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WAUBEEKA LAKE DAM

CT 00065

CONNECTICUT COASTAL BASIN

DANBURY, CONNECTICUT

PHASE I INSPECTION REPORT

NATIONAL DAM INSPECTION PROGRAM

JULY 1981

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Danbury, Conn.	
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INTERNATIONAL ENGINEERING COMPANY, INC. A MORRISON-KNUDSEN COMPANY

EASTERN DISTRICT OFFICE 777 POST ROAD:DARIEN, CONNECTICUT 06820 PHONE (203) 655-3345

1331 2616-110

July 31, 1981

Mr. E. P. Gould
Project Management Branch
Department of the Army
New England Division
Corps of Engineers
424 Trapelo Road
Waltham, Massachusetts 02154

Reference: Contract No. DACW33-81-C-0015

Inspection and Evaluation of Non-Federal Dams

FY-81, Southwestern Connecticut

Dear Mr. Gould:

The inspection of Waubeeka Lake Dam and subsequent hydrologic-hydraulic investigation revealed that the dam should be classified as having a low hazard potential. The following is an abbreviated Phase I Inspection report to substantiate this classification.

Sincerely,

Reynold A. Hokenson, P. E. Project Manager

RAH/daw

Enclosures

DTIC TAB
Unannounced
Justification

By
Distribution/
Availability Codes

Avail and/or
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Special

ACIA,

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NATIONAL DAM INSPECTION PROGRAM

PHASE I INSPECTION REPORT

Identification No.: CT 00065

Name of Dam: Waubeeka Lake Dam

Town: Danbury

County and State: Fairfield, Connecticut

Stream: NOT APPLICABLE

Date of Inspection: June 29 and July 14, 1981

BRIEF ASSESSMENT

Waubeeka Lake is fed primarily by artesian springs, and is impounded by three dikes on the western, eastern, and southern shores. The dikes and lake are currently owned by the Waubeeka Lake Property Owners, Association, Inc., Danbury, Connecticut, (203) 743-9119. The operation and maintenance of the facility is the responsibility of Mr. Peter Couchevich, Caretaker, 404 Post Road, Danbury, Connecticut, (203) 743-9119. The lake is maintained for recreational purposes only.

The formation of what is currently known as Waubeeka Lake, occured over a number of years. The Eastern Dike, which was the first dike constructed according to available information, was erected in 1933. Construction of the West Dike did not occur until the development of the Waubeeka Lake community commenced. The Southern Dike (landfill) was constructed to prevent flow from the lake from entering the Saugatuck River which flows into the Saugatuck Reservoir. The Saugatuck Reservoir, located 5.5 miles from the southern tip of Waubeeka Lake, is currently used as a public water supply.

The West Dike consists of a road embankment built along a natural saddle which lies between hills forming a ridge along the western and southern fringes of the lake. The upstream slope of the dike is a small beach (Waubeeka Lake Beach) which slopes to the lake's edge at about 1V:10H

(Photo 3). The top of the dike (E1- 718 NGVD; Note: all elevations referenced to the National Geodetic Vertical Datum) is a 30-foot-wide two-lane paved road (Post Road). The downstream slope in the vicinity of the culvert was measured at 1V:3H and is entirely covered with small trees (2 to 6 inches in diameter) and brush (Photos 1 and 2). A 3-foot-diameter by 120-foot-long concrete culvert (Intake Invert E1. 712) drains excess water build-up from the lake (Photo 4). During the summer, months the culvert intake is filled with sand to within 1.5 feet of the culvert crown to elevate the recreation pool.

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The East Dike is a 225-foot-long by 3-foot-high earthfill structure with a vertical downstream masonry face and an inclined upstream face which is entirely covered with riprap (Photos 6 and 7). The top of the dike varies from El. 713.3 to El. 714.5 and is approximately 15 feet wide. This variation in elevation occurs in the vicinity of the repaired breach. During the 1955 flood, flow over the top of the dike swept away a 20 to 30-foot-wide section of the dike. This breached area was later restored and no further incidents of this nature have occurred to date. The top and downstream toe are overgrown with trees ranging from 2 to 10 inches in diameter. In addition, a two-lane paved road (Carol Street) was constructed 20 feet from the downstream face of the dike during the development of the Waubeeka Lake community. A storm drain located at the roads edge, adjacent to the dam, collects runoff and pipes it under Carol Street to a point approximately 50 feet downstream of the dike. At the time of the inspection, fine tan deposits were found at the bottom of the storm drain discharge channel (Photo 8). The origin of this material could not be identified.

The southern Dike consists of a landfill deposited in a saddle between hills skirting the southern perimeter of the lake. The width of the South Dike is over 100 feet, while the height is only two to three feet (Photo 5). The lowest point of the South Dike is approximately El. 716.

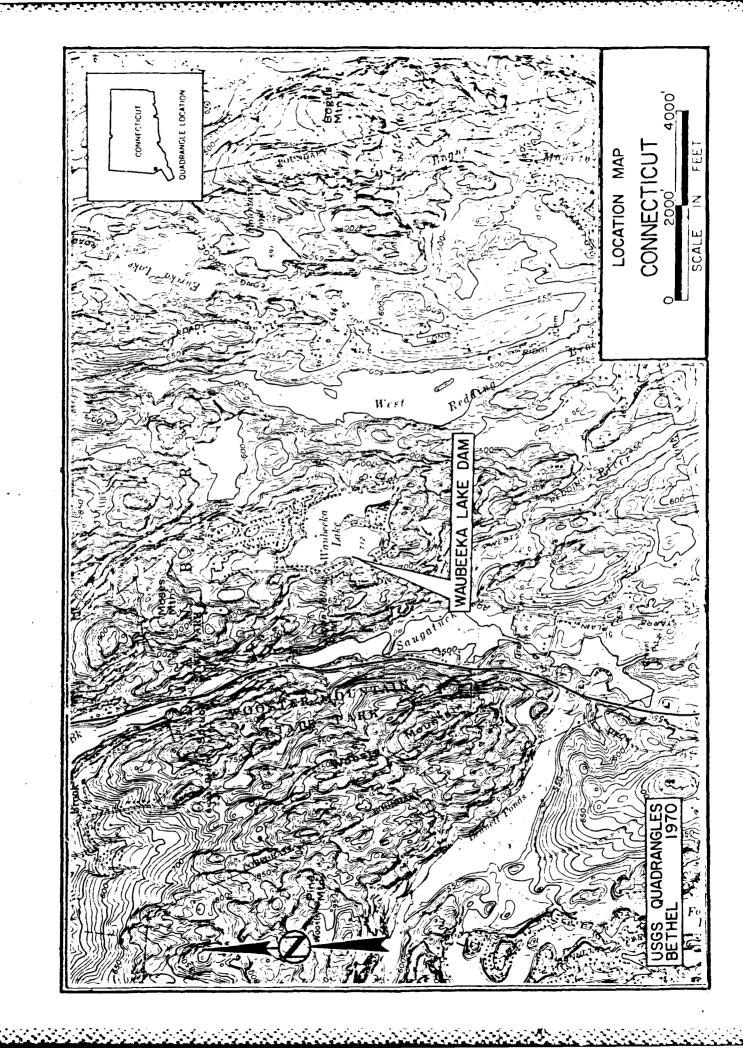
Visual inspection of the dikes indicate that the East and West Dikes are in poor condition. The growth of trees along the dikes will deteriorate

these structures and induce seepage along the root networks. Also, high winds may uproot the larger trees, causing an immediate failure. In addition, the presence of many trees in the vicinity of a discharge channel(s) will contribute to the amount of debris in the channel(s) and impair discharge from the site(s).

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The structures impounding Waubeeka Lake have a maximum storage capacity of 360 acre-feet (ac-ft). Storage capacity was based on the projected runoff from the Probable Maximum Precipitation (8.4 inches over the 0.238 square mile drainage area); the surface area of the lake at E1. 712; and a maximum depth of 14 feet. The height of the structure selected for the dam breach analysis (West Dike) is 5.46 feet. Since the West Dike falls within the Corps' criteria for the small size category based on storage (between 50 and 1,000 ac-ft), the dike (dam) is considered to be SMALL in size. The dam breach analysis was conducted in accordance with the "Rule of Thumb Guidance for Estimating Downstream Dam Failure Hydrographs", dated April 1978, and the potential impact area defined. The invert of the 3-foot-diameter concrete culvert in the West Dike is 1.5 feet below the top of the East Dike. Consequently, the East Dike acts as an emergency spillway since weir flow occurs over this structure before pressure flow begins in the concrete cuvlert. With 1.5 feet of head on the crown of the culvert, weir flow begins over the southern Dike. Flow over the West Dike will not occur until approximately 5 feet of head is on the crown of the concrete culvert. Consequently, dishcarge will occur initially through the concrete culvert and over the East Dike as the lake surface rises. Since the East Dike will pass the entire Probable Maximum Flood (PMF), the water surface will never reach the top of the West Dike. As a result, the maximum water surface elevation achieved during the PMF (El. 715) was used in the dam breach analysis. The breach analysis was performed using that portion of the road enbankment adjacent to the 3-foot diameter concrete culvert (see Appendix B; pg. B-3; Plan and Section). This area was selected since it has the smallest cross-section and; therefore, represents the weakest area and the most likely section to fail. Failure of the West Dike would cause the water surface within the potential impact area, immediately downstream of the

dike, to rise to 2.2 feet at a failure outflow of 730 cfs. The breach width was assumed to include the concrete culvert. The two homes in the potential impact area have first floor elevations of 3 feet and 7.5 feet above ground level. No structures are located downstream of the East Dike and a failure of the South Dike is improbable in that it is a shallow landfill deposited in a saddle between two hills. Consequently, no damage due to the breach of the structures impounding Waubeeka Lake is anticipated and the hazard classification of the West Dike, specifically, is LOW based on the dam breach analysis.



APPENDIX A

VISUAL CHECK LIST WITH COMMENTS

VISUAL INSPECTION CHECK LIST PARTY ORGANIZATION

PROJECT: Waubeeka Lake Dam

DATE: 6/29 & 7/14/81

TIME: 11:00 A.M.

WEATHER: Clear, 80°-85°

W.S. ELEV. 712

PARTY:

1. Reynold A. Hokenson

2. Miron Petrovsky MP

3. Jerry Waugh

4. Ernst Buggisch

PROJECT FEATURE:

1. West Dike

2. Concrete Culvert Intake

3. East Dike

4. South Dike

5. Concrete Culvert

6. Concrete Culvert Discharge Channel

INSPECTED BY:

RAH, EB

INITIALS:

RAH

JW

EB

RAH, MP, JW

RAH, MP, JW, EB

RAH, EB

RAH, MP, JW

RAH, MP, JW

PROJECT: Waubeeka Lake Dam DATE 6/29 & 7/14/81

PROJECT FEATURE: Concrete Culvert Intake NAME RAH, MP, JW

AREA EVALUATED CONDITION

OUTLET WORKS - INTAKE CHANNEL AND

INTAKE STRUCTURE

Approach Channel Waubeeka Lake

Slope Conditions

Bottom Conditions

Rock Slides or Falls

Log Boom

Debris

Condition of Concrete Lining

Drains or Weep Holes

b. Intake Structure

Condition of Concrete Very little of the 3 foot

diameter conduit was exposed.
No deterioration was noted on
exposed portions. Intake
filled with sand to within 1.5

feet of crown.

Stop Logs and Slots Slots in conduit headwall, but

no stop logs were in place.

PROJECT: Waubeeka Lake Dam

DATE 6/29 & 7/14/81 NAME RAH, EB

PROJECT FEATURE: West Dike

PROJECT FEATURE: West Dike	NAME RAH, EB
AREA EVALUATED	CONDITION
DIKE EMBANKMENT	
Crest Elevation	718.0
Current Pool Elevation	712.0
Current root Bievation	712.0
Maximum Impoundment to Date	Approximately El. 715, October,
	1955
Surface Cracks	N/A
Sullace Clacks	N/A
Pavement Condition	Road surface on crest intact
Movement or Settlement of Crest	None observable
Lateral Movement	None
Vertical Alignment	Good
Horizontal Alignment	Good
norizontal Alighment	9000
Condition at Abutment and at Concrete	
Structures	N/A
Taliantina of Managart of Champhons	
Indications of Movement of Structural Items on Slopes	N/A
•	
Trepassing on Slopes	Beach on upstream slope
Sloughing or Exercise of Sloves on	·
Sloughing or Erosion of Slopes or Abutments	None
<i>;</i>	
•	

PROJECT: Waubeeka Lake Dam

DATE 6/29 & 7/14/81

PROJECT FEATURE: West Dike

AREA EVALUATED

NAME RAH, EB

CONDITION

DIKE EMBANKMENT (continued) Rock Slope Protection - Riprap

Failures

No riprap

Unusual Movement or Cracking at or

near Toes

None

Unusual Embankment or Downstream Seepage

Piping or Boils

None

Foundation Drainage Features

Unknown

Toe Drains

Unknown

Instrumentation System

Unknown

PERIODIC INSPECTION CHE	CCK LIST
PROJECT: Waubeeka Lake Dam	DATE 6/29 & 7/14/81
PROJECT FEATURE: East Dike	NAME RAH, MP, JW, EB
AREA EVALUATED	CONDITION
EAST DIKE EMBANKMENT Crest Elevation	713.3
Current Pool Elevation	712.0
Maximum Impoundment to Date	Approximately El. 715 October, 1955
Surface Cracks	N/A
Pavement Condition	N/A
Movement or Settlement of Crest	Crest varies approximately 1.2 feet in elevation.
Lateral Movement	None
Vertical Alignment	Downstream masonry bulging slightly.
Horizontal Alignment	Good
Condition at Abutment	Good, no seepage, seperation or cracking.
Indications of Movement of Structural Items on Slopes	N/A
Trepassing on Slopes	Trees ranging from 2 to 10 inches in diameter and a footpath worn along top of dam.
Sloughing or Erosion of Slopes or Abutments	None, previously repaired breach also sound.
,	

PROJECT: Waubeeka Lake Dam

DATE 6/29 & 7/14/81

SCHOOL MANAGER AND TO THE THE STATE OF THE S

PROJECT FEATURE: East Dike	NAME RAH, MP, JW, EB
AREA EVALUATED	CONDITION
EAST DIKE EMBANKMENT (continued) Rock Slope Protection - Riprap Failures	Some areas of displaced riprap but no exposed embankment material was observed.
Unusual Movement or Cracking at or near Toes	None
Unusual Embankment or Downstream Seepage	Seepage in natural channel bed has deposited tan silty material.
Piping or Boils	None
Foundation Drainage Features	Unknown
Toe Drains	Unknown
Instrumentation System	None
	•
,	
,	

PROJECT: Waubeeka Lake Dam PROJECT FEATURE: South Dike

DATE 6/29 & 7/14/81 NAME RAH, EB,

AREA EVALUATED

CONDITION

SOUTH DIKE EMBANKMENT

Crest Elevation

Current Pool Elevation

Maximum Impoundment to Date

Surface Cracks

Pavement Condition

Movement or Settlement of Crest

Lateral Movement

Vertical Alignment

Horizontal Alignment

Condition at Abutment

Indications of Movement of Structural Items on Slopes

Trepassing on Slopes.

Sloughing or Erosion of Slopes or Abutments

Rock Slope Protection - Riprap Failures

Unusual Movement or Cracking at or near Toes

Unusual Embankment or Downstream Seepage

Piping or Boils

Foundation Drainage Features

Toe Drains

Instrumentation System

Note: The South Dike is a shallow landfill in a natural saddle between two hills. The landfill was placed to prevent flow from the lake from entering the Saugatuck River which flows into the Saugatuck Reservoir. The top width of the landfill is approximately 100 feet and is 2 to 3 feet deep.

PERIODIC INSPECTION	CHECK LIST
PROJECT: Waubeeka Lake Dam	DATE 6/29 & 7/14/81
PROJECT FEATURE:	NAME
AREA EVALUATED	CONDITION
OUTLET WORKS - CONTROL TOWER a. Concrete and Structural	N/A
General Condition	
Condition of Joints	
Spalling	
Visible Reinforcing	
Rusting or Staining of Concrete	
Any Seepage or Efflorescence	
Joint Alignment	
Unusual Seepage or Leaks in Gate Chamber	
Cracks	
Rusting or Corrosion of Steel	
,	

Pi DIC INSPECTION	CHECK LIST
PROJECT: Waubeeka Lake Dam	DATE 6/29 & 7/14/81
PROJECT FEATURE:	NAME
AREA EVALUATED	CONDITION
OUTLET WORKS - CONTROL TOWER (continued) b. Mechanical and Electrical	N/A
Air Vents	
Float Wells	
Crane Hoist	
Elevator	
Hydraulic System	
Service Gates	
Emergency Gates	
Lightning Protection System	
Emergency Power System	
Wiring and Lighting System in Gate Chamber	
•	

PROJECT: Waubeeka Lake Dam

DATE 6/29 & 7/14/81

PROJECT FEATURE: Concrete Culvert

AREA EVALUATED

NAME RAH, MP, JW

CONDITION

OUTLET WORKS - TRANSITION AND CONDUIT

General condition of Concrete

Ends of conduit appear to be in good condition.

Rust or Staining on Concrete

Spalling

Unknown Unknown

Erosion or Cavitation

None of exposed ends.

Cracking

Unknown

Alignment of Monoliths

Unknown - none exposed.

Alignment of Joints

Unknown - none exposed.

Numbering or Monoliths

Unknown - none exposed.

Note: Very little of the concrete conduit was exposed; therefore, it was impossible to examine it closely.

PROJECT: Waubeeka Lake Dam

DATE 6/29 & 7/14/81

AREA EVALUATED

PROJECT FEATURE: Concrete Culvert Discharge

Channel.

NAME RAH, MP, JW

CONDITION

OUTLET WORKS - OUTLET STRUCTURE AND OUTLET CHANNEL

General Condition of Concrete

Rust or Staining

Spalling

Erosion or Cavitation

Visible Reinforcing

Any Seepage or Efflorescence

Condition at Joints

Drain Holes

Channel

Loose Rock or Trees Overhanging Channel

trees 2 to 6 inches in diameter, were found in and adjacent to the channel. Additional 3-foot diameter concrete culverts are within the channel and support private driveways.

Loose rocks and overhanging

Condition of Discharge Channel

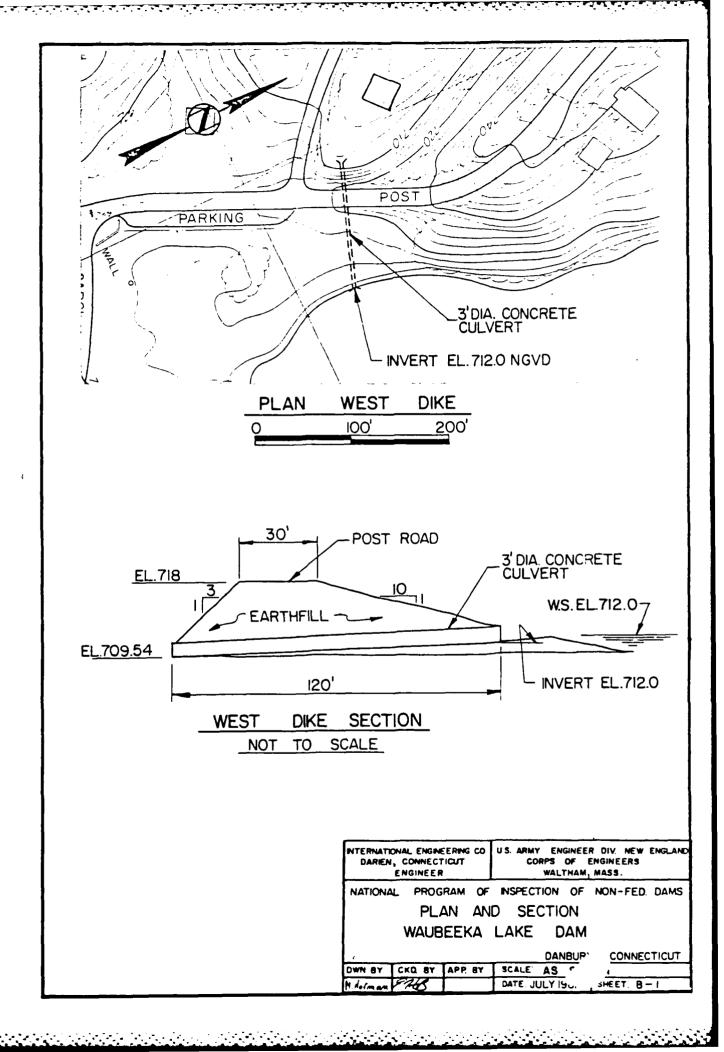
Poor.

PERIODIC INSPECTION	CHECK LIST
PROJECT: Waubeeka Lake Dam PROJECT FEATURE:	DATE 6/29 & 7/14/81 NAME
AREA EVALUATED	CONDITION
OUTLET WORKS - SPILLWAY WEIR, APPROACH AND DISCHARGE CHANNELS a. Approach Channel	N/A
General Condition	
Loose Rock Overhanging Channel	
Trees Overhanging Channel	
Floor of Approach Channel	
b. Weir and Training Walls	N/A
General Condition of Concrete	
Rust or Staining	
Spalling	
Any Visible Reinforcing	
Any Seepage or Efflorescence	
Drain Holes	
c. Discharge Channel	N/A
General Condition	
Loose Rock Overhanging Channel	
Trees Overhanging Channel	
Floor of Channel	
Other Obstructions	
,	

PERIODIC INSPECTION	CHECK LIST
PROJECT: Waubeeka Lake Dam	DATE 6/29 & 7/14/81
PROJECT FEATURE:	NAME
AREA EVALUATED	CONDITION
OUTLET WORKS - SERVICE BRIDGE a. Super Structure	N/A
a. Super Structure Bearings	N/A
Anchor Bolts	
Bridge Seat	
Longitudinal Members	
Under Side of Deck	
Secondary Bracing	
Deck	
Drainage System	
Railings	
Expansion Joints	
Paint	
b. Abutment & Piers	N/A
General Condition of Concrete	·
Alignment of Abutment	
Approach to Bridge	
Condition of Seat & Backwall	
· ·	

APPENDIX B

ENGINEERING DATA



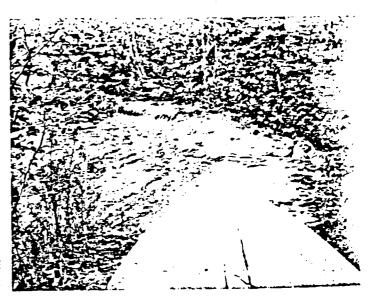
SUMMARY OF CORRESPONDENCE

PAGE	B-4	B-6	В-7	B-8	В-9	B-10	B-12	B-13	B-14	B-11
SUBJECT	Water Resources Inventory Data Sheet	Recommendations following State inspection	Inspection of repairs	Request to revisit site	Progress of repairs	Recommendations	Progress of repairs	Confirmation of inspection	Recommendations	Question concerning letter received from W.H. O'Brien
FROM	1	William P. Sanders Engineer - Geologist State of Connecticut	A.M. McKenzie Civil Engineer	William P. Sanders	Milton Livingson	William P. Sanders	Willaim H. O'Brien III	H. Rooner	William H. O'Brien III	Milton Livingston
TO	1	Mr. Milton Livingston Secretary The Lake Waubeeka Property Owner's, Inc. Danbury, Connecticut	Water Resources Commission State of Connecticut	A.M. McKenzie	William P. Sanders	Milton Livingston	Ľake Waubeeka Property Owner's, Inc.	William H. O'Brien III	Lake Waubeeka Property Owner's, Inc.	William H. O'Brien III State of Connecticut Water Resources Commission
DATE	1	9/26/66	9/56/66	99/8/6	99/L/6	99/8/8	7/25/66	5/23/66	3/29/66	8/1/66

SUMMARY OF CORRESPONDENCE (Continued)

	WATER RESOURCES UNIT			
entoried	SUPERVISION OF DAMS INVENTORY DATA	lat:	41° 20.9'	
y			73° 27.8'	
e		-	rshed = 145 ac	res
Name of Dam or	Pond WAUBEEKA LAKE			·
Code No.				
	Location			
	у			
	d.Bethel			
	am <u>West Redding Brook</u>			
Owner <u>Lake W</u>	laubeeka Property Owners, Inc.	_		
Address Peter	Dotti, President	_		•
291 Wa	ter Street (256 Carol Str	eet)		
Donth	Amboy, NJ (Lake Waubeeka	}	1933?	
rean				
		rainage Area	.17 sq.m	i
Pond Used For _	Dr			
Pond Used For _ Dimensions of P		jth	Area <u>35</u>	ac.
Pond Used For _ Dimensions of P Total Length of	Ond: WidthLeng	th	Area <u>35</u> way ω= 3' s=	ac.
Pond Used For _ Dimensions of P Total Length of Location of Spi	Dr Pond: WidthLeng Dam <u>w140' &80' e 50'</u> Leng 11wayw/&/e	oth	Area <u>35</u> way w=3' &=	ac. e=
Pond Used For _ Dimensions of P Total Length of Location of Spi Height of Pond	Dr Pond: WidthLeng Dam <u>w140' &80' e 50'</u> Leng	oth oth of Spill e5'	Area <u>35</u> way ω=3' s=	ac. e=
Pond Used For _ Dimensions of P Total Length of Location of Spi Height of Pond Height of Emban	Drond: Width Leng Dam <u>w140' &80' e 50'</u> Leng 11way <u>w/s/e</u> Above Stream Bed w15' &2' e	oth	Area <u>35</u> way w= 3' s= washout; 1' ge	ac. e= neral
Pond Used For Dimensions of P Total Length of Location of Spi Height of Pond Height of Emban Type of Spillwa	Dr. Pond: Width Leng F Dam <u>w140' &80' e 50' Leng</u> 11way <u>w/\$/e</u> Above Stream Bed <u>w15' &2' e</u> kment Above Spillway <u>w3' &2</u>	oth	Area <u>35</u> way w= 3' s= washout; 1' ge	ac. e= neral
Pond Used For Dimensions of P Total Length of Location of Spi Height of Pond Height of Emban Type of Spillwa Type of Dike Co	Pond: Width Leng F Dam w140' \$80' e 50' Leng 11way w/s/e Above Stream Bed w15' \$2' e kment Above Spillway w3' \$2' by Construction 36" concrete part enstruction earth	oth	Area <u>35</u> way w= 3' s= washout; 1' ge	ac. e= neral
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Pond Used For Dimensions of P Total Length of Location of Spi Height of Pond Height of Emban Type of Spillwa Type of Dike Co Downstream Cond	Pond: Width Leng F Dam w140' &80' e 50' Leng 11way w/s/e Above Stream Bed w15' &2' e kment Above Spillway w3' &2 by Construction 36" concrete part enstruction earth litions woods; road	gth of Spill e5' 2' e 0' at ipe .	Area <u>35</u> way w=3' s= washout;1' ge	ac. e= neral
Pond Used For Dimensions of P Total Length of Location of Spi Height of Pond Height of Emban Type of Spillwa Type of Dike Co Downstream Cond Summary of File	Pond: Width Leng Floam w140' &80' e 50' Leng Ilway w/s/e Above Stream Bed w15' &2' e Ekment Above Spillway w3' &2 By Construction 36" concrete part Enstruction earth Ititions woods; road Data letter 3/11/66 from McI	gthgth of Spill' e5' 2' e 0' at ipe . Kenzie state	Area 35 way w=3' s= washout;1' ge es that "it is	ac. e=
Pond Used For Dimensions of P Total Length of Location of Spi Height of Pond Height of Emban Type of Spillwa Type of Dike Co Downstream Cond Summary of File consider	Pond: Width Leng F Dam w140' &80' e 50' Leng 11way w/s/e Above Stream Bed w15' &2' e kment Above Spillway w3' &2 by Construction 36" concrete part enstruction earth litions woods; road	gthgth of Spill' e5' 2' e 0' at ipe . Kenzie state d".Letter se	Area 35 way w=3' s= washout;1' ge es that "it is ent to owner	ac. e= neral







September 26, 1966

Mr. Milton Livingston, Secretary The Lake Waubeeka Property Owners, Inc. Danbury, Connecticut

Dear Mr. Livingston:

The Lake Waubeeka dikes were inspected on September 20, 1966 by a consulting engineer retained by this Commission. The inspection was made in the company of your Superintendent, Mr. Peter Couchevich.

The dikes were found to be in good repair as a result of maintenance performed during the past few months. Our engineer recommended that rip-rap be placed on the water side of the east dike to provide proper protection against wave action. When this is accomplished, the recommendations in our letter dated August 8, 1966 will be completed. Thank you for your cooperation.

Very truly yours,

William P. Sander Engineer - Geologist

WPS:js

A. M. MCKENZIE CIVIL ENGINEER M. AM. SOC. C. E.

HYDRAULICS WATER SUPFLY LAND DEVELOR INT

1300 MAIN STREET

September 21, 1966.

Water Resources Commission. state of Connecticut. State Office Building. Hartford, 15. Connecticut.

Ref: Waubeeka Lake. Town of Danbury.

Gentlemen:

As requested in your letter of September 8, I visited the Waubeeka Lake Development again yesterday and, with Mr. Couchevich, looked over the repairs which have recently been made to the dikes.

On the west side of the Lake a "sort" of headwall has been built at the intake end of the 36" of concrete pipe which is the only outlet for water from the Lake. This is referred to at the end of the 3rd. paragraph of my letter of March 11. The headwall extends up to the top of the pipe only and is not very well done, but, in this location it will serve to keep the pipe from being pushed out of place. Slots have been left in the concrete so that boards can be dropped in to form a gate over the face of the pipe. However, the present freeboard on the dikes is only about 2'-6" and I cautioned Mr. Couchevich against raising the water surface more than 6" to 8" above it's present level.

On the east side of the Lake there was a break in the dike which is clearly shown in photo #10. This area has been filled in with rock and earth which appears to have been well compacted and the surface is at the same elevation as the rest of the dike. The Superintendent agreed to put some rip-rap on the water side of the new fill to provide proper protection against wave action.

When the rip-rap is placed the repairs suggested in the last paragraph of my letter Of March 12th. will have been completed. A couple photographs of the recent work will be forwarded as soon as they are processed.

Yours very truly

mme/lengre

A. M. McKenzie

September 8, 1966

Mr. A. M. McKenzie 1300 Main Street South Meriden, Connecticut

Dear Mr. McKenzie:

Enclosed is a copy of a letter which we received from the Lake Waubeeka Property Owners.

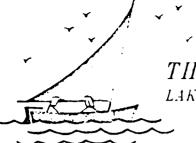
We sent them the recommendations that were contained in your report dated March 11, 1966. In view of comments made by Mr. Livingston, we would like you to reinspect the dam, preferably in the company of Mr. Couchevich, the Superintendent. Upon completion of the inspection, please report again on the condition of the dam.

Very truly yours,

William P. Sander Engineer - Geologist

WPS:js

enclosure

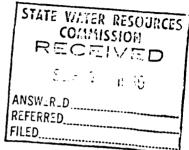


THE LAKE WAUBEEKA PROPERTY OWNERS. / NO LAKE WAUBEEKA DANBURY CONNECTICUT

September 7, 1966

Mr. William P. Sander Water Resources Commission State of Connecticut State Office Building Hartford 15, Conn.

Dear Mr. Sander,



This will acknowlede receipt of your letter of August 8, 1966 about the consultants report after inspecting Lake Waubeeka at Danbury, Connecticut owned by the Lake Waubeeka Property Owners Inc.

Our superintendent, Mr. PeterCouchevith who can be reached at 404 Post Road in our community and whose phone number is 743-9119, is in the process of constructing the head wall at the intake end of the 36 inch pipe on the beach now that the heavy Summer use of the beach is ebbing.

We are puzzled however about your recommendations about the break in the dike at the East end of the Lake which indicates that rock had apparently recently been placed there. The fact is that we placed the rock there during the first stage of our contemplated repair of the dike but early in July of this year we completely covered the rock and built up the entire section to a level of approximately two Beet above the water level. Is it possible that your inspection was made prior to this even though yourletter is dated August 8th. It would be most helpful if you can check to determine when the inspection was made. If there is still work to be done on the dake our superintendent asks that your office tell him exactly what he is to do since we even had a contractor go over the ground with him this past week and they both feel that any futther attempt to raise the level along the entire dike would kill the trees and probably cause erosion where it has not taken place.

We are most happy to hear that your report indicates no immediate hazards and please be assured that we are prepared to cooperate with your office in every possible way and as quickly as possible.

Very truly yours,

Milton Livingston, Secretary

August 8, 1966

Mr. Milton Livingston, Secretary Lake Waubeeka Property Owners, Inc. Danbury, Connecticut

Dear Mr. Livingston:

In the process of inventoring all the dams in the State, the dams and dikes on Waubeeka Lake in Danbury have recently been inspected by an engineering firm acting as a consultant to this office.

According to the Danbury assessors office, the dams and dikes on Waubeeka Lake, Danbury, are owned by the Lake Waubeeka Property Owners, Inc.

Whereas no immediate hazards were noted, there are a few points which should receive attention. The State Water Resources Commission, as further explained in the enclosed copy of the pertinent State statutes, has jurisdiction over all such structures "which by breaking away or otherwise, might endanger life or property."

In the consultant's report, it was recommended that a head-wall be constructed at the intake end of the thirty six inch pipe on the beach, which pipe runs under the roadway on the dike or dam on the west side of the lake. The break in the dike on the east side of the lake (where loose rock has apparently recently been placed) should be repaired with properly compacted earth fill and, "where required, raised so that there will be a freeboard of at least two feet for its entire length. Also, some stone rip-rap should be placed along the water side of the dike to prevent any erosion in case of even slight wave action."

In light of this report, it is evident that this work should be done to reduce possible future hazards. May we hear from you soon in this regard?

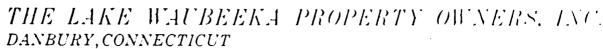
Very truly yours,

William P. Sander Engineer - Geologist

WPS:WO:8

enclosur:

iffe



August 1, 1966

Mr. William H. O'Brien III State of Connecticut WaterResources Commission State Office Building Hartford 15, Conn.

Dear Sir,

Your letter of July 25 addressed to the Lake Waubeeka Property Owners Inc. at 1 EzraeStreet, Danbury, Conn. puzzles us since you are obviously communicating with some other community and not the Lake Waubeeka Property Owners Inc. since we know nothing of a letter dated March 29, 1966 to us nor a reply you received from Mr. H. Roomer of Trumbull stating that Mr. Shhefkind had been ill and would look into the matter.

The Danbury phone book does list a Morris Schefkind at 1 Ezrasstreet, Danbury so we suggest you write to him and see what you can find out from him. Our address is Lake Waubeeka, Danbury, Conn. and we know nothing about the two gentlemen you mention.

Yours very truly,

Miken Livingston, Secretary

STATE WATER RESOURCES
COMMISSION
RECEIVED
ACC 1 1933
ANSWERLD
REFERRED
FILED

July 25, 1966

Lake Waubeeka Property Owners, Inc. 1 Ezra Street Danbury, Connecticut

Gentlemen:

On March 29, 1966 we wrote to you listing certain items which should receive attention on the dams and dikes on Waubeeks Lake in Danbury. On May 24, 1966 we received a letter from a Mr. H. Rooner, 108 Chestnut Hill Road, Trumbull, stating that Mr. Schefkind had been ill but would look into the matter when he was able.

I hope that Mr. Schefkind's health has improved. We would like to hear from someone in your association regarding what action you intend to take regarding the suggestions in our letter of March 29, 1966.

Very truly yours,

William H. O'Brien, III Civil Engineer

WHO:dj

Mr. William H. O' Green I State of Connections Water Resources Commission State office Building May v3, 19 Hartford 15, Com. Dear Jun O. Brien: a heart attack, and this is the relation for the delay in answering your letter to lim of march 29 th in reference to trabe Wankelea in Danhuy is able, he will look into the matter and the suggestions you have outlined Very truly yours 168 Chestrut Hill Rd. Trumbull Come. COMMISSION RECEIVED 47 24 Mis

March 29, 1966

Lake Waubeeka Property Owners, Inc. c/o Morris Schefkind 1 Ezra Street Danbury, Connecticut

Gentlemen:

In the process of inventoring all of the dams in the State, the dams and dikes on Waubeeka Lake in Danbury have recently been inspected by an engineering firm acting as a consultant to this office.

Whereas no immediate hazards were noted, there are a few points which should receive attention. The State Water Resources Commission, as further explained in the enclosed copy of the pertinent State statutes, has jurisdiction over all such structures "which, by breaking away or otherwise, might endanger life or property".

In the consultant's report, it was recommended that a head-wall be constructed at the intake end of the thirty-six inch pipe on the beach, which pipe runs under the roadway on the dike or dam on the west side of the lake. The break in the dike on the east side of the lake [where loose rock fill has apparently recently been placed) should be repaired with properly compacted earth fill and, "where required, raised so that there will be a freeboard of at least two feet for its entire length. Also, some stone rip-rap should be placed along the water side of the dike to prevent any erosion in case of even slight wave action".

In light of this report, it is evident that this work should be done to reduce possible future hazards. May we hear from you soon in this regard?

Very truly yours,

William H. O'Brien, III Civil Engineer

WHO: js

A. M. MCKENZIE CIVIL ENGINEER

M. AM. SOC. C. E.

HYDRAULICS WATER SUPPLY LAND DEVELOPME

1300 MAIN STREE SOUTH MERIDEN, CO

March 11, 1966.

water Resources Commission, State of Connecticut, State Office Building, Hartford, 15, Connecticut. STATE WATER RESC COMMISSION RECEIVE

ANSWLRLD REFERRED

FILED

Ref: Waubeeka Lake Dam, Town of Danbury, Bethel Quad.

Gentlemen:

As instructed in your letter of February 21 I have inspected the Waubeeka Lake Dam and submit the following report for your imformation.

Waubeeka Lake is a real estate development with a large number of houses, mostly for summer occupancy, and, 'according to the sign at the entrance, is privately owned and controled. It is used for recreational purposes only.

The Lake fills a depression in the hills and appears to be almost entirely spring fed as the water shed draining into it is very small - about 145 acres. There are three places around the Lake where low points have been filled in to raise the water to it's present level. The principal "dam", or dike, is on the west side of the Lake at about the center. It is an earth fill with an overall length of about 140', a maximum height of 18' and carries on it one of the roads running thru the development. The fill is approximately 30' wide on top, with an oiled suface forming the road and the area between the edge of the road and the Lake has been filled in with clean sand to form a beach. See photos 4 + 9. The center of the road is 2' to 2'-6" above the water surface. Running under the beach and the road is a 36"\$ concrete, which is the only real drain from the Lake. There is no headwall at the intake nor is there any gate on the line; on this date about 10" of water was flowing into the pipe. The water is very clear and there is no indication of much variation in the elevation of the shore line which further indicates that the lake is spring fed.

On the east side of the Lake, also at about the center point, there is another low dike about 150' long with a maximum height of six feet. This dike is 8' to 12' wide on top and the face away from the Lake is supported by a dry stone wall. See photo. The freeboard here is from 0 to 18". At one point in the dike there has been some erosion and a small stream has recently flowed thru the opening. Some loose rock fill has been thrown into the opening within a few days and there is still a trible of water running thru. The edge of the road along the eside of the Lake is not more than 15' to 20' from the dike and a double catchbasin has been installed on the Lake side of the road

Fr. NE

23 Squi

A. M. MCKENZIE

CIVIL ENGINEER

M. AM. SOC. C. E.

HYDRAULICS WATER SUPPLY LAND DEVELOR OF NO

1300 MAIN STREET

Page - 2 -

which takes the runoff and carries it under the road thru a pipe. See photos *// . From the appearance of the area this overflow has been going on for a number of years without producing any serious break in the dike. Judging from the size of the trees growing on the dike it has been in place 25 - 30 years, or more.

At the south-east corner of the Lake, where the road makes a sharp, rightangle bend, there is another spot where a low point has been filled in and a house built on a part of the fill. The filled area is nearly a hundred feet wide with an elevation of perhaps 18" above the water surface and there is a slight trickle coming thru. However, I do not consider this spot of any importance.

The Lake covers some 40 acres and the area draining into it is about 145 acres. The drainage area is fairly steep, heavily wooded with many irregular low spots and considerable rock exposed on the surface. There are probably more than a hundred houses in the development. Using a stendard formula for calculating a 100 year flood the figures are as follows:

 $Q = R \times C \times A \times S$ where R = 3.7, C = .55 and S = 40

then Q = 3.7 x .55 x .25 x 40 = 20 c.f.s.

The 36" of drain has a capacity of about 31c.f.s. which is ample. The earth fill forming the dam is in good condition and shows no signs of erosion. It is my opinion that the Lake is principally spring fed and that there is very little variation in the water level from season to season and from year to year. There does not appear to be any reason for failure of the dam and it is not considered that any hazard is involved. The overflow from the Lake discharges into the Saugatyck River about 3/4 of a mile to the west. The flow from the break in the dike on the east side goes into a large swamp about a mile to the east and then into Redding Brook. This flow will cease when the dike is repaired.

A. M. MCKENZIE

CIVIL ENGINEER

M. AM. SOC. C. E.

HYDRAULICS WATER SUPPLY LAND DEVI TWEE

1300 MAIN SCHET-

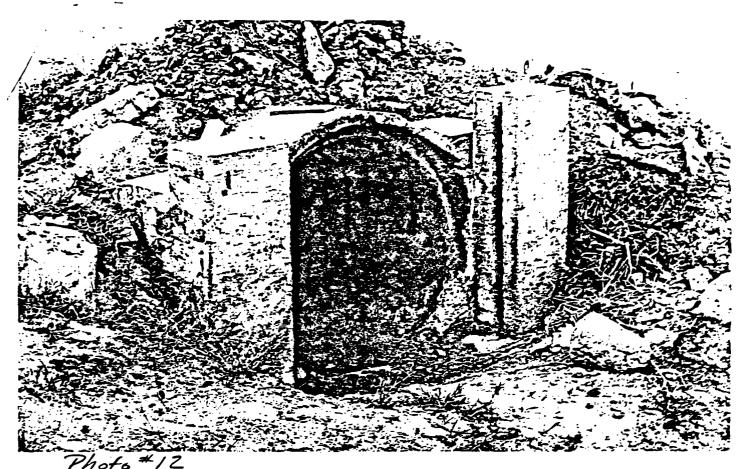
Page - 3 -

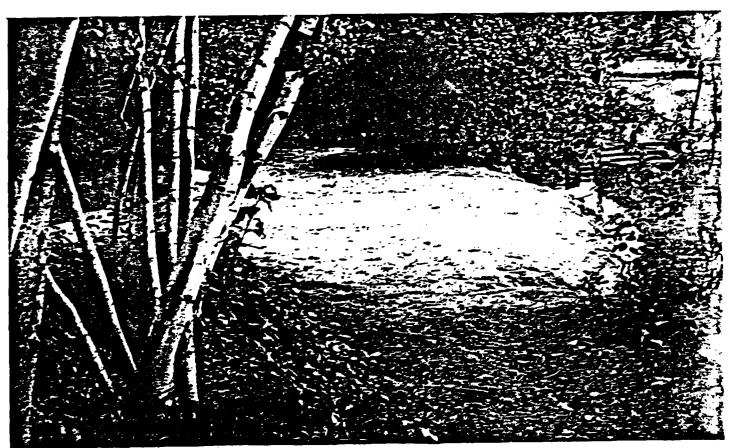
It is recommended that a head wall be constructed at the intake end of the 36" pipe culvert on the west side. On the east side the break in the dike should be repaired with properly compacted earth fill and, where required, raised so that there will be a freeboard of at least 2' for it's entire length. Also, some stone rip-rap should be placed along the water side of the dike to prevent any erosion in case of even slight wave action

Yours very truly

A. M. McKenzie

Enclosure - 5 Photographs.





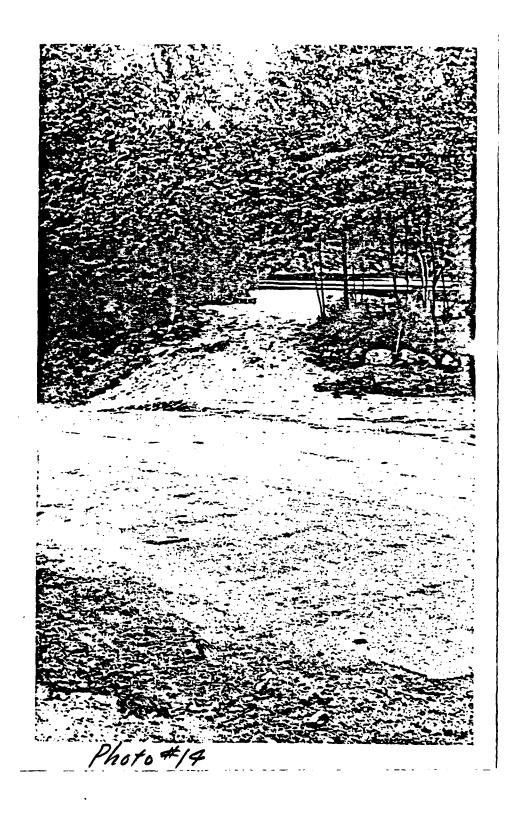
#13 Photo

Manhaci Lake
Transham

This is close-up of headwell recently enstructed at inlet end it 36" b down from Lahe. See plato #6, and sold sold from Lahe. See plato #6, and the slote of a/21/66, or the profit, in I continued a sinst placing stop-like in the slots show in the shoes.

Town of Donbury.

to dike on E. = 1 = 1.00 = 10 mode sporte condition here see place = 10 mode sporte this shot is lacking along dike in apposite direction from photo=10.



B-20

Mind to The Tomberry

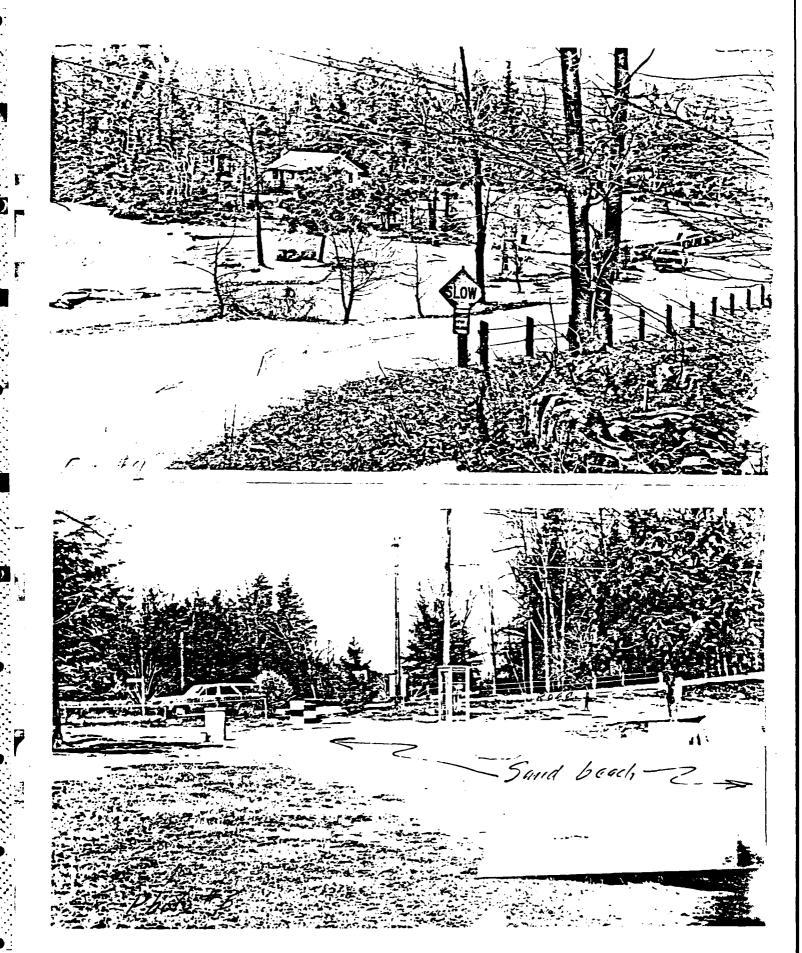
Town - Danhary

Alis photo stars some

repair to E. side dike as

ther shown in #13. Locking

tran real, over dike, I do



Wanbeelta Lake Danburg.

Looking & S.E. over road and bouch

Wanbeeka Lake fll Danbarg

Looking & N.W. over part of back.

Road in background. Photo #6 shins

area to right of this photo.





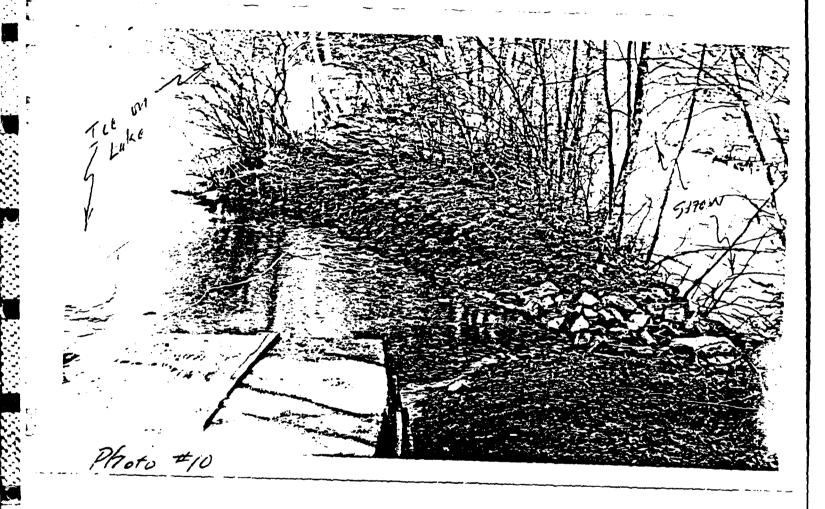
Wanbecka Lake, " 1
Danbury.

Main "DAM", or dike-locking ± N. overbeach and road. The intake out of 36" force pipe in right center of photo

> Waubeeka Lake Danbury.

Dike of E. side of Lake. Stone wall, Visible above snow, supports carth fill. Road in foreground.

のチュ



Wanbeeka Lake

Danbury.

Break in dike on E. Shore

of Lake - note loose stone

thrown into break.

This view looking ± N. over dike.

February 21, 1966

Mr. A. M. McKenzie 1300 Main Street South Meriden, Connecticut

Dear Mr. McKenzie:

Under your contract as consultant to this Commission, will you please inspect and report on the Waubeeka Lake Dam in Danbury. This dam is located on the Bethel Quadrangle about a half mile east of Route 7.

Very truly yours,

William P. Sander Engineer - Geologist

WPS:js

APPENDIX C

PHOTOGRAPHS

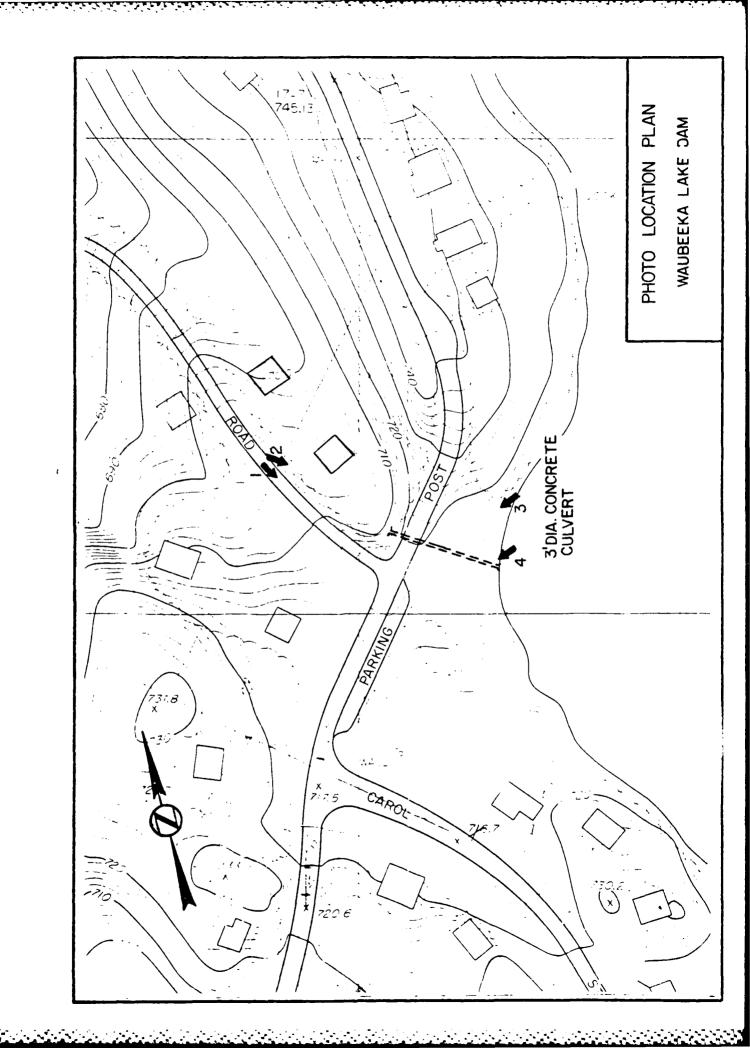




Photo 1 Downstream face and right abutment of dam on west side of Waubeeka Lake. First floor of home in impact area is defined by the deck, left.



Photo 2 Downstream face of dam and Alan Road (West Dike).



Photo 3 Upstream face of dam - Waubeeka Lake Beach (West Dike).

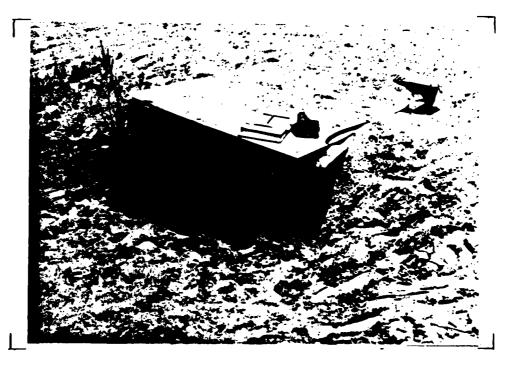


Photo 4 Intake of 3-foot diameter concrete conduit at dam (West Dike).

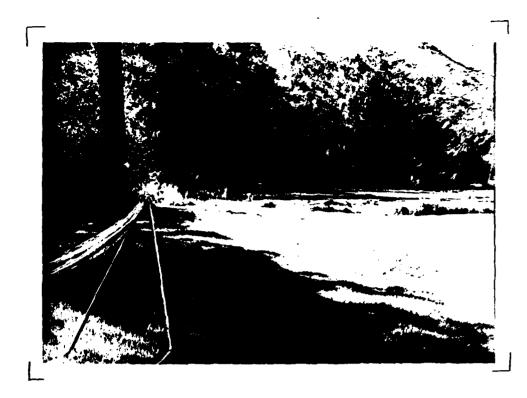


Photo 5 Landfill at southern end of lake.

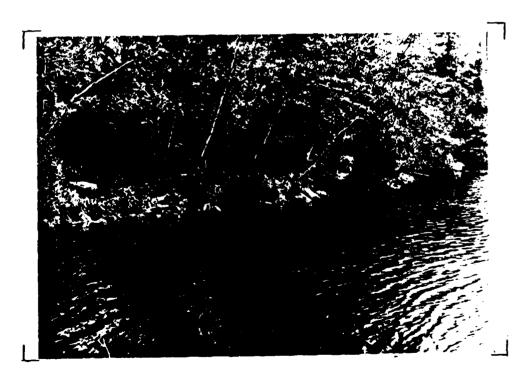


Photo 6 Upstream face of East Dike.



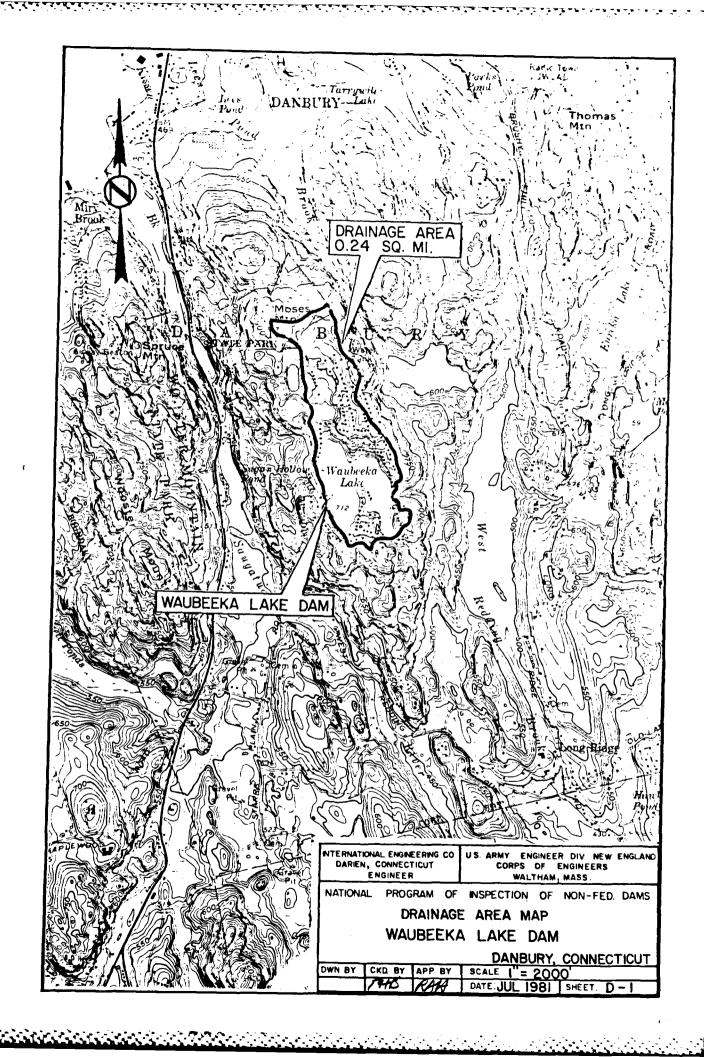
Photo 7 Downstream face of East Dike



Photo 8 Discharge stream from storm drain located on road downstream of East Dike.

APPENDIX D

HYDROLOGIC AND HYDRAULIC COMPUTATIONS



INTERNATIONAL ENGINEERING COMPANY, INC.	Sheet 2 4.22
Project N47/W/AL Now I NSPESION PRISONN	Contract No Zelf 025 File No.
Feature WAVAELLA LIKE Tum	Designed Date
tem	Checked Date

HYDROLOGIC | HYDRAULIC INSPECTION

WAUBEEKA LAKE DAM, CTOODES, DAWBURY, CONVECTICAT

DRAWAGE ARRAND SITE DESERIPTION

THE DROWAGE AREA IS 0.238 SQUARE MILES, AND WAS PLANIMETERED FROM THE 75 USGS QUADRANGLE, BETHEL, CT. . THE LAKE IS FEO PRIMARILY BY AN ARTESIAN SPRING. THE IMPOUNDMENT IS SECURED BY THREE DIKES OF VARYING FACEBOARDS. A THREE-FOOT DIAMETER RCP CULVERT DRAINS LINCESS WATER BUILD-UP IN THE LAKE. HOWEVER THE INVEST OF THE INSET IS ONLY 1.5 FEET BELOW THE TOP OF ANOTHER DIKE SITUATED ON THE EASTERN SHORE OF THE LAKE. WEIR FLOW OCCURS OVER THE EASTERN DIKE BEFORE PRESSURE FLOW BEGINS IN THE 3 CULVERT SPILLWAY. WITH 1.5 FEET OF MEAD ON THE CROWN OF THE CULVERT, WEIR FLOW BEGINS OVER THE THIRD, OR SOUTHERN SINE. WEIR FLOW WILL NOT OCCUR OVER THE DIRE SUPPORTING THE SPILMAY WESTERN DIRE) UNTIL THE CAONN IS SUBMERSED BY APPRIXIMATELY 5 FRET OF WATER.

HOUSES ARE STITUTED BELOW THE WEST AND SOUTH DIKES. AT PRESENT



Project National Engineering Company, Inc.		Sheet 3 # 22
Project National Dam Taspection Passam	Contract No. 26/6 - 025	File No.
Feature	Designed	Date 7-22-8/
Item	Checked	Date

MO POTENTIAL HARARD APPEARS TO EXIST OLION THE EAST DIRE WHICH,

BUTG THE LOWEST, AS EFFECTIVELY AN EMERGENCY SPILLINGY. FAILURE

OF THE SOUTH DIRE IS IMMOBBABLE IN THAT IT IS APPARENTLY A LAWD

FILL DEPOSITED IN A SHOOLE RETWEEN HILLS SKIPTING THE

SOUTHERN PERMETER OF THE LAKE. THE WIOTH OF THE SOUTH DIRE

IS OVER 100 FEET, WHILE THE NEIGHT AS ONLY TWO TO THREE

FEET. THE LOWEST POINT OF THE SOUTH DIRE IS EL. 716;

THIS ELEVATION WILL NEVER OF REACHED BY THE PMF EVEN IF

THE 3 FOOT CULVERT IS PLUGGED, SINCE THE EAST DIRE WILL

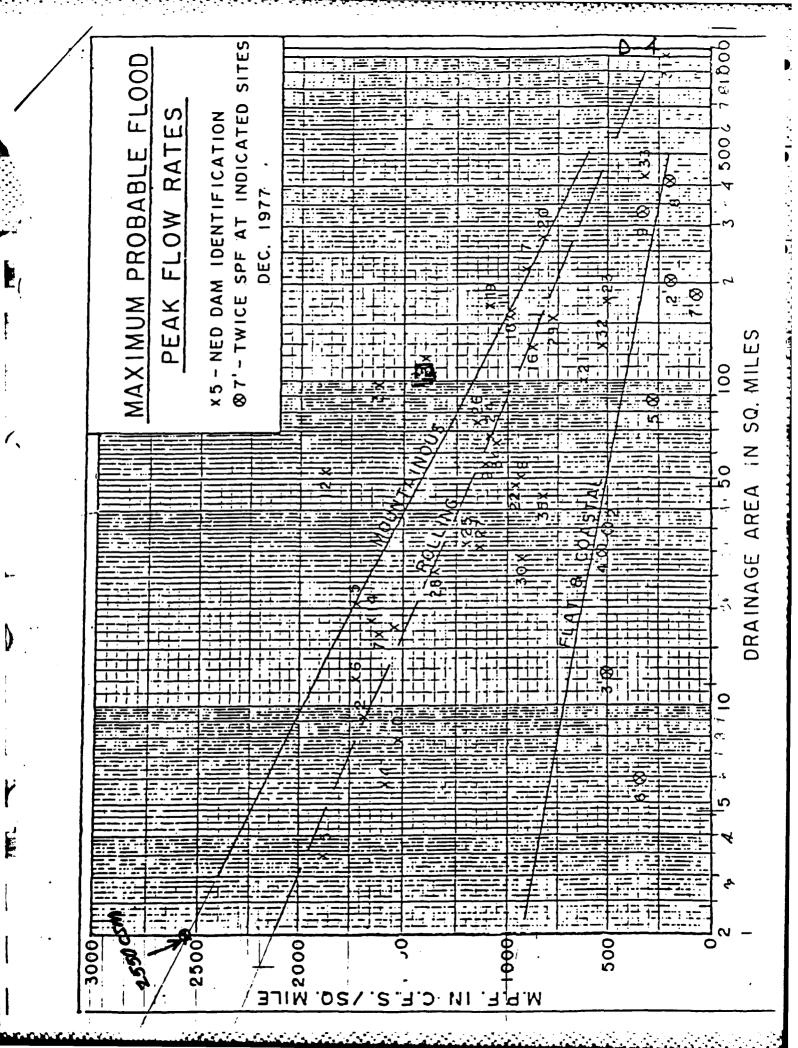
PASS THE ENTIRE PMF.

THE REASERVOIR SURFACE AREA IS 34.6 AC AT GL 712 NGVO.

AS SUCH IF COMPRISES 23 % OF THE TOTAL DRAINAGE AREA

* PLANIMETERED ON G.S. BETWEL QUAD, 7 ± SERIES.





Project Project WATIWAL CHAIL TOSPECTION PROGRAM Feature WARRERA LIKE DAM		Sheet 50/22
Project NATIONAL CHAM INSPECTION PROGRAM	Contract No. 2616-025	File No.
Feature WANDERKA LIKE DAWN	Designed	Date 7-24-8/
ltem	Checked	Date

- 1) PERFORMANCE AT PEAK FLOOD CONDITIONS
 - a) WATERSHED CLASSIFIED AS MOUNTAINOUS
 - b) WATERSHED AREA:

C) PEAK FLOOS (FROM NED-ACE GUIDELINES; GUIDE CORVES FOR PMF)

SECAUSE THE WATERSHED AREA IS LESS THAN 2 mi2, AS
CSM OF 2550 (CF FOLLOWING PP) WAS SELECTED

- 2) SURCHARGE AT PEAK INFLOWS (PMF & & PMF)
 - a) OUTFLOW KATING CORVE
 - 1) SPILLMAY

THE SPILLWAY IS A 3'\$ RCP PIPE ON A

STOPE OF 0.021. ITS INVERT IS AT EL TIZ MSL.



((E) INTERNATIONAL ENGINEERING COMPANY, INC.		Sheet 6 0/22
Project NATIONAL SAME TASPECTION ROGRAM	Contract No. 26/6-025	File No.
Feature WANBLEKA LAKE DAM	Designed	Date 7-24-8/
tem	Checked	Date

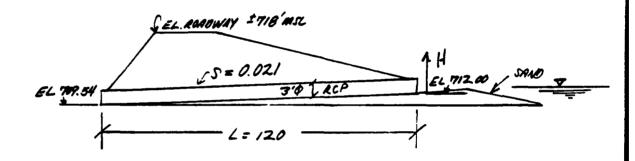
THE DAM CONSISTS OF A ROAD EMBANEMENT BUILT ALDEG A

NATURAL SADDLE WARN LIES BETWEEN HILLS FORMING A RIDGE

ALONG THE WESTERN AND SOUTH WESTERN FRAGES OF THE LAKE.

I SECTION THROUGH THE ROAD EMBANKMENT AND & OF

CALVERT IS SHOWN BELOW.



A PLAN VIEW OF THE SYSTEM IS SHOWN ON PAGE D-ZI.

A RATENG CUXVE FOR THE CULVERT IN TERMS OF AN FHA CULVERT

NOMOGRAPH IS INCLUDED, PAGE D-7. A PROFILE THROUGH

THE EAST DIKE APPEARS ON SHEET D-8. BECAUSE

WATER WAS NOT EXPECTED TO RISE TO THE ROADWAY (IN

THE ABOVE SKETCH) A PROFILE WAS NOT NECESSARY.

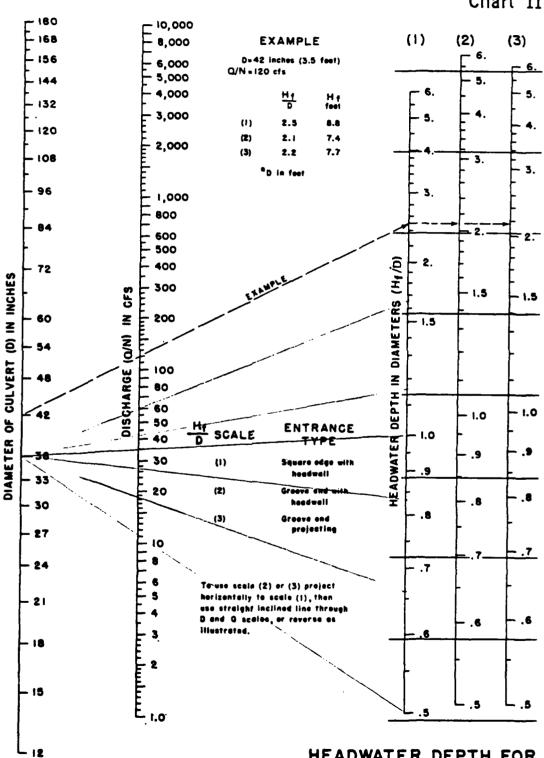
THE PROCEDURE USED IN DEVELOPING THE TOTAL PATING CURVE

FOR THE CHIVERT, EAST DIRE SYSTEM WAS TO SUM THE

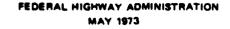
TWO RATINGS, PAGE D-11, APPROETHER WITH THE STAGE

Project NATIONAL ENGINEERING COMPANY, INC. Project NATIONAL DAM TASPECTION PROGRAM DE Teature NAUBULLA LILLE DAM DE Item		Sheet 7 # 22 File No Date 7-22-6/_ Date
---	--	---

Chart 11

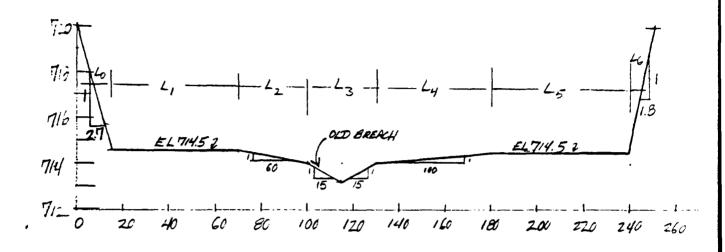


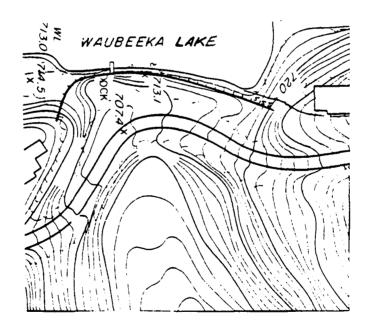
HEADWATER DEPTH FOR CONCRETE PIPE CULVERTS WITH INLET CONTROL





Project MATINAL DOM TASPECTION PROGRAM	C. Contract No. 25/6-05	Sheet 8 4 ZZ File No.
eature WANDERN LAKE CHAM	Designed	Date
tem	Checked	Date





PROFILE ALONG & OF EAST DIKE ESTABLISHED FROM DAVBURY TOWN TOPOGRAPHY, I"=100', AND FIELD SURVEY MEASURE MENTS.

DISCHARGE CURVE FOR EAST DIKE: (READ FLOW AT EL 7/2, INVERT OF 3'\$ RCI WIDER WEST DIKE).



Project NATIONAL ENGINEERING COMPANY, INC. Project NATIONAL THE TOTAL PROJECTION PROGRAM MANAGERA (AMERICA)		Sheet 9 of ZZ
Project NATIONAL DAY SASPECTION PROGRAM	Contract No. 26/6-025	File No.
Feature MANDECKA LAKE JAM	Designed	Date 7-22-8/
Item	Checked	Date

SEGMENT: DISCHARGE OVER EAST DIKE.

$$L_0: Q_4 = 2.3(\frac{2}{5})(2.7)(4-2.5)^{\frac{5}{2}} = 2.48(4-2.5)^{\frac{5}{4}} H \ge 2.5$$

$$Q_{L} = 2.3(\frac{2}{5})60(H-2)^{\frac{5}{2}} \left[1 - \left(1 - \frac{.5}{(H-2)} \right)^{\frac{5}{2}} \right], H \ge 2.5$$

$$Q_{24} = 2.3(\frac{2}{5})(100)(4-2)^{\frac{5}{4}}\left(1-\left(1-\frac{.5}{(4-2)}\right)^{\frac{5}{4}}\right), 4 \ge 2.5$$

Project NATIONAL ENGINEERING COMPANY, INC. Project NATIONAL DAM TOSECTION PROFILED ALABAGE ET LA LANG DAMA	Contract No. 26/6-027	Sheet 6 22 File No.
Feature NAUBEERI LIKE DAM	Designed	Date

DISCHARGE EAST DIKE	H	DUCHARGE 36	RCP (SES MANUSAAPH D-
3 (c/s)	1.5	// (cf)	5 10
21	2	18	<i>39</i>
85	2.5	28	//3
, 325	3	<i>35</i>	360
698	3.5	42	740
1164	4.0	60	1224

DISCHARGE FORMULAS: EAST DIKE

$$2 \leq 1! \leq 2.5 \qquad Q = 27.6(H-1.1)^{\frac{2}{5}} \left(1 - \left(1 - \frac{9}{(H-1.1)}\right)^{\frac{5}{5}}\right) + 147.2(H-2.)^{\frac{5}{5}}$$

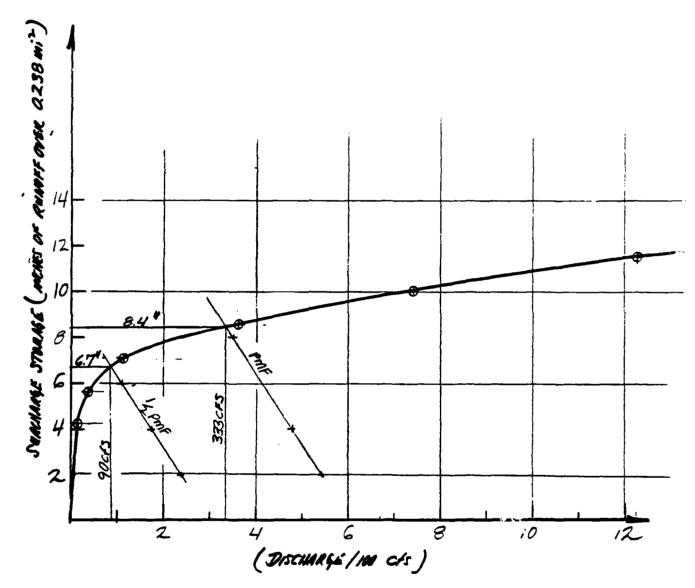
$$2.5 < H$$

$$Q = 27.6(H-1.1)^{\frac{5}{2}} \left[1 - \left(1 - \frac{9}{(H-1.1)} \right)^{\frac{5}{2}} \right] +$$

$$147.2 \left[1 - \left(1 - \frac{5}{(H-2)} \right)^{\frac{5}{2}} \right] (H-2)^{\frac{5}{2}} +$$

Project NATIONAL ENGINEERING COMPANY, INC.).	Sheet 11 of 22	
Project	Z. Contract No. 4	29/07/23 File No	
Feature	Designed		
Item	Checked	Date	

COMMISTE STORAGE - DISCHARGE CURVE OF 3-DIKE RESERVOIR



STAGE STORAGE AT WAUBEEKA LAKE:

H	ELEVATION	INCHES (CUMULATIVE)
1	7/3	2.77
2	714	<i>5.</i> 62
3	7/5	8.54
4	716	11.54
5	717	14.63
6	7/8	/7.80



Project NATIONAL ENGINEERING COMPANY, INC. Project NATIONAL DAM TAISSCIEN PLOCUM		Sheet 12 of 22
Project NATIONAL DAM INCOMO PROGRAM	Contract No. 2616-025	File No.
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JETERMINIATION OF POUTED INFLOW PMF PEAK AND & PMF PEAK

$$\frac{PMF:}{Q_{P_2} = Q_{P_1} \left(1 - \frac{S}{19}\right)}$$

	4PZ	
8	351	RESULTS: THANHUM RISE OF POOL; TO EL. 715.0 TISL
	479	FROM 712.0 WSL
•	EV2	MAXIMI ON FLOW Q ; 333 cfs

$$Q_2' = Q_1' \left(1 - \frac{s}{9.5} \right)$$

5	Q_	
6	112	RESULTS: MAXIMILM RISE OF POOL; TO EL. 714.4 WSL FROM 712.0 NISL
4	176	
7	240	maximum outflow Q ; 90 cfs

& BASEN IN MORMANOUS DA OF Z MIT.



Item Date Date	Designed Date	
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STORAGE CURVE OF THE RESERVOIR, PAGE D-11 IN TABULAR

FORM, A STORAGE-DISCHARGE CURVE WAS OBTAINED, PAGE D-11.

3) SAMWAY CAPACITY RATIOTO PEAK INFLOW & OUTFLOW.

THE CAPACITY OF THE 3'D RCP CHIVERT FLOWING FULL IS 35cfs.

TO CAP. OF INFLOW PMF : 5.8%

" " ONTFOW " : 10.5%

" " " INFLOW & PMF : 11.5%

" " OUTFLOW " : 38.9%

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tem		Date

II DOWNSTREAM FAILURE NATARO

1) POTENTIAL IMPACT AREA

THE POTENTIAL IMPACT AREA & SHOWN ON ENCLOSED TOPOGRAPHICAL MAIL, PAGE D-21. THE APPROXIMATE WATER LINE CREATED

BY A BREACH THROUGH THE ROAD-EMBANKMENT DIRE IS

ALSO INDICATED. ONLY TWO PRIVATE RESIDENCES IMPLICATELY

POWNSTAEAM OF THE CULVERT (100-200 FEET) WILL BE

AFFECTED BY SEVERAL FEET OF WATER AROUND THE

FOUNDATIONS, BUT NO WATER REACHING FIRST FLOOR

ELEVATIONS.

- 2) FAILURY AT WAUBELRA LAKE DOM.
 - a) BREACH WIOTH
 - i) HEIGHT OF DOM: WAS TAKEN FROM MAXIMUM

 RISH OF POND TO INVERT OF OUTLET: 715-709.54 =

 5.46
 - ii) MID HEIGHT OF DAM = 2.73'
 iii) APPRIXIMATE MID-NEIGHT LENGTH = 84'
 iv) BREACH WIOTH .4(84) = 34



Project INTERNATIONAL ENGINEERING COMPANY, INC. Project NATIONAL DAM INSPECTION PROGRAM		Sheet 154 22
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- b) PEAK FAILURE OUTHOW
 - i) HEIGHT AT TIME OF FAILURE

ii) SPILLWAY DISCHARGE AT TIME OF FAILURE:

iii) BREACH OUTFLOW:

IN) PEAK FAILURE OUTFLOW

THE BREACH WILL INCLUDE THE SPILLWAY AND

THEREFORE THE PEAK OUTFLOW WILL BE 730 CFS

C) FLOOD DEPTH IMMEDIATELY DIS OF DAMS
$$d = 0.44 \, Y_0 = 2.4'$$

d) ESTIMATE OF DIS FAILURE CONDITIONS AT POTENTIAL IMPACT AREA.

THE DEPTH OF FLOW WAS COMPUTED FOR n = 0.08, FOR Q = 730 CFS THROUGH THE IMPACT AREA. IT WAS EXPECTED (AND SESSEPHENTLY VERIFIC) THAT THE MAY PAYED WOULD BE USED, AND DOWNSTREAM ROWING

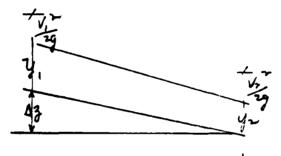


WOULD BE WARECESTARY.

REPRESENTATIVE CROSS SECTIONS AND PROFILE ARE SHOWN ON PAGE D-18.

TABULATIONS OF PERTINENT HYDRAULIC PARAMETERS ARE PRESENTED ON PAGE 0-19.

A MODIFIED STANDARD STEP APPROACH WAS USED AS EXCLOSES:



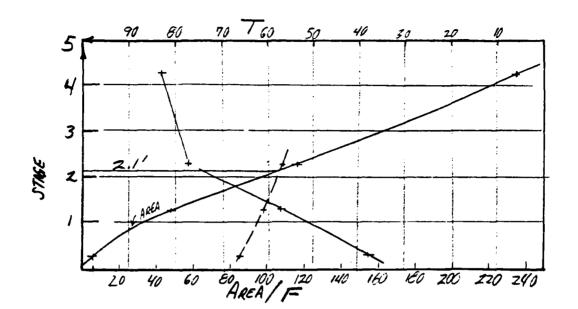
$$\frac{V_i^2}{2q} = \left(\frac{Q}{A_i}\right)_{2q}^2 = \left(\frac{k_i}{A_i}\right)^2 \frac{f_i}{2q}$$

ASSUMED THAT FLOW PASSES THROUGH CRITICAL DEPTH IN SEC-1.

$$\dot{H} = \left[\frac{Q^2 T}{g}\right]^{\frac{1}{3}}$$

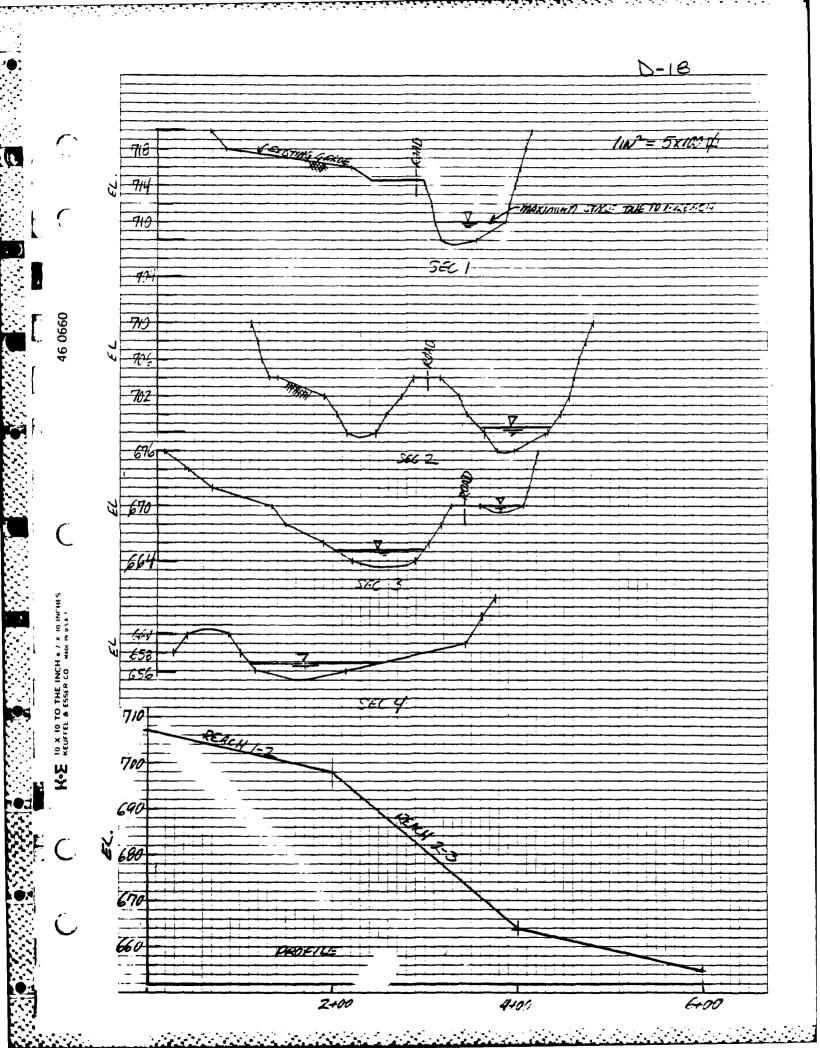


Project NATIONAL CAM TASPECTION PROGRAM	20 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Sheet /7 of 22
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INITAL STAGE AT CRITICAL SECTION-1: 2.1' (EL 709.6)





Project NATIONAL SAM INDECTION PROGRAM	Contract No. 26/6-025	Sheet $\frac{19 + 27}{1}$ File No.
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SEC 1

EL	Ų	A	K	(4A) 2/20,	4(4)
709.6	z./	93	2274	8.37	13.26

SEC 2

EL	¥	A	K	(NA) 1/29	44
697	1,2	34	537	3.97	193
678	2.2	100	2387	8.90	12.39
699	3.2	181	5568	14.65	5.17

SEC 3

<u>::-</u>	-1/	1 4	K	(K/A)=/20	7140
664	6.5	23	202	1.24	1320
·	1.4	84	1490	4.89	76.6 4
655	1.5	91	1690	5.36	71.2
667	3.5	327	10,660	16.56	4.05
666	2.5	187	4870	10.58	4.98

sec 4

EL	4	A	TK	(k/A)=/29	1 1/19
656	1	64.	907	3.12	67.8
656.8 657	1.8	156	3270 4200	6.83	7.12 5.26
658	3	_	_	_	-



Project Name International Engineering Company, Inc.

Project Name International Engineering Company, Inc.

Feature Name International Engineering Program Contract No. 26/6-125 File No.

Designed Date 724-8/

Checked Date $\frac{\text{EENCH } 1-2:}{\text{Y}_1 + (70.5-96.8) + (8.37-100)} \frac{730}{2.274} = 4.36$ INTERPOLATING AT SEC-2, $Y_2 = 2.2^{1}$ EL 698

100

K 2387

(4/1/70 890

Yy, 12.42133

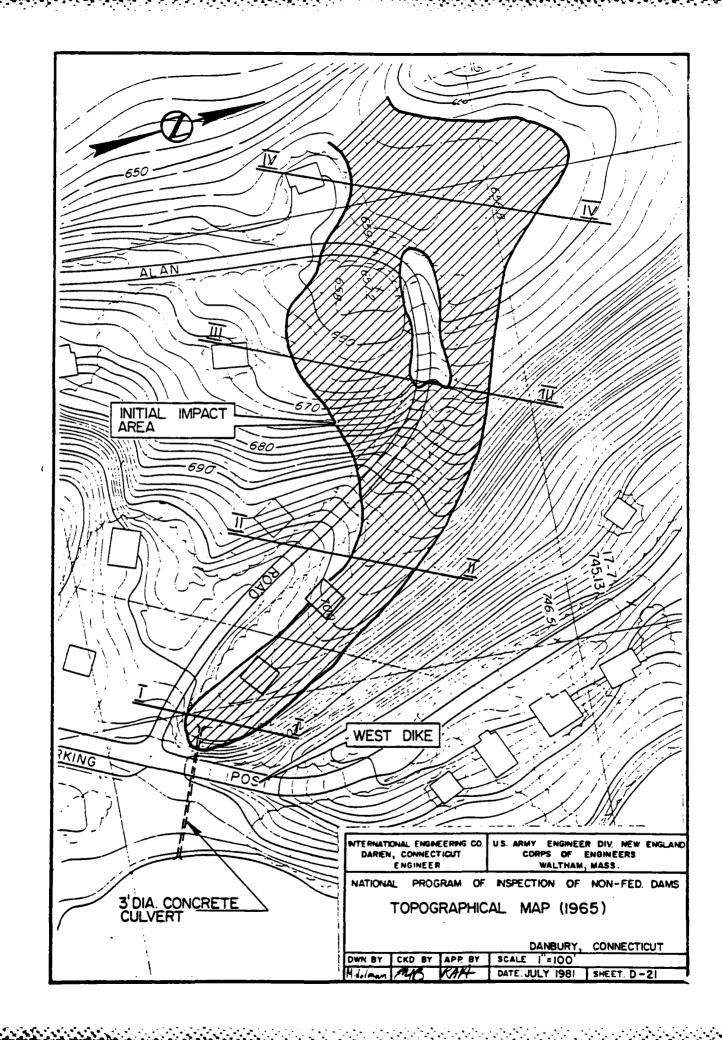
REACH 2-3

7/2+(6953-6635)+(8.90-100)(730) = 26.0

$$V_3 = 1.4'$$
 $A_3 = 84$
 $K = 1490$
 $(K/A)^2 h_9 = 4.89$
 $4y) = 26.6 = 26.$

LEACH 3-4

$$-\frac{1}{3} + \frac{(63.5 - 655)}{(63.5 - 655)} + \frac{(4.89 - 100)}{(1490)} \frac{720}{1490} = -12.9 \text{ in No. With Months.}$$
Assume $\int_{3}' = \int_{f_{y}} = \int_{0}' = \int_{$



	INTERNATIONAL ENGINEERING COMPANY, INC. NATIONAL DAM TROSPERS WALBERS LAKE DAM	- 2 <i>// 121</i> ~	Sheet 22 of 22
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Feature	WAUBERRA LAKE DAM	Designed	Date 7-24-44
Item		Checked	Date

III SELECTION OF TEST FLOOD

MAXIMUM RISE OF WATER SURFACE FROM CHANNEL—

OVERLAND INVEST IS 2.2'. Thin imm HEIGHT OF

FIRST FLOOR OF HOUSE IN POTENTIAL IMPACT ALEA IS

3 FEET. THEREFORE; NO DAMAGE WILL RESULT.

a) Size < 130 Ac-ft ... SMALL

HEIGHT = 6' (BASED ON MAXIMUM RISE OF TU/S)

6) HARARO POTENTIAL: BASED ON BREACH ANALYSIS;

LOW