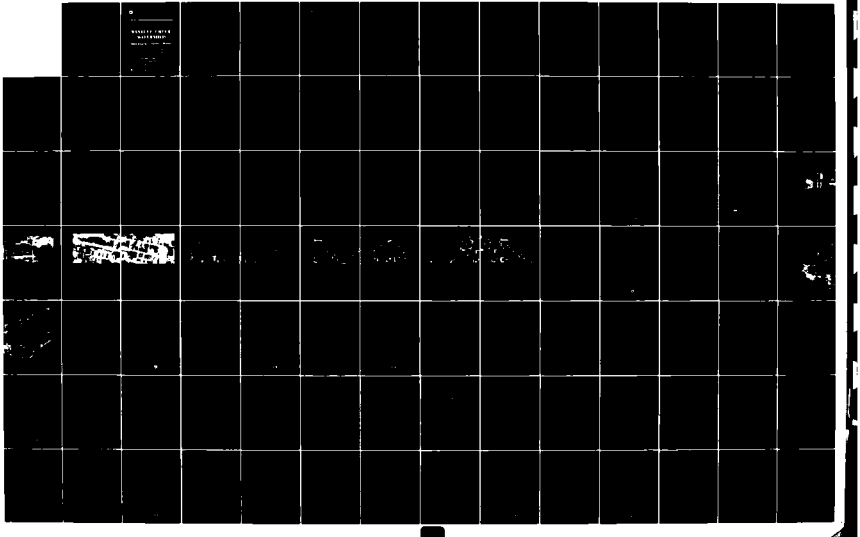
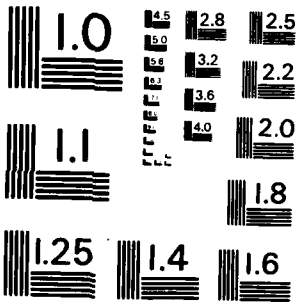
AD-A133 795 BASSETT CREEK WATERSHED HENNEPIN COUNTY MINNESOTA FEASIBILITY REPORT FOR FLOOD CONTROL MAIN REPORT(U) CORPS OF ENGINEERS ST PAUL MN ST PAUL DISTRICT SEP 82 1/7

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US Army Corps of Engineers
St. Paul District

SEPTEMBER 1982

AD-A133 295

Flood Control

BASSETT CREEK WATERSHED

Hennepin County, Minn.

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Design Memorandum No. 2
Phase II
General Project Design
and
Environmental Assessment

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REPORT DOCUMENTATION PAGE	READ INSTRUCTIONS BEFORE COMPLETING FORM
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	6. PERFORMING ORG. REPORT NUMBER
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See AD-A119 394/5 for appendices.

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19. KEY WORDS *(Continue on reverse side if necessary and identify by block number)*

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20. ABSTRACT *(Continue on reverse side if necessary and identify by block number)*

The Bassett Creek watershed is located within the Minneapolis-St. Paul metropolitan area and entirely within Hennepin county. Potential for flooding is expected to increase with urbanization of the watershed. The sstudy reviews the water and related land resource problems and potential solutions to these problems.

The main report is a general nontechnical presentation of the feasibility study of water and related land resources in the Bassett Creek watershed.

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This basic document presents a broad view of the overall study and is intended for the general reviewer. Included in the main report are plan formulation and selection procedures, division of project responsibilities between Federal and non-federal interests and recommendations for implementing the selected plan.

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
1135 U S POST OFFICE & CUSTOM HOUSE
ST PAUL, MINNESOTA 55101

REPLY TO
ATTENTION OF: NCSED-M

30 September 1982

SUBJECT: Flood Control, Bassett Creek Watershed, Hennepin County, Minnesota, Design Memorandum No. 2, Phase II, General Project Design

Commander, North Central Division
ATTN: NCDED-T

1. The subject design memorandum presents designs and discussions of engineering studies and the environmental assessment for flood control improvements in the Bassett Creek Watershed and is submitted in accordance with ER 1110-2-1150.

2. By NCSED-PB letter dated 30 August 1979, subject: Bassett Creek Watershed, Hennepin County, Minnesota, Phase I GDM, Study Classification, the St. Paul District recommended deletion of the Phase I GDM and initiation of Phase II GDM studies. This recommendation was approved by 1st Indorsement, NCDED-T, 4 September 1979. Design Memorandum No. 1, Hydrology and Hydraulics, presented hydrology and hydraulics design information normally presented in a Phase I GDM study.

3. I have met with local officials of the cities of Minneapolis and Golden Valley and with representatives of the Bassett Creek Flood Control Commission to discuss project sponsorship. The city of Minneapolis understands and fully supports this project and has indicated an interest to discuss innovative project financing proposed by the current Administration.

Edward G. Rapp

EDWARD G. RAPP
Colonel, Corps of Engineers
Commanding

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St. Paul, Minnesota 55101

FLOOD CONTROL

BASSETT CREEK

HENNEPIN COUNTY, MINNESOTA

DESIGN MEMORANDUM NO. 2, PHASE II

AND

ENVIRONMENTAL ASSESSMENT

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Department of the Army
St. Paul District, Corps of Engineers
1135 U.S. Post Office & Custom House
St. Paul, Minnesota 55101

FLOOD CONTROL
BASSETT CREEK WATERSHED
HENNEPIN COUNTY, MINNESOTA

DESIGN MEMORANDUM NO. 2

PHASE II
GENERAL PROJECT DESIGN
AND
ENVIRONMENTAL ASSESSMENT

Design Memorandum Index

D.M. No.	Item	Scheduled Completion	Submittal NCD	Submittal OCE	Approved
1	Hydrology & Hydraulics	Jul 81	May 81	Jul 81	Aug 81
2	Phase II - General Project Design	Aug 82	Sep 82		
3	Tunnel Feature Design	Oct 83			
	Environmental Assessment and FONSI		Sep 82		

Department of the Army
St. Paul District, Corps of Engineers
1135 U.S. Post Office & Custom House
St. Paul, Minnesota 55101

FLOOD CONTROL
BASSETT CREEK WATERSHED
HENNEPIN COUNTY, MINNESOTA

GENERAL DESIGN MEMORANDUM
PHASE II

Pertinent Data

Project Document - House Document No. 95-360, 95th Congress, 2d Session.

Project Authorization - 1976 Water Resources Development Act (Public Law 94-587).

Project Purpose - Flood Control.

Location - The project is located in Hennepin County and Minneapolis, Minnesota, on Bassett Creek, a tributary of the Mississippi River.

Hydrology and Hydraulics

Watershed drainage area	43 sq. miles
Design flood frequency	Once in 100 years
Design flood flow (tunnel)	660 cfs

Principal Items of Work

Tunnel, 10' Diameter Equivalent	6,600 L.F.
Tunnel, 12' Diameter Equivalent	4,800 L.F.
Flood Control Structures	7
Earth Embankments for Control Structures	5
Bridge Removals	2
Railroad Bridge Replacement	1
Railroad Bridge Modification	1
Low Head Dam Removal & Replacement	1

Culvert Replacements	9
Culvert Removal	1
Floodproofing Residential Structures	24
Channel Improvement & Bank Protection	2,085 L.F.
Bank Protection	2,085 L.F.
Storage Area Excavation (2 areas)	282,000 cy

Economics

Federal First Cost	$22,300,000
Non-Federal First Cost	6,240,000
Total First Cost	28,540,000
Average Annual Operation & Maintenance Cost	17,500
Total Average Annual Cost	2,329,200
Average Annual Benefits	2,802,100
Benefit-Cost Ratio	1.20
Benefit-Cost Ratio including Area Redevelopment Benefits	1.31

AUTHORIZATION

1. The authority for this project is contained in Section 173 of the 1976 Water Resources Development Act (Public Law 94-587) which states:

> "The project for flood protection in the Bassett Creek Watershed, Minnesota, is hereby adopted and authorized to be prosecuted by the Secretary of the Army, acting through the Chief of Engineers, substantially in accordance with the plans and subject to the conditions contained in the report of the Board of Engineers for Rivers and Harbors dated July 26, 1976, at an estimated cost of $7,953,000. This shall take effect upon submittal to the Secretary of the Army by the Chief of Engineers and notification to Congress of the approval of the Chief of Engineers."

2. Submittal of the favorable report of the Board of Engineers for Rivers and Harbors to the Secretary of the Army occurred on 30 March 1977. Notification to Congress of the approval of the Chief of Engineers occurred in a letter from the Secretary of the Army dated 19 June 1978.

LOCAL COOPERATION

3. The local project sponsor is the city of Minneapolis, Minnesota. By resolution dated 11 December 1981, the city of Minneapolis has indicated its support for the project and willingness to provide the required items of local cooperation. The items of local cooperation include:

a. Provide without cost to the United States all lands, easements, and rights-of-way, including borrow areas and disposal areas for excavated material determined suitable by the Chief of Engineers and necessary for construction of the project;

b. Hold and save the United States free from damages that may result from construction of the project, not including damages due to the fault or negligence of the United States or its contractors;

c. Maintain and operate all the works after completion in accordance with regulations prescribed by the Secretary of the Army;

d. Share in the cost of the nonstructural portion of the project in accordance with cost-sharing provisions being developed under Section 80 of Public Law 93-251;

e. Accomplish without cost to the United States all relocations and alterations of buildings, transportation facilities, storm and sanitary sewer systems, public and private utilities, local betterments, drainage facilities, and other structures and improvements made necessary by construction of the recommended plan, excluding railroad bridges and approaches and facilities necessary for the normal interception and disposal of local interior drainage at the line of protection;

f. Prescribe and enforce regulations to prevent obstruction or encroachment on channels and temporary storage areas which would reduce their flood-carrying or storage capacity or hinder maintenance and operation;

g. Provide a cash contribution for recreation equal to 50 percent of the final separable cost allocated to this function less a credit for the value of lands, easements, rights-of-way, alterations, and relocations allocated therefor;

h. Publicize flood plain information in the areas concerned and provide this information to zoning and other regulatory agencies for their guidance and leadership in preventing unwise future development in the floodplain and in adopting such regulations as may be necessary to insure compatibility between future development and protection levels provided by the project; and

i. At least annually inform affected interests regarding the limitations of the protection afforded by the project.

4. Sponsoring communities within the watershed have a long history of providing leadership in preventing unwise future development in floodplain areas. Most communities in the watershed had established floodplain development policies well before the Federal Insurance Administration's floodplain management requirements were completed. The Bassett Creek Flood Control Commission was established in 1969 under a joint powers agreement to provide leadership in floodplain management for the nine watershed communities. The Commission immediately acquired the services of a water resources consulting engineering firm to provide the technical expertise required in providing wise management leadership. Periodically, the Commission distributes newsletters to keep area residences and businesses informed on floodplain development issues and of progress on the flood control project. An appraisal report prepared in August 1982 for the city of Golden Valley reveals that over 470 acres of floodplain properties in five communities have been acquired at a cost of $3,970,000 to prevent floodplain development. Although the properties have been turned into recreational areas the intent in acquisition was to prevent floodplain development.

5. Since adoption of the Commission's overall plan in 1972, no development has occurred along the main stem or tributaries which would be damaged by the project's design event. The Commission has had review authority (individual communities have enforced the Commission's recommendations) on any development in or near a management envelope which includes from 1 to 3 feet of freeboard over the 100-year flood as determined in 1972.

6. On 23 July 1982, the St. Paul District Engineer met with Mayor Donald Fraser, city of Minneapolis, and other elected and staff officials of the city of Minneapolis, the city of Golden Valley and with representatives of the Bassett Creek Flood Control Commission to reaffirm the sponsoring agency's understanding and support for the project and interest in providing the required items of local cooperation under the cost-sharing policies now being developed. The city of Minneapolis has indicated that it fully understands and supports the project and by letter dated 17 August 1972, has indicated an interest to discuss innovative project financing as proposed by the current Administration.

LOCATION OF PROJECT AND TRIBUTARY AREA

7. The Bassett Creek watershed is located within the Minneapolis-St. Paul Metropolitan area and entirely within Hennepin County. The total area of the watershed is approximately 43 sq. miles and contains portions of nine municipalities: Minneapolis, Robbinsdale, Golden Valley, St. Louis Park, Crystal, New

Hope, Plymouth, Medicine Lake and Minnetonka. Located in a rapidly expanding metropolitan area, approximately 70% of the watershed is in a state of total or partial urban development with urbanization varying from total within the city of Minneapolis to negligible in the western watershed.

8. The average slope of Plymouth Creek, in the upper watershed, from its source to Medicine Lake is approximately 17 feet per mile. From Medicine Lake to the Mississippi River, the slope of Bassett Creek averages approximately 9 feet per mile over its 12.1 mile length. The lower half of the tributary North Branch Bassett Creek drops at approximately 25 feet per mile as does a short reach of the Sweeney Lake Branch, another contributor. Quick runoff combined with relatively few storage areas and existing constrictions such as channel crossings and the tunnel, the only watershed outlet, combine to create flooding all along Bassett Creek.

ALTERNATE PLANS INVESTIGATED

9. The 1976 Feasibility Report has generally provided the project formulation. Authorized for construction by the 1976 Water Resources Development Act on the basis of the Feasibility Report, the project had, and still maintains, a very strong backing from local sponsors and their Congressional representatives. Also, no significant objections were ever made to this flood control project, although several agencies commented on project feature designs or requested that general environmental concerns be acknowledged during construction. There was a consensus among the affected municipalities that this array of features was most appropriate for the highly developed watershed.

10. Upon initiation of post authorization studies in fiscal year 1979, the project sponsors, represented by the Bassett Creek Flood Control Commission, requested the Corps to expedite project design to allow earlier project completion. Upon examination of the project formulated for the watershed, it was determined that the array of proposed structural and non-structural features including the cooperative tunnel and open space flood storage was, indeed, most appropriate and that remaining design could be most productively directed toward verifying project hydrology and hydraulic design; optimizing designs for structural measures, and finalizing layout for project features.

11. On 4 September 1979, a St. Paul District request to delete the Phase I GDM study and advance directly to Phase II was approved. In May 1981, Design Memorandum No. 1, Hydrology and Hydraulics, redeveloped the project hydrology and hydraulic design and supported the Feasibility Report formulation.

12. This submittal of Design Memorandum No. 2, Phase II-General Project Design, advances design for project features. While the general project formulation remains unchanged, many alternative plans for individual project features or groups of features were reviewed. As a result, some authorized project features including road raises, creek clearing and snagging and a reach of floodwall are no longer proposed. Proposed new or modified project features include the slight realignment and altered construction concept for the conduit and a new railroad bridge.

HYDROLOGY

General

13. The basic hydrologic studies were reported in Design Memorandum No. 1, Hydrology and Hydraulics, May 1981. The hydrologic studies included descriptions of the watershed characteristics and climate, the storm characteristics for the basin, the watershed model used to develop runoff hydrographs for synthetic floods and hypothetical floods. It also provided an estimate of the probable maximum flood and standard project flood and an analysis of the flood frequency curves. A brief summary of these studies is given in the following sections.

Climate

14. The project area has a moderate climate typified by warm, humid summers and cold moderately humid winters. The mean annual temperature for the watershed is 44°F. The mean monthly temperature varies from 12°F in January to 72°F in July. Annual precipitation averages 26 inches and average annual snowfall is 44 inches.

Watershed Model

15. The hydrologic analysis of the basin was conducted using the Hydrologic Engineering Center's HEC-1 computer program (723-X6-L2010). The model developed for this study evaluates storm water runoff from 55 subwatersheds forming the Bassett Creek watershed. The subwatersheds vary in size from .06 to 5.77 square miles. The total basin area is approximately 43 squar2 miles. Generally, the outlet for each subwatershed is a restrictive channel crossing which acts as a control for the ponding area upstream.

Discharge-Frequency Curves

16. The watershed model was used to determine discharge frequency curves at 20 locations in the Bassett Creek watershed for ultimate land use and ultimate land use with project in place. The 100-year discharges for these 2 cases are shown in Table 1.

Standard Project Flood

17. The Standard Project Flood was determined for the Bassett Creek watershed using the HEC-1 runoff model. A comparison of Standard Project Flood discharges for ultimate watershed development versus ultimate development with project is shown in Appendix A. There are three reaches where the Standard Project Flood discharges are higher for the with project case than for the without project. These increases do not increase damage potential because the water surface profiles are the same as or lower than the without project condition due to channel modifications.

GEOLOGY AND SOILS

18. Most of the features for this project are small and affect the subsurface to only a shallow depth; therefore, accurate definition of the depth to bedrock and the bedrock statigraphy are not important considerations except for a portion of the proposed tunnel. The geology pertinent to the project consists

of soft alluvial silts and clays, bog deposits of peat, organic rich silts and %lays resting on medium dense glacial sands and tills. Along the proposed tunnel alignment the materials consist of Mississippi River terrace deposits of soft clays, silts, and medium dense sand, medium dense to dense glacial drift and sandstone bedrock of the St. Peter Formation. A detailed discussion of geology and soils is contained in Appendix C. Subsurface explorations, stability analyses, and discussions concerning all elements of geotechnical design can also be found in Appendix C.

INTERIOR DRAINAGE

19. The four areas along Bassett Creek protected by the proposed flood barriers, namely the Western Avenue-Winnetka Avenue area, the Highway 100 North area, the Fruen Mill area below Glenwood Avenue and Bermed area by the conduit entrance, are subject to flooding from interior runoff. The contributing watershed for each area is determined by natural topography, streets, or existing city storm sewer systems. The proposed interior flood control plan for each area includes an outlet in conjunction with a ponding area. An interior ditch is also included in the Highway 100 North plan. The proposed ponding areas and outlets have sufficient capacity to prevent damages from the 100-year event, therefore, pumping stations cannot be justified. Details of the interior flood control analysis and plan are presented in Appendix B.

RECREATION

20. A full analysis of recreation demand, supply, and needs was conducted as part of the March 1976 Feasibility Report. That evaluation, which is still considered to accurately reflect local needs, indicated that there is a need for additional trail developments and parklands in the project area. Recreational concepts/potentials associated with the proposed project have been explored with non-Federal sponsors. However, at this time, a lack of local commitment to development of project-related recreation facilities, availability of local funds for recreation, federal recreational policies, and the general health of the economy have made it impossible to refine the recreation plan contained in the Feasibility Report. Therefore, a recreation plan of development and the associated cost-sharing contract have not been included as project features at this time, although a recognized need and potential for trail developments and park lands still exist.

BEAUTIFICATION AND ENVIRONMENTAL QUALITY PLAN

21. Implementation of the proposed flood control measures involve the removal of many old and/or substandard structures and the construction of new structures through urban areas. Such construction unavoidably impacts existing vegetation in the project area. This loss of vegetation will have varying degrees of adverse visual impacts upon the communities affected. To offset these impacts and thereby make the Bassett Creek project more socially and environmentally acceptable, beautification measures such as landscaping and rustification are proposed as an integral part of the overall project. See Appendix G for details of the beautification plan.

DESCRIPTION OF AUTHORIZED PROJECT

22. The authorized plan for flood control in the Bassett Creek watershed, Hennepin County, Minnesota, involves a combination of both nonstructural and structural measures. A summary description of the project taken from the authorization document reads as follows:

a. "Nonstructural measures include flood proofing of 19 residences and evacuation of 3 residences in the upper watershed and continuance of local flood plain ordinances modified to reflect proposed conditions.

b. The plan includes two principal structural measures; one, a new conduit to replace the existing conduit outletting Bassett Creek to the Mississippi River and the other, a series of small control structures in the upper watershed to temporarily impound flood waters. The lower portion of the new conduit would be constructed in cooperation with the Minnesota Department of Highways under its proposed Interstate Highway program in Minneapolis. Cooperative construction of the new conduit would result in estimated cost savings of about $5.5 million and would prevent severe damages that could occur to industrial developments under present conditions.

c. Other structural measures included in the selected plan in the upper watershed include a limited reach of channel widening, snagging and clearing, road raises, bridge removals, culvert replacements and construction of a new weir at the outlet of Medicine Lake.

d. The temporary flood water storage impoundments and other structural measures acting in conjunction would reduce flood damages to residential developments in the upper Bassett Creek watershed. The new weir structure at the outlet of Medicine Lake would reduce flood water levels of the more infrequent floods and thereby reduce damages to residential properties abutting the lake.

e. None of the features of the selected plan would alter normal streamflow or lake level conditions.

f. Associated features of the selected plan include the development of a permanent marsh impoundment at the entrance to the proposed conduit, which reduces flood damage, provides wildlife enhancement, and contributes to the development of a recreation trail system for bicycling and walking. The trail system would provide a link between existing parks and trails at the lower end of the watershed."

DEPARTURES FROM AUTHORIZED PLAN

23. Post-authorization studies have provided more detailed information on improvements needed to provide design flood protection in the Bassett Creek watershed. A revised study of project hydrology and hydraulic design has altered project design as follows:

a) The conduit sizes were increased from 11 feet to 12 feet diameter for the downstream tunnel portion and from 8.5 feet to 10 feet diameter for the upstream portion.

b) The upstream portion of the conduit has been lowered significantly to go under existing utilities and because of the high cost of cut and cover in this developed area.

c) The authorized plan included the development of a permanent marsh impoundment at the entrance to the proposed conduit. The proposed plan still maintains a flood storage area, but, instead of a permanent marsh, a normally dry surge area providing the required storage volume would be constructed. The creek channel would be relocated and 91 acre feet of storage would be provided in a low surge area graded to allow surface drainage to the creek. The surge area would remain dry at normal creek flows and would provide a potential site for recreational activities.

d) Clearing and snagging from the tunnel storage area to below Glenwood Avenue was eliminated since it did not lower flood stages greatly.

e) Channel enlargement and construction of floodwalls downstream of Glenwood Avenue have been eliminated since adequate freeboard (1.0 foot) below damage elevation is possible within the existing channel. This is now possible because discharges in this reach have been lowered from the Feasibility Report. However, replacement of the lowhead dam (a control on the elevation of Wirth Lake) and some walls for bank stabilization are still required.

f) Replacement of the Soo Line (formerly the Minneapolis, Northfield and Southern) Railroad bridge just downstream from Glenwood Avenue was found necessary since the existing bridge is very obstructive to flow.

g) Relandscaping of Rice Lake to provide additional flood storage was found to be unnecessary. Construction of a roadway across Rice Lake increased the effective storage by reducing the outlet capacity.

h) The culvert replacement at Adair Avenue has been eliminated. The culvert provided drainage for a small corner area at an intersection and has since been covered and is no longer effective. It is not necessary for local storm drainage.

i) Clearing and snagging of Bassett Creek upstream from the Trunk Highway 100 embankment was eliminated since it would have little effect on flood stages in a storage area.

j) All road raises previously proposed to maintain access during the design flood have been eliminated due to the high cost and limited benefits because of short flood durations. Also, conflicts with utilities and storm drainage would occur. In requesting that these project features be eliminated, local authorities have assured that emergency vehicle access throughout the watershed can be accomplished by using other routes. It is believed that no residual safety hazard remains as a result of the road raise deletions.

k) A berm along Winnetka Avenue will be required to prevent water stored by the Wisconsin Avenue control structure from backing into a low, swampy area east of Winnetka Avenue and north of Western Avenue. Normal runoff from the low area will be maintained with a 24" culvert.

PROPOSED STRUCTURES AND IMPROVEMENTS

GENERAL

24. Principal structural flood protection measures for Bassett Creek include a new conduit outlet to the Mississippi River and a series of small control structures in the watershed to temporarily impound floodwaters. Also included in the plan are bridge removals, culvert replacements, a replacement weir structure at Medicine Lake, replacement of a railway bridge and excavation for flood storage. Nonstructural measures include floodproofing 24 residences, acquisition of one residence and continuation of existing floodplain regulations.

CONDUIT

25. The plan for providing an outlet for Bassett Creek flows to the Mississippi River includes abandoning the existing Bassett Creek tunnel beginning at its inlet near 2nd Avenue North and Dupont Avenue and constructing a new tunnel along a new alignment, parts .f which will be completed on a cooperative basis with the Minnesota Department of Transportation (MnDOT). The tunnel will be completed in three major segments with the lower two segments providing cooperative storm drainage for Bassett Creek and new area Interstate Highways.

26. The lower segment of tunnel has been constructed by MnDOT (Feature No. 1) and has been sized to accommodate design Bassett Creek, I-94 and I-394 3rd Avenue Distributor flows. Work has been completed from the tunnel outlet at the Mississippi River, just below the Upper St. Anthony Falls Lock and Dam, to the wye under 2nd Street North, between 2nd Avenue North and 3rd Avenue North. The 2nd Street tunnel is now in use, providing storm drainage for I-94. The 4800 feet of cooperative tunnel in use has been constructed with MnDOT funds and cost allocation is to be based on a ratio of peak design flows generated by each project. A similar methodology and the same frequency of occurrence has been used in the computation of each project's design flows. This cost allocation formula also applies to the middle tunnel segment (Feature No. 2), a cooperative facility to provide storm drainage for Bassett Creek and the 3rd Avenue Distributor of I-394. This tunnel reach from the existing wye upstream to the proposed drop inlet location for I-394 flows near 7th Street North and 3rd Avenue North will be constructed by the Corps of Engineers. The Corps will also construct the proposed drop inlet structure, as requested by MnDOT.

27. The upper reach of tunnel (Feature No. 3) from the I-394 inlet drop structure to the control/drop structure near 2nd Avenue North and Dupont Avenue will be constructed by the Corps solely for Bassett Creek flows.

28. Plates 3 and 4 show the lower tunnel segment plan and profile as constructed by MnDOT. Plates 5 through 7 show the upper two segments of tunnel in plan and profile. All 3 reaches of tunnel will receive additional study in a tunnel feature design memorandum due for submittal in October 1983. The tunnel feature design memorandum will also advance design on the tunnel inlet storage area and the drop structure to the storage area.

CONTROL STRUCTURES

29. Control structures with overflow weirs and low flow orifices will be constructed at six locations throughout the watershed to temporarily impound

runoff. Three such structures will be located in the city of Golden Valley - one at the location of a proposed new main stem Bassett Creek crossing for Wisconsin Avenue (Feature No. 20), one in the Golden Valley Country Club golf course (Feature No. 19), and one at the Bassett Creek crossing of Trunk Highway 55 (Feature No. 10).

30. Two control structures will be located in the city of Crystal - one, the Edgewood Embankment (Feature No. 31), near the intersection of 36th Avenue North and Douglas Drive, and the other at the main stem Bassett Creek crossing of Trunk Highway 100 (Feature No. 16). One control structure will be provided in the city of Minneapolis at the tunnel inlet near the intersection of 2nd Avenue North and Dupont Avenue (Feature No. 5A). A flood storage area (Feature No. 5) will be excavated immediately upstream, southwest of the control structure, in an undeveloped area bounded by the Burlington Northern railroad on the south, Cedar Lake Road on the west and the existing creek bed on the north. The existing control structure for Medicine Lake will be removed and replaced (Feature No. 21) just downstream, south of South Shore Drive, at the Northwestern Railroad bridge over Bassett Creek. The railroad embankment will serve as the tie-back for the control structure.

31. Each control structure will leave the creek virtually unaffected during normal flow conditions. Storage occurs with increasing runoff and quantities stored vary with with the site and timing and intensity of rainfall. Storage under design 100 year event conditions are generally measured in hours and storage levels will begin to recede when runoff to the storage areas is reduced to levels below the discharge at the control structures. For lesser frequency events, storage upstream for control structures will generally result in higher water levels under proposed conditions and relatively more maintenance will likely be required in park areas after rainfall events. The effect of each control structure will be to lower peak discharges immediately downstream. The combined effect of all storage will be to decrease the size of the outlet conduit and reduce measures to increase stream capacity.

BRIDGE REMOVALS

32. The timber bridge over Bassett Creek at Minnaqua Avenue (Feature No. 15A) represents an unnecessary constriction since this crossing is not needed by local traffic. The creek can be crossed at Regent Avenue, approximately 1 city block away. The bridge and the abutments will be removed and relandscaped to match the adjoining grounds. The nine residences on Minnaqua Avenue east of the creek will front a dead-end street with a cul-de-sac near the creek. Just west of the creek, Minnaqua Avenue immediately intersects with Scott Avenue. Through traffic along Scott Avenue and Minnaqua Avenue will not be affected. At the Penn Avenue crossing of Bassett Creek in Minneapolis (Feature No. 8) an old bridge crossing remains intact under a new bridge built well overhead. Although the old bridge, a large concrete box structure, lies in a park area, it is not used as a crossing except occasionally by pedestrians. Access across the creek can be accomplished on the new bridge above. The old Penn Avenue crossing will be removed and relandscaped.

CROSSINGS

33. Proposed new culvert crossings are required for increased capacity at 6 locations in the city of Crystal and 3 locations in the city of Golden Valley. The Golden Valley replacements are located at Noble Avenue (Feature No. 13),

Regent Avenue (Feature No. 14) and Westbrook Road (Feature No. 18). Replacement culverts to be provided in Crystal are located at 32nd Avenue (Feature No. 23), Brunswick Avenue (Feature No. 24), 34th Avenue (Feature No. 28), Douglas Drive (Feature No. 29), Georgia Avenue (Feature No. 33) and the crossing at 36th and Hampshire Avenues (Feature No. 34).

34. All of the proposed culvert crossings except for the Westbrook Road and Douglas Drive culverts will be constructed and in use by the end of the 1982 construction season. The city of Golden Valley will complete this work under the Section 215 (P.L. 90-483) agreement. Two of the crossings, at Noble and Regent Avenues, will use a new arch-bridge type structure which maintains the creeks natural bed. The arch-bridge structures were bid on as alternate designs. Based on bids received, the city of Golden Valley decided to use the alternate designs which are considered to be hydraulic equivalents of the designs shown on Plate 21. See Appendix I for the alternate designs.

RAILROAD BRIDGES

35. The Soo Line Railroad bridge crossing of Bassett Creek in Minneapolis near the intersection of Glenwood and Inglewood Avenues (Feature No. 9B) represents a potentially serious creek obstruction capable of causing localized flooding in the industrial mill area. This bridge will be removed and replaced along a slightly modified alignment to utilize a shorter bridge span and allow a larger open flow area.

36. The Northwestern Railroad bridge over Bassett Creek just downstream from the outlet of Medicine Lake in the city of Plymouth (Feature No. 21) will require minor modifications. A concrete channel bottom slab and new lagging to prevent abutment earth slippage are proposed.

DIVERSION DURING CONSTRUCTION

37. Many of the project features along Bassett Creek, including each of the control structures and culverts, will require the creek to be diverted temporarily during construction. Other than the tunnel, none of the structures are massive or difficult to construct so that the duration of creek diversion will be short. Normal creek flows are small with most areas of the creek having discharges less than 100 cfs. Therefore, control and diversion of the creek during construction is not considered a significant problem. During the lengthy tunnel construction, the creek will continue to flow through the existing conduit and only minor earth work may be required to keep potential high water levels from flooding the tunnel inlet.

ACCESS ROADS

38. Existing public roads will be used for access to project construction sites. U.S., State, county, and city roadways provide access to all project areas. Most roadways are well maintained and of high quality, capable of accommodating transportation of construction equipment and materials. City streets are generally adequate for transportation to project areas and it is possible to route heavy or oversized loads around bridges or other structures with limited capacity. Access can be gained to each construction site by more than one route. No new roads other than short, temporary construction accesses off existing public roadways are necessary. Such temporary accesses will be restored to their original condition upon completion of construction.

CONSTRUCTION MATERIALS

General

39. Much of the berm and embankment material required for the project will be generated by excavation of project flood storage areas. The remainder of construction materials required can be acquired from area commercial sources.

Levee and Berm Fill

40. Fill to be used in the Edgewood embankment and the Golden Valley Country Club embankment is to be acquired from excavation of the Edgewood storage area. Fill to be used in the tunnel storage area berm will be acquired either from the tunnel excavation or excavation of the adjoining flood storage area. Fill to be used in the Wisconsin Avenue embankment, the Winnetka Avenue dike and the T.H. 100 embankment will be acquired from available sources in the Twin City metropolitan area.

Other Construction Materials

41. Suitable materials for riprap, bedding, concrete aggregates and concrete are produced from several tested sources in or near the metropolitan area. Further discussion of material sources is given in Appendix C.

ENVIRONMENTAL

42. After completion of the feasibility report in March 1976 and the final environmental impact statement (FEIS) in March 1977, a number of project components were modified, added, or deleted. Deletions include both channel (i.e., clearing and snagging, channel widening) and structural (i.e., road raises, floodwalls, culvert replacement) components that were eliminated as a result of improved hydrologic data and a need to lessen social disruption. The modifications to project components and component additions also resulted from hydrologic studies and a need to reduce social disruption and environmental impacts. Basically, the modifications and additions include: changing the size (from 8.5 feeO (ft) to 10.0 ft in diameter) and construction methods (from surface to subsurface excavation) of the outlet conduit, redesigning the tunnel inlet storage area to make it a temporary ponding area instead of a shallow wetland, relocatng the Minneapolis-Northfield and Southern Railroad bridge plus part of the creek channel, sheet-piling a portion of the creek bank and constructing an interior drainage site in the Fruen Mill and Glenwood-Inglewood area, relocating and realigning the T.H. 100 control structure, widening the outlet channel, and reducing and changing the nature of the outlet control structure (from a levee to a sheet-piled weir) downstream of Medicine Lake.

43. These changes in the project would not have a significant cumulative impact on the social, cultural, and natural resources within the project area. Although each modified and additional component would have some effect on the natural resources of the project area, the cumulative effects would be less than the impacts identified in the FEIS.

44. The effects of disposing of excavated material and placing fill material in the aquatic environment were also evaluated. Of particular importance was the placement of excavated materials from the tunnel inlet storage area. This area had been used for approximately 30 years as a construction debris disposal site,

and there was some concern about the quality of the material to be excavated because most of this material would be placed around the proposed storage area. as a result, two alternative disposal sites were identified.

45. Placement of fill material into the aquatic environment would result in both temporary and permanent losses of aquatic habitat. However, using clean fill, placing temporary levees around some construction sites, and conducting construction activities during low-flow periods should minimize the potential for adverse impacts.

46. These changes in the proposed project and the effects of placing fill material into the aquatic environment are discussed more fully in the environmental assessment and Section 404(b)(1) ib oages EA-1 through 18.

REAL ESTATE REQUIREMENTS

47. The local sponsor will be required to provide without cost to the United States all lands, easements, and rights-of-way, including borrow areas, disposal areas, and ponding areas, as determined by the Chief of Engineers, for construction, operation and maintenance of the project. All real estate interests will be acquired in compliance with the Uniform Relocation Assistance and Real Property Acquisition Policies Act of 1970, Public Law 91-646.

48. In accordance with Section 221 of the Flood Control Act of 1970, the local sponsor will furnish assurances satisfactory to the Secretary of the Army that it will:

a. Comply with the applicable provisions of the Uniform Relocation Assistance and Real Property Acquisition Policies Act of 1970, Public Law 91-646, approved 2 January 1971, in acquiring lands, easements and rights-of-way for construction and subsequent maintenance of the project and inform affected persons of pertinent benefits, policies and procedures in connection with said Act; and

b. Comply with Section 601 of Title IV of the Civil Rights Act of 1964 (P.L. 88-352) and Department of Defense Directive 5500.11 issued pursuant thereto with the construction and operation of the project.

49. Plan drawings showing required project lands are now being prepared for delivery to the City of Minneapolis. These drawings will present detailed information suitable for preparing easement and parcel descriptions of property to be acquired. Surveying information defining these properties will be based on existing nearby land ownership property corner points, section or sub-section corner points, State plain coordinate grid points or other suitable reference points.

RELOCATIONS

Telephone and Electrical Utilities

50. No permanent telephone or electrical relocations are anticipated. Temporary interruptions to service may be required to accommodate construction of individual project features but facilities are expected to be restored to their original condition. Costs of any such relocations are considered non-Federal expenses.

Natural Gas Utilities

51. Approximately 100' of 12" steel gas main will have to be lowered to accommodate the removal of the concrete box culvert at Penn Avenue. Cost of this work is considered a non-Federal expense.

Railroads

52. Two railroad modifications are anticipated in this project. The Chicago and Northwestern Railroad bridge in Plymouth just downstream from the outlet of Medicine Lake will require minor modifications to the abutments and creek bed area. A second railroad modification involves the removal and replacement of the Soo Line railroad bridge near Glenwood-Inglewood Avenues in Minneapolis. The bridge will be replaced with a shorter span structure along a new alignment and will not be as obstructive to flow as the existing structure. The realignment will require modifications to the tracks leading to the structure from both directions for distances dictated by minimum radius requirements. These railroad modifications are considered Federal cost items.

Sanitary Sewers

53. The existing Currie Avenue 86" brick and stone sanitary sewer in Minneapolis from just west of Freemont Avenue North to Dupont Avenue North will be abandoned and replaced by a 84" RCP-A pipe approximately 862 feet long as shown on Plate 12. This work involves taking up and relaying some trackage from the Soo Line (formerly Minneapolis, Northfield and Southern) Railroad. Any other alignment would be longer and increase construction conflicts with existing buildings, streets and utilities. The work involving the Railroad is generally considered a Federal cost item, and the remainder of the work is considered a non-Federal cost item.

54. At the T.H. 100 embankment, approximately 360 feet of 8" V.C.P. will be replaced with 8" P.V.C. pipe.

55. At the Edgewood embankment and storage area site, approximately 1365 feet of 10" V.C.P. sanitary sewer would be replaced with cast iron pipe along a new alignment.

56. Provision will be made to maintain service throughout construction and this work will be coordinated with the appropriate municipality and the Metropolitan Waste Control Commission. Modifications to sanitary sewers are considered local cost items.

Watermain

57. Approximately 85 feet of 24" ductile iron watermain will have to be lowered to accommodate removal of the concrete box culvert at Penn Avenue.

58. To complete the removal of Georgia Avenue, approximately 100' of 6" watermain will be insulated as a result of the storage area excavation. This will be completed under the Section 215 program.

59. With the removal of the Soo Line Railroad bridge near Glenwood and Inglewood Avenues in Minneapolis, a 6-inch water line attached to the bridge providing water to the Glenwood-Inglewood Company will be removed. It will be

relocated on the new bridge and provisions will be made for temporary water service during bridge construction.

60. Construction of the T.H. 100 embankment will require replacement of approximately 360 feet of 6" watermain with ductile iron pipe.

61. It is believed that water service to all properties near these construction sites can be maintained during construction, but that pressure may be reduced.

62. Modifications to water mains are considered local cost items.

Fences

63. Up to several hundred feet of security fencing will be removed and replaced, if necessary, at the Golden Valley Country Club near the proposed embankment. This work is necessary to allow construction access to the embankment site and is considered a Federal cost item incidental to the embankment construction.

Low Head Rock Dam

64. The low head rock dam near the Fruen Mill in Minneapolis has been recommended to be relocated as a non-Federal cost item by the Board of Engineers for Rivers and Harbors. In 1976, the Office of the Chief of Engineers concurred with this recommendation. Therefore, this item is considered a non-Federal cost item.

Medicine Lake Outlet

65. The Board of Engineers for Rivers and Harbors recommended this work item to be completed at local expense. The Office of the Chief of Engineers concurred and therefore, this item is considered a non-Federal cost item.

Road Raises

66. The Board of Engineers for Rivers and Harbors recommended that the raising and resurfacing of roads at 29th Avenue and Boone Avenue be completed at local expense, the necessity due to downstream controls and storage area real estate policies. The Office of the Chief of Engineers concurred. The Boone Avenue road raise is now no longer recommended as a project feature and, therefore, will not be considered a local cost item. The 29th Avenue road raise, however, is necessary as a part of the T.H. 100 embankment structure. It is recommended that the fill portion of the road raise required for the embankment and related construction be considered Federal cost items and that ramp material not necessary for the embankment, resurfacing materials and related construction be considered local cost items. All other road raises proposed in the Feasibility report are no longer proposed for construction.

COST ESTIMATE

Basis of Estimate

67. This general design memorandum estimate of costs is based on October 1982 price levels and, wherever possible, reflects recent prices for similar work in the St. Paul District. A summary comparison of project costs between the

current approved estimate (PB-3, 26 March 1982) and the revised estimate prepared for this design memorandum is shown in Table 1. Details of the revised cost estimate can be found in Appendix F.

Table 1 - Summary Comparison of Estimated First Costs

	Current Approved Est. (PB-3, 26 Mar 82)	Phase II GDM Estimate
Federal First Costs and Non-Federal Contributions		
Relocations	$ 629.0	$ 540.0
Channels	9,031.0	16,612.0
Floodwalls	704.0	0.0
Recreation Facilities	271.0	0.0
Flood Control Structures	790.0	1,105.0
Engineering and Design	1,791.0	2,720.0
Supervision and Administration	737.0	1,783.0
Inspection	(417.0)	(822.0)
Overhead	(320.0)	(961.0)
Total Cost (Federal First Costs & Non-Federal Contributions)	$13,953.0	$22,760.0
Non-Federal Contributions	253.0	460.0
Total Federal First Costs	$13,700.0	$22,300.0
Non-Federal First Costs		
Lands and Damages	$ 3,040.0	$ 3,740.0
Relocations	1,847.0	2,040.0
Non-Federal Contributions	253.0	460.0
Total Non-Federal First Costs	5,140.0	6,240.0
Total Project First Costs	$18,840.0	$28,540.0

The difference in Federal first cost (an increase of $8,600,000) between this design memorandum cost estimate ($22,300,000) and the current approved estimate from PB-3 effective October 1982 ($13,700,000) is attributable to the following:

a.	Relocations	$ -89,000
	(1) Increase in residences to be floodproofed from 19 to 24	(+129,000)
	(2) Decrease to elimination of three home relocations	(-360,000)
	(3) Increase due to addition of railroad bridge relocation	(+142,000)
b.	Channels - Conduit	+6,855,000
	(1) Increase due to Federal share of costs for Mn/DOT 2nd St. Tunnel	(+919,000)
	(2) Increase due to Federal share of costs for 3rd Ave. Tunnel	(+1,820,000)
	(3) Increase due to redesign of tunnel, increase in length of tunnel, change from open-cut to tunnel construction, and increase in unit tunnel costs based on current construction in area	(+4,116,000)
c.	Channels - Channel Modifications	+1,279,000
	(1) Increase due to transfer of work from Floodwall category and redesign at Fruen Mill area	(+264,000)
	(2) Increase due to bridge removal and associated channel work at Penn Avenue	(+51,000)
	(3) Increase due to change from open channel cut to closed conduit RCP at Markwood Area	(+967,000)
	(4) Decrease to estimated cost for bridge removal at Minnaqua Avenue	(-3,000)
d.	Channels - Storage Areas	-544,000
	(1) Decrease due to refined design and current unit costs at tunnel inlet ponding area	(-556,000)
	(2) Decrease in estimated cost of beautification	(-23,000)
	(3) Increase due to addition of storage area at Edgewood	(+154,000)
	(4) Decrease due to elimination of road raises	(-119,000)
e.	Flood Control Structures	+315,000
	(1) Increase due to redesign of tunnel inlet drop structure	(+78,000)
	(2) Decrease due to redesign of Highway 55 control structure	(-41,000)
	(3) Increase due to redesign of Highway 100 flood control structure	(+126,000)
	(4) Increase due to redesign of Golden Valley Country Club flood control structure.	(+123,000)
	(5) Increase due to redesign of Edgewood	(+56,000)
	(6) Increase due to redesign of Wisconsin Avenue	(+29,000)
	(7) Decrease due to redesign of Winnetka Avenue berm to project	(-56,000)

f. Floodwalls

Decrease due to reclassification and inclusion with channel modifications	-704,000

g. Recreation

Decrease due to elimination of recreation facilities from project	-271,000

h. Engineering and Design

Increase due to direct proportion of estimated construction costs	+929,000

i. Supervision and Administration

Increase due to direct proportion of estimated construction costs	+1,046,000

j. Non-Federal Contributions

Increase due to floodproofing and tunnel cost share	+207,000

The difference in non-Federal first cost ($1,100,000) between this design memorandum cost estimate ($6,240,000) and the current approved estimate from PB-3 effective October 1982 ($5,140,000) is attributable to the following:

a. Lands and Damages

Increase due to project damages and current price levels	+700,000

b. Relocations

Increase due to more accurate estimates and bids received for work	+193,000

c. Non-Federal Contributions

(1) Increase in residences to be floodproofed	(+27,000)
(2) Increase in Mn/DOT participation in 3rd Ave. Tunnel	(+180,000)

OPERATION AND MAINTENANCE

68. Local Interests will maintain the project in accordance with procedures and schedules set forth in a maintenance manual to be furnished by the Corps of Engineers. Maintenance will consist of periodic inspection and repair as required on the tunnel, control structures including the Medicine Lake control structure, and the crossings. Maintenance schedules and instructions will be furnished to the appropriate local officals for completed features of the project as soon as they become functional. This will ensure proper operation of the partially completed project during the extended period required for construction of the total project.

SCHEDULE FOR DESIGN AND CONSTRUCTION

69. The schedule for design and construction is presented in Table 2 and is subject to the availability of funds. In scheduling construction, two reaches are used. Construction in the upper watershed is being scheduled ahead of construction in the lower watershed, largely due to the need for a feature design memorandum for the tunnel in the lower watershed. Previously, the point defined as separating the upper reach from the lower reach was the point where Golden Valley Road crosses the main stem Bassett Creek in the city of Golden Valley. It is now recommended that the lower watershed be defined as that portion of the watershed lying within the boundaries of the city of Minneapolis. Only one feature, the T.H. 55 control structure (Feature No. 10), is affected by this change in reach definition. This feature will now go to construction earlier with the upper reach features.

TABLE 2 - Schedule for Design and Construction

Reach	Submit Feature Design Memo	Submit P&S	Advertise	Award	Construction Completion
Upper	N/A	Aug 83	Feb 84	Mar 84	Oct 85
Lower	N/A	Apr 84	Jun 84	Jul 84	Dec 86
Lower	Oct 83	Apr 84	Jun 84	Jul 84	Dec 86

STATEMENT OF FINDINGS

INTRODUCTION

I have reviewed and evaluated, in light of the overall public interest, the documents concerning the proposed plan and the stated views of other interested agencies and the concerned public in formulating and developing a plan for flood damage reduction in the Bassett Creek Watershed, Hennepin County, Minnesota.

BACKGROUND

The Bassett Creek Watershed flood control project was authorized in the 1976 Water Resources Development Act of 1976 (P.L. 94-587). The project will provide protection against a 100-year frequency flood occuring in any given year for residential development in the upper watershed and commercial and industrial development in the lower watershed. These areas together with several major highways and railroads are subject to frequent flooding. A large commercial and industrial area in Minneapolis considered especially susceptible to flood damage is located immediately upstream of the 1.5 mile tunnel which serves as an outlet for the entire Bassett Creek watershed. A 50 percent blockage of this tunnel would cause flooding on more than 250 acres including 450 residential dwellings and 80 industries. Flood losses could exceed $12.5 million in the proximity of the existing tunnel entrance. Severe flood damages also occur in the communities of Golden Valley, New Hope, and Crystal.

Recent severe flooding in the Bassett Creek watershed occurred in 1974, 1975, and 1978. Summer rainstorms and snowmelt during the spring often increased by rainfall are the major causes of flood damage in the watershed. A related problem caused by the rain and flooding is sanitary sewer backup in many homes. A more severe flood occurred in June 1942. This flood was significant because of the damage which was incurred by the commercial and industrial establishments in Minneapolis. If a flood of this magnitude were to occur at this time, damages of approximately $2.9 million would occur in Minneapolis and damages of approximately $7.7 million would be sustained through the remainder of the watershed.

Flood damages presently average about annually without flood protection. The project will provide flood protection for some residences and industries located in the floodplain, and an indeterminate amount of commercial and industrial development in Minneapolis.

ALTERNATIVES

In addition to the no action alternative, all of the other available means of reducing flood damages were investigated as alternatives to the proposed action.

The no action alternative was not considered responsive to the needs of the people.

Nonstructural alternatives such as flood insurance, improved flood warning systems and reliance on emergency protective measures, flood plain regulations and flood proofing techniques by themselves or in combination would partially reduce the present and potential flood problems of the watershed, but would not feasibly reduce potential damages to existing developments in the flood plain.

A permanent evacuation plan eliminating all damages is economically infeasible, especially in the lower watershed where the primary problem is the deteriorated condition of the existing conduit and the flooding that could result with its failure. In addition, evaucation is unacceptable to the people who would be directly affected.

Structural alternatives involving large-scale channel modifications and leve2s alone or in combination in the upper watershed would be economically infeasible or would be environmentally and socially undesirable.

Alternatives involving repair of the existing conduit, a new conduit in non-highway right-of-way, and others involving a new conduit along various other highway right-of-way alignments were investigated and found to be feasible but more costly than the selected conduit alignment.

An alternative involving an open channel with open space corridor in lieu of a new conduit was viewed by some local interests as a more environmentally desirable plan since it would restore the creek to its natural state. The Feasibility Report recognized this and identified the open channel-open space alternative as the best plan from the standpoint of environmental quality (EQ). However, the report concluded that the open channel-open space alternative is unacceptable since the costs of development would far outweigh the benefits obtainable and to await the results of more detailed studies of the open channel-open space alternative would result in losing the opportunity to develop a cooperative conduit plan with the Minnesota Department of Highways. Development of the new conduit proposal does not preclude future development of the open channel-open space plan by local interests.

THE SELECTED PLAN

The selected plan for the Bassett Creek watershed involves a combination of both nonstructural and structural measures. Nonstructural measures include flood proofing of 24 residences and evacuation of 1 residence in the upper watershed and continuance of local flood plain ordinances modified to reflect proposed conditions.

The plan includes two principal structural measures; one, a new conduit to replace the existing conduit outletting Bassett Creek to the Mississippi River and the other, a series of small control structures in the upper watershed to temporarily impound flood waters. The lower portion of the new conduit has been constructed by the Minnesota Department of Transportation under its Interstate Highway program in Minneapolis. This new conduit has been sized to accomodate future Bassett Creek flows. Cooperative construction of the conduit system will result il estimated cost savings to about $5.7 million and would prevent severe damages that could occur to industrial developments under present conditions.

Other structural measures included in the selected plan in the upper watershed include a limited reach of channel widening, bridge removals, culvert replacements and construction of a new weir at the outlet of Medicine Lake.

The temporary flood water storage impoundments and other structural measures acting in conjunction would reduce flood damages to residential developments in the upper Bassett Creek watershed. The new weir structure at the outlet of Medicine Lake would reduce flood water levels of the more infrequent floods and thereby reduce damages to residential properties abutting the lake.

None of the features of the selected plan would alter normal streamflow or lake level conditions.

Another feature of the selected plan includes the development of a flood storage area at the entrance to the proposed conduit, which will reduce flood damage, provide a potential recreational area, and contribute to the potential development of a recreation trail system for bicycling and walking. Such a trail system would provide a link between existing parks and trails at the lower end of the watershed.

EVALUATION

Environmental Considerations - The selected plan was developed in response to concerns expressed by local interests who wished to preserve the aesthetic quality of Bassett Creek. Accordingly, such environmentally degrading features as channel modification and snagging and clearing were eliminated in the more environmentally sensitive areas of the creek in favor of temporary flood water storage which, because of short duration, would not cause a significant impact on the environment.

Coordination with the U.S. Fish and Wildlife Service and the Minnesota Department of Natural Resources resulted in development of a plan that would minimize adverse impacts on northern pike spawning habitat and yet adequately reduce flood damages to residences on Medicine Lake.

The selected plan includes excavation of two of the temporary flood water storage areas. Adverse environmental impacts could be expected during construction of these and other features of the selected plan. Adverse impacts on water quality and aquatic organisms would be temporary. Some long-term adverse effects would occur from embankment construction due to the removal of vegetation and the associated effects on aesthetic qualities and wildlife habitat. The new outlet structure at Medicine Lake would be constructed in a wetland about 500 feet downstream of the lake.

Some long-term beneficial environmental impacts would result with implementation of the selected plan. The new conduit will be constructed with a drop which will effectively prevent the migration of rough fish from the Mississippi River into Bassett Creek. In the upper watershed the storage impoundments would retard high velocity flood flows thereby possibly reducing downstream degradation and aggradation problems.

Social Considerations - The selected plan would provide the flood protection considered necessary for Minneapolis and the other suburban communities in the Bassett Creek watershed. Direct benefits would accrue from the protection of some residences and industries located in the flood plain and an indeterminable amount of commercial and industrial development in Minneapolis that could be severely damaged if the existing conduit failed or became blocked during a flood. The selected plan would reduce the threat of life and the anxieties commonly associated with flooding.

The selected plan would place a social burden upon one family which will be evacuated from the flood plain and possibly upon those property owners whose residences would be raised or floodproofed. Although flood proofing measures are considered optional by the Federal government, the city of Golden Valley has determined that each residential structure will be protected against the design flood. To date, the homeowners have participated in the program.

Cultural Considerations

Cultural resources studies completed for this Phase II study by the St. Paul District, Corps of Engineers have failed to identify any archaeological or historical sites in the project area. However, a potential for previously unknown culturally valuable sites is felt to exist and construction activities will be required to remain cognizant of this potential.

Economic Considerations - Annual benefits and costs are $2,802,100 and $2,329,200, respectively for the project. The benefit-cost ratio is 1.2. In additiol to an estimate for the prevention of present and future flood damages, the benefit estimate includes an estimate for early replacement of existing facilities, an estimate of the economic gains resulting from increased recreation opportunities and employment, and an estimate of early real estate acquisition activities by sponsoring communities to prevent development in floodplain areas prior to recent adoption of floodplain management regulations.

The opportunity to develop the plan cooperatively with the Minnesota Department of Transportation also results in a significant economic gain. With out this opportunity the plan would most likely not be economically feasible.

The selected plan would cause no significant adverse impact on the existing tax base of school districts. On the contrary, property values and hence the tax base could increase with implementation of the plan.

CONCLUSION

I find that the action proposed herein is based on a thorough analysis and evaluation of all reasonable alternative means for achieving the stated objectives; that wherever unavoidable adverse effects are found to be involved, they cannot be avoided by reasonable alternative courses of action which would achieve the congressionally specified project purpose; that the recommended action is consonant with national policy, statutes, and administrative directives; that where the proposed action results in an adverse effect, this effect is either mitigated or outweighed by other considerations. In addition, the city of Minneapolis, the Minnesota Department of Transportation, the Bassett Creek Flood Control Commission and the communities it represents find the plan acceptable in concept. Accordingly, it is my decision that the public interest would best be served by implementation of the plan.

RECOMMENDATION

I recommend that the United States construct the flood storage and conveyance facilities, flood proofing modifications and related features in the Bassett Creek watershed at Hennepin County, Minnesota, for flood control, in general accordance with the plan selected herein, with such modifications thereof as in the discretion of the Chief of Engineers may be advisable at an estimated Federal first cost of $22,300,000, non-Federal first cost of $6,240,000

and non-Federal annual operation, maintenance, and replacement costs of $17,500 provided that prior to construction, and in accordance with Section 221 of the Flood Control Act of 1970, local interests furnish assurances satisfactory to the Secretary of the Army that they will:

a. Provide without cost to the United States all lands, easements, and rights-of-way, including borrow areas and disposal areas for excavated material determined suitable by the Chief of Engineers and necessary for construction of the project;

b. Hold and save the United States free from damages that may result from construction of the project, not including damages due to the fault or negligence of the United States or its contractors;

c. Maintain and operate all the works after completion in accordance with regulations prescribed by the Secretary of the Army;

d. Share in the cost of the nonstructural portion of the project in accordance with cost-sharing provisions being developed under Section 80 of Public Law 93-251;

e. Accomplish without cost to the United States all relocations and alterations of buildings, transportation facilities, storm and sanitary sewer systems, public and private utilities, local betterments, drainage facilities, and other structures and improvements made necessary by construction of the recommended plan, excluding railroad bridges and approaches and facilities necessary for the normal interception and disposal of local interior drainage at the line of protection;

f. Prescribe and enforce regulations to prevent obstruction or encroachment on channels and temporary storage areas which would reduce their flood-carrying or storage capacity or hinder maintenance and operation;

g. Provide a cash contribution for recreation equal to 50 percent of the final separable cost allocated to this function less a credit for the value of lands, easements, rights-of-way, alterations, and relocations allocated therefor;

h. Publicize flood plain information in the areas concerned and provide this information to zoning and other regulatory agencies for their guidance and leadership in preventing unwise future development in the floodplain and in adopting such regulations as may be necessary to insure compatibility between future development and protection levels provided by the project; and

i. At least annually inform affected interests regarding the limitations of the protection afforded by the project.

j. Comply with the applicable provisions of the Uniform Relocation Assistance and Real Property Acquisition Policies Act of 1970, Public Law 91-646, approved 2 January 1971, in acquiring lands, easements and rights-of-way for construction and subsequent maintenance of the project and inform affected persons of pertinent benefits, policies and procedures in connection with said Act; and

k. Comply with Section 601 of Title VI of the Civil Rights Act of 1964 (P.L. 88-352) and Department of Defense Directive 5500.11 issued pursuant thereto and published in part 300 of Title 32, Code of Federal Regulations, in connection with the construction and operation of the project.

Further, I recommend that non-Federal interests (the Minnesota Department of Transportation or the local sponsor) be reimbursed for the full amount of any costs they have incurred toward payment of the allocated Federal share of the cooperative drainage facilities currently estimated at approximately $3,000,000 for advance work on the cooperative tunnel and $450,000 for advance engineering and construction work on upper watershed project features under the Section 215 agreement.

EDWARD G. RAPP
Colonel, Corps of Engineers
District Engineer

BASSETT C

FLOOD CONTROL

FEATURE NUMBER	SHEET NUMBER	DRAWING NUMBER	DESCRIPTION
	1	M34.3-R-5/180	TITLE AND INDEX
	2	M34.3-R-5/181	PROJECT LOCATION AND VICINITY MAP
1	3	M34.3-R-5/182	MNDOT TUNNEL PLAN AND PROFILE STA 15+00 TO 50+00
1	4	M34.3-R-5/183	MNDOT TUNNEL PLAN AND PROFILE STA 50+00 TO 85+00
2	5	M34.3-R-5/184	TUNNEL STA. 40+00 TO 66+50 PLAN & PROFILE
3	6	M34.3-R-5/185	TUNNEL STA 20+00 TO 40+00 PLAN & PROFILE
4	7	M34.3-R-5/186	TUNNEL STA. 0-25 TO 20+00 PLAN & PROFILE
	8	M34.3-R-5/187	TUNNEL SECTIONS AND DETAILS
5A	9	M34.3-R-5/188	INLET STRUCTURE
5	10	M34.3-R-5/189	TUNNEL INLET PONDING AREA
5	11	M34.3-R-5/190	TUNNEL INLET PONDING AREA BORING LOGS AND DROP STRUCTURE
5	12	M34.3-R-5/192	INLET PONDING AREA SEWER BETWEEN CURRIE AND DUPONT
5	13	M34.3-R-5/193	DELETED
8	14	M34.3-R-5/194	PENN AVE CULVERT REMOVAL
9 & 9A	15	M34.3-R-5/195	FRUEN MILL CHANNEL IMPROVEMENTS PLAN
9	16	M34.3-R-5/196	FRUEN MILL PONDING AREA
9	17	M34.3-R-5/197	FRUEN MILL AREA RETAINING WALL DETAILS
9	18	M34.3-R-5/198	FRUEN MILL AREA RETAINING WALL AND SECTIONS
9B	19	M34.3-R-5/199	FRUEN MILL AREA RAILROAD BRIDGE AND DROP STRUCTURE DETAILS
10	20	M34.3-R-5/201	HIGHWAY 55 CONTROL STRUCTURE PLAN & SECTIONS
13 & 14	21*	M34.3-R-5/202	CULVERT REPLACEMENT AT NOBLE AND REGENT AVE
15A & 28	22*	M34.3-R-5/203	BRIDGE REMOVAL @ MINNAQUA AVE AND CULVERT REPLACEMENT @ 34TH AVE
16	23	M34.3-R-5/204	HIGHWAY 100 EMBANKMENT PLAN AND PROFILE
6	24	M34.3-R-5/205	HIGHWAY 100 EMBANKMENT PROFILE AND SECTIONS
6	25	M34.3-R-5/206	HIGHWAY 100 EMBANKMENT TYPICAL SECTIONS
6	26	M34.3-R-5/207	HIGHWAY 100 EMBANKMENT EXCAVATION, SECTION AND CONDUIT PROFILE

FEATURE NUMBER	SHEET NUMBER
6	27
18	28
19	29
19	30
20	31
20	32
20A	33
21	34
23 & 24	35
29	36
29	37
31	38
31	39
31A & 32	40
31A	41
33	42
34	43
35	44

NOTES

1. FEATURE NUMBERS REFER TO FEATURES INDICATED ON SHEET 2
2. FEATURE NUMBERS MARKED * INDICATE DESIGNS WERE DEVELOPED FOR THE CITY OF GOLDEN VALLEY, MINNESOTA BY BARR ENGINEERING CO IN ACCORDANCE WITH AN AGREEMENT BETWEEN THE CITY OF GOLDEN VALLEY AND THE CORPS OF ENGINEERS UNDER SECTION 215 OF P L 90-483

LEGEND

NUMBER OF SHEET ON WHICH CONDITION IS SHOWN - - -



DETAIL OR SECTION ID

NUMBER OF SHEET ON SECTION OF THE COND

ETT CREEK

ONTROL PROJECT

FEATURE NUMBER	SHEET NUMBER	DRAWING NUMBER	DESCRIPTION
16	27	M34.3-R-5/208	HIGHWAY 100 EMBANKMENT CONTROL STRUCTURE PLAN AND SECTIONS
18	28	M34.3-R-5/210	CULVERT REPLACEMENT @ WESTBROOK ROAD
19	29	M34.3-R-5/211	GOLDEN VALLEY COUNTRY CLUB EMBANKMENT PLAN PROFILE AND SECTIONS
19	30	M34.3-R-5/212	GOLDEN VALLEY COUNTRY CLUB EMBANKMENT BORING LOGS AND SECTIONS
20	31*	M34.3-R-5/213	WISCONSIN AVE EMBANKMENT PLAN AND PROFILE
20	32*	M34.3-R-5/214	WISCONSIN AVE EMBANKMENT SECTION AND DETAILS
20A	33	M34.3-R-5/216	WINNETKA AVE PLAN AND PROFILE
21	34	M34.3-R-5/217	MEDICINE LAKE OUTLET STRUCTURES PLAN AND DETAILS
23 & 24	35*	M34.3-R-5/218	CULVERT REPLACEMENTS @ BURNSWICK AND 32ND AVE
29	36	M34.3-R-5/219	CULVERT REPLACEMENTS @ DOUGLAS DRIVE
29	37	M34.3-R-5/220	CULVERT REPLACEMENTS @ DOUGLAS DRIVE DETAILS
31	38	M34.3-R-5/221	EDGEWOOD EMBANKMENT GENERAL PLAN AND PROFILE
31	39	M34.3-R-5/222	EDGEWOOD EMBANKMENT EMBANKMENT DETAILS
31A & 32	40	M34.3-R-5/223	EDGEWOOD STORAGE AREA EXCAVATION
31A	41	M34.3-R-5/224	EDGEWOOD STORAGE AREA UTILITIES
33	42*	M34.3-R-5/226	CULVERT REPLACEMENT @ GEORGIA AVE
34	43*	M34.3-R-5/227	CULVERT REPLACEMENT @ 36TH AVE
35	44	M34.3-R-5/228	MARKWOOD AREA CHANNEL IMPROVEMENTS

LEGEND

NUMBER OF SHEET ON WHICH CONDITION SHOWN

9 / 9|43

9 — DETAIL OR SECTION IDENTIFICATION NUMBER

43 — NUMBER OF SHEET ON WHICH DETAIL OR SECTION OF THE CONDITION IS DRAWN

SAFETY PAYS

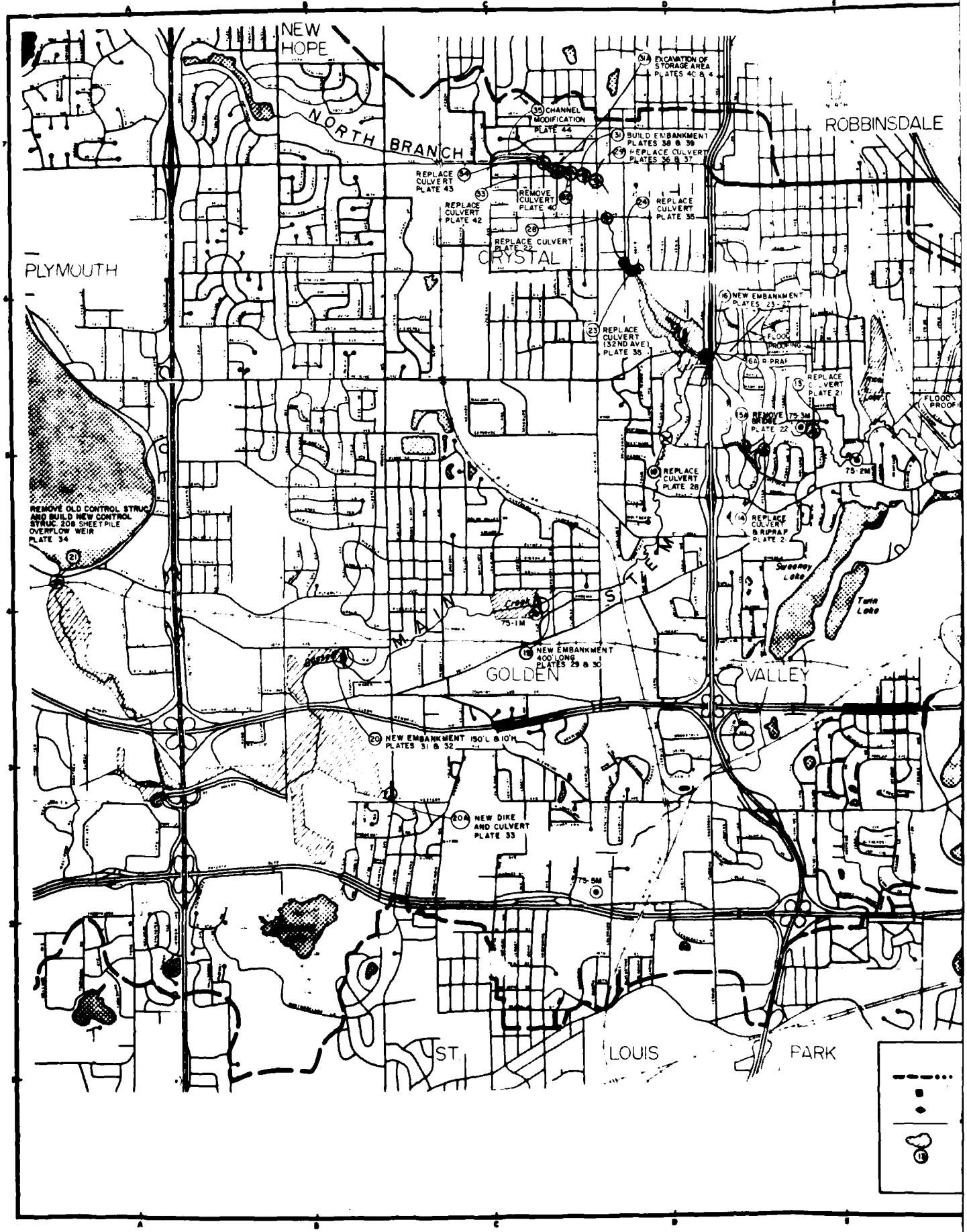
DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY: L A R
DRAWN BY: P W
CHECKED BY: L A R
SUBMITTED BY:
APPROVED
APPROVED:

PHASE II
FLOOD CONTROL
DESIGN MEMORANDUM
BASSETT CREEK MINNESOTA
TITLE AND INDEX
DATE AUGUST 1982

DRAWING NUMBER
M34.3-R-5/180
SHEET 1 OF 44

PLATE 1


NEW HOPE
NORTH BRANCH
ROBBINSDALE
PLYMOUTH
CRYSTAL
GOLDEN VALLEY
ST LOUIS PARK
EXCAVATION OF STORAGE AREA PLATES 4C & 4
CHANNEL MODIFICATION PLATE 44
BUILD EMBANKMENT PLATES 38 & 39
REPLACE CULVERT PLATES 36 & 37
REPLACE CULVERT PLATE 43
REMOVE CULVERT PLATE 40
REPLACE CULVERT PLATE 42
REPLACE CULVERT PLATE 22
REPLACE CULVERT PLATE 35
NEW EMBANKMENT PLATES 23-27
FLOOD PROOFING
REPLACE CULVERT (32ND AVE.) PLATE 35
R-PRAP
REPLACE CULVERT PLATE 21
REMOVE BRIDGE PLATE 22
75-3M
75-2M
REPLACE CULVERT PLATE 28
REMOVE OLD CONTROL STRUC AND BUILD NEW CONTROL STRUC 208 SHEET PILE OVERFLOW WEIR PLATE 34
REPLACE CULVERT & RIPRAP PLATE 2
Sweeney Lake
Twin Lake
Creek
75-1M
NEW EMBANKMENT 400' LONG PLATES 29 & 30
NEW EMBANKMENT 150'L & 10'H PLATES 31 & 32
NEW DIKE AND CULVERT PLATE 33
75-5M

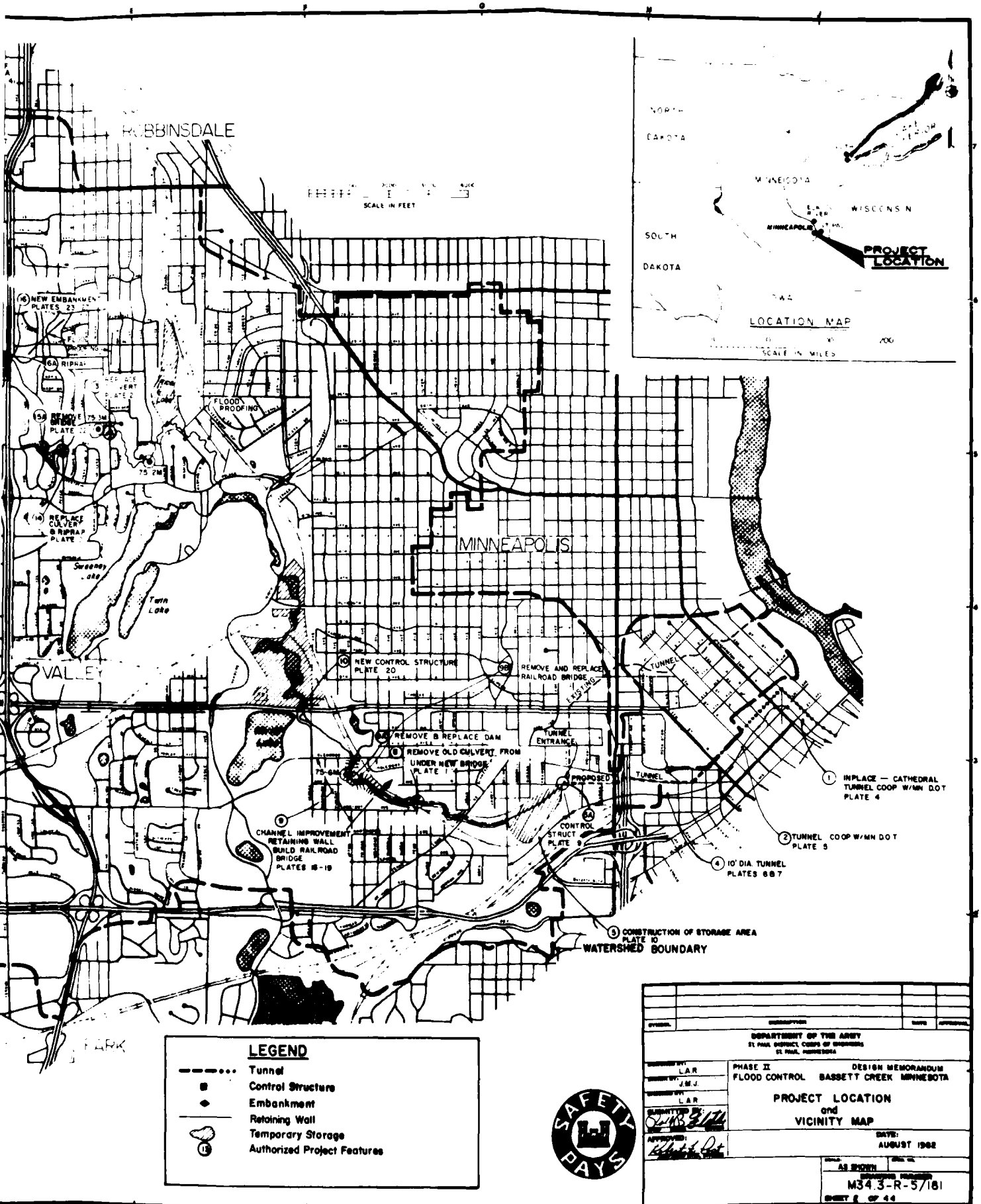
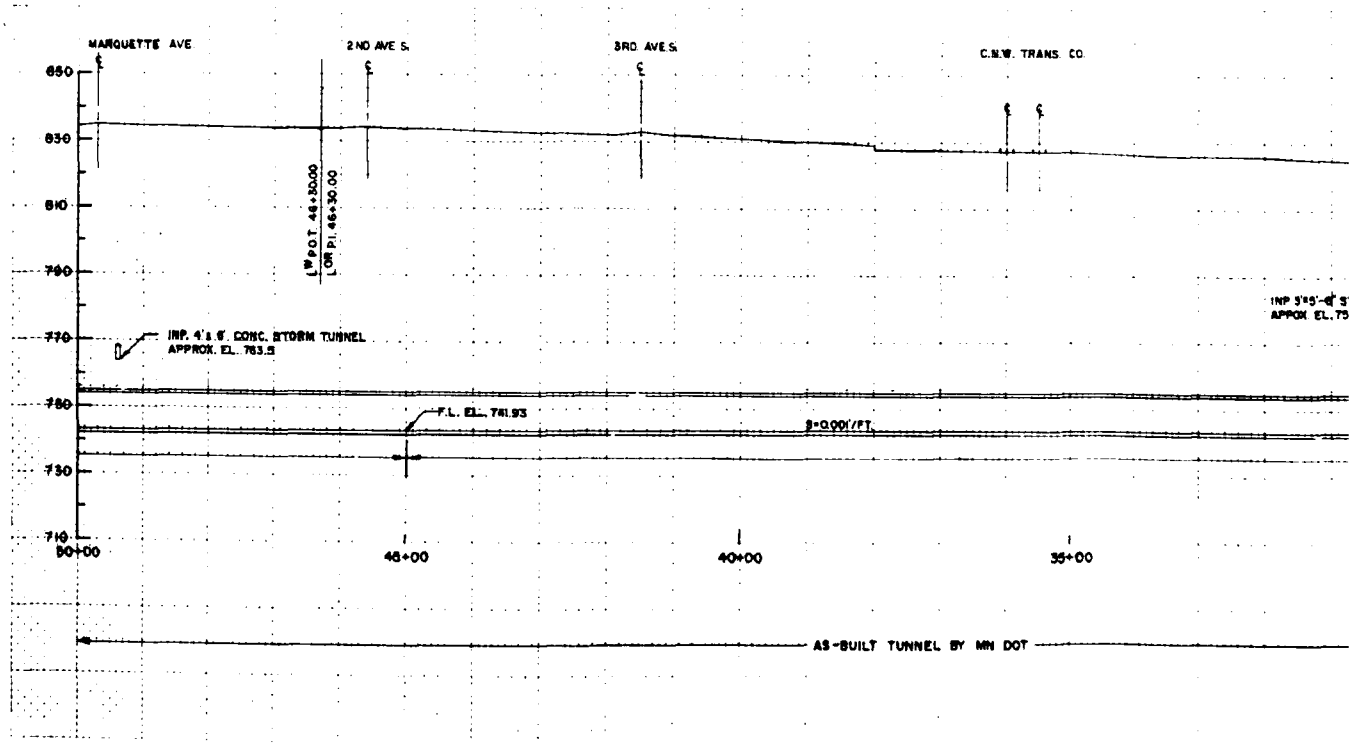
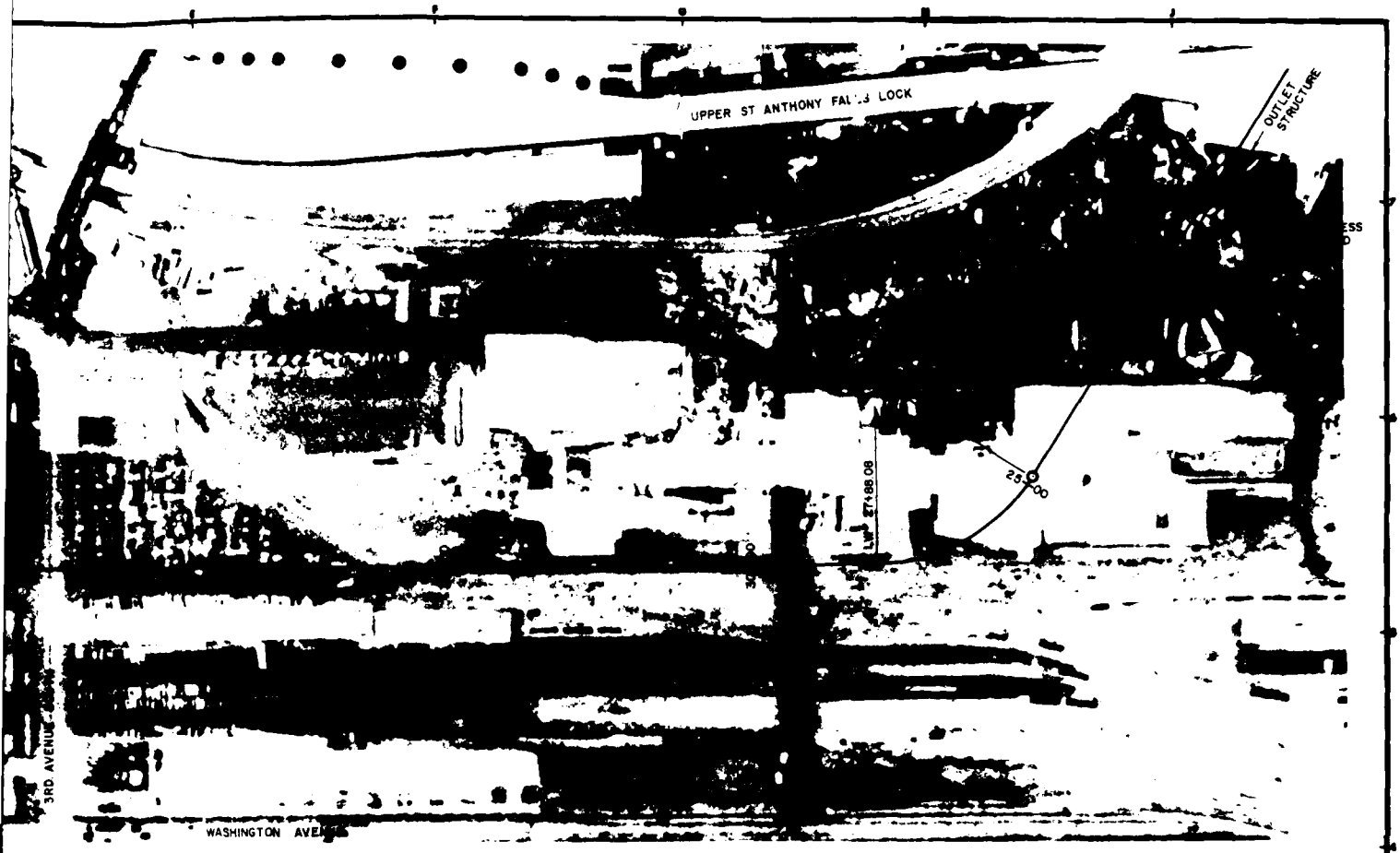

ROBBINSDALE
MINNEAPOLIS
VALLEY
PARK
SCALE IN FEET
NORTH DAKOTA
SOUTH DAKOTA
MINNESOTA
WISCONSIN
MINNEAPOLIS
PROJECT LOCATION
LOCATION MAP
SCALE IN MILES
(16) NEW EMBANKMENT PLATES 23
(6A) RIPRAP
(15A) REMOVE BRIDGE PLATE 17
75-2M
(14) REPLACE CULVERT & RIPRAP PLATE
Sweeney Lake
Twin Lake
FLOOD PROOFING
(10) NEW CONTROL STRUCTURE PLATE 20
(9B) REMOVE AND REPLACE RAILROAD BRIDGE
EXISTING TUNNEL
(8) REMOVE & REPLACE DAM
(8) REMOVE OLD CULVERT FROM UNDER NEW BRIDGE PLATE 1
75-6M
TUNNEL ENTRANCE
PROPOSED TUNNEL
(9) CHANNEL IMPROVEMENT RETAINING WALL BUILD RAILROAD BRIDGE PLATES 16-19
(9A) CONTROL STRUCT PLATE 9
(1) INPLACE — CATHEDRAL TUNNEL COOP W/MN DOT PLATE 4
(2) TUNNEL COOP W/MN DOT PLATE 5
(4) 10' DIA. TUNNEL PLATES 6&7
(5) CONSTRUCTION OF STORAGE AREA PLATE 10
WATERSHED BOUNDARY
LEGEND
Tunnel
Control Structure
Embankment
Retaining Wall
Temporary Storage
Authorized Project Features
SAFETY PAYS
DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA
DESIGNED BY LAR
DRAWN BY J.M.J.
TRACED BY LAR
SUBMITTED BY
APPROVED:
PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
PROJECT LOCATION and VICINITY MAP
DATE: AUGUST 1982
SCALE AS SHOWN
DRAWING NUMBER M34.3-R-5/181
SHEET 2 OF 44

PLATE 2



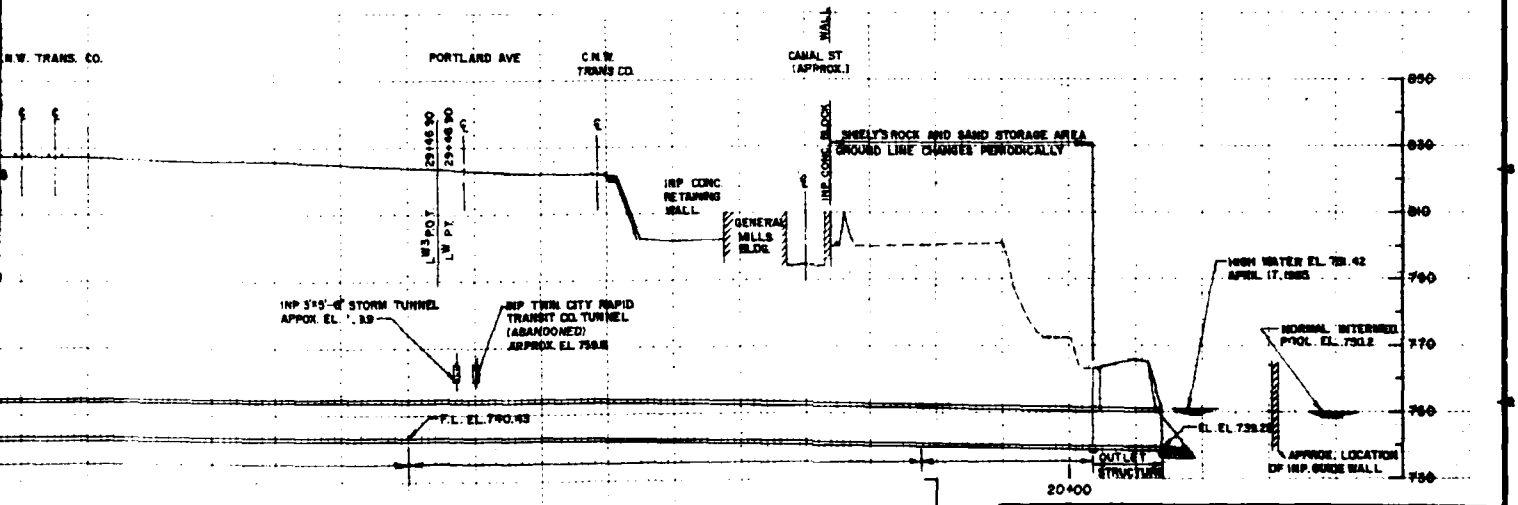
⊙ ALIGNMENT CHANGE





PHOTOGRAPH NOT TO SCALE

NMENT CHANGE



N DOT

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

MN. DOT
J.M.J.
L.A.R.

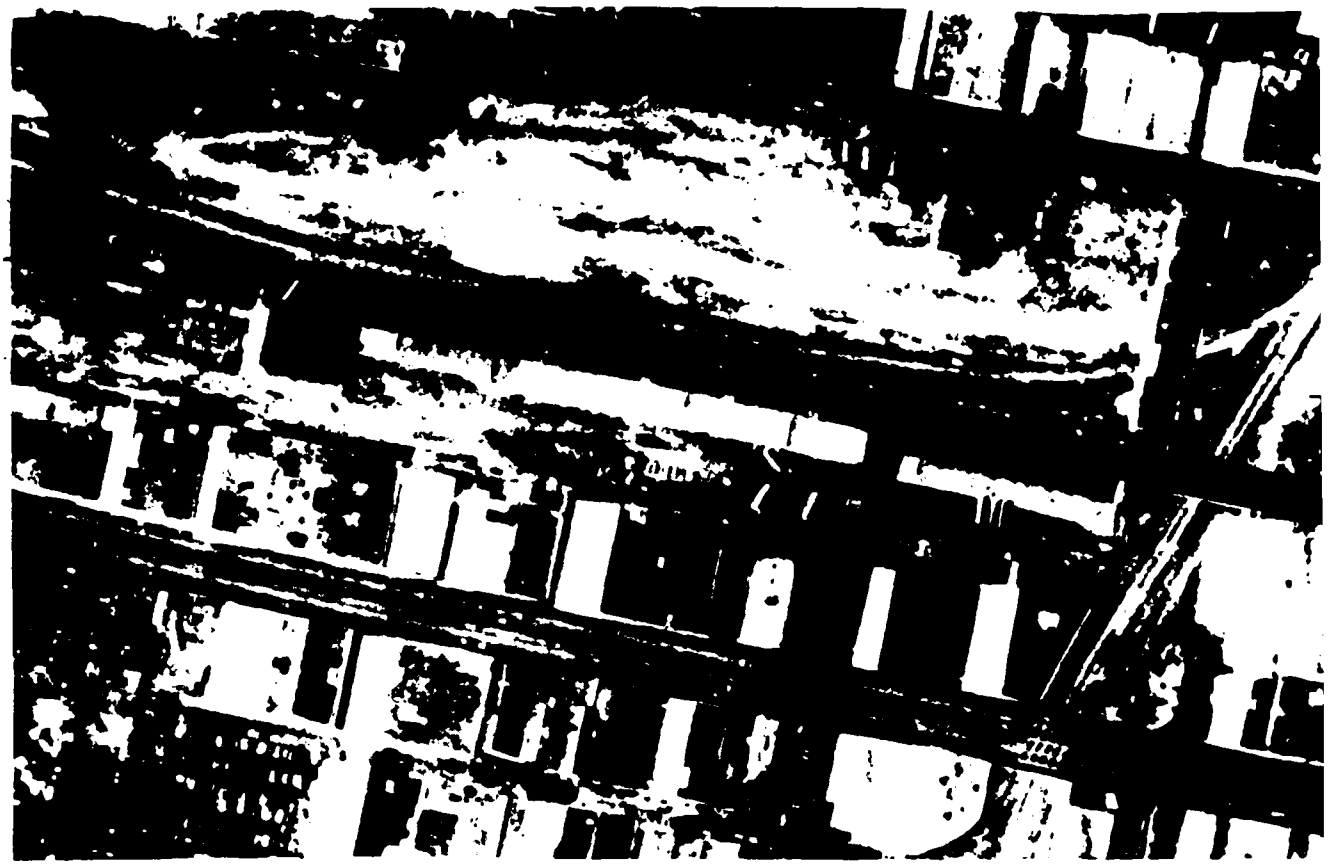
PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
MN. D.O.T. TUNNEL PLAN AND PROFILE
STA. 15+00 TO 50+00

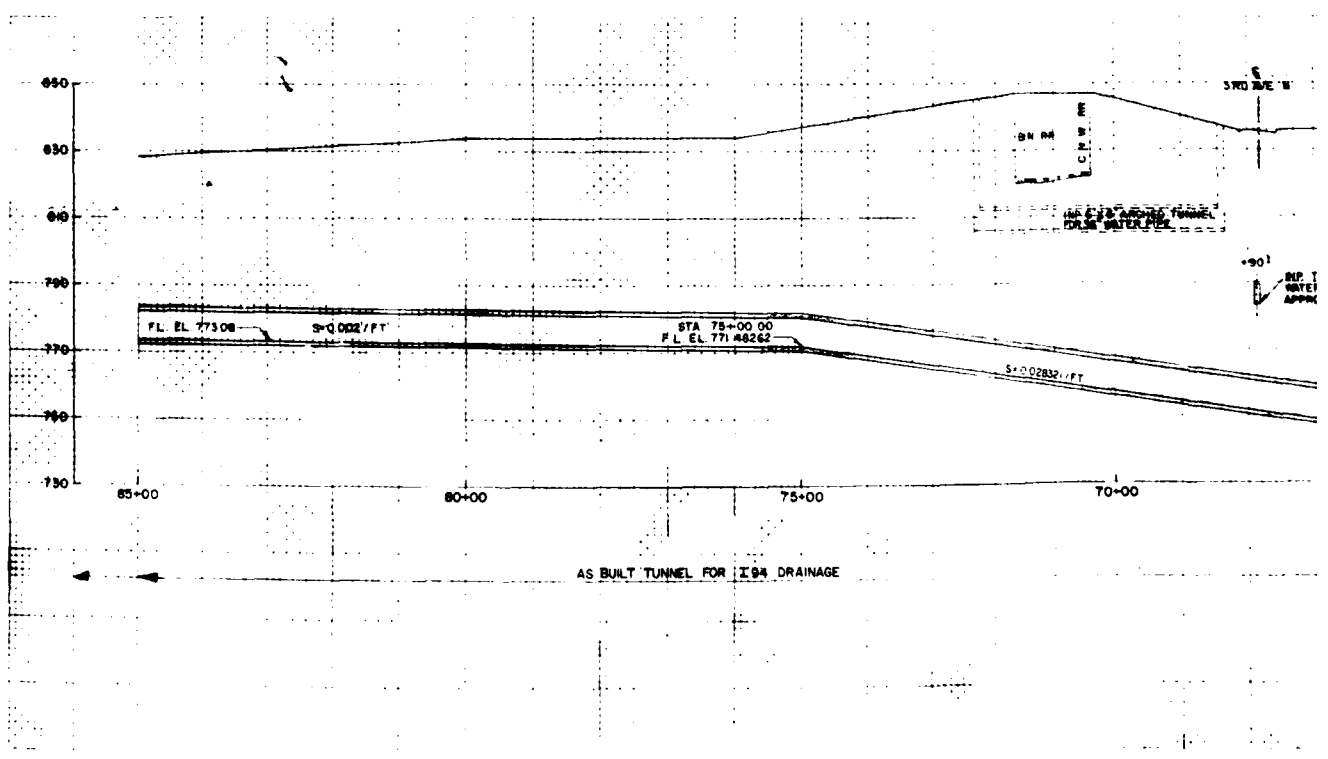
DATE: AUGUST 1962

SCALE: AS SHOWN

DRAWING NUMBER
M34.3-R-5/182
SHEET 3 OF 44

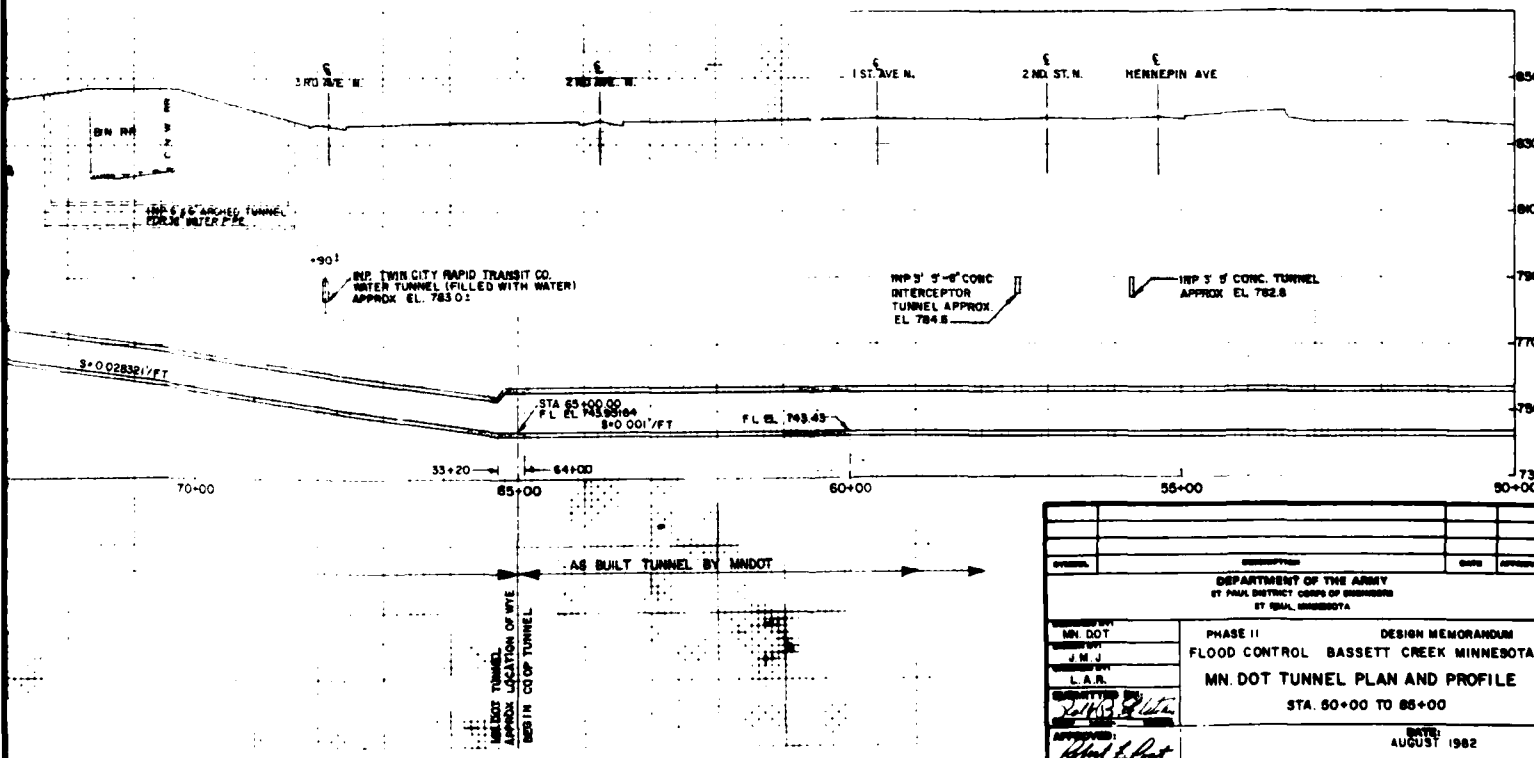
PLATE 3




FL EL 773.08
S=0.002'/FT
STA 75+00.00
F L EL 771.48262
S=0.02832'/FT
85+00
80+00
75+00
70+00
AS BUILT TUNNEL FOR I94 DRAINAGE



PHOTOGRAPH NOT TO SCALE



DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

MN. DOT
J.M.J.
L.A.R.

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
MN. DOT TUNNEL PLAN AND PROFILE
STA. 50+00 TO 65+00

DATE: AUGUST 1982

AS SHOWN

M34 3-R-5/183

SHEET 4 OF 44

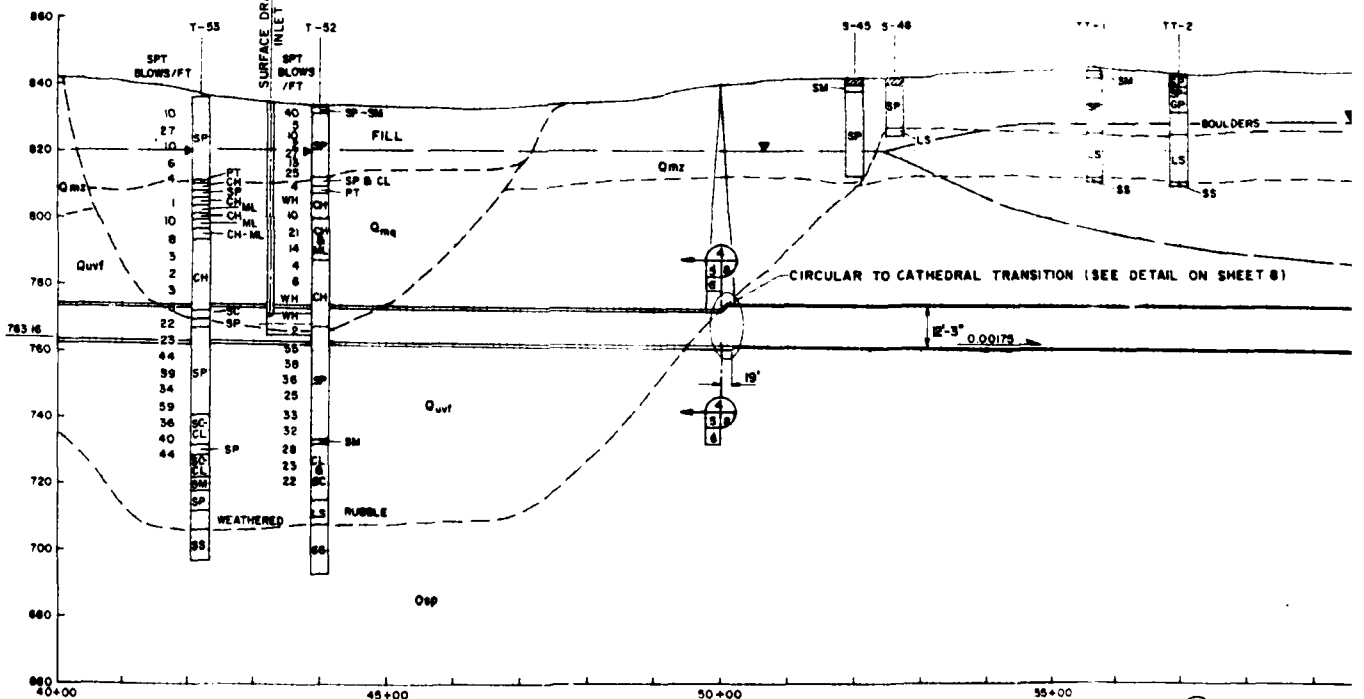
PLATE 4



PLAN

APPROXIMATE SCALE IN FEE

NOTE
SURFACE DRAINAGE INLET IS NOT ON THE CENTER LINE, SHOWN ON PROFILE FOR CLARITY



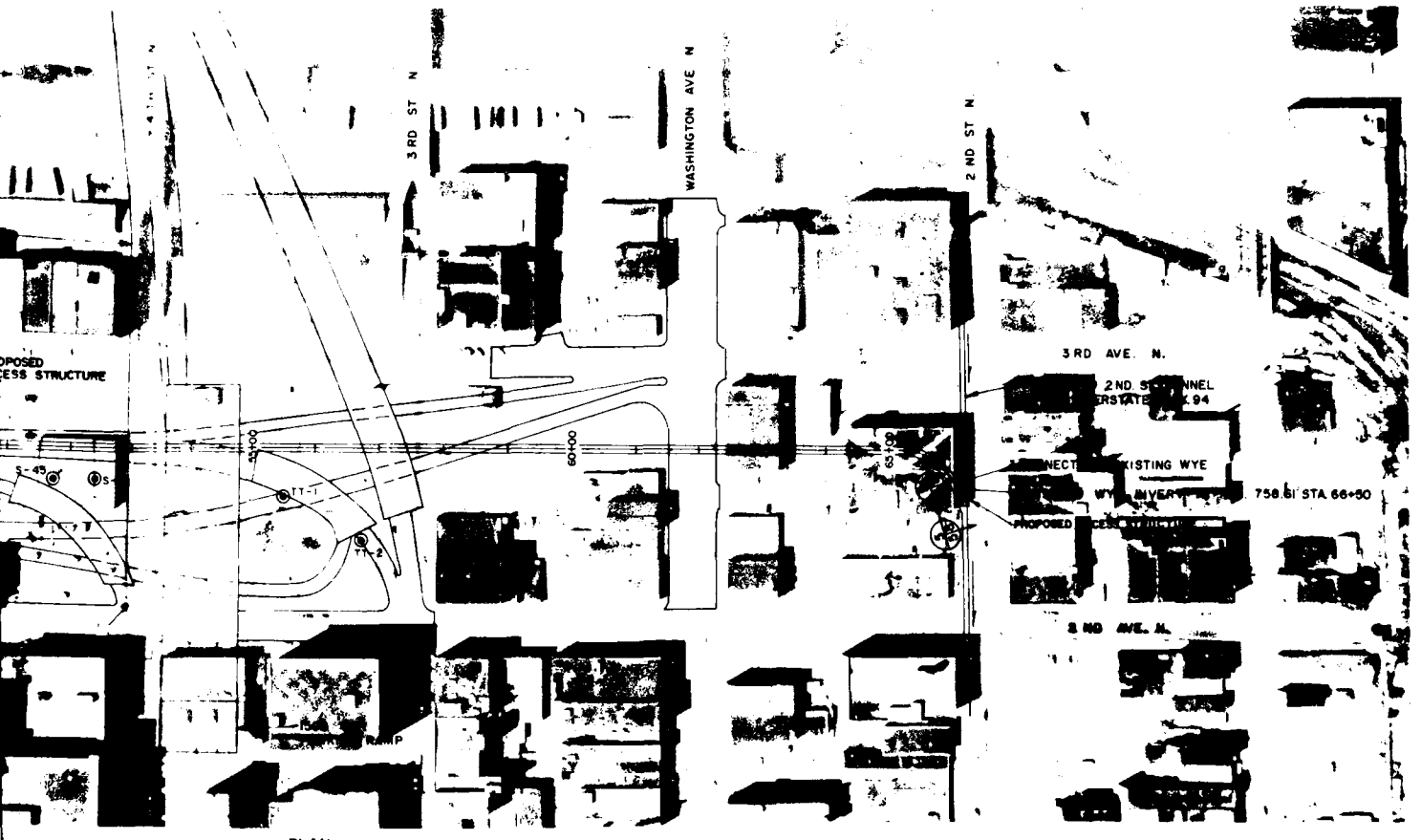
SECTION 4/5/6

(10'DIA. CIRCULAR TUNNEL

PROFILE

SECTION 4/5/6

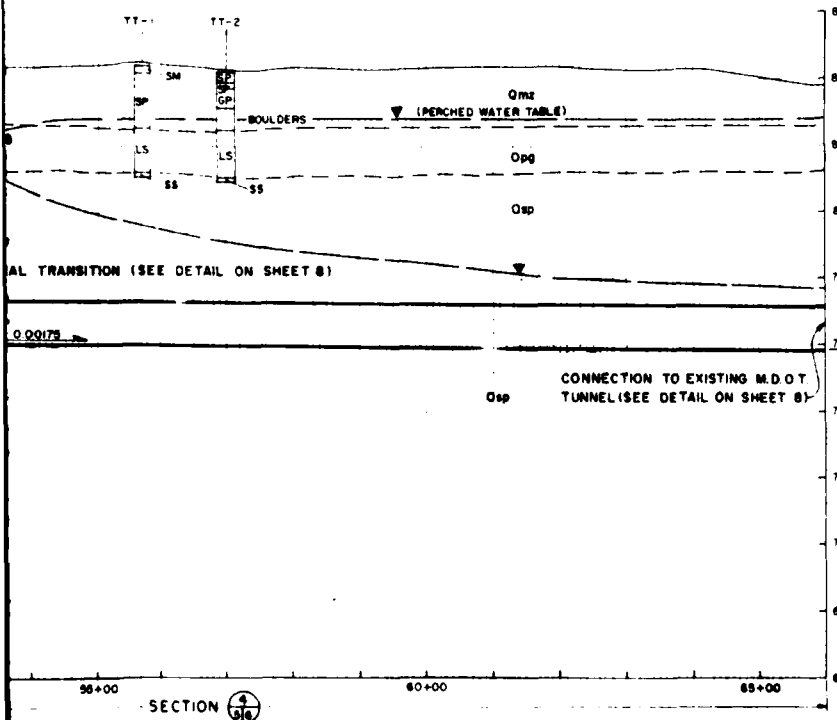
(9'-3 1/4" X 12'-3" CATHEDRAL TUNNEL)



PLAN

100 0 100 200

APPROXIMATE SCALE IN FEET



SECTION 4/16

(9'-3 1/2" X 12'-3" CATHEDRAL TUNNEL)

LEGEND

Qmq	FINE GRAINED LOWER TERRACE DEPOSITS
Qmz	COARSE GRAINED LOWER TERRACE DEPOSITS
Quvf	UNDIFFERENTIATED GLACIAL TILL, OUTWASH SAND AND ALLUVIUM IN BURIED BEDROCK VALLEYS
Osp	ST PETER SANDSTONE
Opq	PLATTEVILLE LIMESTONE AND GLENWOOD SHALE
▽	APPROXIMATE WATER TABLE

NOTES:

1. MATERIALS CLASSIFIED ACCORDING TO SYSTEM PRESENTED IN U S G S MAP I-1157, GEOLOGIC AND HYDROLOGIC ASPECTS OF TUNNELING IN THE TWIN CITIES AREA, MINNESOTA, PLATES 2 & 5 PROJECT BORINGS SHOW FILL AT THE GROUND SURFACE AT SEVERAL LOCATIONS FILL IS DIFFERENTIATED FROM OTHER MATERIALS BETWEEN STATIONS 40+00 & 45+00 ONLY
2. DETAILED BORING LOGS ARE SHOWN ON PLATES C-9 TO C-11

REFERENCES

1. TUNNEL PLAN & PROFILE STA -0+25 TO STA 20+00 SHEET 7
2. TUNNEL PLAN & PROFILE STA 20+00 TO STA 40+00 SHEET 8
3. TUNNEL SECTIONS & DETAILS SHEET 8
4. LEGEND OF MATERIALS SHOWN ON THIS SHEET & ON SHEET C-11
5. THE SCALE OF PLAN VIEW IS APPROXIMATE DUE TO RELIEF DISPLACEMENT OF PHOTO

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DR RW
DESIGNED BY: J M J
DRAWN BY: D R
SUBMITTED BY:
APPROVED:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

TUNNEL PLAN & PROFILE
STA 40+00 TO STA 66+50

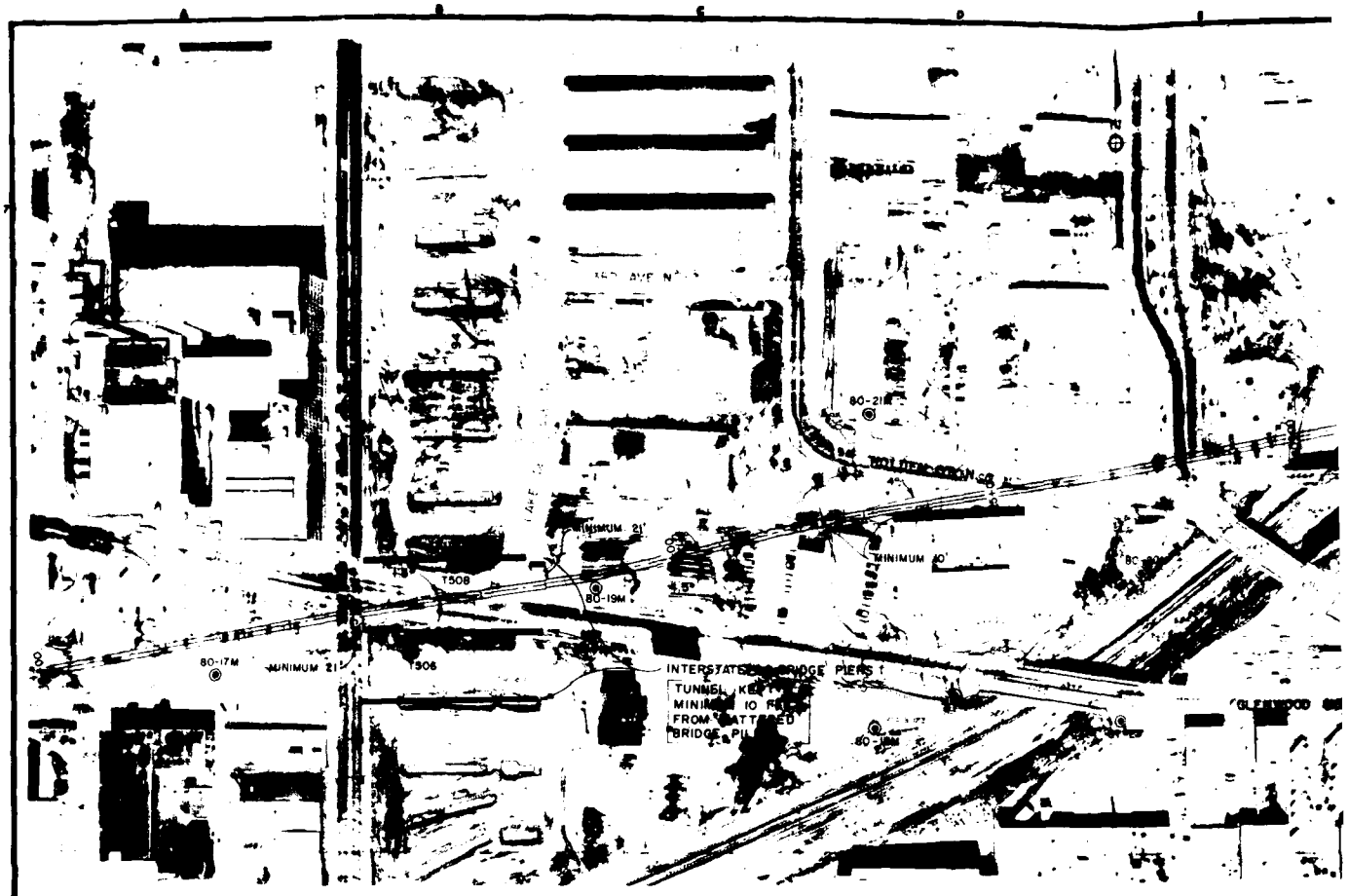
DATE: AUGUST 1962

SCALE: AS SHOWN

M34.3-R-5/184

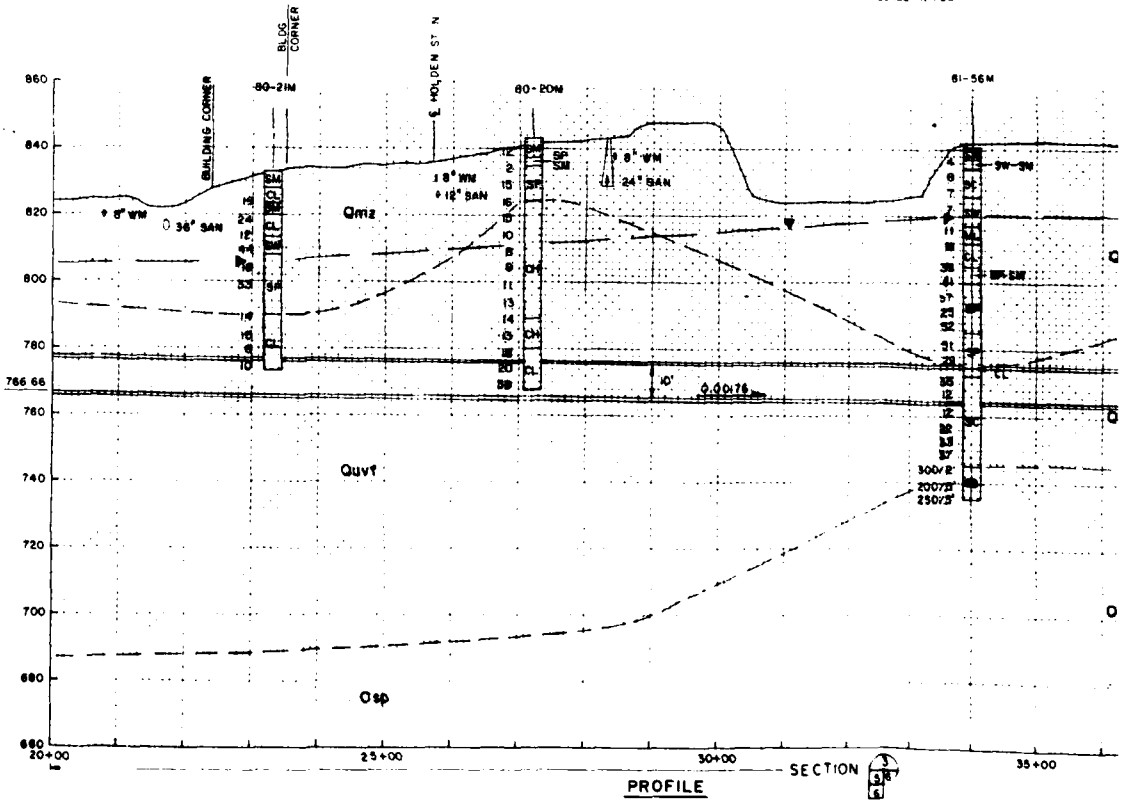
SHEET 3 OF 44

PLATE 6



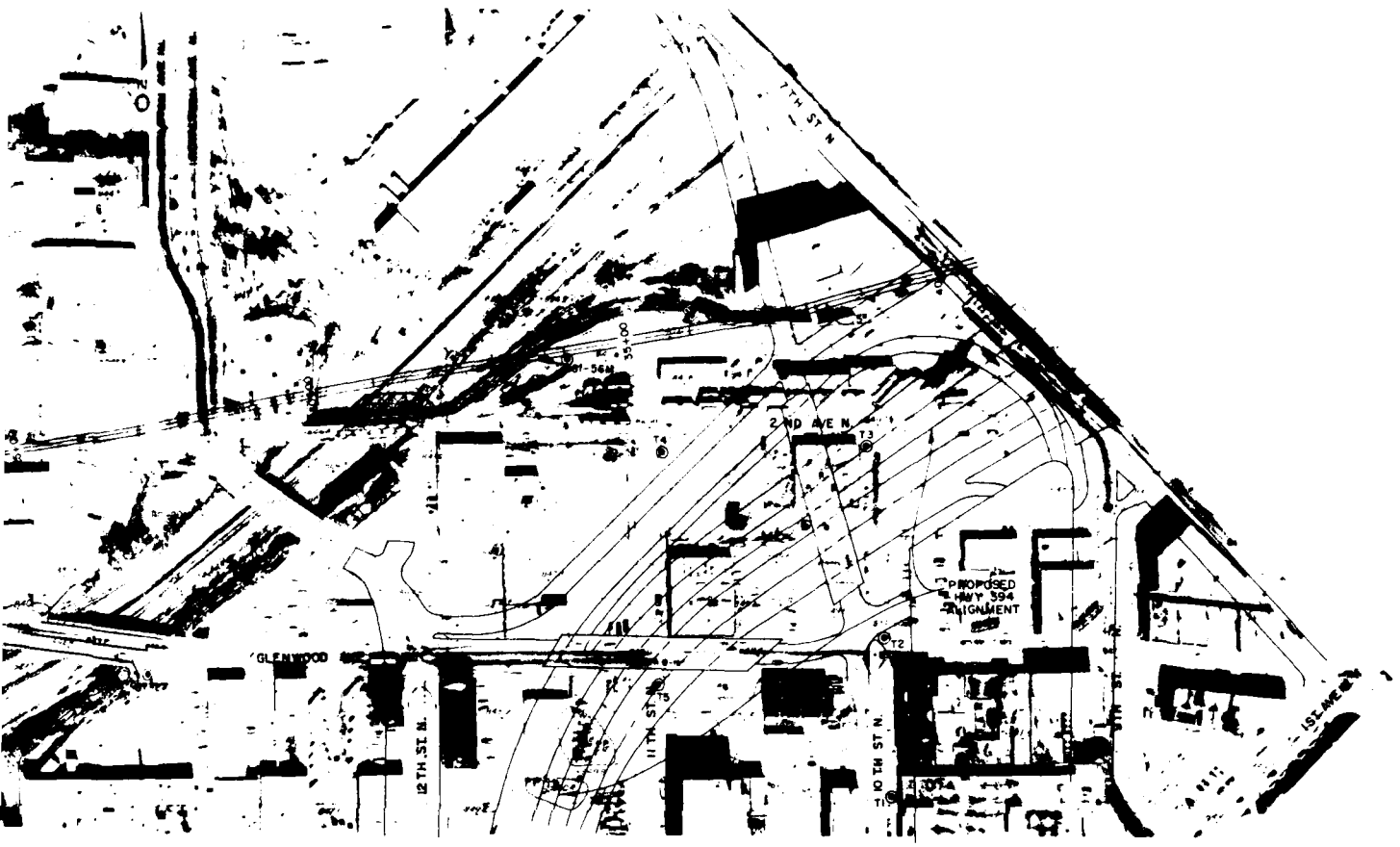
PLAN

SCALE IN FEET



PROFILE

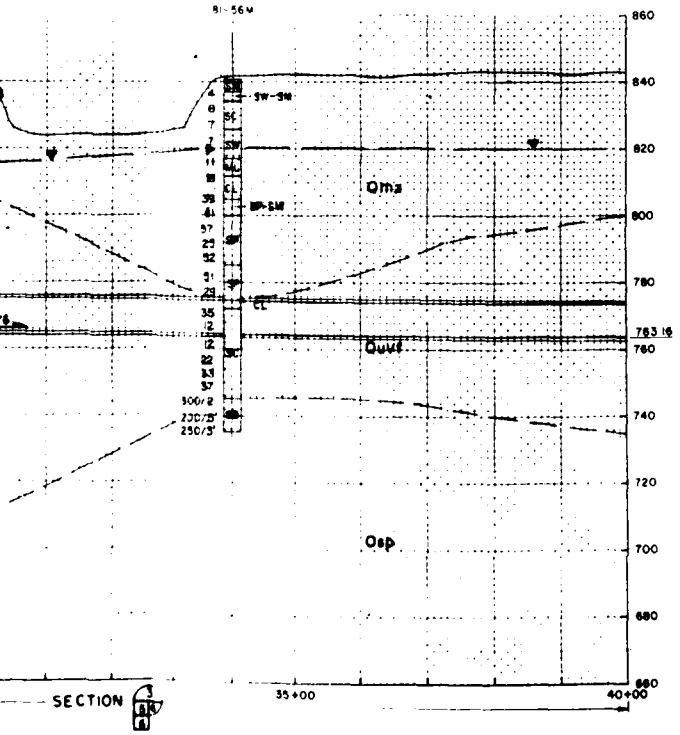
SECTION



PLAN

100 0 100 200

SCALE IN FEET



LEGEND

Qmq	FINE GRAINED LOWER TERRACE DEPOSITS
Qmz	COARSE GRAINED LOWER TERRACE DEPOSITS
Quvf	UNDIFFERENTIATED GLACIAL TILL, OUTWASH SAND AND ALLUVIUM IN BURIED BEDROCK VALLEYS
Osp	ST PETER SANDSTONE
Opq	PLATTEVILLE LIMESTONE AND GLENWOOD SHALE
▽	APPROXIMATE WATER TABLE

NOTES:

1. MATERIALS CLASSIFIED ACCORDING TO SYSTEM PRESENTED IN U.S.G.S. MAP I-1157, GEOLOGIC AND HYDROLOGIC ASPECTS OF TUNNELING IN THE TWIN CITIES AREA, MINNESOTA, PLATES 2 & 5. PROJECT BORINGS SHOW FILL AT THE GROUND SURFACE AT SEVERAL LOCATIONS. FILL IS DIFFERENTIATED FROM OTHER MATERIALS BETWEEN STATIONS 40+00 & 45+00 ONLY
2. DETAILED BORING LOGS ARE SHOWN ON PLATES C-9 TO C-11

REFERENCES

1. TUNNEL PLAN & PROFILE STA 0-25 TO STA 20+00 ..... SHEET 7
2. TUNNEL PLAN & PROFILE STA 40+00 TO STA 66+50 ..... SHEET 5
3. TUNNEL SECTIONS & DETAILS ..... SHEET 8
4. LEGEND OF MATERIALS SHOWN ON SHEET C-11

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY: W.DWR
DRAWN BY: J.B.J., J.M.J
CHECKED BY: D.W.R
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

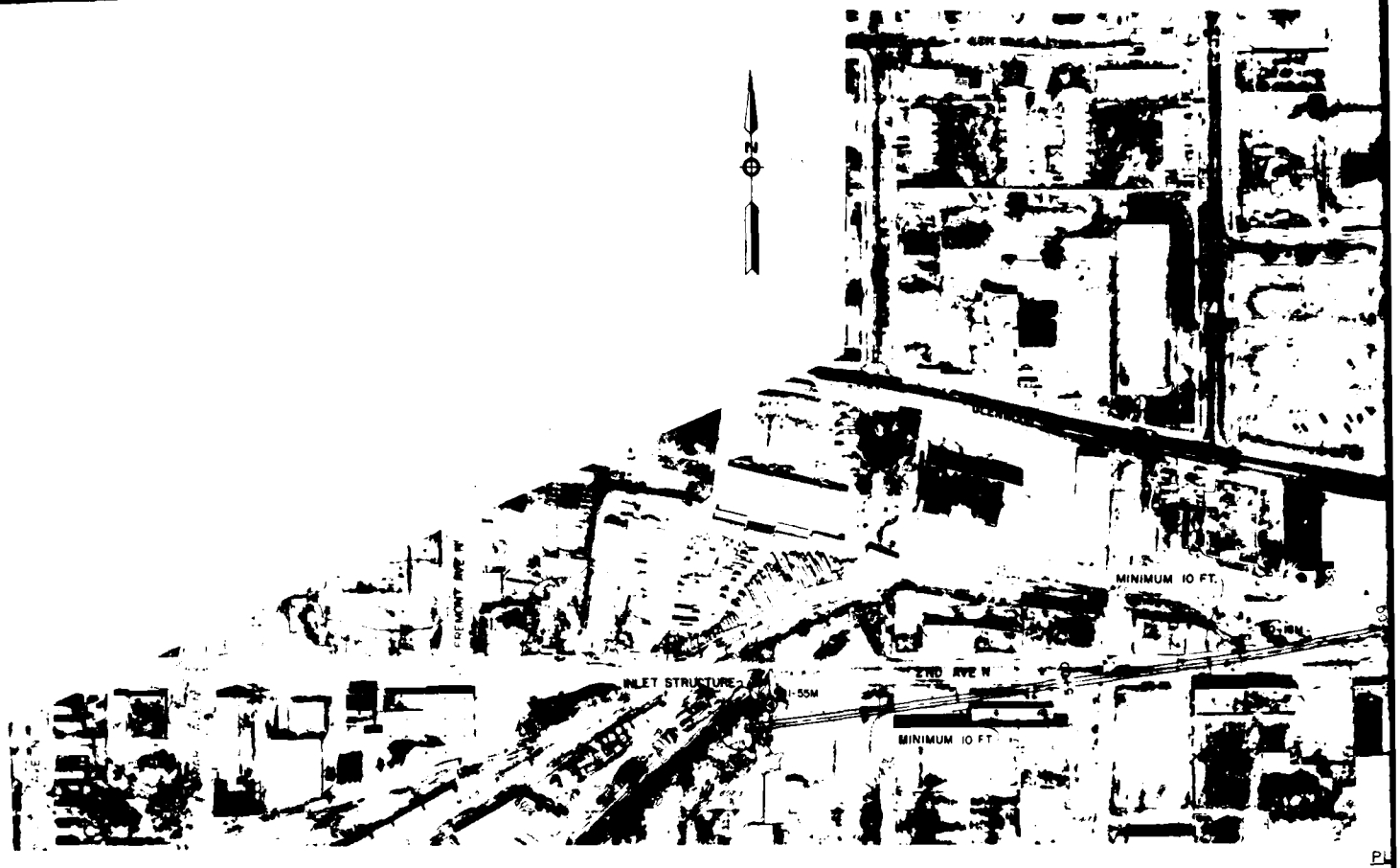
TUNNEL PLAN & PROFILE
STA 20+00 TO 40+00

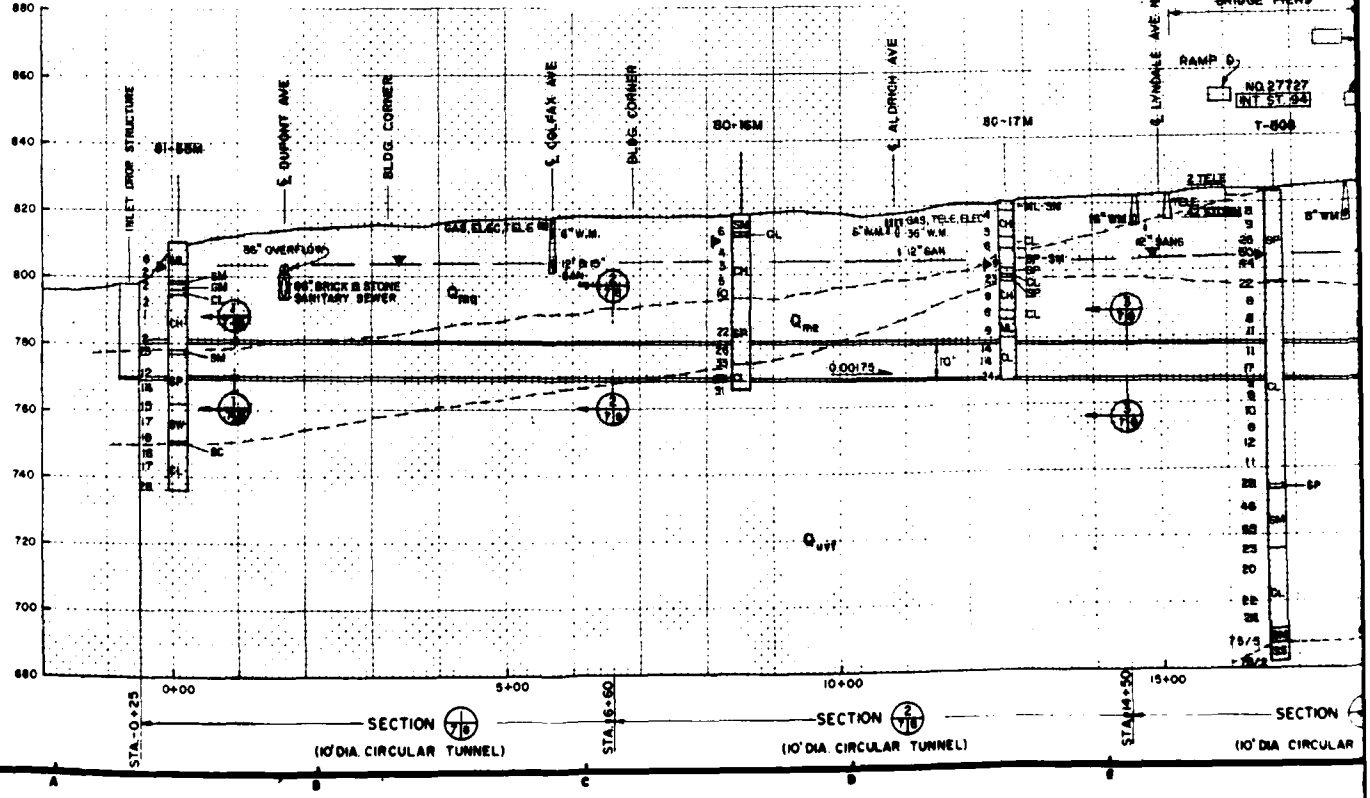
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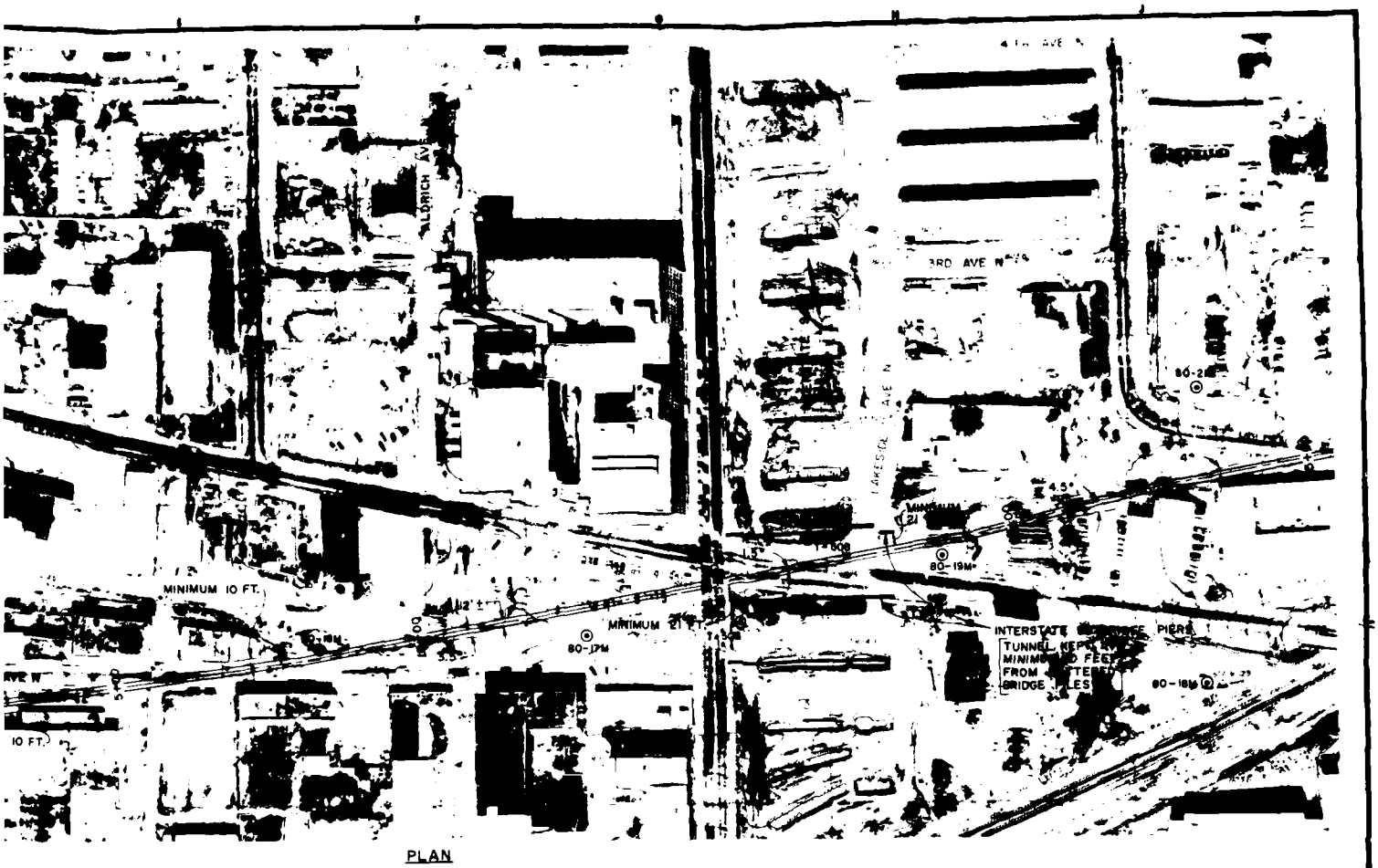
SCALE: AS SHOWN

DRAWING NUMBER: M34.3-R-5/185
SHEET 6 OF 44

PLATE 6


FREMONT AVE N
INLET STRUCTURE
MINIMUM 10 FT.
2ND AVE N
MINIMUM 10 FT.
100
0
SCALE

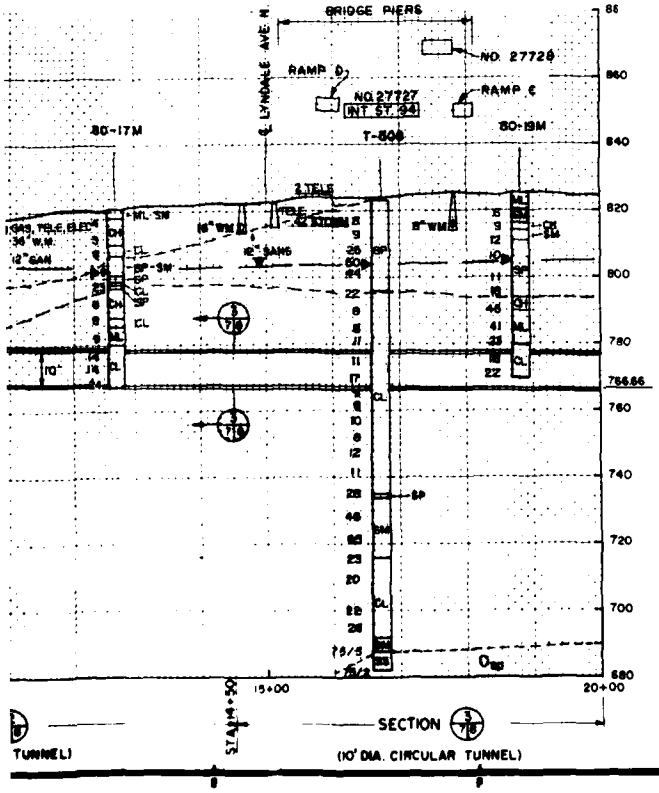

INLET DROP STRUCTURE
E. DUPONT AVE.
BLDG. CORNER
E. GOLFAX AVE
BLDG. CORNER
E. ALDRICH AVE
E. LYNDALE AVE N
BRIDGE PIERS
RAMP D
NO. 27727
INT ST 94
T-508
81-55M
80-16M
80-17M
36" OVERFLOW
66" BRICK & STONE SANITARY SEWER
0.00175
880
860
840
820
800
780
760
740
720
700
680
0+00
5+00
10+00
15+00
STA. 0+25
STA. 16+60
STA. 14+50
SECTION 1/7B
(10' DIA. CIRCULAR TUNNEL)
SECTION 2/7B
(10' DIA. CIRCULAR TUNNEL)
SECTION
(10' DIA. CIRCULAR



PLAN

100 0 100 200

SCALE IN FEET



TUNNEL

SECTION

(10' DIA. CIRCULAR TUNNEL)

LEGEND

Qmg	FINE GRAINED LOWER TERRACE DEPOSITS
Qmz	COARSE GRAINED LOWER TERRACE DEPOSITS
Quvf	UNDIFFERENTIATED GLACIAL TILL, OUTWASH SAND AND ALLUVIUM IN BURIED BEDROCK VALLEYS
Osp	ST PETER SANDSTONE
Opq	PLATTEVILLE LIMESTONE AND GLENWOOD SHALE
— W —	APPROXIMATE WATER TABLE

NOTES:

1. MATERIALS CLASSIFIED ACCORDING TO SYSTEM PRESENTED IN U.S.G.S. MAP I-115?, GEOLOGIC AND HYDROLOGIC ASPECTS OF TUNNELING IN THE TWIN CITIES AREA, MINNESOTA, PLATES 2 & 5. PROJECT BORINGS SHOW FILL AT THE GROUND SURFACE AT SEVERAL LOCATIONS. FILL IS DIFFERENTIATED FROM OTHER MATERIALS BETWEEN STATIONS 40+00 & 45+00 ONLY.
2. DETAILED BORING LOGS ARE SHOWN ON SHEETS C-9 TO C-11

REFERENCES — SHEET NO.

1. TUNNEL PLAN & PROFILE: STA 20+00 TO STA 40+00 SHEET ..6
2. TUNNEL PLAN & PROFILE: STA 40+00 TO STA 66+50 SHEET ..5
3. TUNNEL SECTIONS & DETAILS SHEET ..8
4. LEGEND OF MATERIALS SHOWN ON SHEET ..C-11
5. INLET STRUCTURE ..9

DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

TUNNEL PLAN & PROFILE
STA 0-25 to 20+00

DR. RW
J.G.J. J.M.J.
D.R.

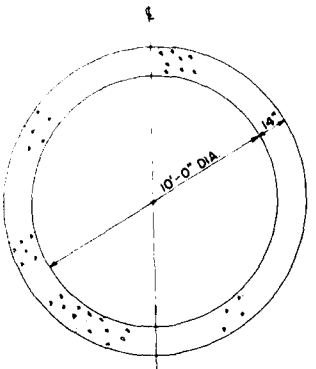
DATE: AUGUST 1962

AS SHOWN

M34.3-R-5/186

SHEET 7 OF 9

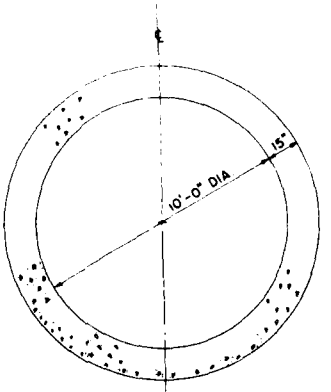
PLATE 7



SECTION 1/7|8

STA 0+25 TO 6+60

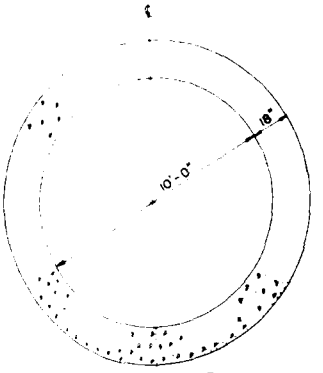
CIRCULAR TUNNEL



SECTION 2/7|8

STA 6+60 TO 14+50

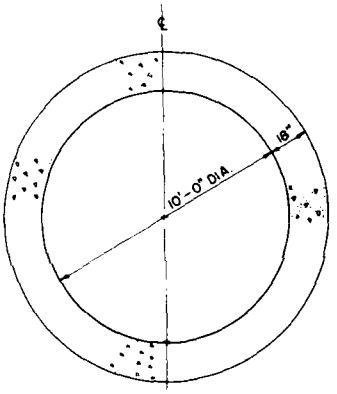
CIRCULAR TUNNEL



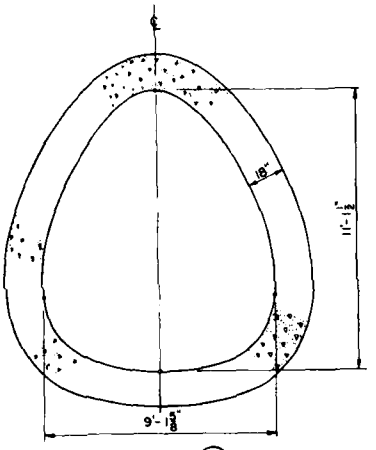
SECTION 3/5|8/6

STA 14+50 TO 50+00

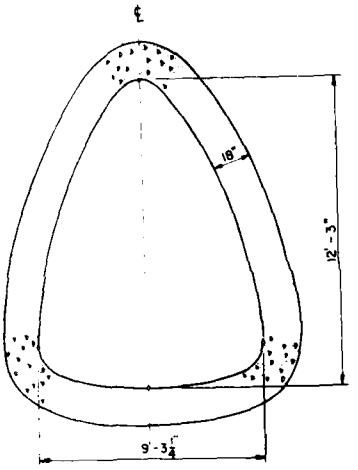
CIRCULAR TUNNEL



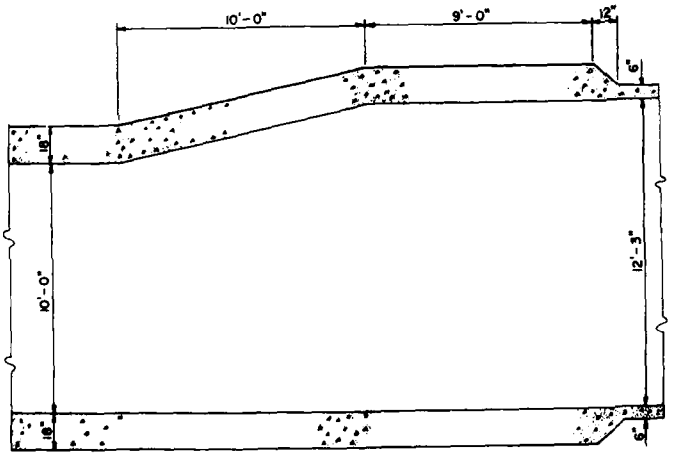
SECTION (2)



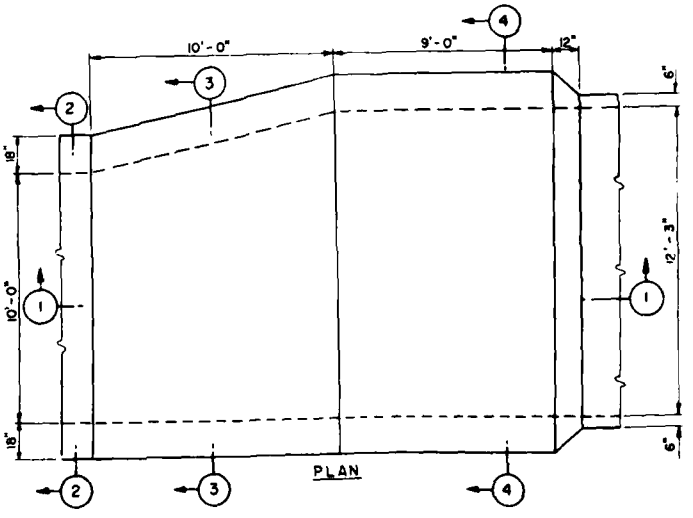
SECTION (3)



SECTION (4)



SECTION (1)

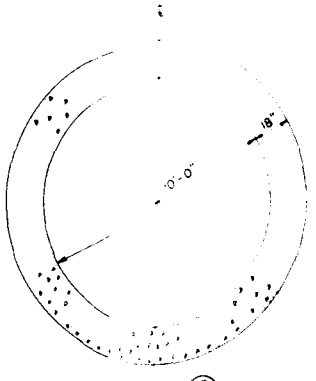


PLAN

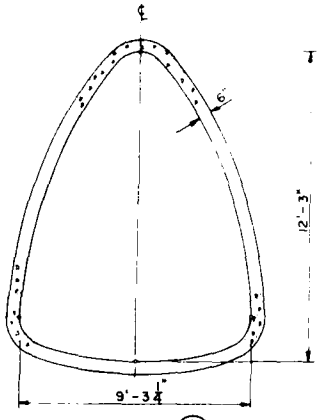
CIRCULAR TO CATHEDRAL TRANSITION

0

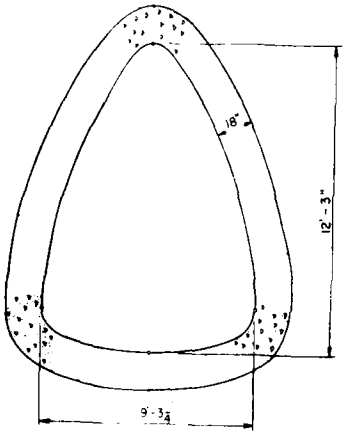
SCALE



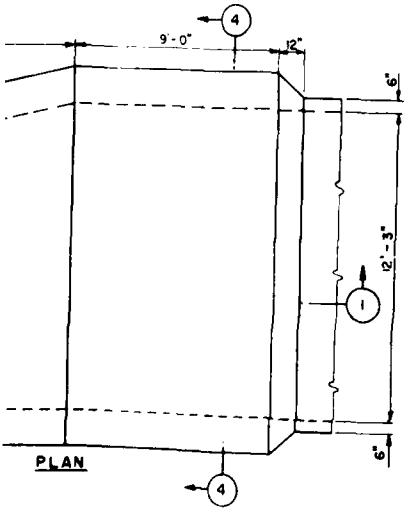
SECTION (3/5B/6)
STA. 14+50 TO 50+00
CIRCULAR TUNNEL



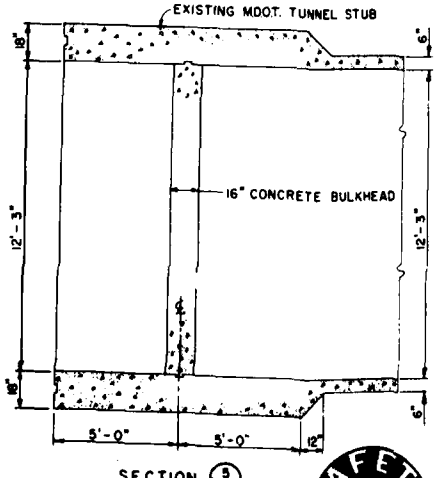
SECTION (4/5B)
STA. 50+00 TO 66+00
CATHEDRAL TUNNEL



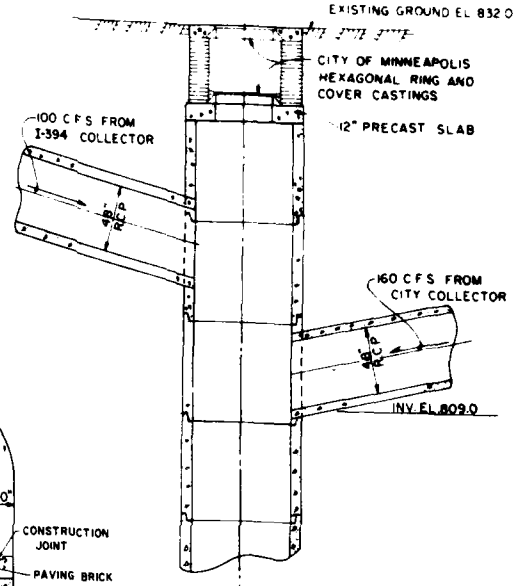
SECTION (4)

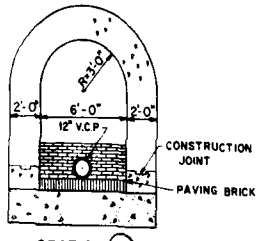


PLAN

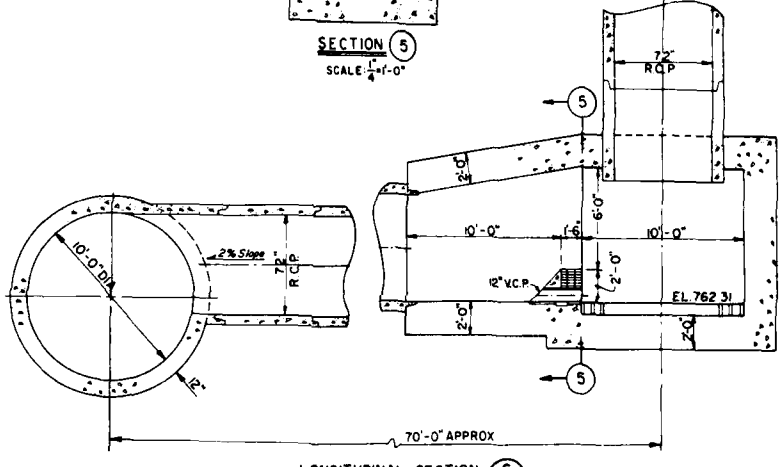


SECTION (B/5B)
STA. 66+00





SECTION (5)
SCALE: 1/4"=1'-0"



LONGITUDINAL SECTION (6/5B)
SURFACE DRAINAGE INLET
SCALE: 1/4"=1'-0"

NOTES:
1. TUNNEL LINER THICKNESSES ARE GIVEN AS A MINIMUM THICKNESS REQUIRED FOR THE SECTION.
2. DUE TO RESTRICTIONS OF TUNNELING METHOD CHOSEN A THICKNESS EQUAL TO THE MAXIMUM LINER THICKNESS REQUIRED MAY BE USED IN CONSTRUCTION OF THE TUNNEL.

SCALE IN FEET
0 1 2 3 4

SAFETY PAYS

SYMBOL	DESCRIPTION	DATE	APPROVED

DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

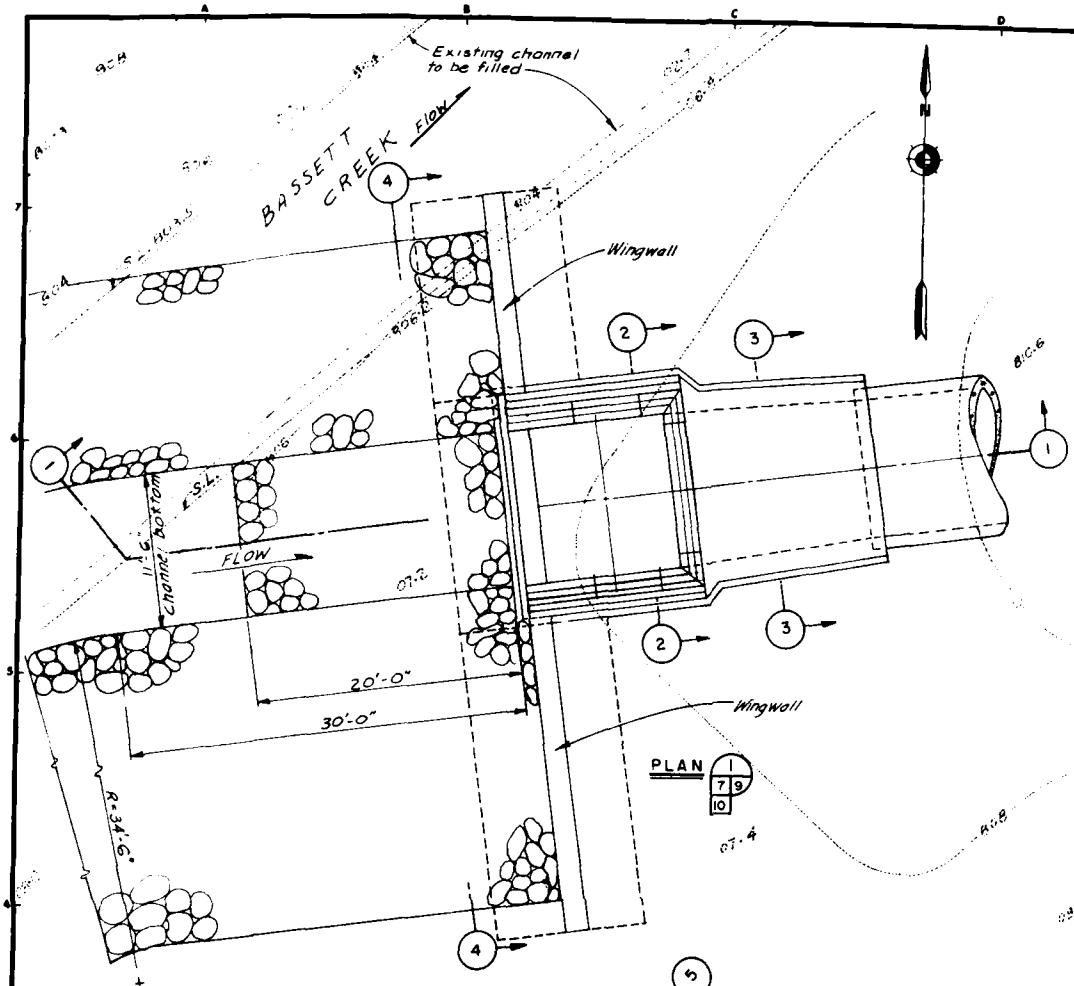
DESIGNED BY: S.P.L.
DRAWN BY:
TRACED BY:
CHECKED BY: S.P.L.
SUBMITTED BY:
APPROVED:

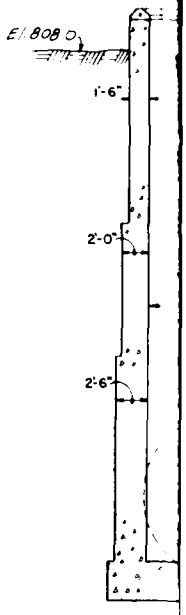
PHASE II FLOOD CONTROL
DESIGN MEMORANDUM BASSETT CREEK MINNESOTA

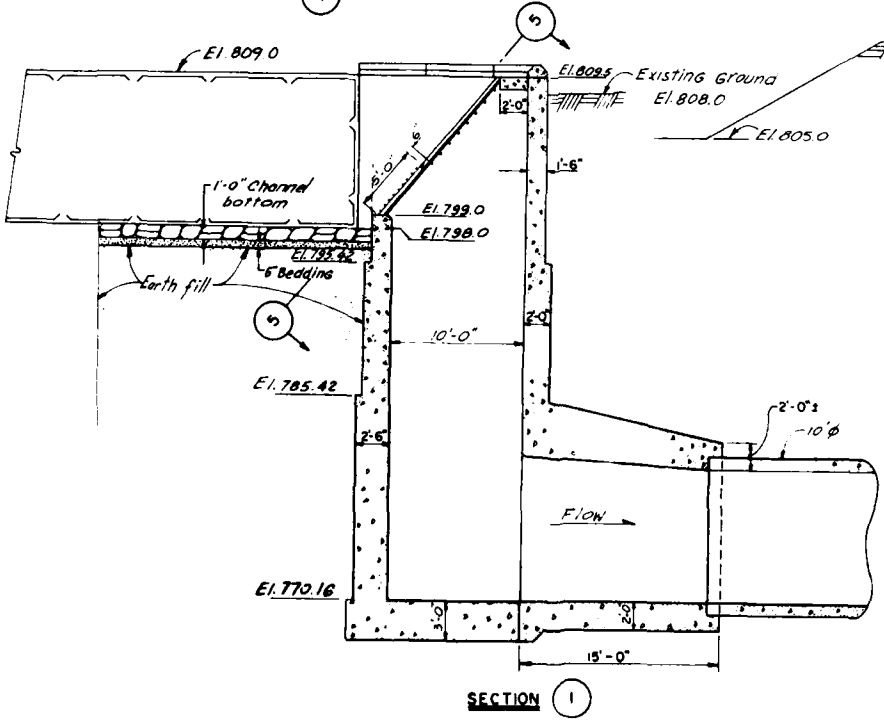
BASSETT CREEK
TUNNEL SECTIONS AND DETAILS

DATE: AUGUST 1962
SCALE: AS SHOWN
SPEC. NO.
DRAWING NUMBER: M34.3-R-5/187
SHEET 8 OF 44

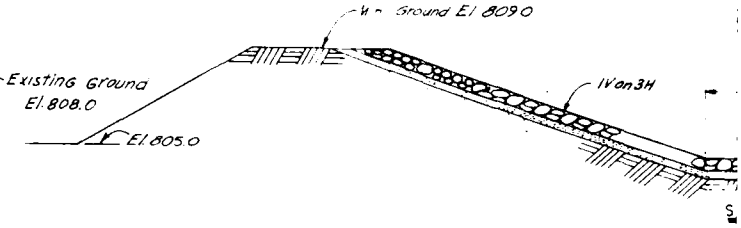
PLATE 8

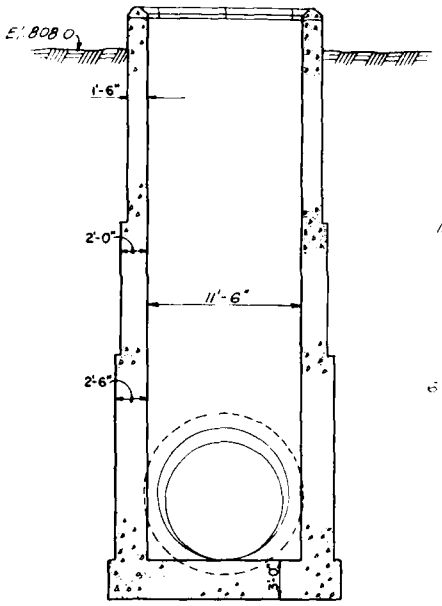

Existing channel to be filled
FLOW
BASSETT CREEK
Wingwall
CHANNEL
FLOW
20'-0"
30'-0"
R=34'6"
Wingwall
PLAN
1
7
9
10


El. 808.0
1'-6"
2'-0"
2'-6"

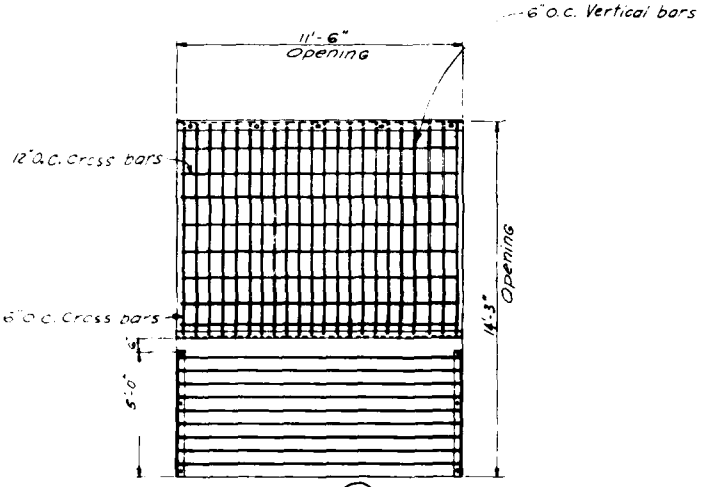

El. 809.0
El. 809.5
Existing Ground
El. 808.0
El. 805.0
1'-0" Channel bottom
El. 799.0
El. 798.0
El. 795.42
6" Bedding
Earth fill
2'-0"
1'-6"
6'-0"
10'-0"
El. 785.42
2'-6"
2'-0"
10'ø
FLOW
El. 770.16
15'-0"

SECTION (1)


Ground El. 809.0
1Van3H

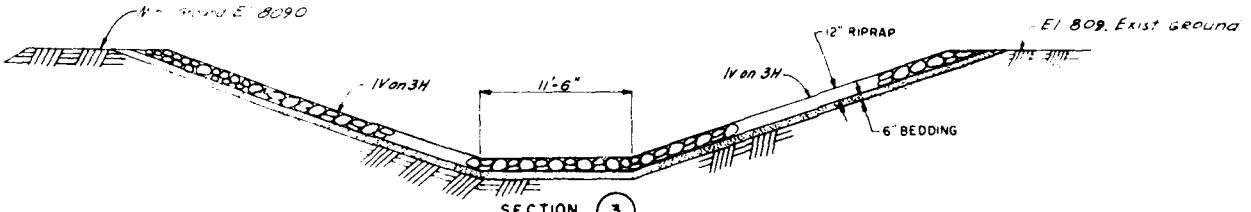


SECTION (2)

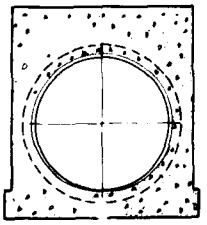


SECTION (5)

TRASH RACK DETAIL
SCALE 3/8" = 1'-0"



SECTION (3)



SECTION (3)

5 4 3 2 1 0
SCALE IN FEET

SAFETY PAYS

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

S P L — PHASE II FLOOD CONTROL — DESIGN MEMORANDUM BASSETT CREEK MINNESOTA

BASSETT CREEK
INLET STRUCTURE

DATE: AUGUST 1982

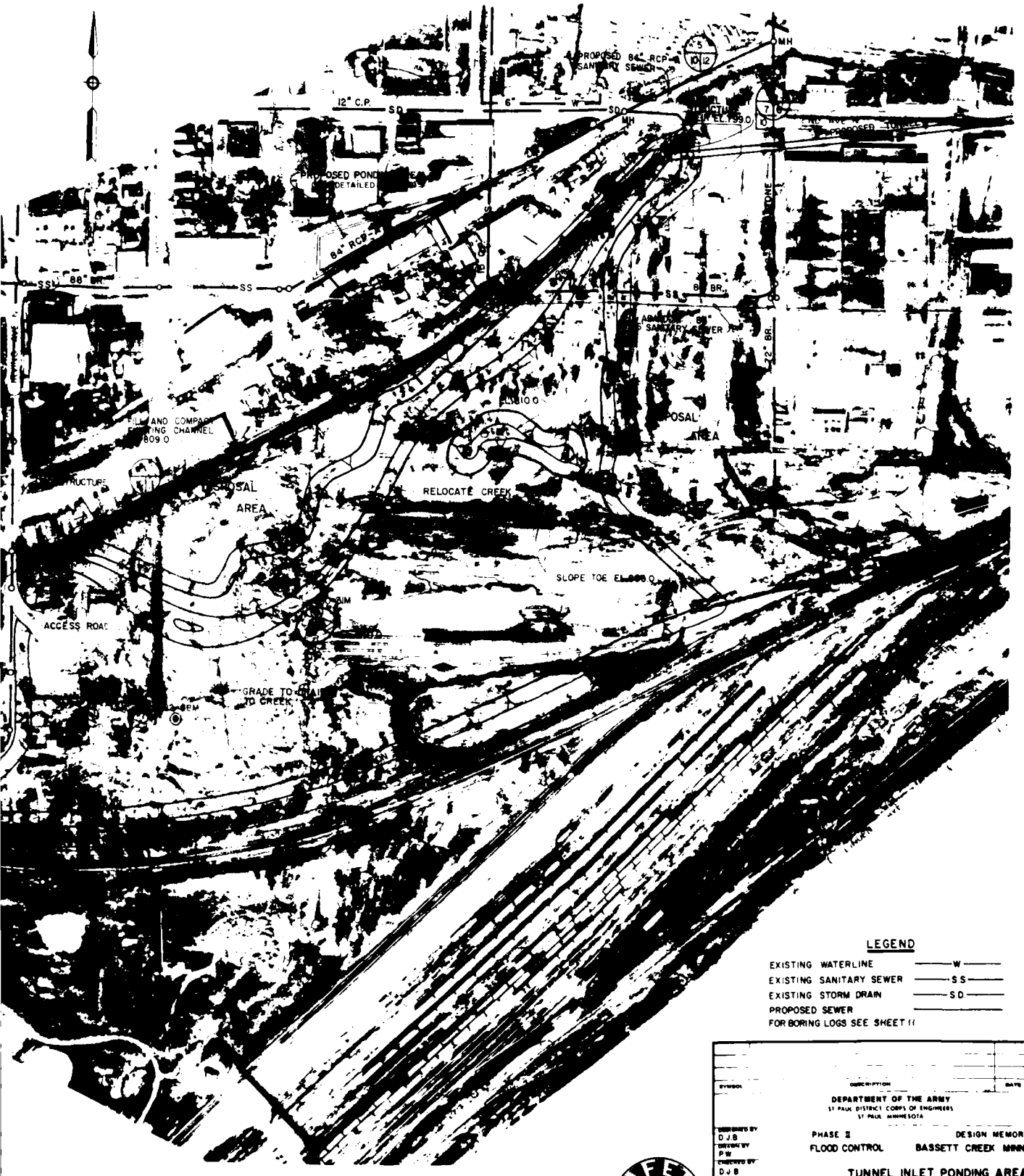
SCALE: AS SHOWN

DRAWING NUMBER
M34 3-R-5/188

SHEET 9 OF 44

PLATE 9


805
800.3
800.0
275'
300'
5
NOTE ALL SIDE SLOPES ARE IV ON 3H
PROPOSED PONDING AREA
DETAILED DRAWING
50 0 50 100
SCALE IN FEET



LEGEND

EXISTING WATERLINE ——W——
EXISTING SANITARY SEWER ——S S——
EXISTING STORM DRAIN ——S D——
PROPOSED SEWER ————
FOR BORING LOGS SEE SHEET II

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL MINNESOTA

DESIGNED BY D J B
DRAWN BY P W
CHECKED BY D J B
SUBMITTED BY
APPROVED

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

TUNNEL INLET PONDING AREA

DATE AUGUST 1982

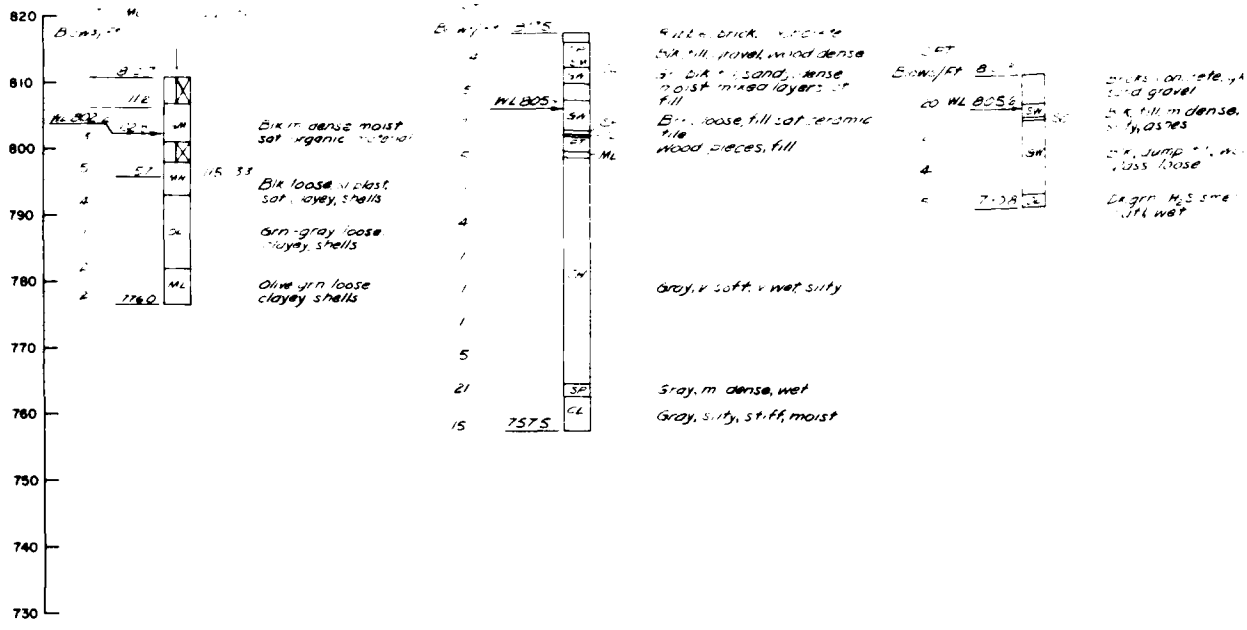
DRAWING NUMBER M34.3-R-5/189
SHEET 10 OF 44

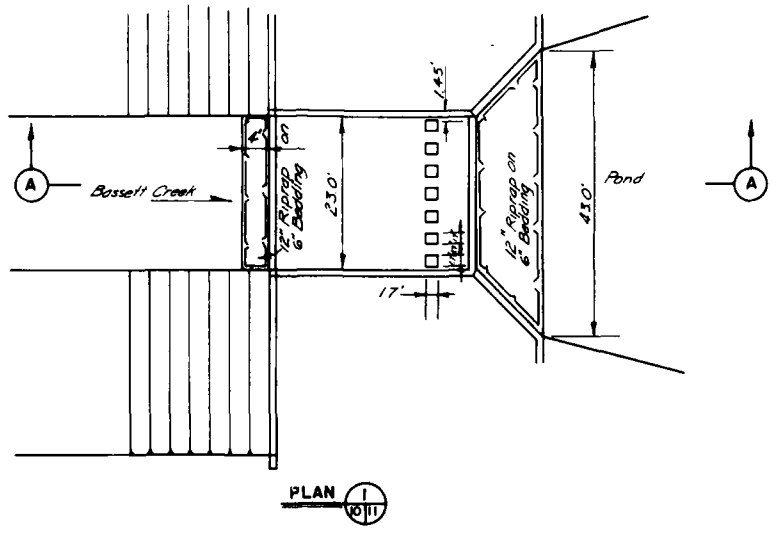
PLATE 10

80-31M
19 AUGUST 1980

82-67M
10 JUNE 1982

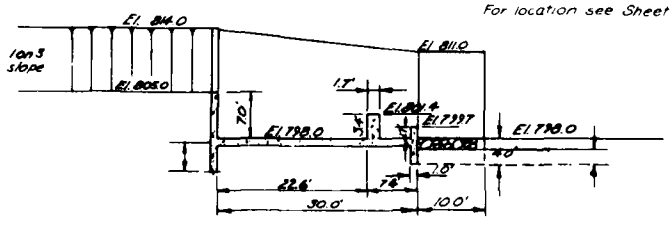
82-68M
JUNE 1982





PLAN (1/10/11)

For location see Sheet 10



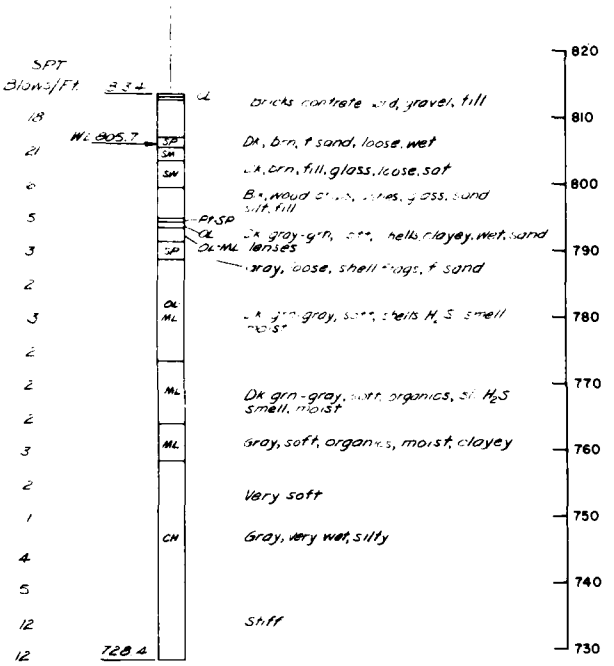
SECTION A-A

SCALE IN FEET

10 5 0 10 20 30

82-68M
JUNE 1982

82-69M
10-11 JUNE 1982



NOTE

BORING LEGEND SHOWN ON PLATE C-11

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

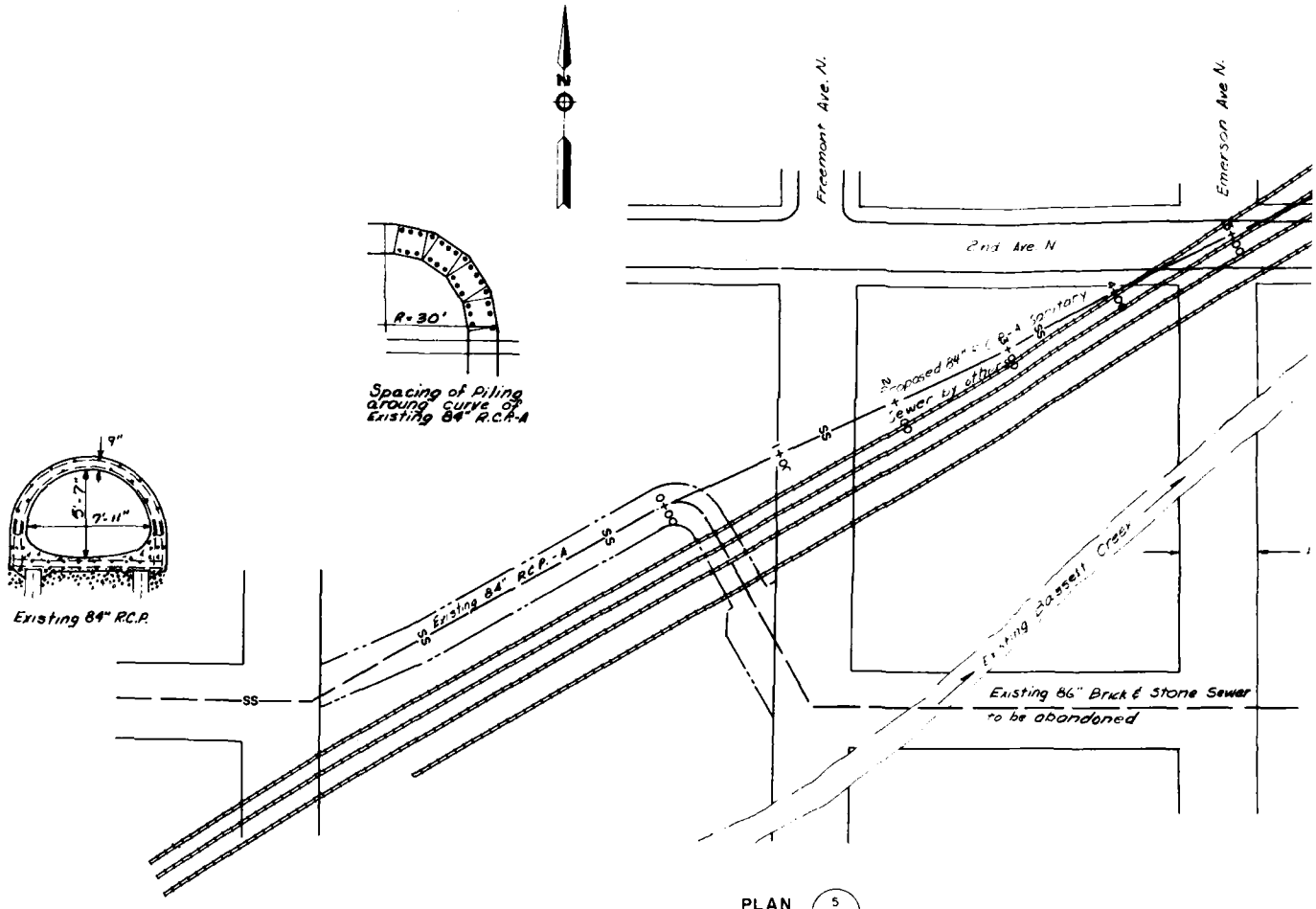
DR L.A.R.
DESIGN BY: J.M.J.
DR L.A.R.
SUBMITTED BY:
APPROVED:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
TUNNEL INLET PONDING AREA
BORING LOGS AND DROP STRUCTURE
PLAN, SECTION AND DETAILS
DATE: AUGUST 1982

SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/190
SHEET 11 OF 44

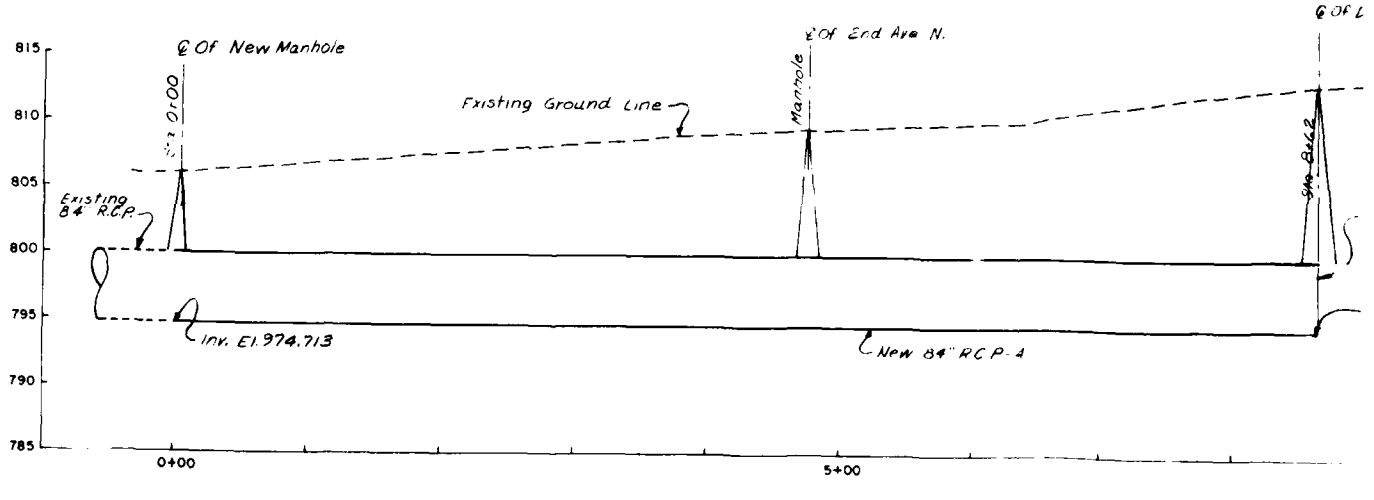
SAFETY PAYS

PLATE 11

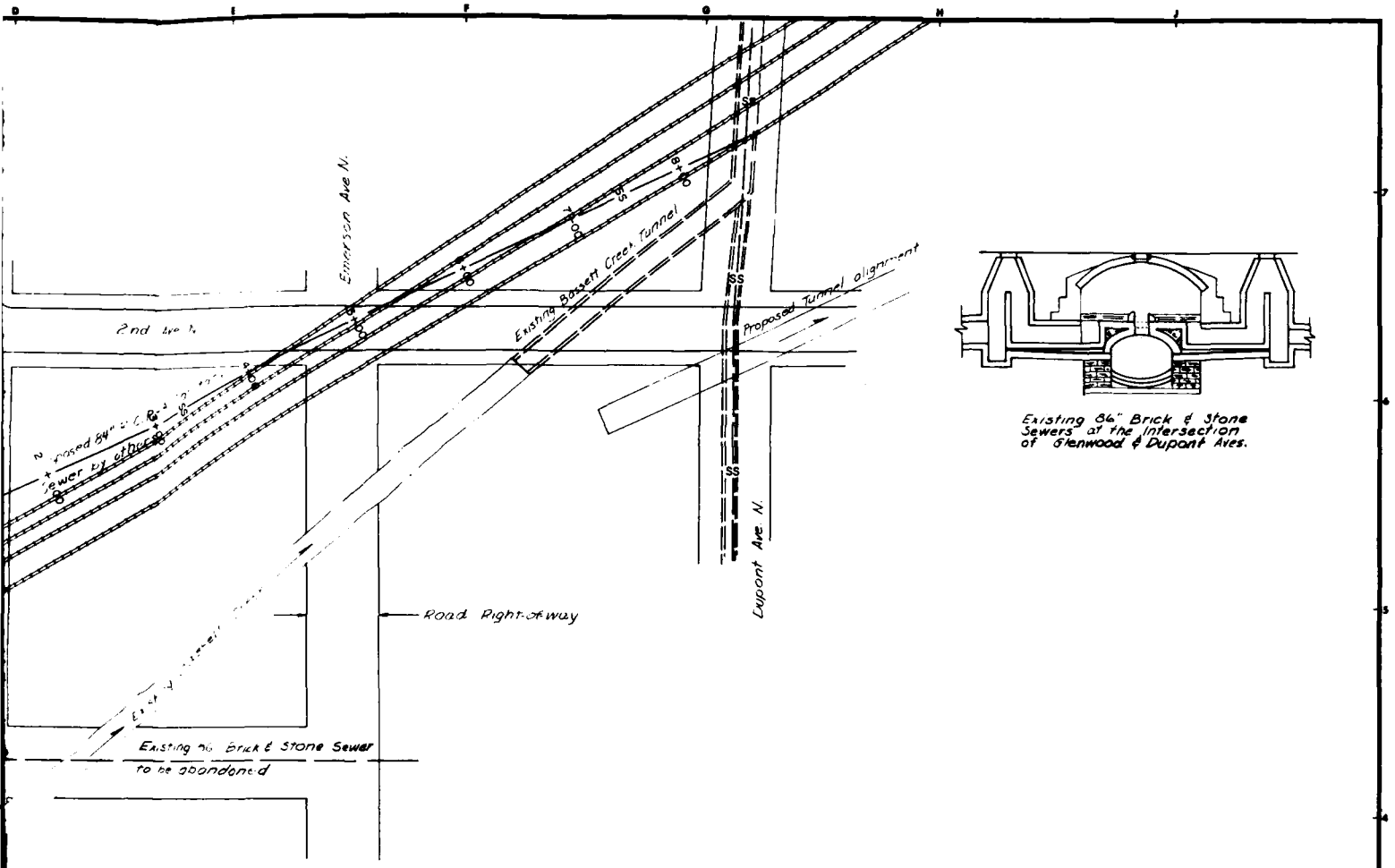


PLAN

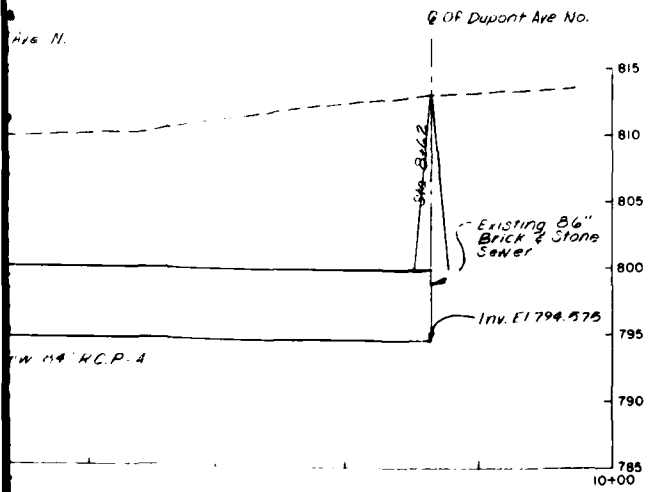
SCALE IN FEET



PROFILE



Existing 84" Brick & Stone Sewers at the Intersection of Glenwood & Dupont Aves.



DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

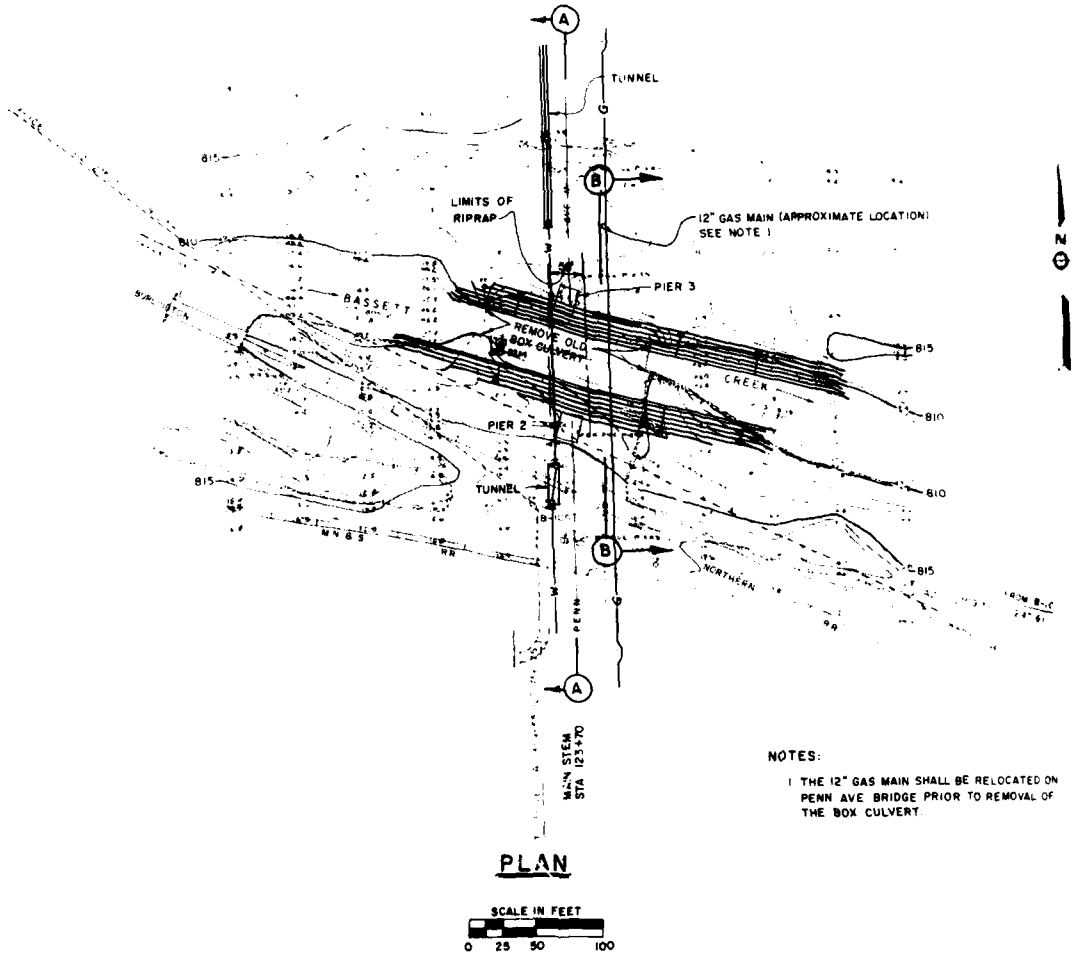
PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
INLET PONDING AREA
SEWER BETWEEN CURRIE & DUPONT AVES

L A R
P W
L A R

APPROVED: Colonel Corps of Engineers
DATE: AUGUST 1962
SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/192
SHEET 12 OF 44

SAFETY PAYS

PLATE 12

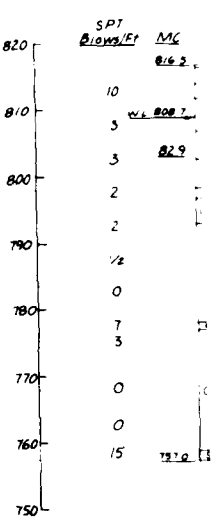


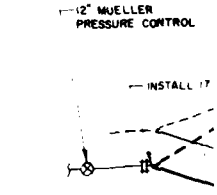
PLAN

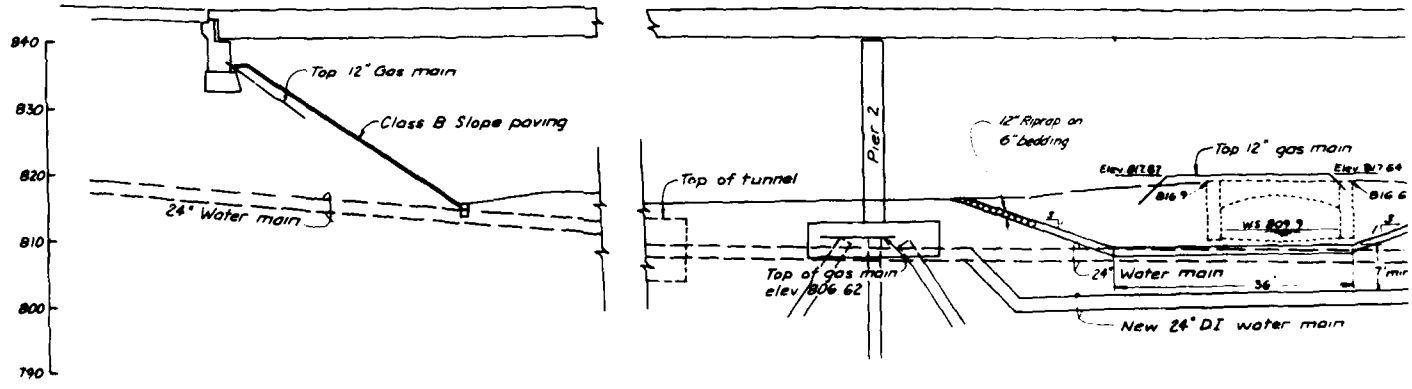
SCALE IN FEET
0 25 50 100

NOTES:

1. THE 12" GAS MAIN SHALL BE RELOCATED ON PENN AVE BRIDGE PRIOR TO REMOVAL OF THE BOX CULVERT.

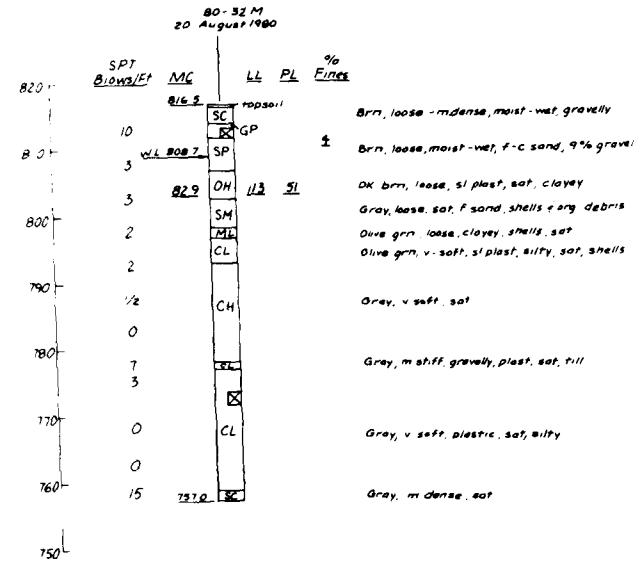




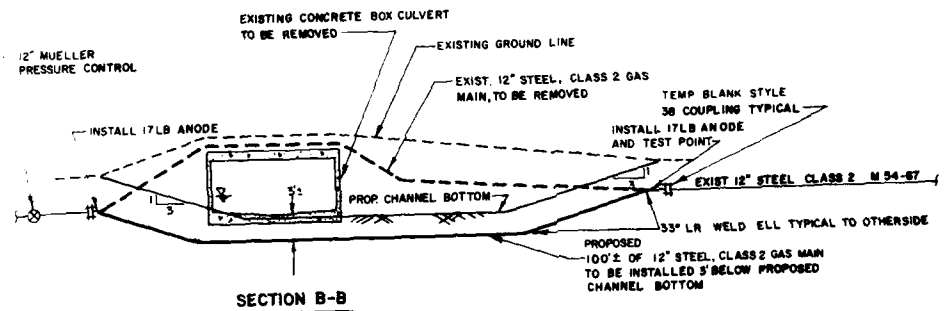


SECTION A-A

10 5 0 10 20
SCALE IN FEET

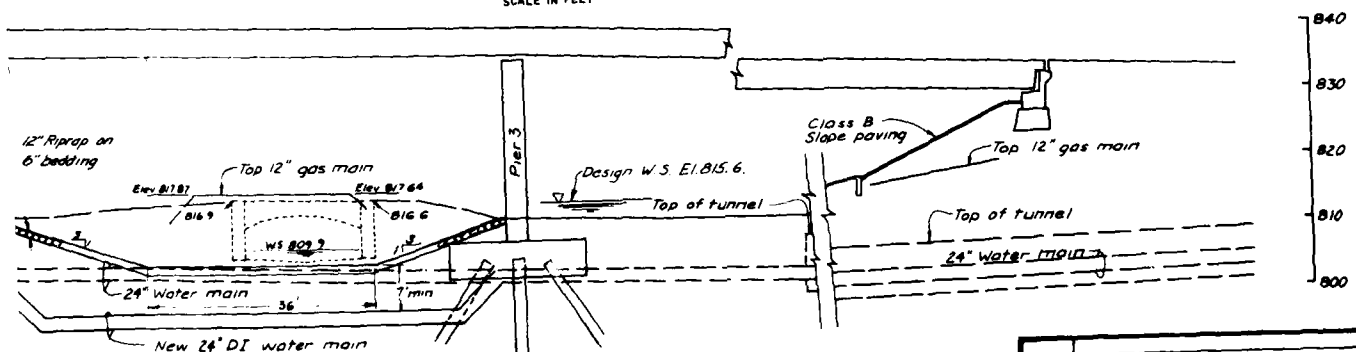


BE RELOCATED ON
R TO REMOVAL OF



SECTION B-B

10 5 0 10 20 30
SCALE IN FEET



SECTION A-A

10 5 0 10 20
SCALE IN FEET

DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

L.A.R., D.W.R.
J.M.J.
L.A.R.

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

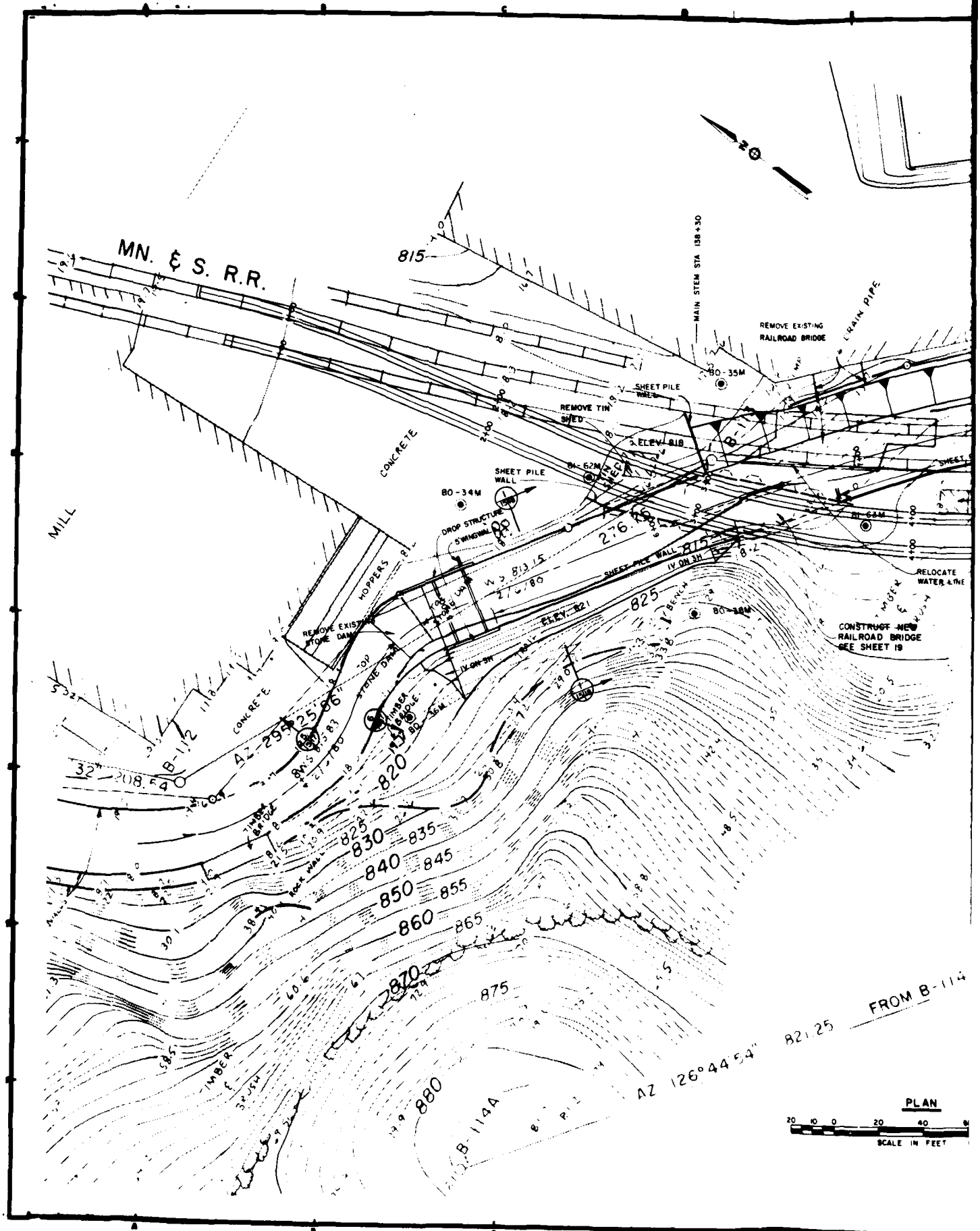
PENN AVE. CULVERT REMOVAL

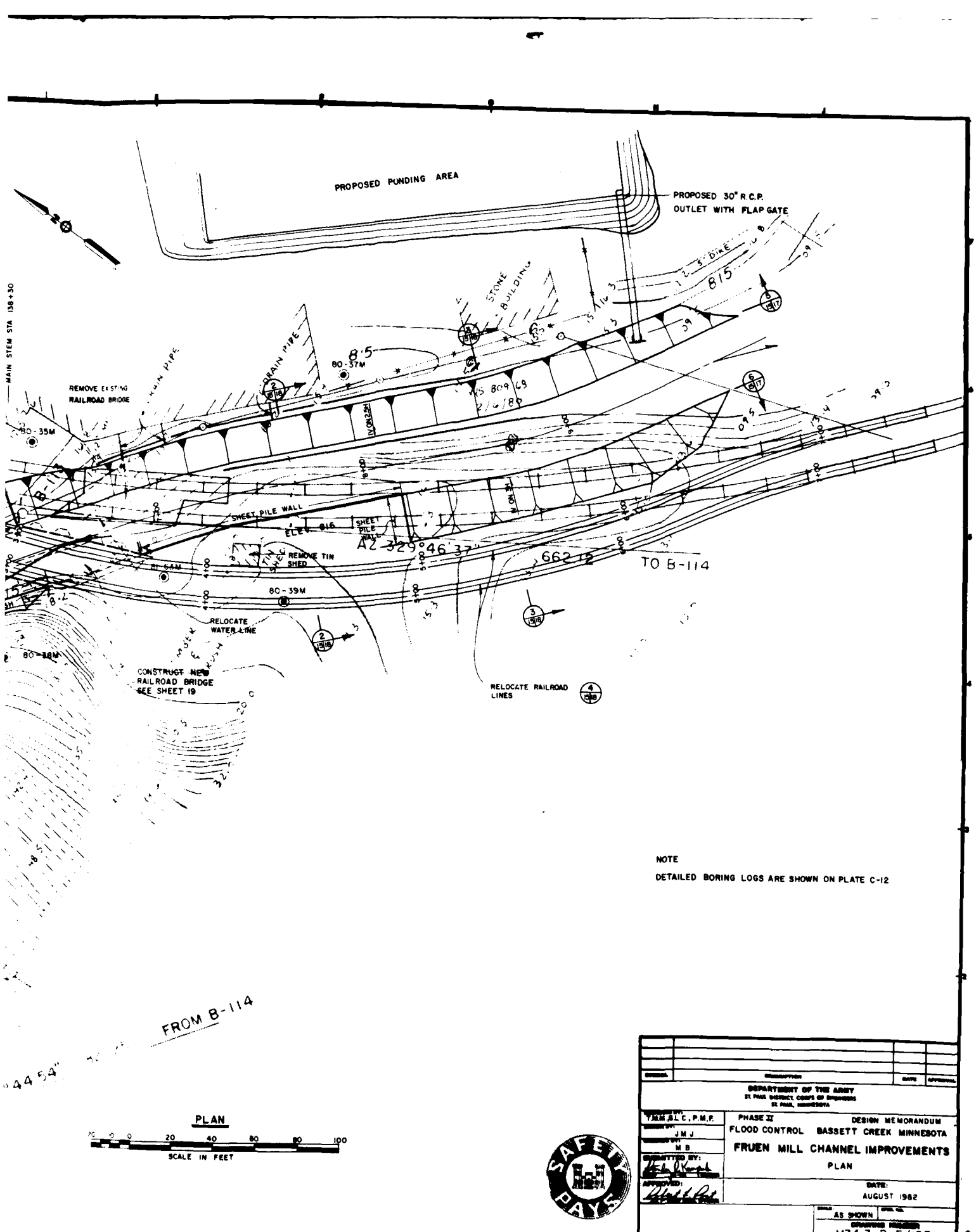
AUGUST 1982

AS SHOWN

M34 3-R-5/194

PLATE 14


MN. & S. R.R.
MILL
CONCRETE
815
MAIN STEM STA 138+30
REMOVE EXISTING RAILROAD BRIDGE
12" DRAIN PIPE
80-35M
SHEET PILE WALL
REMOVE TIN SHED
ELEV. 818
84-62M
80-34M
DROP STRUCTURE
5' WINGWALL
W.S. 813.5
2/6/80
SHEET PILE WALL
1V ON 3H
ELEV. 821
825
1' BENCH
80-38M
RELOCATE WATER LINE
CONSTRUCT NEW RAILROAD BRIDGE SEE SHEET 19
HOPPERS
REMOVE EXISTING STONE DAM
STONE DAM
TIMBER & RUBBLE
80-36M
B-112
AZ 295°25'56"
208.64
820
830
835
840
845
850
855
860
865
870
875
880
TIMBER & BRUSH
ROCK WALL
B-114A
AZ 126°44'54"
82.25
FROM B-114
PLAN
20 10 0 20 40
SCALE IN FEET



NOTE

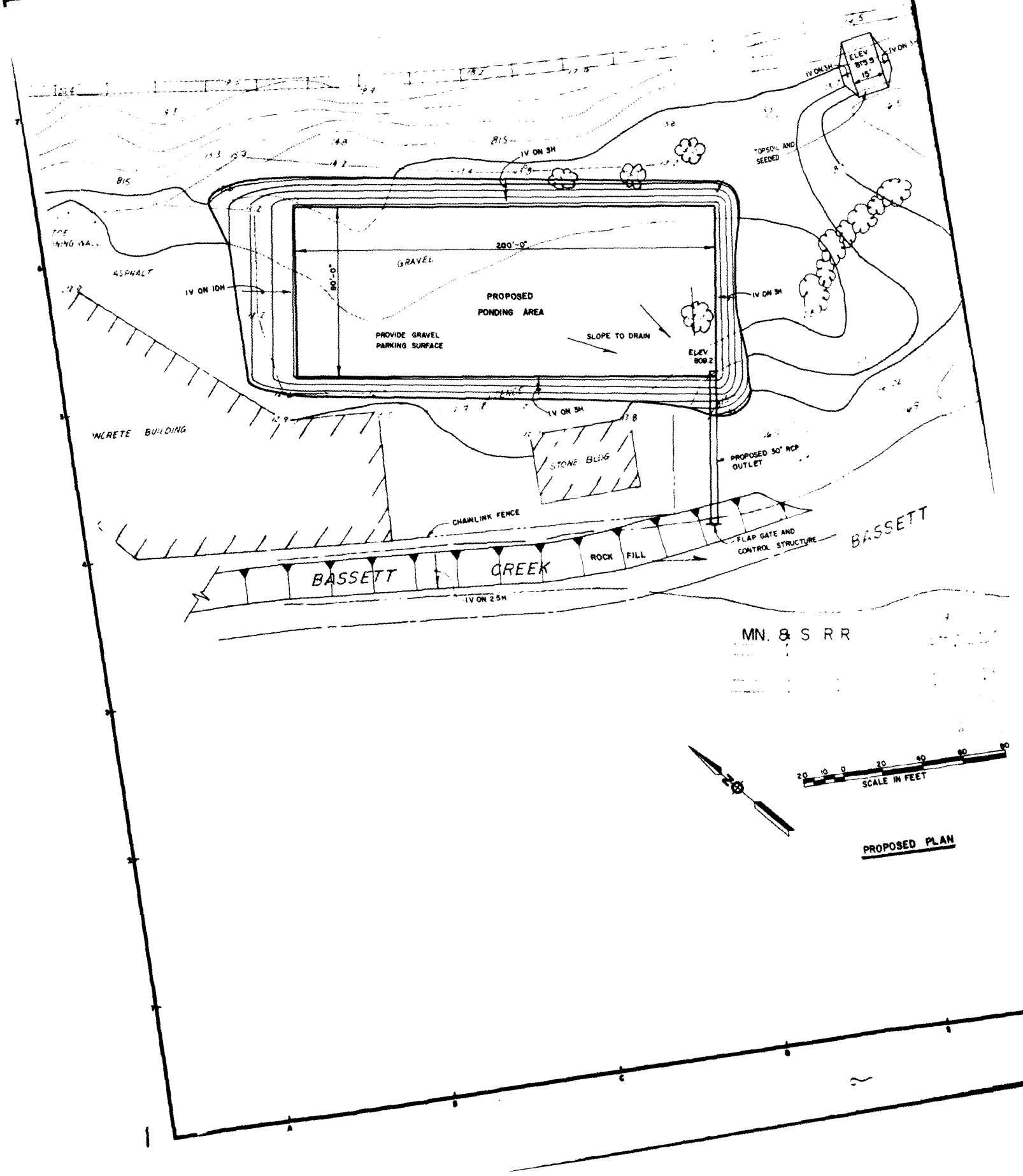
DETAILED BORING LOGS ARE SHOWN ON PLATE C-12

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

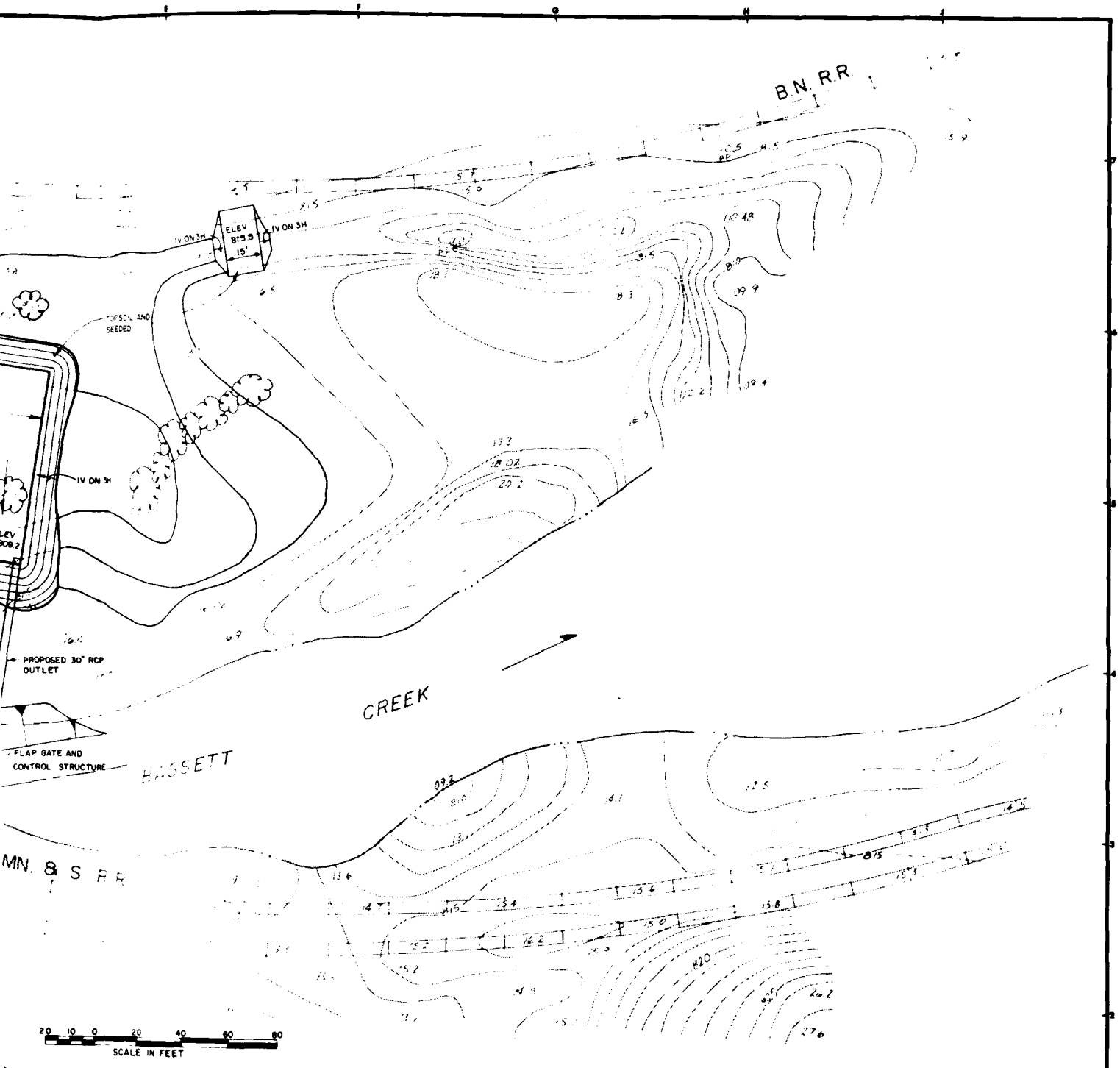
DESIGNED BY:	F.M.M., L.L.C., P.M.P.
DRAWN BY:	J.M.J.
TRACED BY:	M.B.
SUBMITTED BY:	
APPROVED:	

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
FRUEN MILL CHANNEL IMPROVEMENTS
PLAN
DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34 3-R-5/195
SHEET 15 OF 44

PLATE 15



PROPOSED PLAN



PROPOSED PLAN

DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

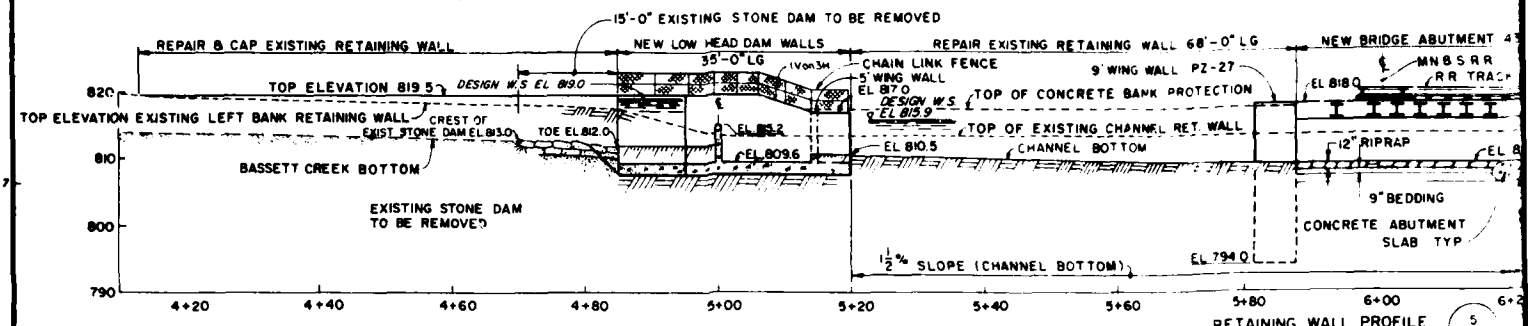
DESIGNED BY: M.D.L. T.M.M.
DRAWN BY: J.G.V.
TRACED BY: M.B.
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
FRUEN MILL PONDING AREA

DATE: AUGUST 1982

SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/196
SHEET 16 OF 44

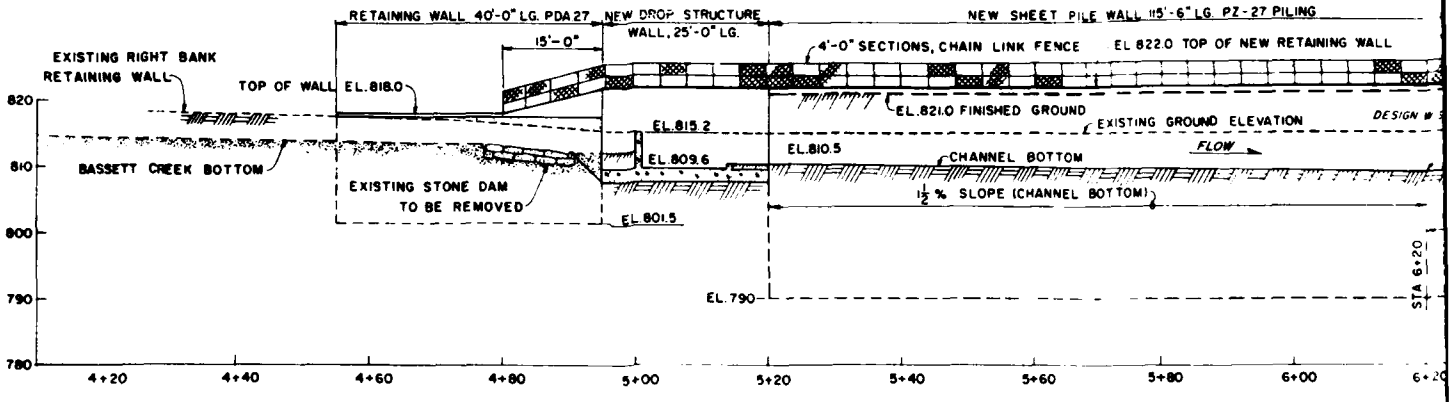
PLATE 16



RETAINING WALL PROFILE (5 / 15 | T)

LEFT BANK

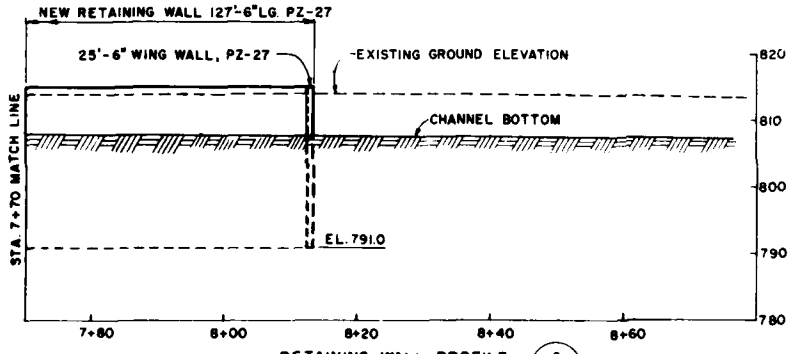
SECTION TAKEN AT 5' FROM FACE OF WALL



RETAINING WALL PROFILE (6 / 8 | 17)

RIGHT BANK

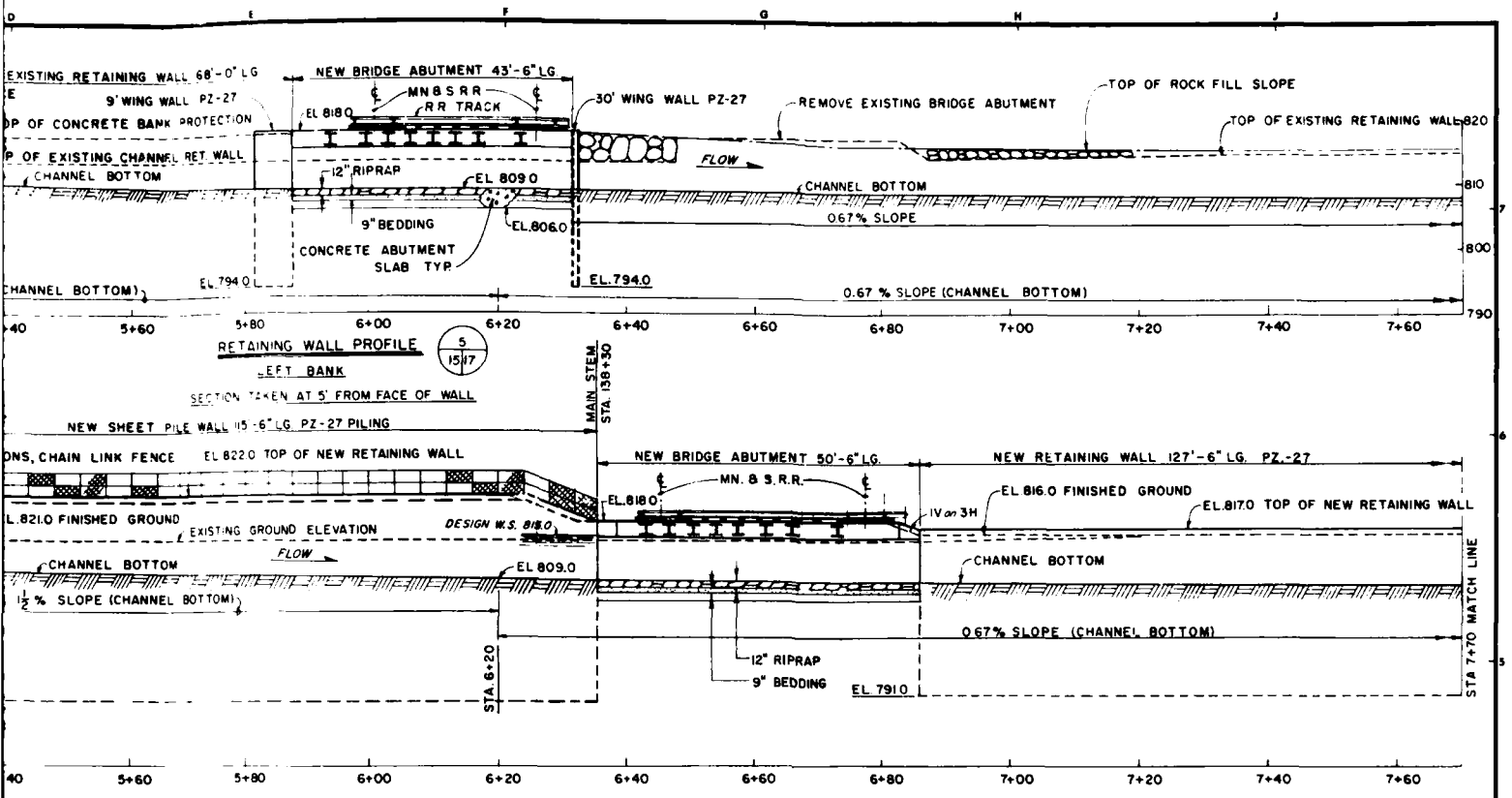
SECTION TAKEN AT 5' FROM FACE OF WALL



RETAINING WALL PROFILE (6 / 15 | 17)

RIGHT BANK

SECTION TAKEN AT 5' FROM FACE OF WALL



RETAINING WALL PROFILE (5 / 15 | 17)

LEFT BANK

SECTION TAKEN AT 5' FROM FACE OF WALL

RETAINING WALL PROFILE (6 / 8 | 17)

RIGHT BANK

SECTION TAKEN AT 5' FROM FACE OF WALL

NOTE:
1. REMOVE EXISTING BRIDGE ABUTMENTS

10 5 0 10 20
SCALE IN FEET

SAFETY PAYS

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY: TMM & GLC
DRAWN BY: K.M.
CHECKED BY: G.L.C.
SUBMITTED BY: [signature] CHIEF, DESIGN BRANCH
APPROVED: [signature] CHIEF, ENGR. DIV.

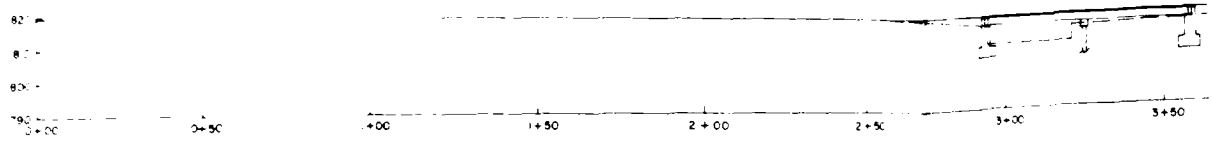
PHASE II
FLOOD CONTROL

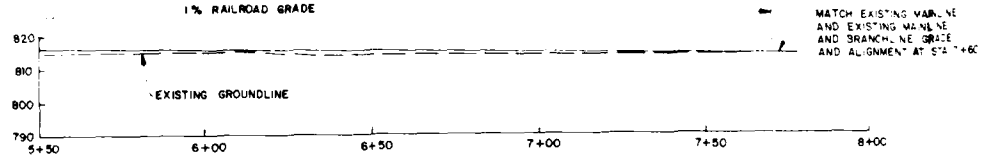
DESIGN MEMORANDUM
BASSETT CREEK MINNESOTA

FRUEN MILL AREA
RETAINING WALL DETAILS

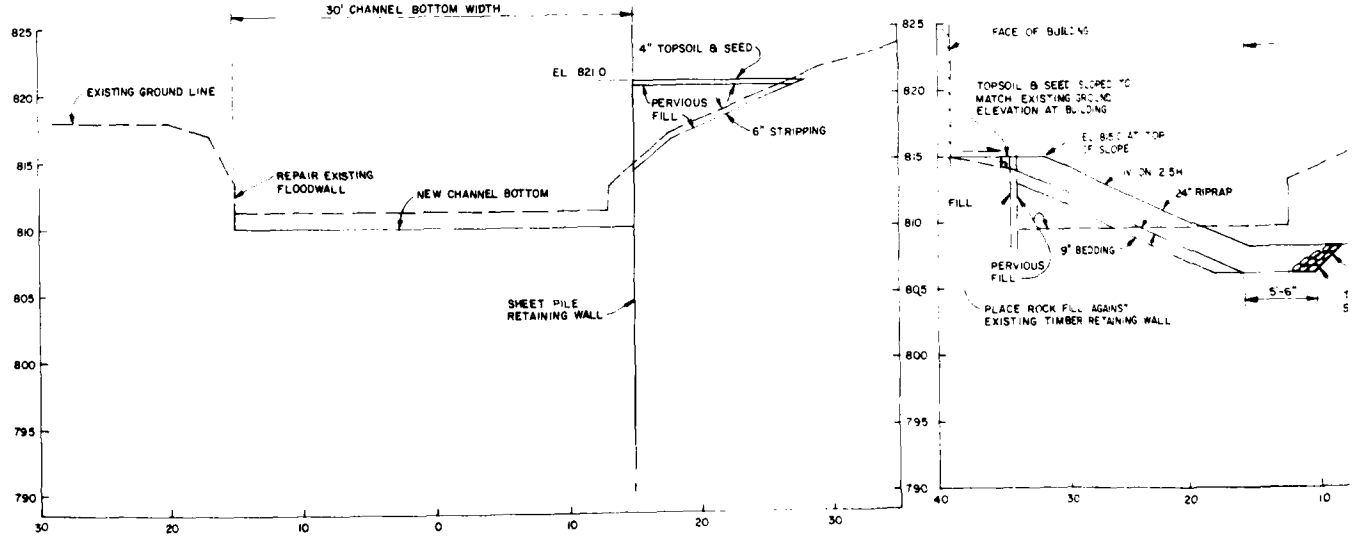
DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/197
SHEET 17 OF 44

PLATE 17

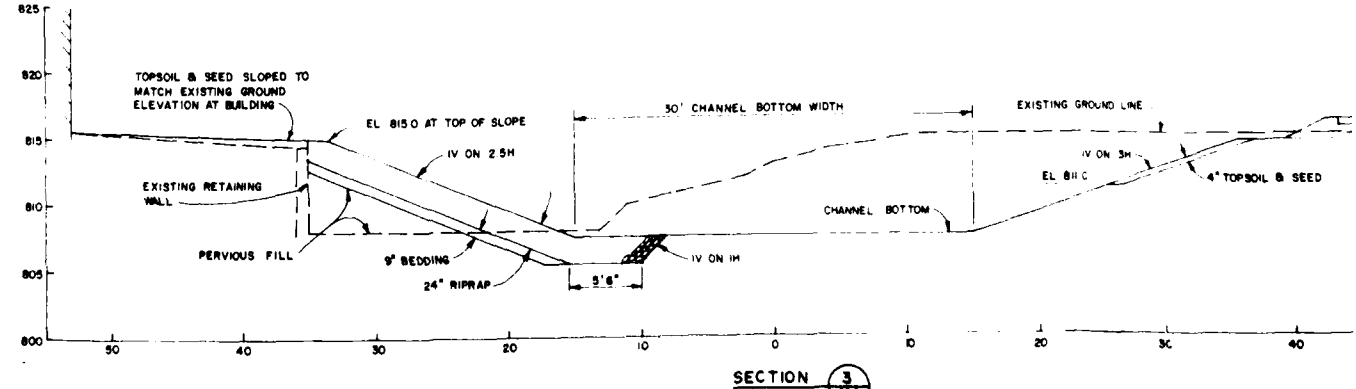




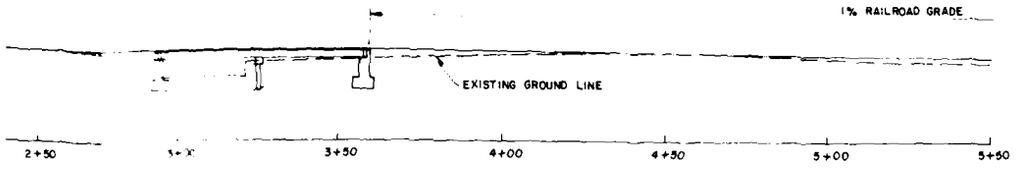
RAILROAD MAINLINE AND BRANCHLINE PROFILE (4 | 15 | 18)
SCALE 1" = 20'



SECTION (1 | 15 | 18)
SCALE 1" = 5'



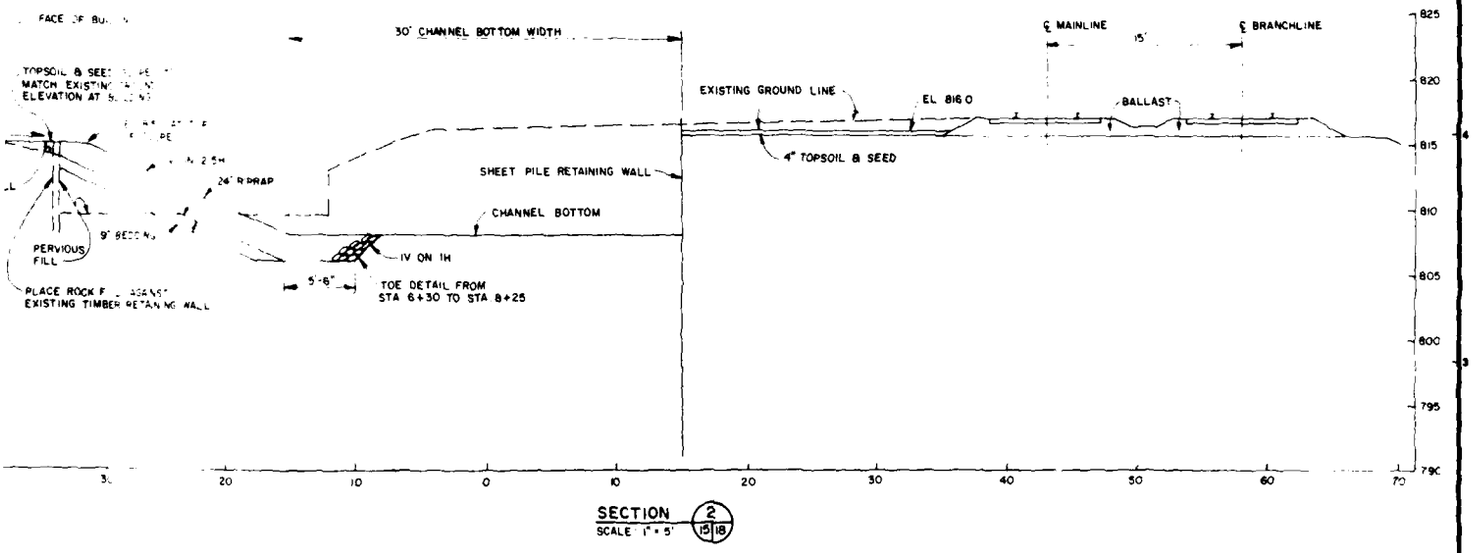
SECTION (3 | 15 | 18)
SCALE 1" = 5'



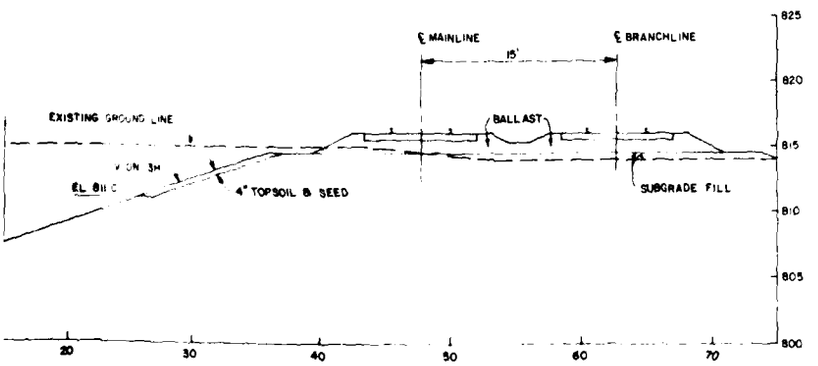
TCH EXISTING MAINLINE
) EXISTING MAINLINE
) BRANCHLINE GRADE
) ALIGNMENT AT STA 7+65

8+00

D MAINLINE AND BRANCHLINE PROFILE (4 / 15 18)
20'



SECTION (2 / 15 18)
SCALE 1" = 5'



SAFETY PAYS

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

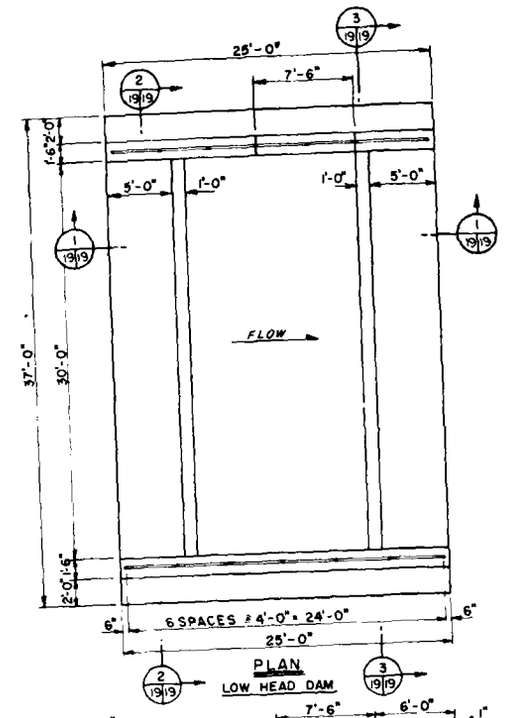
DESIGNED BY	T M
DRAWN BY	J I V L
CHECKED BY	M B
SUBMITTED BY	
APPROVED	

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL — BASSETT CREEK MINNESOTA

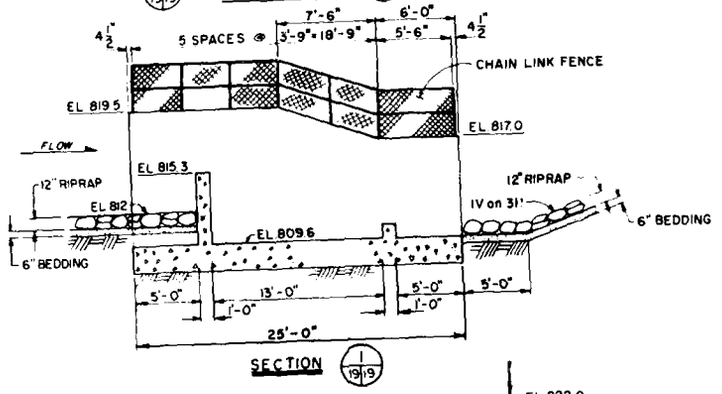
FRUEN MILL AREA
RETAINING WALL AND SECTION

DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34 3-R-5/198
SHEET 18 OF 44

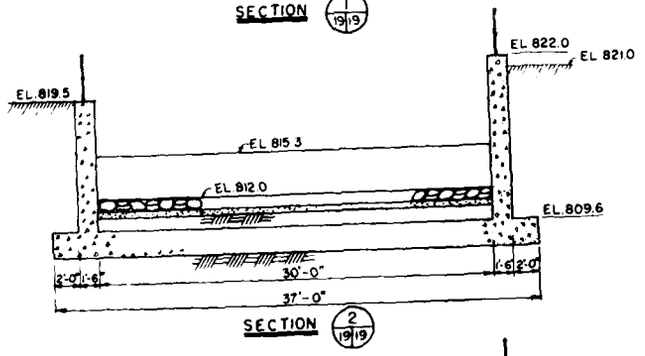
PLATE 18



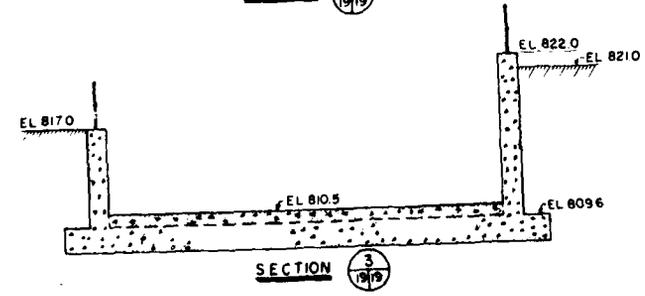
PLAN
LOW HEAD DAM



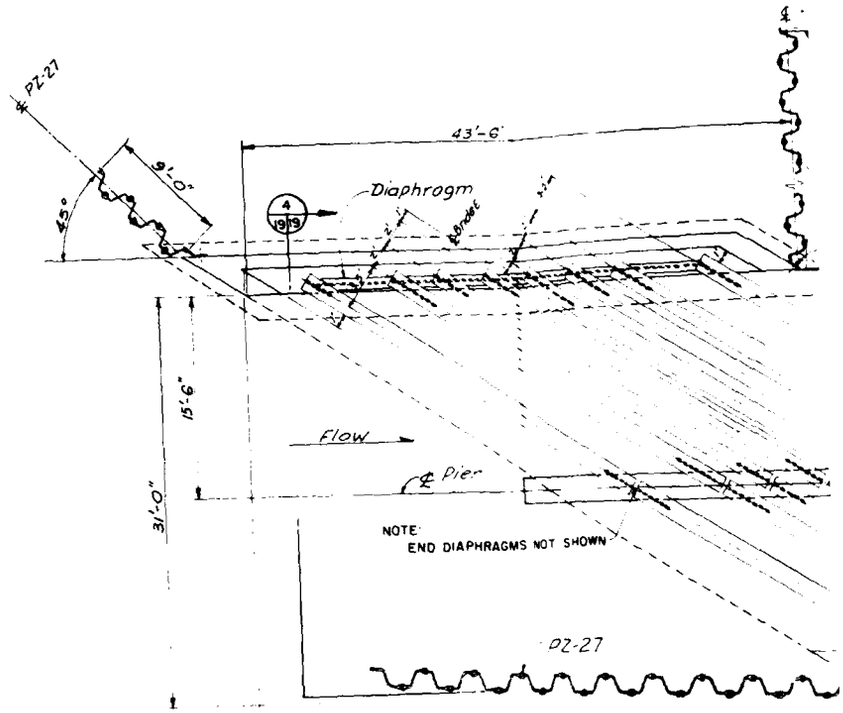
SECTION

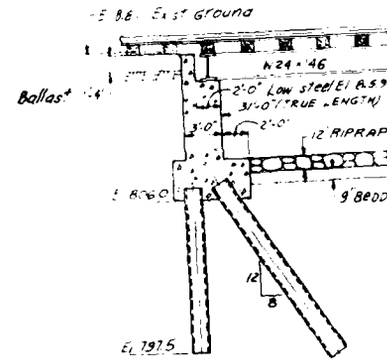


SECTION

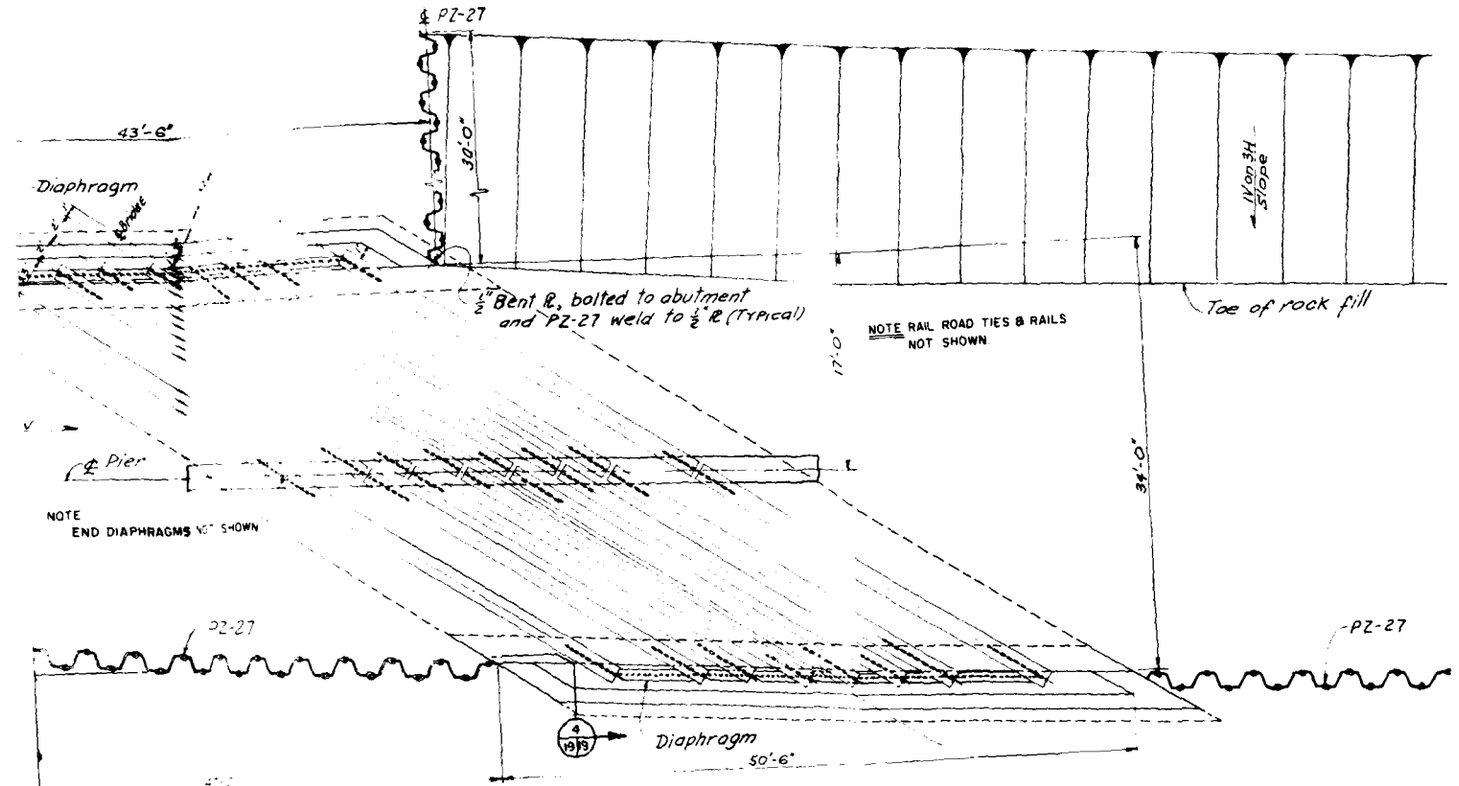


SECTION



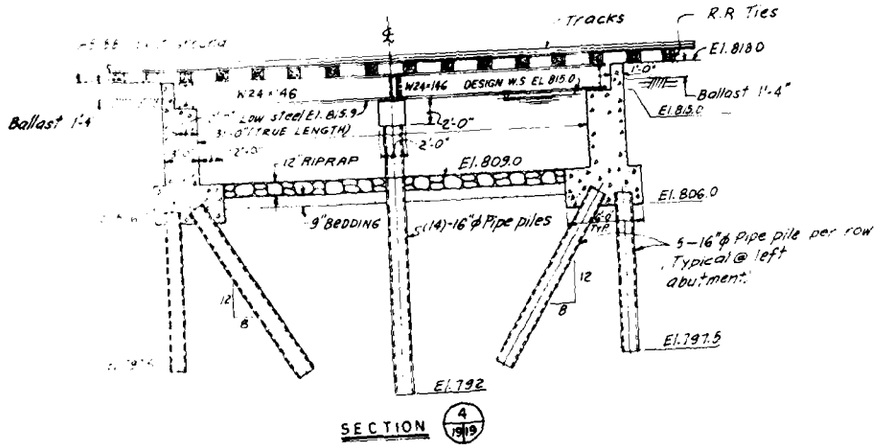


SCALE IN FEET



PLAN

MN & S RAILROAD BRIDGE



SECTION

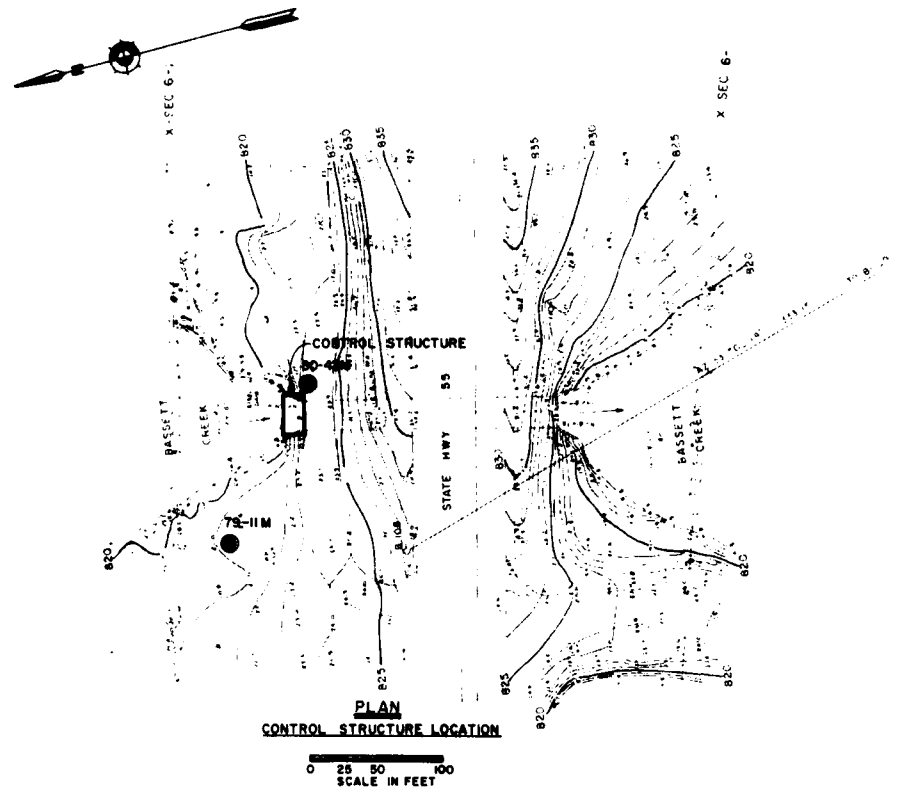
SCALE IN FEET

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL MINNESOTA

DESIGNED BY LMBBGLC
DRAWN BY
CHECKED BY GLC
SUBMITTED BY
CHIEF DESIGN BRANCH
APPROVED
CHIEF ENGR DIV

PHASE II
FLOOD CONTROL
DESIGN MEMORANDUM
BASSETT CREEK MINNESOTA
FRUEN MILL AREA
RAILROAD BRIDGE & DROP STRUCTURE DETAILS
DATE
AUGUST 1982
AS SHOWN
DRAWING NUMBER
M34 3-R-5/199
SHEET 19 OF 44

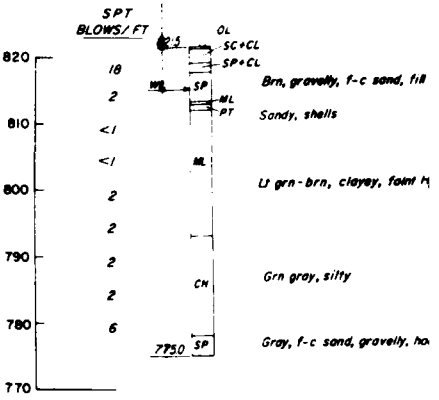
PLATE 19

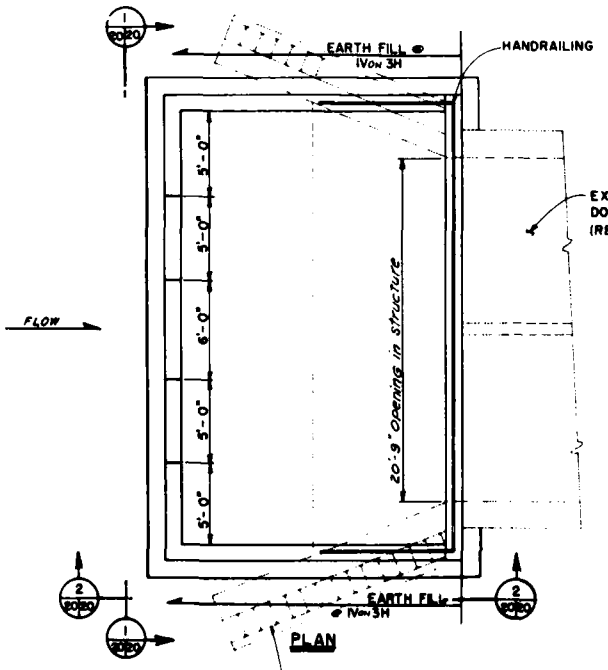


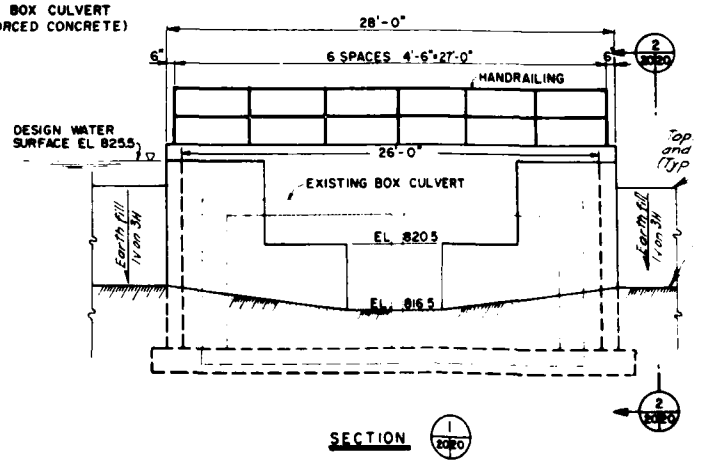
PLAN
CONTROL STRUCTURE LOCATION

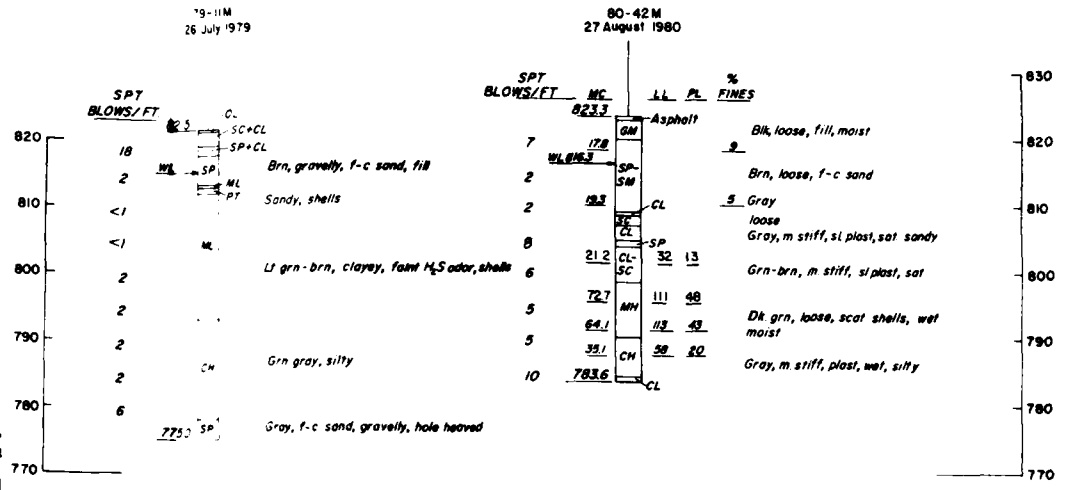
0 25 50 100
SCALE IN FEET

79-11M
26 July 1979



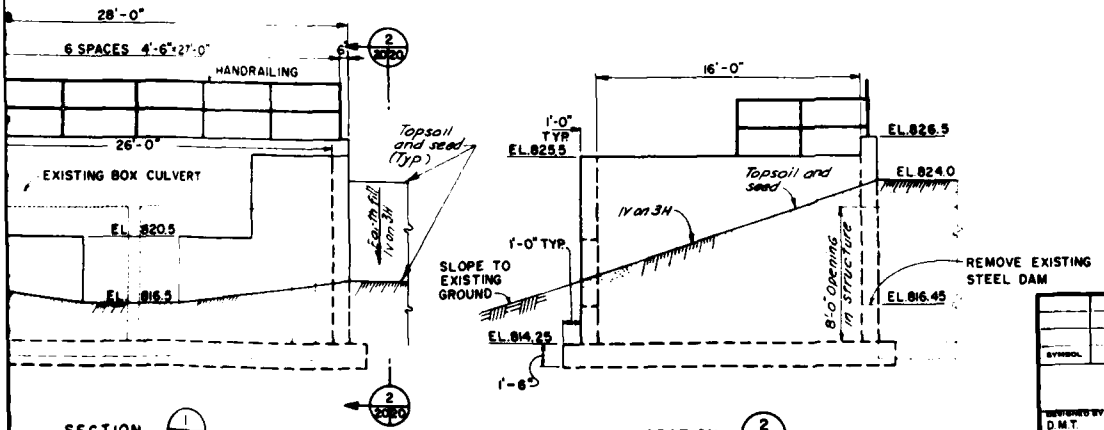






NOTE:

BORING LEGEND SHOWN ON PLATE C-11



DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

DESIGNED BY D.M.T
DRAWN BY
CHECKED BY G.L.C
SUBMITTED BY
APPROVED:

PHASE I DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

HIGHWAY 55 CONTROL STRUCTURE
PLAN & SECTIONS

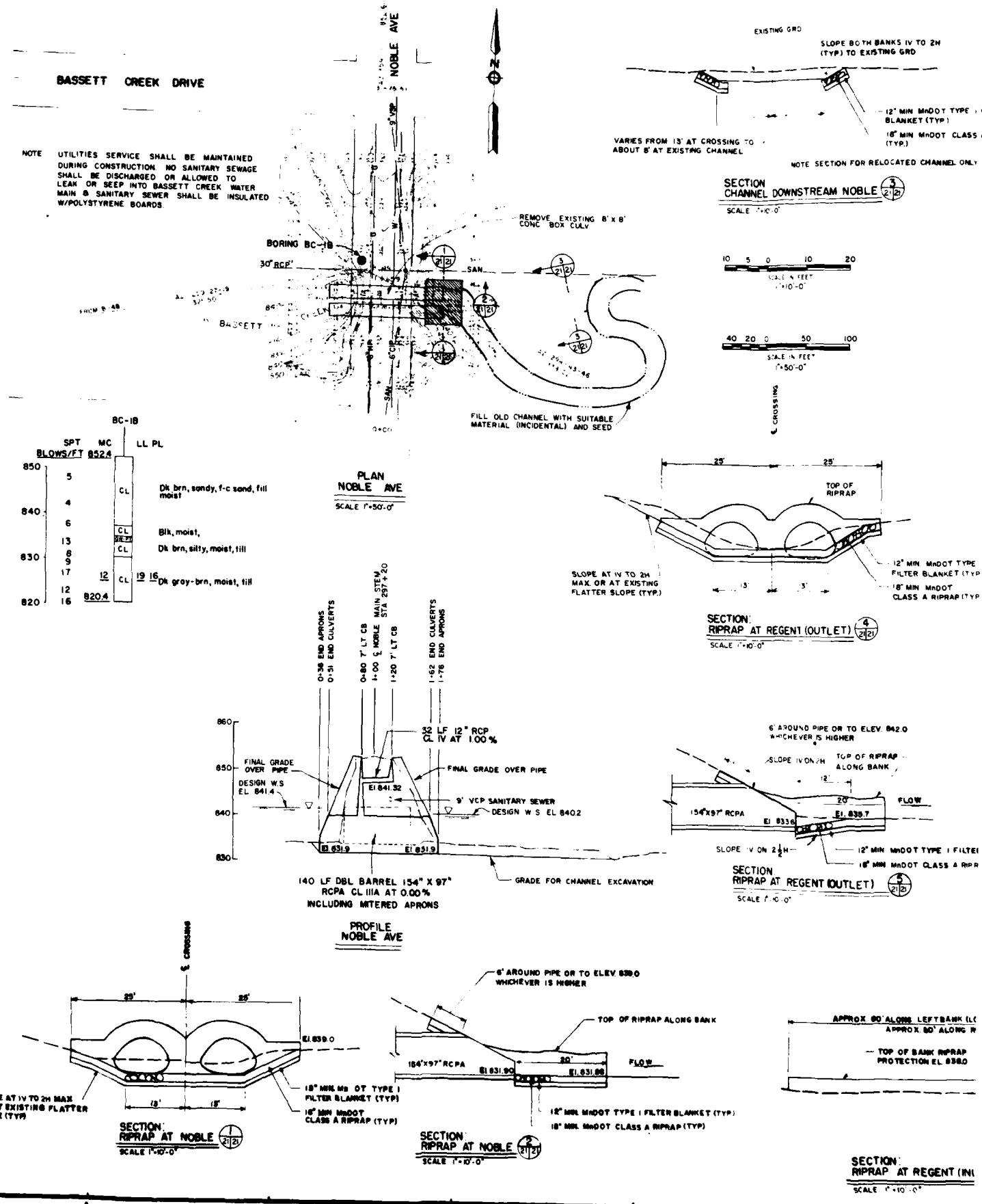
DATE: AUGUST 1982

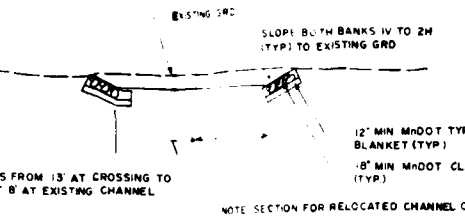
AS SHOWN

DRAWING NUMBER
M34.3-R-5/201
SHEET 20 OF 44

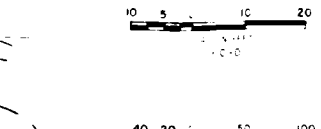
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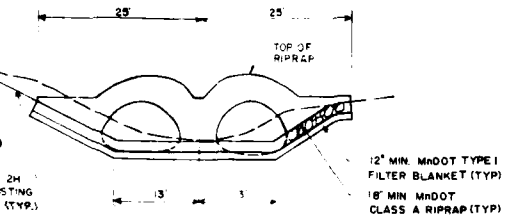
PLATE 20


BASSETT CREEK DRIVE
NOBLE AVE
NOTE UTILITIES SERVICE SHALL BE MAINTAINED DURING CONSTRUCTION. NO SANITARY SEWAGE SHALL BE DISCHARGED OR ALLOWED TO LEAK OR SEEP INTO BASSETT CREEK. WATER MAIN & SANITARY SEWER SHALL BE INSULATED W/POLYSTYRENE BOARDS.
BORING BC-1B
30" RCP
REMOVE EXISTING 8' x 8' CONC BOX CULV
FILL OLD CHANNEL WITH SUITABLE MATERIAL (INCIDENTAL) AND SEED
BC-1B
SPT BLOWS/FT
MC
LL PL
Dk brn, sandy, f-c sand, fill moist
Blk, moist,
Dk brn, silty, moist, till
Dk gray-brn, moist, till
PLAN NOBLE AVE
SCALE 1"=50'-0"
EXISTING GRD
SLOPE BOTH BANKS IV TO 2H (TYP.) TO EXISTING GRD
12" MIN MnDOT TYPE I FILTER BLANKET (TYP.)
18" MIN MnDOT CLASS A RIPRAP (TYP.)
VARIES FROM 13' AT CROSSING TO ABOUT 8' AT EXISTING CHANNEL
NOTE SECTION FOR RELOCATED CHANNEL ONLY
SECTION CHANNEL DOWNSTREAM NOBLE 3/21/21
SCALE 1"=10'-0"
10 5 0 10 20
SCALE IN FEET 1"=10'-0"
40 20 0 50 100
SCALE IN FEET 1"=50'-0"
℄ CROSSING
25' 25'
TOP OF RIPRAP
SLOPE AT IV TO 2H MAX. OR AT EXISTING FLATTER SLOPE (TYP.)
12" MIN MnDOT TYPE I FILTER BLANKET (TYP.)
18" MIN MnDOT CLASS A RIPRAP (TYP.)
SECTION RIPRAP AT REGENT (OUTLET) 4/21/21
SCALE 1"=10'-0"
0+36 END APRONS
0+51 END CULVERTS
0+80 7' LT CB
1+00 ℄ NOBLE MAIN STEM STA 297+20
1+20 7' LT CB
1+62 END CULVERTS
1+76 END APRONS
152 LF 12" RCP CL IV AT 1.00 %
FINAL GRADE OVER PIPE
DESIGN W.S EL 841.4
EL 841.32
9' VCP SANITARY SEWER
DESIGN W.S EL 840.2
EL 831.9
EL 831.9
GRADE FOR CHANNEL EXCAVATION
140 LF DBL BARREL 154" X 97" RCPA CL IIIA AT 0.00% INCLUDING MITERED APRONS
PROFILE NOBLE AVE
6' AROUND PIPE OR TO ELEV. 842.0 WHICHEVER IS HIGHER
SLOPE IV ON 2H
TOP OF RIPRAP ALONG BANK
154"X97" RCPA
EL 833.6
EL. 838.7
FLOW
SLOPE IV ON 2½H
12" MIN MnDOT TYPE I FILTER
18" MIN MnDOT CLASS A RIPR
SECTION RIPRAP AT REGENT (OUTLET) 5/21/21
SCALE 1"=10'-0"
℄ CROSSING
25' 25'
EL 839.0
SLOPE AT IV TO 2H MAX OR AT EXISTING FLATTER SLOPE (TYP)
18' 18'
18" MIN MnDOT TYPE I FILTER BLANKET (TYP)
18" MIN MnDOT CLASS A RIPRAP (TYP)
SECTION RIPRAP AT NOBLE 1/21/21
SCALE 1"=10'-0"
6' AROUND PIPE OR TO ELEV 839.0 WHICHEVER IS HIGHER
TOP OF RIPRAP ALONG BANK
184"X97" RCPA
EL 831.90
EL. 831.88
FLOW
12" MIN MnDOT TYPE I FILTER BLANKET (TYP)
18" MIN MnDOT CLASS A RIPRAP (TYP)
SECTION RIPRAP AT NOBLE 2/21/21
SCALE 1"=10'-0"
APPROX 80' ALONG LEFT BANK
APPROX 80' ALONG R
TOP OF BANK RIPRAP PROTECTION EL 838.0
SECTION RIPRAP AT REGENT (IN
SCALE 1"=10'-0"

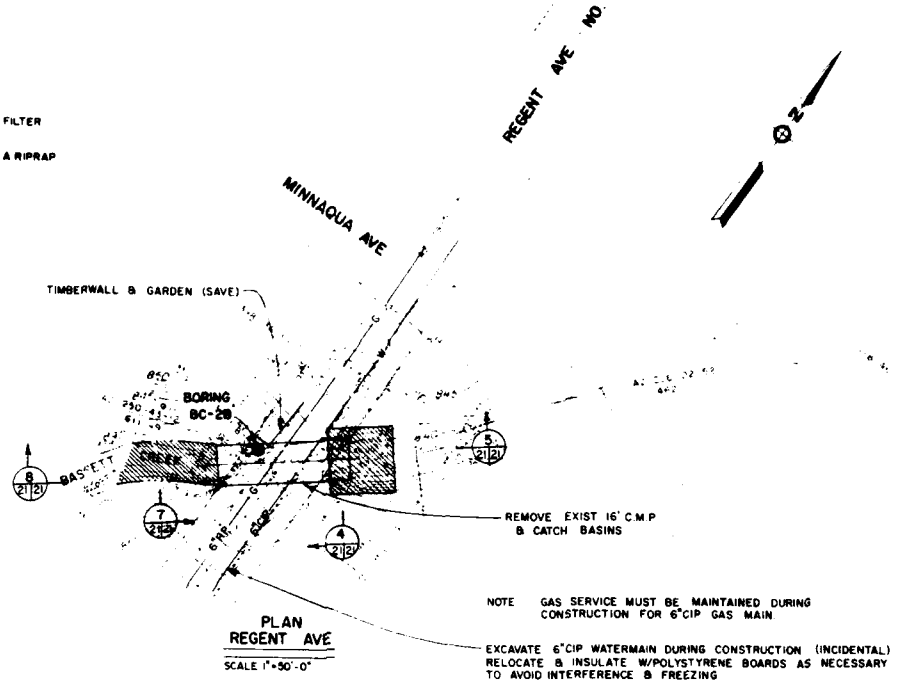


SECTION
CHANNEL DOWNSTREAM NOBLE (3 21 21)
SCALE 1"=

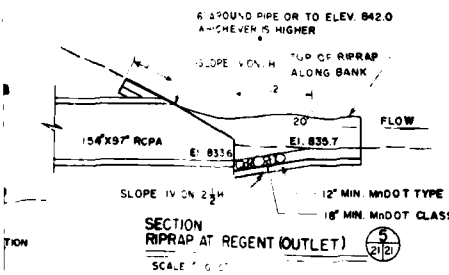




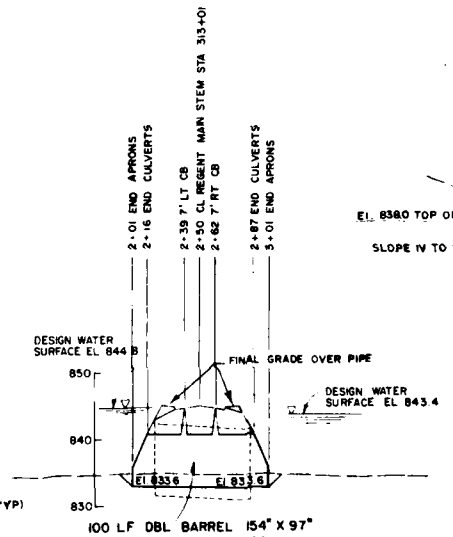
SECTION:
RIPRAP AT REGENT (OUTLET) (4 21 21)
SCALE 1"=10'-0"



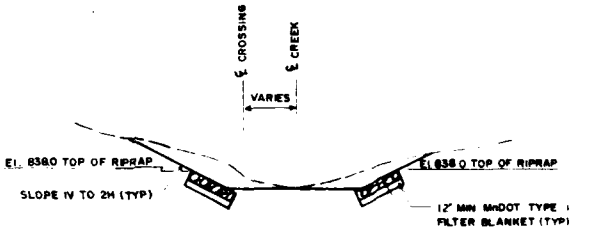
PLAN
REGENT AVE
SCALE 1"=50'-0"



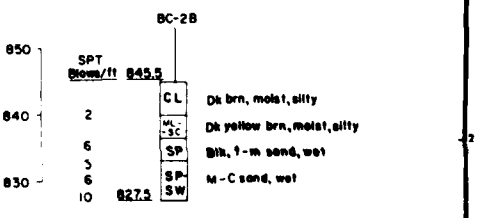
SECTION
RIPRAP AT REGENT (OUTLET) (5 21 21)
SCALE 1"=

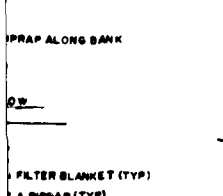


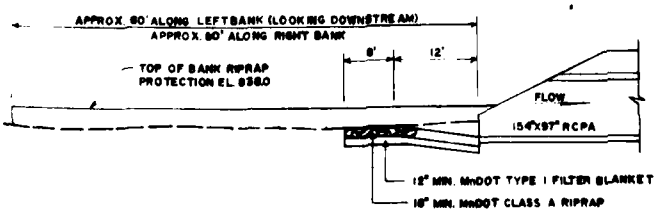
PROFILE
REGENT AVE



SECTION
RIPRAP AT REGENT (INLET) (7 21 21)
SCALE 1"=10'-0"







SECTION:
RIPRAP AT REGENT (INLET) (6 21 21)
SCALE 1"=10'-0"

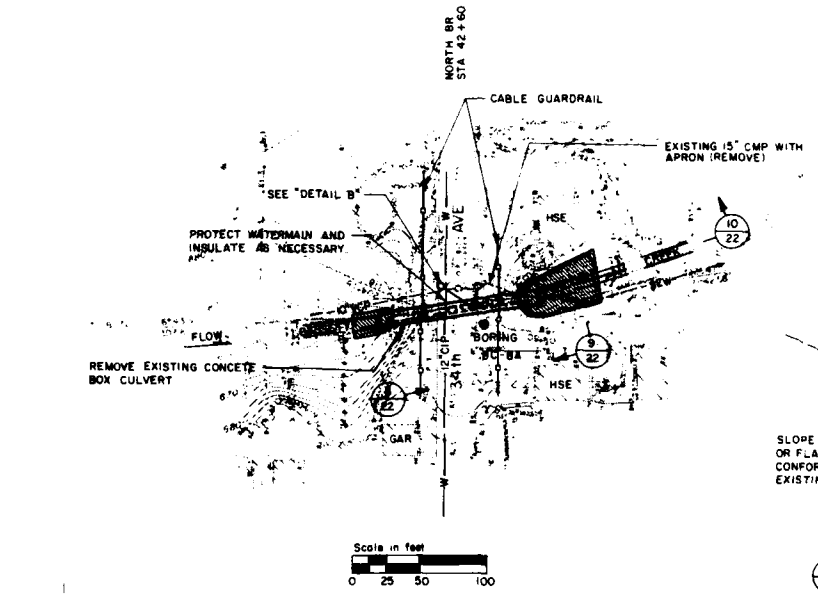
SYMBOL	DESCRIPTION	DATE	APPROVAL

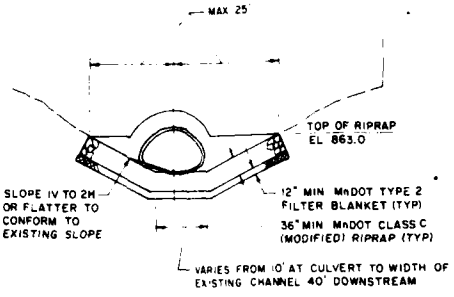
DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

DESIGNED BY: BARR ENG. CO.	PHASE II	DESIGN MEMORANDUM
DRAWN BY: MJZ, RWC	FLOOD CONTROL	BASSETT CREEK MINNESOTA
CHECKED BY: L A R	CULVERT REPLACEMENT AT NOBLE & REGENT AVES	
SUBMITTED BY:	DATE: AUGUST 1982	
APPROVED:	SCALE: AS SHOWN	

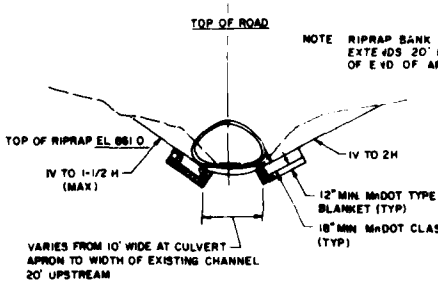
DRAWING NUMBER
M34.3-R-5/202
SHEET 21 OF 44

PLATE 21

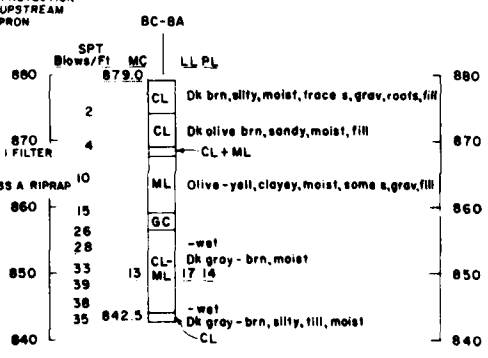


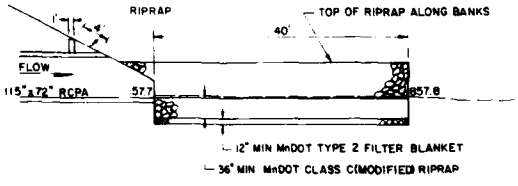


9/22 SECTION: RIPRAP AT 34TH (OUTLET)
SCALE 1" = 10'

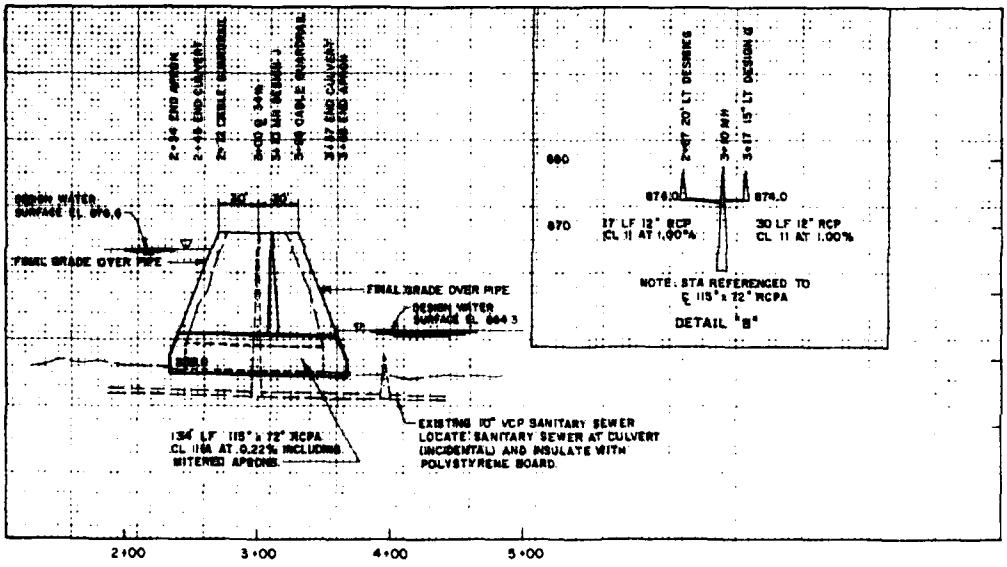


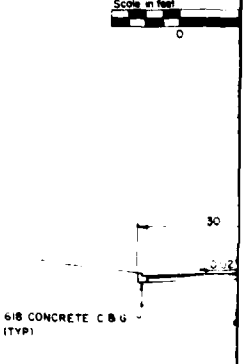
8/22 SECTION: RIPRAP AT 34TH (INLET)
SCALE 1" = 10'





10/22 SECTION: RIPRAP AT 34TH (OUTLET)
SCALE 1" = 10'

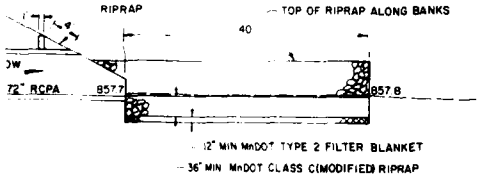




1/22 SECTION: C...

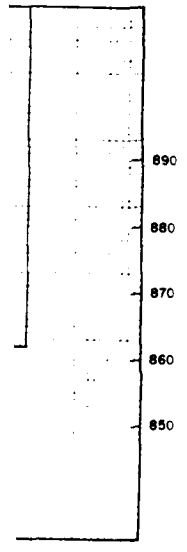
9/22 SECTION: RIPRAP AT 34TH (OUTLET)

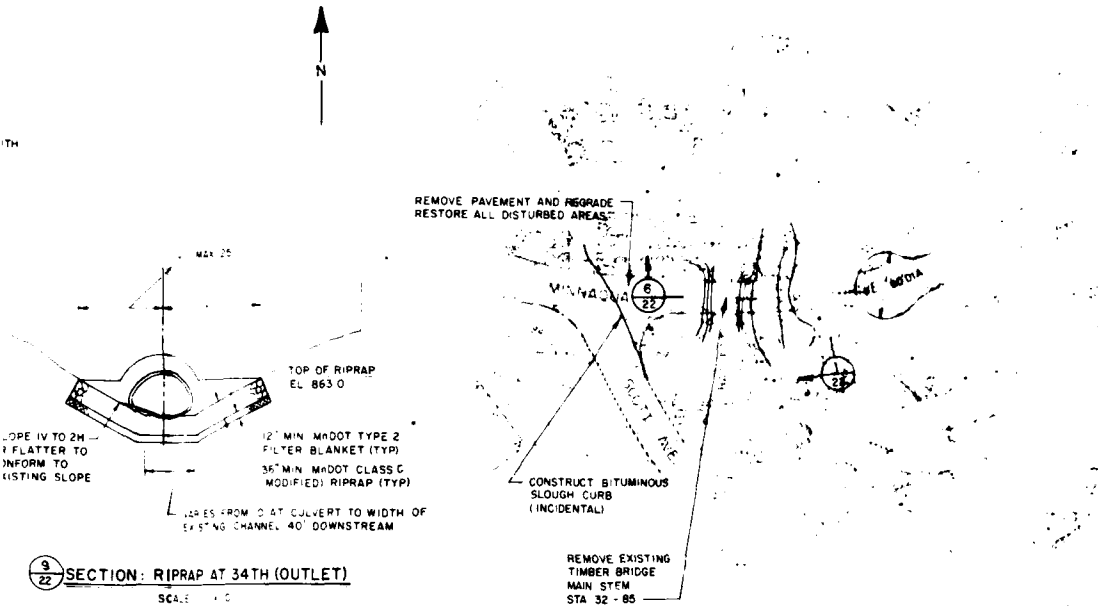
SCALE 1" = 10'

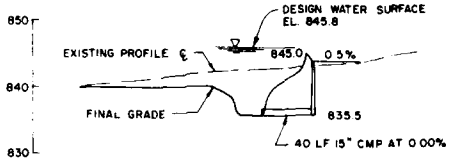


10/22 SECTION: RIPRAP AT 34TH (OUTLET)

SCALE 1" = 10'



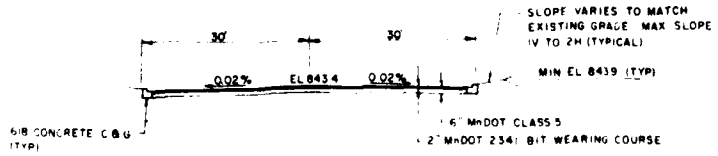




6/22 SECTION: MINNAQUA DRIVE CUL-DE-SAC STORM SEWER

1" = 10' VERTICAL
1" = 50' HORIZONTAL





1/22 SECTION: CUL-DE-SAC PAVEMENT SECTION

SCALE 1" = 10'

DIVISION	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL MINNESOTA

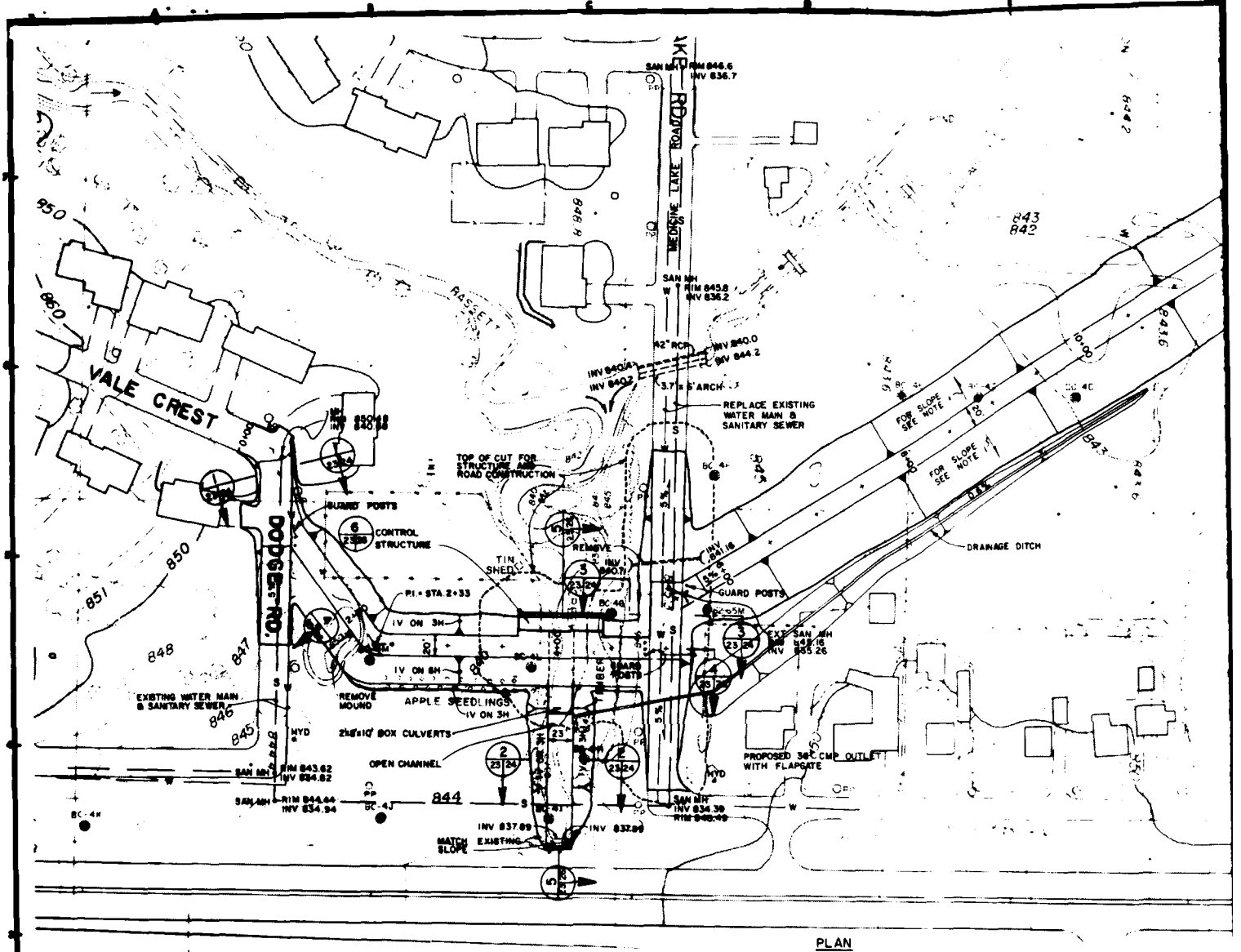
DESIGNED BY BARR ENG CO.
DRAWN BY J M J
CHECKED BY L A R
SUBMITTED BY
CHIEF
APPROVED
CHIEF

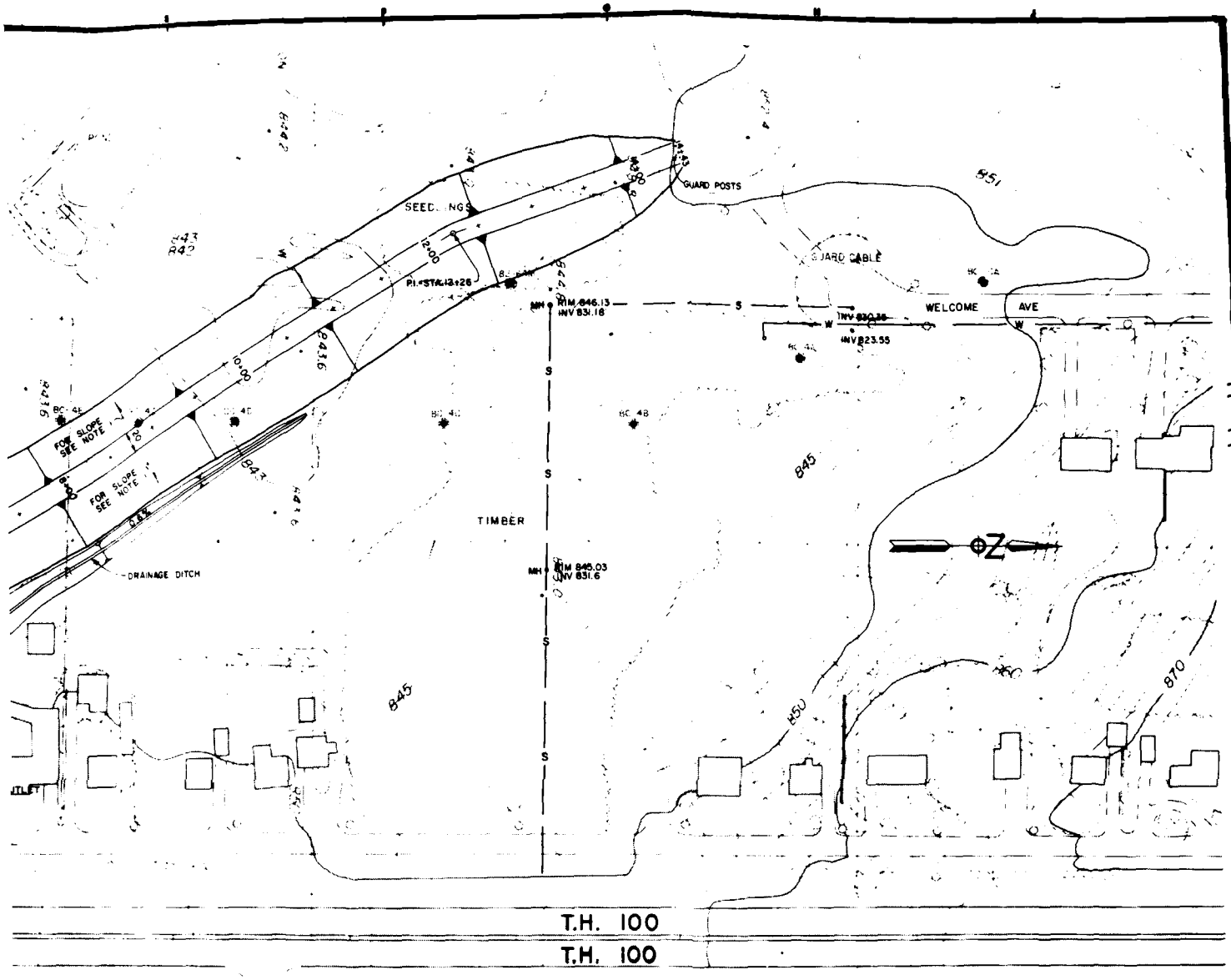
PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
BRIDGE REMOVAL @ MINNAQUA AVE
AND CULVERT REPLACEMENT
@ 34TH AVE

DATE
AUGUST 1982
SCALE AS SHOWN
DRAWING NUMBER
M34 3-R-5/203
SHEET 22 OF 44

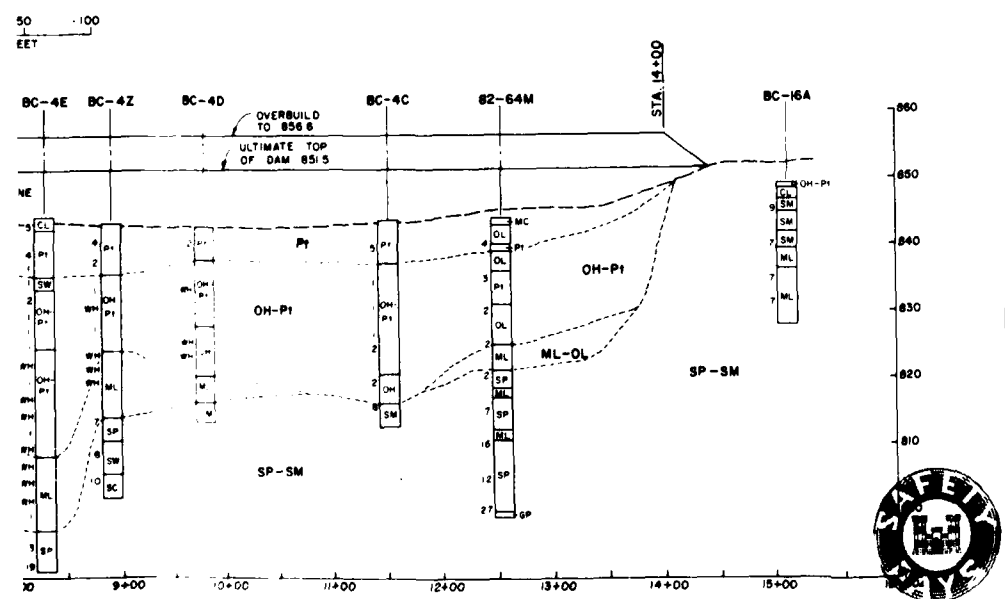
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PLATE 22


VALE CREST
DODGE RD.
MEDICINE LAKE ROAD
RASSET
SAN MH RIM 846.6 INV 836.7
SAN MH RIM 845.8 INV 836.2
42" RCP INV 840.0 INV 844.2
3.7'x 5' ARCH
INV 840.4 INV 840.2
REPLACE EXISTING WATER MAIN & SANITARY SEWER
TOP OF CUT FOR STRUCTURE AND ROAD CONSTRUCTION
GUARD POSTS
CONTROL STRUCTURE
TIN SHED
REMOVE
INV 840.7
P.I. = STA. 2+33
1V ON 3H
1V ON 6H
1V ON 3H
APPLE SEEDLINGS
REMOVE MOUND
EXISTING WATER MAIN & SANITARY SEWER
2'x5'x10' BOX CULVERTS
OPEN CHANNEL
SAN MH RIM 843.62 INV 834.82
SAN MH RIM 844.44 INV 834.94
INV 837.89
INV 837.89
MATCH EXISTING SLOPE
DRAINAGE DITCH
SLOPE NOTE 1
SLOPE NOTE 1
EXIST SAN MH RIM 848.16 INV 835.26
PROPOSED 36" CMP OUTLET WITH FLAPGATE
HYD
BC-4K
BC-4J
BC-4L
BC-4M
BC-4G
BC-4F
BC-4E
BC-4Z
BC-4D
BC-4C
PLAN
50 0 50 100
SCALE IN FEET
STA. 1+00
STA. 3+50
STA. 6+00
ULTIMATE TOP OF DAM 851.0
OVERBUILD TO 851.3
851.5
OVERBUILD TO 856.6
ULTIMATE TOP OF DAM 851.5
EXISTING GROUND LINE
SP-SM
SP-SW
OH-Pt
ML-OL
Pt
STRUCTURE AND ROAD EXCAVATION
0+00 1+00 2+00 3+00 4+00 5+00 6+00 7+00 8+00 9+00 10+00 11+00 12+00
PROFILE ALONG CENTERLINE OF EMBANKMENT



T.H. 100



RLINE OF EMBANKMENT

NOTES:

1. Due to large amounts of settlement upstream slope will change from 1 V on 3 H at End to Construction to 1 V on 4.75 H after 100 years of secondary consolidation. Likewise the downstream slop will vary from 1 V on 3.7 H to 1 V on 6 H.
2. The embankment from Sta. 6 + 00 to 14 + 00 will be constructed in three stages over a two year period. (See Sheet 25)
3. The numbers to the left of the boring columns are standard blows per foot.
4. Detailed Boring logs are shown on Sheet C-13
5. For typical sections See Sheet 25

DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

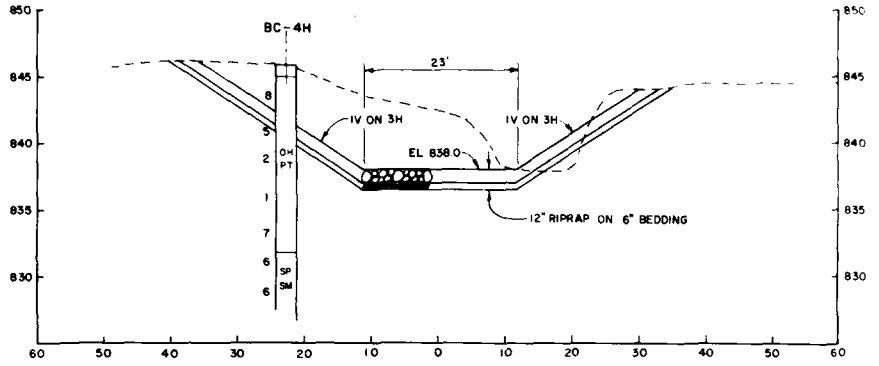
DESIGNED BY: D.W.R.
DRAWN BY: W.J.V.
TRACED BY: M.B.
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
HIGHWAY 100 EMBANKMENT
PLAN AND PROFILE

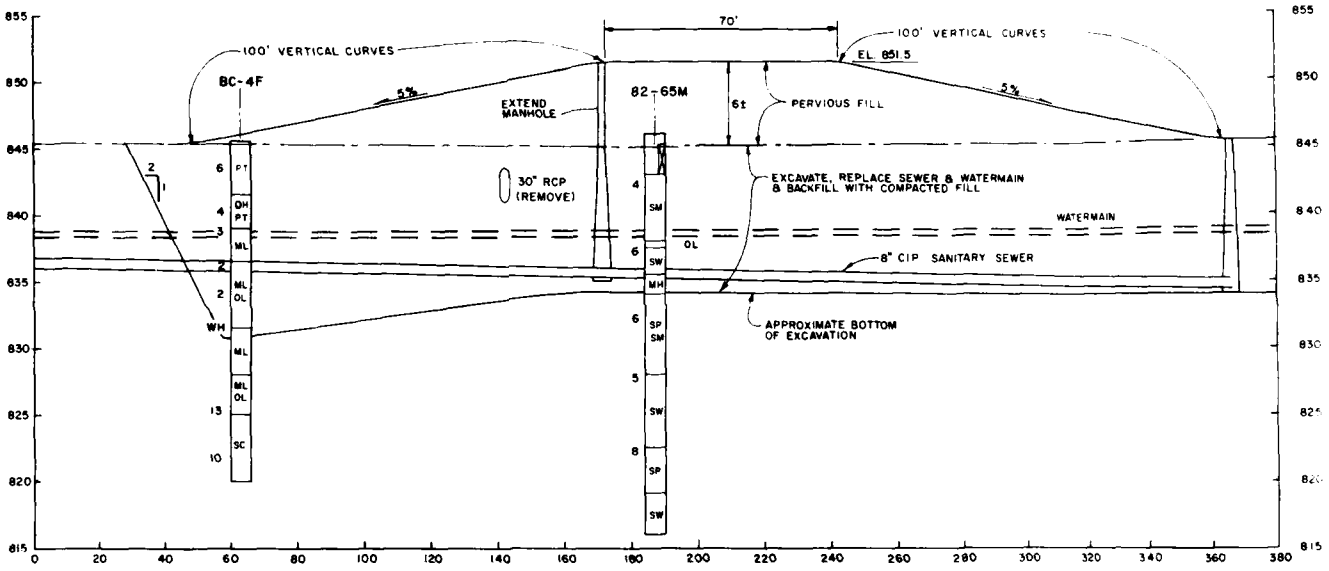
DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/204
SHEET 23 OF 44

PLATE 23

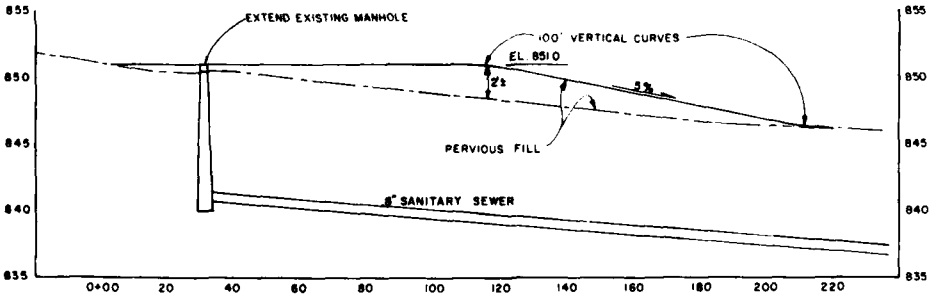
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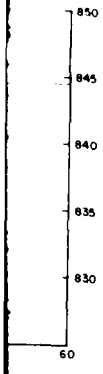
TYPICAL SECTION (2 / 23&24)

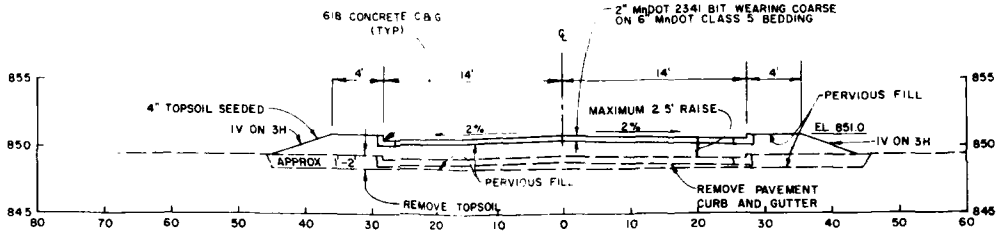


PROFILE ALONG MEDICINE LAKE ROAD CENTERLINE

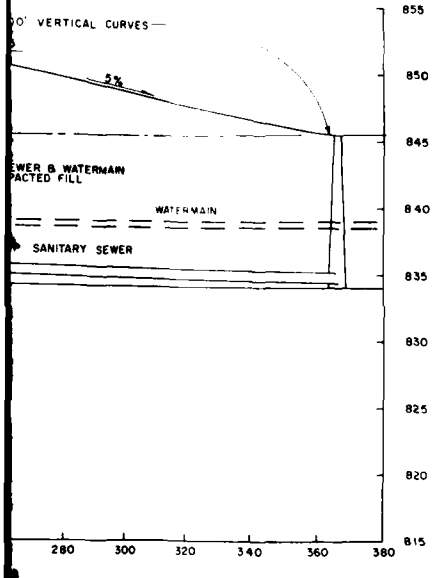


PROFILE ALONG DODGE ROAD CENTERLINE

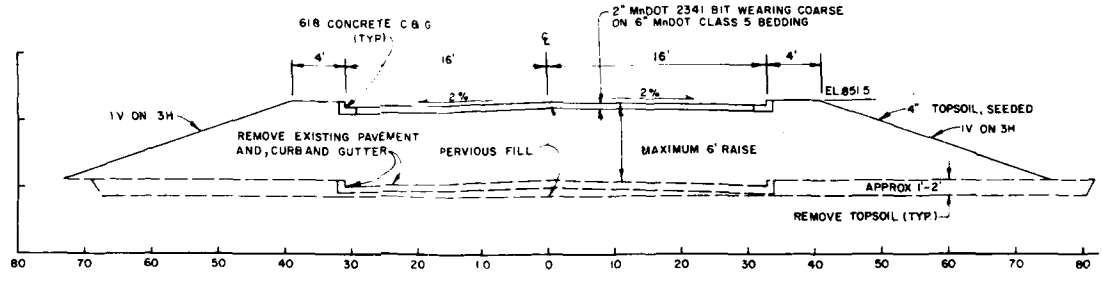




TYPICAL SECTION
DODGE ROAD RAISE
1 / 23 24



LINE



TYPICAL SECTION
MEDICINE LAKE ROAD EMBANKMENT
3 / 23 24

SAFETY PAYS

SYMBOL	DESCRIPTION	DATE	APPROVAL

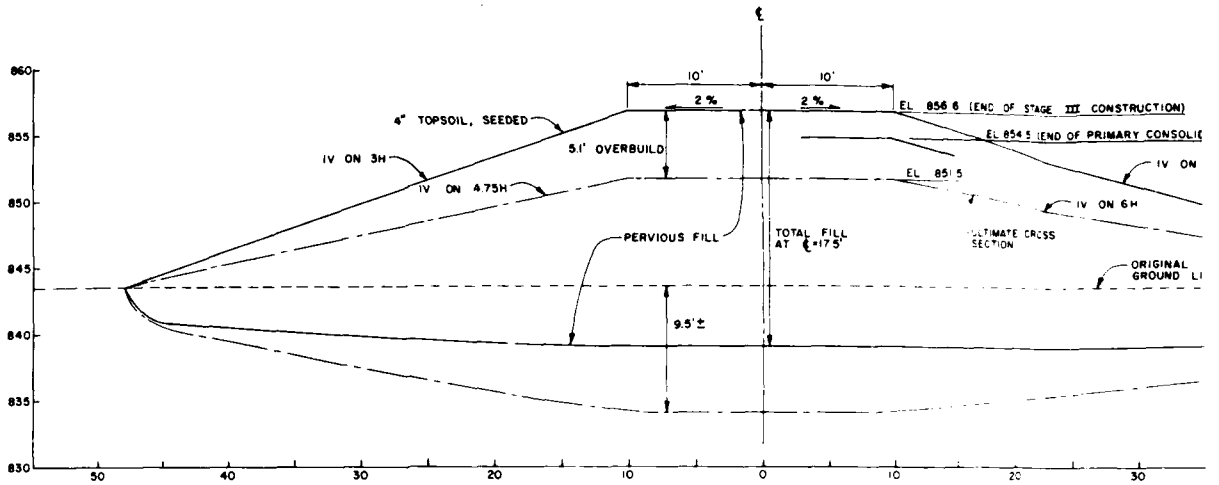
DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY: D.W.R
DRAWN BY: J.M.J
CHECKED BY: M.B.
SUBMITTED BY:
APPROVED:

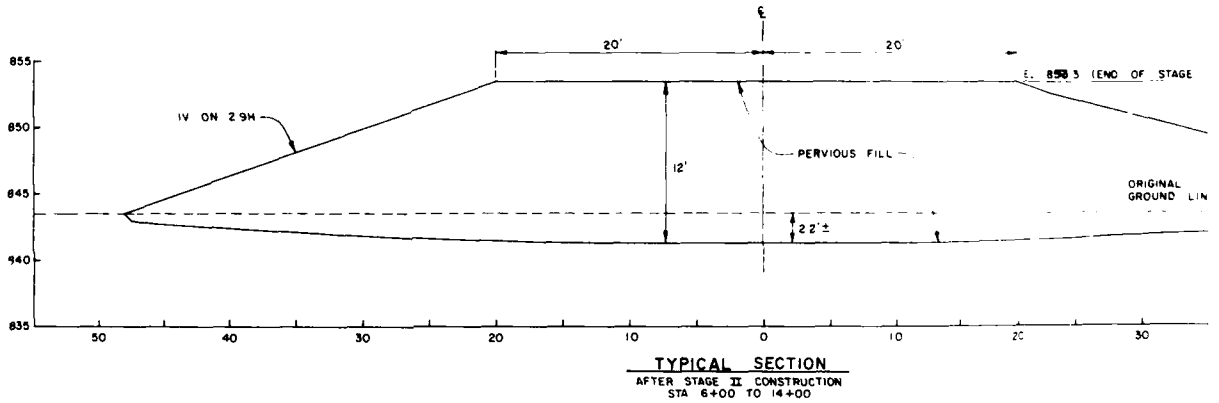
PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
HIGHWAY 100 EMBANKMENT
PROFILES AND SECTIONS

DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34 3-R-5/205
SHEET 24 OF 44

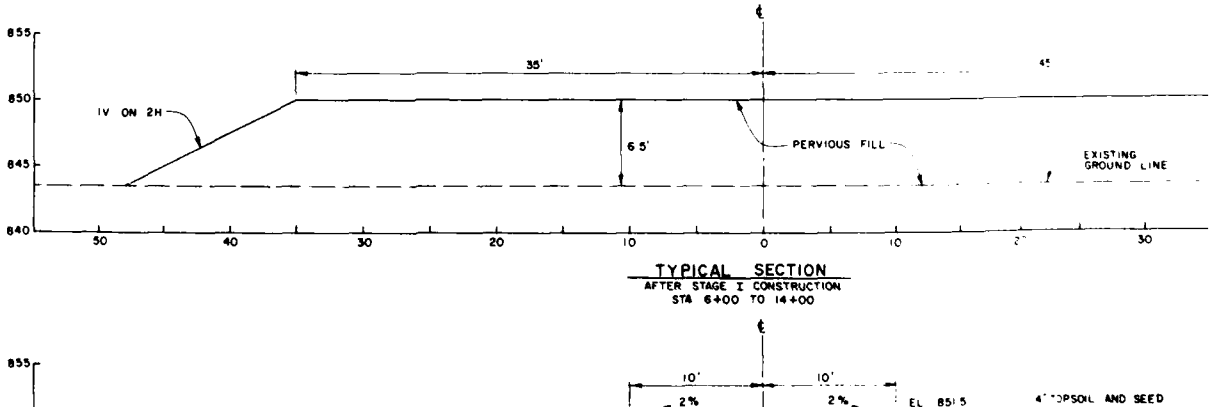
PLATE 24



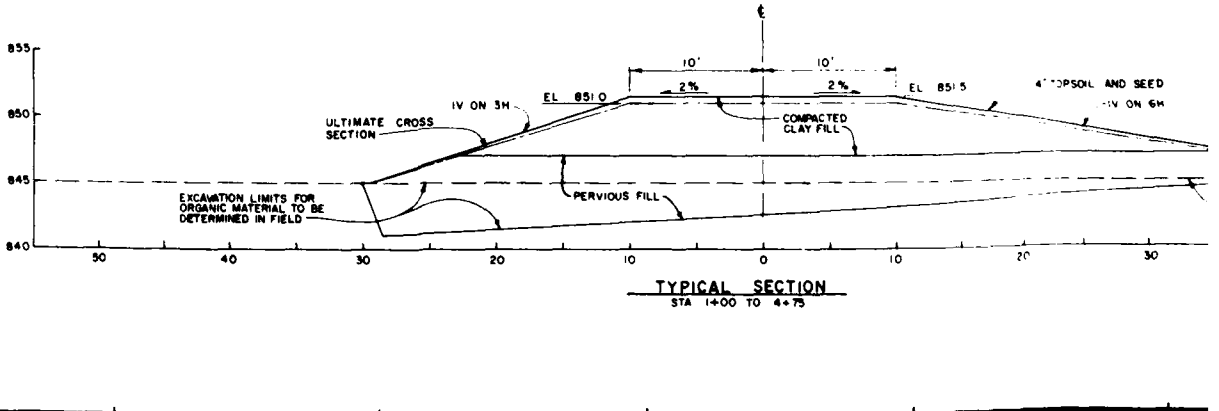
TYPICAL SECTION
AFTER STAGE III CONSTRUCTION
STA 6+00 TO 14+00



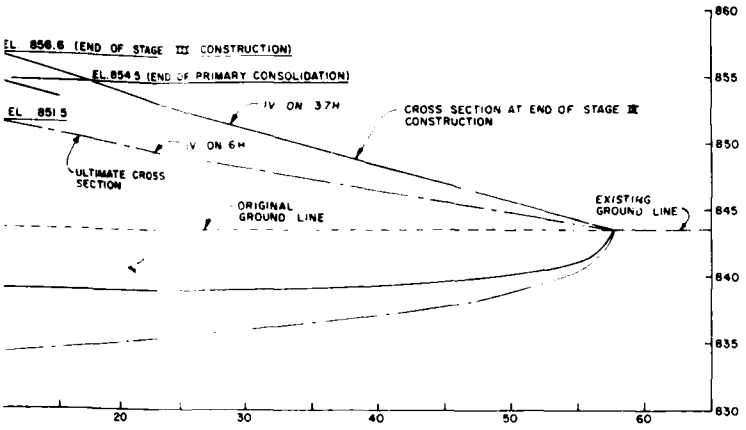
TYPICAL SECTION
AFTER STAGE II CONSTRUCTION
STA 6+00 TO 14+00

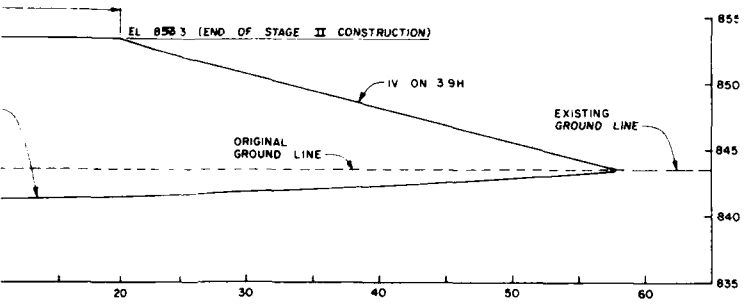


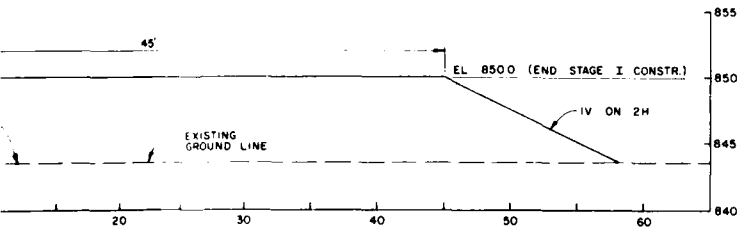
TYPICAL SECTION
AFTER STAGE I CONSTRUCTION
STA 6+00 TO 14+00

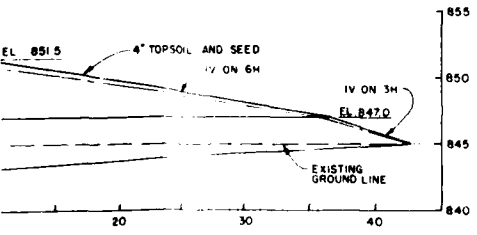


TYPICAL SECTION
STA 1+00 TO 4+75











SYMBOL	DESCRIPTION	DATE	APPROVAL

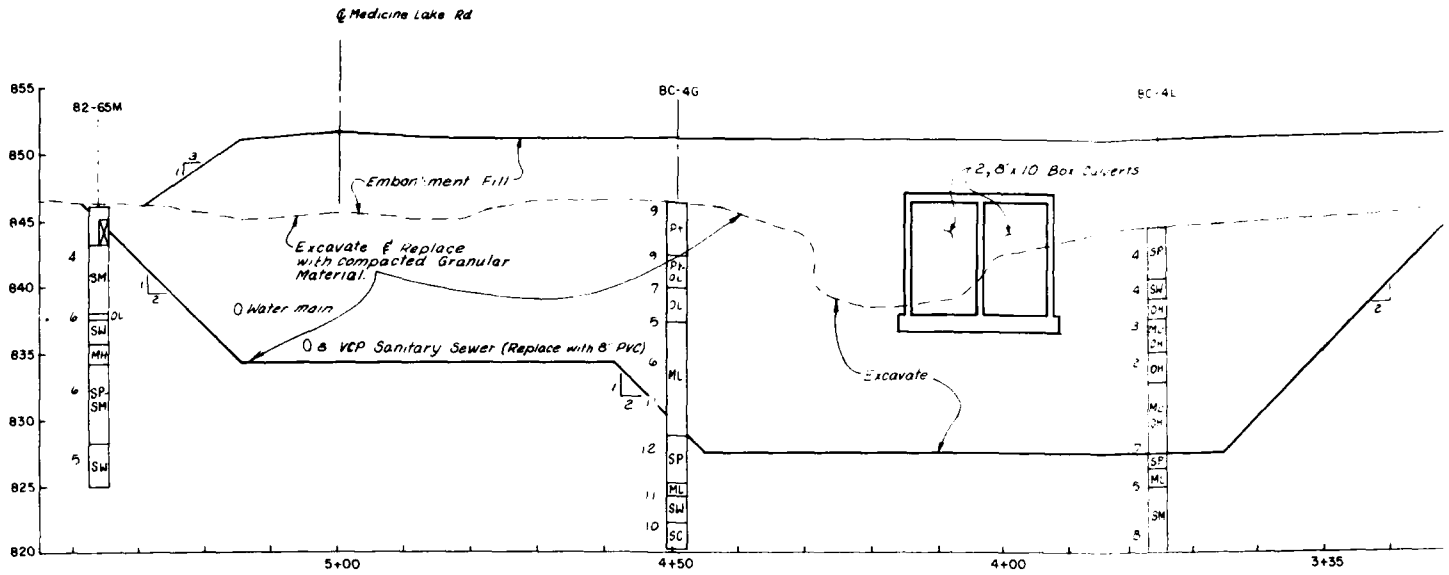
DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY: D W R
DRAWN BY: W J V
CHECKED BY: M B
SUBMITTED BY:
APPROVED:

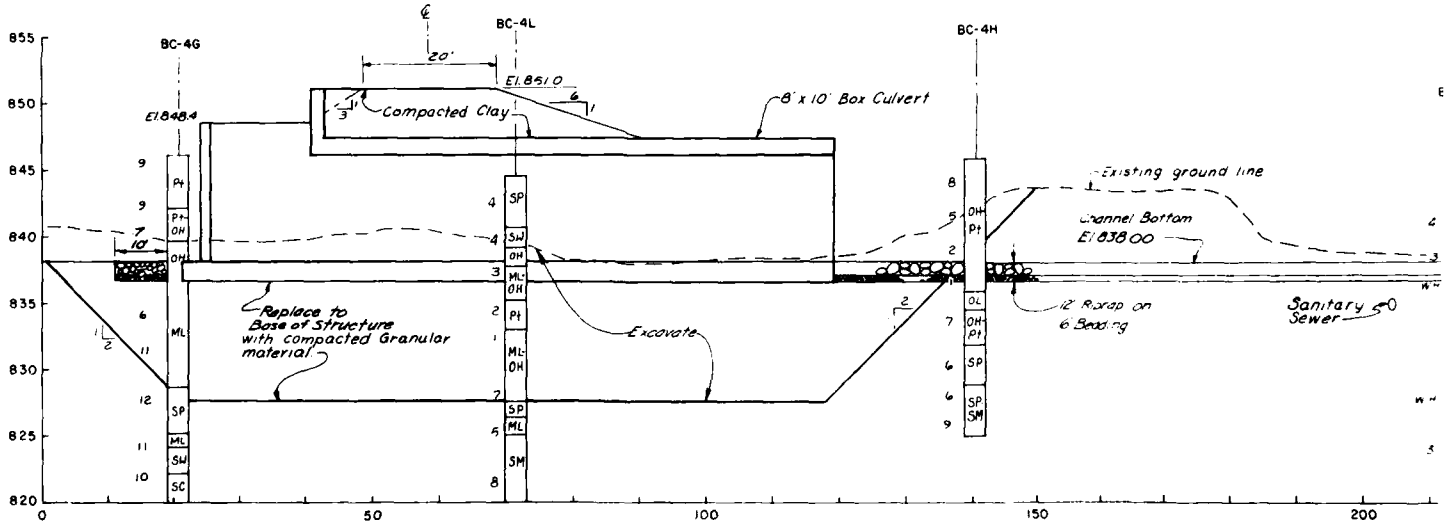
PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
HIGHWAY 100 EMBANKMENT
TYPICAL SECTIONS

DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/206
SHEET 25 OF 44

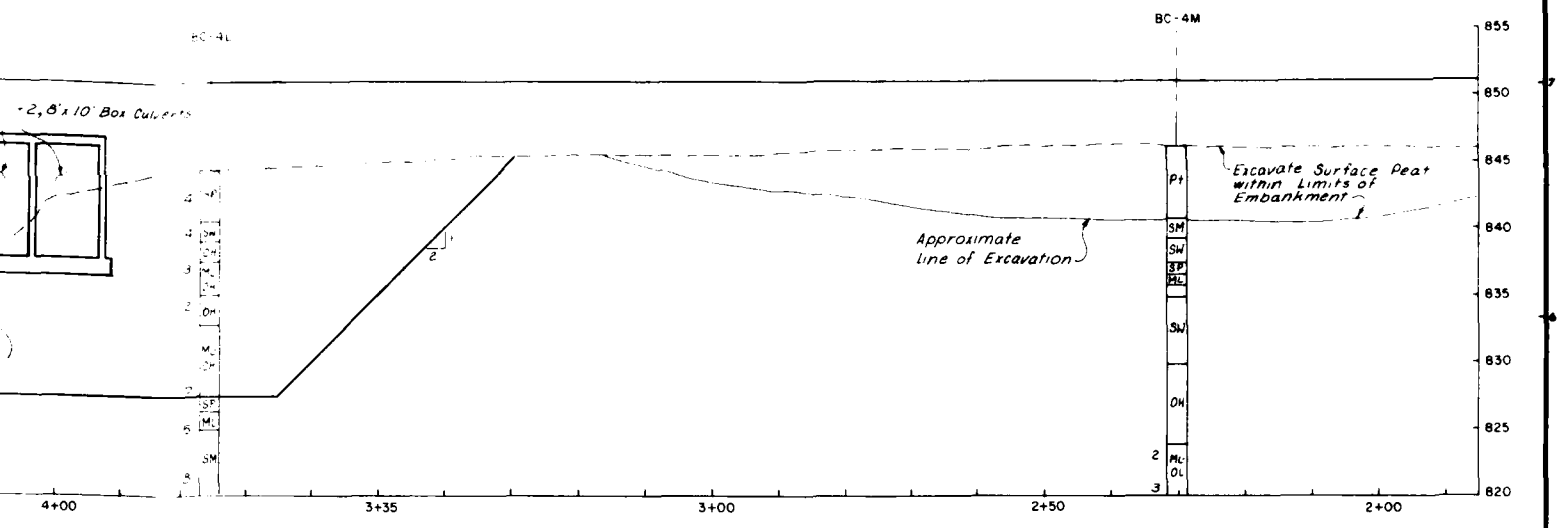
PLATE 25



EMBANKMENT PROFILE 4 (23|26)
STA 1+85 TO 5+00



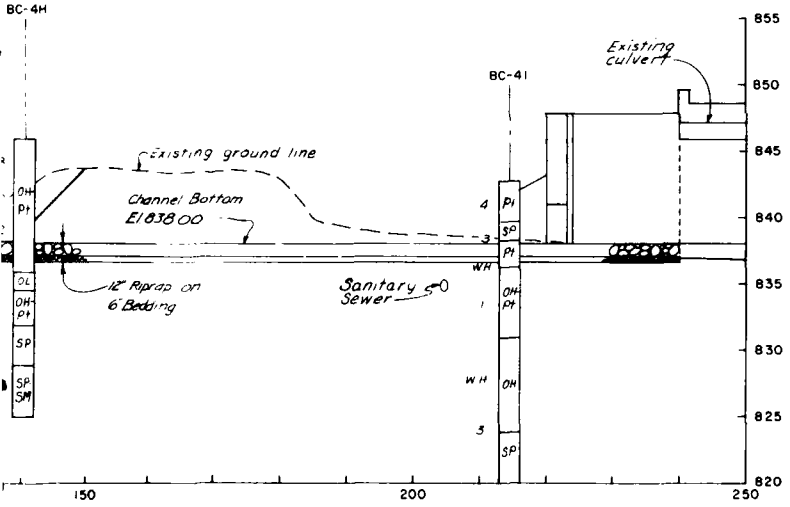
CONDUIT PROFILE 5 (23|26)



EMBANKMENT PROFILE

STA 1+85 TO 5+00

4 / 23 26



5 / 23 26

SAFETY PAYS

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

D.W.R.
P.W.
M.B.

PHASE II — DESIGN MEMORANDUM

FLOOD CONTROL BASSETT CREEK MINNESOTA

HIGHWAY 100 EMBANKMENT

EXCAVATION SECTION & CONDUIT PROFILE

APPROVED

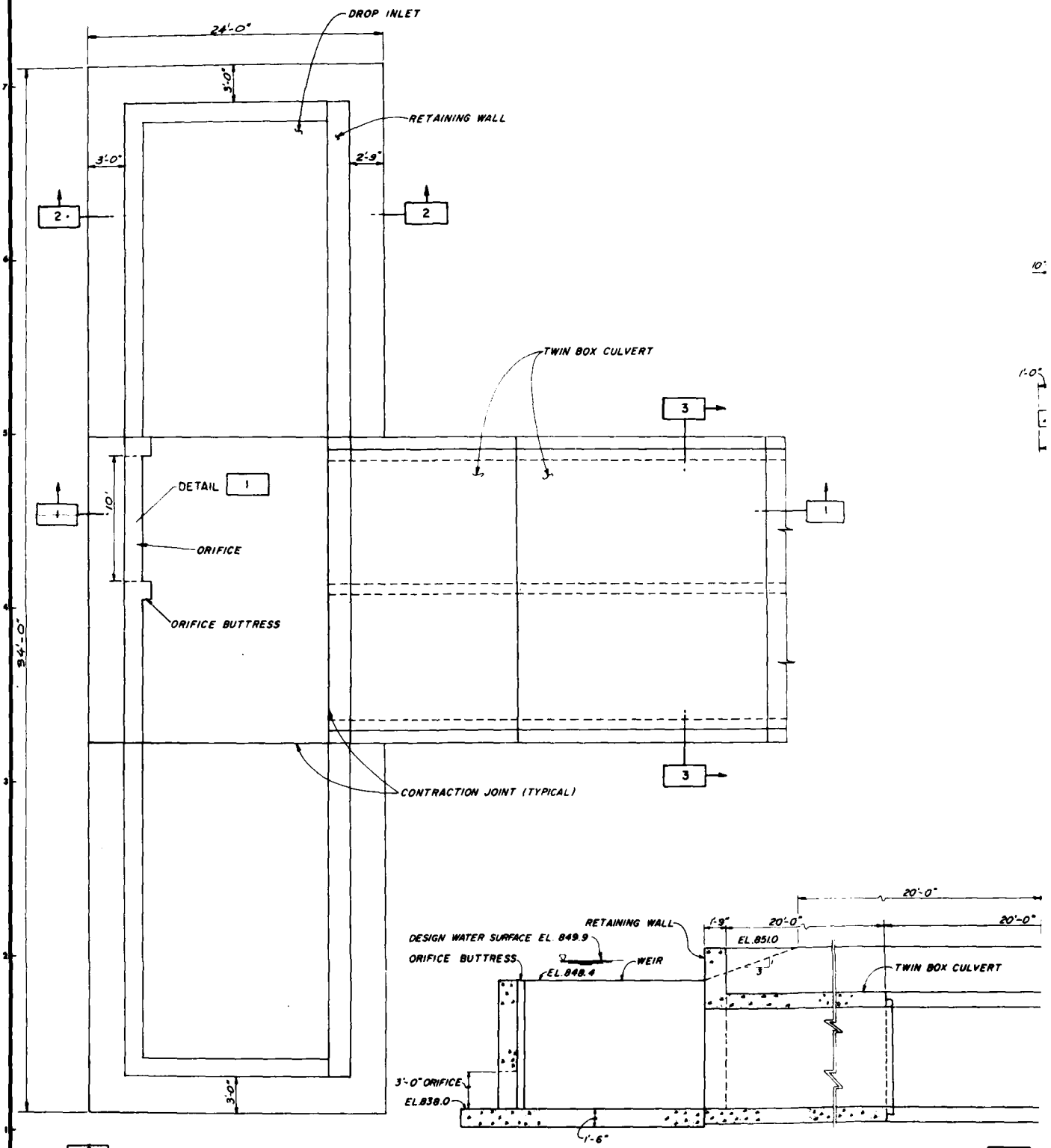
DATE: AUGUST 1962

SCALE: AS SHOWN

DRAWING NUMBER: M34 3-R-5/207

SHEET 26 OF 43

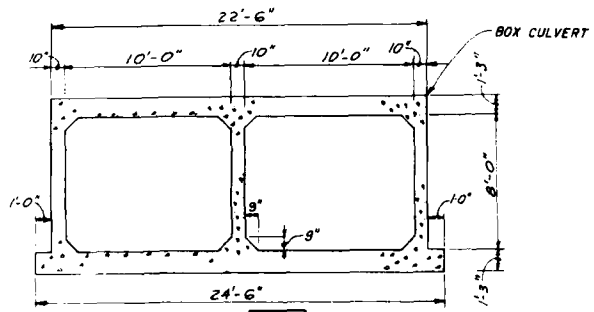
PLATE 26



6
23 27 CONTROL STRUCTURE PLAN

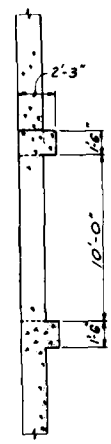
SECTION 1

4 0 4
SCALE IN FEET



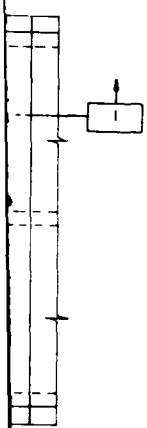
SECTION 3

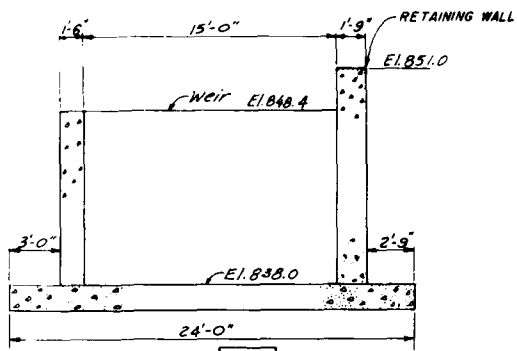
TWIN BOX CULVERT



DETAIL 1

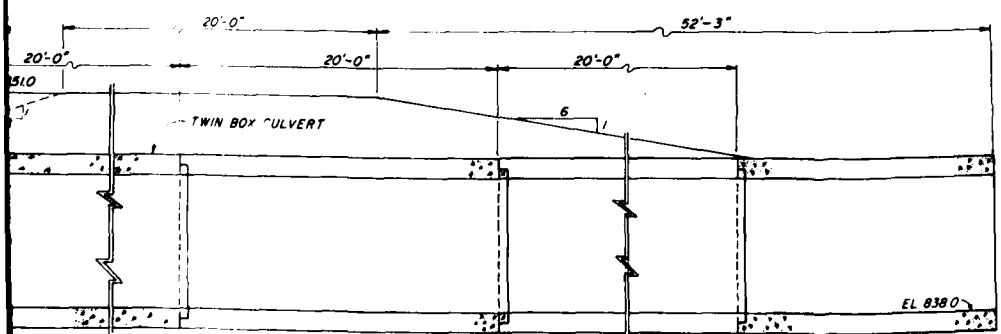
ORIFICE BUTTRESS





SECTION 2

WEIR AND RETAINING WALL



SECTION 1

4 0 4

SCALE IN FEET

SAFETY PAYS

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

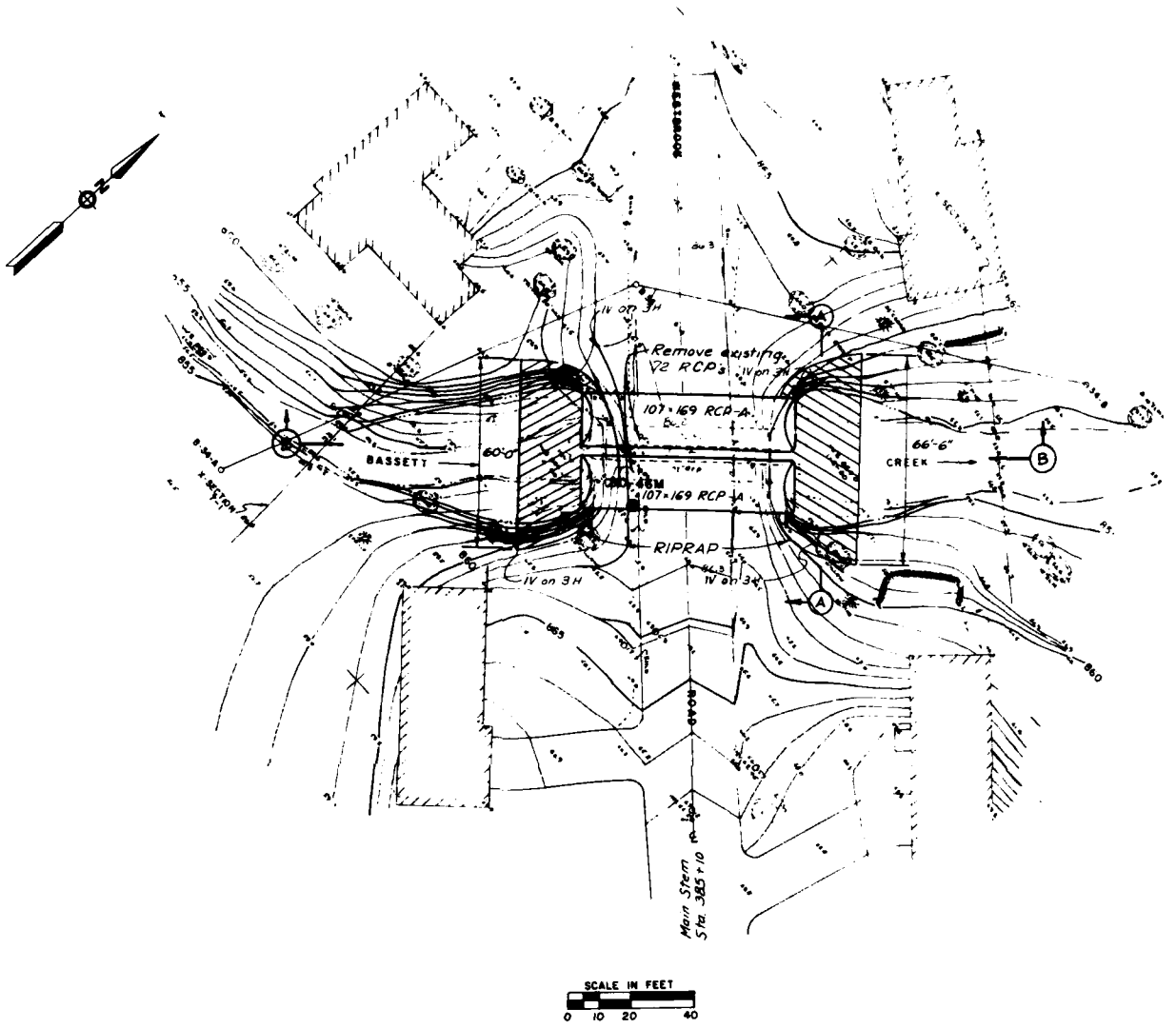
DESIGNED BY: BARR ENGR & G L C
DRAWN BY:
CHECKED BY: L A R
SUBMITTED BY: [signature] CHIEF DESIGN BRANCH
APPROVED: [signature] CHIEF ENGINEERING DIVISION

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
HIGHWAY 100 CONTROL STRUCTURE
PLAN AND SECTIONS

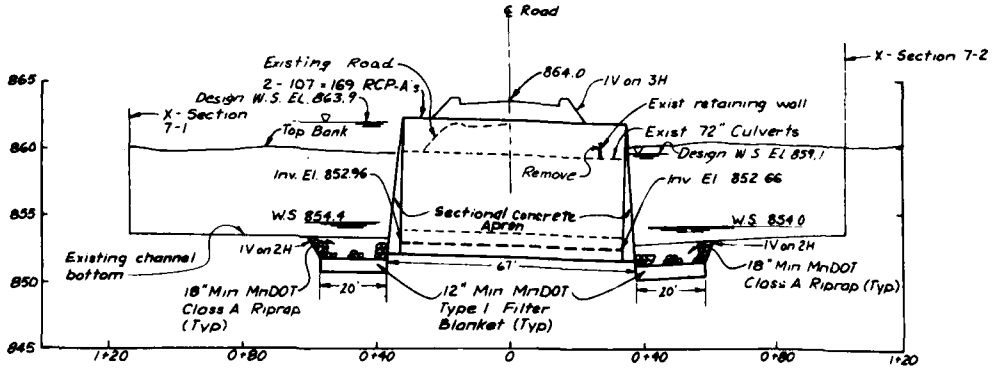
DATE: AUGUST 1982

SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-57208
SHEET 27 OF 44

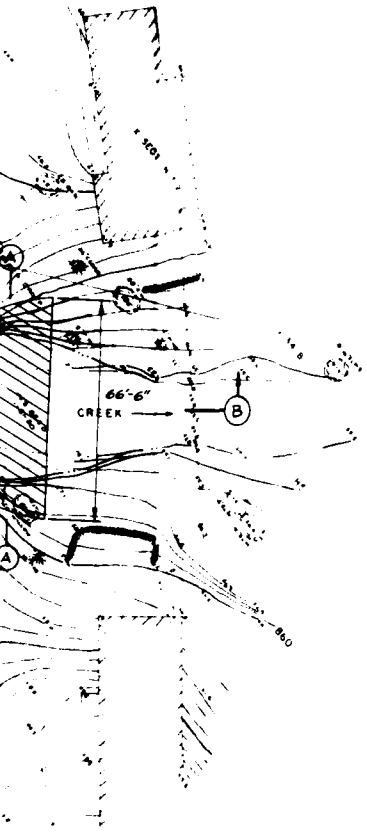
PLATE 27

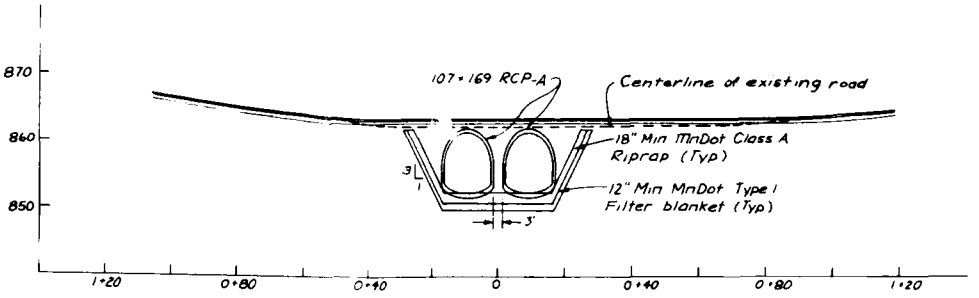


SCALE IN FEET
0 10 20 40

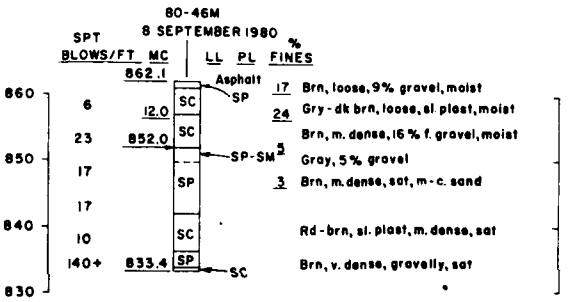


SECTION B-B



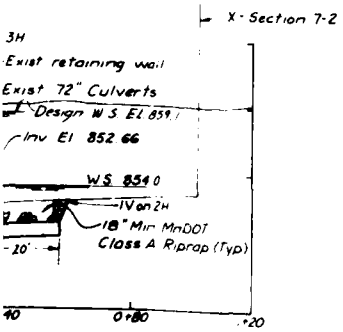


SECTION A-A



NOTE:

1. Pipe bedding shall be class A.
2. 107 x 169 RC pipe shall be class III A
3. Boring Legend shown on Plate C-11.





DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

DESIGNED BY: LAR
DRAWN BY: JGJ
CHECKED BY: LAR
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM

FLOOD CONTROL BASSETT CREEK MINNESOTA

CULVERT REPLACEMENT @ WESTBROOK RD

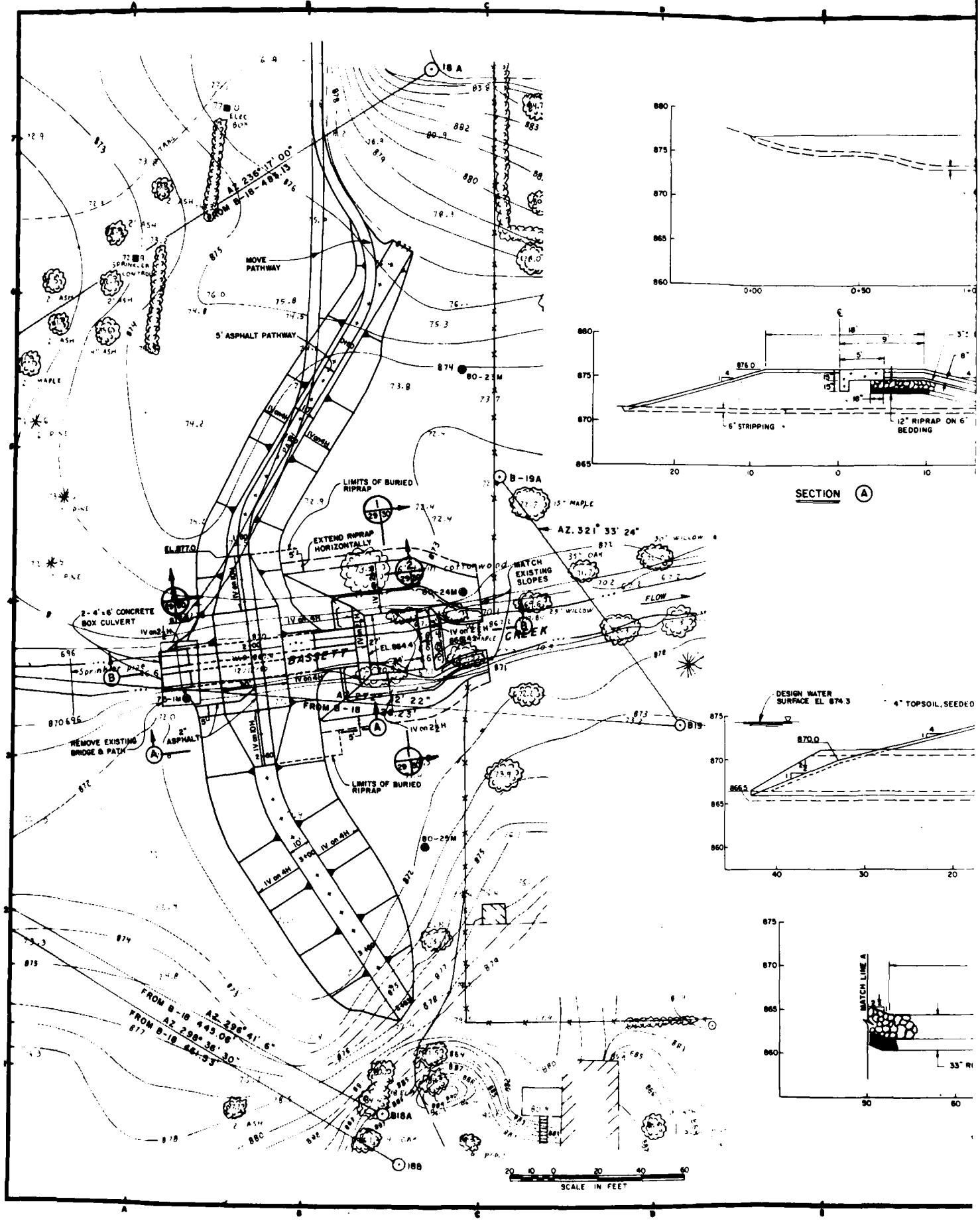
DATE: AUGUST 1982

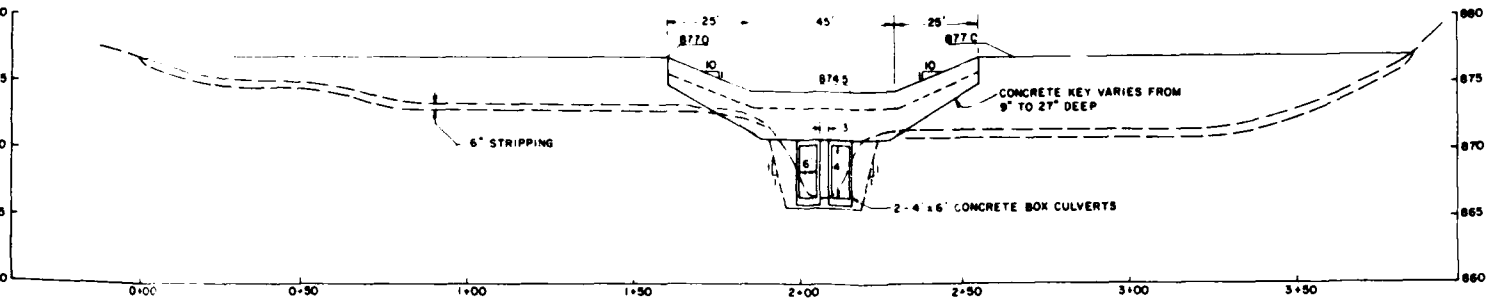
SCALE: AS SHOWN

DRAWING NUMBER: M34.3-R-5/210

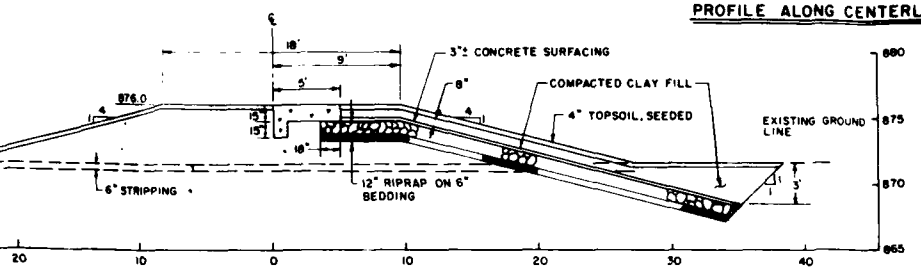
SHEET 28 OF 44

PLATE 28

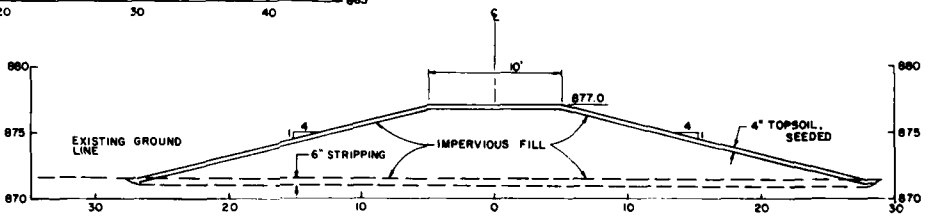

18 A
B-19A
B19
B18A
B18
AZ. 321° 33' 24"
FROM B-18 485.13
AZ. 236° 17' 00"
FROM B-18 445.06
AZ. 296° 41' 6"
FROM B-18 664.93
AZ. 298° 36' 30"
MOVE PATHWAY
5' ASPHALT PATHWAY
LIMITS OF BURIED RIPRAP
EXTEND RIPRAP HORIZONTALLY
MATCH EXISTING SLOPES
2-4'x6' CONCRETE BOX CULVERT
REMOVE EXISTING BRIDGE & PATH
2" ASPHALT
BASSETT CREEK
FLOW
EL. 877.0
EL. 864.4
80-24M
80-23M
80-29M
72-1M
SECTION (A)
6" STRIPPING
12" RIPRAP ON 6" BEDDING
DESIGN WATER SURFACE EL 874.3
4" TOPSOIL, SEEDED
MATCH LINE A
SCALE IN FEET



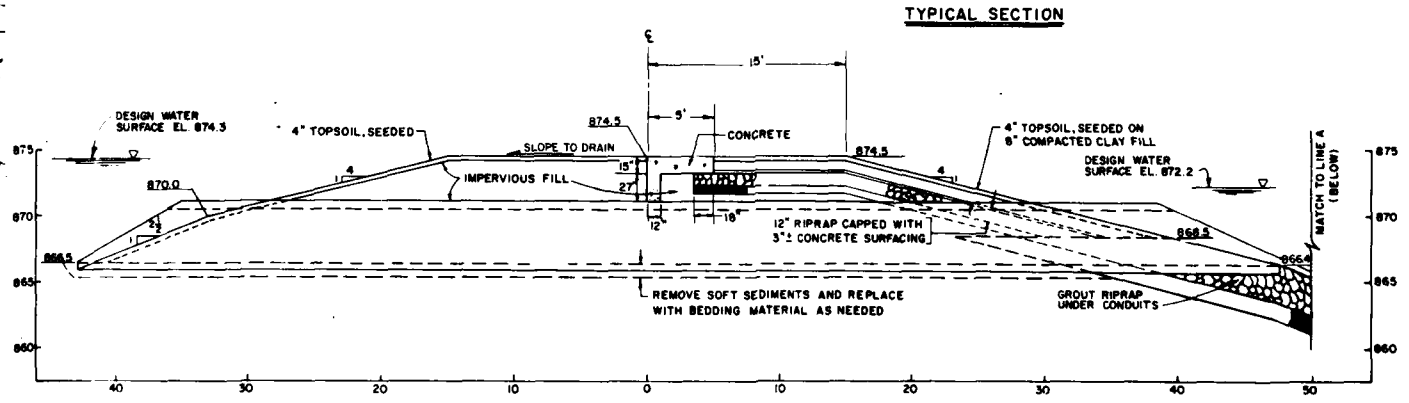
PROFILE ALONG CENTERLINE



SECTION (A)

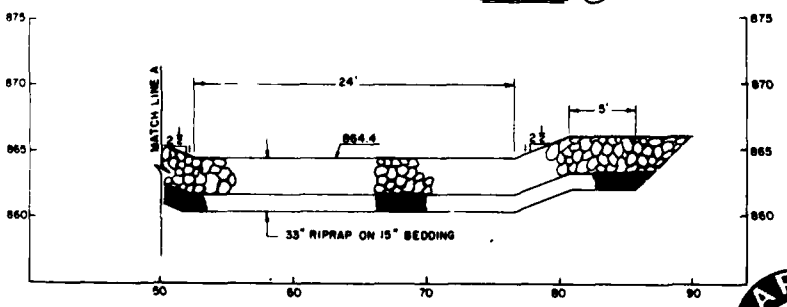


TYPICAL SECTION



SECTION (B)

NOTE:
SEE SHEET 30 FOR DETAILED BORING LOGS



SECTION (B)

SAFETY PAYS

DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

D.R., P.M.F.

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

GOLDEN VALLEY COUNTRY CLUB EMBANKMENT
PLAN, PROFILE AND SECTIONS

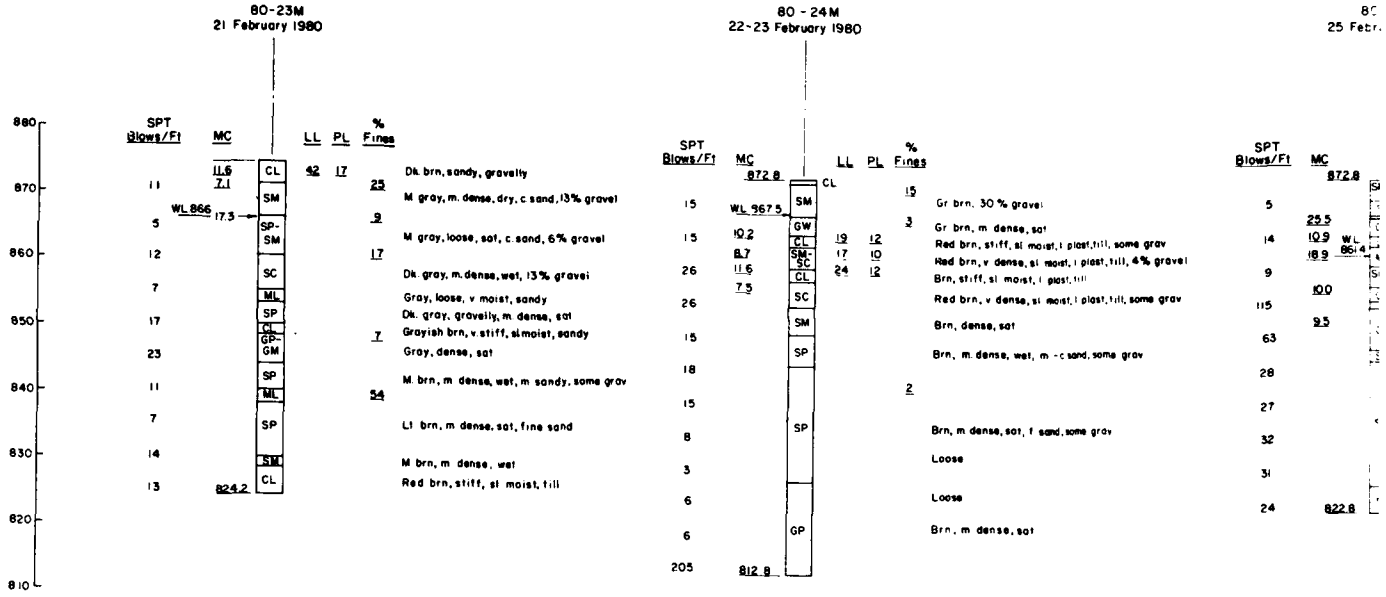
DATE: AUGUST 1962

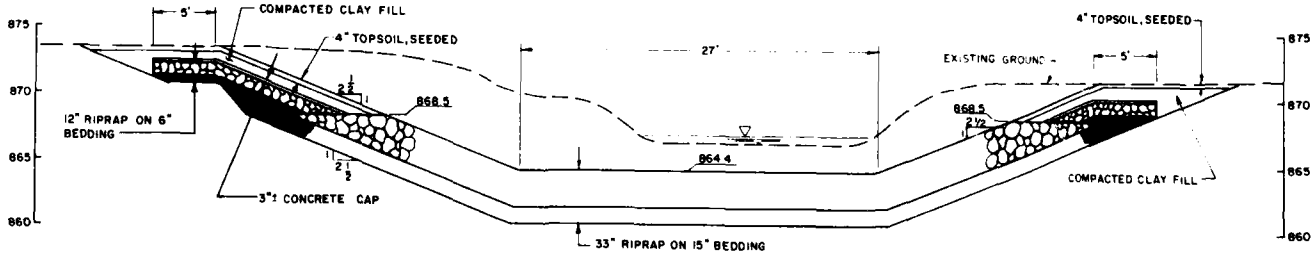
AS SHOWN

M34.3-R-57211

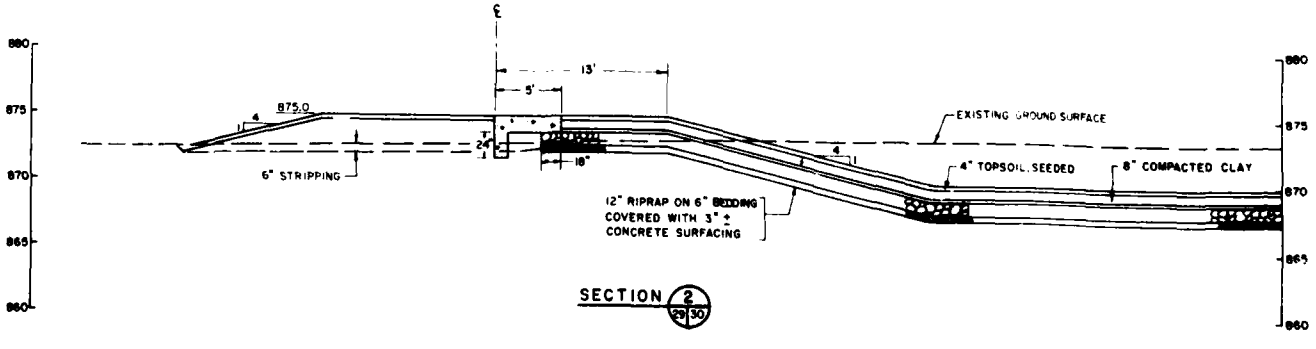
SHEET 29 OF 44

PLATE 29





SECTION 1 (29/30)



SECTION 2 (29/30)

80-25M
25 February 1980

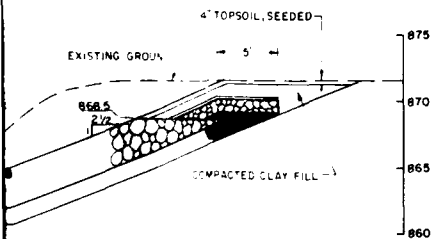
% Fines		SPT Blows/Ft	MC		LL	PL	% Fines	
			872.8	SP-SM			6	Brn, 38% gravel
15	Gr brn, 30% gravel	5		SP				Lt brn, loose, moist, m. sand
3	Gr brn, m dense, sat		25.5		wet 28	20		Gray, silty, loose, v moist
	Red brn, stiff, sl moist, plastic till, some grav	14	10.9 WL	CL			62	Brn, stiff, sl moist, till
	Red brn, v dense, sl moist, plastic till, 4% gravel		18.9 861.4	CL				Gray, loose, sat, 4% gravel
	Brn, stiff, sl moist, t plastic	9		ML			9	Gray, loose, sat, 2% gravel, f - m sand
	Red brn, v dense, sl moist, plastic till, some grav		10.0	SP-SM				Gr brn, v stiff, sl moist, l plast, till
	Brn, dense, sat	115		CL				
			9.5	SP, sat	21	12		Brn, hard, sl moist, l plast, till
	Brn, m dense, wet, m sand, some grav	63		CL				
2		28		SC				Brn, v dense, sl moist, till, some grav
		27					2	
	Brn, m dense, sat, f sand, some grav	32		SP				Lt brn, dense, sat, m - f sand, 2% gravel
	Loose	31						
	Loose	24	822.8	SP				Brn, dense, sat, gravelly
	Brn, m dense, sat							

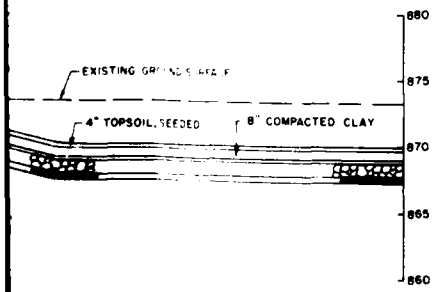
75 - 1M
16 March 1975

SPT Blows/Ft		
	871.5	
5	WL 866.5	OL & Pt — Bituminous & crushed rock
5		CL — Dk brn - blk, soft - loose, moist
6		ML — Gray, sandy, soft, moist
9		CL GP — Gray, loose, low plast, moist
5		SP — Gray, loose, sat, f sand
21		ML — Red - brn, m dense, sandy, grav, moist
10		SP — Red - brn, loose, sat, f sand
8		
11		
14		SP — Red - brn, m dense, sat, f sand
23		
18	841.5	

NOTE

BORING LEGEND SHOWN ON PLATE C-11





SAFETY PAYS

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

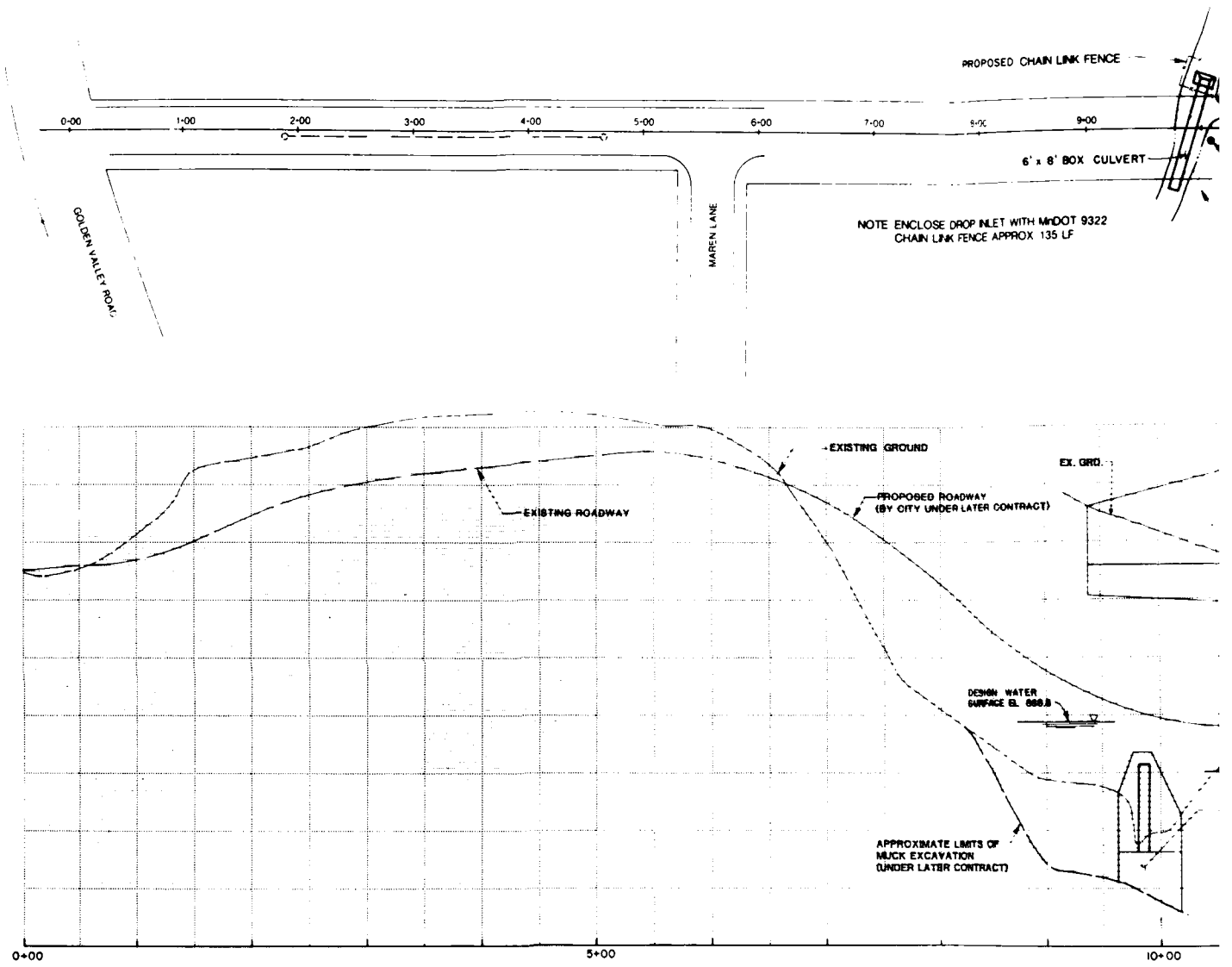
DESIGNED BY: D.R., P.M.F.
DRAWN BY: J.G.J.
CHECKED BY: M.B.
SUBMITTED BY:
APPROVED:

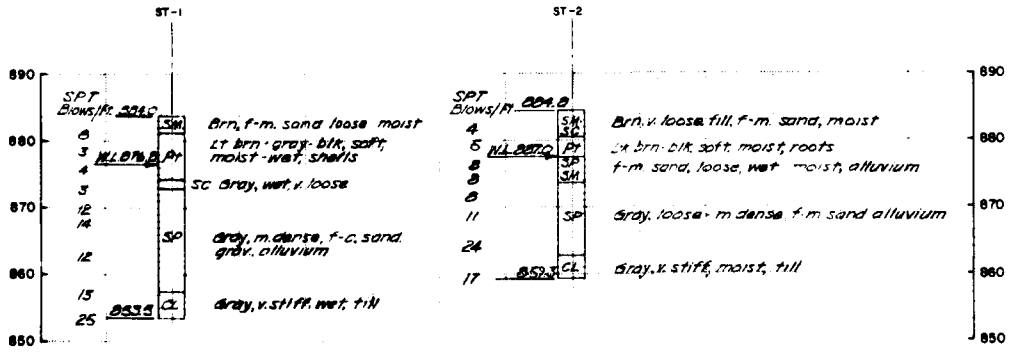
PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
GOLDEN VALLEY COUNTRY CLUB
EMBANKMENT
BORING LOGS AND SECTIONS

DATE: AUGUST 1982

DRAWING NUMBER
M34.3 R-5/212
SHEET 30 OF 44

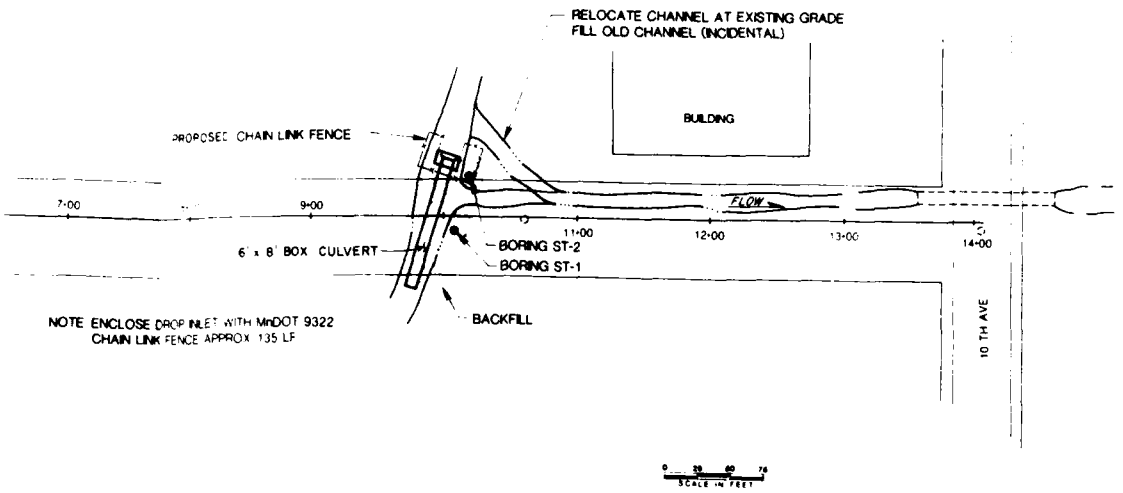
PLATE 30

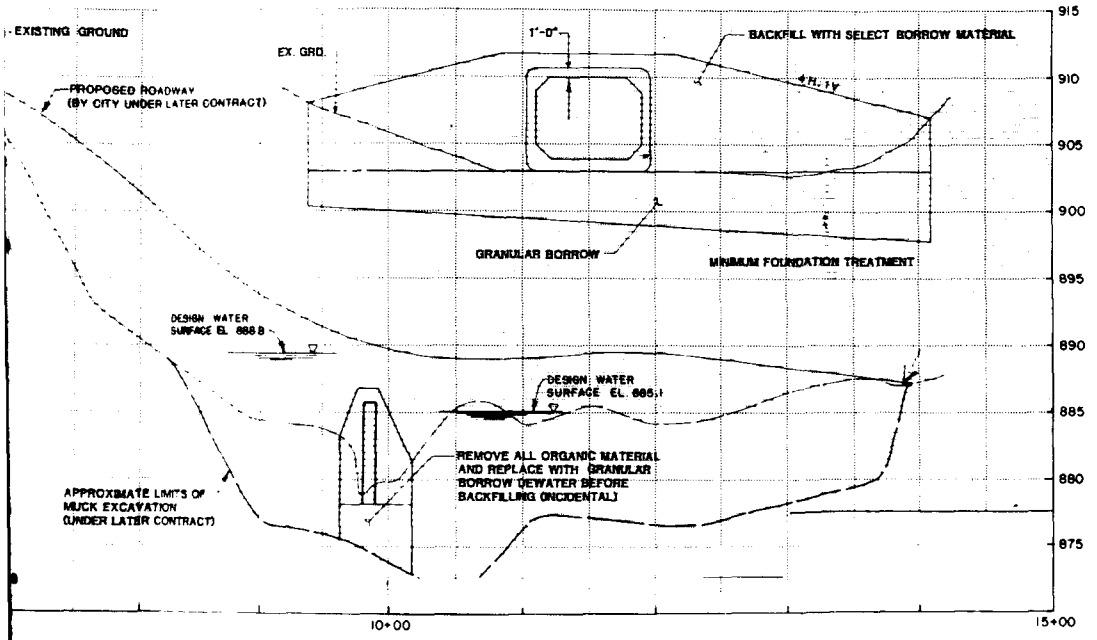




NOTE

BORING LEGEND SHOWN ON PLATE C-11





890
880
870
860
850

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL MINNESOTA

DESIGNED BY BARR ENG CO
DRAWN BY P.W. JMJ
CHECKED BY L.A.R.
SUBMITTED BY
CHIEF ENGINEERING BRANCH
APPROVED
CHIEF ENGINEERING BRANCH

PHASE II
FLOOD CONTROL

DESIGN MEMORANDUM
BASSETT CREEK MINNESOTA

WISCONSIN AVE EMBANKMENT
PLAN AND PROFILE

DATE: AUGUST 1982

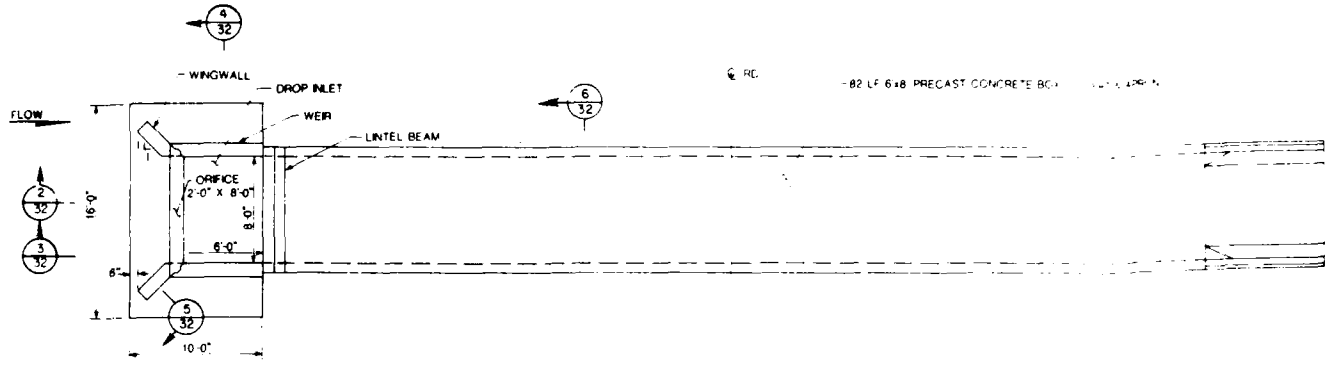
SCALE AS SHOWN

DRAWING NUMBER
M34.3-R-5213

SHEET 31 OF 44

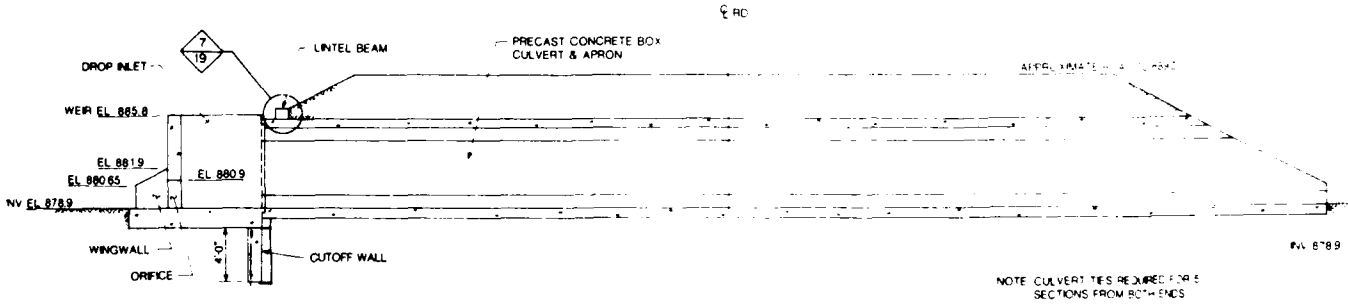
SAFETY PAYS

PLATE 31



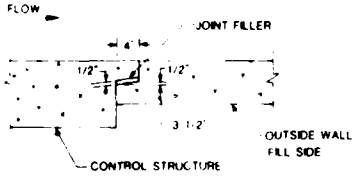
PLAN CONTROL STRUCTURE

SCALE IN FEET



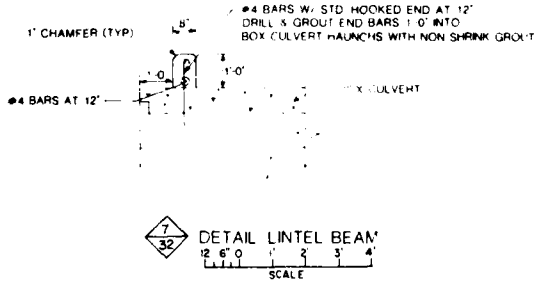
SECTION CONTROL STRUCTURE

SCALE IN FEET



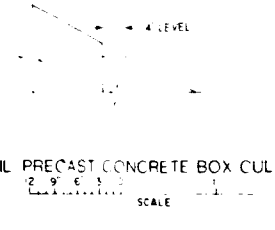
DETAIL TONGUE AND GROOVE JOINT

SCALE



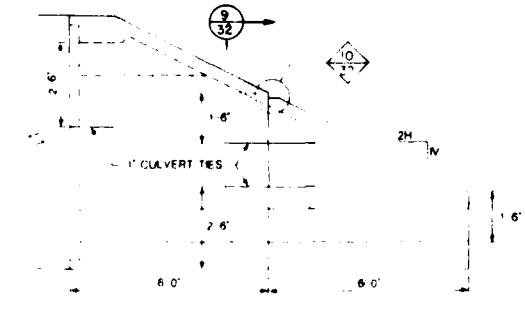
DETAIL LINTEL BEAM

SCALE



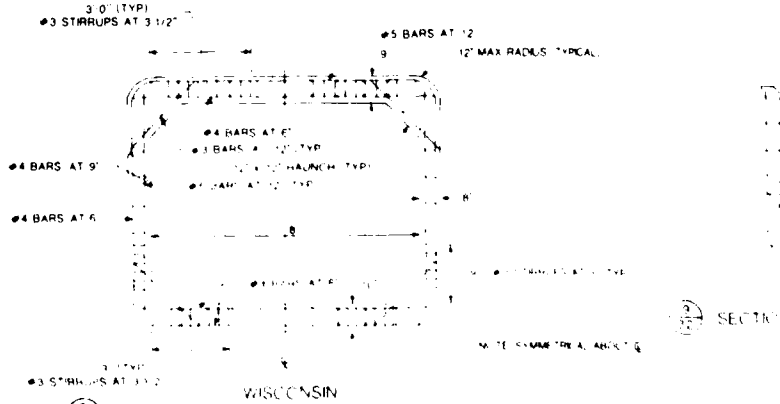
DETAIL PRECAST CONCRETE BOX CULVI

SCALE



SECTION PRECAST CONCRETE BOX CULVERT APRON

SCALE



SECTION

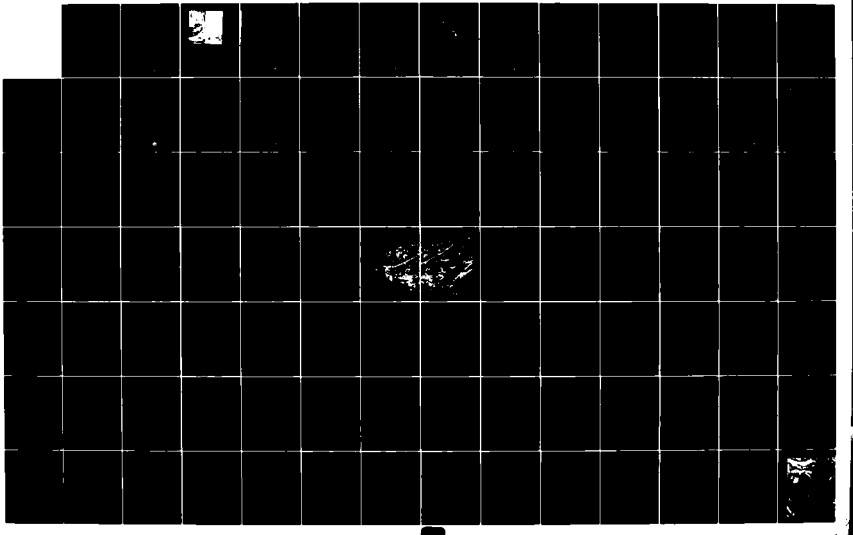
SECTION 6 x 8 BOX CULVERT REINFORCEMENT

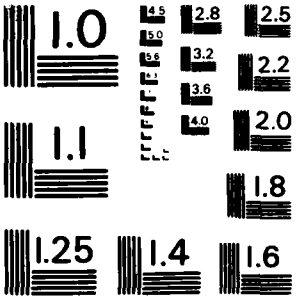
WISCONSIN

AD-A133 795 BASSETT CREEK WATERSHED HENNEPIN COUNTY MINNESOTA FEASIBILITY REPORT FOR FLOOD CONTROL MAIN REPORT(U) CORPS OF ENGINEERS ST PAUL MN ST PAUL DISTRICT SEP 82 F/G 13/2 NL

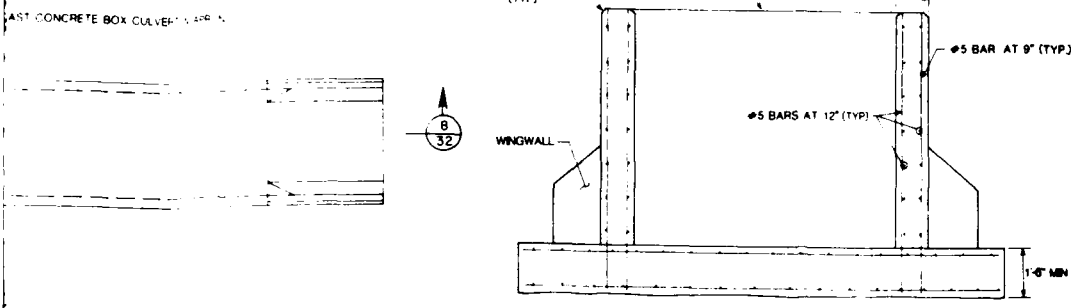
2/7

UNCLASSIFIED

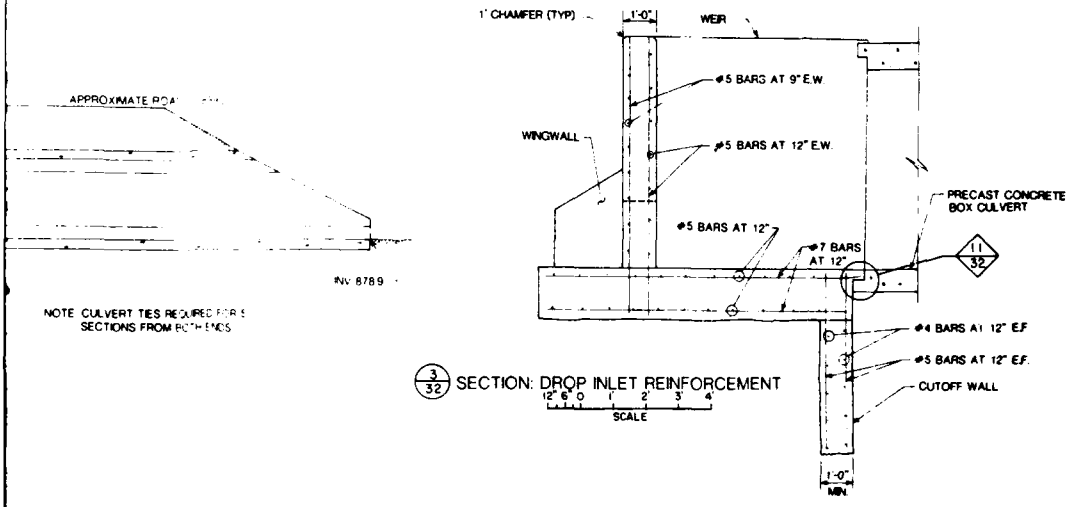




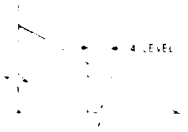
MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS - 1963 - A

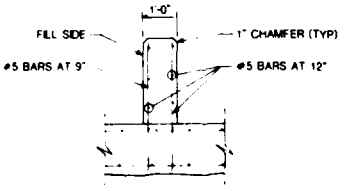


4/32 SECTION: DROP INLET REINFORCEMENT
SCALE 12" 6" 0 1' 2' 3' 4'



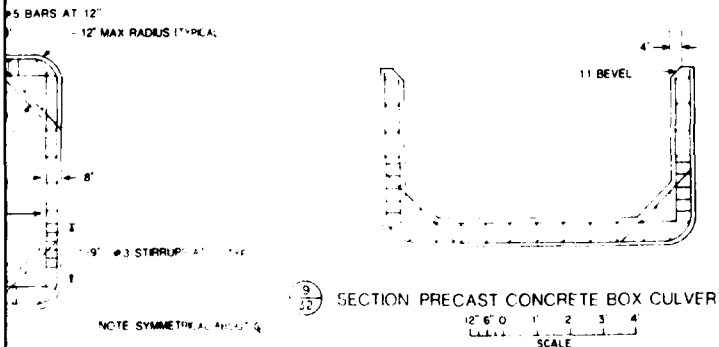
3/32 SECTION: DROP INLET REINFORCEMENT
SCALE 12" 6" 0 1' 2' 3' 4'





10/32 DETAIL PRECAST CONCRETE BOX CULVERT APRON
SCALE 12" 9" 6" 3" 0 2'

5/32 SECTION: WING WALL REINFORCEMENT
SCALE 12" 6" 0 1' 2' 3' 4'



NOTE REINFORCEMENT SAME AS PRECAST BOX CULVERT

9/32 SECTION PRECAST CONCRETE BOX CULVERT APRON
SCALE 12" 6" 0 1' 2' 3' 4'

REINFORCEMENT

SYMBOL	DESCRIPTION	DATE	APPROVED

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY: BARR ENG. CO
DRAWN BY: P.W.
CHECKED BY: L.A.R.
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL — BASSETT CREEK MINNESOTA

WISCONSIN AVE EMBANKMENT
SECTION AND DETAILS

DATE: AUGUST 1982
SCALE: AS SHOWN
WORK UNIT NO.
DRAWING NUMBER: M34.3-R-5/214
SHEET 32 OF 44

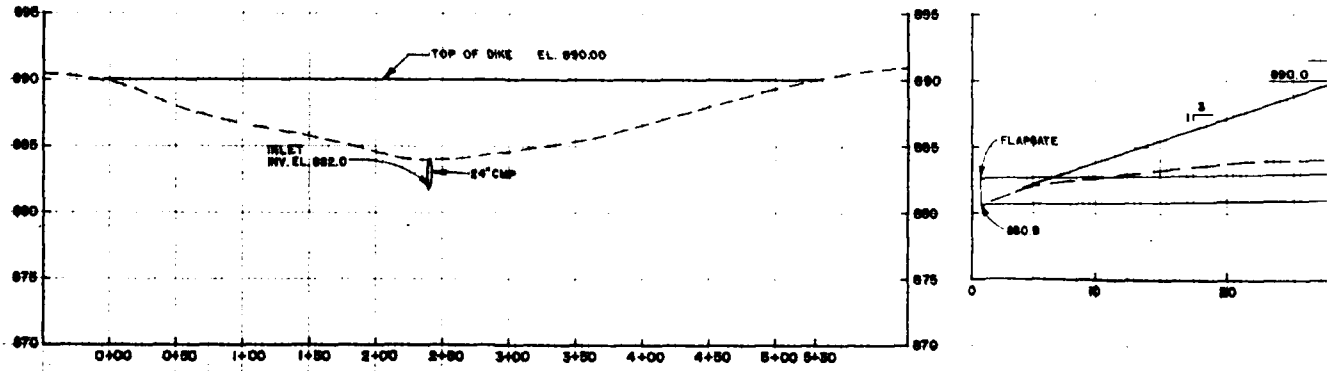
SAFETY PAYS

2

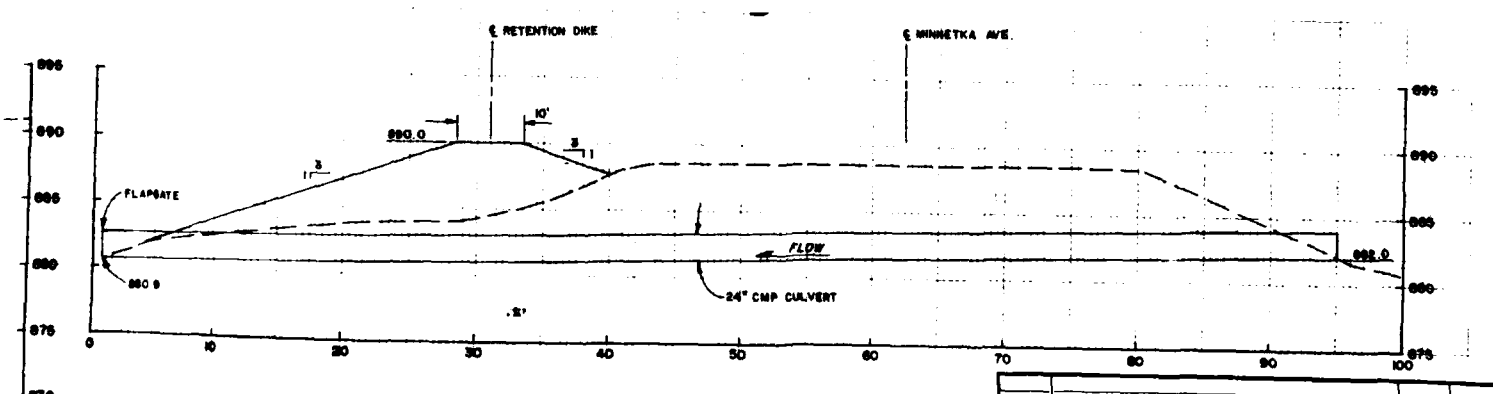


PLAN

50 0 50 100 150
SCALE IN FEET



PROFILE



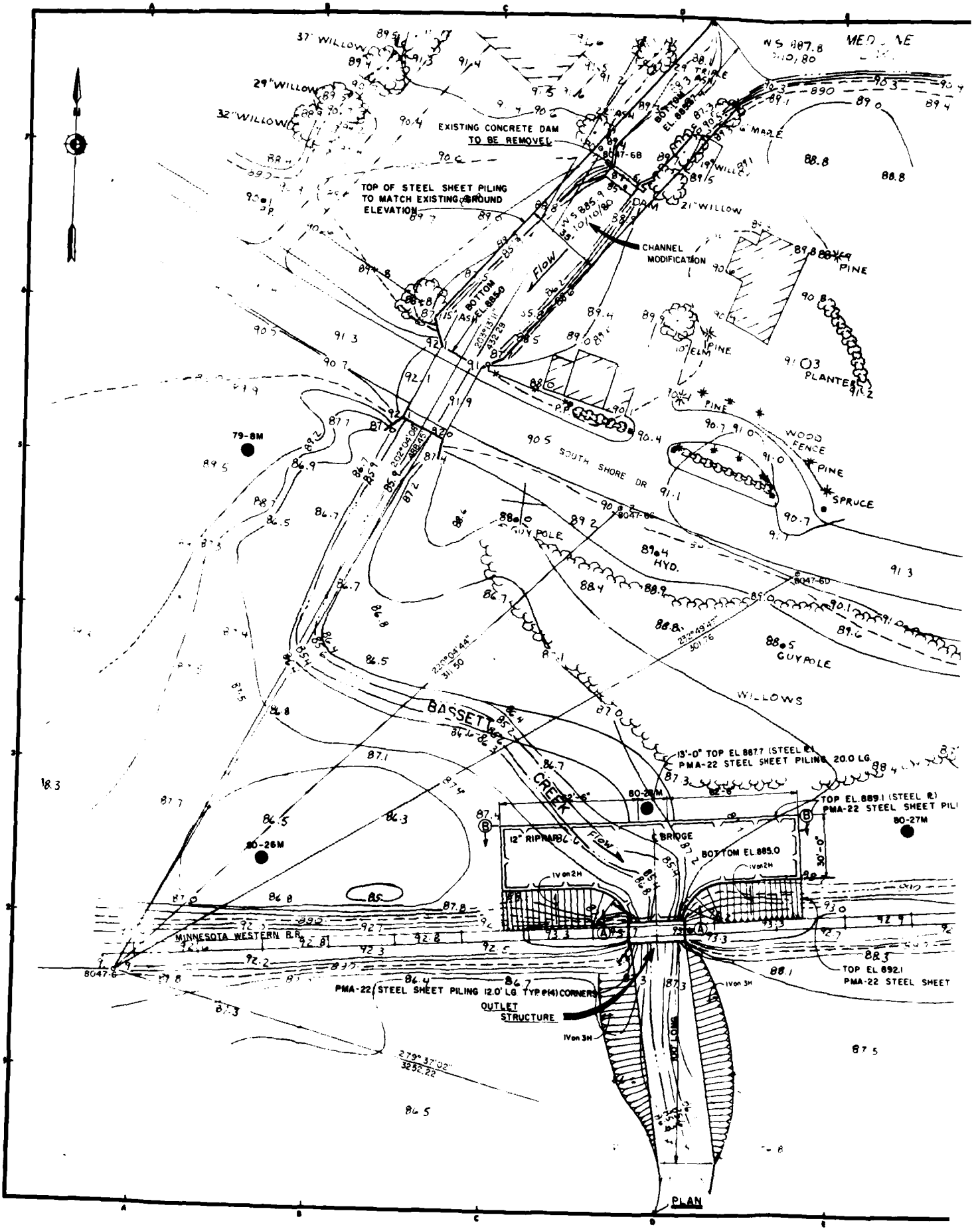
SYMBOL	DESCRIPTION	DATE	APPROVAL

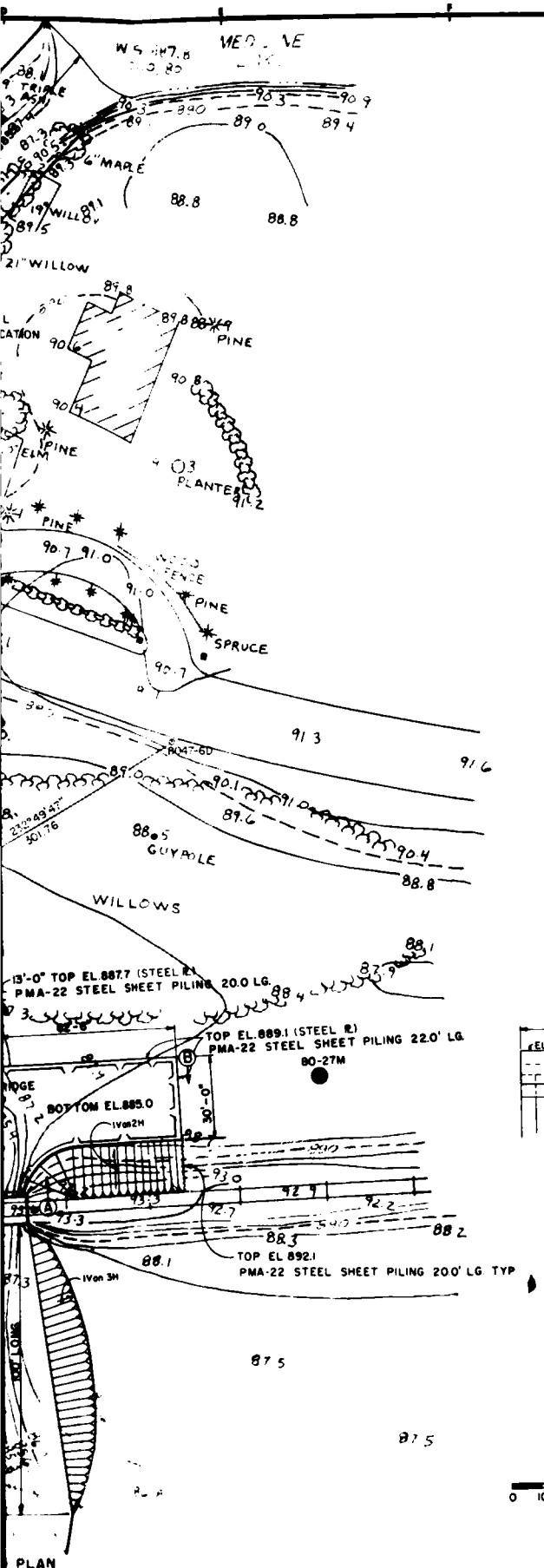
DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

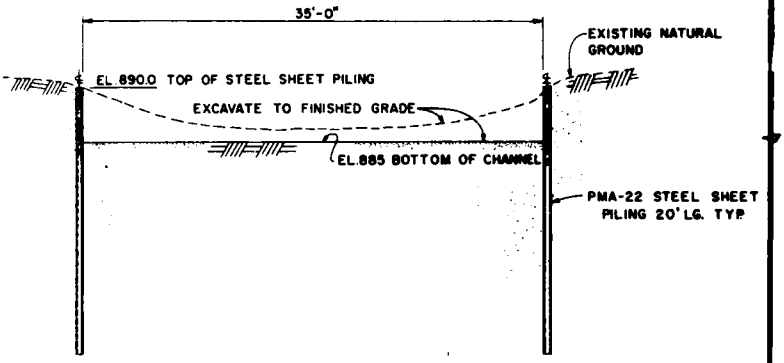
DESIGNED BY: D.W.R.	PHASE II DESIGN MEMORANDUM
DRAWN BY: J.M.J.	FLOOD CONTROL BASSETT CREEK MINNESOTA
TRACED BY: D.W.R.	WINNETKA AVENUE
SUBMITTED BY:	PLAN AND PROFILE
APPROVED:	DATE: AUGUST 1982
	SCALE: AS SHOWN
	DRAWING NUMBER: M34.3-R-5/216 SHEET 33 OF 44

SAFETY PAYS

PLATE 33


EXISTING CONCRETE DAM TO BE REMOVED
TOP OF STEEL SHEET PILING TO MATCH EXISTING GROUND ELEVATION
CHANNEL MODIFICATION
Flow
BOTTOM EL. 885.0
SOUTH SHORE DR
BASSETT CREEK
WILLOWS
PINE
SPRUCE
WOOD FENCE
PLANTER
GUYPOLE
HYD.
79-8M
80-26M
80-27M
BRIDGE
12" RIPRAP
MINNESOTA WESTERN R.R.
PMA-22 STEEL SHEET PILING 12.0' LG TYP @(4) CORNERS
OUTLET STRUCTURE
PLAN

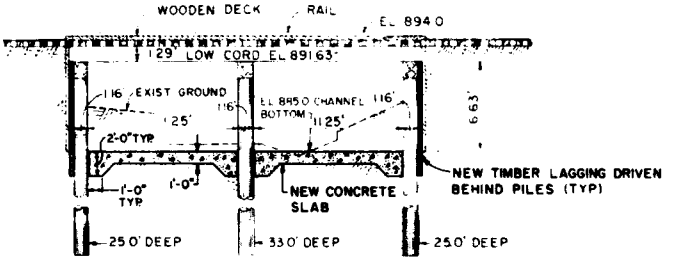




TYPICAL CHANNEL CROSS SECTION

CHANNEL MODIFICATION

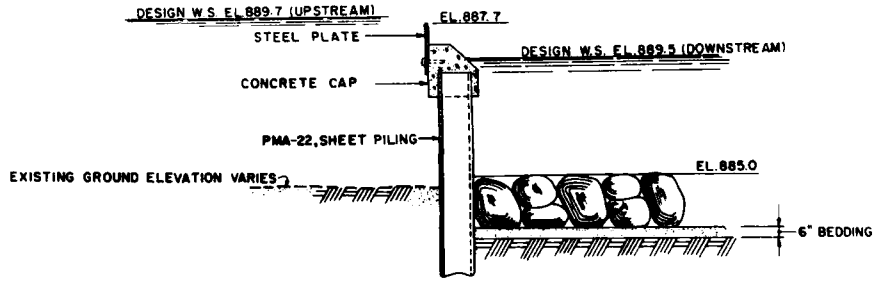
SCALE: 1"=5'-0"



SECTION A-A

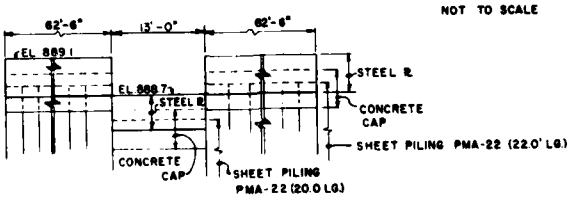
EXISTING BRIDGE

SCALE: 1"= 8'



CONTROL WEIR DETAIL

NOT TO SCALE



SECTION B-B

NOT TO SCALE

NOTE:

1 FOR BORING LOGS SEE SHEET NO C-14

0 10 20 30 40 50

SCALE IN FEET

SAFETY PAYS

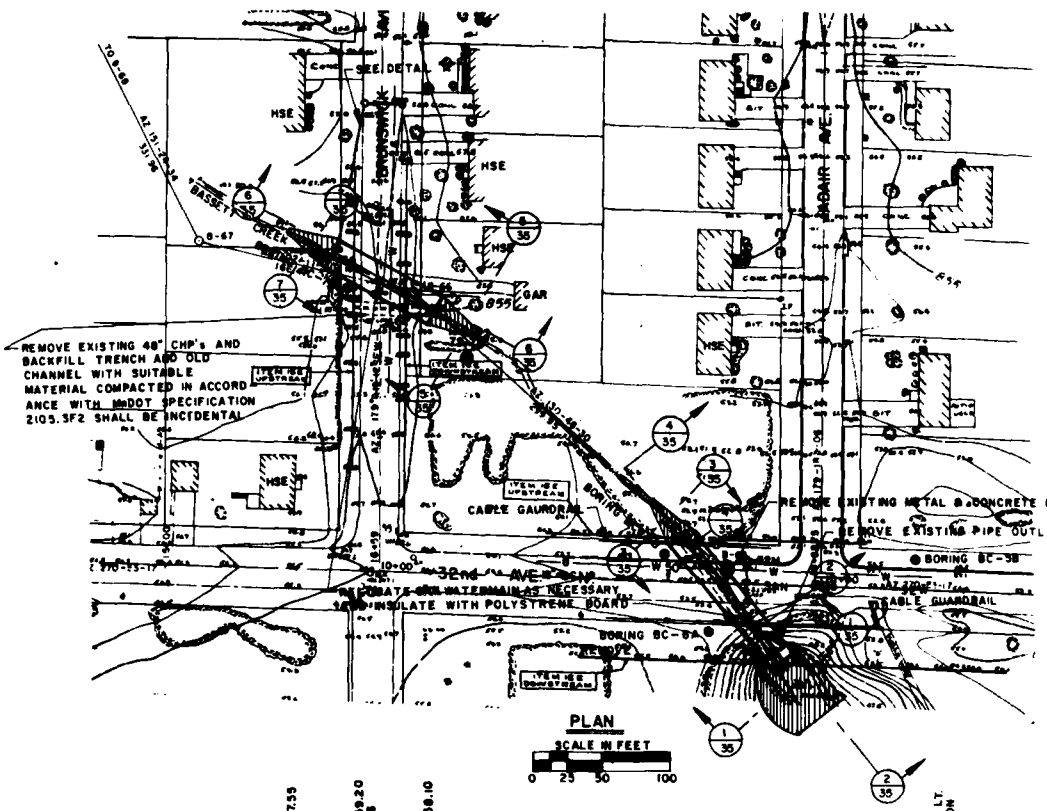
DEPARTMENT OF THE ARMY

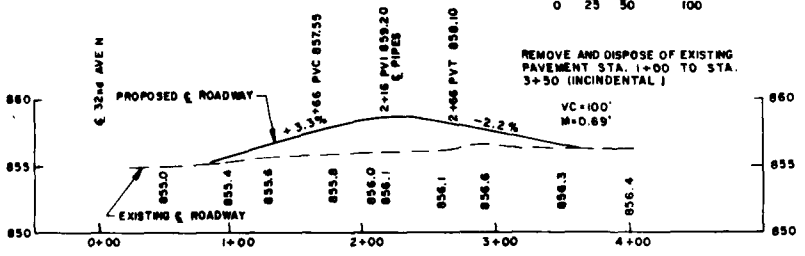
ST. PAUL DISTRICT, CORPS OF ENGINEERS

ST. PAUL, MINNESOTA

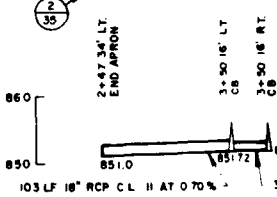
S.P.L.	PHASE II	DESIGN MEMORANDUM
	FLOOD CONTROL	BASSETT CREEK MINNESOTA
S.P.L.	MEDICINE LAKE OUTLET STRUCTURES	
	PLAN & DETAILS	
APPROVED:	DATE: AUGUST 1962	
	AS SHOWN	
	M34.3-R-5/217	
	SHEET 34 OF 44	

PLATE 34



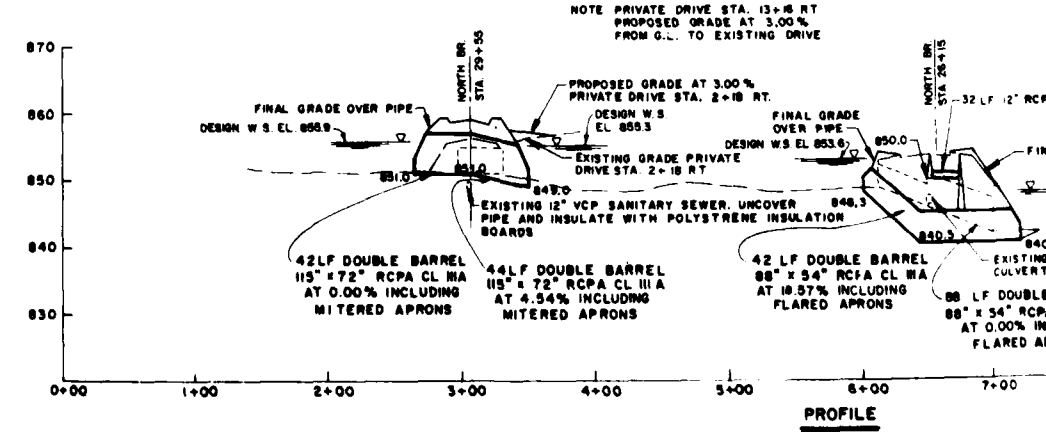


BRUNSWICK AVE N ROADWAY PROFILE



DETAIL "A"





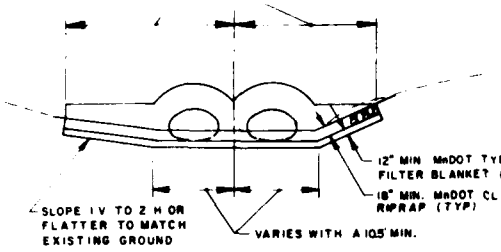
PROFILE

NOTE

DETAILED BORING LO

NOTE EACH PIPE BAR BEND SECTION OF THE BEND

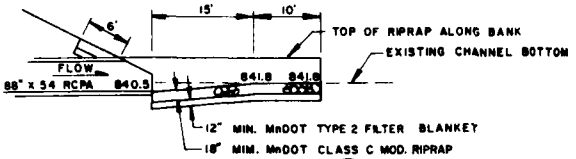
RIPRAP VARIES TO ELEV. 845.5 BUT DOES NOT EXCEED 25' MAX. ON EITHER SIDE



SECTION (1/35)

RIPRAP AT 32ND (OUTLET)

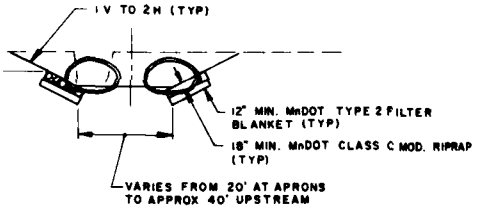
SCALE 1"=10'

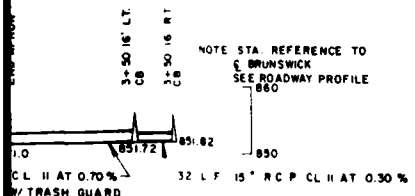


SECTION (2/35)

RIPRAP AT 32ND (OUTLET)

SCALE 1"=10'





DETAIL "A"

SECTION (3/35)

RIPRAP AT 32ND (INLET)

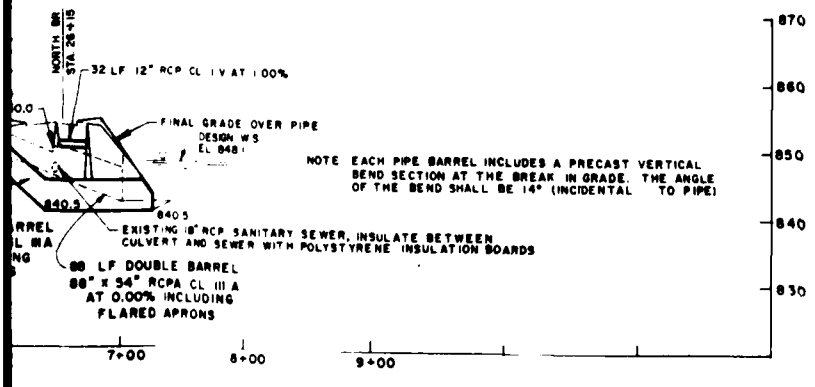
SCALE 1"=10'

10 5 0 10 20

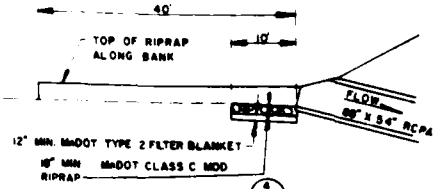
SCALE IN FEET

NOTE

DETAILED BORING LOGS SHOWN ON PLATE C-15



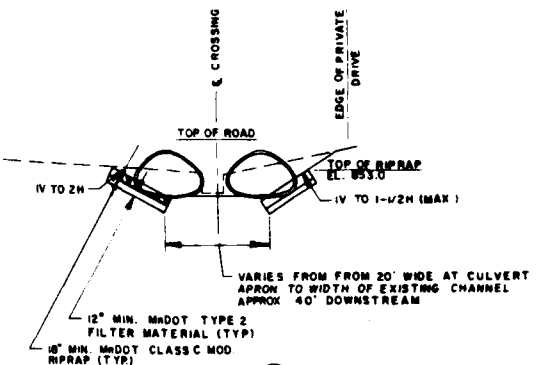
NOTE EACH PIPE BARREL INCLUDES A PRECAST VERTICAL BEND SECTION AT THE BREAK IN GRADE. THE ANGLE OF THE BEND SHALL BE 14° (INCIDENTAL TO PIPE)



SECTION (4/35)

RIPRAP AT 32ND (INLET)

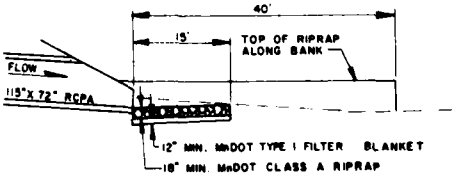
SCALE 1"=10'



SECTION (5/35)

RIPRAP AT BRUNSWICK (OUTLET)

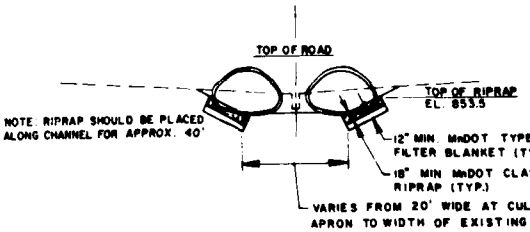
SCALE 1"=10'



SECTION (6/35)

RIPRAP AT BRUNSWICK (OUTLET)

SCALE 1"=10'



SECTION (7/35)

RIPRAP AT BRUNSWICK (INLET)

SCALE 1"=10'

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

Barr Eng Co.

PHASE II DESIGN MEMORANDUM

FLOOD CONTROL BASSETT CREEK MINNESOTA

CULVERT REPLACEMENT @ BRUNSWICK AND 32 AVES

J.G.J

L.A.R

DATE: AUGUST 1982

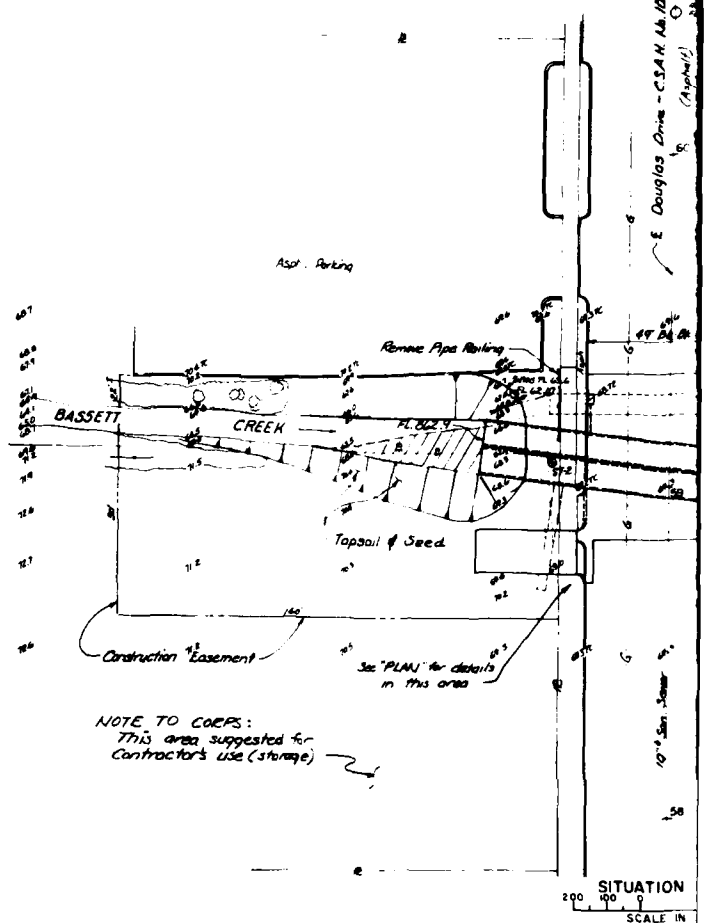
AS SHOWN

DRAWING NUMBER M34.3-R-5/218

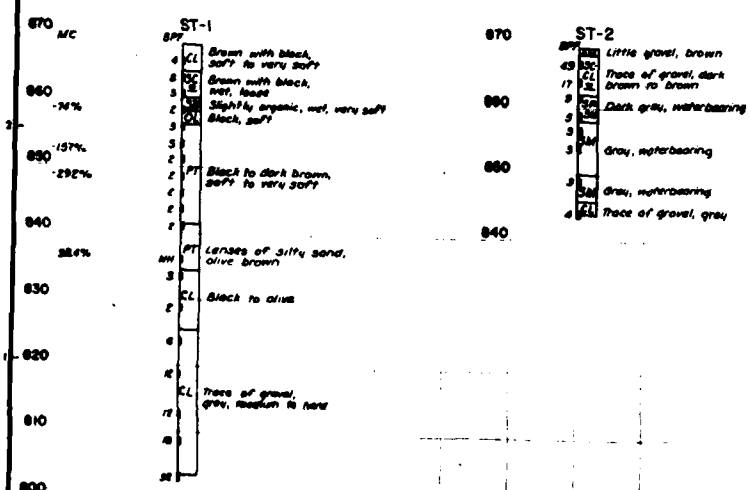
SHEET 35 OF 44

SAFETY PAYS

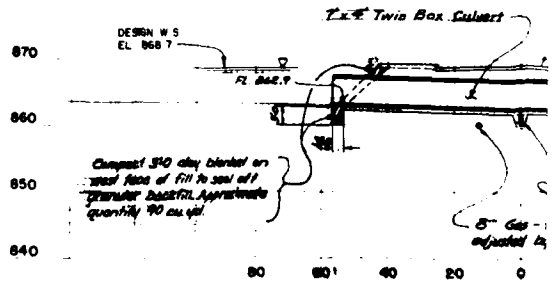
PLATE 35

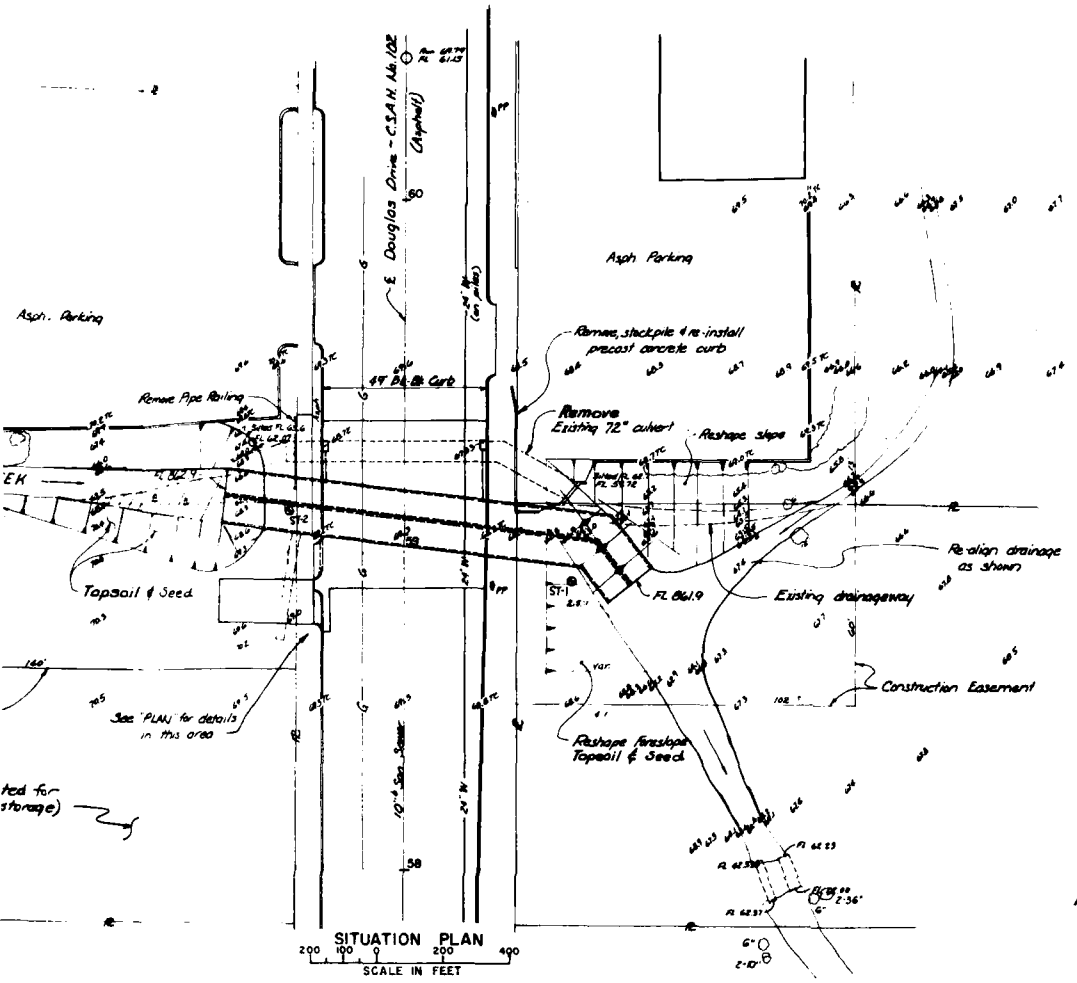


SOIL BORINGS



See report from Braun Engineering Testing dated March 19, 1982 for complete soil boring.





SITUATION PLAN

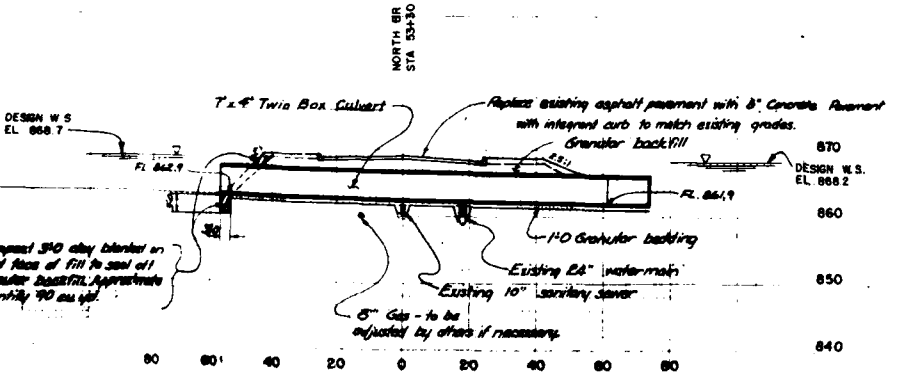
200 100 0 200 400

SCALE IN FEET

Bench Marks

S.W. corner of conc. base So. light std. Stally - 34th & Douglas - Elev. 891.62

Top nut hydrant N.E. corner 34th & Douglas - Elev. 894.87



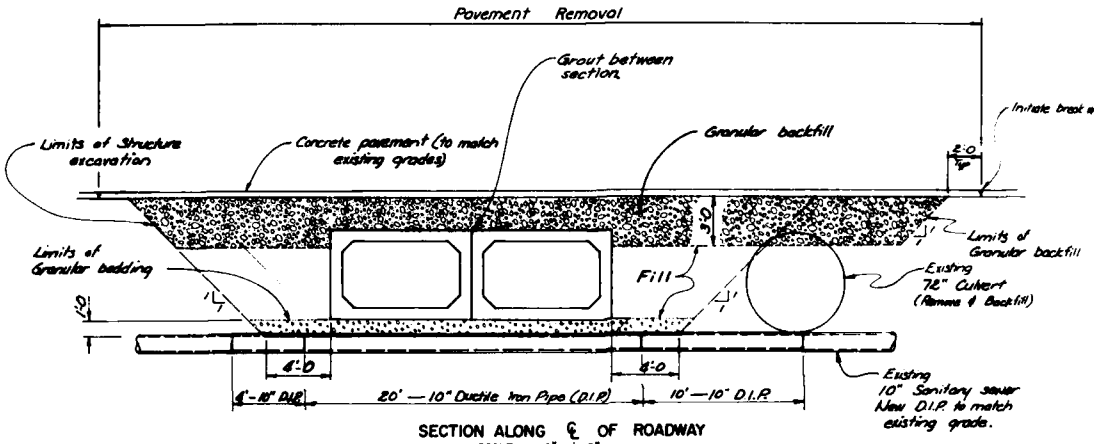
SECTION ON ℄ CULVERT

WALLACE HOLLAND KASTLER SCHMITZ AND COMPANY, ROCHESTER MINNESOTA		DEPARTMENT OF THE ARMY, ST. PAUL DISTRICT CORPS OF ENGINEERS, ST. PAUL MINNESOTA	
DESIGNED BY G.W.H. DRAWN BY D.C.L. J.M.J. CHECKED BY L.A.R. SUBMITTED BY Paul B. Gloth CHIEF, DESIGN BRANCH APPROVED Robert L. Hart CHIEF, ENGINEERING DIVISION	PHASE II FLOOD CONTROL BASSETT CREEK MINNESOTA	DESIGN MEMORANDUM CULVERT REPLACEMENTS AT DOUGLAS DRIVE PLAN, SOIL BORINGS AND SECTION	DATE AUGUST 1962 SCALE AS SHOWN DRAWING NUMBER M34.3-R-5/219 SHEET 36 OF 44

SAFETY PAYS

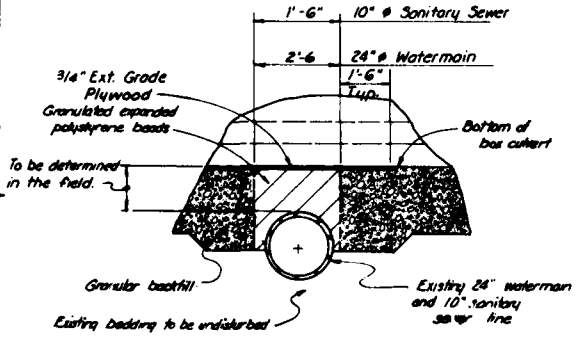
2

PLATE36



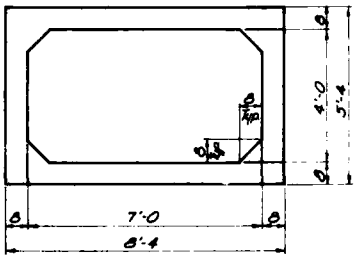
SECTION ALONG ℄ OF ROADWAY
SCALE 1/4" = 1'-0"

SCALE IN FEET



SECTIO[illegible]
SCALE 1/2 = 1'-0"

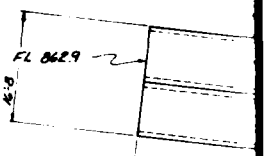
Section shown shall extend full width of culvert and a minimum of 3'-0" beyond each side.



TYPICAL PRECAST REINFORCED CONCRETE BOX CULVERT SECTION
SCALE 1/2" = 1'-0"

SCALE IN FEET

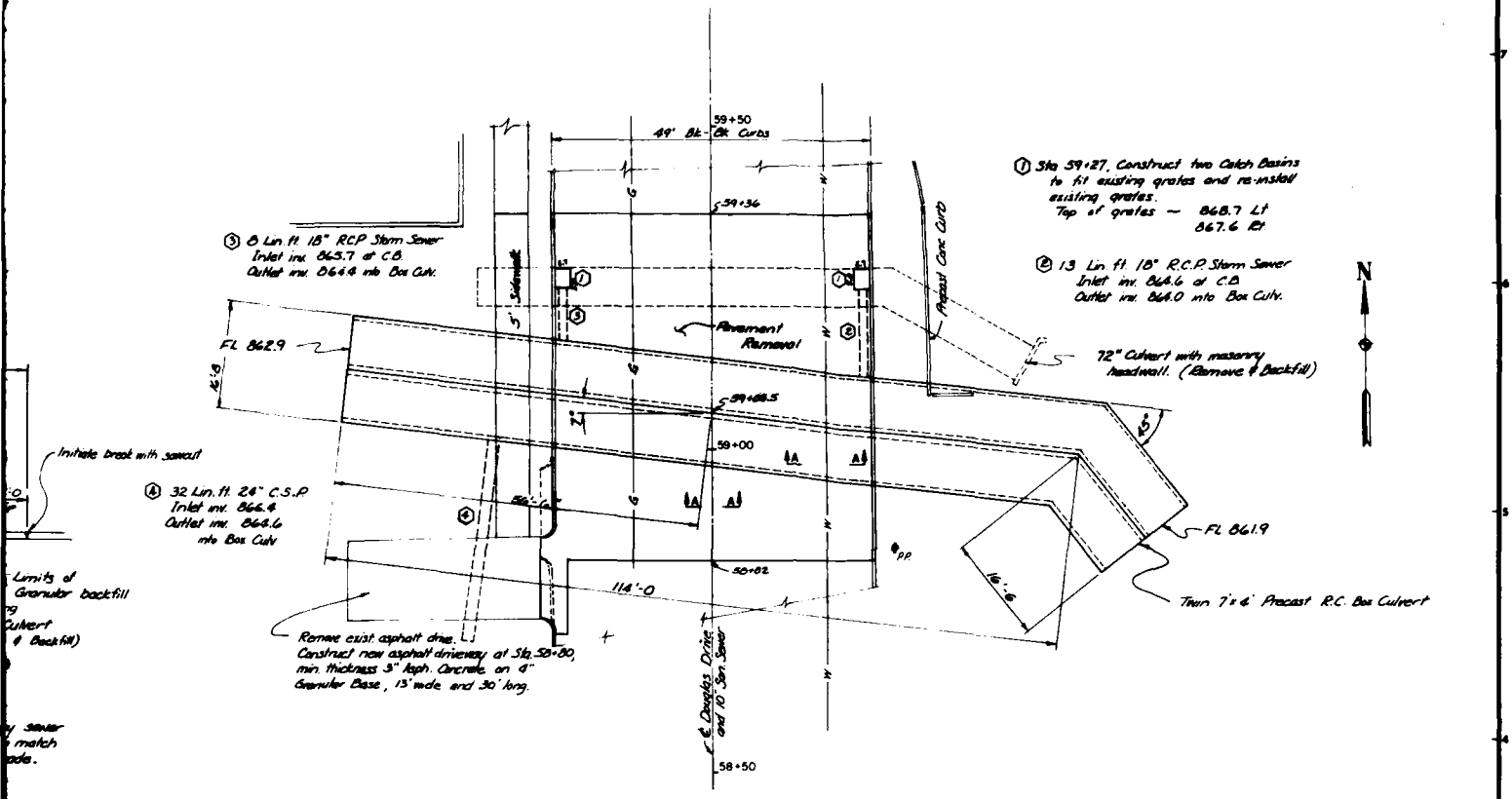
③ 8 Lin ft 18" RCP Storm Sewer
Inlet inv. 865.7 at C.B
Outlet inv. 864.6 into Box Culv



Initiate break with sawcut

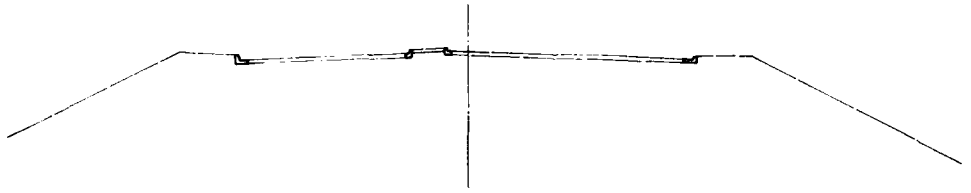
④ 32 Lin. ft. 24" C.S.P
Inlet inv. 866.4
Outlet inv. 864.6
into Box Culv

Remove exist asphalt drive
Construct new asphalt driv
min thickness 3" Asph Co
Granular Base, 13' wide a



PLAN
SCALE 1" = 10'

10 5 0 10 20 30
SCALE IN FEET



FUTURE ROADWAY SECTION
SCALE 1" = 10' HOR
1" = 5' VERT

10 5 0 10 20 30
HORIZONTAL SCALE IN FEET

5 0 5 10 15
VERTICAL SCALE IN FEET

SAFETY PAYS

SYMBOL	DESCRIPTION	DATE	APPROVAL

WALLACE HOLLAND KASTLER SCHMITZ AND COMPANY
ROCHESTER MINNESOTA

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

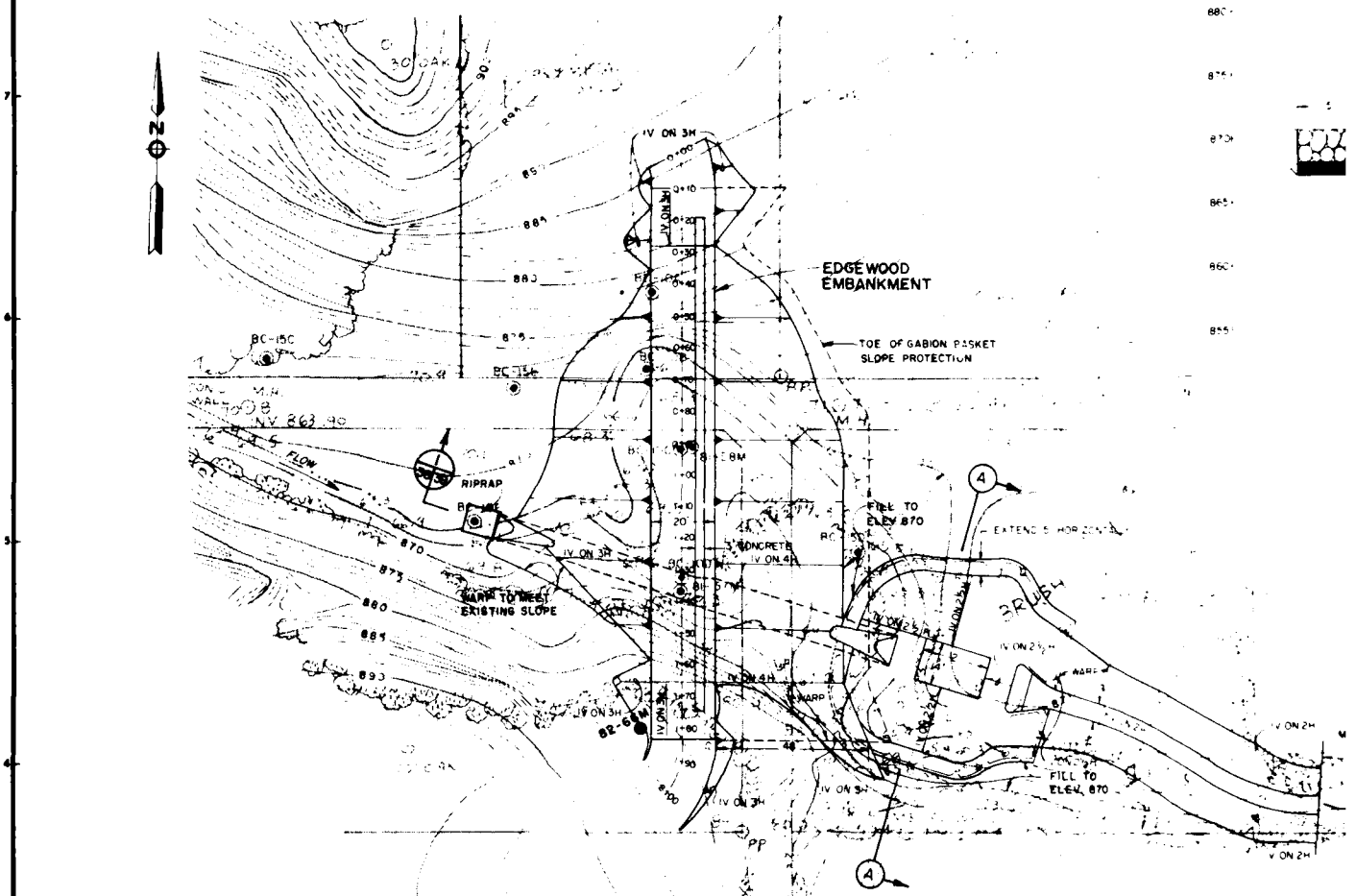
DESIGNED BY: GKH
DRAWN: RGL
CHECKED BY: LAR
SUBMITTED BY:
APPROVED:

PHASE II
DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
CULVERT REPLACEMENTS AT DOUGLAS DRIVE
DETAILS

DATE: AUGUST 1982

SCALE: AS SHOWN
SPEC. NO.
DRAWING NUMBER
M34.3-R-5/220
SHEET 37 OF 44

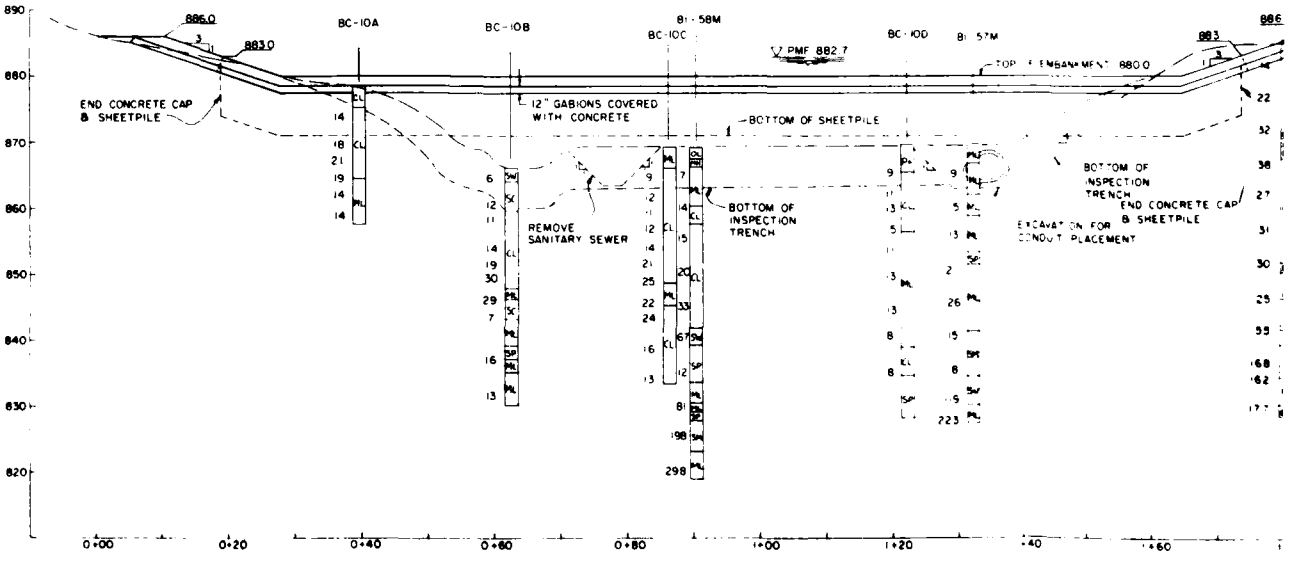
PLATE 37



PLAN

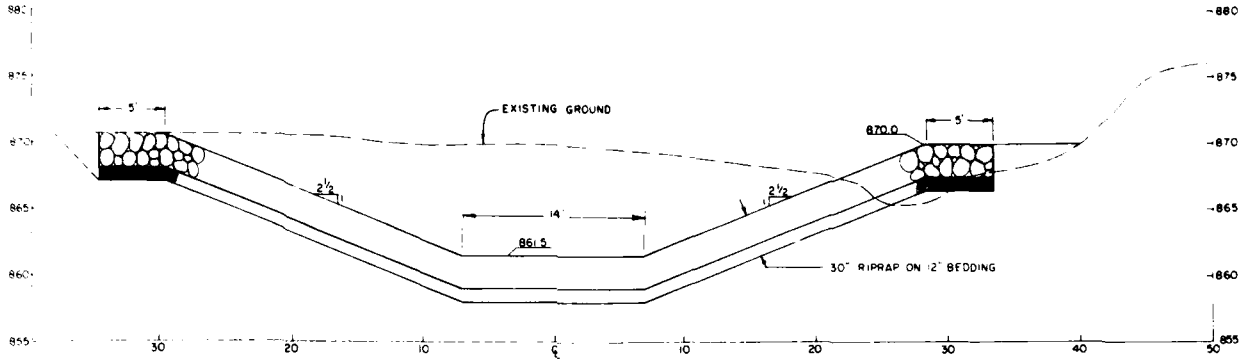
20 0 10 20

SCALE IN FEET

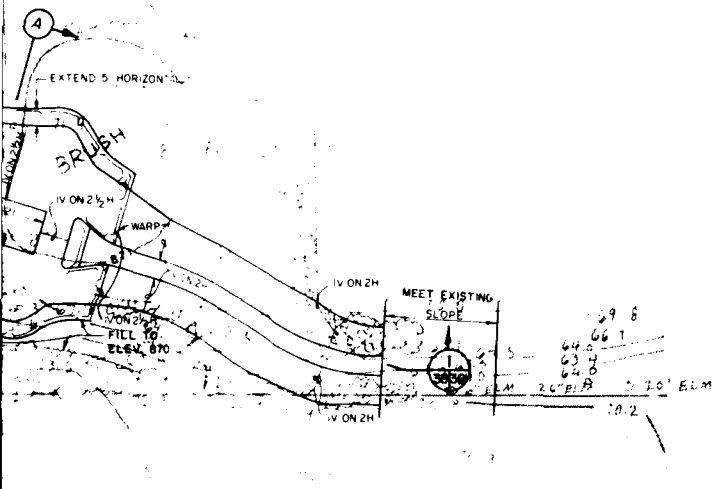


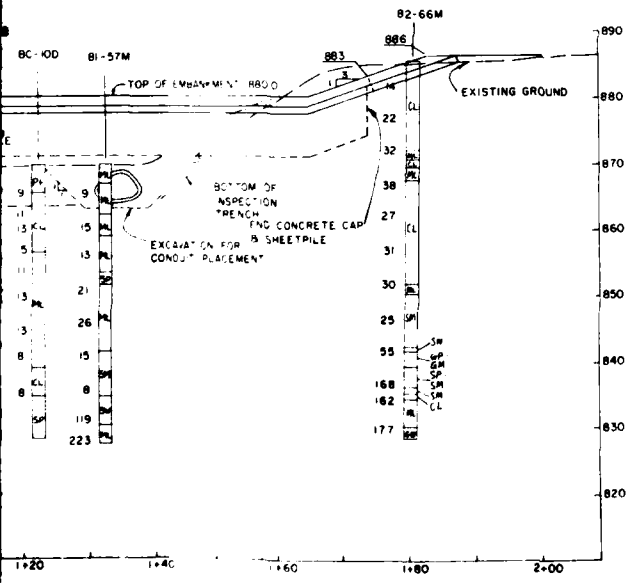
PROFILE

NORTH BR STA 57+80



SECTION A





NOTE:
SEE SHEET C-16 FOR
DETAIL BORING LOGS.

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

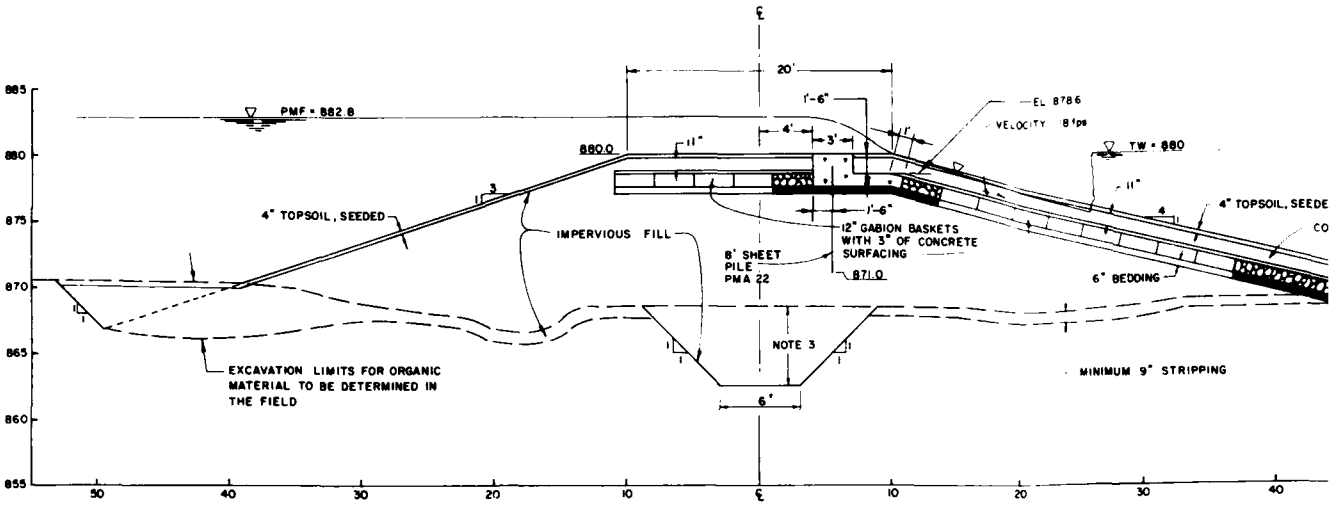
DR. PMF
JGJ
M B

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
EDGEWOOD EMBANKMENT
GENERAL PLAN & PROFILE
DATE: AUGUST 1982

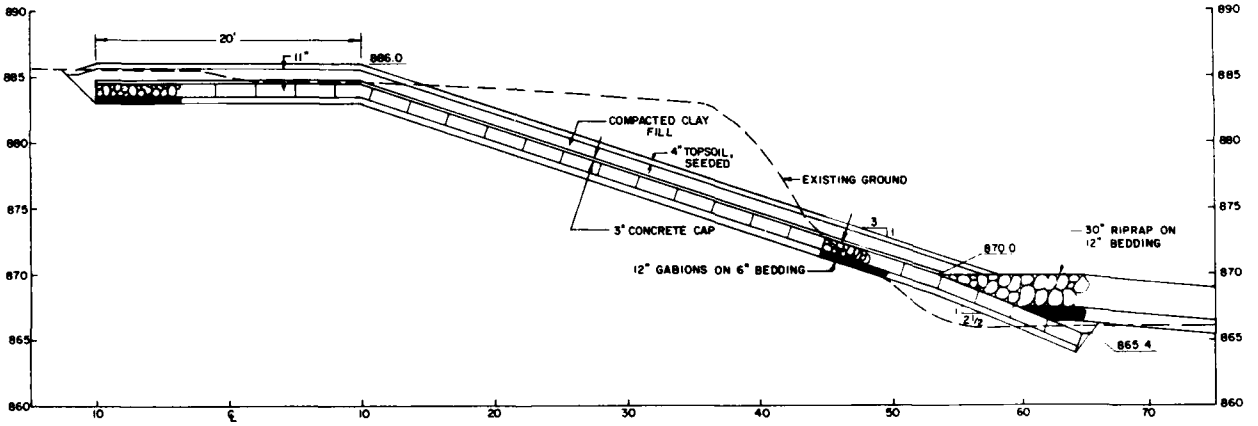
AS SHOWN

DRAWING NUMBER
M34.3-R-5/22
SHEET 38 OF 44

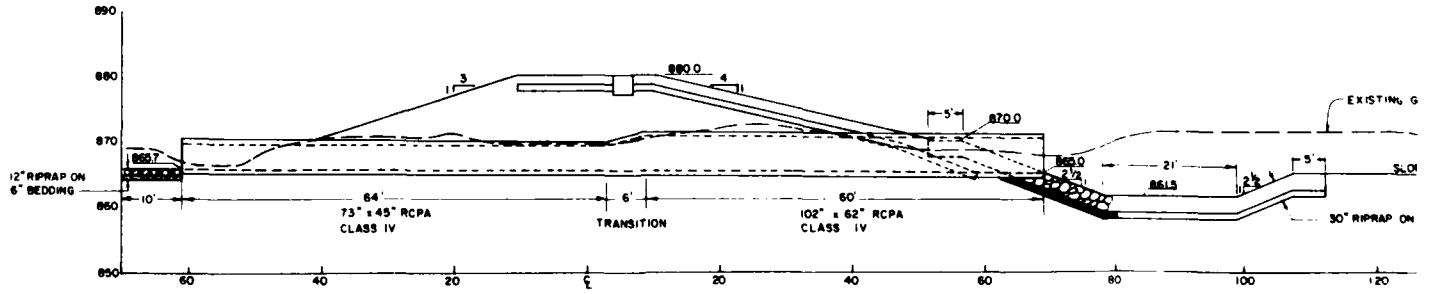
PLATE 38



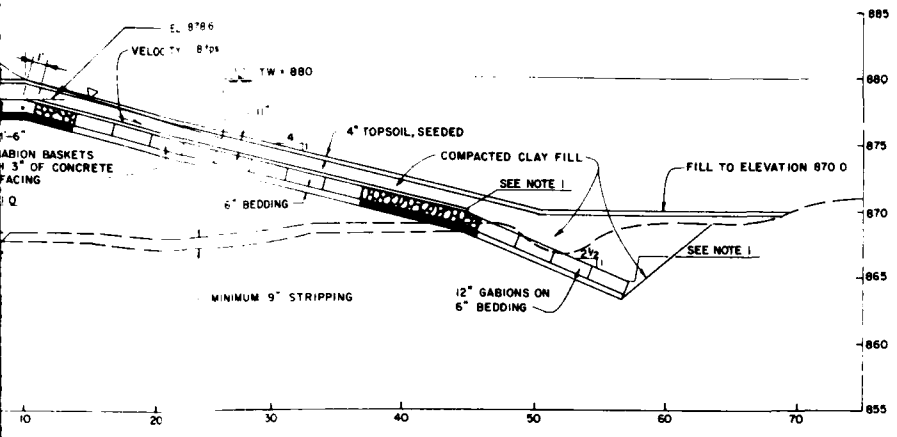
TYPICAL SECTION (Sta 0+28 to Sta 1+66)



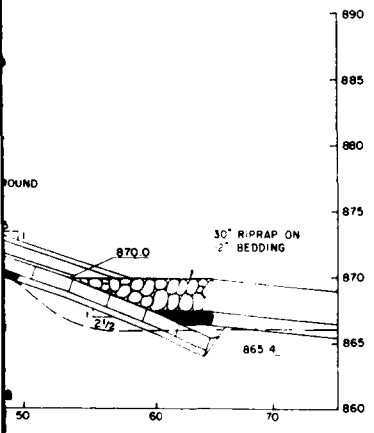
STATION 1+83

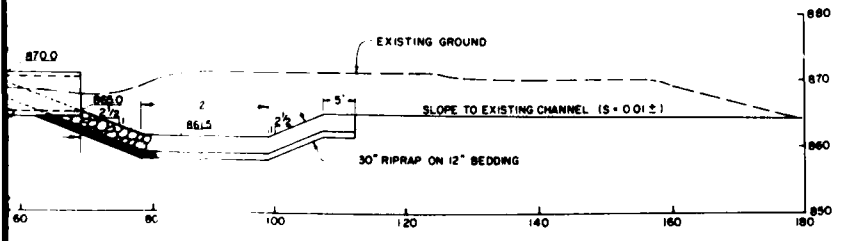


CONDUIT PROFILE



0+28 to Sta 1+66)





NOTES:

1. FROM STA 0+10 TO STA 0+80, THE GABIONS SLOPE AT IV ON 4H TO THE EXISTING GROUND SURFACE, THEN IV ON 2 1/2 H TO A DEPTH OF 5' BELOW THE GROUND SURFACE. FROM STA 0+80 TO STA 1+66 THE GABIONS SLOPE AT IV ON 4H TO ELEVATION 870, THEN AT IV ON 2 1/2 H TO ELEVATION 864.4. AT STA 1+83 THE GABIONS SLOPE AT IV ON 3H TO ELEVATION 870, THEN AT IV ON 2 1/2 H TO ELEVATION 865.4 BETWEEN STA 1+66 AND STA 1+83 THE SLOPE IS WARPED.
2. AN EROSION-RESISTANT CLAY HAVING A LIQUID LIMIT GREATER THAN 30 AND A PLASTICITY INDEX GREATER THAN 15 WILL BE PLACED IN THE ZONE ABOVE THE GABION BASKETS.
3. WHEN THE EMBANKMENT IS GREATER THAN SIX FEET IN HEIGHT, THE DEPTH OF THE INSPECTION TRENCH IS SIX FEET. WHEN THE EMBANKMENT IS LESS THAN SIX FEET IN HEIGHT, THE DEPTH OF THE INSPECTION TRENCH EQUALS THE HEIGHT OF THE EMBANKMENT.
4. PIPE BEDDING SHALL BE FIRST CLASS BEDDING.

SAFETY PAYS

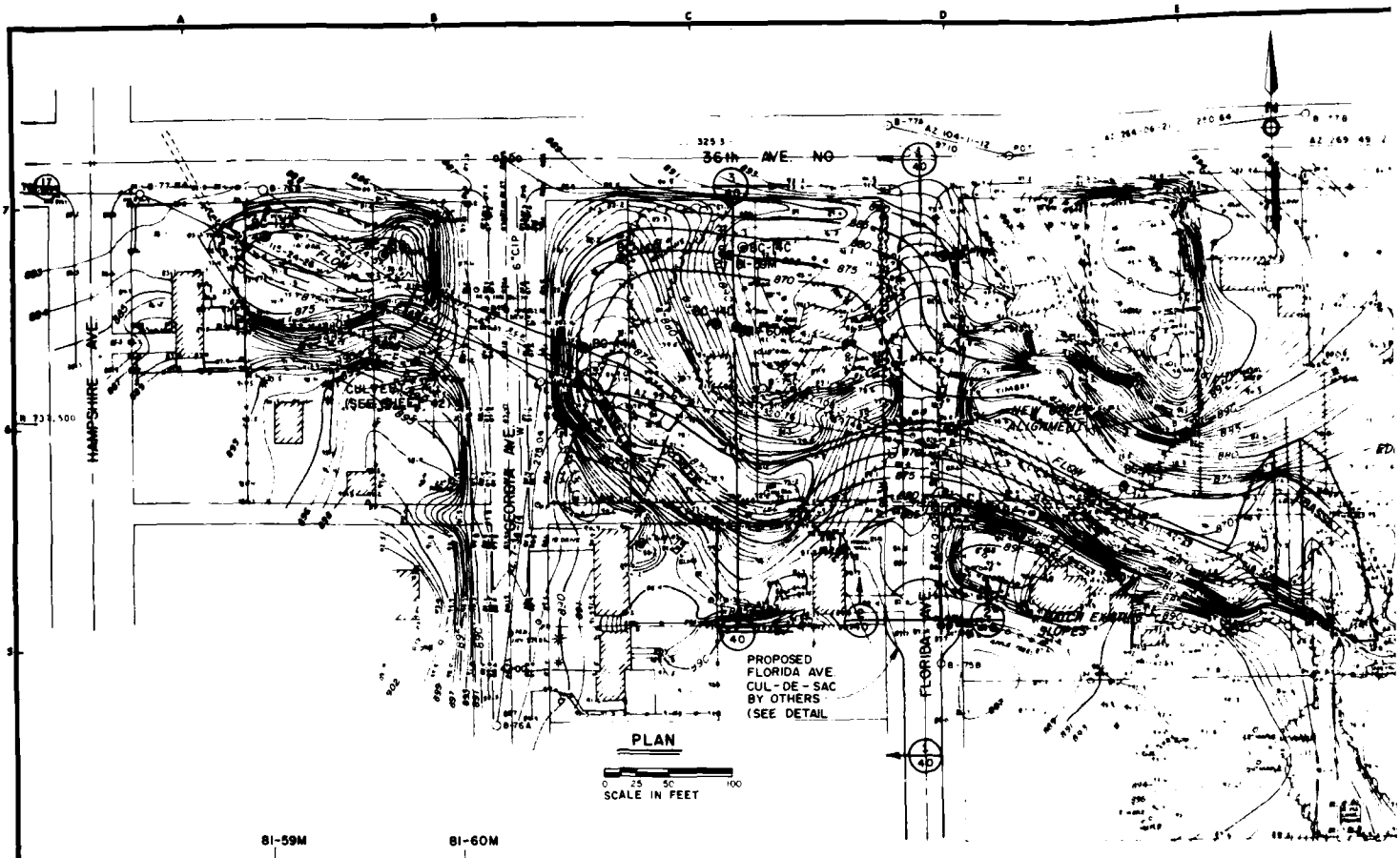
DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

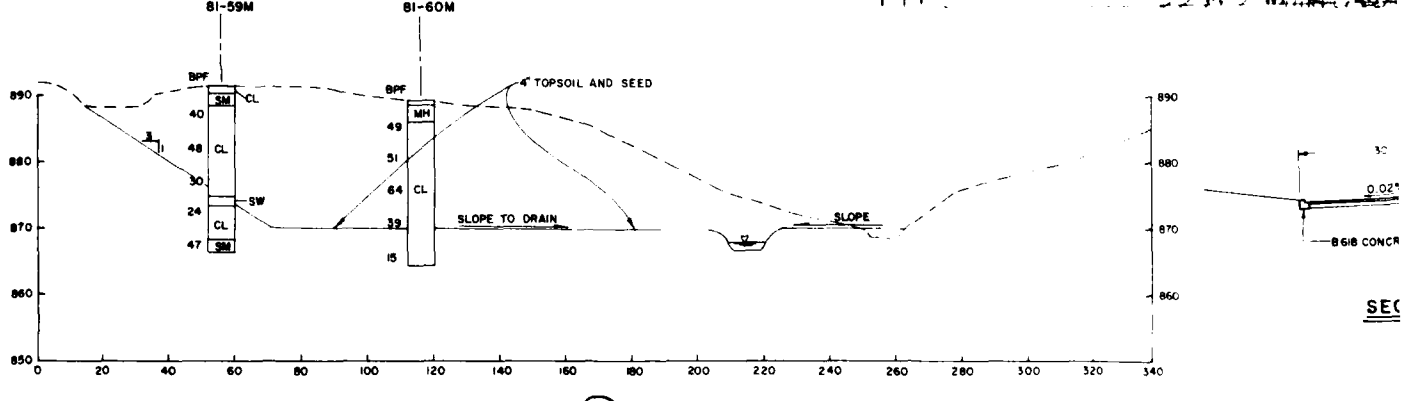
DR, PMF
JGJ
MB

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
EDGEWOOD EMBANKMENT
EMBANKMENT DETAILS

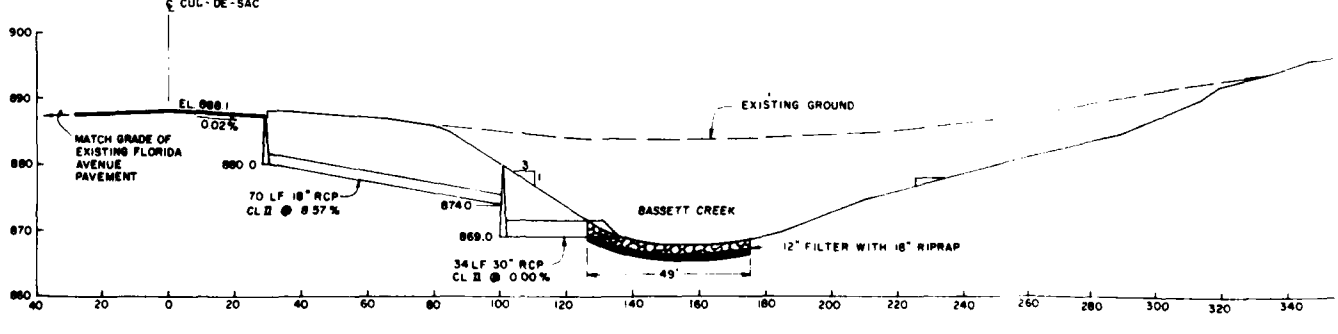
DATE: AUGUST 1962
SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/222
SHEET 39 OF 44

PLATE 39

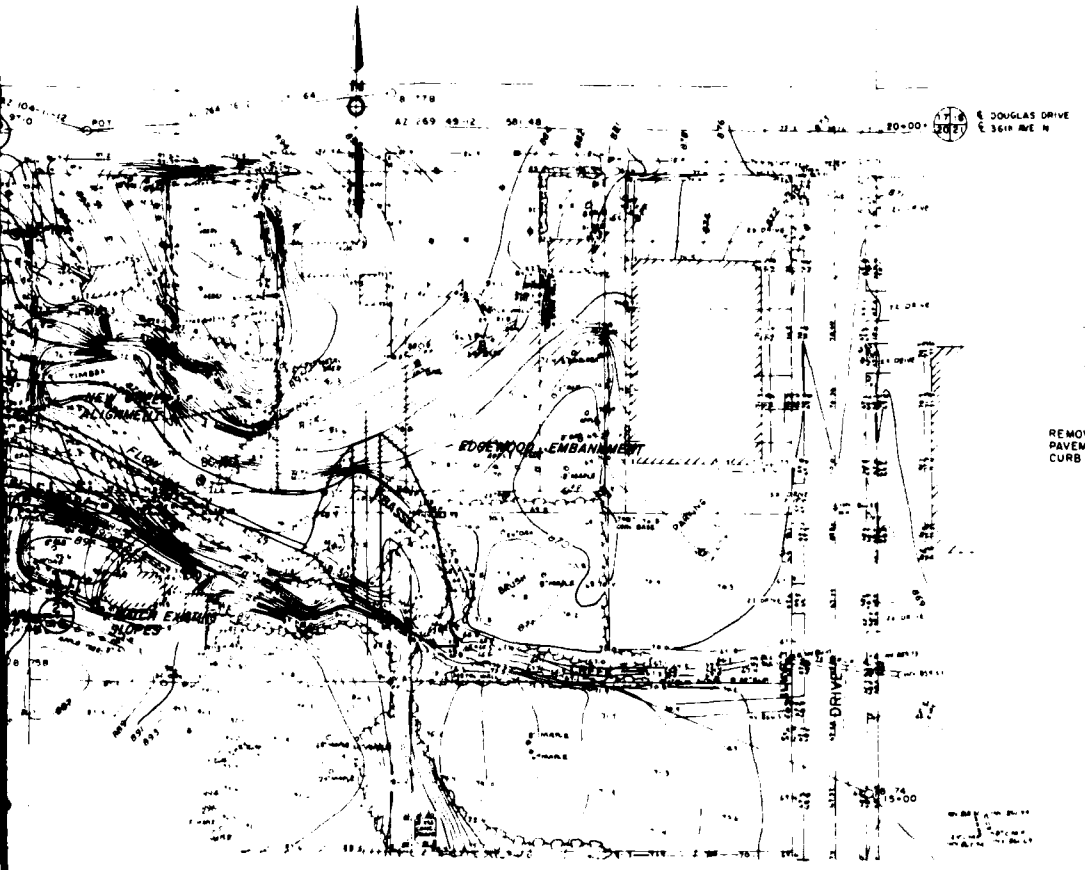


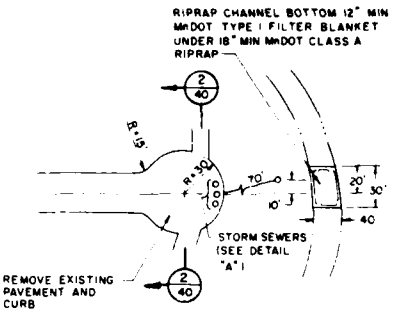


SECTION (3/40)



SECTION (1/40)

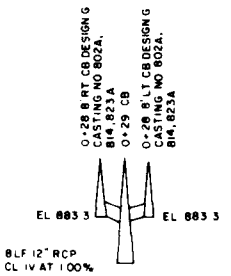




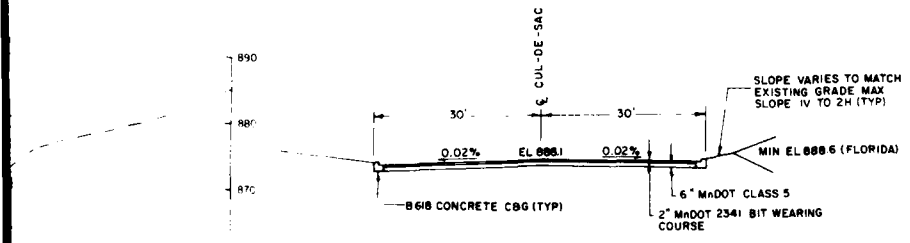
FLORIDA AVE. CUL-DE-SAC

PLAN VIEW

NO SCALE

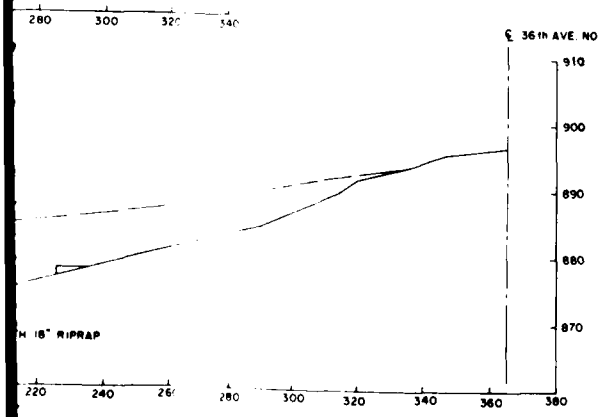


DETAIL "A"



SECTION (2/40)

NOTE: SEE SHEET C-16 FOR BORING LOGS



SAFETY PAYS

SYMBOL	DESCRIPTION	DATE	APPROVAL

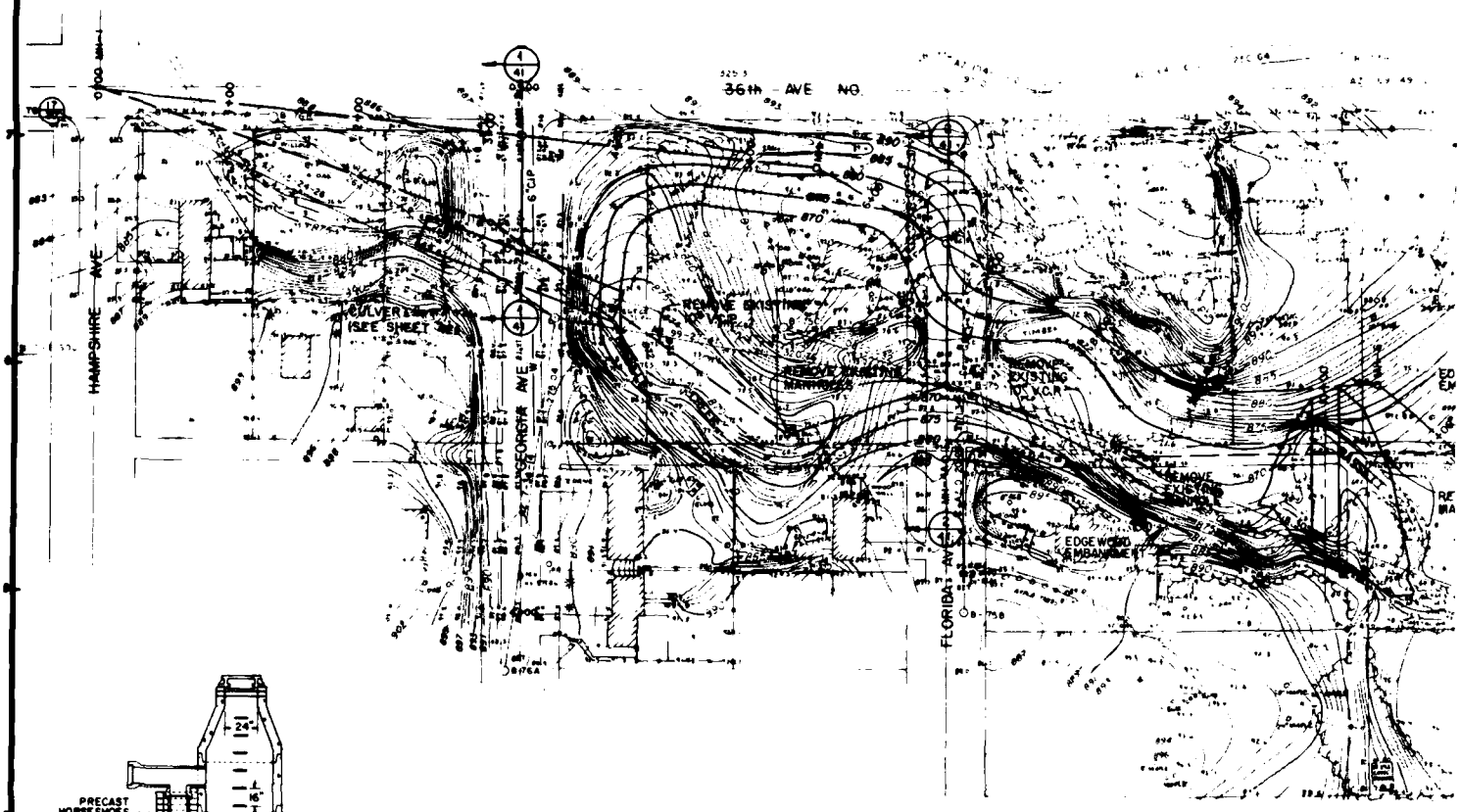
DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

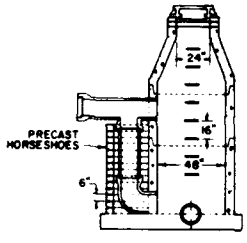
DESIGNED BY: L A R DWR
DRAWN BY: JGJ, J M J
CHECKED BY: L.A.R.
SUBMITTED BY:
APPROVED:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
EDGEWOOD STORAGE AREA
EXCAVATION

DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER
M34 3-R-5/223
SHEET 40 OF 44

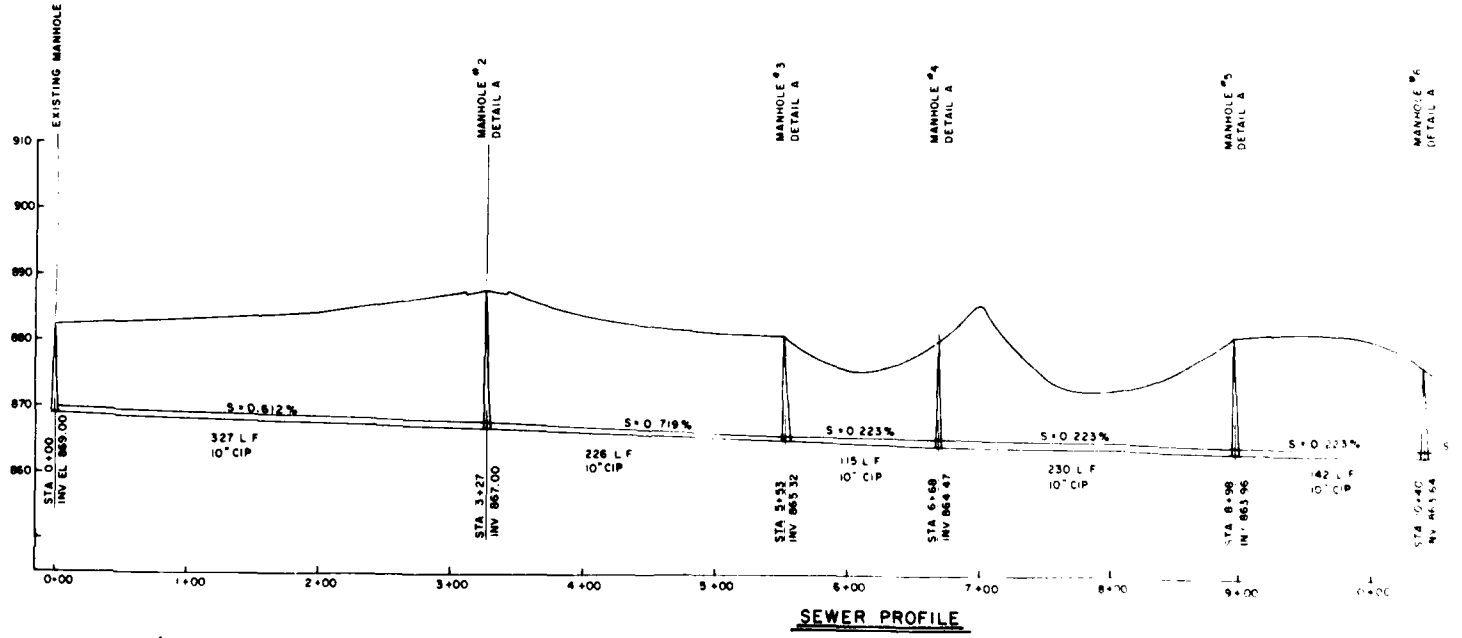
PLATE 40



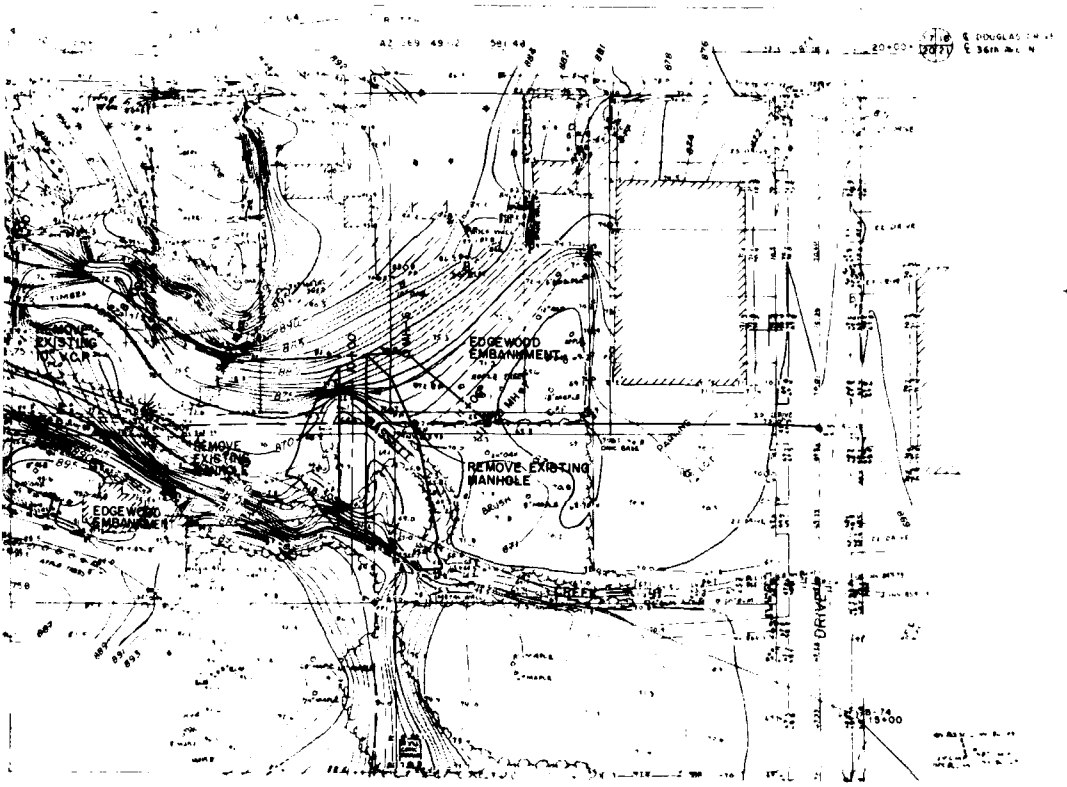


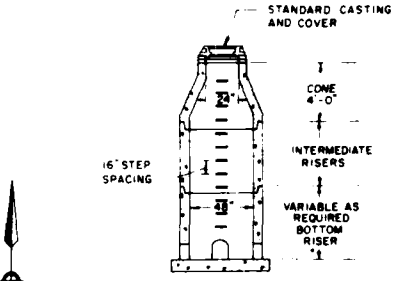
DROP INLET MANHOLE

DETAIL B



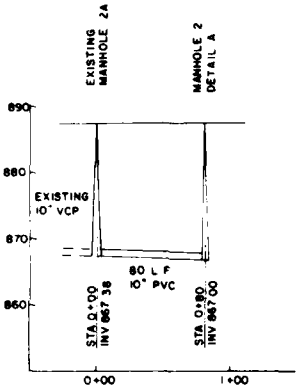
SEWER PROFILE



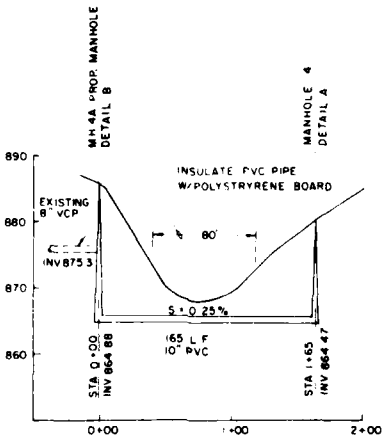


CONCENTRIC MANHOLE

DETAIL A



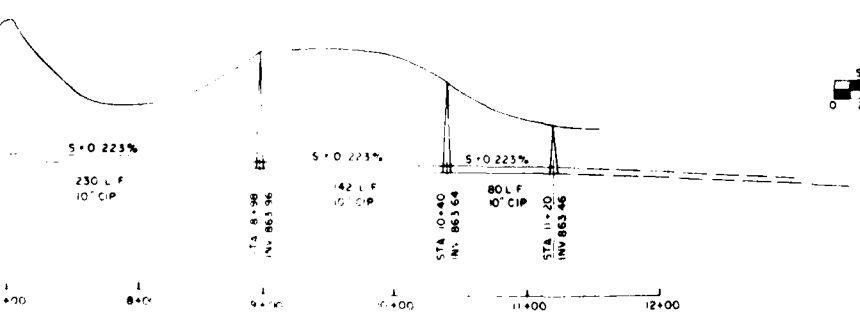
SECTION (1/41)



SECTION (2/41)

NOTES

1. ALL SANITARY SEWER MANHOLES SHALL BE CONSTRUCTED OF PRECAST CONCRETE SECTIONS
2. ALL JOINTS SHALL BE MADE WATER TIGHT BY USE OF BITUMASTIC JOINT COMPOUND
3. ALL INVERTS OF EXISTING SANITARY LINES SHALL BE VERIFIED PRIOR TO CONSTRUCTION



SCALE IN FEET
0 25 50 100

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY	L A R
DRAWN BY	J G J
TRACED BY	L A R
SUBMITTED BY	
APPROVED	

PHASE II — DESIGN MEMORANDUM

FLOOD CONTROL BASSETT CREEK MINNESOTA

EDGEWOOD STORAGE AREA

UTILITIES

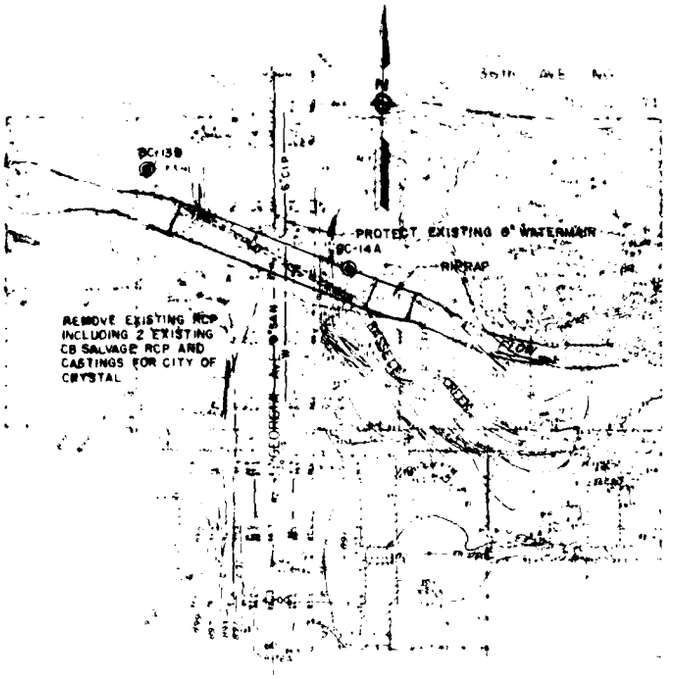
DATE: AUGUST 1982

AS SHOWN

DRAWING NUMBER M34.3-R-5/224

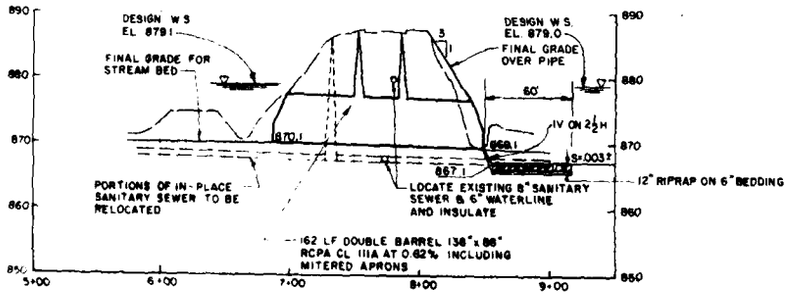
SHEET 41 OF 44

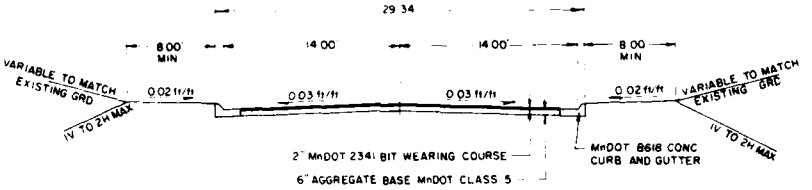
PLATE 41



NOTE
SEE SHEET C-15
FOR DETAILED BORING LOGS

6+88 END APRONS
7+01 END CULVERTS
7+53 7' LT CB DESIGN J
7+73 & GEORGIA NORTH BR STA 66+90
7+86 7' LT CB DESIGN J
8+37 END CULVERTS
8+50 END APRONS





GEORGIA AVE.
ROADWAY SECTION
NO SCALE

15
BORING LOGS

SYMBOL	DESCRIPTION	DATE	APPROVED

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY: BARR ENG CO
DRAWN BY: JGJ
CHECKED BY: LAR
SUBMITTED BY:
APPROVED:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

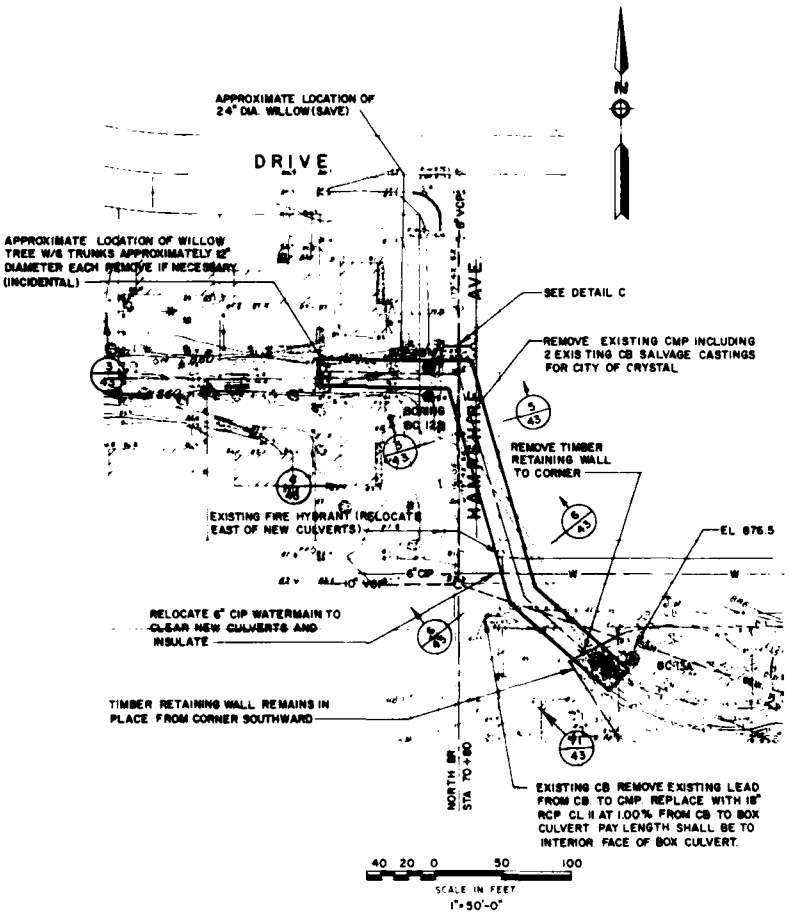
CULVERT REPLACEMENT
AT GEORGIA AVE

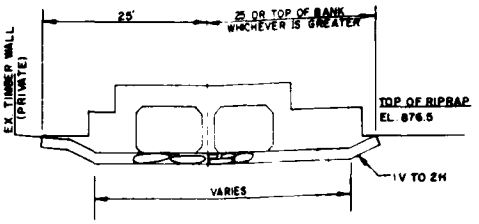
DATE: AUGUST 1982

SCALE: AS SHOWN
DRAWING NUMBER: 173-3-3-5/226
SHEET 42 OF 44

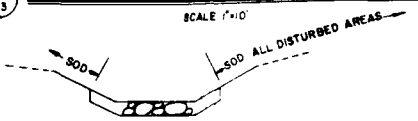
SAFETY PAYS

PLATE 42

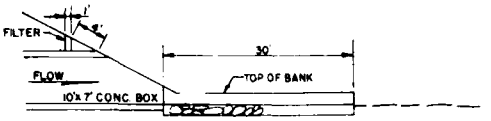




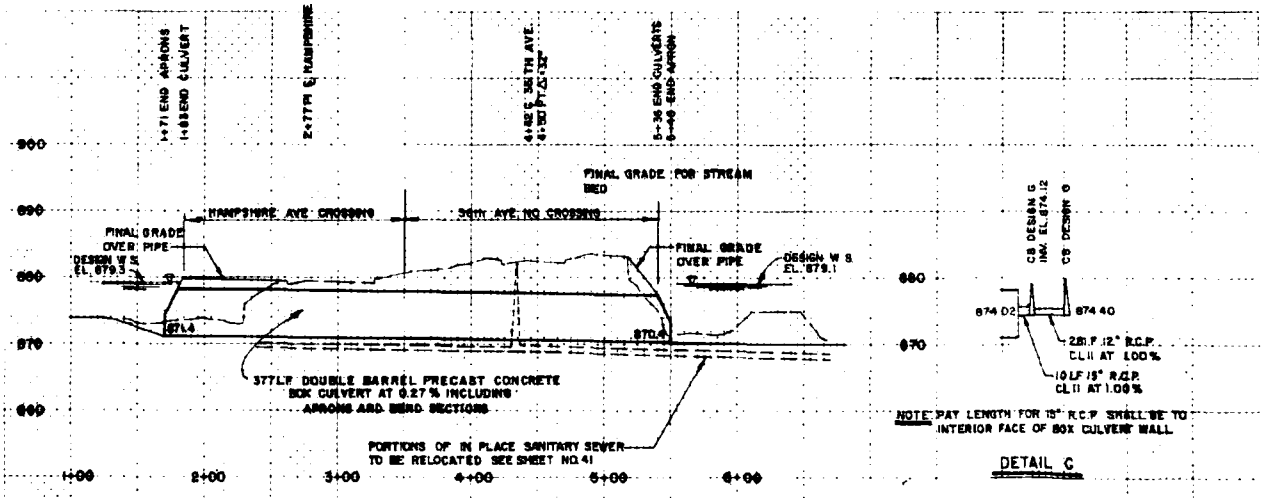
(11/43) SECTION: RIPRAP AT 36TH - HAMPSHIRE (OUTLET)
SCALE 1"=10'



(4/43) TYPICAL SECTION: STATION 1+05 TO STATION 1+70
SCALE 1"=10'

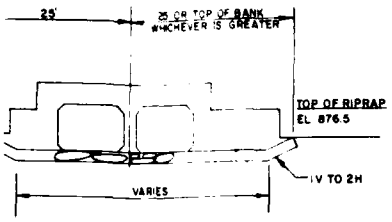


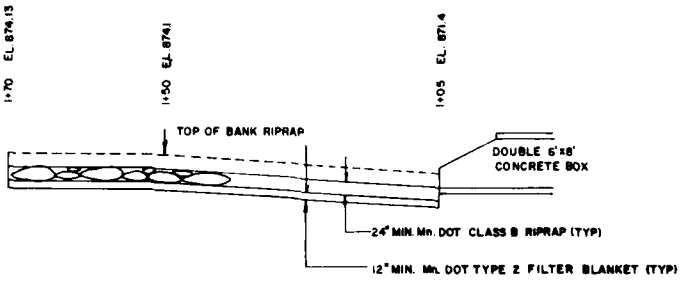
(12/43) SECTION: RIPRAP AT 36TH - HAMPSHIRE (OUTLET)
SCALE 1"=10'



NOTE: PAY LENGTH FOR 15" R.C.P. SHALL BE TO INTERIOR FACE OF BOX CULVERT WALL

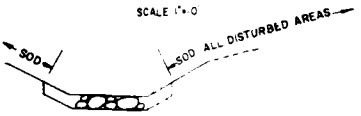
DETAIL C





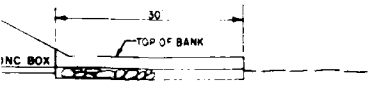
SECTION: RIPRAP AT 36TH- HAMPSHIRE (OUTLET)

SCALE 1"=10



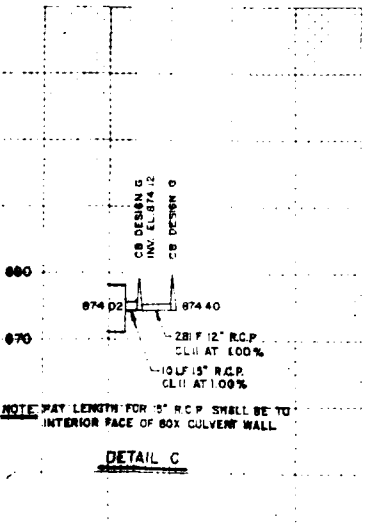
SECTION: STATION 1+05 TO STATION 1+70

SCALE 1"=10



SECTION: RIPRAP AT 36TH-HAMPSHIRE (OUTLET)

SCALE 1"=10'

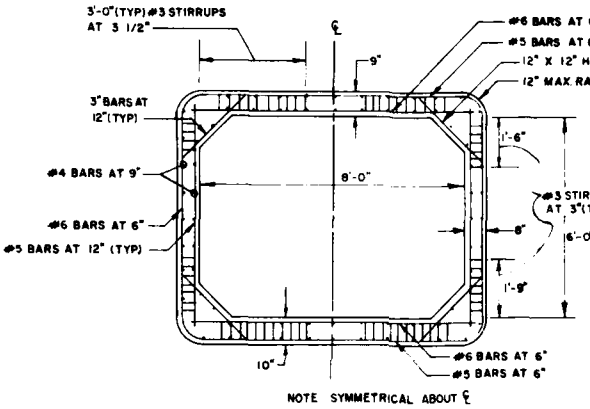


NOTE: PAY LENGTH FOR 15" R.C.P. SHALL BE TO INTERIOR FACE OF BOX CULVERT WALL

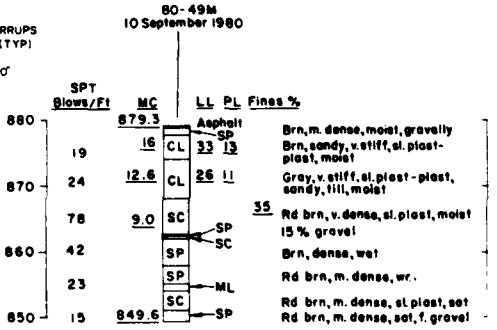
DETAIL C

(3/43) SECTION: STATION 1+05 TO STATION 1+70



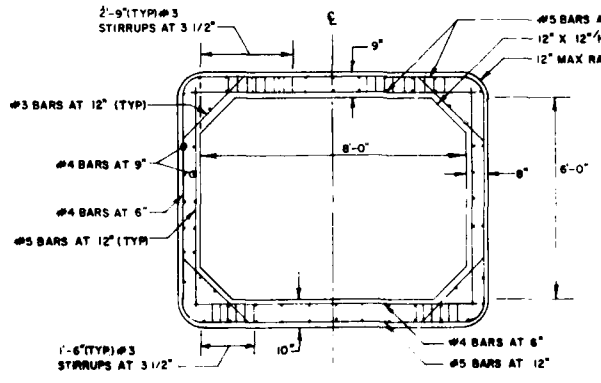


NOTE SYMMETRICAL ABOUT ℄

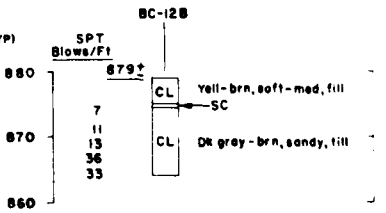


(6/43) SECTION: 36TH AVE. BOX CULVERT

SCALE 1"=2'



NOTE SYMMETRICAL ABOUT ℄



(5/43) SECTION: HAMPSHIRE AVE BOX CULVERT

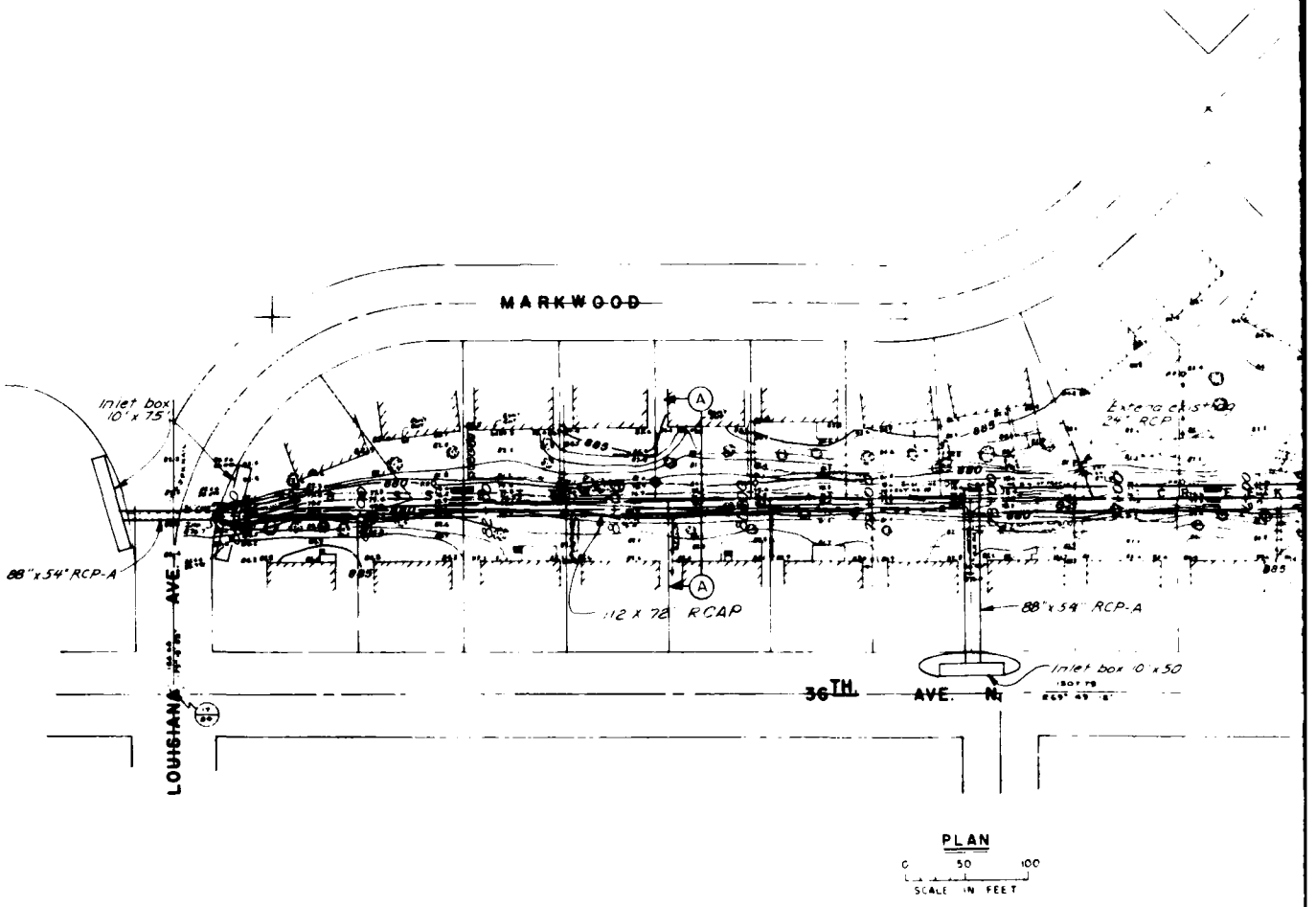


DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY: BARR ENG CO	PHASE II — DESIGN MEMORANDUM
DRAWN BY: J.M.J	FLOOD CONTROL BASSETT CREEK MINNESOTA
CHECKED BY: L.A.R	CULVERT REPLACEMENT @ 36TH AVE
SUBMITTED BY: [signature]	DATE: AUGUST 1982
APPROVED: [signature]	

DRAWING NUMBER: M34.3-R-5/227
SHEET 43 OF 44

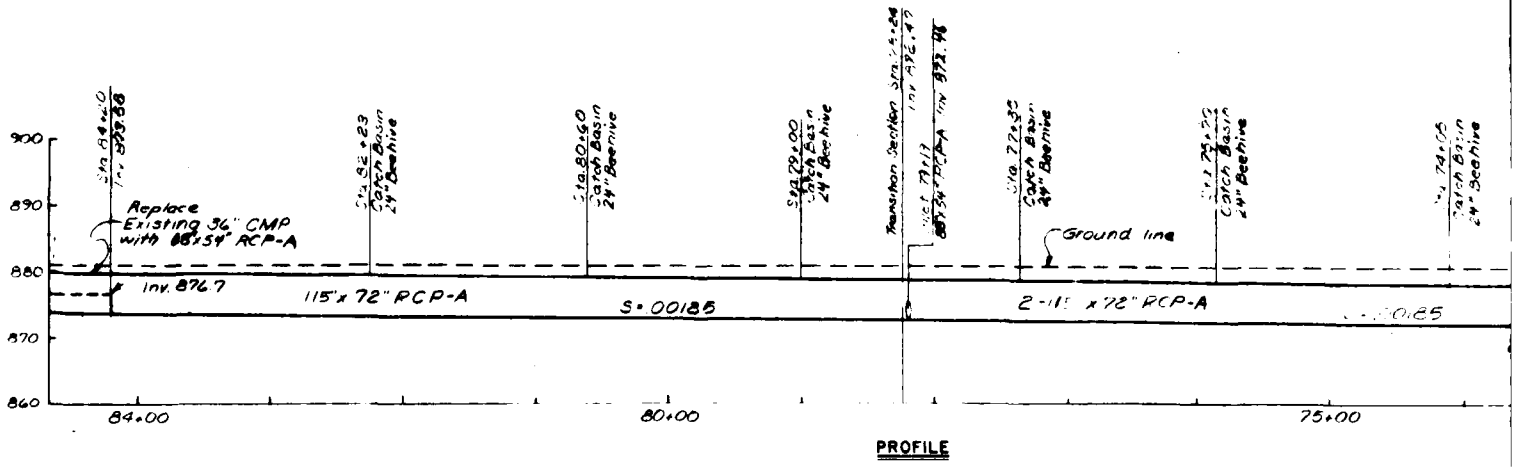
PLATE 43



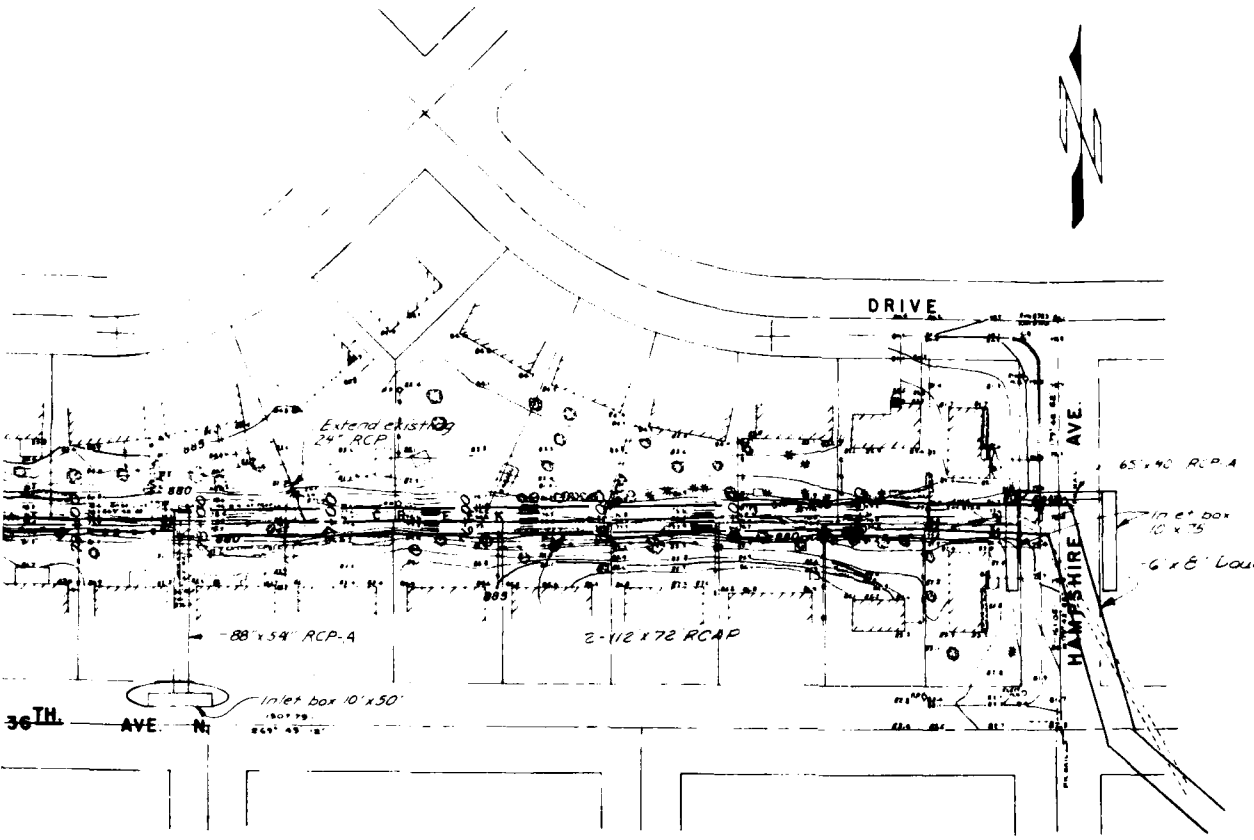
PLAN

0 50 100

SCALE IN FEET

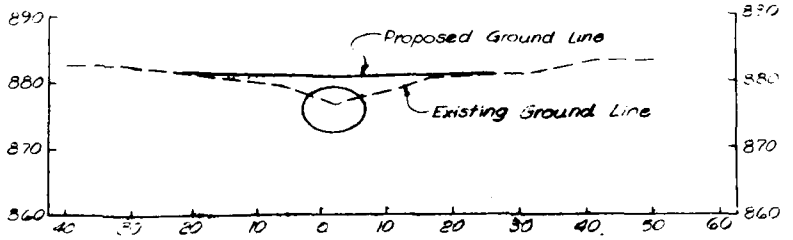


PROFILE

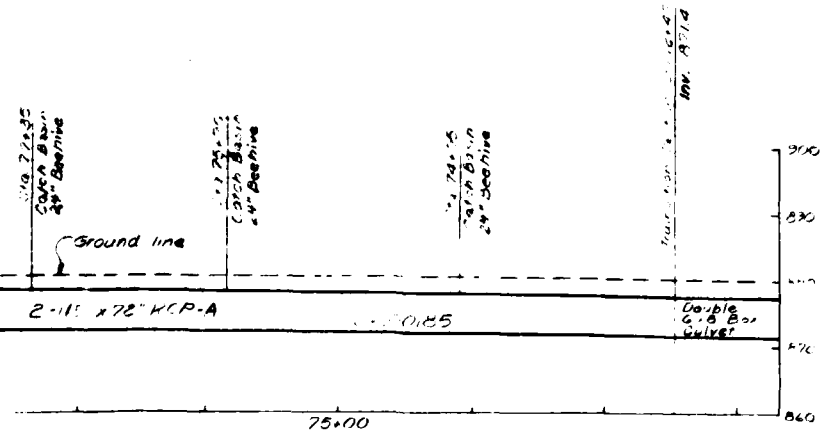


PLAN

0 50 100
SCALE IN FEET



SECTION A-A



DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
MARKWOOD AREA
CHANNEL IMPROVEMENTS
DATE: AUGUST 1982

DRAWING NUMBER
M343-R-5/228
SHEET 44 OF 44

SAFETY PAYS

PLATE 44

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
1135 U S POST OFFICE & CUSTOM HOUSE
ST PAUL, MINNESOTA 55101

REPLY TO
ATTENTION OF:

NCSPD-ER

FINDING OF NO SIGNIFICANT IMPACT

In accordance with the National Environmental Policy Act of 1969, the St. Paul District has assessed the environmental impacts of the following action:

FLOOD CONTROL
BASSETT CREEK WATERSHED
HENNEPIN COUNTY
MINNESOTA

The proposed action incorporates several design modifications from 2nd Street North to Cedar Lake Road and at the Fruen Mill and Glenwood-Inglewood facilities, the Trunk Highway 100 flood control structure, and the Medicine Lake outlet structure. The proposed actions would have no significant impact on the social, cultural, and natural resources within the project area.

The environmental review process indicates that the proposed action does not constitute a major Federal action with significant effects on the human environment. The cumulative effects of the proposed actions are not considered significant, and their overall effects would be less than the impacts identified in the final EIS. Therefore, an environmental assessment and finding of no significant impact have been prepared instead of a supplement to the final environmental impact statement for the Bassett Creek flood control project.

_____
Date

EDWARD G. RAPP
Colonel, Corps of Engineers
District Engineer

ENVIRONMENTAL ASSESSMENT

FLOOD CONTROL

BASSETT CREEK WATERSHED

HENNEPIN COUNTY

MINNESOTA

St. Paul District, Corps of Engineers

August 1982

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ENVIRONMENT ASSESSMENT
FLOOD CONTROL
BASSETT CREEK WATERSHED
HENNEPIN COUNTY
MINNESOTA

1.00 Need for and Objective of Action

1.01 Introduction - This assessment considers design layout modifications plus feature additions and deletions made to a number of project components after completion of the feasibility report in March 1976 and the final environmental impact statement (FEIS) in March 1977. These changes are a direct result of more detailed hydrologic studies and the need to lessen social disruptions. The hydrologic studies indicated that the overall scope of the proposed project could be reduced (hence lowering costs and construction activities) while maintaining sufficient flood protection. Disposal of excavated materials (including quantity, potential sites, and effects on the natural environment) is also considered in this assessment.

1.02 During Phase II studies, preparation of a supplement to the FEIS was considered. According to the Council on Environmental Quality guidelines (40 CFR 1502.9(c)), a supplement to an FEIS must be prepared if the agency makes substantial changes relevant to environmental concerns, or if significant new circumstances or information relevant to environmental concerns bear on the proposed action or its impacts. Following a detailed review and analysis of all relevant factors associated with the proposed action, the St. Paul District determined that the proposed design modifications are not substantial changes relevant to environmental concerns and that they are not significant new circumstances or information relevant to environmental concerns. Therefore, preparing an environmental assessment and finding of no significant impact is the most appropriate way to address the impacts of these design changes.

1.03 Study Authority - Authorization for the proposed project is provided by resolutions adopted by the Committee on Public Works of the United States Senate and the House of Representatives, through a review of House Document No. 669, 76th Congress, 3rd Session. The project is further authorized by the Water Resources Development Act of 1976 (Public Law 94-587).

1.04 Project Purpose - The proposed action is intended to reduce the overall scope, costs, and social disruptions of the project while maintaining its overall integrity.

2.00 Alternative Features

2.01 Project Deletions - The following features were deleted from the project after completion of the feasibility report:

1. Approximately 3,000 feet of clearing and snagging from miles 1.52 to 1.85 on the main stem.*

2. Repairs to the upstream wing walls on the Cedar Lake Road bridge.

3. Construction of a 1,020-foot floodwall and channel widening downstream from Glenwood Avenue (river miles 2.61 to 2.81) on the main stem.

4. Replacement of an existing culvert, on a side inlet channel, in Theodore Wirth Park and at Adair Avenue on the North Branch (river mile 0.57).

5. The Rice Lake floodwater storage area.

6. Riprap for approximately 200 feet of the creek channel downstream of Trunk Highway (T.H.) 100.

7. Modifications for 1,100 feet of channel from river miles 1.9 to 2.1 on the North Branch.

8. Raises for 300 feet of Dresden Lane, 1,800 feet of Toledo Avenue and West Bend Road, and 800 feet of Boone Avenue on the main stem, and 2,300 feet of Brunswick, 32nd, and Adair Avenues, and Douglas Drive on the North Branch.

2.02 Based on recent hydrologic data, items 1 and 3 through 7 were determined unnecessary for project operation; and item 2 would be accomplished by the Hennepin County Highway Department. The road raises listed under item 8 were eliminated at the request of the local sponsors.

2.03 Additions and Design Modifications - To facilitate this discussion, each of the structural features is presented under a separate category that identifies the specific construction location.

2.04 2nd Street North to Dupont Avenue - A new Bassett Creek outlet conduit would run from 2nd Street North and 3rd Avenue (beneath the proposed Interestate 394 alignment) to 6th Street North. From 6th Street North, this conduit would run diagonally under the intersection of Glenwood Avenue and Lyndale Avenue North (beneath the right-of-way of Interstate 94) to the existing outlet structure on 2nd Avenue North near Dupont Avenue. The entire length (approximately 5,950 linear feet) would be excavated as a 10-foot diameter tunnel. Tunnel excavation would be accomplished by both water-jetting and mechanical diggers. Approximately 15,000 cubic yards (yd^3) of clay and gravel plus 5,000 yd^3 of St. Peter sandstone would be removed from the tunnel.

* Miles refers to the upstream distance on the main stem or North Branch above the confluence of Bassett Creek and the Mississippi.

This tunnel would connect (at 2nd Street North) with the recently constructed Minnesota Highway Department tunnel that outflows below the St. Anthony Falls lock and dam complex (see FEIS, p. 3, paragraph 1.12, for a description and the route of this tunnel).

2.05 The planned tunnel would pass through a well developed, primarily commercial and industrial area. During construction, portions of the tunnel would need dewatering. Although this dewatering would tend to affect the ground water in those areas, the dewatering would be of short duration, with no long-lasting effects. Construction of this portion of the outlet conduit as a tunnel instead of as a surface excavation is intended principally to lessen social disruption and to avoid the many public utility lines in this area.

2.06 Dupont Avenue North to Cedar Lake Road - From Dupont Avenue North (mile 1.52) to Cedar Lake Road (mile 2.15), approximately 30 acres of land along the south side of Bassett Creek would be purchased for development of a temporary floodwater ponding area and a storage site for excavated materials. The ponding area would cover approximately 18 acres from miles 1.52 to 1.85 and would provide 90 acre-feet of floodwater storage. The remaining 12 acres, principally located between miles 1.85 and 2.15, would be used to store approximately 100,000 to 150,000 cubic yards (yd^3) of the estimated 302,000 yd^3 of material excavated from the floodwater storage area (see paragraph 2.14, on Excavated Materials, for a discussion of the characteristics of this material and the identification of alternative disposal sites). Within this area, the entire length of the Bassett Creek channel would be relocated south of the current alignment. The proposed channel would wind through the disposal site and along the north edge of the floodwater storage area. The old channel would be only partially filled so that it would retain some of the existing vegetation and drain the adjacent area.

2.07 Through this area, the Bassett Creek channel is bordered by relatively high banks composed of various types of fill material, asphalt, and concrete. Trees and brush cover the bank tops. To the north of the channel lies an industrial complex while to the south are two distinct tracts of land. One of these tracts has been used as a construction debris disposal area for approximately 30 years, and the other is a switching and storage yard owned by the Burlington Northern Railroad. The latter tract is covered predominantly by invader plant species, with only a few trees, and the first tract, a triangular-shaped disposal area, is bordered by a fairly wide band of trees. The quality of the aquatic environment through this area is rather poor because of runoff from the surrounding area. Aquatic plant and animal species are almost nonexistent in this area.

2.08 Fruen Mill and Glenwood-Inglewood Area - In addition to the planned removal and relocation of the small rock dam adjacent to the Fruen Mill, the proposed action includes several structural modifications:

1. Removal and relocation of the Minneapolis-Northfield and Southern Railroad (MN & SRR) bridge (including a portion of two spur lines) upstream from its current alignment.

2. Relocation of approximately 360 linear feet (lf) of the river channel (downstream from the MN & SRR bridge) through the existing railroad right-of-way. The old channel would be filled with riprap to stabilize a wooden retaining wall that currently protects the Glenwood-Inglewood Spring Water bottling plant some 5 feet away.

3. The stone retaining wall along the left side of the channel between the dam and railroad bridge would be replaced with a sheet-pile retaining wall. The right embankment downstream from the dam would be faced with approximately 290 lf of sheet piling and 160 lf of riprap.

4. A stormwater ponding area would be constructed east of the Glenwood-Inglewood bottling plant in an existing parking facility. This ponding area would collect runoff from the entire Fruen Mill and Glenwood-Inglewood complex. Water in the ponding area would drain into the creek through a 36-inch reinforced concrete pipe (RCP) in its southwest corner. A small earthen plug would be placed in a natural drain southeast of the proposed stormwater ponding area to prevent water from backing up during high-flow periods in the creek.

2.09 At this location Bassett Creek is characterized by a well defined channel containing a small rock dam and ponding area followed by a relatively turbulent zone with a rocky substrate. The left side of the channel consists of concrete, rock, and timber retaining walls, while the right embankment is heavily wooded and steep before it flattens out downstream and forms a grassy plain. The area surrounding the creek includes both residential and industrial properties with the latter coming within several feet of the banks. Bassett Creek water quality tends to be fairly good except for short periods when discharges and turbidity levels are high. This segment of the creek channel maintains a few minnows and a fair population of benthic invertebrates (i.e., Ephemeroptera, Tricoptera, and Diptera), some snails, and flatworms.

2.10 Trunk Highway (T.H.) 100 Floodwater Control Structure - As discussed in the FEIS, p. 6, paragraph 1.28, a flood storage structure consisting of an earthen embankment, large-diameter culverts, and an overflow weir would be constructed immediately upstream of T.H. 100. As currently proposed, the entire embankment would be relocated approximately 200 feet farther upstream to avoid utility lines (electric, sewer), to avoid altering T.H. 100 stormwater drainage patterns, and to obtain a better overall layout of the proposed structure. In addition, the south arm of the embankment would be shortened and angled toward the southwest to tie into higher ground along Dodge Road. The reach of channel between the embankment and T.H. 100 would then be excavated and straightened, and lined with stone riprap. This feature was added so that flows discharged through the embankment would approach more directly the double-box culverts beneath T.H. 100.

2.11 At this location, the area bordering the creek can be considered predominantly residential, although recreational open space areas are located immediately upstream and downstream of the proposed structure. The creek itself is ordinarily rather slow-moving, somewhat narrow (less than 5 feet), and relatively shallow. The substrate is a sandy-clay material. Aquatic life

in this reach is limited due to these conditions. The channel embankments are slightly sloping and maintain a narrow band of trees consisting of cottonwood, ash, and willow. A small portion of the embankment near T.H. 100 maintains a relatively dense cover of grass.

2.12 Medicine Lake Outlet Structure - As previously proposed, the existing outlet structure for Medicine Lake would be removed and a new control structure built adjacent to the Minnesota Western Railroad bridge located approximately 500 feet downstream from Medicine Lake (see FEIS, p. 8, paragraph 1.33). The control structure has been reduced to a 138-foot sheet-piled dam with a 13-foot wide low-flow weir. The entire structure would be built on the upstream side of the railroad bridge. Rock riprap would be placed behind the dam to prevent scour while a concrete bed would be placed beneath the railroad bridge for stability. In addition, the outlet channel from Medicine Lake to South Shore Drive would be lined with sheet piling and widened to a maximum of 35 feet.

2.13 From Medicine Lake to the site of the proposed control structure, the creek channel runs first between two residential properties and then through a relatively undisturbed Type 2/3 wetland.* The channel is well defined throughout this area and has a sandy-silt (humus) type substrate. In the residential area, the shoreline is covered with grass, while in the Type 2/3 wetland, bulrush and cattails predominate. Discharges from Medicine Lake through this area are relatively constant most of the year. Flow rates tend to increase after spring snowmelt and summer rainstorms, while minimum flow or no flow tends to occur in late winter or during long periods of dry weather. During periods of no flow, pools develop in low areas along the creekbed. Fish (mostly minnows) and benthic populations vary throughout the year, with some small game fish appearing following high discharge periods.

2.14 Excavated Materials - Of the estimated 348,000 cubic yards (yd^3) of material to be excavated during the construction of the proposed project, approximately 322,000 yd^3 must be disposed of in an approved manner. This material would principally come from two sites: the new outlet conduit (20,000 yd^3) and the tunnel inlet storage area (302,000 yd^3) (see FEIS, p. 4, paragraph 1.15). The excavated materials would consist of clay, gravel, and sand from the outlet conduit plus construction debris (concrete, asphalt, bricks, wood, and coarse gravel), clay, and humus or organic material from the tunnel inlet storage area.

2.15 Minnesota Pollution Control Agency (MPCA) records show that the inlet storage excavation area was used for many years as a construction debris disposal site. MPCA records also show that some debris from an agricultural-chemical warehouse fire was dumped at this location; however, MPCA removed all of this material and placed it in an approved disposal site. Other than this one incident, MPCA records indicate that no hazardous material or refuse has been placed at this site.

*A Type 2/3 wetland contains soils that are waterlogged to within a few inches of the surface and covered by at least 6 inches of water.

2.16 Further discussions with MPCA indicated that, because the area was an uncontrolled dump site, hazardous materials could be present. As a result, a sampling program was initiated to analyze chemically both the soils and ground water in the proposed ponding area. Samples were obtained from 10 test pits randomly selected throughout the proposed storage area. Each sampling (i.e., soil or water) was analyzed for both heavy metals and purgeable organics (see Table 3 for a list of the parameters under each of the above headings). Following the excavation of each test pit, a visual description of its contents was recorded. Most of the debris was located in layers at various depths. The most abundant type of material observed in all test pits was clay bricks, followed by glass containers of various shapes and sizes. Other materials observed included clay, cinders and ash, wood debris, reddish sand, tar, domestic rubbish (i.e., various types of containers), electrical wiring, insulation and electrical conduits, gravel, and reddish-orange soil giving the appearance of rusting metal.

2.17 The test results indicated that the material was relatively clean, except for several areas of concern. There was a general low-level presence of nickel and zinc in the filtered water samples from each of the test pits. PCB's were present in water samples from test pit 82-76. Derivatives of creosote were found in soil samples from test pits 82-73, 82-74, and 82-78. The presence of these toxic substances indicates a need for further analyses to determine if the proposed disposal methods comply with the Toxic Substance Control Act (TSCA), the Resource Conservation and Recovery Act (RCRA), and applicable State standards. These analyses will be conducted during the next stage of project planning.

2.18 It has been estimated that approximately 100,000 to 150,000 yd^3 of the excavated materials, from both the outlet conduit and tunnel inlet storage areas, could be disposed of in the vicinity of the latter area. An estimated 12 acres could be available for such disposal (if the city of Minneapolis can buy the needed land). Several alternative disposal sites have been identified for the placement of these materials:

1. Herbst and Sons Construction Co., Inc. - Demolition Landfill, T. 31N. R.23W., Section 22, in New Brighton, Minnesota.

2. Port Crosby Association

    a. T.28N., R.22W., NW 1/4 of Section 16 (80 acres) in South St. Paul, Minnesota, along the Mississippi River (river mile 835.3).

    b. T.27N., R.22W., Section 27 in Inver Grove Heights, Minnesota, along Highways 55 and 52, 10825 East Courthouse Boulevard.

2.19 Other methods that could be used for the placement of suitable fill material include:

1. Storing the material in a holding area until a suitable disposal site is identified.

2. Stockpiling some of the material such as sand and gravel for later use on areas such as winter roadways, and on lake and river shorelines as either shoreline nourishment or protection.

2.19 Most of the proposed fill site lies to the west of the tunnel inlet storage area. It consists of an open area filled with piles of construction debris and bordered by a variety of trees (e.g., elm, ash, cottonwoods) that parallel Bassett Creek to the north and the Burlington Northern Railroad tracks to the south. Immediately north of the proposed inlet storage area is the Bassett Creek channel. As currently planned, the channel in this area would be diverted through the inlet storage area and the old channel would be partially filled with clean excavated material. The material excavated from construction of the new conduit would be used for this fill.

3.00 Environmental Effects

3.01 The following is a description of the effects of the proposed modifications on the environmental resources of the area. Environmental effects associated with the construction activities outlined in section 2.00 are expected to be minor and short term. The cumulative effects of the proposed actions are expected to be less than those previously identified in the FEIS. Impact categories in Table 1 were reviewed and evaluated during the assessment of the proposed modifications. A summary of this review is presented below. Table 2 shows the relationships of the proposed project to environmental protection statutes and other environmental requirements.

3.02 Social and Cultural Effects - No appreciable social and cultural effects would result from the proposed modifications. No properties in the project area are listed on or eligible for inclusion on the National Register of Historic Places. In addition, there are not any known archaeological or historical sites that would be affected. Because of previous disturbance in the area of the proposed changes and because of previous surveys of the project, no additional surveys would be necessary prior to construction.

3.03 Social Effects - The proposed modifications to the new conduit, the Fruen Mill and Glenwood-Inglewood area, the Trunk Highway 100 embankment, and the Medicine Lake outlet structure would lessen the overall negative effects on public and private facilities and services while ensuring an adequate degree of flood protection.

3.04 Natural Resource Effects - The proposed structural modifications would affect an additional 1,700 linear feet (lf) of existing river channel in addition to those effects discussed in the FEIS, pages 28 to 38, paragraphs 4.00 to 4.43. The areas of impact include the tunnel inlet storage site (1,150 lf), the Fruen Mill and Glenwood-Inglewood site (360 lf), the T.H. 100 embankment (150 lf) site, and the Medicine Lake outlet (40 lf) site. These effects would either modify the existing substrate or eliminate the entire creekbed through relocation of the channel. At the Trunk Highway 100 embankment and the Medicine Lake outlet structure, the existing substrate

would be modified to include rock riprap. Since few benthic invertebrates inhabit the area under present conditions, addition of rock material would diversify the habitat and promote development of a more diverse grouping of benthic communities.

3.05 As discussed in paragraphs 2.06 and 2.08, item 2, approximately two sections of creek channel measuring 1,150 and 360 lf would be relocated, resulting in the loss, through burial, of the existing channel along with the aquatic communities in those areas. When completed, the new channel above the tunnel inlet storage area would contain both slack and turbulent areas, thereby producing habitat for a greater diversity of both fish and benthic communities. In the Fruen Mill and Glenwood-Inglewood area, the new channel would retain some of the qualities of the old channel (i.e., rocky substrate and turbulent water) but provide less turbulent areas with relatively smooth substrates. Hence, a more diverse assemblage of organisms would develop in this area.

3.06 Excavated Material - As discussed in paragraphs 2.14 to 2.19 of this assessment, identification of potential disposal sites for an estimated 177,000 to 222,000 yd^3 of excavated material (from the tunnel inlet storage area) became increasingly important during Phase II planning, partly because of concerns expressed by the MPCA about potential contamination of the soils to be excavated. The ponding area excavation site has been used as a demolition debris disposal area, and a portion of it is still used for such disposal. The rest of the site contains the remnants of a railroad switching and storage facility.

3.07 Chemical analyses (discussed in paragraph 2.16) were conducted to respond to MPCA's concerns and to determine if any environmentally-harmful contaminants were present. Test results (discussed in paragraph 2.17) indicated low levels of nickel, zinc, and PCB's in some of the water samples while some of the soil samples contained moderate levels of a number of creosote compounds. Even though most of the area sampled appeared relatively clean, the presence of some potentially toxic substances indicates a need for further analysis to determine if the proposed disposal methods for the excavated materials would comply with the Toxic Substance Control Act, the Resource Conservation Recovery Act, and any applicable State standards. Because of time and money limitations, these additional studies cannot be conducted until the next phase of planning. As the test results become available, the St. Paul District will assess the impacts of both the ponding area and proposed disposal methods and will prepare the appropriate National Environmental Policy Act documentation (environmental assessment, environmental impact statement supplement, or other appropriate documents).

4.00 Coordination

4.01 Throughout development of the project design modifications, coordination with the public and governmental agencies was maintained. The draft reports, including the environmental assessment and 404(b)(1) evaluation, will be sent to interested citizens, and to local, State, and Federal agencies for review and comment.

TABLE 1
ENVIRONMENTAL IMPACT ASSESSMENT MATRIX
BASSETT CREEK WATERSHED

NAME OF PARAMETER	MAGNITUDE OF PROBABLE IMPACT						
	← INCREASING BENEFICIAL IMPACT			NO APPRECIABLE EFFECT	INCREASING ADVERSE IMPACT →		
	SIGNIFICANT	SUBSTANTIAL	MINOR		MINOR	SUBSTANTIAL	SIGNIFICANT
A. SOCIAL EFFECTS							
1. Noise Levels				X			
2. Aesthetic Values			X				
3. Recreational Opportunities				X			
4. Transportation				X			
5. Public Health and Safety				X			
6. Community Cohesion (Sense of Unity)				X			
7. Community Growth and Development				X			
8. Business and Home Relocations			X				
9. Existing/Potential Land Use			X				
10. Controversy				X			
B. ECONOMIC EFFECTS							
1. Property Values			X				
2. Tax Revenues				X			
3. Public Facilities and Services				X			
4. Regional Growth				X			
5. Employment				X			
6. Business Activity			X				
7. Farmland/Food Supply				X			
8. Commercial Navigation				X			
9. Flooding Effects			X				
10. Energy Needs and Resources			--	X			
C. NATURAL RESOURCE EFFECTS							
1. Air Quality				X			
2. Terrestrial Habitat				X			
3. Wetlands				X			
4. Aquatic Habitat					X		
5. Habitat Diversity and Interspersion				X			
6. Biological Productivity				X			
7. Surface Water Quality				X			
8. Water Supply				X			
9. Groundwater				X			
10. Soils				X			
11. Threatened or Endangered Species				X			
D. CULTURAL EFFECTS							
1. Historic Architectural Values				X			
2. Prehistoric and Historic Archaeological Values				X			

TABLE 2
RELATIONSHIP OF PLANS TO ENVIRONMENTAL PROTECTION STATUTES AND OTHER ENVIRONMENTAL REQUIREMENTS
BASSETT CREEK WATERSHED
HENNEPIN COUNTY, MINNESOTA

Federal Statutes:	Plan 1[1]
Archaeological and Historic Preservation Act, as amended, 16 U.S.C. 469 et seq.	FC
Clean Air Act, as amended, 42 U.S.C. 7401 et seq.	FC
Clean Water Act, as amended (Federal Water Pollution Control Act) 33 U.S.C. 1251 et seq.	FC
Coastal Zone Management Act, 16 U.S.C. 1451 et seq.	NA
Endangered Species Act, as amended, 16 U.S.C. 1531 et seq.	FC
Estuary Protection Act, 16 U.S.C. 1221 et seq.	NA
Federal Water Project Recreation Act, as amended, 16 U.S.C. 460-1(12) et seq.	FC
Fish and Wildlife Coordination Act, as amended, 16 U.S.C. 661 et seq.	FC
Land and Water Conservation Fund Act, as amended, 17 U.S.C. 4601 - 4601-11 et seq.	FC
Marine Protection, Research and Sanctuaries Act, 22 U.S.C. 1401 et seq.	NA
National Historic Preservation Act, as amended, 16 U.S.C. 407a et seq.	FC
National Environmental Policy Act, as amended, 42 U.S.C. 4321 et seq.	FC
Rivers and Harbors Act, 33 U.S.C. 401 et seq.	FC
Watershed Protection and Flood Preservation Act, 16 U.S.C. 1001 et seq.	NA
Wild and Scenic Rivers Act, as amended, 16 U.S.C. 1271 et seq.	FC
Executive Orders, Memoranda, etc.	
Floodplain Management (E.O. 11988)	FC
Protection of Wetlands (E.O. 11990)	FC
Environmental Effects Abroad of Major Federal Actions (E.O. 12114)	NA
Analysis of Impacts on Prime and Unique Farmlands (CEQ Memorandum, 30 August 1976)	NA
Required Federal Entitlements	
None	NA
State and Local Policies	
Minnesota Code of Agency Rules, Pollution Control Agency, WPC 14	FC
Land Use Plans	
None	NA

[1] FC - Full Compliance. The plan has met all requirements of the statute, E.O., or other environmental requirements for the current stage of planning.
PC - Partial Compliance. The plan has not met some of the requirements normally met in the current stage of planning.
NC - Non-compliance. The plan is in violation of a requirement of the statute, E.O., or other environmental requirement.
NA - Not Applicable. The statute, E.O. or other environmental requirement does not apply to the current stage of planning.

TABLE 3
CHEMICAL PARAMETERS FOR SOIL AND GROUNDWATER ANALYSIS

Purgeable Organics	Heavy Metals
Acolein	Antimony
Acrylonitrile	Arsenic
Benzene	Beryllium
Bis (2 chlorethyl) ether	Cadmium
Bromoform	Chromium
Carbon tetrachloride	Copper
Chlorobenzene	Cyanide
Chloroethane	Lead
Chloroform	Mercury
Chlorodibromomethane	Nickel
Creosote	Selenium
Dichlorobromomethane	Silver
Dichlorodifluoromethane	Thalium
Ethylbenzene	Zinc
Methylene chloride	
Methyl chloride	
Methyl bromide	
Toluene	
Trichlorofluoromethane	
Trichloroethylene	
Tetrachloroethylene	
Vinyl chloride	
2 Chloroethylvinyl ether	
1,1 Dichloroethane	
1,2 Dichloroethane	
1,1,1 Trichloroethane	
1,1,2 Trichloroethane	
1,1,2,2 Tetrachloroethane	
1,1 Dichloroethylene	
1,2 Trans-dichloroethylene	
1,2 Dichloropropane	
1,3 Dichloropropene	
Polychlorobiphenyls (totals PCB's)	
Creosote, compounds of:	
Acenaphthene	
Acenaphthylene	
Anthracene	
Benzo(a)anthracene	
Benzo(a)pyrene	
Benzo(b) fluoranthene	
Benzo(ghi)perylene	
Benzo(k)fluoranthene	
Chrysene	
Dibenzo(ah)anthracene	
Fluoranthene	
Fluorene	
Indeno(1,2,3-c,d) pyrene	
Naphthalene	
Phenanthrene	
Pyrene	

PRELIMINARY
SECTION 404(b)(1) EVALUATION
OF THE
BASSETT CREEK WATERSHED FLOOD CONTROL
PROJECT
HENNEPIN COUNTY, MINNESOTA

I. Project Description

a. Location - The proposed fill activities would take place in the main stem of Bassett Creek between its outlet from Medicine Lake and the point where the creek enters the conduit in West Minneapolis and along the North Branch Bassett Creek between its outlet from Northwood Pond and its junction with the main stem.

b. General Description - The proposed fill activities would include five types of structural measures:

(1) Complete closure and filling of stream segments in conjunction with construction of a new channel.

(2) Placement of temporary sandbag levees for stream diversion and dewatering of construction areas.

(3) Placement of riprap and riprap filters across the stream channel.

(4) Placement of sheet-pile floodwalls on one or both sides of the channel.

(5) Placement or construction of flow-control structures (weirs, culverts, drop structures, etc.) at various points along the stream.

c. Authority - These actions are undertaken through authority provided by resolutions adopted by the Committee on Public Works of the U.S. Senate on 7 April 1962 and the House of Representatives on 24 September 1970 in order to provide flood protection for areas in the Bassett Creek watershed.

d. General Description of Fill Material

(1) General Characteristics of Fill Material - Fill material would consist of sandbags (clean sand inclosed within burlap bags, riprap (quarried stone), riprap filters (crushed stone or coarse sand), sheet pile (metal), liquid and solid premixed concrete, and a silt/sand gravel soil.

(2) Quantity of Material - Approximately 100 cubic yards (yd^3) of sandbags would be required for the two temporary levees. Approximately 8,370 yd^3 of rock and 2,300 yd^3 of sand would be needed for the riprap and filter. The sheet metal sidewall

structures would require 800 linear feet of material. Approximately 35,000 yd^3 of the silt/sand/gravel material would be needed to fill one section of streambed.

(3) Source of Material - Burlap sandbags, concrete, and sheet metal would be obtained from local commercial sources. Rock and sand material would be obtained from a local State-approved source. The silt/sand/gravel material would be obtained from the St. Peter sandstone formation during the excavation of the tunnel segment of the project.

e. Description of the Proposed Fill Sites

(1) Location - Locations of the eight proposed fill sites are shown on Exhibit 1.

(2) Size - At sites 1 and 2, 3,250 and 360 square feet (ft^2) of stream channel, respectively, would be completely filled and covered. The area of streambed to be covered by riprap would be 7,200 ft^2 at site 3, 1,600 ft^2 at site 5, 900 ft^2 at site 6, 4,140 ft^2 at site 7, and 1,800 ft^2 at site 9. A total of 8,100 ft^2 of stream channel would be covered by new structures (weirs, culverts, drop structures, etc.) at sites 2, 3, 4, 5, 6, 7, and 8. During construction, approximately 400 ft^2 of streambed would be covered by temporary sandbag levees at sites 4 and 6.

(3) Type of Site - Fill placed behind the temporary levees at sites 4 and 6 would be confined. All other fill activities would be carried out in unconfined open water areas.

(4) Types of Habitat - All of the proposed sites are located along the main stem or the North Branch of Bassett Creek, a small, gradually-sloped stream. Site 1 is located in an area of high industrial development and has a degraded aquatic environment. Sites 2, 3, 4, 5, 6, and 8 are located in areas of moderate to heavy residential development. Although the habitat present provides aesthetically pleasing areas, it has been significantly altered from an undisturbed state. Most of the area surrounding site 7 is a Type 2/3 wetland* and is relatively undisturbed.

(5) Timing and Duration of Discharge - Construction activities would be scheduled between March 1984 and December 1986, during periods of moderate to low flows.

* A Type 2/3 wetland contains soils that are waterlogged to within a few inches of the surface and covered by at least 6 inches of water.

II. Factual Determinations

a. Physical Substrate Determinations

(1) Substrate Elevation and Slope - The proposed fill activities would have no significant impact on the creek's slope or elevation except for the probable development of small pools immediately above or below the flow-regulation structures. Fill placed at sites 1 and 2 would completely close off the channel, but related project activities would construct a nearby channel with approximately the same slope and relative elevation.

(2) Sediment Type - Placement of rock, concrete, and gravel would significantly change or alter the project area's existing sediment type (silt/sand).

(3) Dredged/Fill Material Movement - The size of the fill material would be large enough and the stream velocity low enough during construction to ensure that no significant movement of fill material would occur.

(4) Physical Effects on Benthic Organisms - A complete and permanent loss of existing benthos would occur in those areas covered at sites 1 and 2. The new channels constructed in conjunction with these fill activities, however, would eventually support a similar benthic assemblage. The riprap placed at sites 3, 4, 5, 6, 7, and 8 would eliminate or displace some benthic organisms but would probably have a positive long-term impact by providing for a more diverse habitat. The cement structures at sites 3, 4, 5, 6, 7, and 8 would destroy any habitat that they cover and would provide minimal replacement habitat. The sheet-pile structures at sites 2 and 7 would eliminate a certain amount of bank habitat. The temporary levees around construction areas at sites 4 and 6 would have a short-term negative impact on the habitat that they covered. These affected areas would probably recover quickly, however, once the levees were removed.

(5) Actions Taken to Minimize Impacts - Clean fill material would be used and, whenever possible, construction would be carried out at low-flow periods.

b. Water Circulation, Fluctuation, and Salinity Determinations

(1) Water

(a) Salinity - Not applicable.

(b) Water Chemistry, Color, Odor, Taste, Dissolved Gas Levels, Nutrients, Eutrophication, and Temperature - The proposed fill activities should have no significant impact on any of these parameters.

(c) Clarity - Some increase in turbidity may be caused by construction activities associated with the fill discharge, but this would be a short-term situation minimized by the use of clean fill material and temporary levees to inclose major construction areas.

(2) Current Patterns and Circulation

(a) Current Patterns and Flow - Fill activities at sites 1 and 2 are designed to move the channel and therefore the current pattern from its existing position. The permanent ponding area at site 1 would slow the creek's flow through this area. Flow-control structures at sites 1, 3, 4, 6, and 7 would temporarily hold water behind them at times of high flow.

(b) Velocity - During high-flow periods, the structures at sites 1, 3, 4, 6, and 7 would lessen flows by diverting some of the water into ponding areas.

(c) Stratification and Hydrologic Regime - The proposed fill activities would have no significant impact on either stratification or hydrologic regime.

(3) Normal Water Level Fluctuations - The proposed retarding structures would increase the stage levels in the ponding areas but decrease these levels downstream.

(4) Salinity Gradient - Not applicable.

(5) Actions Taken to Minimize Impacts - Clean fill material would be used to minimize the potential for negative water quality impacts. At sites 4 and 6 where major fill activities would take place, temporary levees would be constructed to dewater the areas, thus reducing the chance for negative impacts on the environment.

c. Suspended Particulate/Turbidity Determinations - The fill material used at site 1 would be a mixture of silt, sand, and gravel. Although the silty material would be susceptible to resuspension, it would still not be considered suspended particulate matter. In addition, the nature of the fill action would leave all but a small portion of this material protected from the force of the flowing water. The fill material to be used at the other sites would be sand or larger particle size mineral substances. This fill would contain no significant amounts of suspendable particulate or organic matter. Because of these conditions, no significant increase would be expected in turbidity or in the concentration of suspended particulates caused by project activities.

d. Contiminent Determinations - The proposed fill materials -- clean silt, sand, gravel, rock, and cement -- would not introduce contaminants into the aquatic system. Likewise, neither the materials nor the placement would significantly relocate or increase contaminants already in the aquatic system.

e. Aquatic Ecosystem and Organism Determinations

(1) Effects on Plankton - Lentic areas established behind retarding structures and in ponding areas would provide improved habitat for plankton.

(2) Effects on Benthos - Some benthic organisms would be killed or displaced by the project's permanent and temporary fill activities. This impact would be short term in areas where the fill material would be removed. Areas that would be covered by rock and gravel would probably provide better benthic habitat than the present predominantly silt/sand substrate. The cement structures would provide little replacement habitat.

(3) Effects on Nekton - Placement of the flow control structures could impede or prevent migration of various nektonic species through the project area. The relative importance of this impact would be minimal, however, because of the limited number of such organisms which inhabit the area.

(4) Effects on Aquatic Flood Web - Placement of fill material would have no significant impact on the project area's food web.

(5) Effects on Wetlands - The structure at site 8 would change approximately 0.5 acre of Type 2/3 wetland to Type 3/4 wetland.[a]

(6) Threatened and Endangered Species - The proposed fill activities would have no significant impact on any threatened or endangered species.

(7) Actions to Minimize Impacts - Clean fill material would be used in all cases. The temporary levees would be made of sandbags to facilitate levee removal. Fill and construction activities would take place as much as possible during low-flow periods.

f. Proposed Disposal Site Determinations

(1) Mixing Zone Determinations - The maximum depth of the creek in areas that would receive fill material is 3 feet. The fill would be placed during periods of low flow, be protected by temporary levees in certain cases, be free of contaminants, and consist primarily of sand, gravel, rock, and premixed concrete, which would all quickly come out of suspension. Some silt would be in the fill placed at site 1 but the placement method and the small amount of silt present indicate that only a small quantity of material would go into and remain in suspension. Under these conditions, the mixing zone would be acceptable.

[a]A Type 3/4 wetland contains soil that is waterlogged throughout the growing season and is usually covered with 6 inches to 3 feet or more of water.

(2) Determination of Compliance With Applicable Water Quality Standards - The Minnesota Pollution Control Agency has classified Bassett Creek in Fisheries and Recreation Category (2B). This classification requires water of a quality adequate to propagate and maintain both sport and commercial fishes as well as aquatic recreation of all kinds, including swimming. The fill material would be obtained from uncontaminated sources; selected sites would be protected by temporary levees during fill placement; and, whenever possible, fill would be placed during low-flow periods. These procedures should insure that water quality conditions remain within a range acceptable to State regulatory agencies.

(3) Potential Effects on Human Use Characteristics - It has been determined that there would be no potential for significant effects on human use characteristics, based on the present and projected human use characteristics, existing physical conditions, the proposed construction methods, and the clean nature of the fill material.

(4) Determination of Cumulative Effects on the Aquatic Ecosystem - The cumulative impact of the proposed fill activities would include:

(a) A reduction in flow velocity in Bassett Creek.

(b) Temporary increases in stage levels behind the new structures at sites 3, 4, 6, and 7.

(c) Short-term increases in turbidity caused by construction activities.

g. Determination of Secondary Effects on the Aquatic Ecosystem - Secondary impacts associated with the proposed fill activities include:

(1) A long-term reduction in turbidity due to a reduction in bank erosion and the settling that would occur in the ponding areas.

(2) A possible reduction in water quality due to ponding of nutrient-rich water behind the flow control structures.

III. Findings of Compliance or Noncompliance With the Restrictions on Discharge - The proposed fill activities would comply with the Section 404(b)(1) Guidelines of the Clean Water Act. Of the alternatives considered, only plans 4C, 6C, and 6D satisfied the economic requirement of practicability, a benefit/cost ratio of 1.00 or greater (i.e., benefits must equal or exceed costs). Plan 4A would have had greater negative environmental impact than would the selected plan, 6C. The environmental impacts of Plans 6C and 6D are identical, but 6C has better engineering aspects.

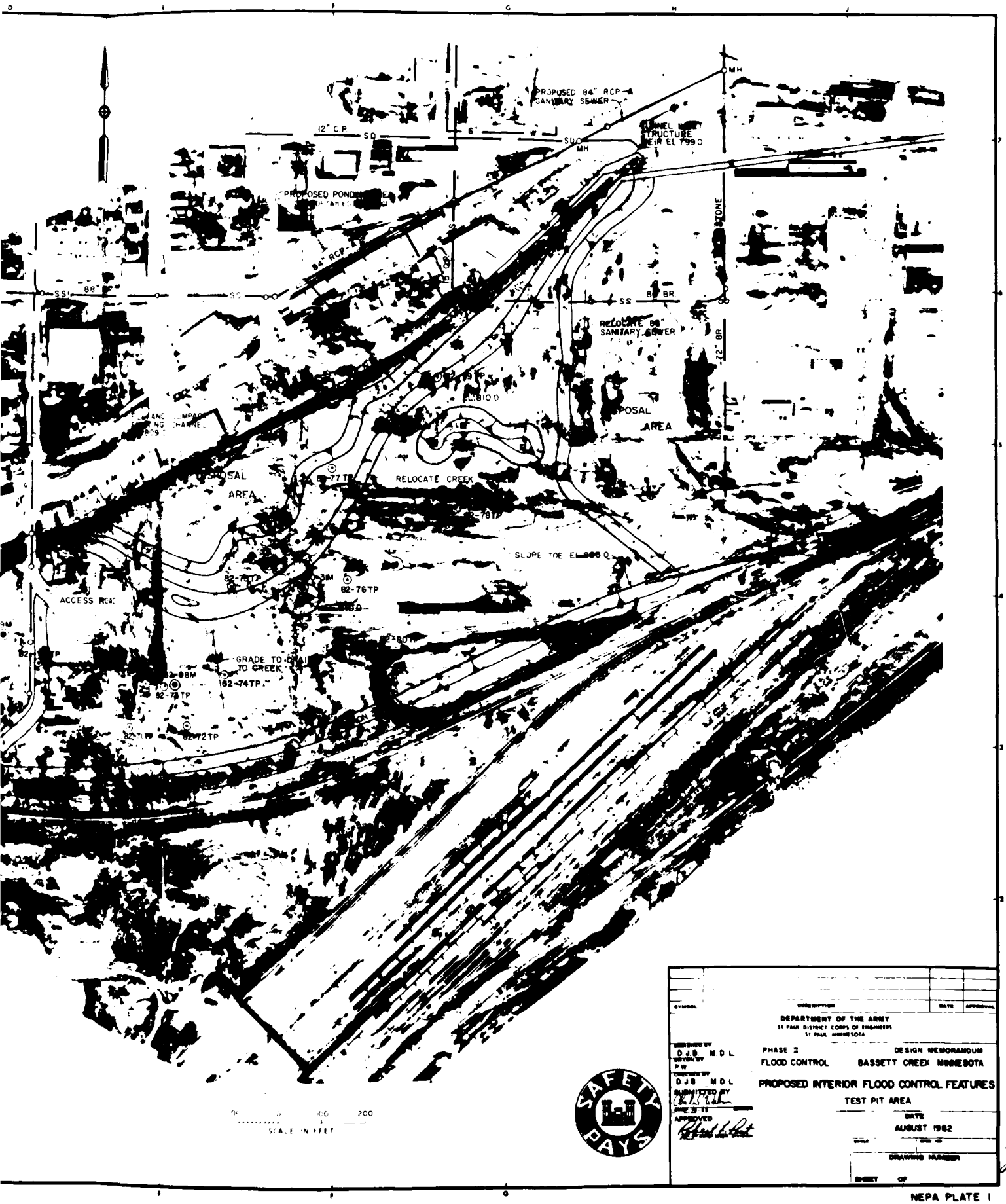
The proposed fill activities could result in turbidity readings temporarily in excess of the State standards, but this impact would be a short-term condition associated with construction activities. The proposed action would comply with Section 307 of the Clean Water Act and the Endangered Species Act of 1973, as amended. The proposed fill activity would have no significant adverse impact on human health or welfare. To minimize the potential for adverse impacts, clean fill would be used; construction would take place during periods of low flow; and, where practicable, temporary levees would be placed around construction sites. On the basis of this evaluation, the proposed disposal sites are specified as complying with the requirements of the guidelines for discharge of fill material.

16 Sep 82

Date

EDWARD G. RAPP
Colonel, Corps of Engineers
District Engineer


805
800 S
800 C
NOTE ALL SIDE SLOPES ARE IV ON 3H
PROPOSED PONDING AREA
DETAILED DRAWING
50 0 50 100
SCALE IN FEET



DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY D.J.B. M.D.L.
DRAWN BY P.W.
CHECKED BY D.J.B. M.D.L.
SUBMITTED BY
APPROVED

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL — BASSETT CREEK MINNESOTA

PROPOSED INTERIOR FLOOD CONTROL FEATURES

TEST PIT AREA

DATE
AUGUST 1982

DRAWING NUMBER

SHEET OF

SAFETY PAYS

NEPA PLATE 1

DEPARTMENT OF THE ARMY
St. Paul District Corps of Engineers
1135 U.S. Post Office & Custom House
St. Paul, Minnesota 55101

FLOOD CONTROL

BASSETT CREEK

HENNEPIN COUNTY, MINNESOTA

DESIGN MEMORANDUM NO. 2, PHASE II

APPENDIX A

HYDRAULICS

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APPENDIX A

GENERAL

1. Because the presented project is basically the same as previously defined in Design Memorandum No. 1, only changes from the original design are presented in this appendix. The changes concern:

a. Medicine Lake Outlet Structure.

b. Golden Valley Golf Course Control Structure.

c. Highway 100 Control Structure.

d. Fruen Mill Channel Improvement.

e. New Drop Structure upstream of the Tunnel's Ponding Area.

f. Tunnel Inlet Structure and Pond.

g. Edgewood Control Structure.

2. MEDICINE LAKE OUTLET STRUCTURE

Because of potential settlement problems, the previously proposed 150-foot overflow embankment was replaced by a 185-foot sheet pile structure with a concrete cap, tied to the existing railroad embankment. The concrete 13-foot weir was also replaced by sheet piling with a concrete cap. The existing railroad bridge is also slightly modified to increase its capacity and to eliminate the two previously proposed additional RCPA's. The creek channel between Medicine Lake and South Shore Drive is widened to 35-foot bottom width to decrease friction losses. New data on the Medicine Lake Outlet are given in Tables A-1 and A-2 and on Plate A-34.

3. GOLDEN VALLEY GOLF COURSE CONTROL STRUCTURE

The control structure proposed in Design Memorandum No. 1 was redesigned for reasons of economy and better protection of the overflow embankment and river bottom downstream of the structure. The 25-foot long broad crested weir and 1 - 73" x 45" and 1 - 88" x 55" RCPA were replaced by a 75-foot long trapezoidal broad crested weir and 2 - 4' x 6' concrete box culverts. A scour hole type stilling basin was added. New data on the Golden Valley Golf Course Control Structure are given in Tables A-1 and A-2 and on Plates A-29 and A-30. The preformed scour hole stilling basin is designed in accordance with USAE WES MPH-72-5.

4. HIGHWAY 100 CONTROL STRUCTURE

For reasons of economy, the control structure was moved upstream of its location in Design Memorandum No. 1. The details of the structure are unchanged. Moving the structure allowed for shorter embankments. Details are shown on Plates A-23 and A-24.

5. FRUEN MILL CHANNEL IMPROVEMENT

To reduce costs the ogee dam and stilling basin proposed in Design Memorandum No. 1 was replaced with a straight drop structure and stilling basin designed in accordance with USAE WES HDC 623. Details are shown on Plates A-15 to A-19.

6. DROP STRUCTURE UPSTREAM OF TUNNEL PONDING AREA

To obtain the required storage area at the tunnel inlet, it was necessary to lower the channel bottom. A drop structure is presently proposed at the upstream end of the ponding area to return the bottom elevation to the natural condition. Details of this structure were not contained in Design Memorandum No. 1. The structure is designed in accordance with USAE WES HDC 624. Details of this structure are shown in Tables A-1 and A-2 and on Plate A-11.

7. TUNNEL INLET STRUCTURE AND POND

More detailed investigation subsequent to Design Memorandum No. 1 found that the design pond elevation upstream of the tunnel inlet should be lowered one foot, from 807.5 to 806.5, to give adequate freeboard on the left bank. The weir at the inlet structure was lowered from elevation 800 to elevation 799 to accomplish this. Outflow discharge hydrographs from the pond were unaffected. Details of the structure are shown in Tables A-1 and A-2 and on Plate A-9.

8. EDGEWOOD CONTROL STRUCTURE

The proposed design of the Edgewood Avenue control structure has been changed from that approved in Design Memorandum No. 1, Hydrology and Hydraulics. The structure has been changed from a non-overflow to an overflow embankment. This was done to save costs and to increase safety. The hydraulic design for the proposed overflow embankment follows.

9. The design presented in D.M. No. 1 consisted of a 40-foot fixed crest concrete drop inlet, double 169" x 107" RCPA's out of the drop inlet, a low flow 73" x 45" RCPA into the drop inlet and an embankment with a top elevation 3 feet above the probable maximum flood headwater elevation. The presently proposed design consists of a 73" x 45" RCPA transitioning to a 102" x 62" RCPA, a protected overflow embankment with a 127-foot crest at about 1 foot above the 100-year flood headwater elevation, and erosion protection at the outlet of the 73" x 45" RCPA. The proposed design is shown on Plates A-38 and A-39. As shown in Table 24 of D.M. No. 1, the non-overflow structure was classified as high hazard; the proposed overflow structure is similarly classified as high hazard and erosion protection is designed to be stable for all floods up to the probable maximum flood. It is felt the proposed overflow structure has a smaller chance of catastrophic failure than the design presented in D.M. No. 1. The previous structure was designed to have all flow through the pipes. If the drop inlet or the pipes were to plug, the embankment would be overtopped by floods. Since it was not to be protected from overtopping, there would be a risk of catastrophic failure. The presently proposed structure should be safer since it is designed to not fail when overtopped and the crest is about 8 feet lower than the non-overtopping design, reducing the size of the potential flood wave.

10. TAILWATER RATING CURVE

The tailwater rating curve at the Edgewood control structure is based on proposed conditions at downstream crossings. The headwater rating curve for Douglas Avenue (about 250 feet downstream) is used for the tailwater curve for the control structure. For large floods, the tailwater at Douglas Drive is equal to the headwater at the 34th Avenue crossing. The headwater curve at 34th Avenue is based on the proposed 115" x 72" RCPA. Headwater elevations at 34th Avenue are determined from storage outflow routings using the existing storage area from 34th Avenue to Douglas Drive. Weir flow starts at Douglas Drive at elevation 869.3 and when the headwater at 34th Avenue is greater than about elevation 871, there is negligible head loss across Douglas Drive, and the headwater for Douglas Drive is assumed equal to the headwater for 34th Avenue. The storage and rating curves for 34th Avenue are shown on Plates A-1 and A-2, respectively. Inflow and outflow hydrographs for the 34th Avenue ponding area are shown on Plates A-3 and A-4 for the standard project and probable maximum floods. The headwater rating curve for Douglas Drive (used for tailwater for the Edgewood control structure) is shown on Plate A-5.

11. STORAGE CURVE

The storage curve for the ponding area above the Edgewood control structure is based on removal of one home at the corner of Florida Avenue and 36th Avenue, removal of the Florida Avenue crossing and excavation of the storage area. The storage area is excavated to give a total storage of 19.8 acre-feet at elevation 880.0. The curve is shown on Plate A-6. The storage area is shown in plan view on Plate 40. As described in paragraphs 93, 94, 145A and 145B of D.M. No. 1, easements to preserve the storage area will be required up to a ground elevation equal to the design flood headwater elevation 879.3. Plate A-6 also shows the storage required to be preserved by easement. The area above ground elevation 879.3 is very developed. Thus, the existing total storage curve is considered to be acceptable for representing future project conditions and was used for the standard project and probable maximum flood routings.

12. HEADWATER RATING CURVE

The proposed headwater rating curve, Plate A-7, is a combination of flow through the outlet pipe and over the embankment. For flows less than about 310 cfs, all flows are through the pipe and it operates under inlet control. The computations for a discharge of 300 cfs are shown on Pages A-7 and A-8. The larger 102" x 62" RCPA is used at the outlet to reduce outflow velocities. For discharges larger than about 310 cfs, the headwater exceeds elevation 880.0, and the embankment is overtopped. The top of the embankment is 127 feet long at elevation 880.0. The weir coefficient and submergence impact were obtained from "Hydraulics of Bridge Waterways" by the BPR. The selected weir coefficient is 3.05.

13. FLOOD ROUTING

The design, standard project, and probable maximum floods were routed through the storage area using the total storage curve on Plate A-6 and the headwater rating curve on Plate A-7. A storage routing or modified Puls method was used. The inflow and outflow hydrographs are shown on Plates A-8, A-9, and A-10. Peak discharges are summarized below.

	Peak Discharge		Peak
Flood	Inflow	Outflow	Stage
Design	700	310	879.3
SPF	1044	800	881.1
PMF	2915	2000	882.7

Stage hydrographs for the headwater and tailwater for the Edgewood control structure and the headwater for 34th Avenue are shown on Plates A-11 and A-12 for the SPF and PMF. The tailwater for Edgewood is equal to the headwater for Douglas Drive.

14. OVERTOPPING PROTECTION

The proposed protection is designed to remain stable for floods up to the probable maximum flood. The stability of the proposed gabions was checked by ETL 1110-2-194. Riprap size for an alternative design was found using USAE WES TR 2-650. Velocities down the slope and location of the hydraulic jump were found by downwater computations using Chezy's equation with k set equal to 0.25 foot for the concrete-faced gabions and set equal to the riprap min D_{50} for the riprap alternative. For both alternatives, it was assumed that a maximum settlement of 0.5 foot could occur in the embankment, causing concentration of flow. Thus, the unit discharge used to size the erosion protection was found by assuming the weir crest was 0.5 foot below the average crest elevation, or at 879.5 instead of 880.0. The downstream slope of 1V:4H was based on the results of TR 2-650, page 2, paragraph 4. Normal flow velocities were found to be only 1-2 fps less for a 1V:6H slope, and thus the flatter slope was not considered necessary.

15. From Plate A-12, it can be seen that the tailwater increases rapidly with time after flow over the embankment starts. Because of this it was not obvious what was the worst case. Thus, several different time periods were checked. A downwater was performed for the peak discharge and the location of the hydraulic jump and maximum velocity were found. This was then compared to the normal depth velocities for lesser discharges that would occur earlier when the tailwater is low. It was found that the velocities reached for the peak discharge were higher than normal velocities for the smaller discharges.

Alternatives

	Gabion	Riprap
k (Chezy's)	0.25 ft.	1.56 ft.
Q (peak)	1700	1700
Hw	882.7	882.7
Tw	880.8	880.8
q (crest = 880.0)	13.4 cfs/ft	13.4 cfs/ft
q (crest - 789.5)	17.3 cfs/ft	17.3 cfs/ft
V max (at jump)	17.3 fps	14.5 fps
Horizontal Distance to jump (from edge of crest)	13 ft	9 ft

16. To determine the riprap size needed for the riprap alternative, Plate 47 of TR 2-650 was used. Several different time periods were checked and it was found that a W_{50} rock size somewhat larger than that used in the model (W_{50} = 200 bls ±) was needed. A 48-inch layer with W_{50} = 328 bls. from Inclosure 3 of the ETL 1110-2-120 was selected.

17. The stability of the concrete covered gabions was checked by ETL 1110-2-194, Plate 5. The gabions were found to be well within the stable zone; thus the use of 1V:4H downstream slope in lieu of the model's 1V:6H slope is considered acceptable.

18. STILLING BASIN AT PIPE OUTLET

A preformed scour hole is proposed at the outlet of the 102" x 62" pipe. The size of the hole and the riprap was obtained from USAE WES H-72-5, Figures 11 and 12.

19. THE OVERFLOW EMBANKMENT SECTION

The concrete-faced gabion basket alternative was chosen over the riprap alternative because it offers better overtopping protection and is more aesthetically acceptable. A typical embankment section and plan view for this alternative is shown on Plates 38 and 39. The embankment will have a top width of 20 feet, an average height of about 10 feet and a 1V on 3H upstream side slope. Twelve-inch gabion baskets on 6 inches of bedding will be placed across the top of the embankment and down the downstream slope. The baskets will be covered with 3± inches of concrete to act as a wearing surface during overtopping. The concrete will also protect and extend the life of the exposed gabion wires. The gabion baskets will be placed on a 1V on 4H downstream slope to either elevation 870.0 or the ground surface, whichever is the higher elevation. At that point, the slope of the gabions changes to 1V on 2½H to a vertical depth of 5 feet. This protection will extend the length of the embankment and continue up the abutment to elevation 886, 3 feet above the probable maximum flood level.

20. For aesthetic reasons, the concrete faced gabion baskets will be covered by 11 inches of erosion resistant clay having a liquid limit greater than 30

and a plasticity index greater than 15. Four inches of seeded topsoil will be placed over this. Four feet downstream of the embankment centerline, the gabion basket blanket will be interrupted by a concrete section having a top elevation of 880.0, extending the length of the embankment and acting as an overflow weir. To help maintain the top of the concrete weir at a permanent elevation, 8-foot long steel sheetpiling will be driven into the embankment and embedded into the concrete.

21. The embankment will be constructed of impervious fill obtained from the borrow area upstream of the embankment. The existing ground surface will be stripped to a depth of 9 inches except where layers of organic materials exist at the surface, in which case excavation will be required to suitable material.

22. RATING CURVES

All redesigned structures were hydraulically designed for "no change" in headwater elevations in comparison with structures presented and approved in Design Memorandum No. 1. For this reason there are no significant changes in the rating curves presented in Design Memorandum No. 1.

23. DISCHARGES

A summary of design (1% exceedence frequency) and standard project flood discharges are shown on Table A-3. The data on this table are taken from Table 21 of Design Memorandum No. 1, Hydrology and Hydraulics.

24. RIPRAP

A summary of hydraulic data for the design of riprap is shown on Table A-4.

25. REFERENCES

a. "Hydraulics of Bridge Waterways," U.S. Department of Transportation, Federal Highway Administration, March 1978.

b. ETL 1110-1-194, Gabion Channel Control Structures, 30 August 1974.

c. USAE WES R 2-650, Stability of Riprap and Discharge Characteristics, Overflow Embankments, Arkansas River, Arkansas, June 1964.

d. ETL 1110-2-120, Additional Guidance for Riprap Channel Protection, 14 May 1971.

e. USAE WES H-72-5, Practical Guidance for Estimating and Controlling Erosion at Culvert Outlets, May 1972.

COMPUTATION SHEET

Date 12 JAN 82 | Page 2 of

Name of Office NCSED-GH | Project Bassett Creek

H.W at Edgewood for Q = 300 cfs

Computed by RWF | Checked by | Approved by | Price Level

Data

Q 300 cfs — $R_{73\times45}$ = [illegible]

T.W. 868.65 — $R_{102\times62}$ = [illegible]

72' - 73" x 45" - Inlet invert = 865.7

60' - 102" x 62" - Outlet invert = 865.0

Outlet Control

For 102"x62" Avg depth = 868.65 - 865 = 3.65'

From hydraulic elements for RCP A

A = 26.3 + .3(29.9 - 26.3) = 27.4 sf

R = 1.86 + .2(1.90 - 1.86) = 1.87'

V = 10.95 fps

$$h_{Lf} = \frac{29.2 n^2}{2g} \frac{L V^2}{R^{4/3}} = \frac{29.2(.015)^2}{64.4} \frac{(60)(10.95)^2}{(1.87)^{4/3}}$$

$$= .32'$$

Expansion $h_e = .2\left(\frac{\Delta V^2}{2g}\right) = .2\left(\frac{16.9^2}{64.4} - \frac{10.95^2}{64.4}\right) = .52'$

73"x45" - full

$$h_{Lf} = \frac{29.2(.015)^2}{64.4} \frac{(72)(16.95)^2}{(1.13)^{4/3}} = 1.79'$$

Outlet $h_o = \frac{V_2^2}{2g} = \frac{10.95^2}{2g} = 1.86'$

Inlet $h_e = .8\frac{V_1^2}{2g} = .8\frac{(16.95)^2}{64.4} = 3.57'$

$h_{Tot} = .32 + .52 + 1.79 + 1.86 + 3.57 = 8.06'$

H.W. = 8.06 + 868.65 = 876.7

COMPUTATION SHEET

Name of Office	Project	Date	Page 2 of 2

Computed by	Checked by	Approved by	Price Level

Inlet Control

For Q = 300 cfs, 72"x45" RCBA, Square edge

Get $\frac{H_W}{D} = 3.55$ from nomograph

$H_W = \frac{(3.55)(45")}{12} = 13.3'$

$H_W = 13.3 + 865.7 = \boxed{879.0}$

879.0 > 876.7 ∴ Inlet control

NCS FORM 34
I NOV 1976

TABLE A-1

CONTROL STRUCTURES - DESCRIPTION

Control Structure	Station	Main Weir	Low Flow	Outlet	Embankment
Conduit Inlet	80+07	11.6' weir at El. 800.0	None	10' diameter conduit invert at El. 765.3	None-additional storage from excavation
Pond Inlet	95+00	23' weir at El. 805.0	None	23'x30' stilling basin	None
Highway 55	163+80	10' weir at El. 820.5 40' weir at El. 825.5	6' wide notch at El. 816.5	Existing 8'x10' double box, invert at El. 814.16	Existing Highway 55, low point at El. 826.6
Highway 100	350+73	Drop box 105' (75'x15') at El. 848.4	3' high x 10' wide orifice invert at El. 838.0	8'x10' double box	Top El. 851.0 to 853.5
Golden Valley Golf Course	466+70	75' trapezoidal broad-crested weir at El. 874.5 2-4'x6' concrete box culverts, invert at El. 866.5	None	2-4'x6' concrete box culverts, invert to El. 866.4	Earth embankment with 10' top width at El. 877.0
Wisconsin Avenue	525+15	Drop box - 20' (6'x8') at El. 885.5	2' high x 3.5' wide orifice, invert at El. 878.9	6'x8' box, invert at El. 878.9	Minimum top El. 889.0
Medicine Lake	675+80	13' sharp-crested weir at El. 887.7 185' sharp-crested weir at El. 889.1	13' sharp-crested weir at El. 887.7	The existing railroad bridge slightly modified by removing all soil and old timber walls to El. 885.0 and to the total bottom width 26.7'. Concrete slab 10' thick below El. 855.0	185' overflow sheet piling wall tied to existing railroad embankment
Edgewood Avenue (North Branch)	57+80	127' weir at El. 880.0 73"x45" RCPA transitioning to a 102"x62" RCPA at El. 865.7	73"x45" RCPA transitioning to 102"x62" RCPA invert to El. 865.7	102"x62" RCPA at El. 865.0	Protected overflow embankment with a 127' crest at El. 886.0

TABLE A-2 Control Structures - Hydraulic Data

Control Structure	Station	100-YEAR FLOOD								STANDARD PROJECT FLOOD						
		DISCHARGE (cfs)		TAILWATER		HEADWATER		Storage (ac-ft) Proposed	Outlet Velocity (fps) Design	DISCHARGE (cfs)		TAILWATER		HEADWATER		Storage (ac-ft) Proposed
		Inflow to Pond	Outflow Through Structure	Proposed	Existing	Proposed	Existing			Inflow to Pond	Outflow Through Structure	Proposed	Existing	Proposed	Existing	
Conduit Inlet	80+07	1,124	660	791.5	---	806.5	812.4	91	8.4	2,050	934	818.9	---	822.3	822.3	2,700
Pond Inlet	95+00	1,124	1,124	806.5	---	811.5	---	---	5.1	2,050	2,050	822.3	---	822.3	822.3	---
Highway 55	163+80	3,040	621	821.6	822.8	825.5	825.3	630	3.9	4,939	2,100	825.6	826.1	827.3	827.3	990
Highway 100	350+20	2,610	950	843.3	845.7	849.9	845.9	205	5.9	4,020	3,264	850.4	852.2	853.5	852.5	297
Golden Valley Golf Course	466+70	919	420	872.2	875.3	874.3	875.6	29	8.8	1,623	1,135	876.4	880.1	877.2	880.1	63
Wisconsin Avenue	525+15	3,000	370	885.1	888.1	888.8	888.2	450	7.7	4,006	1,075	890.4	890.4	890.5	890.5	1,020
Medicine Lake	675+80	9,458	245	889.5	889.5	889.7	890.1	2,000	2.5	15,873	530	890.9	891.9	891.3	891.9	5,580
Edgewood Avenue (North Branch)	57+80	700	300	868.7	877.0	879.0	877.0	18.5	2.8	1,040	676	870.9	878.5	881.2	878.5	29

TABLE A-3
COMPARISON OF DISCHARGES
ULTIMATE DEVELOPMENT VERSUS ULTIMATE DEVELOPMENT WITH PROJECT

Subwatershed Designation		100-Yr. Flood (cfs) Ultimate	100-Yr. Flood (cfs) W/Project	SPF (cfs) Ultimate	SPF (cfs) W/Project
ML-1	Medicine Lake Outlet	168	245	520	530
MS-15	Winnetka Avenue (Ult.) Wisconsin Avenue (W/Proj.)	811	370	1,035	1,076
MS-14A	Hampshire Avenue (Ult.) Golden Valley Golf Course (W/Proj.)	918	420	1,797	1,135
MS-14B	Douglas Drive	908	435	1,868	1,135
MS-10	T.H. 100	1,331	955	4,084	3,280
MS-9A	Regent Avenue	1,082	855	4,110	2,422
MS-7D	Golden Valley Road	1,081	735	2,347	1,931
NB-2A	Louisiana Avenue	310	310	442	442
NB-2B	Florida Avenue (Ult.) Edgewood Control (W/Proj.)	280	300	292	676
NB-2C	34th Avenue	500	420	680	650
MS-4	T.H. 55	898	621	2,465	2,099
MS-2B	Cedar Lake Road	895	670	2,404	2,074
MS-1B	Conduit Storage Area	1,224	1,124	2,405	2,074
MS-1B	Conduit Entrance	820	659	1,033	934

Table A-4 Riprap Required

Feature Number	Location	Design Flood Frequency	Maximum Average Velocity (fps)	Turbulence Level	Required Minimum W50 (lbs) from HDC 712-1	Required Thickness (Inches)
	MAIN STEM					
5	Tunnel Inlet Channel	100-Year	2.25	Low	Less Than 10	12
5A	Upstream from Tunnel Inlet	100-Year	8.0	High	45	21
5	Ponding Area Inlet Drop Structure					
	Upstream from Drop Inlet	100-Year	7.5	Low	Less Than 10	12
	Downstream from Stilling Basin	100-Year	5.1	High	Less Than 10	12
8	Penn Avenue	100-Year	2.6	High	Less Than 10	12
9A	Fruen Mill Drop Structure					
	Upstream from Drop Inlet	100-Year	3.1	Low	Less Than 10	12
	Downstream from Stilling Basin	100-Year	4.8	High	Less Than 10	12
9B	RR Bridge	100-Year	4.2	High	Less Than 10	12
10	Highway 55 Control Structure					
	Upstream from Drop Inlet	100-Year	1.0	Low	Less Than 10	None
13	Noble Avenue Culvert					
	Upstream from Culvert Inlet	100-Year	5.5	Low	Less Than 10	12
	Downstream from Culvert Outlet	100-Year	5.5	High	Less Than 10	12
14	Regent Avenue Culvert					
	Upstream from Culvert Outlet	100-Year	5.2	Low	Less Than 10	12
	Downstream from Culvert Outlet	100-Year	5.2	High	Less Than 10	12
15A	Bridge Removal Minnagua Avenue					
	River Channel	100-Year	2.3	Low	Less Than 10	None
16	Highway 100 Control Structure					
	Downstream from Culvert Outlet	100-Year	5.9	High	Less Than 10	12
18	Culvert Replacement Westbrook					
	Upstream from Culvert Inlet	100-Year	5.1	Low	Less Than 10	12
	Downstream from Culvert Inlet	100-Year	5.1	High	Less Than 10	12

Table A-4 (Cont'd) Riprap Required

Feature Number	Location	Design Flood Frequency	Maximum Average Velocity (fps)	Turbulence Level	Required Minimum W50 (lbs) from HDC 712-1	Required Thickness (Inches)
19	Golden Valley Country Club Structure					
	Upstream from Culvert Inlet	100-Year	8.8	Low	11	12
	Downstream from Culvert Outlet	100-Year	8.8	High	84	33
20	Wisconsin Avenue Control Structure					
	Upstream from Inlet Control Structure	100-Year	7.7	Low	Less Than 10	12
	Downstream from Culvert Outlet	100-Year	7.7	High	36	24
21	Medicine Lake					
	Upstream from Control Weir	100-Year	2.5	Low	Less Than 10	None
	Downstream from Control Weir Through RR Bridge Outlet	100-Year	2.5	High	Less Than 10	12
	NORTH BRANCH					
23	Culvert Replacement 32rd Avenue					
	Upstream from Culvert Inlet	100-Year	4.4	High	Less Than 10	12
	Downstream from Culvert Outlet	100-Year	7.3	High	27	21
24	Culvert Replacement Brunswick Avenue					
	Upstream from Culvert Inlet	100-Year	4.4	Low	Less Than 10	12
	Downstream from Culvert Outlet	100-Year	4.4	High	Less Than 10	12
28	Culvert Replacement 34th Avenue					
	Upstream from Culvert Inlet	100-Year	10.7	Low	37	18
	Downstream from Culvert Outlet	100-Year	10.7	High	270	48
29	Culvert Replacement Douglas Drive					
	Upstream from Culvert Inlet	100-Year	4.2	Low	Less Than 10	12
	Downstream Culvert Outlet	100-Year	4.2	High	Less Than 10	12
31	Edgewood Control Structure					
	Upstream from Culvert Inlet	100-Year	8.7	Low	11	12
	Downstream from Culvert Outlet	100-Year	16.9	High	80*	30

*From USAEWES TMP H-72-5

Table A-4 (Cont'd) Riprap Required

Feature Number	Location	Design Flood Frequency	Maximum Average Velocity (fps)	Turbulence Level	Required Minimum W50 (lbs) from HDC 712-1	Required Thickness (inches)
33	Culvert Replacement Georgia Avenue					
	Upstream from Culvert Inlet	100-Year	6.4	Low	Less Than 10	12
	Downstream from Culvert Outlet	100-Year	6.4	High	13	18
34	Culvert Replacement 36th Street					
	Downstream from Culvert Outlet	100-Year	6.0	High	Less Than 11	12

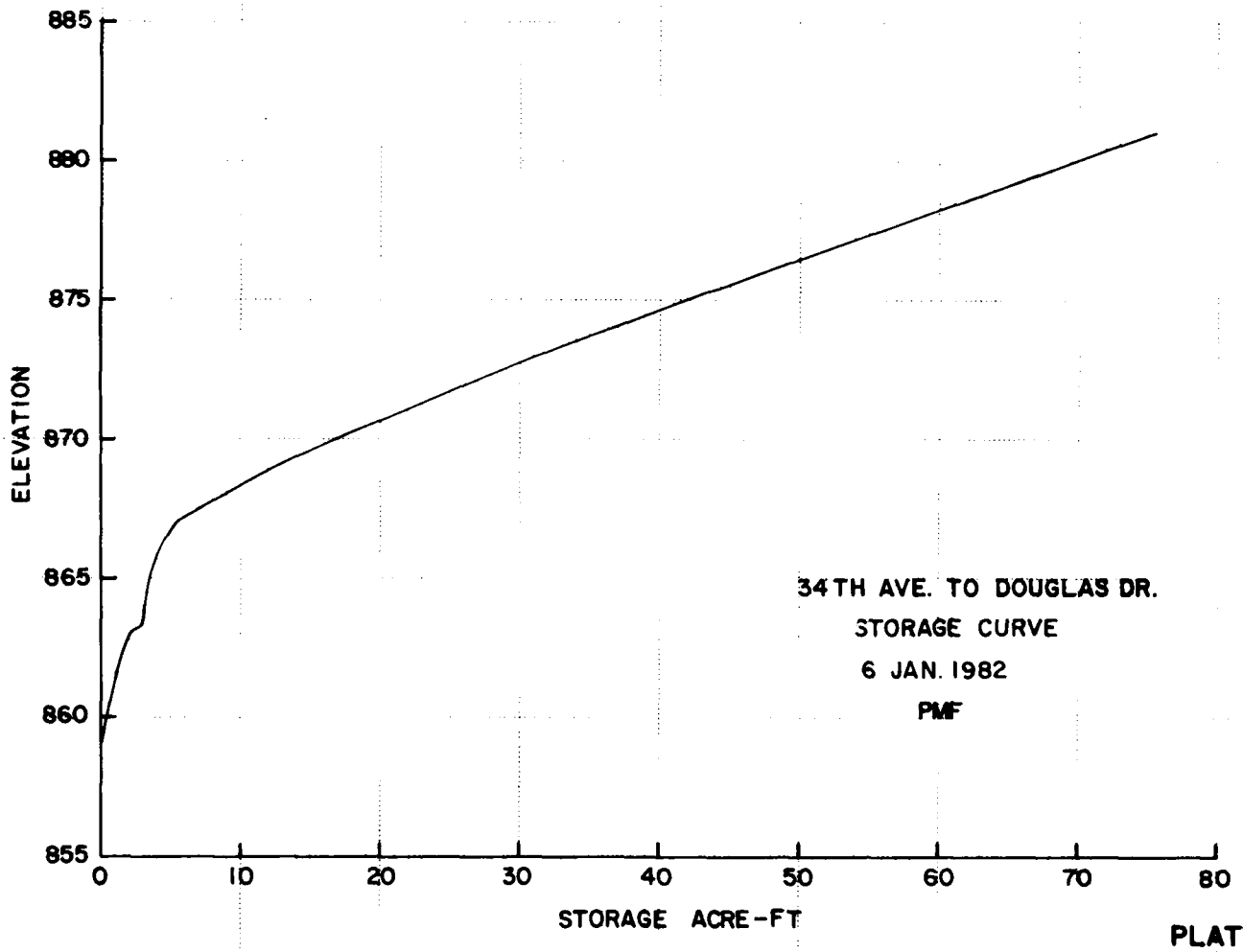

885
880
875
870
865
860
855
ELEVATION
0
10
20
30
40
50
60
70
80
STORAGE ACRE-FT
34TH AVE. TO DOUGLAS DR.
STORAGE CURVE
6 JAN. 1982
PMF

PLATE A-1

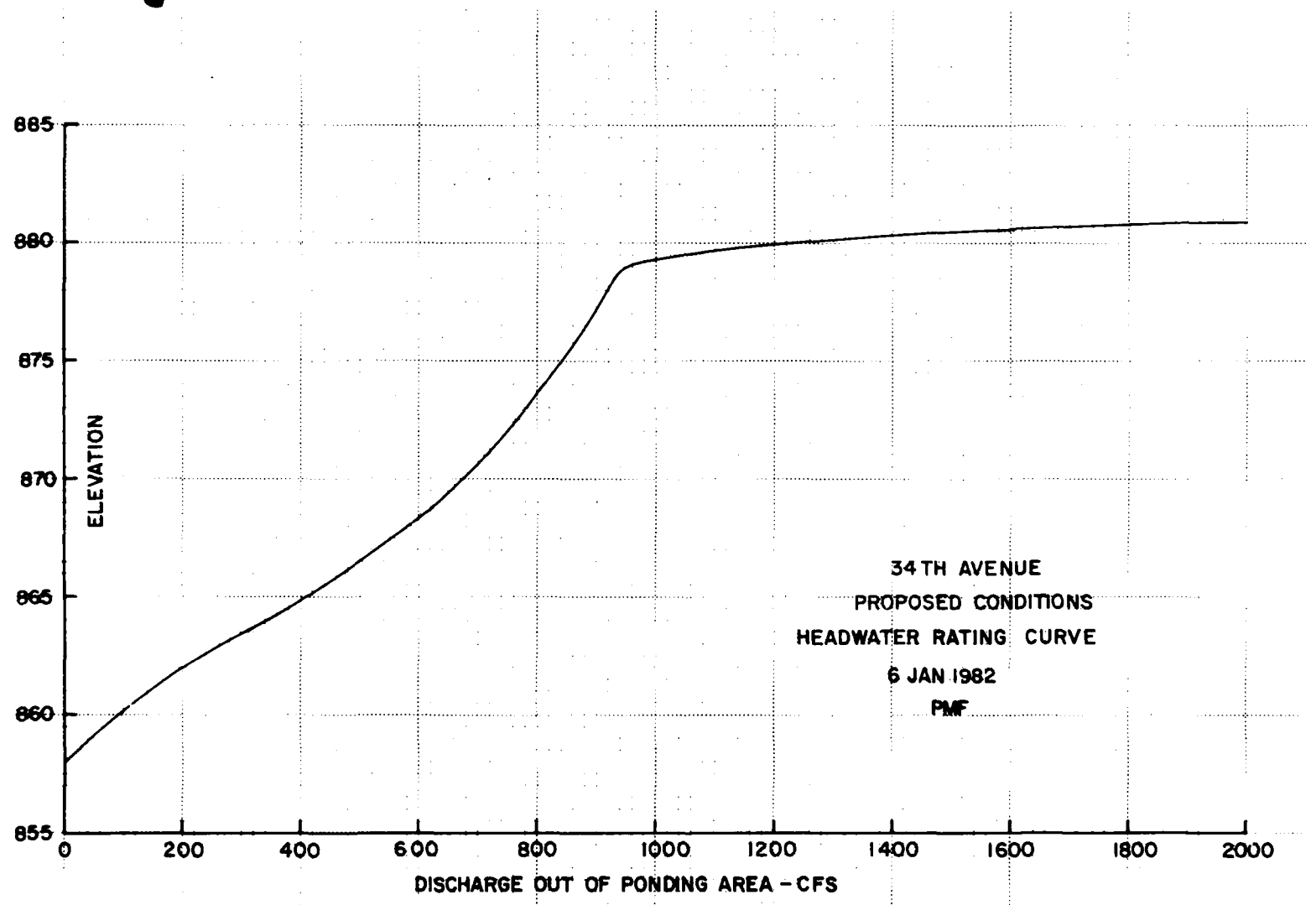

885
880
875
870
865
860
855
ELEVATION
0
200
400
600
800
1000
1200
1400
1600
1800
2000
DISCHARGE OUT OF PONDING AREA - CFS
34TH AVENUE
PROPOSED CONDITIONS
HEADWATER RATING CURVE
6 JAN 1982
PMF

PLATE A-2

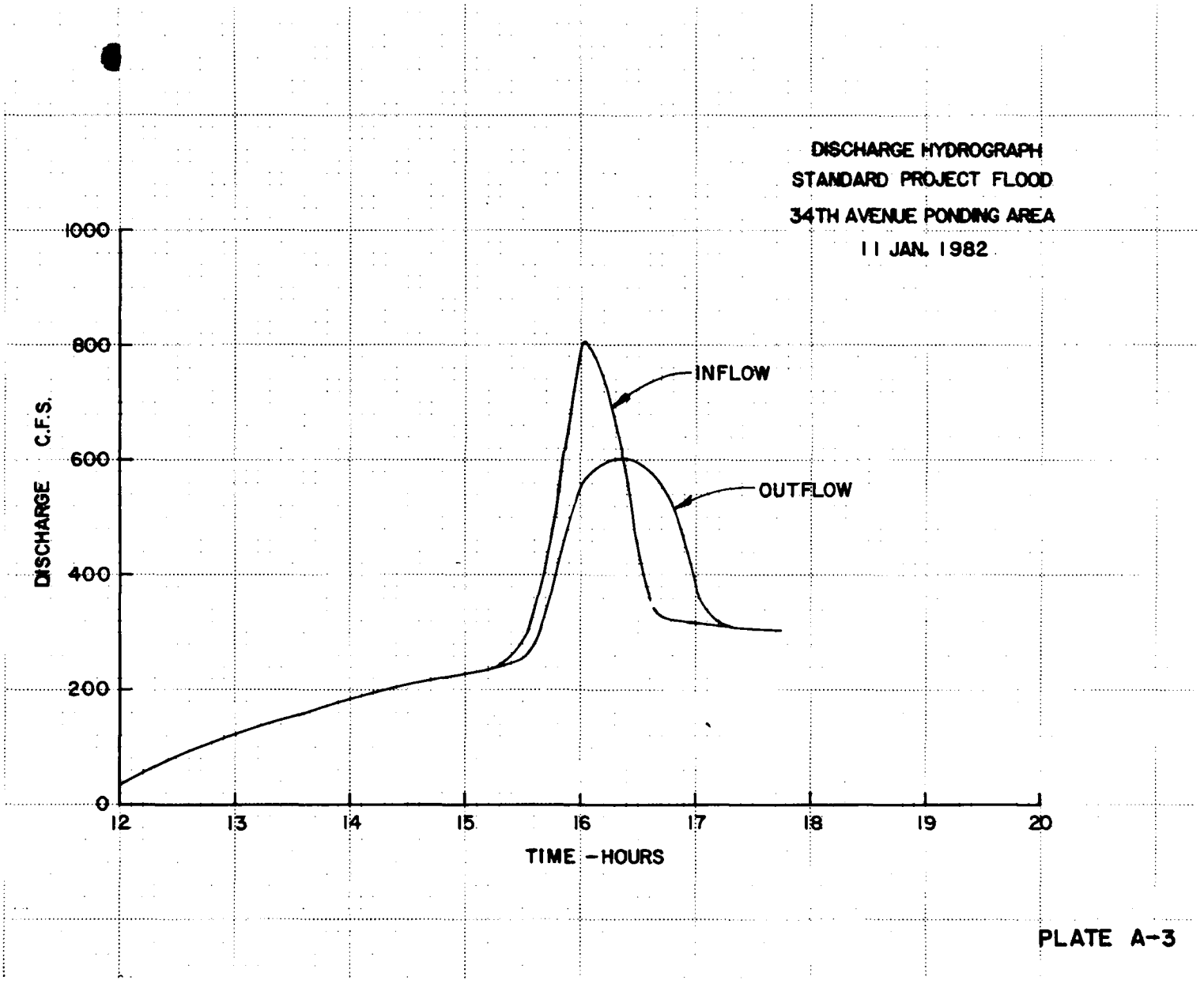

DISCHARGE HYDROGRAPH
STANDARD PROJECT FLOOD
34TH AVENUE PONDING AREA
11 JAN. 1982
INFLOW
OUTFLOW
DISCHARGE C.F.S.
0
200
400
600
800
1000
12
13
14
15
16
17
18
19
20
TIME - HOURS

PLATE A-3

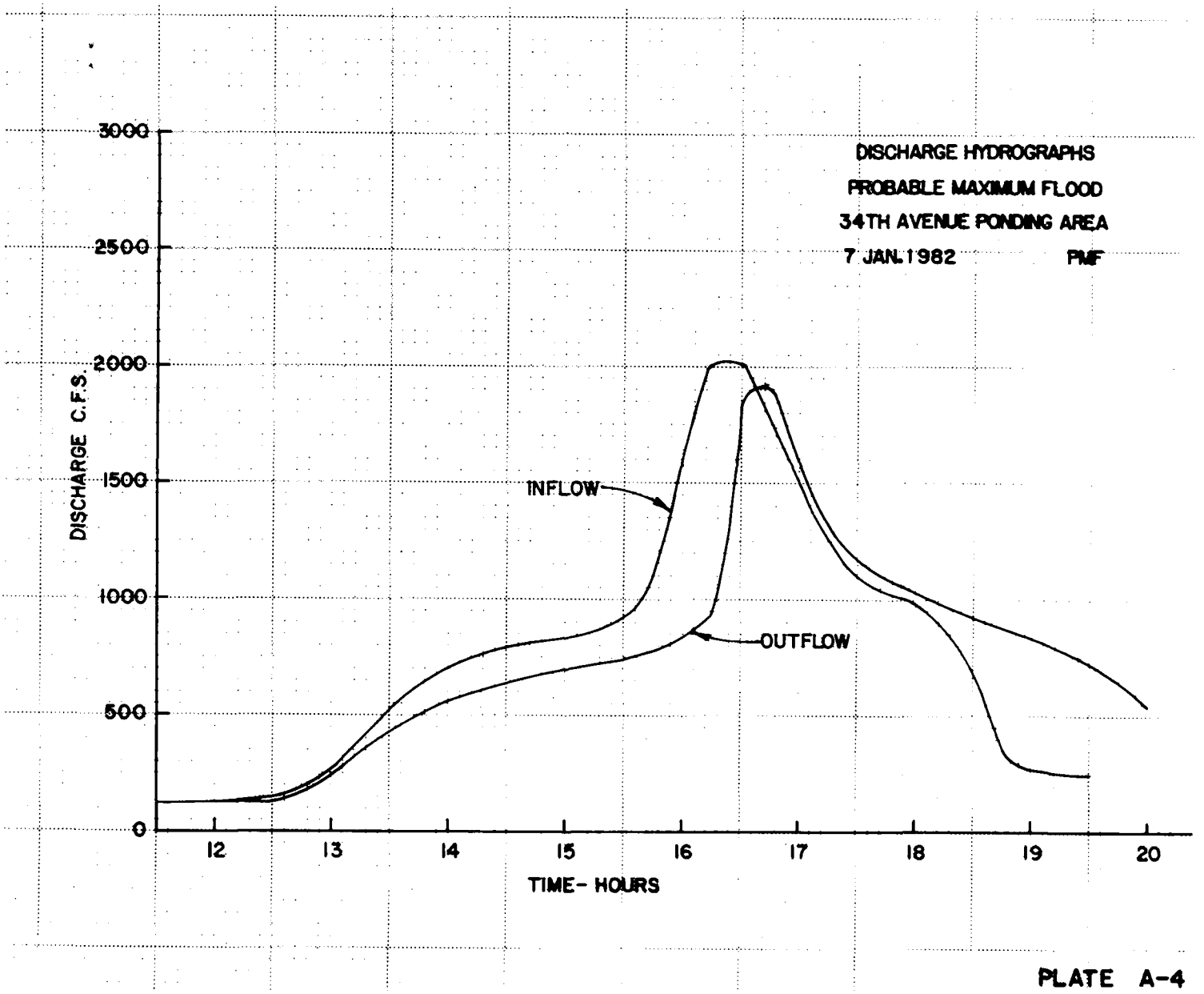

DISCHARGE HYDROGRAPHS
PROBABLE MAXIMUM FLOOD
34TH AVENUE PONDING AREA
7 JAN. 1982 PMF
DISCHARGE C.F.S.
3000
2500
2000
1500
1000
500
0
INFLOW
OUTFLOW
12
13
14
15
16
17
18
19
20
TIME- HOURS

PLATE A-4

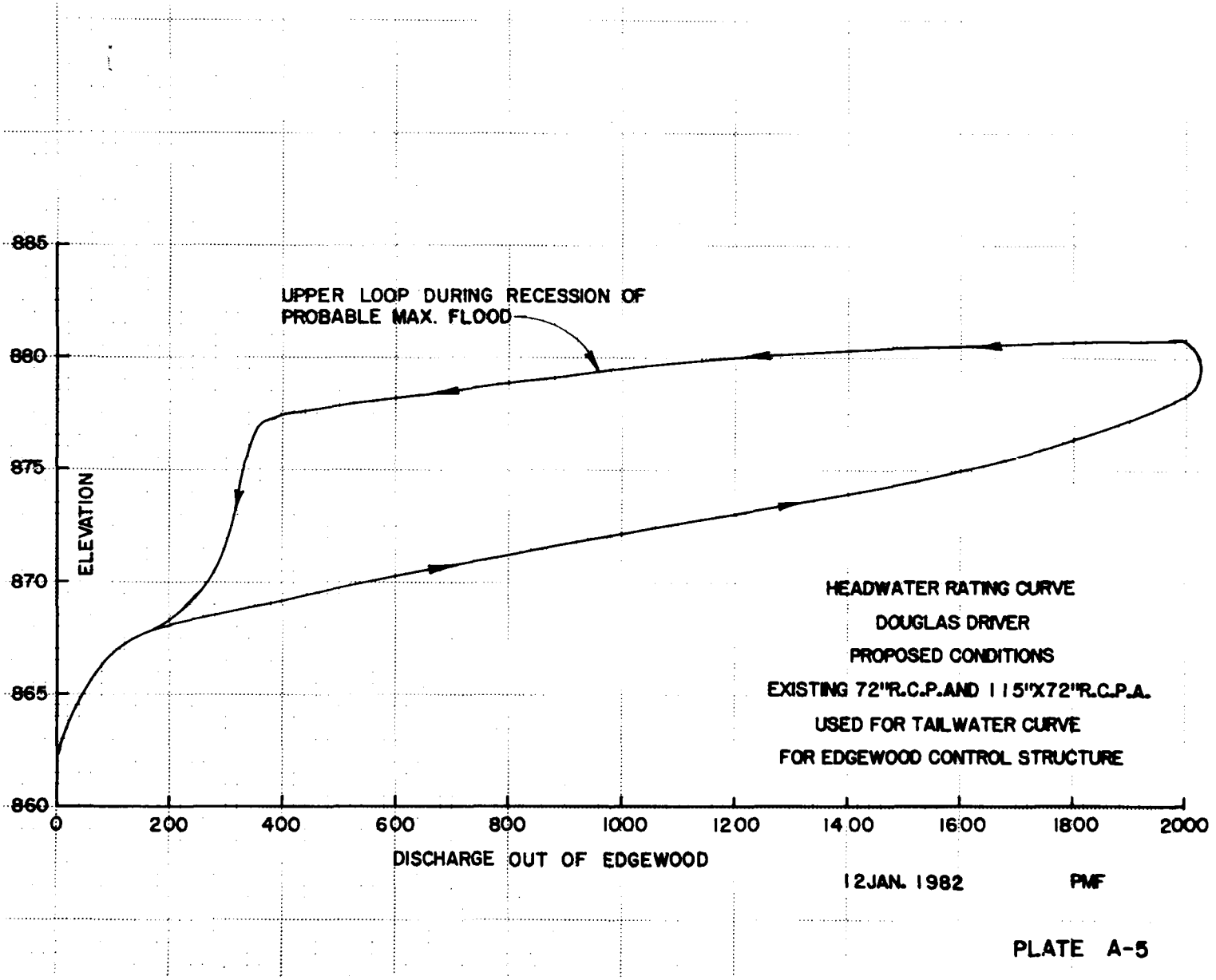

885
880
875
870
865
860
ELEVATION
UPPER LOOP DURING RECESSION OF
PROBABLE MAX. FLOOD
0
200
400
600
800
1000
1200
1400
1600
1800
2000
DISCHARGE OUT OF EDGEWOOD
HEADWATER RATING CURVE
DOUGLAS DRIVER
PROPOSED CONDITIONS
EXISTING 72"R.C.P.AND 115"X72"R.C.P.A.
USED FOR TAILWATER CURVE
FOR EDGEWOOD CONTROL STRUCTURE
12JAN. 1982
PMF

PLATE A-5

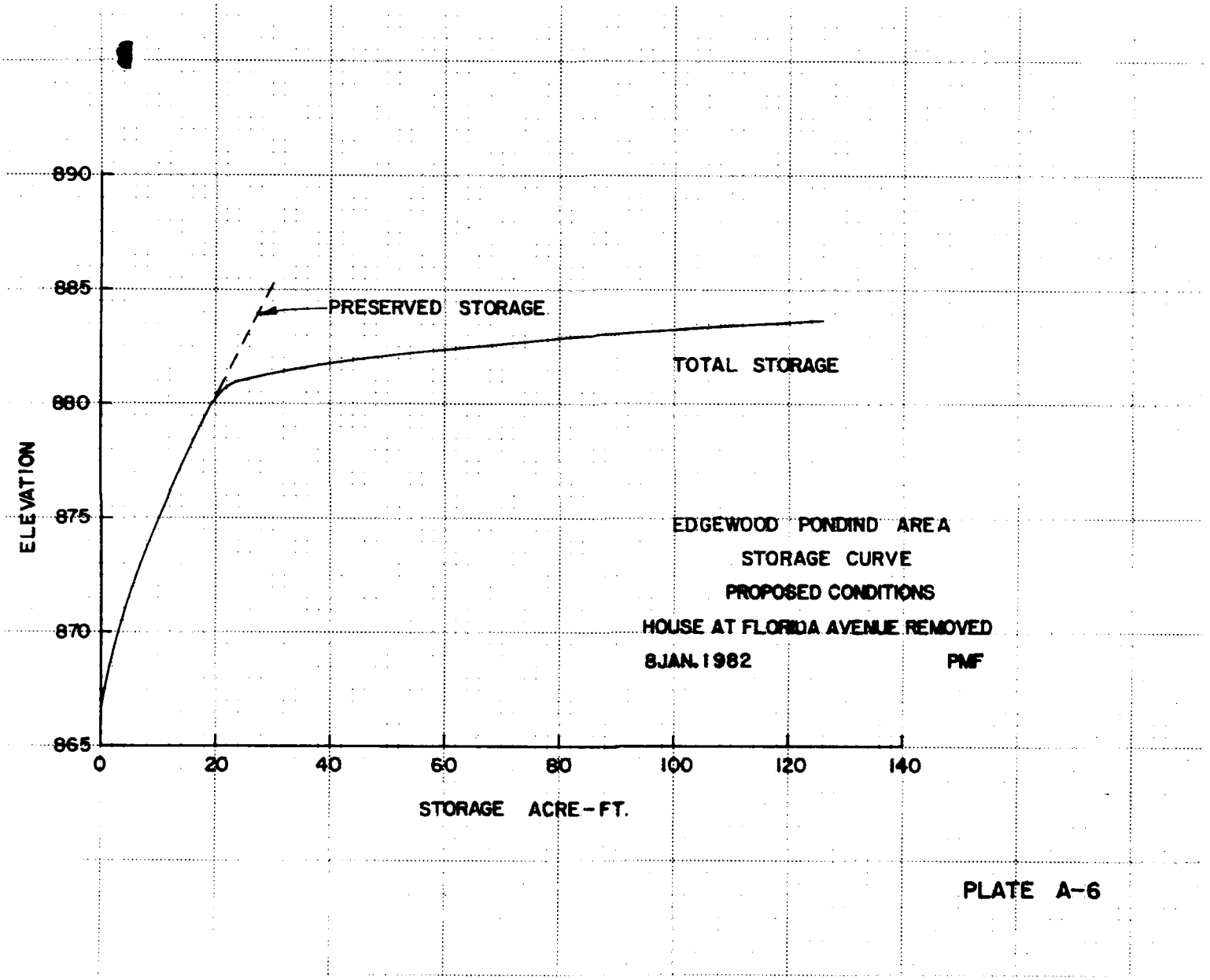

890
885
880
875
870
865
ELEVATION
PRESERVED STORAGE
TOTAL STORAGE
0
20
40
60
80
100
120
140
STORAGE ACRE-FT.
EDGEWOOD PONDIND AREA
STORAGE CURVE
PROPOSED CONDITIONS
HOUSE AT FLORIDA AVENUE REMOVED
8JAN. 1982
PMF

PLATE A-6

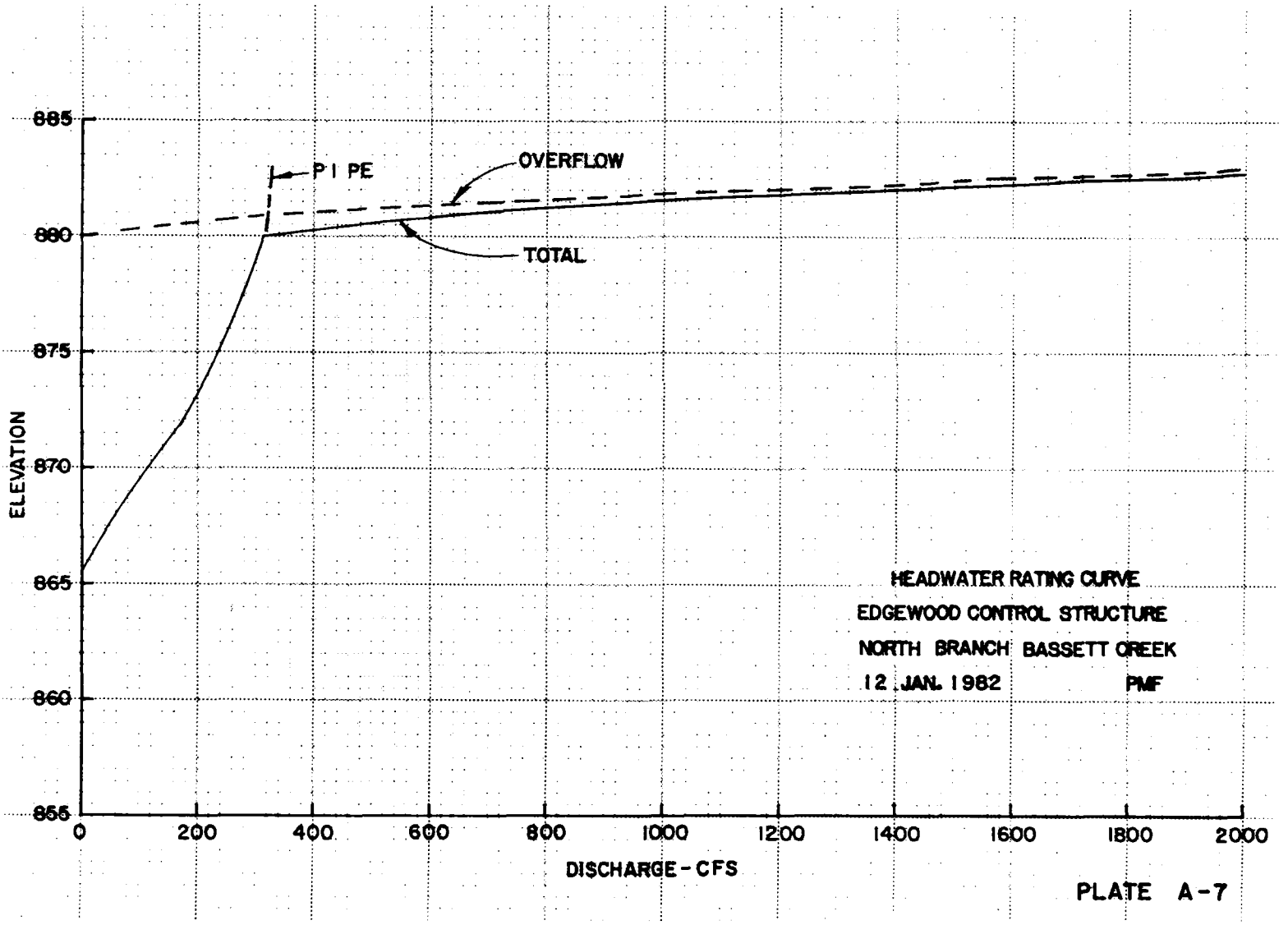

885
880
875
870
865
860
855
ELEVATION
PIPE
OVERFLOW
TOTAL
0
200
400
600
800
1000
1200
1400
1600
1800
2000
DISCHARGE - CFS
HEADWATER RATING CURVE
EDGEWOOD CONTROL STRUCTURE
NORTH BRANCH BASSETT CREEK
12 JAN. 1982
PMF

PLATE A-7

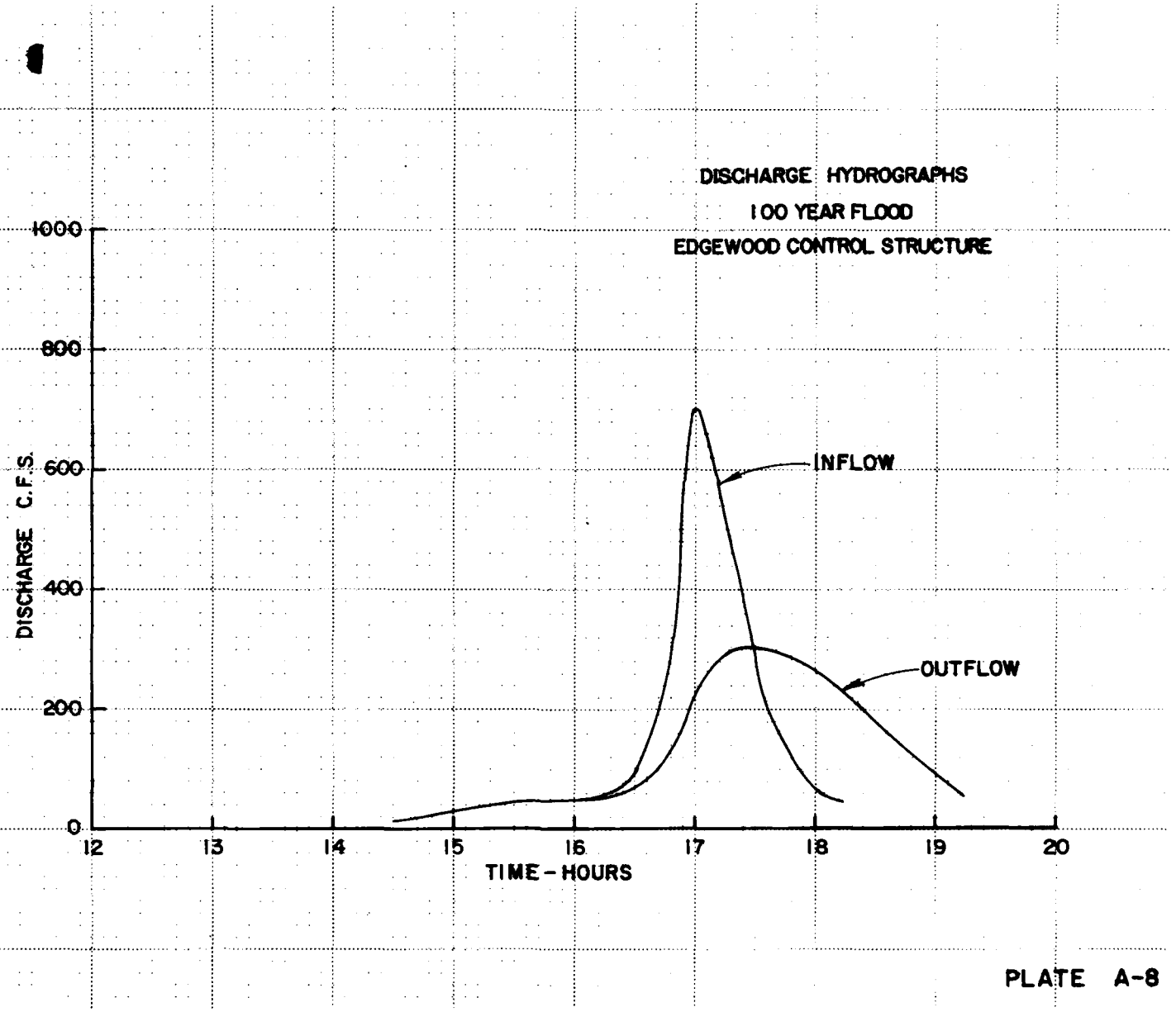

DISCHARGE HYDROGRAPHS
100 YEAR FLOOD
EDGEWOOD CONTROL STRUCTURE
1000
800
600
400
200
0
DISCHARGE C.F.S.
INFLOW
OUTFLOW
12
13
14
15
16
17
18
19
20
TIME - HOURS

PLATE A-8

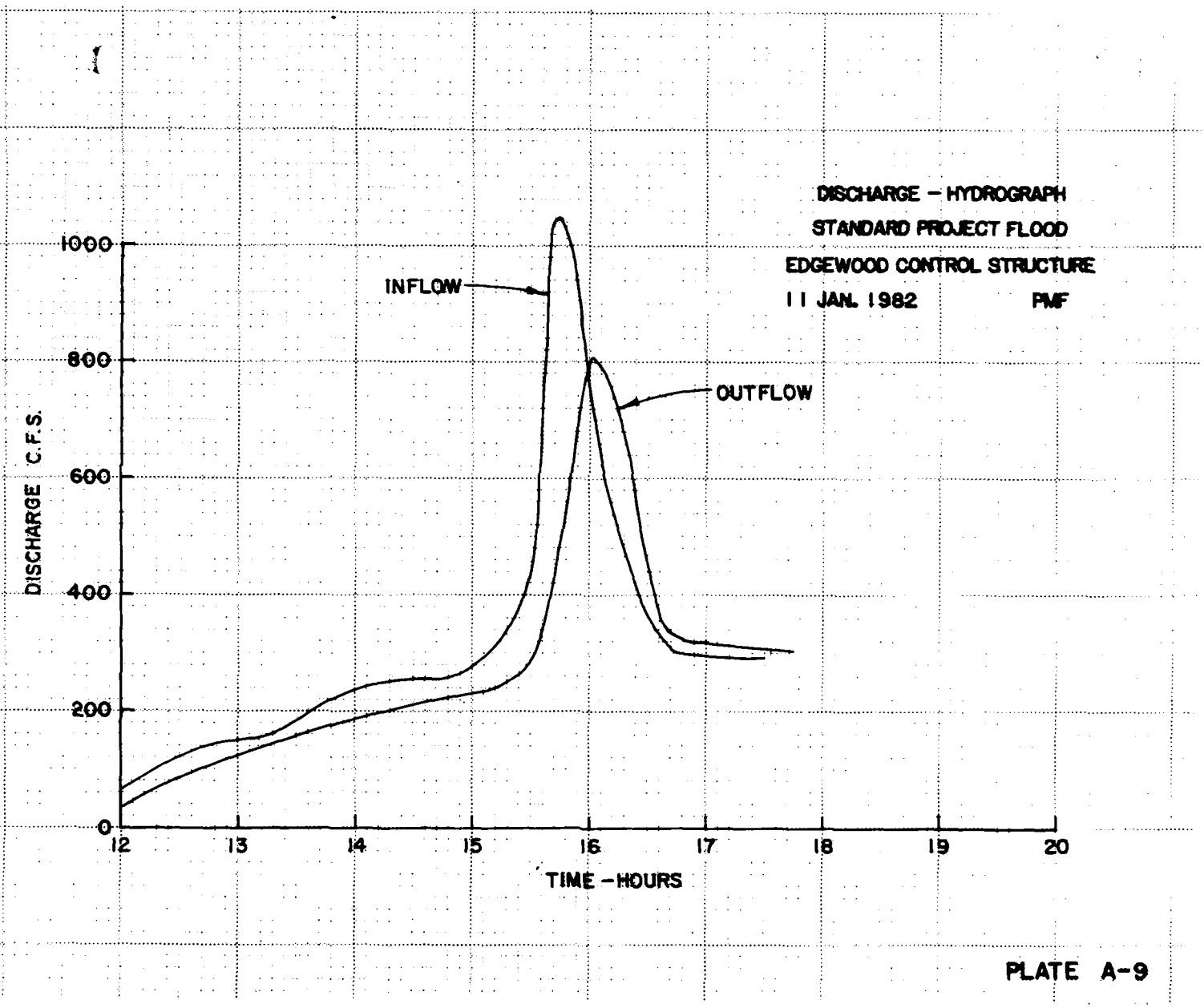

DISCHARGE – HYDROGRAPH
STANDARD PROJECT FLOOD
EDGEWOOD CONTROL STRUCTURE
11 JAN. 1982 PMF
INFLOW
OUTFLOW
DISCHARGE C.F.S.
1000
800
600
400
200
0
12
13
14
15
16
17
18
19
20
TIME – HOURS

PLATE A-9

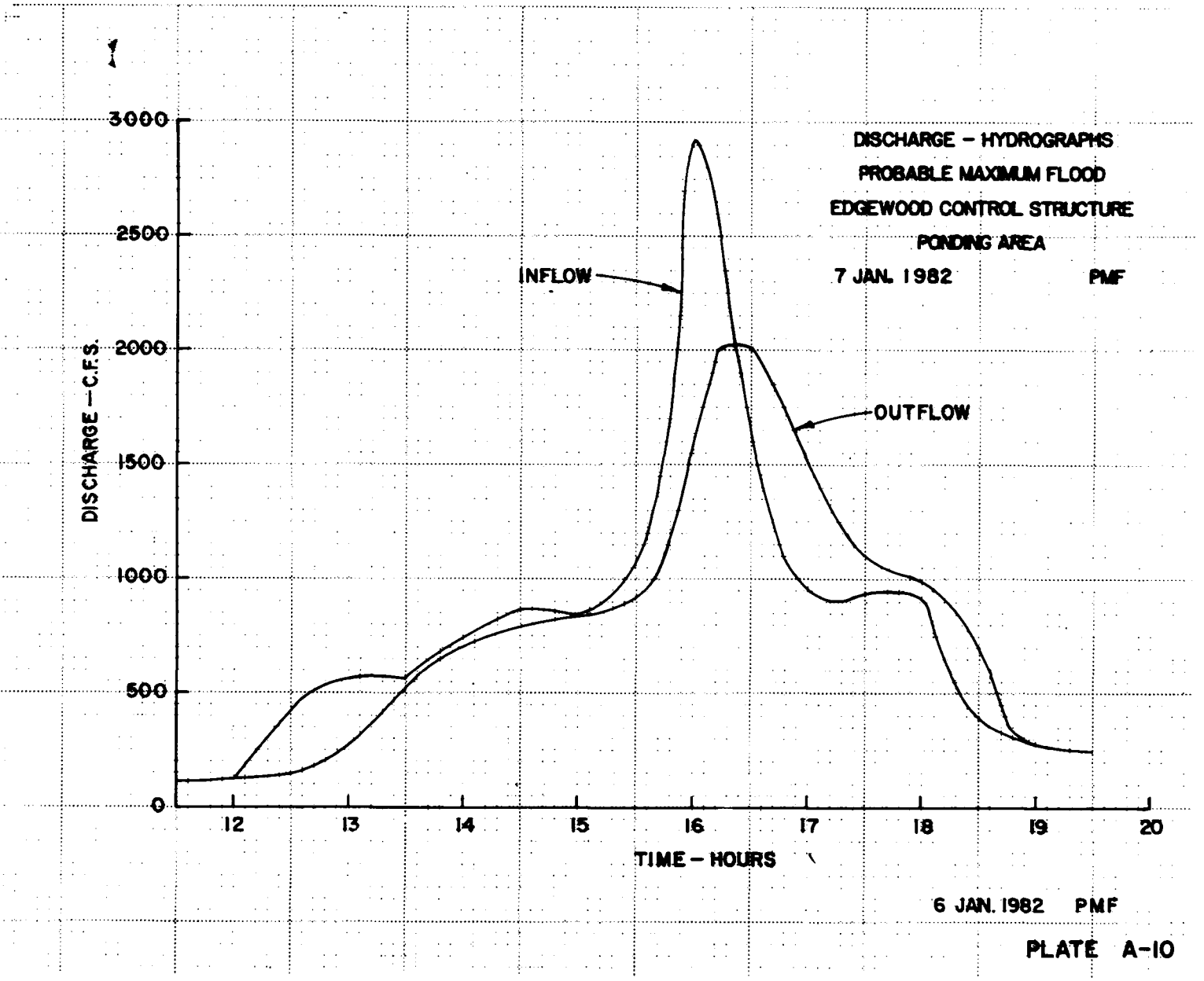
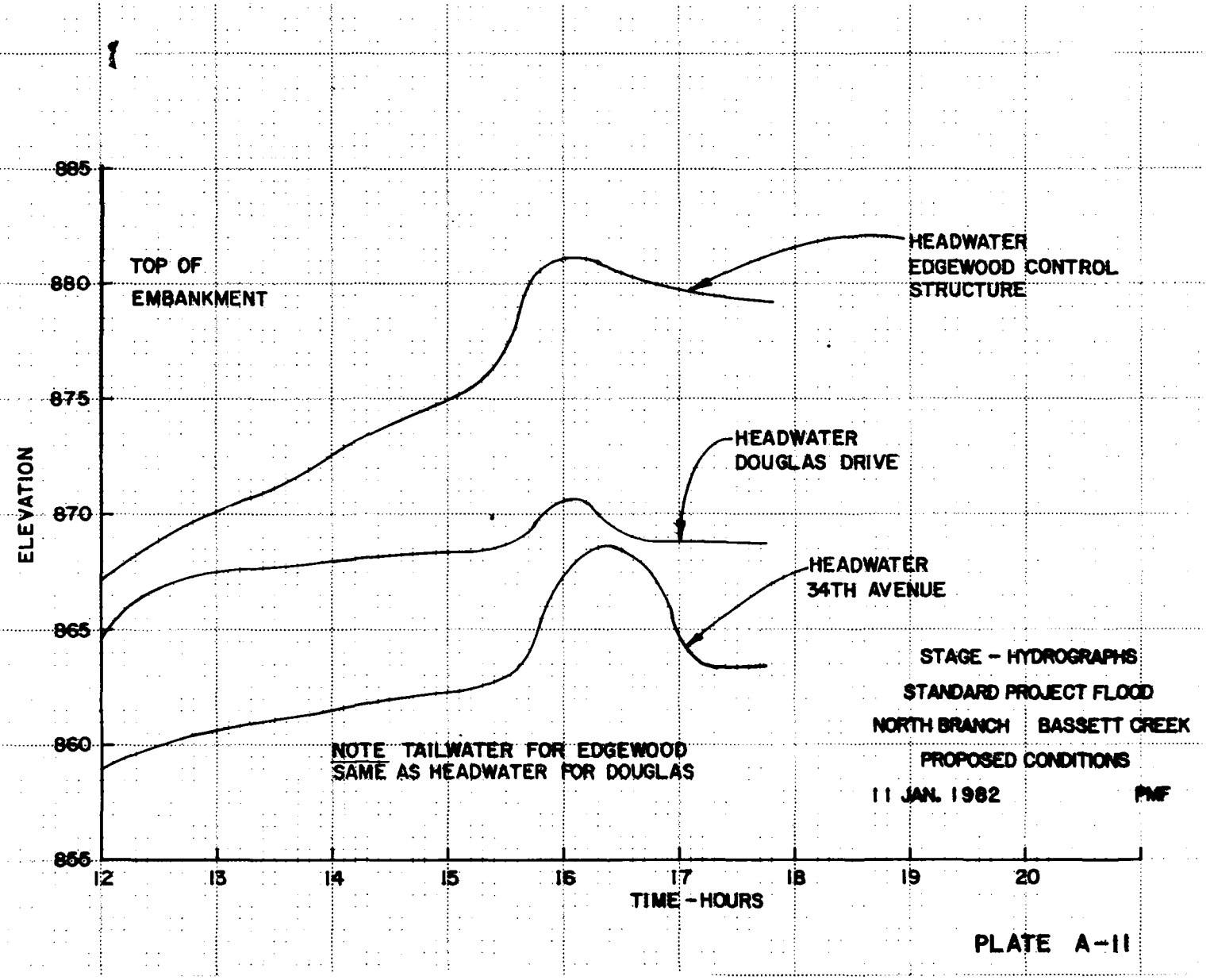

DISCHARGE – HYDROGRAPHS
PROBABLE MAXIMUM FLOOD
EDGEWOOD CONTROL STRUCTURE
PONDING AREA
7 JAN. 1982 PMF
INFLOW
OUTFLOW
DISCHARGE – C.F.S.
3000
2500
2000
1500
1000
500
0
12
13
14
15
16
17
18
19
20
TIME – HOURS
6 JAN. 1982 PMF

PLATE A-10



STAGE - HYDROGRAPHS

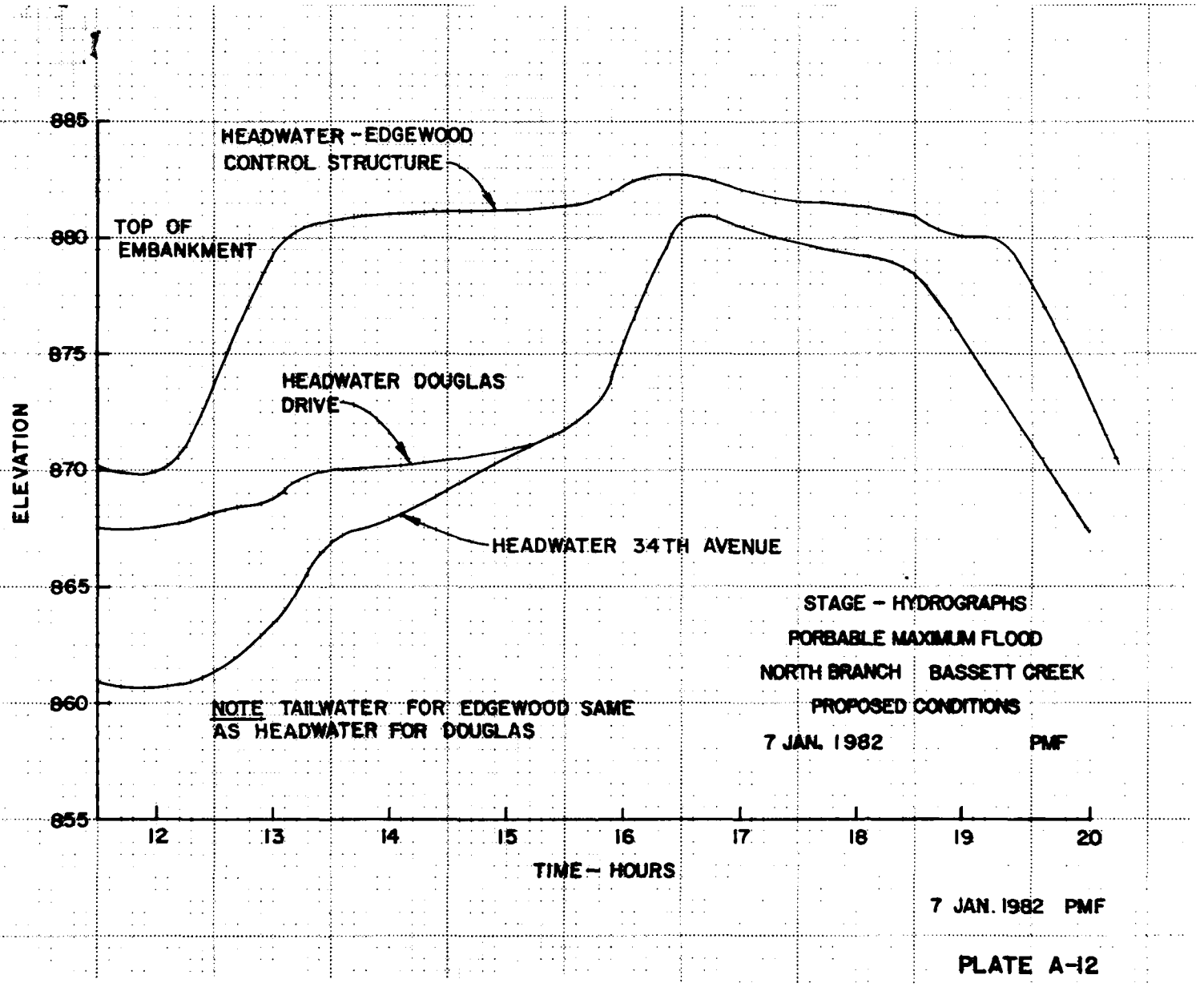
STANDARD PROJECT FLOOD

NORTH BRANCH BASSETT CREEK

PROPOSED CONDITIONS

11 JAN. 1982 PMF

PLATE A-11



STAGE - HYDROGRAPHS
PORBABLE MAXIMUM FLOOD
NORTH BRANCH BASSETT CREEK
PROPOSED CONDITIONS
7 JAN. 1982 PMF

7 JAN. 1982 PMF

PLATE A-12

DEPARTMENT OF THE ARMY
St. Paul District Corps of Engineers
1135 U.S. Post Office & Custom House
St. Paul, Minnesota 55101

FLOOD CONTROL

BASSETT CREEK WATERSHED

HENNEPIN COUNTY, MINNESOTA

DESIGN MEMORANDUM NO. 2, PHASE II

APPENDIX B

INTERIOR FLOOD CONTROL

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Plates (Cont.)

Number	
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APPENDIX B

INTERIOR FLOOD CONTROL

GENERAL

1. PROJECT PLAN

Interior flood control features are required for four areas of the Bassett Creek watershed. The four areas are referred to as the Western Avenue-Winnetka Avenue area (shown on Plate B-5), the Highway 100 North area (shown on Plate B-6), the Fruen Mill area below Glenwood Avenue (shown on Plate B-7) and the Bermed area by the conduit entrance (shown on Plate B-8). Flood control features for the low area northeast of the intersection of Western Avenue and Winnetka Avenue, as shown on Plate B-1, consist of a ponding area to elevation 886.1 and a flap-gated 24-inch CMP culvert under Winnetka Avenue and the proposed berm. Proposed features required north of the Highway 100 control structure, as shown on Plate B-2, include a ditch with 2-foot base width and 1 on 3 side slopes parallel to the levee, a 2-acre ponding area upstream and adjacent to the ditch, and a flap-gated 36-inch CMP culvert under Medicine Lake Road from the proposed interior ditch to the proposed ditch between the control structure and existing box culverts under Highway 100. The proposed interior flood control plan for the Fruen Mill area, as shown on Plates B-3 and B-7, consists of an excavated ponding area in the Glenwood-Inglewood parking lot and a flap-gated 36-inch RCP outlet at the southeast corner of the lot. Proposed features required for the Bermed area along the north side of the conduit entrance storage area, as shown on Plate B-4, include an excavated ponding area near Girard Avenue and the railroad tracks and a flap-gated 48-inch RCP culvert at the southwest corner of the ponding area.

2. CHANGES FROM DESIGN MEMORANDUM NO. 1

Since the completion of Design Memorandum No. 1, interior flood control features have been added for the Western Avenue-Winnetka Avenue area and the Bermed area along the north side of the conduit entrance storage area. These areas were added when it was found that water stored above the Wisconsin Avenue control structure and the conduit entrance could back up into each of these respective areas. Proposed features for the Highway 100 south area were eliminated when the control structure was moved about 150 feet upstream. Features for the Highway 100 North area were altered slightly because of this move. The interior flood control plan for the Fruen Mill area remains essentially the same as in Design Memorandum No. 1.

3. DESCRIPTION OF WATERSHEDS AND DRAINAGE PATTERNS

The Western Avenue-Winnetka Avenue watershed consists of about 83.6 acres of residential and undeveloped property as shown on Plate B-5. Most of the undeveloped area is very low and swampy. The area is generally bounded by Winnetka Avenue and high ground on the west and by high ground on the other sides. Runoff presently flows into the low area located in the middle of the watershed from all directions. Water ponded in the low area is eventually drained through a small CMP culvert of unknown size under Winnetka Avenue. Even after the proposed berm is constructed, runoff will continue to flow as it presently does, except that the ponded water will flow under Winnetka Avenue and the berm through the proposed 24-inch CMP culvert. In instances when Bassett Creek overtops its right bank and prevents outflow through the proposed culvert, runoff will pond in the low area until the creek recedes.

4. The area north of the Highway 100 control structure consists of about 23.6 acres of residential and undeveloped property as shown on Plate B-6. It is bounded by high ground to the north and west, Highway 100 to the east, and the proposed levee to the south and west. Runoff presently flows southwest into the creek. Once the proposed levee is constructed, runoff will temporarily pond in the low area along the levee, then flow along the proposed ditch and through the 36-inch CMP culvert into the ditch between the control structure and the existing box culverts under Highway 100.

5. The Fruen Mill watershed consists of about 6.3 acres of commercial property as shown on Plate B-7. It is bounded by Glenwood Avenue to the north, Bassett Creek to the west, and the Minneapolis Northfield and Southern Railroad tracks to the east. On the south side, the area will be bounded by a small proposed embankment and high ground. Runoff from the area presently flows to the south into the Glenwood-Inglewood parking lot and on into the creek. With construction of the proposed embankment, runoff will flow into the proposed ponding area in the Glenwood-Inglewood parking lot. From there it will discharge into Bassett Creek through the proposed 30-inch RCP outlet.

6. The watershed for the Bermed area northwest of the conduit entrance, as shown on Plate B-8, includes about 52.7 acres of residential and industrial property. It is bounded by high ground along Glenwood Avenue to the north, high ground to the east and west, and the proposed berm and high ground to the south. Under present conditions, runoff generally flows south into Bassett Creek and is aided by a RCP storm sewer system, consisting of pipe up to 21 inches in diameter, along 2nd Avenue North which discharges into the existing Bassett Creek tunnel. Once the proposed berm is constructed, runoff will flow into the proposed excavated ponding area near Girard Avenue and the railroad tracks. From there it will discharge into Bassett Creek through the proposed 48-inch RCP outlet.

7. PONDING AREAS

Elevation-area-storage curves for each of the four ponding areas are shown on Plates B-9 through B-12. Ponding will occur in the Western Avenue-Winnetka Avenue area at the low area northeast of the intersection, in the Highway 100 North area at the low area along the proposed levee, in the Fruen Mill area at the Glenwood-Inglewood parking lot, and in the Bermed area northwest of the conduit entrance at the excavated area near Girard Avenue and the railroad tracks.

8. DAMAGE-ELEVATION RELATIONSHIPS

Elevation-damage curves for the four areas are not available at this time. Approximate zero damage elevations are 887.3 in the Western Avenue-Winnetka Avenue area, 845.0 in the Highway 100 North area, 812.2 in the Fruen Mill area, and 805.1 in the Bermed area by the conduit entrance.

DESIGN CRITERIA

9. DEGREE OF PROTECTION

The Western Avenue-Winnetka Avenue area and the Highway 100 North area, are considered to be Class II urban development, as defined in EM 1110-2-1410 (reference 21a)*. The Fruen Mill area and Bermed area by the conduit entrance are considered to be Class I development. The design of features for the non-flood gravity discharge condition for Class II development is generally based on a 50-year rainfall frequency, while the design for Class I development is usually based on a 100-year rainfall frequency. However, since two of the four areas must be designed for the 100-year event and since the 100-year peak inflow rates for the other two areas are not substantially greater than the 50-year peak inflow rates, all areas were designed for the 100-year rainfall event. Protection against a standard project flood is not provided.

10. STREAMFLOW DATA

Discharge frequency curves for the main stem of Bassett Creek at Highway 100, at Highway 55 (just above the Fruen Mill area) and at the conduit entrance are shown on Plates 18, 21 and 22, respectively of Design Memorandum No. 1. Discharge rating curves for the main stem just below the Highway 100 control structure and below Glenwood Avenue are shown on Plates B-13 and B-14 while a headwater-discharge curve for the conduit entrance is shown on Plate B-15. Since streamflow records for the Bassett Creek watershed are incomplete, no stage-duration curves are provided.

*This reference and all references which follow are described in paragraph 21.

11. RAINFALL DATA

The 24- and 96-hour duration rainfall depths for the 2-, 5-, 10-, 25-, 50- and 100-year all-year theoretical rainfall events for the study areas were obtained from U.S. Weather Bureau Reports Nos. 40 and 49, "Rainfall Frequency Atlas of the United States" (references 21b and 21c). These point rainfall depths are shown in Table B-1 along with the 1-year point rainfall depth which was obtained from the exceedence frequency versus point rainfall depths plot shown on Plate B-16. Also shown in Table B-1 are standard project storm values for the 24- and 96-hour durations which were developed in accordance with criteria presented in EM 1110-2-1411 (reference 21d). Because of the small size of the Bassett Creek watershed, rainfall is assumed to occur simultaneously over the entire watershed. Based on this assumption, interior watershed runoff and streamflow runoff are dependent on one another.

12. RAINFALL EXCESS AND RUNOFF HYDROGRAPHS

Key parameters used in development of the runoff hydrographs are shown in Table B-2. Surface cover, slopes and flow lengths were obtained from aerial photos and USGS quad sheets. Flow velocities in each area were obtained using Figure 3-1 in Soil Conservation Service Technical Release (TR) No. 55 (reference 21g). Times of concentration (Tc) were calculated by dividing the overland flow length by the velocity. A SCS unit hydrograph corresponding to the Tc for each area was obtained from Table 5-3 of TR-55 (reference 21g). Runoff curve numbers (CN) for each area were obtained from Table 2-2 of TR-55 and used to obtain the runoff hydrographs. Rainfall excess (Q) was calculated using the following formula:

$$Q = \frac{(P - 0.2\,S)^2}{P + 0.8\,S}$$

in which P is the 24-hour precipitation and $S = \frac{1000}{CN} - 10$. Runoff hydrographs were developed by multiplying the SCS unit hydrograph by Q and the drainage area in square miles. The resulting runoff hydrographs are presented in Tables B-3, B-4 and B-5. Runoff hydrographs were not developed for the Western Avenue-Winnetka Avenue area as the ponding area has enough storage capacity to pond the 96-hour duration standard project storm, assuming 70 percent runoff. The 24-inch CMP culvert under Winnetka Avenue will serve to empty the pond once Bassett Creek recedes. The runoff hydrographs for the Bermed area by the conduit entrance were reduced by the capacity of the existing storm sewer, about 33 cfs, which outlets into the existing Bassett Creek tunnel.

13. SEEPAGE

Due to the short duration of flood conditions, seepage was considered to be negligible.

DESIGN OF INTERIOR FLOOD CONTROL FEATURES

14. GENERAL

Each time there is a significant amount of rainfall runoff from each of the protected areas there is also a simultaneous rise in the level of Bassett Creek. Because of this condition, the design of the required gravity flow and blocked gravity flow interior flood control features was performed simultaneously. The location of the proposed interior flood control features are shown on Plates B-1, -2, -3 and -4 and defined in paragraph 1 and in Tables B-6 and B-7. The proposed drainage plan is described in paragraphs 3 through 6. One-percent and standard project storm interior pond levels for both existing and proposed conditions are shown in Table B-9.

15. DESIGN OF OUTLETS AND DITCHES

The hydraulic design of the outlets is based on procedures outlined in TM 5-820-4 (reference 21e) and on the inflow from the 100-year frequency rainfall event. Design data for each outlet are presented in Table B-6. Manning's roughness coefficient "n" is assumed to be 0.014 for RCP and 0.024 for CMP, and the entrance loss coefficient "Ke" is assumed to be 0.5. Pipe sizes were selected to maintain design water surface elevations below the zero damage elevation during a 100-year rainfall event. A discharge rating curve for each of the four outlets is shown on Plates B-17 through B-20. All outlets will be furnished with safety guards to improve safety and reduce debris deposition in pipes. All gravity outlets will be equippped with flap-gates.

16. The design of the one drainage ditch is also based on criteria presented in TM 5-820-4 (reference 21e) and on the peak 100-year outflow through the proposed Highway 100 North outlet. Design data for the ditch are presented in Table B-7. The ditch will be grass-lined with 1 on 3 side slopes. Manning's roughness coefficient "n" is assumed to be 0.040.

17. MAXIMUM POND LEVELS

The maximum interior pond levels shown in Table B-8, for all areas, except the Western Avenue-Winnetka Avenue area, were obtained by routing the runoff hydrographs for each area through the proposed ponding areas and outlets and assuming no outflow after the Bassett Creek stage equalled

the interior pond elevation. Runoff hydrographs for Bassett Creek are only available for the 100-year event and the standard project storm. However, since the standard project storm (SPS) overtops the levee or berm in all three areas, the check for when the Bassett Creek stage equalled the interior pond elevation was done for the 100-year event only. Routing the 100-year storm through each of the three areas resulted in two peak ponding levels. First based on gravity flow conditions and second based on blocked gravity conditions. (During a 100-year storm over the entire basin, it is estimated that almost all the interior runoff will discharge into the creek before the river rises to the level required for blocked gravity conditions.) In all three areas, the maximum elevation obtained during gravity flow condition was considerably higher than the elevation reached by ponding all inflow after the river stage equalled the interior pond elevation. As this was true for the 100-year event, it was assumed to hold true for the lower frequency storms also. Therefore, the elevations in Table B-8 for the Highway 100 North area, the Fruen Mill area and the Bermed area by the conduit entrance are the maximum gravity flow elevations. Since there is excess storage available in the Western Avenue-Winnetka Avenue ponding area, the pond levels in Table B-8 are for ponding the 96-hour duration storm assuming 70 percent runoff. (This area is currently owned and maintained by the city of Golden Valley as a wetland area with no future changes in land uses anticipated.) Since rainfall and river stage are dependent events for the Bassett Creek watershed, the probability of a given river stage is assumed to be equivalent to the probability of the rainfall event which produced it. The frequencies assigned to the maximum interior pond elevations in Table B-8 are based on this assumption. A fence is recommended around the excavated ponding area for the Bermed area by the conduit entrance as water will be ponded more than 4 feet deep.

18. ONE-PERCENT AND SPS POND LEVELS

One-percent (100-year) and standard project storm pond levels are presented in Table B-9. The standard project storm pond levels shown are the SPF elevation for Bassett Creek obtained from the HEC-2 backwater computations. In all four areas, the proposed levee or berm will be overtopped by the SPF on Bassett Creek. No flood damages are expected during the 100-year event, while considerable damages are expected during the SPF.

19. JUSTIFICATION

Based on the dependent rainfall analysis performed, the recommended interior flood control plan appears to be the most economical and will keep the 100-year pond level below the apparent zero damage elevation. Pumping stations are not necessary as the majority, if not all, of the interior runoff will pass through the outlets before the creek rises enough to close the flap-gates.

20. OTHER ALTERNATIVES CONSIDERED

In addition to the recommended design, various outlet sizes and alternate ponding conditions were considered for each of the four areas. Each of these alternatives, however, were found to be more costly than the recommended plan. For the Bermed area by the conduit entrance, a larger outlet which would eliminate the need for a ponding area was considered, but not recommended because a larger outlet would require a gatewell with sluice gate in addition to the flap-gate, and therefore result in a more costly plan. Also, the use of large flap-gates is very unreliable.

21. REFERENCES

a. EM 1110-2-1410, Interior Drainage of Leveed Urban Areas; Hydrology.

b. National Weather Service Technical Report No. 40, "Rainfall Frequency Atlas of the United States," May 1961.

c. National Weather Service Technical Report No. 49, "Two - To Ten-Day Precipitation for Return Periods of 2 to 100 Years in the Contiguous United States," 1964.

d. EM 1110-2-1411, Standard Project Flood Determinations (Civil Works Engineer Bulletin No. 52-8, March 1952).

e. TM 5-820-4, Drainage for Areas other than Airfields. (EM 1110-345-284)

f. EM 1110-2-1601, "Hydraulic Design of Flood Control Channels."

g. Soil Conservation Service Technical Release No. 55, "Urban Hydrology for Small Watersheds."

TABLE B-1

BASSETT CREEK WATERSHED

POINT RAINFALL VALUES IN INCHES

	DURATION	
Return Period	24-hour	46-hour
1-year	2.17	3.17
2-year	2.75	4.00
5-year	3.52	5.00
10-year	4.14	5.71
25-year	4.75	6.72
50-year	5.28	7.51
100-year	5.92	8.30
SPS*	11.2	13.6

*SPS estimates are for the entire Bassett Creek Watershed rather than point rainfall values.

TABLE B-2

INTERIOR WATERSHED PARAMETERS

Section	Area Mi^2	Area Acres	Slope Ft/Ft	Velocity Ft/Sec	Overland Flow Length Ft.	T_c Hours	SCS Curve Number CN (1)
Highway 100 North	0.037	23.6	0.025	0.9	2,000	0.62	87
Fruen Mill Area	0.010	6.3	0.035	2.5	800	0.10	89
Bermed Area by conduit entrance	0.082	52.7	0.042	1.4	500(2)	0.88	92
			0.005	0.5	1,400		

(1) Antecedent moisture condition III
(2) The overland flow length was divided into two parts because of the change in slope and cover type.

TABLE B-3

RUNOFF HYDROGRAPHS
HIGHWAY 100 NORTH

RAINFALL EVENT IN YEARS

Time in Hours	1	2	5	10	25	50	100	SPS
10.0	0	0	0	0	0	0	0	0
10.5	0	1	1	1	1	1	1	3
11.0	1	1	1	2	2	2	3	6
11.5	1	2	3	3	4	5	5	12
12.0	16	23	34	43	51	59	68	148
12.5	12	18	25	32	38	44	51	111
13.0	5	7	10	12	14	17	19	42
13.5	3	4	5	7	8	9	11	23
14.0	2	3	4	5	6	7	8	17
14.5	1	2	3	4	5	5	6	13
15.0	1	2	3	3	4	5	5	11
15.5	1	2	2	3	3	4	4	10
16.0	1	1	2	2	3	3	4	8
16.5	1	1	1	2	2	3	3	6
17.0	1	1	1	2	2	2	2	5
17.5	0	1	1	1	1	2	2	4
18.0		1	1	1	1	1	2	4
18.5		1	1	1	1	1	1	3
19.0		0	1	1	1	1	1	3
19.5			1	1	1	1	1	2
20.0			0	1	1	1	1	2
20.5				1	1	1	1	2
21.0				0	0	1	1	1
21.5						0	0	1
22.0								1
22.5								0

TABLE B-4

RUNOFF HYDROGRAPHS
FRUEN MILL AREA BELOW GLENWOOD AVENUE

Time in Hours	RAINFALL EVENT IN YEARS							
	1	2	5	10	25	50	100	SPS
10.0	0	0	0	0	0	0	0	0
10.5	0	0	0	0	0	0	1	1
11.0	0	0	1	1	1	1	1	2
11.5	1	1	1	2	2	2	2	5
12.0	12	17	24	29	35	40	46	98
12.5	1	2	3	3	4	4	5	11
13.0	1	1	2	2	2	3	3	6
13.5	1	1	1	1	2	2	2	5
14.0	0	1	1	1	1	2	2	4
14.5		1	1	1	1	1	2	3
15.0		0	1	1	1	1	1	3
15.5			1	1	1	1	1	2
16.0			0	1	1	1	1	2
16.5				1	1	1	1	2
17.0				0	0	1	1	1
17.5						0	1	1
18.0							0	1
18.5								1
19.0								1
19.5								1
20.0								0

TABLE B-5

RUNOFF HYDROGRAPHS*
BERMED AREA BY CONDUIT ENTRANCE

Time In Hours	RAINFALL EVENT IN YEARS							
	1	2	5	10	25	50	100	SPS
11.0	0	0	0	0	0	0	0	0
11.5	0	0	0	0	0	0	0	0
12.0	0	5	19	31	42	53	65	167
12.5	7	22	43	61	78	93	111	261
13.0	0	0	3	11	19	26	35	106
13.5			0	0	0	0	4	42
14.0							0	16
14.5								3
15.0								0

*Runoff hydrographs minus the capacity of the existing storm system, about 33 cfs, which outlets into the existing Bassett Creek tunnel.

TABLE B-6

DESIGN OF GRAVITY OUTLETS

Outlet Location	Western Avenue-Winnetka Avenue Area	Highway 100 North	Fruen Mill Area	Bermed Area by Conduit Entrance
Pipe Diameter, In.	24	36	30	48
No. of Pipes Required	1	1	1	1
Type	CMP	CMP	RCP	RCP
Design Discharge, cfs	21	44	23	90
Slope, Ft/Ft	0.0156	0.008	0.0025	0.0031
Upstream Invert Elevation	882.0	840.0	809.2	800.0
Approximate Length, Ft.	90	125	80	320
Maximum Design Water Surface Elevation	886.1	844.3	811.9	805.1
Zero Damage Elevation	887.3	845.0	812.2	805.1

TABLE B-7

DRAINAGE DITCH DATA

Location	Highway 100 North Ponding Area
100-Year Discharge, cfs	44
Channel Length, Ft.	500
Channel Slope, Ft./Ft.	0.006
Channel Invert Elevation, Upstream	843.0
Channel Invert Elevation, Downstream	840.0
Side Slopes	1 on 3
Manning's "n"	0.040
Water Depth at Design Flow, Ft.	1.9
Bottom Width, Ft.	2
Velocity at Design Q, Ft/Sec	3.0

TABLE B-8

INTERIOR POND LEVELS

ALL-YEAR RAINFALL FREQUENCY

1-Year	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
		Western Avenue-Winnetka Avenue Area				
884.7	884.9	885.2	885.4	885.7	885.9	886.1
		Highway 100 North				
841.9	842.3	843.0	843.4	843.8	844.1	844.3
		Fruen Mill Area				
810.1	810.5	810.9	811.1	811.4	811.6	811.9
		Bermed Area by Conduit Entrance				
800.6	801.5	802.5	803.2	803.8	804.5	805.1

TABLE B-9

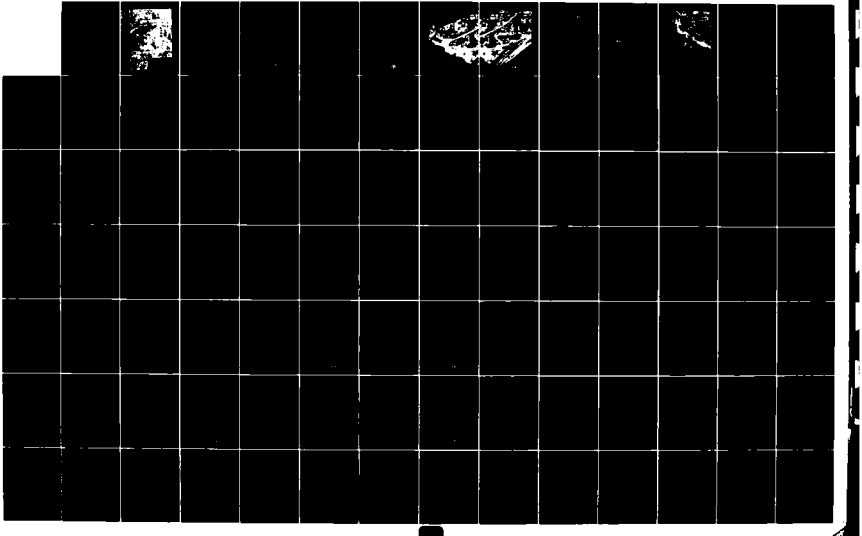
ONE-PERCENT AND STANDARD PROJECT STORM
INTERIOR POND LEVEL DATA

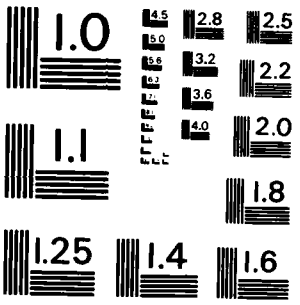
	EXISTING CONDITIONS		PROPOSED CONDITIONS	
Rainfall Event	One-Percent (100-year)	SPS	One-Percent (100-year)	SPS
Western Avenue-Winnetka Avenue Area	888.1	890.4	886.1	890.6
Highway 100 North Area	847.2	852.5	844.3	853.5
Fruen Mill Area	816.1	823.0	811.9	822.8
Bermed Area by Conduit Entrance	812.3	822.2	805.1	822.2


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SCALE

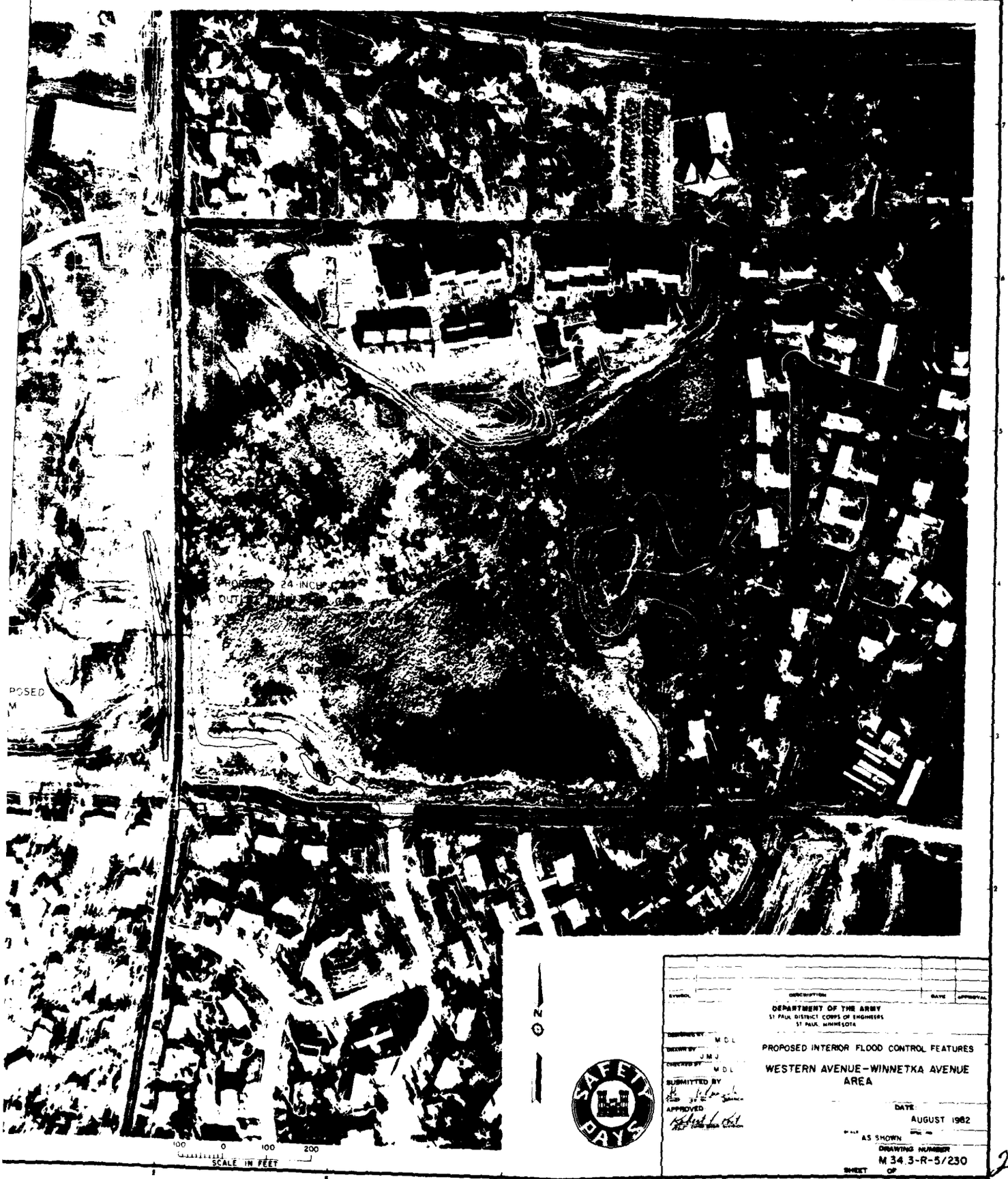
AD-A133 795 BASSETT CREEK WATERSHED HENNEPIN COUNTY MINNESOTA FEASIBILITY REPORT FOR FLOOD CONTROL MAIN REPORT(U) CORPS OF ENGINEERS ST PAUL MN ST PAUL DISTRICT SEP 82 3/7

UNCLASSIFIED F/G 13/2 NL





MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS - 1963 - A



DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY M D L
DRAWN BY J M J
CHECKED BY M D L
SUBMITTED BY
APPROVED

PROPOSED INTERIOR FLOOD CONTROL FEATURES

WESTERN AVENUE-WINNETKA AVENUE AREA

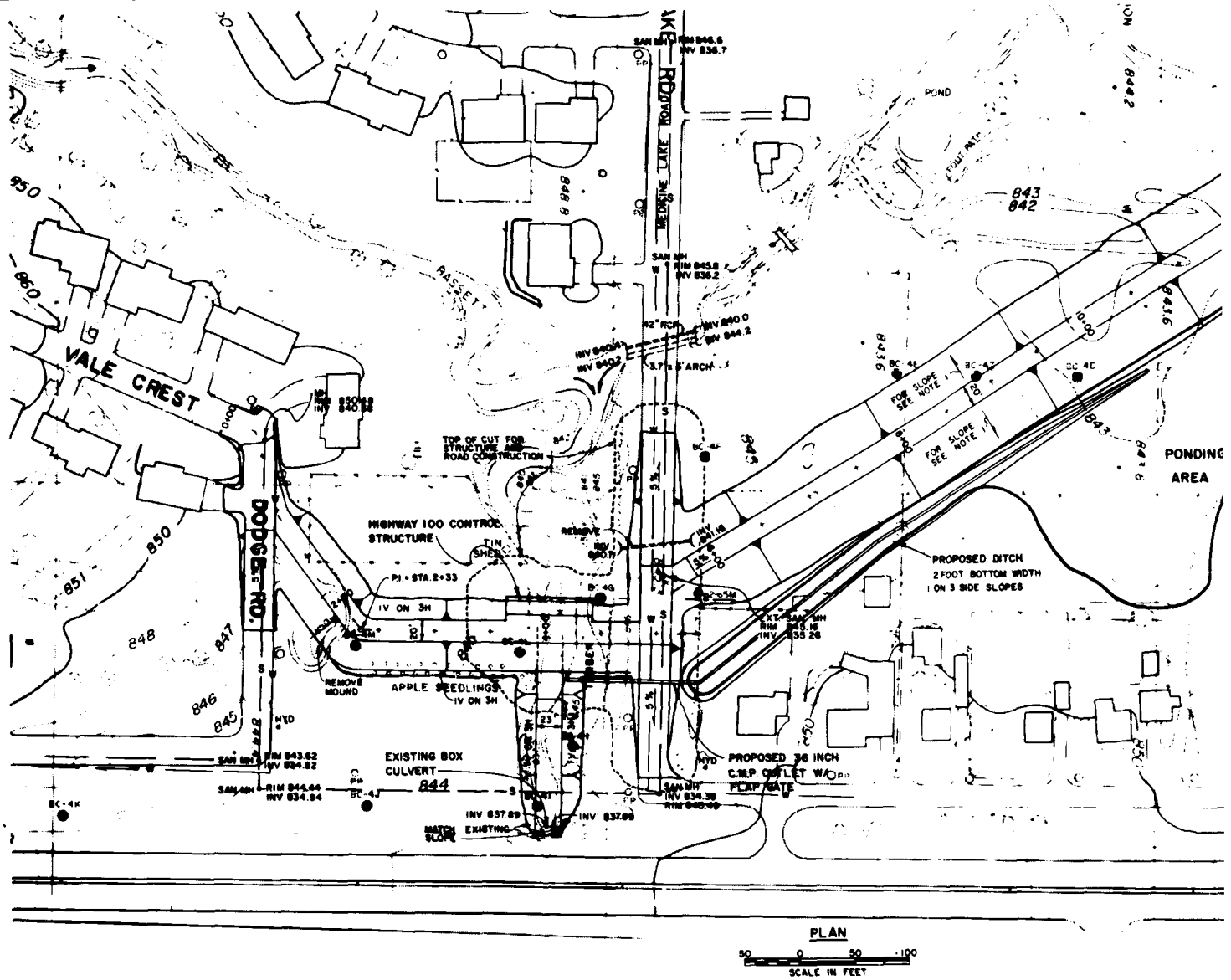
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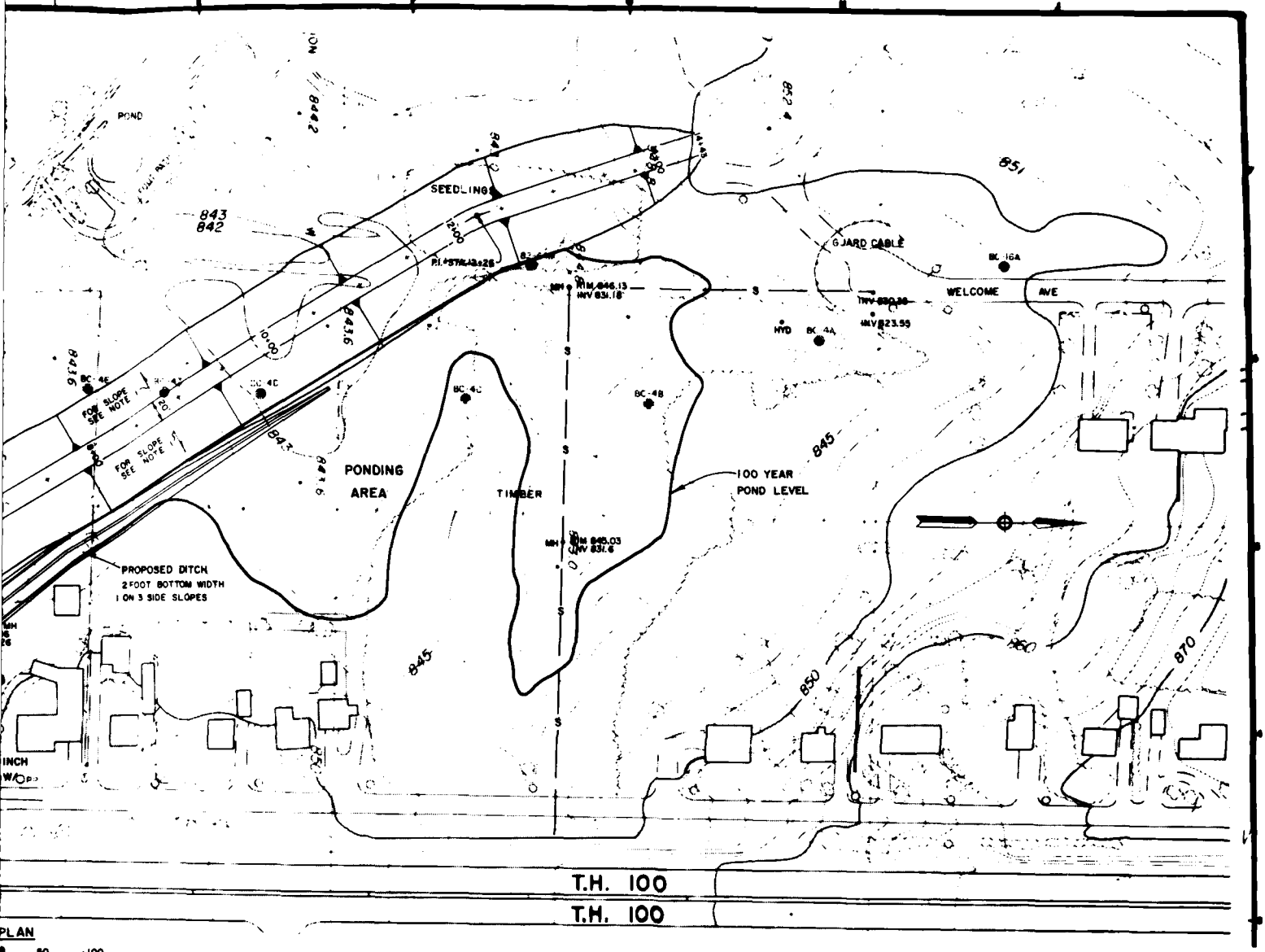
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DRAWING NUMBER
M 34.3-R-5/230

SHEET OF

PLATE B-1


VALE CREST
DODGE RD.
LAKE ROAD
WEDGWINE
POND
PONDING AREA
HIGHWAY 100 CONTROL STRUCTURE
TIN SHED
REMOVE
TOP OF CUT FOR STRUCTURE AND ROAD CONSTRUCTION
PROPOSED DITCH
2 FOOT BOTTOM WIDTH
1 ON 3 SIDE SLOPES
EXISTING BOX CULVERT
844
APPLE SEEDLINGS
1V ON 3H
REMOVE MOUND
PROPOSED 36 INCH C.M.P. OUTLET W/ FLAP GATE
MATCH EXISTING SLOPE
FOR SLOPE SEE NOTE 1
PLAN
50 0 50 100
SCALE IN FEET



PLAN

0 50 100

E IN FEET

NOTES:

1. Due to large amounts of settlement upstream slope will change from 1V on 3H at End to Construction to 1V on 4.75H after 100 years of secondary consolidation. likewise the downstream slop will vary from 1V on 3.7H to 1V on 6H.
2. The embankment from Sta. 6+00 to 14+00 will be constructed in three stages over a two year period. (See Plate)

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

D.W.R. & M.D.L.
W.J.V.
M.C.B.

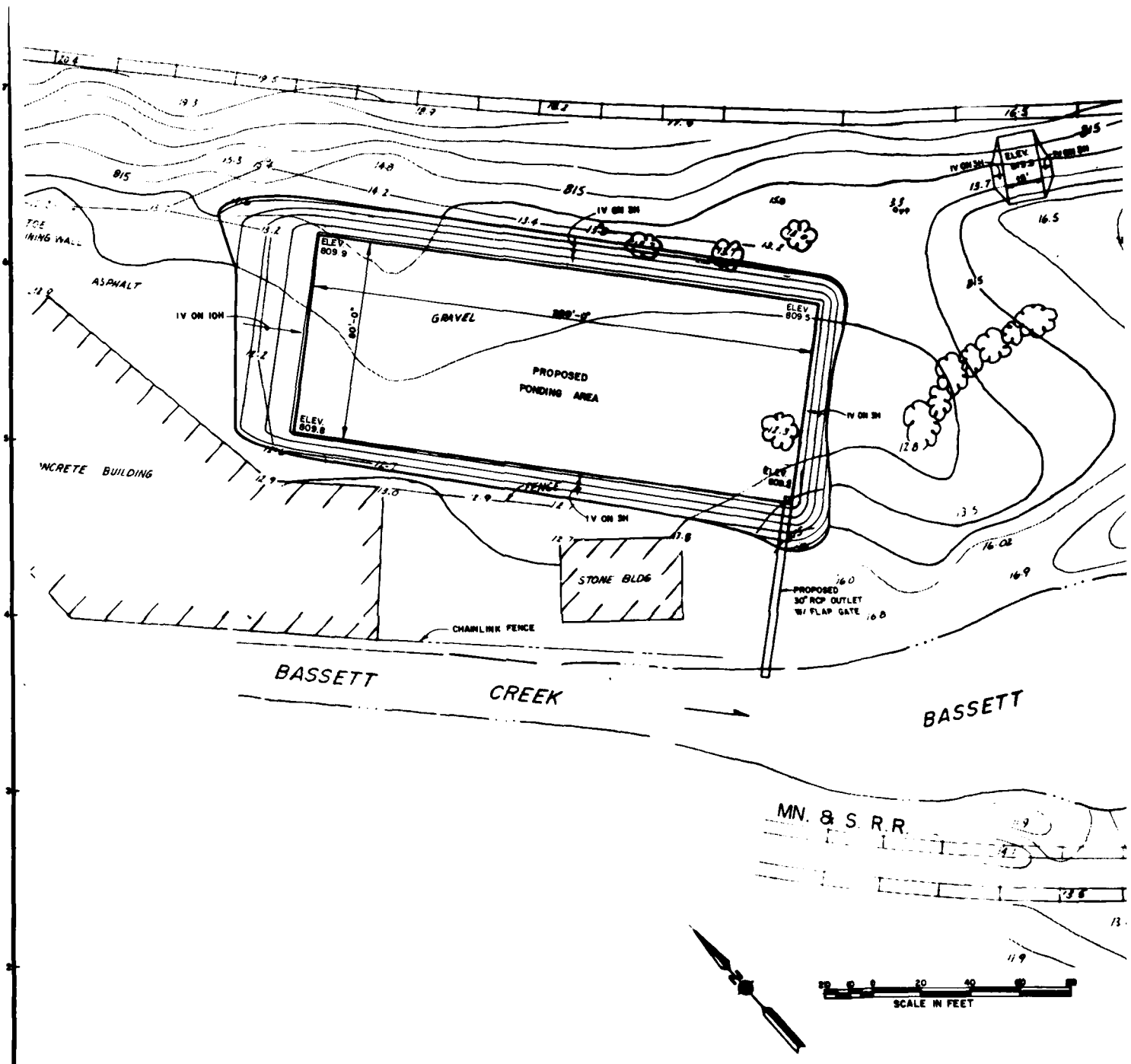
PROPOSED INTERIOR FLOOD CONTROL FEATURES
HIGHWAY 100 NORTH

JUNE 1962

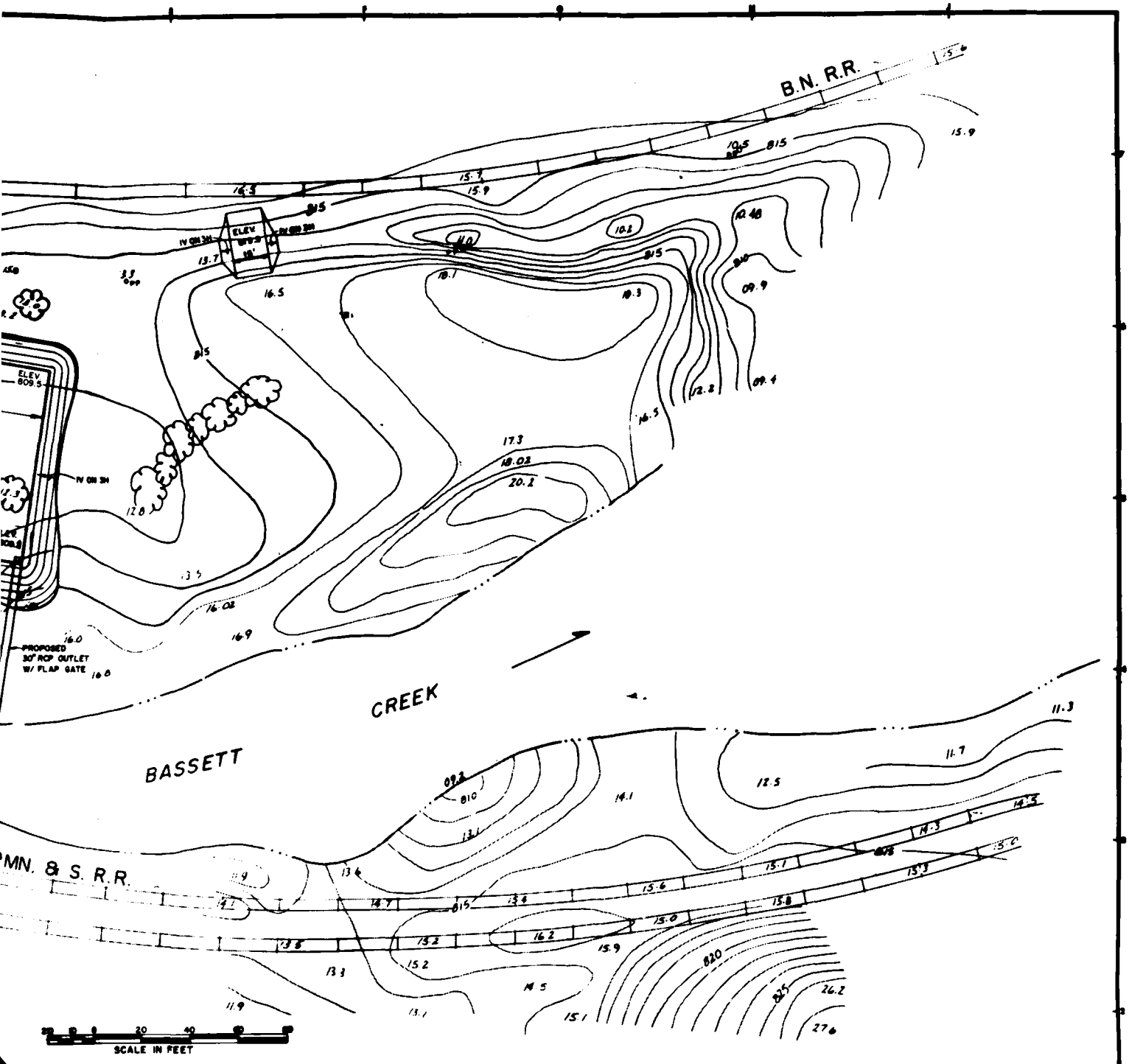
AS SHOWN

SHEET OF

PLATE B-2



PROPOSED PLAN



PROPOSED PLAN

DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

PROPOSED INTERIOR FLOOD CONTROL FEATURES
GLENWOOD - INGLEWOOD PONDING AREA
FRUEN MILL AREA BELOW GLENWOOD AVENUE

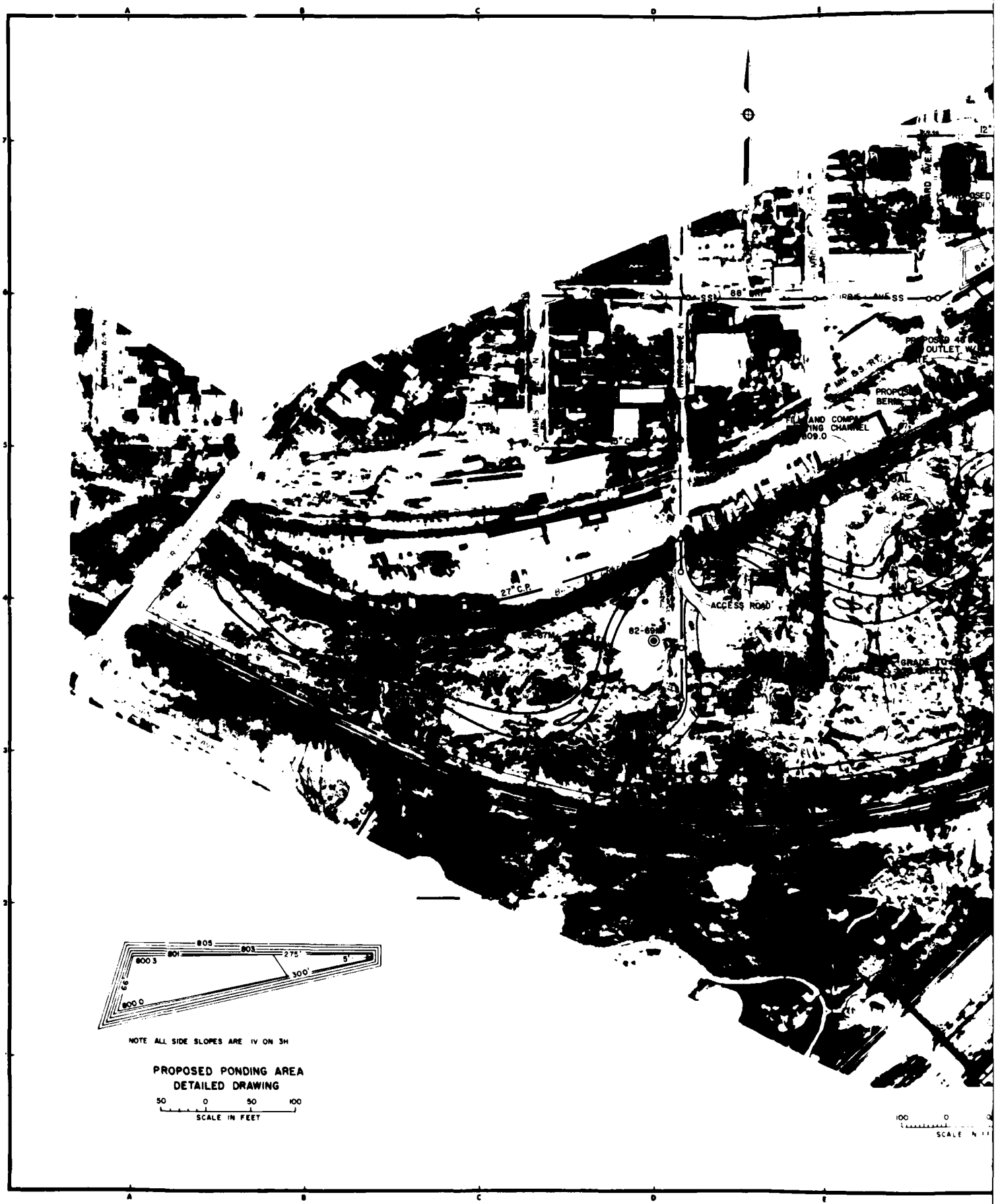
DATE: JUNE 1982

AS SHOWN

SHEET OF

SAFETY PAYS

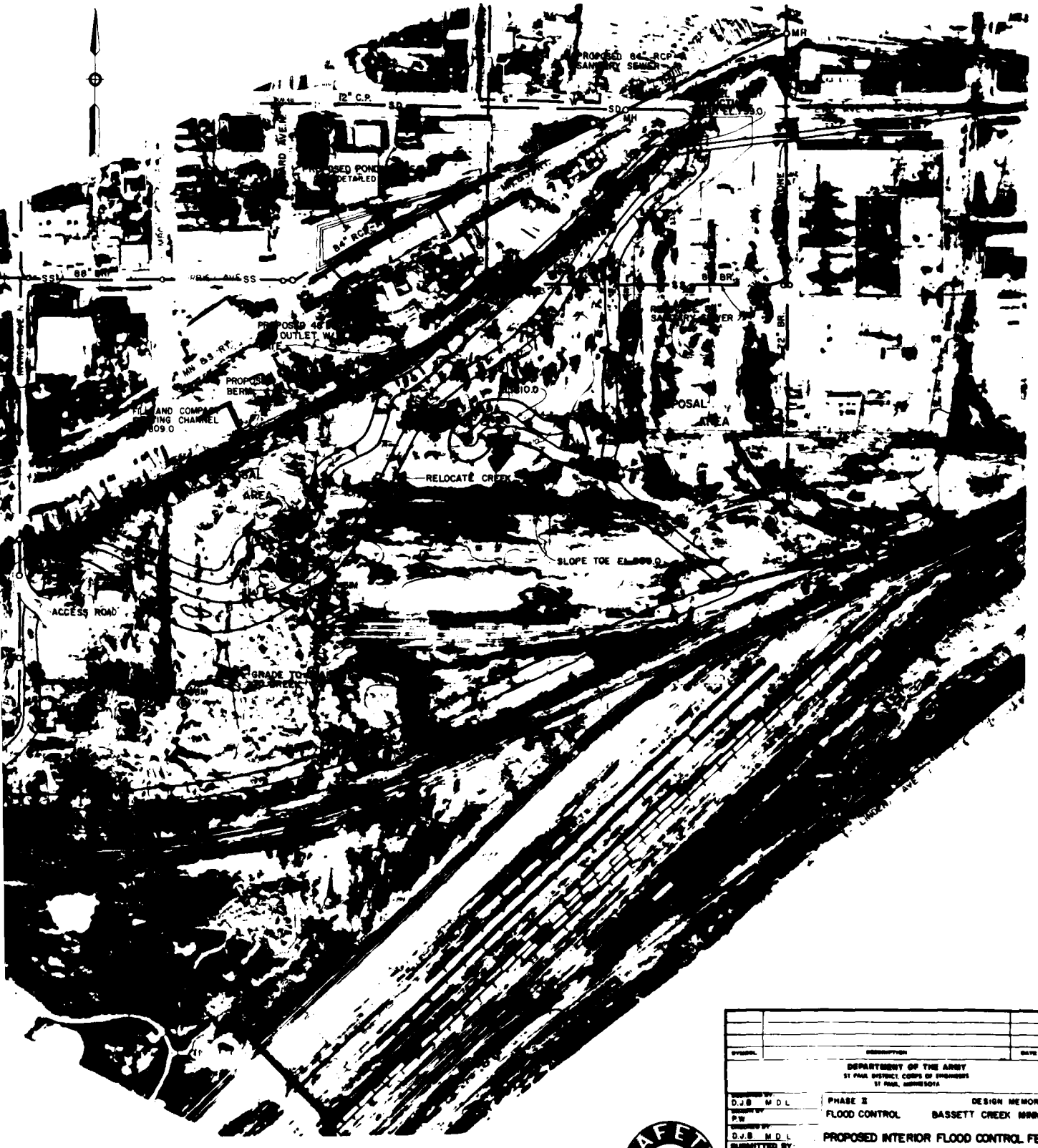
PLATE B-3



NOTE ALL SIDE SLOPES ARE IV ON 3H

PROPOSED PONDING AREA
DETAILED DRAWING

50 0 50 100
SCALE IN FEET



SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY D.J.S M.D.L	PHASE II — DESIGN MEMORANDUM
DRAWN BY P.W	FLOOD CONTROL — BASSETT CREEK MINNESOTA
CHECKED BY D.J.S M.D.L	PROPOSED INTERIOR FLOOD CONTROL FEATURES
SUBMITTED BY	BERMED AREA BY CONDUIT ENTRANCE
APPROVED	DATE: AUGUST 1982
	SCALE / DRAWING NUMBER
	SHEET OF

SAFETY PAYS

2

PLATE B-4

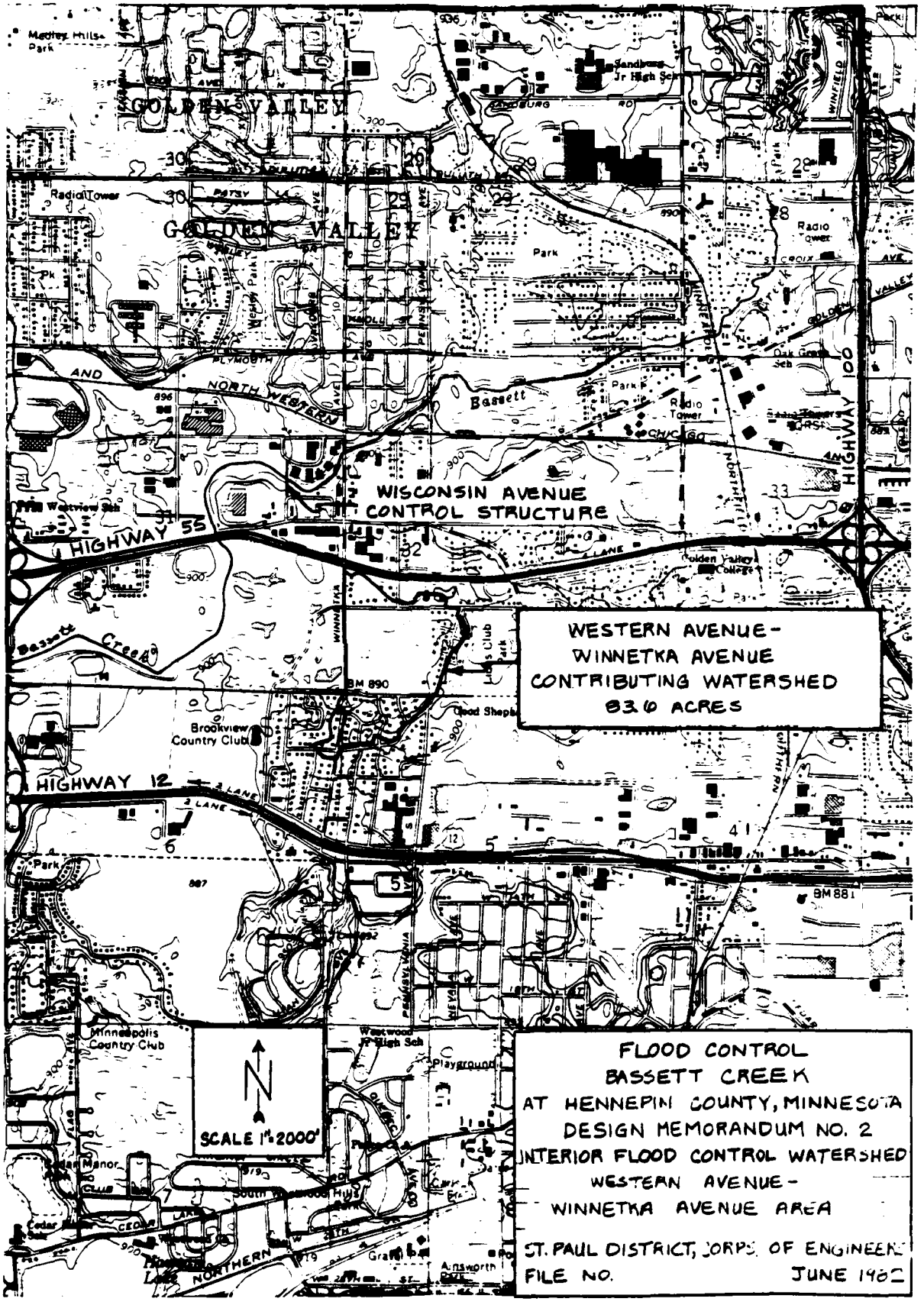

WISCONSIN AVENUE
CONTROL STRUCTURE
WESTERN AVENUE-
WINNETKA AVENUE
CONTRIBUTING WATERSHED
836 ACRES
FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
INTERIOR FLOOD CONTROL WATERSHED
WESTERN AVENUE-
WINNETKA AVENUE AREA
ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO.
JUNE 1962
SCALE 1"=2000'
GOLDEN VALLEY
HIGHWAY 55
HIGHWAY 12
BASSETT CREEK
BM 890
BM 881
Brookview Country Club
Minneapolis Country Club
Golden Valley College
Sandburg Jr High Sch
Westwood Jr High Sch
Playground
Park
Radio Tower
Radial Tower
Bassett
Lions Club Park
Good Shepherd
Westview Sch
Oak Grove Sch
Cedar Lake
Lake Northern
Ainsworth
Manor
Hills

PLATE B-5

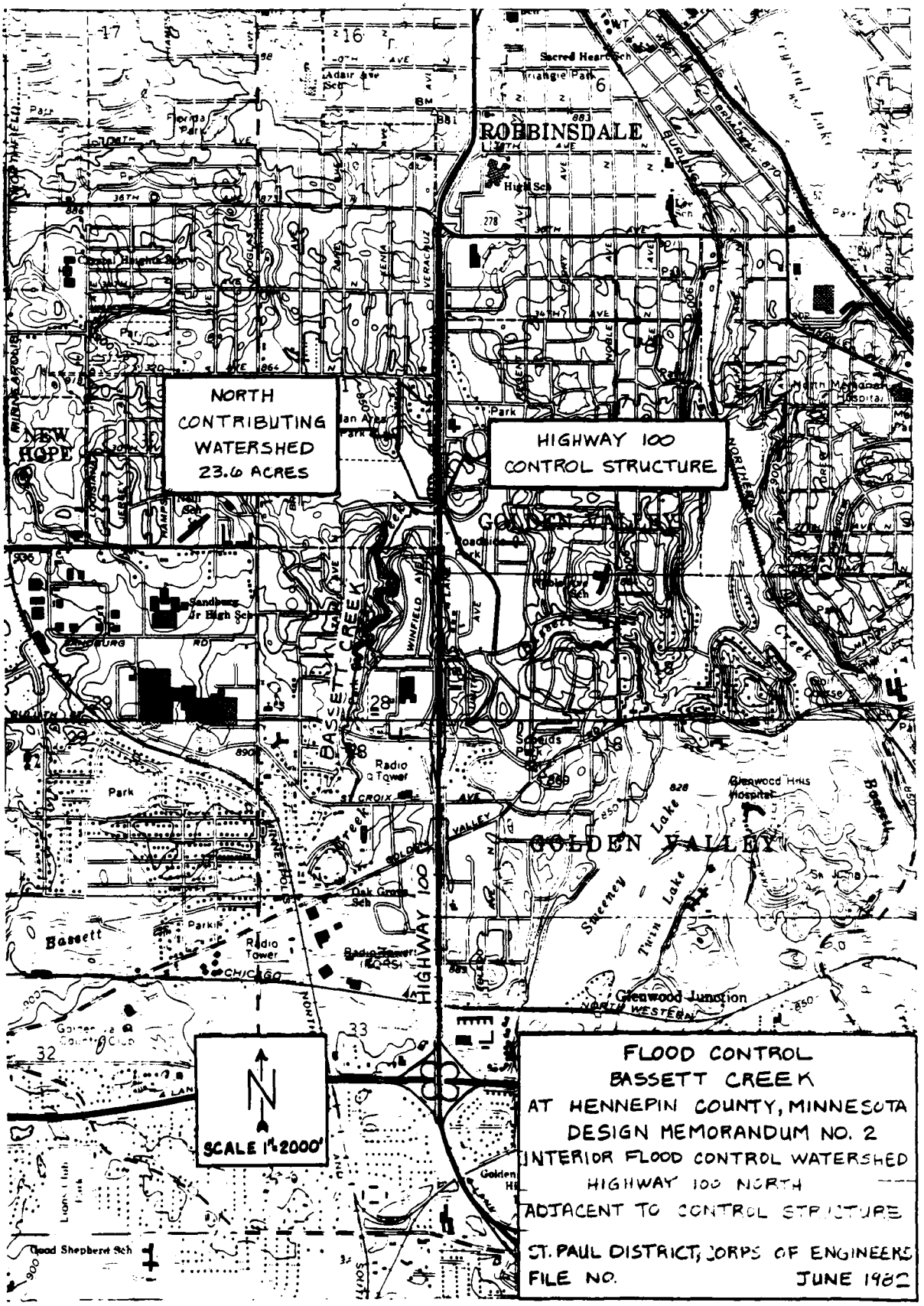
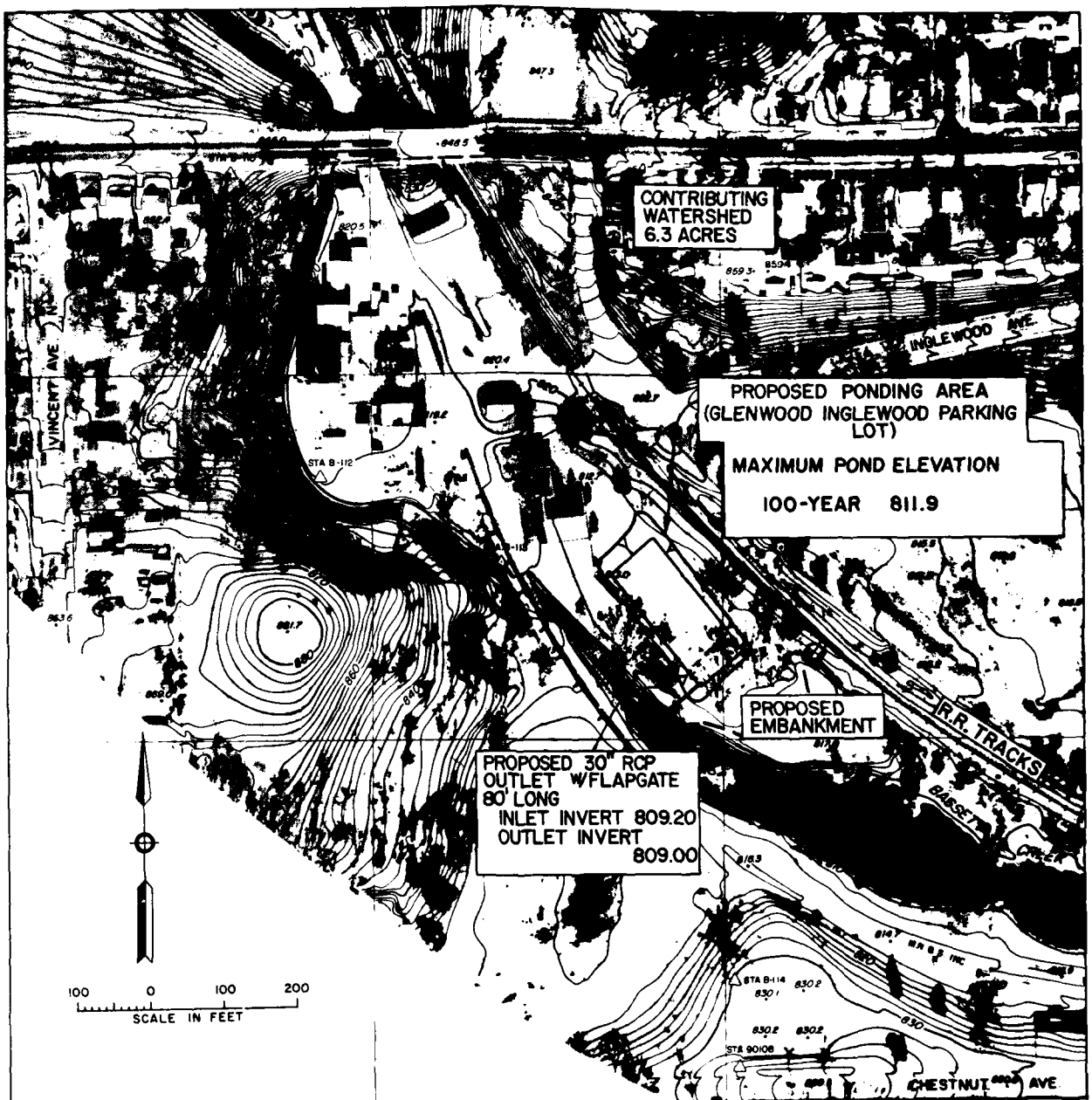

NORTH
CONTRIBUTING
WATERSHED
23.6 ACRES
HIGHWAY 100
CONTROL STRUCTURE
ROBBINSDALE
NEW HOPE
GOLDEN VALLEY
BASSETT CREEK
HIGHWAY 100
Sandburg Jr High Sch
Radio Tower
Park
Oak Grove Sch
Glenwood Hills Hospital
Sweeney Lake
Twin Lake
Glenwood Junction
Good Shepherd Sch
SCALE 1"=2000'
FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
INTERIOR FLOOD CONTROL WATERSHED
HIGHWAY 100 NORTH
ADJACENT TO CONTROL STRUCTURE
ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO.
JUNE 1982

PLATE B-6



DESIGN MEMORANDUM NO. II
PHASE II

FLOOD CONTROL
BASSETT CREEK WATERSHED
MINNEAPOLIS, MINNESOTA

INTERIOR FLOOD CONTROL WATERSHED AND PROPOSED PONDING AREA FRUEN MILL AREA BELOW GLENWOOD AVE.

ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO. M34.3-R-5/135 JUNE 1982

PLATE B-7

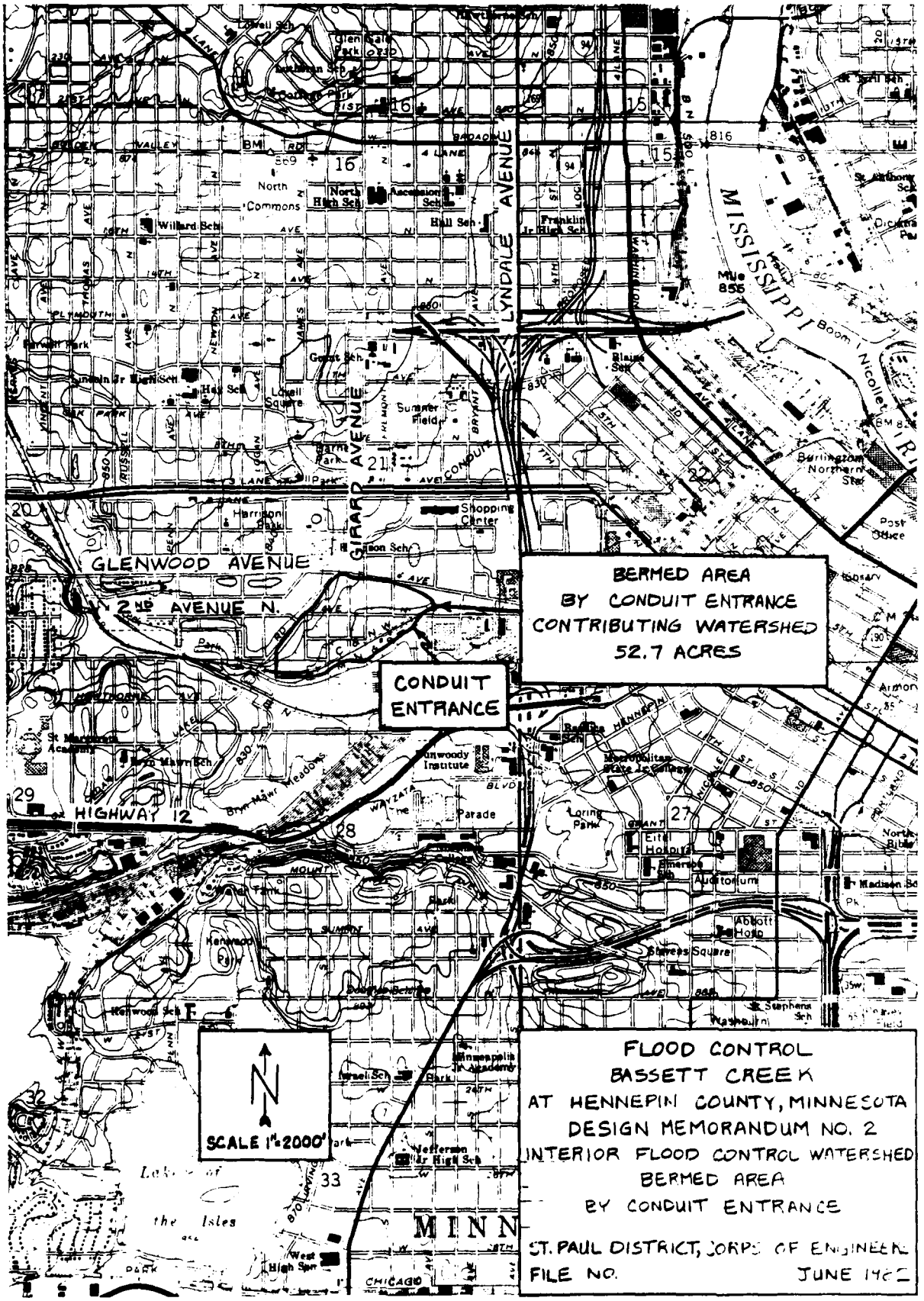
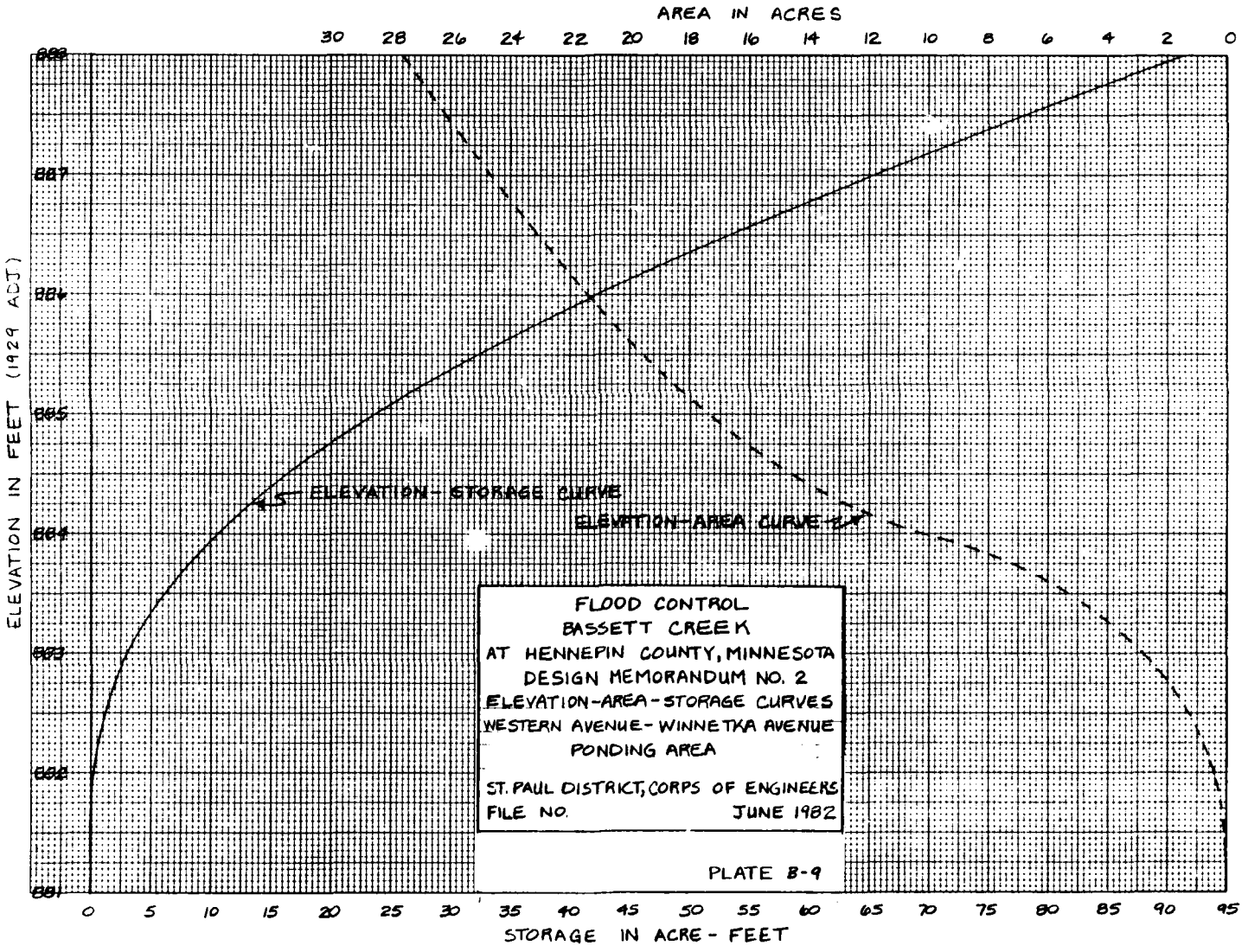

BERMED AREA
BY CONDUIT ENTRANCE
CONTRIBUTING WATERSHED
52.7 ACRES
CONDUIT
ENTRANCE
SCALE 1"=2000'
FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
INTERIOR FLOOD CONTROL WATERSHED
BERMED AREA
BY CONDUIT ENTRANCE
ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO.
JUNE 1962
GLENWOOD AVENUE
2ND AVENUE N.
HIGHWAY 12
GIRARD AVENUE
LYNDALE AVENUE
MISSISSIPPI
MINN
Lake of the Isles

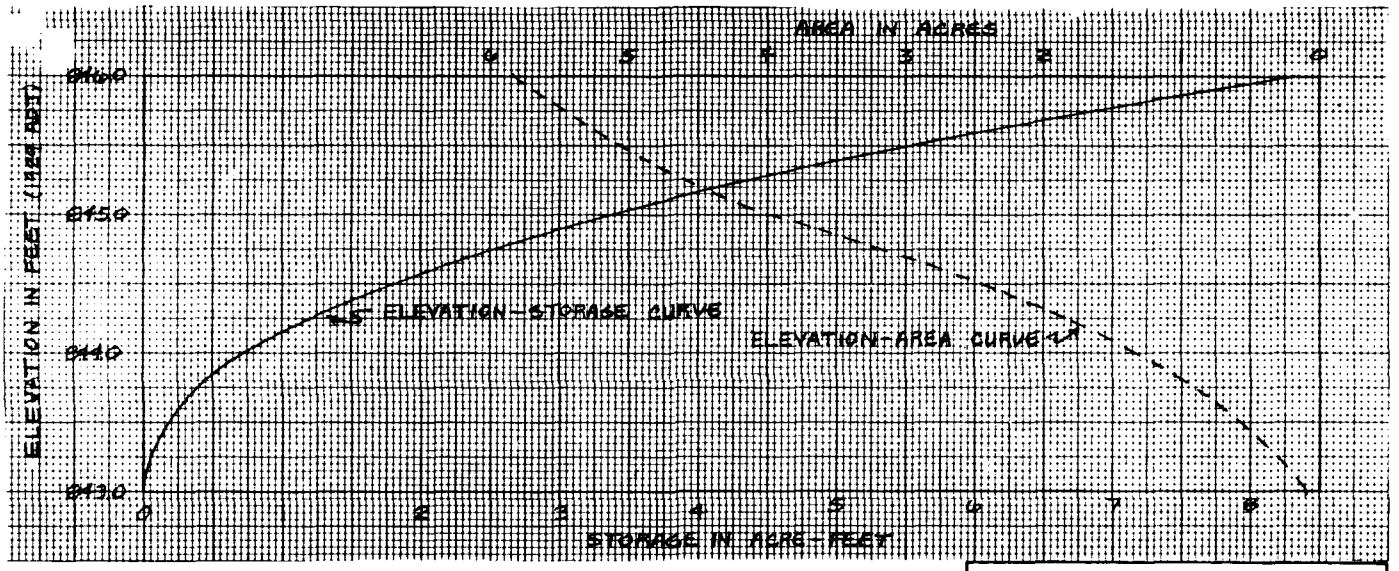
PLATE E-8



FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
ELEVATION-AREA-STORAGE CURVES
WESTERN AVENUE - WINNETKA AVENUE
PONDING AREA

ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO. JUNE 1982

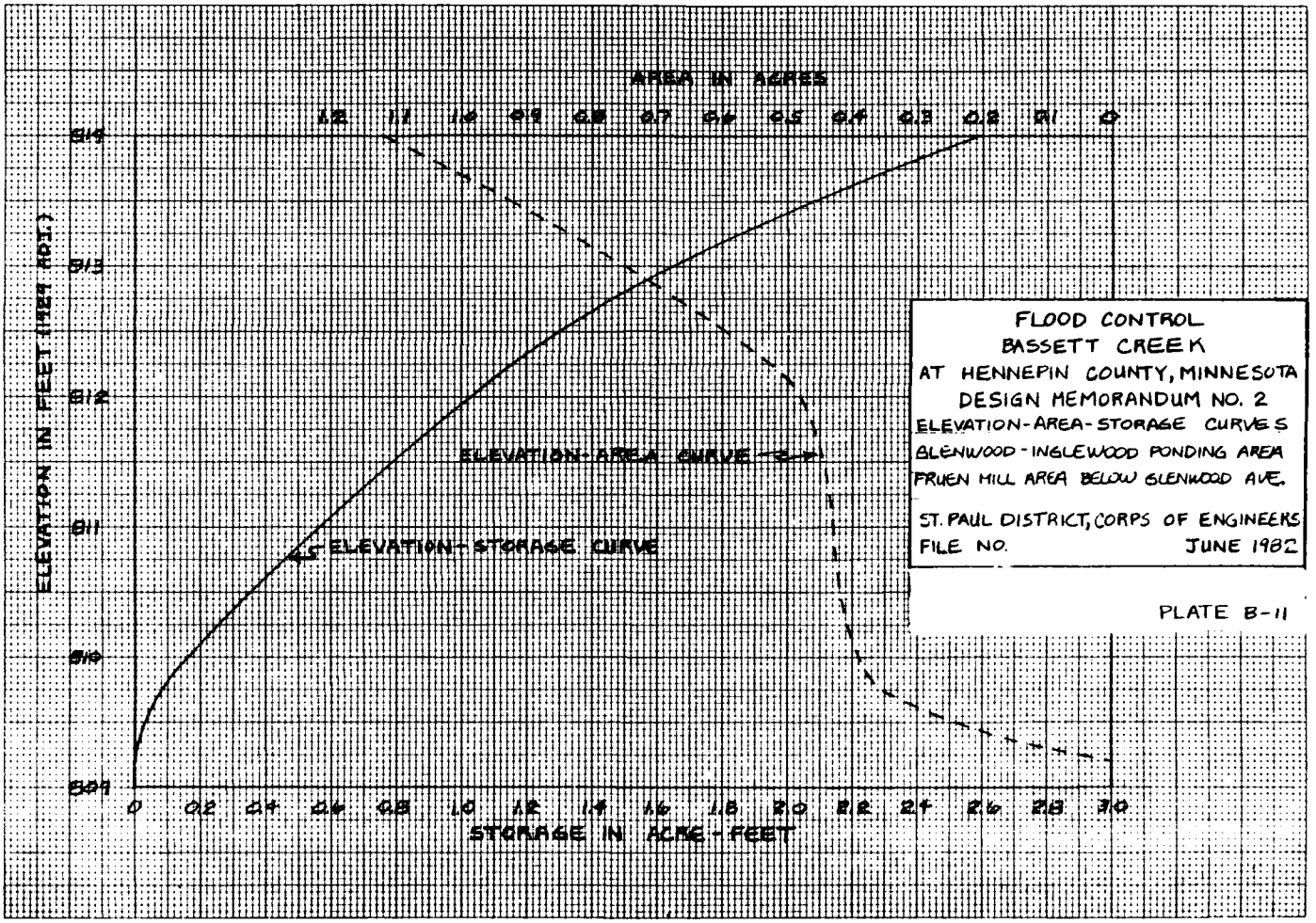
PLATE B-9



FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
ELEVATION-AREA-STORAGE CURVES
HIGHWAY 100 NORTH
PONDING AREA

ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO. JUNE 1982

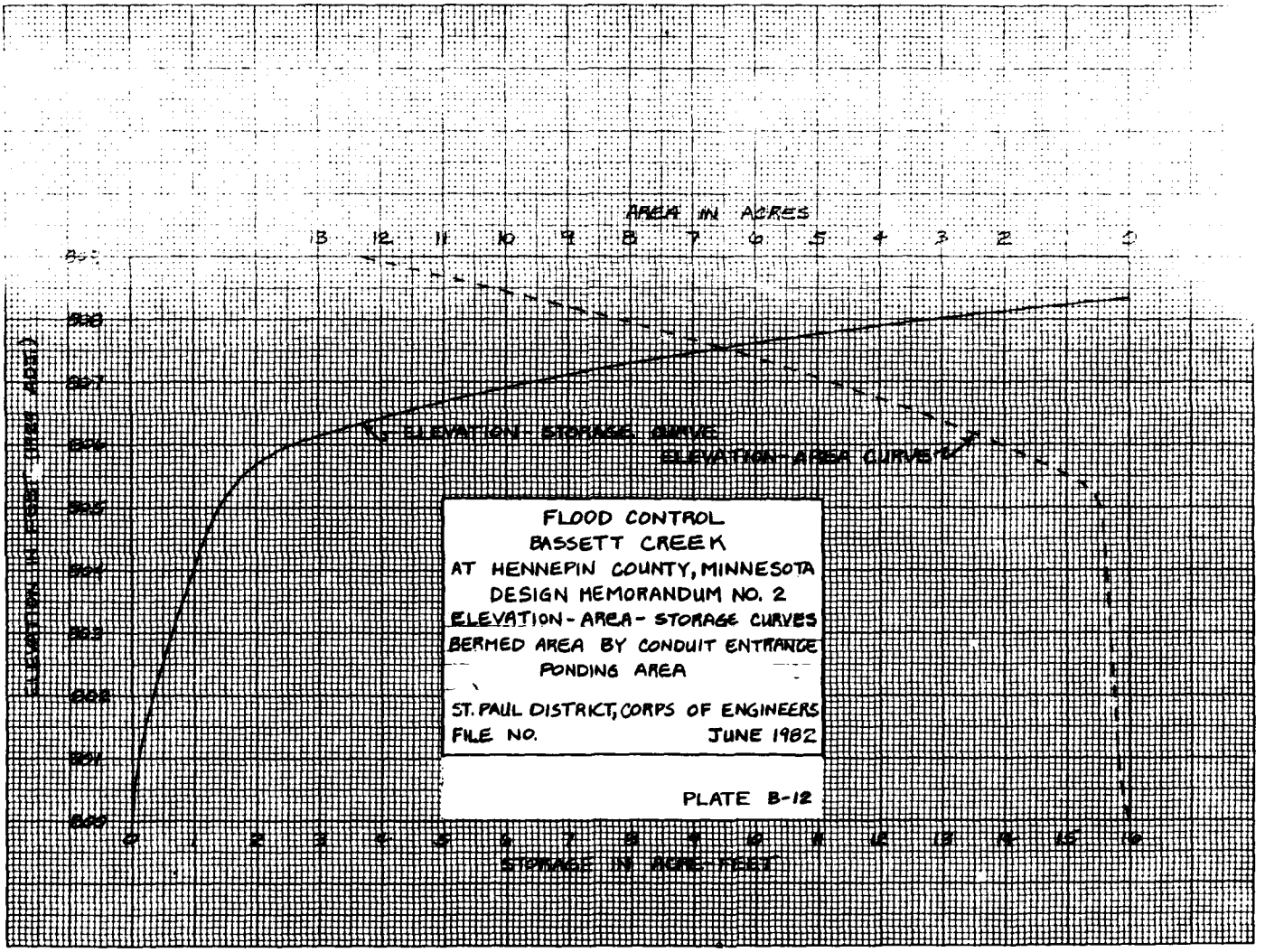
PLATE B-10



FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
ELEVATION-AREA-STORAGE CURVES
GLENWOOD-INGLEWOOD PONDING AREA
FRUEN MILL AREA BELOW GLENWOOD AVE.

ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO. JUNE 1982

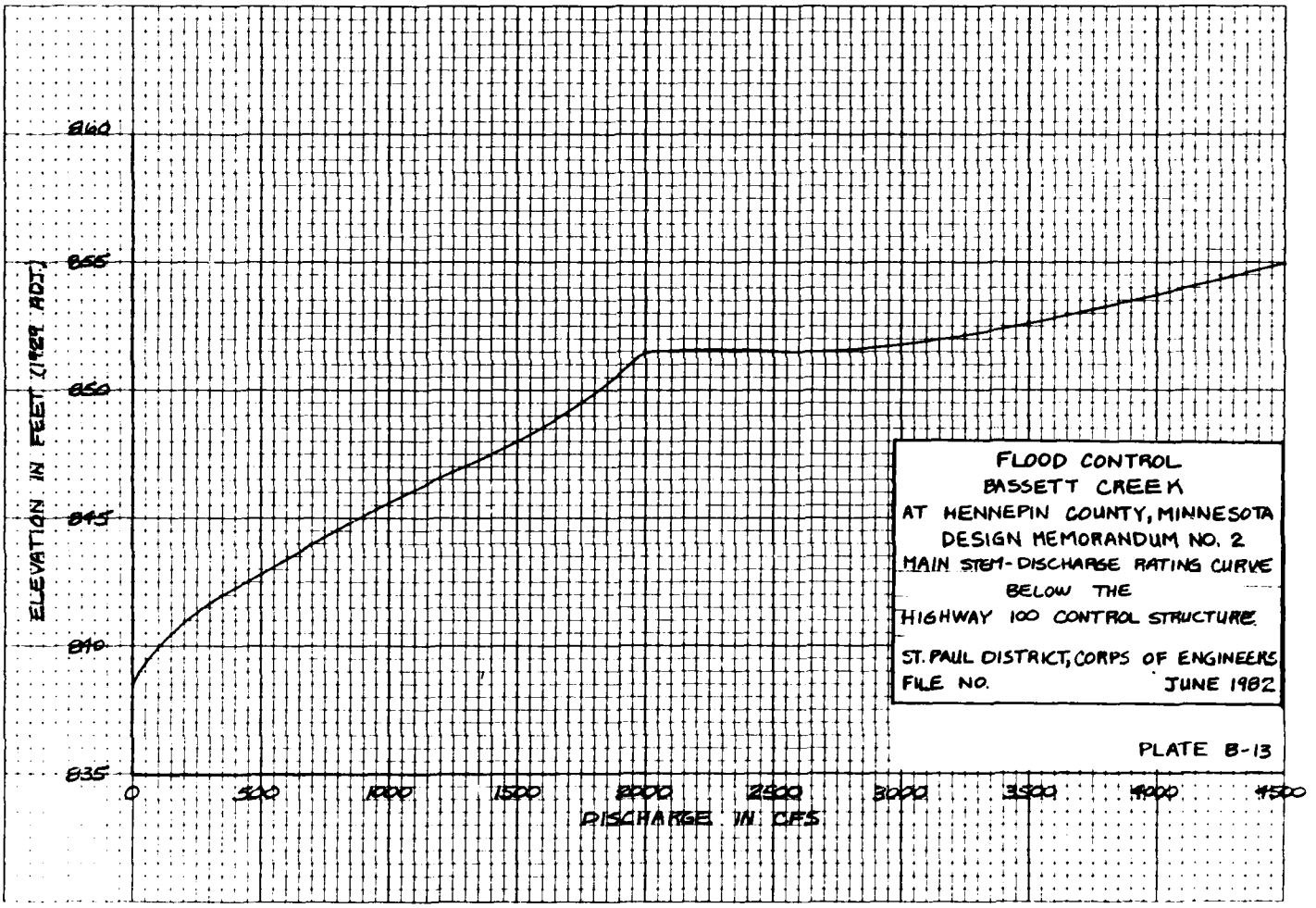
PLATE B-11



FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
ELEVATION - AREA - STORAGE CURVES
BERMED AREA BY CONDUIT ENTRANCE
PONDING AREA

ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO. JUNE 1982

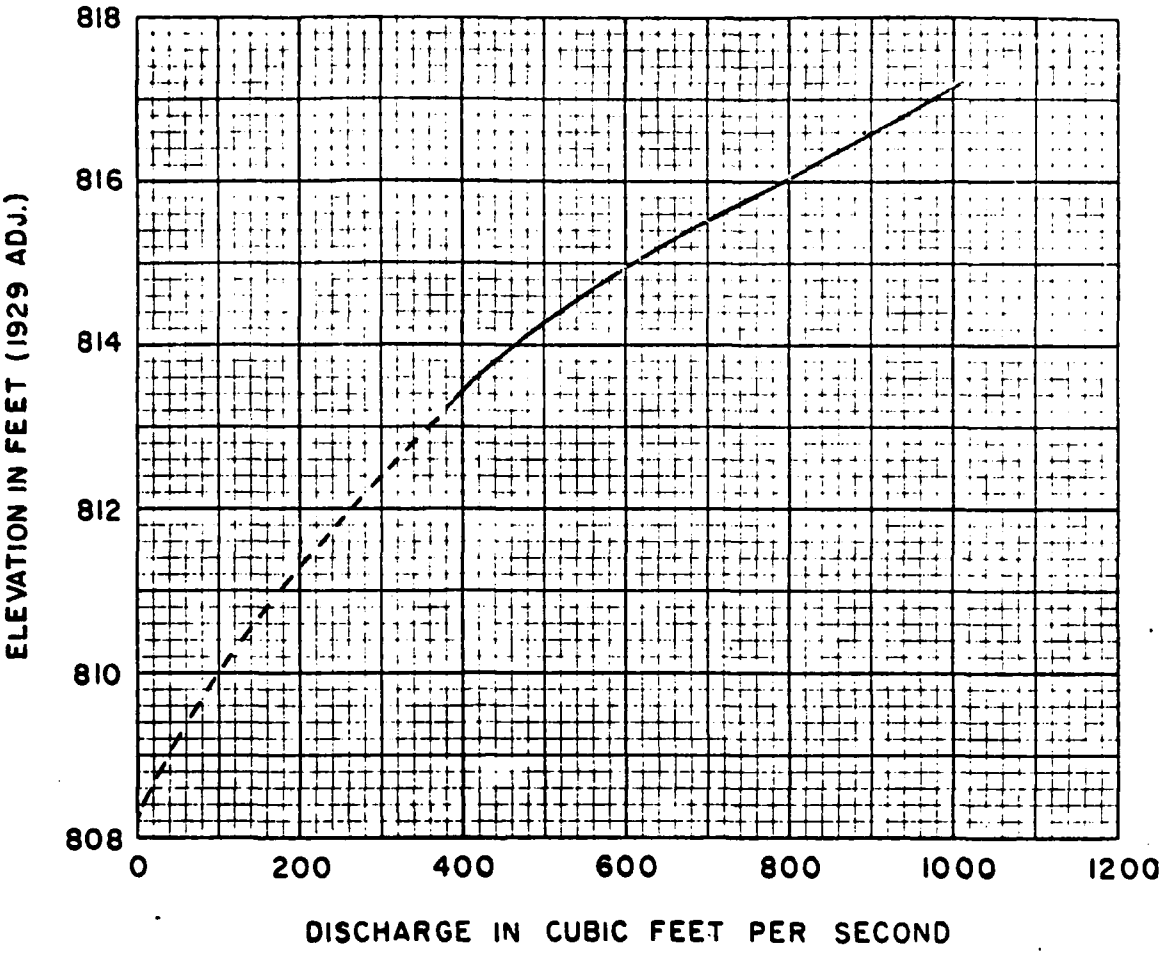
PLATE B-12



FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
MAIN STEM - DISCHARGE RATING CURVE
BELOW THE
HIGHWAY 100 CONTROL STRUCTURE

ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO. JUNE 1982

PLATE B-13

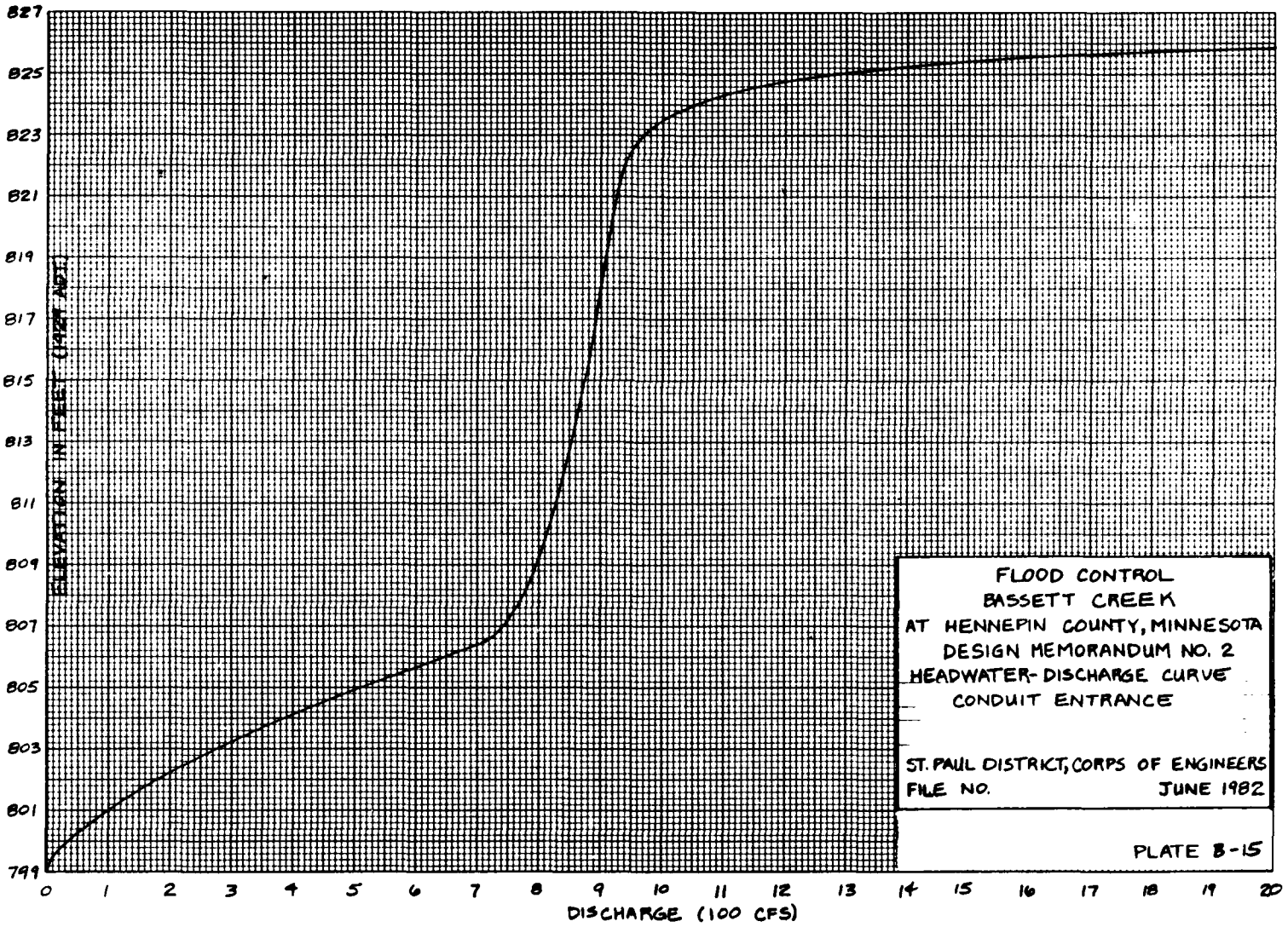


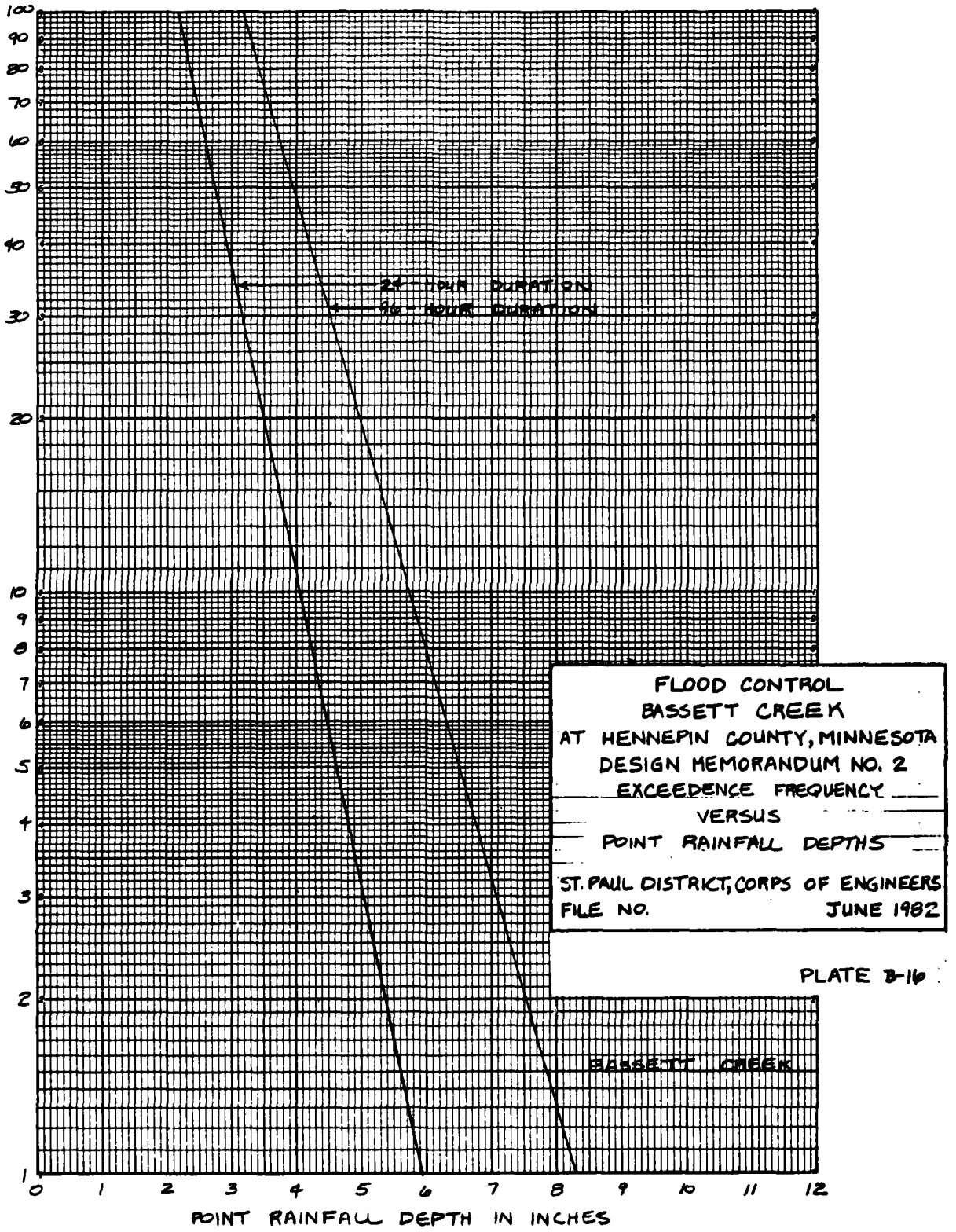
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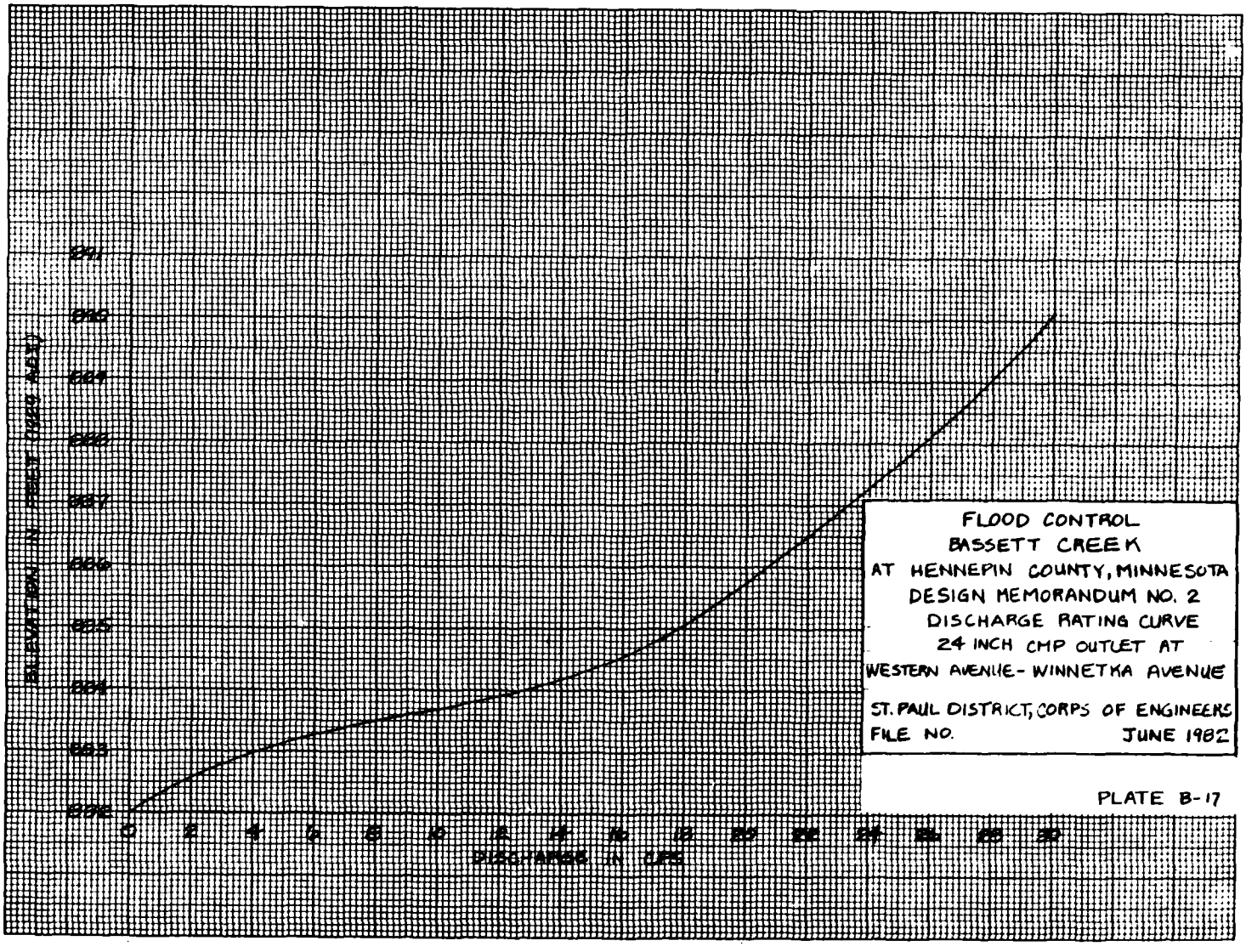
---EXTENSION TO CHANNEL BOTTOM

FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
MAIN STEM - DISCHARGE RATING CURVE
AT CROSS SECTION 4.101
FRUEN MILL AREA BELOW GLENWOOD AVE.
ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO. JUNE 1982

PLATE B-14


827
825
823
821
819
817
815
813
811
809
807
805
803
801
799
ELEVATION IN FEET (USGS DATUM)
0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20
DISCHARGE (100 CFS)
FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
HEADWATER-DISCHARGE CURVE
CONDUIT ENTRANCE
ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO.
JUNE 1982
PLATE B-15

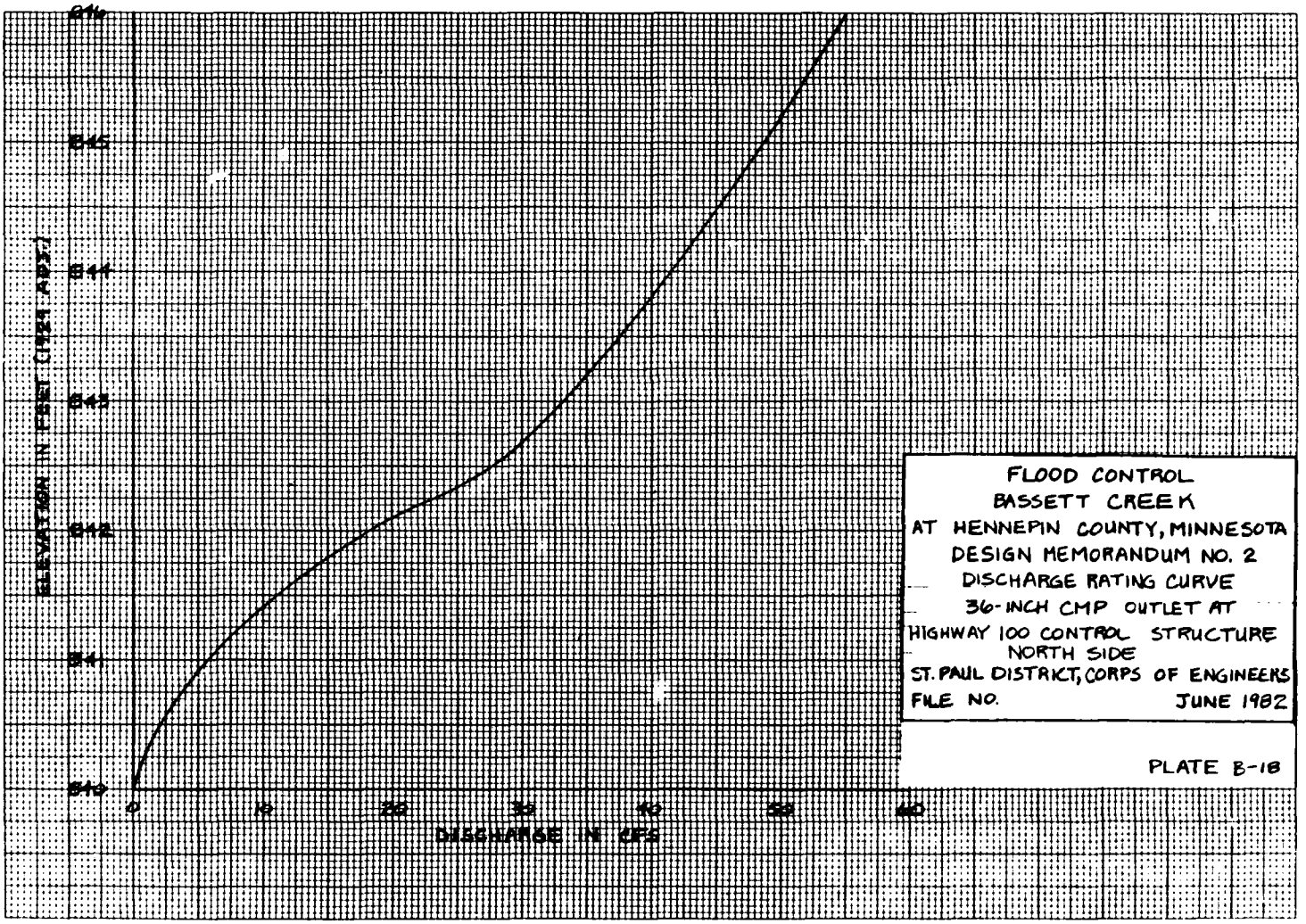

100
90
80
70
60
50
40
30
20
10
9
8
7
6
5
4
3
2
1
24 - HOUR DURATION
96 - HOUR DURATION
0 1 2 3 4 5 6 7 8 9 10 11 12
POINT RAINFALL DEPTH IN INCHES
BASSETT CREEK
FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
EXCEEDENCE FREQUENCY
VERSUS
POINT RAINFALL DEPTHS
ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO.
JUNE 1982
PLATE B-16

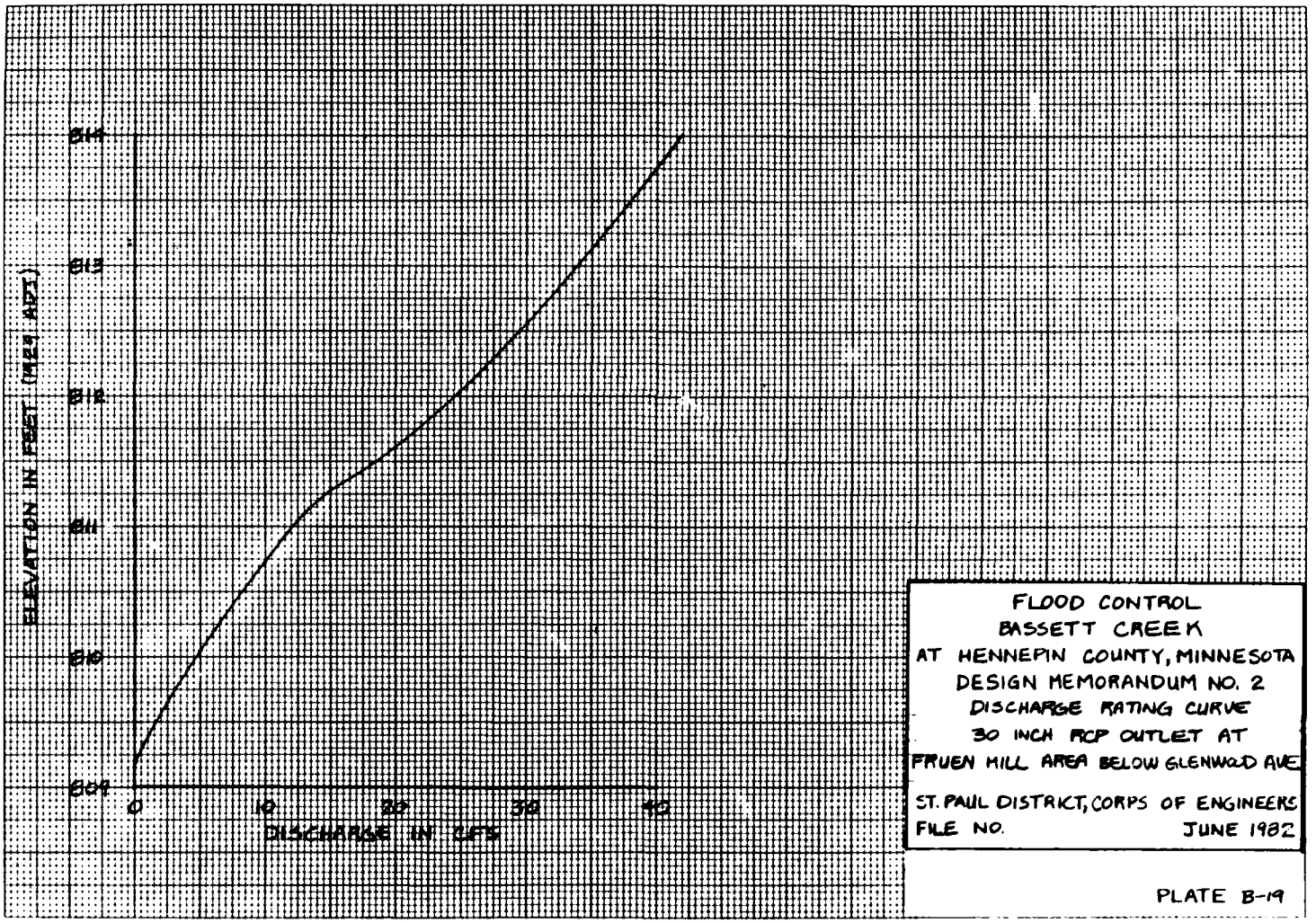


FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
DISCHARGE RATING CURVE
24 INCH CMP OUTLET AT
WESTERN AVENUE - WINNETKA AVENUE

ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO. JUNE 1982

PLATE B-17

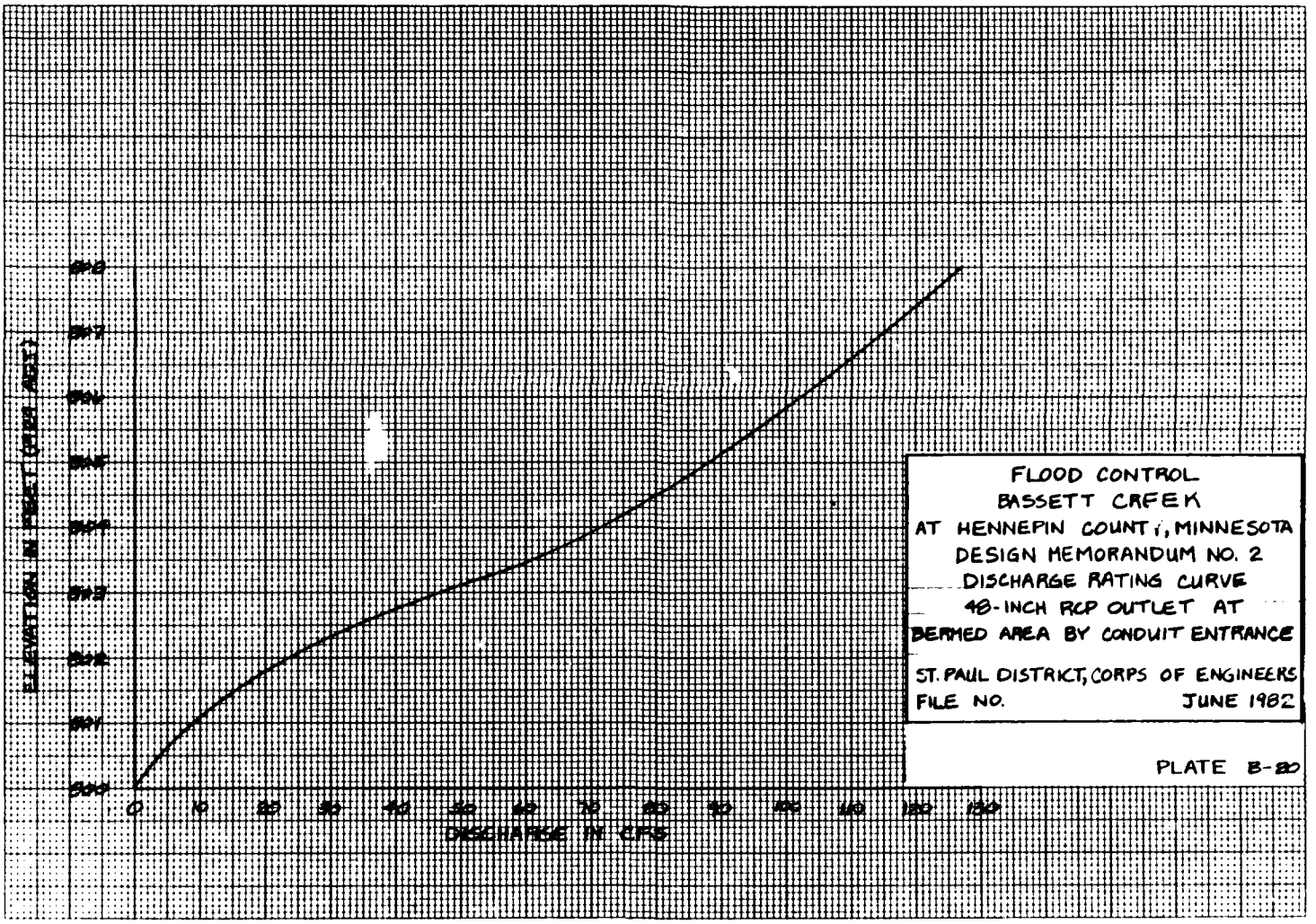

ELEVATION IN FEET (1929 ADJ.)
DISCHARGE IN CFS
0
10
20
30
40
FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
DISCHARGE RATING CURVE
36-INCH CMP OUTLET AT
HIGHWAY 100 CONTROL STRUCTURE
NORTH SIDE
ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO.
JUNE 1982
PLATE B-18



FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
DISCHARGE RATING CURVE
30 INCH RCP OUTLET AT
FRUEN MILL AREA BELOW GLENWOOD AVE.

ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO. JUNE 1982

PLATE B-19



FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
DISCHARGE RATING CURVE
48-INCH RCP OUTLET AT
BERMED AREA BY CONDUIT ENTRANCE

ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO. JUNE 1982

PLATE B-20

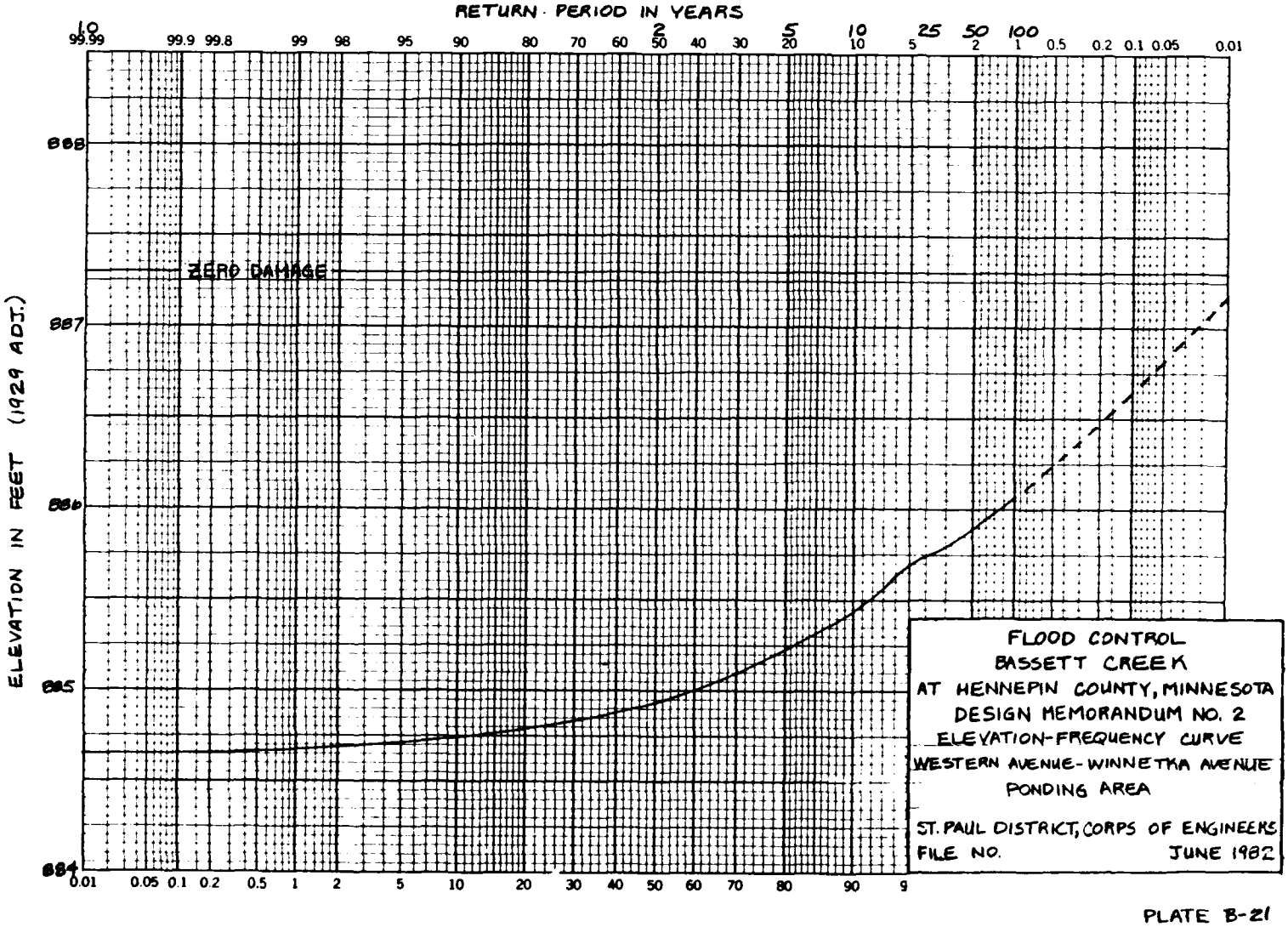
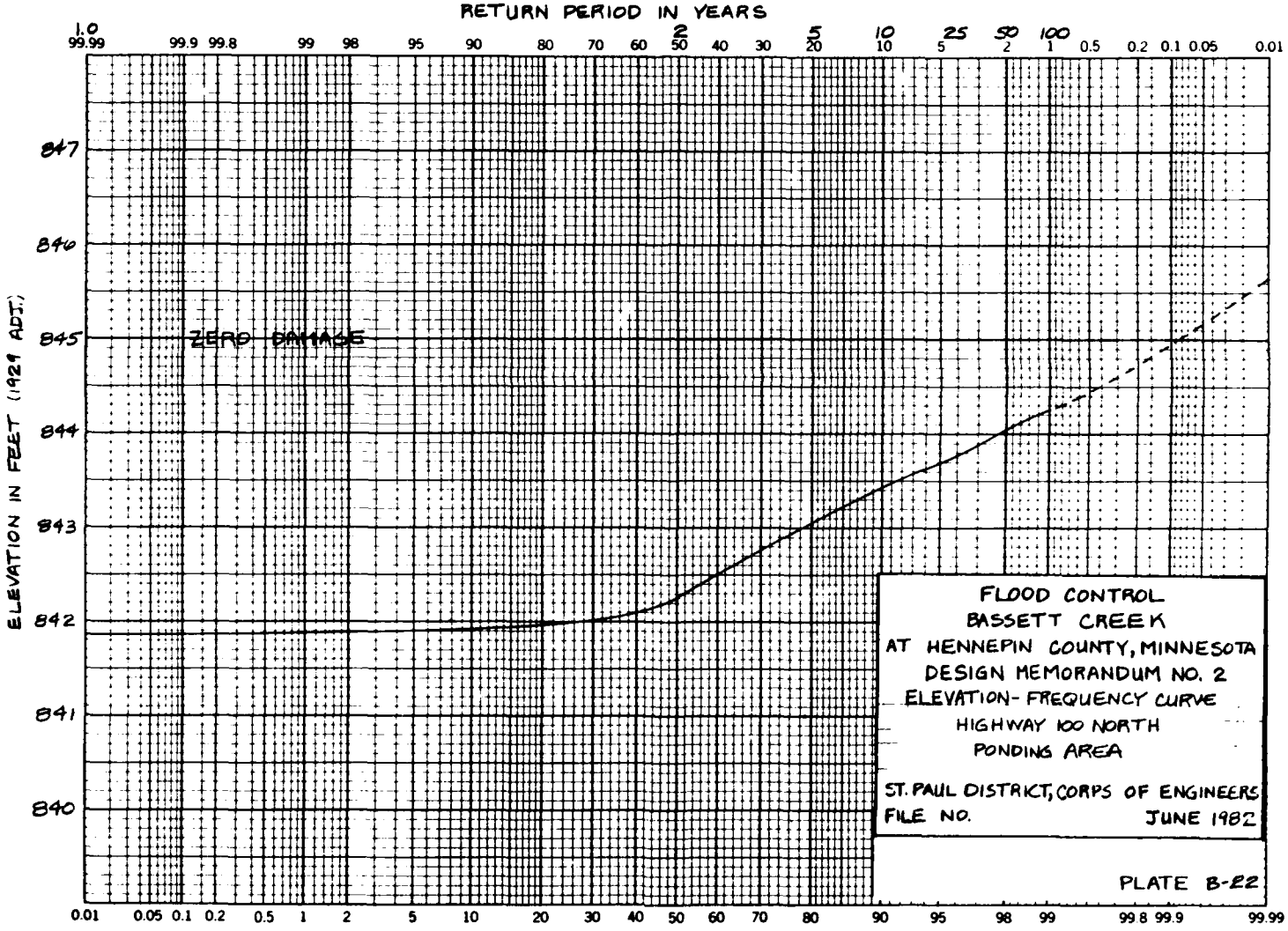

RETURN PERIOD IN YEARS
10 99.99 99.9 99.8 99 98 95 90 80 70 60 2 50 40 30 5 20 10 10 5 25 2 50 1 100 0.5 0.2 0.1 0.05 0.01
ELEVATION IN FEET (1929 ADJ.)
888
887
886
885
884
ZERO DAMAGE
0.01 0.05 0.1 0.2 0.5 1 2 5 10 20 30 40 50 60 70 80 90 9
FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
ELEVATION-FREQUENCY CURVE
WESTERN AVENUE-WINNETKA AVENUE
PONDING AREA
ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO.
JUNE 1982

PLATE B-21


RETURN PERIOD IN YEARS
1.0 99.99 99.9 99.8 99 98 95 90 80 70 60 2 50 40 30 5 20 10 10 25 5 50 2 100 1 0.5 0.2 0.1 0.05 0.01
ELEVATION IN FEET (1929 ADJ.)
847
846
845
844
843
842
841
840
ZERO DAMAGE
0.01 0.05 0.1 0.2 0.5 1 2 5 10 20 30 40 50 60 70 80 90 95 98 99 99.8 99.9 99.99
FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
ELEVATION- FREQUENCY CURVE
HIGHWAY 100 NORTH
PONDING AREA
ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO.
JUNE 1982
PLATE B-22

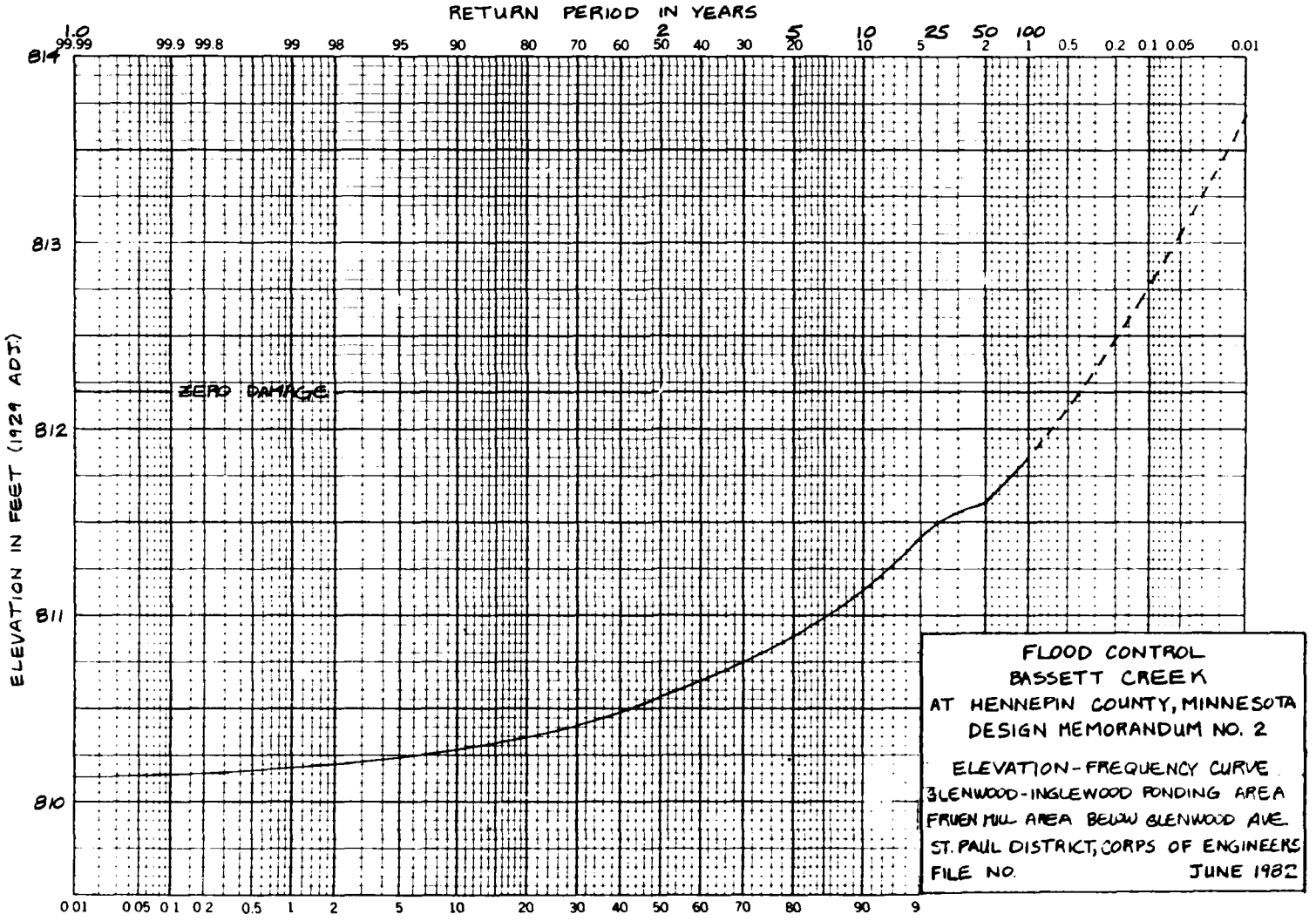
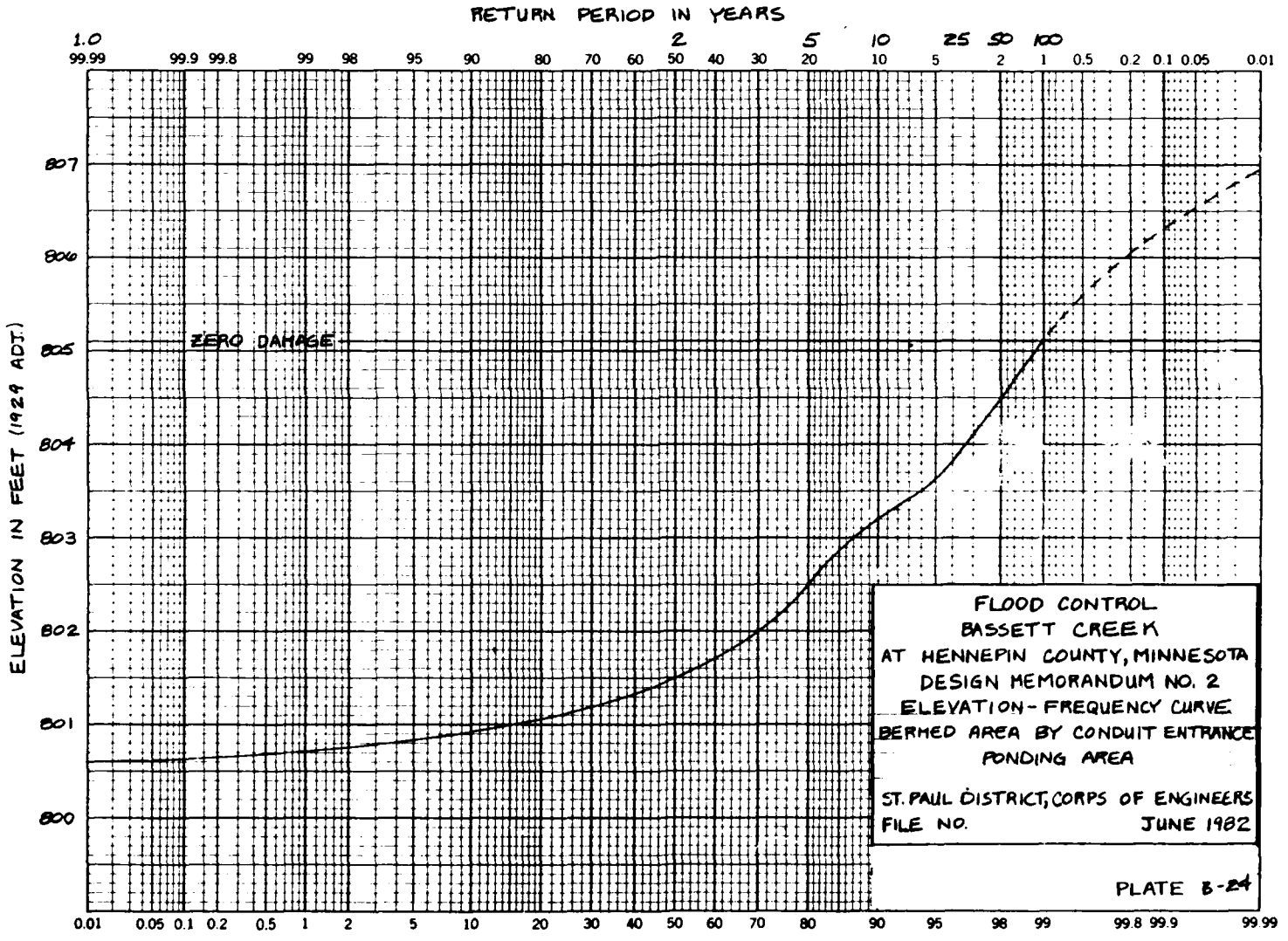

RETURN PERIOD IN YEARS
1.0 99.99 99.9 99.8 99 98 95 90 80 70 60 50 40 30 20 10 5 2 1 0.5 0.2 0.1 0.05 0.01
2 5 10 25 50 100
ELEVATION IN FEET (1929 ADJ.)
814
813
812
811
810
ZERO DAMAGE
0.01 0.05 0.1 0.2 0.5 1 2 5 10 20 30 40 50 60 70 80 90 99
FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
ELEVATION-FREQUENCY CURVE
GLENWOOD-INGLEWOOD PONDING AREA
FRUEN MILL AREA BELOW GLENWOOD AVE.
ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO.
JUNE 1962

PLATE B-23


RETURN PERIOD IN YEARS
1.0
2
5
10
25
50
100
99.99
99.9
99.8
99
98
95
90
80
70
60
50
40
30
20
10
5
2
1
0.5
0.2
0.1
0.05
0.01
807
806
805
804
803
802
801
800
ELEVATION IN FEET (1929 ADJ.)
ZERO DAMAGE
0.01
0.05
0.1
0.2
0.5
1
2
5
10
20
30
40
50
60
70
80
90
95
98
99
99.8
99.9
99.99
FLOOD CONTROL
BASSETT CREEK
AT HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2
ELEVATION - FREQUENCY CURVE
BERMED AREA BY CONDUIT ENTRANCE
PONDING AREA
ST. PAUL DISTRICT, CORPS OF ENGINEERS
FILE NO.
JUNE 1982
PLATE B-24

DEPARTMENT OF THE ARMY
St. Paul District Corps of Engineers
1135 U.S. Post Office & Custom House
St. Paul, Minnesota 55101

FLOOD CONTROL

BASSETT CREEK

HENNEPIN COUNTY, MINNESOTA

DESIGN MEMORANDUM NO. 2, PHASE II

APPENDIX C

GEOLOGY AND SOILS

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C-2	HIGHWAY 100 EMBANKMENT - STABILITY ANALYSIS - PARTIAL POOL, STEADY SEEPAGE AND EARTHQUAKE CONDITIONS
C-3	HIGHWAY 100 EMBANKMENT - STABILITY ANALYSIS - TYPICAL CALCULATION OF CRITICAL FAILURE ARC

Plates (con.)

Number	
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C-7	EDGEWOOD EMBANKMENT - FOUNDATION CLAY TILL - UNDISTURBED SHEAR STRENGTH PARAMETERS
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C-9	TUNNEL BORINGS
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C-11	TUNNEL BORINGS
C-12	FRUEN MILL BORINGS
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C-29	SOIL TEST DATA - HIGHWAY 100 EMBANKMENT
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C-31	SOIL TEST DATA - GOLDEN VALLEY COUNTRY CLUB
C-32	SOIL TEST DATA - EDGEWOOD EMBANKMENT

Plates (con.)

Number	
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C-38	SOIL TEST DATA - EDGEWOOD EMBANKMENT
C-39	SOIL TEST DATA - EDGEWOOD EMBANKMENT

APPENDIX C

GENERAL TOPOGRAPHY AND GEOLOGY

1. The topography and most of the geology important to the Bassett Creek project are products of Pleistocene and Recent sedimentation and erosion. That portion of the geologic history is, therefore, summarized to provide background for understanding the physical setting of the project.

2. At the beginning of the Pleistocene Epoch, the area consisted of broad uplands and deep bedrock valleys similar to conditions that now exist farther south along the Mississippi River. Glaciers advanced several times over this setting throughout the Pleistocene, but little evidence of early glaciations persist due to erosion during long interglacial stages. The final two glacial advances which occurred during the Wisconsin Stage, however, completely filled the old valleys with debris and developed a new landscape with only subtle reflections of the preglacial topography.

3. Immediately following the retreat of the glaciers, the Bassett Creek basin consisted of a hummocky till and outwash plain with scattered blocks of ice that later melted and left depressions which became lakes, bogs and marshes that were subsequently filled with peat, soft clay and silt. Bassett Creek now meanders through this glaciated terrain in a channel less than 50 feet wide and at most a few feet deep. In some areas it has cut a small valley through glacial till knolls, but throughout most of its length it flows through low areas with no identifiable valley and in which drainage has been improved by ditching or enlarging and cleaning the small, natural channel. The creek flows in an underground conduit from 2nd Avenue North to the Mississippi River. The total relief in the basin is 210 feet, but local relief is generally less than 70 feet. In much of the area the natural topography has been altered by leveling and filling for residential and commercial development.

4. The thickness of the overburden and the underlying types of bedrock vary greatly throughout the basin due to the presence of buried preglacial valleys. Most of the project features are small and affect the subsurface to only a shallow depth; therefore, accurate definition of the depth to bedrock and the bedrock stratigraphy are not important considerations. A general geologic column is provided for background information, and additional bedrock information is presented when important in the discussion of specific features.

5. Upstream of the proposed tunnel entrance where Bassett Creek intersects 2nd Avenue North, bedrock is of little concern. The geology pertinent to the project consists of soft alluvial silts and clays, bog deposits of peat, organic-rich silts and clays resting on medium dense glacial sands and tills. Along the proposed tunnel heading the materials consist of Mississippi River terrace deposits of soft clays, silts and medium dense sand, medium dense to dense glacial drift and sandstone bedrock of the St. Peter Formation.

TABLE 1

GENERAL GEOLOGIC COLUMN FOR BASSETT CREEK DRAINAGE BASIN

SYSTEM	FORMATION		APPROXIMATE THICKNESS (feet)	DESCRIPTION
QUATERNARY	Recent Sediments		0-50	Soft clay, silt and peat
	Glacial Drift		25-350	Medium dense to dense sand, gravel and glacial till
ORDOVICIAN	Platteville Formation		Up to 30	Dolomitic limestone, thin to medium bedded, gray
	Glenwood Formation		Up to 5	Shale, soft, blue-gray
	St. Peter Formation		150	Sandstone, fine to medium, well-sorted, quartzose, poorly cemented, 50' of siltstone and moderately cemented sandstone near base, white to gray
	Prairie du Chien Group	Shakopee Formation	50	Dolomite, thin-bedded, light brown
		Oneota Formation	75	Dolomite, thin to thick bedded, brown
CAMBRIAN	Jordan Formation		90	Sandstone, fine to coarse, massive, brown
	St. Lawrence Formation		50	Dolomitic siltstone and fine dolomitic sandstone, gray to gray-green
	Franconia Formation		190	Sandstone with interbeds of siltstone and shale, white to gray and green
	Galesville Formation		35	Sandstone, medium to coarse, yellow
	Eau Claire Formation		Up to 150	Sandstone, siltstone and shale, brown
	Mt. Simnon Formation		Up to 200	Sandstone, medium to coarse, gray
KEWEENAWAN	Hinckley Sandstone		Up to 200	Sandstone, medium to coarse, buff to red
	Red Clastics		Up to 4000	Sandstone, silty, feldspathic, fine, includes red shale
	Volcanic Rocks		Up to 20000	Mafic lava flows with thin interbeds of tuff and breccia

6. Bassett Creek is a local discharge corridor for ground water as well as surface water and, as such, is the low point in the water table throughout most of the basin. Except for unwatering required for construction of the tunnel portion of the project, the proposed work will have no temporary or long-term effect on the ground water of the basin.

SUBSURFACE INFORMATION AND LABORATORY TEST DATA

7. BORINGS

A total of 65 Corps of Engineers (USCE) machine borings were taken for various features throughout the Bassett Creek watershed. In addition, 46 borings were taken in the Highway 100 Embankment, Edgewood Embankment and Tunnel areas by other engineering organizations. The locations of the borings are shown on plan views of the individual structures. Logs of the borings are grouped separately for each structure and are shown either on the plan view sheet in the main report or on separate plates in this appendix. Borings taken for features that have been deleted are not shown in this report.

8. LABORATORY TEST DATA

Laboratory tests performed to date include moisture contents, Atterberg limits, mechanical analysis, undisturbed and remolded triaxial and direct shears, compaction and consolidation tests. Moisture contents, Atterberg limits and standard penetration test results are shown on the boring logs. Laboratory test results are presented on Plates C-17 to C-39.

9. The undisturbed samples from USCE drilling were obtained from borings offset slightly from a pilot hole while undisturbed sampling by others was obtained at preselected depths within the pilot hole. The USCE sampling procedure permits preselection of sampling depths on the basis of information shown on the log of the pilot boring and, therefore, provides samples that are more representative of foundation materials. Where this procedure was used, the offset boring number appears on individual laboratory test results, but subsurface conditions are shown by the detailed log of the pilot boring. Also the boring number of the pilot hole is used to show the location of both borings in plan. Therefore, boring numbers ending with the letters MU (80-42MU) that appear on USCE test results represent borings slightly offset from a pilot boring having the same prefix numbers but ending with the letter M (80-42M).

10. The individual strength test results were used to develop summary plots whenever sufficient data was available for a given material at a specific site. The plates showing the strength plots are shown in this appendix.

BASSETT CREEK TUNNEL

11. GEOLOGY

The geology pertinent to the proposed tunnel is discussed using a

classification system presented in Miscellaneous Investigation Series Map I-1157, Geologic and Hydrologic Aspects of Tunneling in the Twin Cities Area, by Norvitch and Walton and published by the United States Geological Survey in 1979. Information from this publication, boring logs obtained from the Minnesota Department of Transportation and a minimum number of site specific borings and laboratory tests to fill gaps in the information were used for this report. The basic data are considered adequate only for the development of design concepts and realistic cost estimates. Significant additional information will be needed for final design.

12. The interpreted subsurface conditions along the tunnel alignment are shown on Sheets 5 through 7 with materials classified according to the previously referenced system which is based on time and mode of deposition. Although variations of soil types within each overburden unit are known or expected, sufficient similarities of engineering properties of the components of each unit exist to justify the use of the system for the present level of planning. The stratigraphy and general engineering properties of the bedrock units are well established from extensive tunneling in the Minneapolis area. Site specific investigation and testing of bedrock were, therefore, not necessary for this report.

13. The general properties of the classification units are discussed in the following paragraphs. In addition, numerical values accepted as reasonable for the engineering properties of the various materials are summarized in Table 2. A discussion by stationing along the tunnel alignment summarizes identified considerations, problems and solutions in paragraphs 25 through 33.

14. OVERBURDEN

Overburden units within the influence of the tunnel are Fine-Grained Lower-Terrace Deposits (Qmq), Coarse-Grained Lower-Terrace Deposits (Qmz) and Undifferentiated Glacial Till, Outwash and Alluvium in Buried Bedrock Valleys (Quvf). Fill is, of course, present in many areas and is expected to consist of a variety of materials ranging from trash to clean sand. All identified fill material is above the tunnel heading. Fill is, therefore, not discussed as a separate unit.

15. Fine-Grained Lower-Terrace Deposits (Qmq)

Although fine-grained terrace sediments are classified along with the coarse terrace material as being deposited by the Mississippi River when it flowed at a much higher level than at present, the distribution pattern and texture suggest they may have been deposited later in backwater trapped behind a riverward terrace. In either case the sediments have never been consolidated by any natural overlying load. In some areas, however, they have been loaded by fill or structures in the urbanization of the area.

16. Fine-grained terrace deposits are composed of silt and clay with minor amounts of silty and clayey sand. The materials are very soft to soft with a penetration resistance measured by standard penetration tests commonly less than 3 blows per foot. Although blows as high as 21 were recorded for some material classified in this unit, they are exceptions and may be

TABLE 2

ENGINEERING PROPERTIES OF BASSETT CREEK SOILS AND BEDROCK

Units	Type	Dry Unit Weight (pcf)	Saturated Unit Weight (pcf)	Modulus of Elasticity (psf)	Poissons Ratio	Coefficient of Earth Pressure at Rest	Void Ratio	Porosity (%)	Standard Penetration Test (BPF)	Compressive Strength (psf)	∅ (deg)
Qmq*	Clay	74	106	10	0.50	1.0	1.4	--	0-5	600	20
Qmc**	Sand	-	130	2250	0.33	0.5	0.6	--	10-25	-	33
Quvf**	Clay	--	125	900	0.44	0.8	0.9	--	10-25	1500	29
Quvf**	Sand	--	135	9500	0.28	0.4	0.5	--	15-50	-	37
Osp***	Sandstone	126	-	$0.5\text{-}0.8\times10^5$	--	-	-	29	75/0.0-0.3'	$1.4\text{-}4.0\times10^5$	60

* values obtained from test data
** assumed values
*** values obtained from "Miscellaneous Investigation Series Map 1-1157, Geologic and Hydrologic Aspects of Tunneling in the Twin Cities Area."

attributed to obstructions or errors in classification. All material in this unit should be treated for planning purposes as soft clays and silts. These soils will be difficult to unwater and susceptible to subsidence if subjected to sustained unwatering effort. In addition, they will exhibit poor stability at the tunnel heading as well as in open cuts and shafts. Settlement above the tunnel due to loss of ground will, therefore, be extremely critical when tunneling in this material.

17. Coarse-Grained Lower-Terrace Deposits (Qmz)

The coarse-grained terrace deposits are more typical of river terrace sediments than are the soft, fine textured materials. They are composed primarily of fairly clean sand and gravel with lesser amounts of clay and silt. Some prominent clay and silt beds are, however, present. The material is predominantly medium dense to stiff with penetration resistances generally more than 15 blows per foot. The granular nature will facilitate unwatering the unit, but sufficient beds of clay and silt are present to cause problems with perched water and zones of undrained soil that may be unstable when encountered at a tunnel heading. Subsidence due to unwatering is a consideration but is not evaluated as a significant problem. Settlement above the tunnel due to lost ground is an important consideration and ground stabilization may be required in critical areas.

18. Undifferentiated Glacial Till, Outwash and Alluvium in Buried Bedrock Valleys (Quvf)

Information concerning the character of this unit was obtained primarily from boring logs provided by the Minnesota Department of Transportation. First-hand knowledge of the drilling characteristics and field inspection of materials are, therefore, lacking. The available data indicate the tunnel will be in clay with a penetration resistance of 10 to 20 blows per foot throughout most of this unit. The dominance of stiff clay may be a benefit or a liability depending on whether it is fairly homogeneous or highly variable in texture and structure. A good, uniform clay would require no unwatering and allow simple, repetitive tunneling procedures. A stratified or mixed unit with sandy phases would, on the other hand, be difficult to unwater and would cause support problems due to running ground at the tunnel heading. Present plans are based on an interpretation between the two extremes. Minor tunneling problems due to the presence of boulders in glacial till are expected. Boulders and poor quality weathered rock are also expected to be problem elements where the tunnel goes from glacial material into bedrock.

19. BEDROCK

The tunnel will penetrate only the St. Peter Formation and will be located between 40 and 55 feet below the base of the Glenwood Formation. Although the tunnel alignment in bedrock has not been explored specifically for this study, sufficient data are available to allow problems to be identified and reliable cost estimates to be made. The St. Peter Formation has been tunneled extensively in the Twin Cities area. A good description of the formation and tunneling characteristics developed from this experience is presented on Plate 3 of the reference cited in paragraph 11. The description is quoted in its entirety as follows:

The St. Peter Sandstone provides an optimum environment for low-cost tunneling and underground-space development. Except for a few thin, discontinuous shaly beds in its lower half, it is an exceptionally massive, homogeneous sandstone with little or no discernible bedding. Though weakly cemented the rock is well compacted. Cohesion is provided mainly by a small percentage of clay binder. The rock has good confined compressive strength and low to moderate compressibility, but it is friable and easily disaggregated. Pieces can be broken by hand, and the formation is commonly excavated rapidly by a water jet, which reduces the rock to a sand slurry removable by pipeline and slurry pump.

Many miles of unsupported utility and drainage tunnels have been constructed in the St. Peter Sandstone. Unsupported caverns 30 feet or more in width, which were excavated 50 to 100 years ago when the St. Peter Sandstone was mined for foundry and glass sand, still remain open.

Failure takes place gradually in underground openings in the sandstone through grain-by-grain raveling and crumbling of exposed surfaces. This process is most rapid in zones of tensional stress in the crowns of openings. Over a period of time, the crowns of tunnels assume a "gothic arch" form or even a convexly curved "tented" profile. This process can be arrested by preventing skin decomposition. Research at the University of Minnesota (Nelson, 1977) has developed methods of arresting the skin decomposition by spraying. The rock is stabilized by impregnation with a chemical grout. Use of the technique makes the St. Peter Sandstone stable, and easily and rapidly excavatable by low-cost methods.

The principal problems with excavation in the sandstone relate to the occurrence of water, and to zones of diagenetic alteration in the clay binder. Both occur sporadically and are not predictable on the basis of existing information.

The St. Peter Sandstone generally has moderate permability (coefficient of permeability 3.5 to 6.6 x 10^{-3} cm/sec), and because the rock is sparsely jointed, water enters excavations mostly by slow seepage. Difficulties arise where joints are encountered below the water table or where the formation has been disturbed by nearby operations. Once free flow begins in a joint or disturbed zone the rock erodes rapidly and must be protected.

In some places illite, which is the primary clay binder in the rock, has been altered to kaolinite. The illite tends to adhere to sand grains and bind them together, but the secondary kaolinite forms discrete crystalline grains in spaces between sand grains (W. E. Parham, written commun., 1975) and cohesion is greatly reduced. Where this process is well-advanced the rock becomes an almost incoherent, free-running sand. Such conditions cannot be predicted because the geologic factors that control the formation

of kaolinite are not well-enough understood. Tunneling operations should include stand-by preparation for rapid breast-boarding to deal with this phenomenon. Chemical grouting probably can be used as a control.

20. The preceding discussion does not stress the fact that unlined tunnels must not be subjected to running water. Water control during construction is, therefore, very important. Also, prevention of leakage from or into completed tunnels is an important consideration and must be addressed in the tunnel design.

21. Problems in unwatering the sandstone were encountered by the Minnesota Department of Transportation in their 2nd Street tunnel, to which this tunnel will connect, due to shaly seams about 90 feet below the base of the Glenwood Formation. This material served as an aquitard which decreased the drawdown efficiency of the unwatering wells. A similar problem must, of course, be anticipated in the construction of the subject tunnel. Experience in the same tunnel showed that spraying the sandstone with sodium silicate immediately after exposure controlled raveling of the sandstone but that occasional treatment with rock bolts, mesh and shotcrete was required to control excessive overbreak at the crown of the tunnel.

22. GROUND WATER

The entire tunnel will be below the water table for which an approximate, or inferred, location is shown on the geologic profile along the tunnel alignment. Where bedrock is present, a perched body of water is expected on top of the Platteville Formation and the true water table is in the St. Peter Formation. Considerable refinement in the definition of the ground-water conditions and hydraulic properties of the materials will be necessary to allow final evaluation of the tunneling methods and unwatering effort required and the impact of this effort in areas susceptible to subsidence due to drawdown of the water table.

23. TUNNELING PROCEDURE

Options available for tunneling in the overburden range from simple hand mining in a shield with temporary ring-beam and lagging support followed by casting a permanent liner in place to tunneling with compressed air to control ground water and improve ground stability with installation of a permanent liner as the tunnel progresses. Intermediate between the extremes are tunneling with a slurry shield, earth-pressure-balance shields or pipe jacking. Compressed air, slurry shields and possibly pipe jacking require little unwatering, require minimum ground modification and yield the least disturbance to overlying and adjacent features. They are, however, more complex and less common in the construction industry. The earth-pressure-balance and simpler shields generally require unwatering, require a greater amount of ground modification and produce greater disturbance to overlying and adjacent features. Additional data and evaluation are necessary to select the most cost-effective tunneling method; therefore, present plans and costs are based on tunneling with a rudimentary shield, maximum unwatering effort and significant ground modification. As more data become available in the feature design stage, tunneling procedures will be analyzed to

optimize the tunneling method with an appropriate design and disturbance restrictions.

24. A number of options are available for tunneling in the St. Peter Formation, but the method used extensively consists of simple mining with hydraulic jets and removal of muck by pumping. The method requires unwatering which is normally performed with deep wells. As long as such tunnels in the formation remain unwatered they require little stabilization until a permanent liner is installed. No improvement over this tunneling method is expected to evolve from more detailed study.

25. TUNNELING CONSIDERATIONS

The following discussion addresses problems and considerations identified from the interpreted subsurface conditions along the tunnel alignment from the intake structure to Sta. 66+50 where it joins the existing 2nd Street tunnel. The evaluation is based on tunneling in the overburden by jacking a shield followed by ring-beam and lagging support and tunneling in the sandstone by hydraulic jetting with stabilization limited to spraying the sandstone with sodium silicate supplemented by occasional rock bolts, mesh and shotcrete as needed.

26. The intake structure will be located 50 feet upstream from Sta. 0+00. It will penetrate about 20 feet of soft terrace clay and be founded in terrace sand. The soft clay will have to be stabilized with sheet piling and the water table will have to be drawn down at least 30 feet. No significant problems are anticipated.

27. From the intake structure to Sta. 2+50 the tunnel will be in terrace clay and sand. The tunnel is underlain by 15 to 20 feet of sand that rests on clay. Unwatering in the sand will require closely spaced wells due to the shallow depth of sand below the tunnel. Drainage from the clay will, however, be slow, and ground support problems and settlement due to lost ground are anticipated. At Sta. 1+70 the tunnel will pass 11 feet under an 86-inch masonry sanitary sewer. The soft clay between the sewer and tunnel precludes ground modification by grouting; therefore, structural support will be required.

28. From Sta. 2+50 to 7+50 the tunnel will be in terrace sand. Glacial clay rises in a downstream direction and intersects the tunnel at Sta. 7+50. The thinning of the sand will make unwatering increasingly difficult due to the decreasing depth of wells below the tunnel. The tunnel will be offset 10 feet horizontally from buildings at Sta. 3+25 and 6+90. Borings will be taken at each building to determine appropriate stabilization methods and required instrumentation. Based on interpreted conditions, the building at Sta. 6+90 may possibly be stabilized by chemical grouting in the terrace sand or compaction grouting in the clay. Sufficient sand is not indicated at Sta. 3+25 to allow treatment by chemical grouting. Structural support or ground retention may be the only positive solutions for the buildings and will be evaluated. The cost of stabilization must, of course, be weighed against that of no protection and repair of any damage. At Sta. 5+70 the tunnel passes 20 feet under two sewers, a 6-inch water main and utility lines. A tolerable settlement must be defined by the owners and appropriate stabilization incorporated in the design.

29. From Sta. 7+50 to 10+00 the tunnel will be in terrace sand and glacial clay. Unwatering will require closely spaced surface wells or unwatering from the tunnel itself. A 3.5° change in tunnel alignment occurs at Sta. 8+00.

30. From the tunnel intake to Sta. 10+00 the soft terrace clay will be subjected to drainage by unwatering the underlying sand. If sufficient drainage occurs, the clay will consolidate and settlement, or subsidence, at the surface will be evident. The amount of subsidence will depend on the permeability of the clay, the load on the clay, and the duration of the drainage. Available test data show that the clay layer has a saturated unit weight of 100 pcf and a voids ratio of 1.9. In the absence of other test data it is assumed that the permeability ranges from 10^{-6} to 10^{-7} cm/sec, the compression index (C_c) is 0.65, the coefficient of consolidation (C_v) is 0.0066 to 0.066 ft^2/day and that the 20-foot clay layer will drain only to the underlying sand layer. The period of unwatering during which consolidation can occur is estimated to be 18 months. Using these assumptions, subsidence of 1.1 to 3.4 inches is predicted to occur over the unwatered area during the construction of the tunnel. Additional settlement over the tunnel alignment will, of course, occur due to lost ground.

31. From Sta. 10+00 to 30+00 the tunnel will be in glacial sediments indicated by the existing data to be stiff clay. The need for little, if any, unwatering effort is indicated, and disturbance of the overlying strata and structures is not expected to be a significant problem. Additional borings are needed in this reach to confirm such an optimistic interpretation, therefore, tunnel cost estimates for this report are based on more difficult conditions. Between Sta. 14+50 and 19+00 the alignment is overlain by several utilities and adjacent to bridge piles. A more intensive restudy of as-built bridge drawings and utilities will be made to confirm that the tunnel can be completed without damage to these features. The tunnel will pass 10 feet horizontally from building corners at Sta. 22+40 and 23+50. Borings will, of course, be taken at each site to determine if stabilization is cost effective compared with repair of any damage. Known utilities overlie the tunnel at Sta. 20+80, 21+70, 25+75 and 28+30. Tunnel alignment changes of 1.5°, 4.5° and 4.0° occur at Sta. 15+00, 19+10 and 22+40, respectively.

32. From Sta. 30+00 to 50+00 the tunnel is still in glacial sediments; however, available data indicate the material is sandier and greater unwatering and support problems are anticipated. The tunnel will pass under railroad tracks between Sta. 30+80 and 33+00. Existing buildings and utilities downstream from 36+00 will be removed for highway construction prior to tunneling and will not present a problem. Existing data indicate the tunnel will be in soft terrace clay or will have a mixed face of glacial sand and soft terrace clay where it passes under a buried channel in the glacial sediments between Sta. 41+00 and 45+00. Unwatering and support problems are anticipated. An inlet for surface drainage will be constructed between Sta. 43+00 and 44+00. The structure will be in soft terrace sediments which will require unwatering and will require structural support for construction. The tunnel alignment changes 5° at Sta. 37+40 and 36° between Sta. 42+50 and 45+50. Site specific borings will be taken for the inlet and to better define the subsurface profile.

33. At Sta. 50+00 the tunnel will enter the St. Peter Sandstone. Problems with boulders and weathered rock are anticipated at this transition. Unwater-

ing and occasional minor support problems are anticipated in the sandstone, but no problems with surface settlement are anticipated due to the thick cover of bedrock. The tunnel alignment makes a 90 transition between Sta. 65+00 and the existing 2nd Street tunnel. Sufficient borings will be taken to determine the character of the sandstone for tunnel and unwatering design.

TUNNEL PONDING AREA

34. GENERAL

The plan view, sections, and boring logs are shown on Sheets 10 and 11. Four borings were taken in the area, showing 10 to 20 feet of dump fill comprised of bricks, pieces of concrete, glass and wood mixed with gritty sand and gravel underlain by up to 65 feet of very soft silty clay and organic silt. The existing ground surface is at about elevation 810 with 15 to 20-foot mounds of brick rubble scattered over a portion of the area. The borings indicate that the original ground surface in the area, before placement of the fill, ranged from elevation 793 to 799. The water table ranges from elevation 805 at the upstream end of the pond to elevation 803 at tunnel inlet. Since the tunnel inlet structure has a weir elevation at 799.0, the water table in the immediate vicinity of the creek in the ponding area will be permanently lowered. The berm area and old creek channel, north of the pond, will be filled with suitable sandy soil and slopes will be at 1V on 3H or flatter. There is one sanitary sewer crossing the ponding area: an 86-inch brick and stone arch crossing on the Currie Avenue alignment. The pipe would be exposed by excavation to elevation 799, therefore, the 86-inch conduit will be rerouted north of the pond.

35. DISPOSAL AREA

About 300,000 c.y. of material will be excavated from the ponding area, with the bulk of that material being coarse dump fill. About half of the excavated fill material will be placed in the disposal areas shown on Sheet 10. The remainder of the material will have to be disposed of in a satisfactory manner. While no hazardous wastes were identified in the boring logs, the nature of a dump fill area suggests that some polluted materials may be present. A more extensive investigation will be conducted to locate any hazardous materials, and a proper disposal area located for them, if any are found.

36. DROP INLET STRUCTURE

A concrete drop inlet structure 23 feet wide with a 6-foot drop, from elevation 805 to 799, will be constructed at the upstream end of the pond. Borings indicate that the bottom of the structure will be founded on about 35 feet of soft silty clay. Settlement is not considered a problem because 20 feet of excavation will be needed to found the structure. However, bearing piles may be needed beneath the structure because of the low bearing capacity of the soft clay.

PENN AVENUE CULVERT REMOVAL

37. GENERAL

The existing 125-foot long box culvert, approximately 18' x 7', will be removed and replaced with an open channel having a 36-foot bottom width and 1V on 3H slopes. Boring 80-32M indicates that there is about 10 feet of loose to medium dense sand and gravel underlain by over 45 feet of very soft clay. The channel excavation will be very close to Pier No. 3 of the Penn Avenue Bridge, which is on timber bearing piles. The estimated velocities will be less than 3 fps in this area, even during high water levels, and therefore, no erosion should occur. However, to assure that Pier No. 3 is never undercut, a 55-foot section of riprap will be placed as shown on Sheet 14.

FRUEN MILL

38. GENERAL

Proposed improvements in the Fruen Mill area include: widening and realignment of the channel, replacement of the existing stone dam, and replacement of the timber MN & S Railroad bridge. The plan and sections are shown on Sheets 15 through 19. The existing channel will be enlarged to a 30-foot bottom width below the proposed spillway. In addition, the channel will be realigned away from the failing timber retaining wall alongside Glenwood-Inglewood's water plant, with riprap-covered pervious fill forming the left bank of the channel. Due to steep slopes and constraints involved with realigning the MN & S Railroad tracks, a sheet pile retaining wall was used to form the right bank of the channel from below the concrete spillway to 125 feet below the replacement railroad bridge.

39. The existing stone dam has a crest length of 25 feet and is so badly deteriorated that repair and widening of the structure is considered impractical. A concrete spillway with a crest length of 30 feet and a 5.6-foot difference in elevation between the spillway crest and stilling basin floor will be constructed to replace the stone dam. The existing timber railroad bridge supported by timber piles will be removed and replaced with a structure located upstream of existing bridge. The new structure will be constructed with only one center pier in the channel, eliminating the obstructions created by the existing bridge's many timber piles.

40. Soil borings taken in the Fruen Mill area are shown on Plate C-12. Soil test data taken to determine the allowable railroad bridge pile loadings is shown on Plates C-19 through C-21.

HIGHWAY 55 CONTROL STRUCTURE

41. GENERAL

The plan of the Highway 55 Control Structure, sections and boring logs are shown on Sheet 20. Two borings were taken in the area; 79-11M about 90

feet from the structure and 80-42M, within 10 feet of the structure. The borings show that there is a layer of soft to medium stiff clay and loose silt underlying a layer of sandy fill. A 28-foot long by 18-foot wide concrete structure will be added to the existing 8-foot by 10-foot double box culverts.

42. BEARING CAPACITY

Despite the soft layer of soil underlying the proposed control structure, bearing capacity is not considered to be a problem for this structure. Using boring 80-42M to obtain representative undisturbed samples, it was found that cohesive strengths from unconsolidated-undrained tests ranged from 800 to 1200 psf. Using the bearing capacity formula for a saturated clay ($q=NcC$) and an average cohesive value of 800 psf, the ultimate bearing capacity is estimated to be 4000 psf. The loading applied to the ground from the construction of the control structure is only about 500 psf so that the factor of safety against bearing capacity failure is around 8.0.

43. SETTLEMENT

Although soft, compressible soils underlie the proposed control structure, it is anticipated that very little settlement will occur. Consolidation tests were conducted on 2 undisturbed samples from boring 80-42MU. One test was on a sandy clay and the other on a loose silt. The test results indicate both materials are overconsolidated. Using the Boussinesq method of reduction of stresses with depth for the 500 psf structure load, the resulting pressure applied to these compressible layers is less than the preconsolidation pressure. Assuming that the compressible layers extend from the bottom of the structure to a depth of 35 feet, a total settlement of less than 1 inch was calculated for the reloading caused by the structure.

HIGHWAY 100 EMBANKMENT

44. GENERAL

The plan, section views and boring locations of the Highway 100 embankment are shown on Sheets 23 through 27. A total of 18 borings were taken near this site, 16 by Barr Engineering Company and 2 by the Corps. The boring logs are shown on Plate C-13. The borings show that a thick layer of swamp deposits will underlie a major portion of the embankment. These organic swamp deposits are very soft and susceptible to significant amounts of primary and secondary consolidation. Underlying these soft deposits, at depths ranging from 10 to 48 feet, are medium dense sandy outwashes and in some areas a medium dense glacial till. The borings show that the critical portion of the embankment is from Sta. 6+00 to 14+00 where the swamp deposits are thickest. The alignment chosen will minimize the amount of construction over the swamp deposits and maximize the number of homes given 100-year protection while inducing the least amount of damage to private property in the area.

45. EMBANKMENT DESIGN

For design purposes the embankment is divided into 3 sections: Sta. 1+00 to 4+75, Sta. 4+75 to 5+60 and Sta. 5+60 to 14+43. Sta. 0+00 to 1+00 is a small road raise with a 30-foot top width. Between Sta. 0+00 and 4+75 the embankment is designed to contain the 100-year flood and overtop for larger floods. Following overtopping in this reach, the tailwater will rise rapidly and the remainder of the embankment will overtop at 851.5 with no significant head differential; consequently, there will be no danger from catastrophic failure to the homes immediately downstream of this section. The total embankment will overtop only for floods less frequent than the 500-year flood.

46. Sta. 1+00 to 4+75 has a top width of 20 feet, an upstream slope of 1V on 3H and a downstream slope of 1V on 6H from the top to elevation 847.0 then 1V on 3H from 847.0 to the ground surface. The ultimate top elevation after 100 years of secondary consolidation will be 851.0. Therefore, a 0.3-foot overbuild will be added to give a 851.3 top elevation at the end of construction. The relatively thin peat layer at the ground surface, as shown in boring BC-4M, will be excavated and replaced with adequate fill before construction. The embankment will be constructed of pervious fill to elevation 847.0, with 1V on 3H slopes. On top of this an erosion resistant clay having a liquid limit greater than 30 and a plasticity index greater than 15 will be placed to the top elevation and slopes described above. The clay fill and flat downstream slope are required in this reach because when the embankment overtops, there will be a 2.1-foot head differential. This overtopping will occur for only a matter of hours and the tailwater will rise very rapidly. By the time the headwater gets to elevation 851.5, the tailwater will have risen to the same elevation. This is not classified as a high hazard dam, therefore, the clay-capped embankment with flat slopes is considered adequate to withstand the overtopping differential for the limited time it will occur.

47. Sta. 4+75 to 5+60 is a road raise for Medicine Lake Road with a 30-foot top width, a top elevation of 851.5 and maximum grades of 5 percent. All soft sediments in this area will be excavated and backfilled with compacted pervious fill to assure no settlement beneath utilities under road. Side slopes will be 1V on 3H and all embankment fill will be pervious.

48. Sta. 5+60 to 14+43 will be constructed in 3 stages due to the large predicted settlements and the low unconsolidated, undrained strengths of the soft foundation soils. The first stage will be constructed in compacted layers to elevation 850 and allowed to consolidate for 12 months before Stage II construction begins. Stage III construction will begin 12 months after Stage II has been completed. Stage III construction will have a 20-foot top width and a top elevation of 856.6 that includes 5.1 feet of overbuild. Since large amounts of settlements will occur, the slopes will be continually changing. Therefore, the slope after Stage III construction will range from 1V on 3H to 1V on 4.75H upstream and 1V on 3.7H to 1V on 6H downstream. The embankment will be constructed of pervious fill; therefore, an inspection trench is considered unnecessary. This portion of the embankment will be overtopped at something less than the Standard Project Flood. The overtopping will occur with no head differential; consequently

little or no damage to the embankment should occur. See Sheets 24 through 26 for staged construction and typical sections.

49. STABILITY

Sta. 8+25 was chosen as the critical section for stability because boring BC-4E showed the thickest layer of soft sediments. This embankment will pond water for short periods of time minimizing the chance that steady state seepage through the embankment will occur. However, since pervious fill is being used, it was conservatively assumed that steady seepage would develop. Therefore, all conditions of stability discussed in EM 1110-1-1902 were analyzed.

50. The soft foundation sediments were divided into 3 separate types: peat, muck (OH-Pt), and organic silt (ML-OL). See the geologic profile on Sheet 23. All sampling, testing and classification for this site was done by Barr Engineering. When summarizing strength parameters, the strength tests for the peat and muck samples were combined because the test results and index properties were similar. A total of 4 undisturbed samples in the peat-muck soil resulted in 3 Q, 1 S and 3 $\bar{R}$ tests being conducted in addition to 14 vane shear tests. The organic silt had only 1 undisturbed sample on which a Q and $\bar{R}$ test were run. No tests were conducted on borrow samples, but with a pervious fill section adequate values could be estimated. For a summary of the soil strength parameters, see the tabulations on Plates C-4 and C-5.

51. The Q tests and vane shear tests indicate that the peat-muck materials have an undrained compressive strength of about 200 psf, which is not adequate to support the 8 feet of embankment fill plus 10 additional feet needed to allow for settlement. Therefore, construction will be staged to allow consolidation from partial lifts and thereby allow increased undrained strengths to be used as defined by the R-test strength envelope and shown on Plate C-4. Plate C-1 shows the increased strengths used for each stage. In order to use increased Q strengths the soft foundation materials would have to reach equilibrium from the added load. It was assumed that increased strengths could be used when the foundation reached a minimum of 90% primary consolidation. Therefore, from consolidation tests on this material, the time between stages was estimated to be 12 months, making the total time from Stage I to the end of Stage III greater than 2 years. This time could be greatly reduced if a system of vertical drains was installed in the soft soils beneath the embankment between Sta. 5+60 and 14+43 to speed up the consolidation process. Using the increased strength parameters and embankment sections for each stage as shown on Sheet 25, the minimum safety factor of 1.3 for the end of construction condition is satisfied for each stage. By applying a seismic loading coefficient of 0.025 to each of these cases, the earthquake factor of safety is also satisfied.

52. Because the embankment is being constructed of pervious fill, and while it is very unlikely, there is a possibility that high reservoir pools could stay up for a long enough duration to establish steady state seepage through the embankment. The section chosen for Cases II, IV and VI stability analysis has a top elevation of 854.4, a 1V on 3.45H upstream slope and a 1V on 4.4H downstream slope. This represents the embankment at the end of primary

consolidation which will occur about 1 year after Stage III construction. Secondary consolidation of the foundation will eventually flatten the side slopes to about 1V on 4.7H upstream and about 1V on 6H downstream. Thus the slope stability safety factors will increase with time. The sudden drawdown from maximum pool condition for water levels of 851.0 to 843.5 gives a safety factor just slightly greater than the required 1.0, but this is partly because a conservative $\emptyset'$ value of 30° was used for the embankment fill. If a $\emptyset'$ of 35° were used, a factor of safety of 1.26 would be achieved by the infinite slope formula. The minimum factor of safety is also for a failure arc almost tangent to the slope, which would cause no danger to the integrity of the embankment.

53. The partial pool and steady seepage with maximum pool conditions satisfy the required safety factors. The sudden drawdown from spillway crest and steady seepage with surcharge pool were not considered applicable for this embankment. Table 3 shows the required factors of safety and calculated factors of safety for each stability case. The stability analyses are presented on Plates C-1 to C-3.

TABLE 3

STABILITY AT STATION 8+25

Case	Design Condition	Minimum Required Factor of Safety	Minimum Calculated Factor of Safety
I	End of Construction		
	- Stage I	1.3	1.35
	- Stage II	1.3	1.44
	- Stage III	1.3	1.47
II	Sudden Drawdown from Maximum Pool	1.0	1.04
III	Sudden Drawdown from Spillway Crest	1.2	N/A
IV	Partial Pool with Steady Seepage	1.5	1.79
V	Steady Seepage with Surcharge Pool	1.4	N/A
VI	Steady Seepage with Maximum Pool	1.5	1.54
VII	Earthquake		
	- End of Construction		
	-- Stage I	1.0	1.08
	-- Stage II	1.0	1.22
	-- Stage III	1.0	1.29
	- Partial Pool	1.0	1.45
	- Steady Seepage	1.0	1.48

54. SETTLEMENT

The embankment is divided into 3 sections for settlement calculations: Sta. 1+00 to 3+50, Sta. 3+50 to 5+50, and Sta. 5+50 to 14+43. The foundation at Sta. 1+00 to 3+50, according to boring BC-4M, consists of 2 layers of soft sediments, separated by a layer of loose sand. Underlying the soft soils is a medium dense sand. The upper soft layer, about 5 feet thick, will be excavated and backfilled with pervious fill. This will leave a maximum compressible layer of 16 feet at a depth of about 15 feet. Settlement calculations show that with the average 6-foot height embankment, a maximum settlement of less than 0.5 feet will occur and of this 0.3 feet will be secondary consolidation during the next 100 years.

55. From Sta. 3+50 to 5+50, soft sediments will be excavated down to the medium dense sand or till and backfilled with compacted pervious fill to assure adequate stability and negligible settlement for the control structure, conduits and sanitary sewer and water main beneath Medicine Lake Road. The maximum excavation depth in this area is 17 feet.

56. From Sta. 5+50 to 14+43 the foundation consists of 25 to 47 feet of very soft organic sediments underlain by medium dense sand. Boring BC-4E shows the thickest layer of soft soil and boring BC-4C shows an average thickness. Based on consolidation tests and an embankment height of 8 feet plus 10 feet of overbuild, the dam was estimated to have the following settlements:

TABLE 4

SUMMARY OF TOTAL SETTLEMENT AT STA. 8+25 AND STA. 11+25

Boring BC-4 (maximum thickness)

Foundation Material	Thickness (ft)	Primary Settlement (ft)	Secondary* Settlement (ft)	
Peat	7	1.9	0.6	
Muck (OH-Pt)	29	4.6	2.8	
Organic Silt	11	0.1	0.0	
		6.6	3.4	Total=10.0 ft.

Boring BC-4C (average thickness)

Foundation Material	Thickness (ft)	Primary Settlement (ft)	Secondary* Settlement (ft)	
Peat	7	1.7	0.6	
Muck (OH-Pt)	21	4.7	2.1	
Organic Silt	-	-	-	
		6.4	2.7	Total=9.1 ft.

*Secondary Consolidation computed over 100-year period.

57. This section of the embankment extends mostly across an open park area; therefore, the large settlements should not cause problems to nearby structures or utilities. The closest structure is a sanitary sewer manhole about 130 feet from the downstream toe near Sta 13+00.

58. CONTROL STRUCTURE AND OUTLET WORKS

The peat and organic silt will be excavated down to the medium dense sand at the control structure to minimize potential differential settlement. The area will be excavated to elevation 827+ and immediately backfilled with compacted pervious fill to elevation 836.5 which is the base of the control structure. See Sheet 26 for sections through this area.

59. EXISTING UNDERGROUND UTILITIES

The existing vitrified clay pipe (VCP) under Medicine Lake Road will not withstand the additional load from the new embankment. The soft soils would also be susceptible to settlement. Therefore, the soft foundation soils will be removed from beneath the road and replaced with compacted pervious fill. The VCP pipe will be replaced with polyvinyl chloride pipe (PVC). The existing 8-inch cast iron pipe will not require replacement because of its higher crushing strength characteristics.

60. INSTRUMENTATION

Settlement plates will be installed in the area of Sta. 5+60 to 14+43 to measure the amounts of settlement beneath the embankment to compare with estimated amounts. Scheduled periodic surveys will monitor the embankment crest to assure that the required top elevation is maintained.

GOLDEN VALLEY COUNTRY CLUB EMBANKMENT

61. GENERAL

The plan and section views and boring logs of the Golden Valley Country Club embankment are shown on Sheets 29 through 30. Four borings were taken at this site, with 3 (80-23M, 80-24M, and 80-25M) located up to 95 feet downstream of the overflow structure. These borings, taken for a different alignment, show alternating layers of medium stiff and medium dense fine- and coarse-grained alluvium underlain by loose to medium dense sand to a depth of over 50 feet. Boring 75-1M, taken just upstream of the structure, shows soft to medium stiff fine-grained soils in the top 8 to 10 feet, with medium dense sand below.

62. EMBANKMENT DESIGN

The earth embankment is notched to allow necessary controlled overtopping to occur when water levels rise above the 100-year frequency flood level. The maximum head differential across the embankment that will occur during overtopping is about 2.1 feet. The flood flows will decrease rapidly and the overtopping condition will exist for only a matter of hours. Although designing an earth dam for differential overtopping is not standard design practice, it is considered necessary and acceptable for this embankment.

63. The top of the main portion of the embankment is at elevation 877.0 with 1V on 10H slopes down to the bottom of a 45-foot wide notch at elevation 874.5 (approximate 100-year flood level), which is centered over the twin

box culverts. The notched section of the embankment is designed to withstand overtopping by keying a 5-foot wide by 70-foot long concrete slab at the dam crest into the impervious embankment fill to act as a fixed overflow weir. Beneath this slab and extending along the downstream slope, 12 inches below the ultimate embankment surface, is a 12-inch layer of riprap on 6 inches of bedding and covered with $3^{\pm}$ inches of lean concrete. The concrete covered riprap extends down to where it either intersects the 33-inch riprap layer of the stilling basin at elevation 868.5, or to a depth of 3 feet below the existing ground surface. The riprap protection is buried for aesthetic reasons, with the whole embankment except the concrete crest slab covered with topsoil and seeded. If the soil above the riprap erodes during overtopping the rough concrete surface covering the riprap would provide better embankment protection than the riprap alone. To retard the loss of soil upon overtopping, the zone above the concrete-covered riprap will be constructed of an erosion-resistant clay that has a liquid limit greater than 30 and a plasticity index greater than 15.

64. The embankment has a top width of 10 feet at elevation 877.0 and a maximum height of less than 6 feet. The width across the notch at elevation 874.5 is 30 feet. The upstream slope is 1V on 4H from the top of the dam to elevation 870.0 where the slope changes to 1V on $2\frac{1}{2}$H. The downstream slope is 1V on 4H from the top of the dam to elevation 866.4, which is the culvert invert elevation and the point where the slope changes to 1V on $2\frac{1}{2}$H. The relatively flat side slopes were chosen to blend the embankment into the surrounding golf course terrain. The existing ground surface will be stripped to a depth of 6 inches beneath the embankment.

65. Any soft sediments encountered in the excavation for the two 4-foot by 6-foot concrete box culverts will be removed and replaced with suitable backfill.

66. STABILITY

Under normal conditions, the flow through the box culverts will be uncontrolled. High flood levels will occur only for short durations, not allowing the embankment to saturate or establish a phreatic surface through the embankment. Therefore, the only stability conditions that have to be analyzed are the end of construction case and a long-term stability case with no line of seepage through the embankment. The only testing done on undisturbed foundation soil samples were from boring 80-24M, 95 feet downstream of the structure. The following is a tabulation of strength parameters obtained from testing.

TABLE 5

TABULATION OF STRENGTH PARAMETERS

Boring	Depth (ft)	Class	Test	C (tsf)	Ø (deg)	Ø' (deg)
80-24M	11-12.5	SM-SC	Q	2.48	0	-
80-24M	11-12.5	SM-SC	$\bar{R}$	1.20	32.5	36.5
80-24M	13.6-15.0	CL	Q	2.62	0	-
Values Obtained from Edgewood Borrow Test Results			Q	0.38	10	-
			R	0.20	10	-
			S	-	-	31

67. For any condition of stability that needs to be analyzed, the factor of safety requirements are more than satisfied because of the high soil strength parameters, the relatively flat side slopes and the small height of the embankment. For this reason, a stability analysis plate for the embankment is not included in the report.

68. SETTLEMENT

Much of the foundation material is dense, coarse-grained soil and not susceptible to consolidation under the light loads being applied. There are small layers of medium stiff clay shown in borings 80-23M, 80-24M and 80-25M, but not enough to result in any measurable amounts of settlement. Boring 75-1M shows 5 to 8 feet of soft to medium stiff compressible material near the ground surface; however, since the embankment fill height ranges from only 3 to 5.5 feet above the ground surface, negligible settlement should occur. Small settlements under this embankment are acceptable, except beneath the fixed 45-foot long overflow weir and twin box culverts which pass under the weir section. Any soft sediments encountered beneath the concrete weir and twin box culverts will be excavated and replaced with suitable material to minimize settlement.

69. SEEPAGE CONTROL

Borings indicate that in general the foundation soils are pervious below the upper 5 to 10 feet. The maximum pool differential across the embankment is about 2.1 feet, and this occurs for only a short duration; therefore, seepage through the foundation is not a problem.

70. Under normal conditions this embankment will not pond water behind it. When high flood levels occur, there will be insufficient time to create steady seepage through the impervious-fill embankment. Therefore, seepage through the embankment will not be a problem.

WISCONSIN AVENUE EMBANKMENT

71. GENERAL

The plan, profile, sections and boring logs are shown on Sheets 31 through 32. Two soil borings were taken at the structure location indicating that up to 7 feet of soft swamp deposits are underlain by about 16 feet of medium dense sand and very stiff till. A thin layer of fill overlies the swamp deposits. The embankment, which will be constructed by the City of Golden Valley, will have a roadway crest width of 58 feet and a maximum height of 10 feet. The total length of roadway will be 1400 feet with 650 feet in a cut area and about 750 feet in a fill area. Little soils exploration or testing was done at this time because the final alignment is subject to change by the city. The control structure consists of a drop inlet with a 20-foot long weir attached to a 6' x 8' precast concrete box culvert and apron.

72. STABILITY

No test data were obtained from the borings, but using assumed values and test results from similar soils at the Highway 100 embankment, a stability analysis was completed. The following is a summary of strength parameters used for the analysis.

TABLE 6

TABULATION OF STRENGTH PARAMETERS

Soil Type	Q		R		S		γ_{sat}
	C (psf)	Ø	C (psf)	Ø	C (psf)	Ø'	(pcf)
Embankment Fill*	-	-	-	-	0	30	130
Peat**	240	0	400	17	-	-	72
Foundation Sand*	-	-	-	-	0	30	130

*Assmued Values
**From Highway 100 Embankment test results

73. The use of low undrained strengths for the soft organic layer required that 1V on 4H slopes be used to achieve an adequate factor of safety for the End of Construction condition. The Partial Pool and Sudden Drawdown conditions were not considered applicable because of the short duration of high water levels. The following is a tabulation of required and existing factors of safety for applicable stability cases.

TABLE 7

STABILITY AT STA. 10+00 USING 1V ON 4H SLOPES

Case	Design Condition	Minimum Required Factor of Safety	Minimum Calculated Factor of Safety
I	End of Construction	1.3	1.34
VI	Steady Seepage from Maximum Pool	1.5	1.72
-	Infinite Slope - No Seepage	-	2.3

74. If future exploration along the embankment alignment shows that the maximum depth of swamp deposits is no more than 7 feet, it would be recommended that the soft materials be excavated and replaced with adequate backfill. This would allow the embankment slopes to be steepened to about 1V on 2½H while still meeting stability requirements. This would also eliminate any problems with differential settlement due to varying thicknesses of soft, compressible soils beneath the roadway. In any event, the soft soils will be excavated from beneath the control structure and replaced with suitable backfill before constructing the drop inlet and placing the box culvert.

75. WINNETKA AVENUE RETENTION DIKE

The storage pool ponded by the Wisconsin Avenue embankment requires that a retention dike be placed along the west side of Winnetka Avenue, and a 24-inch CMP culvert be jacked beneath the road. The dike will be 540 feet long with a top elevation of 890, a 10-foot top width and 1V on 3H slopes. The maximum height of fill along the centerline of the dike will only be about 4 feet. However, this area is swampy and soft, compressible deposits are expected to underlie the proposed dike which could result in unacceptable amounts of settlement beneath the culvert. Soils investigation will be conducted before the Plans and Specifications phase to assure that there is an adequate design in this area.

MEDICINE LAKE OUTLET STRUCTURE

76. GENERAL

The plan view and sections are shown on Sheet 33. The boring logs are shown on Plate C-14. The borings in this area indicate 15 to 27 feet of soft, organic soils underlain by a stiff clay and dense sandy till. Initial plans called for a 100-foot long overflow earth embankment in this area. However, because of the highly compressible soils having poor compressive strengths and the erodible qualities of earth fill, it was decided that a sheetpile structure would cause fewer problems and be a better alternative. The proposed three-sided outlet structure consists of sheetpile driven to an adequate depth to allow ponding of less than 3 feet of water upstream of

the structure. Using boring 80-28M, the length of pile required is only 22 feet. Twelve inches of riprap and 6 inches of bedding will be placed inside the outlet structure to prevent erosion of materials during overtopping. The control structure will be embedded in the railroad embankment, and the slope of the embankment will be flattened to 1V on 2H with clay fill, topsoiled and seeded inside the structure. The structure will have a 13-foot low flow section at elevation 887.1 and a 150-foot long overflow weir at elevation 889.1. The 15-foot section of piles connecting the structure to the railroad embankment will have a top elevation of 892.1. The capacity of the existing opening in the railroad embankment will be increased by driving vertical piles at each abutment and placing concrete across the bottom of the channel.

77. Upstream of South Shore Drive an existing control structure will be removed and the channel will be widened with 20-foot steel sheetpiles driven to form the vertical sides of the channel. The top of the piles will be level with the existing ground surface (elevation 890$\pm$) and the bottom of the channel will be at 885.0.

EDGEWOOD EMBANKMENT

78. GENERAL

The plan and section views of the Edgewood embankment are shown on Sheets 37 through 38. Barr Engineering Company took 7 borings at the dam site and 8 more in the proposed reservoir. Three USCE borings were taken at the dam site and 2 in the reservoir area. The embankment borings show that the foundation consists of stiff to very stiff clay till and silt and is underlain by very dense sand at a depth of about 40 feet. The reservoir borings were taken to identify soils suitable for use as borrow in the construction of the Edgewood and Golden Valley Country Club embankments. These borings show that the borrow is a sandy clay till that will have relatively high remolded strength. The borings are shown on Plate C-16.

79. EMBANKMENT DESIGN

This embankment is designed to be overtopped at floods greater than the 100-year level without subsequent failure due to erosion of the downstream slope. The standard project flood (SPF) and probable maximum flood (PMF) will overtop the embankment with a maximum differential between headwater and tailwater of 11 feet. However, the duration of overtopping is very short with the SPF lasting about 1 hour and the PMF about 6 hours. Velocities during overtopping exceed 17 fps and a hydraulic jump will occur on the downstream slope. The embankment is classified as a high hazard structure and is designed to be stable for the PMF.

80. The earth embankment will have a top width of 20 feet, an average height of about 10 feet and a 1V on 3H upstream slope. Across the top and down the downstream slope, buried under 15 inches of soil, will be 12-inch rockfilled gabion baskets on 6 inches of bedding. The baskets will be covered with 3 inches of concrete to provide greater erosion protection

during overtopping. From Sta. 0+10 to Sta. 0+80 the gabions are placed at a 1V on 4H slope down to the existing ground surface and are then buried 5 feet at a 1V on 2½H slope. From Sta. 0+80 to Sta. 1+66 the gabion baskets will be placed on a 1V on 4H slope to elevation 870 where the slope changes to 1V on 2½H and extends to elevation 864.4. Between Sta. 1+66 and Sta. 1+83 the slope is warped from 1V on 4H to 1V on 3H. The crest of the dam is at elevation 880.0, which is approximately the 100-year flood level, and extends from Sta. 0+28 to Sta. 1+66. At both ends of the embankment the gabion protection continues up the abutment at a 1V on 3H slope to elevation 886, which gives 3 feet of freeboard above the PMF.

81. For aesthetic reasons, the concrete-faced gabion baskets will be covered by 11 inches of compacted fill and 4 inches of seeded topsoil. To retard the loss of this soil during overtopping, an erosion resistant clay having a liquid limit greater than 30 and a plasticity index greater than 15 will be used as fill.

82. Four feet downstream of the embankment centerline, the gabion basket blanket will be interrupted by a concrete section extending from Sta. 0+19 to Sta. 1+75. The top of the concrete will be at the crest of the embankment and will act as a fixed overflow weir. To help maintain the top of the concrete weir at a permanent elevation, 8-foot long steel sheetpiling will be driven into the embankment and embedded into the concrete.

83. The main embankment will be constructed of compacted sandy clay fill obtained from the reservoir excavation upstream of the embankment. The existing ground surface will be stripped to a depth of 9 inches except where layers of organic materials exist at the surface, in which case excavation will be required to suitable material. An inspection trench will be excavated along the centerline to a depth of 6 feet.

84. STABILITY

The uncontrolled discharge outlet will allow reservoir pools to stay up for only short periods of time. Stage hydrographs show that reservoir pools above elevation 1070 stay up for periods ranging from 2 hours for the 100-year flood to about 8 hours for the PMF. Since the embankment will be constructed of impervious fill, there is no opportunity for saturation of the embankment or steady state seepage to occur in this short time span. Therefore, the sudden drawdown, partial pool and steady seepage conditions that require a long-term pool to establish a seepage line, as defined in EM 1110-2-1902, need not be analyzed for this embankment. The only applicable stability conditions to be analyzed are the end of construction case and a long-term stability case with no line of seepage through the embankment. A complete set of Q and R or $\bar{R}$ tests were conducted on each soil type to assure satisfactory stability because of the high hazard classification. Six undisturbed samples at the embankment site and 2 sack samples from the upstream borrow area were obtained. A total of 5 Q tests and 5 $\bar{R}$ tests were performed on the undisturbed samples. The sack samples resulted in 2 Q, 1 $\bar{R}$ and 2 R tests on remolded specimens. The following is a summary of strength parameters obtained from testing.

TABLE 8

TABULATION OF STRENGTH PARAMETERS

Soil Type	Q C(psf)	Q Ø	R C(psf)	R Ø	S (from $\bar{R}$ tests) C(psf)	S (from $\bar{R}$ tests) Ø'	γ_m (pcf)	γ_{sat} (pcf)
Embankment Fill	750	10	400	10	0	31	106	130
Foundation Clay	1500	0	400	22	0	30	134	138
Foundation Silt	2400	34	1600	35	0	39	131	131

See Plates C-6 to C-8 for strength plots and Plates C-32 to C-39 for individual test results.

85. Using these parameters, a stability analysis for the end of construction and long-term stability condition was conducted at Sta. 0+90, which was determined to be the most critical section. A minimum factor of safety greater than 8.0 was obtained for the end of construction condition, reflecting the very stiff composition of the foundation materials. The long-term stability condition was analyzed with a full pool at elevation 880 on the upstream side, and a tailwater and phreatic surface through the embankment at elevation 870, resulting in a minimum factor of safety greater than 2.4. Both of these minimum safety factors are much greater than the required values, therefore, stability sections were not drawn up for this embankment. A check of embankment stability due to a long-term pool at elevation 880, establishing steady state seepage at the downstream toe, gives a minimum factor of safety greater than 1.8. While this condition is very improbable and not possible under normal situations, the safety factor is significantly greater than the 1.4 which is required. The following is a tabulation of required and existing factors of safety for each stability case.

TABLE 9

STABILITY AT STATION 0+90

Case	Design Condition	Minimum Required Factor of Safety	Minimum Calculated Factor of Safety
I	End of Construction	1.3	8.3
II-V	Sudden Drawdown, Partial Pool, Steady Seepage from Maximum Pool	1.0-1.5	N/A
VI	Steady Seepage from Surcharge Pool	1.4	1.8
VII	Earthquake		
	- End of Construction	1.0	9.1
	- Partial Pool	1.0	-
	- Steady Seepage from Maximum Pool	1.0	-
-	Maximum Pool with No Phreatic Surface	-	2.4

86. SETTLEMENT

The foundation consists of about 35 feet of stiff to very stiff clayey till and silt underlain by very dense sand. Borings BC-10D, BC-15D, and 81-57M, near the outlet conduit, show mostly silt and very little clay in this area. Two undisturbed silt samples from 81-57M and 1 clay sample from BC-15D were used for consolidation tests in this area. The silt samples proved to be just slightly compressible for the loads applied. Because the conduit will be placed beneath the existing ground surface and because the embankment is only 10 feet in height, there will be very little increase in foundation stress. Therefore, negligible settlement will occur beneath the conduit. At Sta. 0+90, near borings BC-10C and 81-58M, the foundation consists of mostly clay with very little silt to a depth of over 35 feet. Two undisturbed samples from 81-58M and 1 from BC-10C were used for consolidation tests in this area. The tests showed that the clay samples were overconsolidated so that with the relatively small load applied to the foundation by the 10-foot embankment, a maximum total settlement of about 2 inches will occur. This small amount of settlement is acceptable in this portion of the embankment.

87. SEEPAGE CONTROL

The overall impervious nature of the foundation soils, the clay fill embankment with inspection trench and the very short duration of reservoir pool will preclude any seepage problems at this embankment.

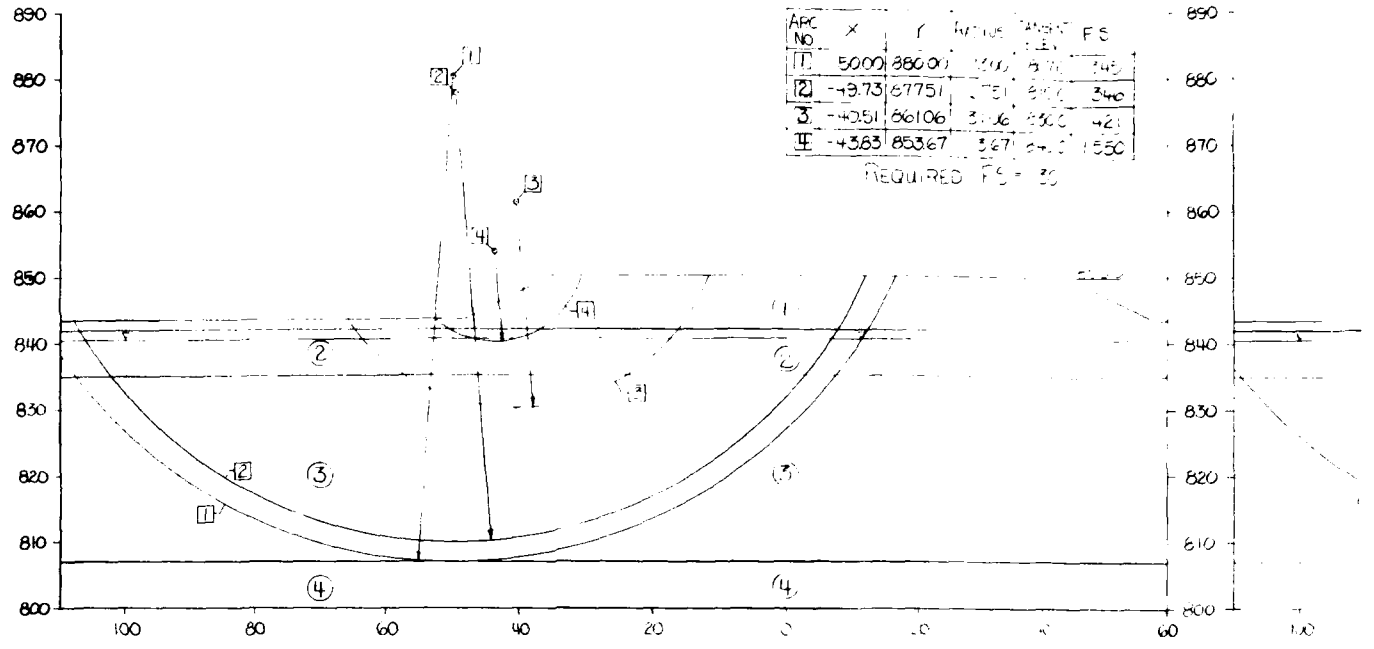
88. BORROW AREAS

Borrow for this embankment and the Golden Valley Country Club embankment will come from the reservoir area upstream of the dam. The reservoir will be excavated as shown on Sheet 39 to achieve adequate flood storage and will have 1V on 3H or flatter slopes for all areas of excavation. Borings show that the excavated material is a very stiff sandy clay till, indicating that the reservoir slopes will be stable for the proposed cut slopes. A new low flow channel will be excavated from the Georgia Avenue culvert to the Edgewood embankment inlet. Riprap will be required for 60 feet downstream of the Georgia Avenue culvert and for 10 feet upstream of the embankment inlet.

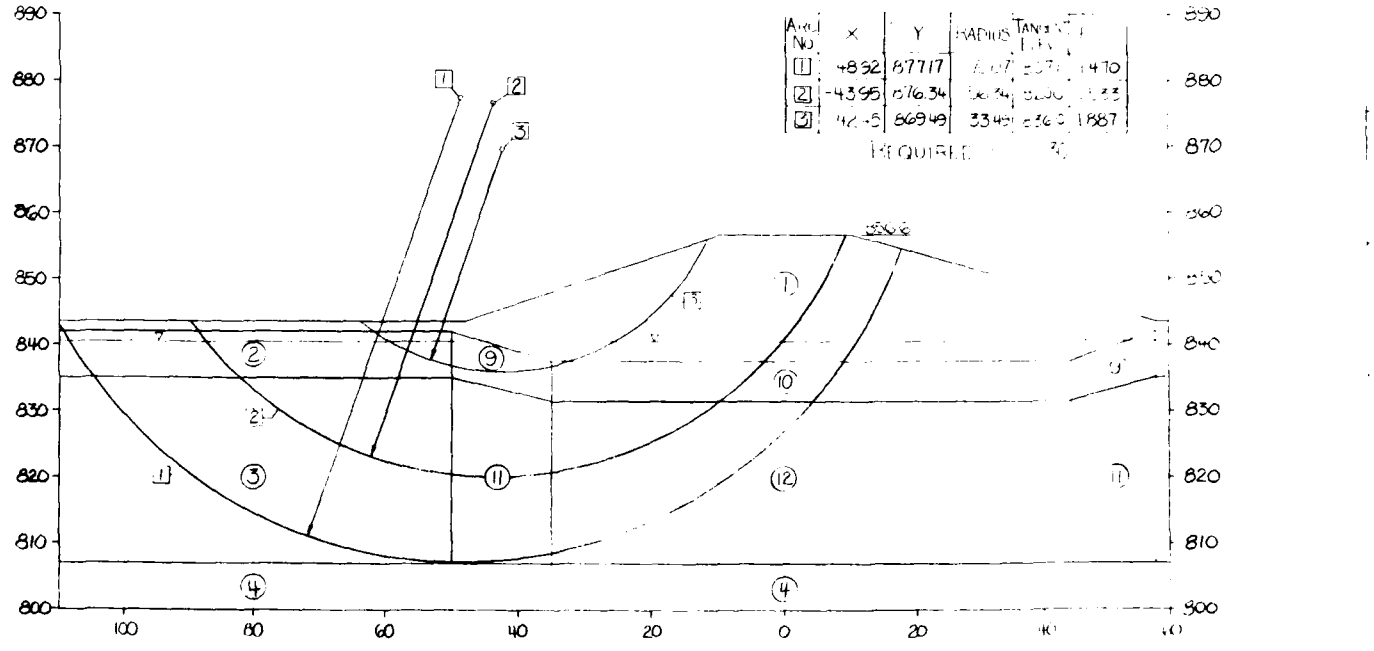
CONSTRUCTION MATERIALS

89. RIPRAP AND CONCRETE AGGREGATE

Riprap and concrete aggregate produced from several sources in the Twin Cities metropolitan area have been used successfully on numerous Corps of Engineers projects. No problems in obtaining adequate supplies of good quality material will be encountered.



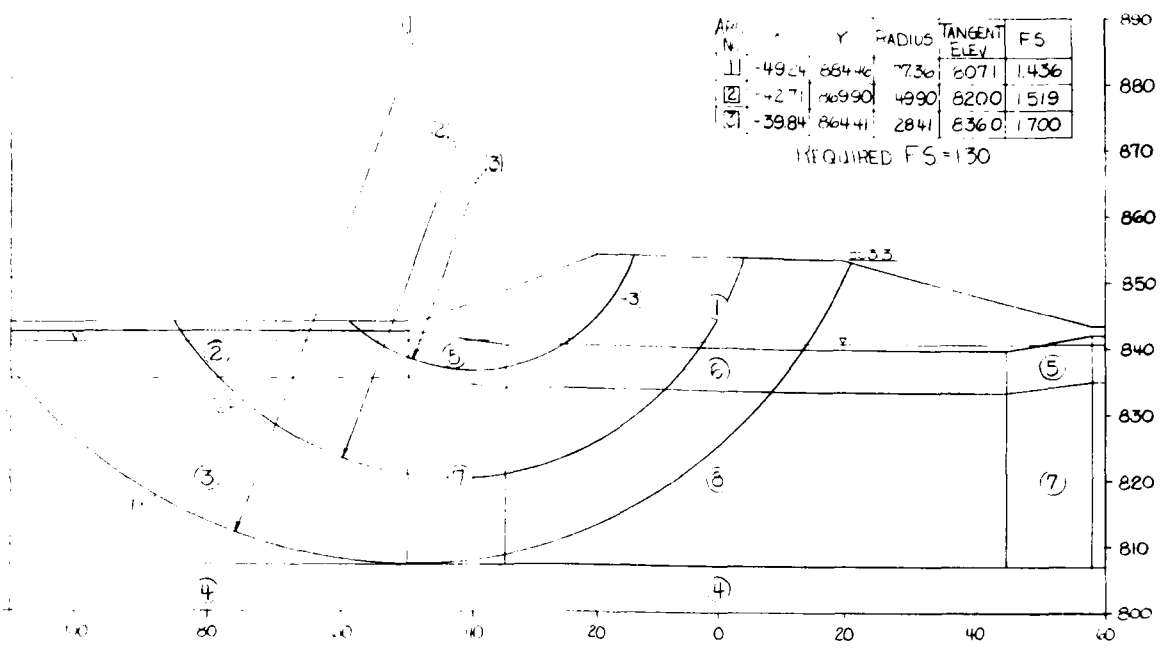
STAGE I - END OF CONSTRUCTION
STA 8+25



STAGE III - END OF CONSTRUCTION
STA 8+25

ARC N.	X	Y	RADIUS	TANGENT ELEV	FS
1	-49.24	884.46	77.36	807.1	1.436
2	-42.71	869.90	49.90	820.0	1.519
3	-39.84	864.41	28.41	836.0	1.700

REQUIRED FS = 1.30



STAGE II END OF CONSTRUCTION
STA 9+25

	NO	MATERIAL TYPE	γ_m PCF	γ_{sat} (PCF)	C PSF	ϕ DEGS
STAGE I	1	PERVIOUS EMBANKMENT FILL	130	130	0	30
	2	UNCONSOLIDATED PEAT	650	670	200	0
	3	UNCONSOLIDATED MUCK (OR PEAT)	750	750	200	0
	4	SILT (ML-OL)	1240	1240	0	22.5
STAGE II	5	PEAT (CONSOLIDATED UNDER STAGE I EMBANKMENT)	650	670	650	0
	6	PEAT (CONSOLIDATED UNDER STAGE II EMBANKMENT)	650	670	400	0
	7	MUCK (CONSOLIDATED UNDER STAGE II EMBANKMENT)	750	750	200	0
	8	MUCK (CONSOLIDATED UNDER STAGE II EMBANKMENT)	750	750	450	0
	9	PEAT (CONSOLIDATED UNDER STAGE II EMBANKMENT)	650	670	430	0
STAGE III	10	PEAT (CONSOLIDATED UNDER STAGE II EMBANKMENT)	650	670	800	0
	11	MUCK (CONSOLIDATED UNDER STAGE II EMBANKMENT)	810	810	480	0
	12	UNCONSOLIDATED PEAT	810	810	650	0

NOTE: (2) STRENGTHS WERE INCREASED WITH NORMAL STRESS
MAY INCREASED WITH DEPTH ENVELOPE
(4) USED FOR SOIL STRENGTH (SHEET C-4)

SYMBOL	DESCRIPTION	DATE	APPROVAL

DESIGNED BY: D.R.
DRAWN BY: D.R.
CHECKED BY: M.B.
SUBMITTED BY:
APPROVED:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

HIGHWAY 100

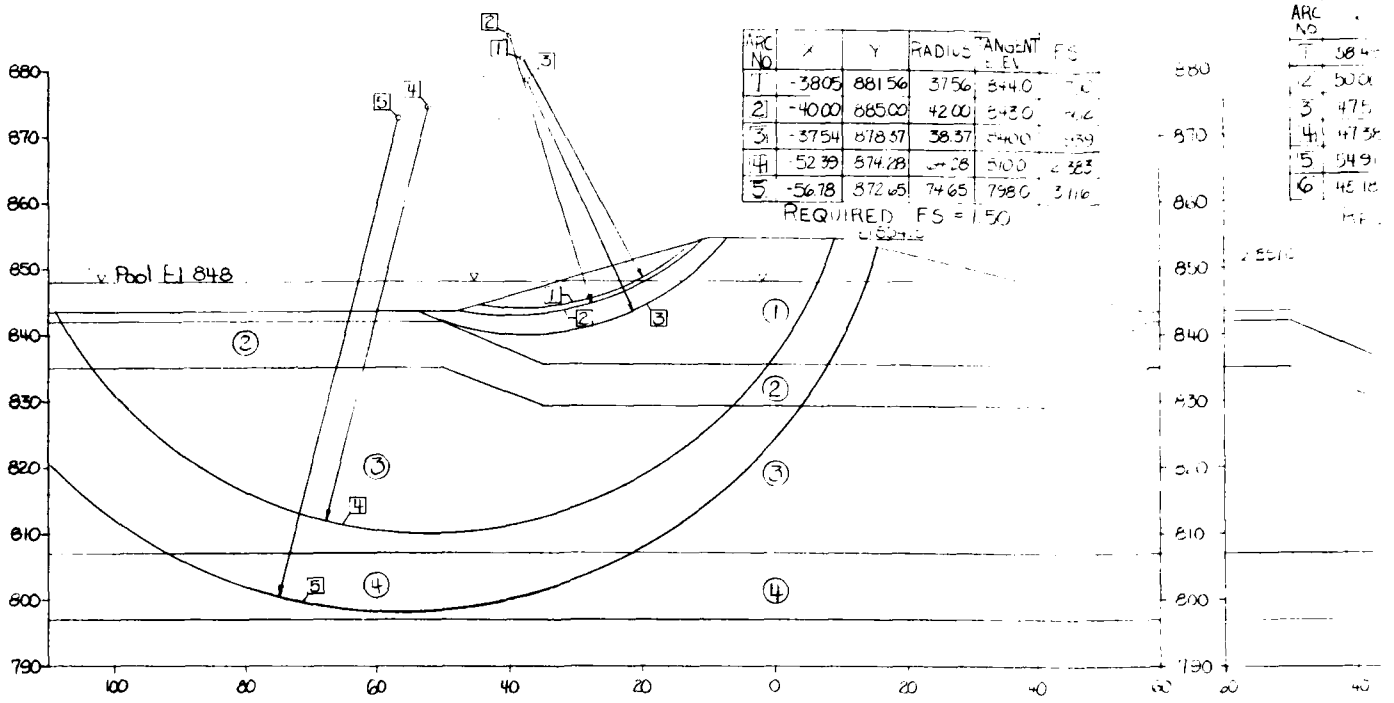
STABILITY ANALYSIS
END OF CONSTRUCTION CONDITIONS

DATE: AUGUST 1982

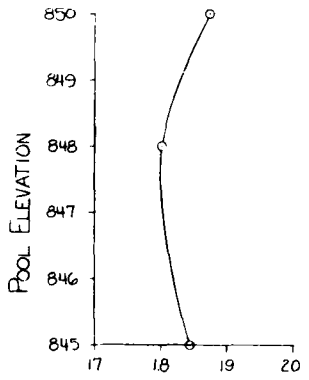
SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/229
SHEET C-1 OF C-39

SAFETY PAYS

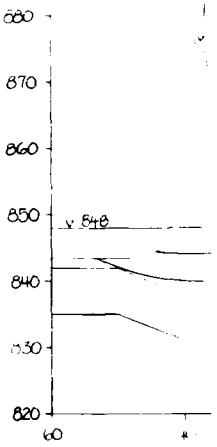
2



PARTIAL POOL AT END OF PRIMARY CONSOLIDATION
STA 8+25



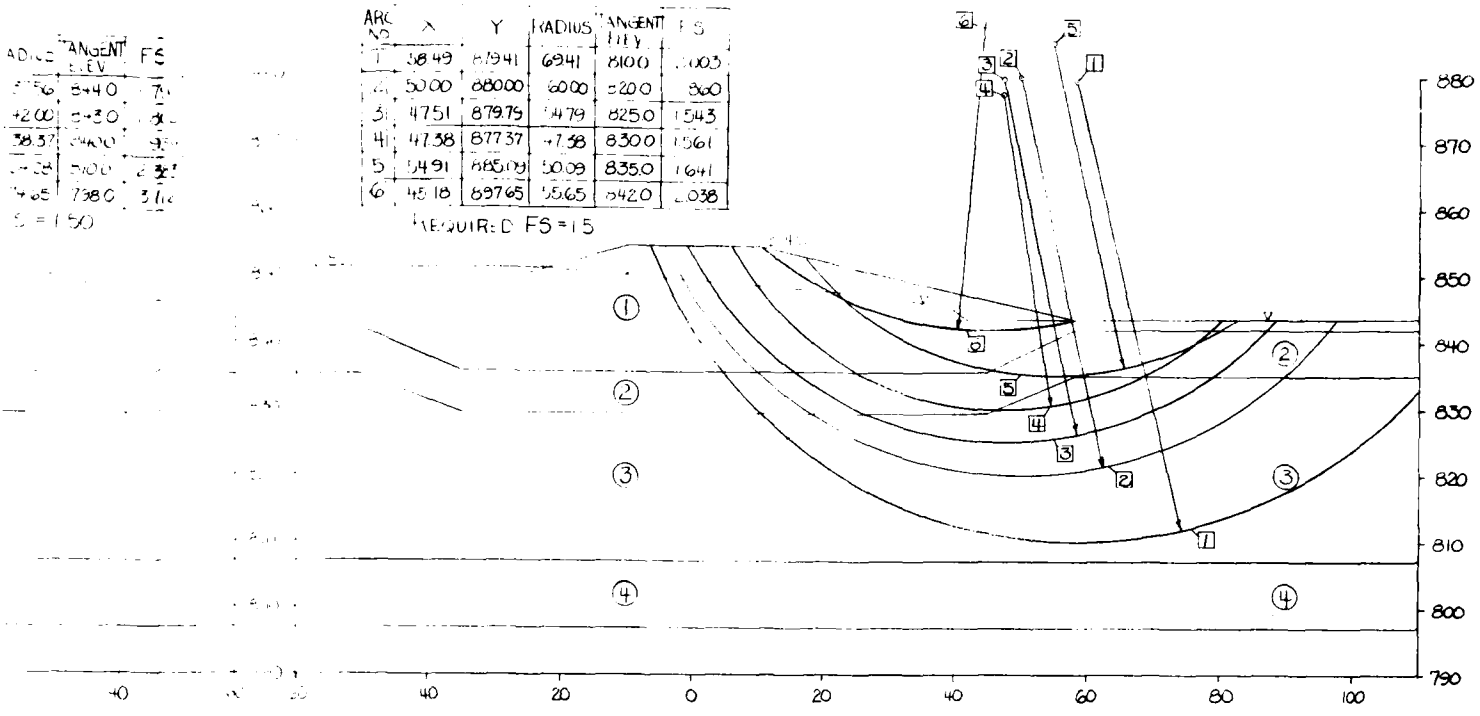
PARTIAL POOL ANALYSIS



NO	MATERIAL TYPE	U STRENGTH C (psf)	U STRENGTH Φ (DEG)	R STRENGTH C (psf)	R STRENGTH Φ (DEG)	(R+U)/2 C (psf)	(R+U)/2 Φ (DEG)	γ_m (pcf)	γ_{sat} pcf
①	EMBANKMENT FILL	0	30	0	30	0	30	130	135
②	PEAT	0	43.5	150	20.5	75	32	66	68
③	MUCK (OH-PT)	0	43.5	150	20.5	75	32	81	81
④	SILT (ML-OL)	0	40.5	200	38	100	39.3	124	124

ARC NO	X	Y	RADIUS	TANGENT ELEV	F.S.
1	38.49	879.41	69.41	810.0	1.003
2	50.00	880.00	60.00	820.0	1.360
3	47.51	879.79	54.79	825.0	1.543
4	47.38	877.37	47.38	830.0	1.561
5	54.91	885.09	50.09	835.0	1.641
6	45.18	897.65	55.65	842.0	2.038

REQUIRED FS = 1.5

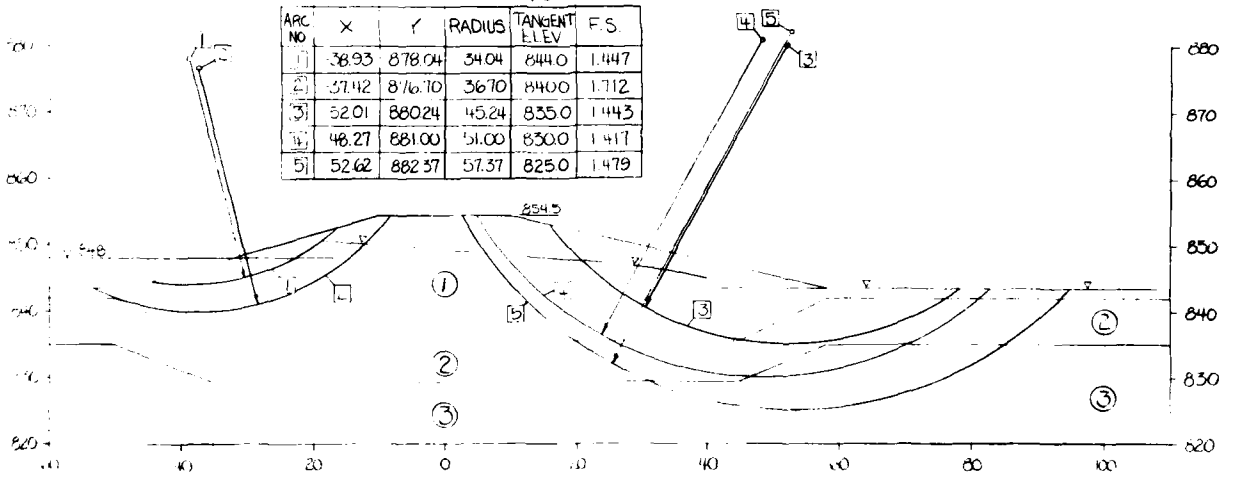


STEADY SEEPAGE FROM 851.0 AT END OF PRIMARY CONSOLIDATION

STA 8+25

REQUIRED F.S = 1.0

ARC NO	X	Y	RADIUS	TANGENT ELEV	F.S.
1	38.93	878.04	34.04	844.0	1.447
2	37.42	876.70	36.70	840.0	1.712
3	52.01	880.24	45.24	835.0	1.443
4	48.27	881.00	51.00	830.0	1.417
5	52.62	882.37	57.37	825.0	1.479



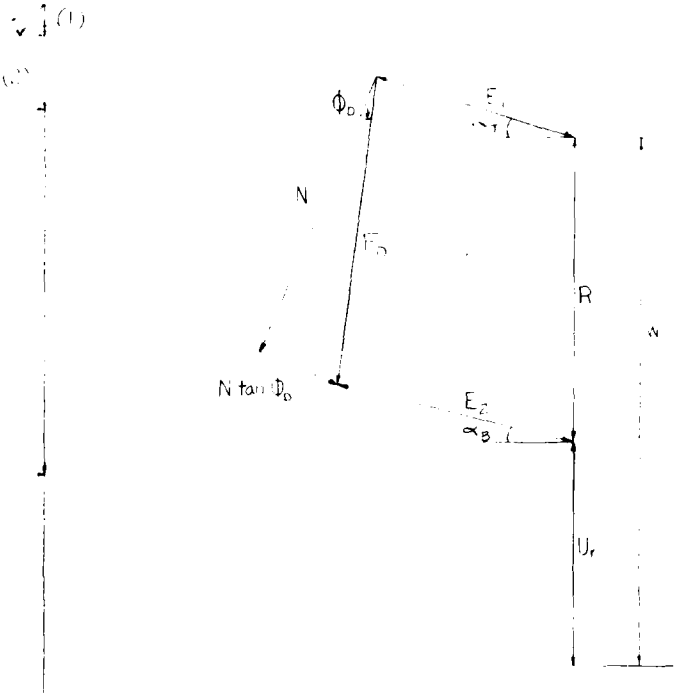
STEADY SEEPAGE & PARTIAL POOL WITH EARTHQUAKE

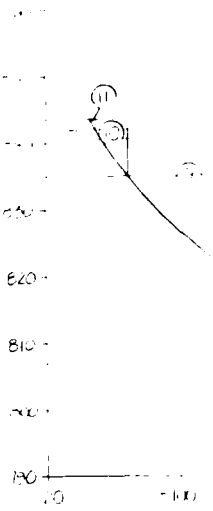
STA 8+25

DRAWN BY: D.W.R	PHASE II	DESIGN MEMORANDUM
CHECKED BY: D.W.R	FLOOD CONTROL	BASSETT CREEK MINNESOTA
ENGINEER BY: M.B	HIGHWAY 100	
SUBMITTED BY:	STABILITY ANALYSIS, PARTIAL POOL, STEADY SEEPAGE & EARTHQUAKE CONDITIONS	
APPROVED:	DATE: AUGUST 1982	
	SCALE: AS SHOWN	DRAWING NUMBER: M34.3-R-5/230
	SHEET C-2 OF C-39	

SAFETY PAYS

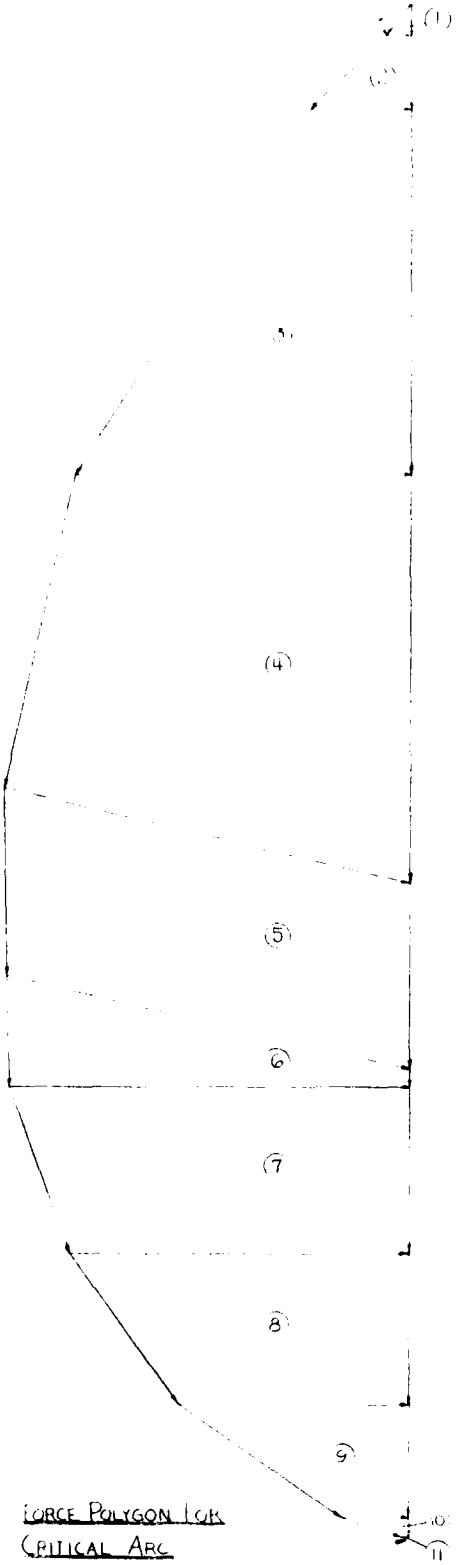
2





DEFINITION OF TERMS

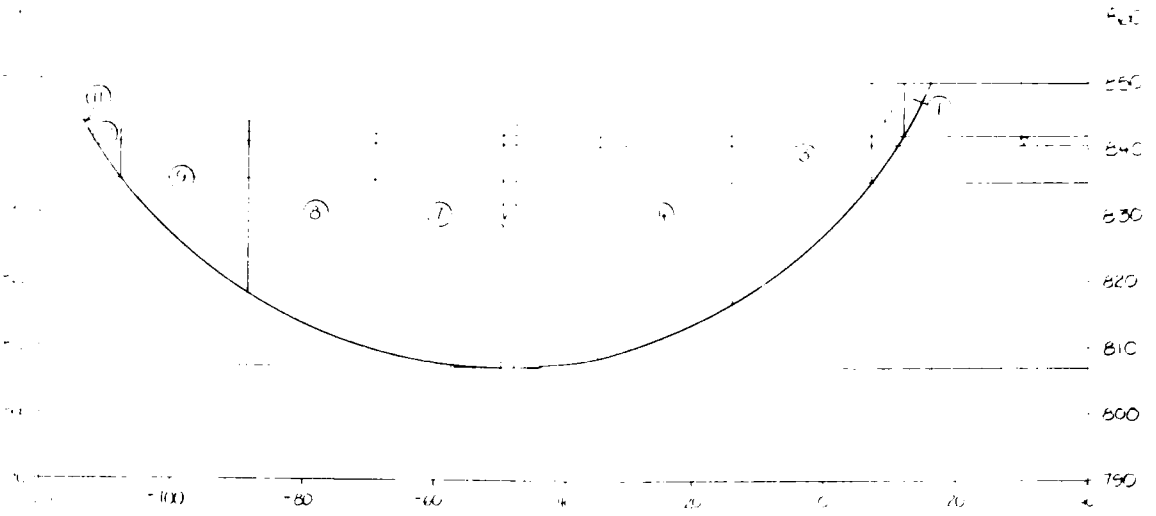
- n = SLICE NUMBER
- W = TOTAL WEIGHT OF SLICE
- U_r = WATER FORCE
- R = RESULTANT OF TOTAL WT + WATER
- Φ_d = DEVELOPED Φ PARAMETER
- E_1 = RESULTING SIDE FORCE BETWEEN SLICE n & n-1
- E_2 = RESULTING SIDE FORCE BETWEEN SLICE n & n+1
- α_T & α_B = DIRECTION OF RESULTANT SIDE FORCE
- N = NORMAL FORCE ON BOTTOM OF SLICE
- F_D = RESULTANT OF NORMAL & FRICTIONAL COMPONENTS



FORCE POLYGON FOR CRITICAL ARC

SCALE 1" = 5 Kips

CLOSURE = 0.00 Kips



CRITICAL ARC STAGE 1 END OF CONSTRUCTION

TABULATION OF SLICE DATA

SLICE NO	X COORD	SLICE WIDTH	TOTAL WT (W)	WATER FORCE (W)	DIRECTION OF U	FORCE (U)	EFFECTIVE FORCE	NORMAL FORCE (N)	NORMAL FORCE (N)	ALPHA TOP	ALPHA BOT	F1	F2
1	14.44	4.22	.76	0.00	0.00	0.00	4.23	2198	0.31	.78	0.0	0.0	0.00	89
2	9.91	4.85	6.24	0.67	90.00	1.26	0.0	5515	1.93	[illegible]	0.0	0.0	[illegible]	38
3	-3.14	21.24	49.47	21.83	90.00	4.12	0.00	39.94	1.18	36.55	0.0	0.0	.28	25.44
4	-24.38	21.24	69.20	38.25	90.00	3.37	0.0	26.55	1.06	1.401	0.0	0.0	25.44	31.17
5	-41.50	13.00	40.90	26.77	90.00	1.95	0.0	[illegible]	1.07	14.07	[illegible]	[illegible]	[illegible]	31.26
6	-49.00	2.00	3.51	4.17	90.00	1.30	0.00	.79	4.26	8.52	[illegible]	0.0	31.26	40.25
7	-59.58	9.16	51.99	39.30	90.00	1.87	0.0	[illegible]	68	3.19	0.0	0.0	40.25	45.67
8	-78.74	19.16	44.43	33.00	90.00	3.10	0.00	[illegible]	0.00	3.76	[illegible]	0.0	45.67	[illegible]
9	-91.90	19.16	27.16	18.63	90.00	[illegible]	0.00	41.01	0.57	14.18	[illegible]	0.0	[illegible]	[illegible]
10	-109.91	4.85	2.14	2.67	90.00	1.26	0.00	[illegible]	.52	[illegible]	0.0	0.0	[illegible]	.67
11	-112.17	0.89	0.09	0.00	0.00	0.00	[illegible]	[illegible]	.10	.62	0.0	0.0	.67	0.0

ALL ANGLES MEASURED FROM POSITIVE X AXIS FORCES ARE IN KIPS

SYMBOL	DESCRIPTION	DATE	APPROVAL

DESIGNED BY: D W R
DRAWN BY: D W R
CHECKED BY: M B
SUBMITTED BY:
APPROVED:

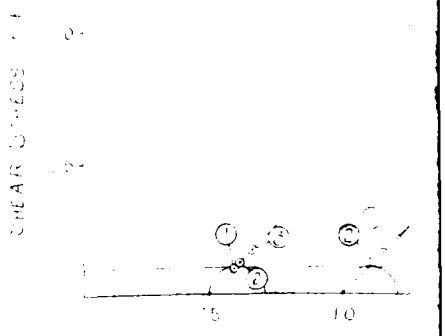
PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

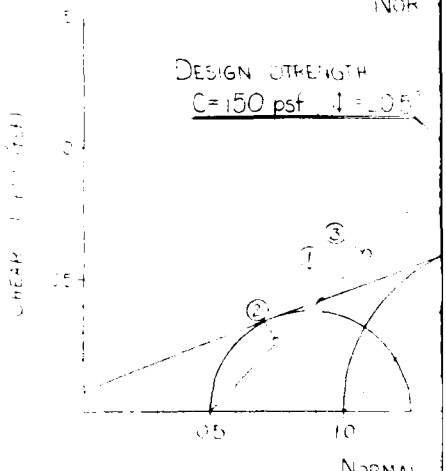
HIGHWAY 100
STABILITY ANALYSIS - TYPICAL CALCULATION OF CRITICAL FAILURE ARC

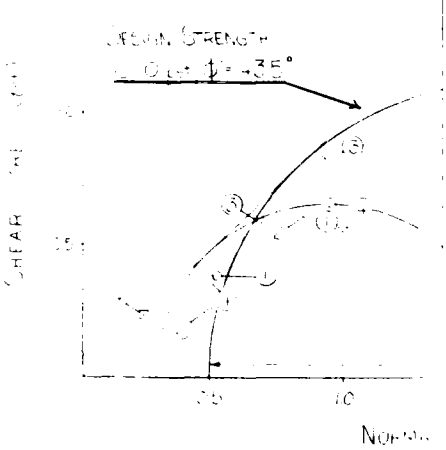
DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34 3-R-5/231
SHEET C-3 OF C-39

SAFETY PAYS

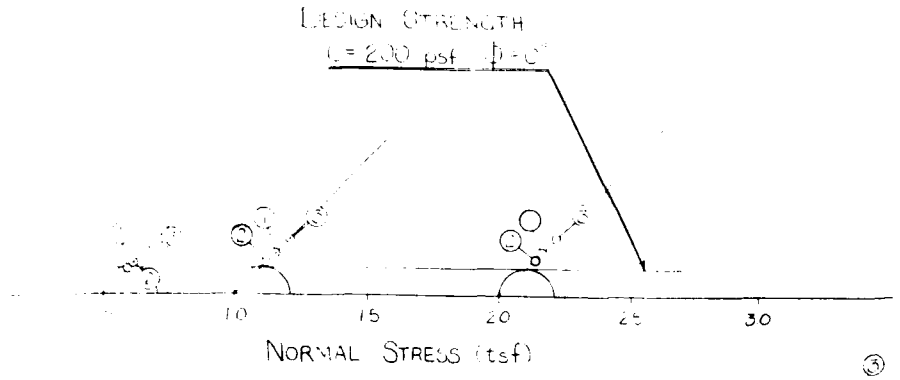
TEST No	BORING No	DEPTH (FT)	DRY UNIT WT (pcf)	MOIST UNIT WT	SAT UNIT WT (pcf)	MOISTURE CONTENT	SPECIFIC GRAVITY	VOID RATIO	LIQUID LIMIT	PLASTIC LIMIT
①	BC-4B	17-19	27.2	77.4	78.0	18.7	2.35	.39	.65	21
②	BC-4Z	75-185	49.2	92.0	92.7	87.0	2.59	2.28	.00	44
③	BC-4J	95-11	23.1	73.1	73.6	218.4	.94	4.24	2.66	178
④	BC-4C	2-4	12.3	69.3	66.6	431.2	1.52	6.71	—	—



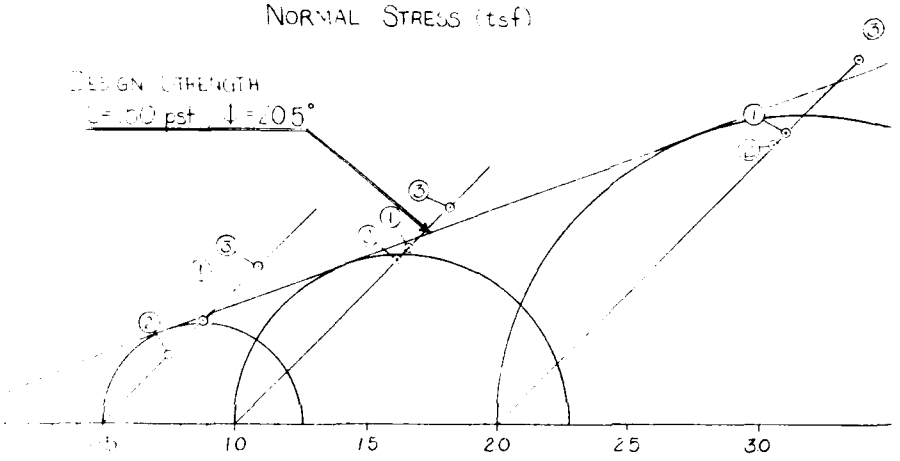




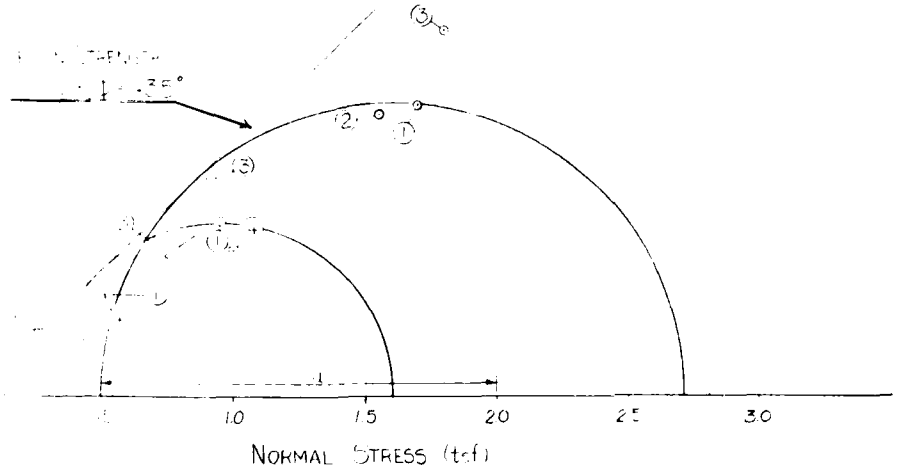
NOTE ALL STRENGTH VALUES FR



Q - STRENGTHS



R - STRENGTHS



S - STRENGTHS
(FROM R TESTS*)

* TEST NO 4 FROM DIRECT SHEAR

ALL STRENGTH VALUES PLOTTED AT 2% STRAIN

SAFETY PAYS

SYMBOL	DESCRIPTION	DATE	APPROVAL

DWR
DWR
MB
SUBMITTED BY:
APPROVED:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

HIGHWAY 100

PEAT - MUCK
UNDISTURBED SHEAR STRENGTH PARAMETERS

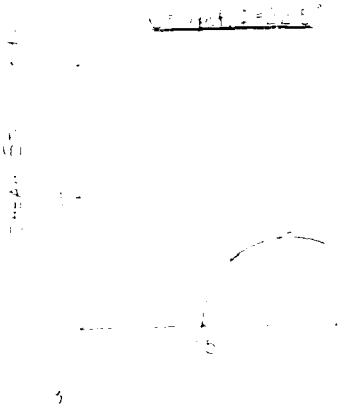
DATE
AUGUST 1982

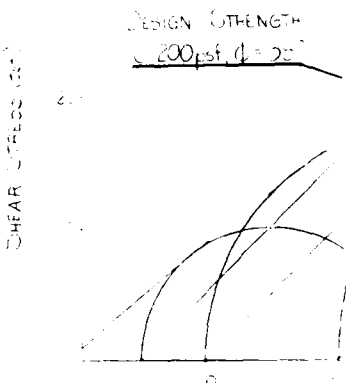
SCALE
AS SHOWN

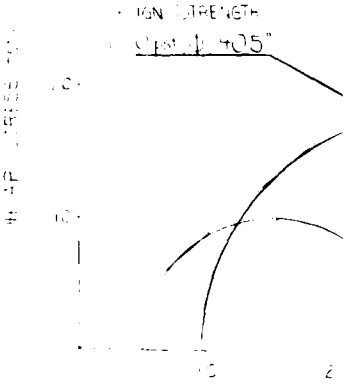
DRAWING NUMBER
M34.3-R-5/232
SHEET C-4 OF C-39

2

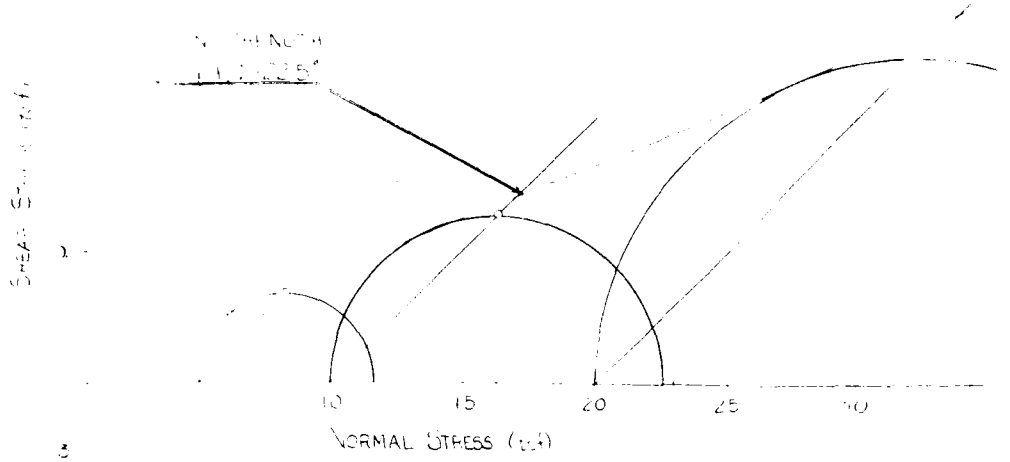
TEST NO	BORING NO	DEPTH (FT.)	DRY UNIT WT (PCF)	MOIST UNIT WT	SAT UNIT WT (PCF)	MOISTURE CONTENT	SPECIFIC GRAVITY	VOID RATIO	LIQUID LIMIT	PLASTIC LIMIT
1	BC-4Z	27-29	98.3	123.0	124.1	25.1	2.67	0.69	24	22





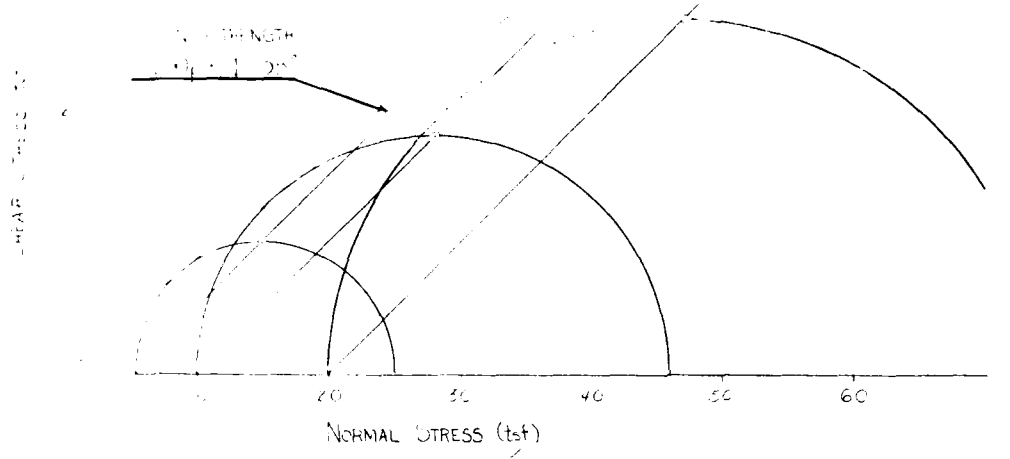


NOTE ALL STRENGTH VAL

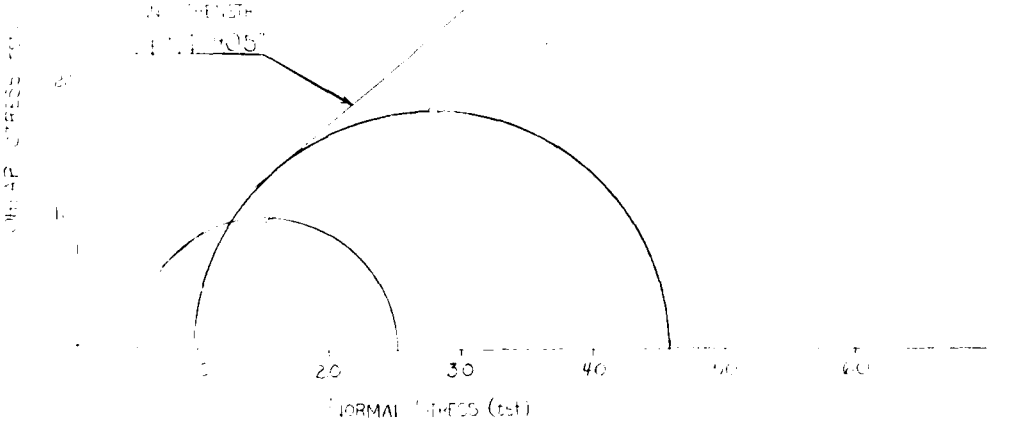


Q STRENGTH*

* ANALYSIS NOT INCORPORATED



R STRENGTH



S STRENGTH
(FROM R TEST)

TOTAL STRENGTH VALUES PLOTTED AT 0% STRAIN

SYMBOL	DESCRIPTION	DATE	APPROVAL

DESIGNED BY: DWR
DRAWN BY: DWR
CHECKED BY: MB
SUBMITTED BY:
APPROVED:

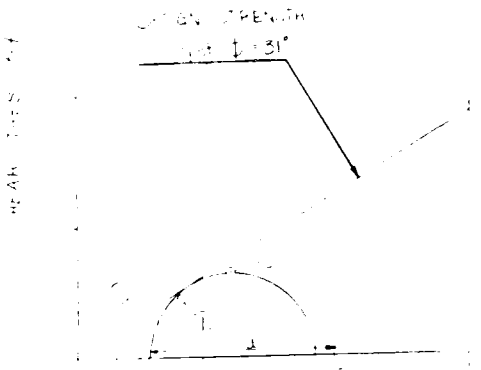
PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
HIGHWAY 100
SILT
UNDISTURBED SHEAR STRENGTH PARAMETERS
DATE: AUGUST 1962
SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/233
SHEET C-5 OF C-39

SAFETY PAYS

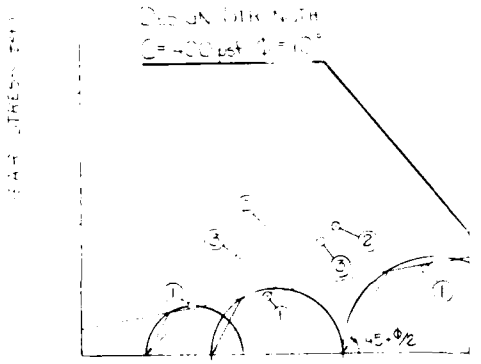
2

TEST No	BORING No	DEPTH	LIQUID LIMIT	PLASTIC LIMIT	OPTIMUM M.C.	SAMPLE M.C.	Max γ_d (pcf)	SAMPLE γ_d (pcf)
①	B1-60M	0-20'	33	14	15.4	15.6	111.1	100.3
②	BC-14C	5.5-77'	---	—	15.4	18.4	113.6	107.9
③	BC-14C	5.5-77'	31	15	15.4	15.4	113.6	107.9
④*	BC-10A	7-95'	—	—	16.1	16.1	113.5	107.8

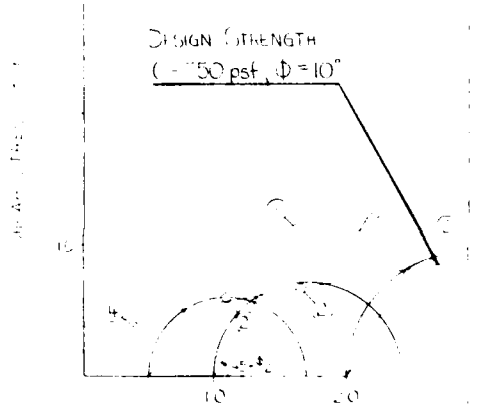
*UNCONFINED COMPRESSION TEST



NORMAL STRESS (psf)

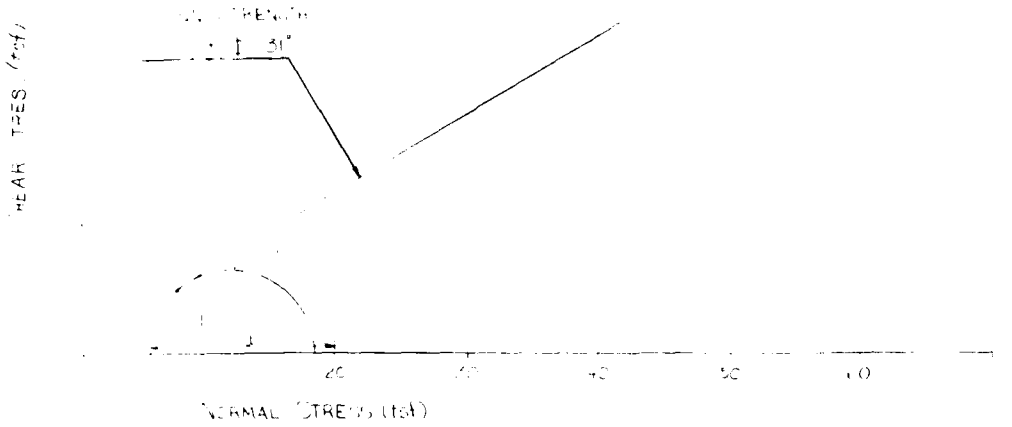


NORMAL STRESS (psf)

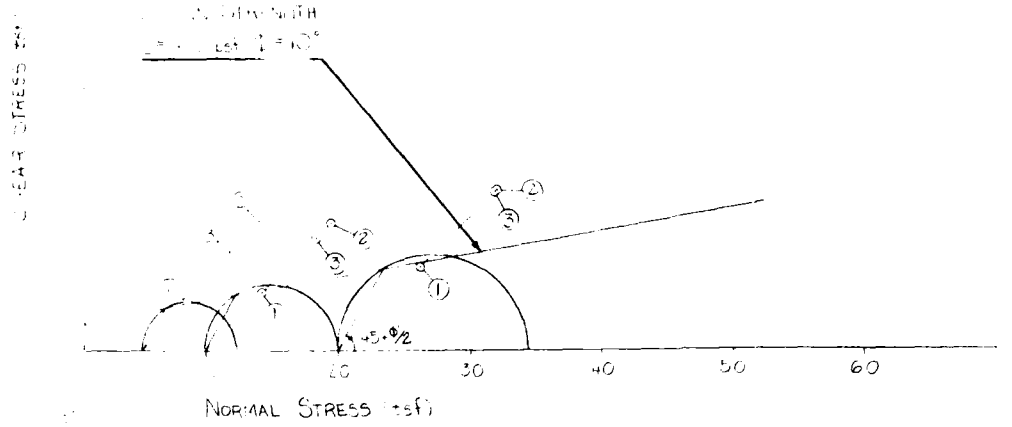


NORMAL STRESS (psf)

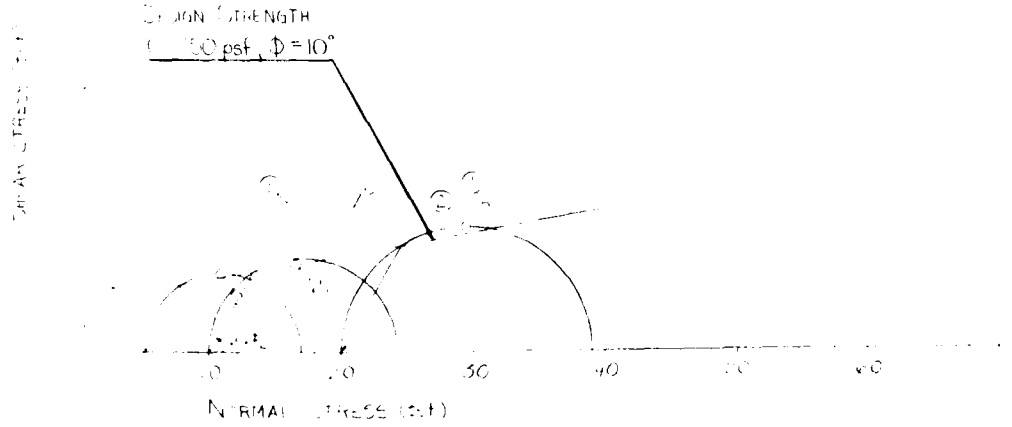
NOTE: ALL STRENGTH VALUES FROM



S STRENGTH
(FROM $\bar{R}$ TEST)



R STRENGTHS



Q STRENGTHS

STRENGTH VALUES PLOTTED AT 15% STRAIN

SAFETY PAYS

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

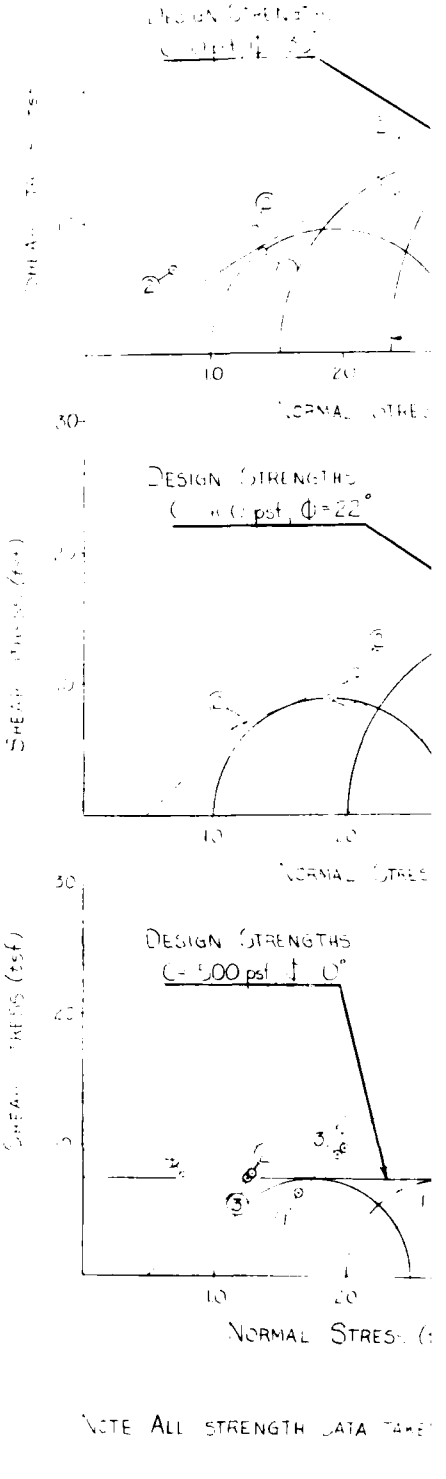
DESIGNED BY: D.W.R
DRAWN BY: J.M.J.
CHECKED BY: M.B
SUBMITTED BY:
APPROVED:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
EDGEWOOD EMBANKMENT
BORROW AREA - CLAY TILL
REMOLDED SHEAR STRENGTH PARAMETERS

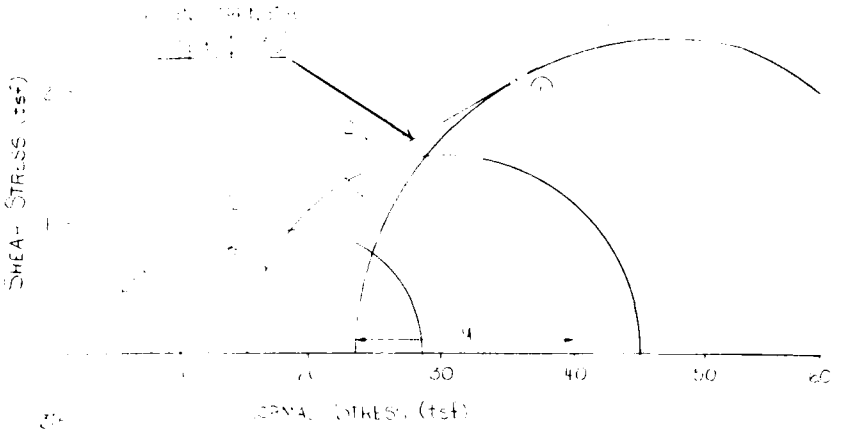
DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/234
SHEET C-6 OF C-39

TEST NO	BORING NO	DEPTH (FT)	DRY UNIT WT (PCF)	MOIST UNIT WT	SAT UNIT WT (PCF)	MOISTURE CONTENT	SPECIFIC GRAVITY	VOID RATIO	LIQUID LIMIT	PLASTIC LIMIT
①	B1-58M	11-13	112.1	131.8	136.3	17.6	2.67	0.49	32	12
②	BC-15D	9.3-11.4	116.1	134.7		16.0	—	—	26	14
③	BC-10B	4.5-7	114.0	134.5		18.0	—	—	30	15
④ *	BC-10C	6.5-7	113.5	132.9	—	17.1	—	—	25	15

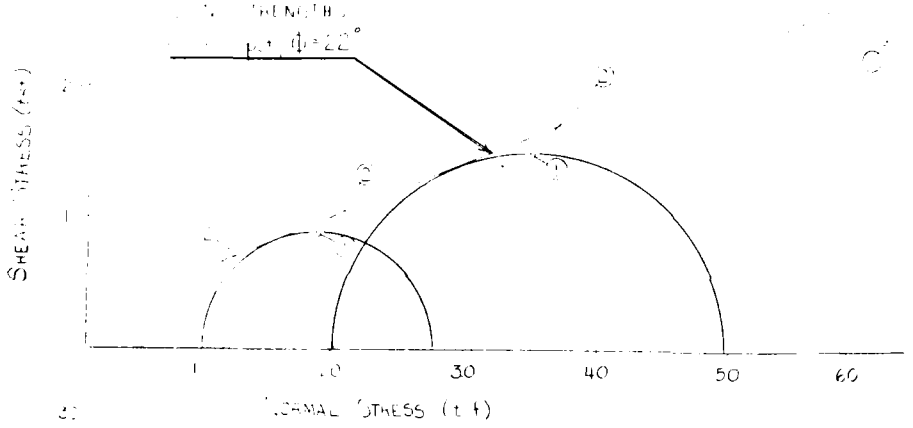
* UNCONFINED COMPRESSION TEST



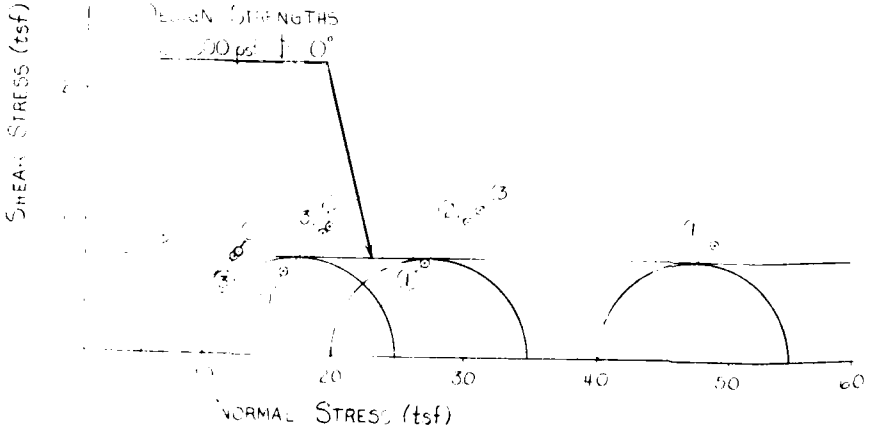
NOTE ALL STRENGTH DATA TAKE



Q STRENGTHS
(FROM $\bar{R}$ TESTS)



R STRENGTHS



Q STRENGTHS

NOTE ALL STRENGTH DATA TAKEN AT 15% STRAIN

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY: D.W.R.
DRAWN BY: J.M.J.
TRACED BY: M.B.
SUBMITTED BY:
APPROVED:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
EDGEWOOD EMBANKMENT
FOUNDATION CLAY
UNDISTURBED SHEAR STRENGTH PARAMETERS
DATE: AUGUST 1982

SCALE: AS SHOWN
DRAWING NUMBER
M34.3-R-5/235
SHEET C-7 OFC-39

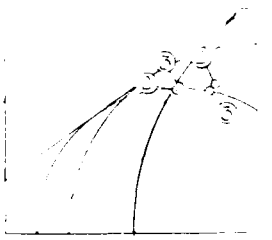
SAFETY PAYS

2

TEST NO.	BORING NO.	DEPTH (FT.)	DRY UNIT WT (PCF)	MOIST. UNIT WT	SAT UNIT WT (PCF)	MOISTURE CONTENT	SPECIFIC GRAVITY	VOID RATIO	[illegible] UNIT	PLASTIC [illegible]
11	B1-58M	1.125	108.9	131.3	131.3	20.6	2.69	0.54		
12	B6-15D	[illegible]								
13	B6-15D	[illegible]								

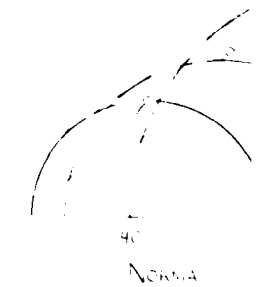


NORMAL



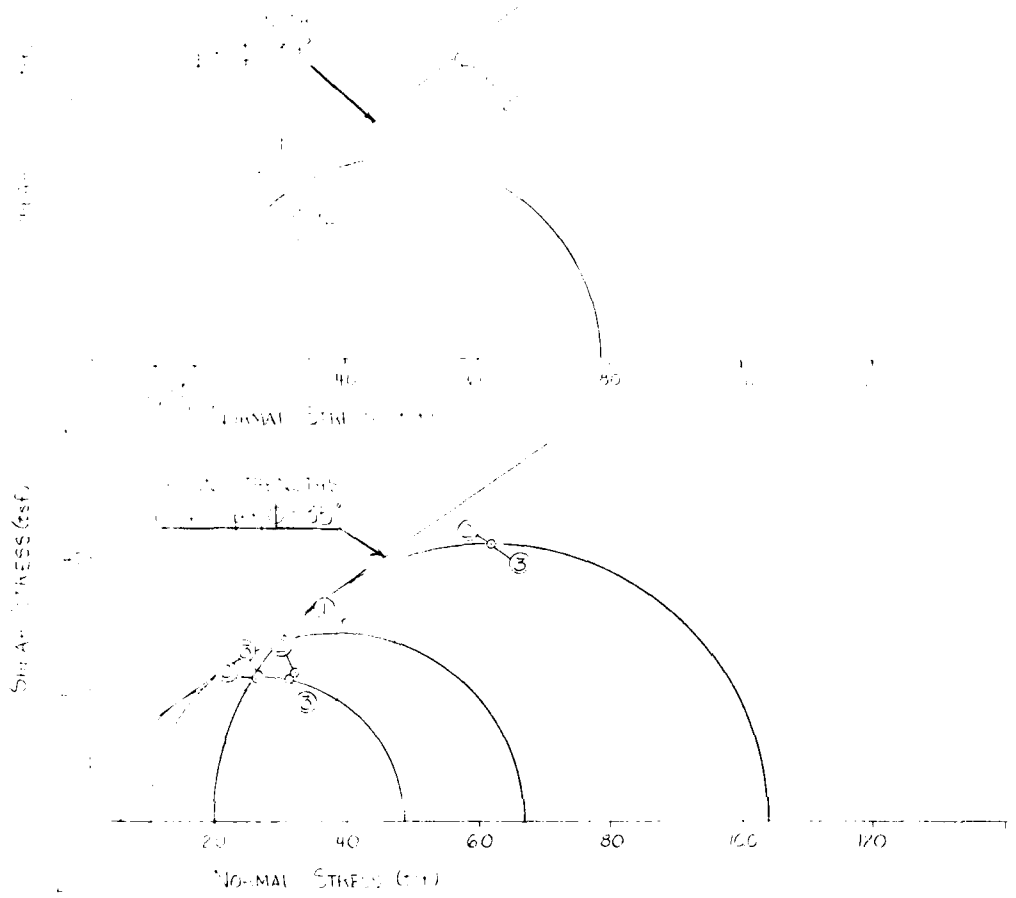
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NORMAL



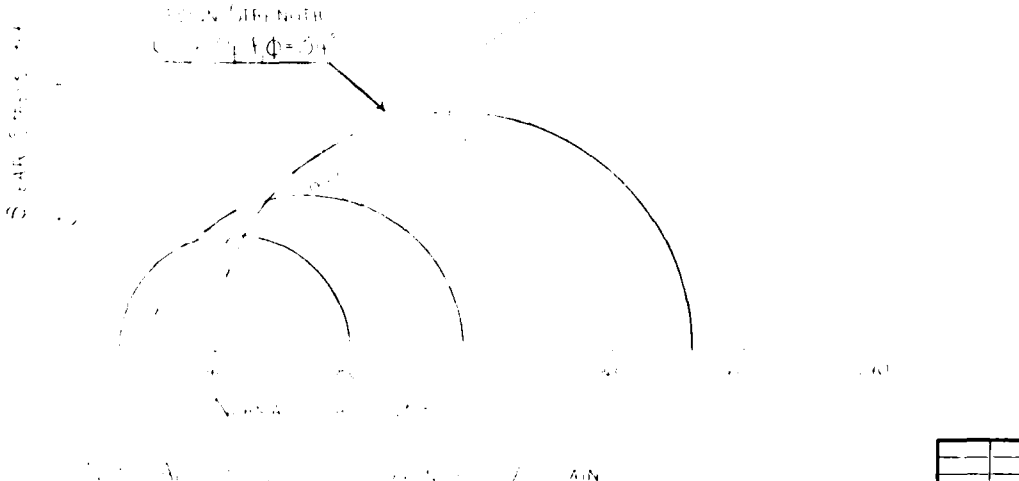
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NORMA



U STRENGTHS
(From UU Tests)

R STRENGTHS



Q STRENGTHS

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY: DWR
DRAWN BY: JMJ
CHECKED BY: MB
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
EDGEWOOD EMBANKMENT
FOUNDATION SILT
UNDISTURBED SHEAR STRENGTH PARAMETERS

DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34 3-R-5/236
SHEET C-8 OF C-39

SAFETY PAYS

79-14M
30 JULY 1979

SPT Blows/Ft	MC	LL	PL	% FINES	
	811.0				
10					Brn, f-c, cinders
	57.8				Sandy, organic, wood, ashes, brn
3					Org, grn-gray
					Blk, oily odor, glass frag
3					Clayey, blk
					Clayey, sandy, blk
2					Org, plant debris, gray-grn
					Silty, some shells
1½	57.9	88	23		Silty, scat silt lams
3		113	32		
	783.0				

81-55M
10 JULY 1981

SPT Blows/Ft		
	811.1	
		Cinders
6		Brn, loose, much organics, sandy
		Blk, cinder fill w/bricks
	W.L 803.1	
2		Red-brn, peaty, sandy, loose
2		Dk gray, loose, clayey
2		
1		Gray, silty, v soft, wet
1½		
23		Blk, m dense, sat
		Gray, m dense, sat
12		
		Gray, loose, gravelly
14		
15		Gray, m dense, sat
17		Gray, m dense, sat
18		
16		Till, hard, dk brn & dk grn layers, sandy, moist
17		Till, dk gray, v stiff, moist, sandy
28	BOH 736.1	Till, dk brn, v stiff to hard, sandy

80-16M
24 JANUARY 19

SPT Blows/Ft	MC	
	817.2	
6	8.4	
	84.3	
4	89.4	W.L 805.2
	120.4	
3		
	36.9	
5		
	26.6	
10		
	12.5	
22		
26		
33		
25		
31	765.5	

T-506

SPT Blows/Ft		
	826.5	
5		Clay, brn, dense, hard
32		
20		
28		Fine sand, brn, dry
25		
25		
15		Silty clay w/some silty loam seams, moist, gray
10		
12		Clay, fat, gray, moist, some silty loam seams
9		
8		
9		Sandy, loam, brn, gray, moist
16		
14		
9		Clay w/some seams of silty loam, gray, moist, firm
7		
17		
14		
15		
19		Clay loam till, gray, moist, firm
25		
15		
75/2		
28		Lean clay, till, gray, moist, firm
25		
22		
25		
28		Sandy loam, gray, moist, firm
26		
20		
35		Clay loam, gray, moist, firm
27		
26		
26		
24		Loamy sand, brn, wet
24	685.0	Top of Sandstone
	681.5	St Peter Sandstone

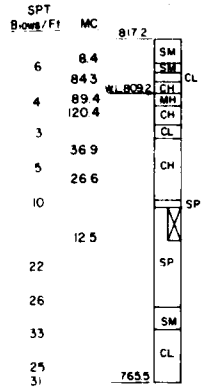
T-506

SPT Blows/Ft		
	822.5	
		Blacktop & fill
8		
9		Loam, blk, moist
		Sandy loam, plastic, brn, moist
25		
50		
44		Fine-med sand, brn, moist
27	W.L 804.5	
21		
24		
22		
18		
8		Silty clay, gray, moist
11		
		Clayey loam, sl pl sandy loam, gray, brn, moist
11		
17		Clay w/some silty loam patches, gray, moist
9		
8		Clay-silty clay & silty loam interbedded
10		
8		Silty clay, gray, moist
12		
11		
60		
26		Fine sand
75		
46		
65		Loamy sand & gravel
60		
65		
60		
23		Clay, gray, moist
24		
20		
22		Clay loam, gray
22		
26		Clay w/small amounts of fine sand
W/H		
75/.8	686.0	Top of Sandstone
75/.5	682.0	St Peter Sandstone
75/.2		

80-1
28 JANUA

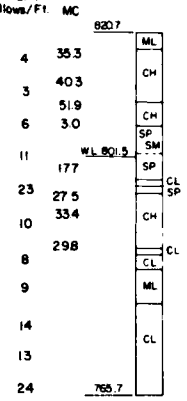
SPT Blows/Ft	MC	
	824.1	
6	7.9	
	18.8	
9		
12	5.0	
	16.8	
10		W.L 804.6
11	17.8	
18	28.8	
45	22.4	
43		
23		
16		
22	769.4	

80-16M
24 JANUARY 1980



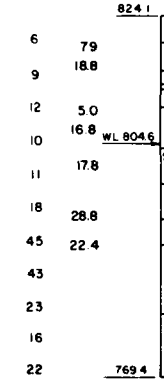
SPT Blows/Ft	MC	LL	PL	% FINES	
	8.4				Dk brn, loose, gravelly, dry
6	84.3	117	42		Lt brn, loose, dry Gray, plast, stiff
4	89.4	95	45		Blk, organic, clayey, low plast, m stiff, moist
	120.4	176	15		White, clayey, soft, non-plast, moist, shell & plants
3					Lt gray, silty, low plast, soft, moist, shells
	36.9	84	24		Gray, silty, low plast, v soft, wet
5	26.6	116	24		Gray, silty, plast, med stiff, moist
10				4	Brn, loose, 5% Gravel Sand heaved
	12.5			2	1% gravel
22					Gray, f - m sand, scat gravel, wet
26				3	2% gravel
33					Dense, wet, f sand
25					Brn, stiff, plast, moist, till
31					

80-17M
24 JANUARY 1980



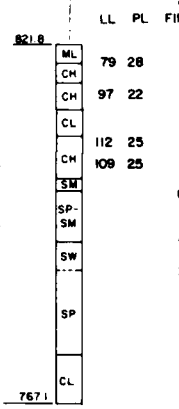
SPT Blows/Ft.	MC	LL	PL	% FINES	
					Dk. brn, dry, fill & topsoil
4	35.3				Gry, silty, m stiff, plast, moist
3	40.3	95	23		Lam silt layers
	51.9	98	25		Dk gry, m stiff, plast, v moist
6	3.0			5	2% gravel
					Gray, m dense, dry
11	17.7			2	1% gravel, wet
23	27.5			3	
10	33.4	54	18		
		67	19		Dk gray, stiff, plast, moist, gravelly, till
8	29.8	28	19		
9					Gray, non - plast, wet, soft. M stiff, plast, moist, lam, silty, gry
					Gry - brn, plast, m dense, moist
14					
13					Gray, stiff - v stiff, plast, moist, silty
24					

80-19M
28 JANUARY 1980



SPT Blows/Ft	MC	LL	PL	% FINES	
6	7.9				Brn, sandy, grav, loose, dry Conc slab
9	18.8	63	18		Brn, loose - m dense, dry, brick & glass debris
					Blk, loose, moist
12	5.0				Grn, loose, dry, lam
10	16.8			4	Lt grn - grn, gravelly, loose, wet 6% gravel
11	17.8			5	5% gravel Grn, m dense, sat, gravelly
18	28.8	63	18	1	2% gravel
45	22.4				Gray, hard, plast, moist, silty
43					Gray, dense, wet, v f sand, non - plastic
23					
16					Gray, v stiff, plast, moist
22					

80-18M
24-25 JANUARY 1980



SPT Blows/Ft.	MC	LL	PL	% FINES	
7	23.7	79	28		Sandy, blk, loose, moist
12	40.3	97	22		Gray, stiff, scat carb frag
1½	71.9				Gray, soft - v soft, moist, plast, scat silt lam
2	68.5	112	25		Gray, v soft, wet
		109	25		
9				6	Gray, moist - wet, loose 1% grave
14	14.6			7 4	Grn-brn, wet, loose, gravelly 25% gravel
17	13.1			3	7% gravel
22					Brn, dense - m dense, wet, gravelly
17				3	32% gravel
23					Gray, v stiff - hard, moist, plast, till gravelly
32					

NOTE:
BORING LEGEND IS SHOWN ON SHEET NO. C-11

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

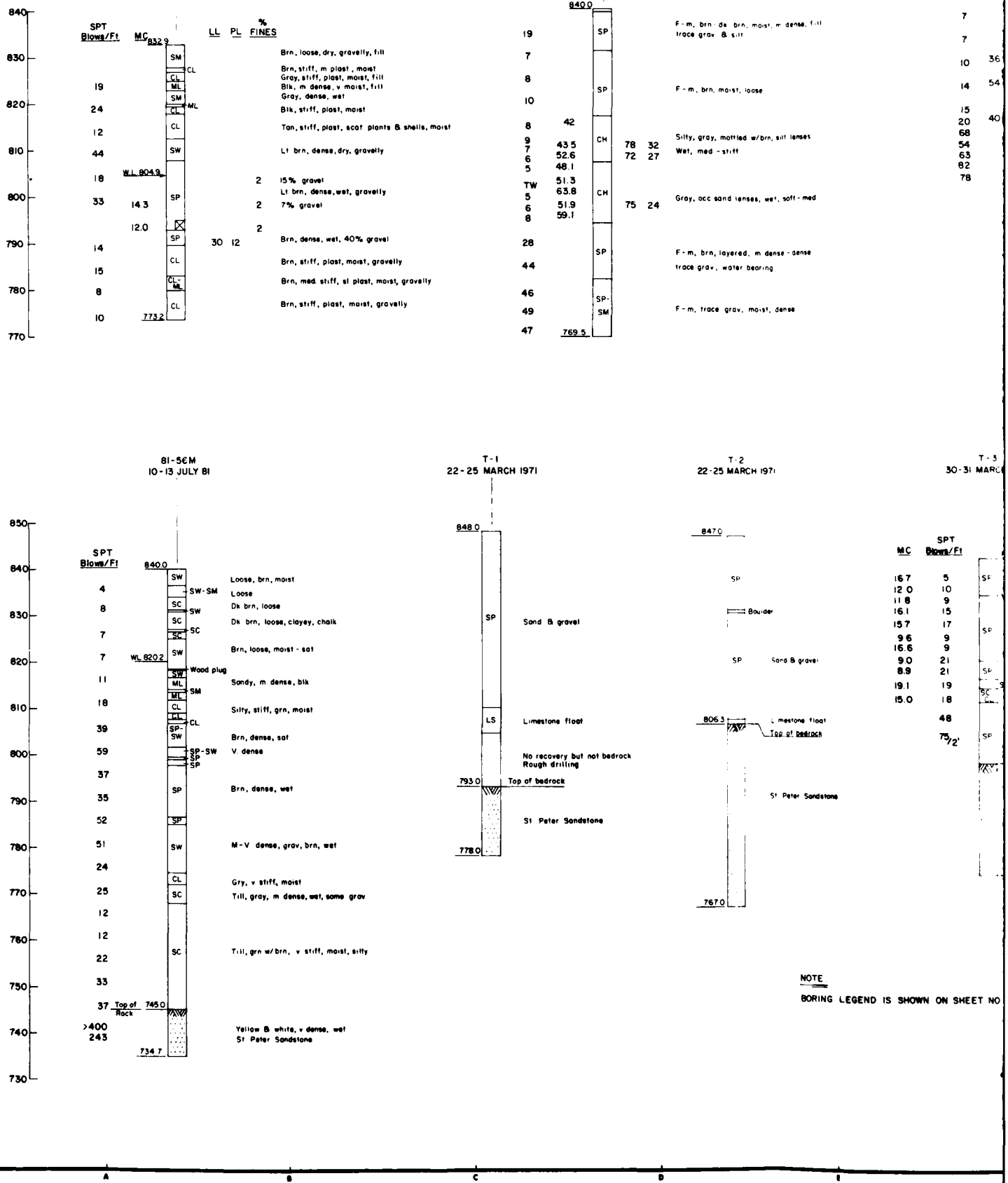
DRAWN BY: D.W.R., J.F.J.
TRACED BY: J.G.J., J.M.J.
CHECKED BY: D.W.R.
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

TUNNEL BORING LOGS
79-14M, 80-16M-19M, 81-56M, T-506 & T-508

DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/237
SHEET C-9 OF C-39

SAFETY PAYS


80-21M
30 JANUARY 1980
SPT Blows/Ft
MC
LL PL % FINES
832.9
SM
CL
CL
ML
SM
ML
CL
CL
SW
WL 804.9
SP
SP
CL
CL-ML
CL
773.2
Brn, loose, dry, gravelly, fill
Brn, stiff, m plast, moist
Gray, stiff, plast, moist, fill
Blk, m dense, v moist, fill
Gray, dense, wet
Blk, stiff, plast, moist
Tan, stiff, plast, scat plants & shells, moist
Lt brn, dense, dry, gravelly
15% gravel
Lt brn, dense, wet, gravelly
7% gravel
Brn, dense, wet, 40% gravel
Brn, stiff, plast, moist, gravelly
Brn, med stiff, sl plast, moist, gravelly
Brn, stiff, plast, moist, gravelly
840
830
820
810
800
790
780
770
ST-1
4 SEPTEMBER 1979
SPT Blows/Ft
MC
LL PL
840.0
SP
SP
CH
CH
SP
SP-SM
769.5
F-m, brn-dk brn, moist, m dense, f-ll trace grav & silt
F-m, brn, moist, loose
Silty, gray, mottled w/brn, silt lenses
Wet, med-stiff
Gray, occ sand lenses, wet, soft-med
F-m, brn, layered, m dense-dense trace grav, water bearing
F-m, trace grav, moist, dense
SPT Blows/Ft
M
81-56M
10-13 JULY 81
SPT Blows/Ft
840.0
WL 820.2
Wood plug
745.0
Top of Rock
734.7
SW
SW-SM
SC
SW
SC
SC
SW
SW
ML
ML
CL
CL
SP-SW
SP
SP
SW
CL
SC
SC
Loose, brn, moist
Loose
Dk brn, loose
Dk brn, loose, clayey, chalk
Brn, loose, moist-sat
Wood plug
Sandy, m dense, blk
Silty, stiff, grn, moist
Brn, dense, sat
V dense
Brn, dense, wet
M-V dense, grav, brn, wet
Gry, v stiff, moist
Till, gray, m dense, wet, some grav
Till, grn w/brn, v stiff, moist, silty
Yellow & white, v dense, wet
St Peter Sandstone
850
840
830
820
810
800
790
780
770
760
750
740
730
T-1
22-25 MARCH 1971
848.0
793.0
778.0
SP
LS
Top of bedrock
Sand & gravel
Limestone float
No recovery but not bedrock
Rough drilling
St Peter Sandstone
T-2
22-25 MARCH 1971
847.0
806.3
767.0
SP
Boulder
SP
Sand & gravel
Limestone float
Top of bedrock
St Peter Sandstone
T-3
30-31 MARC
MC
SPT Blows/Ft
16.7 5
12.0 10
11.8 9
16.1 15
15.7 17
9.6 9
16.6 9
9.0 21
8.9 21
19.1 19
15.0 18
48
75/2"
SP
SP
SC
SP
NOTE
BORING LEGEND IS SHOWN ON SHEET NO

F-m, brn dk brn, moist - dense trace grav & silt

F-m, brn, moist, loose

Silty, gray, mottled w/brn, silt lenses
Wet, med - stiff

Gray, occ sand lenses, wet soft - med

F-m, brn, layered, m dense dense
trace grav, water bearing

F-m, trace grav, moist, dense

ST-2
12-13 JUNE 1979

SPT Blows/Ft	MC		
		843.0	
7		SP	F-m, brn, loose, moist, fill
7			
10	36.5		
14	54.4	CH	Silty, gray, layered, wet, stiff - v stiff
15			
20	40.0		
68			
54			
63		SP	F-m, moist, v dense, trace gravel
82		LS	Pieces gray Dolo limestone
78		CL	hi plast, yell, moist, v stiff
			Top of Bedrock 801.0

80-20M
28-29 JANUARY 1980

SPT Blows/Ft	MC		LL	PL	% FINES	
		842.4				
12		SM				Brn, m dense, fill - debris
2		SP				Brn, m dense, dry, c sand
15		SM				
16		SP				Brn, loose, dry, gravelly, c sand
15		SP				Lt brn, m dense, dry, m sand
10						
8	25.0	CH	56	14		Gray, stiff, plast, moist
9						
11	28.5		65	18		
13	27.2		65	17		
14	19.5	SM	60	15		
13	19.8	CH				Gray, v stiff, plast, moist, fill, gravelly
18						
20		CL				Gray, v stiff, plast, moist
29		767.7				

T-2
22-25 MARCH 1971

847.0
SP
Boulder
SP Sand & gravel
806.3 Limestone float
Top of bedrock
St Peter Sandstone
767.0

T-3
30-31 MARCH 1971

MC	SPT Blows/Ft		
		842.0	
16.7	5	SP	Some organics
12.0	10		
11.8	9		
16.1	15	SP	Wet - moist
15.7	17		
9.6	9		
16.6	9		
9.0	21	SP	Sand & gravel
8.9	21	SC-CL	Moist, stiff
19.1	19		Moist - v moist
15.0	18		
	48	SP	Sand & gravel
	75/2"		moist - wet
			Top of Bedrock 798.0
			St Peter Sandstone
		774.0	

T-4
31 MARCH - 8 APRIL 1971

MC	SPT Blows/Ft		
		841.0	
4.6		SP	Sand & gravel
19.0	10		brn, wet - moist
15.2	10		peat layers
14.9	15	SP	Sand & gravel
14.4	26		brn, wet - moist
13.9	33	SM	Brn & gray brn
8.0	44		moist - v moist
8.4	75		Boulders from
9.8	75/8		14.5 to 28'
15.5	75/7	SP	Sand & gravel
12.1	75/4		brn, wet
13.3	75/3	LS	Limestone float
	75/1		Igneous boulder
	75/4	SP	Sand & gravel
10.2	75/2	CL	Till, brn, v moist
		SP	Sand & gravel
		LS	Top of Bedrock 784.0
			St Peter Sandstone
		761.0	

T-5
6-8 APRIL 1971

MC	SPT Blows/Ft		
		848.0	
13.7	9	SP	Some grav, moist - wet
13.7	8		
18.6	8	SP	F sand, gray, v moist
20.0	6		
16.6	11	SP	Sand & gravel
14.3	15		brn - gray, wet, silt layer
13.1	25		Boulders
7.5	42		
15.0	81	SM	Br, v moist, firm
10.6	59		Brn, wet, pieces of limestone
			Top of Bedrock 799.3
7.7	70		St Peter Sandstone
6.9	70		
8.3	50/1		
11.7	75/4	769.0	

NOTE

BORING LEGEND IS SHOWN ON SHEET NO. C-11

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY: DWR JFJ
DRAWN BY: JGJ, JMJ
CHECKED BY: DWR
SUBMITTED BY:
APPROVED:

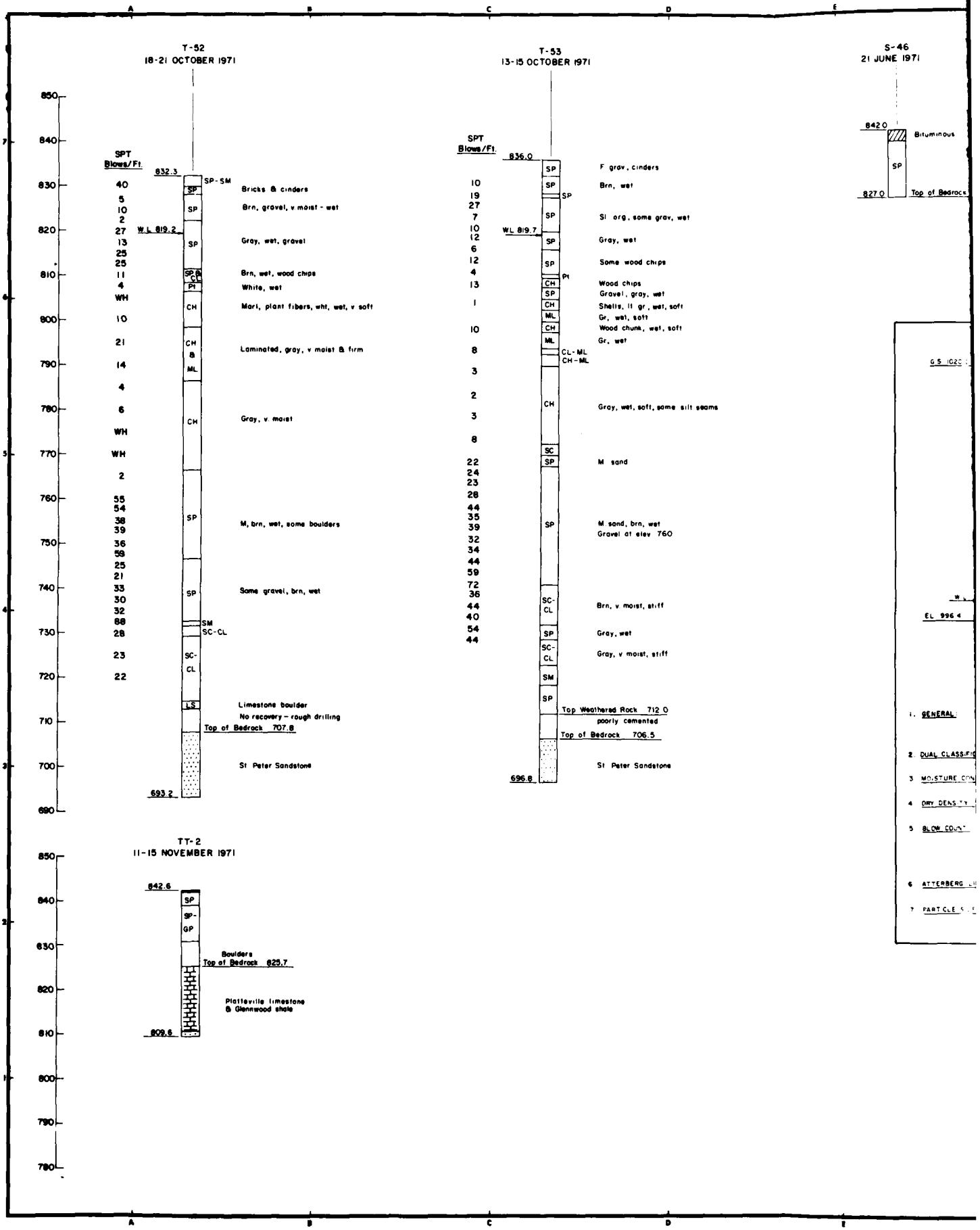
PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

TUNNEL BORING LOGS
80-20M, 21M, 81-56M, ST-1, ST-2, T-1 TO T-5

DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER
M34.3-R-5/238
SHEET C-10 OF C-59

SAFETY PAYS

2


T-52
18-21 OCTOBER 1971
SPT Blows/Ft.
832.3
SP-SM
Bricks & cinders
Brn, gravel, v moist - wet
W.L. 819.2
Gray, wet, gravel
Brn, wet, wood chips
White, wet
Marl, plant fibers, wht, wet, v soft
Laminated, gray, v moist & firm
Gray, v. moist
M, brn, wet, some boulders
Some gravel, brn, wet
SM
SC-CL
Limestone boulder
No recovery - rough drilling
Top of Bedrock 707.8
St Peter Sandstone
693.2
T-53
13-15 OCTOBER 1971
836.0
F grav, cinders
Brn, wet
Sl org, some grav, wet
W.L. 819.7
Gray, wet
Some wood chips
Wood chips
Gravel, gray, wet
Shells, lt gr, wet, soft
Gr, wet, soft
Wood chunk, wet, soft
Gr, wet
CL-ML
CH-ML
Gray, wet, soft, some silt seams
M sand
M sand, brn, wet
Gravel at elev 760
Brn, v moist, stiff
Gray, wet
Gray, v moist, stiff
Top Weathered Rock 712.0
poorly cemented
Top of Bedrock 706.5
St Peter Sandstone
696.8
S-46
21 JUNE 1971
842.0
Bituminous
827.0
Top of Bedrock
TT-2
11-15 NOVEMBER 1971
842.6
Boulders
Top of Bedrock 829.7
Platteville limestone & Glenwood shale
809.6

S-46
21 JUNE 1971

842.0 Bituminous
SP
827.0 Top of Bedrock (limestone)

S-45
21 JUNE 1971

842.0 Bituminous
SM
SP
SP F gravel
SP
SP C. sand
813.0

TT-1
9-10 NOVEMBER 1971

844.3
SM
SP
SP
SP Gravelly
SP
LS Weathered limestone
Top of Bedrock 825.7
Platteville limestone & Glenwood shale
810.8

850
840
830
820
810
800
790
780
770
760
750
740
730
720
710
700
690

GENERAL BORING LEGEND

GS 1020.2 — GROUND SURFACE ELEVATION AT BORING

GW	WELL GRADED GRAVELS, GRAVEL-SAND MIXTURE, LITTLE OR NO FINES
GP	POORLY GRADED GRAVELS, LITTLE OR NO FINES
GM	SILTY GRAVELS, GRAVEL-SAND-SILT MIXTURES
GC	CLAYEY GRAVELS, GRAVEL-SAND-CLAY MIXTURES
SW	WELL GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
SP	POORLY GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
SM	SILTY SANDS, SAND-SILT MIXTURES
SC	CLAYEY SANDS, SAND-CLAY MIXTURES
ML	INORGANIC SILTS, LIQUID LIMIT LESS THAN 50
MH	INORGANIC SILTS, LIQUID LIMIT GREATER THAN 50
CL	INORGANIC CLAYS, LOW TO MEDIUM PLASTICITY, LIQUID LIMIT LESS THAN 50
CH	INORGANIC CLAYS, HIGH PLASTICITY, LIQUID LIMIT GREATER THAN 50
OL	ORGANIC SILTS OR CLAYS, LOW PLASTICITY, LIQUID LIMIT LESS THAN 50
OH	ORGANIC SILTS OR CLAYS, MEDIUM TO HIGH PLASTICITY, LIQUID LIMIT GREATER THAN 50
PT	PEAT

W.L. — WATER LEVEL AT DATE OF BORING

EL 996.4 — NO RECOVERY; ELEVATION AT BOTTOM OF BORING

66-IM — MACHINE BORING

66-4A — AUGER BORING

GENERAL NOTES

1. GENERAL: THE UNIFIED SOIL CLASSIFICATION SYSTEM IS USED TO IDENTIFY BASIC SOIL TYPE THE LEGEND REPRESENTS ONLY THE BASIC SOILS. TO COMPLETE THE CLASSIFICATION PERTINENT INFORMATION IS ADDED TO THE RIGHT OF THE BORING LOG.

2. DUAL CLASSIFICATION: WHERE A SOIL IS CONSIDERED TO BE ON THE BORDER LINE OF TWO (OR MORE) GROUPS A DOUBLE SYMBOL IS USED i.e. (GP-GM) OR (CL-ML)

3. MOISTURE CONTENT: THE NATURAL MOISTURE CONTENT IN PERCENT OF DRY WEIGHT (MC) IS SHOWN TO THE LEFT OF THE BORING LOG

4. DRY DENSITY: THE DRY DENSITY IN POUNDS PER CUBIC FOOT (γ_d) IS SHOWN TO THE LEFT OF THE BORING LOG

5. BLOW COUNT: BLOW COUNTS ARE SHOWN TO THE LEFT OF THE BORING LOGS AND EXCEPT AS NOTED ARE THE NUMBER OF BLOWS NECESSARY TO DRIVE THE SAMPLER USED A DISTANCE OF 12 INCHES. STANDARD BLOW COUNTS ARE FOR A STANDARD PENETRATION TEST, USING A 1 3/8" X 2" SAMPLER, 140 LB HAMMER AND A 30" DROP FOR NON-STANDARD BLOW COUNTS, SAMPLER SIZE, HAMMER WEIGHT AND/OR HEIGHT OF DROP ARE AS SHOWN

6. ATTERBERG LIMITS: LIQUID LIMIT (LL) AND PLASTIC LIMIT (PL) ARE SHOWN TO THE RIGHT OF THE BORING LOG

7. PARTICLE SIZE: FINE, MEDIUM AND COARSE SAND ARE DESIGNATED BY THE LETTERS F, M & C TO THE RIGHT OF THE BORING LOG FOR GRAVELS THE MAXIMUM PARTICLE SIZE FOUND IN SAMPLE IS GIVEN. FOR MIXED MATERIALS APPROXIMATE PERCENTAGES MAY ALSO BE GIVEN

SAFETY PAYS

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

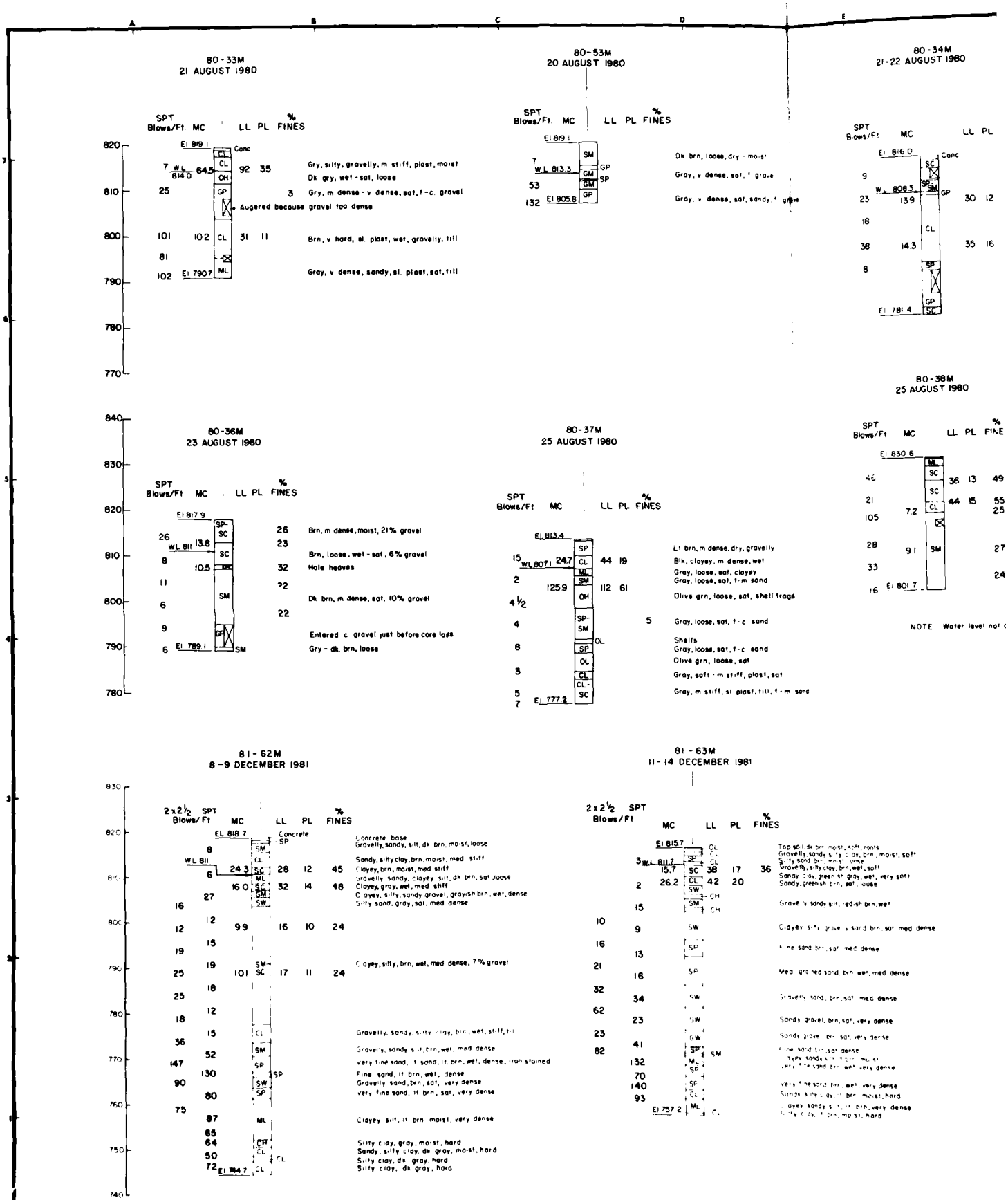
DESIGNED BY: DWR, JFJ
DRAWN BY: JGJ, JMJ
CHECKED BY: DWR
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

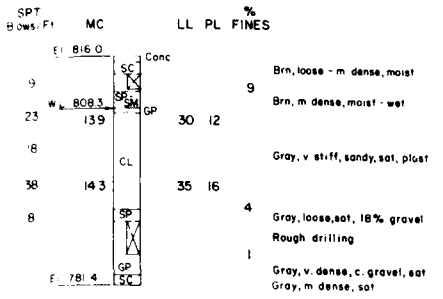
TUNNEL BORING LOGS
T-52, T-53, S-45, S-46, TT-1

DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/239
SHEET C-11 OF C-39

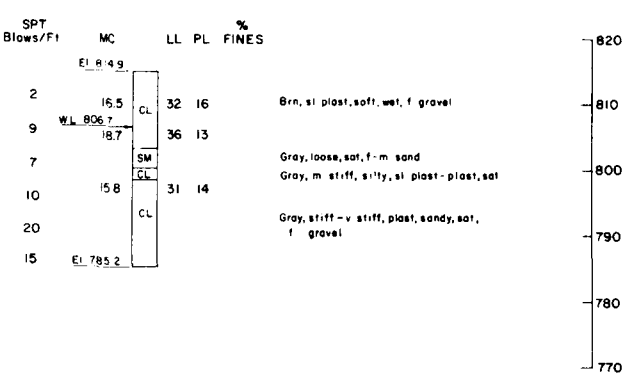
2


A
B
C
D
E
80-33M
21 AUGUST 1980
SPT Blows/Ft
MC
LL
PL
% FINES
820
810
800
790
780
770
El 819.1
Conc
CL
OH
GP
ML
7
WL 814.0
64.5
92
35
Gry, silty, gravelly, m stiff, plast, moist
Dk gry, wet - sat, loose
25
3
Gry, m dense - v dense, sat, f - c gravel
Augered because gravel too dense
101
102
10.2
CL
31
11
Brn, v hard, sl. plast, wet, gravelly, till
81
El 790.7
Gray, v dense, sandy, sl. plast, sat, till
80-53M
20 AUGUST 1980
SPT Blows/Ft
MC
LL
PL
% FINES
El 819.1
SM
GP
GM
SP
7
WL 813.3
53
132
El 805.8
Dk brn, loose, dry - moist
Gray, v dense, sat, f gravel
Gray, v dense, sat, sandy, f gravel
80-34M
21-22 AUGUST 1980
SPT Blows/Ft
MC
LL
PL
El 816.0
Conc
SC
SP
SM
GP
CL
9
WL 808.3
23
13.9
30
12
18
38
14.3
35
16
8
El 781.4
840
830
820
810
800
790
780
80-36M
23 AUGUST 1980
SPT Blows/Ft
MC
LL
PL
% FINES
El 817.9
SP-SC
SC
SM
GP
26
WL 811
13.8
8
10.5
11
6
9
6
El 789.1
26
Brn, m dense, moist, 21% gravel
23
Brn, loose, wet - sat, 6% gravel
32
Hole heaves
22
22
Dk brn, m dense, sat, 10% gravel
Entered c gravel just before core loss
Gry - dk. brn, loose
80-37M
25 AUGUST 1980
SPT Blows/Ft
MC
LL
PL
% FINES
El 813.4
SP
CL
ML
SM
OH
SP-SM
OL
SP
OL
CL
CL-SC
15
WL 807.1
24.7
44
19
2
125.9
112
61
4 1/2
4
8
3
5
7
El 777.2
Lt brn, m dense, dry, gravelly
Blk, clayey, m dense, wet
Gray, loose, sat, clayey
Gray, loose, sat, f-m sand
Olive grn, loose, sat, shell frags
5
Gray, loose, sat, f - c sand
Shells
Gray, loose, sat, f-c sand
Olive grn, loose, sat
Gray, soft - m stiff, plast, sat
Gray, m stiff, sl plast, till, f - m sand
80-38M
25 AUGUST 1980
SPT Blows/Ft
MC
LL
PL
% FINE
El 830.6
ML
SC
CL
46
36
13
49
21
44
15
55
105
7.2
25
28
9.1
SM
27
33
24
16
El 801.7
NOTE Water level not
830
820
810
800
790
780
770
760
750
740
81-62M
8-9 DECEMBER 1981
2x2 1/2 SPT Blows/Ft
MC
LL
PL
% FINES
EL 818.7
Concrete
SP
SM
CL
SC
ML
CH
SW
8
WL 811
6
24.3
27
16.0
16
12
12
9.9
15
19
25
19
101
25
18
12
18
15
36
52
147
130
90
80
75
87
65
64
50
72
El 764.7
28
12
45
32
14
48
16
10
24
17
11
24
Concrete base
Gravelly, sandy, silt, dk. brn, moist, loose
Sandy, silty clay, brn, moist, med stiff
Clayey, brn, moist, med stiff
Gravelly, sandy, clayey silt, dk brn, sat, loose
Clayey, gray, wet, med stiff
Clayey, silty, sandy gravel, grayish brn, wet, dense
Silty sand, gray, sat, med dense
Clayey, silty, brn, wet, med dense, 7% gravel
Gravelly, sandy, silty clay, brn, wet, st-ff, f-l
Gravelly, sandy silt, brn, wet, med dense
very fine sand, f sand, lt. brn, wet, dense, iron stained
Fine sand, lt. brn, wet, dense
Gravelly sand, brn, sat, very dense
very fine sand, lt brn, sat, very dense
Clayey silt, lt brn moist, very dense
Silty clay, gray, moist, hard
Sandy, silty clay, dk gray, moist, hard
Silty clay, dk gray, hard
Silty clay, dk gray, hard
81-63M
11-14 DECEMBER 1981
2x2 1/2 SPT Blows/Ft
MC
LL
PL
% FINES
El 815.7
OL
CL
SP
SC
CL
SW
OH
SM
CH
SW
SP
SW
GW
GW
SP
SM
ML
SP
SP
CL
ML
CL
3
WL 811.7
15.7
38
17
36
2
26.2
42
20
15
10
9
16
13
21
16
32
34
62
23
23
41
132
70
140
93
El 757.2
Top soil, dk brn, moist, soft, roots
Gravelly, sandy silty clay, brn, moist, soft
Silty sand, brn, moist, loose
Gravelly, silty clay, brn, wet, soft
Sandy clay, green-sh gray, wet, very soft
Sandy, greenish brn, sat, loose
Gravelly sandy silt, red-sh brn, wet
Clayey silty gravelly sand, brn, sat, med dense
Fine sand, brn, sat, med dense
Med. grained sand, brn, wet, med dense
Gravelly sand, brn, sat, med dense
Sandy gravel, brn, sat, very dense
Sandy gravel, brn, sat, very dense
Fine sand, brn, sat, dense
Clayey sandy silt, brn, moist
very fine sand, brn, wet, very dense
very fine sand, brn, wet, very dense
Sandy silty clay, brn, moist, hard
Clayey sandy silt, brn, very dense
Silty clay, brn, moist, hard

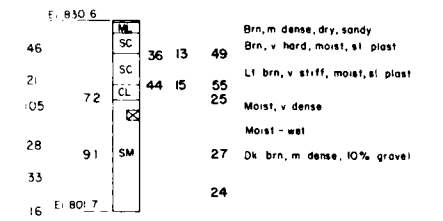
80-34M
21-22 AUGUST 1980



80-35M
22-23 AUGUST 1980

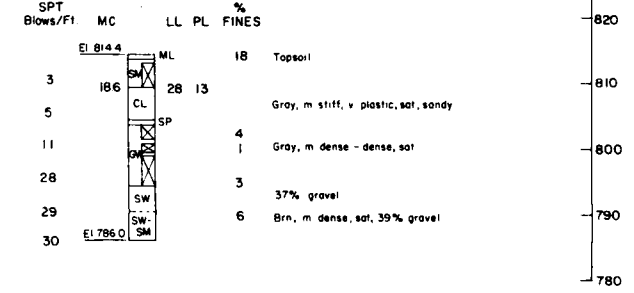


80-38M
25 AUGUST 1980



NOTE Water level not determined

80-39M
25-26 AUGUST 1980



NOTE
BORING LEGEND IS SHOWN ON SHEET NO C-11

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

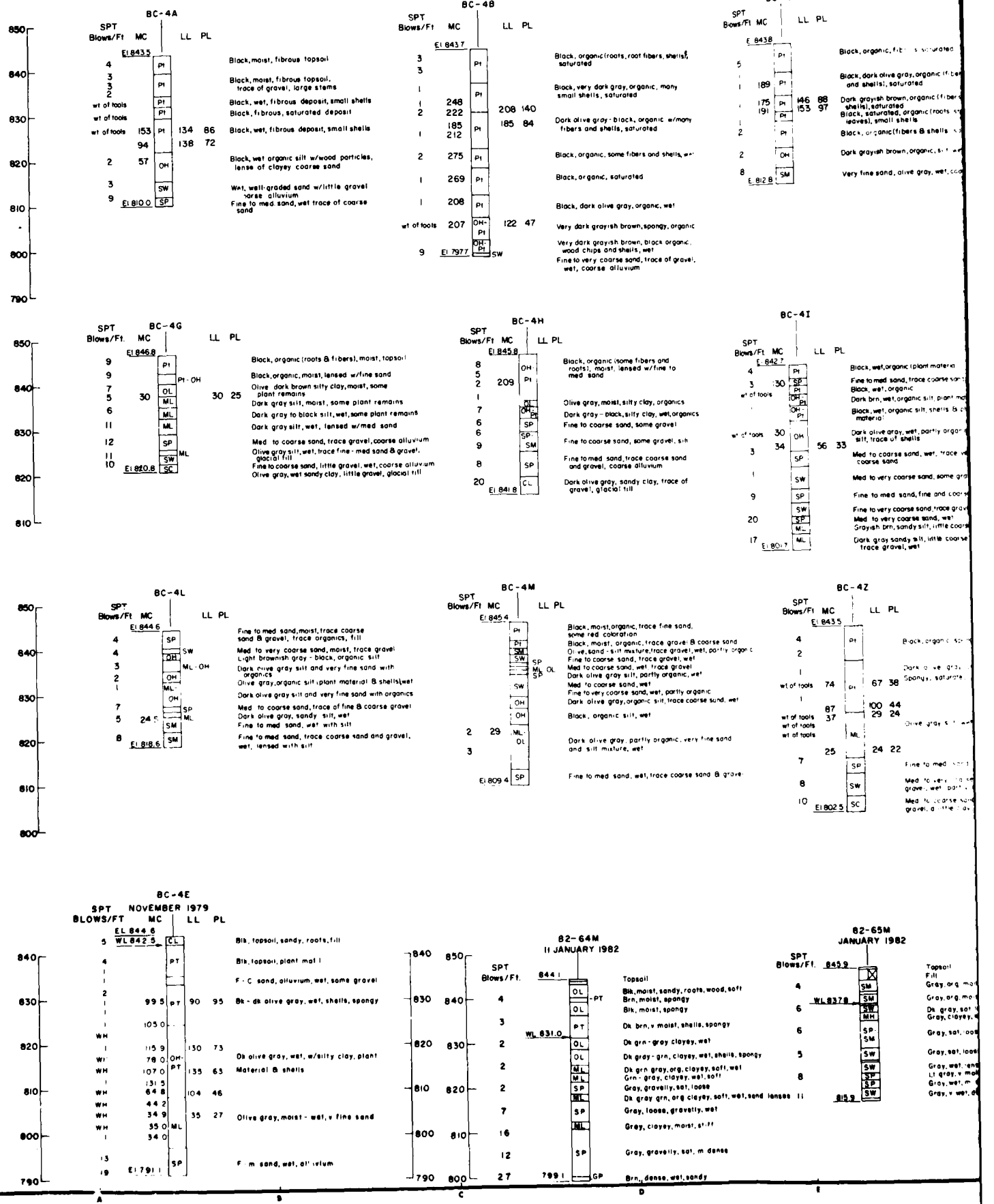
DESIGNED BY T M M & D W R
DRAWN BY J G L, J M J
CHECKED BY M B
SUBMITTED BY
CHIEF, ENGR BRANCH
APPROVED
CHIEF, ENGR DIVISION

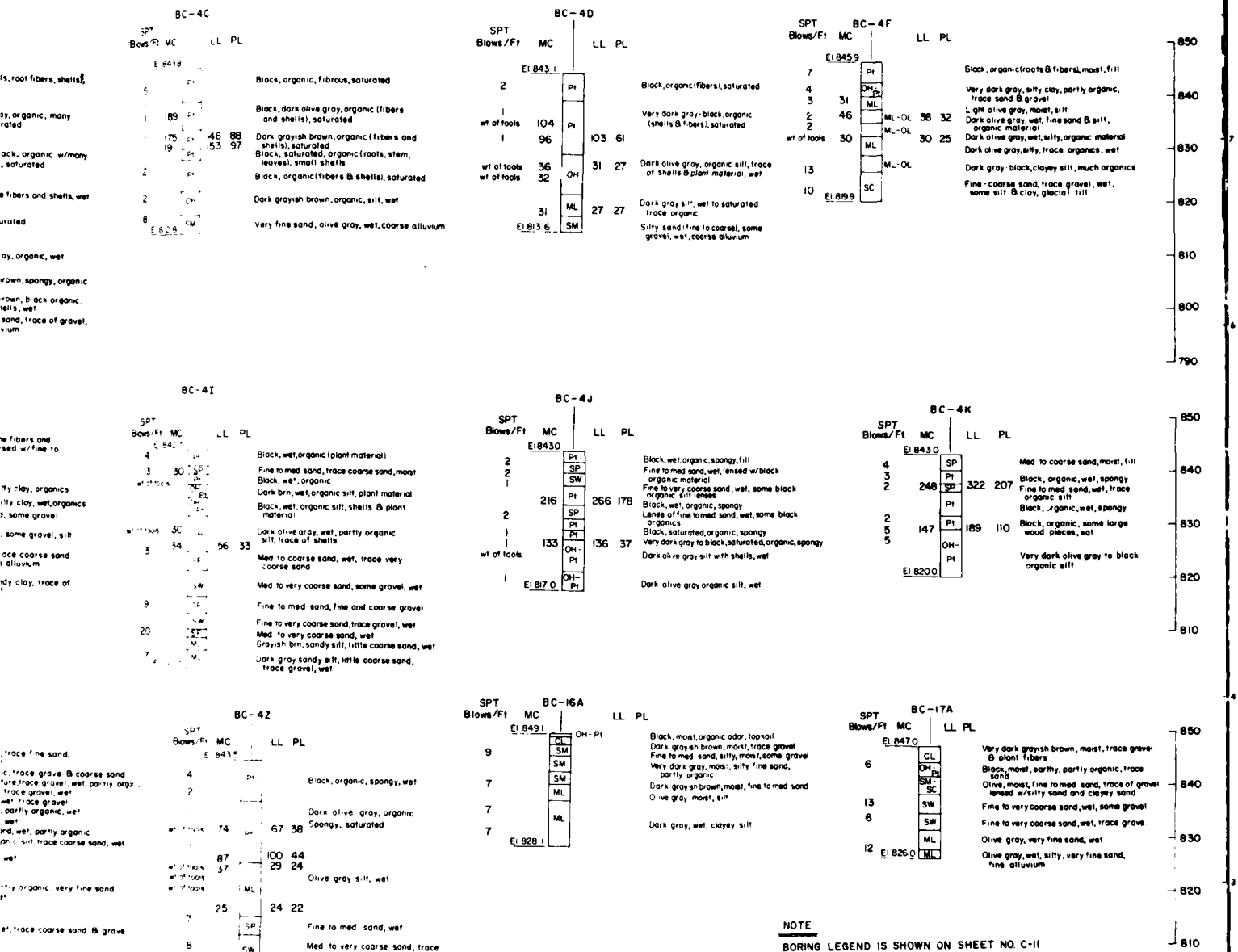
PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
FRUEN MILL AREA BORING LOGS
80-33M THRU 8-39M
80-53M, 81-62M, 81-63M

DATE AUGUST 1982
SCALE AS SHOWN
DRAWING NUMBER M34.3-R-5/240
SHEET C-12 OF C-39

SAFETY PAYS

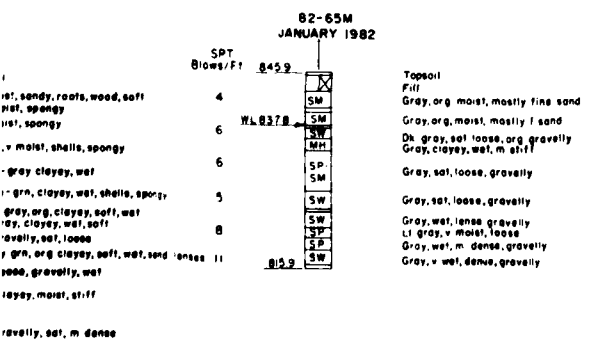
2


BC-4A
SPT Blows/Ft
MC
LL PL
EL 843.5
EL 810.0
Black, moist, fibrous topsoil
Black, moist, fibrous topsoil, trace of gravel, large stems
Black, wet, fibrous deposit, small shells
Black, fibrous, saturated deposit
Black, wet, fibrous deposit, small shells
Black, wet organic silt w/wood particles, lense of clayey coarse sand
Wet, well-graded sand w/little gravel coarse alluvium
Fine to med. sand, wet trace of coarse sand
wt of tools
wt of tools
wt of tools
153
94
57
134 86
138 72
BC-4B
EL 843.7
EL 797.7
Black, organic (roots, root fibers, shells), saturated
Black, very dark gray, organic, many small shells, saturated
Dark olive gray - black, organic w/many fibers and shells, saturated
Black, organic, some fibers and shells, wet
Black, organic, saturated
Black, dark olive gray, organic, wet
Very dark grayish brown, spongy, organic
Very dark grayish brown, black organic, wood chips and shells, wet
Fine to very coarse sand, trace of gravel, wet, coarse alluvium
wt of tools
248
222
185
212
275
269
208
207
208 140
185 84
122 47
BC-4C
EL 843.8
EL 812.8
Black, organic, fibers saturated
Black, dark olive gray, organic (fibers and shells), saturated
Dark grayish brown, organic (fibers shells), saturated
Black, saturated, organic (roots leaves), small shells
Black, organic (fibers & shells
Dark grayish brown, organic,
Very fine sand, olive gray, wet,
189
175
191
146 88
153 97
BC-4G
EL 846.8
EL 820.8
Black, organic (roots & fibers), moist, topsoil
Black, organic, moist, lensed w/fine sand
Olive dark brown silty clay, moist, some plant remains
Dark gray silt, moist, some plant remains
Dark gray to black silt, wet, some plant remains
Dark gray silt, wet, lensed w/med sand
Med to coarse sand, trace gravel, coarse alluvium
Olive gray silt, wet, trace fine - med sand & gravel, glacial till
Fine to coarse sand, little gravel, wet, coarse alluvium
Olive gray, wet sandy clay, little gravel, glacial till
30
30 25
BC-4H
EL 845.8
EL 841.8
Black, organic (some fibers and roots), moist, lensed w/fine to med sand
Olive gray, moist, silty clay, organics
Dark gray - black, silty clay, wet, organics
Fine to coarse sand, some gravel
Fine to coarse sand, some gravel, silt
Fine to med. sand, trace coarse sand and gravel, coarse alluvium
Dark olive gray, sandy clay, trace of gravel, glacial till
209
BC-4I
EL 842.7
EL 801.7
Black, wet, organic (plant material
Fine to med sand, trace coarse sand
Black, wet, organic
Dark brn, wet, organic silt, plant mat
Black, wet, organic silt, shells & material
Dark olive gray, wet, partly organ silt, trace of shells
Med to coarse sand, wet, trace v coarse sand
Med to very coarse sand, some gra
Fine to med sand, fine and coar
Fine to very coarse sand, trace gra
Med to very coarse sand, wet
Grayish brn, sandy silt, little coars
Dark gray sandy silt, little coarse trace gravel, wet
wt of tools
30
34
56 33
BC-4L
EL 844.6
EL 818.6
Fine to med sand, moist, trace coarse sand & gravel, trace organics, fill
Med to very coarse sand, moist, trace gravel
Light brownish gray - black, organic silt
Dark olive gray silt and very fine sand with organics
Olive gray, organic silt (plant material & shells), wet
Dark olive gray silt and very fine sand with organics
Med to coarse sand, trace of fine & coarse gravel
Dark olive gray, sandy silt, wet
Fine to med sand, wet with silt
Fine to med sand, trace coarse sand and gravel, wet, lensed with silt
24.5
BC-4M
EL 845.4
EL 809.4
Black, moist, organic, trace fine sand, some red coloration
Black, moist, organic, trace gravel & coarse sand
Olive, sand - silt mixture, trace gravel, wet, partly organic
Fine to coarse sand, trace gravel, wet
Med to coarse sand, wet, trace gravel
Dark olive gray silt, partly organic, wet
Med to coarse sand, wet
Fine to very coarse sand, wet, partly organic
Dark olive gray, organic silt, trace coarse sand, wet
Black, organic silt, wet
Dark olive gray, partly organic, very fine sand and silt mixture, wet
Fine to med sand, wet, trace coarse sand & grave
29
BC-4Z
EL 843.5
EL 802.5
Black, organic
Dark olive gra
Spongy, saturate
Olive gray
Fine to med
Med to very
gravel, wet, part
Med to coarse sand
gravel, a little
wt of tools
wt of tools
wt of tools
74
87
37
25
67 38
100 44
29 24
24 22
BC-4E
NOVEMBER 1979
EL 844.6
WL 842.5
EL 791.1
Blk, topsoil, sandy, roots, fill
Blk, topsoil, plant mat'l
F - C sand, alluvium, wet, some gravel
Bk - dk olive gray, wet, shells, spongy
Dk olive gray, wet, w/silty clay, plant
Material & shells
Olive gray, moist - wet, v fine sand
F - m sand, wet, alluvium
99.5
105.0
115.9
78.0
107.0
131.5
64.8
44.2
34.9
35.0
34.0
90 95
130 73
135 63
104 46
35 27
82-64M
11 JANUARY 1982
844.1
WL 831.0
799.1
Topsoil
Blk, moist, sandy, roots, wood, soft
Brn, moist, spongy
Blk, moist, spongy
Dk brn, v moist, shells, spongy
Dk grn - gray clayey, wet
Dk gray - grn, clayey, wet, shells, spongy
Dk grn gray, org. clayey, soft, wet
Grn - gray, clayey, wet, soft
Gray, gravelly, sat, loose
Dk gray grn, org clayey, soft, wet, sand lenses
Gray, loose, gravelly, wet
Grey, clayey, moist, stiff
Gray, gravelly, sat, m dense
Brn, dense, wet, sandy
82-65M
JANUARY 1982
845.9
815.9
WL 837.8
Topsoil
Fill
Gray, org
Gray, org, mo
Dk gray, sat
Gray, clayey,
Gray, sat, loos
Gray, sat, loos
Gray, wet, sand
Lt gray, v mo
Gray, wet, m
Gray, v wet, d



NOTE

BORING LEGEND IS SHOWN ON SHEET NO. C-11



SAFETY DAYS

SYMBOL	DESCRIPTION	DATE	APPROVAL

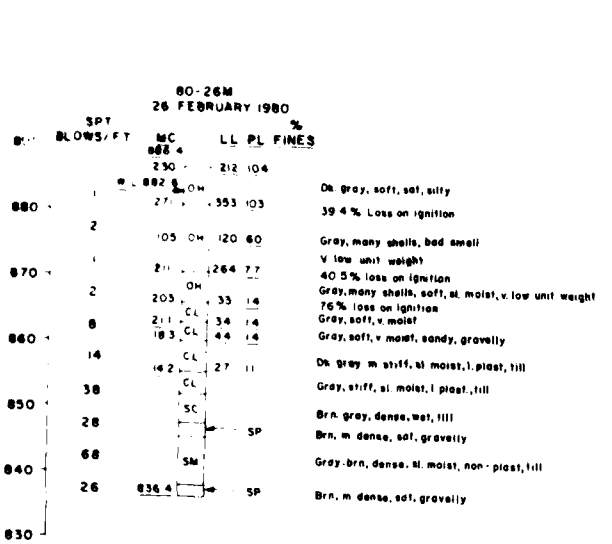
DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

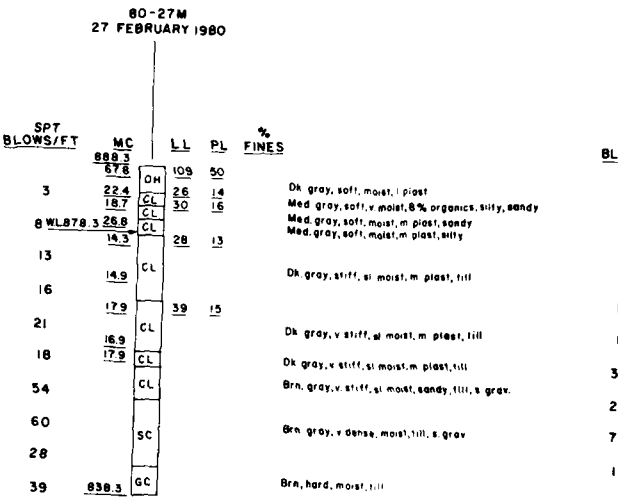
DESIGNED BY BARR ENGR, D W R
DRAWN BY J M J
CHECKED BY M B
SUBMITTED BY
APPROVED

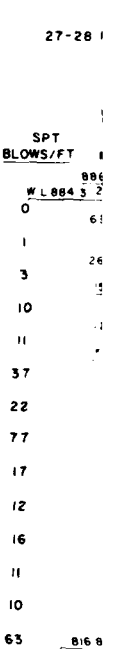
PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

HIGHWAY 100
BORING LOGS
BC-4A - 82-65M

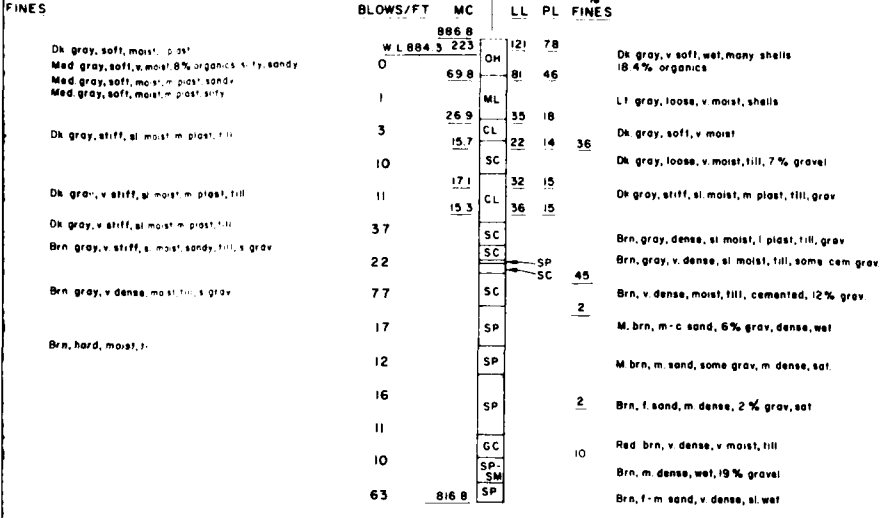
DATE: AUGUST 1982
SCALE
DRAWING NUMBER
M34.3-R-5/241
SHEET C-13 OF C-39


80-26M
26 FEBRUARY 1980
SPT BLOWS/FT
MC
LL
PL
% FINES
888.4
290
212
104
882.5
OH
1
27
353
103
Dk gray, soft, sat, silty
39.4% Loss on ignition
880
2
105
OH
120
60
Gray, many shells, bad smell
V low unit weight
1
211
264
77
40.5% loss on ignition
870
2
OH
203
33
14
Gray, many shells, soft, sl. moist, v. low unit weight
76% loss on ignition
CL
8
211
34
14
Gray, soft, v. moist
860
CL
183
44
14
Gray, soft, v. moist, sandy, gravelly
14
CL
162
27
11
Dk gray m stiff, sl moist, l. plast, till
CL
38
Gray, stiff, sl. moist, l plast., till
850
SC
28
Brn. gray, dense, wet, till
SP
Brn, m dense, sat, gravelly
68
SM
Gray-brn, dense, sl. moist, non-plast, till
840
26
836.4
SP
Brn, m dense, sat, gravelly
830

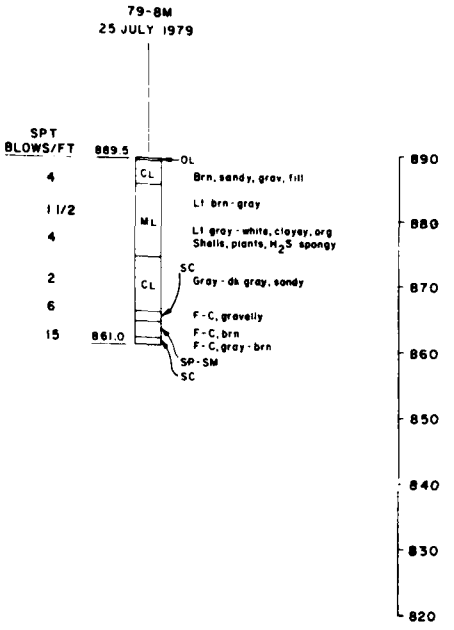

80-27M
27 FEBRUARY 1980
SPT BLOWS/FT
MC
LL
PL
% FINES
888.3
67.6
OH
109
50
3
22.4
CL
26
14
Dk gray, soft, moist, l plast
18.7
CL
30
16
Med gray, soft, v moist, 8% organics, silty, sandy
8
WL 878.3
26.8
CL
Med. gray, soft, moist, m plast, sandy
14.3
28
13
Med. gray, soft, moist, m plast, silty
13
CL
14.9
16
Dk gray, stiff, sl moist, m plast, till
17.9
39
15
21
CL
Dk gray, v stiff, sl moist, m plast, till
16.9
18
17.9
CL
Dk gray, v stiff, sl moist, m plast, till
CL
54
Brn. gray, v. stiff, sl moist, sandy, till, s grav.
60
SC
Brn gray, v dense, moist, till, s grav
28
39
838.3
GC
Brn, hard, moist, till


27-28
SPT BLOWS/FT
WL 884.3
0
1
3
10
11
37
22
77
17
12
16
11
10
63
816.8

80-28M
27-28 FEBRUARY 1980



79-8M
25 JULY 1979



NOTE:
BORING LEGEND IS SHOWN ON SHEET NO. C-11

SYMBOL	DESCRIPTION	DATE	APPROVAL

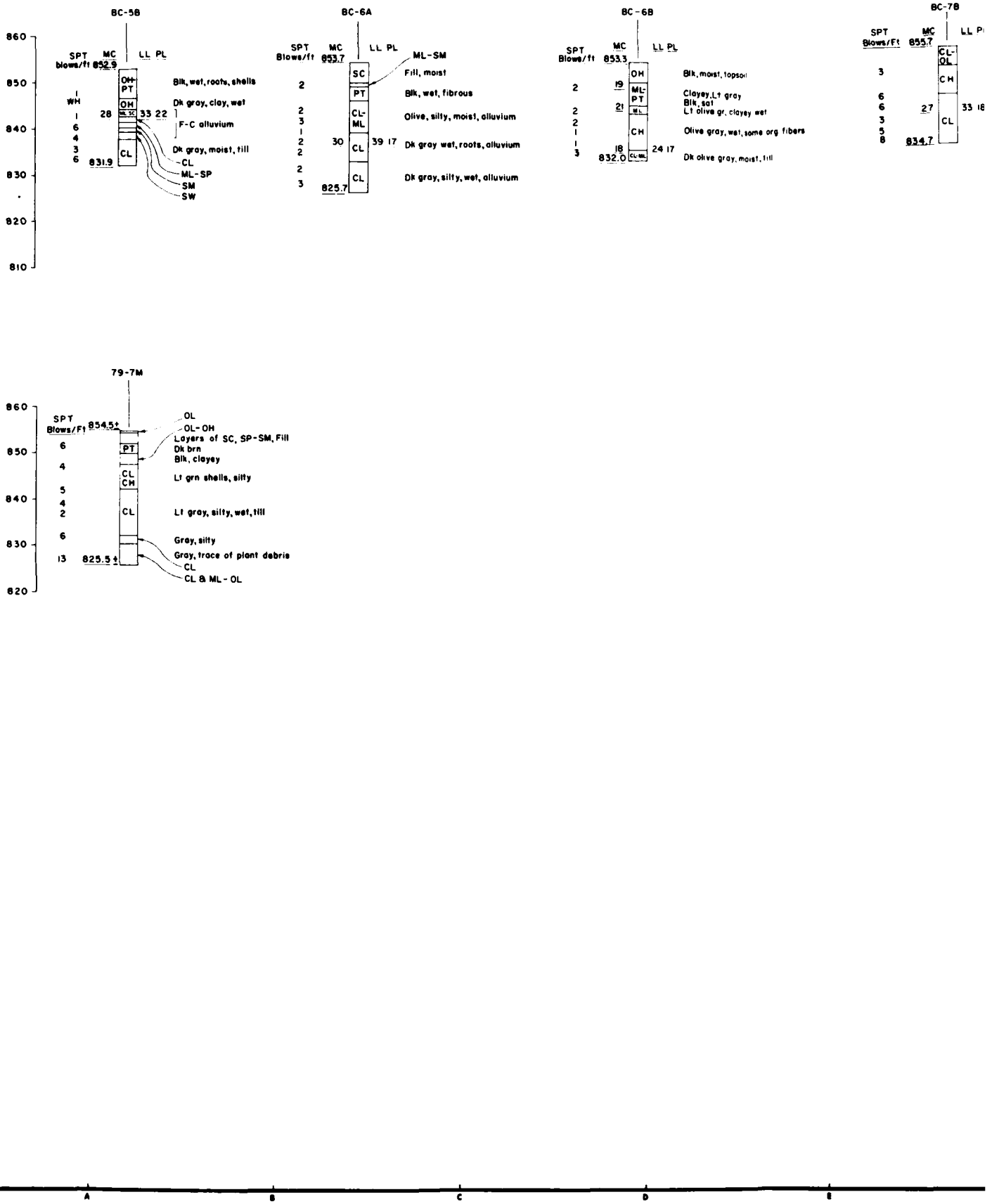
DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

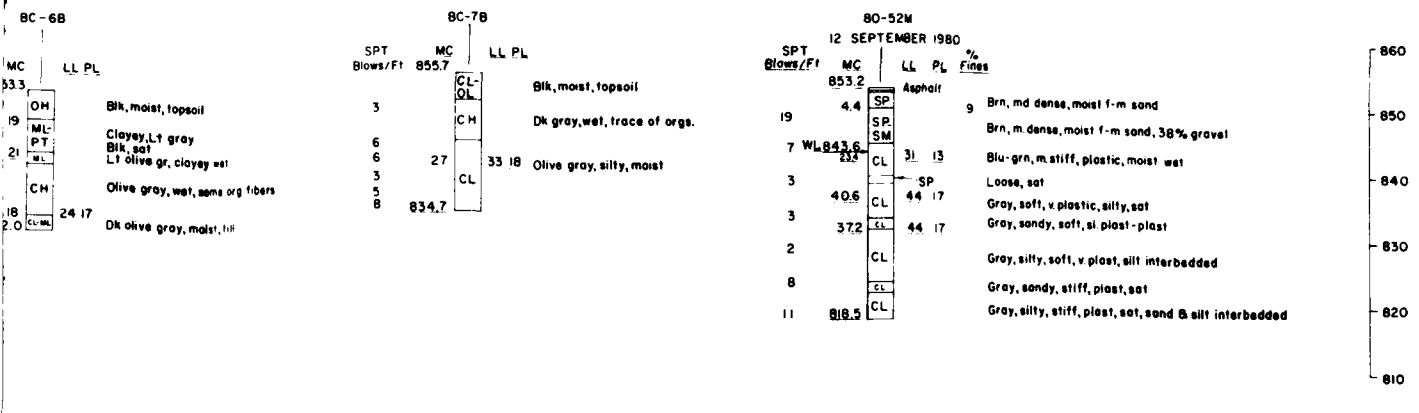
DESIGNED BY: D R
DRAWN BY: J G J
CHECKED BY: D W R
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
MEDICINE LAKE OUTLET
BORING LOGS

DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/242
SHEET C-14 OF C-39

SAFETY PAYS


BC-5B
SPT Blows/ft
MC 852.9
LL PL
WH
1
28
6
4
3
6
831.9
OH
PT
OH
ML SC
33 22
CL
Blk, wet, roots, shells
Dk gray, clay, wet
F-C alluvium
Dk gray, moist, till
CL
ML-SP
SM
SW
860
850
840
830
820
810
BC-6A
SPT Blows/ft
MC 853.7
LL PL
2
2
3
1
2
2
2
3
825.7
SC
PT
CL-ML
CL
30
39 17
ML-SM
Fill, moist
Blk, wet, fibrous
Olive, silty, moist, alluvium
Dk gray wet, roots, alluvium
Dk gray, silty, wet, alluvium
BC-6B
SPT Blows/ft
MC 853.3
LL PL
2
2
2
1
1
3
832.0
OH
19
ML-PT
21
ML
CH
18
24 17
CL-ML
Blk, moist, topsoil
Clayey, Lt gray
Blk, sat
Lt olive gr, clayey wet
Olive gray, wet, some org fibers
Dk olive gray, moist, till
BC-7B
SPT Blows/Ft
MC 855.7
LL PL
3
6
6
3
5
8
834.7
CL-OL
CH
27
CL
33 18
79-7M
SPT Blows/Ft
854.5±
6
4
5
4
2
6
13
825.5±
PT
CL CH
CL
OL
OL-OH
Layers of SC, SP-SM, Fill
Dk brn
Blk, clayey
Lt grn shells, silty
Lt gray, silty, wet, till
Gray, silty
Gray, trace of plant debris
CL
CL & ML-OL
860
850
840
830
820
A
B
C
D
E
1
2
3
4
5
6
7
8



NOTE:
BORING LEGEND IS SHOWN ON SHEET NO. C-11

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

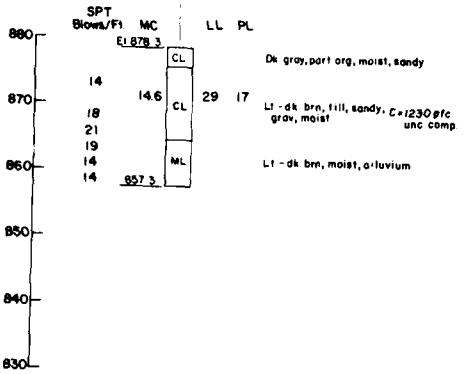
DESIGNED BY: D.W.R.
DRAWN BY: C.R.C.
CHECKED BY: D.W.R.
SUBMITTED BY:
APPROVED:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
BORING LOGS @ BRUNSWICK AND 32 AVES
DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER: M34.3-R-5/243
SHEET C-15 OF C-39

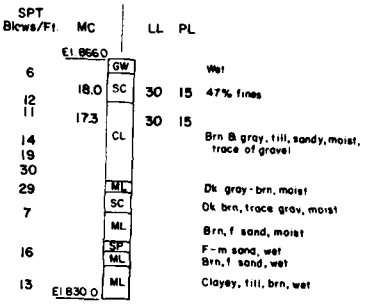
SAFETY PAYS

2

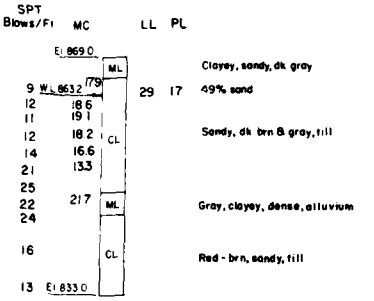
BC-10A
4 DECEMBER 1979



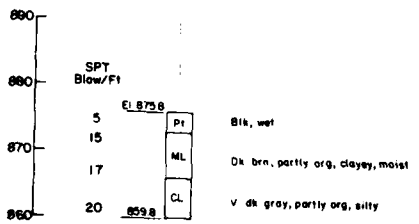
BC-10B
5 DECEMBER 1979



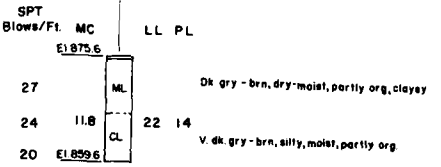
BC-10C
1 DECEMBER 1979



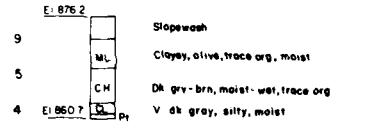
BC-13A
26 MARCH 1980



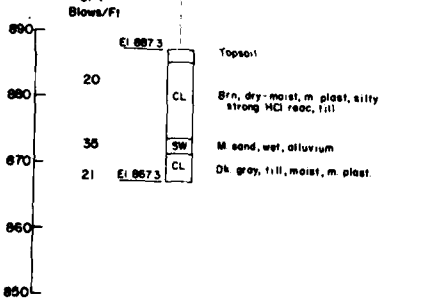
BC-13B
26 MARCH 1980



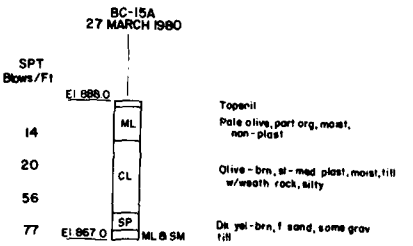
BC-14A
26 MARCH 1980



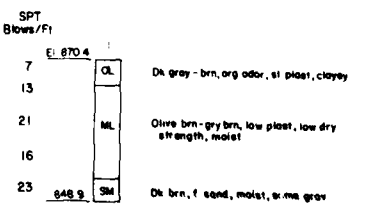
BC-14D
27 MARCH 1980



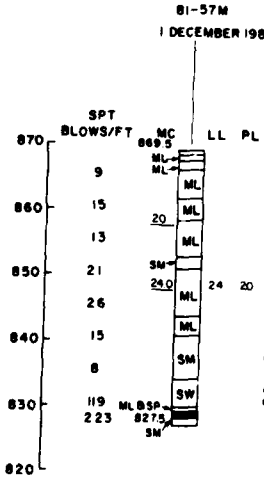
BC-15A
27 MARCH 1980



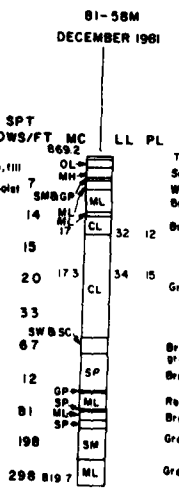
BC-15B
27 MARCH 1980



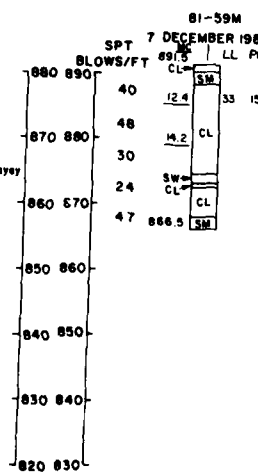
81-57M
1 DECEMBER 1981



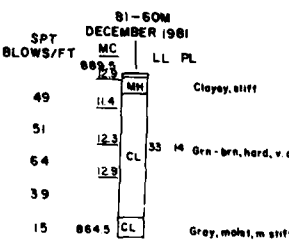
81-58M
DECEMBER 1981



81-59M
7 DECEMBER 1981



81-60M
DECEMBER 1981



NOTE
BORING LEGEND IS SHOWN ON SHEET NO C-11

BC-10C
1 DECEMBER 1979

SPT Blows/Ft | MC | LL | PL

El 869.2
ML — Clayey, sandy, dk gray
9 WL 863.2 — 29 — 17 — 49% sand
12 — 18.6
11 — 19.1
12 — 18.2 — CL — Sandy, dk brn & gray, till
14 — 16.6
21 — 13.3
25
22 — 21.7 — ML — Gray, clayey, dense, alluvium
24
16 — CL — Red-brn, sandy, till
13 — El 833.0

BC-10D
1 DECEMBER 1979

SPT Blows/Ft | MC | LL | PL

El 869.4
Pt — Blk, trace of sand
9 — 18.1
11 — 17.3 — CL — 31 — 17 — Sandy, dk brn & gray, till, little grav
13
15
WL 854.1
11 — 27.4
13 — 23.7 — ML — Dk gray, clayey, alluvium
13 — 22.1
8 — 23.1
12.4
8 — 11.8 — CL — Rd-brn, sandy, till
18.3
19.0 — SP — M-sand, brn, alluvium
El 828.4

BC-10E
4 DECEMBER 1979

SPT Blows/Ft

El 869.0
WL 866.7 — OL — Blk, soft
7 — CL — Sandy, till, gray
Gradation
11 — El 858.0 — ML — Clayey, gray, alluvium

680
670
660
650
640
630

BC-14A
26 MARCH 1980

SPT Blows/Ft

El 876.2
9 — Stopewash
ML — Clayey, olive, trace org, moist
5
CH — Dk gry-brn, moist-wet, trace org
4 — El 860.7 — CL — Pt — V dk gray, silty, moist

BC-14B
26 MARCH 1980

SPT Blows/Ft.

El 880.0
Stopewash
12 — CL — Lt olive, sandy, trace org
ML — Gry-brn, moist
11 — CL — Olive, little org, sandy
9 — El 864.5 — CL — V dk gry-brn, silty, m plast, till

BC-14C
26 MARCH 1980

SPT Blows/Ft | MC | LL | PL

El 891.9
14
15.4 — 31 — 15
25 — CL-SC — Olive brn, silty, till gravelly, sl plast
20
38 — El 866.4 — SM-SP — Dk brn, moist, trace of gravel

890
880
870
860

BC-15B
27 MARCH 1980

SPT Blows/Ft.

El 870.4
7 — CL — Dk gray-brn, org odor, sl plast, clayey
13
21 — ML — Olive brn-gry brn, low plast, low dry strength, moist
16
23 — 848.9 — SM — Dk brn, f sand, moist, some grav

BC-15C
27 MARCH 1980

SPT Blows/Ft.

El 871.7
SM — Gray-brn, f sand, organics, grav.
11
CL — Gray-brn, sandy, part org, m plast
17
CL — Dk olive gray, till, sandy, m. plast
15 — 857.7 — ML — Gray-brn, low plast., moist, low dry strength

BC-15D
28 MARCH 1980

SPT Blows/Ft | MC | LL | PL

El 868.0
CL — Topsoil
SC — Dk brn, wet, partly org
5
9
160 — CL-SC — 26 — 14 — Dk olive gray, partly org, sandy
WL 852.7
11 — ML — Dk gray, low dry strength, moist alluvium, sl plast
20
10 — El 837.0 — SC — Dk brn, moist

890
880
870
860
850
840
830

59M
ER 1981
LL PL
Topsoil
Grav, brn, moist, loose
33 15
Brn, silty, hard, moist, till
Brn, dry - moist, v dense
Gray, silty, moist, stiff

81-60M
DECEMBER 1981

SPT BLOWS/FT | MC | LL | PL

885.5
2.3 — MH — Clayey, stiff
49 — 11.4
51
12.3 — CL — 33 — 14 — Grn-brn, hard, v. dry, till, silty
64 — 12.9
39
15 — 864.5 — CL — Gray, moist, m. stiff - stiff

82-66M
13-14 JANUARY 1982

SPT BLOWS/FT | 885.7

Topsoil
14
CL — Grn-brn, till, silty, moist, m stiff iron staining, l. plast
22
32 — ML — Tan, dry, hard, alluvium
CL — Brn
ML — Tan
38
CL — Brn, silty, dry, stiff, alluvium
27
31 — CL — Brn-gray, silty, moist, alluvium
CL — DF.sand — Grn-brn, silty, moist-dry, alluvium
30 — ML — Red-brn, clayey, wet, m. dense
25
SM — Red-brn, gravelly, wet, alluvium
SW
59 — GP-GM — Dk red-brn, sat, dense, alluvium
168 — SP-SM — Brn, wet, grav, v. dense, alluvium
192 — SM
SP
ML — Red-brn, dry, v. dense, alluvium
177 — 827.7 — Till
GW

890
880
870
860
850
840
830

NOTE
BORING LEGEND IS SHOWN ON SHEET NO. C-11

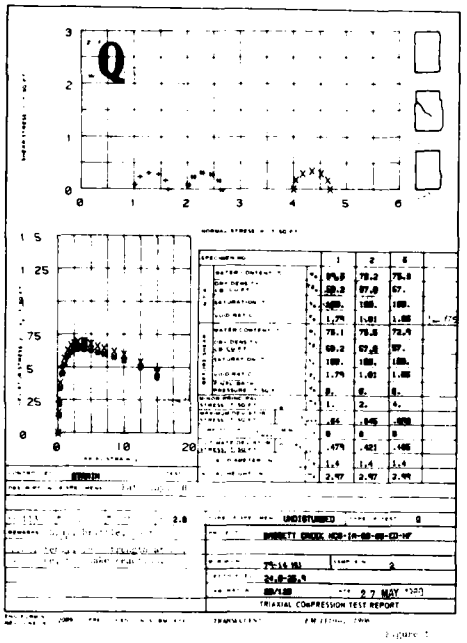
SYMBOL	DESCRIPTION	DATE	APPROVAL

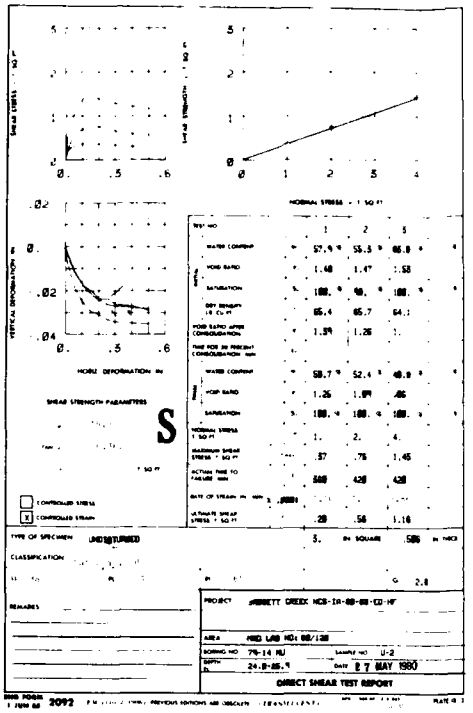
DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

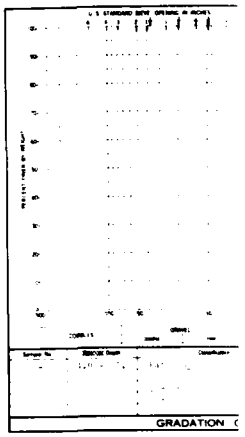
DESIGNED BY: BARR ENGR, D.W.R
DRAWN BY: J.G.J
CHECKED BY: B.M.
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
EDGEWOOD EMBANKMENT
BORING LOGS
DATE: AUGUST 1982
SCALE: AS SHOWN
DRAWING NUMBER
M34.3-R-5/244
SHEET C-10 OF C-39

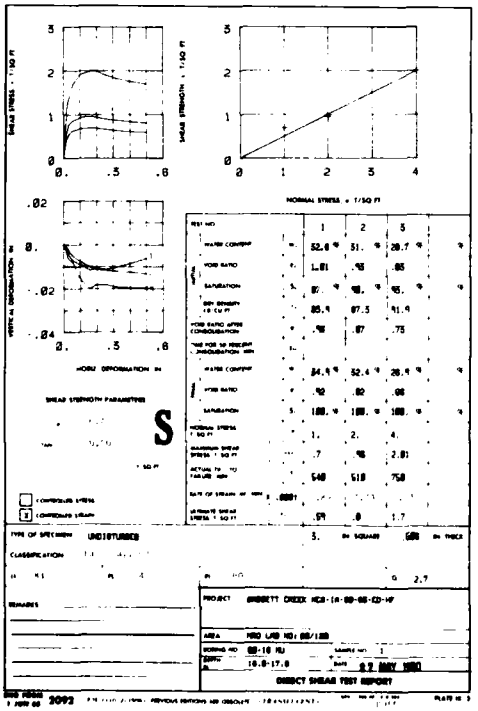
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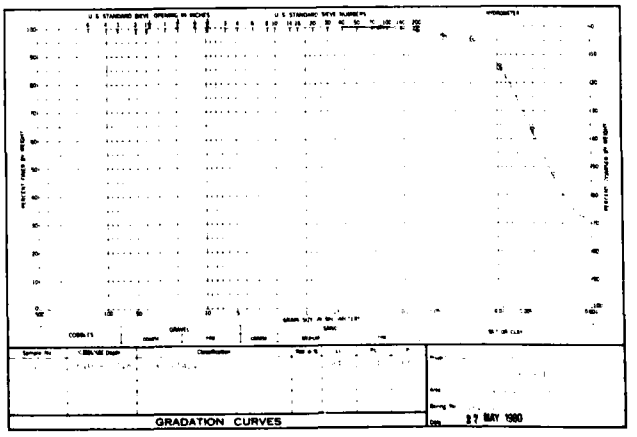




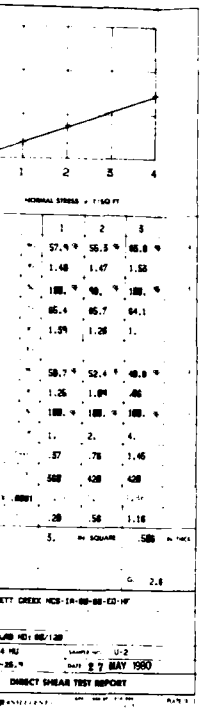


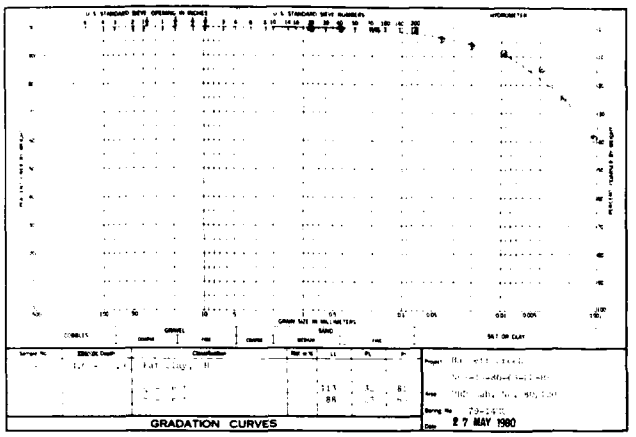
ENG 2087



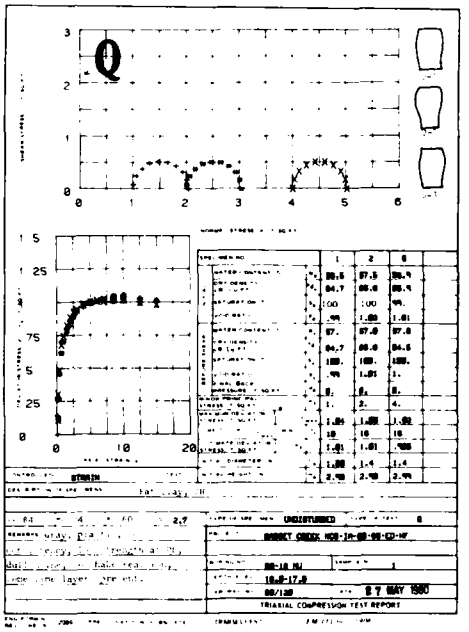


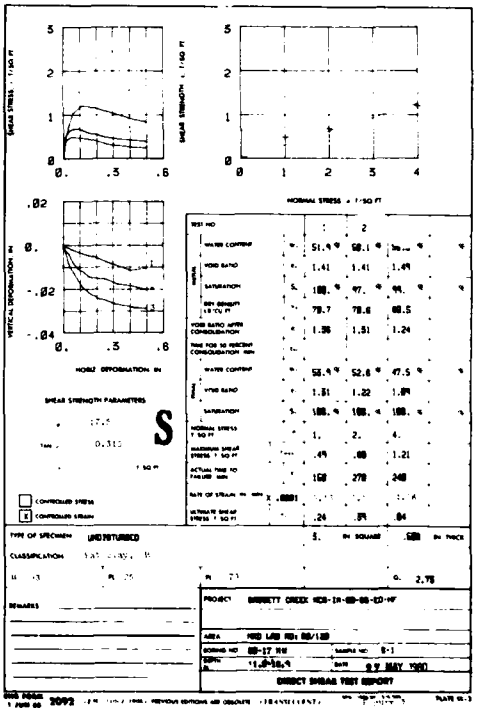
ENG 2087

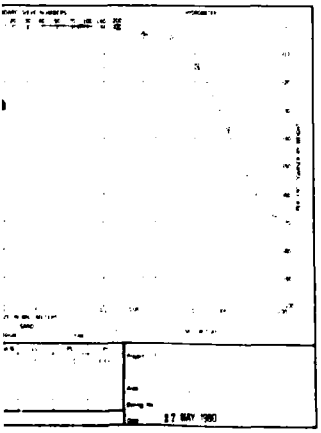


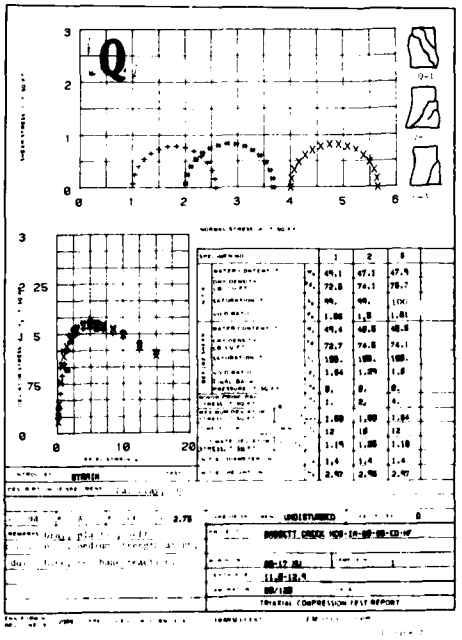


ENG [illegible], 2087









SAFETY DAYS

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

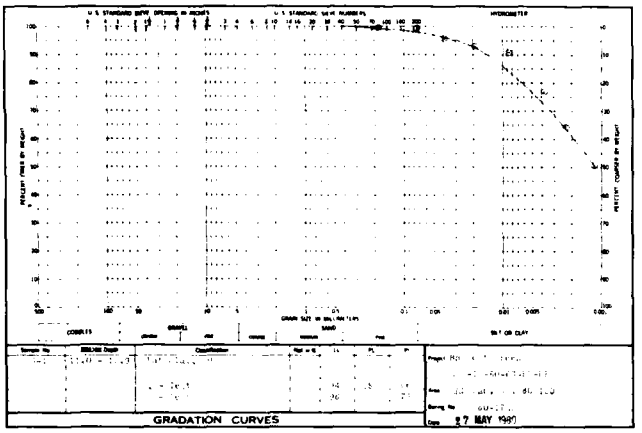
PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
TUNNEL ALIGNMENT
BORINGS 79-14MU, 80-16MU & 80-17MU

DESIGNED BY:
DRAWN BY:
TRACED BY:
SUBMITTED BY:
APPROVED:

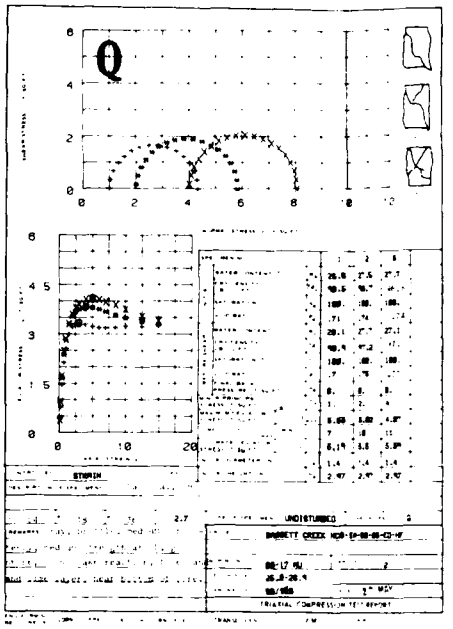
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AUGUST 1982

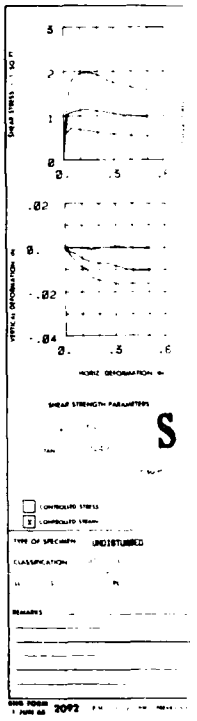
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M34.3-R-5/245
SHEET C-17 OF C-39

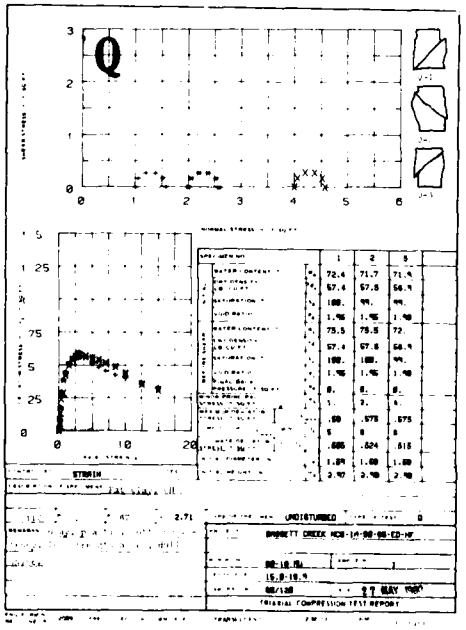
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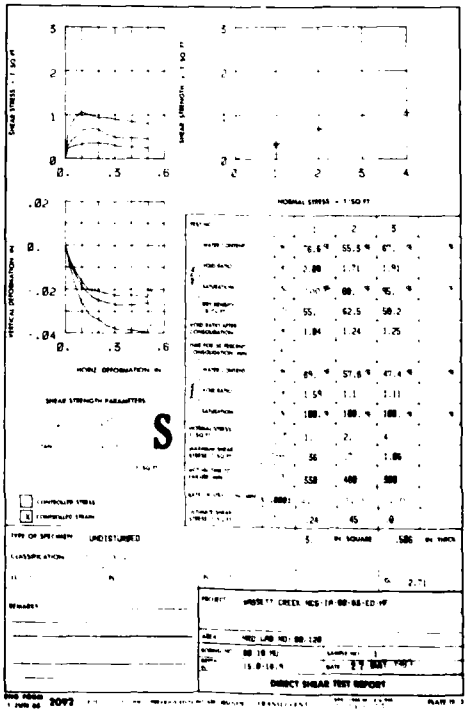

GRADATION CURVES
27 MAY 1987

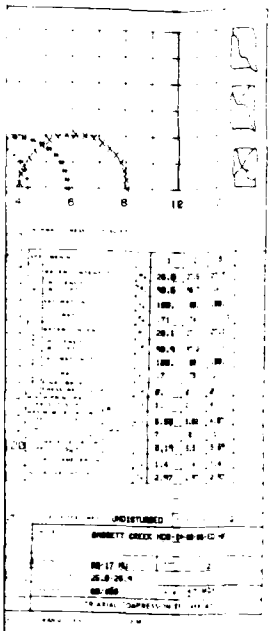
ENG 2087

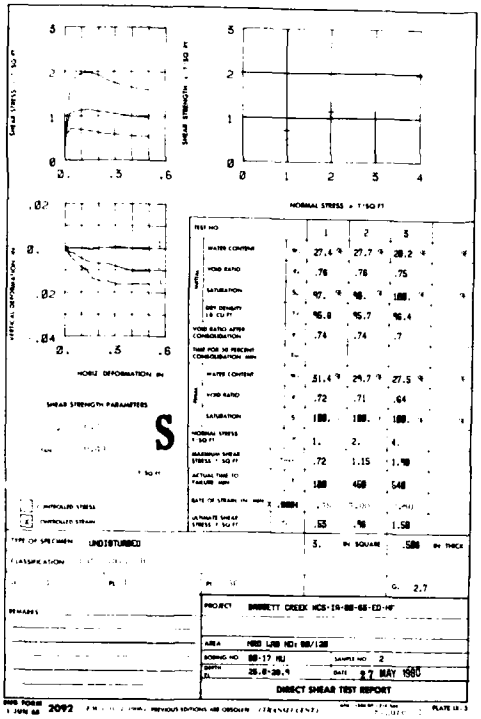

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UNDISTURBED
TRIAXIAL COMPRESSION TEST REPORT

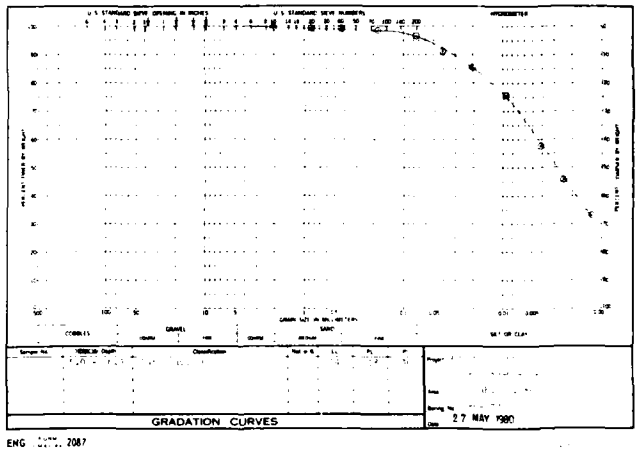

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2092


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TRIAXIAL COMPRESSION TEST REPORT

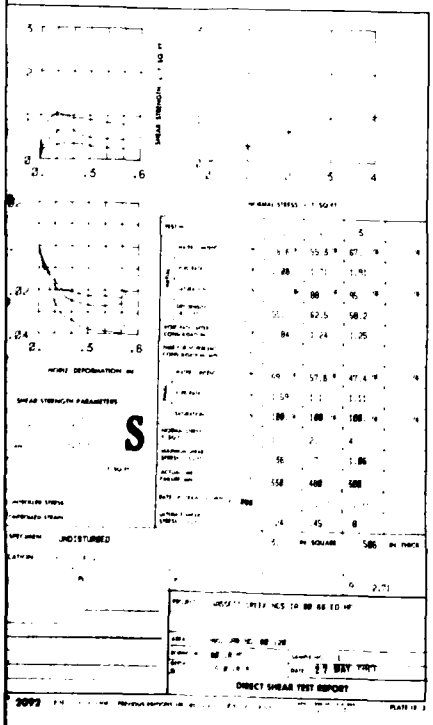

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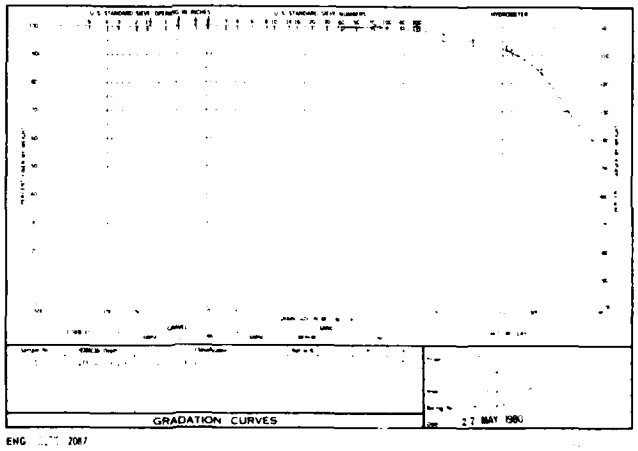






ENG FORM 2087





ENG FORM 2087

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
TUNNEL ALIGNMENT
BORINGS 80-17MU & 80-18MU

DESIGNED BY
DRAWN BY
CHECKED BY
SUBMITTED BY
APPROVED

DATE
AUGUST 1982

DRAWING NUMBER
M34.3-R-5/246
SHEET C-180F C-39

SAFETY PAYS

2.

SOIL CLASSIFICATION RECORD SHEET

Project | Boring No | MRD Lab No

Station | Range | Surf Elev | Depth To Water Table | Bottom Of Hole

Sample No	Depth To Bottom Of Sample	Moisture (%)	Plasticity (Att Limits) LL	PI	Grading (Cumulative Percents Finer)	Gradation Curve Analysis	Classification Tech MEMO 3-357, May 67	Remarks

MRD FORM NOV 75 16 EDITION OF MAY 70 IS OBSOLETE

SOIL CLASSIFICATION RECORD SHEET

Project | Boring No | MRD Lab No

Station | Range | Surf Elev | Depth To Water Table | Bottom Of Hole

Sample No	Depth To Bottom Of Sample	Moisture (%)	Plasticity (Att Limits) LL	PI	Grading (Cumulative Percents Finer)	Gradation Curve Analysis	Classification Tech MEMO 3-357, May 67	Remarks

MRD FORM NOV 75 16 EDITION OF MAY 70 IS OBSOLETE

SOIL CLASSIFICATION RECORD SHEET

Project | Boring No | MRD Lab No

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MRD FORM NOV 75 16 EDITION OF MAY 70 IS OBSOLETE

SOIL CLASSIFICATION RECORD SHEET

Project			Boring No.	MRD Lot No.
Station	Range	Surf. Elev.	Depth To Water Table	Bottom of Hole

Sample No.	Depth To Bottom of Sample	Moisture (%)	Physical Properties (Atterberg limits) L.L.	P.I.	Grading (Cumulative Percents Finer)	Gradation Curve Analysis D60	D30	D10	Cc	Classification Tech. MEMO 3-357, May 67	Remarks

MRD Form [illegible] EDITION OF MAY 70 IS OBSOLETE

SOIL CLASSIFICATION RECORD SHEET

Project			Boring No.	MRD Lot No.
Station	Range	Surf. Elev.	Depth To Water Table	Bottom of Hole

Sample No.	Depth To Bottom of Sample	Moisture (%)	Physical Properties (Atterberg limits) L.L.	P.I.	Grading (Cumulative Percents Finer)	Gradation Curve Analysis D60	D30	D10	Cc	Classification Tech. MEMO 3-357, May 67	Remarks

MRD Form [illegible] EDITION OF MAY 70 IS OBSOLETE

SAFETY DAYS

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY
DRAWN BY
CHECKED BY
SUBMITTED BY
APPROVED

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
TUNNEL, FRUENHOLZ, HWY 55 CONTROL STRUCTURE
BORINGS 80-15M TO 80-22M & 80-28M TO 80-44M

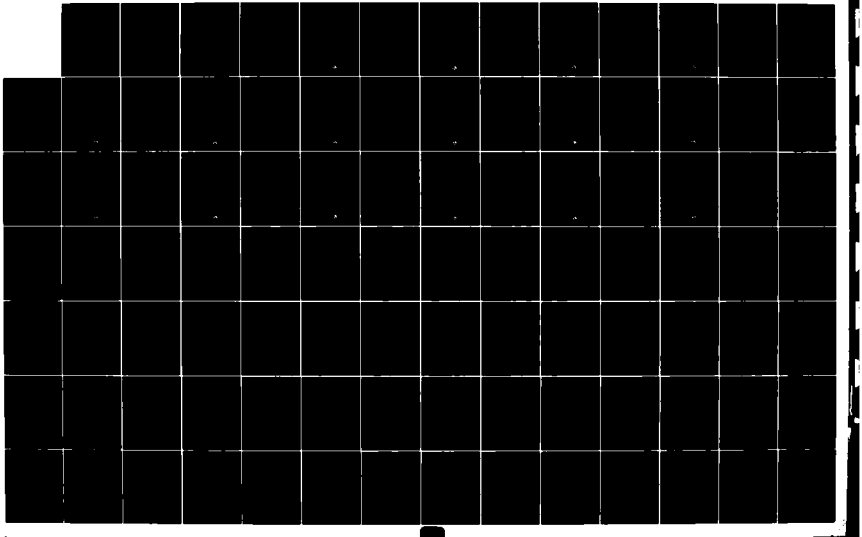
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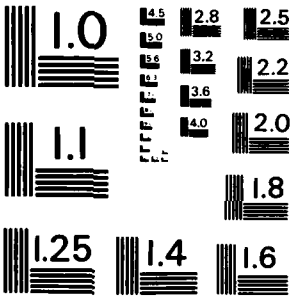
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M34.3-R-5/247

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AD-A133 795 BASSETT CREEK WATERSHED HENNEPIN COUNTY MINNESOTA FEASIBILITY REPORT FOR FLOOD CONTROL MAIN REPORT(U) CORPS OF ENGINEERS ST PAUL MN ST PAUL DISTRICT SEP 82 4/7

UNCLASSIFIED F/G 13/2 NL





MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS - 1963 - A

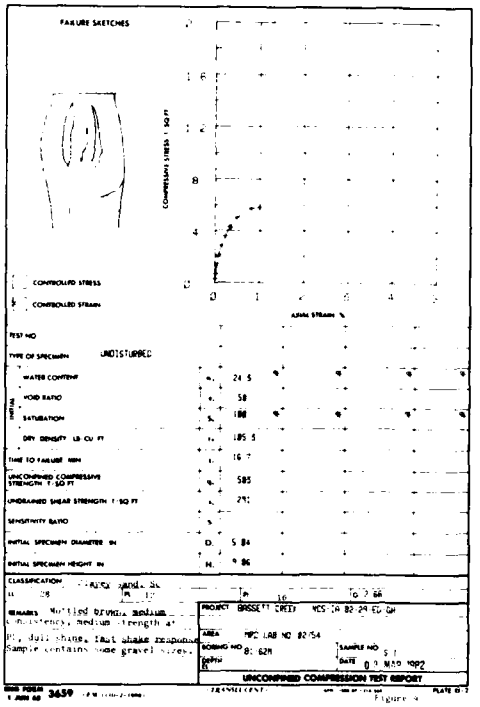
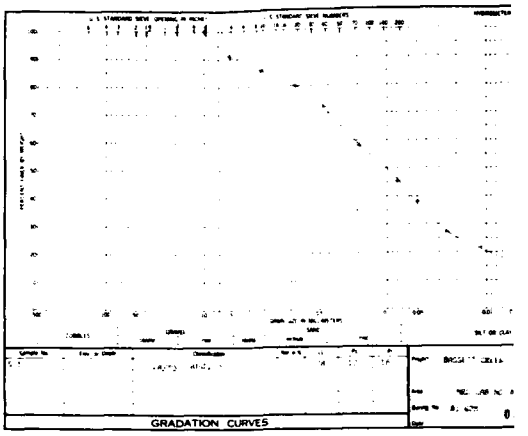
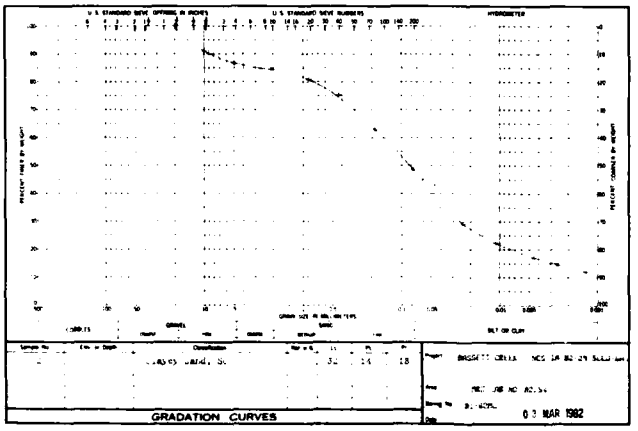


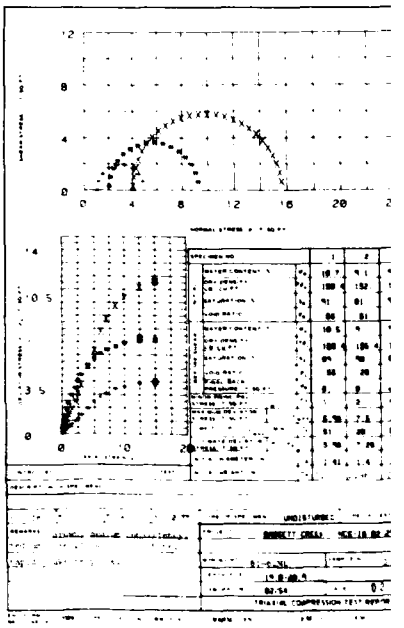
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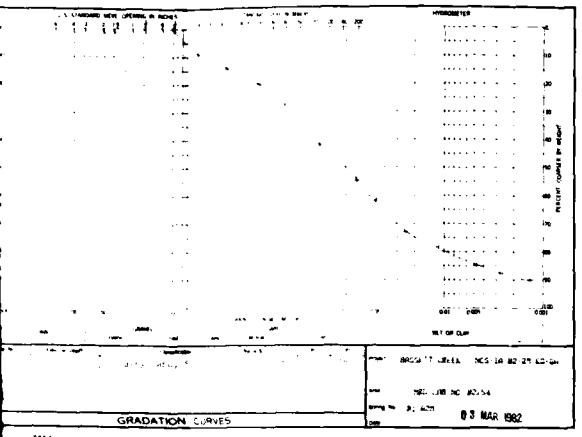


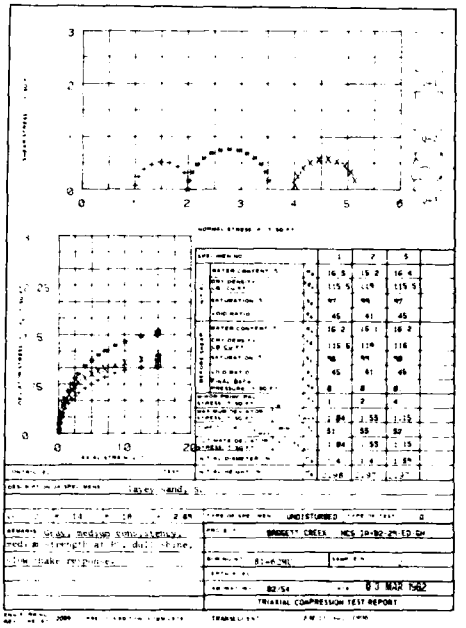
ENG 2087

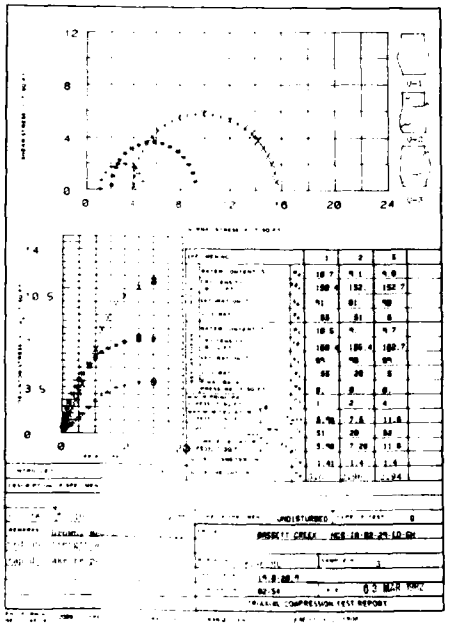


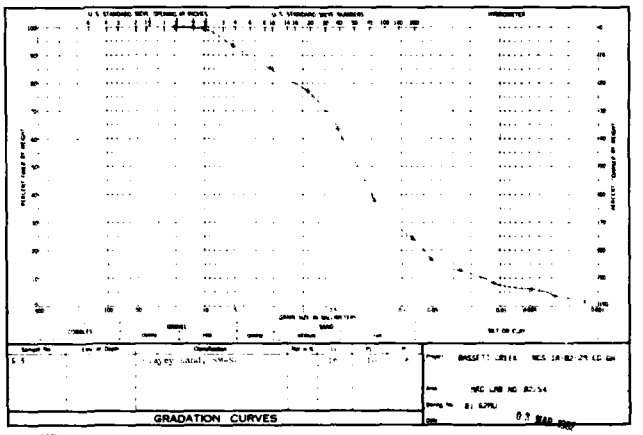
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DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

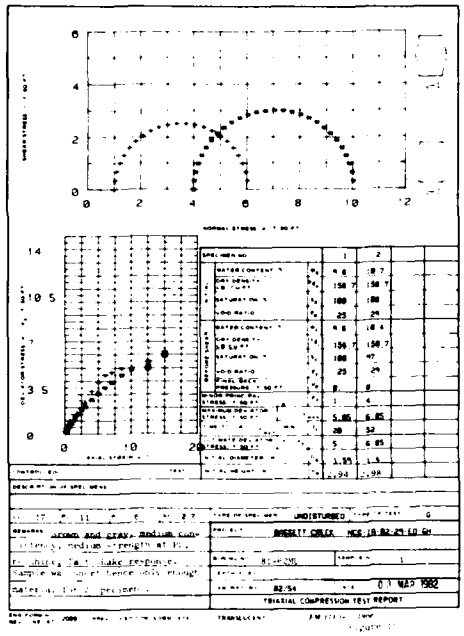
PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
FRUEN MILL BRIDGE
BORING 81-62MU

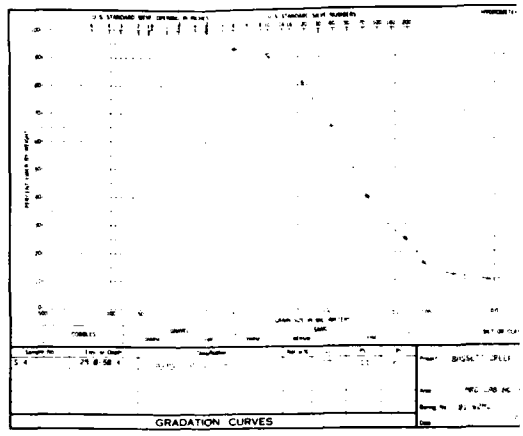
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AUGUST 982

DRAWING NUMBER
M34.3-R-5/248

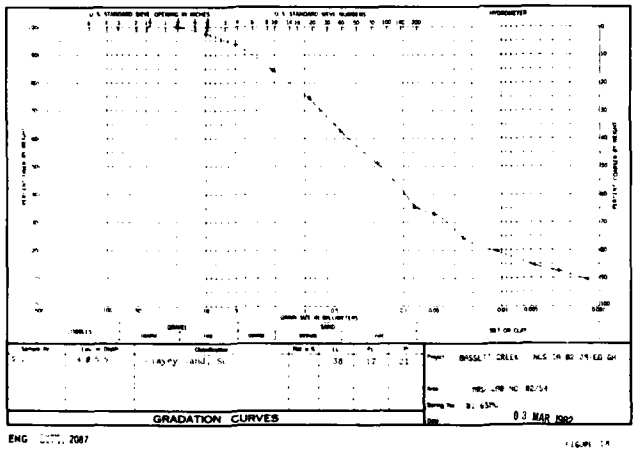
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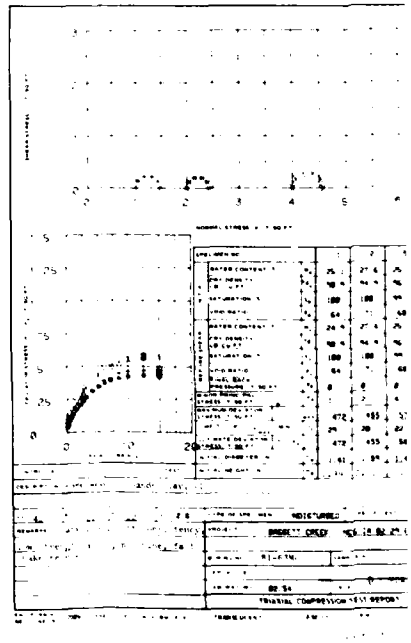

TRIAXIAL COMPRESSION TEST REPORT

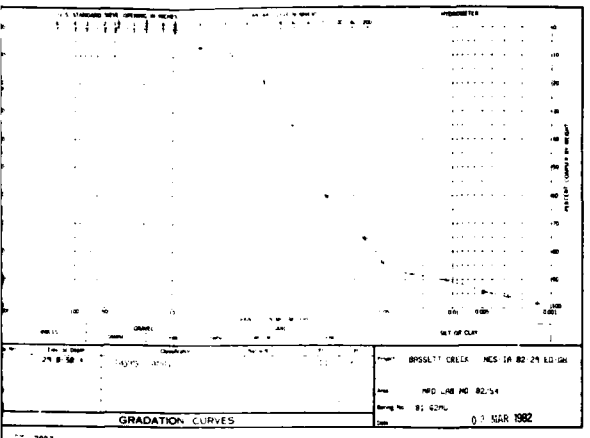

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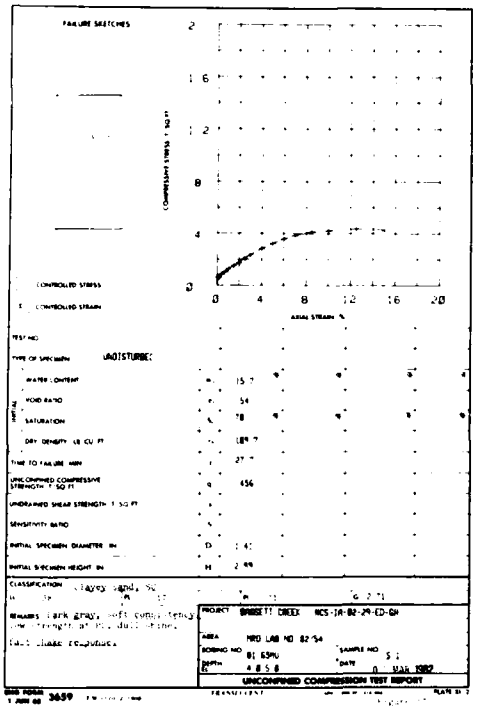
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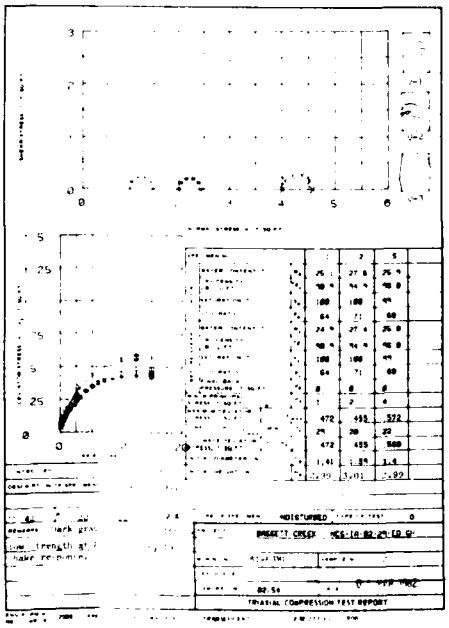

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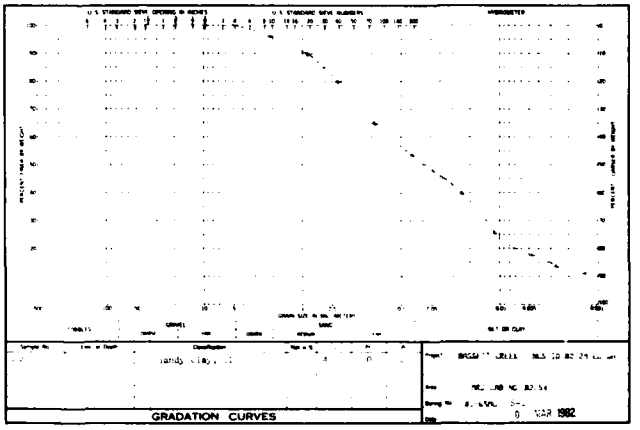
ENG











ENG 2087

SYMBOL	DESCRIPTION	DATE	APPROVED

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY:
DRAWN BY:
CHECKED BY:
SUBMITTED BY:
APPROVED:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
FRUEN MILL BRIDGE
BORINGS 81-62MU & 81-63MU

DATE:
AUGUST 1982

DRAWING NUMBER
M34.3-R-5/249
SHEET 2 OF 2 SHEETS

SAFETY PAYS

2

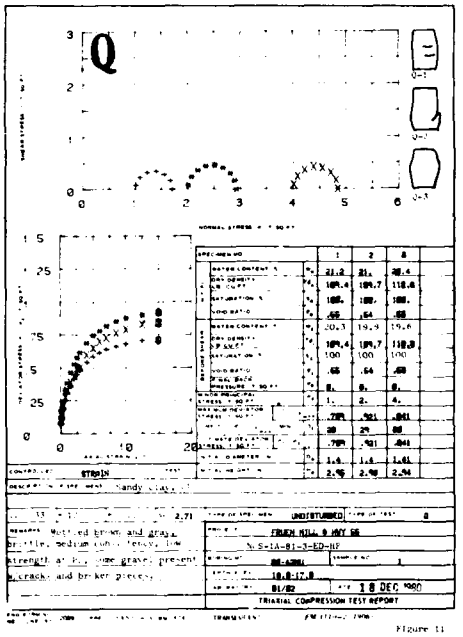
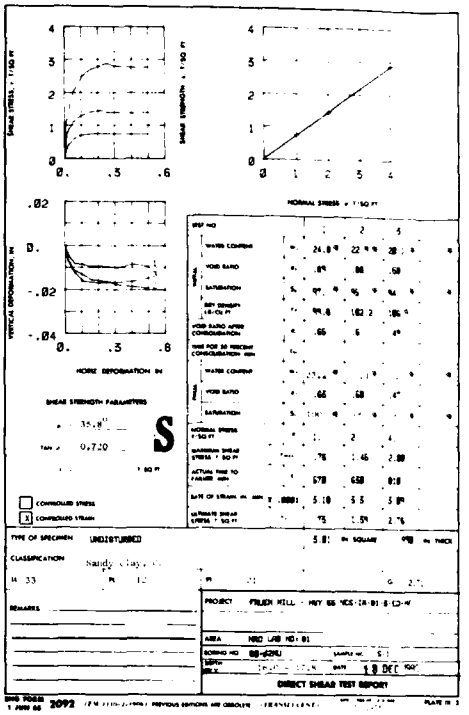
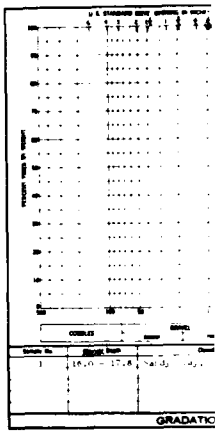


Figure 11





ENG

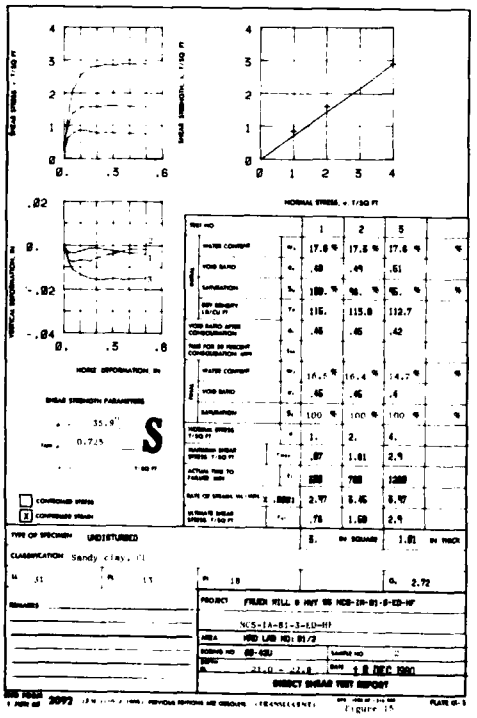
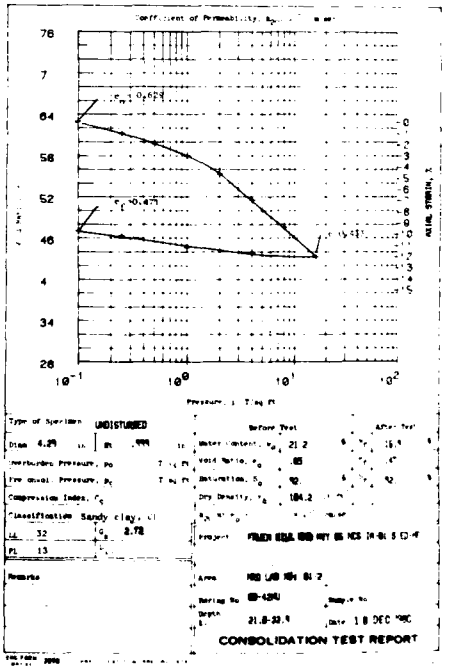
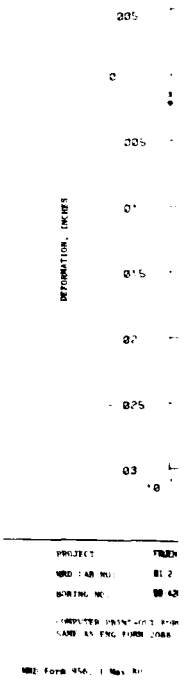
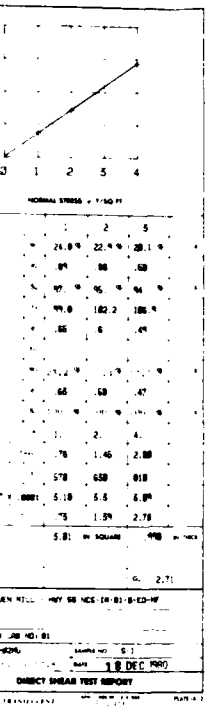


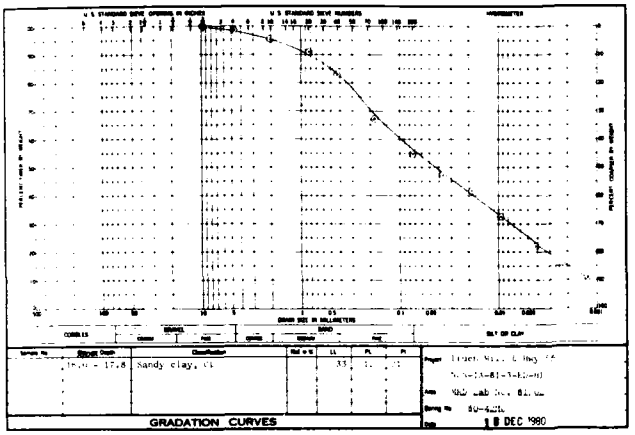
Figure 15





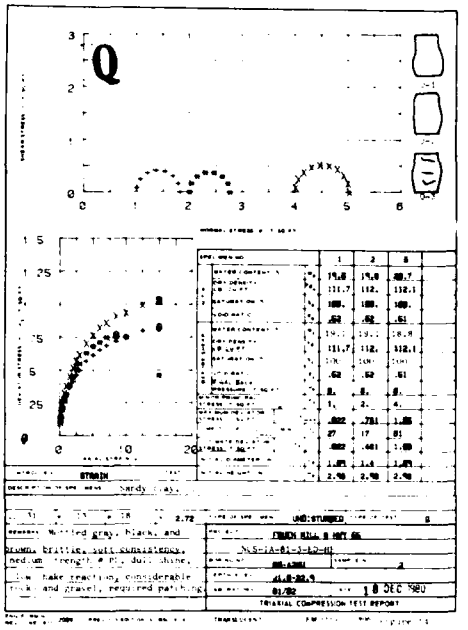


DIRECT SHEAR TEST REPORT

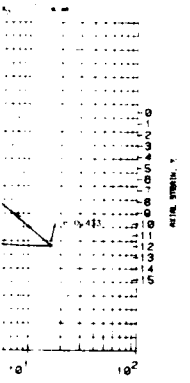


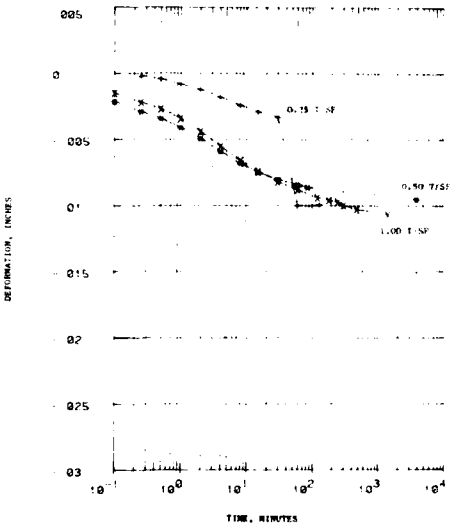
GRADATION CURVES

ENG Form 2087



TRIAXIAL COMPRESSION TEST REPORT





DEFORMATION, INCHES

TIME, MINUTES

PROJECT: FRUEH HILL AND HWY 55 HCS-IA-81-5-ED-HF

NRD LAB NO: 81-2

BORING NO: 80-42MU SAMPLE NO: DEPTH/ELEV: 21.8-22.4

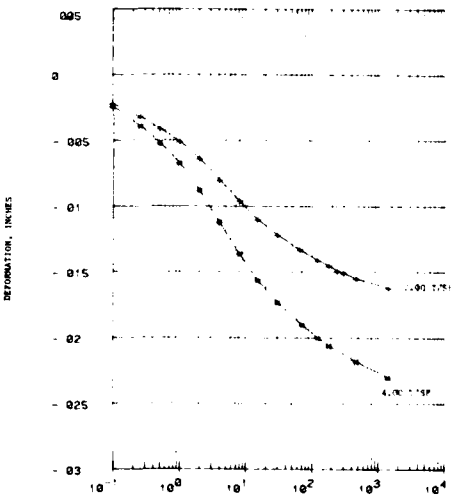
COMPUTER PRINT-OUT FORMAT SAME AS ENG FORM 2088

CONSOLIDATION TEST--TIME CURVES

FIGURE: 1A

DATE: 18 DEC 1980

NRD Form 956, 1 May 80



DEFORMATION, INCHES

TIME, MINUTES

PROJECT: FRUEH HILL AND HWY 55 HCS-IA-81-5-ED-HF

NRD LAB NO: 81-2

BORING NO: 80-42MU SAMPLE NO: DEPTH/ELEV: 21.8-22.4 ft

COMPUTER PRINT-OUT FORMAT SAME AS ENG FORM 2088

CONSOLIDATION TEST--TIME CURVES

FIGURE: 1A

DATE: 18 DEC 1980

NRD Form 956, 1 May 80

SAFETY PAYS

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

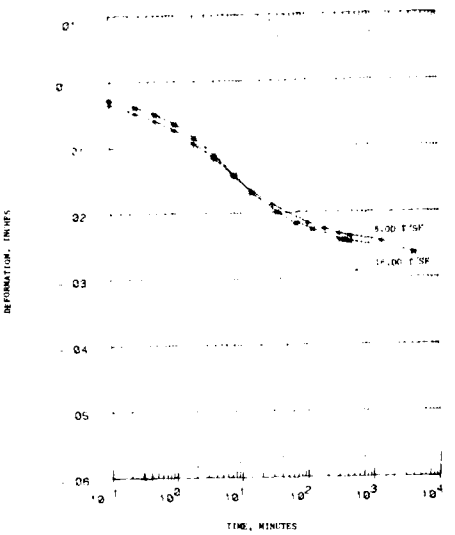
PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
HIGHWAY 55 CONTROL STRUCTURE
BORING 80-42MU

DESIGNED BY:
DRAWN BY:
CHECKED BY:
SUBMITTED BY:
APPROVED:

DATE: AUGUST 1982

DRAWING NUMBER
M34.3-R-5/250
SHEET C-22 OF C-39

2



PROJECT: FRUEN MILL AND HWY 66 NCS-IR-81-8-ED-HF

MRD LAB NO: 81-2 DATE: 18 DEC 1980

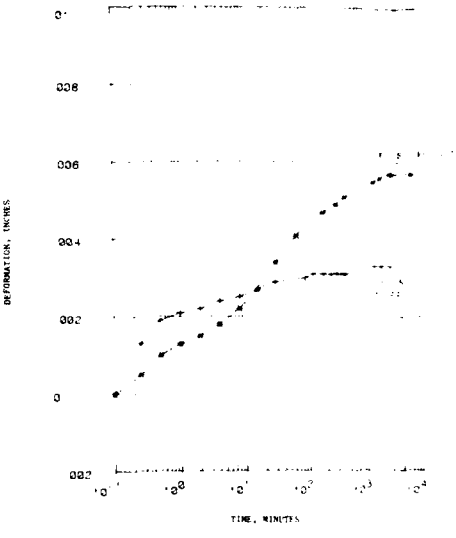
BORING NO: 80-4290 SAMPLE NO: DEPTH/ELEV: 21.8-22.4

COMPUTER PRINT-OUT FORMAT SAME AS ENG FORM 2088

CONSOLIDATION TEST—TIME CURVES

FIGURE:

MRD Form 956, 1 May 80



PROJECT: FRUEN MILL AND HWY 66 NCS-IR-81-8-ED-HF

MRD LAB NO: 81-2 DATE: 18 DEC 1980

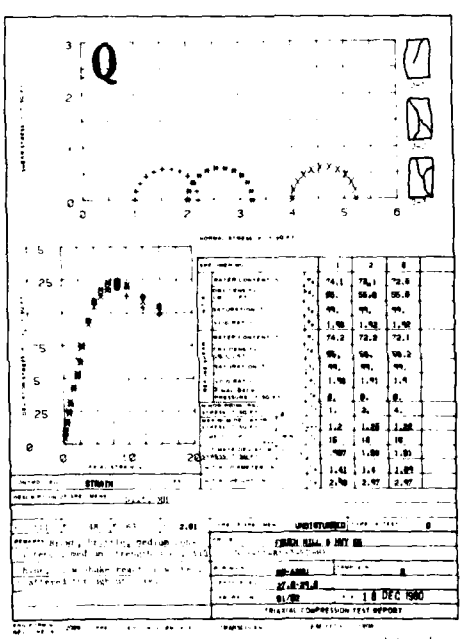
BORING NO: 80-4290 SAMPLE NO: DEPTH/ELEV: 21.8-22.4

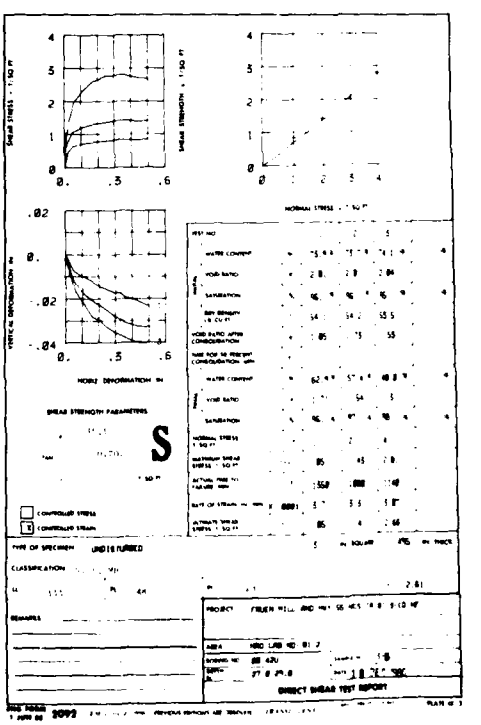
COMPUTER PRINT-OUT FORMAT SAME AS ENG FORM 2088

CONSOLIDATION TEST—TIME CURVES

FIGURE

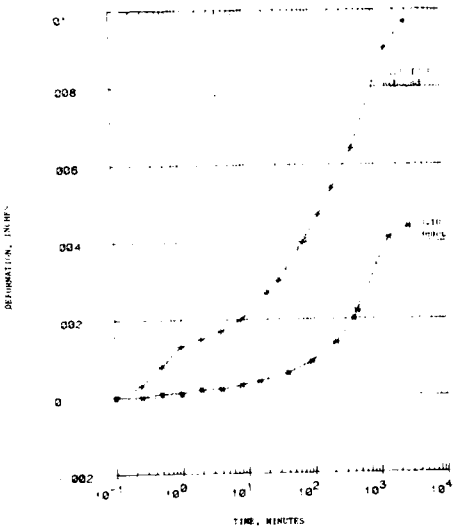
MRD Form 956, 1 May 80

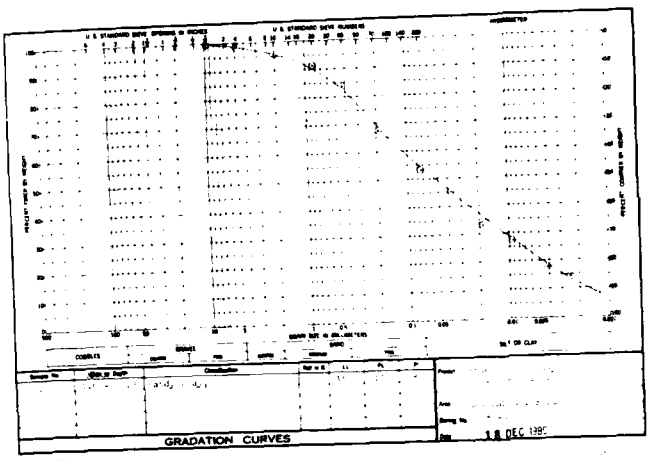






NOTES



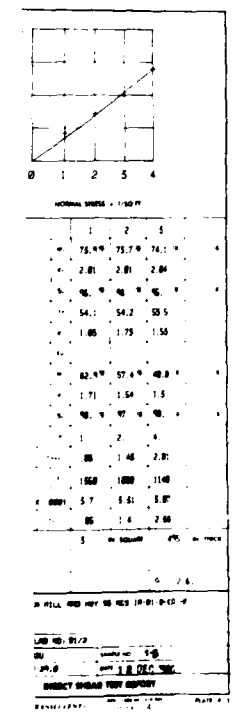


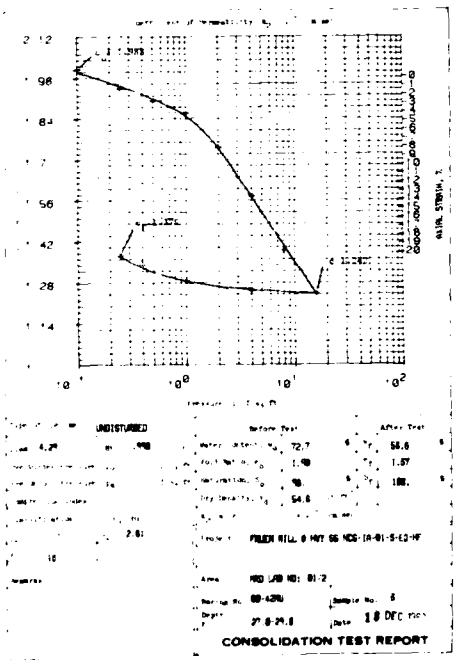
ENG Form 2087

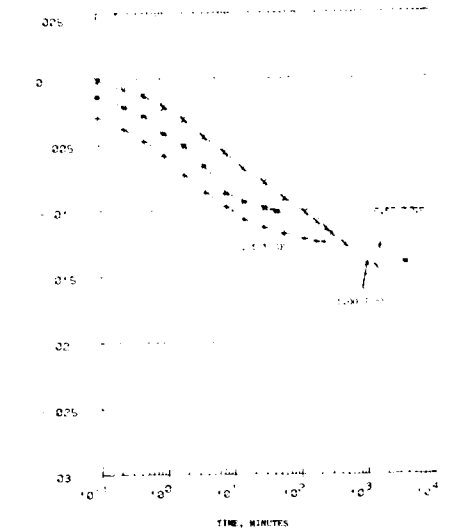
DATE: 18 DEC 1980
DEPTH/ELEV: 21.8-22.
CONSOLIDATION TEST--TIME CURVES
FIGURE:

PROJECT: FRUEN MILL RND HWY 55 MCS-IR-01-5-ED-HF
MRD LAB NO: 81:2
BORING NO: 80-42MU SAMPLE NO:
DATE: 18 DEC 1980
DEPTH/ELEV: 21.8-22.9
COMPUTER PRINT-OUT FORMAT SAME AS ENG FORM 2088
CONSOLIDATION TEST--TIME CURVES
FIGURE: 1

MRD Form 95b, 1 May 80







PROJECT: FRUEN MILL & HWY 55 MCS-IR-01-5-ED-HF
MRD LAB NO: 81:2
BORING NO: 80-42MU SAMPLE NO: 5
DATE: 18 DEC 1980
DEPTH/ELEV: 27.8-29.8
COMPUTER PRINT-OUT FORMAT SAME AS ENG FORM 2088
CONSOLIDATION TEST--TIME CURVES
FIGURE:

MRD Form 95h, 1 May 80

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY:
DRAWN BY:
CHECKED BY:
SUBMITTED BY:
APPROVED:

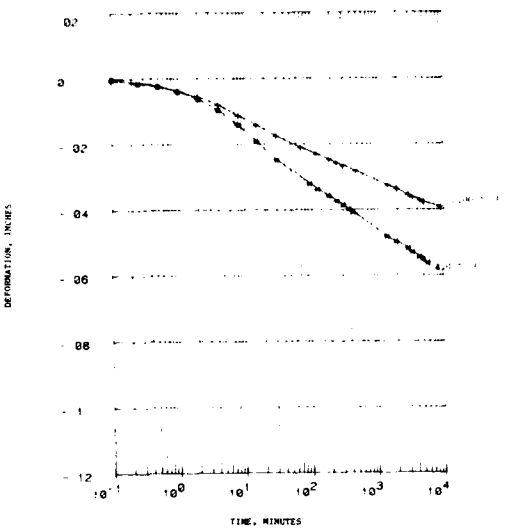
PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
HIGHWAY 55 CONTROL STRUCTURE
BORING 80-42MU

DATE: AUGUST 1982

DRAWING NUMBER
M34.3-R-5/251
SHEET 0-23 OF C-39

SAFETY PAYS

2



PROJECT: FRUEN MILL 8 MAY 85 NCS-IA-81-5-CO-HF

NRD LAB NO: 81/2 DATE: 18 DEC 1980

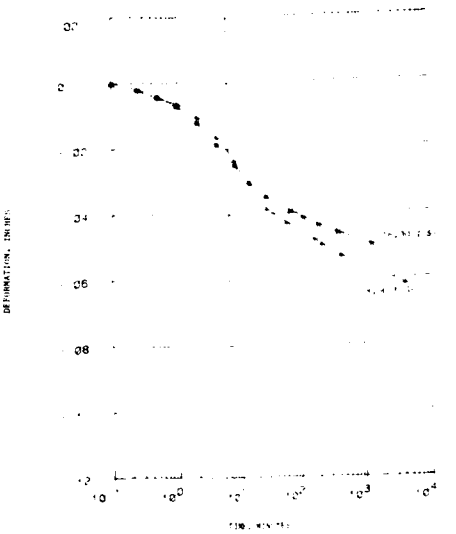
BORING NO: 80-42MU SAMPLE NO: 5 DEPTH/ELEV: 27.8-29.8

COMPUTER PRINT-OUT FORMAT
SAME AS ENG FORM 2088

CONSOLIDATION TEST—TIME CURVES

FIGURE: 27

NRD Form 956, 1 May 80



PROJECT: FRUEN MILL 8 MAY 85 NCS-IA-81-5-CO-HF

NRD LAB NO: 81/2 DATE: 18 DEC 1980

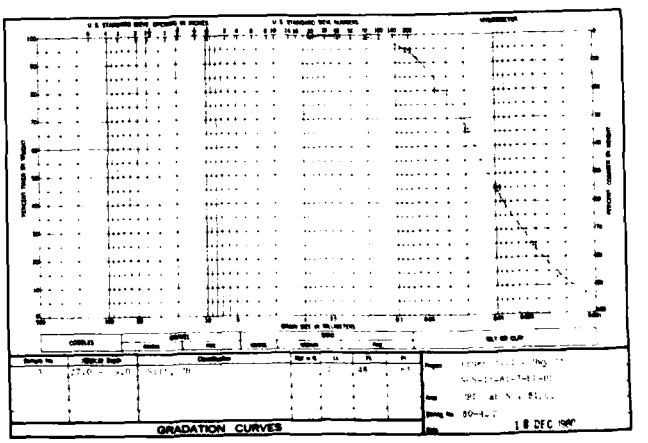
BORING NO: 80-42MU SAMPLE NO: 5 DEPTH/ELEV: 27.8-29.8

COMPUTER PRINT-OUT FORMAT
SAME AS ENG FORM 2088

CONSOLIDATION TEST—TIME CURVES

FIGURE: 27

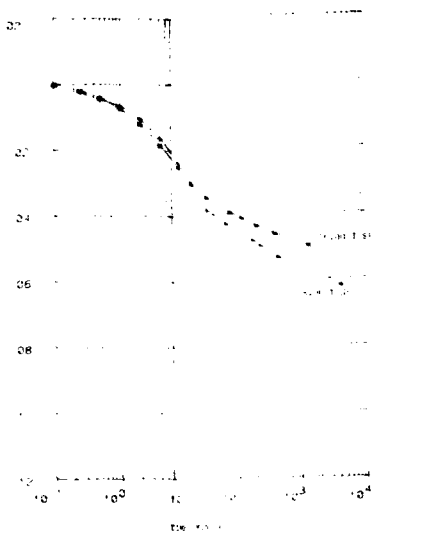
NRD Form 956, 1 May 80

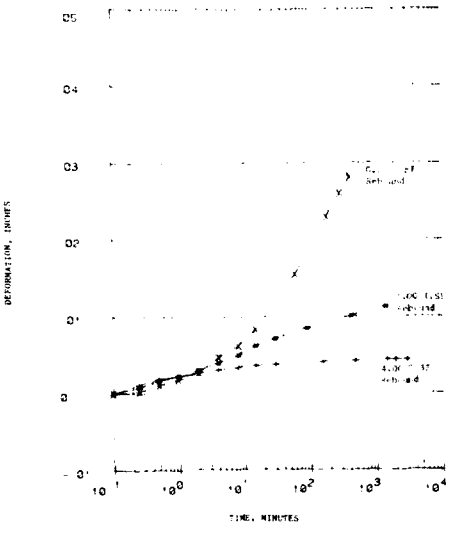


GRADATION CURVES

18 DEC 1980

ENG 2087





PROJECT: FRUEN MILL & HWY 55 MCS-IR-81-5-C2-HF

MRD LAB NO: 81/2 DATE: 18 DEC 1981

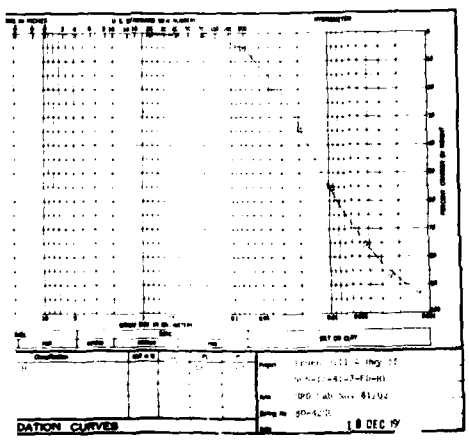
BORING NO: 80-42MU SAMPLE NO: 5 DEPTH/ELEV: 27.8-29.8

CONSOLIDATION TEST—TIME CURVES

COMPUTER PRINT-OUT FORMAT
SAME AS ENG FORM 2088

FIGURE:

MRD Form 956, 1 May 80



DATION CURVES

18 DEC 19

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY:
DRAWN BY:
CHECKED BY:
SUBMITTED BY:
APPROVED:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
HIGHWAY 55 CONTROL STRUCTURE
BORING 80-42MU

DATE:
AUGUST 1982

SCALE: GRID NO.

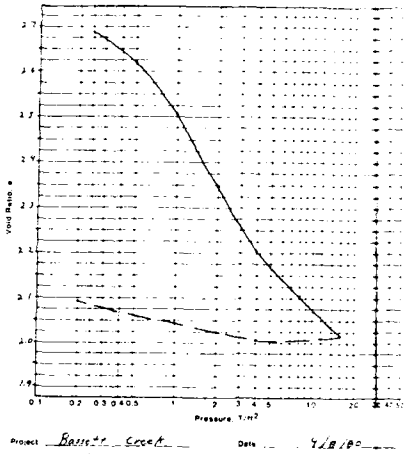
DRAWING NUMBER
M34.3-R-5/252
SHEET 0-24 OF C-39

SAFETY PAYS

Original Moisture Content = [illegible]%
Original Dry Density = [illegible] pcf
Plastic Index = 65 %
Specific Gravity = 2.56
C_c = 2.56

Loading	Void Ratio, e	Permeability, k (cm/sec)	C_v
.25 TSF	2.686		2.35×10^{-2}
.5 TSF	2.620		8.30×10^{-3}
1 TSF	2.502		2.40×10^{-3}
2 TSF	2.349		2.26×10^{-3}
4 TSF	2.159		2.50×10^{-3}
8 TSF	2.105		1.62×10^{-3}
16 TSF	2.017		2.46×10^{-3}
4 TSF (unloading)	2.084		
1 TSF (unloading)	2.044		
.25 TSF (unloading)	2.088		

CONSOLIDATION TEST
VOID RATIO VS PRESSURE

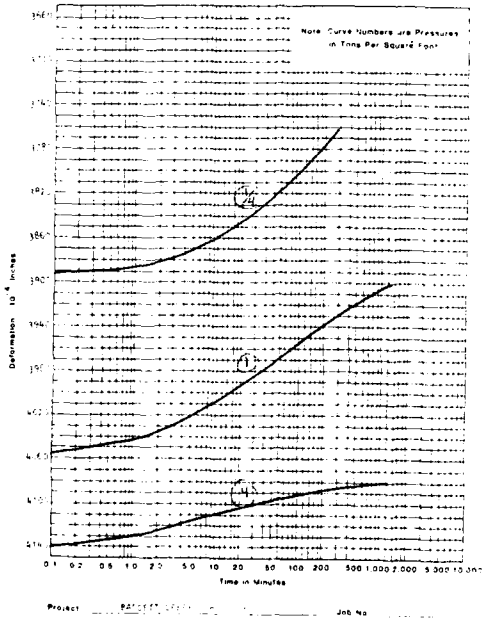


Project: Bassett Creek
Job No: 2362-144 ECR 04
Boring No.: BC-4A
Sample No.:
Depth: 192'-215'
Initial Sample Height: .75"
Sample Diameter: 2.5"
Description of Sample: Organic Muck

Date: 4/8/80
Tested By: MAR & JAJ
Original Moisture Content: 93.3 %
Original Dry Density: 42.2 pcf
Liquid Limit: 138.0 %
Plastic Limit: 65.6 %
Overburden Pressure, Po:
Preconsol. Pressure, Pc:
Compression Index, Cc: .45

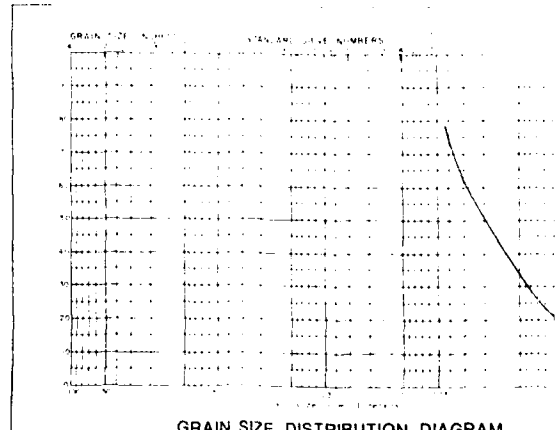
BARR ENGINEERING CO.

CONSOLIDATION TEST - TIME CURVES



Project: Bassett Creek
Job No:
Sample No: Organic Mucks
Boring No.: BC-4A
Depth: 192'-215'

BARR ENGINEERING CO.

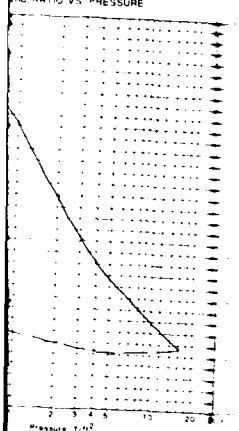


GRAIN SIZE DISTRIBUTION DIAGRAM

Project: Bassett Creek
Job No:
Sample No.: Organic Mucks
Boring No.: BC-4A
Depth: 192'-215'

ONSOLIDATION TEST

IO RATIO VS. PRESSURE



Date 4/8/82

Tested by MAR

Original Moisture Content 2

Original Dry Density 42.2

Liquid Limit 178 %

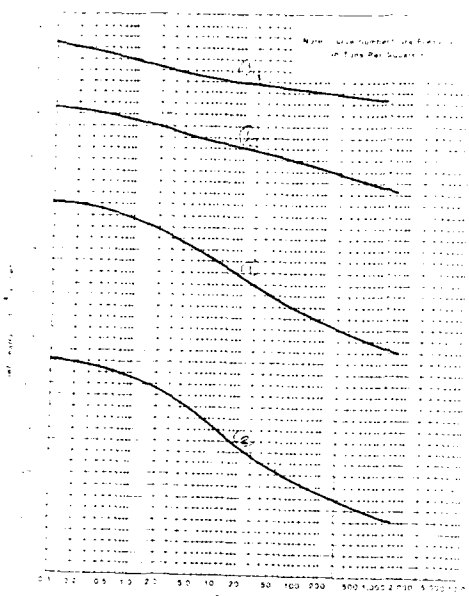
Plastic Limit 65 %

Overburden Pressure Po

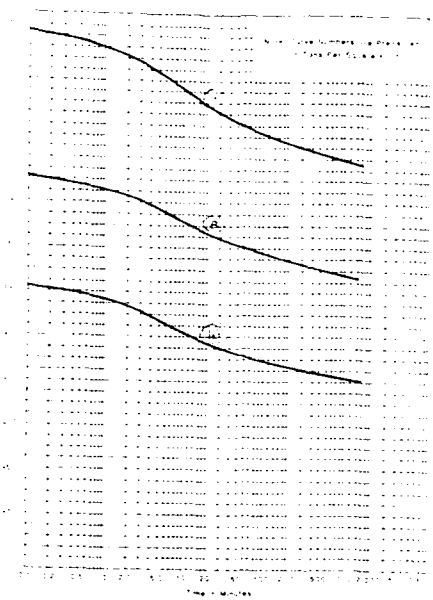
Preconsol Pressure Pc

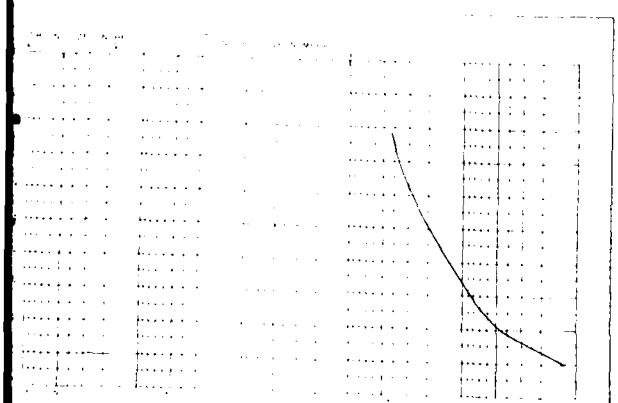
Compression Index Cc .4

CONSOLIDATION TEST - TIME CURVES

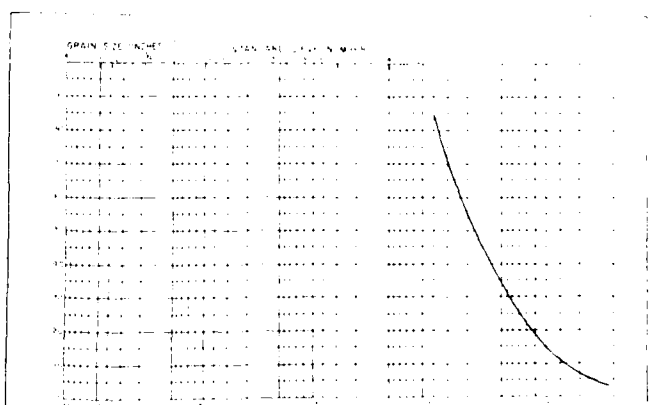


CONSOLIDATION TEST - TIME CURVES





GRAIN SIZE DISTRIBUTION DIAGRAM



GRAIN SIZE DISTRIBUTION DIAGRAM

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL MINNESOTA

DESIGNED BY

DRAWN BY

CHECKED BY

SUBMITTED BY

APPROVED

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
HIGHWAY 100 EMBANKMENT
BORING BC-4A

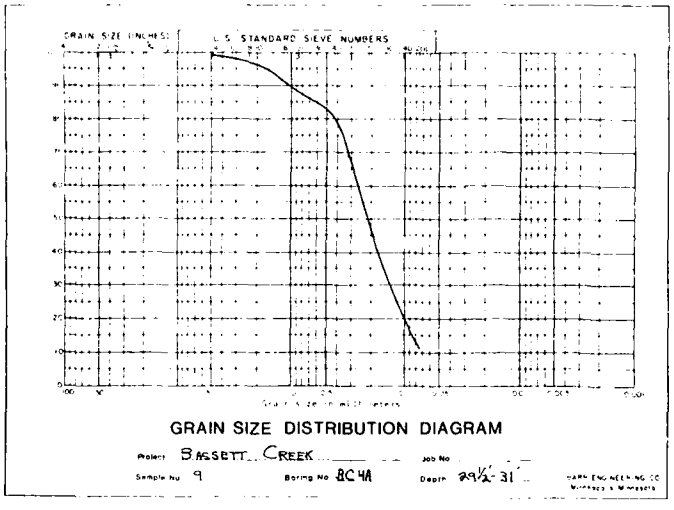
DATE
AUGUST 1982

DRAWING NUMBER
M34.3-R-5/253

SHEET C-25 OF C-39

SAFETY PAYS

2



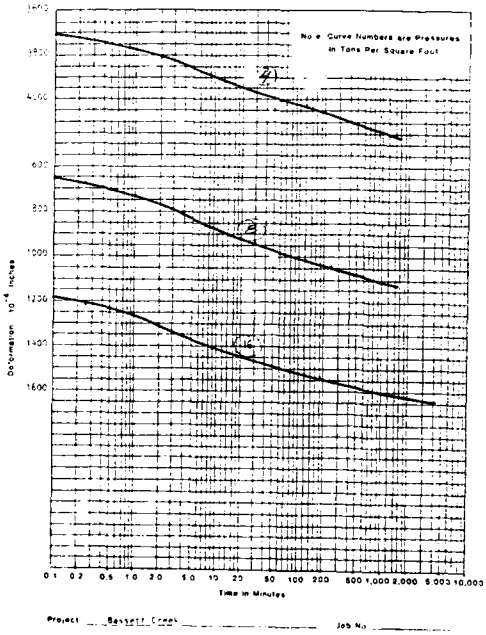
GRAIN SIZE DISTRIBUTION DIAGRAM

Project BASSETT CREEK Job No

Sample No 9 Boring No BC4A Depth 29½-31'

BARR ENGINEERING CO
Minneapolis, Minnesota

CONSOLIDATION TEST - TIME CURVES

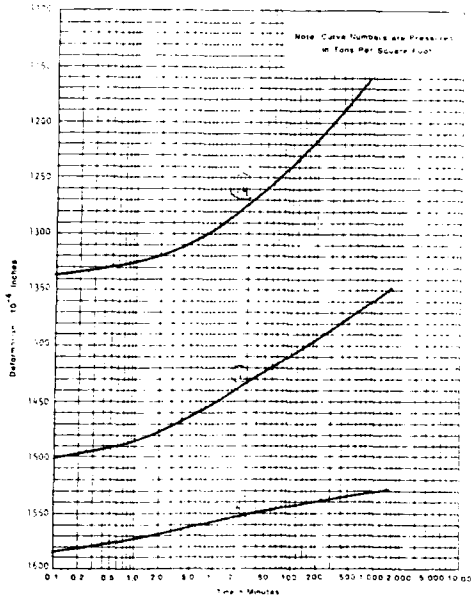


Project Bassett Creek Job No

Sample No BC4A Boring No BC4A Depth 14½ - 16½

BARR ENGINEERING CO
CONSULTING ENGINEERS

CONSOLIDATION TEST - TIME CURVES



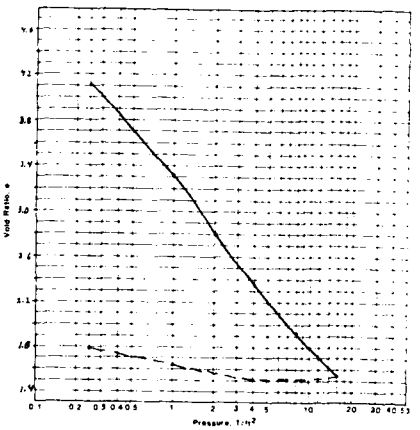
Project Bassett Creek Job No

Sample No BC4A Boring No BC4A Depth 14½ - 16½

BARR ENGINEERING CO
CONSULTING ENGINEERS

CONSOLIDATION TEST

VOID RATIO VS PRESSURE

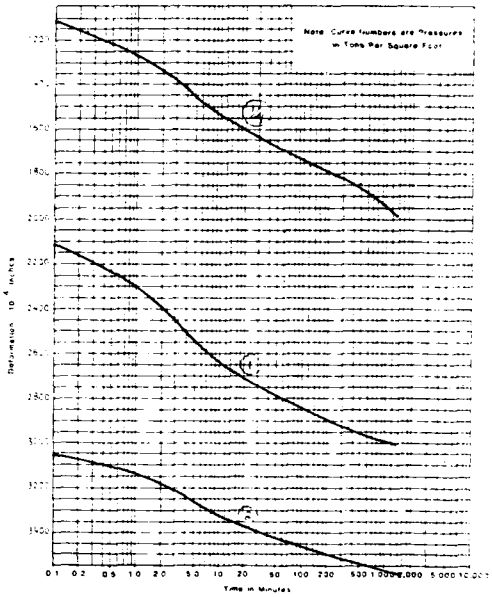


Project Bassett Creek
Job No 23-20-144 EGR 04
Boring No BC-4C
Sample No Thomas II
Depth 14½-16½
Initial Sample Height .75"
Sample Diameter 2.5"
Description of Sample Muck

Date 3/29/80
Tested by MAR + DAP
Original Moisture Content 191.0%
Original Dry Density 26 lb/ft³
Liquid Limit 153.0%
Plastic Limit 96.7%
Overburden Pressure, Po
Preconsol Pressure, Pc
Compression Index, Cc 1.5

BARR ENGINEERING CO

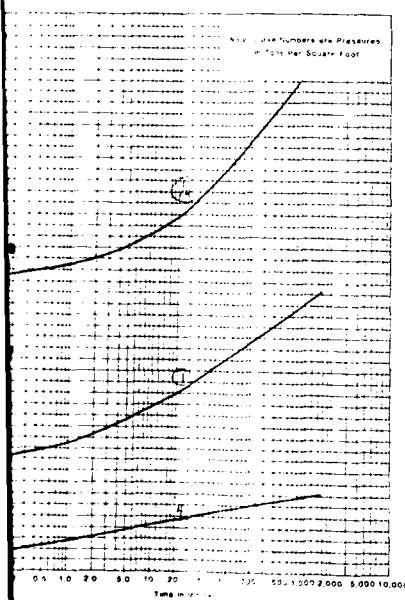
CONSOLIDATION TEST - TIME CURVES



Project Bassett Creek
Job No
Sample No
Boring No BC-4C
Depth

BARR ENGINEERING CO

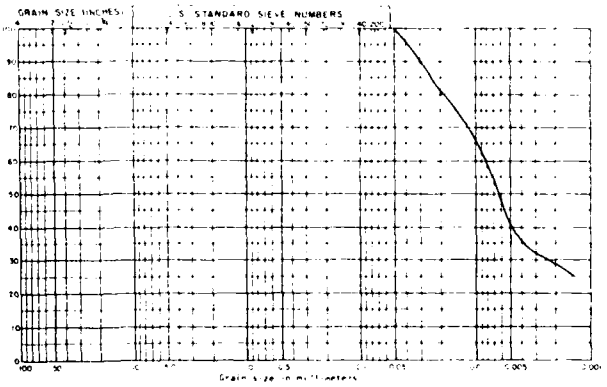
CONSOLIDATION TEST - TIME CURVES



Bassett Creek
Boring No
Depth

BARR ENGINEERING CO

GRAIN SIZE DISTRIBUTION DIAGRAM



Project Bassett Creek
Job No
Sample No Muck
Boring No BC4C
Depth 14½-16½

BARR ENGINEERING CO

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY:
DRAWN BY:
CHECKED BY:
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
HIGHWAY 100 EMBANKMENT
BORINGS BC-4A & BC-4C

DATE
AUGUST 1982

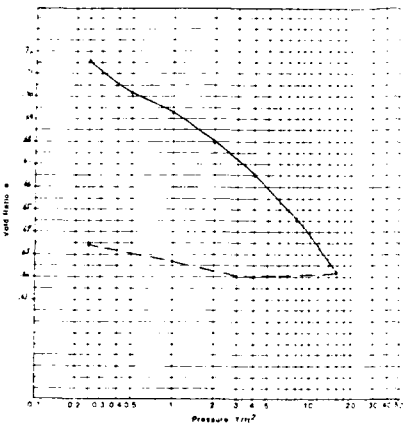
SCALE
DRAWING NUMBER
M34.3-R-5/254
SHEET 0-25 OF C-39

SAFETY PAYS

Original Moisture Content = [illegible]
Original Dry Density = [illegible]
Plastic Index = Non Plastic
C_c = .037
Specific Gravity = 2.75
Ash Content = 74.9%
Organic Content = 1.1%

Loading	Void Ratio e	Permeability (k)	C_v
.25 TSF	.717	1.68×10^{-5}	1.85×10^{-3}
.5 TSF	.702		2.31×10^{-3}
1 TSF	.693		2.26×10^{-3}
2 TSF	.681		2.25×10^{-3}
4 TSF	.665	6.84×10^{-5}	2.21×10^{-3}
8 TSF	.648		2.16×10^{-3}
16 TSF	.622		2.16×10^{-3}
4 TSF (unloading	.629		
1 TSF (unloading	.623		
.25 TSF (oil drained	.614		

CONSOLIDATION TEST
VOID RATIO VS PRESSURE

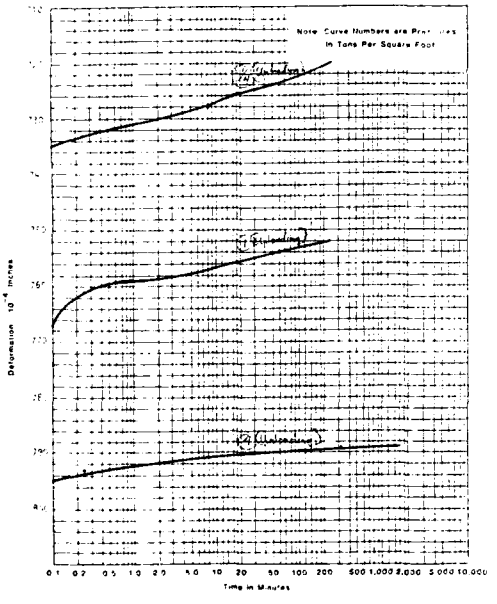


Project: Bassett Creek
Job No: A1127.177 E&P 04
Boring No: B-40
Sample No: Thomas II
Depth: 24'-26'
Initial Sample Height: 75"
Sample Diameter: 2.5"
Description of Sample: Organic Silt

Date: 3/4/80
Tested by: MAR & DA
Original Moisture Content: 76.6%
Original Dry Density: 95.2 #/ft³
Liquid Limit: 26.5%
Plastic Limit: 27.8%
Overburden Pressure Po:
Preconsol. Pressure Pc:
Compression Index C_c: .037

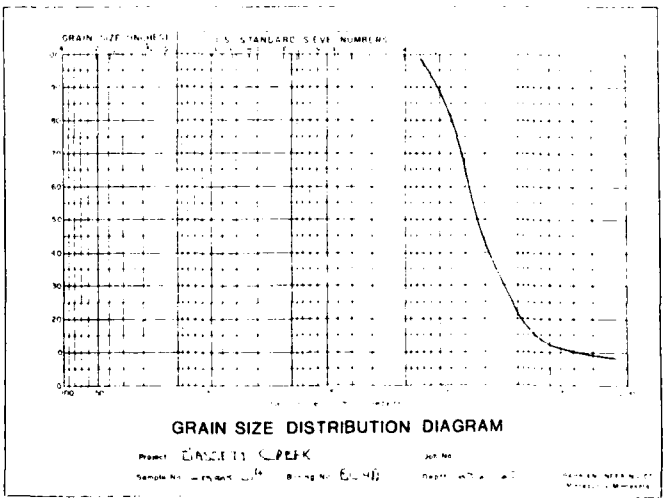
BARR ENGINEERING CO.

CONSOLIDATION TEST - TIME CURVES



Project: Bassett Creek
Job No:
Sample No: Grab - S-12
Boring No: B-40
Depth: 22' - 27'

BARR ENGINEERING CO.

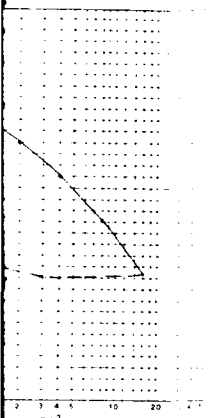


GRAIN SIZE DISTRIBUTION DIAGRAM

Project: BASSETT CREEK
Job No:
Sample No: GRAB S-12
Boring No: B-40
Depth:

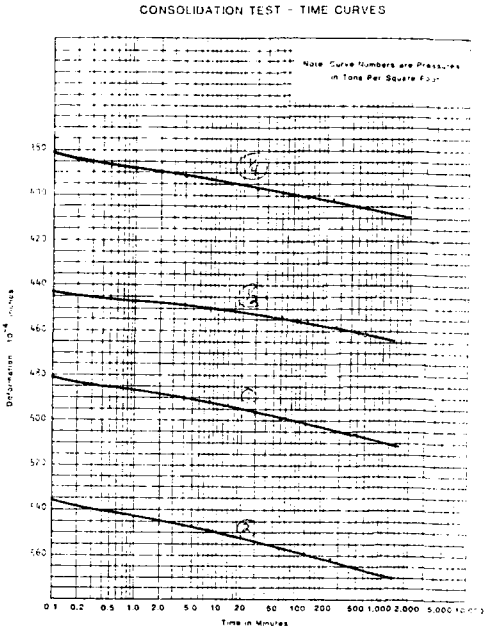
BARR ENGINEERING CO.

IDATION TEST
TIO VS PRESSURE



Date 3/4/82
Tested by MAR + LC
Original Moisture Content 31 %
Original Dry Density 95.2 pcf
Liquid Limit 26.5 %
Plastic Limit 21.8 %
Overburden Pressure, Po
Preconsol. Pressure, Pc
Compression Index, Cc .037

CONSOLIDATION TEST - TIME CURVES

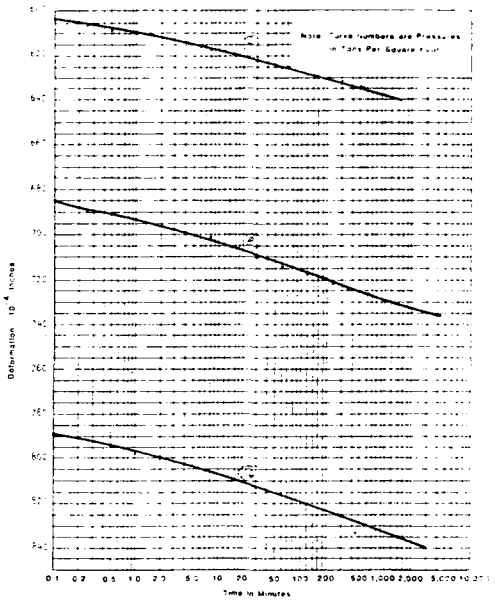


Project Bassett Creek
Job No
Sample No Organic Silt
Boring No BC-4D
Depth 24½ - 27

22

BARR ENGINEERING CO.

CONSOLIDATION TEST - TIME CURVES

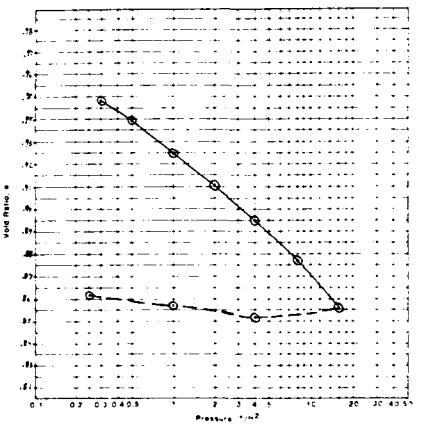


Project Bassett Creek
Job No
Sample No Organic Silt
Boring No BC-4D
Depth 24½ - 27

23

BARR ENGINEERING CO.

CONSOLIDATION TEST
VOID RATIO VS. PRESSURE

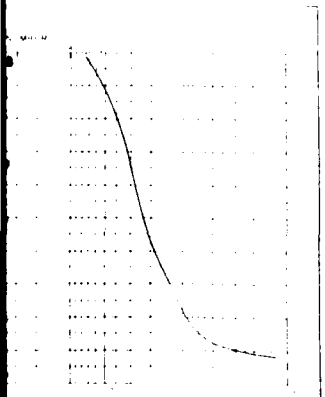


Project Bassett Street
Job No 23,127.125 C&B 04
Boring No BC-41
Sample No Thinwall
Depth 17 - 19½
Initial Sample Height .75"
Sample Diameter 2.5"
Description of Sample Silty Muck

Date 3/27/82
Tested by MAR + DC
Original Moisture Content 22.9%
Original Dry Density 84.3 pcf
Liquid Limit 56 %
Plastic Limit 33 %
Overburden Pressure, Po
Preconsol. Pressure, Pc
Compression Index, Cc .054

BARR ENGINEERING CO.

BUTION DIAGRAM



Job No
Depth 49½

Original Moisture Content
Original Dry Density = 84.3 pcf
Plastic Index = 23
Cc = .054
Specific Gravity = 2.64
Ash Content 32.0%
Organic Matter = 2.7%

Loading	Void Ratio, e	Permeability, k	Cv
.25 TSF	.948		
.5 TSF	.939		
1 TSF	.935		
2 TSF	.911		
4 TSF	.895		
8 TSF	.857		
16 TSF	.856		
4 TSF (unloading)	.862		
1 TSF (unloading)	.862		
2 TSF (unloading	.862		

24

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY
DRAWN BY
CHECKED BY
SUBMITTED BY
APPROVED

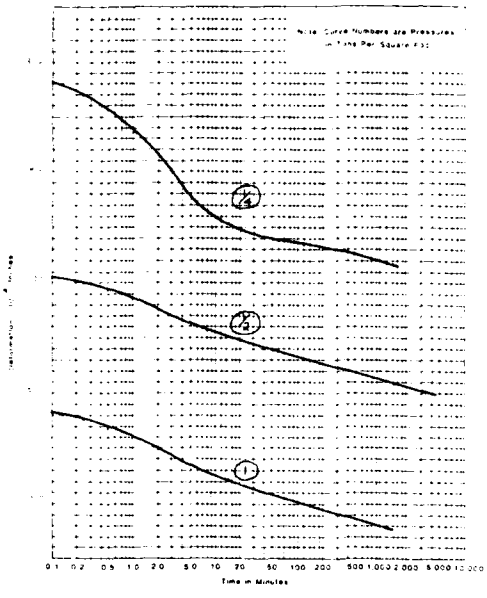
PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
HIGHWAY 100 EMBANKMENT
BORINGS BC-4D & BC-41

DATE: AUGUST 1982
SCALE
SPEC NO

DRAWING NUMBER
M34.3-R-5/255
SHEET C-27 OF C-39

SAFETY PAYS

CONSOLIDATION TEST - TIME CURVES

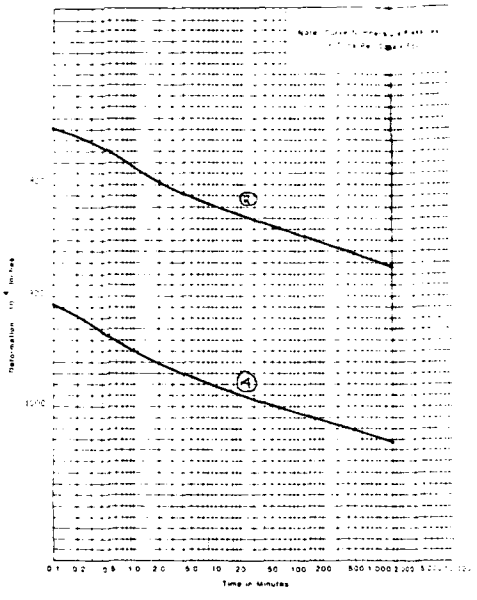


Project Bassett Creek Job No

Sample No Silt-Muck Boring No BC-4E Depth

BARR ENGINEERING CO

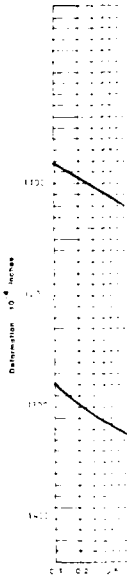
CONSOLIDATION TEST - TIME CURVES



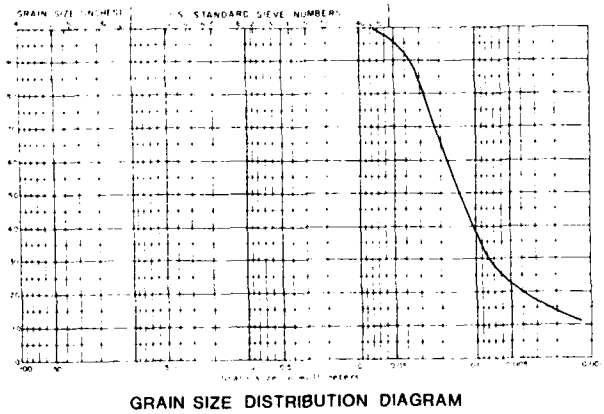
Project Bassett Creek Job No

Sample No Silt-Muck Boring No BC-4E Depth

BARR ENGINEERING CO



GRAIN SIZE DISTRIBUTION DIAGRAM

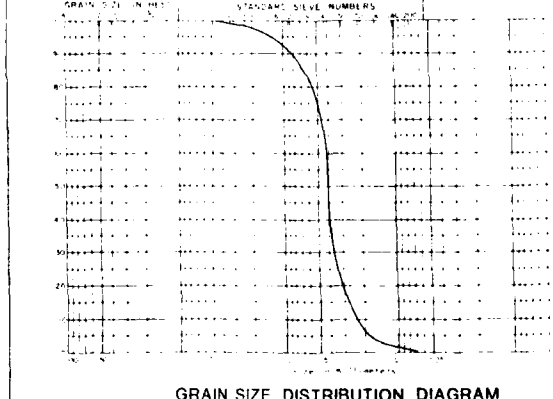


Project BASSETT CREEK Job No

Sample No Silt-Muck Boring No BC-4E Depth 17'-19½'

BARR ENGINEERING CO

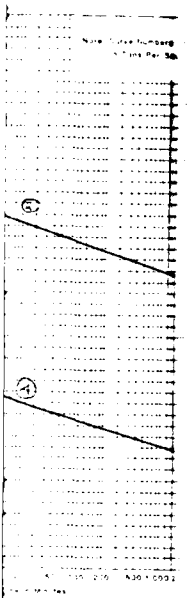
GRAIN SIZE DISTRIBUTION DIAGRAM



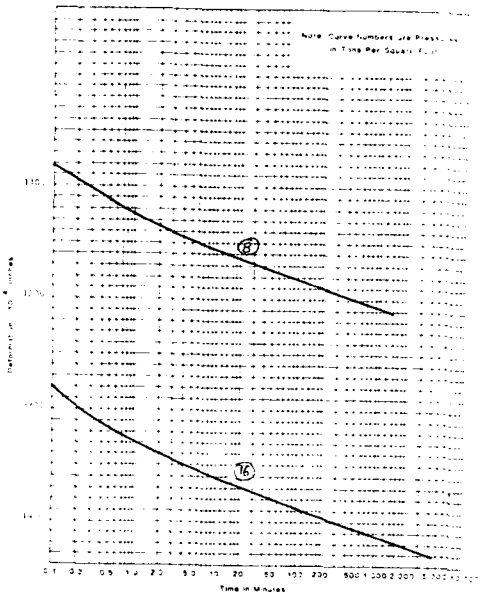
Project BASSETT CREEK Job No

Boring No BC-4E Depth 19½'-21'

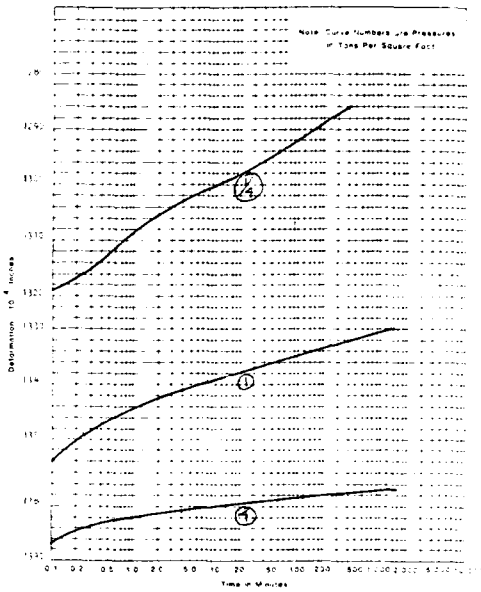
CONSOLIDATION TEST - TIME CURVES

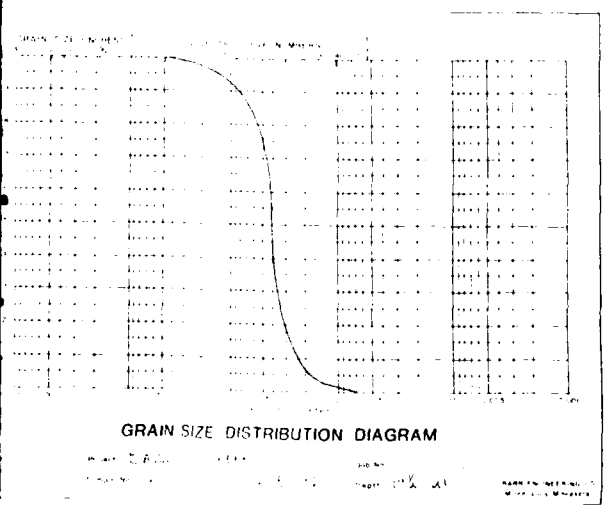


CONSOLIDATION TEST - TIME CURVES

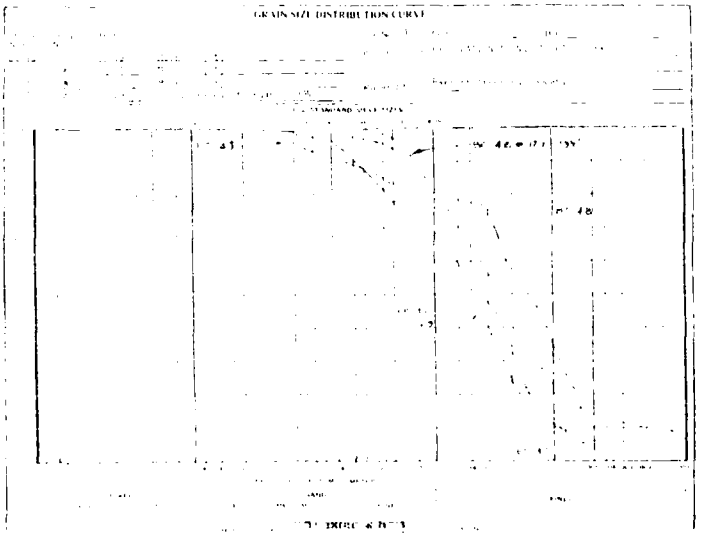


CONSOLIDATION TEST - TIME CURVES





GRAIN SIZE DISTRIBUTION DIAGRAM



SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL MINNESOTA

DESIGNED BY
DRAWN BY
TRACED BY
SUBMITTED BY
APPROVED

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
HIGHWAY 100 EMBANKMENT
BORINGS BC-4I, BC-4C, BC-4J & BC-4Z

DATE
AUGUST 1982

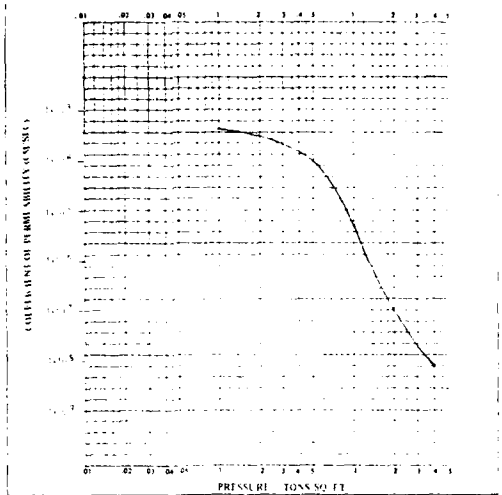
DRAWING NUMBER
M34.3-R-5/256

SHEET C-28 OF C-39

SAFETY PAYS

PERMEABILITY TEST CURVE

PERMEABILITY VS. PRESSURE



PRESSURE - TONS/SQ. FT.

Project

Job No.

Boring No.

Sample No.

Depth (FT.)

Initial Sample Height (IN.)

Sample Diameter (IN.)

Date

Soil Type (ASTM D2487)

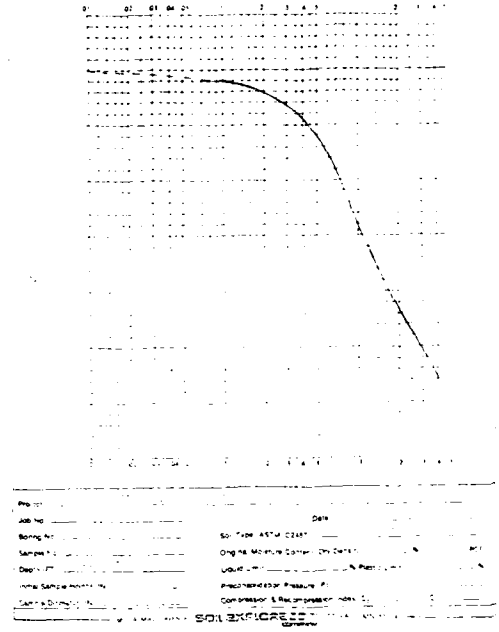
Original Moisture Content, Dry Density % PCF

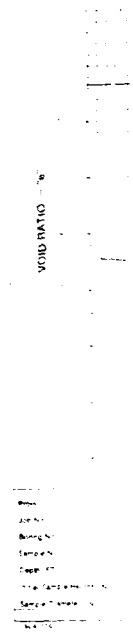
Liquid Limit % Plastic Limit %

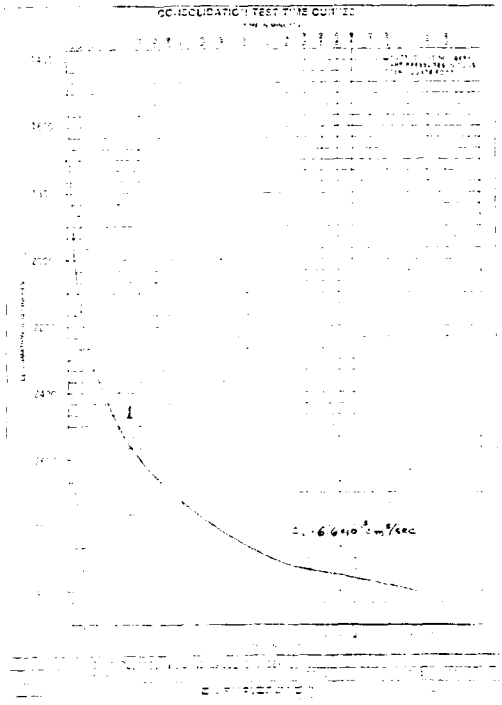
Preconsolidation Pressure (Pc)

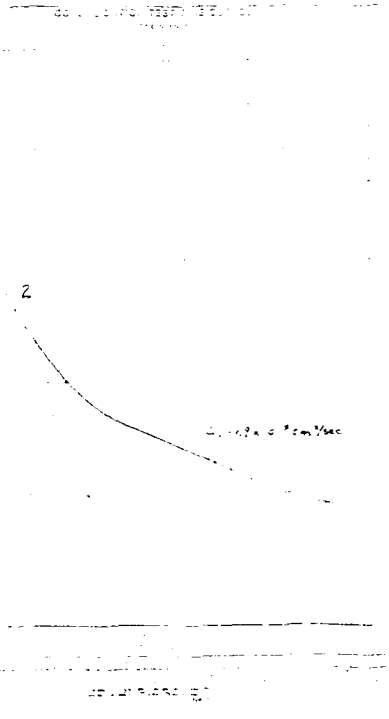
Compression & Recompression Index Cc Cr

SOILEXPLORERS



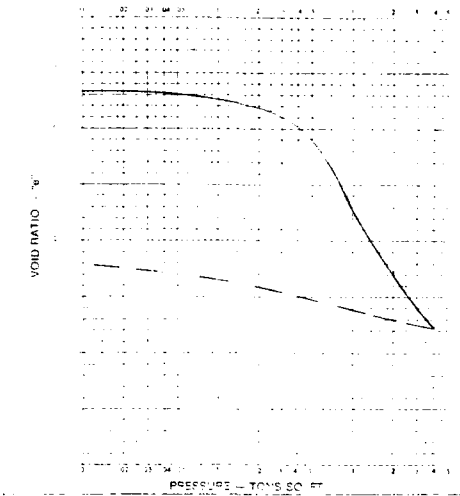


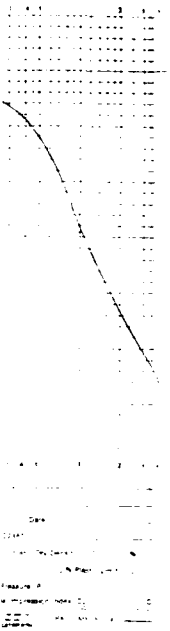


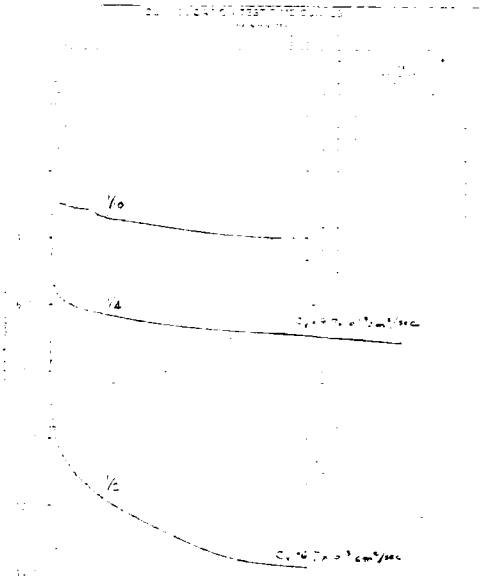


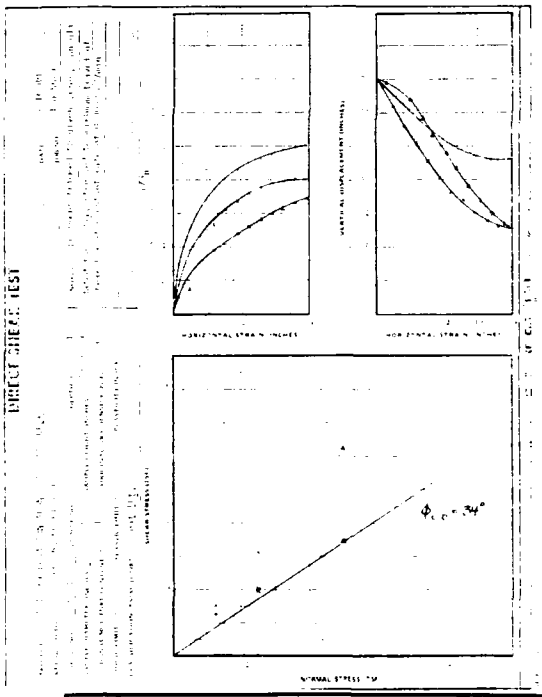
CONSOLIDATION TEST CURVE

VOID RATIO VS PRESSURE









SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
HIGHWAY 100 EMBANKMENT
BORING BC-4C

DESIGNED BY:
DRAWN BY:
CHECKED BY:
SUBMITTED BY:
APPROVED:
DATE: AUGUST 1982

DRAWING NUMBER
M34.3-R-5/257
SHEET 0-29 OF C-39

SAFETY PAYS

2

TRIAXIAL TEST DATA

Date

Job No

Project

Boring No

Sample No

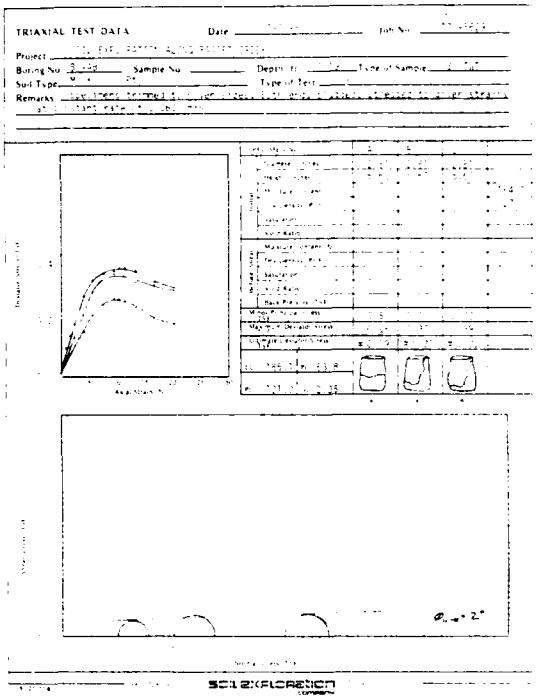
Depth ft

Type of Sample

Soil Type

Type of Test

Remarks



SOILEXPLORATION

TRIAXIAL TEST DATA

Date

Project

Boring No

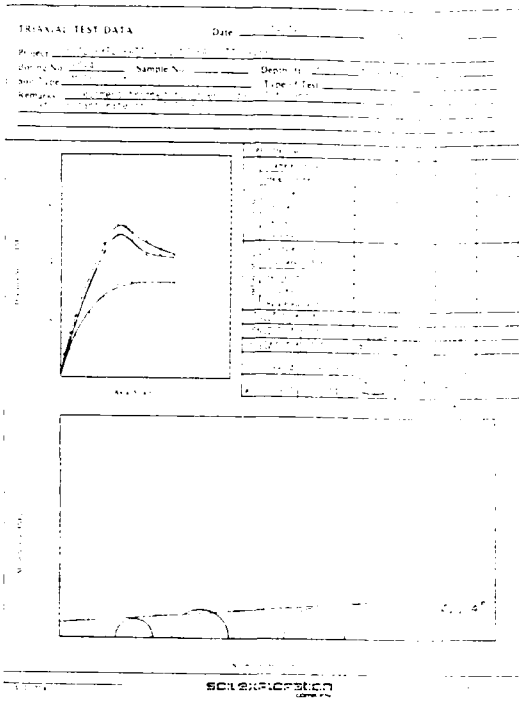
Sample No

Depth ft

Soil Type

Type of Test

Remarks



SOILEXPLORATION

TRIAXIAL TEST DATA

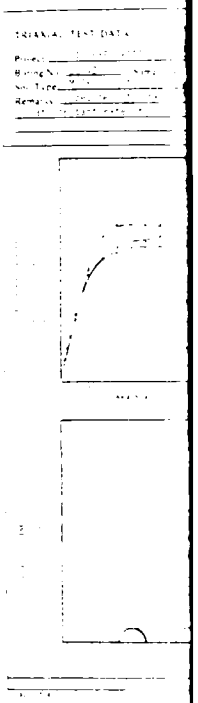
Project

Boring No

Sample No

Soil Type

Remarks



TRIAXIAL TEST DATA

Date

Job No

Project

Boring No

Sample No

Depth ft

Type of Sample

Soil Type

Type of Test

Remarks

SPECIMEN NO

Diameter inches

Height inches

Moisture Content %

Dry Density PCF

Saturation %

Void Ratio

Moisture Content %

Dry Density PCF

Saturation %

Void Ratio

Back Pressure TSF

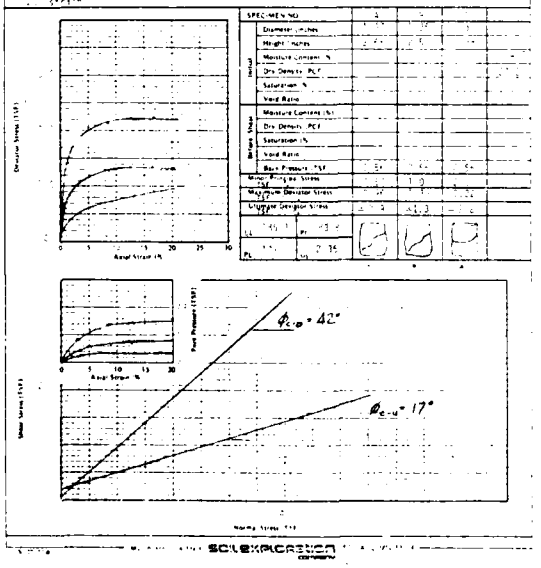
Minor Principal Stress

Maximum Deviator Stress

Ultimate Deviator Stress

LL

PL



SOILEXPLORATION

TRIAXIAL TEST DATA

Date

Job No

Project

Boring No

Sample No

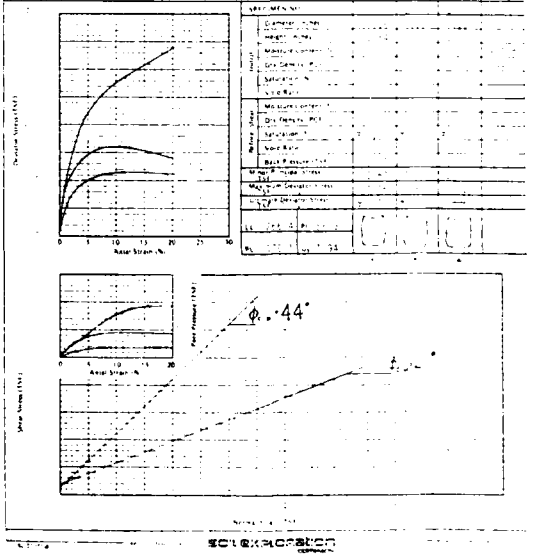
Depth ft

Soil Type

Type of Test

Remarks

SPECIMEN NO



SOILEXPLORATION

TRIAXIAL TEST DATA

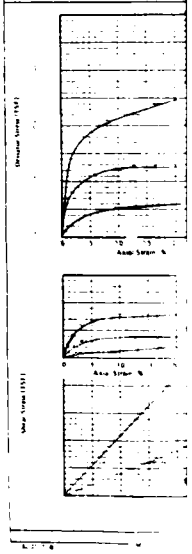
Project

Boring No

Sample No

Soil Type

Remarks



TRIAXIAL TEST DATA

Date

Job No

Project

Boring No

Sample No

Depth (ft)

Type of Sample

Soil Type

Type of Test

Remarks

SOIL EXPLORATION

TRIAXIAL TEST DATA

Date

Job No

Project

Boring No

Sample No

Depth (ft)

Type of Sample

Soil Type

Type of Test

Remarks

SOIL EXPLORATION

TRIAXIAL TEST DATA

Date

Job No

Project

Boring No

Sample No

Depth (ft)

Type of Sample

Soil Type

Type of Test

Remarks

SOIL EXPLORATION

TRIAXIAL TEST DATA

Date February 29, 1980

Job No 80-5629

Project SOIL EXPLORATION ALONG BASSETT CREEK

Boring No BC-4J

Sample No

Depth (ft) 27.5-29

Type of Sample 3" ø

Soil Type SILTY CLAY, TRACE OF ORGANIC

Type of Test CU with Pore Pressure

Remarks Specimens trimmed to given sizes, Drainage strips applied, Saturated and consolidated for total of 3 Days, Back Pressure applied, Drainage lines closed, stressed to given strains at constant rate of 1.0024 min. Pore Pressures recorded via transducers. Mohr Circles based on maximum stress ratio.

SPECIMEN NO	A	B	C
Diameter, inches	1.37	1.39	1.34
Height, inches	2.81	2.79	2.64
Initial: Moisture Content, %			25
Initial: Dry Density, PCF			96
Initial: Saturation, %			
Initial: Void Ratio			
Before Shear: Moisture Content, %			
Before Shear: Dry Density, PCF			
Before Shear: Saturation, %	100	100	
Before Shear: Void Ratio			
Before Shear: Back Pressure, TSF	2.58	2.58	2.58
Minor Principal Stress, TSF	2.3	3.6	5.1
Maximum Deviator Stress			
Ultimate Deviator Stress	4.6	6.1	—
LL 23.3	PL 21		
PL 21	Gs 2.6*		

$\phi_{c\,p} = 38°$

$\phi_{c\,u} = 32°$

SOIL EXPLORATION

TRIAXIAL TEST DATA

Date 2-6-80

Job No

Project SOIL EXPLORATION ALONG BASSETT CREEK

Boring No BC-4J

Sample No

Depth (ft)

Type of Sample

Soil Type

Type of Test

Remarks Specimens trimmed to sizes shown, Drainage strips applied, saturated, consolidated for total of 3 Days, Back Pressure applied, Lines closed, stressed to given strains at constant rate of 0.0024 min. pore-pressure recorded. Maximum stress ratio at 20.6 psi

SPECIMEN NO	A	B	C
Diameter, inches	1.36		
Height, inches	2.86		
Initial: Moisture Content, %			
Initial: Dry Density, PCF			
Initial: Saturation, %			
Initial: Void Ratio			
Before Shear: Moisture Content, %			
Before Shear: Dry Density, PCF			
Before Shear: Saturation, %			
Before Shear: Void Ratio			
Before Shear: Back Pressure, TSF	2.8		
Minor Principal Stress, TSF	2.3	3.6	5.1
Maximum Deviator Stress			
Ultimate Deviator Stress	20.6	21.2	—
LL 99.7	PL 55.9		
PL 43.9	Gs 2.69		

$\phi_{c\,p} = 47°$

$\phi_{c\,u} = 22\frac{1}{2}°$

SOIL EXPLORATION

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY:
DRAWN BY:
CHECKED BY:
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
HIGHWAY 100 EMBANKMENT
BORINGS BC-4B, BC-4J, & BC-4Z

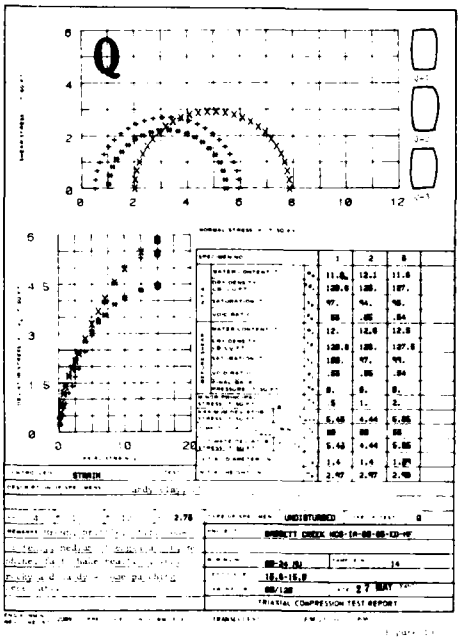
DATE:
AUGUST 1982

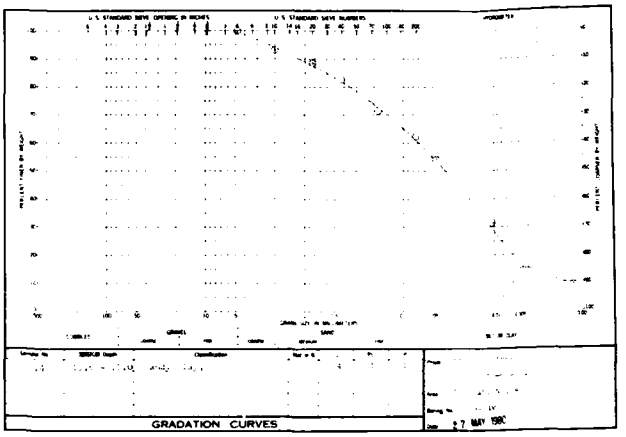
SCALE — USCE NO

DRAWING NUMBER
M34.3-R-5/258
SHEET 0-30 OF C-39

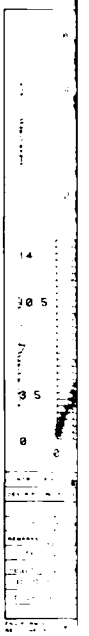
SAFETY PAYS

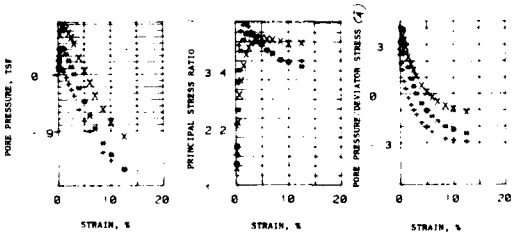
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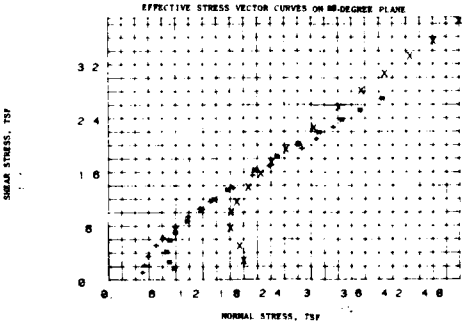


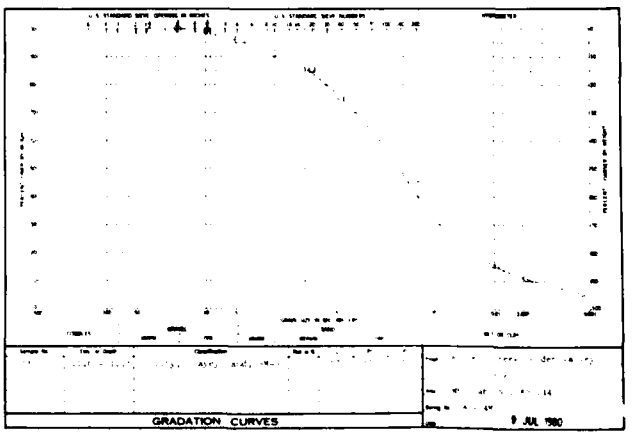


ENG 2087









ENG 2087

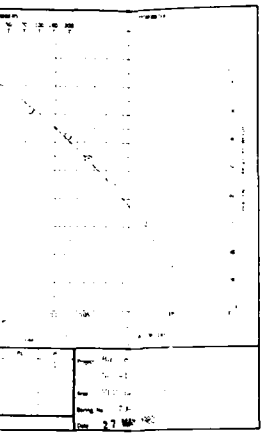
REMARKS: COMPUTER PRINT-OUT
SYMBOLS SAME AS FORM 2089
R TRIAXIAL TEST: PORE PRESSURE MONITORED DURING SHEAR

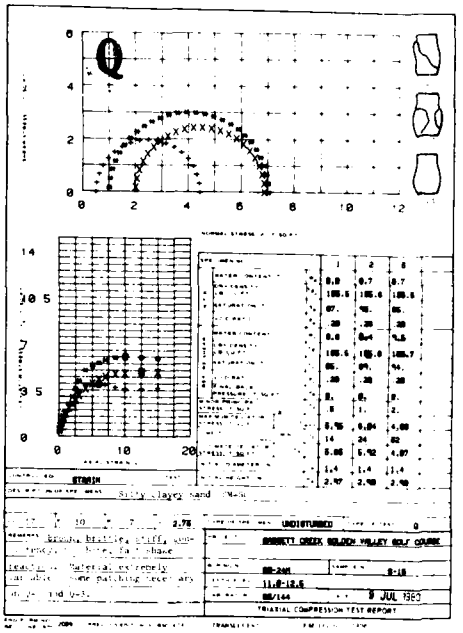
MRD Form 756, 1 May 80

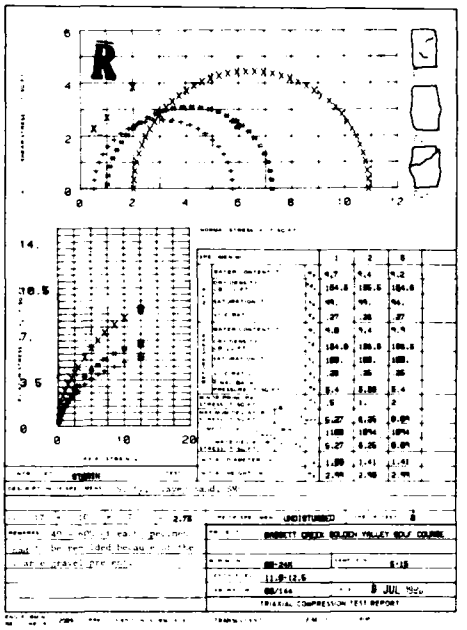
PROJECT: BARRETT CREEK GOLDEN VALLEY GOLF COURSE
BORING NO: 80-34R SAMPLE NO: S-15
DEPTH/ELEV: 14.0-16.5
MRD LAB NO: 80/144 DATE: 9 JUL 1980

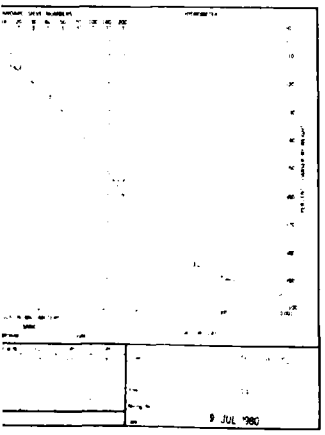
TRIAXIAL COMPRESSION TEST REPORT

FIGURE: 1









SOIL CLASSIFICATION RECORD SHEET

Project | Boring No | MRD Lab No
Station | Range | Surf Elev | Depth To Water Table | Bottom Of Hole

Sample No	Depth To Bottom Of Sample	Moisture (%)	Plasticity (Att Limits) LL	PI	Grading (Cumulative Percents Finer) Hyd Analysis	U.S. Standard Sieve Sizes	Gradation Curve Analysis D_{60} (mm)	D_{30} (mm)	D_{10} (mm)	C_u	C_c	Classification *Tech MEMO 3-357, May 67*	Remarks
Hole	80-23M												
1	0-2.0	13.5	42	22									
3	2.0-4.0	17.3											
5	4.0-6.0	17.3											
8	6.0-8.5												
10	8.5-10.0												
Hole	80-24M												
1	0-2.0												
3	2.0-4.0	10.4	23	7									
5	4.0-6.0	17.4											
10	8.5-10.0												
Hole	80-25M												
1	0-2.0												
3	2.0-4.0	15.4	28	8									
5	4.0-6.0	20.2											
6	6.0-8.5	19.6											
7	8.5-10.0	9.2	NP	NP									
10	10.0-11.5												

MRD FORM NOV 75 16 EDITION OF MAY 70 IS OBSOLETE

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY:
DRAWN BY:
CHECKED BY:
SUBMITTED BY:
APPROVED:

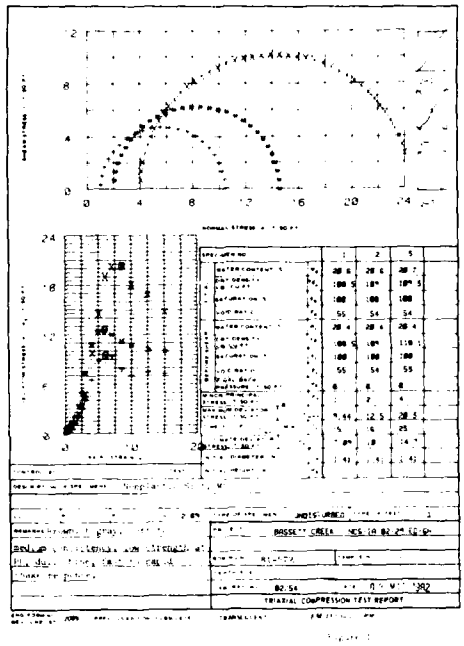
PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
GOLDEN VALLEY COUNTRY CLUB EMBANKMENT
BORINGS 80-23M, 80-24M & 80-25M

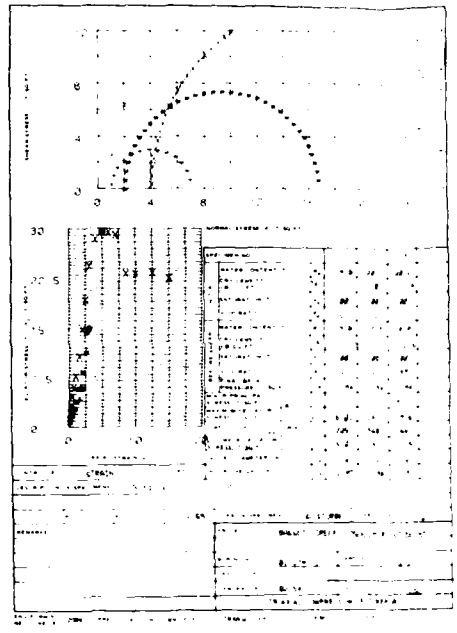
DATE: AUGUST 1982

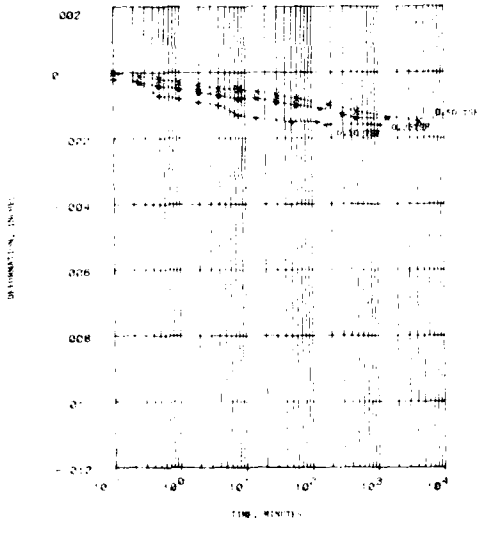
DRAWING NUMBER
M34.3-R-5/259
SHEET C-31 OF C-39

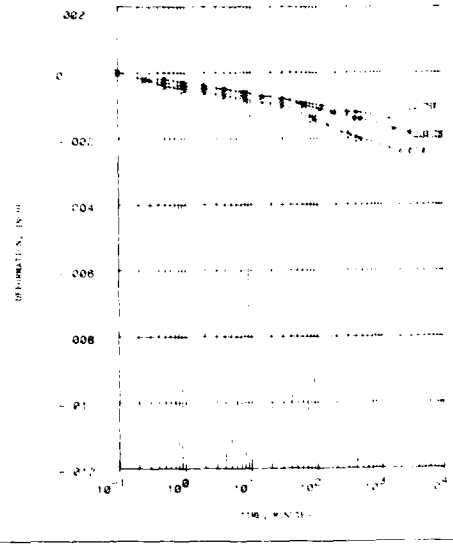
SAFETY PAYS

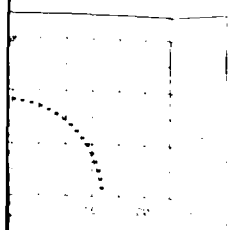
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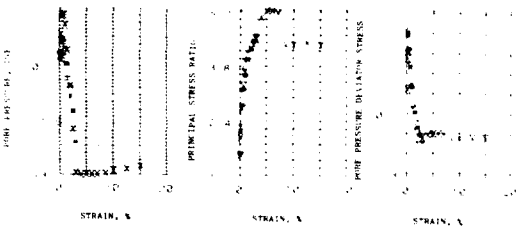


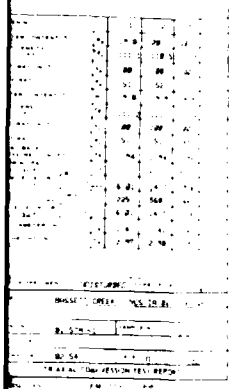


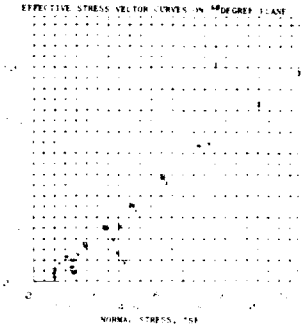


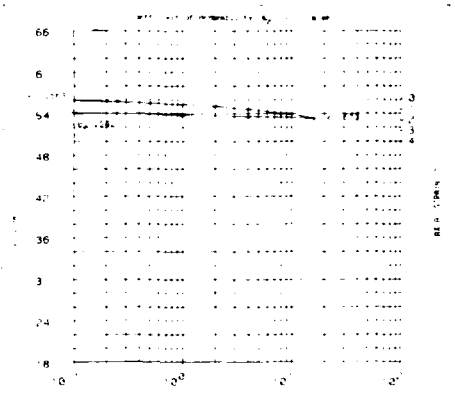


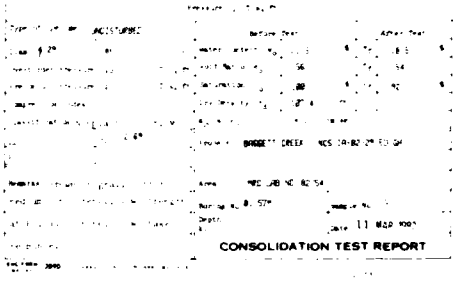












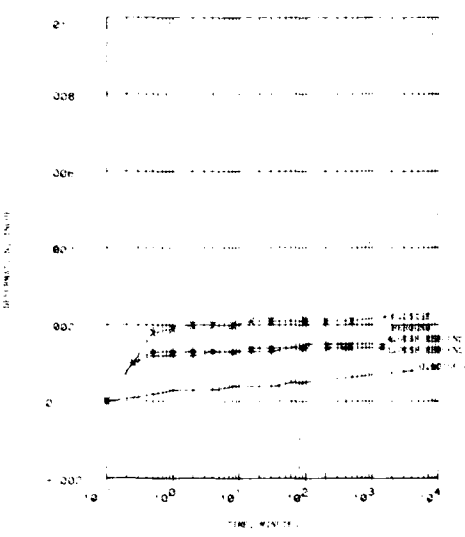
REMARKS (METER PRINTOUT)
SYMBOLS SAME AS FORM 2089
R "TRIAXIAL TEST, PORE PRESSURE MONITORED DURING SHEAR"

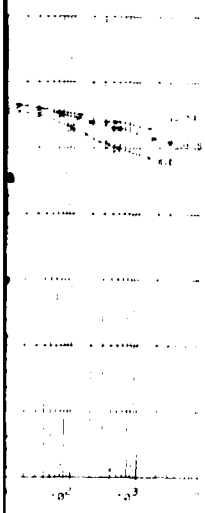
MRD Form 756, 1 May 80

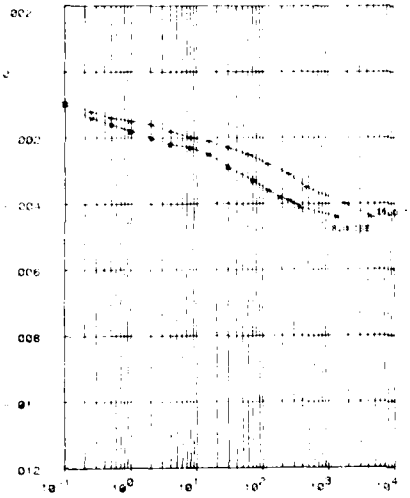
PROJECT	BASSETT CREEK MCS-1A-82-2M-ED-SM
BORING NO.	B1-57M SAMPLE NO. S-1
DEPTH FEET	
MRD LAB NO.	82-54 DATE 8 MAY 82

TRIAXIAL COMPRESSION TEST REPORT

FIGURE







BASSETT CREEK MCS-1A-82-2M-ED-SM
82-54 DATE: 11 MAR 1982
B1-57M SAMPLE NO: S-1 DEPTH FEET:
CONSOLIDATION TEST--TIME CURVE
FIGURE 4

BASSETT CREEK MCS-1A-82-2M-ED-SM
82-54 DATE: 11 MAR 1982
B1-57M SAMPLE NO: S-1 DEPTH FEET:
CONSOLIDATION TEST--TIME CURVE
FIGURE

MRD Form 757, 1 May 81

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY
DRAWN BY
CHECKED BY
SUBMITTED BY
APPROVED

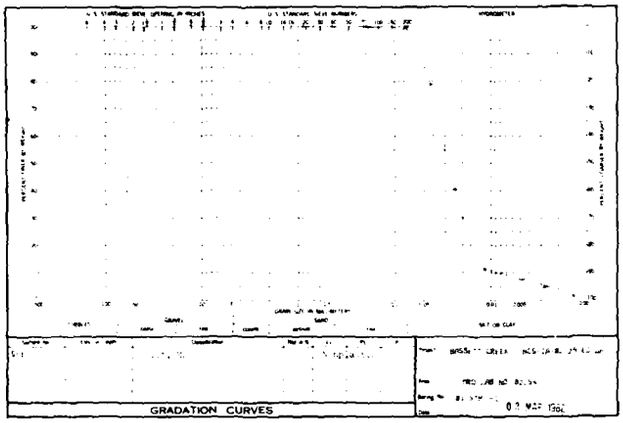
PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
EDGEWOOD EMBANKMENT
BORING B1-57MU

DATE
AUGUST 1982

DRAWING NUMBER
M34.3-R-5/260
SHEET C-32 OF C-39

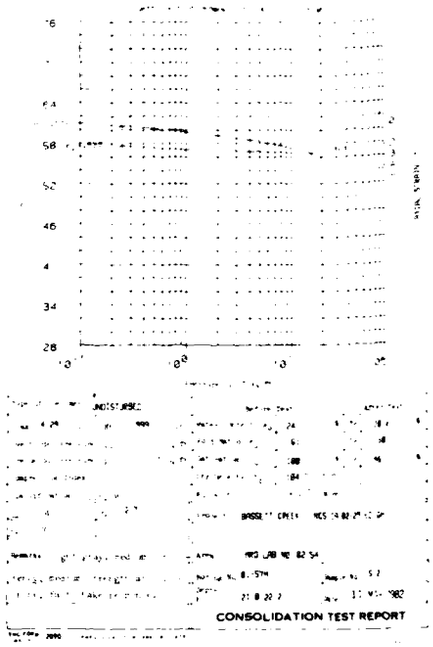
SAFETY PAYS

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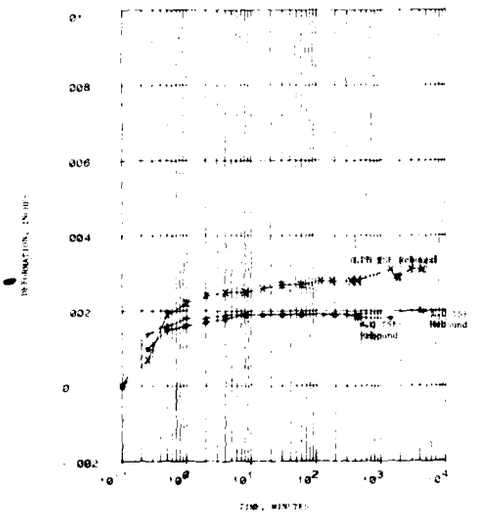


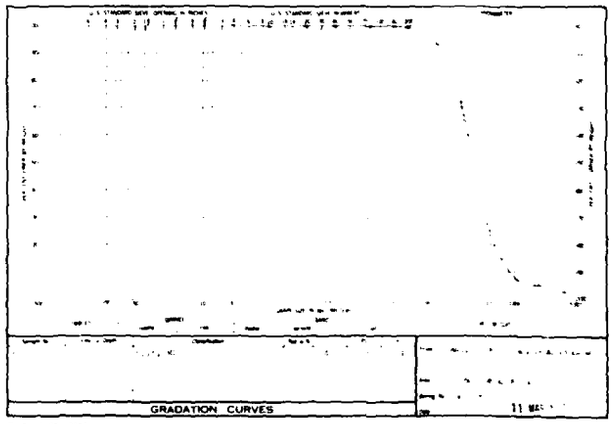
GRADATION CURVES

ENG [illegible] 2087



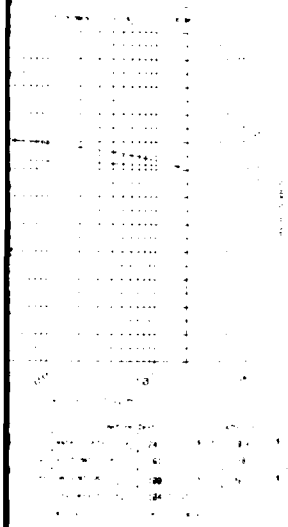
CONSOLIDATION TEST REPORT



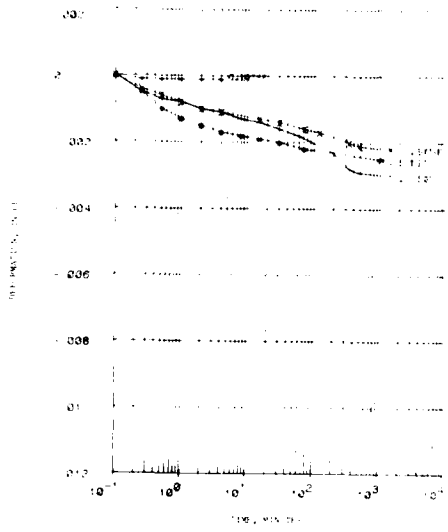


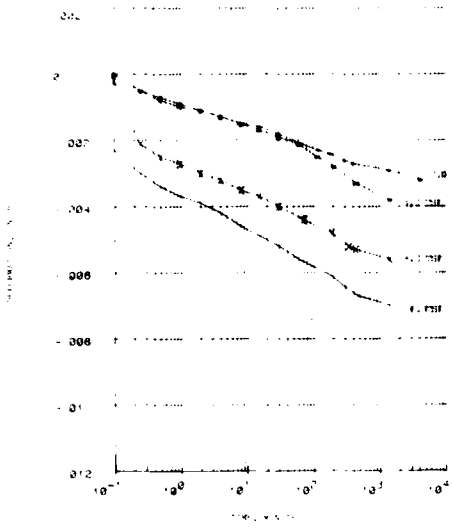
GRADATION CURVES

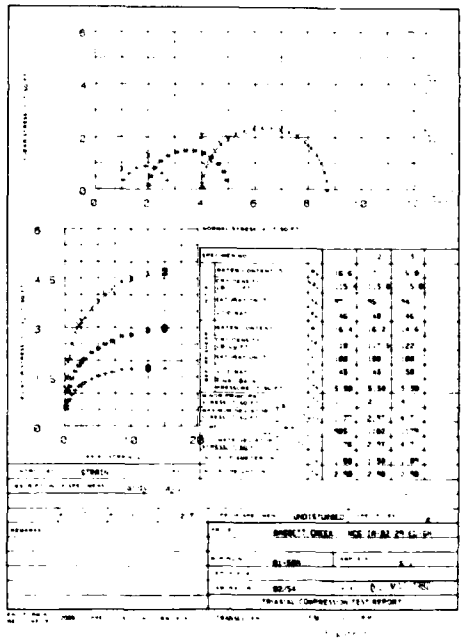
ENG [illegible] 2087

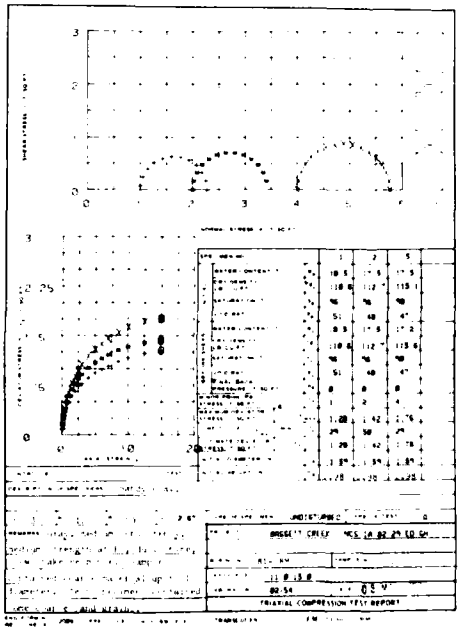


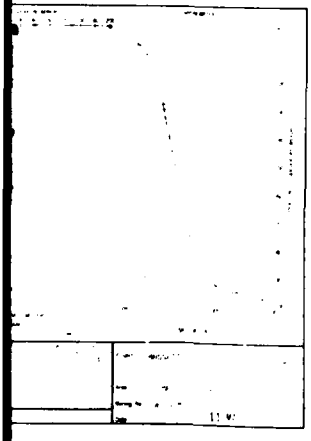
CONSOLIDATION TEST REPORT











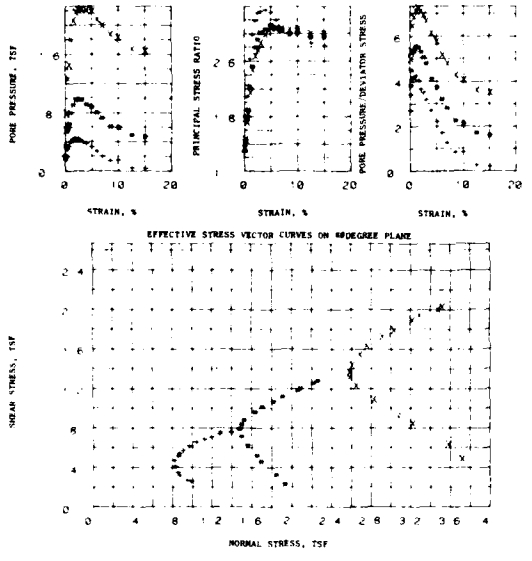
DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
EDGEWOOD EMBANKMENT
BORINGS 81-57MU & 81-58MU

DATE
AUGUST 1982

DRAWING NUMBER
M34.3-R-5/261
SHEET C-33 OF C-39

SAFETY PAYS



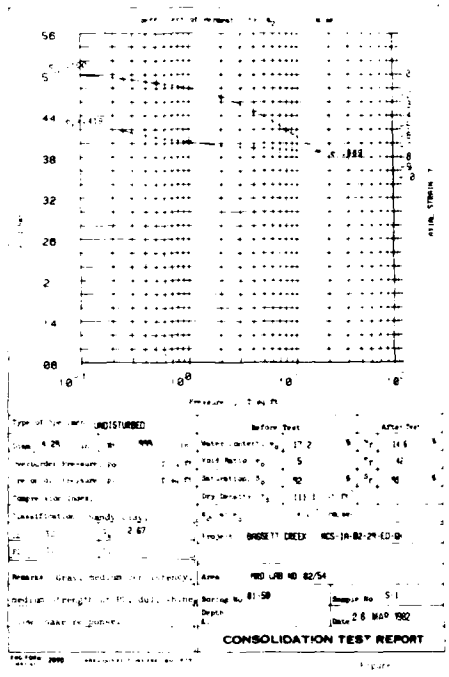
REMARKS: COMPUTER PRINTOUT
SYMBOLS SAME AS FORM 2089
R TRIAXIAL TEST: PORE PRESSURE
MONITORED DURING SHEAR

MRD Form 956, 1 May 80

PROJECT: BASSETT CREEK HCS-1A-82-29-ED-BH
BORING NO: B1-50M SAMPLE NO: S-1
DEPTH/ELEV:
MRD LAB NO: 82-54 DATE: 03 MAR 1982

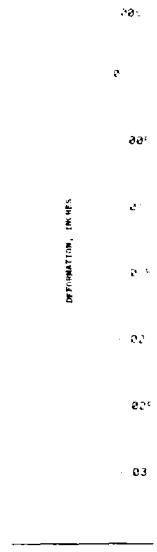
TRIAXIAL COMPRESSION TEST REPORT

FIGURE: 7



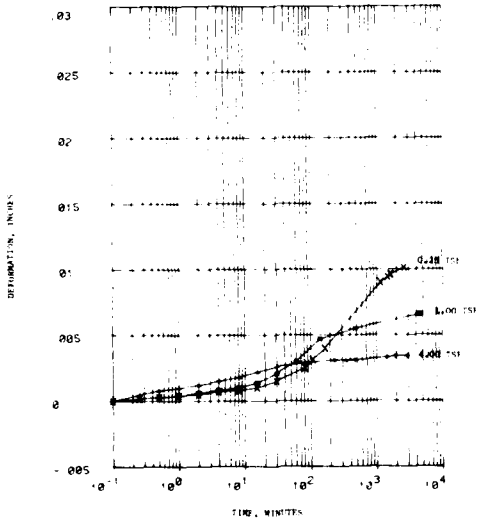
CONSOLIDATION TEST REPORT

ENG FORM 2090



PROJECT
MRD LAB NO:
BORING NO
COMPUTER PRINTO
SAME AS ENG FORM

MRD Form 956, 1 May 80



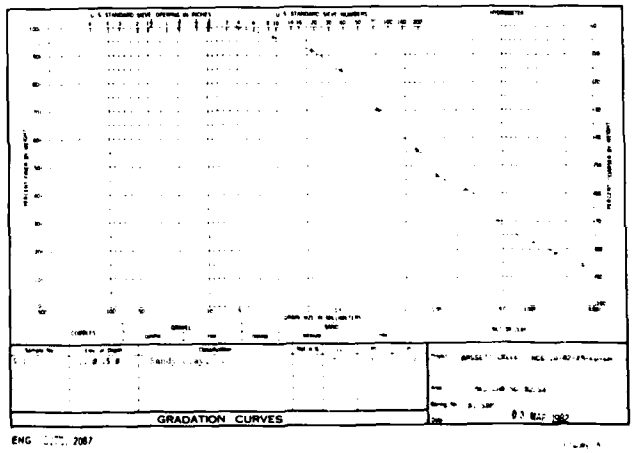
PROJECT: BASSETT CREEK HCS-1A-82-29-ED-BH
MRD LAB NO: 82/54 DATE: 26 MAR 1982
BORING NO: B1-50 SAMPLE NO: S-1 DEPTH/ELEV:

COMPUTER PRINTOUT FORMAT
SAME AS ENG FORM 2088

CONSOLIDATION TEST--TIME CURVES

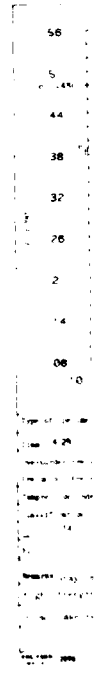
FIGURE: 8

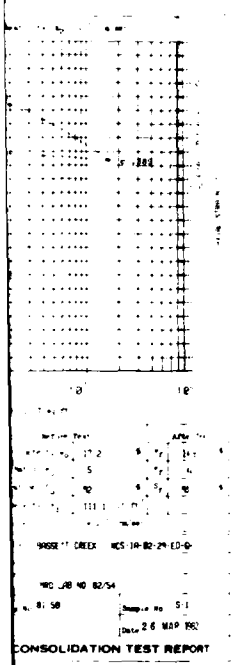
MRD Form 956, 1 May 80



GRADATION CURVES

ENG FORM 2087





BASSETT CREEK MCS-1A-82-2M-ED-GH

MRD LAB NO 82/54

BORING NO: 81-58 SAMPLE NO: S-1

DEPTH/ELEV:

CONSOLIDATION TEST—TIME CURVES

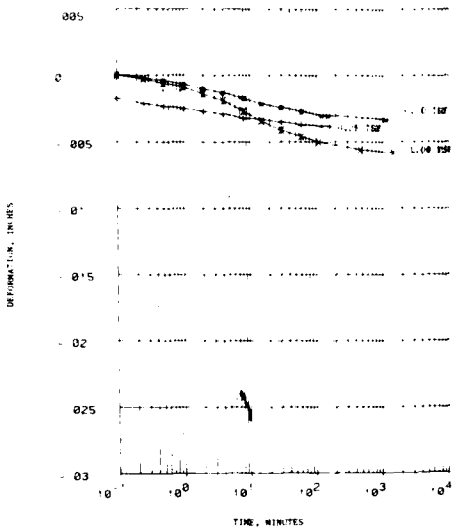
COMPUTER PRINT-OUT FORMAT
SAME AS ENG FORM 2088

FIGURE:

DATE: 26 MAR 1982

MRD Form 956, 1 May 80

CONSOLIDATION TEST REPORT



PROJECT: BASSETT CREEK MCS-1A-82-2M-ED-GH

MRD LAB NO: 82/54 DATE: 26 MAR 1982

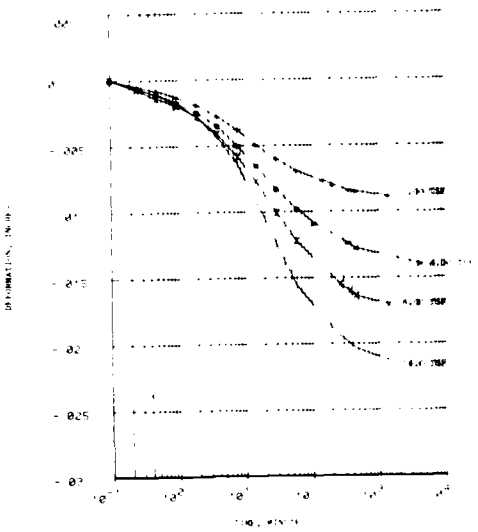
BORING NO: 81-58 SAMPLE NO: S-1 DEPTH/ELEV:

COMPUTER PRINT-OUT FORMAT
SAME AS ENG FORM 2088

CONSOLIDATION TEST—TIME CURVES

FIGURE:

MRD Form 956, 1 May 80



PROJECT: BASSETT CREEK MCS-1A-82-2M-ED-GH

MRD LAB NO: 82/54 DATE: 26 MAR 1982

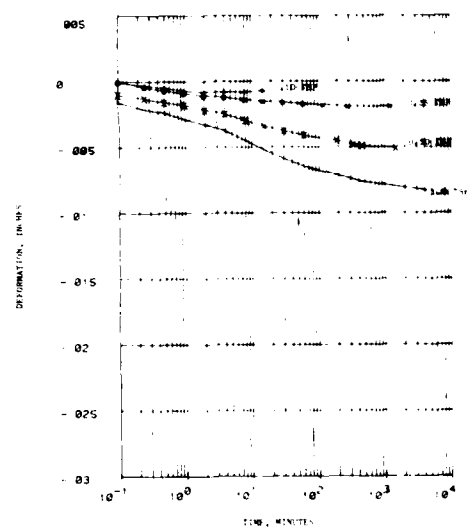
BORING NO: 81-58 SAMPLE NO: S-1 DEPTH/ELEV:

COMPUTER PRINT-OUT FORMAT
SAME AS ENG FORM 2088

CONSOLIDATION TEST—TIME CURVES

FIGURE:

MRD Form 956, 1 May 80



PROJECT: BASSETT CREEK MCS-1A-82-2M-ED-GH

MRD LAB NO: 82/54 DATE: 11 MAR 1982

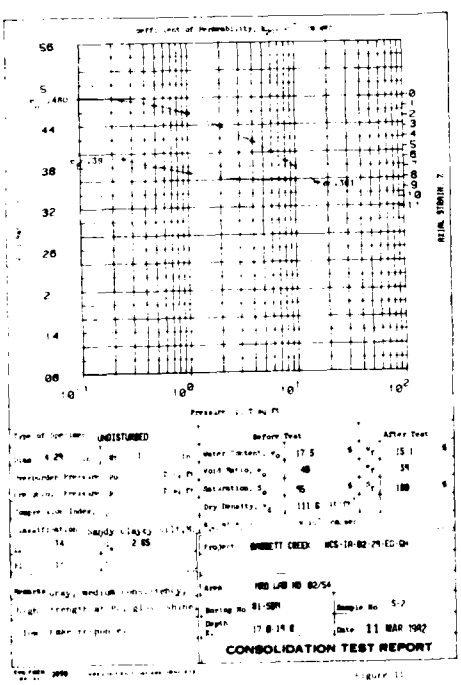
BORING NO: 81-58M SAMPLE NO: S-2 DEPTH/ELEV:

COMPUTER PRINT-OUT FORMAT
SAME AS ENG FORM 2088

CONSOLIDATION TEST—TIME CURVES

FIGURE:

MRD Form 956, 1 May 80



Type of Specimen: UNDISTURBED

	Before Test	After Test
Water Content, %	17.5	15.1
Void Ratio, e	.48	.39
Saturation, %	95	100
Dry Density, pcf	111.6	

Classification: Sandy Clay

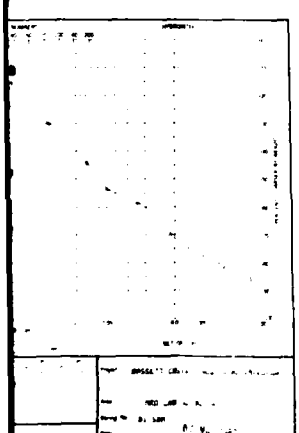
Project: BASSETT CREEK MCS-1A-82-2M-ED-GH

Area: MRD LAB NO 82/54

Boring No: 81-58M Sample No: S-2

Depth: 17.0-19.0 Date: 11 MAR 1982

CONSOLIDATION TEST REPORT





SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY:
DRAWN BY:
CHECKED BY:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
EDGEWOOD EMBANKMENT
BORING 81-58MU

SUBMITTED BY:

APPROVED:

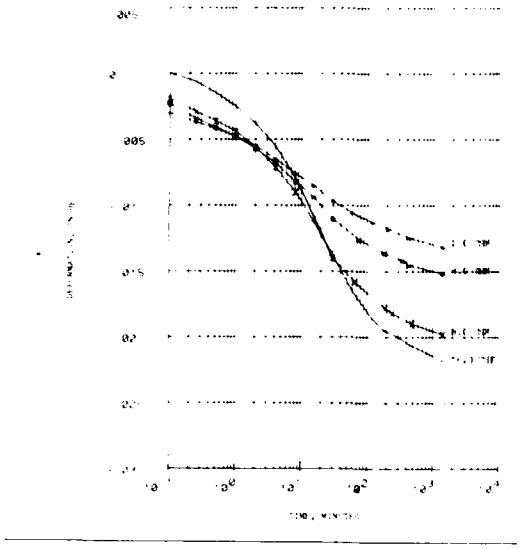
DATE:
AUGUST 1982

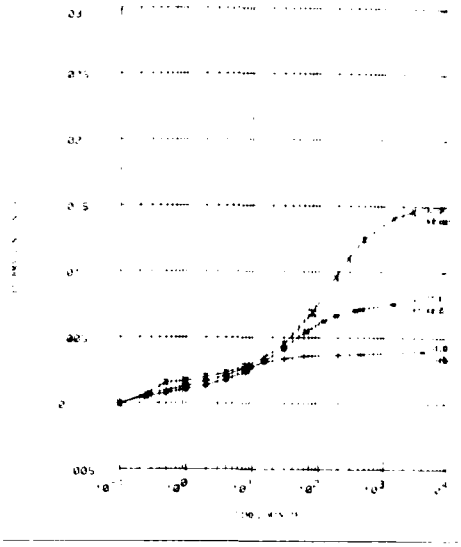
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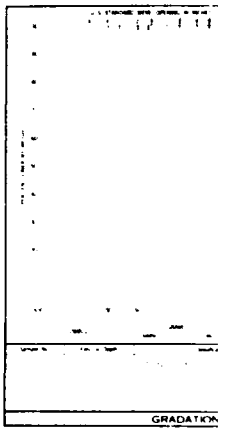
DRAWING NUMBER
M34.3-R-5/262

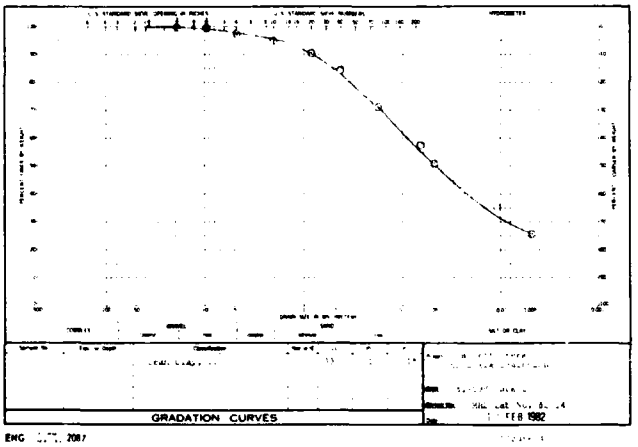
SHEET C-34 OF C-39

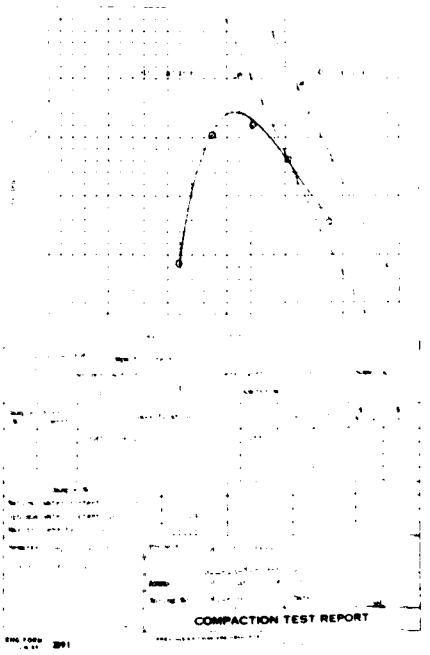
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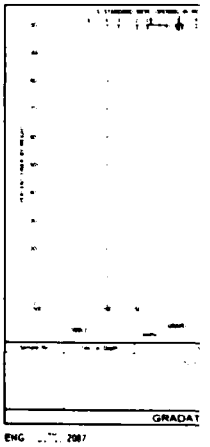


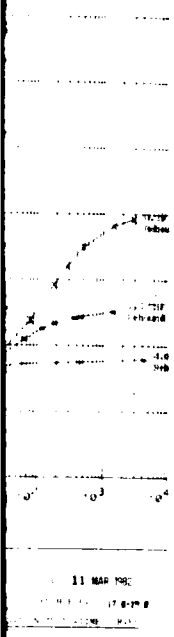




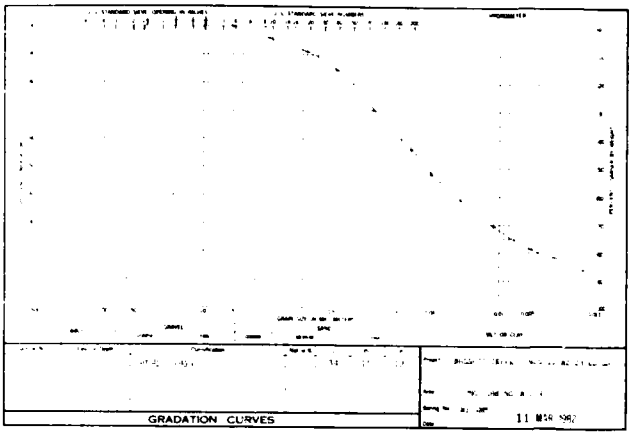








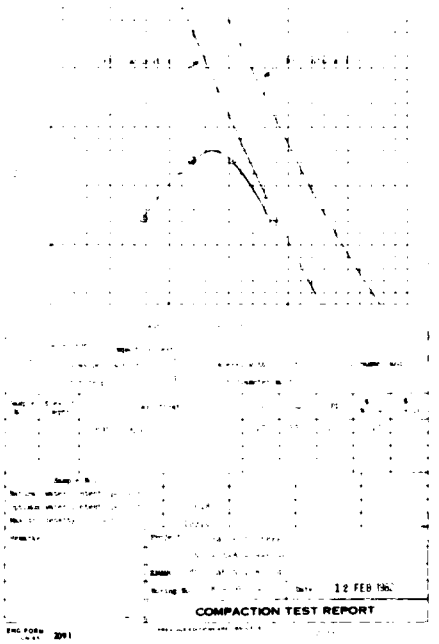
11 MAR 1982



GRADATION CURVES

11 MAR 1982

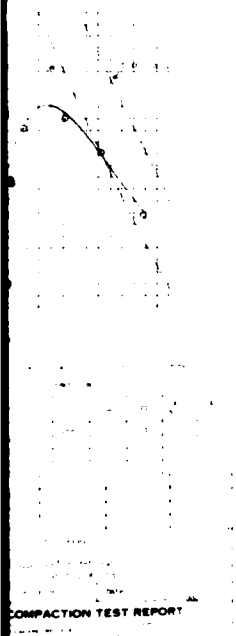
ENG 2087



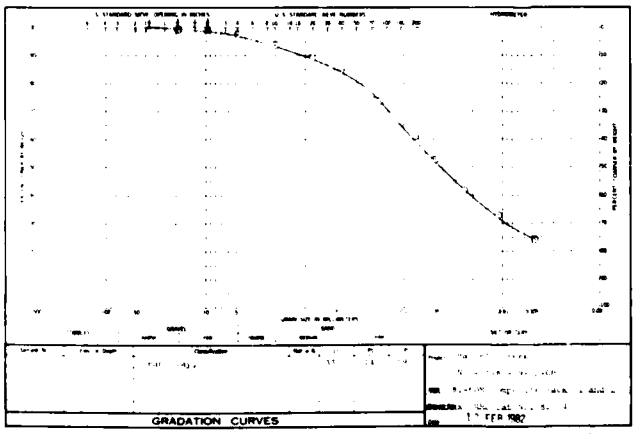
12 FEB 1982

COMPACTION TEST REPORT

ENG FORM 2087



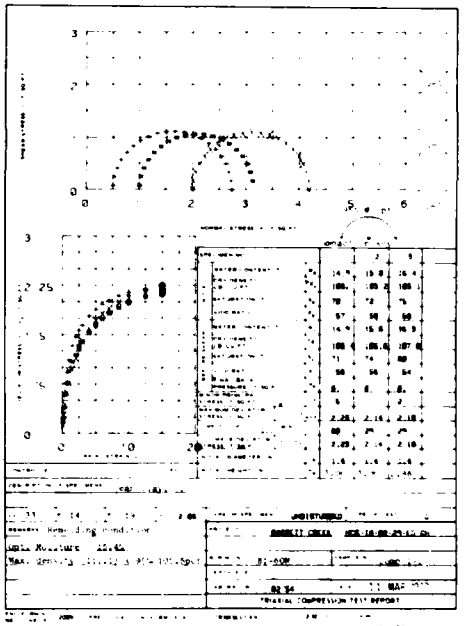
COMPACTION TEST REPORT



GRADATION CURVES

1 FEB 1982

ENG 2087



UNDISTURBED

BASSETT CREEK

81-60M

82 34

11 MAR

TRIAXIAL COMPRESSION TEST REPORT

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

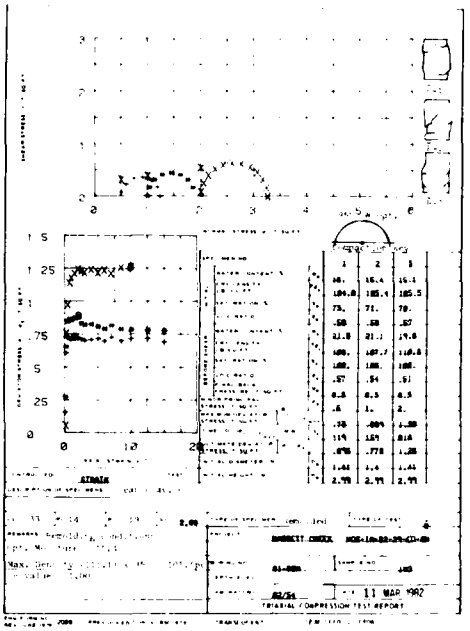
PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
EDGEWOOD EMBANKMENT
BORINGS 81-58MU, 81-59MU & 81-60

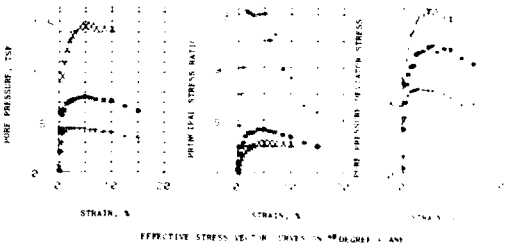
DATE
AUGUST 1982

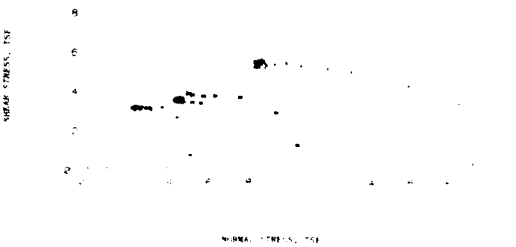
DRAWING NUMBER
M34.3-R-5/263
SHEET 0-35 OF C-39

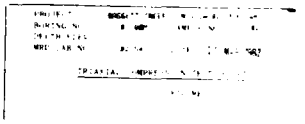
SAFETY PAYS

2



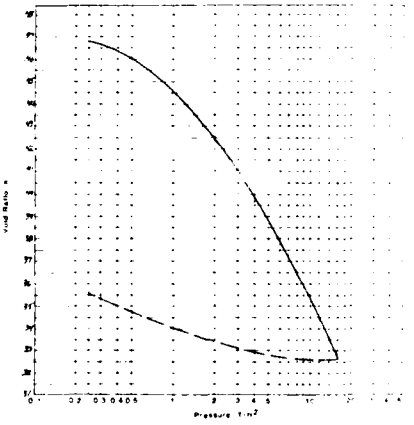






CONSOLIDATION TEST

VOID RATIO VS PRESSURE



Project: Bassett Creek

Job No.:

Boring No.: B-16

Sample No.: Tube #1

Depth: 48.7'

Initial Sample Height: 0.75"

Sample Diameter: 2.5"

Description of Sample: Clay-Till

Date: 4/18/80

Tested By: MRC

Original Moisture: 17.9%

Original Dry Density: 113.8

Liquid Limit: 22%

Plastic Limit: 16%

Overburden Pressure:

Pre-Const. Press.:

Compression Index:

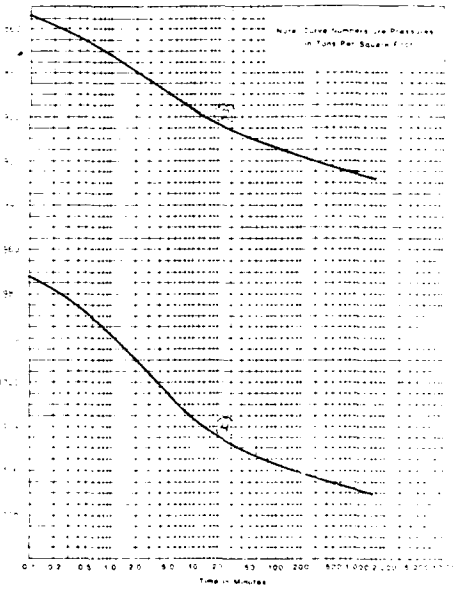
SOIL CLASSIFICATION RECORD SHEET

Project			Boring No	MRD Lab No
Station	Range	Surf Elev	Depth To Water Table	Bottom Of Hole

Sample No	Depth To Bottom Of Sample	Moisture (%)	Plasticity (Att Limits) LL	PI	Grading (Cumulative Percents Finer)	Gradation Curve Analysis	Classification Tech MEMO 3-357, May 67	Remarks

MRD FORM NOV 75 16 EDITION OF MAY 70 IS OBSOLETE

CONSOLIDATION TEST - TIME CURVES

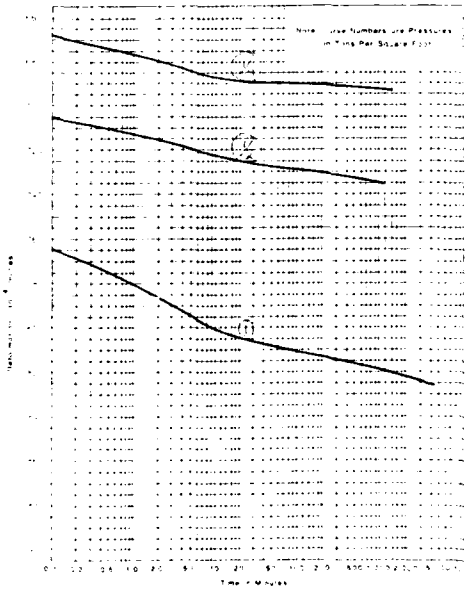


Project Job No

Sample No Boring No Depth

BARR ENGINEERING CO.

CONSOLIDATION TEST - TIME CURVES



Project Job No

Sample No Boring No Depth

BARR ENGINEERING CO.

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY:
DRAWN BY
CHECKED BY:
SUBMITTED BY:
APPROVED:

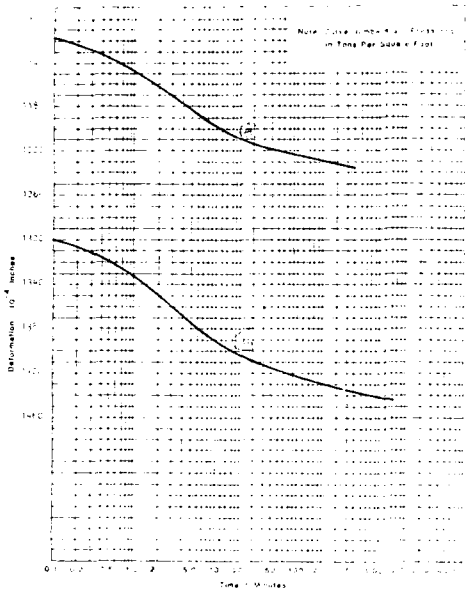
PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
EDGEWOOD EMBANKMENT
BORINGS B1-60M & BC-10C

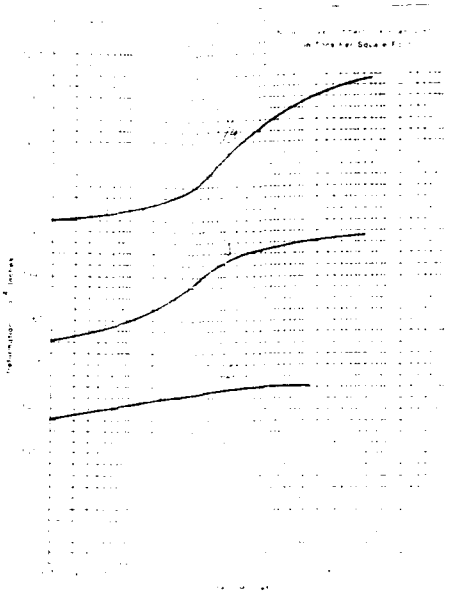
DATE
AUGUST 1982

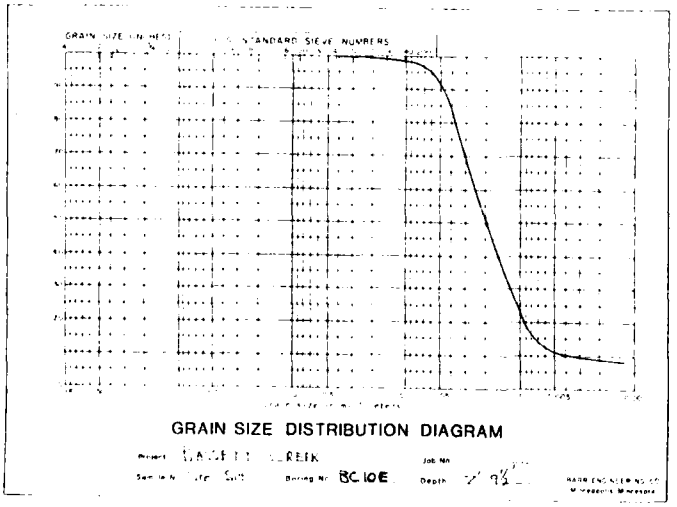
DRAWING NUMBER
M34.3-R-5/264
SHEET 36 OF C-39

SAFETY PAYS

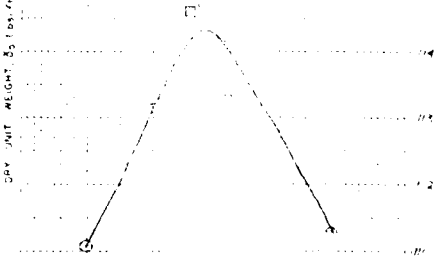
CONSOLIDATION TEST - TIME CURVES

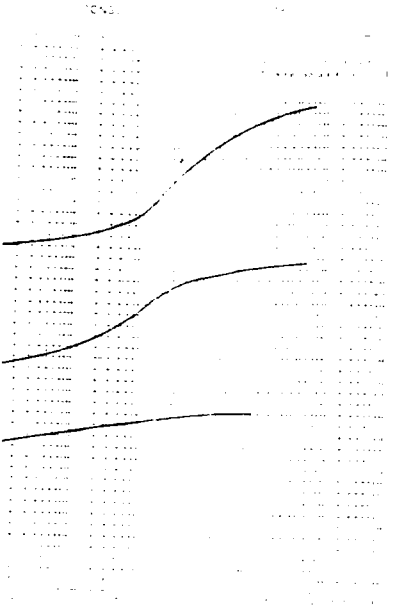


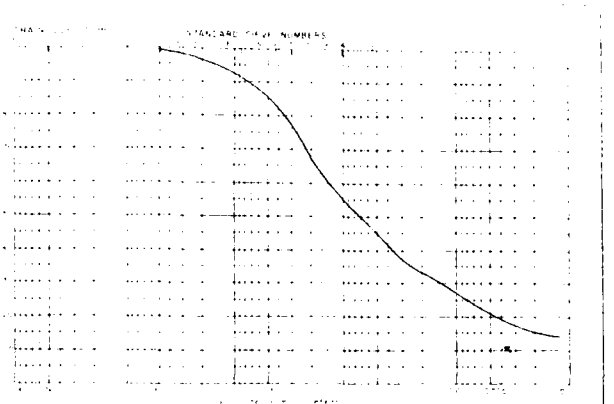




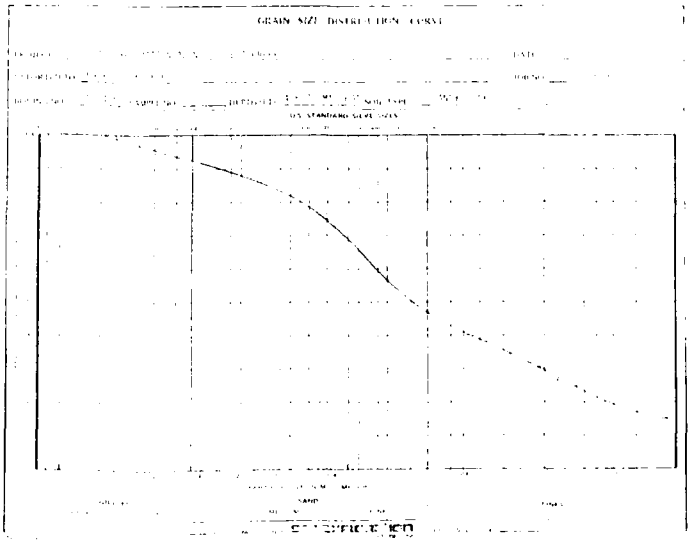
GRAIN SIZE DISTRIBUTION DIAGRAM







GRAIN SIZE DISTRIBUTION DIAGRAM



STANDARD PROCTOR

MATERIAL Brown Clay BC 10D

PROJECT Bassett Creek

LOCATION

DATE TAKEN 3/26/82

DATE TESTED 4/1/82

TESTED BY DAS

ORIGINAL M.C. (%)

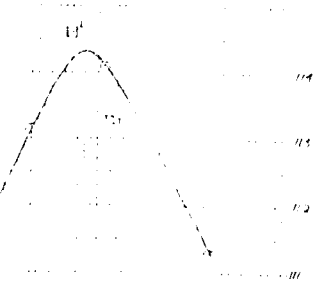
OPTIMUM M.C. (%) 19.8

MAXIMUM DENSITY (PCF) 113

95% DENSITY (PCF) 1086

COMMENTS

[illegible] Taken from Soils Engineering and Rock Manual Typical Moisture Density Curves



PARR ENGINEERING CO

SAFETY PAYS

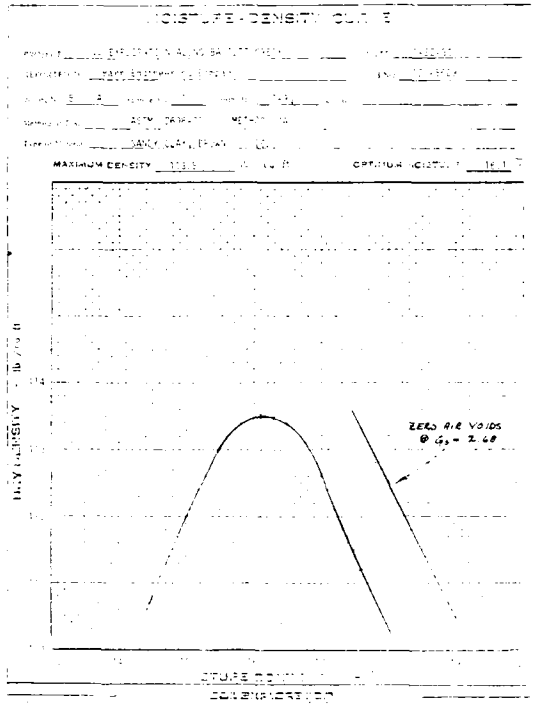
DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

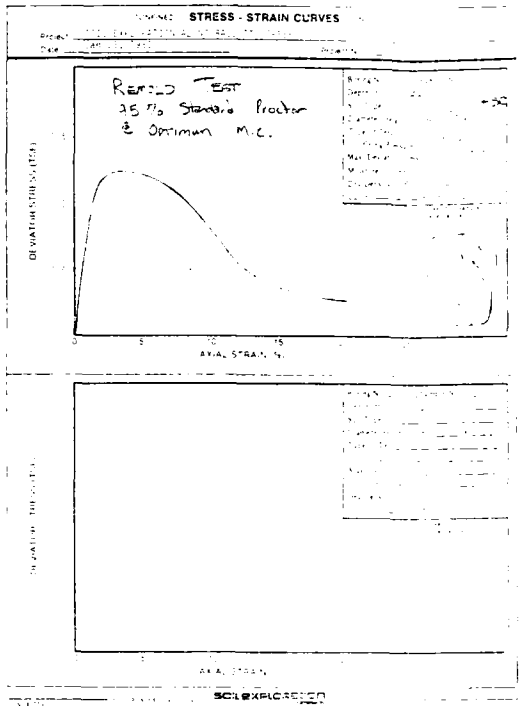
PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
EDGEWOOD EMBANKMENT
BORINGS BC-10C, BC-14D, BC-10E & BC-10B

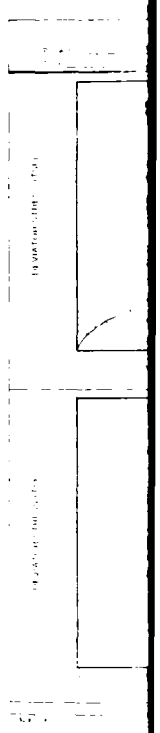
DESIGNED BY:
DRAWN BY:
CHECKED BY:
SUBMITTED BY:
APPROVED:
DATE: AUGUST 1982

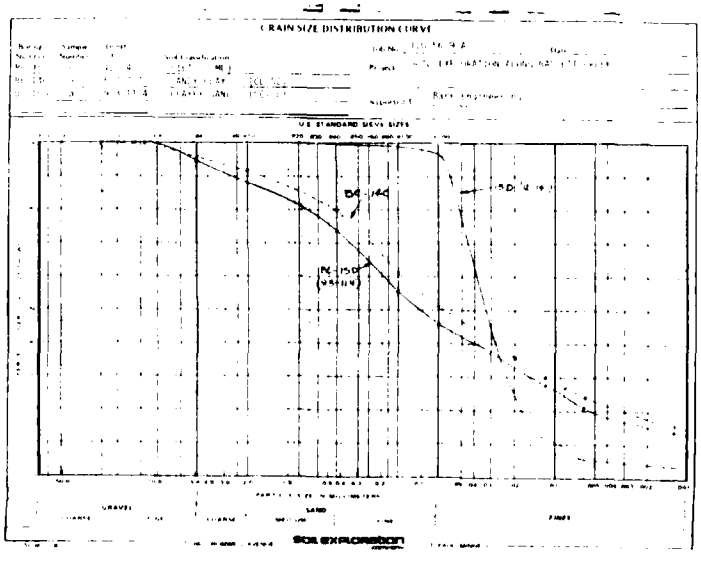
DRAWING NUMBER
M34.3-R-5/265
SHEET 0-37 OF C-39

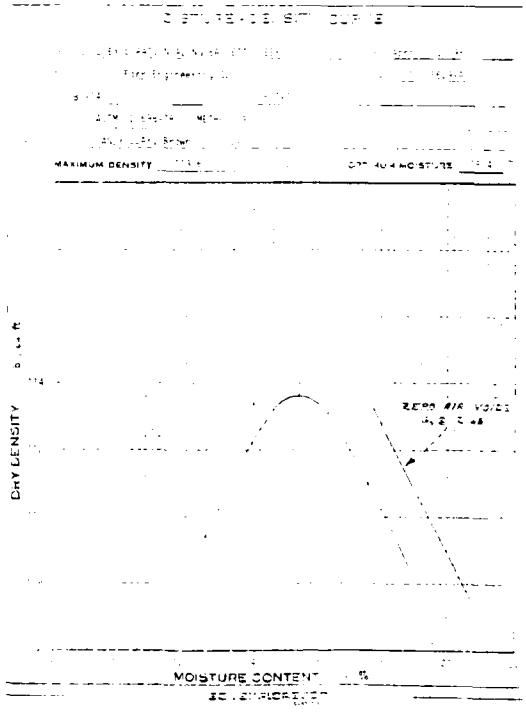
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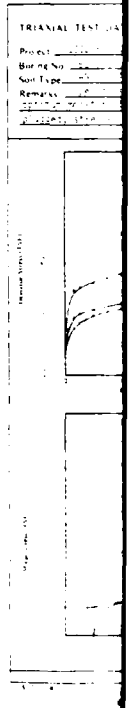

MOISTURE - DENSITY CURVE
MAXIMUM DENSITY
OPTIMUM MOISTURE
DRY DENSITY
ZERO AIR VOIDS
@ G_s = 2.68
MOISTURE CONTENT
SOILEXPLORATION


STRESS - STRAIN CURVES
REPEAT TEST
25 Blows Standard Proctor
@ Optimum M.C.
DEVIATOR STRESS (PSI)
AXIAL STRAIN %
SOILEXPLORATION

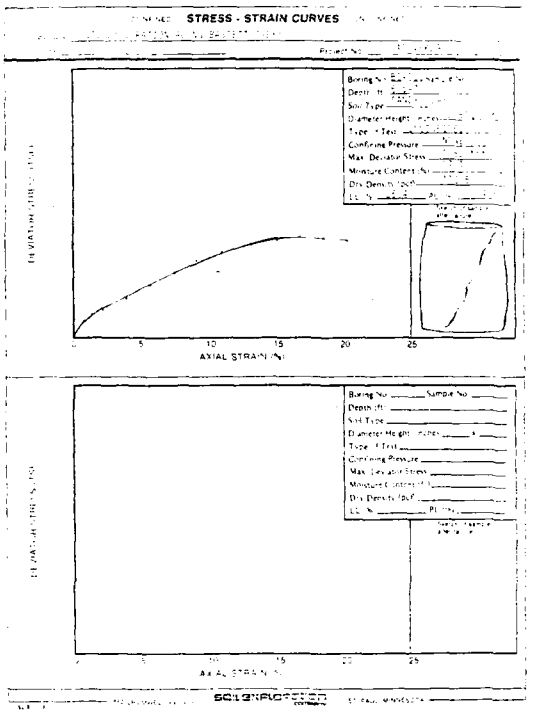



GRAIN SIZE DISTRIBUTION CURVE
U.S. STANDARD SIEVE SIZES
D4 #4
D50 #10
D85 #50
GRAVEL
SAND
FINES
SOILEXPLORATION


MOISTURE - DENSITY CURVE
MAXIMUM DENSITY
OPT. MOISTURE
DRY DENSITY
ZERO AIR VOIDS
MOISTURE CONTENT
SOILEXPLORATION


TRIAXIAL TEST

STRESS - STRAIN CURVES



TRIAXIAL TEST DATA

Date 4-30-80 Job No 100-86[illegible]

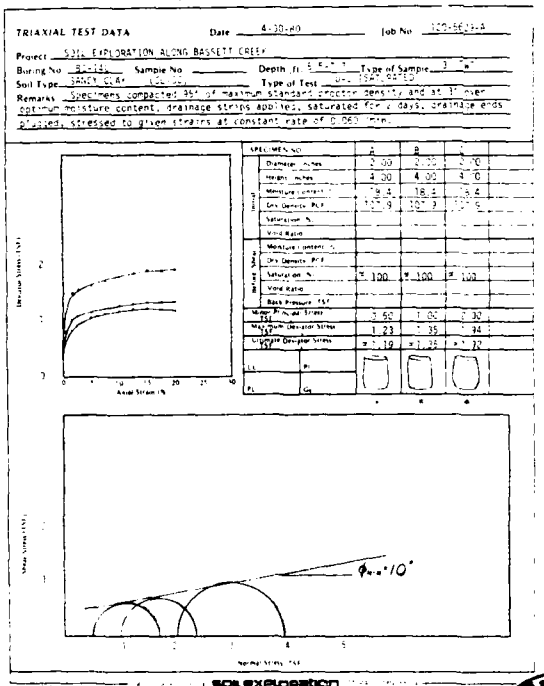
Project SOIL EXPLORATION ALONG BASSETT CREEK

Boring No BC-14A Sample No — Depth (ft) 5.5-7 Type of Sample 3" ø

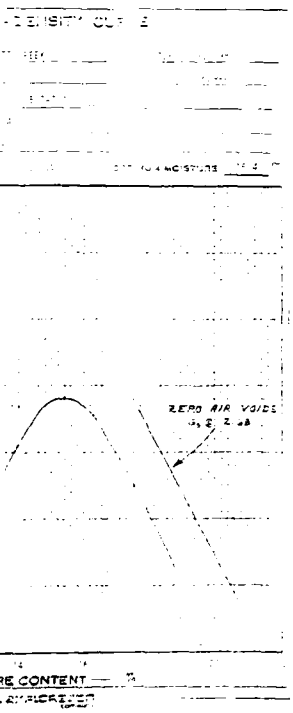
Soil Type SANDY CLAY (CL) Type of Test UU - 150 PSI CP

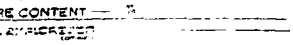
Remarks Specimens compacted 95% of maximum standard proctor density, and at 3% over optimum moisture content, drainage strips applied, saturated for 7 days, drainage ends closed, stressed to given strains at constant rate of 0.067 min.

SPECIMEN NO	A	B	C
Diameter, inches	2.00	2.00	2.00
Height, inches	4.00	4.00	4.00
Moisture Content %	18.4	18.4	18.4
Dry Density PCF	105.9	105.9	105.9
Saturation %			
Void Ratio			
Moisture Content %			
Dry Density PCF			
Saturation %	@ 100	@ 100	@ 100
Void Ratio			
Back Pressure TSF			
Minor Principal Stress TSF	0.50	1.00	1.50
Max Deviator Stress TSF	1.23	1.35	1.44
Ultimate Deviator Stress TSF	@ 1.19	@ 1.35	@ 1.72
LL			
PL			



SOIL EXPLORATION





TRIAXIAL TEST DATA

Date 4-28-80 Job No [illegible]

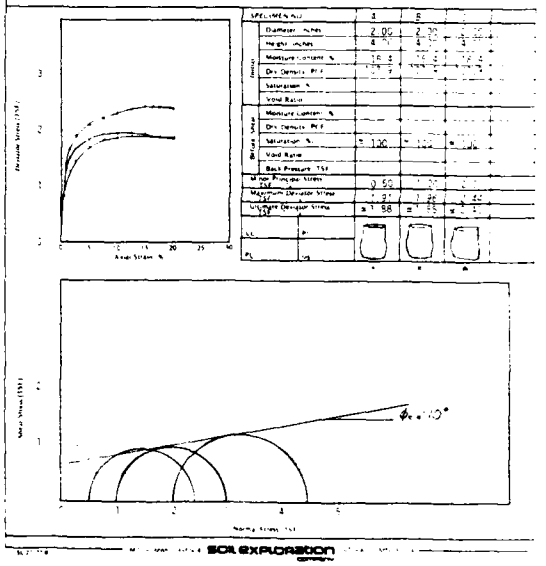
Project SOIL EXPLORATION ALONG BASSETT CREEK

Boring No BC-14A Sample No — Depth (ft) 5.5-7 Type of Sample 3" ø

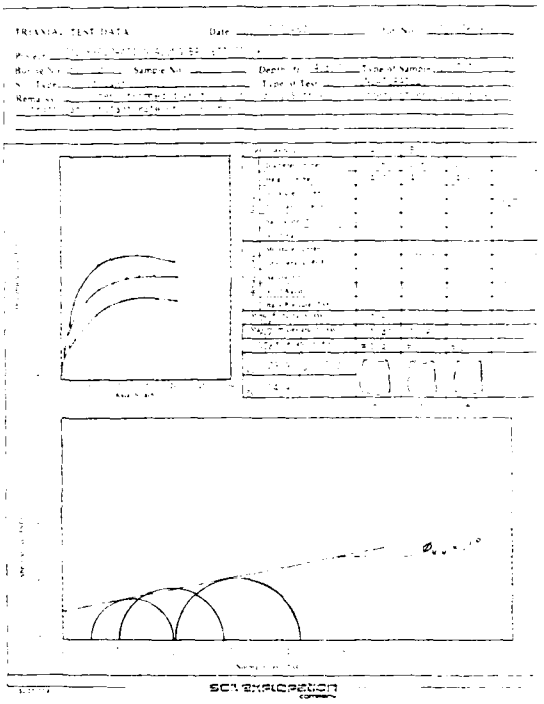
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Diameter, inches	2.00	2.00	2.00
Height, inches	4.00	4.00	4.00
Moisture Content %	18.4	18.4	18.4
Dry Density PCF	105.9	105.9	105.9
Saturation %			
Void Ratio			
Moisture Content %			
Dry Density PCF			
Saturation %	@ 100	@ 100	@ 100
Void Ratio			
Back Pressure TSF			
Minor Principal Stress TSF	0.50	1.00	1.50
Max Deviator Stress TSF	1.23	1.35	1.44
Ultimate Deviator Stress TSF	@ 1.19	@ 1.35	@ 1.72
LL			
PL			



SOIL EXPLORATION



SOIL EXPLORATION

SYMBOL	DESCRIPTION	DATE	APPROVED

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY:
DRAWN BY:
CHECKED BY:
SUBMITTED BY:
APPROVED:

PHASE II — DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
EDGEWOOD EMBANKMENT
BORINGS BC-10A, BC-10B, BC-10C & BC-14C

DATE
AUGUST 1982

DRAWING NUMBER
M34.3-R-5/266
SHEET C-38 OF C-39

SAFETY PAYS

2

TRIAXIAL TEST DATA

Date

Job No

Project

Boring No

Sample No

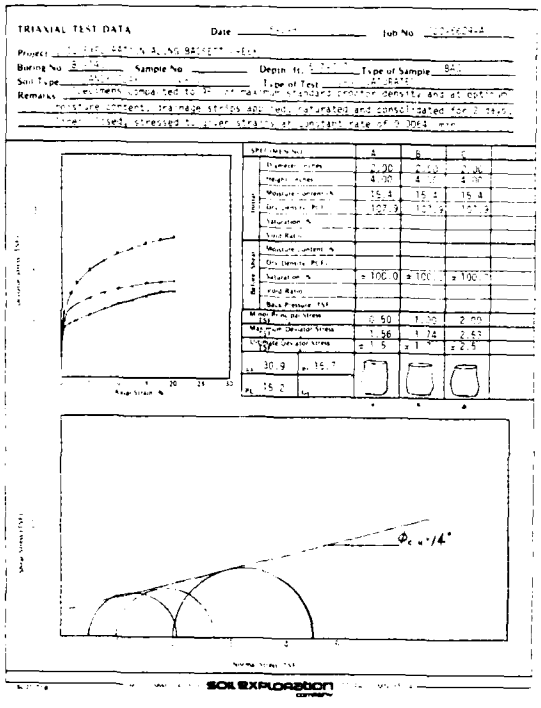
Depth (ft)

Type of Sample

Soil Type

Type of Test

Remarks



SOIL EXPLORATION COMPANY

TRIAXIAL TEST DATA

Date

Project

Boring No

Sample No

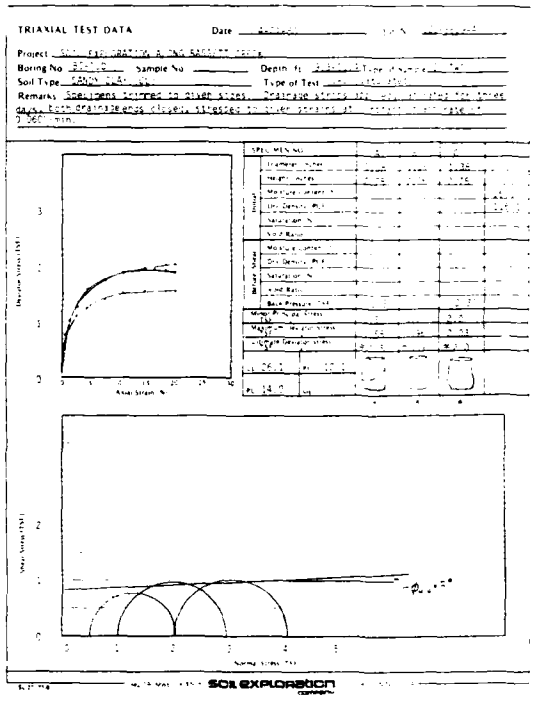
Depth (ft)

Type of Sample

Soil Type

Type of Test

Remarks



SOIL EXPLORATION COMPANY

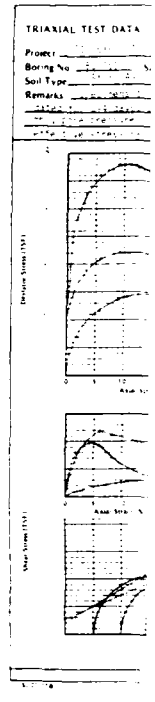
TRIAXIAL TEST DATA

Project

Boring No

Soil Type

Remarks



TRIAXIAL TEST DATA

Date

Job No

Project

Boring No

Sample No

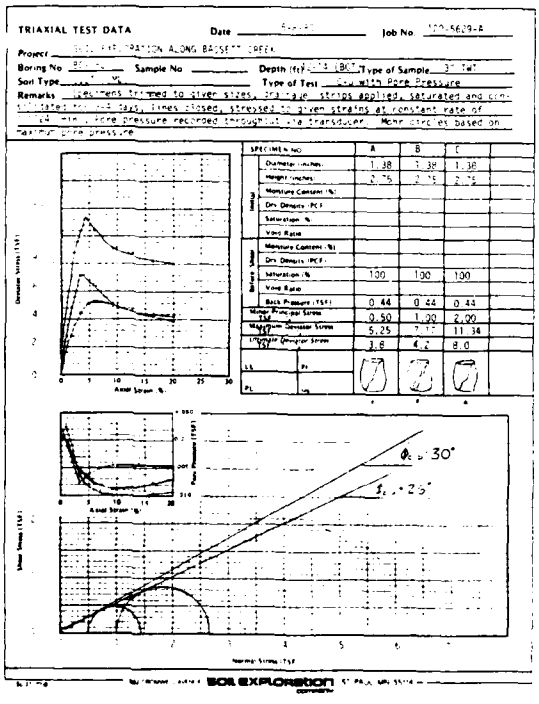
Depth (ft)

Type of Sample

Soil Type

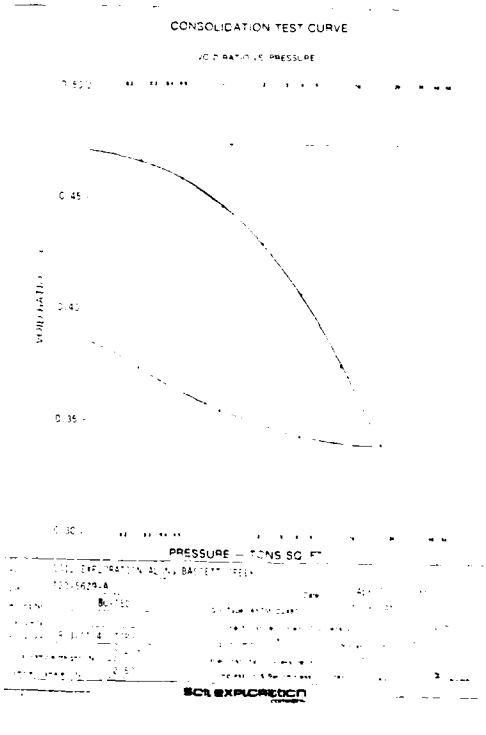
Type of Test

Remarks



SOIL EXPLORATION COMPANY

CONSOLIDATION TEST CURVE



PRESSURE — TONS SQ. FT.

SOIL EXPLORATION COMPANY

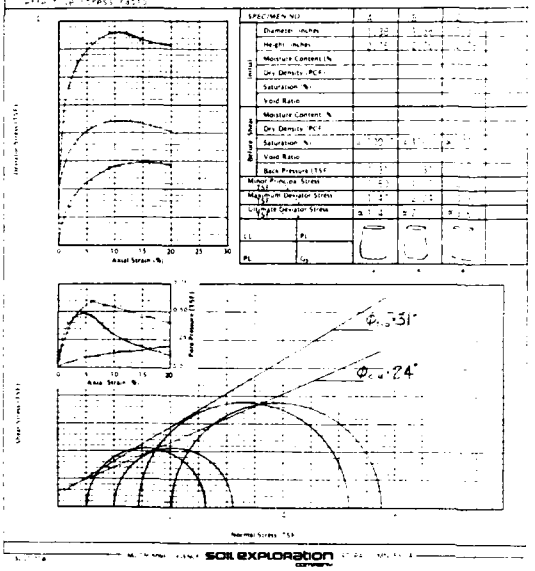
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Project

Boring No Sample No Depth ft Type of Sample

Soil Type Type of Test

Remarks



SOIL EXPLORATION

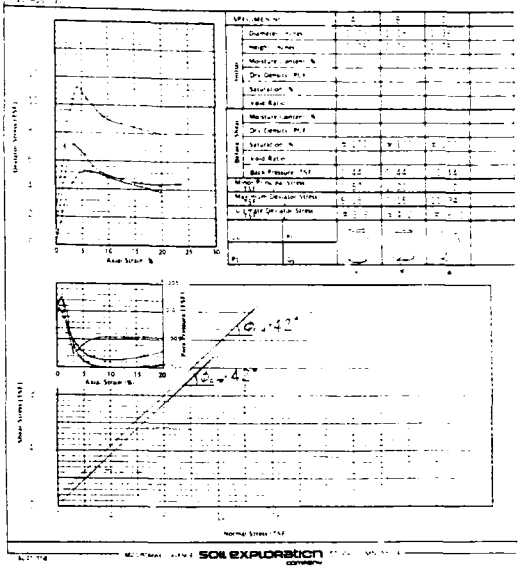
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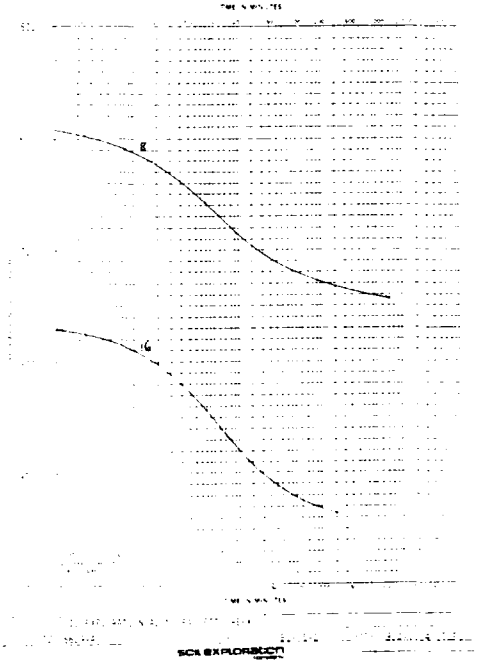
Soil Type Type of Test

Remarks



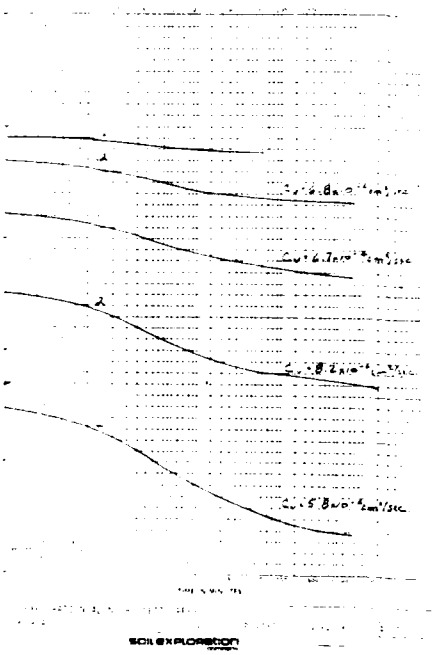
SOIL EXPLORATION

CONSOLIDATION TEST TIME CURVES



SOIL EXPLORATION

CONSOLIDATION TEST TIME CURVES



SOIL EXPLORATION

SYMBOL	DESCRIPTION	DATE	APPROVAL

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT, CORPS OF ENGINEERS
ST PAUL, MINNESOTA

DESIGNED BY:
DRAWN BY:
CHECKED BY:
SUBMITTED BY:
APPROVED:

PHASE II DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA
SOIL TEST DATA
EDGEWOOD EMBANKMENT
BORINGS BC-14C AND BC-15D

DATE: AUGUST 1982

SCALE DRWG NO

DRAWING NUMBER
M34.3-R-5/267

SHEET 0-39 OF 0-39

SAFETY PAYS

DEPARTMENT OF THE ARMY
St. Paul District Corps of Engineers
1135 U.S. Post Office & Custom House
St. Paul, Minnesota 55101

FLOOD CONTROL

BASSETT CREEK

HENNEPIN COUNTY, MINNESOTA

DESIGN MEMORANDUM NO. 2, PHASE II

APPENDIX D

STRUCTURAL

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APPENDIX D

STRUCTURAL

1. This appendix describes the methods used in designing the retaining walls, drop structure, control structures, railroad bridge, tunnel, tunnel inlet structure, and interior drainage interceptors. Supporting information is included, such as design criteria, basic data and assumptions, loads and loading conditions.

DESIGN CRITERIA

2. WORKING STRESSES

Working stresses used in the design conform to those specified in Engineering Manual (EM) 1110-1-2101, dated November 1963, "Working Stresses for Structural Design", including changes 1 and 2. Loading conditions, design assumptions and other criteria are based on the applicable parts of the following references:

a. EM 1110-2-2000, Standard Practices for Concrete (including changes through 4, 30 July 1973).

b. EM 1110-2-2103, Details of Reinforcement and Hydraulic Structures (May 1971).

c. EM 1110-2-2902, Conduits, Culverts, and Pipes (including changes through 3, March 1969).

d. EM 1110-2-2906, Design of Pile Structures and Foundations (July 1958).

e. EM 1110-2-2502, Retaining Walls (including changes through 3, January 1965).

f. ETL 1110-2-265, Strength Design Criteria for Reinforced Concrete Hydraulic Structures (15 September 1981).

3. REINFORCED CONCRETE STRUCTURES

Reinforced concrete structures were designed by the strength method. The structures will be constructed of concrete having a 28-day minimum compressive strength of 4,000 pounds per square inch (psi).

4. CONCRETE REINFORCEMENT

Concrete reinforcement design was based on the use of deformed billet-steel bars conforming to the requirements of ASTM A615-72, Grade 40, with a minimum yield stress of 40,000 psi. Concrete cover and minimum reinforcement will comply with EM 1110-2-2103. The spacing, splicing and bending requirements of the bars will be in accordance with ACI-318-77 and the Manual of Standard Practice for Detailing Reinforced Concrete Structures.

5. ALUMINUM

Aluminum for the trash racks will be an alloy known commercially as 6061-T6. Working stresses used in the design in accordance with the EM 1110-1-2101.

6. STEEL SHEET PILING

Steel for sheet piling will conform to the requirements of the American Society for Testing Material A-328-75. The maximum working stress used in the design of sheet piling was limited to 0.5 Fy (yield strength) in accordance with Guide Specification CW-02411,February 1976. The yield strength of the sheet piling was assumed at 33,000 psi.

7. MISCELLANEOUS STRUCTURAL STEEL

Steel for miscellaneous structural steel will be in accordance with ASTM A-36. Working stresses for the design of structural steel will be in accordance with EM 1110-1-2101 for hydraulic structures.

SOILS DATA

8. SOILS CONSTANTS

Table 1 gives the soils constants of the foundation material used in the design of typical structures. The use of this soil data is explained in the description of the design of typical structures.

DESIGN OF STRUCTURES

9. GENERAL

The design of the retaining walls, drop structure, culverts, control structures, tunnel and tunnel inlet is described in the following paragraphs in sufficient details to explain the application of the design criteria and assumptions used.

10. RETAINING WALLS

The design of the cantilever sheet pile retaining walls in the Fruen Mill area was accomplished using computer program CSHTWAL (called X0031 in the CORPS Library). See Plates 15 and 17 for details. The walls were designed with the water at the design water surface elevation and at normal water surface elevation. The effect of the railroad was applied at the applicable section. The resulting pile penetration are those required for stability but increased by 20 percent.

11. DROP STRUCTURE

The drop structure's concrete thicknesses were designed to satisfy requirements for flexure and shear. Overturning stability, foundation pressures and sliding stability factor of safety were determined. For normal water surfaces, the sliding factor of safety is 3.8; for design water surface, the factor of safety is 3.0. Both satisfy the requirements of ETL 1110-2-256. The maximum soil pressure is .79 ksf which is well within the allowable soil pressure. See Plates 15 and 19 for the details of the drop structure.

TABLE 1

SOILS DATA
BASSETT CREEK

Location	Soil-Type Backfill	∅	Unit Weight Sliding Stab. Moist pcf	Saturated pcf	C pcf	∅	Bearing Capacity tsf
Highway 100	sand	35°	115	125	0	35	4*
Edgewood	clay	30°	121	128	750	10°	3
Golden Valley CC	clay	30°	121	128	750	10°	2
Highway 55	sand	30°	115	125			2
		*Excavate beneath structure					
Tunnel	Qmq (CH)		106	43.6	600	20°	
	Qmz (SP)			130		30°	
	Quvf (CL)			125	1500-2000	27°	
	Soft (CH)			106	500	20°	
Medicine Lake	OL			87		34	
	SC			140-145++		35°-37°++	
	CL			134-140++		32°++	
	OH			85++		34°++	
	SC-SM			145++		37°++	
Fruen Mill	Sand Backfill		120	130		30	
	Foundation		120	130	0	27	
			120	130	4000	0	

++ Estimated.

12. RAILROAD BRIDGE

The railroad bridge was designed according to the applicable portions of the American Railway Engineering Association (AREA) Manual for Railway Engineering. See Plates 15 and 19 for details. Two different bridges were designed to determine the most cost effective structure. A single span welded plate girder bridge proved to be more expensive than a two span rolled girder bridge. Both alternatives were analyzed using the computer program Analysis and Rating of Plate Girder Bridges (called X0045 in the CORPS Library). The abutment was designed for at-rest conditions. Pile loads were determined by the classical method. The pile penetration was determined by skin friction.

The concrete dimensions of the intermediate pier were checked for flexure and shear. The pile penetration was determined assuming that the piles are friction piles.

The bridge was designed for a Cooper E-80 loading but after discussions with the Minneapolis Northfield and Southern Railroad they felt that designing for Copper E-60 would be adequate. The bridge is on a spur line and due to the condition of the roadway, train speeds do not exceed 10 miles per hour.

13. TUNNEL

The design of the tunnel was of a preliminary nature since a Feature Design Memorandum will be prepared for this feature. Three different tunnel sections were designed using loads determined by the formulas presented in "Analysis of Ground Liner Interaction for Tunnels" prepared for the U.S. Department of Transportation by the University of Illinois. The report is dated October 1978. The loads were used in the computer program "NEWTUN" (X0055 in the CORPS Library) to determine the moments, shears and axial loads for the design of the tunnel lining. The Feature Design Memorandum will explore precast vs. cast-in-place tunnel liners. See Plate 8 for the different tunnel sections.

14. TUNNEL INLET

The tunnel inlet was not designed but was drawn based upon experience with similar structures that have been used successfully in the past. See Plate 9 for the site plan and details.

15. CONTROL STRUCTURES

The control structures' concrete thicknesses were designed for flexure and shear. Overturning stability was checked and the foundation pressures were determined. All were within the allowables. The design computations are not included since they are similar to the drop structure computations.

16. CULVERTS

All culverts were designed according to EM 1110-2-2902. The precast reinforced concrete culverts will have watertight joints. Wherever the culvert is used at an embankment, the culvert will have tied joints. The design of the pre-cast box culvert was based upon ASTM D 789.

17. MEDICINE LAKE CONTROL STRUCTURE

The control structure is a sheet pile weir capped with concrete. See Plate 34 for details. The computer program "Soil Structure Interaction Analysis of Sheetpiles"(X0060 in the CORPS Library) was used for the analysis due to the multi-layered soils. A penetration into the sand layer was assumed and then bending moments, shears and deflections were computed.

18. 215 WORK ITEMS

The design computations for structures covered under the 215 agreement have previously been submitted.

19 TYPICAL DESIGN COMPUTATIONS

	Pages
Drop Structure	D6 - D23
Retaining Wall	D24 - D33
Railroad Bridge	D34 - D66
Medicine Lake Control Structure	D67 - D70
Tunnel Design	D71 - D100

(JUNE 79 (REPLACES NCO 14, 1 APR 69)
NCS 14

ST. PAUL DISTRICT COMPUTATION SHEET	DATE Mar '82	PAGE 1 OF 2	FILE NUMBER
NAME OF OFFICE		COMPUTATION Stability Analysis	
SUBJECT Bassett Creek - Truen mill drop structure near		SOURCE DATA	
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	

Concrete Weights

Sym	Factors	+ ↓ Forces (lb) →		Arm (Ft)	↺+ Moments (Ft-Lb) ↻	
C_1	25'(37')(2')(150 lb/ft³)	277,500		12.5	3,468,750	
C_2	30'(5.7')(1')(150 lb/ft³)	25,650		19.5	500,175	
C_3	30'(0.9')(1')(150 lb/ft³)	4,050		5.5	22,275	
C_4	25'(12.4')(1.5')(150 lb/ft³)	69,750		12.5	871,875	
C_5	25'(7.4')(1.5')(150 lb/ft³)	41,625		12.5	520,313	
C_6	½(7.5')(2.5')(1.5')(150)	2,109		11.0	23,203	
C_7	11.5'(2.5')(1.5')(150)	6,469		19.25	124,523	
C		427,153		12.949	5,531,114	

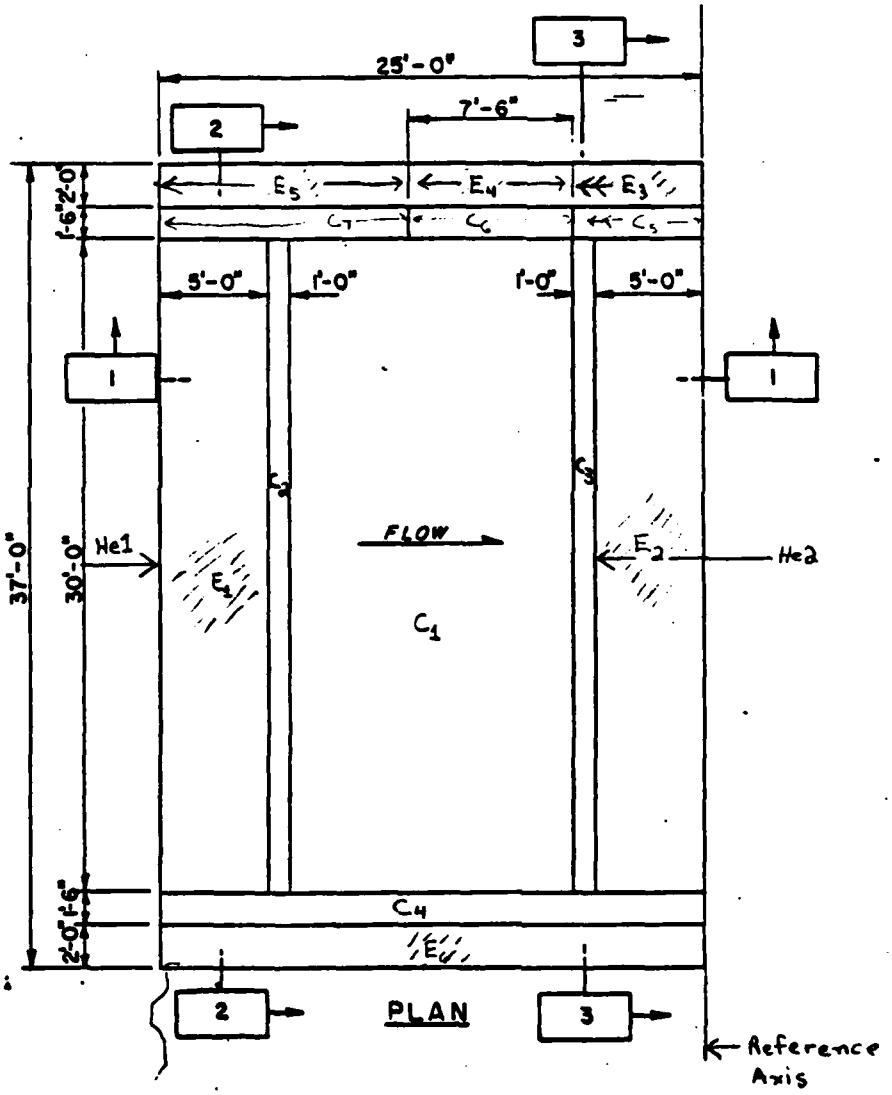
Case 1) Construction Case (No water) $\gamma_M = 120$ lb/ft³, $\gamma_{sat} = 125$ lb/ft³, $\phi = 30°$

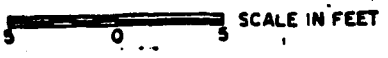
Sym	Factors	+ ↓ Forces (Lb) →		Arm	↺+ Moments (F-K) ↻	
E_1	30'(5')(2.4')(120 lb/ft³)	43,200		22.5	972,000	
E_2	30'(5')(0.9')(120 lb/ft³)	16,200		2.5	40,500	
E_3	25'(7.4')(2')(120 lb/ft³)	44,400		12.5	555,000	
E_4	½(7.5')(2.5')(2')(120)	2,250		11.0	24,750	
E_5	11.5'(2.5')(2')120 lb/ft³	6,900		19.25	132,825	
E_6	25'(12.4')(2')(120 lb/ft³	74,400		12.50	930,000	
He1	$\frac{(120)(4.4')^2}{2}(0.5)(30')$		17,424	1.47		-25,555
He2	$\frac{(120)(2.9)^2}{2}(0.5)(30)$		- 7,569	0.97	7,317	
C	(See above)	427,153		12.949	5,531,114	
		614,503	9,855		8,193,506	-25,555

$\Sigma M = 8,167,951$

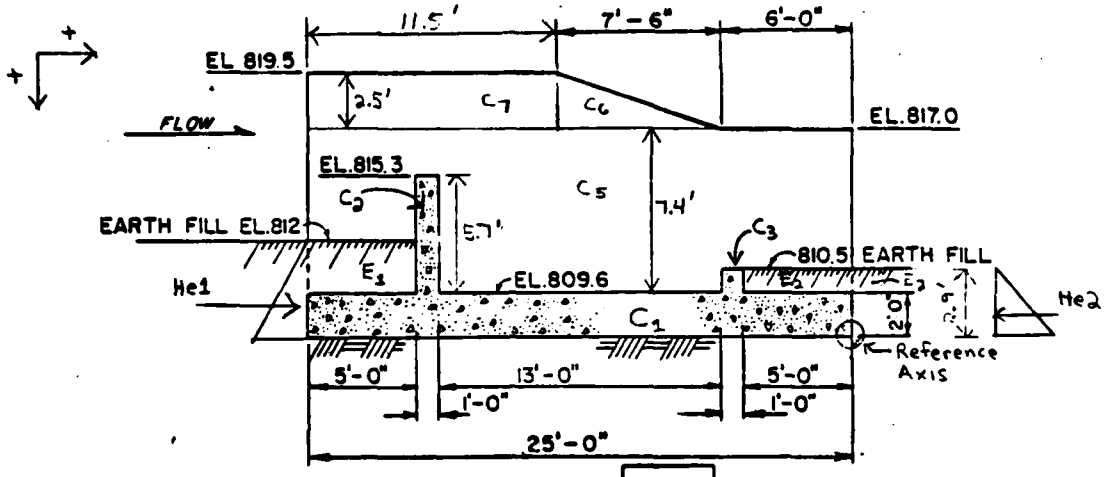
$He1 = \frac{\gamma_m H^2}{2} \cdot K_p \cdot L$

ST. PAUL DISTRICT COMPUTATION SHEET	DATE Mar '82	PAGE 2 OF 18	FILE NUMBER
NAME OF OFFICE		COMPUTATION Stability Analysis	
SUBJECT Bassett Creek Drop Structure near Fruen mill		SOURCE DATA	
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	

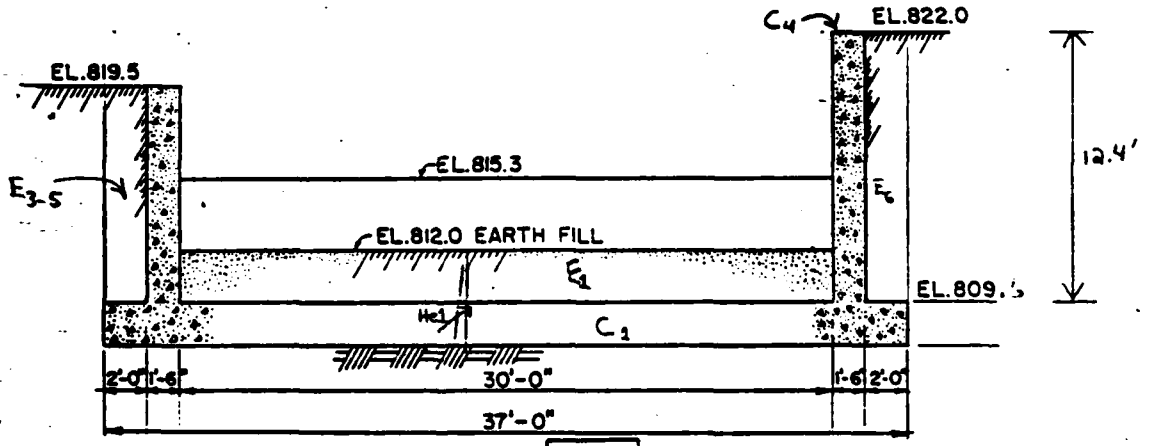




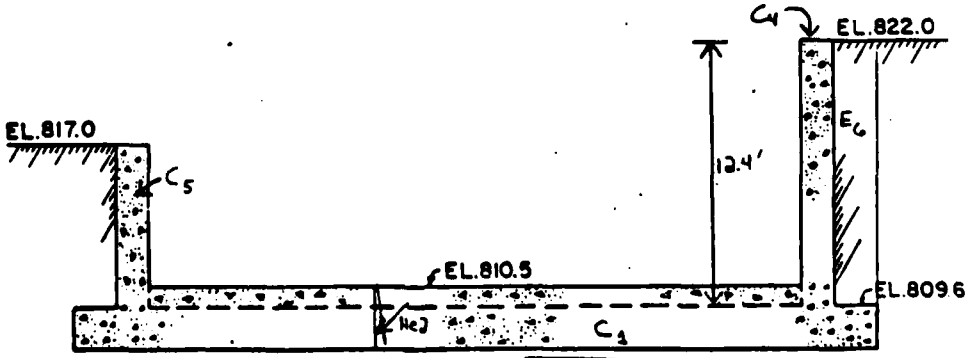
ST. PAUL DISTRICT COMPUTATION SHEET	DATE Mar '82	PAGE 3 OF 18	FILE NUMBER
NAME OF OFFICE		COMPUTATION Stability Analysis	
SUBJECT Bassett Creek Drop Structure near Fruen mill		SOURCE DATA	
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	



SECTION 1



SECTION 2



SECTION 3

D-8

NCS 14
1 JUNE 79 (REPLACES NCD 14, 1 APR 63)

ST. PAUL DISTRICT COMPUTATION SHEET	DATE Mar '82	PAGE 4 OF 18	FILE NUMBER
NAME OF OFFICE	COMPUTATION Stability Analysis		
SUBJECT Bassett Creek Drop Structure near Friesen mill	SOURCE DATA		
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	

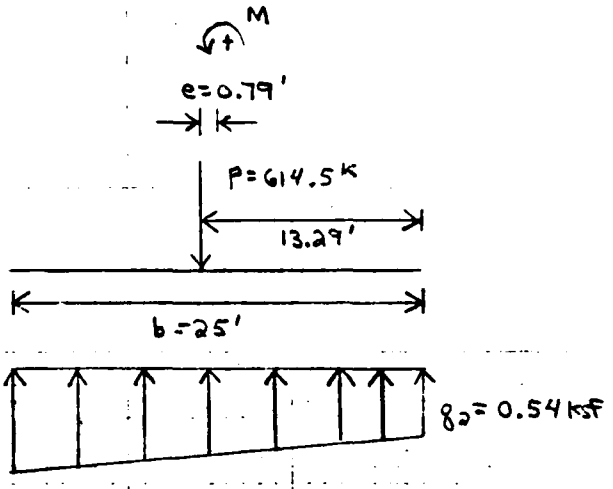
- Find Soil Pressure for Case 1

(See pg 8-64 "CE Handbook")

$$\text{arm} = \frac{\Sigma M}{\Sigma V} = \frac{8,167,951}{614,503} = 13.29$$

(in Kern)

$A = 25' \times 37' = 925\ ft^2$



$$q_1 = \frac{P}{A}\left(1 + \frac{6e}{b}\right)$$

$$q_2 = \frac{P}{A}\left(1 - \frac{6e}{b}\right)$$

$$q_1 = \frac{614.5^K}{925\ ft^2}\left(1 + \frac{6(0.79')}{25'}\right) = 0.79\ Ksf$$

$$q_2 = \frac{614.5}{925}\left(1 - \frac{6(0.79)}{25}\right) = 0.54\ Ksf$$

NCS 14
1 JUNE 79 (REPLACES NCD 14, 1 APR 63)

ST. PAUL DISTRICT COMPUTATION SHEET	DATE Mar'82	PAGE 5 OF 13	FILE NUMBER
NAME OF OFFICE		COMPUTATION Stability Analysis	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	

- Case 2) Pool = 815.8 , T.W. = 811.0 , $\gamma_M = 120^{Lb/Ft^3}$, $\gamma_{Sat} = 125^{Lb/Ft^3}$

$\phi = 30°$, $\gamma_{Sub} = \gamma_{Sat} - \gamma_{water}$

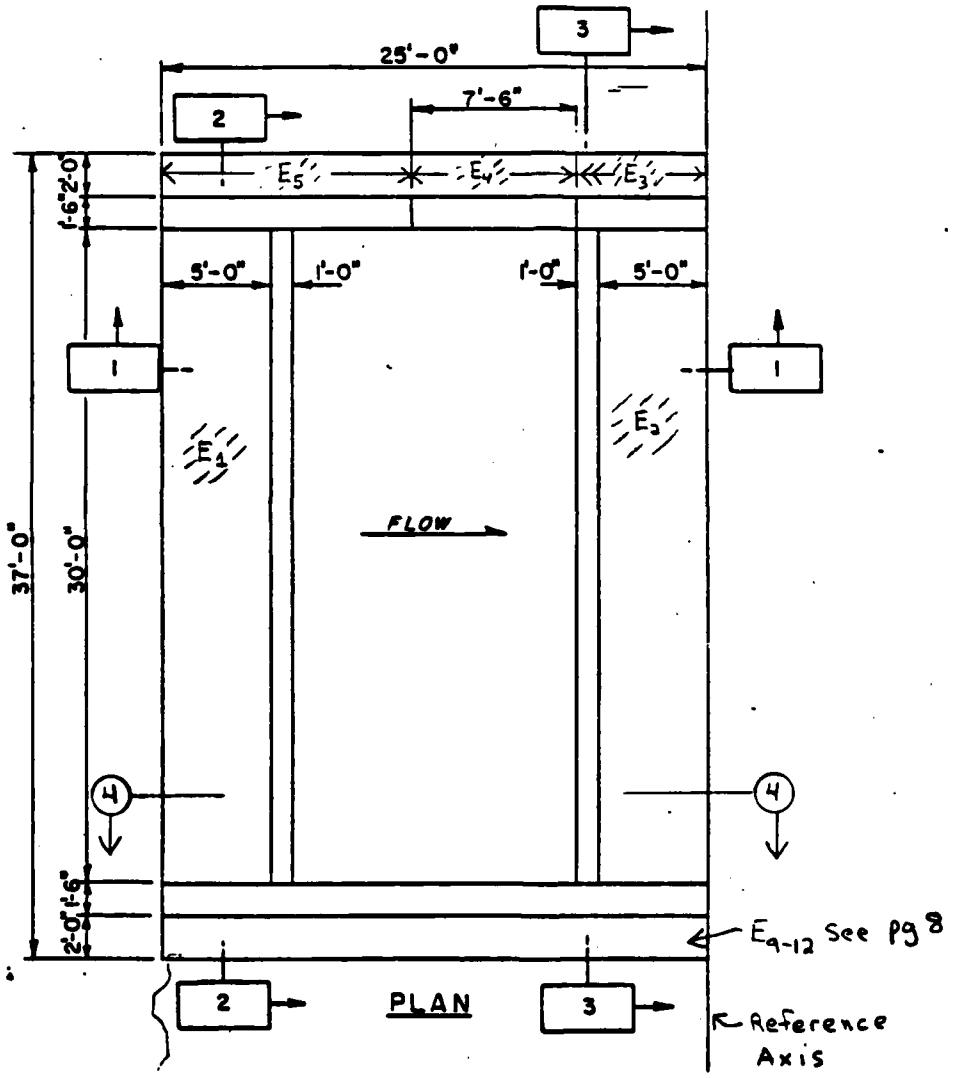
$= 125 - 62.5 = 62.5^{Lb/Ft^3}$

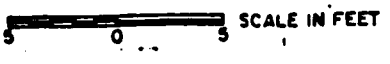
$He1 = \frac{\gamma_M H^2}{2} \cdot K_a \cdot L$

$Hw1 = \frac{1}{2} \gamma_w H^2 \cdot L$

	Sym	Factors	+↓ Forces (Lb)	+→	Arm (Ft)	+↺ Moments (Ft-Lb)	±
	C	(See pg 1)	427,153		12.949	5,531,114	
	E_1	$5'(30')(2.4')(62.5^{Lb/Ft^3})$	22,500		22.5	506,250	
	E_2	$5'(30')(0.9')(62.5^{Lb/Ft^3})$	8,438		2.5	21,094	
	E_3	$2'(25')(1.4')(62.5^{Lb/Ft^3})$	4,375		12.5	54,688	
	E_4	$\frac{1}{2}(4.8')(25')(2')(62.5)$	7,500		$\frac{2}{3}(25)$	125,000	
	E_5	$\frac{1}{2}(4.8')(25')(2')(120)$	14,400		$\frac{1}{3}(25')$	120,000	
	E_6	$2'(1.2')(25')(120^{Lb/Ft^3})$	7,200		12.5	90,000	
	E_7	$\frac{1}{2}(7.5')(2.5')(2')(120)$	2,250		11.0	24,750	
	E_8	$2'(2.5')(11.5')(120)$	6,900		19.25	132,825	
	E_9	$1.4'(2')(25')(62.5)$	4,375		12.5	54,688	
	E_{10}	$\frac{1}{2}(25')(4.8')(2')(62.5)$	7,500		$\frac{2}{3}(25)$	125,000	
	E_{11}	$\frac{1}{2}(25')(4.8')(2')(125)$	15,000		$\frac{1}{3}(25)$	125,000	
	E_{12}	$2'(25')(6.2')(120)$	37,200		12.5	465,000	
-	W_1	$5'(30')(6.2')(62.5^{Lb/Ft^3})$	58,125		22.5	1,307,813	
-	W_2	$(5.5')(30')(1.4')(62.5)$	14,438		16.25	234,609	
-	W_5	$\frac{1}{2}(5.5')(4.8')(30')(62.5)$	24,750		17.17	424,875	
-	W_3	$(7.5')(30')(1.4')(62.5)$	19,688		9.75	191,953	
-	W_6	$(5')(0.9')(30')(62.5)$	8,438		2.5	21,094	
-	W_4	$6'(30')(0.5')(62.5^{Lb/Ft^3})$	5,625		3.0	16,875	
	He1	$\frac{(62.5^{Lb/Ft^3})(4.4')^2}{2}(0.5)(30')$		9,075	$\frac{1}{3}(4.4')$		-13,310
	He2	$\frac{(62.5)(2.9)^2}{2}(0.5)(30')$		-3,942	$\frac{1}{3}(2.9)$	3,811	
	Hw1	$\frac{1}{2}(62.5^{Lb/Ft^3})(8.2)^2(30')$		63,038	$\frac{1}{3}(8.2')$		-172,303
	Hw2	$\frac{1}{2}(62.5)(3.4)^2(30')$		-10,838	$\frac{1}{3}(3.4)$	12,283	
	U_1	(See pg 9)	-196,562		12.5		-2,457,025
	U_2	(See pg 9)	-138,750		$\frac{2}{3}(25')$		-2,312,500
			360,543	57,333		9,588,722	-4,955,138

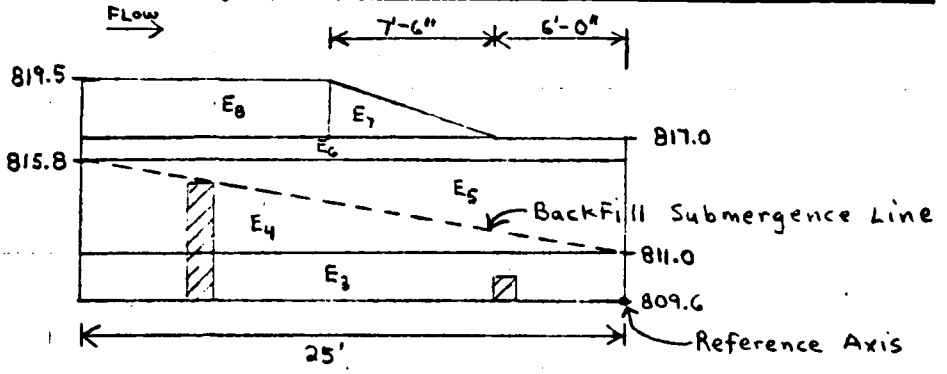
ST. PAUL DISTRICT COMPUTATION SHEET	DATE Mar '82	PAGE 6 OF 18	FILE NUMBER
NAME OF OFFICE		COMPUTATION Stability Analysis	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	



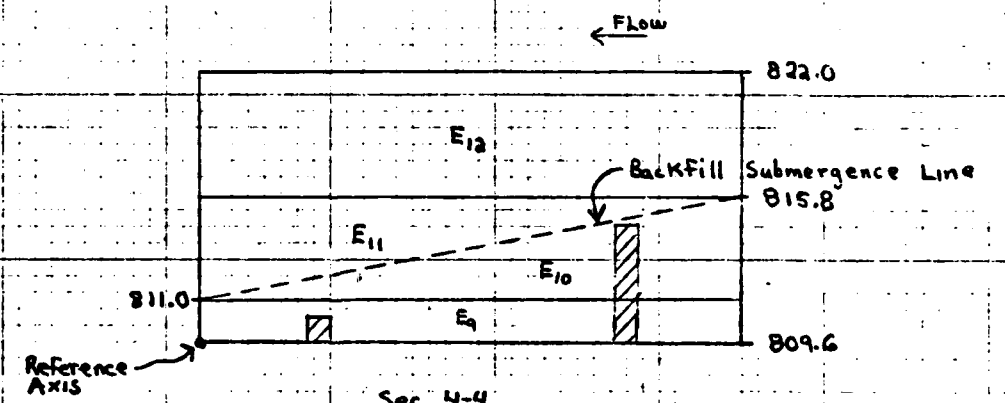


NCS 14
1 JUNE 79 (REPLACES NCD 14, 1 APR 63)

ST. PAUL DISTRICT COMPUTATION SHEET	DATE Mar '82	PAGE 7 OF 18	FILE NUMBER
NAME OF OFFICE	COMPUTATION Stability Analysis		
SUBJECT Bassett Creek	SOURCE DATA		
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	

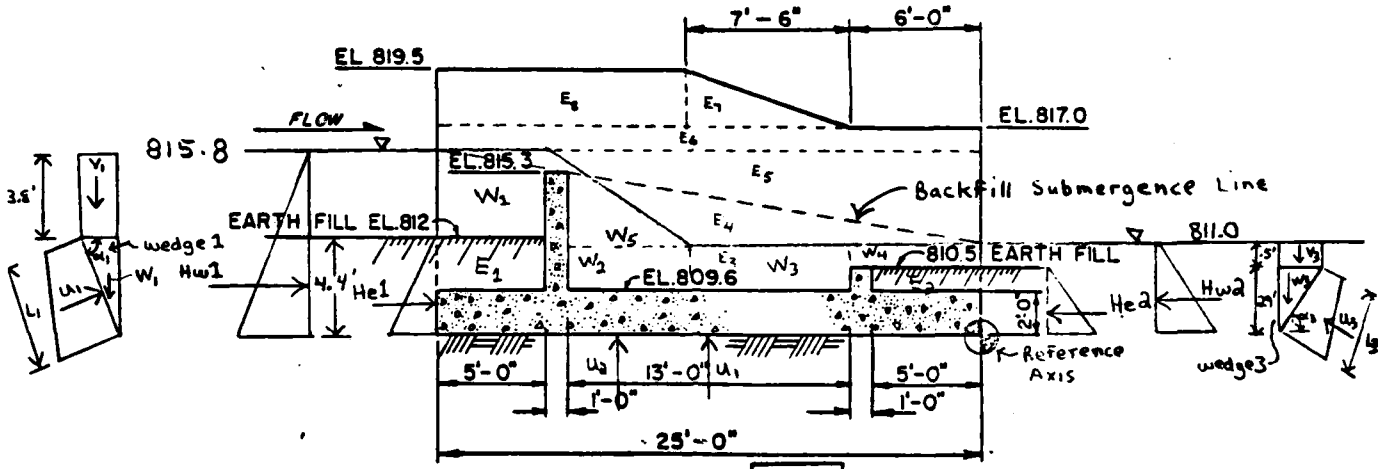


from Sec 1-1

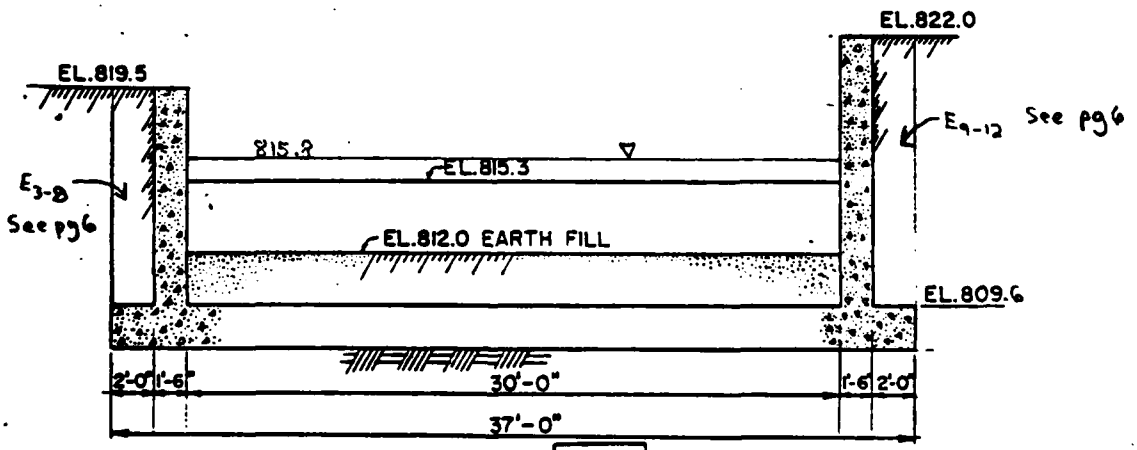


Sec. 4-4

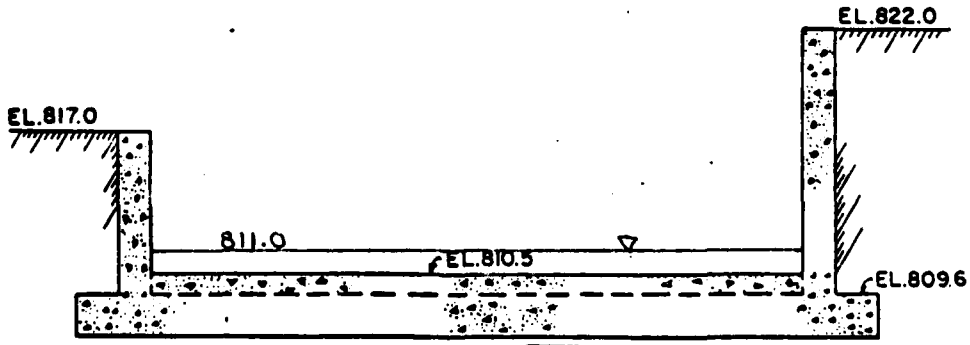
ST. PAUL DISTRICT COMPUTATION SHEET	DATE Mar '82	PAGE 8 OF 18	FILE NUMBER
NAME OF OFFICE		COMPUTATION	
SUBJECT Basset Creek		SOURCE DATA	
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	



SECTION 1



SECTION 2

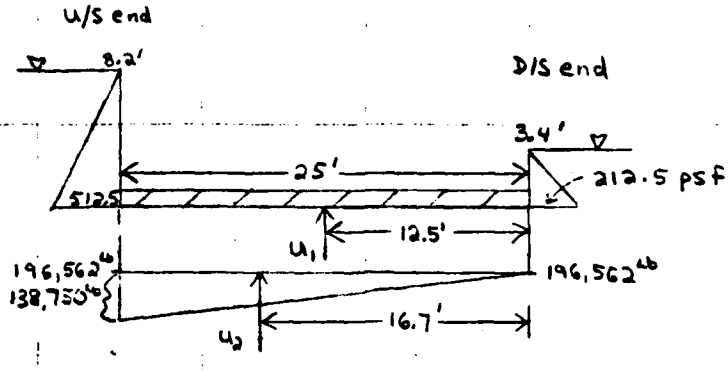


SECTION 3

D-13

ST. PAUL DISTRICT COMPUTATION SHEET	DATE Mar '82	PAGE 9 OF 18	FILE NUMBER
NAME OF OFFICE		COMPUTATION Stability Analysis	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	

- Find uplift forces:



Pressure at U/S end = $\gamma_w h = (62.5^{\#}/ft^3)(8.2') = 512.5$ psf

" " D/S " = $(62.5)(3.4) = 212.5$ psf

bottom area of slab = $(25')(37') = 925\ ft^2$

$U_1 = (212.5\ psf)(925\ ft^2) = 196,562^{\#}$

$U_2 = \frac{1}{2}(512.5 - 212.5\ psf)(925\ ft^2) = 138,750^{\#}$

- Find soil pressure for Case 2.

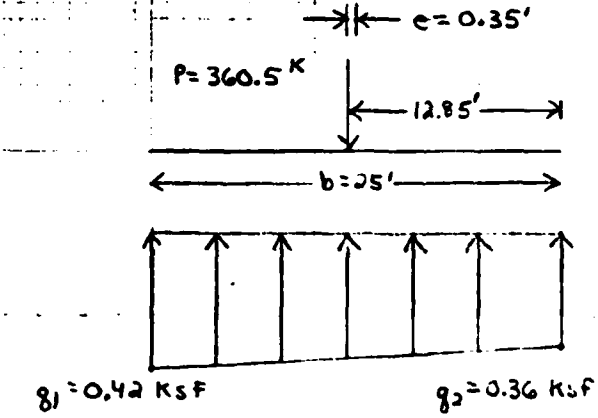
$arm = \frac{\Sigma M}{\Sigma V} = \frac{9,588,722 - 4,955,138}{360,543} = 12.85\ Ft$ (within kern)

$q_1 = \frac{P}{A}\left(1 + \frac{6e}{b}\right)$

$q_2 = \frac{P}{A}\left(1 - \frac{6e}{b}\right)$

$q_1 = \frac{360.5}{925\ ft^2}\left(1 + \frac{6(0.35)}{25'}\right) = 0.42\ ksf$

$q_2 = \frac{360.5}{925\ ft^2}\left(1 - \frac{6(0.35')}{25'}\right) = 0.36\ Ksf$



NCS 14
1 JUNE 79 (REPLACES NCO 14, 1 APR 63)

ST. PAUL DISTRICT COMPUTATION SHEET	DATE Apr '82	PAGE 10 OF 18	FILE NUMBER
NAME OF OFFICE	COMPUTATION Stability Analysis		
SUBJECT Bassett Creek	SOURCE DATA		
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	

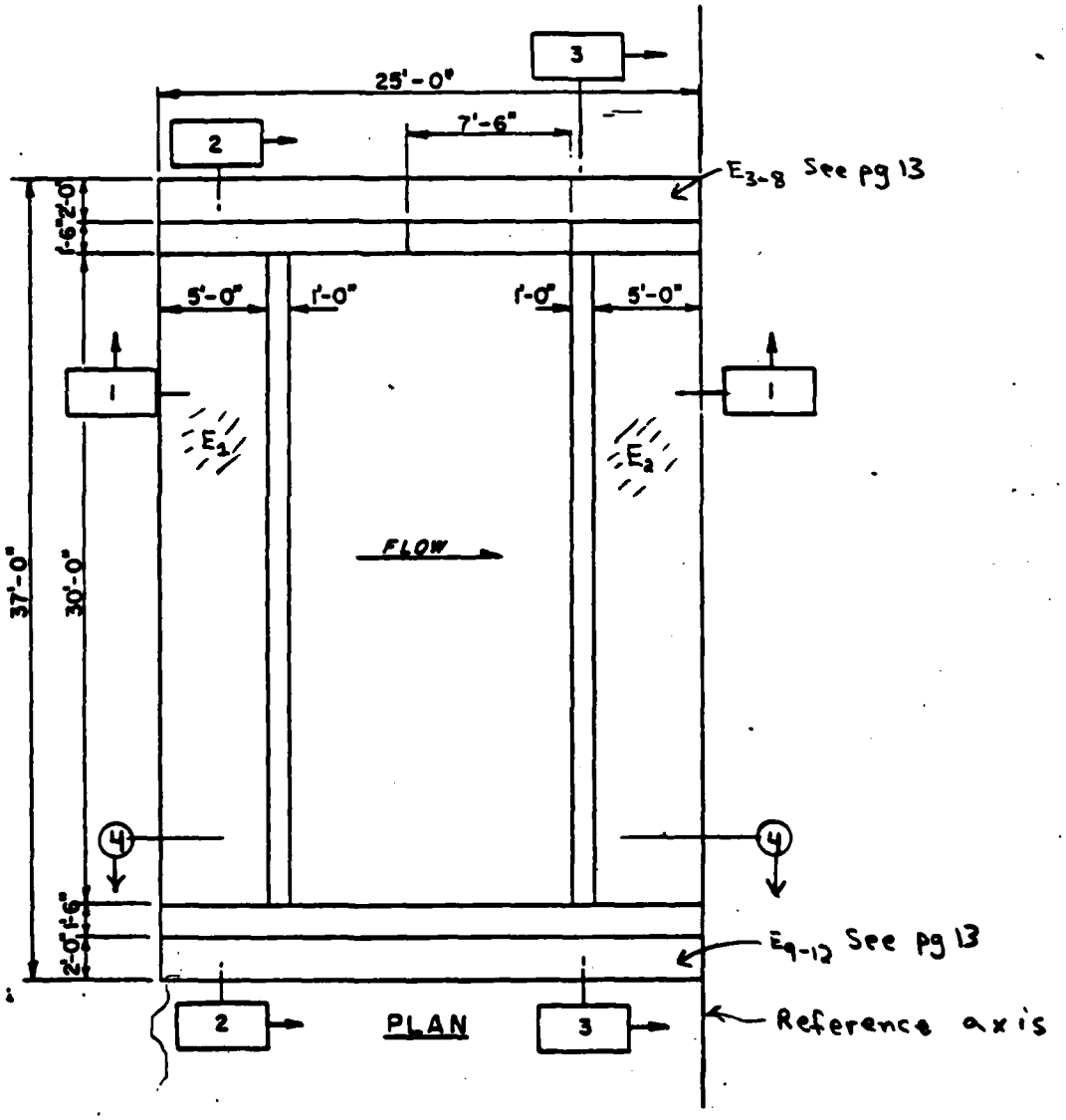
– Case 3) Pool = 819.0, T.W. = 815.9, $\gamma_M = 120^{lb/ft^3}$, $\gamma_{Sat} = 125^{lb/ft^3}$
$\Phi = 30°$, $\gamma_{sub} = 62.5^{lb/ft^3}$, $\gamma_w = 62.5^{lb/ft^3}$

$He1 = \frac{\gamma_w H^2}{2} \cdot K_o \cdot L$

$Hw1 = \frac{1}{2} \gamma_w H^2 \cdot L$

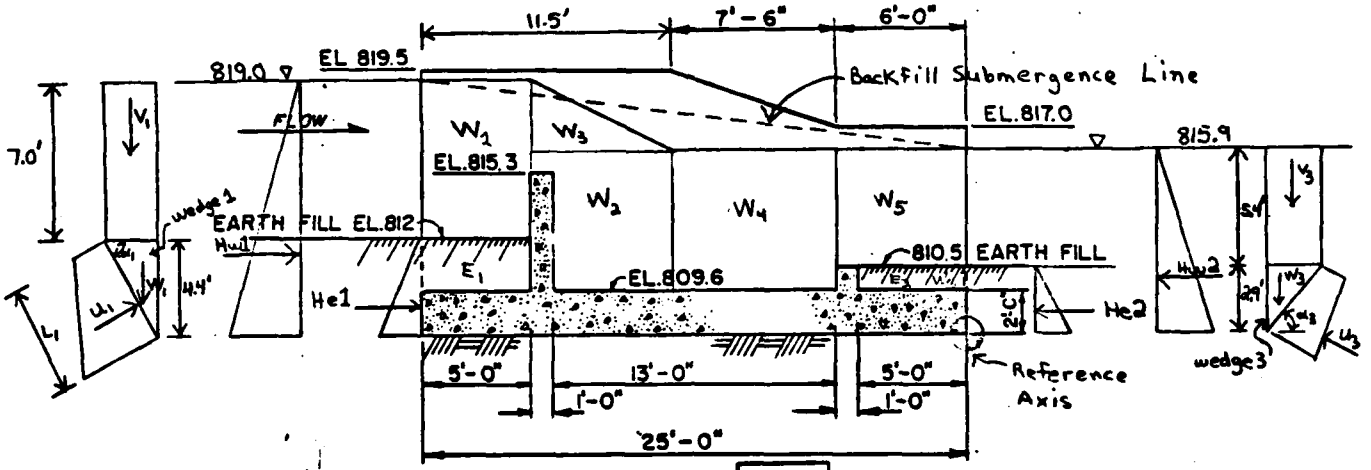
Sym.	Factors	+↓ Forces (Lb)	+ →	Arm (Ft.)	⊕ Moment (Ft-Lb) ⊖	
C	(See pg 1)	427,153		12.949	5,531,114	
E_1	(See pg 5)	22,500		22.5	506,250	
E_2	(See pg 5)	8,438		2.5	21,094	
E_3	$2'(25')(6.3')(62.5^{lb/ft^3})$	19,688		12.5	246,094	
E_4	$\frac{1}{2}(25')(3.1')(2')(62.5)$	4,844		$\frac{2}{3}(25')$	80,729	
E_5	$\frac{1}{2}(11.5')(1.4')(2')(120^{lb/ft^3})$	1,932		17.33	33,488	
E_6	$2'(11.5')(0.5')(120^{lb/ft^3})$	1,380		19.25	26,565	
E_7	$\frac{1}{2}(2.5')(2.0')(7.5')(120)$	2,250		11.0	24,750	
E_8	$\frac{1}{2}(6')(1.1')(2')(120)$	792		2.0	1,584	
E_9	$2(25')(6.3')(62.5)$	19,688		12.5	246,094	
E_{10}	$\frac{1}{2}(2')(25')(3.1')(62.5)$	4,844		$\frac{2}{3}(25')$	80,729	
E_{11}	$\frac{1}{2}(2')(25')(3.1')(120)$	9,300		$\frac{1}{3}(25')$	77,500	
E_{12}	$2'(25')(3')(120^{lb/ft^3})$	18,000		12.5	225,000	
W_1	$30'(5')(9.4')(62.5^{lb/ft^3})$	88,125		22.5	1,476,563	
W_2	$(30'\times 6.3')(5.5')(62.5)$	64,969		16.25	1,982,813	
W_3	$\frac{1}{2}(6.5')(3.1')(30')(62.5)$	18,891		17.83	336,883	
W_4	$7.5'(6.3')(30')(62.5)$	88,594		9.75	863,789	
W_5	$6'(5.4')(30')(62.5)$	60,750		3.0	182,250	
W_6	$5(0.9)(30)(62.5)$	8,438		2.5	21,094	
He1	(See pg 5)		9,075	$\frac{1}{3}(4.4)$		-13,612
He2	(See pg 5)		-3,942	$\frac{1}{3}(2.7)$	3,811	
Hw1	$\frac{1}{2}(62.5^{lb/ft^3})(11.4')^2(30')$		121,838	$\frac{1}{3}(11.4')$		-462,983
Hw2	$\frac{1}{2}(62.5^{lb/ft^3})(8.3')^2(30')$		-64,584	$\frac{1}{3}(8.3)$	178,683	
U_1	(See pg 14)	-479,844		12.5		-5,998,050
U_2	(See pg 14)	-89,609		$\frac{2}{3}(25')$		-1,493,490
		301,123	62,387		12,146,877	-7,968,135

ST. PAUL DISTRICT COMPUTATION SHEET	DATE Apr '82	PAGE 11 OF 18	FILE NUMBER
NAME OF OFFICE		COMPUTATION Stability Analysis	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	

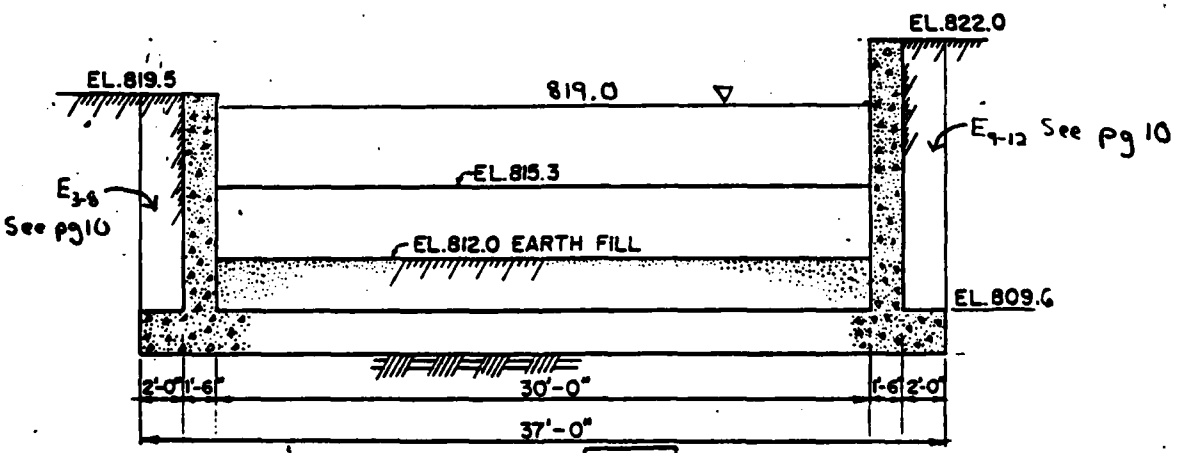




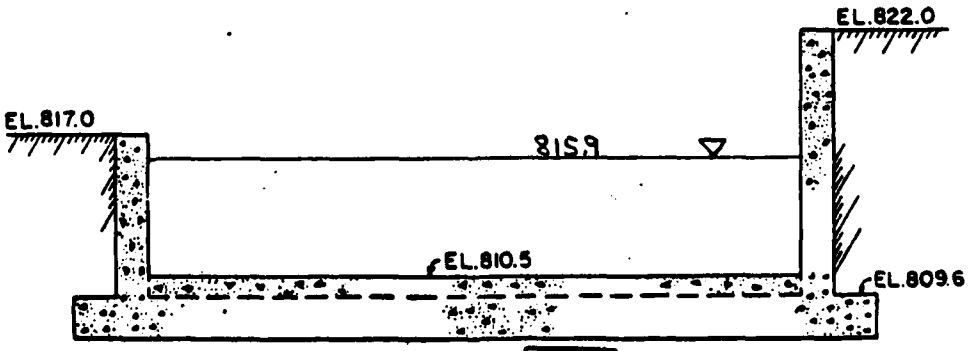
ST. PAUL DISTRICT COMPUTATION SHEET	DATE Apr '82	PAGE 12 OF 18	FILE NUMBER
NAME OF OFFICE		COMPUTATION Stability Analysis	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	



SECTION 1



SECTION 2

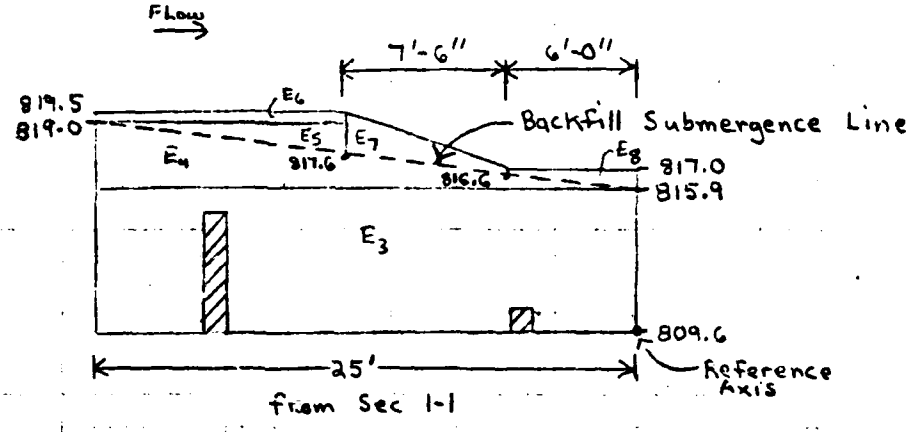


D-17

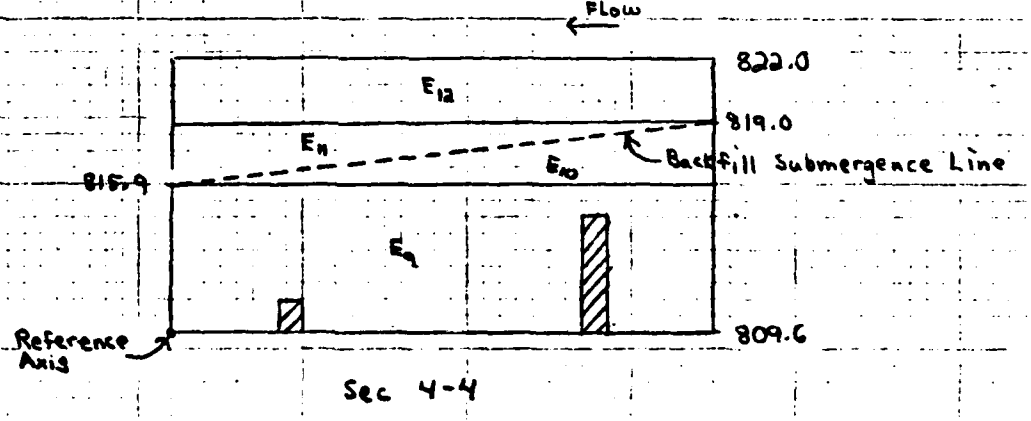
SECTION 3

NCS 14
1 JUNE 79 (REPLACES NCD 14, 1 APR 63)

ST. PAUL DISTRICT COMPUTATION SHEET	DATE Apr '82	PAGE 13 OF 18	FILE NUMBER
NAME OF OFFICE	COMPUTATION Stability Analysis		
SUBJECT Bassett Creek	SOURCE DATA		
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	



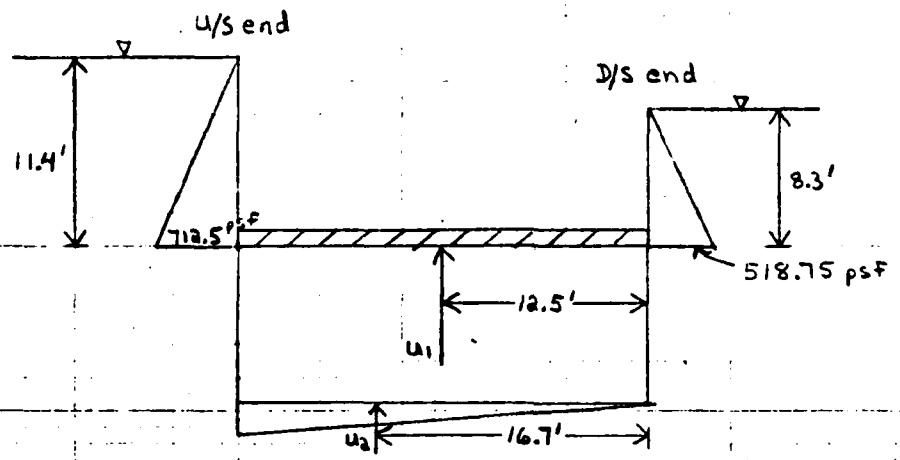
from Sec 1-1



Sec 4-4

ST. PAUL DISTRICT COMPUTATION SHEET	DATE Apr '82	PAGE 14 OF 18	FILE NUMBER
NAME OF OFFICE	COMPUTATION Stability Analysis		
SUBJECT Bassett Creek	SOURCE DATA		
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	

-Find uplift forces:



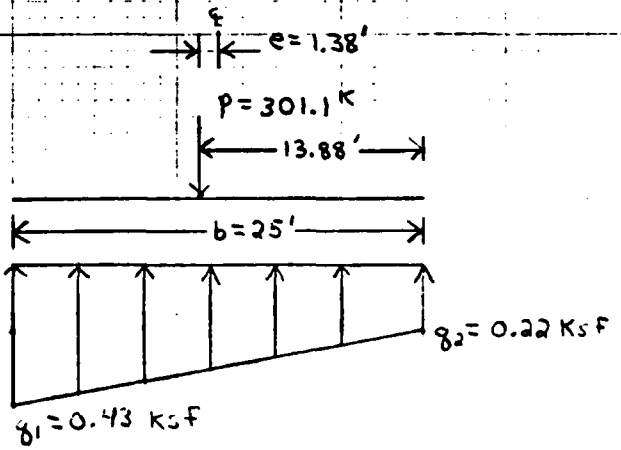
Pressure at U/S end = $\gamma_w h = (62.5\ lb/ft^3)(11.4\ ft) = 712.5$ psf
Pressure at D/S end = $(62.5\ lb/ft^3)(8.3\ ft) = 518.75$ psf

bottom area of slab = $25' \times 37' = 925\ ft^2$
$U_1 = (518.75\ psf)(925\ ft^2) = \underline{479,844\ lb}$
$U_2 = \frac{1}{2}(712.5 - 518.75\ psf)(925\ ft^2) = \underline{89,609\ lb}$

-Find soil pressure for case 3

$arm = \frac{\Sigma M}{\Sigma V} = \frac{12,146,877 - 7,968,135\ ft\text{-}lb}{301,123\ lb}$

$= 13.88$ Ft (within kern)



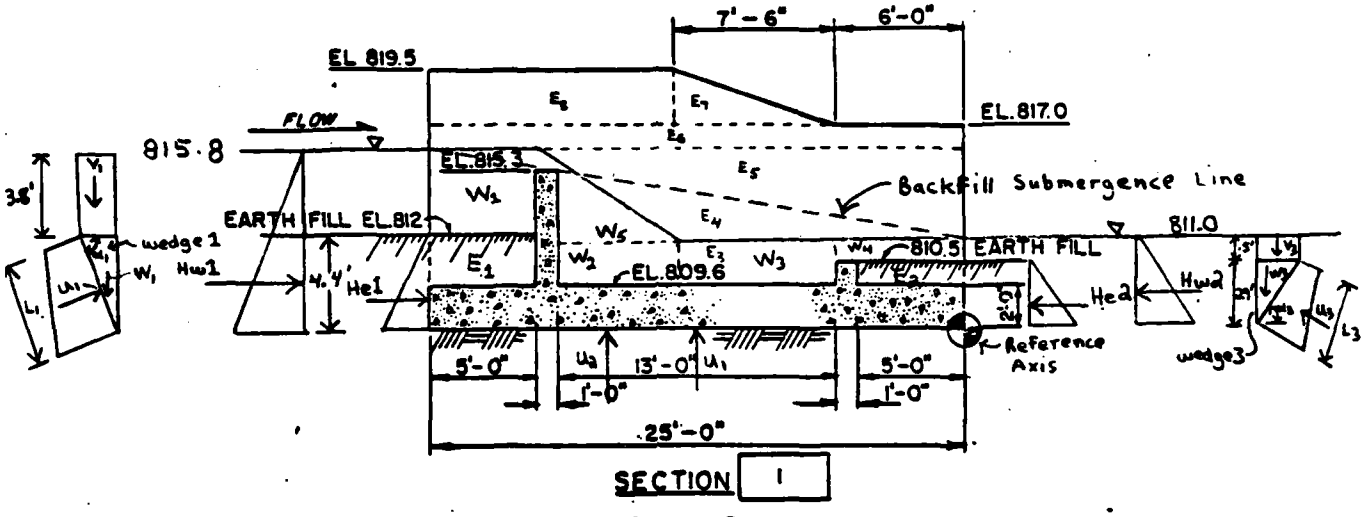
$q_1 = \frac{P}{A}\left(1 + \frac{6e}{b}\right)$

$q_2 = \frac{P}{A}\left(1 - \frac{6e}{b}\right)$

$q_1 = \frac{301.1^K}{925\ ft^2}\left(1 + \frac{6(1.38')}{25'}\right) = 0.43$ Ksf

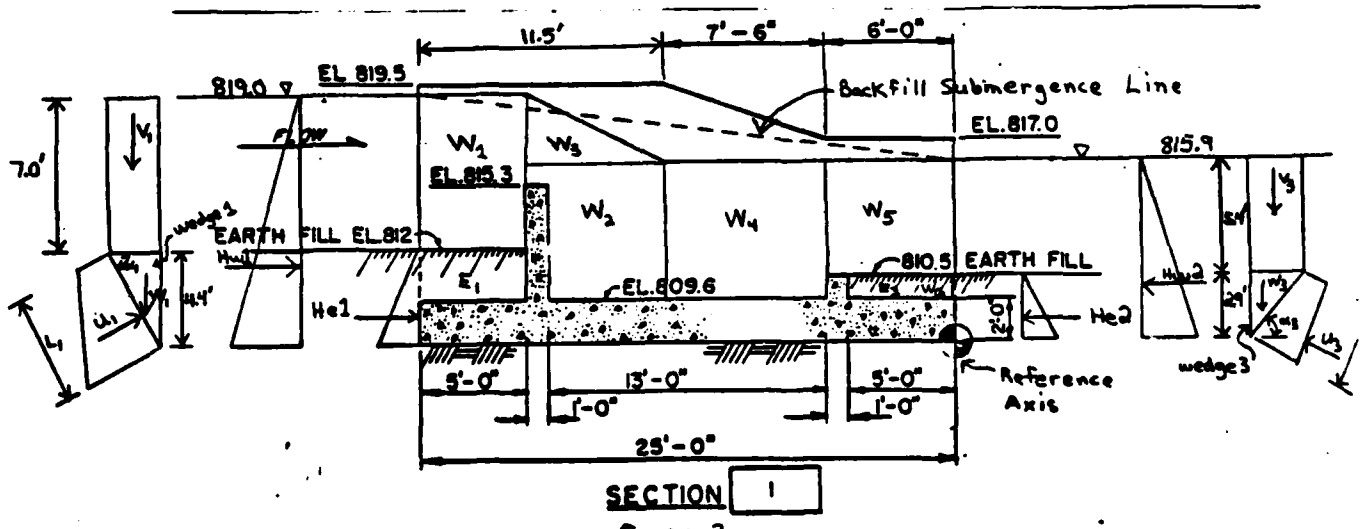
$q_2 = \frac{301.1^K}{925\ ft^2}\left(1 - \frac{6(1.38')}{25'}\right) = 0.22$ Ksf

ST. PAUL DISTRICT COMPUTATION SHEET	DATE Mar '82	PAGE 15 OF 18	FILE NUMBER
NAME OF OFFICE		COMPUTATION	
SUBJECT Basset Creek		SOURCE DATA	
COMPUTED BY DMT	CHECKED BY GLC	APPROVED BY	



SECTION 1

Case 2



SECTION 1

Case 3

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 29 April 82	PAGE 16 OF 18	FILE NUMBER
NAME OF OFFICE	COMPUTATION Drop Structure Sliding Stability		
SUBJECT Bassett Creek Drop Structure	SOURCE DATA		
COMPUTED BY GLC, DMT	CHECKED BY GLC, DMT	APPROVED BY	

Case 2 Upper Pool 815.8 Tailwater 811.0

Try FS = 3.75

$\phi_d = \text{Tan}^{-1}\left(\frac{\tan 30°}{FS}\right) =$ Wedge 1 $\alpha_1 = -(45 + \frac{\phi_d}{2}) = -49.38°$

$L_1 = 4.4/\sin\alpha_1 = 5.80'$

$V_1 = 3.8(62.5)(L_1)(\cos\alpha_1)(30) = 26893$
$W_1 = \frac{1}{2}(125)(4.4)(L_1)(\cos\alpha_1)(30) = 31,239$ } $V_1 + W_1 = 58032$
$U_1 = \left(\frac{3.8 + 8.2}{2}\right)(62.5)(L_1)(30) = 65217$

$P_0 - P_1 = -62856$

Wedge 2

$W_2 = 69\,5855$
$U_2 = 335312$
$H_L = \frac{1}{2}(62.5)(3.8)^2 30 = 13538$
$H_R = \frac{1}{2}(62.5)(.5)^2 30 = 234$
$H_L - H_R = 13304$
$\alpha_2 = 0$
$P_1 - P_2 = 42,205$

Wedge 3

$\phi_d = \text{Tan}^{-1}\left(\frac{\tan 30°}{FS}\right) = 8.75°$ $\alpha_3 = 45 - \frac{\phi_d}{2} = 40.62°$

$L_3 = 2.9/\sin\alpha_3 = 4.45$ $W_3 = \frac{1}{2}(125)(2.9)(L_3)(\cos\alpha_3)(30)$
$V_3 = .5(62.5)(L_3)(\cos\alpha_3)(30) = 3169$ $W_3 = 1838^2$
$W_3 + V_3 = 21552$
$U_3 = \left(\frac{.5 + 3.4}{2}\right)(62.5)(L_3)(30) = 16285$

$P_2 - P_3 = 21318$

$\sum P = -62856 + 42205 + 16285 = 667$

Try FS = 3.8

Wedge 1: $W_1 + V_1 = 58148$
$\alpha_1 = -49.32$ $L_1 = 5.80'$ $W_1 = 31201$
$V_1 = 26947$ $U_1 = 65273$ $P_0 - P_1 = -62909$

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 29 April 1982	PAGE 17 OF 18	FILE NUMBER
NAME OF OFFICE		COMPUTATION Sliding Stability	
SUBJECT Bassett Creek Drop Structure		SOURCE DATA	
COMPUTED BY G.L.C., D.M.T.	CHECKED BY D.M.T., G.L.C.	APPROVED BY	

Wedge 2

Forces same as FS = 3.75

$P_1 - P_2 = 41475$

Wedge 3

$\alpha_3 = 40.68°$ $L_3 = 4.45'$

$V_3 = 3163$ $W_3 = 18346$

$V_3 + W_3 = 21509$ $U_3 = 16266$

$P_2 - P_3 = 21275$

$\Sigma P = -62909 + 41475 + 21275 = -159$ $\therefore FS \approx 3.8$

Case 3 Pool 819.0 Tailwater 815.9

Try FS = 3.00

Wedge 1

$\alpha_1 = -50.45$ $L_1 = 4.4/\sin\alpha_1 = 5.71'$

$V_1 = 7(62.5)(L_1)(\cos\alpha_1)(30) = 47696$

$W_1 = \frac{1}{2}(125)(4.4)(L_1)(\cos\alpha_1)(30) = 29980$

$V_1 + W_1 = 77676$

$U_1 = \left(\frac{7+4.4}{2}\right)(62.5)(L_1)(30) = 98439$

$P_0 - P_1 = -88280$

Wedge 2

$W_2 = 870576$ $H_L = \frac{1}{2}(62.5)(7)^2(30) = 45938$

$U_2 = 569453$ $H_R = \frac{1}{2}(62.5)(7)^2(30) = 27538$

$H_L - H_R = 18600$ $\alpha_2 = 0$

$P_1 - P_2 = 39351$

Wedge 3

$\phi_d = 10.89°$ $\alpha_3 = 45 - \frac{\phi_d}{2} = 39.55°$

$L_3 = 2.9/\sin\alpha_3 = 4.55'$

$V_3 = 5.4(62.5)(L_3)(\cos\alpha_3)(30) = 35552$

$W_3 = \frac{1}{2}(125)(2.9)(L_3)(\cos\alpha_3)(30) = 19093$

$V_3 + W_3 = 54645$

$U_3 = \left(\frac{5.4 + 8.3}{2}\right)62.5(L_3)(30) = 58491$

$P_2 - P_3 = 48806$

NCS 14
1 JUNE 79 (REPLACES NCS 14, 1 APR 63)

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 29 April 82	PAGE 18 OF 18	FILE NUMBER
NAME OF OFFICE		COMPUTATION Sliding Stability	
SUBJECT Bassett Creek Drop Structure		SOURCE DATA	
COMPUTED BY GLC, DMT	CHECKED BY DMT, GLC	APPROVED BY	

$\Sigma P = -88280 + 39351 + 48806 = -123$

$\therefore$ F.S. = 3.00

NCS 14
1 JUNE 79 (REPLACES NCD 14, 1 APR 63)

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 11/20/80	PAGE 1 OF 10	FILE NUMBER
NAME OF OFFICE	COMPUTATION Sheet Pile Wall		
SUBJECT Bassett Creek	SOURCE DATA		
COMPUTED BY G.L.C.	CHECKED BY	APPROVED BY	

F.S. = 1.0 Retaining Wall at Bridge - Left Bank

Loading Case	Penetration (ft.)	Bottom Elevation	Maximum Moment	Deflection Scaled	Deflection Actual (in)
Normal Water	9.31	799.69	10937	1.76 E 9	.35
Normal Water with RR	12.32	796.68	23781	5.31 E 9	.99
Design Water Surface	8.09	800.91	7820	1.09 E 9	.20
DWS with RR	11.31	797.69	19161	3.85 E 9	.72

$S_{req'd} = \frac{23781(12)}{.6(33000)} = 14.41\ in^3$

Use PZ-27 $S_x = 30.2\ in^3$ $I = 184.2\ in^4/ft$ of wall

Actual Deflection = $\frac{\text{Scaled Deflection}}{29 \times 10^6 (184.2)}$

Increase penetration by 20%

∴ 1.2(12.32) = 14.78' Elevation = 794.22 ~ 794.0

```
PROGRAM SHTVAL - DESIGN/ANALYSIS OF ANCHORED
OR CANTILEVER SHEET PILE WALLS BY CLASSICAL METHODS
DATE: 11/20/81                                  TIME: 13:28:40

1. INPUT DATA

1.A.--HEADING

BASSETT CREEK WINGWALL AT BRIDGE LEFT SIDE
NORMAL WATER CASE WITH RAILROAD LOAD


1.B.--WALL TYPE, MODE, METHOD
     CANTILEVER WALL DESIGN


1.C.--WALL DESCRIPTION
     TOP OF WALL ELEVATION =  818.00 (FT)
     FACTOR OF SAFETY      =    1.00


1.D.--RIGHT SIDE SOIL DESCRIPTION
     NUMBER OF RIGHT SIDE SURFACE POINTS =  1
     NUMBER OF RIGHT SIDE SOIL LAYERS    =  1

     RIGHT SIDE SURFACE POINT COORDINATES
     POINT       ELEVATION       X-COORD
      NO.          (FT)           (FT)
       1          818.00           0.00

RIGHT SIDE SOIL LAYER DATA
                 INTERNAL                        WALL      BOTTOM
LAYER    UNIT    FRICTION                     FRICTION     ELEV      BOTTOM
 NO.    WEIGHT    ANGLE      COHESION          ANGLE     AT WALL     SLOPE
        (PCF)     (DEG)       (PSF)            (DEG)       (FT)     (FT/FT)
  1    120.00     28.00       0.00             18.60

1.E.--LEFT SIDE SOIL DESCRIPTION
     NUMBER OF LEFT SIDE SURFACE POINTS    =  1
     NUMBER OF LEFT SIDE SOIL LAYERS       =  1

     LEFT SIDE SURFACE POINT COORDINATES
     POINT       ELEVATION       X-COORD
      NO.          (FT)           (FT)
       1          809.00           0.00

LEFT  SIDE SOIL LAYER DATA
                 INTERNAL                        WALL      BOTTOM
LAYER    UNIT    FRICTION                     FRICTION     ELEV      BOTTOM
 NO.    WEIGHT    ANGLE      COHESION          ANGLE     AT WALL     SLOPE
        (PCF)     (DEG)       (PSF)            (DEG)       (FT)     (FT/FT)
  1    120.00     28.00       0.00             18.60

1.F.--WATER DATA

     RIGHT SIDE ELEVATION        =  809.50 (FT)
     LEFT SIDE ELEVATION         =  809.50 (FT)
     WATER UNIT WEIGHT           =   62.40 (PCF)
     SEEPAGE GRADIENT            =    0.00 (FT/FT)

1.G.--SURCHARGE LOADS
     NUMBER OF LINE LOADS          =  0
     DISTRIBUTED LOAD DISTRIBUTION =  STRIP

     STRIP SURCHARGE LOAD
     DISTANCE FROM WALL =   10.00 FT
     WIDTH              =    5.00 FT
     LOAD               =  2024.00 PSF

1.H.--HORIZONTAL LOADS
     NUMBER OF HORIZONTAL LINE LOADS          =  0
     NUMBER OF HORIZONTAL PRESSURE POINTS     =  0
     EARTHQUAKE ACCELERATION                  =  0.00 (G'S)


DO YOU WANT A PLOT OF INPUT GEOMETRY?
ENTER 'YES' OR 'NO'
?>
```

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PROGRAM SHTWAL - DESIGN/ANALYSIS OF ANCHORED
OR CANTILEVER SHEET PILE WALLS BY CLASSICAL METHODS
DATE: 11/20/81                                  TIME: 13:29:29

8. RESULTS

8.A.--HEADING

BASSETT CREEK WINGWALL AT BRIDGE LEFT SIDE
NORMAL WATER CASE WITH RAILROAD LOAD


8.B.--SUMMARY OF RESULTS FOR CANTILEVER WALL DESIGN

     SOIL PRESSURES DETERMINED BY COULOMB
     COEFFICIENTS AND THEORY OF ELASTICITY
     EQUATIONS FOR SURCHARGE LOADS


     WALL BOTTOM:
          PENETRATION =     12.32 (FT)
          ELEVATION   =    796.68 (FT)

     BENDING MOMENT:
          MAXIMUM     =   -23781. (LB-FT)
          ELEVATION   =     802.6 (FT)

     SCALED DEFLECTION:
          MAXIMUM     =  5.31E+00 (LB-IN3)
          ELEVATION   =     818.0 (FT)


          (NOTE: DIVIDE SCALED DEFLECTION BY MODULUS
          OF ELASTICITY IN PSI TIMES PILE MOMENT OF
          INERTIA IN IN##4 TO OBTAIN DEFLECTION IN INCHES)


     DO YOU WANT COMPLETE RESULTS OUTPUT?
     ENTER 'YES' OR 'NO'
?>
```

D-26

1

3/10

PROGRAM SNTUAL - DESIGN/ANALYSIS OF ANCHORED
OR CANTILEVER SHEET PILE WALLS BY CLASSICAL METHODS
DATE: 11/20/81 TIME: 13:34:13

1. INPUT DATA

1.A.--HEADING

BASSETT CREEK WINGWALL AT BRIDGE LEFT SIDE
DESIGN WATER CASE WITH RAILROAD LOAD

1.B.--WALL TYPE, MODE, METHOD
CANTILEVER WALL DESIGN

1.C.--WALL DESCRIPTION
TOP OF WALL ELEVATION = 818.00 (FT)
FACTOR OF SAFETY = 1.00

1.D.--RIGHT SIDE SOIL DESCRIPTION
NUMBER OF RIGHT SIDE SURFACE POINTS = 1
NUMBER OF RIGHT SIDE SOIL LAYERS = 1

RIGHT SIDE SURFACE POINT COORDINATES

POINT NO.	ELEVATION (FT)	X-COORD (FT)
1	818.00	0.00

RIGHT SIDE SOIL LAYER DATA

LAYER NO.	UNIT WEIGHT (PCF)	INTERNAL FRICTION ANGLE (DEG)	COHESION (PSF)	WALL FRICTION ANGLE (DEG)	BOTTOM ELEV AT WALL (FT)	BOTTOM SLOPE (FT/FT)
1	120.00	28.00	0.00	18.00		

1.E.--LEFT SIDE SOIL DESCRIPTION
NUMBER OF LEFT SIDE SURFACE POINTS = 1
NUMBER OF LEFT SIDE SOIL LAYERS = 1

LEFT SIDE SURFACE POINT COORDINATES

POINT NO.	ELEVATION (FT)	X-COORD (FT)
1	809.00	0.00

LEFT SIDE SOIL LAYER DATA

LAYER NO.	UNIT WEIGHT (PCF)	INTERNAL FRICTION ANGLE (DEG)	COHESION (PSF)	WALL FRICTION ANGLE (DEG)	BOTTOM ELEV AT WALL (FT)	BOTTOM SLOPE (FT/FT)
1	120.00	28.00	0.00	18.00		

1.F.--WATER DATA
RIGHT SIDE ELEVATION = 814.00 (FT)
LEFT SIDE ELEVATION = 814.00 (FT)
WATER UNIT WEIGHT = 62.40 (PCF)
SEEPAGE GRADIENT = 0.00 (FT/FT)

1.G.--SURCHARGE LOADS
NUMBER OF LINE LOADS = 0
DISTRIBUTED LOAD DISTRIBUTION = STRIP

STRIP SURCHARGE LOAD
DISTANCE FROM WALL = 10.00 FT
WIDTH = 5.00 FT
LOAD = 2084.00 PSF

1.H.--HORIZONTAL LOADS
NUMBER OF HORIZONTAL LINE LOADS = 0
NUMBER OF HORIZONTAL PRESSURE POINTS = 0
EARTHQUAKE ACCELERATION = 0.00 (G'S)

DO YOU WANT A PLOT OF INPUT GEOMETRY?
ENTER 'YES' OR 'NO'

D-37

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PROGRAM SNTUAL - DESIGN/ANALYSIS OF ANCHORED
OR CANTILEVER SHEET PILE WALLS BY CLASSICAL METHODS
DATE: 11/20/81                                  TIME: 13:35:04

B. RESULTS

B.A.--HEADING

BASSETT CREEK WINGWALL AT BRIDGE LEFT SIDE
DESIGN WATER CASE WITH RAILROAD LOAD


B.B.--SUMMARY OF RESULTS FOR CANTILEVER WALL DESIGN

     SOIL PRESSURES DETERMINED BY COULOMB
     COEFFICIENTS AND THEORY OF ELASTICITY
     EQUATIONS FOR SURCHARGE LOADS


     WALL BOTTOM:
          PENETRATION =     11.31 (FT)
          ELEVATION   =    797.69 (FT)

     BENDING MOMENT:
          MAXIMUM     =  -19181. (LB-FT)
          ELEVATION   =    803.3 (FT)

     SCALED DEFLECTION:
          MAXIMUM     =  3.85E+09 (LB-IN3)
          ELEVATION   =    818.0 (FT)

          (NOTE: DIVIDE SCALED DEFLECTION BY MODULUS
          OF ELASTICITY IN PSI TIMES PILE MOMENT OF
          INERTIA IN IN##4 TO OBTAIN DEFLECTION IN INCHES)


DO YOU WANT COMPLETE RESULTS OUTPUT?
ENTER 'YES' OR 'NO'
I>
```

D-28

PROGRAM SNTWAL - DESIGN/ANALYSIS OF ANCHORED
OR CANTILEVER SHEET PILE WALLS BY CLASSICAL METHODS
DATE: 11/20/81 TIME: 13:30:24

1. INPUT DATA

1.A.--HEADING

BASSETT CREEK WINGWALL AT BRIDGE LEFT SIDE
NORMAL WATER CASE WITHOUT RAILROAD LOAD

1.B.--WALL TYPE, MODE, METHOD
CANTILEVER WALL DESIGN

1.C.--WALL DESCRIPTION
TOP OF WALL ELEVATION = 818.00 (FT)
FACTOR OF SAFETY = 1.00

1.D.--RIGHT SIDE SOIL DESCRIPTION
NUMBER OF RIGHT SIDE SURFACE POINTS = 1
NUMBER OF RIGHT SIDE SOIL LAYERS = 1

RIGHT SIDE SURFACE POINT COORDINATES

POINT NO.	ELEVATION (FT)	X-COORD (FT)
1	818.00	0.00

RIGHT SIDE SOIL LAYER DATA

LAYER NO.	UNIT WEIGHT (PCF)	INTERNAL FRICTION ANGLE (DEG)	COHESION (PSF)	WALL FRICTION ANGLE (DEG)	BOTTOM ELEV AT WALL (FT)	BOTTOM SLOPE (FT/FT)
1	130.00	28.00	0.00	18.60		

1.E.--LEFT SIDE SOIL DESCRIPTION
NUMBER OF LEFT SIDE SURFACE POINTS = 1
NUMBER OF LEFT SIDE SOIL LAYERS = 1

LEFT SIDE SURFACE POINT COORDINATES

POINT NO.	ELEVATION (FT)	X-COORD (FT)
1	809.00	0.00

LEFT SIDE SOIL LAYER DATA

LAYER NO.	UNIT WEIGHT (PCF)	INTERNAL FRICTION ANGLE (DEG)	COHESION (PSF)	WALL FRICTION ANGLE (DEG)	BOTTOM ELEV AT WALL (FT)	BOTTOM SLOPE (FT/FT)
1	130.00	28.00	0.00	18.60		

1.F.--WATER DATA
RIGHT SIDE ELEVATION = 809.50 (FT)
LEFT SIDE ELEVATION = 809.50 (FT)
WATER UNIT WEIGHT = 62.40 (PCF)
SEEPAGE GRADIENT = 0.00 (FT/FT)

1.G.--SURCHARGE LOADS
NUMBER OF LINE LOADS = 0
DISTRIBUTED LOAD DISTRIBUTION = NONE

1.H.--HORIZONTAL LOADS
NUMBER OF HORIZONTAL LINE LOADS = 0
NUMBER OF HORIZONTAL PRESSURE POINTS = 0
EARTHQUAKE ACCELERATION = 0.00 (G'S)

DO YOU WANT A PLOT OF INPUT GEOMETRY?
ENTER 'YES' OR 'NO'
1>

D-29

```
PROGRAM SHTVAL - DESIGN/ANALYSIS OF ANCHORED
OR CANTILEVER SHEET PILE WALLS BY CLASSICAL METHODS
DATE: 11/20/91                                  TIME: 13:31:08

8. RESULTS

8.A.--HEADING

BASSETT CREEK WINGWALL AT BRIDGE LEFT SIDE
NORMAL WATER CASE WITHOUT RAILROAD LOAD


8.B.--SUMMARY OF RESULTS FOR CANTILEVER WALL DESIGN

     SOIL PRESSURES DETERMINED BY COULOMB
     COEFFICIENTS AND THEORY OF ELASTICITY
     EQUATIONS FOR SURCHARGE LOADS


     WALL BOTTOM:
          PENETRATION =      9.31 (FT)
          ELEVATION   =    788.69 (FT)

     BENDING MOMENT:
          MAXIMUM     =  -10937. (LB-FT)
          ELEVATION   =     804.2 (FT)

     SCALED DEFLECTION:
          MAXIMUM     =  1.76E+09 (LB-IN3)
          ELEVATION   =     818.0 (FT)

          (NOTE: DIVIDE SCALED DEFLECTION BY MODULUS
          OF ELASTICITY IN PSI TIMES PILE MOMENT OF
          INERTIA IN IN**4 TO OBTAIN DEFLECTION IN INCHES)


   DO YOU WANT COMPLETE RESULTS OUTPUT?
   ENTER 'YES' OR 'NO'
1>
```

D-30

PROGRAM SHTVAL - DESIGN/ANALYSIS OF ANCHORED
OR CANTILEVER SHEET PILE WALLS BY CLASSICAL METHODS
DATE: 11/20/81 TIME: 13:35:58

I. INPUT DATA

I.A.--HEADING

BASSETT CREEK WINGWALL AT BRIDGE LEFT SIDE
DESIGN WATER CASE WITHOUT RAILROAD LOAD

I.B.--WALL TYPE, MODE, METHOD
CANTILEVER WALL DESIGN

I.C.--WALL DESCRIPTION
TOP OF WALL ELEVATION = 818.00 (FT)
FACTOR OF SAFETY = 1.00

I.D.--RIGHT SIDE SOIL DESCRIPTION
NUMBER OF RIGHT SIDE SURFACE POINTS = 1
NUMBER OF RIGHT SIDE SOIL LAYERS = 1

RIGHT SIDE SURFACE POINT COORDINATES

POINT NO.	ELEVATION (FT)	X-COORD (FT)
1	818.00	0.00

RIGHT SIDE SOIL LAYER DATA

LAYER NO.	UNIT WEIGHT (PCF)	INTERNAL FRICTION ANGLE (DEG)	COHESION (PSF)	WALL FRICTION ANGLE (DEG)	BOTTOM ELEV AT WALL (FT)	BOTTOM SLOPE (FT/FT)
1	120.00	28.00	0.00	18.00		

I.E.--LEFT SIDE SOIL DESCRIPTION
NUMBER OF LEFT SIDE SURFACE POINTS = 1
NUMBER OF LEFT SIDE SOIL LAYERS = 1

LEFT SIDE SURFACE POINT COORDINATES

POINT NO.	ELEVATION (FT)	X-COORD (FT)
1	800.00	0.00

LEFT SIDE SOIL LAYER DATA

LAYER NO.	UNIT WEIGHT (PCF)	INTERNAL FRICTION ANGLE (DEG)	COHESION (PSF)	WALL FRICTION ANGLE (DEG)	BOTTOM ELEV AT WALL (FT)	BOTTOM SLOPE (FT/FT)
1	120.00	28.00	0.00	18.00		

I.F.--WATER DATA

RIGHT SIDE ELEVATION = 814.00 (FT)
LEFT SIDE ELEVATION = 814.00 (FT)
WATER UNIT WEIGHT = 62.40 (PCF)
SEEPAGE GRADIENT = 0.00 (FT/FT)

I.G.--SURCHARGE LOADS
NUMBER OF LINE LOADS = 0
DISTRIBUTED LOAD DISTRIBUTION = NONE

I.H.--HORIZONTAL LOADS
NUMBER OF HORIZONTAL LINE LOADS = 0
NUMBER OF HORIZONTAL PRESSURE POINTS = 0
EARTHQUAKE ACCELERATION = 0.00 (G'S)

DO YOU WANT A PLOT OF INPUT GEOMETRY?
ENTER 'YES' OR 'NO'
I>

D-31

```
PROGRAM SHTWAL - DESIGN/ANALYSIS OF ANCHORED
OR CANTILEVER SHEET PILE WALLS BY CLASSICAL METHODS
DATE: 11/20/81                          TIME: 13:36:44

B. RESULTS

B.A.--HEADING

BASSETT CREEK WINGWALL AT BRIDGE LEFT SIDE
DESIGN WATER CASE WITHOUT RAILROAD LOAD


B.B.--SUMMARY OF RESULTS FOR CANTILEVER WALL DESIGN

     SOIL PRESSURES DETERMINED BY COULOMB
     COEFFICIENTS AND THEORY OF ELASTICITY
     EQUATIONS FOR SURCHARGE LOADS

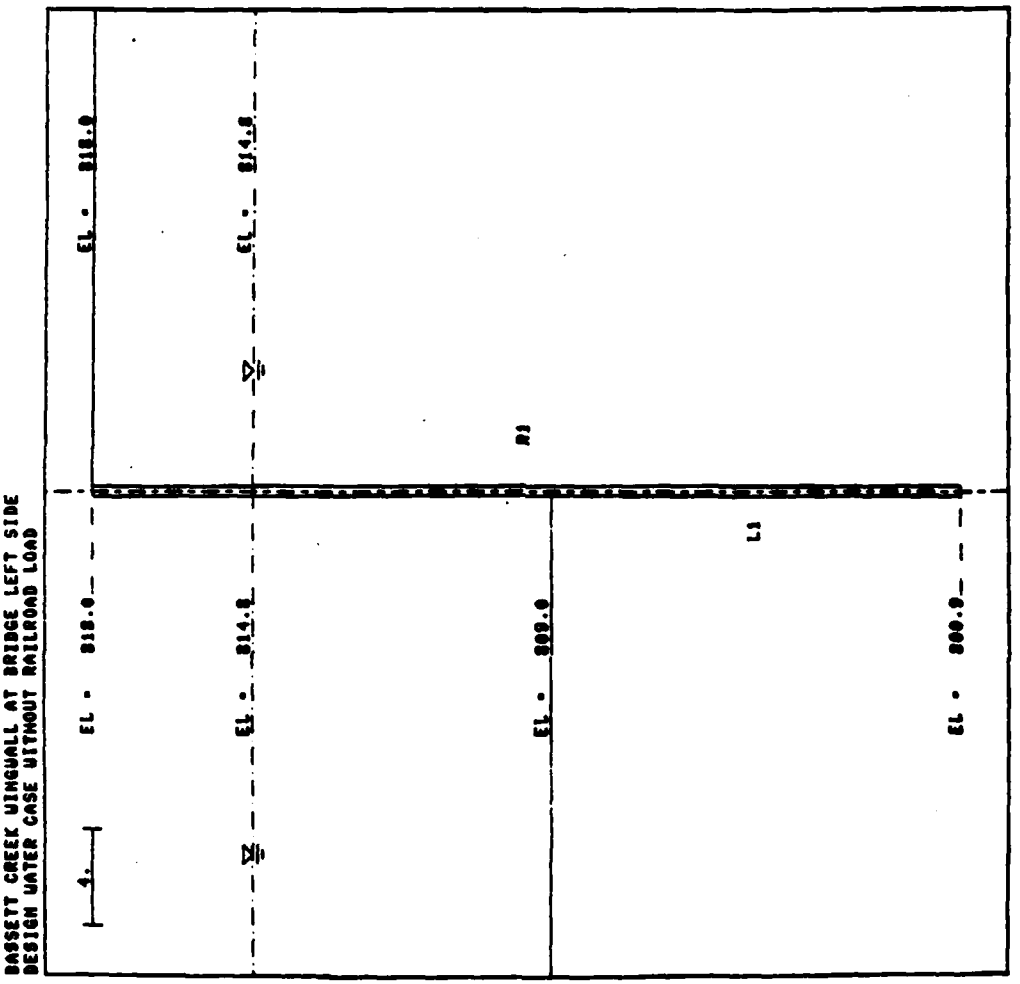
     WALL BOTTOM:
          PENETRATION =      8.00 (FT)
          ELEVATION   =    808.81 (FT)

     BENDING MOMENT:
          MAXIMUM     =    -7820. (LB-FT)
          ELEVATION   =     805.1 (FT)

     SCALED DEFLECTION:
          MAXIMUM     =  1.05E+00 (LB-IN3)
          ELEVATION   =     818.0 (FT)

          (NOTE: DIVIDE SCALED DEFLECTION BY MODULUS
          OF ELASTICITY IN PSI TIMES PILE MOMENT OF
          INERTIA IN IN##4 TO OBTAIN DEFLECTION IN INCHES)

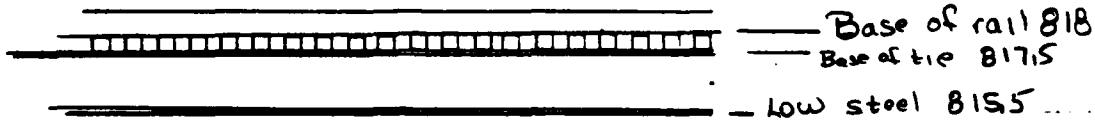

DO YOU WANT COMPLETE RESULTS OUTPUT?
ENTER 'YES' OR 'NO'
?>
```


BASSETT CREEK WINGWALL AT BRIDGE LEFT SIDE
DESIGN WATER CASE WITHOUT RAILROAD LOAD
EL = 818.0
EL = 814.0
EL = 809.0
EL = 800.0
R1
L1
LEFTSIDE SOIL PROPERTIES
LAYER C PHI GAMMA DELTA
L1 0. 28.0 120.0 18.6
RIGHTSIDE SOIL PROPERTIES
LAYER C PHI GAMMA DELTA
R1 0. 28.0 120.0 18.6

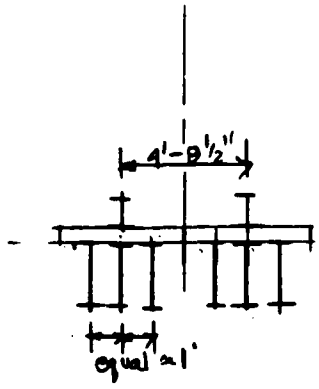
ST. PAUL DISTRICT COMPUTATION SHEET	DATE 11 August 1981	PAGE 1 OF 33	FILE NUMBER
NAME OF OFFICE		COMPUTATION Fruen Mill R.R. Bridge	
SUBJECT Bassett Creek		SOURCE DATA AREA Section 15-1	
COMPUTED BY GLC	CHECKED BY	APPROVED BY	

Clear span = 67'
Due to skew, use 76' for span length
with 2 spans, and 4' pier, then each span is $\frac{76-4}{2} = 36'$

$M_{max\ LL} = 1097.3^{1K}$ $V_{max} = 141.12^{K}$
from AREA pg 15-1-35



∴ 2' max depth of beam



$M = \frac{1097.3\ (12)}{3}$

$M = 438920^{"K}$
(1.3.4.1(c))

for $F_b = 20$ ksi $S_x = \frac{4389.20^{"K}}{20\ ksi} = 219.5\ in^3$

Member	S_x	Depth
W21X111	273	21.51"
~~W24X94~~	~~222~~	~~24.31~~
W24X104	258	24.06
W24X117	291	24.26

D-34

Add DL moment

$M = \frac{.115(36)^2(12)}{8} = 223.56^{1K}$

$M_{Total} = 4612.76$ $S_x = 231\ in^3$

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 11 August 1981	PAGE 2 OF 35	FILE NUMBER
NAME OF OFFICE		COMPUTATION Fruen Mill R.R. Bridge	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY GLC	CHECKED BY	APPROVED BY	

Determine Allowable for W21X111 (use lower depth to provide more flexibility and clearance)

$r_T = 3.28 \quad d/A_f = 1.99 \quad I_x = 2670 \quad I_y = 274$

$r_x = 9.05 \quad r_y = 2.90$

$F_b = 20000 - 0.4(\ell/r_T)^2 = 20000 - .4\left(\frac{36 \times 12}{3.28}\right)^2$

$F_b = 13061$ psi ← Governs

$F_b = \frac{10.5 \times 10^6}{\ell\left(\frac{d}{A_f}\right)} = \frac{10.5 \times 10^6}{36(12)(1.99)} = 12214$ psi

$f_b = \frac{4612.76}{1.273} = 16.90$ ksi Overstressed

$S_{x\,req'd} = \frac{4612.76}{13} = 355\ in^3$

There is no 21" deep member with sufficent section modulus

∴ try 24" deep member

Try W24 X 146 $d = 24.74 \quad r_T = 3.43 \quad d/A_f = 1.76 \quad S_x = 371$

$F_b = 20000 - .4\left(\frac{36 \times 12}{3.43}\right)^2 = 13655$ psi

$F_b = \frac{10.5 \times 10^6}{36(12)(1.76)} = 13810$ psi ← Governs

$W_{DL} = 146 + \frac{200}{5} \text{ rail} = 212.67 \qquad M_{DL} = \frac{.212(36)^2}{8} 12 = 413.42$"k

$M_{Total} = 4389.20 + 413.42 = 4802.62$"k

$f_b = \frac{4802.62}{371} = 12.95$ ksi OK

Check shear

$V = \frac{142.12}{3} + .212\left(\frac{36}{2}\right) = 51.2^k$

$f_v = \frac{V}{A_w} = \frac{51.2}{(24.74 - 2 * 1.090).65} = 3.5$ ksi < 20ksi OK

D-35

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 11 August 198	PAGE 3 OF 33	FILE NUMBER
NAME OF OFFICE		COMPUTATION Fruen Mill R.R. Bridge	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY GLC	CHECKED BY	APPROVED BY	

Check $\frac{\ell}{r_{min}} = \frac{36(12)}{3.01} = 143.5 \quad = \quad 151$

$\frac{\ell}{r_T} \leq 157 \qquad \frac{36(12)}{3.43} = 126 \leq 157$

$\therefore$ 1.7.1(6) satisfied

Check use X0045

ASSOCIATION OF AMERICAN RAILROADS TECHNICAL CENTER
CHICAGO, ILLINOIS
REVISED DEC.1975

M N & S BRIDGE OVER BASSETT CREEK AT FRUEN MILL

GIRDER ANALYSIS

DECK PLATE GIRDER 36.00 FT SPAN

C-C GIRDERS 4.71 .167 AXLE LOAD TO EACH GIRDER

UNIFORM DEAD LOAD .213 KIPS/FT

DIESEL LOCOMOTIVE

UNBALLASTED DECK
1969 AREA SPECIFICATIONS

LOADING E 80.0

SECTION PROPERTIES

SECTION 1 TYPE 5 0.00 FT FROM LEFT SUPPORT

ROLLED SECTION
AREA 43.000 CENTROID 12.370 MOM INERTIA 4580.00

OPEN HOLES
TOP FLANGE 0.0 0.000 IN. O.H.
BOT FLANGE 0.0 0.000 IN. O.H.
WEB 0.0 O.H.
SIZE 0.000
SPACE 0.000

CENTROID 12.370

GROSS MOMENT OF INERTIA 4580.00 SECTION MODULUS 370.25
NET MOMENT OF INERTIA 4580.00 SECTION MODULUS 370.25

RADIUS OF GYRATION OF COMP AREA ABOUT WEAK AXIS 3.02

SECTION 2 TYPE 5 9.00 FT FROM LEFT SUPPORT

ROLLED SECTION
AREA 43.000 CENTROID 12.370 MOM INERTIA 4580.00

OPEN HOLES
TOP FLANGE 0.0 0.000 IN. O.H.
BOT FLANGE 0.0 0.000 IN. O.H.
WEB 0.0 O.H.
SIZE 0.000
SPACE 0.000

CENTROID 12.370

GROSS MOMENT OF INERTIA 4580.00 SECTION MODULUS 370.25
NET MOMENT OF INERTIA 4580.00 SECTION MODULUS 370.25

RADIUS OF GYRATION OF COMP AREA ABOUT WEAK AXIS 3.02

SECTION 3 TYPE 5 18.00 FT FROM LEFT SUPPORT

ROLLED SECTION
AREA 43.000 CENTROID 12.370 MOM INERTIA 4580.00

OPEN HOLES
TOP FLANGE 0.0 0.000 IN. O.H.
BOT FLANGE 0.0 0.000 IN. O.H.
WEB 0.0 O.H.
SIZE 0.000
SPACE 0.000

CENTROID 12.370

GROSS MOMENT OF INERTIA 4580.00 SECTION MODULUS 370.25
NET MOMENT OF INERTIA 4580.00 SECTION MODULUS 370.25

RADIUS OF GYRATION OF COMP AREA ABOUT WEAK AXIS 3.02

SECTION 4 TYPE 5 27.00 FT FROM LEFT SUPPORT

ROLLED SECTION
AREA 43.000 CENTROID 12.370 MOM INERTIA 4580.00

OPEN HOLES
TOP FLANGE 0.0 0.000 IN. O.H.

BOT FLANGE 0.0 0.000 IN. O.H.
WEB 0.0 O.H.
SIZE 0.000
SPACE 0.000

CENTROID 12.370

GROSS MOMENT OF INERTIA 4580.00 SECTION MODULUS 370.25
NET MOMENT OF INERTIA 4580.00 SECTION MODULUS 370.25

RADIUS OF GYRATION OF COMP AREA ABOUT WEAK AXIS 3.02

SECTION 5 TYPE 5 36.00 FT FROM LEFT SUPPORT

ROLLED SECTION
AREA 43.000 CENTROID 12.370 MOM INERTIA 4580.00

OPEN HOLES
TOP FLANGE 0.0 0.000 IN. O.H.
BOT FLANGE 0.0 0.000 IN. O.H.
WEB 0.0 O.H.
SIZE 0.000
SPACE 0.000

CENTROID 12.370

GROSS MOMENT OF INERTIA 4580.00 SECTION MODULUS 370.25
NET MOMENT OF INERTIA 4580.00 SECTION MODULUS 370.25

RADIUS OF GYRATION OF COMP AREA ABOUT WEAK AXIS 3.02

ASSOCIATION OF AMERICAN RAILROADS TECHNICAL CENTER
CHICAGO, ILLINOIS
REVISED DEC.1975

M N & S BRIDGE OVER BASSETT CREEK AT FRUEN MILL

GIRDER ANALYSIS

DECK PLATE GIRDER 36.00 FT SPAN

C-C GIRDERS 4.71 .167 AXLE LOAD TO EACH GIRDER

UNIFORM DEAD LOAD .213 KIPS/FT

DIESEL LOCOMOTIVE

UNBALLASTED DECK
1969 AREA SPECIFICATIONS

DEAD LOAD VALUES

UNIFORM DEAD LOAD .21 KIPS PER FT (EACH GIRDER)

POINT	DIST FROM LEFT SUPPORT (FT)	DEAD MOMENT (FT-KIPS)	DEAD SHEAR (KIPS)	PANEL (FT)
1	0.00	0.	4.	0.00
2	9.00	26.	2.	0.00
3	18.00	35.	0.	0.00
4	27.00	26.	-2.	0.00
5	36.00	0.	-4.	0.00

LIVE LOAD
(NO IMPACT)

POINT	DIST FROM LEFT SUPPORT (FT)	MOMENT (FT-KIPS)	POSITIVE SHEAR (KIPS)	NEGATIVE SHEAR (KIPS)	GROSS I (IN-4)
	MODULUS (COMP) (IN-3)	COMP STRESS (KSI)	NET I (IN-4)	MODULUS (TEN) (IN-3)	TEN STRESS (KSI)	WEB AREA (IN-2)	WEB SHEAR (KSI)
1	0.00	0.	47.	0.	4580.	
	370.	0.00	4580.	370.	0.00	16.08	2.93
2	9.00	284.	30.	-2.	4580.	
	370.	9.20	4580.	370.	9.20	16.08	1.84

8/33

3	18.00	366.	14.	-14.	4580.		
	370.	11.86	4580.	370.	11.86	16.08	.85
4	27.00	284.	2.	-30.	4580.		
	370.	9.20	4580.	370.	9.20	16.08	1.84
5	36.00	0.	0.	-47.	4580.		
	370.	0.00	4580.	370.	0.00	16.08	2.93

ASSOCIATION OF AMERICAN RAILROADS TECHNICAL CENTER
CHICAGO, ILLINOIS
REVISED DEC.1975

M H & S BRIDGE OVER BASSETT CREEK AT FRUEN MILL

GIRDER ANALYSIS

DECK PLATE GIRDER 36.00 FT SPAN

C-C GIRDERS 4.71 .167 AXLE LOAD TO EACH GIRDER

UNIFORM DEAD LOAD .213 KIPS/FT

DIESEL LOCOMOTIVE

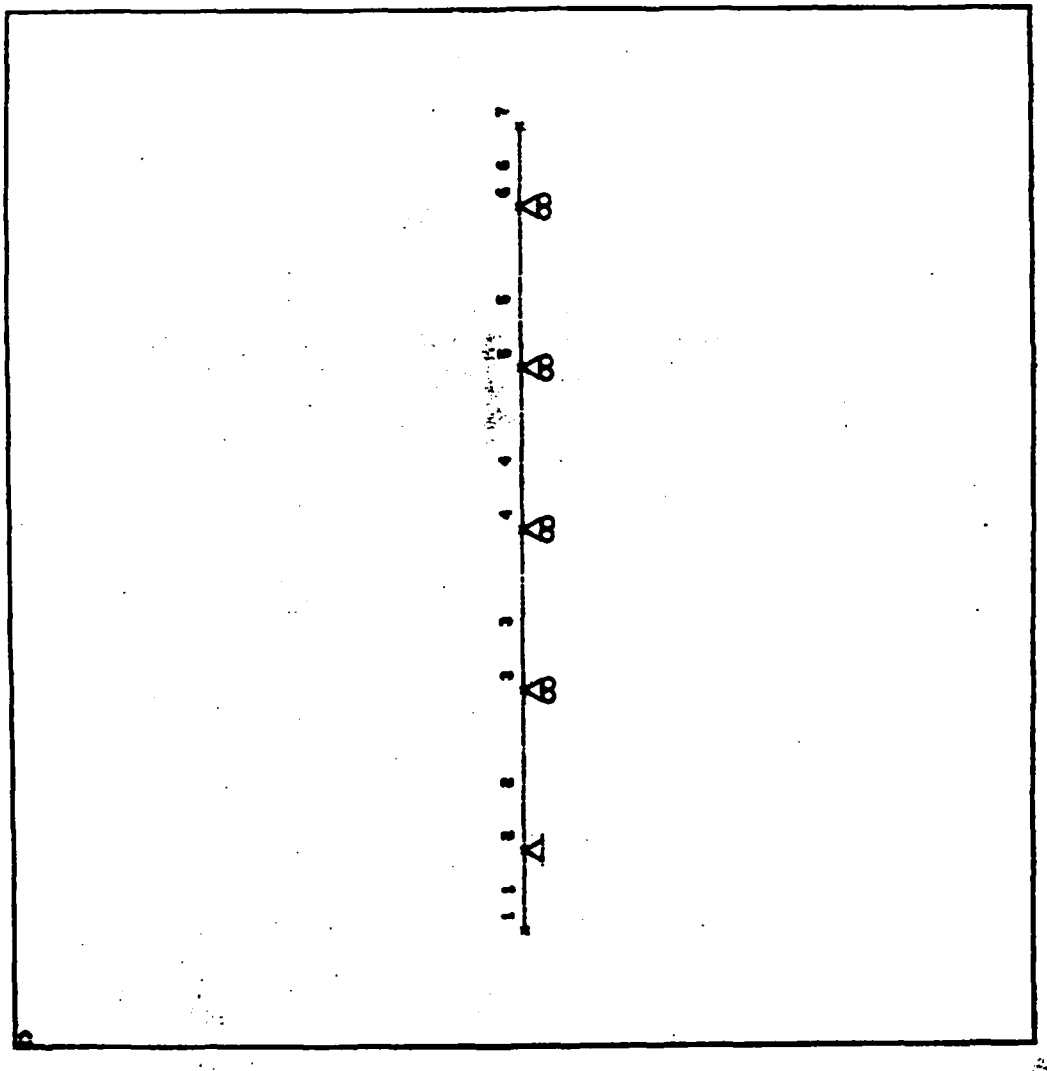
UNBALLASTED DECK
1969 AREA SPECIFICATIONS

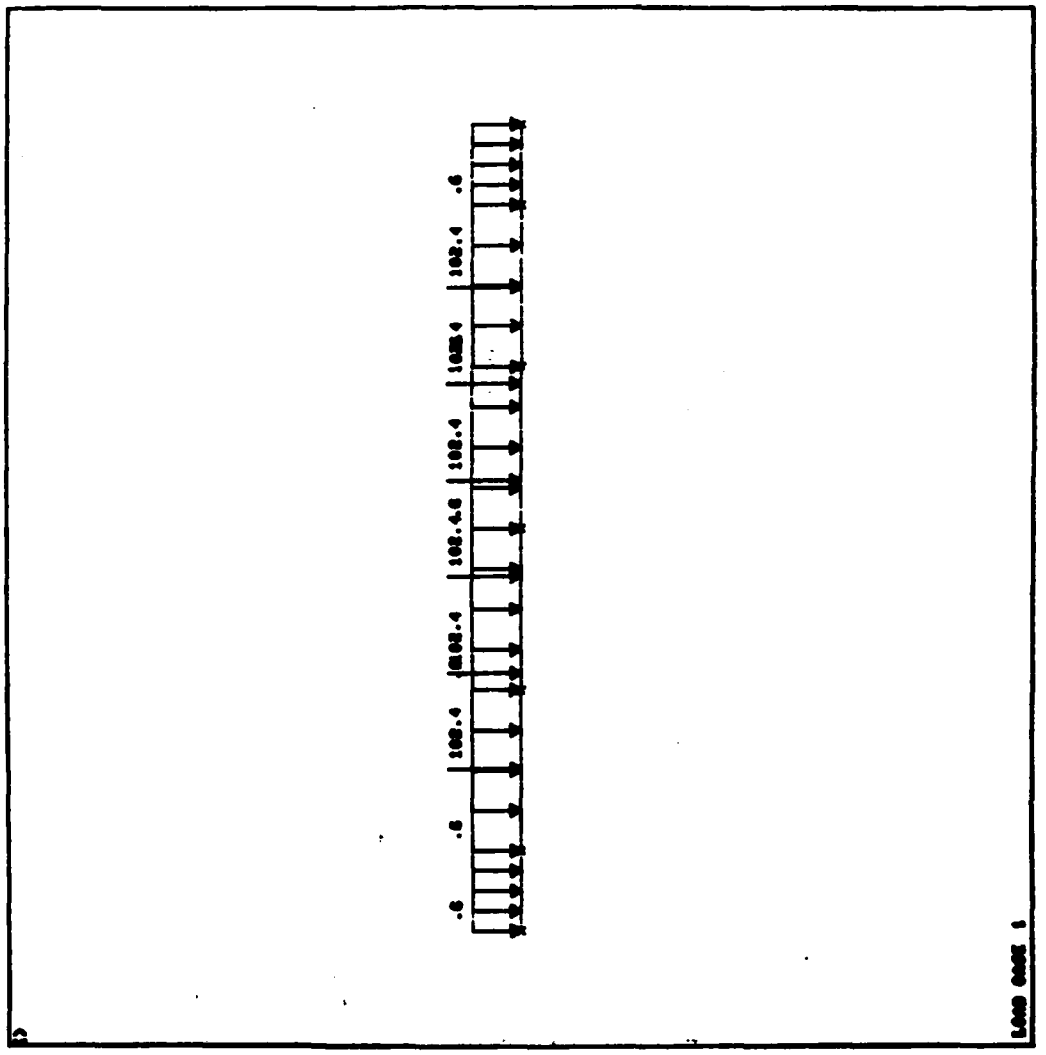
TOTALS

POINT	DIST FROM LEFT SUPPORT (FT)	DEAD MOMENT (FT-KIPS)	LIVE MOMENT (FT-KIPS)	IMPACT (FT-KIPS)	TOTAL MOMENT (FT-KIPS)	
	DEAD SHEAR (KIPS) / LIVE SHEAR (KIPS)	IMPACT (KIPS)	TOTAL SHEAR (KIPS)	COMP STRESS (KSI)	TEN STRESS (KSI)	SHEAR STRESS (KSI)
1	0.00	0.	0.	0.	0.	
	4. 47.	28.	79.	0.00	0.00	4.88
2	9.00	26.	284.	167.	477.	
	2. 30.	17.	49.	15.45	15.45	3.05
3	18.00	35.	366.	215.	615.	
	0. 14.	8.	22.	19.95	19.95	1.35
4	27.00	26.	284.	167.	477.	
	-2. -30.	-17.	49.	15.45	15.45	3.05
5	36.00	0.	0.	0.	0.	
	-4. -47.	-28.	79.	0.00	0.00	4.88

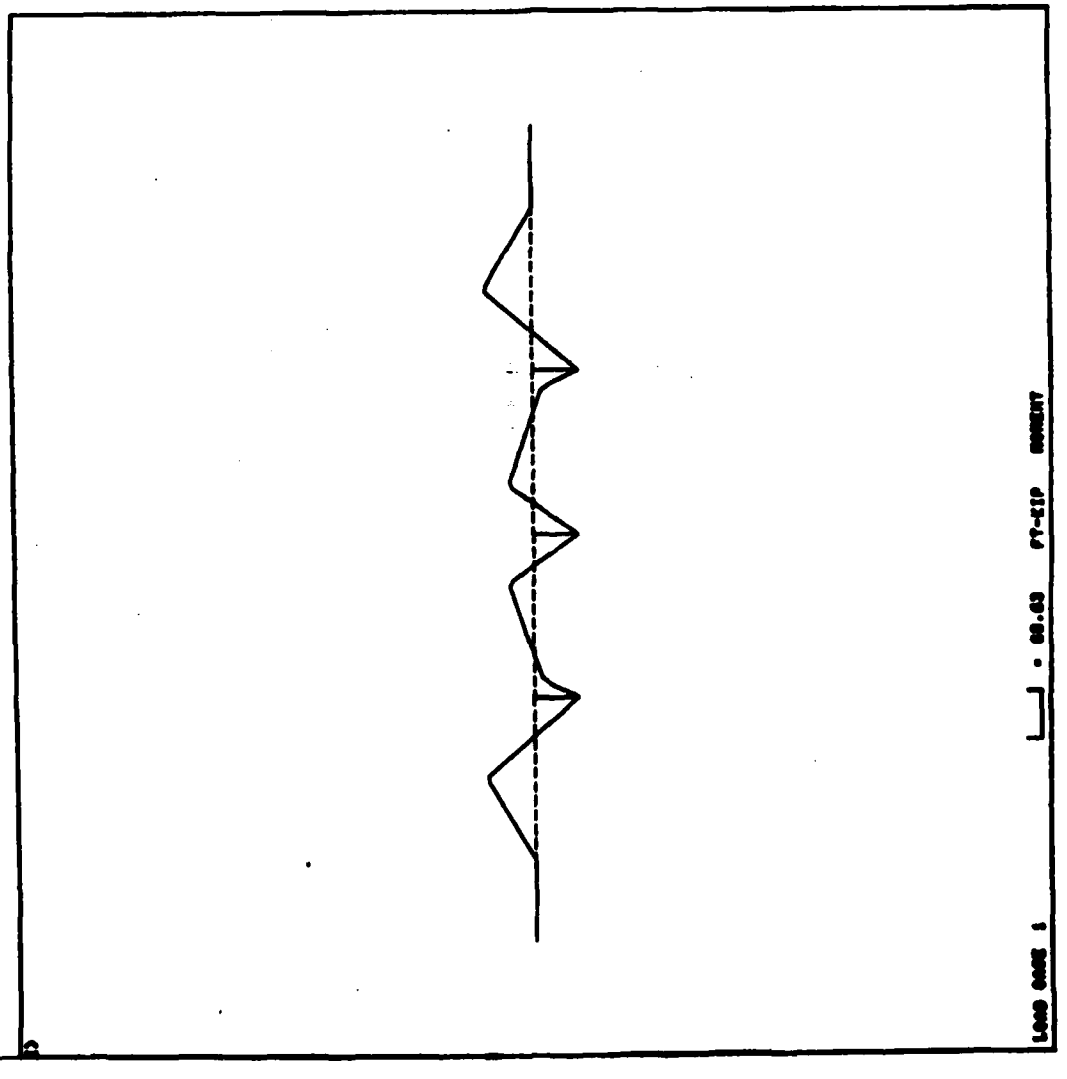
10/53

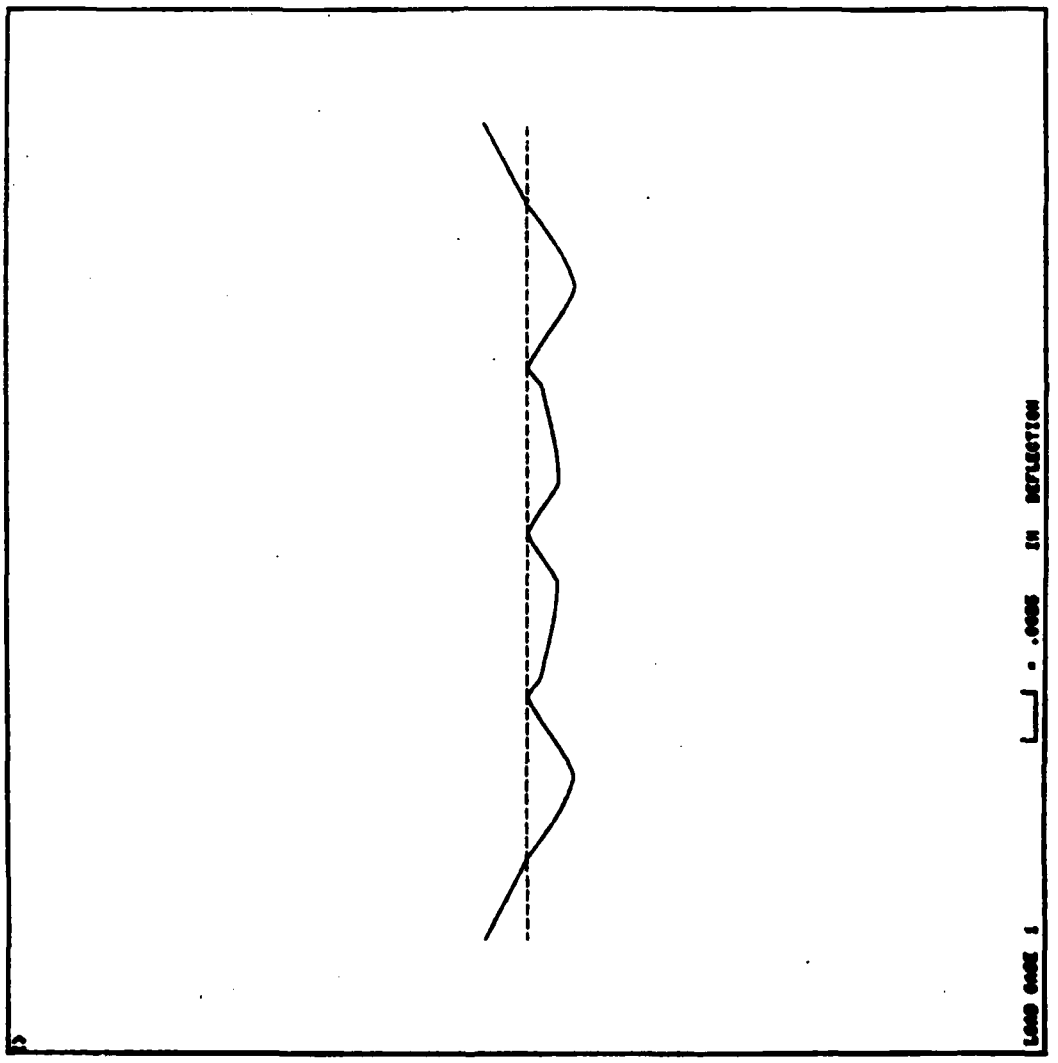
Intermediate Pier, Railroad Bridge Near Fruen Mill

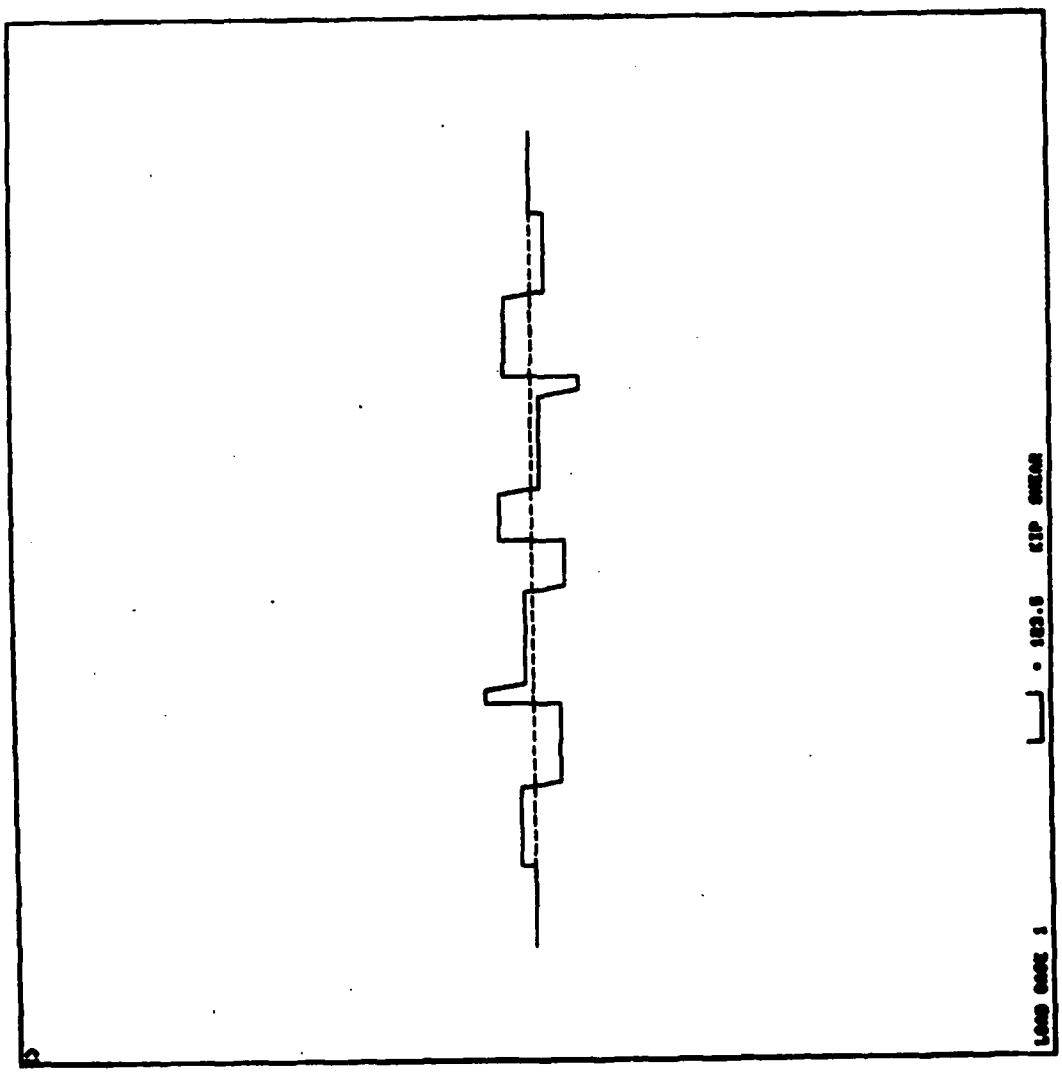


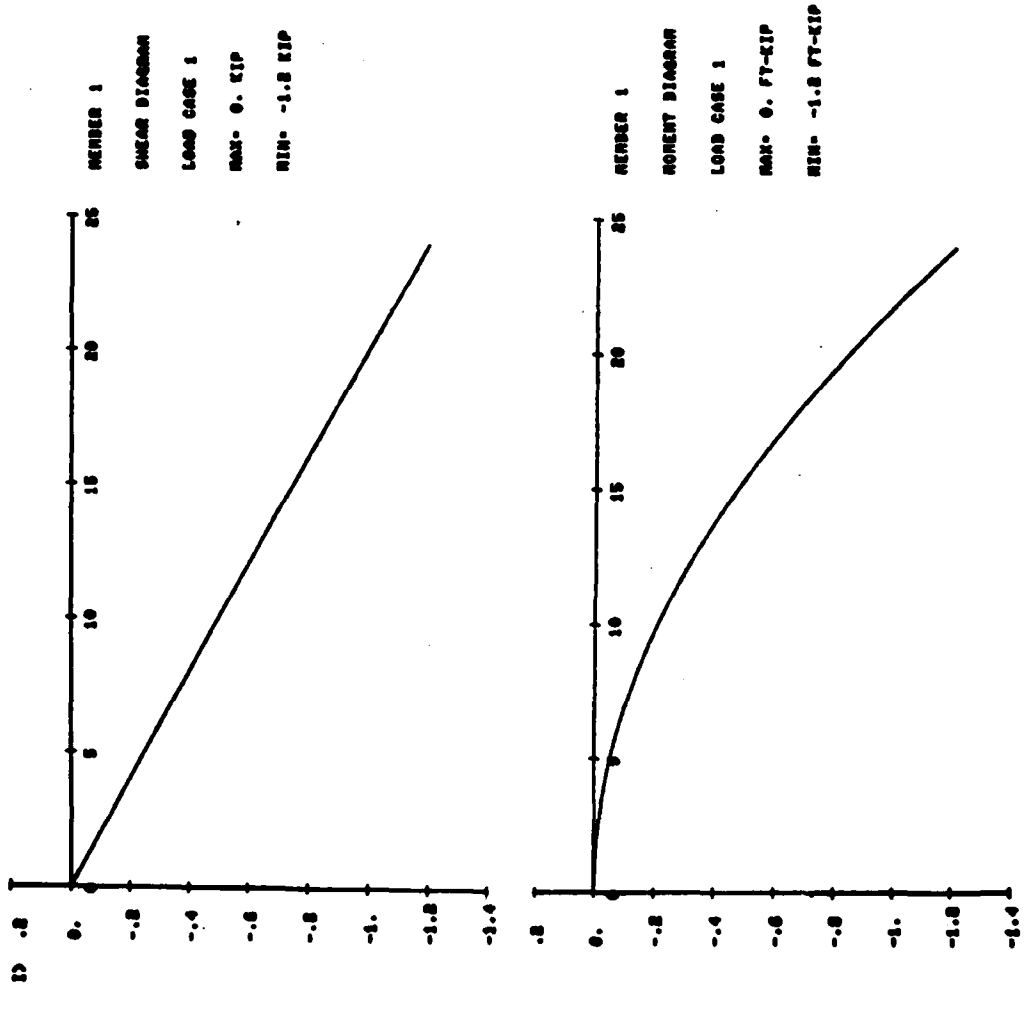

.6
102.4
102.4
102.4
102.4
102.4
.6
.6

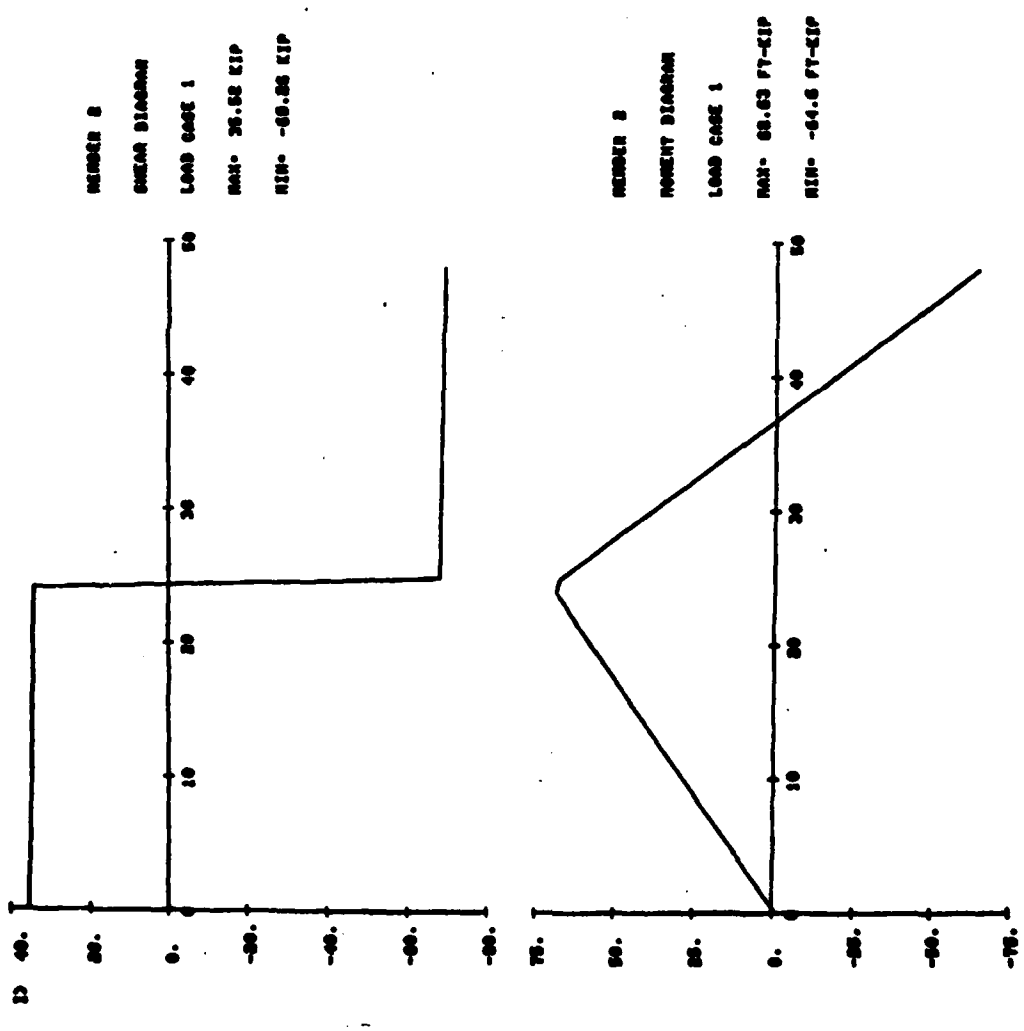
12/33

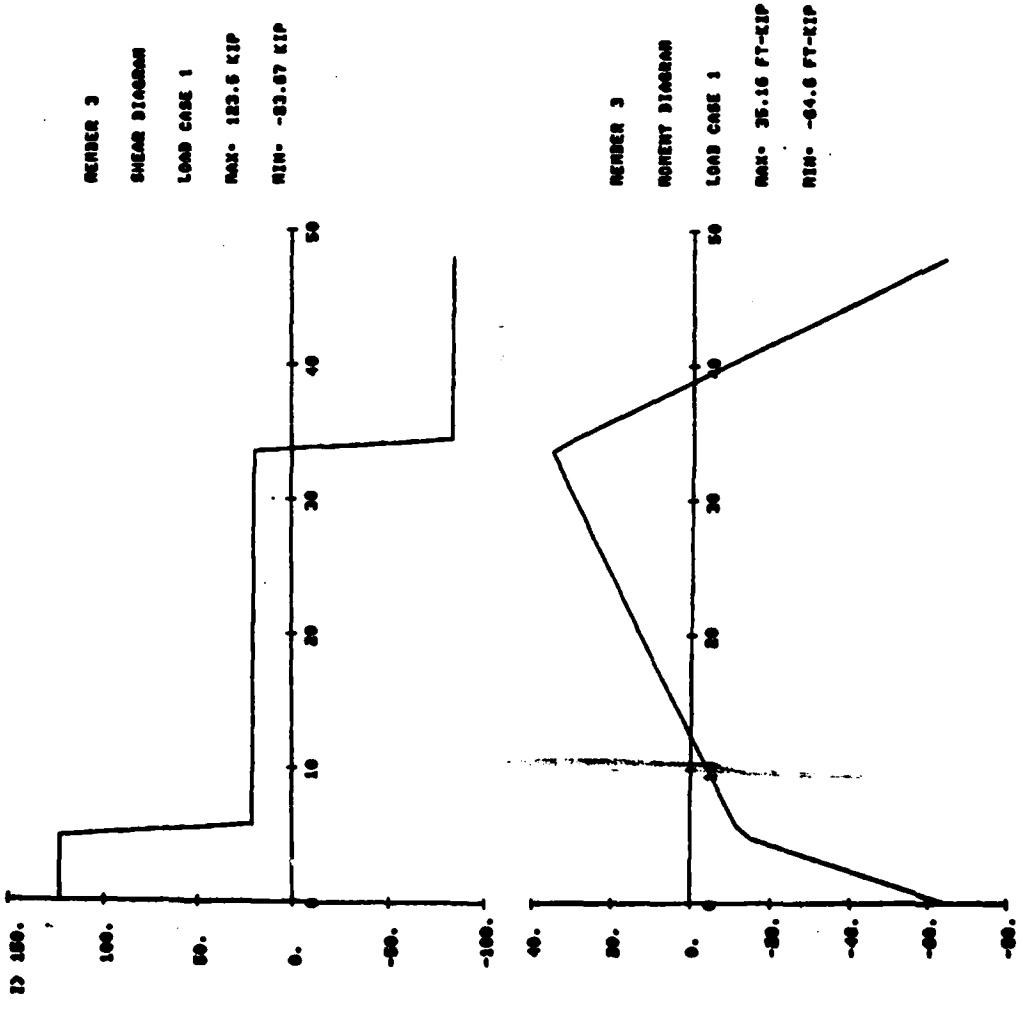


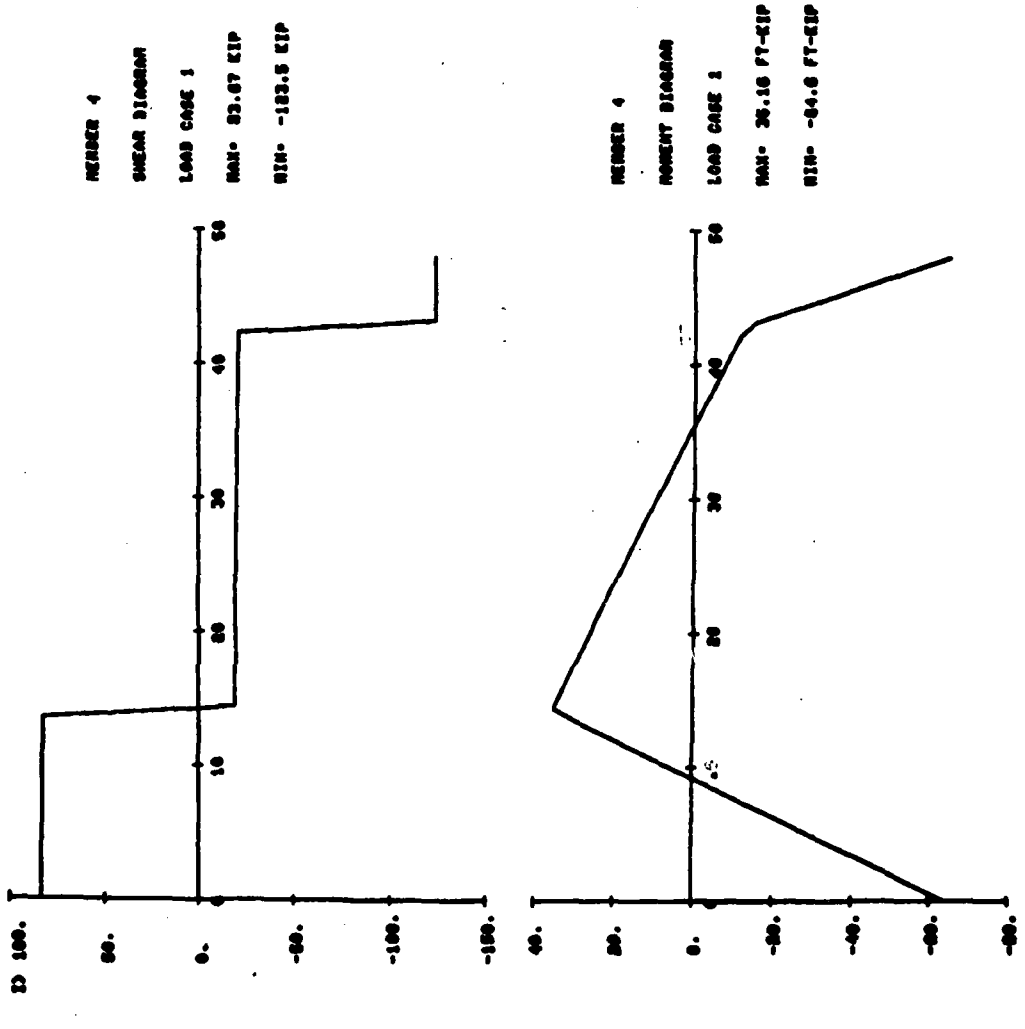

LOAD CASE 1
= .0005 IN DEFLECTION

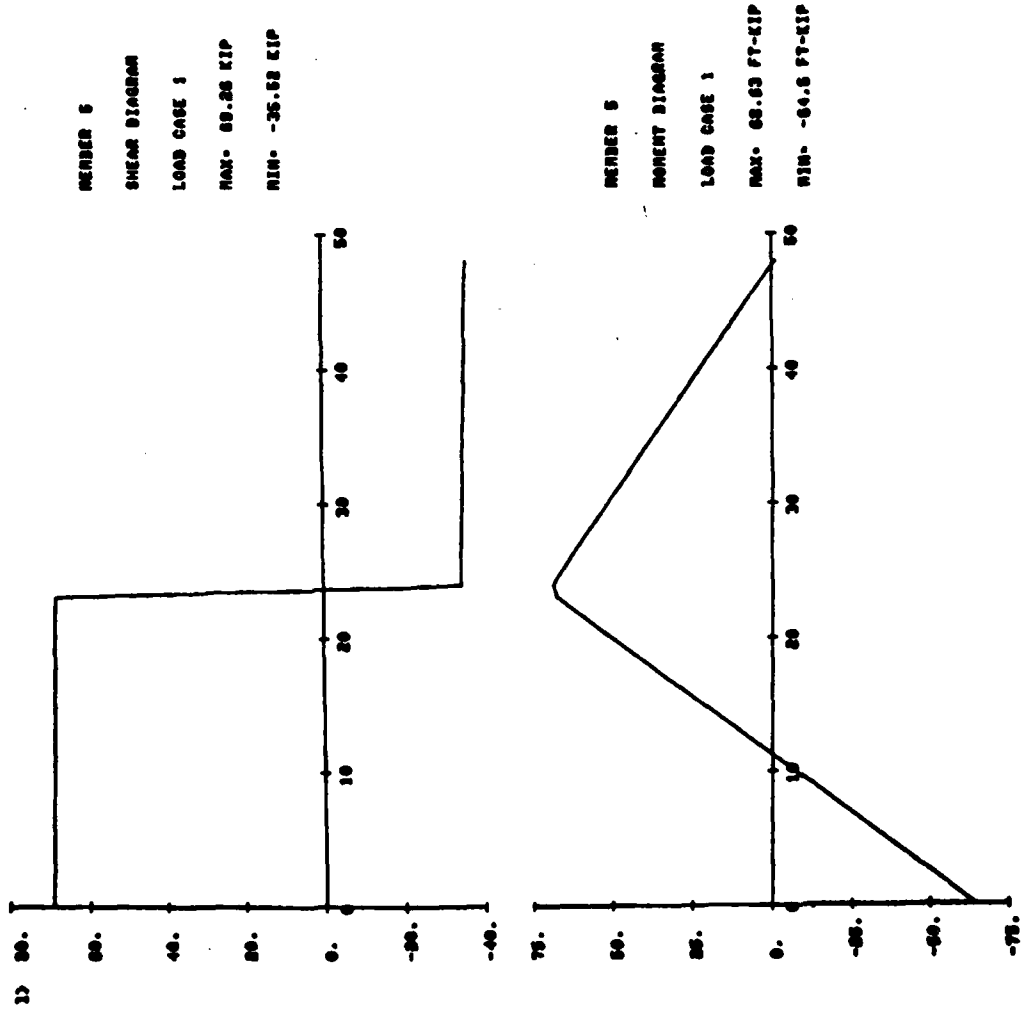



MEMBER 1
SHEAR DIAGRAM
LOAD CASE 1
MAX= 0. KIP
MIN= -1.8 KIP
MEMBER 1
MOMENT DIAGRAM
LOAD CASE 1
MAX= 0. FT-KIP
MIN= -1.8 FT-KIP

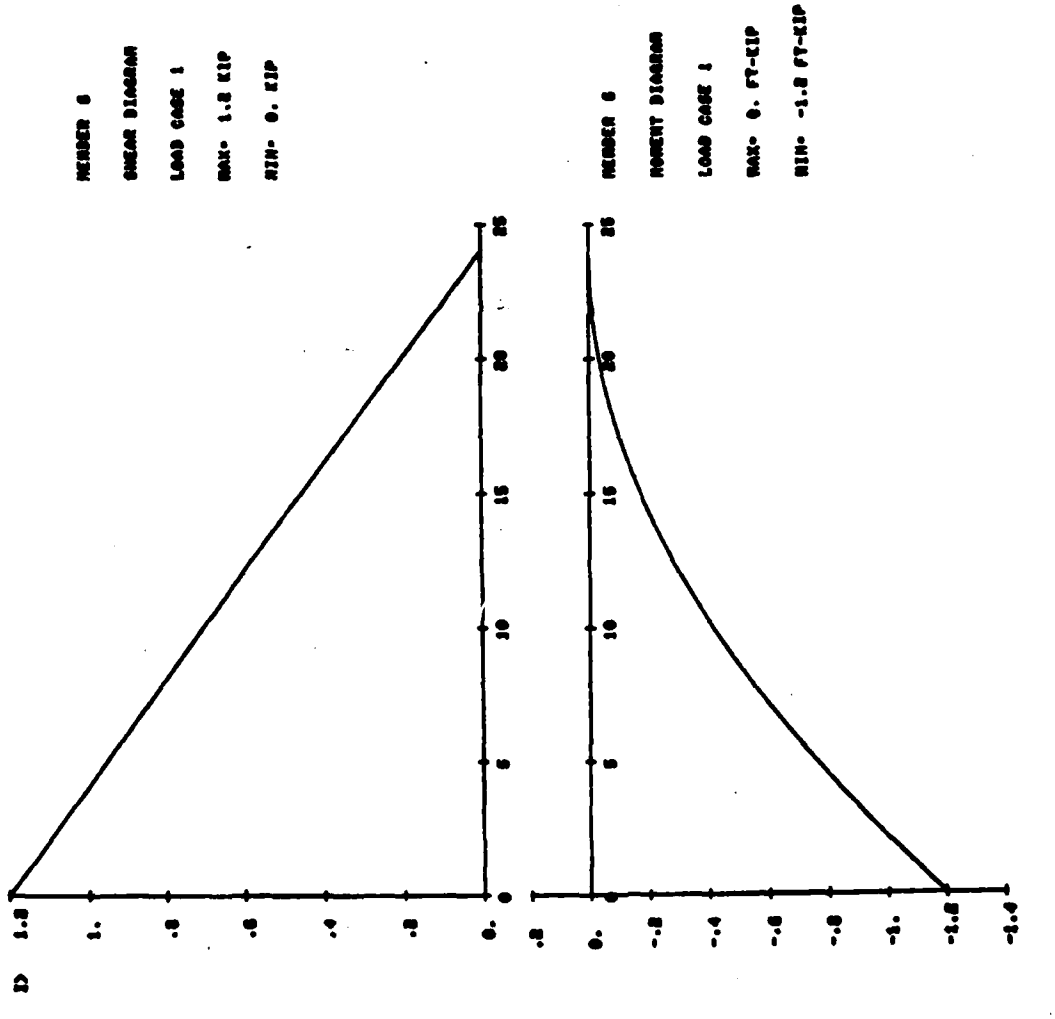

MEMBER 8
SHEAR DIAGRAM
LOAD CASE 1
MAX= 36.62 KIP
MIN= -60.86 KIP
MEMBER 8
MOMENT DIAGRAM
LOAD CASE 1
MAX= 69.63 FT-KIP
MIN= -64.6 FT-KIP


MEMBER 3
SHEAR DIAGRAM
LOAD CASE 1
MAX= 123.6 KIP
MIN= -83.87 KIP
MEMBER 3
MOMENT DIAGRAM
LOAD CASE 1
MAX= 35.16 FT-KIP
MIN= -64.6 FT-KIP


MEMBER 4
SHEAR DIAGRAM
LOAD CASE 1
MAX= 83.67 KIP
MIN= -183.5 KIP
100.
80.
0.
-80.
-100.
-180.
MEMBER 4
MOMENT DIAGRAM
LOAD CASE 1
MAX= 36.16 FT-KIP
MIN= -84.6 FT-KIP
40.
20.
0.
-20.
-40.
-60.
-80.


MEMBER 6
SHEAR DIAGRAM
LOAD CASE 1
MAX= 80.26 KIP
MIN= -35.58 KIP
50
40
30
20
10
80.
60.
40.
20.
0.
-20.
-40.
MEMBER 6
MOMENT DIAGRAM
LOAD CASE 1
MAX= 68.63 FT-KIP
MIN= -84.6 FT-KIP
50
40
30
20
10
75.
50.
25.
0.
-25.
-50.
-75.

20/33


MEMBER 6
SHEAR DIAGRAM
LOAD CASE 1
MAX= 1.8 KIP
MIN= 0. KIP
MEMBER 6
MOMENT DIAGRAM
LOAD CASE 1
MAX= 0. FT-KIP
MIN= -1.8 FT-KIP

P 50

```
1S-S-S-S-S-S-S-S-S-S-S-S-S-S-S-S-S-S-S-S-S
 PROGRAM CFRAME V01.05 06JUL78
 S-S-S-S-S-S-S-S-S-S-S-S-S-S-S-S-S-S-S-S-S

 RUN DATE =  81/08/18.
 RUN TIME =  06.01.08.

 INTERMEDIATE PIER FOR M N & S RR NEAR FRUEH MILL
1 SSS JOINT DATA SSS

                                  ------------------FIXITY-------------------
 JOINT      X         Y          X  Y  R       KX          KY          KR
            ----FT----                          ----KIP/IN----      IN-KIP/RAD

    1      0.00      0.00
    2      2.00      0.00     S  S
    3      6.00      0.00        S
    4     10.00      0.00        S
    5     14.00      0.00        S
    6     18.00      0.00        S
    7     20.00      0.00
1 SSS MEMBER DATA SSS

          END END
 MEMBER    A   B    LENGTH       I           A          AS          E          G
                      FT       FTSS4       FTSS2       FTSS2       KSI        KSI

    1     1   2      2.00   .1333E+01   .4000E+01   .4000E+01   .3834E+04   .1598E+04
    2     2   3      4.00   .1333E+01   .4000E+01   .4000E+01   .3834E+04   .1598E+04
    3     3   4      4.00   .1333E+01   .4000E+01   .4000E+01   .3834E+04   .1598E+04
    4     4   5      4.00   .1333E+01   .4000E+01   .4000E+01   .3834E+04   .1598E+04
    5     5   6      4.00   .1333E+01   .4000E+01   .4000E+01   .3834E+04   .1598E+04
    6     6   7      2.00   .1333E+01   .4000E+01   .4000E+01   .3834E+04   .1598E+04
1 SSS LOAD CASE  1 DATA SSS

 MEMBER  LA        PA        LB        PB        ANGLE
         FT      KIP/FT      FT      KIP/FT      DEG

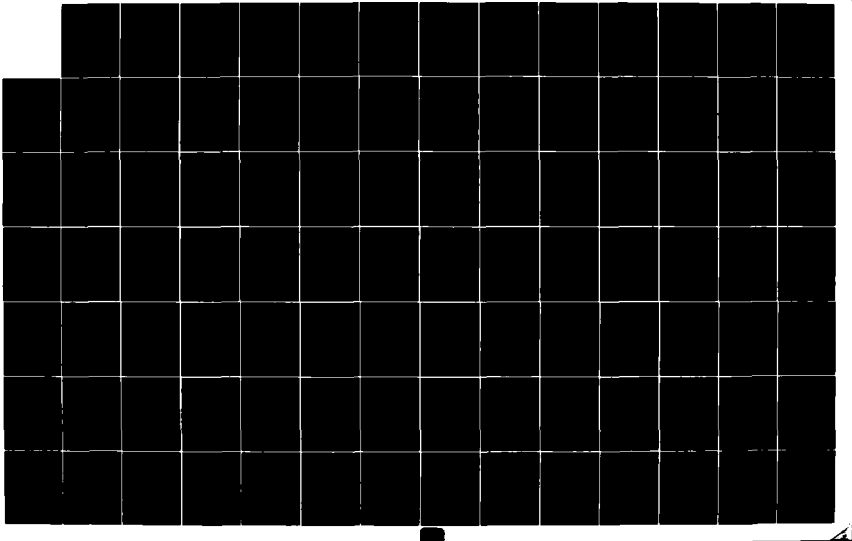
    1     0.00   .0000E+00     2.00   .0000E+00     0.00
    2     0.00   .0000E+00     4.00   .0000E+00     0.00
    3     0.00   .0000E+00     4.00   .0000E+00     0.00
    4     0.00   .0000E+00     4.00   .0000E+00     0.00
    5     0.00   .0000E+00     4.00   .0000E+00     0.00
    6     0.00   .0000E+00     2.00   .0000E+00     0.00

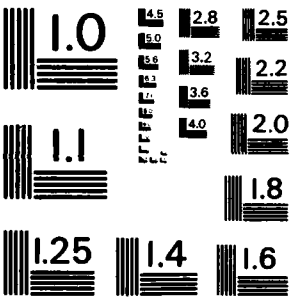
ED
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D-54

AD-A133 795 BASSETT CREEK WATERSHED HENNEPIN COUNTY MINNESOTA FEASIBILITY REPORT FOR FLOOD CONTROL MAIN REPORT(U) CORPS OF ENGINEERS ST PAUL MN ST PAUL DISTRICT SEP 82 5/7

UNCLASSIFIED F/G 13/2 NL

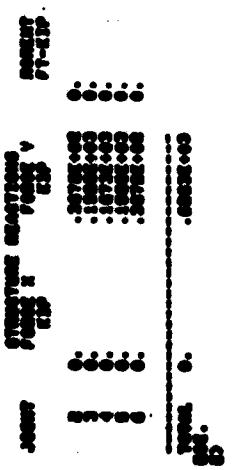




MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS - 1963 - A

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P 50

1$-$-$-
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ST. PAUL DISTRICT COMPUTATION SHEET	DATE 17 August 1981	PAGE 24 OF 33	FILE NUMBER
NAME OF OFFICE		COMPUTATION Intermediate Pier	
SUBJECT Bassett Creek		SOURCE DATA AREA	
COMPUTED BY GLC	CHECKED BY		APPROVED BY

Check Adequacy of 2'x2' Pile Cap

$M_{max} = 68.63$ 1K

$M_u = \frac{1.8(68.63)}{.9} = 137.26$ 1K

$R_u = \frac{M_u}{bd^2} = \frac{137.26(12000)}{24(21.5)^2} = 148.47$ psi

Use $f'_c = 4000$ psi $\quad f_y = 60{,}000$ psi $\quad m = \frac{f_y}{.85f'_c} = 17.65$

$\rho = \frac{1}{m}\left(1 - \sqrt{1 - \frac{2mR_u}{f_y}}\right) = .00253$

$\rho_{min} = \frac{200}{f_y} = .00333$

$A_s = \rho bd = .00333(24)(21.5) = 1.72$ in^2

Max V = 123.5

$v_u = \frac{V_u}{\phi bd} = \frac{1.8(123.5)(1000)}{.85(24)(21.5)} = 506$ psi

$v_c = 2\sqrt{f'_c} = 126.49$ Pg 8-2-41

$v_u - v_c \leq 8\sqrt{f'_c} = 506$ Section 2.35.3 Pg 8-2-42

∴ OK

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 18 Aug 1981	PAGE 25 OF 35	FILE NUMBER
NAME OF OFFICE		COMPUTATION Piling Design	
SUBJECT Bassett Creek, Fruen Mill		SOURCE DATA	
COMPUTED BY G.L.C.	CHECKED BY	APPROVED BY	

For 16"OD pipe pile, using skin friction, then

surface area / LF. of pile = $\pi \frac{16}{12} = 4.19\ ft^2$

$\therefore$ Capacity = $\sum$(Depth of pile in layer)(4.19 ft²)(q_b {#/ft² of surface area})

Boring 80-39M

Elevation	Material	q_b	P	ΣP
809				
	CL	1000	12.57	12.57
806				
	SP	200	.84	13.40
805				
	GP	2000	75.40	88.80
796				
	SP	2000	83.78	172.58
786				

Boring 80-34M

Elevation	Material	q_b	P	ΣP
809				
	GP	2000	8.38	8.38
808				
	CL	1500	94.25	102.63
793				
	SP	800	6.70	109.33
791				
	GP	2000	58.64	167.97
784				

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 18 Aug 1981	PAGE 26 OF 33	FILE NUMBER
NAME OF OFFICE		COMPUTATION Intermediate Piev	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY GLC	CHECKED BY		APPROVED BY

Redesign Piles

Max P = 192.8 K

Boring 80-39M (Page 2)

Below 786 = $\frac{192.8 - 172.58}{2(4.17)} = 2.41$

∴ Bottom eleu at 786 - 2.4 = 783.59 use 783.5

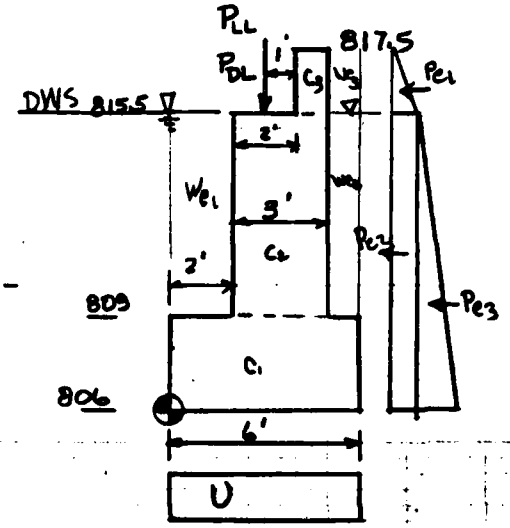
Boring 80-34M

Below 784 = $\frac{192.8 - 167.97}{2(4.17)} = 2.96$

784 - 2.96 = 781.04 use 781

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 12/15/81	PAGE 27 OF 33	FILE NUMBER
NAME OF OFFICE		COMPUTATION MN&S RR. Abutment	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY GLC	CHECKED BY	APPROVED BY	

Abutment Stability (One Foot Strip)



P_{DL} is girder weight. Since P_{DL} is 1.5' on centers, then

$P_{DL} = 3828^{\#}/1.5 = 2552^{\#}$

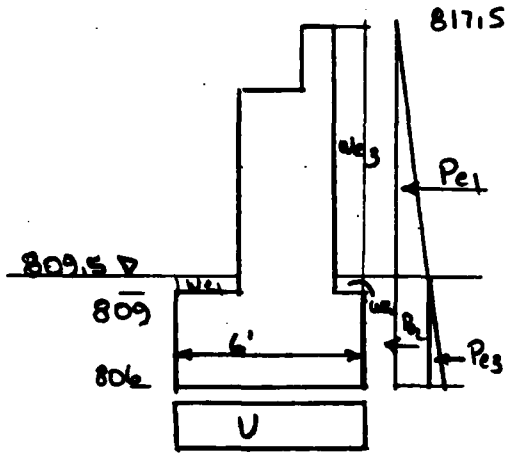
$P_{LL} = \frac{141.12^{K}}{3} = 47040^{\#}$

@ 1.5' on centers

$\therefore P_{LL} = \frac{47040}{1.5} = 31360^{\#}$

SYM	FACTOR	+ FORCES ↓	± →	ARM	+ MOMENT ↻	± ↻
C_1	3(6)(150)	2700		3.0	8100	
C_2	3(6.5)(150)	2925		3.5	10237.5	
C_3	1(2)(150)	300		4.5	1350	
We_1	.5(2)(6.25)	812.5		1.0	812.5	
We_2	.5(1)(130)	845		5.5	4647.5	
We_3	1(2)(120)	240		5.5	1320	
U	6(9.5)(62.5)	- 35625		3		106875
Pe_1	(2)²(½)(120 × .5)		120	10.17		1220.4
Pe_2	2(120 × .5) 9.5		1140	9.5/2		5415
Pe_3	(130-62.5)(9.5)²(½)(.5)		1522.0	9.5/3		4822.7
P_{DL}		2552		3.0	7656	
		6812	2783	1.76	341235	221456
				Σ	11978	
P_{LL}		31360		3	94080	
		38172	2783	2.78	1282035	221456
					106058	

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 12/15/81	PAGE 28 OF 33	FILE NUMBER
NAME OF OFFICE	COMPUTATION MN&S RR Abutment		
SUBJECT Bassett Creek	SOURCE DATA		
COMPUTED BY G.L.C.	CHECKED BY	APPROVED BY	



SYM	FACTORS	↓ FORCES	←	→	ARM	↻ MOMENT	↺
C_1		2700			3.0	8100	
C_2		2925			3.5	10237.5	
C_3		300			4.5	1350	
We_1	.5(2)(62.5)	62.5			1.0	62.5	
We_2	.5(1)(130)	65.0			5.5	357.5	
We_3	8(1)(120)	960			5.5	5280	
U	6(3.5)(62.5)	(-) 1312.5			3.0		3937.5
Pe1	$(8)^2(1/2)(120\#.5)$			1920	6.17		11840
Pe2	8(120#.5)(3.5)			1680	3.5/2		2940
Pe3	(130-62.5)(3.5)²(1/2)(.5)			206.7	3.5/3		241.2
P_{DL}		2552			3.0	7656	
		8252		3806.7		33093.5	18958.7
					1.71	$\Sigma H = 14084.8$	
P_{LL}		31360			3	94080	
		39612		3806.7	2.73	127173.5	18958.7
						$\Sigma H = 10816.5$	

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 12/15/81	PAGE 29 OF 33	FILE NUMBER
NAME OF OFFICE		COMPUTATION	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY GLC	CHECKED BY		APPROVED BY

Since stability analysis was performed on a one foot strip, the total vertical force must be compute

For DWS without railroad load

$\Sigma V = 37(6812 - 2552) + 6(3828) = 180588^{\#}$

For DWS with railroad load

$\Sigma V = 180588 + 6(47040) = 462828^{\#}$

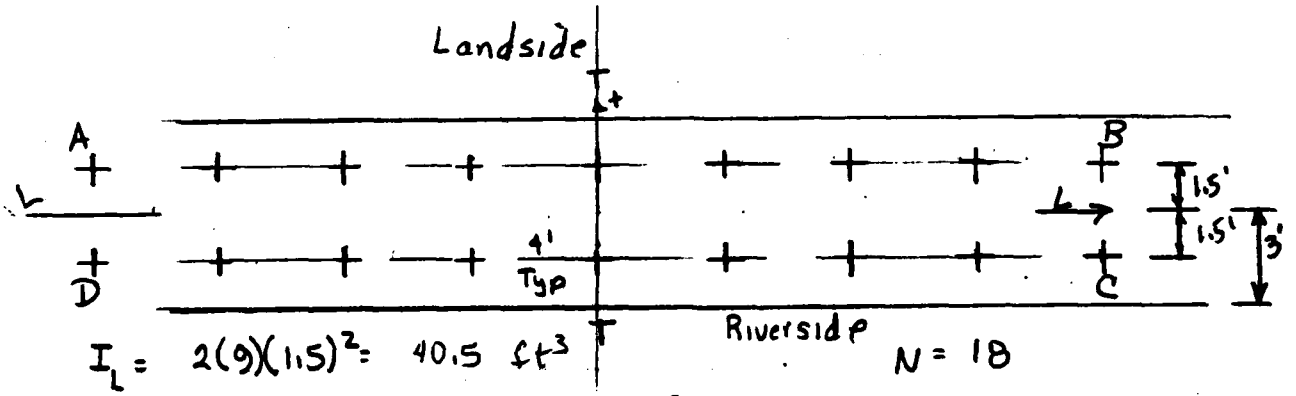
For NWS without railroad load

$\Sigma V = 37(8252 - 2552) + 6(3828) = 233868$

For NWS with railroad load

$\Sigma V = 233868 + 6(47040) = 516108$

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 12/23/81	PAGE 30 OF 33	FILE NUMBER
NAME OF OFFICE		COMPUTATION M N & S R.R. Bridge	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY G.L.C	CHECKED BY	APPROVED BY	



$I_L = 2(9)(1.5)^2 = 40.5\ ft^3$ $N = 18$

$I_T = 4(4)^2 + 4(8)^2 + 4(12)^2 + 4(16)^2 = 1920\ ft^3$

Load Case DWS case

1 Without train load $e = 1.76 - 3 = -1.24$

2 With train load $e = 2.78 - 3 = -0.22$

Normal Water Case

3 Without train load $e = 1.71 - 3 = -1.29$

4 With train load $e = 2.73 - 3 = -0.27$

Pile A & B $\quad \frac{\Sigma V}{18} + \frac{\Sigma V e (1.5)}{40.5}$

Pile	Loading Case	ΣV	e	Pile Load (lbs)	Pile Load (Tons)	
A&B	1	180588	-1.24	1739	.87	
	2	462828	-0.22	21941	10.97	
	3	233868	-1.29	1819	.91	
	4	516108	-0.27	25512	11.76	
C&D	1	180588	-1.24	18326	9.16	
	2	462828	-0.22	29184	14.74	
	3	233868	-1.29	24166	12.08	
	4	516108	-0.27	33534	16.92	← Max pile load

Determine pile penetration below 806

		q	p	Σp
806				
805	SP	200	.84	.84
	GP	4000		

Below 805 = $\frac{33.0 - .8}{2(4.19)} = 3.94'$ Bottom elev = 805 − 4 = 801

D-63

∴ Redesign - Too many piles

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 12/24/81	PAGE 31 OF 33	FILE NUMBER
NAME OF OFFICE		COMPUTATION MNS RR Bridge	
SUBJECT Bassett Creek		SOURCE DATA AREA	
COMPUTED BY G.L.C	CHECKED BY	APPROVED BY	

Check adequacy of 16" ϕ pipe piles

Minimum wall thickness of 3/8" by FP 4.4.3.3

Allowable stresses FP 4.4.5.6

$f_c = .3f'_c < 1200 psi$

$f_s = 12{,}600$ psi

for a 16" ϕ, 3/8" thick $A_s = 18.41 in^2$ $A_c = 182.6 in^2$

From PILETIPS – Design and Installation of Pile Foundations

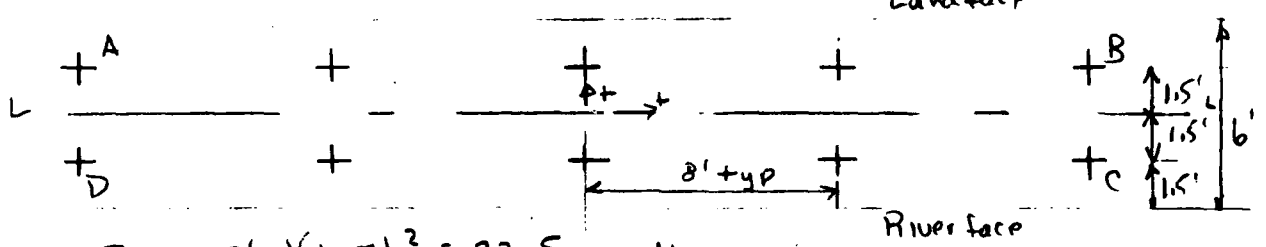
$P = f'_c A_c + f_s A_s = 182.6(.3 \times 4000) + 18.41(12{,}600)$

$= 231966 + 219120 = 451.086 K$

∴ Any pile loads will be significantly less than the pile capacity.

ST. PAUL DISTRICT COMPUTATION SHEET	DATE 12/24/81	PAGE 32 OF 33	FILE NUMBER
NAME OF OFFICE		COMPUTATION M N&S R.R. Bridge	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY G.L.C	CHECKED BY	APPROVED BY	

Revise pile layout



$I_{LL} = 2(5)(1.5)^2 = 22.5$ N = 10 piles

Load Case	DWS	
1	Without train	e = 1.76-3 = -1.24'
2	With train	e = 2.78-3 = -0.22
	Normal Water Surface	
3	Without train	e = 1.71-3 = -1.29'
4	With train	e = 2.73-3 = -0.27'

Piles A&B $P = \frac{\Sigma V}{N} + \frac{\Sigma V e (1.5)}{I_{LL}}$ Piles C&D $P = \frac{\Sigma V}{N} + \frac{\Sigma V e (-1.5)}{I_{LL}}$

Pile	Loading Case	ΣV lb	e ft	Pile Load lbs	Pile Load Tons	
A&B	1	180588	-1.24	3130	1.57	
	2	462828	-0.22	39495	19.75	
	3	233868	-1.29	3274	1.64	
	4	516108	-0.27	42321	21.16	
C&D	1	180588	-1.24	32987	16.49	
	2	462828	-0.22	53071	26.54	
	3	233868	-1.29	43499	21.75	
	4	516108	-0.27	60901	30.45	← Max Pile Load

Determine Pile Penetration

		q	$P_a = m(k_{ips})\,q$	ΣP
806	SD	20		
805	GP	2000	.84	.84

Below 805 = $\frac{60901 - .84}{2(4.19)} = 7.17'$ Bottom elev. 805 - 7.17 = 797.83 Use 797.5

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ST. PAUL DISTRICT COMPUTATION SHEET	DATE 12/24/81	PAGE 35 OF 33	FILE NUMBER
NAME OF OFFICE		COMPUTATION MN&S RR Bridge	
SUBJECT Bassett Creek		SOURCE DATA	
COMPUTED BY GLC	CHECKED BY	APPROVED BY	

Determine Pile Batter

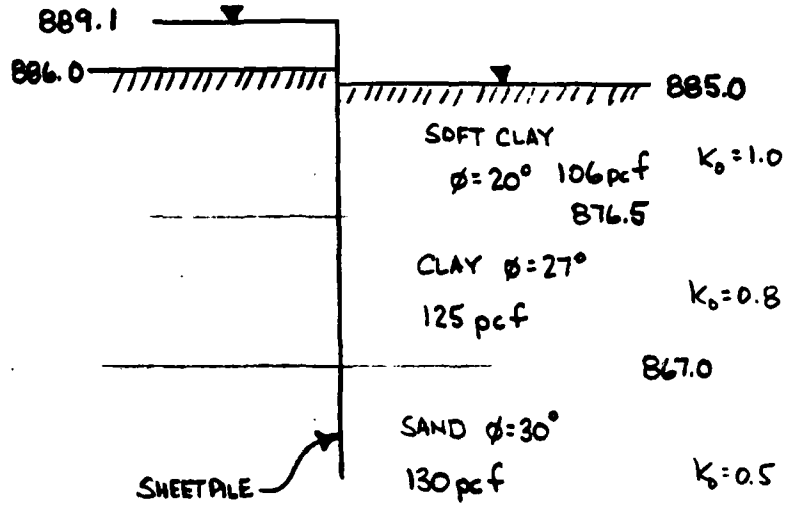
Batter: $m = 12 \frac{\Sigma H}{\Sigma V}$

$\Sigma H = 37(\Sigma H/_{foot})$ $\Sigma V = 5(\text{Pile C\&D})$

LC	$\Sigma H/_{Ft}$	ΣH	$\Sigma V/_{pile}$	ΣV	m
1	2783	102971	32987	164935	7.49
2	2783	102971	53071	265355	4.66
3	3806.7	140848	43499	217495	7.77 ← Governs
4	3806.7	140848	60901	304505	5.55

Use Batter of $\frac{7.77}{12} \simeq 30°$ ∴ use batter of $\frac{8}{12}$

DETERMINE SHEET PILE DEPTH FOR CONTROL STRUCTURE



SOIL PROFILE TAKEN FROM BORING 80-28M

SAND LAYER IN CLAY LAYER IS IGNORED (A CONSERVATIVE ASSUMPTION)

USE PROGRAM SHTSSI (X0060)

COMPUTER INPUT

from SHEETPILE PORTION OF SSI NOTEBOOK

$E_{SAND} = 5.0$ pci for lower MEDIUM SAND

$E_{CLAY} = 87$ pci for $q_f = 1\text{-}2$ Tsf $\quad q_f = 2c = 2\left(\frac{1500}{2000}\right) = 1.5$ Tsf

$E_{CLAY} = 45$ pci est. for $q_f = 0.6$ Tsf $\quad q_f = 2\left(\frac{600}{2000}\right) = 0.6$ Tsf

I PMA 22 SHEETPILE = $22.4\ in^4$ / 19.625" WIDTH = $13.7\ in^4/FT$

A PMA 22 SHEETPILE = $10.6\ in^2$ / 19.625" WIDTH = $6.48\ in^2/FT$

PROGRAM SHELL - SOIL-STRUCTURE INTERACTION ANALYSIS
OF CANTILEVER OR ANCHORED SHEET PILE RETAINING WALL
DATE: 02/08/82 TIME: 12:54:12

I.--INPUT DATA

1.--HEADING

RAT ATT CREEK
NO CONTROL
CANTILEVER
EMA 22

2.--WALL DATA

ELEVATION AT TOP OF WALL	=	889.10	(FT)
ELEVATION AT BOTTOM OF WALL	=	~~869.10~~ 869.1	(FT)
WALL MODULUS OF ELASTICITY	=	29.0E+06	(PSI)
WALL MOMENT OF INERTIA	=	12.70	(IN**4)
WALL CROSS SECTION AREA	=	6.48	(SQIN)

3.--ANCHOR DATA
NONE

4.--RIGHTSIDE SOIL DATA

LAYER NO	TOP ELEV AT WALL (FT)	UNIT WEIGHT (PCF)	INTERN FRICT (DEG)	COH-HESION (PSF)	WALL FRICT (DEG)	AT-REST COEFF	SOIL MODULUS (PCI)	INTERACT DISTANCE (FT)
1	885.00	106.00	20.00	0.00	0.00	1.00	45.00	8.50
2	876.50	125.00	27.00	0.00	0.00	.80	87.00	9.50
3	867.00	130.00	30.00	0.00	0.00	.50	5.00	7.00

5.--LEFTSIDE SOIL DATA

LAYER NO	TOP ELEV AT WALL (FT)	UNIT WEIGHT (PCF)	INTERN FRICT (DEG)	COH-HESION (PSF)	WALL FRICT (DEG)	AT-REST COEFF	SOIL MODULUS (PCI)	INTERACT DISTANCE (FT)
1	885.00	106.00	20.00	0.00	0.00	1.00	45.00	9.50
2	876.50	125.00	27.00	0.00	0.00	.80	87.00	9.50
3	867.00	130.00	30.00	0.00	0.00	.50	5.00	7.00

6.--WATER DATA

WATER UNIT WEIGHT	=	62.50	(PCF)
RIGHTSIDE WATER ELEVATION	=	885.00	(FT)
LEFTSIDE WATER ELEVATION	=	889.10	(FT)

7.--SURFACE SURCHARGE LOADS
NONE

8.--HORIZONTAL LINE LOADS
NONE

9.--HORIZONTAL APPLIED PRESSURES
NONE

PROGRAM CANTI - SOIL-STRUCTURE INTERACTION ANALYSIS
OF CANTILEVER OR ANCHORED SHEET PILE RETAINING WALLS
DATE: 02/05/82 TIME: 13:58:25

III.--SUMMARY OF RESULTS

III.A.--HEADING
BASSETT CREEK
ML CONTROL
CANTILEVER
'PMA 22'

III.B.--MAXIMA

	MAXIMUM POSITIVE	ELEV (FT)	MAXIMUM NEGATIVE	ELEV (FT)
AXIAL DISPLACEMENT (IN) :	0.	0.00	0.	0.00
LATERAL DISPLACEMENT (IN):	2.00E-02	869.10	-4.00E+00	889.10
AXIAL FORCE (LB) :	0.	0.00	0.	0.00
SHEAR (LB) :	1.11E+03	881.00	-2.13E+03	872.55
BENDING MOMENT (LB-FT) :	8.56E+03	876.01	0.	0.00

DO YOU WANT COMPLETE RESULTS OUTPUT? ENTER 'YES' OR 'NO'.
I>Y

IV.--COMPLETE RESULTS

IV.A.--HEADING
BASSETT CREEK
ML CONTROL
CANTILEVER
'PMA 22'

IV.B.--COMPLETE RESULTS

ELEV (FT)	<----DEFLECTIONS---> AXIAL (IN)	LATERAL (IN)	AXIAL FORCE (LB)	SHEAR (LB)	BENDING MOMENT (LB-FT)	SOIL PRESSURE (PSF)
889.10	0.	-4.00E+00	0.	0.	0.	0.00
888.10	0.	-3.67E+00	0.	32.	12.	0.00
887.10	0.	-3.34E+00	0.	127.	86.	0.00
886.10	0.	-3.01E+00	0.	282.	284.	0.00
886.00	0.	-2.97E+00	0.	300.	310.	0.00
885.10	0.	-2.68E+00	0.	510.	674.	-19.19
885.00	0.	-2.64E+00	0.	536.	721.	-21.33
884.10	0.	-2.35E+00	0.	757.	1313.	39.33
883.10	0.	-2.03E+00	0.	940.	2167.	106.72
882.10	0.	-1.72E+00	0.	1056.	3171.	174.12
881.10	0.	-1.42E+00	0.	1105.	4257.	241.51
880.10	0.	-1.14E+00	0.	1086.	5358.	308.91
879.10	0.	-8.86E-01	0.	999.	6406.	376.30
878.10	0.	-6.58E-01	0.	846.	7334.	443.70
877.10	0.	-4.62E-01	0.	624.	8074.	511.09
876.50	0.	-3.60E-01	0.	461.	8407.	551.53
876.50	0.	-3.60E-01	0.	461.	8407.	829.44
876.10	0.	-3.01E-01	0.	217.	8532.	886.62
875.10	0.	-1.77E-01	0.	-484.	8412.	1029.59
874.10	0.	-8.91E-02	0.	-1347.	7514.	1131.18
873.10	0.	-3.36E-02	0.	-1979.	5811.	648.40
872.10	0.	-3.34E-03	0.	-2049.	3720.	57.93
871.10	0.	1.07E-02	0.	-1669.	1800.	-368.82
870.10	0.	1.65E-02	0.	-928.	491.	-587.62
869.10	0.	2.00E-02	0.	0.	0.	-756.39
867.00	0.	2.00E-02	0.	0.	0.	-754.56

VERIFY ADEQUACY OF 20' LONG PMA-22 SHEETPILE.

MAXIMUM BENDING = 8560 FT-lb. /FT

$$f_b = M/S_x = \frac{8560(12)}{5.4\ in^3} = 19{,}022\ psi < 19{,}250\ O.K.$$

$$f_{ALL} = f_y/2 = \frac{38500}{2} = 19{,}250\ psi$$

MAXIMUM SHEAR = 2130 lb. /FT

ALLOWABLE SHEAR = $0.33F_y = \frac{38500}{3} = 12{,}833\ psi$

$$\frac{2130}{L48} = 329\ psi < 12{,}833\ O.K.$$

CREEP

4.1' of RETAINED HEAD.

SHEET PILE IS 30% EFFECTIVE

$$4.1/0.3 = 13.7'$$

MINIMUM PILE LENGTH = 13.7 + 4.1 = 17.8'

DEFLECTION

4" AT MAXIMUM LOAD

THE 4" DEFLECTION WOULD BE INCREASED BY REDUCTION OF PILE LENGTH (3.95" WITH 22' PILE)

THE PLACEMENT OF RIPRAP BEHIND THE SHEETPILE WOULD REDUCE DEFLECTION

USING A CONCRETE CAP TO COVER THE TOP 5' OF THE SHEETPILE WOULD ALSO REDUCE DEFLECTION.

A MORE DETAILED INVESTIGATION OF THE DEFLECTION WILL BE DONE DURING FURTHUR STAGES OF ANALYSIS.

-1 by GLC

REFERENCE: ANALYSIS OF GROUND-LINER INTERACTION FOR TUNNELS
REPORT NO. UMTA-IL-06-0043-78-3
OCTOBER 1978
PREPARED FOR U.S. DEPT OF TRANSPORTATION BY THE UNIV. OF ILLINOIS

OBTAIN EXTERIOR TUNNEL PRESSURES USING EQUATIONS PRESENTED IN THE ABOVE REFERENCE FOR NO-SLIP AND FULL-SLIP CONDITIONS ASSUMING BOTH THIN AND THICK WALL CONDUITS.

EXCAVATION LOADING - NO SLIPPAGE, THIN LINER

EXTERNAL PRESSURES ACTING ON LINER

$$P_r = \frac{\gamma H}{2}\left[(1+K_o)(1-L_\eta^*) - (1-K_o)\left[1 + 3J_\eta^* - 2N_\eta^*\right]\cos 2\theta\right]$$

SEC. B.3.1 PAGE 405

$$T_{r\theta} = \frac{\gamma H}{2}\left[(1-K_o)(1-3J_\eta^* + N_\eta^*)\sin 2\theta\right]$$

SEC. B.3.1 PAGE 405

$$L_\eta^* = \frac{(1-2\nu_m)C}{1+(1-2\nu_m)C + \frac{(1-2\nu_m)C}{6F}}$$

EQ. BOTTOM PAGE 350

$$J_\eta^* = \frac{[2\nu_m + (1-2\nu_m)C]F + (1-\nu_m)(1-2\nu_m)C}{[(3-2\nu_m)+(1-2\nu_m)C]F + \frac{1}{2}(5-6\nu_m)(1-2\nu_m)C + (6-8\nu_m)}$$

SEC B.3.5 PAGE 406

$$N_\eta^* = \frac{[3+2(1-2\nu_m)C]F + \frac{1}{2}(1-2\nu_m)C}{[(3-2\nu_m)+(1-2\nu_m)C]F + \frac{1}{2}(5-6\nu_m)(1-2\nu_m)C + (6-8\nu_m)}$$

SEC. B.3.5 PAGE 406

$$F = \left(\frac{E_m}{E_l}\right)\left(\frac{r}{t}\right)^3\left[\frac{2(1-\nu_l^2)}{(1+\nu_m)}\right]$$

EQ. A.42b PAGE 339

$$C = \left(\frac{E_m}{E_l}\right)\frac{r}{t}\left[\frac{(1-\nu_l^2)}{(1+\nu_m)(1-2\nu_m)}\right]$$

EQ. A.42a PAGE 339

-'d by GLC

EXCAVATION LOADING - FULL SLIPPAGE, THIN LINER

$$P_r = \frac{\gamma H}{2}\left[(1+K_o)(1-L_\eta^*) - 3(1-K_o)(1-2J_\eta^*)\cos 2\theta\right]$$

SEC. B.4.1 PAGE 407

L_η^* EQUATION REMAINS THE SAME.

$$J_\eta^*(J_f^*) = \frac{F+(1-\nu_m)}{2F+(5-6\nu_m)}$$

SEC. B.4.5 page 408

C AND F EQUATIONS REMAIN THE SAME

EXCAVATION LOADING - NO SLIPPAGE, THICK LINER

EXTERNAL PRESSURE ACTING ON LINER

$$P_r = \frac{\gamma H}{2}\left[(1+K_o)\left[1-L_\eta^*\left(\frac{R_o}{r}\right)^2\right] - (1-K_o)\left[1+3J_\eta^*\left(\frac{R_o}{r}\right)^4 - 4N_\eta^*\left(\frac{R_o}{r}\right)^2\right]\cos 2\theta\right]$$

SEC. B.7.2 PAGE 419

$$L_\eta^* = \frac{b_2^*}{\left(\frac{G_l}{G_m}\right)b_1^* + b_2^*} \qquad T_{r\theta} = \frac{\gamma H}{2}\left[(1-K_o)\left[1-3J_\eta^*\left(\frac{R_o}{R_o}\right)^2 + 2N_\eta^*\left(\frac{R_o}{R_o}\right)^2\right]\sin 2\theta\right]$$

$$J_\eta^* = \frac{\left(\frac{G_l}{G_m}\right)\left[b_{11}^* + 16\nu_m b_{12}^*\right] + b_6^*}{(3-4\nu_m)\left(\frac{G_l}{G_m}\right)^2 b_3^* + 2\left(\frac{G_l}{G_m}\right)\left[b_4^* - 2\nu_m b_5^*\right] + b_6^*}$$

SEC. B.7.3 PAGE 420

$$N_\eta^* = \frac{\left(\frac{G_l}{G_m}\right)b_{13}^* + b_6^*}{(3-4\nu_m)\left(\frac{G_l}{G_m}\right)^2 b_3^* + 2\left(\frac{G_l}{G_m}\right)\left[b_4^* - 2\nu_m b_5^*\right] + b_6^*}$$

$$b_1^* = \left[\left(\frac{R_o}{R_i}\right)^2 - 1\right]$$

SEC. B.7.3 PAGE 421

$$b_2^* = \left[(1-2\nu_l)\left(\frac{R_o}{R_i}\right)^2 + 1\right]$$

$$b_3^* = \left[\left(\frac{R_o}{R_i}\right)^2 - 1\right]^4$$

$$b_4^* = \left[(5-6\nu_l)\left(\frac{R_o}{R_i}\right)^6 + (5-2\nu_l)\left(\frac{R_o}{R_i}\right)^4 - (1+2\nu_l)\left(\frac{R_o}{R_i}\right)^2 + (3-2\nu_l)\right]\left[\left(\frac{R_o}{R_i}\right)^2 - 1\right]$$

$$b_5^* = \left[(3-4\nu_1)\left(\frac{R_o}{R_i}\right)^6 + 3\left(\frac{R_o}{R_i}\right)^4 - 3\left(\frac{R_o}{R_i}\right)^2 + 1\right]\left[\left(\frac{R_o}{R_i}\right)^2 - 1\right]$$

$$b_6^* = \left[(3-4\nu_1)\left(\frac{R_o}{R_i}\right)^8 + 4(3-6\nu_1+4\nu_1^2)\left(\frac{R_o}{R_i}\right)^6 - 6\left(\frac{R_o}{R_i}\right)^4 + 4\left(\frac{R_o}{R_i}\right)^2 + (3-4\nu_1)\right]$$

$$b_{11}^* = \left[\left(\frac{R_o}{R_i}\right)^6 - (11-8\nu_1)\left(\frac{R_o}{R_i}\right)^4 + (7-4\nu_1)\left(\frac{R_o}{R_i}\right)^2 + (3-4\nu_1)\right]\left[\left(\frac{R_o}{R_i}\right)^2 - 1\right]$$

Sec. 8.7.3. PAGE 422

$$b_{12}^* = (1-\nu_1)\left[\left(\frac{R_o}{R_i}\right)^6 - \left(\frac{R_o}{R_i}\right)^4\right]$$

$$b_{13}^* = \left[\left(\frac{R_o}{R_i}\right)^6 + (1-4\nu_1)\left(\frac{R_o}{R_i}\right)^4 + (7-4\nu_1)\left(\frac{R_o}{R_i}\right)^2 + (3-4\nu_1)\right]\left[\left(\frac{R_o}{R_i}\right)^2 - 1\right]$$

EXCAVATION LOADING - FULL SLIPPAGE, THICK LINER

$$P_r = \frac{\gamma H}{2}\left[(1+K_o)\left[1-L_n^*\left(\frac{R_o}{r}\right)^2\right] - (1-K_o)\left[1+3J_n^*\left(\frac{R_o}{r}\right)^4 - 2N_n^*\left(\frac{R_o}{r}\right)^2\right]\cos 2\theta\right] \quad ; \quad T_{ro} = 0$$

Sec. 8.8.2. PAGE 424

L_n^* AND b_1^* AND b_2^* EQUATIONS REMAIN THE SAME.

$$J_n^* = \frac{2(1-\nu_m)\left(\frac{G_1}{G_m}\right)d_3^* + d_4^*}{(5-6\nu_m)\left(\frac{G_1}{G_m}\right)d_3^* + d_4^*}$$

Sec. 8.8.2. PAGES 425, 426

$$N_n^* = \frac{\left(\frac{G_1}{G_m}\right)d_3^* + 2d_4^*}{(5-6\nu_m)\left(\frac{G_1}{G_m}\right)d_3^* + d_4^*}$$

$$d_3^* = \left[\left(\frac{R_o}{R_i}\right)^2 - 1\right]^3$$

$$d_4^* = \left[(3-2\nu_1)\left(\frac{R_o}{R_i}\right)^6 + 3(5-6\nu_1)\left(\frac{R_o}{R_i}\right)^4 + 3(3-2\nu_1)\left(\frac{R_o}{R_i}\right)^2 + (5-6\nu_1)\right]$$

LIST OF VARIABLES

γ = UNIT SOIL WEIGHT

H = HEIGHT OF FILL TO CENTER OF TUNNEL

K_o = AT REST EARTH PRESSURE COEFFICIENT

r = RADIUS OF PRESSURE SURFACE

ν_m = POISSON'S RATIO OF SOIL

ν_l = POISSON'S RATIO OF LINER

E_m = MODULUS OF ELASTICITY OF SOIL

E_l = MODULUS OF ELASTICITY OF LINER

R_o = OUTSIDE RADIUS OF LINER

G_l = SHEAR MODULUS OF LINER

G_m = SHEAR MODULUS OF SOIL

$G = \frac{E}{2(1+\nu)}$

R_i = INSIDE RADIUS OF LINER

CONSTANTS FOR ALL CASES

$r = 70"$

$R_i = 60"$

$R_o = 70"$

$E_l = 3605$ Ksi

$\nu_l = 0.17$

$G_l = \frac{3605}{2(1+.17)} = 1541$ Ksi

$t = 10"$

-'d by GLC

COMPUTATIONS FOR TUNNEL SECTION (1)

CONSTANTS: $\nu_m = 0.33$

$K_o = 0.50$

$E_m = 2.25$ ksi

$G_m = \frac{2.25}{2(1+0.33)} = 0.85$ ksi

$\gamma = \frac{14(106) + 14(43.5) + 16(47.5)}{44} = 72.1$ pcf

$H = 818.0 - 774.0 = 44.0'$

EXCAVATION LOADING - NO SLIPPAGE, THIN LINER

$$F = \left(\frac{2.25}{3605}\right)\left(\frac{70}{10}\right)^3\left[\frac{2(1-0.17^2)}{1+0.33}\right] = 0.3126$$

$$C = \left(\frac{2.25}{3605}\right)\left(\frac{70}{10}\right)\left[\frac{(1-0.17^2)}{(1+0.33)(1-2(0.33))}\right] = 0.0094$$

$$N_\eta^* = \frac{[3+2(1-2(0.33))0.0094]0.3126 + \frac{1}{2}(1-2(0.33))0.0094}{[(3-2(0.33))+(1-2(0.33))0.0094]0.3126 + \frac{1}{2}(5-6(0.33))(1-2(0.33))0.0094 + (6-8(0.33))}$$

$$N_\eta^* = \frac{0.9414}{4.0973} = 0.2298$$

$$J_\eta^* = \frac{[2(0.33)+(1-2(0.33))0.0094]0.3126 + (1-0.33)(1-2(0.33))0.0094}{[(3-2(0.33))+(1-2(0.33))0.0094]0.3126 + \frac{1}{2}(5-6(0.33))(1-2(0.33))0.0094 + (6-8(0.33))}$$

$$J_\eta^* = \frac{0.2095}{4.0973} = 0.0511$$

$$L_n^* = \frac{(1-2(0.33))0.0094}{1+(1-2(0.33))0.0094+\frac{(1-2(0.33))0.0094}{6(0.3126)}} = \frac{0.0032}{1.0049} = 0.0032$$

$$P_r = \left(\frac{72.1(44)}{2}\right)\left[(1+0.5)(1-0.0032)-(1-0.5)\left[1+3(0.0511)-2(0.2298)\right]\cos 2\theta\right]$$

$$P_r = 1586\left[1.4952-0.3469\cos 2\theta\right]$$

$$T_{r\theta} = \left(\frac{72.1(44)}{2}\right)\left[(1-0.5)(1-3(0.0511)+0.2298)\sin 2\theta\right]$$

$$T_{r\theta} = 853.7 \sin 2\theta$$

EXCAVATION LOADING - FULL SLIPPAGE, THIN LINER

$$J_n^* = \frac{0.3126+(1-0.33)}{2(0.3126)+(5-6(0.33))} = 0.2696$$

$$P_r = \left(\frac{72.1(44)}{2}\right)\left[(1+0.5)(1-0.0032)-3(1-0.5)(1-2(.2696))\cos 2\theta\right]$$

$$P_r = 1586\left[1.4952-0.6912\cos 2\theta\right]$$

EXCAVATION LOADING - NO SLIPPAGE, THICK LINER

$$b_{13}^* = \left[\left(\frac{70}{60}\right)^6+(1-4(0.17))\left(\frac{70}{60}\right)^4+(7-4(0.17))\left(\frac{70}{60}\right)^2+(3-4(0.17))\right]\left[\left(\frac{70}{60}\right)^2-1\right]$$

$$b_{13}^* = 5.069$$

$$b_{12}^* = (1-0.17)\left[\left(\frac{70}{60}\right)^6-\left(\frac{70}{60}\right)^4\right]$$

$$b_{12}^* = 0.5563$$

$$b_{11}^* = \left[\left(\tfrac{70}{60}\right)^6 - (11-8(0.17))\left(\tfrac{70}{60}\right)^4 + (7-4(0.17))\left(\tfrac{70}{60}\right)^2 + (3-4(0.17))\right]\left[\left(\tfrac{70}{60}\right)^2 - 1\right]$$

$$b_{11}^* = -1.595$$

$$b_6^* = \left[(3-4(0.17))\left(\tfrac{70}{60}\right)^8 + 4(3-6(0.17)+4(0.17)^2)\left(\tfrac{70}{60}\right)^6 - 6\left(\tfrac{70}{60}\right)^4 + 4\left(\tfrac{70}{60}\right)^2 + (3-4(0.17))\right]$$

$$b_6^* = 25.75$$

$$b_5^* = \left[(3-4(0.17))\left(\tfrac{70}{60}\right)^6 + 3\left(\tfrac{70}{60}\right)^4 - 3\left(\tfrac{70}{60}\right)^2 + 1\right]\left[\left(\tfrac{70}{60}\right)^2 - 1\right]$$

$$b_5^* = 3.006$$

$$b_4^* = \left[(5-6(0.17))\left(\tfrac{70}{60}\right)^6 + (5-2(0.17))\left(\tfrac{70}{60}\right)^4 - (1+2(0.17))\left(\tfrac{70}{60}\right)^2 + (3-2(0.17))\right]\left[\left(\tfrac{70}{60}\right)^2 - 1\right]$$

$$b_4^* = 7.044$$

$$b_3^* = \left[\left(\tfrac{70}{60}\right)^2 - 1\right]^4 = 0.0170$$

$$b_2^* = \left[(1-2(0.17))\left(\tfrac{70}{60}\right)^2 + 1\right] = 1.898$$

$$b_1^* = \left[\left(\tfrac{70}{60}\right)^2 - 1\right] = 0.3611$$

$$N_n^* = \frac{\left(\frac{1541}{0.85}\right)(5.069) + 25.75}{(3-4(0.33))\left(\frac{1541}{0.85}\right)^2 0.017 + 2\left(\frac{1541}{0.85}\right)\left[7.044 - 2(0.33)(3.006)\right] + 25.75}$$

$$N_n^* = 0.0821$$

$$J_n^* = \frac{\left(\frac{1541}{0.85}\right)\left[-1.595 + 16(0.33)0.5553\right] + 25.75}{(3-4(0.33))\left(\frac{1541}{0.85}\right)^2 0.017 + 2\left(\frac{1541}{0.85}\right)\left[7.044 - 2(0.33)(3.006)\right] + 25.75}$$

$$J_n^* = 0.0218$$

$$L_n^* = \frac{1.898}{\left(\frac{1541}{0.85}\right)0.3611 + 1.898} = 0.00289$$

$$P_r = \frac{72.1(44)}{2}\left[(1+0.5)\left[1-0.00289\left(\frac{70}{70}\right)^2\right]-(1-0.5)\left[1+3(0.0218)\left(\frac{70}{70}\right)^4-4(0.0821)\left(\frac{70}{70}\right)^2\right]\cos 2\theta\right]$$

$$P_r = 1586\left[1.4957 - 0.3685\cos 2\theta\right]$$

$$T_{r\theta} = \frac{72.1(44)}{2}\left[(1-0.5)\left[1-3(.0218)\left(\frac{70}{70}\right)^2+2(0821)\left(\frac{70}{70}\right)^2\right]\sin 2\theta\right]$$

$$T_{r\theta} = 871.5\sin 2\theta$$

EXCAVATION LOADING - FULL SLIPPAGE, THICK LINER

$$d_4^* = \left[(3-2(0.17))\left(\frac{70}{60}\right)^6+3(5-6(0.17))\left(\frac{70}{60}\right)^4+3(3-2(0.17))\left(\frac{70}{60}\right)^2+(5-6(0.17))\right]$$

$$d_4 = 43.67$$

$$d_3^* = \left[\left(\frac{70}{60}\right)^2-1\right]^3 = 0.0471$$

$$N_n^* = \frac{\left(\frac{1541}{0.85}\right)0.0471+2(43.67)}{(5-6(0.33))\left(\frac{1541}{0.85}\right)0.0471+43.67} = 0.5728$$

$$J_n^* = \frac{2(1-0.33)\left(\frac{1541}{0.85}\right)0.0471+43.67}{(5-6(0.33))\left(\frac{1541}{0.85}\right)0.0471+43.67} = 0.5243$$

$$P_r = \frac{72.1(44)}{2}\left[(1+0.5)\left[1-0.00289\left(\frac{70}{70}\right)^2\right]-(1-0.5)\left[1+3(0.5243)\left(\frac{70}{70}\right)^4-2(.5728)\left(\frac{70}{70}\right)^2\right]\cos 2\theta\right]$$

$$P_r = 1586\left[1.4957 - 0.7137\cos 2\theta\right]$$

SOIL PRESSURES - THIN WALL ASSUMPTION (psf)

	No Slippage		Full Slippage
$\theta°$	$P_r = 1586[1.4952 - 0.3469\cos 2\theta]$	$T_{r\theta} = 853.7 \sin 2\theta$	$P_r = 1586[1.4952 - 0.6912\cos 2\theta]$
90°	2922	0	3468
80°	2888	292	3402
70°	2793	549	3211
60°	2646	739	2920
50°	2467	841	2562
40°	2276	841	2181
30°	2096	739	1823
20°	1950	549	1532
10°	1854	292	1341
0°	1821	0	1275

SEE PAGE 10 FOR NODE CORRELATIONS, NOTES.

SOIL PRESSURES - THICK WALL ASSUMPTION (psf)

	NO SLIPPAGE		FULL SLIPPAGE
θ°	$P_r = 1586[1.4957 - 0.3685 \cos 2\theta]$	$T_{r\theta} = 871.5 \sin 2\theta$	$P_r = 1586[1.4957 - 0.7137 \cos 2\theta]$
90°	2957	0	3504
80°	2921	298	3436
70°	2820	560	3239
60°	2664	755	2938
50°	2474	858	2569
40°	2271	858	2176
30°	2080	755	1806
20°	1924	560	1505
10°	1823	298	1309
0°	1788	0	1240

NODES	
1, 19	90°
2, 18	80°
3, 17	70°
4, 16	60°
5, 15	50°
6, 14	40°
7, 13	30°
8, 12	20°
9, 11	10°
10	0°

FOR $T_{r\theta}$ (-)ANGLES PRODUCE (-)VALUES

USE THE THICK WALL EQUATIONS ONLY FOR LOAD DETERMINATION FOR THE OTHER 2 SECTIONS AS IT PRODUCED THE HIGHEST LOADING.

COMPUTATIONS FOR TUNNEL SECTION (2)

CONSTANTS

$\nu_m = 0.44$

$K_o = 0.80$

$E_m = 0.9$ KSI

$G_m = \frac{0.9}{2(1+0.44)} = 0.31$ KSI

$H = 822.0 - 772.62 = 49.38'$ say 50.0' TO ACCOUNT FOR GROUND IRREGULARITIES

$\gamma = \frac{10(106) + 10(130) + 5(67.5) + 25(62.5)}{50} = 85.2$ pcf

EXCAVATION LOADING, NO SLIPPAGE, THICK WALL

SEE SECTION (1) COMPUTATIONS FOR VALUES FOR: $b_{13}^*, b_{12}^*, b_{11}^*, b_6^*, b_5^*, b_4^*, b_3^*, b_2^*$, AND b_1^*

$$N_n^* = \frac{\left(\frac{1541}{0.31}\right)5.069 + 25.75}{(3-4(0.44))\left(\frac{1541}{0.31}\right)^2 0.017 + 2\left(\frac{1541}{0.31}\right)\left[7.044 - 2(0.44)(3.006)\right] + 25.75}$$

$$N_n^* = 0.0447$$

$$J_n^* = \frac{\left(\frac{1541}{0.31}\right)\left[-1.595 + 16(0.44)(0.5553)\right] + 25.75}{(3-4(0.44))\left(\frac{1541}{0.31}\right)^2 0.017 + 2\left(\frac{1541}{0.31}\right)\left[7.044 - 2(0.44)(3.006)\right] + 25.75}$$

$$J_n^* = 0.0204$$

$$L_n^* = \frac{1.898}{\left(\frac{1541}{0.31}\right)0.3611 + 1.898}$$

$$L_n^* = 0.00106$$

$$P_r = \frac{(85.2)(50)}{2}\left[(1+0.8)\left[1-(0.00106)\left(\frac{70}{70}\right)^2\right]-(1-0.8)\left[1+3(0.0204)\left(\frac{70}{70}\right)^4-4(0.0447)\left(\frac{70}{70}\right)^2\right]\cos 2\theta\right]$$

$$P_r = 2130\left[1.7981 - 0.1765\cos 2\theta\right]$$

$$T_r = \frac{85.2(50)}{2}\left[(1-0.8)\left[1-3(0.0204)\left(\frac{70}{70}\right)^4+2(0.0447)\left(\frac{70}{70}\right)^2\right]\sin 2\theta\right.$$

$$T_r = 438.0 \sin 2\theta$$

EXCAVATION LOADING, FULL SLIPPAGE, THICK WALL

$$d_4^* = \left[(3-2(0.17))\left(\frac{70}{60}\right)^6+3(5-6(0.17))\left(\frac{70}{60}\right)^4+3(3-2(0.17))\left(\frac{70}{60}\right)^2+(5-6(0.17))\right]$$

$$d_4^* = 43.67$$

$$d_3^* = \left[\left(\frac{70}{60}\right)^2-1\right]^3 = 0.0471$$

$$N_n^* = \frac{\left(\frac{1541}{0.31}\right)0.0471+2(43.67)}{(5-6(0.44))\left(\frac{1541}{0.31}\right)(0.0471)+43.67} = 0.5329$$

$$J_n^* = \frac{2(1-0.44)\left(\frac{1541}{0.31}\right)(0.0471)+43.67}{(5-6(0.44))\left(\frac{1541}{0.31}\right)(0.0471)+43.67} = 0.5131$$

$L_n^* = 0.00106$ (SEE PAGE 11 OF THESE COMPUTATIONS)

$$P_r = \frac{85.2(50)}{2}\left[(1+0.8)\left[1-.00106\left(\frac{70}{70}\right)^2\right]-(1-0.8)\left[1+3(0.5131)\left(\frac{70}{70}\right)^4-2(0.5329)\left(\frac{70}{70}\right)^2\right]\cos 2\theta\right]$$

$$P_r = 2130\left[1.7981 - 0.2947\cos 2\theta\right]$$

SOIL PRESSURES - THICK WALL ASSUMPTION (psf)

θ	NODE	NO SLIPPAGE $P_r = 2130[1.7981 - 0.1765 \cos 2\theta]$	NO SLIPPAGE $\tau_{r\theta} = 438.0 \sin 2\theta$ *	FULL SLIPPAGE $P_r = 2130[1.7981 - 0.2947 \cos 2\theta]$
90°	1,19	4206	0	4458
80°	2,18	4183	150	4420
70°	3,17	4118	282	4311
60°	4,16	4018	379	4144
50°	5,15	3895	431	3939
40°	6,14	3765	431	3721
30°	7,13	3642	379	3516
20°	8,12	3542	282	3349
10°	9,11	3477	150	3240
0°	10	3454	0	3202

* FOR $\tau_{r\theta}$ NEG. ANGLES PRODUCE NEG. VALUES

COMPUTATIONS FOR TUNNEL SECTION ③

CONSTANTS

$\nu_m = 0.44$

$K_o = 0.80$

$E_m = 0.9$ KSI

$G_m = 0.31$ KSI

$H = 848 - 770.09 = 77.91'$ use $78.0'$

$\gamma = \frac{33(130) + 45(62.5)}{78} = 91.1$ pcf

EXCAVATION LOADING, NO SLIPPAGE, THICK WALL

SEE SECTION ① COMPUTATIONS FOR VALUES OF: $b_{13}^*, b_{12}^*, b_{11}^*, b_6^*, b_5^*, b_4^*, b_3^*, b_2^*$, AND b_1^*.

SEE SECTION ② COMPUTATIONS FOR VALUES OF N_n^*, J_n^*, AND L_n^*.

$$P_r = \frac{91.1(78)}{2}\left[1.7981 - 0.1765\cos 2\theta\right]$$

$$P_r = 3553\left[1.7981 - 0.1765\cos 2\theta\right]$$

$$T_{r\theta} = \frac{91.1(78)}{2}\left[(1-0.8)\left[1 - 3(0.0204)\left(\frac{70}{70}\right)^4 + 2(0.0447)\left(\frac{70}{70}\right)^2\right]\right]\sin 2\theta$$

$$T_{r\theta} = 730.6 \sin 2\theta$$

EXCAVATION LOADING, FULL SLIPPAGE, THICK WALL

SEE SECTION ① COMPUTATIONS OF VALUES FOR: $b_{13}^*, b_{12}^*, b_{11}^*, b_6^*, b_5^*, b_4^*, b_3^*, b_2^*$, AND b_1^*.

SEE SECTION ② COMPUTATIONS OF VALUES FOR: N_n^*, J_n^*, AND L_n^*.

$$P_r = \frac{91.1(78)}{2}\left[1.7981 - 0.2947\cos 2\theta\right]$$

$$P_r = 3553\left[1.7981 - 0.2947\cos 2\theta\right]$$

SOIL PRESSURES - THICK WALL ASSUMPTION (psf)

θ	NODE	NO SLIPPAGE $P_r = 3553[1.7981 - 0.1765 \cos 2\theta]$	NO SLIPPAGE $T_{r\theta} = 730.6 \sin 2\theta$ *	FULL SLIPPAGE $P_r = 3553[1.7981 - 0.2967 \cos 2\theta]$
90°	1,19	7016	0	7436
80°	2,18	6979	250	7373
70°	3,17	6869	470	7191
60°	4,16	6702	633	6912
50°	5,15	6498	720	6570
40°	6,14	6280	720	6207
30°	7,13	6075	633	5865
20°	8,12	5908	470	5587
10°	9,11	5799	250	5405
0°	10	5762	0	5342

* FOR $T_{r\theta}$ NEG. ANGLES PRODUCE NEG. VALUES

NEWTUN INPUT, SECTION ①

2 LOADING CONDITIONS: 1- NO WATER IN TUNNEL, WATER OUTSIDE TUNNEL
2- MAXIMUM INTERNAL HEAD IN TUNNEL, WATER OUTSIDE TUNNEL

MAXIMUM INTERIOR HEAD $= \left[(818.9-804.1)\left(\frac{5940-2480}{6700-2480}\right)\right] + 804.1 = 816.1$

WATER TABLE ELEV. 804.0

USE FULL SLIPPAGE ASSUMPTION AS ASSUMED WORST LOADING CONDITION.

$F_i = \left[(h_i - R_i) - \cos\theta R_i\right] S_i \gamma_w$; $S_i = \frac{\pi 2R_i}{18} = \frac{\pi 2(5)}{18} = 1.745$

$F_E = \left[(h_e - R_e) - \sin\theta R_e\right] S_e \gamma_w$; $S_e = \frac{\pi 2R_e}{18} = \frac{\pi 2(5.833)}{18} = 2.036$

$F_R = S_e (P_r)$

$F_n = F_R + F_E - F_i$; $F_x = \cos\theta F_n$, $F_y = \sin\theta F_n$

LINING WEIGHT PER 10° SEGMENT OF TUNNEL

$$\frac{\pi (R_e^2 - R_i^2) 150}{36} = \frac{\pi \left(\frac{70^2 - 60^2}{12^2}\right) 150}{36} = 118.16.$$

LOAD CASE 1 - SOIL + LINER + EXTERIOR WATER, FULL SLIPPAGE (SEE PAGE 10 THESE COMPUTATIONS FOR P_r)

$F_E = \left[((807.0-774.0) - 5833\sin\theta\right] 2.036\,(62.5) = 4199 - 742.2\sin\theta$

$F_R = 2.036\,(P_r)$

NODE 19, $\theta = 90°$ (IN POUNDS)

$F_n = 2.036(3504) + (4199 - 742.2\sin 90°) = 10{,}591$

$F_x = F_n \cos\theta = F_n \cos 90° = 0$

$F_y = F_n \sin\theta + 118 = 10591\,(\sin 90°) + 118 = 10709$ ↓

NODE 18, $\theta = 80°$

$F_n = 2.036(3436) + (4199 - 742.2 \sin 80°) = 10,464$

$F_x = 10,464 \cos 80° = 1817 \leftarrow$

$F_y = 10,464 \sin 80° + 118 = 10423 \downarrow$

NODE 17, $\theta = 70°$

$F_n = 2.036(3239) + (4199 - 742.2 \sin 70°) = 10,096$

$F_x = 10096 \cos 70° = 3453 \leftarrow$

$F_y = 10096 \sin 70° + 118 = 9605 \downarrow$

NODE 16, $\theta = 60°$

$F_n = 2.036(2938) + (4199 - 742.2 \sin 60°) = 9,538$

$F_x = 9538 \cos 60° = 4769 \leftarrow$

$F_y = 9538 \sin 60° + 118 = 8378 \downarrow$

NODE 15, $\theta = 50°$

$F_n = 2.036(2669) + (4199 - 742.2 \sin 50°) = 8861$

$F_x = 8861 \cos 50° = 5696 \leftarrow$

$F_y = 8861 \sin 50° + 118 = 6906 \downarrow$

NODE 14, $\theta = 40°$

$F_n = 2.036(2176) + (4199 - 742.2 \sin 40°) = 8152$

$F_x = 8152 \cos 40° = 6245 \leftarrow$

$F_y = 8152 \sin 40° + 118 = 5358 \downarrow$

NODE 13, $\theta = 30°$

$F_n = 2.036(1806) + (4199 - 742.2 \sin 30°) = 7505$

$F_x = 7505 \cos 30° = 6499 \leftarrow$

$F_y = 7505 \sin 30° + 118 = 3870 \downarrow$

NODE 12, $\theta = 20°$

$F_\eta = 2.036(1505) + (4199 - 742.2 \sin 20°) = 7009$

$F_x = 7009 \cos 20° = 6587 \leftarrow$

$F_y = 7009 \sin 20° + 118 = 2515 \downarrow$

NODE 11, $\theta = 10°$

$F_\eta = 2.036(1309) + (4199 - 742.2 \sin 10°) = 6735$

$F_x = 6735 \cos 10° = 6663 \leftarrow$

$F_y = 6735 \sin 10° + 118 = 1288 \downarrow$

NODE 10, $\theta = 0°$

$F_\eta = 2.036(1240) + (4199 - 742.2 \sin 0°) = 6724$

$F_x = 6724 \cos 0° = 6724 \leftarrow$

$F_y = 6724 \sin 0° + 118 = 118 \downarrow$

NODE 9, $\theta = -10°$

$F_\eta = 2.036(1309) + (4199 - 742.2 \sin -10°) = 6993$

$F_x = 6993 \cos -10° = 6887 \leftarrow$

$F_y = 6993 \sin -10° + 118 = 1096 \uparrow$

NODE 8, $\theta = -20°$

$F_\eta = 2.036(1505) + (4199 - 742.2 \sin -20°) = 7517$

$F_x = 7517 \cos -20° = 7064 \leftarrow$

$F_y = 7517 \sin -20° + 118 = 2453 \uparrow$

NODE 7, $\theta = -30°$

$F_\eta = 2.036(1806) + (4199 - 742.2 \sin -30°) = 8247$

$F_x = 8247 \cos -30° = 7142 \leftarrow$

$F_y = 8247 \sin -30° + 118 = 4006 \uparrow$

NODE 6, $\theta = -40°$

$F_\eta = 2.036(2176) + (4199 - 742.2 \sin -40°) = 9106$

$F_x = 9106 \cos -40° = 6976 \leftarrow$

$F_y = 9106 \sin -40° + 118 = 5735 \uparrow$

NODE 5, $\theta = -50°$

$F_\eta = 2.036(2569) + (4199 - 742.2 \sin -50°) = 9998$

$F_x = 9998 \cos -50° = 6427 \leftarrow$

$F_y = 9998 \sin -50° + 118 = 7541 \uparrow$

NODE 4, $\theta = -60°$

$F_\eta = 2.036(2938) + (4199 - 742.2 \sin -60°) = 10824$

$F_x = 10824 \cos -60° = 5412 \leftarrow$

$F_y = 10824 \sin -60° + 118 = 9255 \uparrow$

NODE 3, $\theta = -70°$

$F_\eta = 2.036(3239) + (4199 - 742.2 \sin -70°) = 11491$

$F_x = 11491 \cos -70° = 3930 \leftarrow$

$F_y = 11491 \sin -70° + 118 = 10680 \uparrow$

NODE 2, $\theta = -80°$

$F_\eta = 2.036(3436) + (4199 - 742.2 \sin -80°) = 11926$

$F_x = 11926 \cos -80° = 2071 \leftarrow$

$F_y = 11926 \sin -80° + 118 = 11626 \uparrow$

NODE 1, $\theta = -90°$

$F_\eta = 2.036(3504) + (4199 - 742.2 \sin -90°) = 12075$

$F_x = 12075 \cos -90° = 0$

$F_y = 12075 \sin -90° + 118 = 11957 \uparrow$

LOAD CASE 2 - SOIL + LINER + EXTERIOR WATER + INTERIOR WATER, FULL SLIPPAGE

$F_\lambda = [(816.1 - 774.0) - 5 \sin\theta]\, 62.5(1.745) = 4592 - 545.3 \sin\theta$

$F_n = P_r\, 2.036 + (4199 - 742.2 \sin\theta) - (4592 - 545.3 \sin\theta)$

$F_n = 2.036 P_r - 393 - 196.9 \sin\theta$

NODE 19, $\theta = 90°$

$F_n = 2.036(3504) - 393 - 196.9 \sin 90° = 6544$

$F_x = F_n \cos\theta = 6544 \cos 90° = 0$

$F_y = F_n \sin 90° + 118 = 6544 \sin 90° + 118 = 6662 \downarrow$

NODE 18, $\theta = 80°$

$F_n = 2.036(3436) - 393 - 196.9 \sin 80° = 6409$

$F_x = 6409 \cos 80° = 1113 \leftarrow$

$F_y = 6409 \sin 80° + 118 = 6429 \downarrow$

NODE 17, $\theta = 70°$

$F_n = 2.036(3239) - 393 - 196.9 \sin 70° = 6017$

$F_x = 6017 \cos 70° = 2058 \leftarrow$

$F_y = 6017 \sin 70° + 118 = 5772 \downarrow$

NODE 16, $\theta = 60°$

$F_n = 2.036(2938) - 393 - 196.9 \sin 60° = 5418$

$F_x = 5418 \cos 60° = 2709 \leftarrow$

$F_y = 5418 \sin 60° + 118 = 4810 \downarrow$

NODE 15, $\theta = 50°$

$F_n = 2.036(2569) - 393 - 196.9 \sin 50° = 4687$

$F_x = 4687 \cos 50° = 3013 \leftarrow$

$F_y = 4687 \sin 50° + 118 = 3708 \downarrow$

NODE 14, $\theta = 40^\circ$

$F_n = 2.036(2176) - 393 - 196.9 \sin 40^\circ = 3911$

$F_x = 3911 \cos 40^\circ = 2996 \leftarrow$

$F_y = 3911 \sin 40^\circ + 118 = 2632 \downarrow$

NODE 13, $\theta = 30^\circ$

$F_n = 2.036(1806) - 393 - 196.9 \sin 30^\circ = 3186$

$F_x = 3186 \cos 30^\circ = 2759 \leftarrow$

$F_y = 3186 \sin 30^\circ + 118 = 1711 \downarrow$

NODE 12, $\theta = 20^\circ$

$F_n = 2.036(1505) - 393 - 196.9 \sin 20^\circ = 2604$

$F_x = 2604 \cos 20^\circ = 2447 \leftarrow$

$F_y = 2604 \sin 20^\circ + 118 = 1009 \downarrow$

NODE 11, $\theta = 10^\circ$

$F_n = 2.036(1309) - 393 - 196.9 \sin 10^\circ = 2238$

$F_x = 2238 \cos 10^\circ = 2204 \leftarrow$

$F_y = 2238 \sin 10^\circ + 118 = 507 \downarrow$

NODE 10, $\theta = 0^\circ$

$F_n = 2.036(1240) - 393 - 196.9 \sin 0^\circ = 2132$

$F_x = 2132 \cos 0^\circ = 2132 \leftarrow$

$F_y = 118 \downarrow$

NODE 9, $\theta = -10^\circ$

$F_n = 2.036(1309) - 393 - 196.9 \sin -10^\circ = 2306$

$F_x = 2306 \cos 10^\circ = 2272 \leftarrow$

$F_y = 2306 \sin 10^\circ + 118 = 282 \uparrow$

NODE 8, $\theta = -20^\circ$

$F_n = 2.036(1505) - 393 - 196.9 \sin -20^\circ = 2739$

$F_x = 2739 \cos -20^\circ = 2573 \leftarrow$

$F_y = 2739 \sin -20^\circ + 118 = 819 \uparrow$

NODE 7, $\theta = -30°$

$F_n = 2.036(1806) - 393 - 196.9 \sin -30° = 3382$

$F_x = 3382 \cos -30° = 2929 \leftarrow$

$F_y = 3382 \sin -30° + 118 = 1573 \uparrow$

NODE 6, $\theta = -40°$

$F_n = 2.036(2176) - 393 - 196.9 \sin -40° = 4164$

$F_x = 4164 \cos -40° = 3190 \leftarrow$

$F_y = 4164 \sin -40° + 118 = 2559 \uparrow$

NODE 5, $\theta = -50°$

$F_n = 2.036(2569) - 393 - 196.9 \sin -50° = 4988$

$F_x = 4988 \cos -50° = 3206 \leftarrow$

$F_y = 4988 \sin -50° + 118 = 3703 \uparrow$

NODE 4, $\theta = -60°$

$F_n = 2.036(2938) - 393 - 196.9 \sin -60° = 5759$

$F_x = 5759 \cos -60° = 2880 \leftarrow$

$F_y = 5759 \sin -60° + 118 = 4870 \uparrow$

NODE 3, $\theta = -70°$

$F_n = 2.036(3239) - 393 - 196.9 \sin -70° = 6387$

$F_x = 6387 \cos -70° = 2184 \leftarrow$

$F_y = 6387 \sin -70° + 118 = 5883 \uparrow$

NODE 2, $\theta = -80°$

$F_n = 2.036(3436) - 393 - 196.9 \sin -80° = 6797$

$F_x = 6797 \cos -80° = 1180 \leftarrow$

$F_y = 6797 \sin -80° + 118 = 6575 \uparrow$

NODE 1, $\theta = -90°$

$F_n = 2.036(3504) - 393 - 196.9 \sin -90° = 6938$

$F_x = 6938 \cos -90° = 0$

$F_y = 6938 \sin -90° + 118 = 6820 \uparrow$

DATA FILE BTUN 1

```
100 BATTLE CREEK TUNNEL
101 10 INCH CONCRETE LINER, 10 FOOT DIAMETER
102 PERVIOUS SECTION
104 19,18,2
110 1,101,0.0,0,0,0
111 2,0,11.2,0.98,0,0,0
112 3,0,22.06,3.89,0,0,0
113 4,0,32.25,8.64,0,0,0
114 5,0,41.46,15.09,0,0,0
115 6,0,49.41,23.04,0,0,0
116 7,0,55.86,32.25,0,0,0
117 8,0,60.61,42.44,0,0,0
118 9,0,63.52,53.3,0,0,0
119 10,0,64.5,64.5,0,0,0
120 11,0,63.52,75.7,0,0,0
121 12,0,60.61,86.56,0,0,0
122 13,0,55.86,96.75,0,0,0
123 14,0,49.41,105.96,0,0,0
124 15,0,41.46,113.91,0,0,0
125 16,0,32.25,120.36,0,0,0
126 17,0,22.06,125.11,0,0,0
127 18,0,11.2,128.02,0,0,0
128 19,0,0,129.,0
130 1 1 2 120. 3605000. 2250. 1000. 0 0 0
131 18 18 19 120. 3605000. 2250. 1000. 0 0 0
200 LC 1 SOIL,EXWATER,LINER
201 19,0
202 1,0,11957,0
203 2,-2071,11626,0
204 3,-3930,10680,0
205 4,-5412,9255,0
206 5,-6427,7541,0
207 6,-6976,5735,0
208 7,-7142,4006,0
209 8,-6724,2453,0
210 9,-6887,1096,0
211 10,-6724,-118,0
212 11,-6633,-1288,0
213 12,-6587,-2515,0
214 13,-6499,-3870,0
215 14,-6245,-5358,0
216 15,-5696,-6906,0
217 16,-4769,-8378,0
218 17,-3453,-9605,0
219 18,-1817,-10423,0
220 19,0,-10709,0
221 0,0,0,0,0
300 LC 2 SOIL,IHEXWATER,LINER
301 19,0
302 1,0,6820,0
303 2,-1180,6575,0
304 3,-2184,5883,0
305 4,-2880,4870,0
306 5,-3206,3703,0
307 6,-3190,2559,0
308 7,-2929,1573,0
309 8,-2573,819,0
310 9,-2272,282,0
311 10,-2138,-118,0
312 11,-2204,-507,0
313 12,-2447,-1009,0
314 13,-2759,-1711,0
315 14,-2998,-2632,0
316 15,-3013,-3708,0
317 16,-2709,-4810,0
318 17,-2058,-5772,0
319 18,-1113,-6429,0
320 19,0,-6662,0
321 0,0,0,0,0
400 $END
```

** NEWTUN ** ANALYSIS OF CAST IN PLACE TUNNELS

BASSETT CREEK TUNNEL
10 INCH CONCRETE LINER
PERVIOUS SECTION

NUMBER OF NODES = 19
NUMBER OF ELEMENTS = 18
NUMBER OF LOADINGS = 2

(RESTRAINT CODE =1 SPECIFY RESTRAINT, =0 SPECIFY LOAD)

NODE	RESTRAINT CODE	X-COOR	Y-COOR	SPECIFIED DISPLACEMENTS X	Y	ROT
1	101	0.00	0.00	0.00	0.00	0.00
2	0	11.20	.98	0.00	0.00	0.00
3	0	22.06	3.89	0.00	0.00	0.00
4	0	32.25	8.64	0.00	0.00	0.00
5	0	41.46	15.09	0.00	0.00	0.00
6	0	49.41	23.04	0.00	0.00	0.00
7	0	55.86	32.25	0.00	0.00	0.00
8	0	60.61	42.44	0.00	0.00	0.00
9	0	63.52	53.30	0.00	0.00	0.00
10	0	64.50	64.50	0.00	0.00	0.00
11	0	63.52	75.70	0.00	0.00	0.00
12	0	60.61	86.56	0.00	0.00	0.00
13	0	55.86	96.75	0.00	0.00	0.00
14	0	49.41	105.96	0.00	0.00	0.00
15	0	41.46	113.91	0.00	0.00	0.00
16	0	32.25	120.36	0.00	0.00	0.00
17	0	22.06	125.11	0.00	0.00	0.00
18	0	11.20	128.02	0.00	0.00	0.00
19	101	0.00	129.00	0.00	0.00	0.00

LC 1 OIL+EXWATER+LINER

NUMBER OF NODAL POINT LOAD CARDS = 19

SPECIFIED NODAL POINT LOADS

NODE	FX	FY	MOM
1	0.000	11957.000	0.000
2	-2071.000	11626.000	0.000
3	-3930.000	10680.000	0.000
4	-5412.000	9255.000	0.000
5	-6427.000	7541.000	0.000
6	-6976.000	5735.000	0.000
7	-7142.000	4006.000	0.000
8	-6724.000	2453.000	0.000
9	-6887.000	1096.000	0.000
10	-6724.000	-118.000	0.000
11	-6633.000	-1288.000	0.000
12	-6587.000	-2515.000	0.000
13	-6499.000	-3870.000	0.000
14	-6245.000	-5358.000	0.000
15	-5696.000	-6906.000	0.000
16	-4769.000	-8378.000	0.000
17	-3453.000	-9605.000	0.000
18	-1817.000	-10423.000	0.000
19	0.000	-10709.000	0.000

NODAL POINT DISPLACEMENTS

LC 1

NODE	X-DISPL	Y-DISPL	ROT-DISPL
1	0.	.24049E+00	0.
2	-.85113E-03	.23355E+00	-.11745E-02
3	.25021E-02	.21529E+00	-.20655E-02
4	.12377E-01	.19048E+00	-.26164E-02
5	.28753E-01	.16434E+00	-.28003E-02
6	.49406E-01	.14140E+00	-.26266E-02
7	.70634E-01	.12450E+00	-.21444E-02
8	.88330E-01	.11439E+00	-.14395E-02
9	.99118E-01	.10974E+00	-.61735E-03
10	.10115E+00	.10786E+00	.22465E-03
11	.94312E-01	.10557E+00	.10065E-02
12	.80115E-01	.10003E+00	.16615E-02
13	.61284E-01	.89444E-01	.21369E-02
14	.41163E-01	.73385E-01	.23942E-02
15	.24019E-01	.53009E-01	.24090E-02
16	.93756E-02	.30837E-01	.21708E-02
17	.14972E-02	.10864E-01	.16808E-02
18	-.96040E-03	-.45161E-02	.95017E-03
19	0.	-.10156E-01	0.

END ACTIONS WITH NODAL POINT LOADS

LC 1

ELEMENT	AXIAL I AXIAL J	SHEAR I SHEAR J	MOMENT I MOMENT J
1	55891.234	7112.203	416597.478
	-55891.234	-7112.203	-336636.455
2	57285.826	9064.889	336636.455
	-57285.826	-9064.889	-234718.838
3	58940.013	10328.167	234718.838
	-58940.013	-10328.167	-118602.219
4	60714.080	10608.986	118602.219
	-60714.080	-10608.986	684.779
5	62421.653	9786.678	-684.779
	-62421.653	-9786.678	110716.376
6	63868.715	7807.521	-110716.376
	-63868.715	-7807.521	198503.813
7	64866.869	4750.225	-198503.813
	-64866.869	-5312.652	254588.713
8	65433.851	1106.849	-254588.713
	-65433.851	-2682.716	275616.179
9	65397.167	-1763.858	-275616.179
	-65397.167	-539.553	268682.135
10	64965.646	-4117.501	-268682.135
	-64965.646	1400.868	237827.776
11	64208.153	-5920.086	-237827.776
	-64208.153	3100.428	187478.086
12	63199.781	-7161.300	-187478.086
	-63199.781	4516.559	122344.116
13	62010.188	-7892.275	-122344.116
	-62010.188	5643.180	46858.182
14	60716.685	-8116.078	-46858.182
	-60716.685	6411.267	-34160.606
15	59386.357	-7928.342	34160.606
	-59386.357	6830.170	-116515.476
16	58077.228	-7442.610	116515.476
	-58077.228	6923.804	-196754.427
17	56844.307	-6724.290	196754.427
	-56844.307	6645.730	-271653.201
18	55728.322	-5873.689	271653.201
	-55728.322	5873.689	-337689.873

LC 2 SOIL+THERMWATER+LINER

NUMBER OF NODAL POINT LOAD CARDS = 19

SPECIFIED NODAL POINT LOADS

NODE	FX	FY	MOM
1	0.000	6820.000	0.000
2	-1180.000	6575.000	0.000
3	-2184.000	5883.000	0.000
4	-2880.000	4870.000	0.000
5	-3206.000	3703.000	0.000
6	-3190.000	2559.000	0.000
7	-2929.000	1573.000	0.000
8	-2573.000	819.000	0.000
9	-2272.000	282.000	0.000
10	-2132.000	-118.000	0.000
11	-2204.000	-507.000	0.000
12	-2447.000	-1009.000	0.000
13	-2759.000	-1711.000	0.000
14	-2992.000	-2632.000	0.000
15	-3013.000	-3708.000	0.000
16	-2709.000	-4810.000	0.000
17	-2058.000	-5772.000	0.000
18	-1113.000	-6429.000	0.000
19	0.000	-6662.000	0.000

NODAL POINT DISPLACEMENTS

LC 2

NODE	X-DISPL	Y-DISPL	POT-DISPL
1	0.	.92536E-01	0.
2	-.29083E-03	.87529E-01	-.85717E-03
3	.25100E-02	.74180E-01	-.15296E-02
4	.10208E-01	.55821E-01	-.19665E-02
5	.22937E-01	.36231E-01	-.21420E-02
6	.39213E-01	.18763E-01	-.20618E-02
7	.56468E-01	.56179E-02	-.17598E-02
8	.71720E-01	-.24716E-02	-.12819E-02
9	.82231E-01	-.62215E-02	-.68091E-03
10	.86081E-01	-.74686E-02	-.14836E-04
11	.82549E-01	-.86853E-02	.65501E-03
12	.72265E-01	-.12366E-01	.12664E-02
13	.57095E-01	-.20404E-01	.17587E-02
14	.39772E-01	-.33574E-01	.20748E-02
15	.23337E-01	-.51167E-01	.21633E-02
16	.10442E-01	-.70944E-01	.19883E-02
17	.26208E-02	-.89489E-01	.15462E-02
18	-.25075E-03	-.10297E+00	.86558E-03
19	0.	-.10802E+00	0.

LC 2

END ACTIONS WITH NODAL POINT LOADS

ELEMENT	AXIAL I AXIAL J	SHEAR I SHEAR J	MOMENT I MOMENT J
1	27942.979	4401.047	299592.039
	-27942.979	-4401.047	-250111.973
2	28844.682	6138.447	250111.973
	-28844.682	-6138.447	-181096.685
3	29978.293	7295.401	181096.685
	-29978.293	-7295.401	-99076.586
4	31224.718	7613.428	99076.586
	-31224.718	-7613.428	-13471.516
5	32423.670	6963.592	13471.516
	-32423.670	-7075.853	65077.475
6	33425.862	5421.572	-65077.475
	-33425.862	-6243.198	130141.874
7	34190.462	3656.050	-130141.874
	-34190.462	-5120.077	179066.336
8	34685.386	1807.361	-179066.336
	-34685.386	-3730.956	209936.229
9	34889.206	-60.669	-209936.229
	-34889.206	-2090.946	221255.592
10	34790.449	-1865.877	-221255.592
	-34790.449	-256.930	212297.090
11	34387.275	-3527.976	-212297.090
	-34387.275	1690.958	183216.961
12	33691.626	-4987.643	-183216.961
	-33691.626	3668.136	134966.756
13	32721.966	-6228.458	-134966.756
	-32721.966	5608.978	68933.307
14	31506.304	-7226.628	-68933.307
	-31506.304	7216.277	-12206.519
15	30116.462	-7809.668	12206.519
	-30116.462	7809.668	-100018.104
16	28723.158	-7422.600	100018.104
	-28723.158	7422.600	-183468.273
17	27493.538	-6185.770	183468.273
	-27493.538	6185.770	-253015.613
18	26550.124	-4364.318	253015.613
	-26550.124	4364.318	-302082.743

◆◆◆ END ◆◆◆

CHECK ASSUMED 10" CONCRETE THICKNESS FOR STRUCTURAL ADEQUACY.

MAX. SHEAR LC 1 AT NODE 4 — 10,609 lb., A = 60,714

MAX MOMENT LC 1 AT NODE 1 — M = 416,597 in-lb
A = 55,891 lb.

LC 2 AT NODE 19 — M = 302,083 in-lb
A = 26,550 lb.

USE WORKING STRESS METHOD REF. EM 1110-2-2902 AND REINFORCED CONCRETE DESIGN HANDBOOK WORKING STRESS METHOD ACI SP-3.

CHECK SHEAR AND DIAGONAL TENSION WITH 10" THICKNESS

$$f_t = \frac{f_c}{2} - \sqrt{\left(\frac{f_c'}{2}\right)^2 + v^2} \leq 2\sqrt{f_c'}$$

$$f_c = \frac{60714}{10(12)} = 506.0 \text{ psi}$$

$$v = \frac{10609}{8(12)} = 110.5 \text{ psi}$$

$$f_t = \frac{506.0}{2} - \sqrt{\left(\frac{506.0}{2}\right)^2 + (110.5)^2} = -23.1 \text{ psi} \leq 2\sqrt{f_c'} \text{ O.K.}$$

CHECK THICKNESS REQUIRED FOR REINFORCING.

M = 416,597 in-lb WITH AXIAL OF 55,891 lb.

$f_c = 0.45(4000) = 1800$ psi

$f_s = 20{,}000$ psi

$$n = \frac{29000000}{57000\sqrt{4000}} = 8.04$$

d = 8", d" = 3", b = 12"

$$e = \frac{M}{N} + d'' = \frac{416597}{55891} + 3 = 10.45''$$

DETERMINE DEPTH OF CONCRETE REQUIRED WITHOUT COMPRESSION REINFORCEMENT.

$$E = \frac{10.45}{12} = 0.8711$$

$$NE = 55.6(0.8711) = 48.4$$

$K = 324$

$KF \geq NE$

$F \geq NE/K = \frac{48.4}{324} = 0.1494$

from TABLE 4, $d_{REQ} = 12\frac{1}{2}"$ $F = 0.156$

DETERMINE DEPTH OF CONCRETE REQUIRED WITH COMPRESSION REINFORCEMENT, $h = 12"$, $d = 9.5"$

$d'/d = 2.5/9.5 = 0.2632$

$$c = \frac{20000\,(2(8.04)-1)\,(1-0.2632)(0.419-0.2632)}{12000\,(8.04)\,(1-0.419)} = 0.618$$

$$A_s' = \frac{NE-KF}{cd} = \frac{48.4-324(.090)}{0.618(9.5)} = 3.28$$ ← from TABLE 4, $d = 9.5"$ TOO MUCH STEEL

TRY $h = 14"$, $d = 11.5"$

$d'/d = 2.5/11.5 = 0.2174$

$$c = \frac{20000}{12000}\left[+-\frac{1800}{20000}\left(\frac{0.42(11.5)-2.5}{0.42(11.5)}\right)\right](1-0.2174) = 1.25$$

$$A_s' = \frac{48.4-324\,(0.132)}{1.25(11.5)} = 0.39\ in^2/FT$$

$$A_s = \frac{NE}{cdi} = \frac{48.4}{1.44(11.5)(5.10)}$$

$A_s = 0.57\ in^2$

$j = 0.860$ $\quad e/d = \frac{12.27}{11.5} = 1.07$

$i = 1/1-t/e = 1/1-0.86/1.07 = 5.10$

O.K. USE A THICKNESS OF 14"

$0.38 in^2/FT = A_s' = A_s$ USING ULTIMATE STRENGTH

D100

DEPARTMENT OF THE ARMY
St. Paul District Corps of Engineers
1135 U.S. Post Office & Custom House
St. Paul, Minnesota 55101

FLOOD CONTROL

BASSETT CREEK

HENNEPIN COUNTY, MINNESOTA

DESIGN MEMORANDUM NO. 2, PHASE II

APPENDIX E

ECONOMICS

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APPENDIX E

ECONOMICS

INTRODUCTION

1. This appendix presents an economic analysis of existing flood damages and benefits of the selected flood control plan in the Bassett Creek watershed, for an assumed economic life of 100 years. This analysis includes total and annual project costs and tangible flood control benefits, advance replacement benefits and redevelopment benefits attributable to the proposed project. Redevelopment benefits were not included in the final benefit-cost analysis, but were developed to show the potential redevelopment benefits resulting from the project.

FLOOD DAMAGES

General

2. Flood losses in the Bassett Creek watershed include physical damage caused by inundation, business losses due to decreased production, loss of profits and wages and increased cost of normal operation and living. Additional losses are incurred in flood fighting and flood preparation, but due to the short time of peak and short duration of the flood waters, these losses are small. Damages were obtained for the following categories:

3. Residential - Residential damages include inundation losses to all residences and upper tenant structures, including building contents and damage to grounds.

4. Commercial and Industrial - Damages include physical losses to properties and facilities used for retail and wholesale trade, services, manufacturing, and warehousing. Wage and profit losses where identified, and increased cost of normal operations resulting from evacuation and cleanup are also included.

5. Public - Damages include all costs associated with physical flood losses to

public property such as municipal, sewage, water and park systems. Other losses to the public include additional costs incurred during flood emergencies such as evacuation; flood fighting; disaster relief; and extra duty for police, fire and martial units.

6. Transportation - Transportation damages consist of the cost of additional mileage required because of inundated roadways and the resulting detours required.

DATA COLLECTION

7. Data was not available for major flood damages since major flooding has not recently occurred on Bassett Creek. Consequently, damages were estimated from Hennepin County property records, county traffic records, on-site visits, interviews and community records of cities within the Bassett Creek watershed.

8. Water surface elevation for low frequency floods used to calculate stage damage curves were obtained from water surface profiles and flood on lines developed using the Hydrologic Engineering Center's HEC-1 computer programs.

9. Commercial damages to commercial and industrial businesses were determined by interviews for those businesses in the 100 year floodplain. Damages to businesses in the SPF floodplain were estimated from elevation-damage curves developed by the U.S. Soil Conservation Service and the Baltimore District, U.S. Corps of Engineers, and updated by the St. Paul District in 1979. Damages were derived as a percentage of structure value. The percentages developed from this technique were compared to percentage of structural value for business that were interviewed. The percentages using the curves were lower than those derived from interviews. It was assumed that under estimating damages using this technique would not significantly affect the analysis for two reasons. First, the curves were used for structures in the SPF flood plain. The small change in annual damages would be minimal for low-frequency flooding. Second, benefits from changes in SPF flood plain damages are small for the 100-year design

project.

10. Residential - Residential structures within the SPF flood plain were identified from aerial photos, ortho maps and feasibility study surveys. Structural values were obtained from Hennepin County property records, and assesor's office. Elevations were obtained from two foot contour ortho maps, feasibility study surveys and on-site inspections. Damages were estimated using residential depth damage curves.

11. Public and Transportation - Public damages were derived from estimates of damages that would occur due to major flooding. The damages, were calculated from past low flow damages and estimates from watershed engineers. Transportation damages result from the additional cost of travel due to detours around the flooded area. Average annual daily traffic counts for county and state roads in the Bassett Creek area were obtained from the County Highway Department. Operating vehicle costs were obtained from the Department of Transportation. Areas of road inundation for the 100-year and SPF floods were obtained from flood profiles. Detour routes were derived by assuming traffic would use the county and state roads network to travel around the flooded area.

DAMAGE CALCULATION

12. Average annual flood damages were evaluated for 1981 conditions and were projected to 1986, when it is assumed the project would be completed, by estimating interim economic growth. Flood damages will also increase with ultimate urbanization or future development of the watershed that is currently undeveloped because runoff will increase and hydrologic conditions change. Due to the significant increase in flood discharge and flood levels in the watershed between existing urbanization and ultimate urbanization, damages were calculated for both these conditions. Based upon urban development projections prepared by the Metropolitan Council of the Twin Cities Area, and published in the

Metropolitan Development Guide, December 1977, ultimate urbanization would occur in 1995. With ultimate conditions, during the 100-year flood event, 171 residences would incur direct flood damages and 203 would incur indirect flood damages. For existing conditions, 75 would incur direct damages, and 105 would incur indirect damages.

13. Residential - Residential data was compiled and inputted into the Depth Damage System (DDS) program. The DDS program calculates depth damage curves for individual structures using Residential Depth Damage Curves. Flood elevations are inputted at specific stationing, corresponding to the structure location along the channel. Using water surface profiles developed for existing and ultimate urbanization conditions, frequency damage curves were developed for with and without project conditions. Both direct and indirect damages were calculated.

TABLE 1

Residential Frequency Damage Curves - Bassett Creek
(Damage Values in $1000)

Frequency	Damages without project		Damages with project	
Direct Damages	Existing	Ultimate	Existing	Ultimate
10-year	1350.2	1913.4	0	0
50-year	1750.3	3033.7	0	0
100-year	2497.1	5662.6	986.6	1498.3
SPF	17162.9	19003.6	11652.7	13222.9
Indirect Damages				
10-year	2282.5	2650.1	0	0
50-year	2681.1	2824.1	0	0
100-year	2853.6	2890.8	1384.3	1991.7
SPF	986.1	586.0	2657.7	2455.4
Total Damages				
10-year	3632.7	4563.5	0	0
50-year	4431.4	5857.8	0	0
100-year	5350.7	8553.4	2370.9	3490.0
SPF	18149.0	19589.6	14310.4	15678.3

14. Total damages were used in calculating benefits. Although Bassett Creek flooding is short duration, there is considerable flood damage due to sewer backup. During recent hi-frequency floods, damages were caused primarily by sewer backup. It was assumed that for low frequency floods indirect damages would also be a major part of total damages.

15. Commercial - Stage damage curves for commercial structures were developed for two reaches of Bassett Creek. The first reach, Minneapolis, included structures in the SPF floodplain from the conduit entrance to Glenwood Avenue. Most of the commercial structures in the watershed are in this reach, and consequently, most of the commercial flood damages are in this reach. (See Table 2.) The second reach, Golden Valley, included structures in the remaining SPF floodplain.

TABLE 2

Commercial Flood Damages
(Ultimate, without project conditions)

	Damages ($1000)	
Flood Event	Reach - Minneapolis	Golden Valley
10-year	1348.2	52.7*
50-year	2328.8	245.0*
100-year	2907.6	536.4
SPF	9088.3	1834.0

*Estimate.

16. The Stage damage curves for individual structures were referenced to the starting points of the reaches by using water surface profiles developed for the project. The stage damage curves were then used in conjunction with stage frequency curves at the reference point, to determine average annual damages.

GROWTH OF PROJECTED FLOOD DAMAGES

17. Increase in flood damages due to future growth has been based upon expected increases in damages to residential contents and industrial and commercial establishments. No increase in flood damages due to new structures is expected

as existing flood plain regulations require new structures to be built above the elevation of the 100-year flood.

GROWTH TO EXISTING DEVELOPMENT

18. The per capita income index for the Minneapolis-St. Paul SMSA was used as the growth index for the contents of residential structures. Damages are expected to increase over time due to the increase in residential content value. Flood damages to contents are projected to increase at the same rate of change as real per capita income, up to the point where content value is 75% of structure value. The value of contents is currently estimated to be 25% of the residential structure value. Therefore, content value will increase by a factor of 3.0 (.75 divided by .25). Since real per capita income is projected to grow at a 2.1 percent compounded annual rate, content value will be 75 percent of structure value in 53 years (1.02153 = 3.0). The net factor increase for year 53 in residential damages is 1.5 ((0.25 x 3) x (0.75 x 1.0)). This factor (1.5), takes into account the expected increase in residential content damages.

19. Future increases in industrial and commercial damages were projected independently for each business enterprise within the 100-year flood plain. A personal interview was conducted with each firm in order to jointly determine which existing condition damage source will change in the future. Past patterns of internal growth within the firm and the firm's visible growth horizon were discussed. Based upon these discussions, realistic decisions were made concerning plant production capacities, inventory and/or stock material growth, production equipment improvements, sources of profit growth and changes in labor inputs (loss of wages). Rates of change and timing of changes for damages sustained were uniquely determined for each firm studied. Some near term increase in damages was also recognized based upon pending installation of pollution control devices and facilities currently required by government

regulatory agencies for specific industries. These improvements will be subject to flood damages. Because of the limited number of businesses with the 100-year flood plain sustaining damage and the possibility for unauthorized disclosure, a table of individual company damage projections is not included. However, the percent of damages subject to future increase and the percent determined to remain constant over the project life is presented in Table 4.

TABLE 3
Growth Components for Commercial-Industrial Units

Type of Damage	Firm A	B	C	D	E	F	G
Constant	82%	90%	39%	56%	96%	100%	100%
Subject to Growth	8%	10%	6%	44%	4%	0%	0%

20. Individual company projected damages were summed and an index of change over time determined. Very little growth after the year 2000 can be reasonably projected using this direct interview and study-of-the-firm method because change beyond the year 2000 is very difficult for private industry to anticipate. No growth was assumed for the remaining 50 years of the 100-year project life.

21. An estimate of growth for companies outside the 100-year floodplain was not developed. The same rate of growth was applied to these companies as to those in the 100-year floodplain. The rate of growth would be similar. The impact on the benefit analysis was also considered minimal since the reduction of SPF flooding is minimal and the total benefits from companies in the SPF is minor since the project is a 100-year design. Between 1980 and 2000 a 25 percent growth rate in commercial damages was calculated; between 2000 and 2030 a 4 percent growth rate in commercial damages was calculated.

EXPECTED ANNUAL DAMAGES

22. Average annual damages were estimated using the Expected Annual Damage (EAD) computer program developed at the Hydrologic Engineering Center. A complete description of the general principles of calculating average annual damages is presented in the users' manual for the program.

23. Residential damages for existing 1982 and ultimate (1995) conditions with and without project, with increases in content damages to year 2035 were input to the program. Commercial damages for existing and ultimate conditions with and without project, were input to the program with the increase in economic growth factor (25% to year 2000; 4% from year 200 to year 2030). The average annual residential and commercial damages for without and with project conditions are shown in Table 4. Project benefits for reducing flood damages are also listed. Total project residential and commercial flood reduction annual benefits are $1,417,750.

TABLE 4
Flood Damages and Project Benefits
(With economic and hydrologic changes)

	Average Annual Damages Without Project ($1000)	Average Annual Damages with Project ($1000) (Residual Damages)	Project Benefits ($1000)
Residential	1285.07	154.15	1130.92
Commercial			
Minneapolis Reach	314.14	54.09	260.05
Golden Valley Reach	34.77	8.00	26.78
TOTAL			$1417.75

NOTE: (7 5/8 percent interest rate, 100-year project life)

REDEVELOPMENT BENEFITS

GENERAL

24. In accordance with the Water Resources Council's Principles and Standards (38FR174), in areas having persistent unemployment or underemployment, project

benefits shall be increased by the value of labor and other resources required for project construction. Since parts of the area within the commuting distance of the project were designated in 1975 as areas of persistent unemployment, redevelopment benefits were evaluated. The benefits were not included in the benefit-cost ratio since the designation of these areas has changed. However this section provides an analysis of redevelopment benefits to be included in the final justification, if the designation is changed before project implementation.

METHODOLOGY AND ASSUMPTIONS

25. The recommended plan will have an effect upon the local economy. The initial investment will create new jobs and income flows, thereby directly reducing unemployment and underemployment. There will be demands for both labor and construction materials required for project construction and incomes of individuals in associated industries will be increased indirectly due to the interrelationship and interdependence of these industries. Included are such industries as manufacturing, retail and wholesale trade, etc. These conditions will stimulate the economy which will raise the general level of income.

26. Parts of all of 12 counties within a reasonable commuting distance (50 mile radius) of the project site were eligible for aid in 1975 pursuant to the Area Redevelopment Act (PL 87-27). Redevelopment benefits credited to the relief of unemployment and underemployment are only those allocated to eligible counties. In making the benefit allocation, estimates of unemployment were obtained from published sources while quantitative estimates of underemployment were developed on the basis of per capita income and civilian labor force data. It was determined that approximately 30 percent of the total redevelopment benefits would be the amount applicable to redevelopment counties. 1975 estimates of the Minnesota Department of Manpower Services indicate there are more than 7,000 unemployed construction workers available for work within commuting distance of

the project. Of this total, it is estimated that there are approximately 3,500 skilled, 2,100 semi-skilled and 1,400 unskilled workers. Therefore, an ample supply of labor is readily available.

27. Construction features evaluated include a new conduit, control structures, flood walls, channel modifications and relocations. Total construction cost of the recommended plan is estimated at $17,806,000. On-site labor cost as percentage of total construction cost, is estimated to be approximately 65 percent for urban projects similar to the recommended plan.[1] The distribution of total on-site labor for similar civil works are; skilled 31 percent, semi-skilled and unskilled 59 percent, and administration and supervisory 10 percent.[2] Based on similar projects constructed in the metropolitan area, it was determined that all of the skilled, semi-skilled and unskilled and 50% of the administrative labor required for the project would be hired from the local labor supply and these percentages would result in the amount of wages paid to locally hired unemployed or underemployed labor. Average annual redevelopment benefits credited to the construction phase of the recommended plan were estimated to be $251,700 based on interest and amortization computed at 7 5/8 percent over an economic life of 100-years. Derivation of these benefits is shown on Table 5.

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1. Engineering News Record, McGraw-Hill, Inc., September 18, 1975, Vol. 195, No. 12, p. 14.

2. IBID., June 19, 1975, Vol. 194, No. 25, p. 77.

TABLE 5
Redevelopment Benefits

Estimated On-Site Labor Cost

Construction Cost [1]	$17,806,000
Percent of Cost Allocated to Labor	65
On-Site Labor Cost	11,574,010

On-Site Labor by Employment Category

Classification of Labor	Percent	Wages
Skilled	31	3,588,000
Semi and Unskilled	59	6,829,000
Administrative & Supervisory	10	1,157,000
		$11,574,000

Allocation of Wages to Unemployed and Underemployed Labor

Classification of Labor	Wages	Percent Locally Hired Labor	Wages Paid to Locally Hired Labor
Skilled	$3,588,000	100	$ 3,588,000
Semi and Unskilled	6,829,000	100	6,829,000
Administrative & Supervisory	1,157,000	50	578,500
			$10,995,500

Redevelopment Benefits

Average Annual Value of Local Labor Component

$10,995,500 x .07630 = $838,960

Amount Applicable as Redevelopment Benefits

$838,960 x .30 = $251,700

[1] Includes total construction cost, less costs for engineering and design, supervision and administration, and land acquisition.

ADVANCE REPLACEMENT BENEFITS

28. Construction of the flood control project includes three features that replace four existing structures. These structures are currently in need of major repair or replacement. The existing conduit, outlet for Bassett Creek, and Currie Avenue Sewer, and the floodwalls and railroad bridge downstream of Glenwood Avenue are currently in need of replacement. Construction of the new conduit, floodwalls and bridge would obviate the need for repairing or replacing the existing structures. Accordingly, average annual costs of repair or replacement are considered to be a benefit attributable to the project. (See Table 6).

29. The estimated costs of a complete repair of the existing conduit and Currie Avenue Sewer would be $13,800,000 based on September 1982 construction costs. These costs are estimated to be $1,053,000 based on interest and amortization computed at 7 5/8% over an assumed economic life of 100 years.

30. The estimated replacement cost for the existing flood walls downstream of Glenwood Avenue would be $593,600 based on September 1982 construction costs. Average annual costs are estimated to be $45,200 based on interest and amortization computed at 7 5/8% over an assumed economic life of 100 years and assuming the replacement work would have to be completed by 1986.

31. The estimated costs of replacement of the existing railroad bridge would be $64,000 based upon September 1982 construction costs. Average annual costs are estimated to be $4,900, assuming the replacement work would have to be completed by 1986.

32. Total advance replacement benefits attributable to the project are estimated to be $1,103,100.

TABLE 6
Advance Replacement
(Annual Cost)

Conduit & Sewer	$13,800,000	(Cost of Replacement)
	.07630	(100 year life)
	$ 1,053,000(1)	(Annual Cost)
Floodwalls	$ 592,600	(Cost of Replacement)
	.07630	(100 year life)
	$ 45,200(1)	(Annual Cost)
Railroad Bridge	$ 64,000	(Cost of Replacement)
	.07630	(100 year life)
	$ 4,900	(Annual Cost)

(1) Replacement would be necessary in 1986, therefore benefits for advance replacement are the same as the cost of replacement.

PUBLIC BENEFITS

33. Public benefits were derived from reduction of damages to municipal facilities and reduction of flood emergency costs. The total average annual benefits was estimated at $80,900. This estimate was based on damage reductions developed from community or public works calculations.

TRANSPORTATION BENEFITS

34. Transportation benefits were derived from reduction of detour costs incurred from rerouting traffic around the flooded area. Seventeen county roads and major streets would require detours during the 100-year flood event. The project would eliminate the need for detours. The reductions were based upon an average detour length of 3.5 miles and duration of 6 hours. Using variable operating costs per mile, the reductions in annual detour costs was estimated at $1000.

SUMMARY OF MONETARY BENEFITS

35. Benefits attributable to the selected flood control plan include tangible flood control benefits from the reduction of flood damages, advance replacement benefits, and benefits associated with the decreased threat to human life and public health. Additional tangible benefits which result from the project,

which were not evaluated, include the foregone cost of flood proofing, land enhancement, and the savings in the administrative costs of flood insurance. Based on September 1982 prices, a 100-year economic life and a 7 5/8% interest rate, total average annual flood damage reduction benefits attributable to the project are $2,802,100, and the estimated average annual residual flood damages with the project are $245,000. A summary of Average Annual Benefits is given in Table 7.

36. Redevelopment benefits would increase Average Annual benefits by $251,700.

37. The proposed project at Bassett Creek complies with Executive Order 11988. It would significantly reduce flood risk to existing development with measures that address the dual objectives of national economic development and environmental quality. The proposed measures represent the only practicable alternative. Appropriate consideration has been given in the planning process to nonstructural measures. The area communities have implemented floodplain development restrictions, and have purchased developable floodplain to prevent future flood damages to new development.

TABLE 7
Summary of Average Annual Benefits

Item			Average Annual Benefits
Flood control			
Residential	1,130,920		
Commercial	286,830		
Public	80,900		
	1,498,650[1]	Update Factor[2] X 1.133 =	1,698,000
Advance Replacement			1,103,100
Transportation			1,000
	TOTAL		$2,802,100

(1) October 1981 prices.
(2) To compare with Average Annual Costs, which are in September 1982 prices, the Average Annual Benefits were updated from October 1981 prices to September 1982 prices.

BENEFIT COST ANALYSIS

38. Annual Costs - Annual costs for the project are the same of amortized first costs and annual operations and maintenance costs. First costs include interest during construction for the cost of conduit construction since this feature will be constructed over a 3-year period. Other features will be completed within one construction season.

Calculations below show annual costs to be $2,329,000

Calculation of annual charges for the proposed project.
(September 1982 prices; 7 5/8 percent interest rate)

Item	Amount
First Costs	$28,540,000
Interest During Construction	1,757,000
Total	$30,297,000
Interest & Amortization (30,297,000 x 0.07630)	$ 2,311,700
Operation & Maintenance	17,500
Total Annual Charges	$ 2,329,200

ANNUAL BENEFITS

39. Total Average Annual Benefits were estimated at $2,802,100 in September 1982 prices.

RELATIONSHIP OF BENEFITS TO COSTS

40. The project has annual benefits of $2,802,000 and annual costs of $2,329,200, resulting in a benefit-cost ratio of 1.20.

AREA REDEVELOPMENT

41. If the Bassett Creek area was reclassified as an area of persistent unemployment, redevelopment benefits could be attributable to the project. This would result in an increase in annual benefits to $3,053,800 and a change in the BCR to 1.31.

DEPARTMENT OF THE ARMY

St. Paul District Corps of Engineers
1135 U.S. Post Office & Custom House
St. Paul, Minnesota 55101

FLOOD CONTROL

BASSETT CREEK

HENNEPIN COUNTY, MINNESOTA

DESIGN MEMORANDUM NO. 2, PHASE II

APPENDIX F

DETAILED COST ESTIMATE

APPENDIX F

September 1982

BASSETT CREEK

DETAILED ESTIMATE OF FIRST COSTS

	Unit	Quantity	Unit Cost	Total Cost	
Federal First Costs and Non-Federal Contributions					
Relocations					
Flood Proofing (24 Houses- App. H, Page H-26)	Job	Sum	-	398,600	
Railroad Bridge	-	-	-	-	
Concrete	C.Y.	273	200	54,600	
Bearing Piles	L.F.	504	120	60,480	
Railroad Ties	L.F.	808	10	8,080	
Contingencies				18,240	
Total Relocations					540,000
Channels					
Conduit					
Mn/DOT 2nd St. Tunnel	Job	Sum	-	3,000,000	
10' ϕ Precast Tunnel	L.F.	5,100	1,500	7,650,000	
Cath. Arch Tunnel	L.F.	1,600	1,000	1,600,000	
Contingencies				1,350,000	
Channel Modifications					
Penn Avenue Bridge					
Excavation	C.Y.	1,750	2.50	4,375	
12" Riprap w/6" Bedding	S.Y.	2,620	16.50	43,230	
Old Bridge Removal	Job	Sum		10,000	
Contingencies				8,395	
Fruen Mill Area					
Excavation	C.Y.	7,750	2.00	15,500	
Fill	C.Y.	1,610	2.00	3,220	
Sheet Piling	S.F.	9,240	13.00	120,120	
New Railroad	L.F.	755	60.00	45,300	
36" RCP w/Flapgate	L.F.	72	120.00	8,640	
Chain Link Fence	L.F.	205	10.00	2,050	
Remove Railroad Bridge	Job	Sum		10,000	
Crushed Rock	C.Y.	300	8.00	2,400	
Riprap	C.Y.	900	25.00	22,500	
Contingencies				34,270	

BASSETT CREEK

DETAILED ESTIMATE OF FIRST COSTS

	Unit	Quantity	Unit Cost	Total Cost
Channel Modifications (Continued)				
Markwood Area				
115" x 72" RCP-A	L.F.	2,030	390.00	791,700
Sodding	S.Y.	5,000	2.00	10,000
84" x 54" RCP-A	L.F.	120	300.00	36,000
24" RCP	L.F.	20	90.00	1,800
Catch Basins 24" Beehive	Ea.	6	500.00	3,000
Box Inlets	Ea.	5	9,000	45,000
Connection Box	Ea.	1	6,000	6,000
115" x 72" RCP-A	L.F.	120	390.00	46,800
Contingencies				140,700
Minnaqua Avenue Bridge Removal				
Lump Sum as Bid	Job	Sum	-	6,625
Contingencies				375
Storage Areas				
Tunnel Inlet Ponding				
Excavation	C.Y.	125,000	2.00	250,000
Excavation, Disposal at Landfill	C.Y.	125,000	5.75	718,750
Shaping at Landfill	C.Y.	125,000	.35	43,750
Random Fill	C.Y.	24,000	2.00	48,000
18" Riprap w/9" Bedding	S.Y.	20	25.00	500
Drop Inlet Sturcture	C.Y.	100	250.00	25,000
Contingencies				163,000
Edgewood Area				
Excavation	C.Y.	31,500	2.00	63,000
Cul-de-Sac				
4" Bituminous	S.Y.	320	6.00	1,920
6" Base Coarse	S.Y.	320	2.00	640
18" Riprap w/12" Bedding	S.Y.	3,150	16.20	51,030
48" Manhole (10') SD	Ea.	4	2,750	11,000
12" RCP - SD	L.F.	16	25.00	400
18" RCP - SD	L.F.	70	25.00	1,750
30" CMP - SD	L.F.	34	60.00	2,040
Remove Existing Sanitary Sewer	L.F.	1,150	2.00	2,300
Contingencies				19,920
Beautification (App. G, Page G-4)				200,000
Total Channels				16,621,000

BASSETT CREEK

DETAILED ESTIMATE OF FIRST COSTS

	Unit	Quantity	Unit Cost	Total Cost
Flood Control Structures				
Tunnel Inlet Drop Structure				
Concrete	C.Y.	309	300.00	92,700
Safety Cap	L.F.	42	20.00	840
Trash Rack	Ea.	1	2,000	2,000
Contingencies				14,460
Highway 55				
Control Structure				
Concrete	C.Y.	68	300.00	20,400
Handrailing	L.F.	28	15.00	420
Remove Wing Wall	C.Y.	23	250.00	5,750
Dewatering (Diversion)	Job	Sum	-	2,000
Contingencies				4,430
Highway 100				
Pervious Fill	C.Y.	48,700	2.00	97,400
Impervious Fill	C.Y.	1,075	12.00	12,900
Clearing and Grubbing	Acre	0.1	4,000	400
Topsoil	C.Y.	1,450	3.00	4,350
Seed	Acre	2.7	900.00	2,430
Excavate and Fill (Impervious)	C.Y.	15,000	14.50	217,500
Curb and Gutter	L.F.	1,120	8.50	9,520
36" C.M.P.	L.F.	125	60.00	7,500
Control Structure				
Excavation	C.Y.	1,300	2.50	3,250
18" Riprap	C.Y.	325	36.00	11,700
9" Bedding	C.Y.	160	15.00	2,400
Concrete	C.Y.	457	250.00	114,250
Contingencies				72,400

BASSETT CREEK

DETAILED ESTIMATE OF FIRST COSTS

	Unit	Quantity	Unit Cost	Total Cost
Flood Control Structures (Continued)				
Golden Valley Country Club				
Excavation	C.Y.	700	2.50	1,750
Embankment Fill	C.Y.	1,600	2.00	3,200
Concrete	C.Y.	75	250.00	18,750
Riprap	C.Y.	565	36.00	20,340
Bedding	C.Y.	280	15.00	4,200
Culverts (4' x 6' Box)	L.F.	178	150.00	26,700
Stripping	C.Y.	420	3.00	1,260
Clearing & Grubbing	Acre	0.1	4,000	400
Asphalt Pavement 2"	S.F.	1,050	4.00	4,200
Topsoil	C.Y.	275	1.00	275
Seeding	Acre	0.5	900.00	450
Concrete	C.Y.	200	250.00	50,000
Grouted Riprap	C.Y.	10	50.00	500
Contingencies				19,975
Edgewood Area				
Embankment				
73" x 45" RCP-A CL IV w/ Apron	L.F.	64	180.00	11,520
102" x 62" RCP-A CL IV w/ Apron	L.F.	60	310.00	18,600
73" x 45" to 102" x 62" transition	Ea.	1	700.00	700
Impervious Fill	C.Y.	3,450	2.00	6,900
Stripping	C.Y.	425	2.00	850
Clearing & Grubbing	Acre	0.4	4,000	1,600
Inspection Trench	C.Y.	275	8.00	2,200
Topsoil	C.Y.	235	1.00	235
Seeding	Acre	0.3	900.00	270
Gabions 12"	C.Y.	340	85.00	28,900
Bedding	C.Y.	170	15.00	2,550
Sheet Pile	S.F.	1,270	12.00	15,240
Excavation for Gabions	C.Y.	616	2.50	1,540
Concrete	C.Y.	150	300.00	45,000
Conduit and Stilling Basin				
Excavation	C.Y.	1,700	2.50	4,250
Riprap	C.Y.	350	36.00	12,600
Bedding	C.Y.	140	15.00	2,100
Topsoil	C.Y.	30	1.00	30
Misc. Impervious Fill	C.Y.	300	2.00	600
Channel Excavation	C.Y.	480	2.50	1,200
Contingencies				23,115

BASSETT CREEK

DETAILED ESTIMATE OF FIRST COSTS

	Unit	Quantity	Unit Cost	Total Cost	
Flood Control Structures (Continued)					
Wisconsin Avenue Embankment					
Pervious Fill	C.Y.	1,100	12.00	13,200	
Excavation	C.Y.	830	2.50	2,075	
Impervious Fill	C.Y.	1,000	2.00	2,000	
Concrete	C.Y.	17	500.00	8,500	
Precast Culvert, Conc. 8' x 6'	L.F.	70	390.00	27,300	
Fence	L.F.	100	5.00	500	
Topsoil	C.Y.	100	6.00	600	
Seeding	Acre	0.2	900.00	180	
Contingencies				8,645	
Winnetka Avenue Berm					
Impervious Fill	C.Y.	23	10.00	230	
24" CMP w/Flapgate	L.F.	90	105.00	9,450	
Contingencies				1,320	
Total Flood Control Structures					1,105,000
Engineering and Design					2,711,000
Supervision and Administration					
Inspection				822,000	
Overhead				961,000	
Total Cost (Federal First Costs and Non-Federal Contributions)					22,760,000
Non-Federal Contributions					460,000
TOTAL FEDERAL FIRST COSTS					22,300,000

BASSETT CREEK

DETAILED ESTIMATE OF FIRST COSTS

	Unit	Quantity	Unit Cost	Total Cost
Non-Federal First Costs				
Lands and Damages				3,740,000
Relocations				
Channel Crossings				
Nobel Avenue Regent Avenue 32nd Avenue Brunswick Avenue Adams Avenue 34th Avenue 36th Avenue	Items have been constructed or bids have been received totaling			1,051,000
Westbrook Road				
107" x 169" RCP-A III	L.F.	134	550.00	73,700
18" Riprap w/9" Bedding	S.Y.	350	16.20	5,670
Excavation	C.Y.	370	2.50	925
Road Replacement				
4" Bituminous	S.Y.	4,000	6.00	24,000
6" Base Coarse	S.Y.	4,330	2.00	8,660
Contingencies				17,045
Douglas Drive				
Pavement Removal	S.Y.	337	2.00	674
72" Culvert Removal	Job	Sum		2,400
Pavement, Concrete 8"	S.Y.	307	16.00	4,912
Pavement, Concrete 4"	S.Y.	22	10.00	220
Pavement, Asphalt 1 1/2"	S.Y.	40	4.00	160
Excavation	C.Y.	1,220	2.50	3,050
Backfill, Pervious	C.Y.	440	12.00	5,280
Bedding	C.Y.	120	16.00	1,920
Box Culvert 7' x 4'	L.F.	261	380.00	99,180
24" CMP	L.F.	32	50.00	1,600
18" RCP	L.F.	21	40.00	840
10" D.I.P.	L.F.	34	40.00	1,360
Contingencies				32,344

BASSETT CREEK

DETAILED ESTIMATE OF FIRST COSTS

	Unit	Quantity	Unit Cost	Total Cost	
Utility Relocations					
Tunnel Inlet Ponding					
48" Manhole	Ea.	3	2,750	8,250	
84" RCP Sewer	L.F.	862	370.00	318,940	
Contingencies				48,810	
Penn Avenue Bridge					
24" D.I.P. Watermain	L.F.	85	60.00	5,100	
12" Steel Gasline	L.F.	100	40.00	4,000	
Contingencies				1,400	
Fruen Mill Area					
Drop Structure					
Concrete	C.Y.	106	300.00	31,800	
Remove Rock Dam	Job	Sum	-	1,500	
Replace 6" Watermain	L.F.	60	70.00	4,200	
Contingencies				8,000	
Highway 100					
Remove and replace					
3 1/2" Asphalt Road	S.F.	15,000	0.70	10,500	
Replace 8" VCP with					
8" PVC S.S.	L.F.	360	12.00	4,320	
Replace Waterline	L.F.	360	20.00	7,200	
48" S.S. Manhole	Ea.	1	2,750	2,750	
48" S.S. Manhole Extension	Ea.	1	1,750	1,750	
Contingencies				3,480	
Edgewood Area					
48" Manhole S.S.	Ea.	8	2,750	22,000	
10" PVC S.S.	L.F.	1,360	11.00	14,960	
Contingencies				5,040	
Medicine Lake Outlet Structure					
Sheet Piling	S.F.	12,910	12.00	154,920	
Timber Lagging	S.F.	134	35.00	4,690	
Concrete	C.Y.	26	250.00	6,500	
Steel Plate	Lbs.	4,650	2.00	9,300	
Contingencies				24,590	
Total Relocations					2,040,000

BASSETT CREEK

SUMMARY OF NON-FEDERAL FIRST COSTS

Non-Federal First Costs	
Lands and Damages	$3,740,000
Flood Proofing (20% of Total Cost)	80,000
Relocations	2,040,000
Indirect Costs (Mn/DOT - 3rd Ave.)	380,000
TOTAL NON-FEDERAL FIRST COSTS	$6,240,000

Department of the Army
St. Paul District, Corps of Engineers
1135 U.S. Post Office and Custom House
St. Paul, Minnesota 55101

FLOOD CONTROL

BASSETT CREEK
HENNEPIN COUNTY, MINNESOTA
DESIGN MEMORANDUM NO. 2, PHASE II

APPENDIX G

BEAUTIFICATION MEASURES

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APPENDIX G

BEAUTIFICATION PLAN

INTRODUCTION

1. The basic flood control measures proposed as part of the Bassett Creek project will have varying degrees of adverse visual impacts upon the project area (i.e., construction of some measures will have significant impacts while others will result in slight impacts). To offset these impacts and to make the Bassett Creek project more socially and environmentally acceptable, beautification measures are proposed (e.g., landscaping, rustification, etc.). This appendix provides a brief discussion of the types of aesthetic impacts the basic flood control project could have on the resource and provides a conceptual beautification plan to help offset these impacts (see the following plates for details). A rough estimate of the cost associated with implementation of the beautification plan is also included. Detailed planting plans will need to be fully developed in future studies to coordinate exact locations and species to be implemented.

PROJECT RELATED AESTHETIC IMPACTS AND ASSOCIATED BEAUTIFICATION MEASURES

2. GOLDEN VALLEY COUNTRY CLUB EMBANKMENT

Impacts - A portion of the golf course will be disturbed by the construction of a concrete box culvert. The existing bridge and path will need to be removed. Some visual resources will therefore be affected.

Beautification Measures - The path will be realigned and landscape plantings will be incorporated into the overall plan to minimize visual impacts. The exact species and locations for these plantings will be carefully coordinated with golf course representatives in future studies so as not to affect operations of the golf course. See plate G-1 for details.

3. CULVERT REPLACEMENT AT WESTBROOK ROAD

Impacts - Construction of these culverts will result in the loss of existing vegetation adjacent to the bridge.

Beautification Measures - Plantings of flood tolerant shrubs and trees along the affected construction zone is proposed. See plate G-1 for details.

4. CULVERT REPLACEMENT AT 34TH AVENUE

Impacts - Construction needed for making the necessary improvements to this site will disturb the vegetation in this area.

Beautification Measures - Proposed plantings will help offset the loss of vegetation and restore the visual setting of the neighborhood. See plate G-2 for details.

5. IMPROVEMENTS AT HIGHWAY 100

Impacts - Considerable change in earth form/grade and loss of existing vegetation would result from implementation of this measure.

Beautification Measures - To help offset this disruption, groups of plantings are proposed at strategic locations. The use of warp sections along the levee near Medicine Lake Road is also proposed. Care during construction should help minimize the loss of existing vegetation in this area (e.g., apple trees, etc.). See plate G-2 for details.

6. BRIDGE REMOVAL AT MINNAQUA AVENUE

Impacts - Closure and removal of this bridge will not be a major aesthetic impact. However, some measures to make the feature aesthetically acceptable are warranted.

Beautification Measures - Limited plantings of a horticultural nature are proposed to terminate the road, buffer the disturbed area, and make the project more acceptable to the neighborhood affected. See plate G-2 for details.

7. BASSETT CREEK CLEARING AND WIDENING NEAR 36th AVENUE NORTH

Impacts - The implications of this construction are signficant to neighborhood aethetics. Adverse impacts would be considerable.

Beautification Measures - To help offset the aesthetic impacts associated with this construction, a large number and variety of plant materials would need to be incorporated into the overall plan. Careful coordination with affected landowners will be necessary in future studies to identify the location and species. See plate G-3 for details.

8. PENN AVENUE CULVERT

Impacts - Changes in riparian contours adjacent to the culvert will result in unavoidable impacts on the existing vegetation.

Beautification Measures - The planting of flood-tolerant species of trees and shrubs to restore this area is recommended. See plate G-4 for details.

9. NOBEL AVENUE CULVERT

Impacts - Access requirements for removal of the old culvert and upgrading of structures will impact the site somewhat.

Beautification Measures - Plantings are proposed to offset the loss of vegetation and to restore the project area (as viewed from Noble Avenue). See plate G-4 for details.

10. STATE HIGHWAY 55 CONTROL STRUCTURE

Impacts - Aesthetic implications of construction of this flood control measure are limited to disruption of vegetation adjacent to the proposed control structure.

Beautification Measures - Limited landscape plantings are proposed to restore the project area; particular attention would be given to views from State Highway 55 towards the site. See plate G-4 for details.

11. REGENT AVENUE NORTH CULVERT

Impacts - The extent of visual disruption to the site will be limited to the immediate area of construction/access.

Beautification Measures - Limited landscape plantings are proposed to restore the project as viewed from Regent Avenue North. See plate G-4 for details.

12. FRUEN MILL CHANNEL IMPROVEMENTS

Impacts - The proposed flood control measures at this site would invovle removal of the existing stone dam, addition of floodwalls and riprap along portions of the shoreline, and construction of a new drop structure. Although implementing these features would have aesthetic impacts, some of the area is aesthetically deteriorated so net impacts associated with the flood control features are limited. The area is located in a heavy use area because of the proximity of the Fruen Mill, existing parklands, and a trail. Therefore, relocation and enhancement of the project area is recommended.

Beautification Measures - The inclusion of rustification measures on the concrete floodwalls, landscape planting to buffer disturbed areas and installation of aesthetically pleasing fencing is recommended. These measures would be highly visible to the public and would make the project more acceptable. See plate G-5 for details.

13. MOUTH OF MEDICINE LAKE

Impacts - Channel modifications proposed by the project would have significant visual impacts because of the loss of existing vegetation.

Beautification Measures - Landscape planting is recommended to help offset adverse impacts from South Shore Drive and from the existing nearby residences. See plate G-6 for details.

14. EDGEWOOD STORAGE AREA

Impacts - The development of ponding areas in this area will result in considerable vegetative disruption. Visually, because of the high frequency of observation of this area, the aesthetic impact could be significant.

Beautification Measures - Implementation of substantial landscape plantings and the development of a small permanent pond between Georgia and Florida Avenues is recommended. These measures should help offset project effects on visual resources. See plate G-6 for details.

15. BRUNSWICK AVENUE AND 32nd AVENUE FEATURES

Impacts - Visual impacts caused by construction of these proposed flood control features will be limited to access disruptions.

Beautification Measures - To offset the visual impacts of construction, limited landscape planting are proposed. See plate G-6 for details.

16. BASSETT CREEK OVERFLOW PONDING AREA

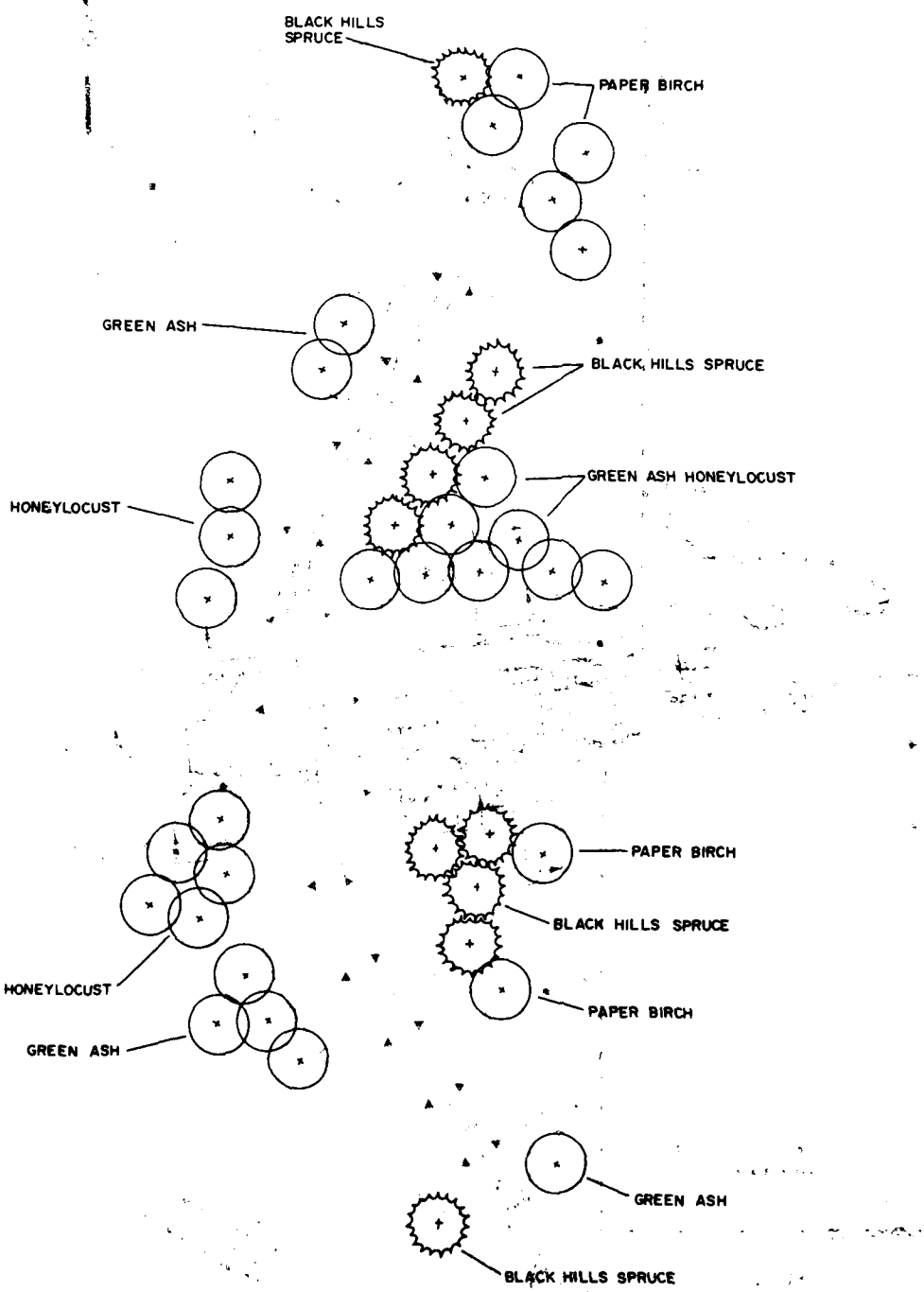
Impacts - This area is currently a railroad yard and landfill area which has limited aesthetic qualities. However, the proposed project would require considerable earthwork resulting in significant vegetation loss.

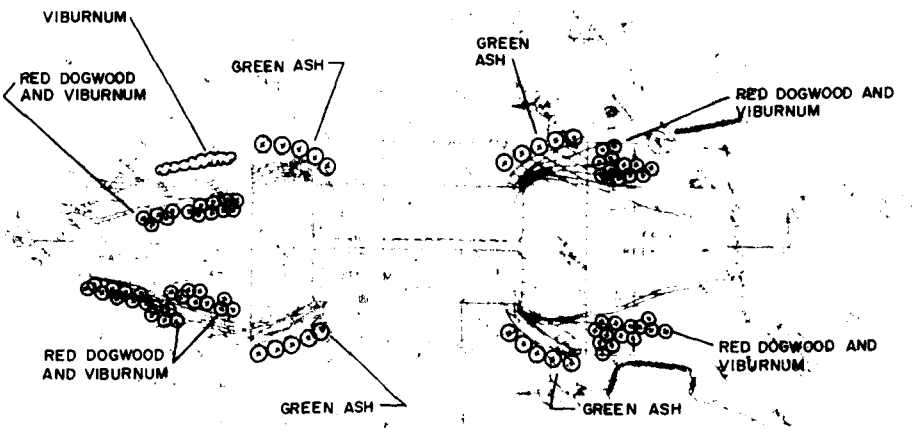
Beautification Measures - To offset visual impacts of the proposed construction activities, considerable landscaping and mounding of earth is proposed. Realignment of the creek channel is also proposed. See plate G-7 for details.

COST ESTIMATE

17. Costs associated with implementing the recommended beautification plan are estimated at $200,000. This estimate does not include the cost of turf establishment. Estimated costs have been broken down as follows (costs do include contingencies):

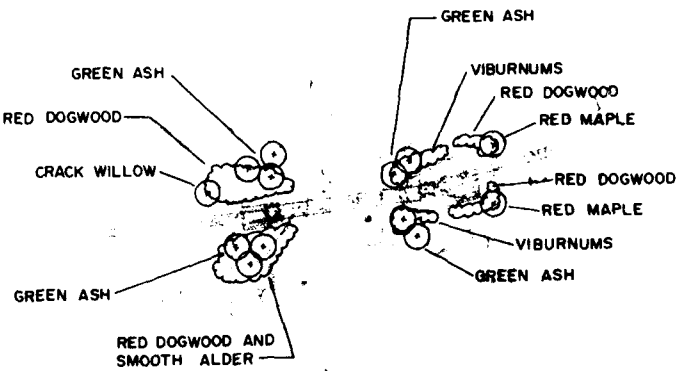
Items	Quantity	Average unit cost (planted)	Subtotal
Trees	700	$150.00	$105,000
Shrubs	2,000	15.00	30,000
Earthwork/shaping	Job	--	25,000
Excavation (create pond)	10,000 cu yd	4.00/cu yd	35,000
Rustification	Job	--	5,000
Estimated total $200,000			

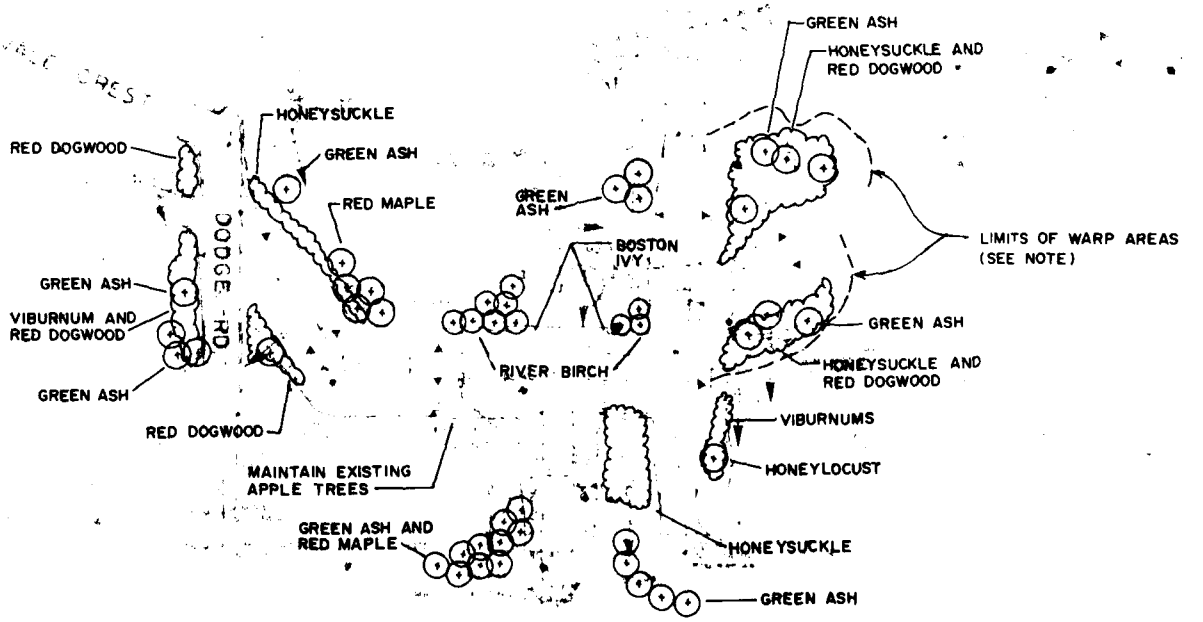

BLACK HILLS SPRUCE
PAPER BIRCH
GREEN ASH
BLACK HILLS SPRUCE
GREEN ASH HONEYLOCUST
HONEYLOCUST
PAPER BIRCH
BLACK HILLS SPRUCE
PAPER BIRCH
HONEYLOCUST
GREEN ASH
GREEN ASH
BLACK HILLS SPRUCE

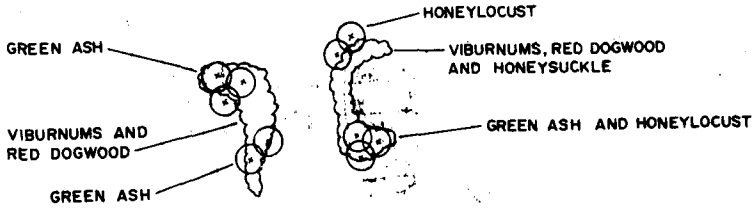


DEPARTMENT OF THE ARMY

PLATE G-1


GREEN ASH
RED DOGWOOD
CRACK WILLOW
GREEN ASH
RED DOGWOOD AND
SMOOTH ALDER
GREEN ASH
VIBURNUMS
RED DOGWOOD
RED MAPLE
RED DOGWOOD
RED MAPLE
VIBURNUMS
GREEN ASH


DODGE RD
RED DOGWOOD
HONEYSUCKLE
GREEN ASH
RED MAPLE
GREEN ASH
VIBURNUM AND
RED DOGWOOD
GREEN ASH
RED DOGWOOD
MAINTAIN EXISTING
APPLE TREES
GREEN ASH AND
RED MAPLE
GREEN
ASH
BOSTON
IVY
RIVER BIRCH
GREEN ASH
HONEYSUCKLE AND
RED DOGWOOD
LIMITS OF WARP AREAS
(SEE NOTE)
GREEN ASH
HONEYSUCKLE AND
RED DOGWOOD
VIBURNUMS
HONEYLOCUST
HONEYSUCKLE
GREEN ASH



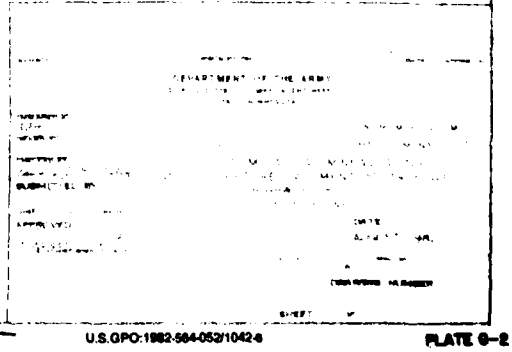
AND

NOTE: WARP SECTIONS ARE AREAS OF ADDITIONAL FILL OVER LEVEE AREAS WHICH ARE CREATED TO ALLOW FOR PLANTINGS ON THE LEVEE (I.e. ALLOWS FOR ROOT FREE ZONE)

— LIMITS OF WARP AREAS
(SEE NOTE)

BH

AND

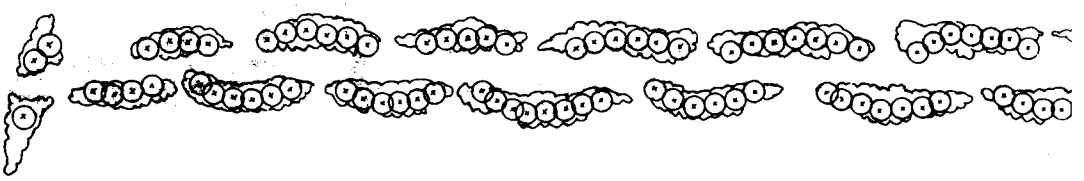




2

U.S.GPO:1982-564-052/1042-6

PLATE G-2

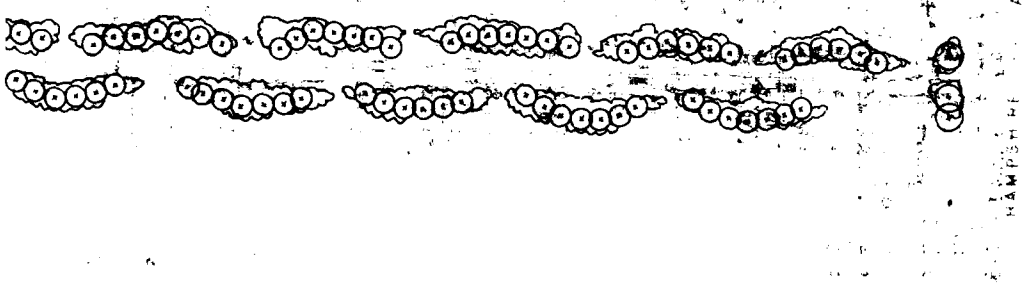


NOTES

1. BASSETT CREEK CHANNEL TO BE WIDENED. EXISTING PLANT SPECIES TO BE REMOVED. A 50' DIA. CHANNEL WAS PROJECTED HERE. THIS BEING THE UPPER LIMIT OF SIZE. ALL NEW PLANT SPECIES MUST BE MOISTURE TOL.

2. POSSIBLE PLANT SPECIES TO BE USED INCLUDE:

TREES	RED MAPLE SILVER MAPLE GREEN ASH AMUR MAPLE WHITE ASH HONEY LOCUST CRACK WILLOW RUSSIAN OLIVE RIVER BIRCH BLACK HILLS SPRUCE
SHRUBS	RED DOGWOOD STAGHORN SUMAC AMERICAN PLUM VIBURNUMS HONEYSUCKLE ALDER



EXISTING
CHANNEL
ER
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DE:

OGWOOD
IORN SUMAC
ICAN PLUM
NUMS
'SUCKLE
R

SCALE IN FEET

DEPARTMENT OF THE ARMY
ST. PAUL DISTRICT, CORPS OF ENGINEERS
ST. PAUL, MINNESOTA

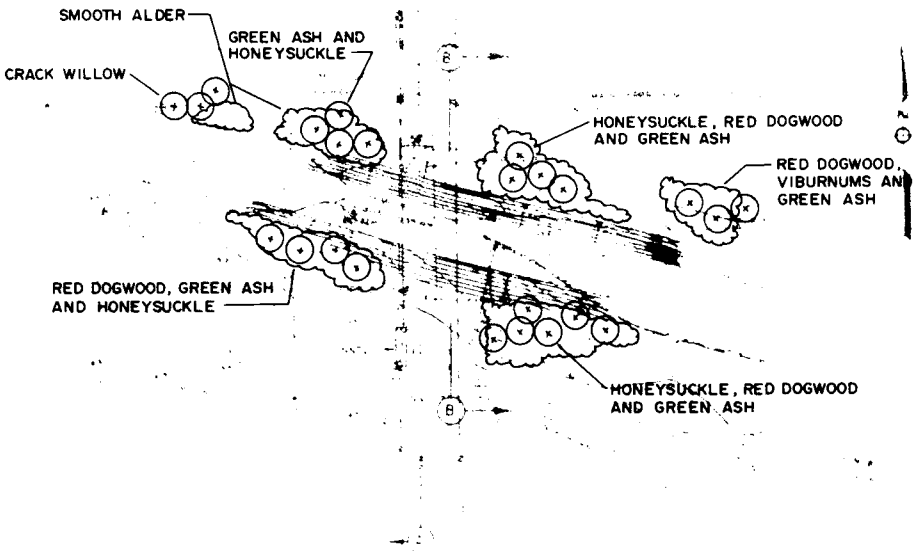
PHASE: DESIGN MEMORANDUM
FLOOD CONTROL BASSETT CREEK MINNESOTA

NORTH BR CHANNEL N 36th AVE
PLANTING PLAN

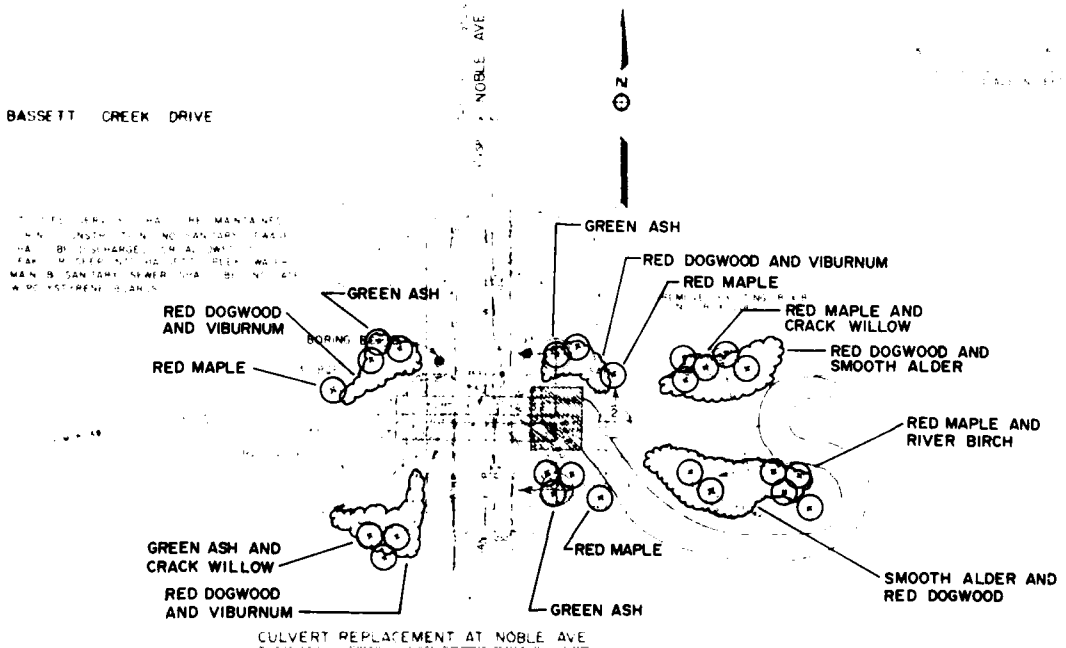
DATE: AUGUST 1968

SAFETY DAYS

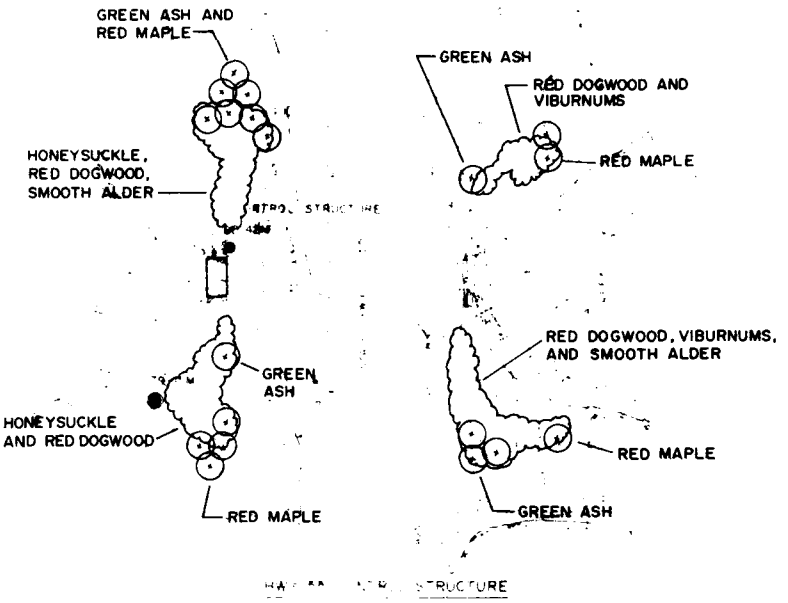
PLATE G-3



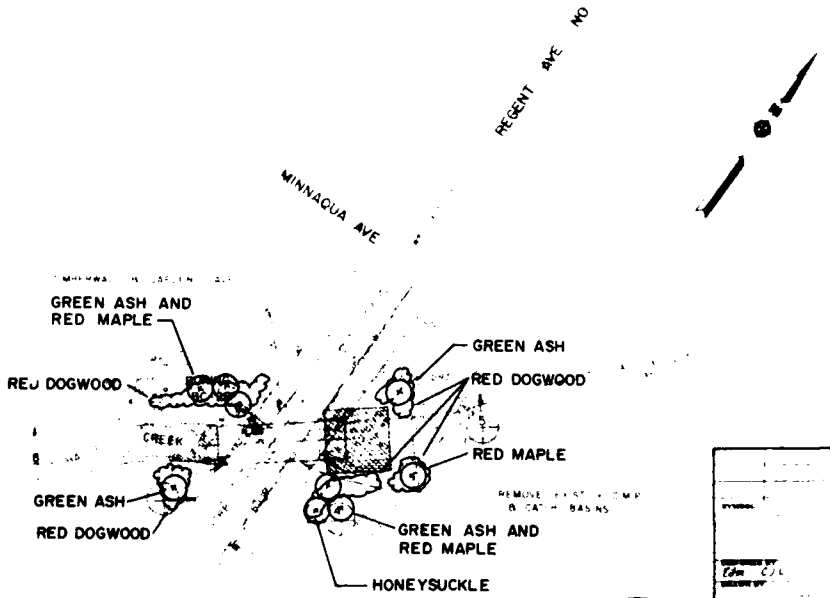
PENN AVE. CULVERT REMOVAL



CULVERT REPLACEMENT AT NOBLE AVE



STRUCTURE



CULVERT REPLACEMENT AT REGENT AVE

DEPARTMENT OF THE ARMY
ST PAUL DISTRICT CORPS OF ENGINEERS
ST PAUL MINNESOTA

PHASE I DESIGN MEMORANDUM

FLOOD CONTROL BASSETT CREEK MINNESOTA

PENN AVE CULVERT REMOVAL HWY 55 CONTROL STRUCTURE CULVERT REPLACEMENT AT NOBLE AVE & CULVERT REPLACEMENT AT REGENT AVE NO

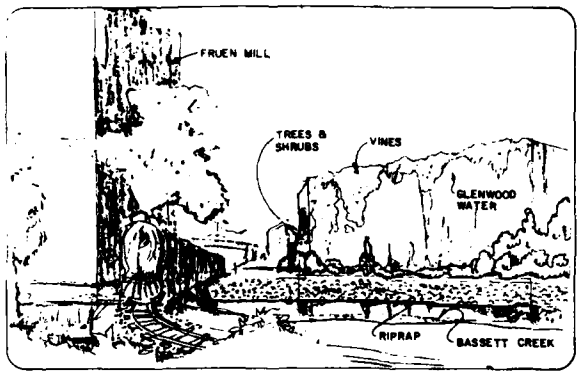
PLANTING PLANS

DATE AUGUST 1982

AS SHOWN

SHEET

PLATE G-4



HONEYLOCUST

STAGHORN SUMAC

VEGETATIVE SCREEN BUFFER TRACKS FROM TRAIL

SHRUB LINE SETBACK MIN 15' FROM TRACK BED

RUSTIFICATION MEASURES TO BE USED ON THIS CONCRETE WALL TO MAKE IT FIT INTO SURROUNDING AREA (I.E. COLOR CONCRETE AND PUT A FLAGSTONE WALL TYPE FACE ON CONCRETE...)

OPEN SPACE

POSSIBLE LOCATION FOR PARK BENCH

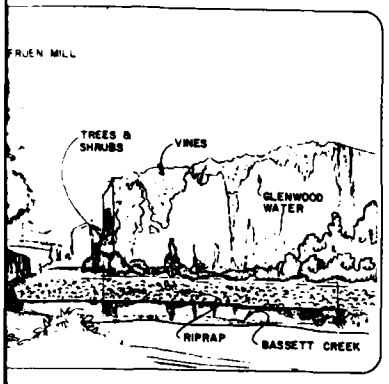
STONE BENCH CLIFF FACE

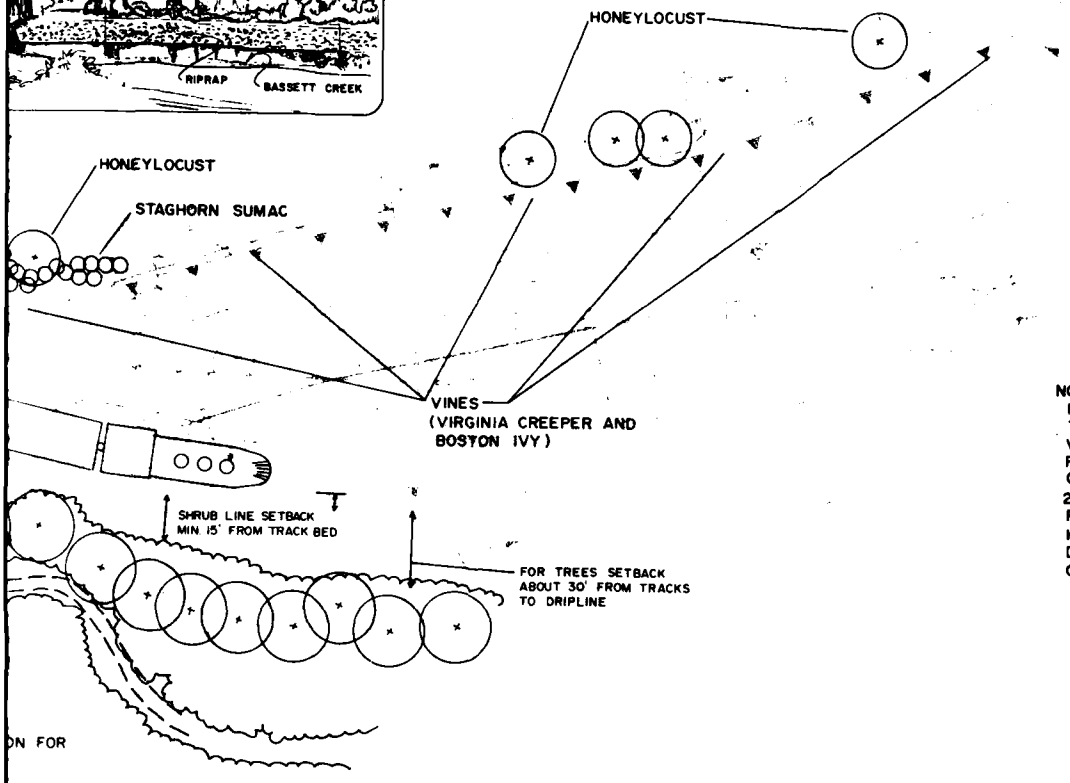
SOME RE-ALIGNMENT OF THE EXISTING TRAIL WILL BE NEEDED

POSTS INSET INTO GROUND 3' CONCRETE REINFORCED

OBJECTIVE
REDUCE NEG VISUAL IMPACT OF PLANNED SHEET PILE RETAINING WALL AND PROVIDE SAFETY RAILING

PLANT MATERIAL TO REDUCE VISUAL IMPACT OF IRON SHEET PILES



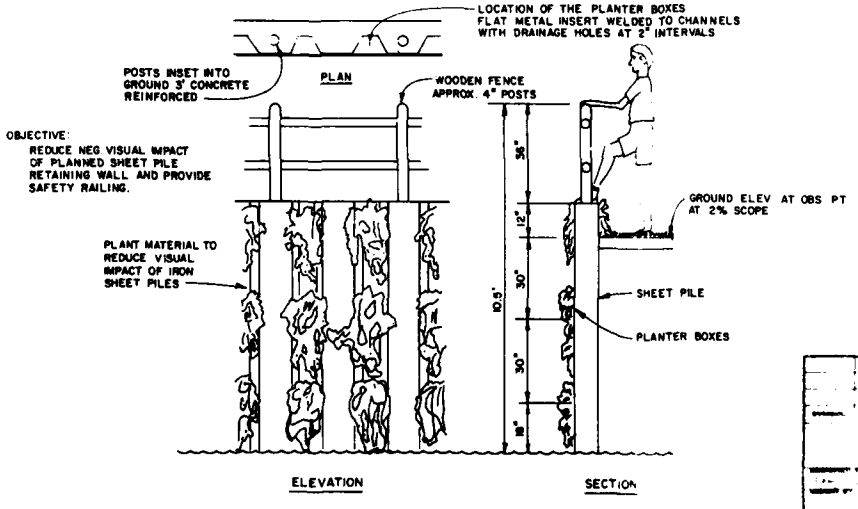


NOTES:

1. OBJECTIVE OF PLANT MATERIALS HERE IS TO SOFTEN BUILDING EDGES, PROVIDE A VISUAL BARRIER FROM TRAIL USERS TO RAILROAD TRACKS, & LIMIT ERODABILITY OF LAND.
2. PLANT MATERIALS TO INCLUDE THE FOLLOWING: GREEN ASH, WHITE ASH, HONEYLOCUST, CRACK WILLOW, HONEYSUCKLE, DOGWOODS, SUMACS, VIBURNUMS, VIRGINIA CREEPER AND BOSTON IVY.

ON FOR

OF THE
BE



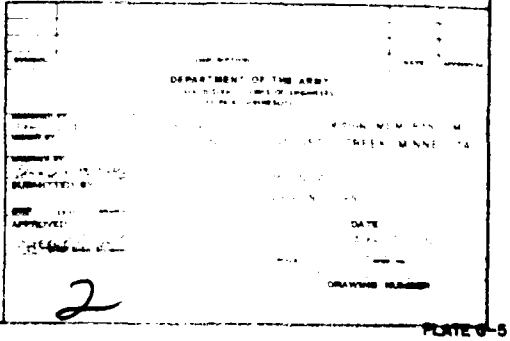
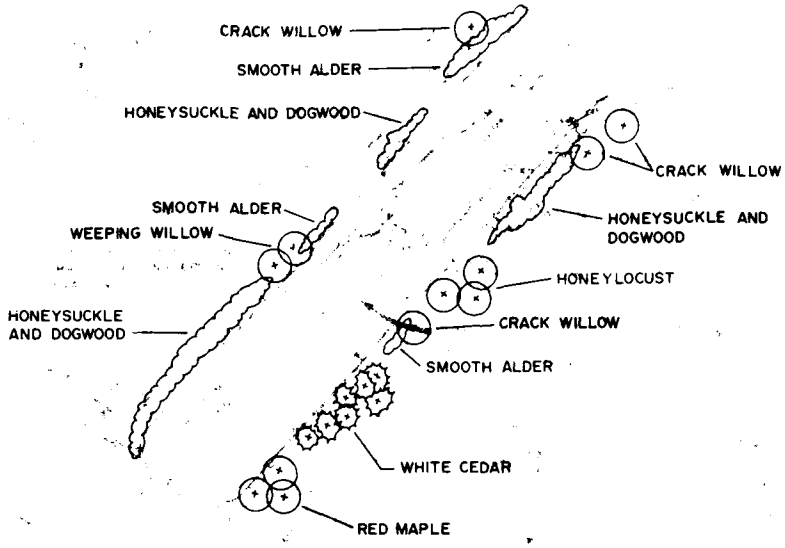
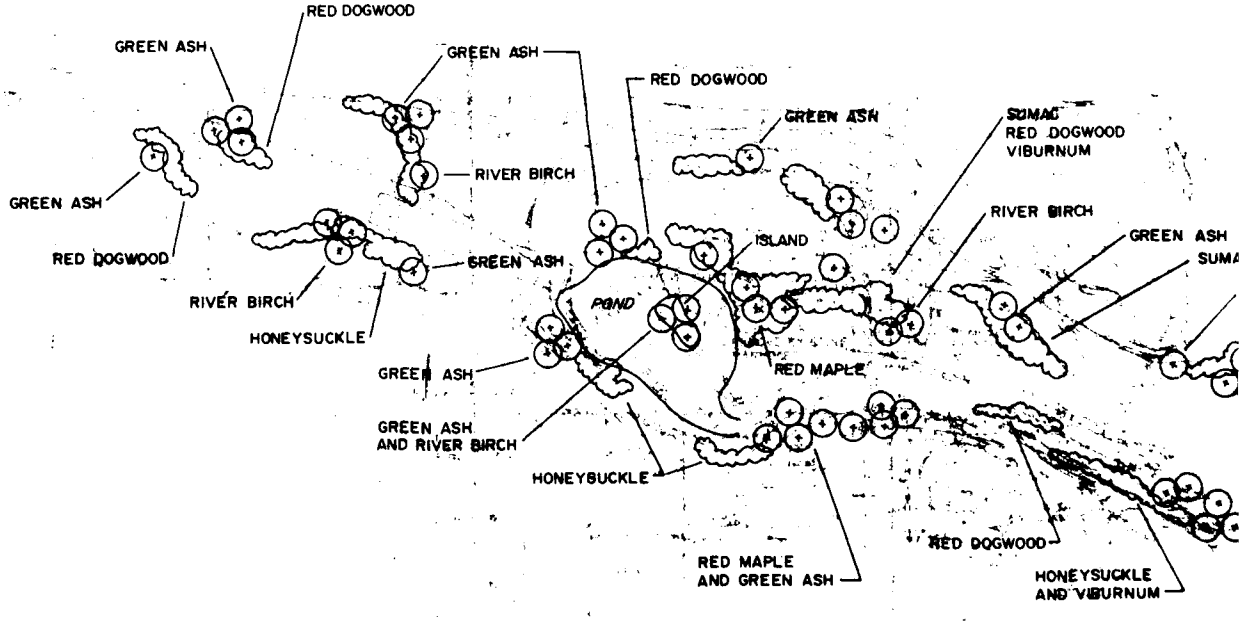


PLATE C-5

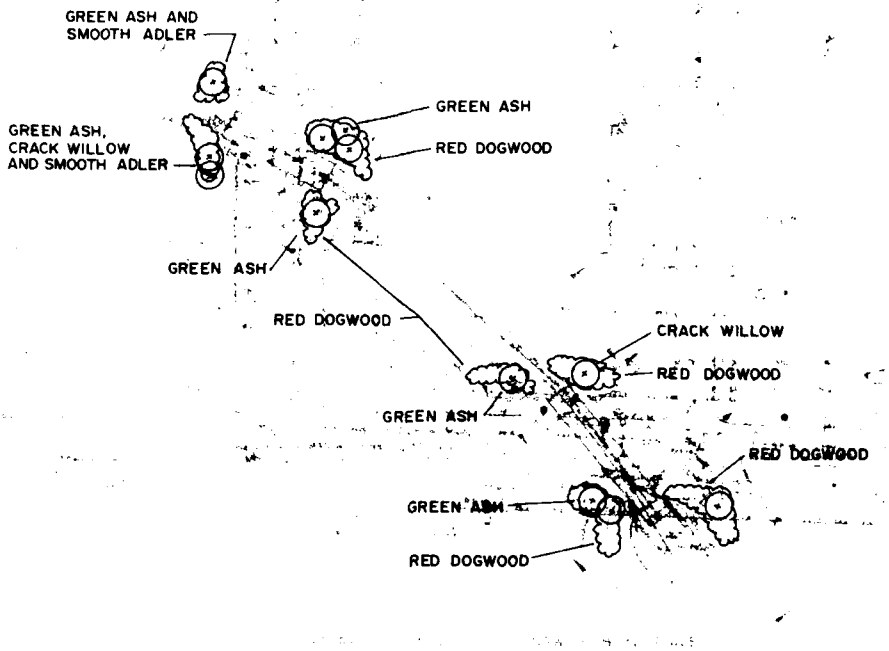

CRACK WILLOW
SMOOTH ALDER
HONEYSUCKLE AND DOGWOOD
CRACK WILLOW
HONEYSUCKLE AND DOGWOOD
SMOOTH ALDER
WEEPING WILLOW
HONEYLOCUST
HONEYSUCKLE AND DOGWOOD
CRACK WILLOW
SMOOTH ALDER
WHITE CEDAR
RED MAPLE


RED DOGWOOD
GREEN ASH
GREEN ASH
RED DOGWOOD
GREEN ASH
SUMAC
RED DOGWOOD
VIBURNUM
RIVER BIRCH
GREEN ASH
RED DOGWOOD
ISLAND
RIVER BIRCH
GREEN ASH
SUMA
GREEN ASH
RIVER BIRCH
POND
HONEYSUCKLE
RED MAPLE
GREEN ASH
GREEN ASH AND RIVER BIRCH
HONEYSUCKLE
RED DOGWOOD
RED MAPLE AND GREEN ASH
HONEYSUCKLE AND VIBURNUM

CRACK WILLOW

EYSUCKLE AND
WOOD

CUST



ASH

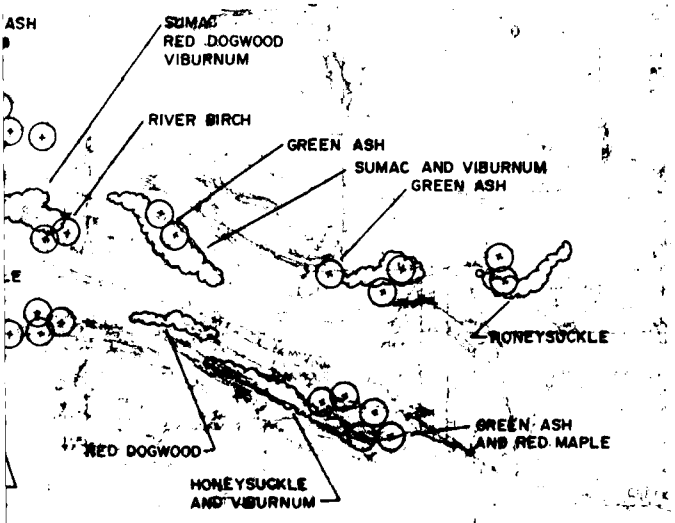


PLATE G-6

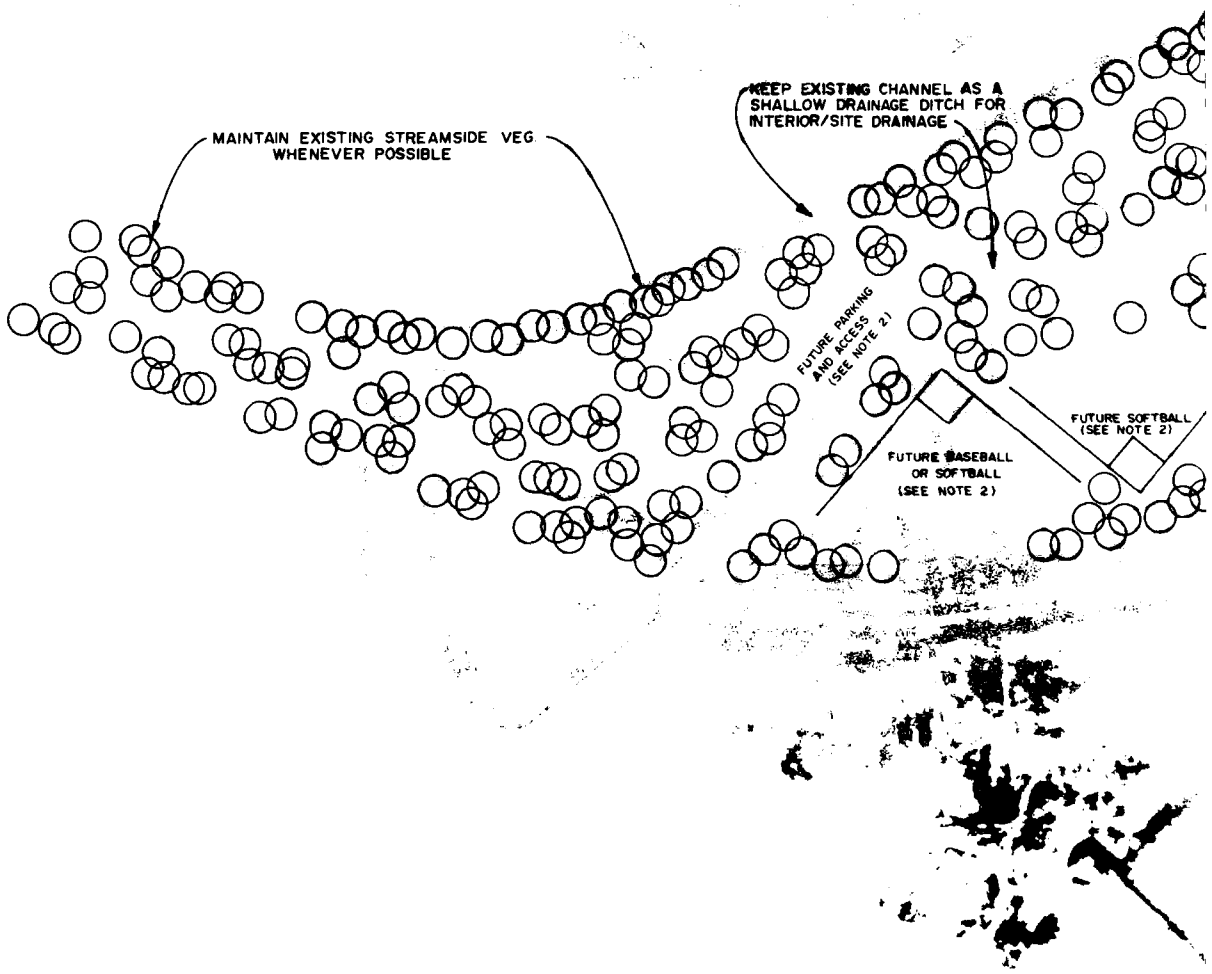
NOTES:

1. PLAN SHOWS ONLY NEW TREE PLANTINGS ACTUAL UNDERSTORY SHRUBS WILL ALSO BE INCLUDED AS PART OF THE FINAL PLANTING PLANS PREPARED FOR SPECIFICATIONS. PLANT SPECIES TO BE INCLUDED IN THIS AREA INCLUDE:

TREES

GREEN ASH	HACKBERRY	RED DOGWOOD	VIBURNUM
RED MAPLES	WILLOW	STAGHORN SUMAC	HAZEL NUT
RIVER BIRCH	SILVER MAPLE	AMERICAN PLUM	
RED CEDAR	WHITE CEDAR	HONEYSUCKLE	

2. ALL RECREATION FEATURES SHOWN ARE A FUTURE NON-FEDERAL RESPONSIBILITY (i.e. NO RECREATION IS INCLUDED AS A PART OF THIS PROJECT).



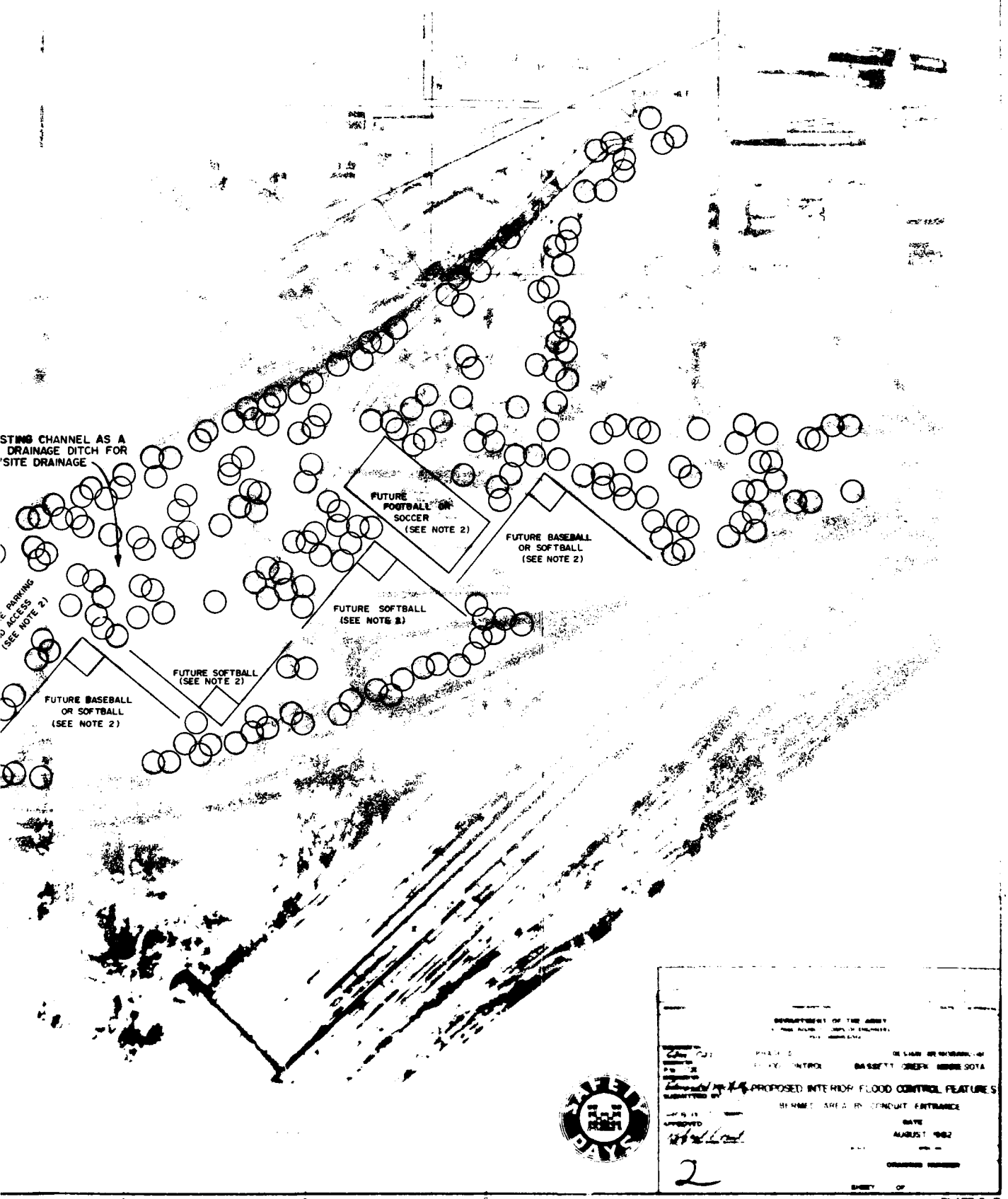

STING CHANNEL AS A
DRAINAGE DITCH FOR
'SITE DRAINAGE
FUTURE
FOOTBALL OR
SOCCER
(SEE NOTE 2)
FUTURE BASEBALL
OR SOFTBALL
(SEE NOTE 2)
FUTURE SOFTBALL
(SEE NOTE 2)
FUTURE SOFTBALL
(SEE NOTE 2)
FUTURE BASEBALL
OR SOFTBALL
(SEE NOTE 2)
PROPOSED INTERIOR FLOOD CONTROL FEATURES
AUGUST 1982
2

PLATE G-7

DEPARTMENT OF THE ARMY
St. Paul District Corps of Engineers
1135 U.S. Post Office & Custom House
St. Paul, Minnesota 55101

FLOOD CONTROL

BASSETT CREEK

HENNEPIN COUNTY, MINNESOTA

DESIGN MEMORANDUM NO. 2, PHASE II

APPENDIX H

FLOODPROOFING

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10-12	ANALYSIS OF THE PROBLEM - FIRST GROUP	H-2
13-14	SOIL CONDITIONS - FIRST GROUP	H-3
15	RECOMMENDATIONS - FIRST GROUP	H-3
16-17	ANALYSIS OF THE PROBLEM - SECOND GROUP	H-8
18-28	SOIL CONDITIONS - SECOND GROUP	H-8
29-39	RECOMMENDATIONS - SECOND GROUP	H-10

Number	Plates
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H-10 - H-18	2655 SCOTT AVENUE NORTH
H-19 - H-27	2665 SCOTT AVENUE NORTH
H-28 - H-36	2675 SCOTT AVENUE NORTH
H-37 - H-39	2685, 2725, 2735 SCOTT AVENUE NORTH
H-40, H-41	2755 SCOTT AVENUE NORTH AND 2775 REGENT AVENUE NORTH
H-42 - H-45	1205 IDAHO AVENUE NORTH

Number	Plates
H-46, H-47	2425, 2445 REGENT AVENUE NORTH
H-48, H-49	2505, 2545 REGENT AVENUE NORTH
H-50 - H-53	3810 BASSETT CREEK DRIVE
H-54 - H-56	3820 BASSETT CREEK DRIVE
H-57	3810 AND 3820 BASSETT CREEK DRIVE
H-58 - H-61	4975 BASSETT CREEK DRIVE
H-63 - H-65	5005 BASSETT CREEK DRIVE
H-66, H-67	5110, 5120, 5126 MINNAQUA DRIVE
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H-71, H-72	5222 MINNAQUA DRIVE
H-73 - H-78	5130 MINNAQUA AVENUE NORTH

APPENDIX H

FLOODPROOFING

GENERAL

1. The 1976 Feasibility Report noted that approximately 20 residential dwellings were located along Bassett Creek which could not be afforded 100 year flood protection by the major structural project features. It was determined that adverse environmental impacts were minimized and project economics were best served by providing a nonstructural floodproofing program for these residences.

2. As a result of post authorization hydrology and hydraulic design studies, it was determined that 24 houses along Bassett Creek would need protection against the 100-year event. These 24 homes form the project's nonstructural floodproofing program.

ALTERNATIVES CONSIDERED

3. In general, three basic alternatives for flood-proofing were considered for each of the 24 residences. The first alternative consists of construction of a low earth berm between the home and the creek to a flood protection elevation (Regulatory Flood Datum) 1-foot above the proposed 100-year flood level. An interior drainage system would have to be provided within the bermed area to handle local storm water runoff.

4. The second alternative considered consists of closing low openings and providing earth fill against the home foundation to prevent entry of flood waters to an elevation of 1-foot above the proposed 100-year flood level. With this alternative, a system of drain tile would be installed to prevent undesirable uplift and seepage pressures on the existing basement floor.

5. The third alternative basically consists of raising the home to 1-foot above the proposed 100-year flood level at its existing location and on its existing foundation. The home would be separated from its existing foundation and the existing foundation would be extended upward to the flood protection elevation and the house would be replaced on its new foundation.

6. In addition, variations of the second alternative were considered, including use of retaining walls around patio areas with interior drainage facilities, use of small landing wells at door openings to avoid losing direct access to the yard via the walk-out and the use of window wells around low windows to prevent blocking openings required to maintain legal emergency exits for living and sleeping quarters.

APPROACH TO THE PROBLEM

7. In 1979, under a Section 215 agreement (see Section 215, P.L. 90-483), Barr Engineering Co., for the city of Golden Valley, began design for the floodproofing program. Affected residents were informed that the city of Golden Valley had adopted a policy requiring each identified residence to conform to recently adopted flood plain management regulations. Non-conforming uses were no longer acceptable for residential structures.

8. On 29 November 1979, a public workshop meeting was held at the Golden Valley City Hall for the purpose of discussing the concept of flood-proofing and the proposed schedules for flood-proofing residential dwellings in the project area. Each of the affected homeowners was individually notified by mail in advance of the meeting and approximately half of the homeowners were able to attend the meeting. Following the November 29 meeting, a series of meetings were held with each homeowner. This series of meetings presented to each home-owner a reiteration of the material discussed on 29 November 1979, and discussed their particular situation and their particular home. The meetings were held in the homes requiring flood-proofing and, thus, an on-site inspection of each property was a part of each meeting. Photographs of each home were obtained to facilitate the design of the flood-proofing measures and to provide a record of the before condition.

9. Following the meeting with each individual homeowner, the needs of each home were evaluated based upon the economical and technical aspects of protecting that property. During this review period, the desires and concerns of each homeowner were also considered.

ANALYSIS OF THE PROBLEM - FIRST GROUP

10. The City of Golden Valley presented 4 separate design submittals analyzing the flooding problems of homes grouped by area or with similar problems.

11. The first design submittal discussed the concerns, needs and solutions for flood-proofing of four homes in the city of Golden Valley along the Main Stem of Bassett Creek. Due to the unique requirements of these 4 homes, the analysis, discussion and recommendations are being presented as the first floodproofing group of residences. These homes are located just east of T.H. 100 along Scott Avenue North. The addresses are:

2605 Scott Avenue North

2655 Scott Avenue North

2665 Scott Avenue North

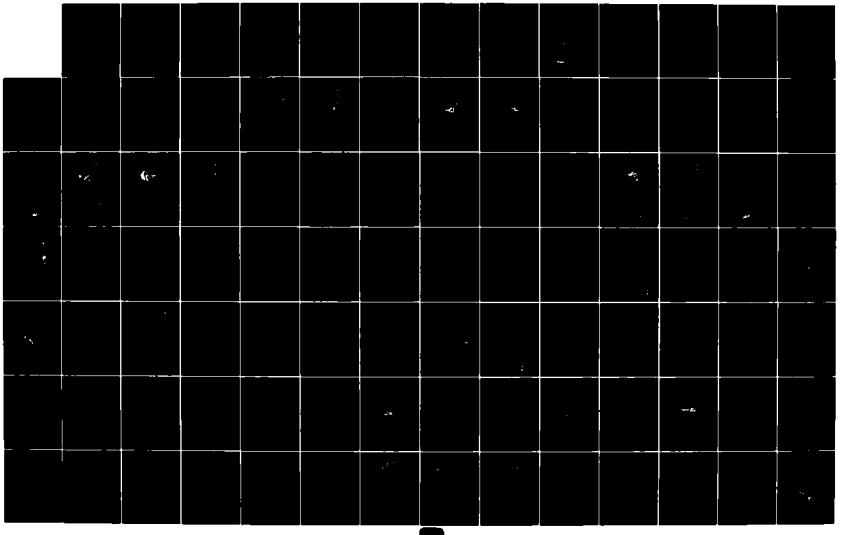
2675 Scott Avenue North

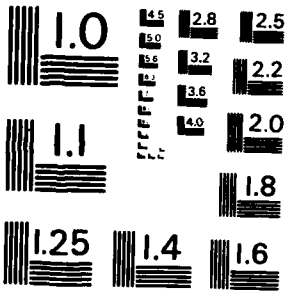
12. These adjoining homes are located just north of the Main Stem of Bassett Creek and each of the homes has been subject to flood damage in the past. The most recent occurrence of flood damage was in July, 1978, when these homes had from 6 inches to more than 2 feet of water in the lower levels of each home. The low openings in these homes are from 1 foot to approximately 2.5 feet below the proposed 100-year flood level in this area. The proposed 100-year flood level is approximately 2.5 to 3 feet below the existing 100-year flood level in this area. Two of the homes have built low level retaining walls around patio areas at the rear of the home in an effort to prevent frequent flood damage. The elevations of these retaining walls are inadequate to prevent flood damage by a flood approaching the 100-year flood event and are obviously structurally inadequate to provide long-term flood protection for the residences. The garage doors are all below the proposed 100-year flood level in this area and since Scott Avenue is inundated by backwater from the Main Stem of Bassett Creek, flood waters can enter the homes through the garage door and this has occurred in the past.

AD-A133 795 BASSETT CREEK WATERSHED HENNEPIN COUNTY MINNESOTA FEASIBILITY REPORT FOR FLOOD CONTROL MAIN REPORT(U) CORPS OF ENGINEERS ST PAUL MN ST PAUL DISTRICT SEP 82

6/9

UNCLASSIFIED F/G 13/2 NL





MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS - 1963 - A

SOIL CONDITIONS - FIRST GROUP

13. Each of the homes is constructed on piling, which are typically approximately 60 feet long. Copies of the piling layout and lot description were prepared. To facilitate technical review of the alternatives considered for each home, a series of hand auger soil borings were obtained in the vicinities of the four homes. These hand soil borings obtained a maximum depth of approximately 15 feet. For these four homes being considered, the soil borings uniformly reflected a thin topsoil layer, a layer of black peat and a layer of olive color muck containing organic debris and snail shells. Occasional thin sand lenses were evident in some of the soil borings near the surface and it is unknown if these are natural deposits or sand fill which remained after home construction.

14. Based upon the adverse soil conditions which exist in the area of the first four homes, it was determined that it would be impractical to attempt to build earth berms or retaining walls at these locations. It was also determined that the closing of low openings and the embankment of earth material against the home was impractical in these locations due to the adverse soils, the need for a concrete block basement wall, and the prospect of fairly large earth settlement in the future. Further, the individual homeowners in each case strongly desired to keep the walkout lower level.

RECOMMENDATIONS - FIRST GROUP

15. Based upon the above discussion, it is recommended that the four homes at 2605, 2655, 2665 and 2675 Scott Avenue North, in the city of Golden Valley, be raised to an elevation of not less than 1 foot above the proposed 100-year flood level in this area. With one exception, the elevation of the garage slabs would remain the same. The garages would be subject to inundation during a 100-year flood, however, the garages would be free-draining as the flood waters receded. At 2665 Scott, however, the existing garage slab is approximately 2 feet below street grade and would not be freedraining following a flood. It is, therefore, recommended that the garage slab be reconstructed at an elevation above street level as part of the raise. Modifications to each home are presented by address on pages H-4 through H-7.

BASSETT CREEK FLOODPROOFING
SPECIFIC RECOMMENDATIONS

THE BRECKENRIDGE RESIDENCE LOCATED AT 2605 SCOTT AVENUE NORTH, GOLDEN VALLEY, MINNESOTA: THE SOIL CONDITIONS AND TOPOGRAPHY WILL NOT ALLOW EITHER THE CONSTRUCTION OF A WALL OR A BERM AS A MEANS OF FLOODPROOFING THIS STRUCTURE. THE BEARING POTENTIAL OF THE SOIL REQUIRES PILING TO SUPPORT A WALL. THE LACK OF TOPOGRAPHY AND CLOSE PROXIMITY TO THE CREEK ELIMINATES BERMING AS A SOLUTION.

WE RECOMMEND RAISING THIS RESIDENCE AS A MEANS OF FLOODPROOFING. THE LEVEL OF THE LOWER LEVEL FLOOR IS 1.64 FT. BELOW THE ONE HUNDRED YEAR FLOOD LEVEL OF 843.2 FT. THE LOWER LEVEL OF THE HOME IS COMPLETELY FURNISHED WITH FIREPLACES ON EACH LEVEL. THE STRUCTURE WILL BE RAISED TO A LEVEL OF 844.75 FT. WHICH WILL RESULT IN A LEVEL ONE FOOT ABOVE THE ONE HUNDRED YEAR FLOOD LEVEL. THE GARAGE WILL REMAIN AT THE EXISTING LEVEL TO ELIMINATE THE NEED FOR THE CONSTRUCTION OF AN EXPENSIVE FLOOR SLAB.

WE ALSO RECOMMEND THE CONSTRUCTION OF A RIPRAP WALL ON THE BANK OF THE CREEK TO ELIMINATE EROSION OF THE SOIL AT THE BEND IN THE CREEK.

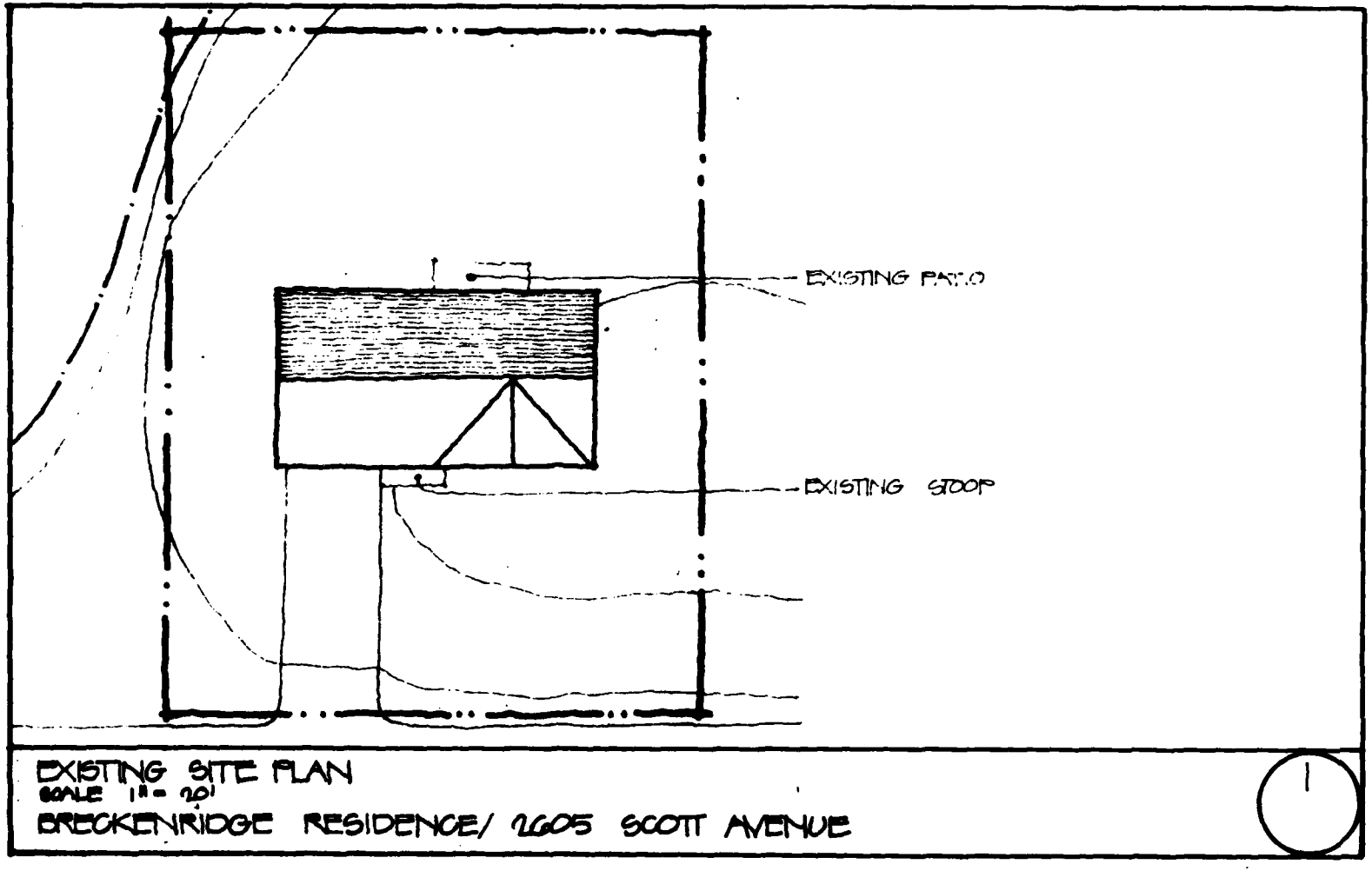
SEE PLATES H-1 THROUGH H-9.

SUMMARY OF ELEVATIONS:

BACK DOOR SILL	842.31
WINDOW SILL	845.50
FIRST FLOOR	850.48
SIDE DOOR	842.17
GARAGE DOOR	842.11

CONSTRUCTION ESTIMATE:
2605 SCOTT AVENUE NORTH, GOLDEN VALLEY, MINNESOTA

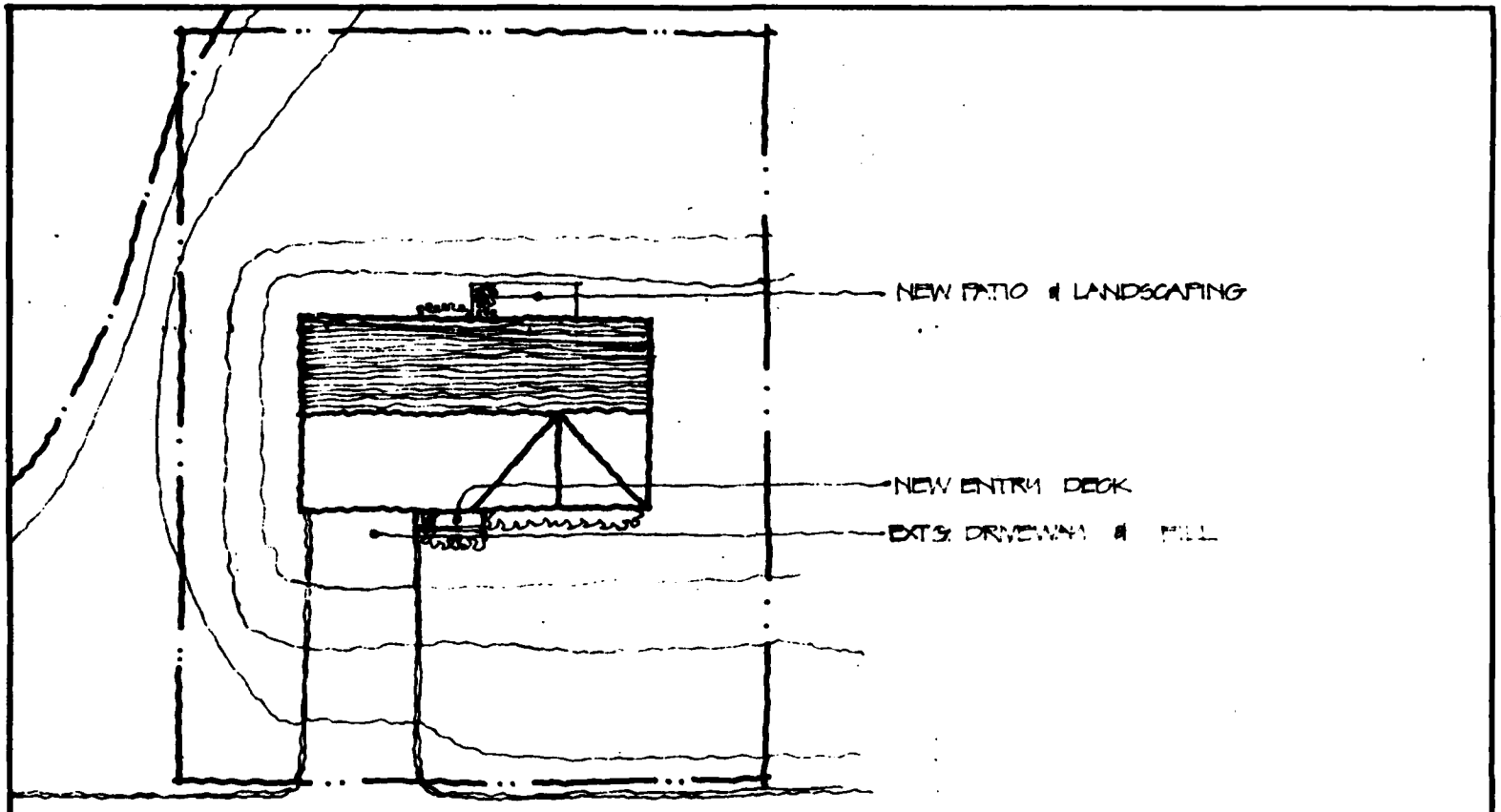
CARPENTRY	$20,100.00
MASONRY	
BLOCK	4,000.00
ELECTRICAL	700.00
PLUMBING	3,400.00
HVAC	2,700.00
HOME RAISE	10,100.00
CRIBBING AT EMBANKMENT	2,000.00
PERMITS & PLAN CHECK	300.00
	100.00
TOTAL ESTIMATE	$43,400.00



EXISTING SITE PLAN
SCALE 1" = 20'
BRECKENRIDGE RESIDENCE/ 2605 SCOTT AVENUE

1

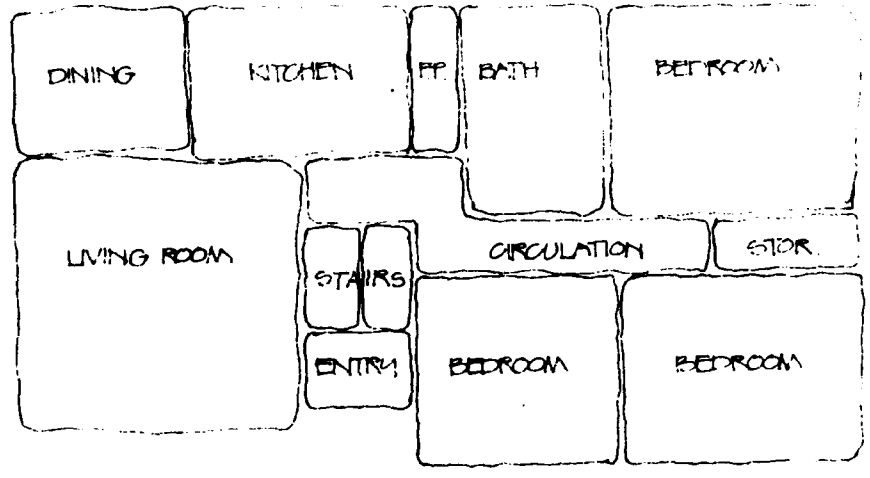
PLATE H-1



PROPOSED SITE PLAN
SCALE 1"=10'
BRECKENRIDGE RESIDENCE / 1605 SCOTT AVENUE

2

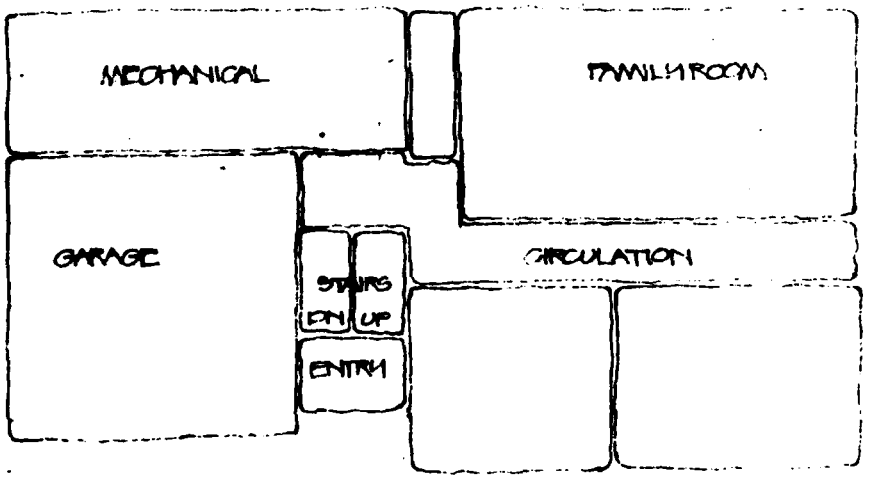
PLATE 4-2



SCHEMATIC FLOOR PLAN / UPPER LEVEL
SCALE: 1/8" = 1'-0"
BRECKENRIDGE RESIDENCE/2608 SCOTT AVENUE

3

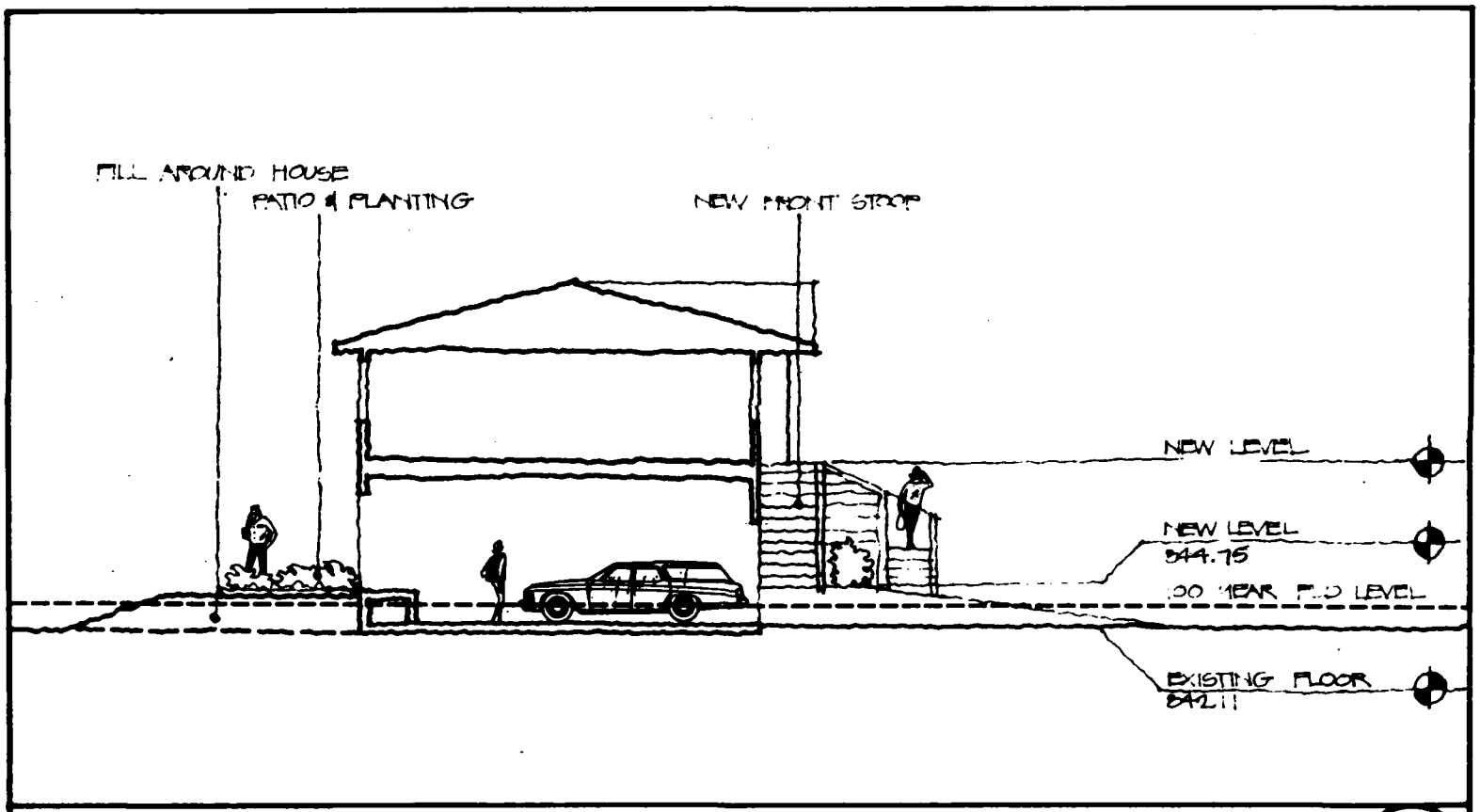
PLATE H-3



SCHEMATIC FLOOR PLAN / LOWER LEVEL
SCALE: 1/8" = 1'-0"
BRECKENRIDGE RESIDENCE / 1608 SCOTT AVENUE

4

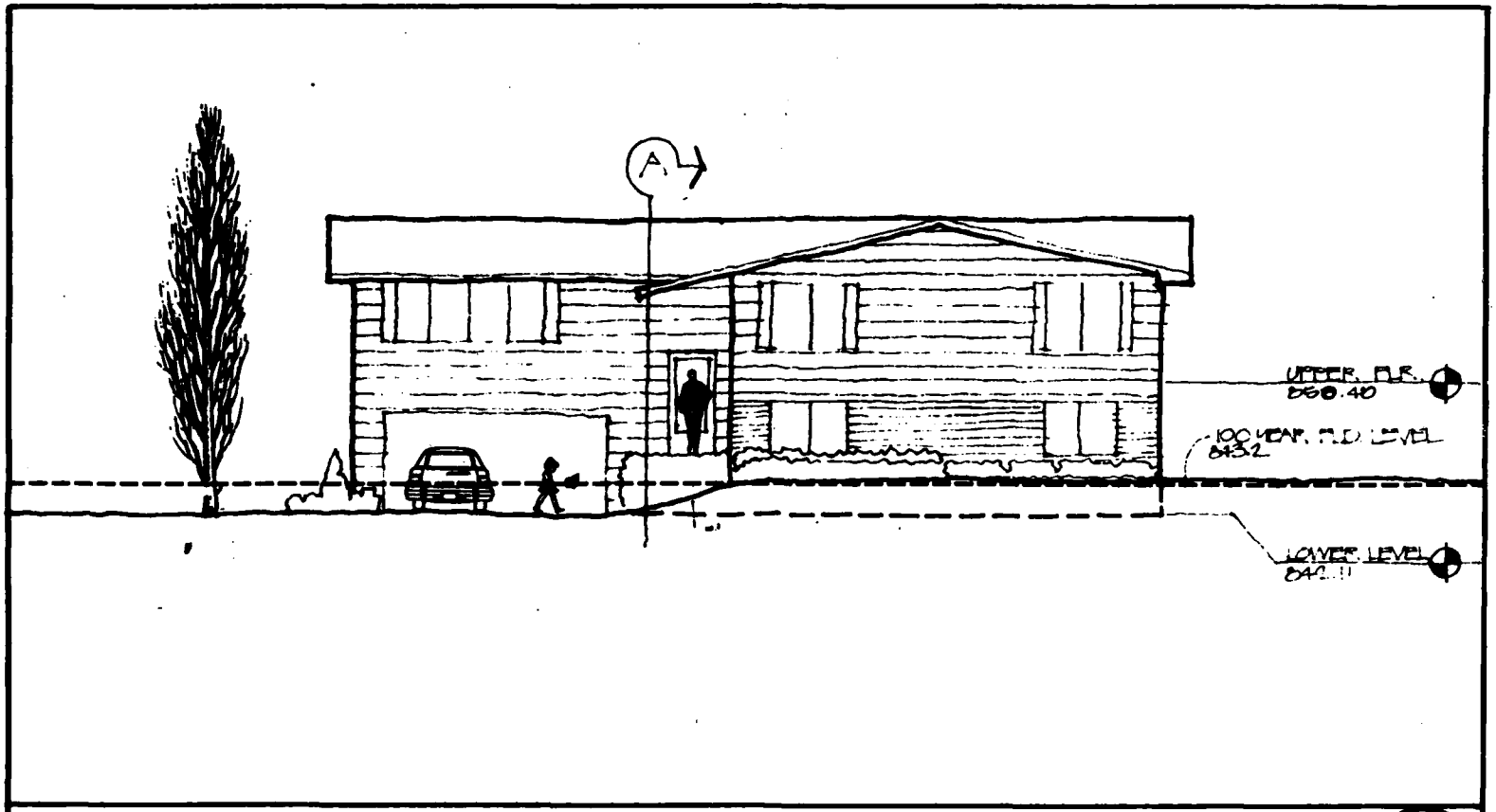
PLATE A-4



PROPOSED SECTION
SCALE 1/8"=1'-0"
BRECKENRIDGE/RESIDENCE 2605 SCOTT AVENUE

5

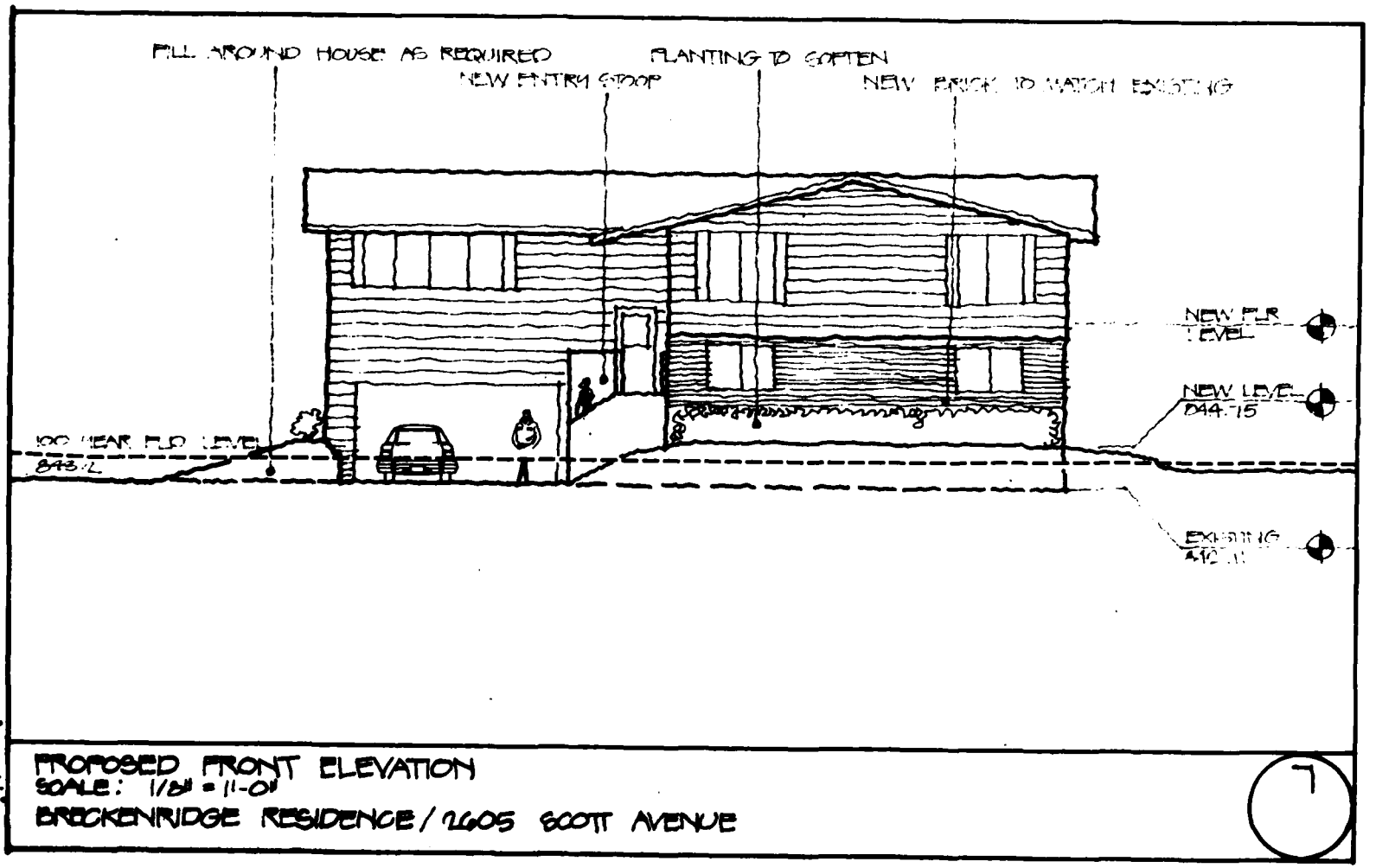
PLATE H-5



EXISTING FRONT ELEVATION
SCALE : 1/8" = 1'-0"
BRECKENRIDGE RESIDENCE/ 2605 SCOTT AVENUE

6

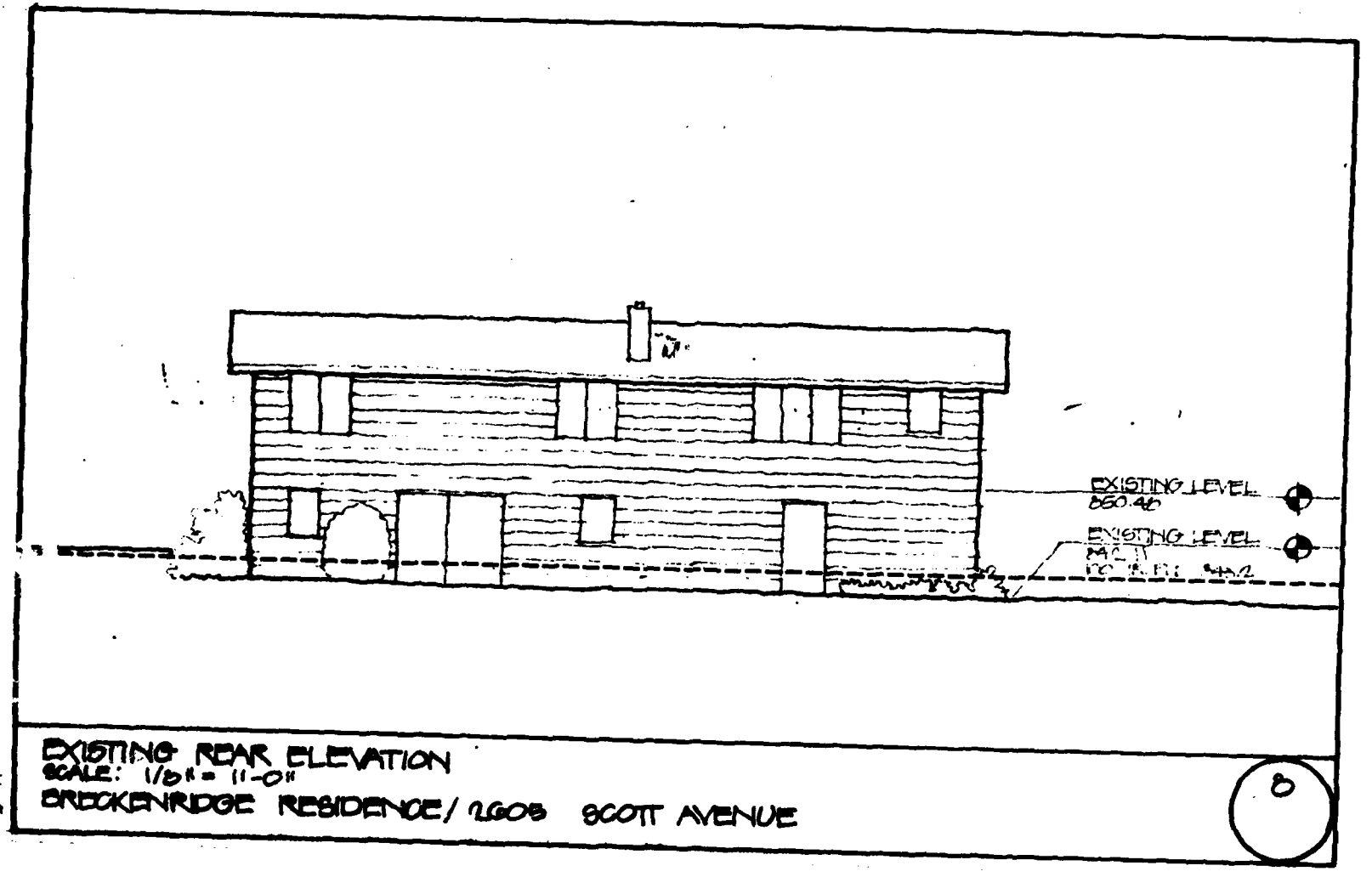
PLATE H-6



PROPOSED FRONT ELEVATION
SCALE: 1/8" = 1'-0"
BRECKENRIDGE RESIDENCE / 2605 SCOTT AVENUE

7

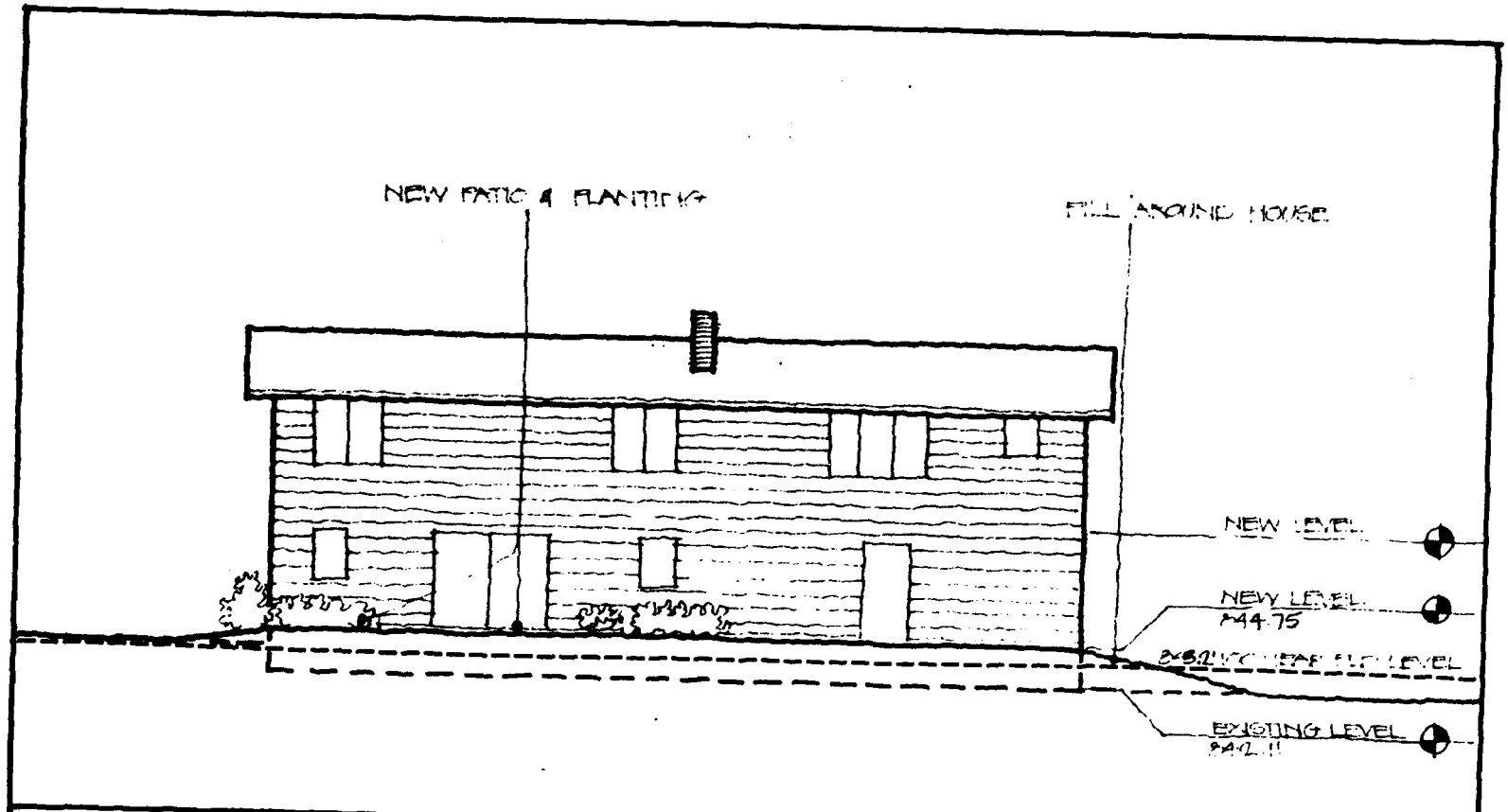
PLATE H-7



EXISTING REAR ELEVATION
SCALE: 1/8" = 1'-0"
BRECKENRIDGE RESIDENCE / 2605 SCOTT AVENUE

8

PLATE NO. 1



PROPOSED REAR ELEVATION
SCALE: 1/8" = 1'-0"
BRECKENRIDGE RESIDENCE / SCOTT AVENUE

9

PLATE H-9

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE MCGOWAN RESIDENCE LOCATED AT 2655 SCOTT AVENUE NORTH, GOLDEN VALLEY, MINNESOTA: THE MCGOWANS HAVE EXPERIENCED FLOODING MOST RECENTLY IN THE 1976 FLOOD. PHOTOS OF THE FLOOD OF 1976 WERE TAKEN BY THE MCGOWANS. THE FLOOD WATERS ENTERED THE HOME THROUGH THE GARAGE. THE RESIDENCE IS 1.64 FT. BELOW THE ONE HUNDRED YEAR FLOOD LEVEL. THE GROUND FLOOR OF THE HOME IS ON THE SAME LEVEL AS THE GARAGE FLOOR.

THE RECOMMENDATION OF THIS STUDY IS THAT THE HOME BE RAISED TO A LEVEL OF ONE FOOT ABOVE THE ONE HUNDRED YEAR FLOOD LEVEL. WE RECOMMEND THE HOME BE RAISED, BUT THAT THE GARAGE REMAIN AT THE EXISTING LEVEL. THIS IS DUE TO THE POOR SOIL CONDITIONS WHICH INCREASE THE PROBLEM OF RAISING THE GARAGE FLOOR. WE FEEL THAT THE PILINGS WOULD PUSH THROUGH THE GARAGE FLOOR UNLESS WE SIGNIFICANTLY INCREASED THE STRUCTURAL CAPACITY OF THE FLOOR. THIS WOULD BE COST PROHIBITIVE AND NOT NECESSARY TO THE FLOOD-PROOFING OF THE RESIDENCE. THE GARAGE WILL NECESSARILY FLOOD DURING HIGH WATER CONDITIONS.

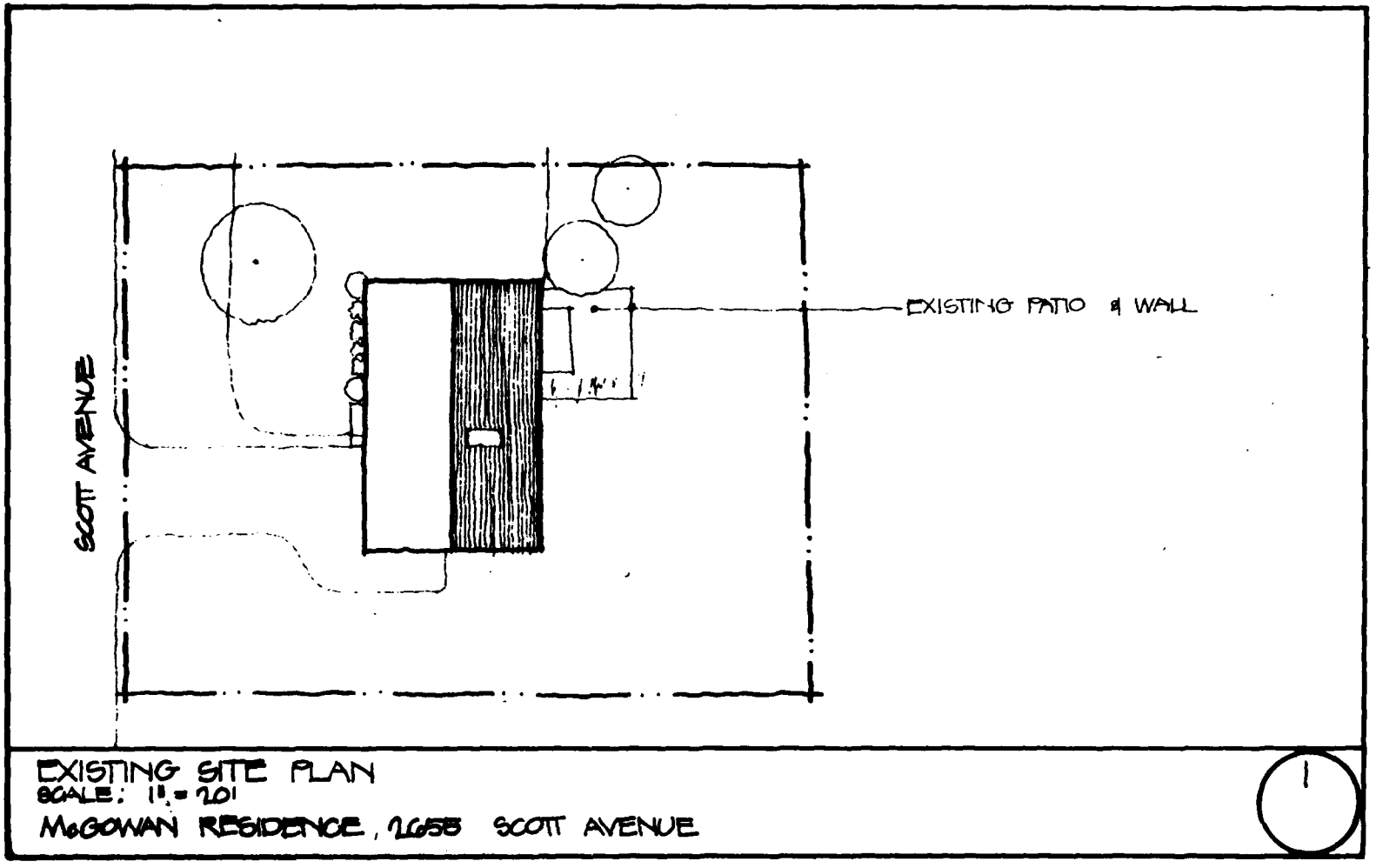
SEE PLATES H-10 THROUGH H-18.

SUMMARY OF ELEVATIONS:

SLIDING DOOR @ PATIO	842.12
WINDOW SILL	845.65
PATIO WALL	842.16
DOOR SILL	842.28
SIDE DOOR GARAGE	842.08
FIRST FLOOR	850.08

CONSTRUCTION ESTIMATE:
2655 SCOTT AVENUE NORTH, GOLDEN VALLEY, MINNESOTA

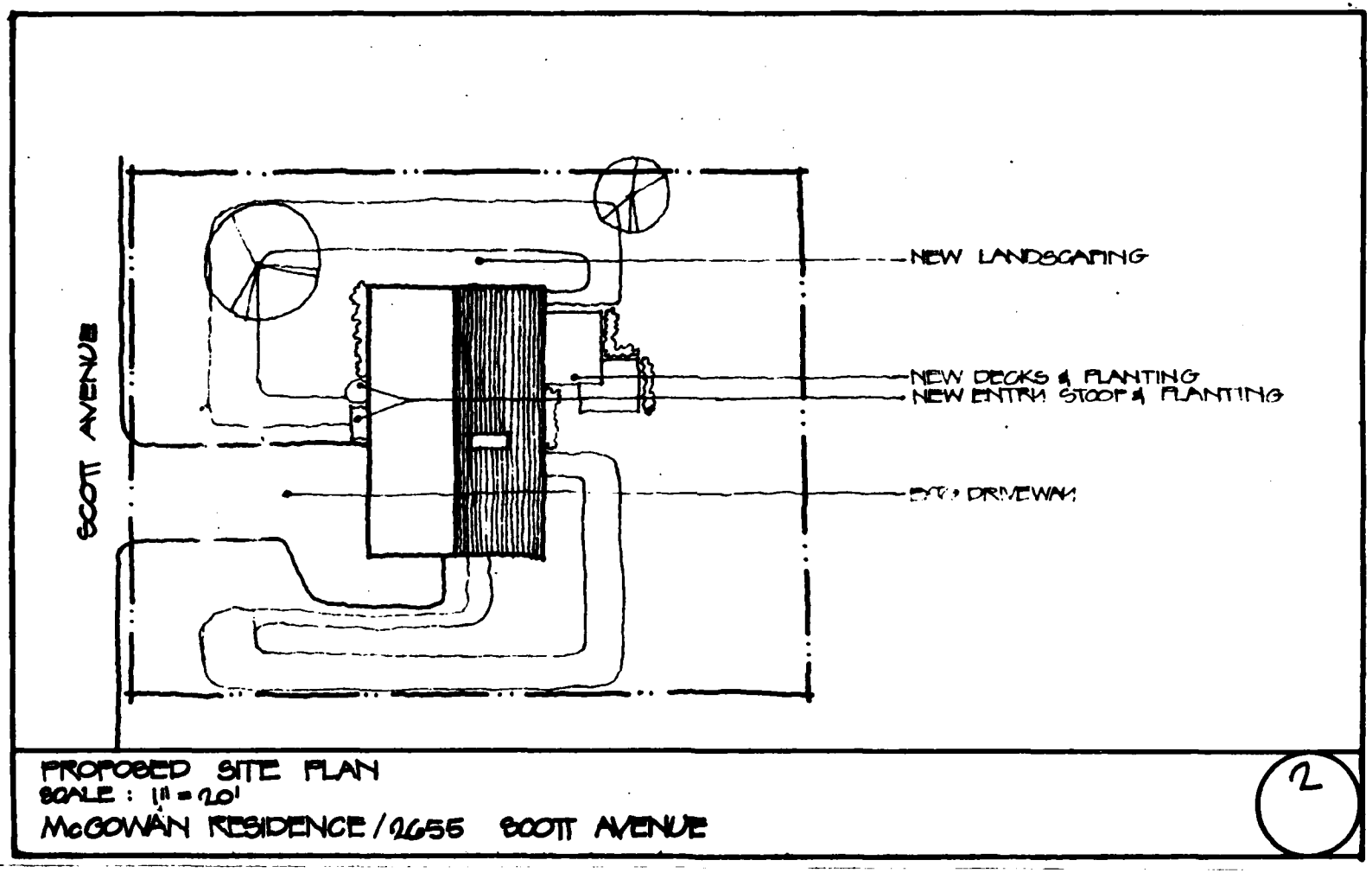
CARPENTRY	$20,000.00
MASONRY	
BLOCK	4,000.00
ELECTRICAL	500.00
PLUMBING	3,800.00
HVAC	2,700.00
HOME RAISE	10,100.00
PERMITS & PLAN CHECK	300.00
	100.00
TOTAL ESTIMATE	$41,500.00



EXISTING SITE PLAN
SCALE: 1" = 20'
McGOWAN RESIDENCE, 2655 SCOTT AVENUE

1

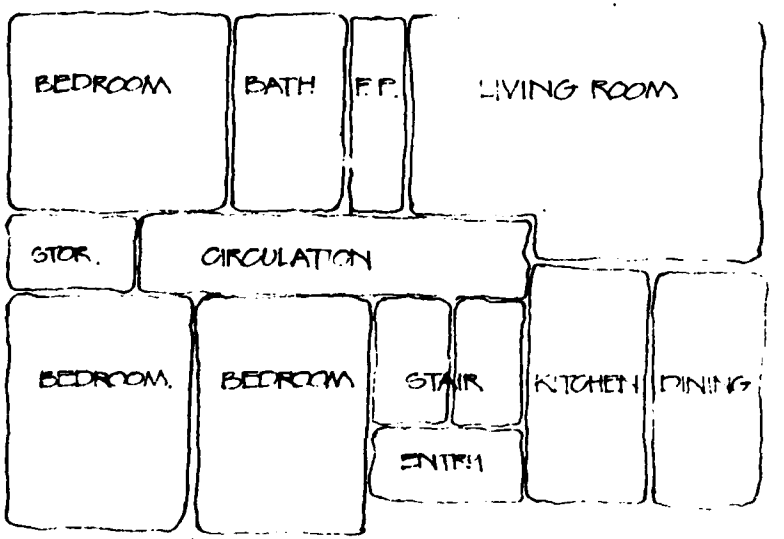
PLATE No. 1



PROPOSED SITE PLAN
SCALE : 1" = 20'
McGOWAN RESIDENCE / 2655 SCOTT AVENUE

2

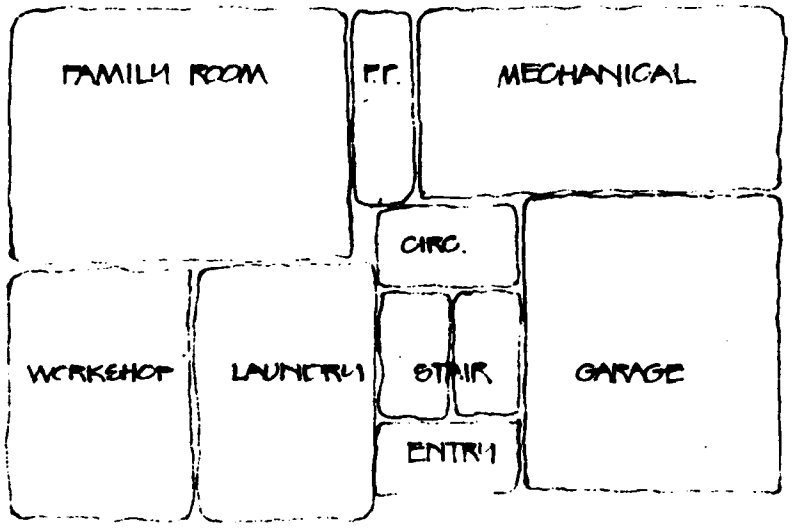
PLATE H-11



SCHEMATIC FLOOR PLAN / UPPER LEVEL
SCALE: 1/8" = 1'-0"
McGOWAN RESIDENCE, 1655 SCOTT AVENUE

3

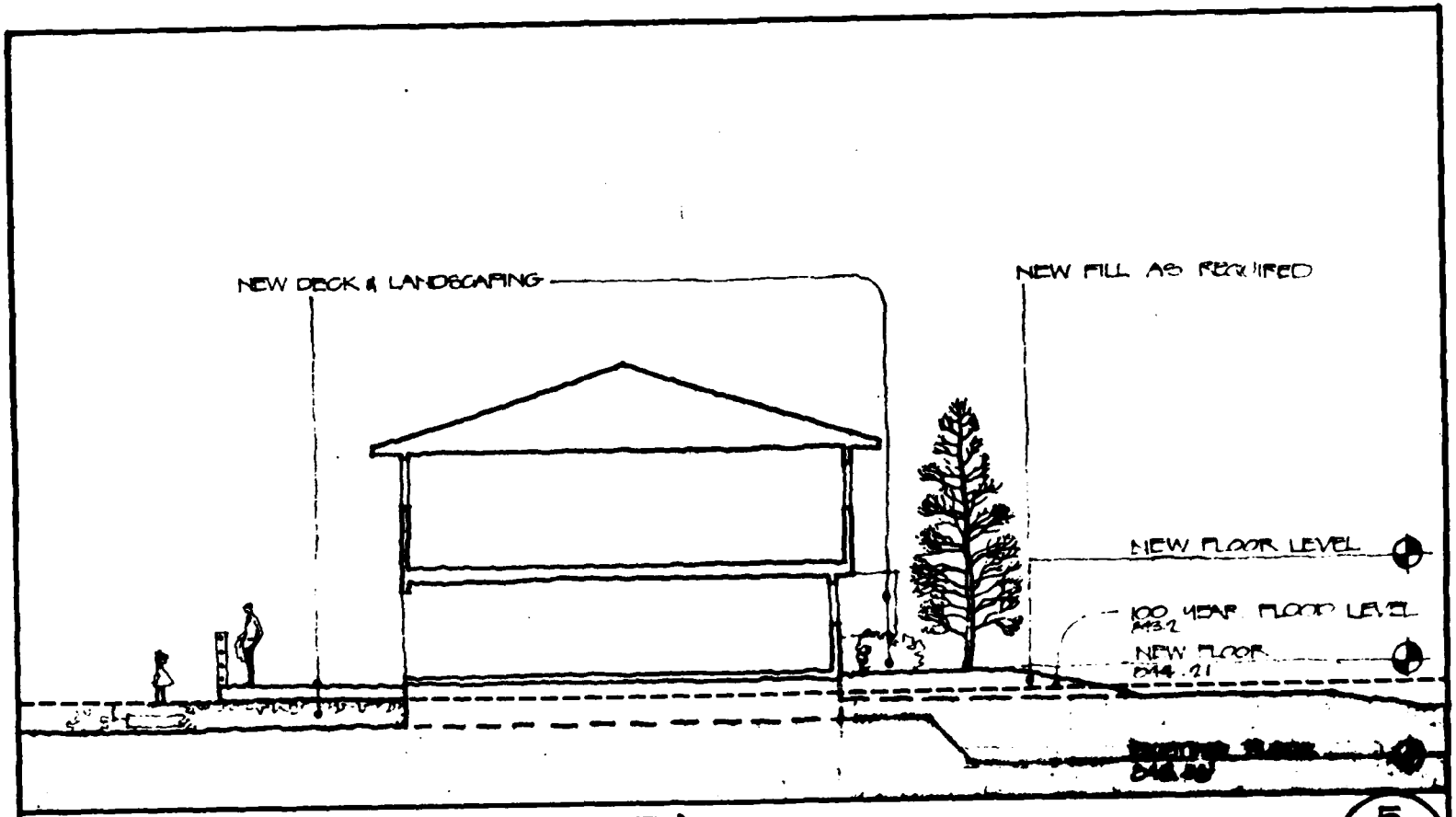
PLATE NO. 2



SCHEMATIC FLOOR PLAN / LOWER LEVEL
SCALE: 1/8" = 1'-0"
McGOWAN RESIDENCE, 1688 SCOTT AVENUE

4

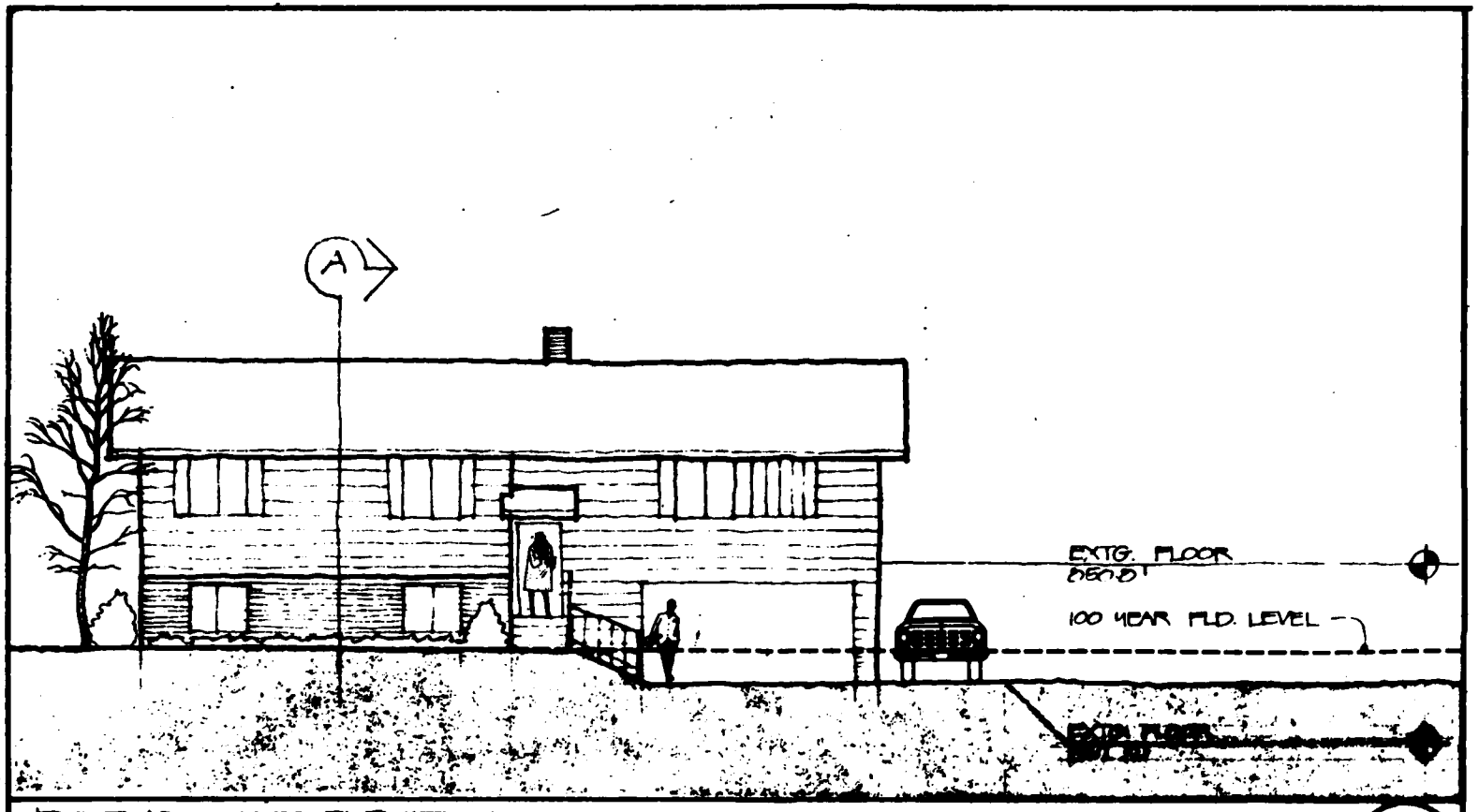
PLATE H-8-9



SCHEMATIC SECTION A/A (PROPOSED)
SCALE: 1/8" = 1'-0"
McGOWAN RESIDENCE / 2655 SCOTT AVENUE

5

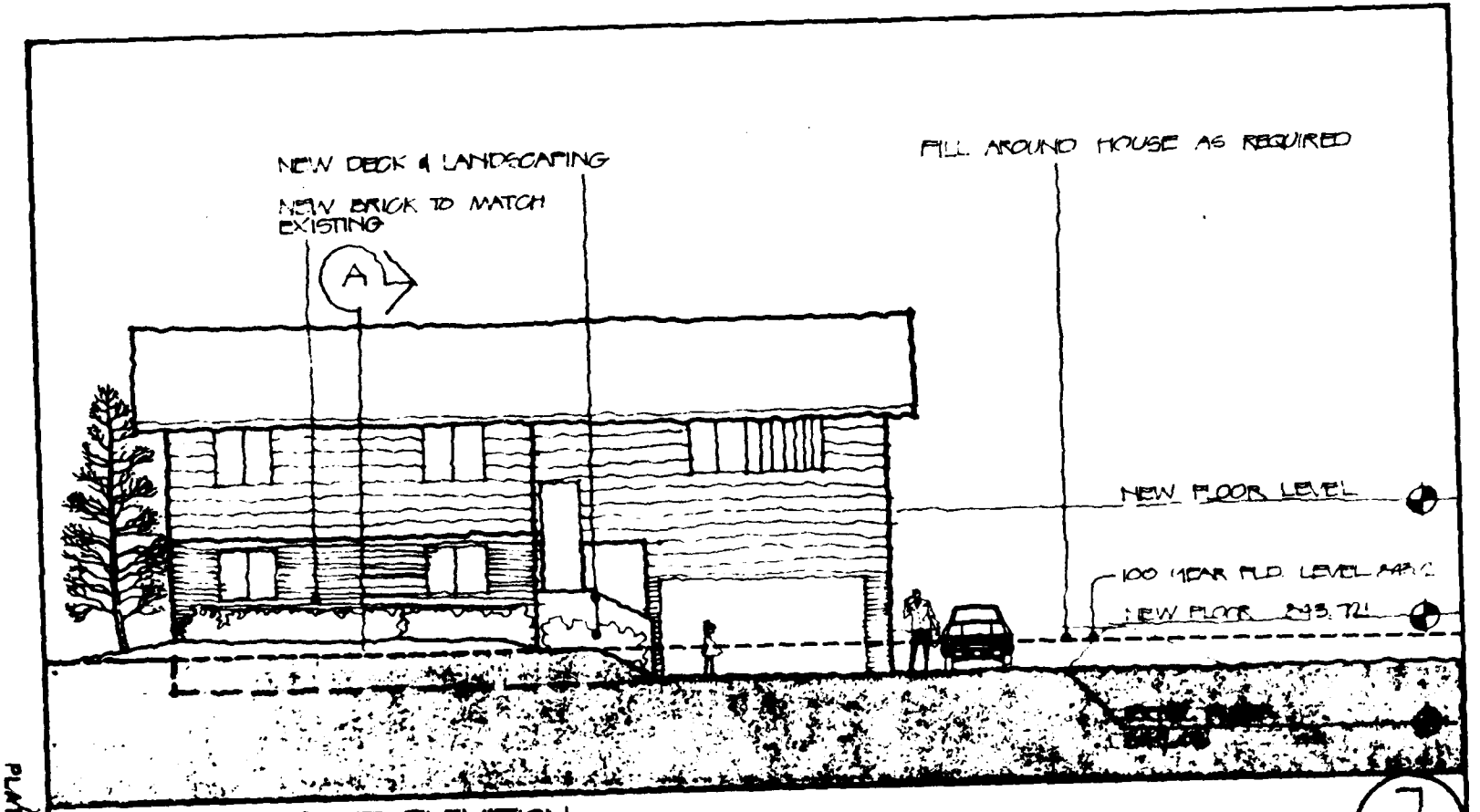
PLATE H-1



EXISTING FRONT ELEVATION
SCALE: 1/8"=1'-0"
McGOWAN RESIDENCE . 1655 SCOTT AVENUE

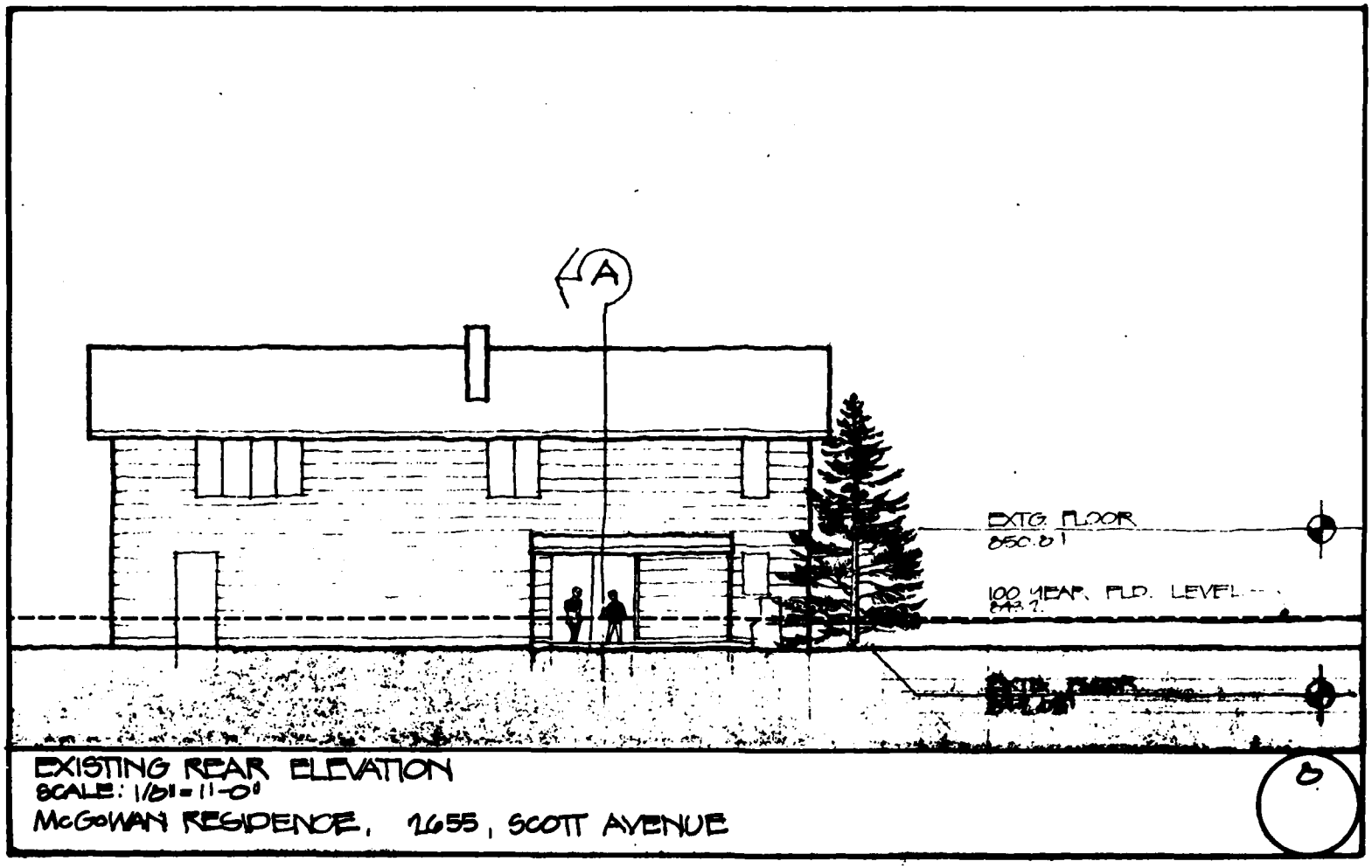
G

PLATE H-1.5



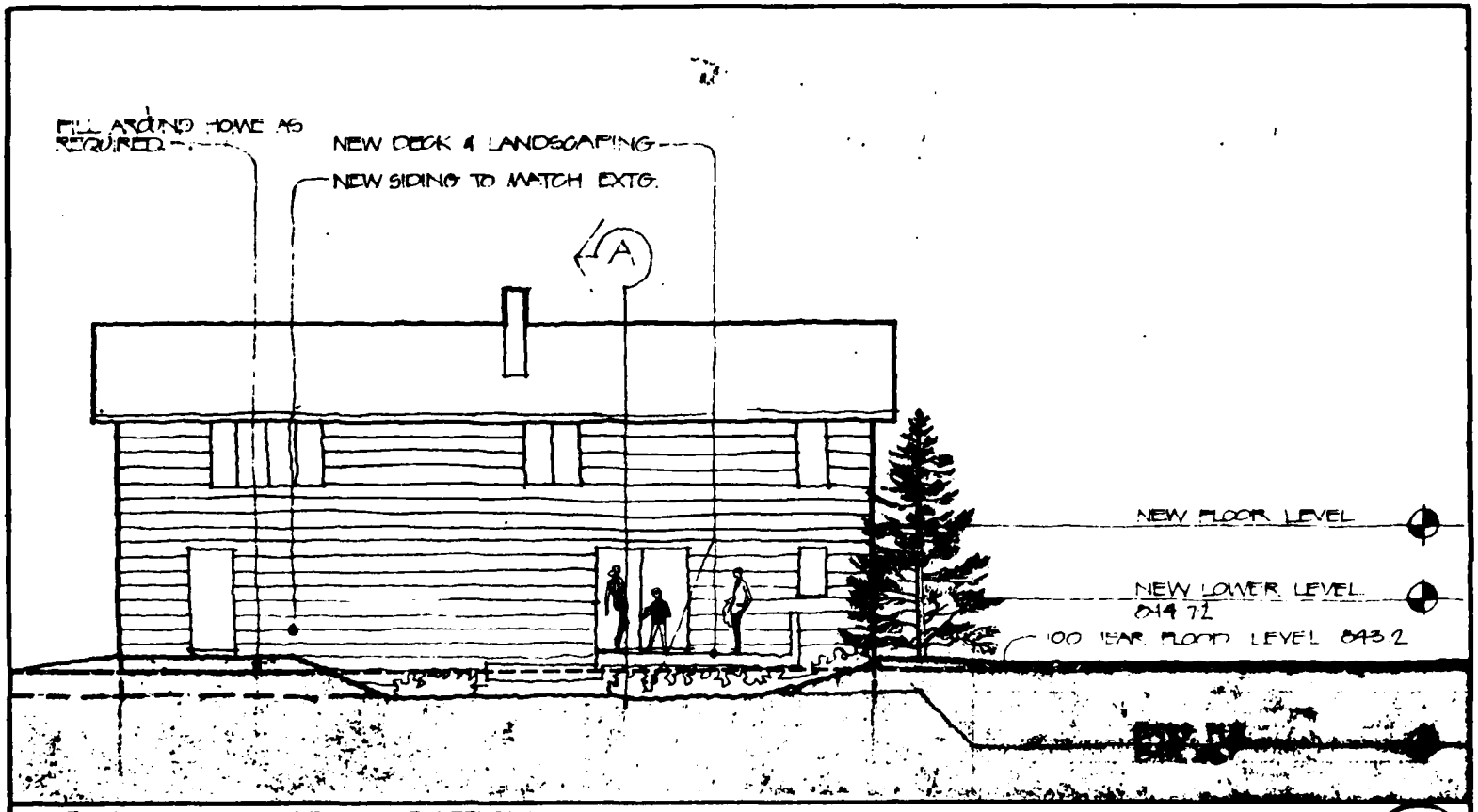
PROPOSED FRONT ELEVATION
SCALE: 1/8" = 1'-0"
McGOWAN RESIDENCE / 2655 SCOTT AVENUE

PLATE NO. 12



EXISTING REAR ELEVATION
SCALE: 1/8" = 1'-0"
McGOWAN RESIDENCE, 2655, SCOTT AVENUE

PLATE H-11-2



PROPOSED REAR ELEVATION
SCALE: 1/8"=1'-0"
McGOWAN RESIDENCE / 2655 SCOTT AVENUE

9

PLATE H-18

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE GETZKIN RESIDENCE LOCATED AT 2665 SCOTT AVENUE NORTH, GOLDEN VALLEY, MINNESOTA: THE SOIL CONDITIONS AND THE TOPOGRAPHY WILL NOT ALLOW EITHER THE CONSTRUCTION OF A WALL OR BERMING AS A SOLUTION. THE BEARING POTENTIAL IS SUCH THAT PILING WOULD BE REQUIRED TO SUPPORT A WALL. SINCE THE WATER ENTERS THE HOME ON TWO SIDES THERE IS NO TOPOGRAPHY TO USE IN AN EFFORT TO BLEND A BERM INTO THE SURROUNDING LANDSCAPE.

IT IS RECOMMENDED THAT THE RESIDENCE BE RAISED. THE LEVEL OF THE BASEMENT FLOOR IS 841.15 FT. THE ELEVATION OF THE ONE HUNDRED YEAR FLOOD LEVEL IS 844.09 FT. THE RESIDENT HAS AN ELABORATE BRICK FIREPLACE ON EACH LEVEL WHICH STACK VERTICALLY. THE LOWER LEVEL IS COMPLETELY FINISHED. THE LOWER LEVEL OF THIS HOME INCLUDING THE GARAGE IS 3.25 FT. LOWER THAN THE ONE HUNDRED YEAR FLOOD LEVEL. CONSEQUENTLY, THE MAJORITY OF THE WATER DAMAGE OCCURS FROM THE STREET SIDE. APPROXIMATELY 3 FT. OF WATER STOOD IN THE BASEMENT IN THE 1978 FLOOD. WE RECOMMEND THAT THE ENTIRE HOME BE RAISED TO A LEVEL OF 845.45 FT. WHICH WOULD THEN BE ONE FOOT ABOVE THE ONE HUNDRED YEAR FLOOD LEVEL.

WE RECOMMEND THAT THE GARAGE SHOULD BE INCLUDED WITHIN THE RAISE DUE TO ITS RELATION TO THE STREET ELEVATION. THIS WILL REQUIRE SOME ADDITIONAL STRUCTURAL MEASURES WITHIN THE FLOOR SLAB TO THE SUPPORT THE FLOOR. IT WILL BE NECESSARY TO REINFORCE THE SLAB OR CONSTRUCT CONCRETE BEAMS BETWEEN THE PILINGS TO SUPPORT THE NEW SLAB AND KEEP THE PILINGS FROM PUSHING THROUGH THE FLOOR.

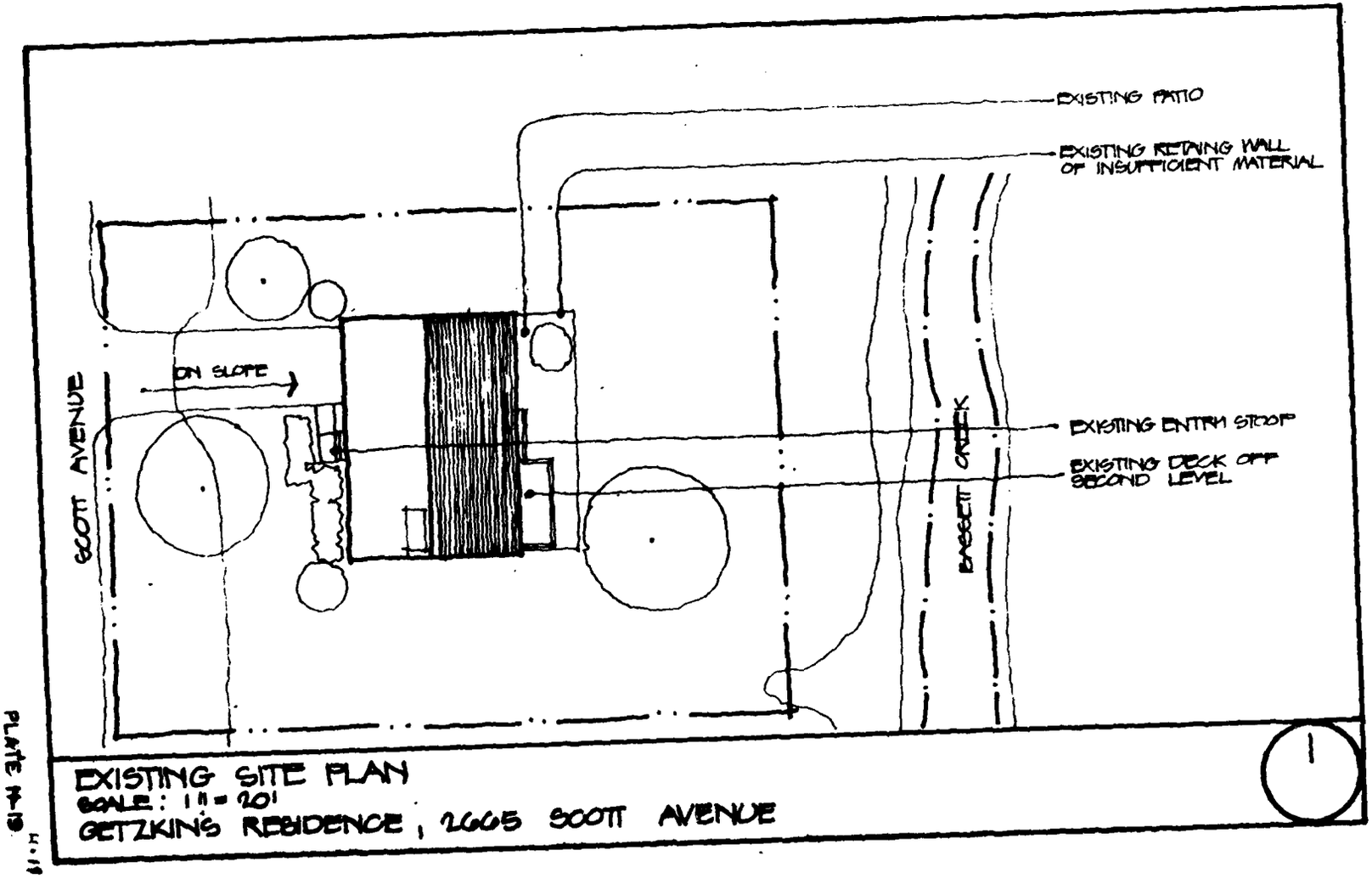
SEE PLATES H-19 THROUGH H-27.

SUMMARY OF ELEVATIONS:

SIDE DOOR	841.15
DOOR @ DECK	849.63
WINDOW SILL	844.68
DOOR SILL	841.21
WALL	842.72

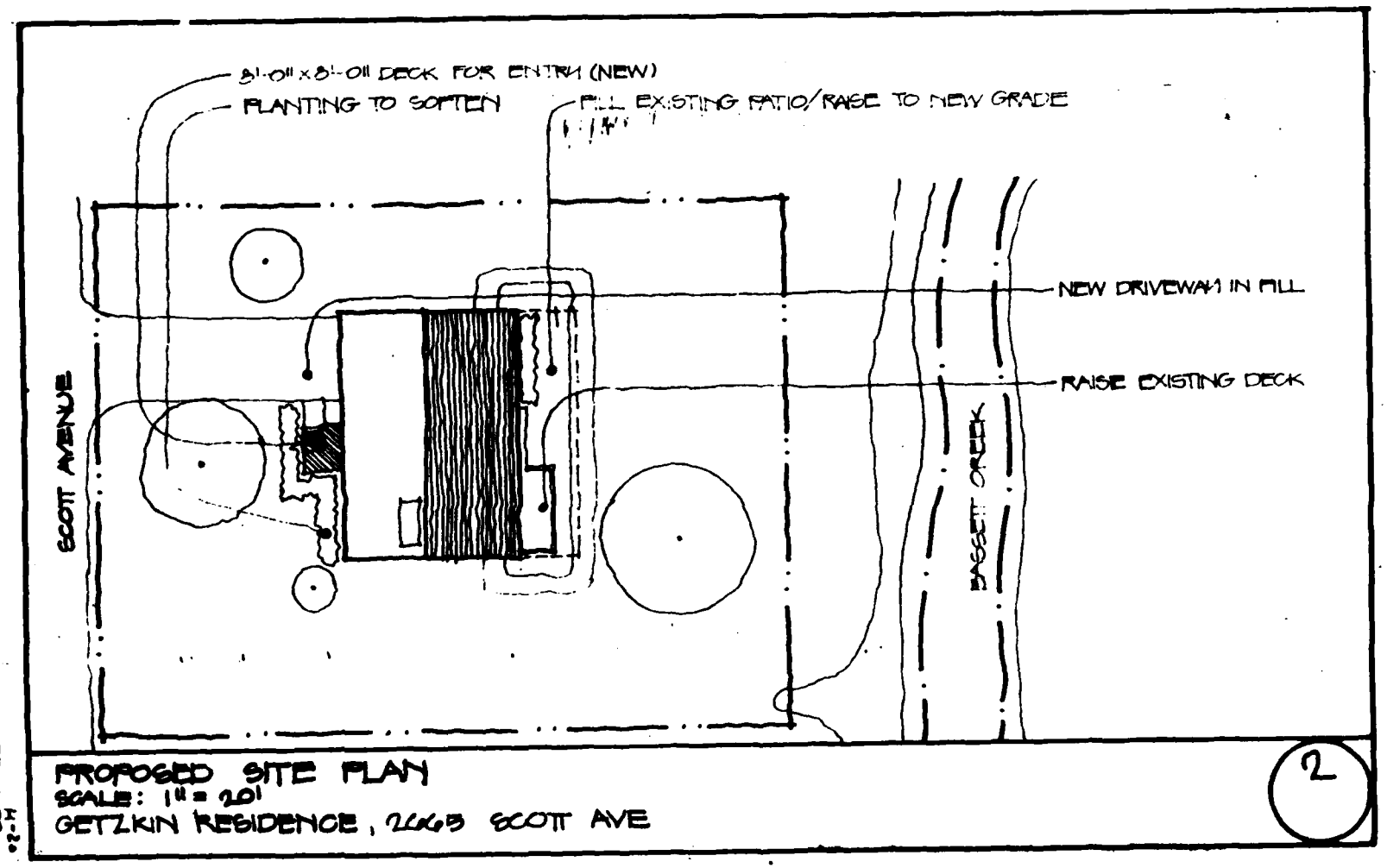
CONSTRUCTION ESTIMATE:
2665 SCOTT AVENUE NORTH, GOLDEN VALLEY, MINNESOTA

CARPENTRY	$22,700.00
MASONRY	
UNDER PIN WALLS	4,000.00
GARAGE GRADE BEAMS @ 8.00 SQ FT	1,300.00
NEW GARAGE FLOOR @ 1.40 SQ FT	1,100.00
ELECTRICAL	700.00
PLUMBING	3,800.00
HVAC	2,700.00
HOME RAISE	10,100.00
PERMITS & PLAN CHECK	300.00
	100.00
TOTAL ESTIMATE:	$46,800.00



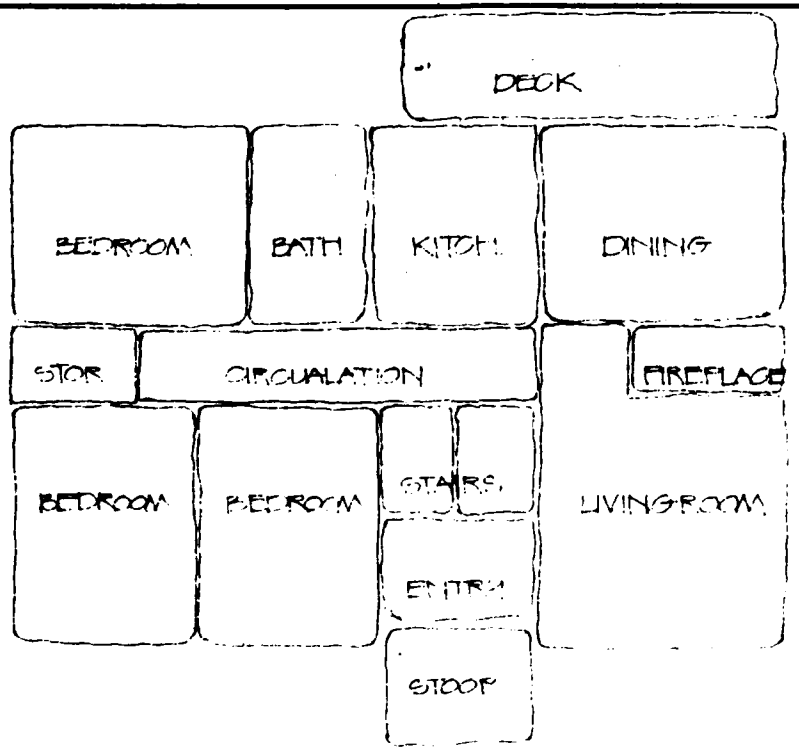
EXISTING SITE PLAN
SCALE: 1" = 20'
GETZKINS RESIDENCE, 2605 SCOTT AVENUE

PLATE #18



PROPOSED SITE PLAN
SCALE: 1" = 20'
GETZKIN RESIDENCE, 2663 SCOTT AVE

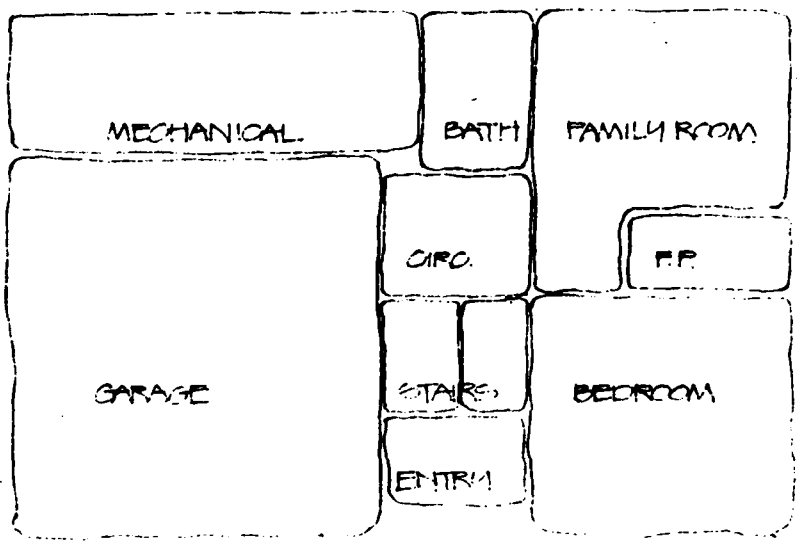
2



SCHEMATIC FLOOR PLAN / UPPER LEVEL
SCALE: 1/8"=1'-0"
GETZKIN RESIDENCE, 2663 SCOTT AVENUE

3

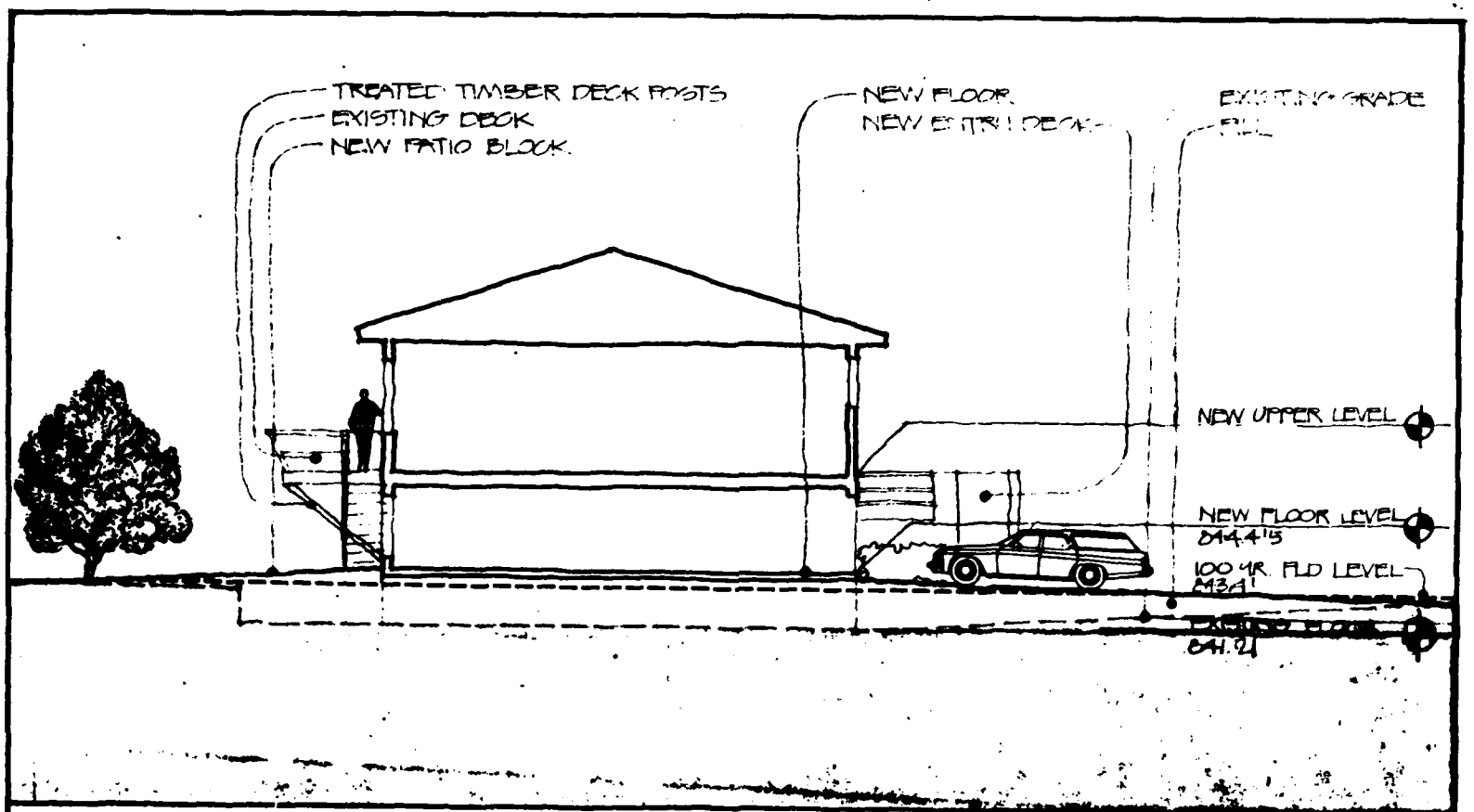
PLATE H-21



SCHEMATIC FLOOR PLAN / LOWER LEVEL
SCALE: 1/8" = 1'-0"
GETZKIN RESIDENCE, 2665 SCOTT AVENUE

4

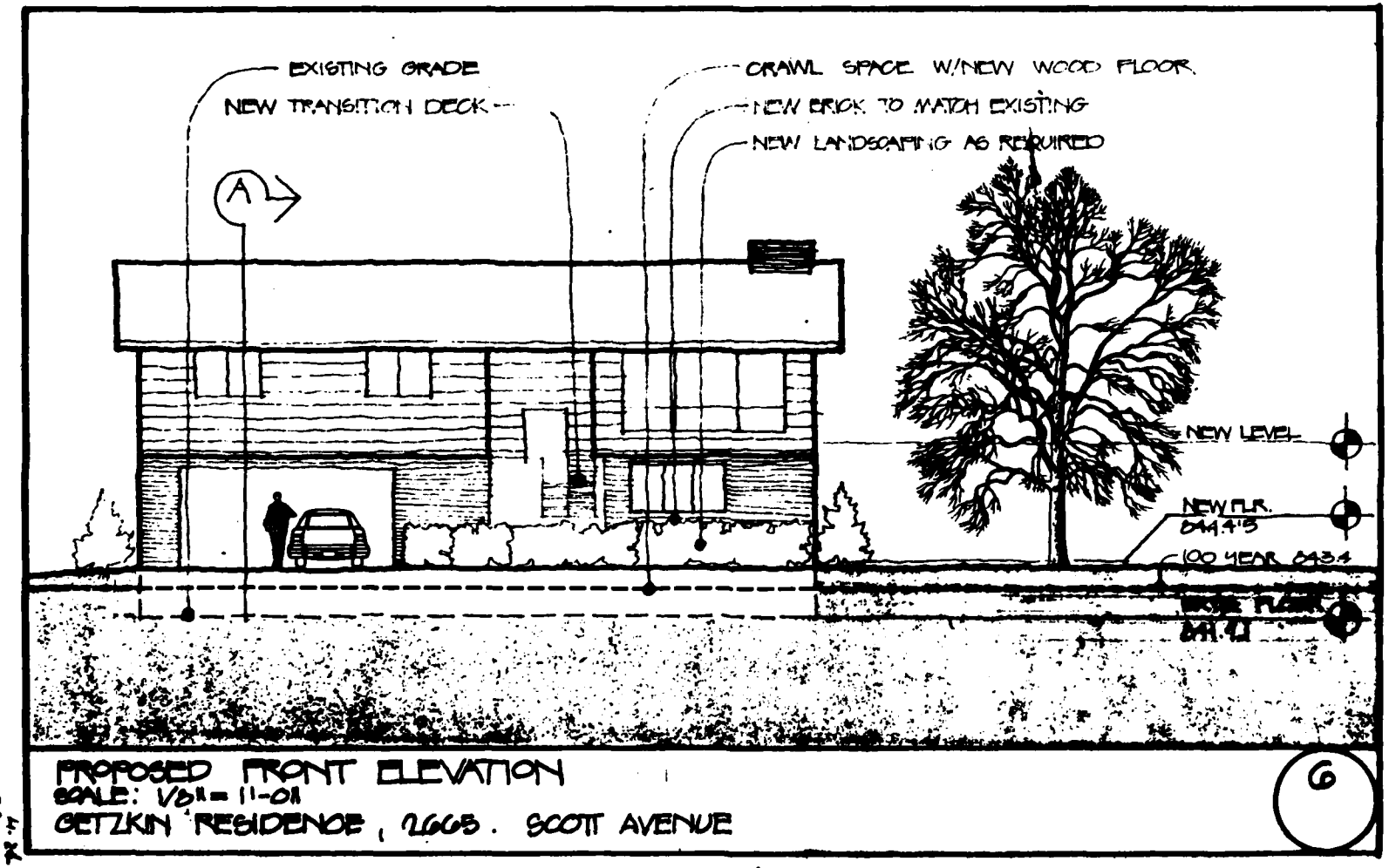
PLATE #2



SCHEMATIC SECTION A/A (PROPOSED)
SCALE : 1/8" = 1'-0"
GETZKIN RESIDENCE 2665, SCOTT AVENUE

5

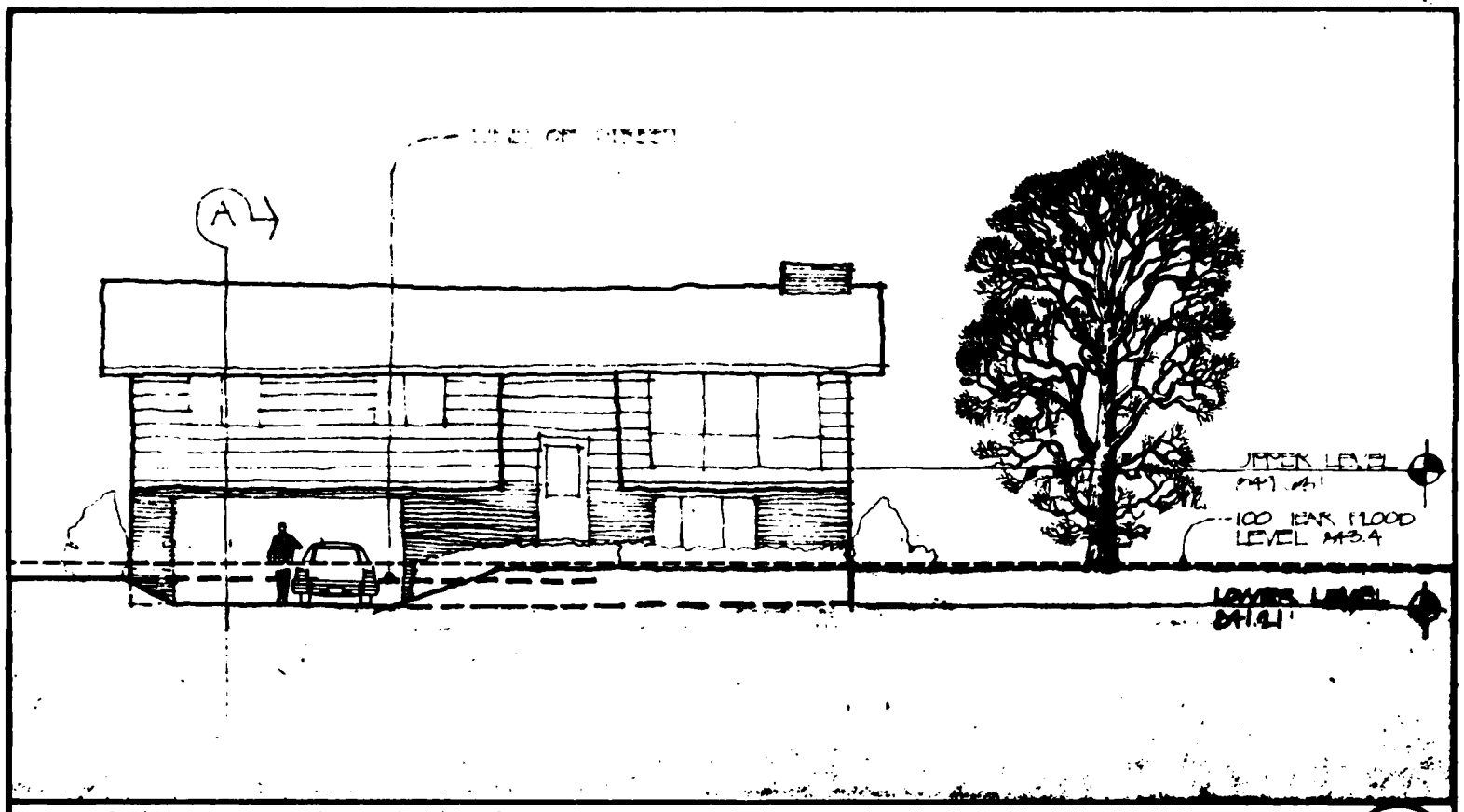
PLATE H-23



PROPOSED FRONT ELEVATION
SCALE: 1/8" = 1'-0"
GETZKIN RESIDENCE, 2665. SCOTT AVENUE

6

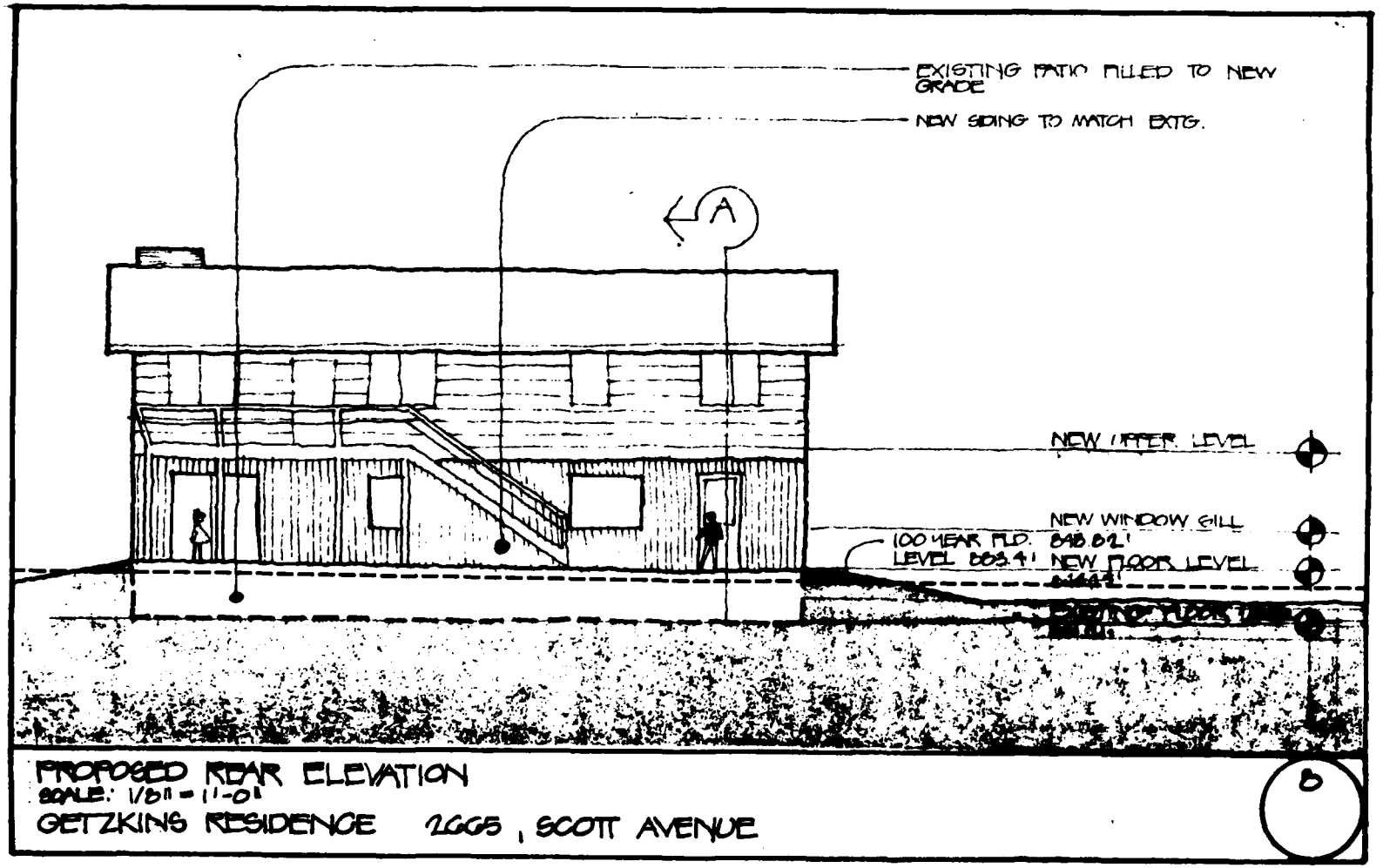
PLATE H-24



EXISTING FRONT ELEVATION
SCALE: 1/8" = 1'-0"
GETZKIN RESIDENCE 2665 SCOTT AVENUE

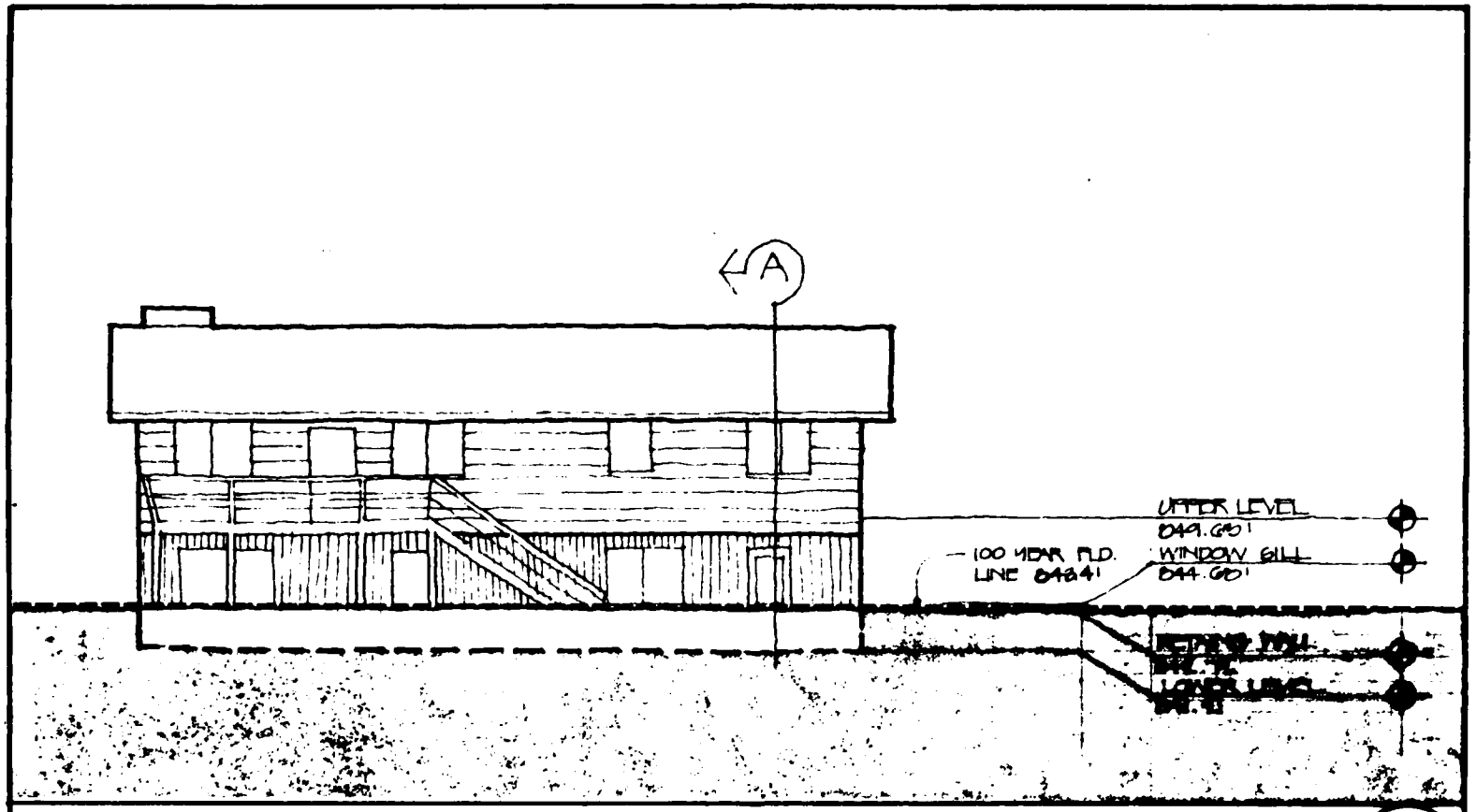
7

PLATE H-25



PROPOSED REAR ELEVATION
SCALE: 1/8" = 1'-0"
GETZKINS RESIDENCE 2665, SCOTT AVENUE

PLATE H-28



EXISTING REAR ELEVATION
SCALE: 1/8" = 1'-0"
GETZKING RESIDENCE 2665, SCOTT AVENUE

9

PLATE H-27

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE STECKER RESIDENCE LOCATED AT 2675 SCOTT AVENUE NORTH, GOLDEN VALLEY, MINNESOTA: THE SOIL CONDITIONS AND THE SURROUNDING TOPOGRAPHY WILL NOT ALLOW EITHER THE CONSTRUCTION OF A WALL OR A BERM AS A MEANS OF FLOOD-PROOFING THIS STRUCTURE. THE BEARING POTENTIAL OF THE SOIL REQUIRES PILING TO SUPPORT ANY WALLS. THE LACK OF TOPOGRAPHY TO THE SIDES ELIMINATES THE CONSIDERATION OF A BERM.

IT IS RECOMMENDED THAT THIS RESIDENCE BE RAISED AS A MEANS OF FLOOD-PROOFING. THE LEVEL OF THE BASEMENT FLOOR IS 841.09 FT. THE ONE HUNDRED YEAR FLOOD LEVEL IS 843.4 FT. THE LOWER LEVEL OF THE HOME IS COMPLETELY FINISHED. THERE ARE TWO FIREPLACES; ONE ON EACH LEVEL. THE BASEMENT LEVEL OF THE HOME WOULD BE RAISED TO A LEVEL OF 844.39 FT., EXCLUSIVE OF THE ATTACHED GARAGE. THIS WILL PRECLUDE ANY STRUCTURAL MODIFICATIONS TO THE FLOOR SLAB.

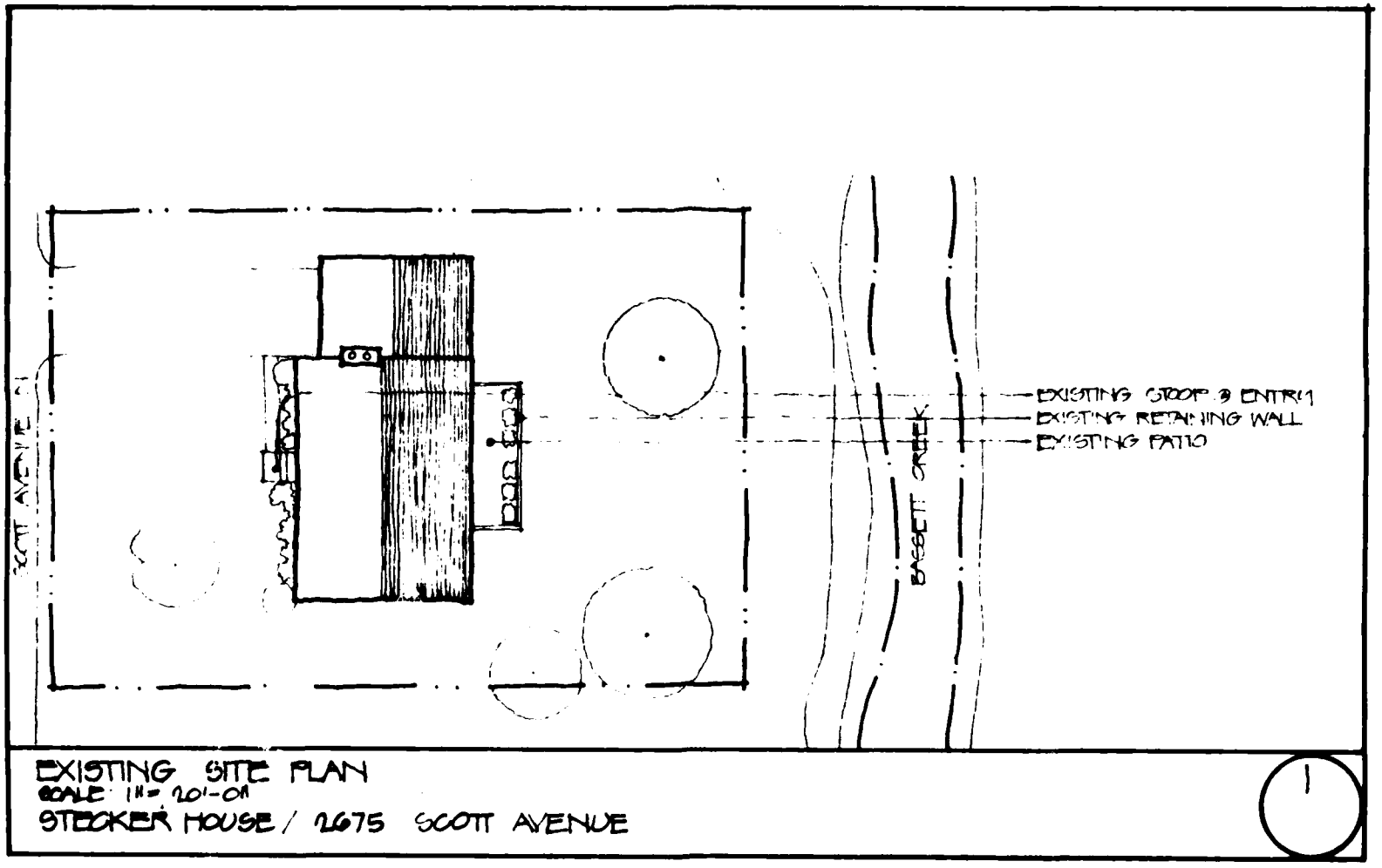
SEE PLATES H-28 THROUGH H-36.

SUMMARY OF ELEVATIONS:

DOOR SILL	841.09
SIDE DOOR	841.15
RETAINING WALL	842.99
FRONT STEPS	842.82
GARAGE DOOR	843.56
FIRST FLOOR	849.59 (APPROX.)

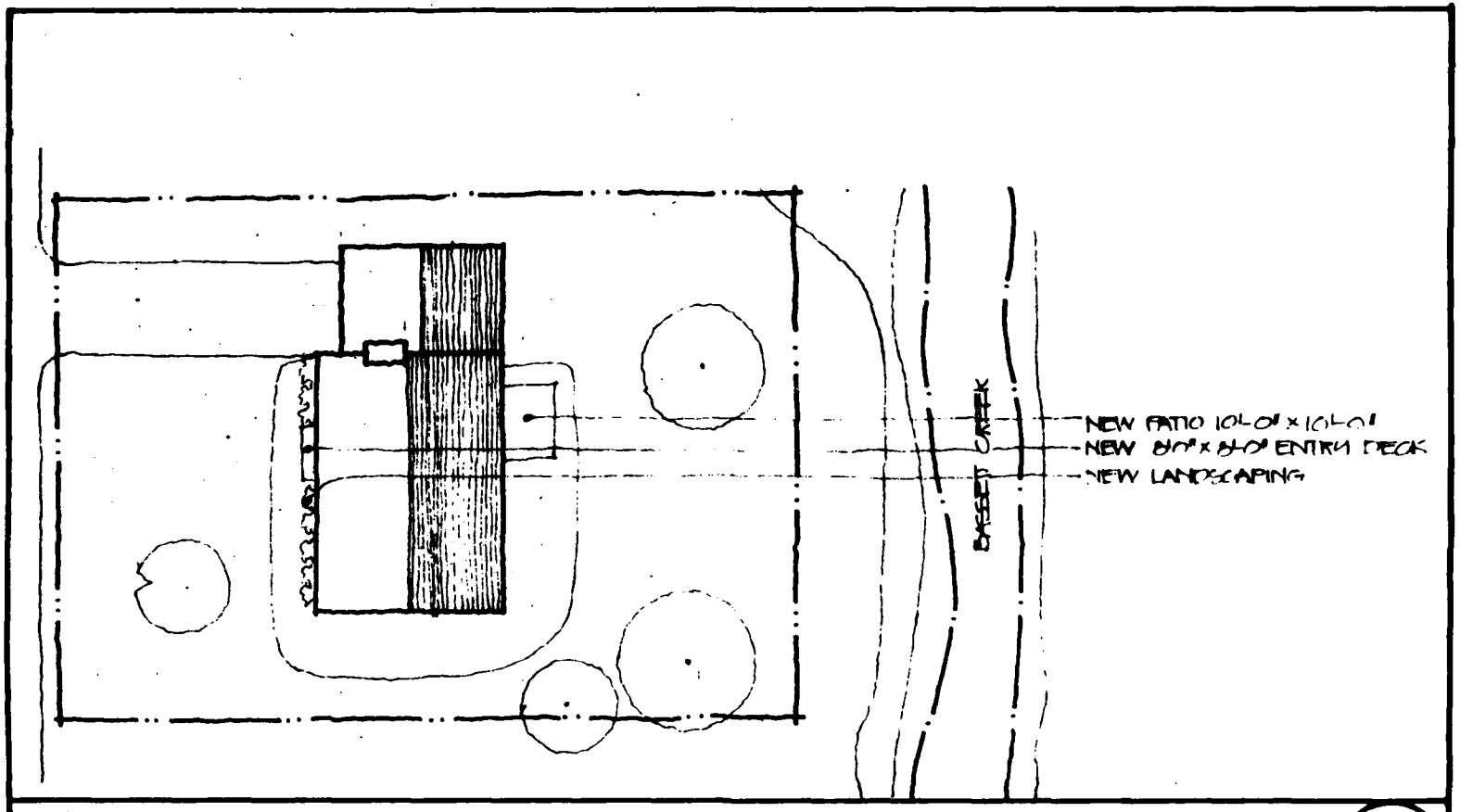
CONSTRUCTION ESTIMATE:
2675 SCOTT AVENUE NORTH, GOLDEN VALLEY, MINNESOTA

CARPENTRY	$22,700.00
MASONRY	
BLOCK	4,000.00
ELECTRICAL	500.00
PLUMBING	3,800.00
HVAC	2,700.00
HOME RAISE	10,100.00
PERMITS & PLAN CHECK	300.00
	100.00
TOTAL ESTIMATE	$44,200.00



EXISTING SITE PLAN
SCALE 1"= 20'-0"
STECKER HOUSE / 2675 SCOTT AVENUE

PLATE H-28

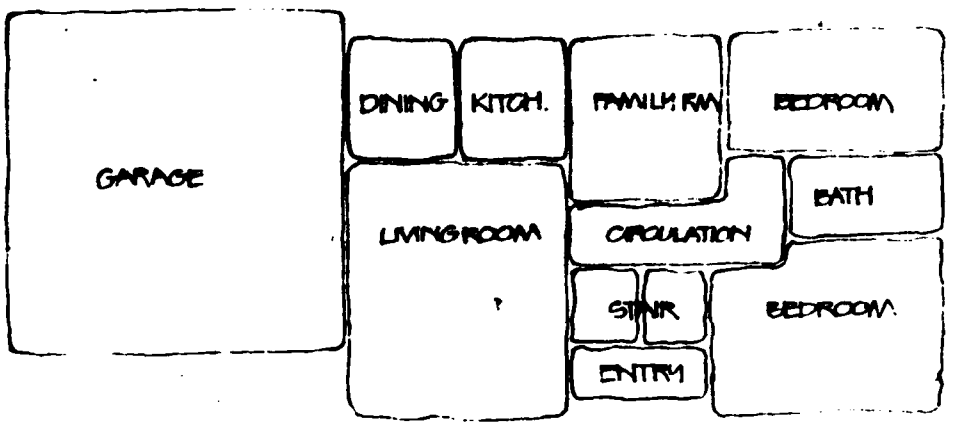


PROPOSED SITE PLAN
SCALE: 1" = 20'
STECKER HOUSE/1675 SCOTT AVENUE

2

PLATE H-23

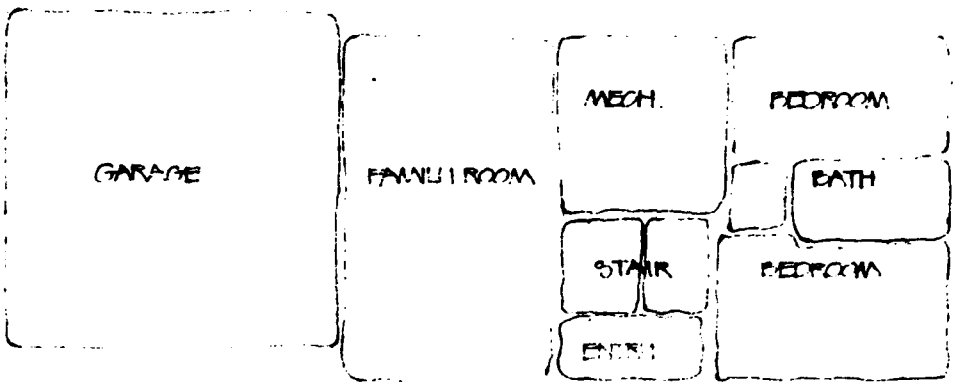
H-21



SCHEMATIC FLOOR PLAN / UPPER LEVEL
SCALE: 1/8" = 1'-0"
STECKER HOUSE, 2675 SCOTT AVENUE

3

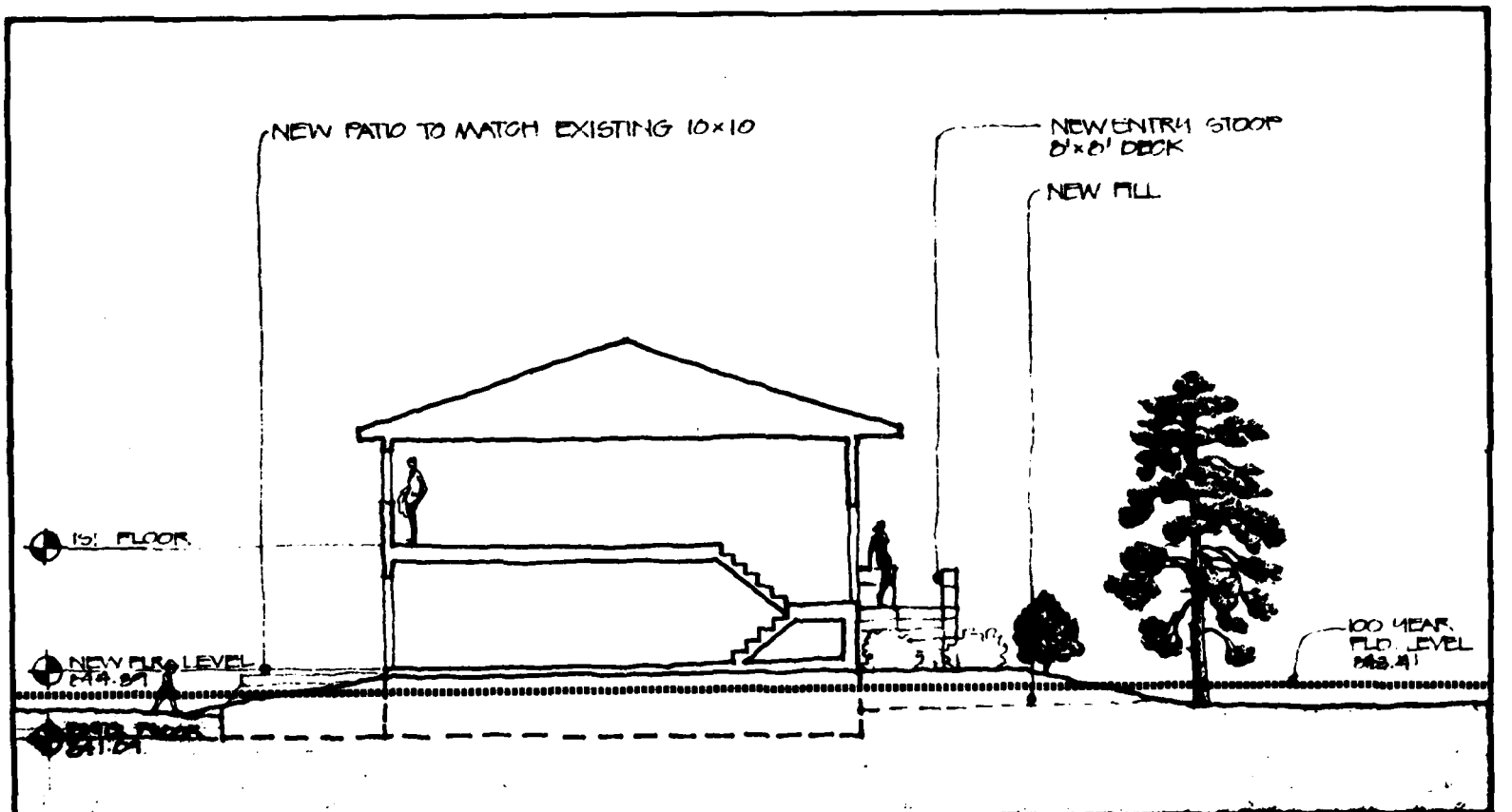
PLATE N-36



SCHEMATIC FLOOR PLAN / LOWER LEVEL
SCALE 1/8" = 1'-0"
STECKER HOUSE / 2675 SCOTT AVENUE

4

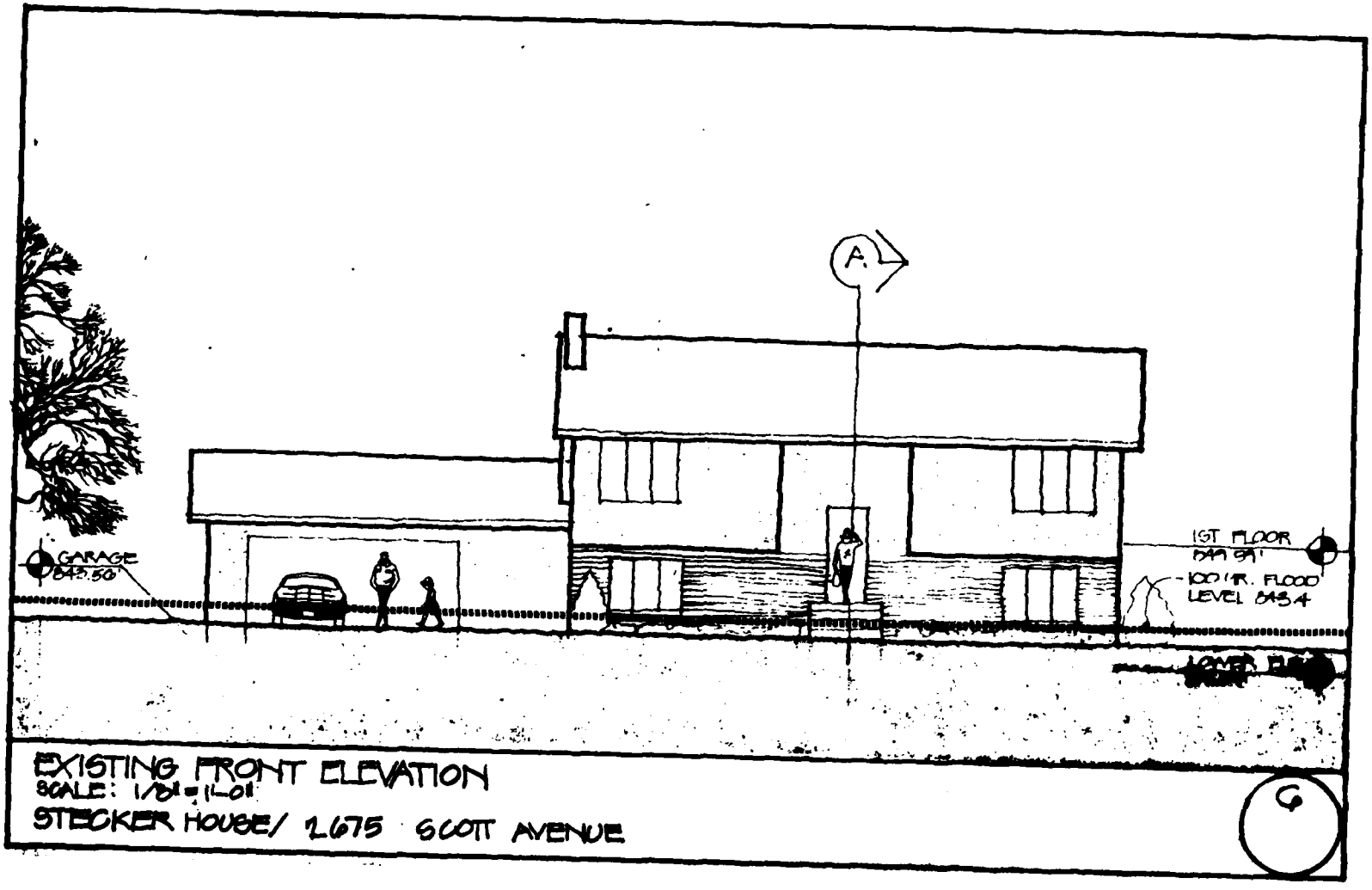
PLATE H-31
11-31



SCHEMATIC SECTION A/A (PROPOSED)
SCALE: 1/8" = 1'-0"
STECKER HOUSE/ 2078 SCOTT AVENUE

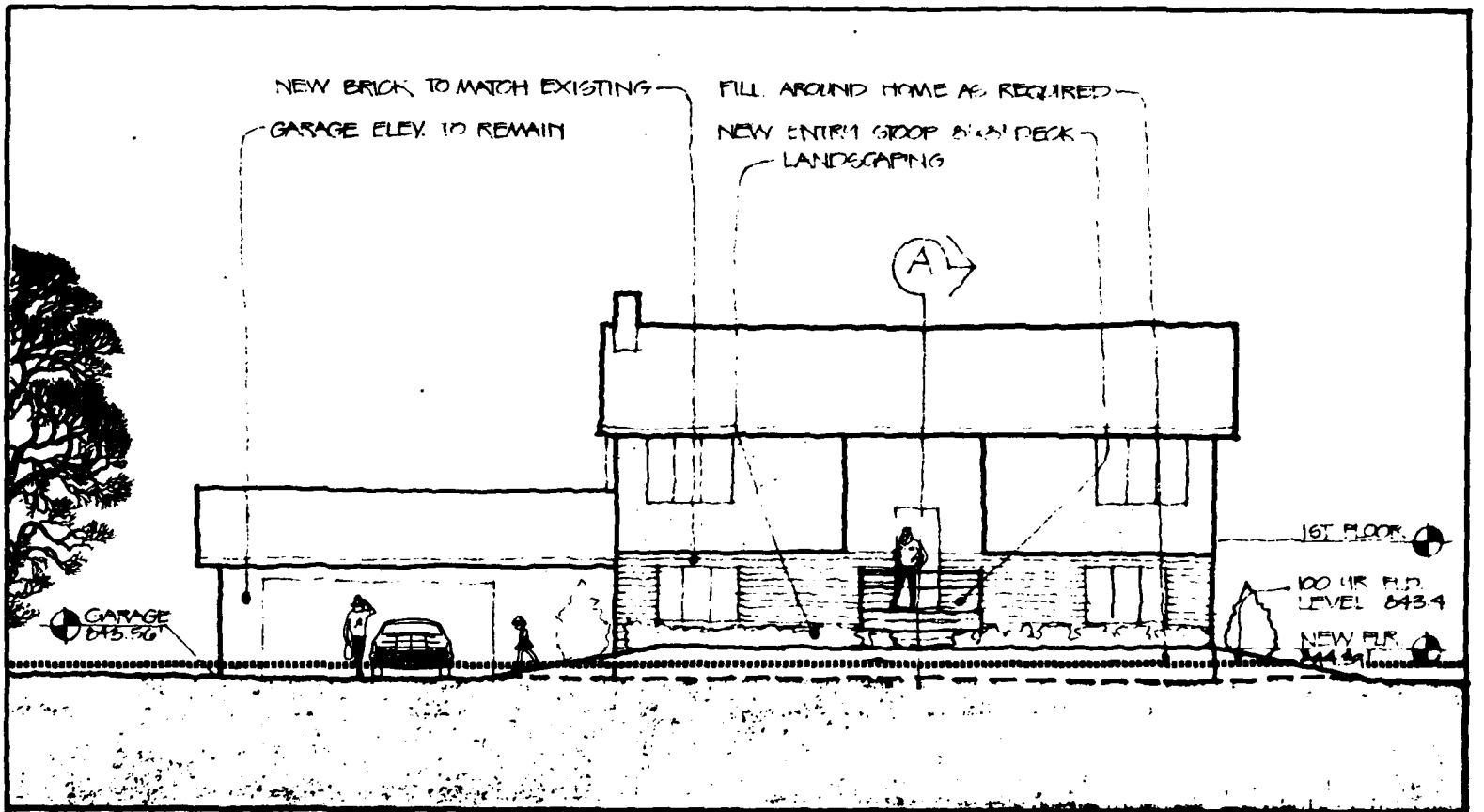
5

PLATE H-52 H-12



EXISTING FRONT ELEVATION
SCALE: 1/8" = 1'-0"
STECKER HOUSE/ 2675 SCOTT AVENUE

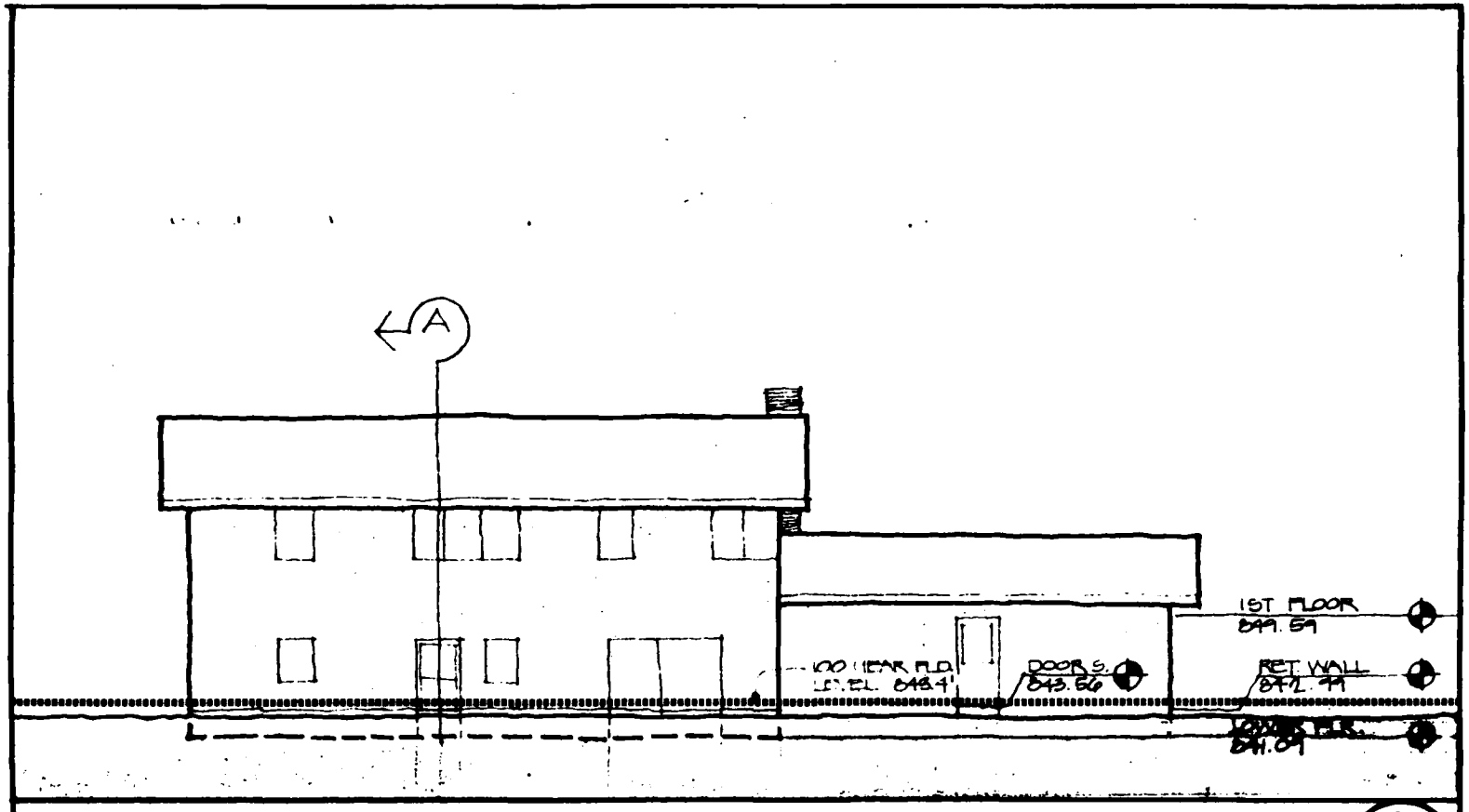
PLATE H-55



PROPOSED FRONT ELEVATION
SCALE: 1/8" = 1'-0"
STECKER HOUSE / 2675 SCOTT AVENUE

7

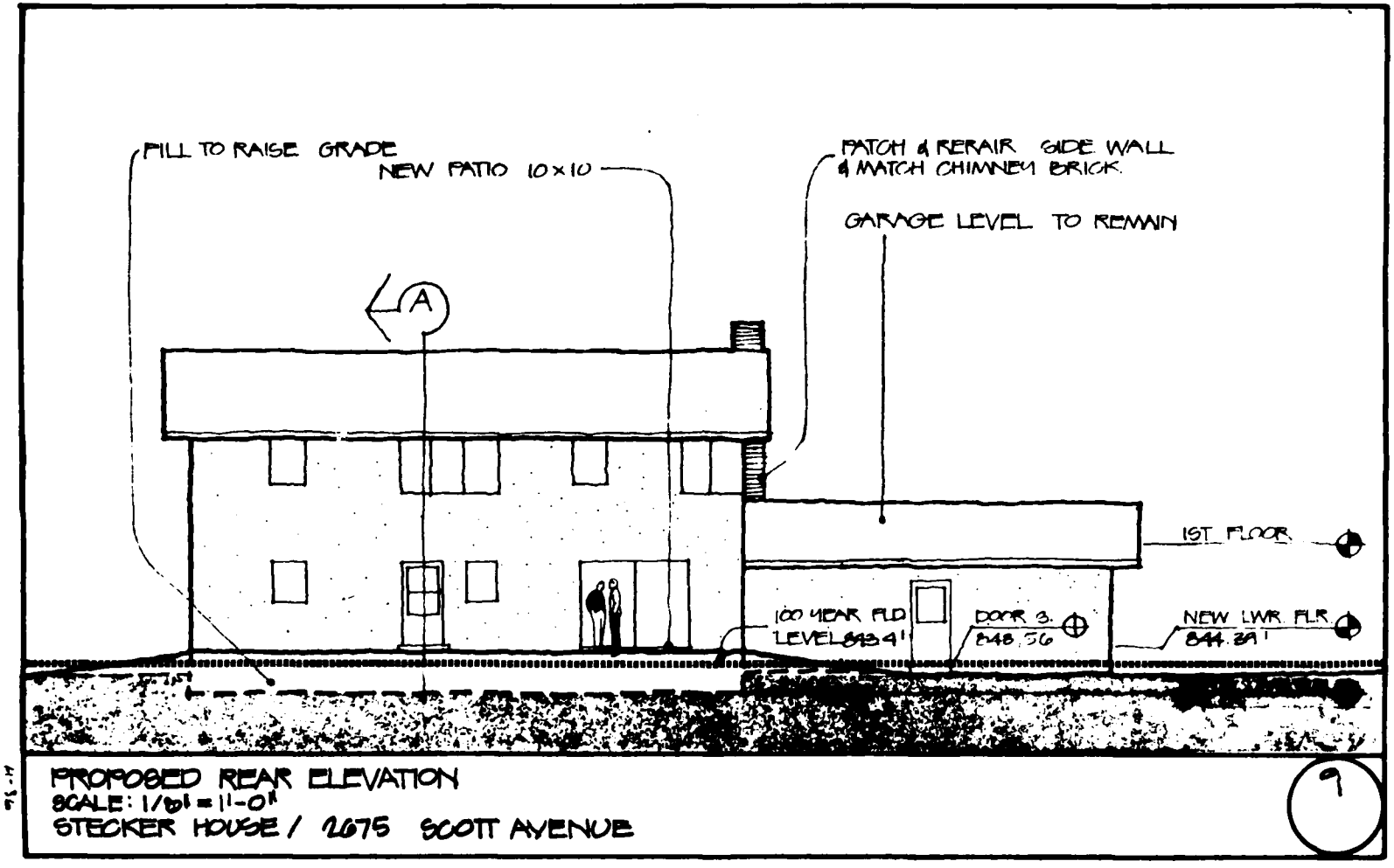
PLATE H-34



EXISTING REAR ELEVATION
SCALE: 1/01 = 1'-0"
STECKER HOUSE / 2675 SCOTT AVENUE

8

PLATE H-35



PROPOSED REAR ELEVATION
SCALE: 1/8" = 1'-0"
STECKER HOUSE / 2675 SCOTT AVENUE

9

PLATE H-36

ANALYSIS OF THE PROBLEM - SECOND GROUP

16. A series of three additional design submittals were prepared by Golden Valley which discussed the remaining 20 homes, all located in the city of Golden Valley in an area east of T.H. 100 and north of Golden Valley Road. The addresses are:

2685 Scott Avenue North	3810 Bassett Creek Drive
2725 Scott Avenue North	3820 Bassett Creek Drive
2735 Scott Avenue North	4975 Bassett Creek Drive
2755 Scott Avenue North	5005 Bassett Creek Drive
2775 Regent Avenue North	5110 Minnaqua Drive
1205 Idaho Avenue North	5120 Minnaqua Drive
2425 Regent Avenue North	5126 Minnaqua Drive
2445 Regent Avenue North	5125 Minnaqua Drive
2505 Regent Avenue North	5222 Minnaqua Drive
2545 Regent Avenue North	5130 Minnaqua Avenue North

17. These homes are all located along Bassett Creek. Most have been subject to flood damage in the past. The most recent occurance was in July 1978 when up to 2.5 feet of flood water entered the lower level of these homes.

SOIL CONDITIONS - SECOND GROUP

18. To facilitate technical review of the alternatives considered for each home, a series of hand auger soil borings were generally obtained in the vicinity of each home. These hand soil borings obtained a maximum depth of approximately 10 feet.

19. At the 2685, 2725, 2735 and 2755 Scott Avenue North and the 2775 Regent Avenue North sites, no hand auger borings were made. However, hand auger borings made for homes at 2655 through 2675 Scott Avenue North tend to substantiate the soil borings taken prior to construction of the homes. All five homes have pile foundations. Soil boring logs and reports and the pile driving logs and reports were obtained from the Building Inspector's records.

20. At the 1205 Idaho Avenue North site, no hand auger borings were made, however, the current owner had the home built for himself and reports that the home's foundation is on a granular material. Based on the observations and the lack of settlement problems in the existing home, we do not anticipate adverse soil conditions at this site.

21. At the 2425 and 2445 Regent Avenue sites, three hand auger borings were obtained on the 2445 lot. The first hole, which was located about 20 feet west of the rear center of the house, indicated a fine brown sand with some gravel to a depth of about 2 1/2 feet, a medium brown sand between the depth of 2 1/2 and

3 1/2 feet, a olive organic soil with some clay between the depths of 3 1/2 and 6 feet and a very wet coarse grey sand and gravel to a depth of 4 1/2 feet where the hole was ended because the sand was sloughing into the hole. The second hole was located about 185 feet west of the rear center of the house, indicated black peat to a depth of 3 feet, a brown coarse sand between the depth of 3 and 4.2 feet and olive color muck containing organic debris and snail shells to a depth of 10 feet where the hole was ended. Water was encountered at a depth of about 4 feet. The third hole was located within 5 feet of the rear of the house indicated 1/2 foot of topsoil over fine sand to a depth of 2.3 feet, a coarse sand between the depths of 2.3 and 4.5 feet and a wet sand with gravel to a depth of 5.5 feet where the hole was ended. Water was encountered at a depth of about 4 1/2 feet.

22. At the 2050 and 2545 Regent Avenue North site, two hand auger borings were made. Both borings indicated a thin layer of topsoil, a layer of black peat followed by a layer of olive color muck containing organic debris and snail shells. The olive color muck is very similar to that found along Scott Avenue and west of T.H. 100 at the T.H. 100 embankment.

23. At the 3810 and 3820 Bassett Creek Drive site, two hand auger borings were obtained. Both holes indicated about 2 feet of clayey sand fill followed by more than 6 feet of soft wet clay. Both holes were ended at a depth of about 8.5 feet. These hand auger borings tend to substantiate the soil borings taken prior to construction of the homes. Both homes have pile foundations. The soil boring logs and reports, and the pile driving logs and reports as obtained from the Building Inspector's records were compiled.

24. At the 4975 and 5005 Bassett Creek Drive sites, two hand auger borings were obtained. The first hole, at 4975, indicated about 1 foot of sandy fill over about 2 1/2 feet of topsoil, followed by medium sand at depths of 3.5 to 4.5 feet, then sandy silt from 4.5 to 5.3 feet, coarse sand from 5.3 to 7.0 feet and silt from 7 to 10 feet. The water level was about 6 feet deep. The second hole, at 5005, indicated topsoil to a depth of 3.5 feet, followed by clean, medium to fine sand to a depth of 6.5 feet. The water level was at about 5.0 feet. The boring was ended at a depth of 6.5 feet because the saturated sand sloughed into the bottom of the hole.

25. At the 5110, 5120 and 5126 Minnaqua Drive site, a series of four hand auger borings were obtained. Three of the holes indicated about 2 feet of topsoil, followed by layers of organic silts and clays and at a depth of 4 to 4.5 feet coarse sand was encountered. These holes were ended at depths of 6.0 to 6.5 feet, because the saturated sand sloughed into the bottom of the holes. Water was encountered at a depth of about 4.5 feet in all three holes. The fourth hole, located in the northwest corner of the 5120 property had about 3.5 feet of black topsoil and peat, followed by about 3.5 feet of muck, followed by fibrous muck to the end of the hole at a depth of 10 feet.

26. At the 5125 Minnaqua Drive site, two hand auger borings were obtained. The first hole located 20 feet from the house indicated about 2.5 feet of topsoil, then 1 foot of soft clay, followed by 6.5 feet of peat, muck and fibrous muck to a depth of 10 feet where the hole was ended. Water was found at a depth of about 6 feet. The second hole, located 80 feet from the house, indicated about 3 feet of topsoil following by a 1-foot layer of sand, then a layer of fibrous muck to a depth of 6 feet. Below the depth of 6 feet, coarse sand was encountered to a depth of 7.5 feet where the hole was ended because the saturated sand

sloughed into the bottom of the hole. Water was encountered at a depth of about 4 feet.

27. At the 5222 Minnaqua Drive site, no hand auger borings were obtained, however, a soil boring dated September 20, 1977, was obtained from the Building Inspector's record and is attached. The profile seems to be very similar to that found along the north end of Regent Avenue and along Scott Avenue.

28. At the 5130 Minnaqua Avenue site, two hand auger borings were obtained. The first hole indicated about 2 1/2 feet of topsoil with some gravel, followed by about 1 foot of coarse sand mixed with clay and at a depth of about 3 1/2 feet water was encountered in a gravelly sand layer. The hole was ended at a depth of 4 1/2 feet because the saturated sand sloughed into the bottom of the hole. The second hole indicated about 2 feet of topsoil followed by about 1 1/2 feet of medium brown sand and about 1 foot of peat. At a depth of about 4 1/2 feet, a coarse sand with some gravel was encountered before the hole was ended at a depth of 6 1/2 feet because the sand was sloughing into the hole. The first hole was located approximately 20 feet west of the existing walkout door, while the second hole was located approximately 25 feet west and north of the rear center of the house.

RECOMMENDATIONS - SECOND GROUP

29. Based on the above discussion and an analysis of the available basic alternatives, it is recommended that the adjoining homes at 2685, 2725 and 2735 Scott Avenue North, be flood-proofed by providing a common timber wall located between the rear of the homes and the creek. For the soils which exist at this site, it is anticipated that the wall section indicated will be structurally adequate. A system of drain tile will be provided to intercept seepage under the wall. A sump and sump pump would be provided to collect and discharge seepage and runoff from interior drainage areas. See page H-12 & Plates H-37 through H-39.

30. For the homes at 2755 Scott Avenue North and 2775 Regent Avenue North, it is recommended that the homes be flood-proofed by providing a common earth berm through the backyards between the homes and the creek. A sump and sump pump would be provided to collect and discharge runoff from interior drainage areas. See page H-13 and Plates H-40 and H-41.

31. For the home at 1205 Idaho Avenue North, it is recommended that the home be floodproofed by modifying the existing walkout. The existing homeowner is opposed to blocking the walkout since it would have a significant adverse effect on the property value. The existing windows would be removed, concrete blocks would be added to raise the window sills to above the flood damage elevation, and new windows would be installed. The existing doorway would be lowered to the same elevation as the basement floor. A reinforced concrete masonry landing and steps up to a new patio deck will be provided. Earthfill will be added to berm against the basement foundation and landing. Drain tile would be installed to reduce seepage pressures on the basement floor. A sump pump would be provided to collect and discharge seepage and runoff from interior drainage areas. See page H-14 and Plates H-42 through H-45.

32. For the homes at 2425 and 2445 Regent Avenue North, it is recommended that the homes be flood-proofed by providing a common earth berm across the backyard between the homes and the creek. A sump and sump pump would be provided to

collect and discharge runoff from interior drainage areas. See page H-15 and Plates H-46 and H-47.

33. For the homes at 2505 and 2545 Regent Avenue North, it is recommended that the homes be flood-proofed by providing a common earth berm through the backyards between the homes and the creek. A sump and sump pump would be provided to collect and discharge runoff from interior drainage areas. In addition, an existing storm sewer outlet will be extended to assure that the discharge of stormwater is beyond the berm. See page H-16 and Plates H-48 and H-49.

34. It is recommended that the homes at 3810 and 3820 Bassett Creek Drive be flood-proofed by providing a common timber wall located betwen the homes and the creek. For the soils which exist at this site, it is anticipated that the wall section indicated will be structurally adequate. A system of drain tile will be provided to intercept seepage under the wall. A sump and sump pump would be provided to collect and discharge seepage and runoff from interior drainage areas. See pages H-17 and H-18, Plates H-50 through H-53 and Plate H-57.

35. For 4975 Bassett Creek Drive, it is recommended that the home be flood-proofed by providing a reinforced concrete and reinforced masonry wall between the home and the creek. A system of drain tile will be provided to intercept seepage under the wall. A sump and sump pump would be provided to collect and discharge seepage and runoff from interior drainage areas. See page H-19 and Plates H-58 through H-61.

36. For 5005 Bassett Creek Drive, it is recommended that the home be flood-proofed by providing a reinforced concrete and reinforced masonry wall between the home and the creek. A system of drain tile will be provided to intercept seepage under the wall. A sump and sump pump would be provided to collect and discharge seepage and runoff from interior drainage areas. See page H-20 and Plates H-63 through H-65.

37. For the homes at 5110, 5120 and 5126 Minnaqua Drive, it is recommended that the homes be flood-proofed by providing a common earth berm through the backyards between the homes and the creek. A sump and sump pump would be provided to collect and discharge runoff from interior drainage areas. See pages H-21 and H-22 and Plates H-66 and H-67.

38. For homes at 5125 and 5222 Minnaqua Drive, it is recommended that each home be flood-proofed by providing separate earth berm - one across each backyard between the house and creek. A sump and sump pump would be provided to collect and discharge runoff from each interior drainage area. See page H-23 and H-24 and Plates H-68 through H-72.

39. It is recommended that the home at 5130 Minnaqua Avenue North be flood-proofed by providing earthfill against the foundation at the walkout level. The door on the walkout level would be replaced with a window and earthfill would be placed against the foundation to the flood protection elevation. Window wells would to be used to allow earthfill to be placed to an elevation about 0.5 feet above the window sills. Negligible settlement of the earthfill is expected on the sandy soils which exist around the walkout. A system of drain tile with a sump pump would be provided along the west and north sides between the house and the creek to prevent undesirable seepage and uplift pressures on the existing basement floor. See page H-25 and Plates H-73 through H-78.

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE PETERSON RESIDENCE LOCATED AT 2685 SCOTT AVENUE, THE ROBERTS RESIDENCE LOCATED AT 2725 SCOTT AVENUE, AND THE SVIHEL RESIDENCE LOCATED AT 2735 SCOTT AVENUE, GOLDEN VALLEY, MINNESOTA: THE LOW OPENINGS ARE 841.98, 842.62, AND 843.17, RESPECTIVELY. THE 100-YEAR FLOOD LEVEL IS 843.5. WE MUST FLOOD-PROOF TO A LEVEL OF 844.5.

IN CONSIDERATION OF THE PROXIMITY OF THE THREE HOMES, AND THE FACT THAT THE ELEVATIONS ARE APPROXIMATELY THE SAME, THE BEST SOLUTION WOULD BE A TREATED TIMBER RETAINING WALL THROUGH THE REAR YARDS OF THESE THREE HOMES. THIS RETAINING WALL WOULD TIE INTO THE EXISTING CONTOURS OF 2735 AND 2685 SCOTT AVENUES. WE MUST THEN PROVIDE AN INTERIOR DRAINAGE SYSTEM WITH A SUMP PUMP.

SEE PLATES H-37 THROUGH H-39.

SUMMARY OF ELEVATIONS - 2685 SCOTT AVENUE

DOOR SILL	841.98
DOOR SILL	844.89
UPPER DECK	849.59
100-YEAR FLOOD LEVEL	843.5
FLOOD-PROOF LEVEL	844.5

SUMMARY OF ELEVATIONS - 2725 SCOTT AVENUE

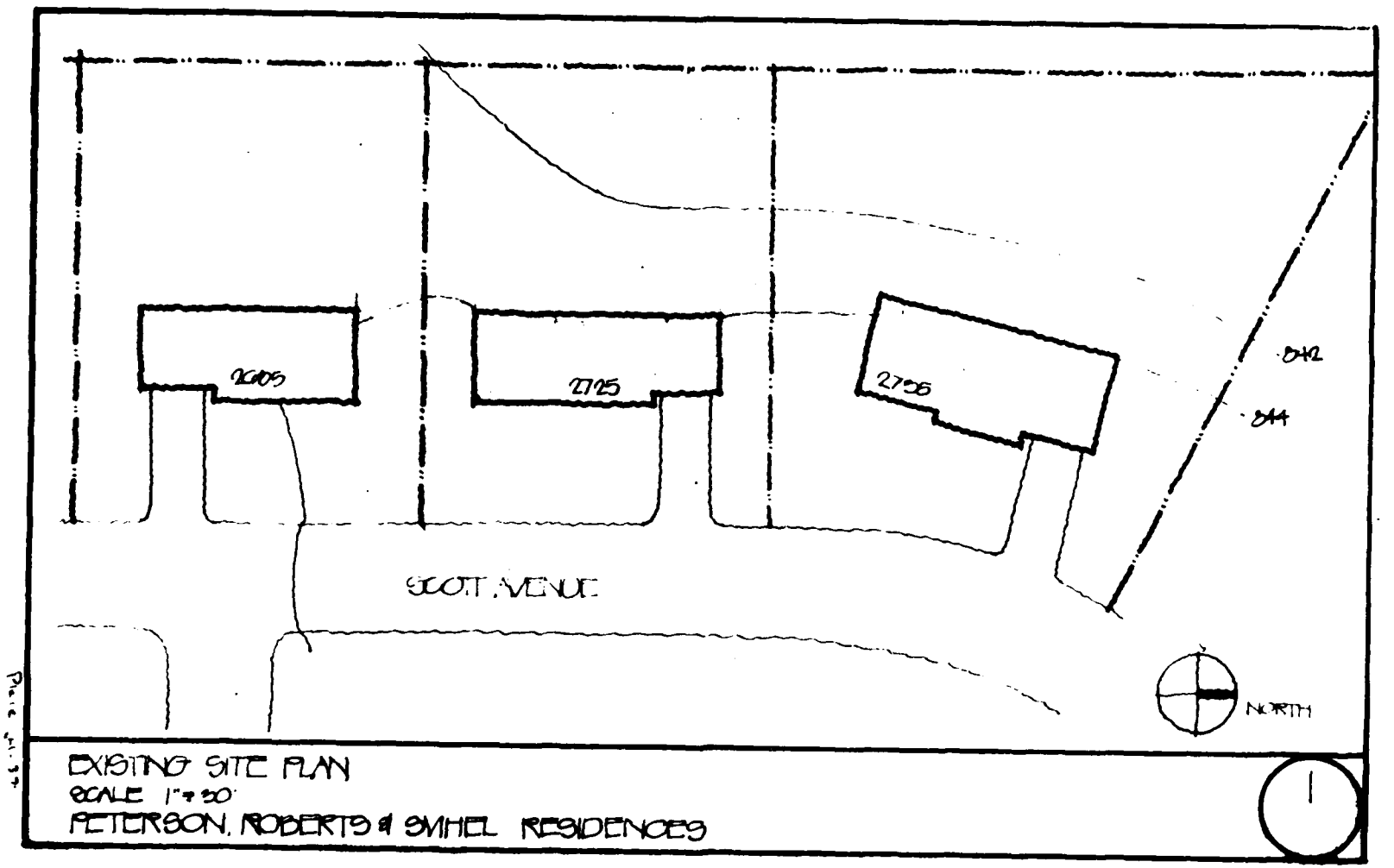
DOOR SILL (PATIO DOOR)	842.63
DOOR SILL (PANEL DOOR)	842.62
WINDOW SILL	845.87
100-YEAR FLOOD LEVEL	843.5
FLOOD-PROOF LEVEL	844.5

SUMMARY-OF ELEVATIONS - 2735 SCOTT AVENUE

DOOR SILL (LOWER)	843.17
DOOR SILL (UPPER)	845.85
100-YEAR FLOOD LEVEL	843.5
FLOOD-PROOF LEVEL	844.5

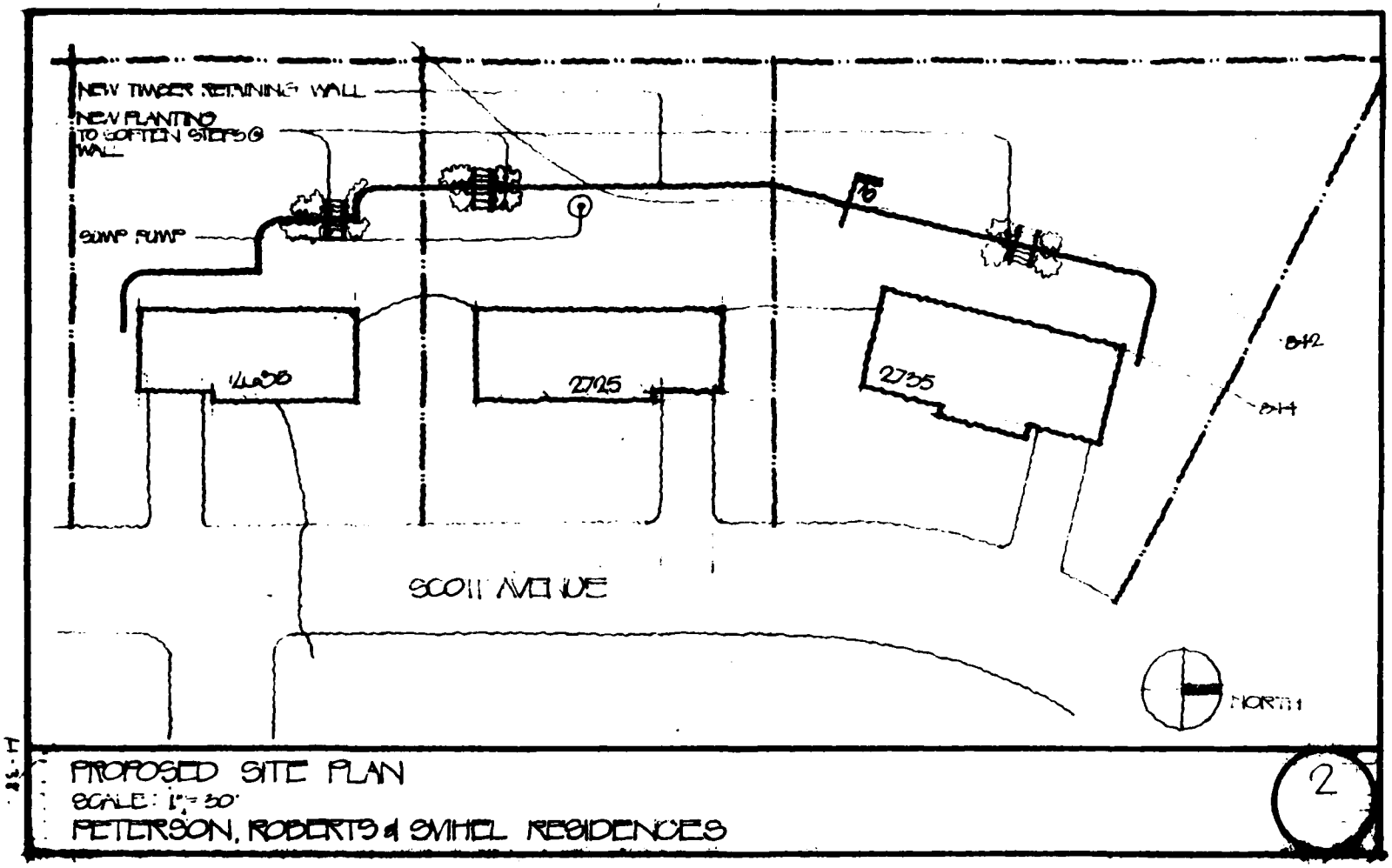
CONSTRUCTION ESTIMATE - 2685, 2725 AND 2735 SCOTT AVENUE

EARTHWORK	
EXCAVATION AND BACKFILL	$ 2,100.00
SOD WITH TOPSOIL	950.00
CARPENTRY	
TIMBER WALL	9,340.00
STEPS	1,360.00
DRAINAGE	
SUMP PUMP WITH SUMP	6,050.00
DRAINTILE WITH FILTER	2,350.00
TOTAL ESTIMATE	$22,150.00
OR	$ 7,383.00/HOME



EXISTING SITE PLAN
SCALE 1"=30'
PETERSON, ROBERTS & SMITH RESIDENCES

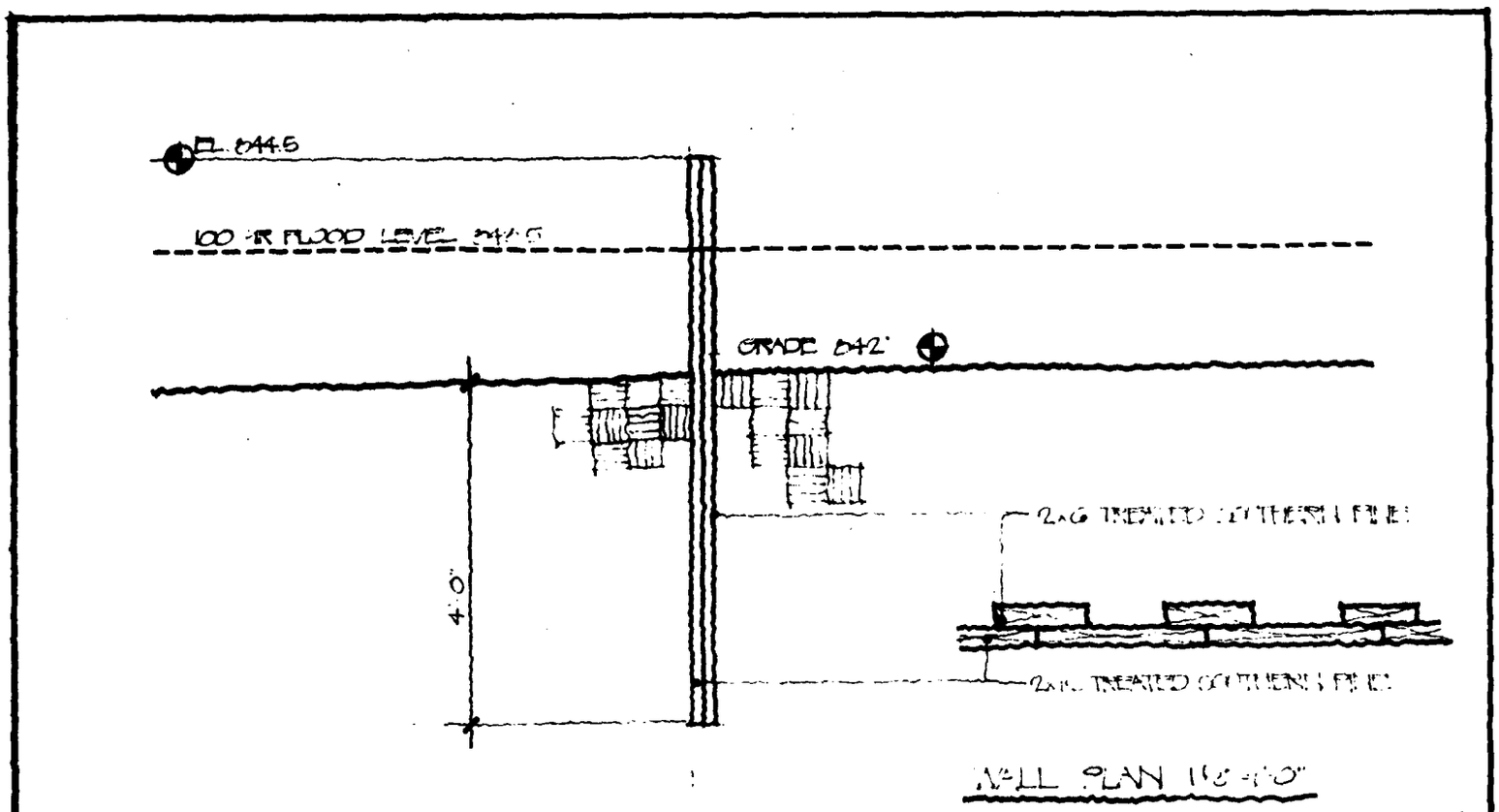
PLATE H-37



PROPOSED SITE PLAN
SCALE: 1" = 30'
PETERSON, ROBERTS & SMITH RESIDENCES

2

PLATE H-38



WALL SECTION & PLAN

SCALE: 3/4"=1'-0"

PETERSON, ROBERTS & SWIHEL RESIDENCES / 2685, 2725, & 2735 SCOTT AVE.

3

PLATE H-39

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE SCHMIDT RESIDENCE LOCATED AT 2775 REGENT AVENUE, AND THE ADEMITE RESIDENCE LOCATED AT 2755 SCOTT AVENUE, GOLDEN VALLEY, MINNESOTA: THE LOW OPENINGS FOR THESE TWO RESIDENCES ARE 843.17 AND 843.22, RESPECTIVELY. THE 100-YEAR FLOOD LEVEL IS 843.6. WE MUST FLOOD-PROOF TO A LEVEL OF 844.6.

THE ADEMITE RESIDENCE HAD 3 INCHES OF WATER IN ITS LOWER LEVEL IN 1978. THE SCHMIDT RESIDENCE ALSO EXPERIENCED FLOOD DAMAGE IN 1978.

THE BEST SOLUTION TO THE PROBLEM OF FLOODING IN THIS INSTANCE WOULD BE THE CONSTRUCTION OF A BERM WHICH WOULD TIE INTO THE EXISTING SLOPES OF THE LAND. IT WOULD THEN BE COVERED WITH EARTH AND SODDED OR SEEDED. AN INTERIOR DRAINAGE SYSTEM WITH A SUMP PUMP WOULD ALSO BE CONSTRUCTED.

SEE PLATES H-40 AND H-41.

SUMMARY OF ELEVATIONS - 2775 REGENT AVENUE

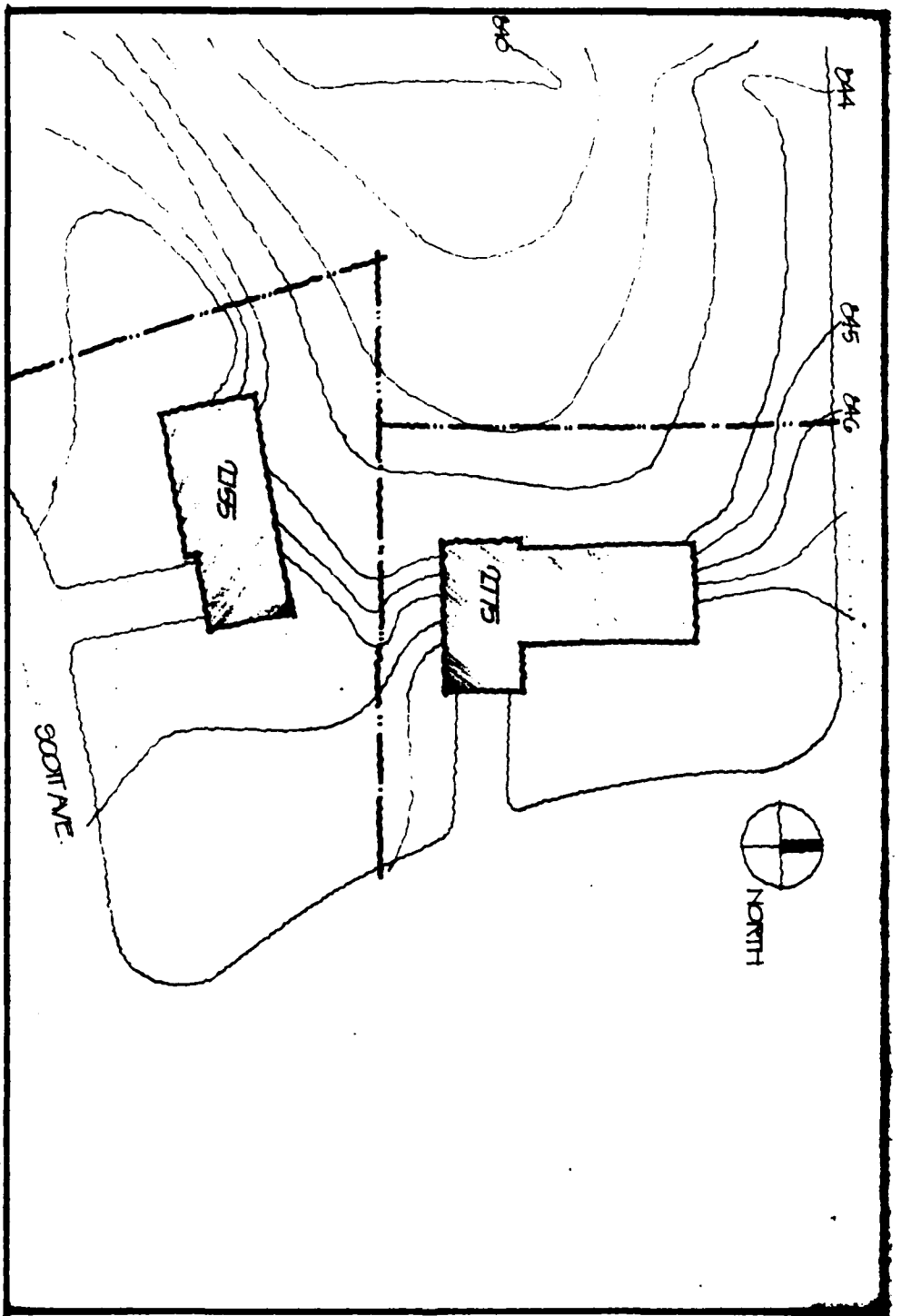
DOOR SILL (PATIO DOOR)	843.19
DOOR SILL (REAR DOOR)	843.17
FIRST FLOOR	851.3

SUMMARY OF ELEVATIONS - 2755 SCOTT AVENUE

DOOR SILL (REAR DOOR)	843.30
DOOR SILL (PATIO DOOR)	843.22
FIRST FLOOR	851.6

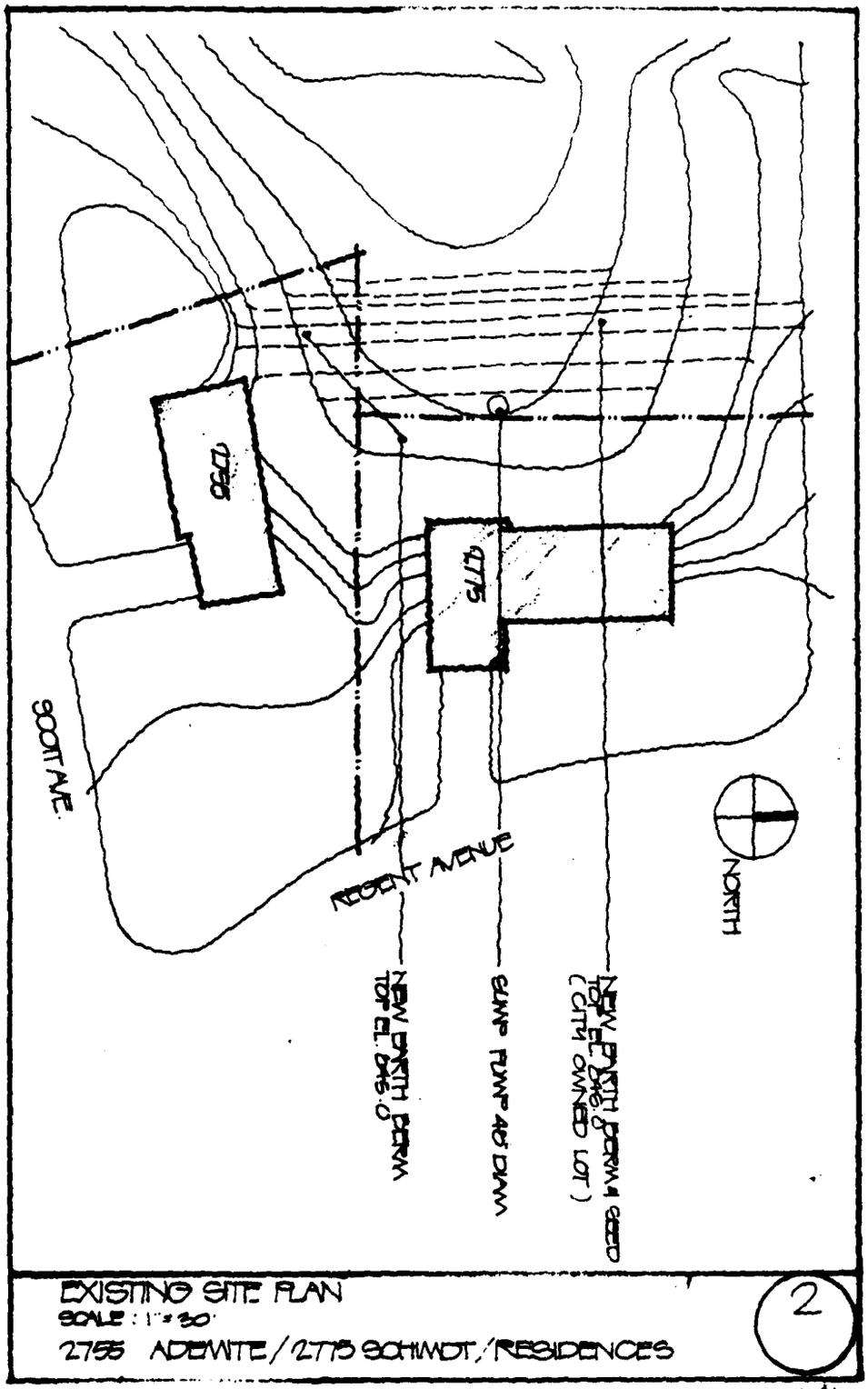
CONSTRUCTION ESTIMATE - 2755 REGENT AVENUE AND 2775 SCOTT AVENUE

EARTHWORK	
FILL	$ 2,220.00
SOD OR SEED WITH TOPSOIL	2,830.00
DRAINAGE	
SUMP PUMP WITH SUMP	6,050.00
TOTAL ESTIMATE	$11,100.00
OR	$ 5,550.00/HOME



EXISTING SITE PLAN
SCALE 1"=30'
2755 ADEMITE / 2775 SCHWOT / RESIDENCES

PLATE H-10



EXISTING SITE PLAN
SCALE: 1" = 30'
2755 ADEWITE / 2775 SCHMIDT / RESIDENCES

2

PLATE M-941

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE KNIGHT RESIDENCE LOCATED AT 1205 IDAHO AVENUE, GOLDEN VALLEY, MINNESOTA: THE LOW OPENING IS 871.30. THE ONE HUNDRED YEAR FLOOD LEVEL IS 871.7. WE MUST FLOOD-PROOF TO A LEVEL OF 872.7.

THE BEST SOLUTION TO THE FLOODING PROBLEM OF THIS HOME WOULD BE THE CONSTRUCTION OF A NEW CONCRETE RETAINING WALL AND A SLAB AT THE DOOR. THIS AREA WOULD SERVE AS A LANDING WITH STEPS GOING UP TO A DECK. THE DECK WOULD REPLACE THE EXISTING PATIO, WHICH WILL BE LOST, AS WELL AS BE ABOVE THE ONE HUNDRED YEAR FLOOD LEVEL. WE MUST ALSO PROVIDE AN INTERIOR DRAINAGE SYSTEM WITH A SUMP PUMP.

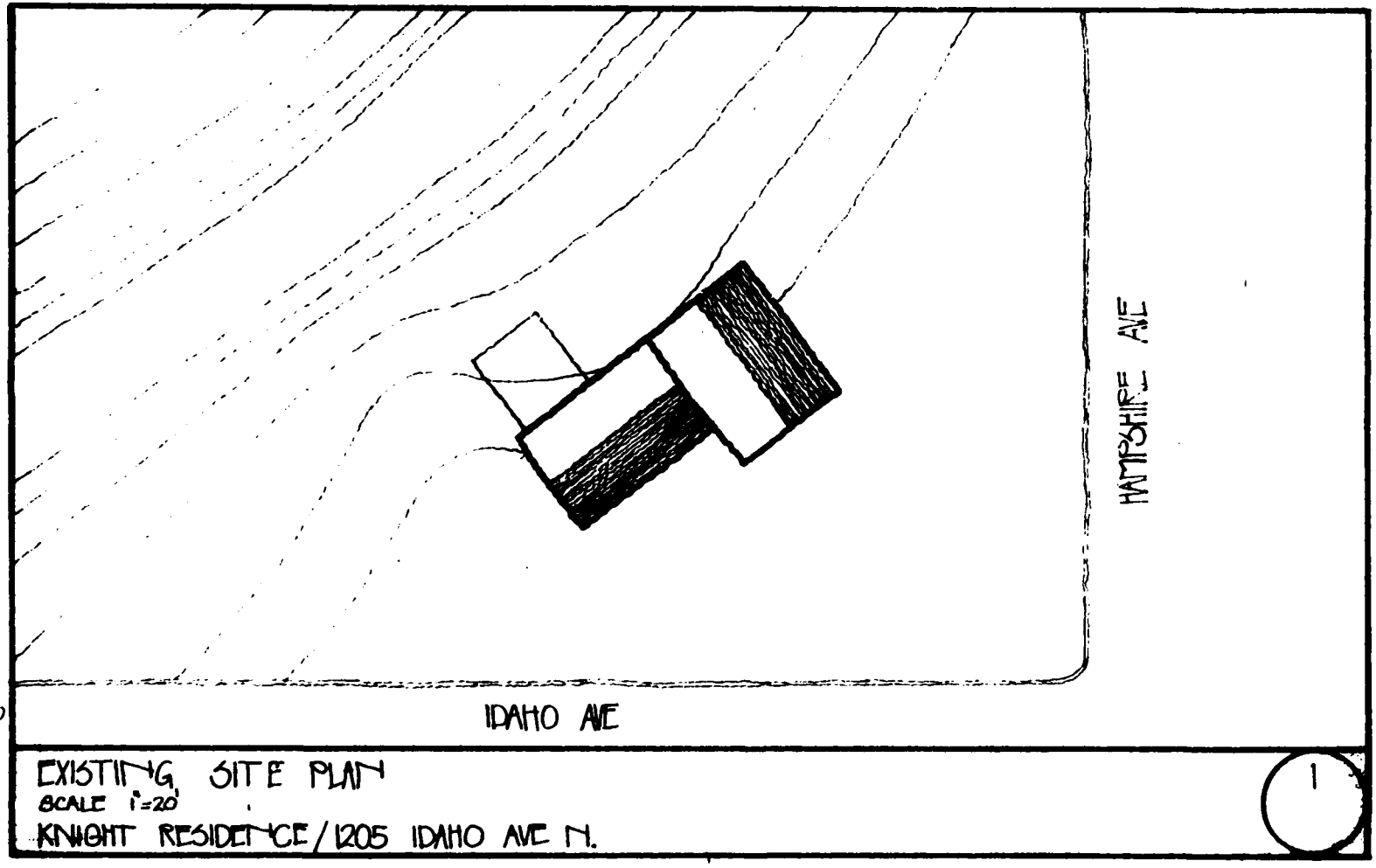
SEE PLATES H-42 THROUGH H-45.

SUMMARY OF ELEVATION

LOW OPENING (DOOR SILL)	871.38
ONE HUNDRED YEAR FLOOD LEVEL	871.7
FLOOD-PROOFING LEVEL	872.7

CONSTRUCTION ESTIMATE
1205 IDAHO AVENUE NORTH, GOLDEN VALLEY, MINNESOTA

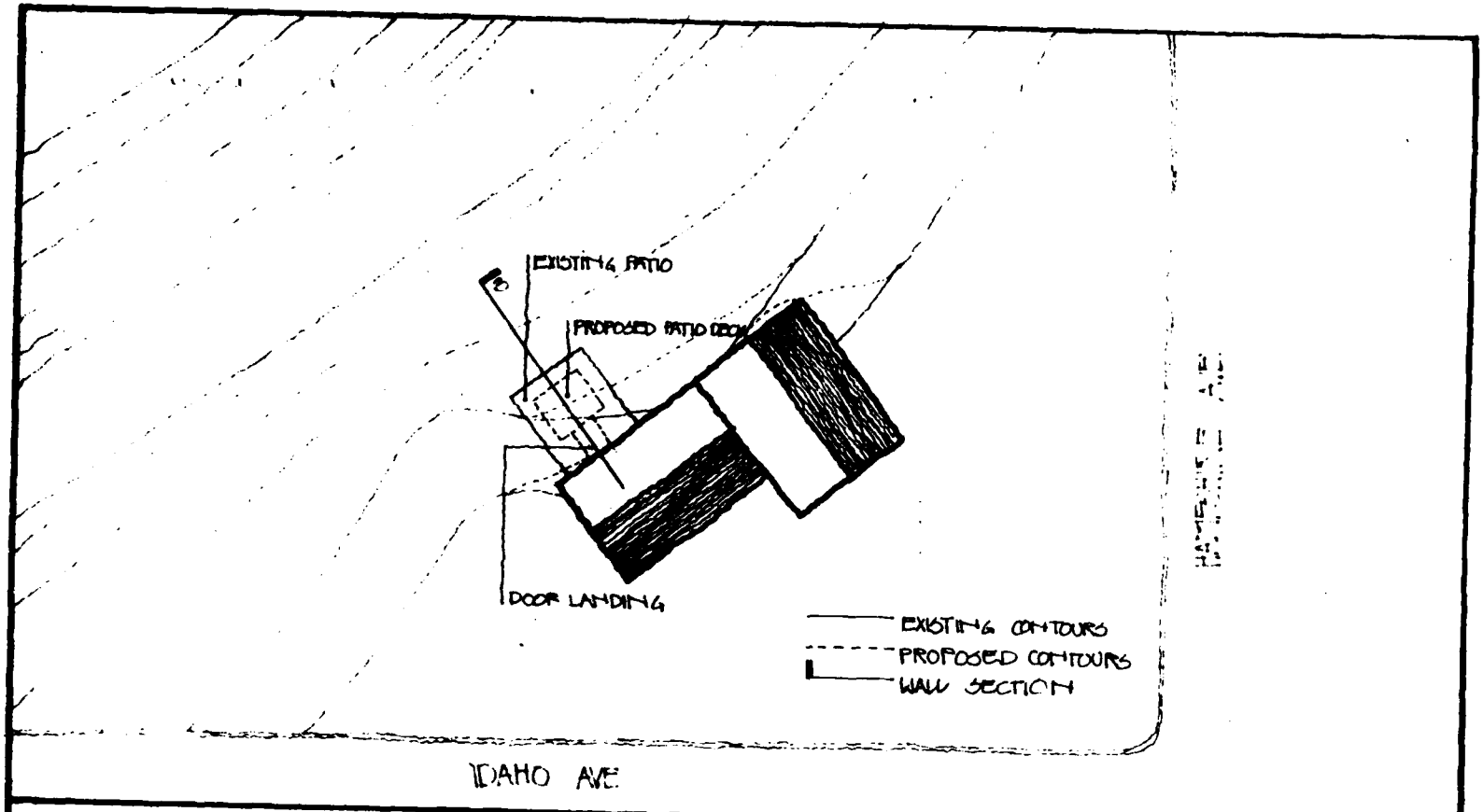
CARPENTRY	
LABOR & MATERIALS	$ 2,300.00
PATIO (LABOR & MATERIALS)	1,350.00
EARTHWORK	
EXCAVATION AND BACKFILL	950.00
SOD WITH TOPSOIL	350.00
CONCRETE AND MASONRY	
REINFORCED CONCRETE	1,350.00
REINFORCED MASONRY	475.00
UNREINFORCED MASONRY	275.00
DRAINAGE	
SUMP PUMP WITH SUMP	4,050.00
DRAIN TILE WITH FILTER	575.00
TOTAL ESTIMATE	$11,675.00



EXISTING SITE PLAN
SCALE 1"=20'
KNIGHT RESIDENCE / 1205 IDAHO AVE N.

1

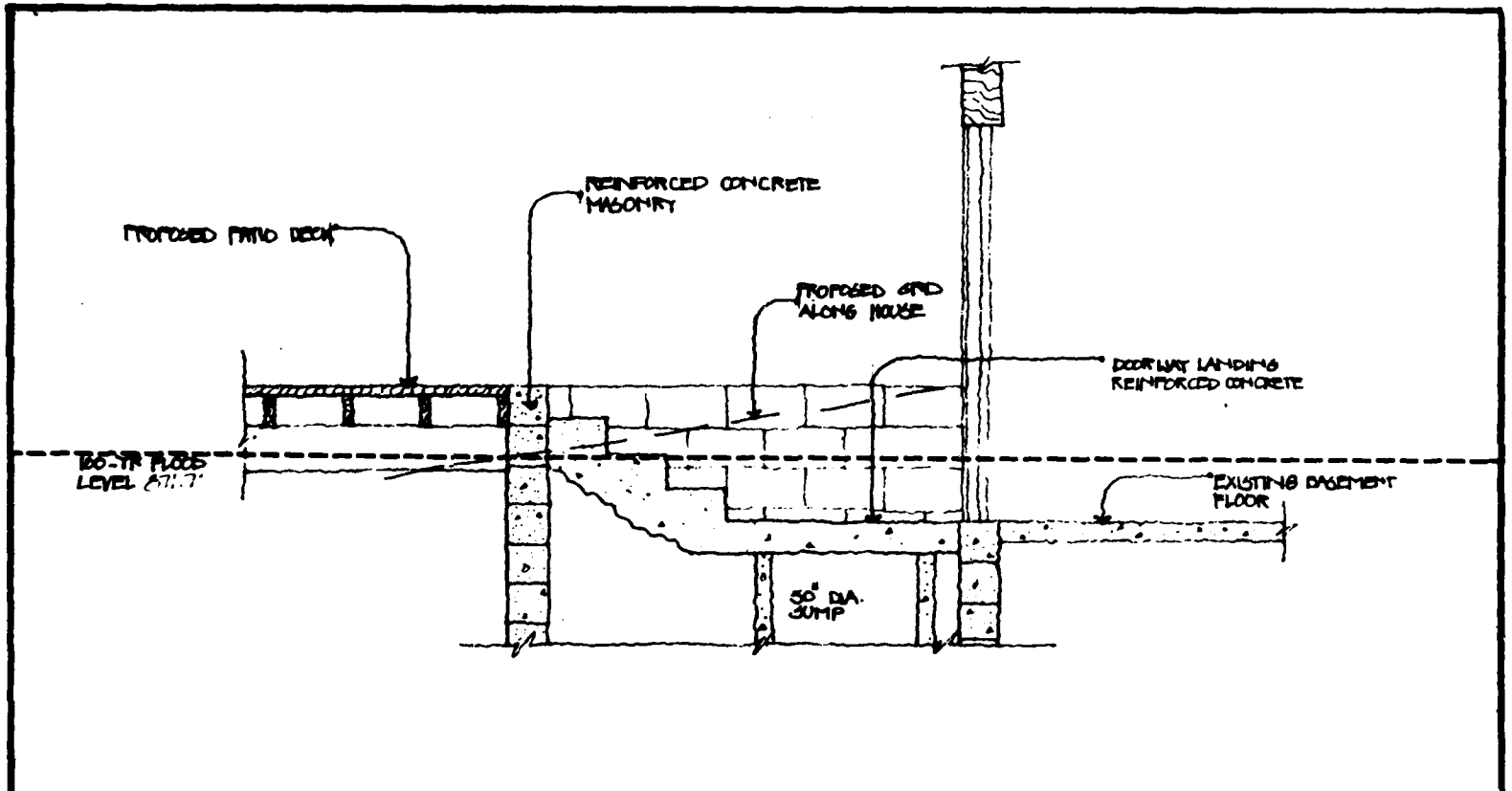
PLATE H-42



PROPOSED SITE PLAN
SCALE 1"=20'
KNIGHT RESIDENCE / 1205 IDAHO AVE N.

2

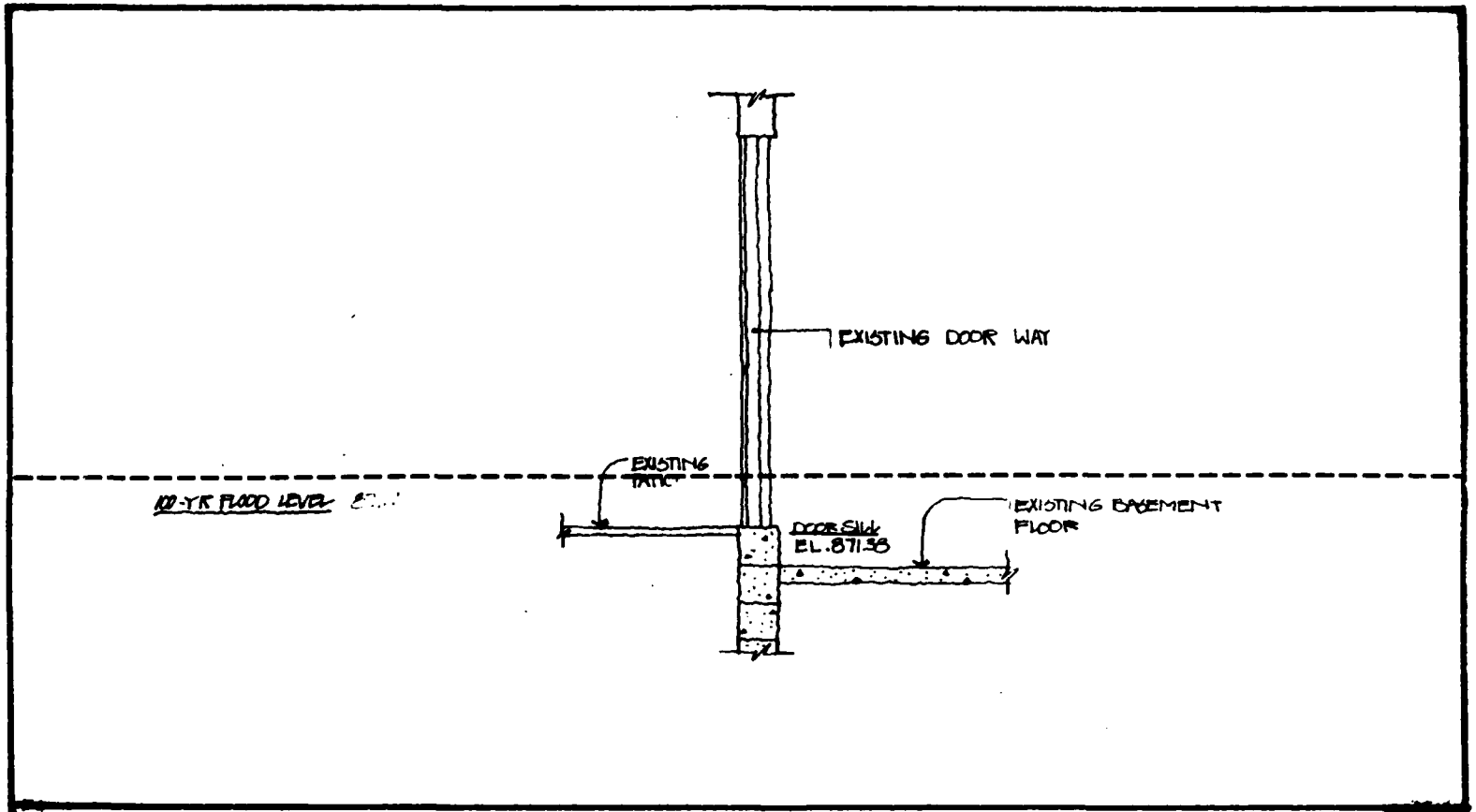
PLATE H-43



PROPOSED WALL SECTION
SCALE 1½"=1'
KNIGHT RESIDENCE/1205 IDAHO AVE N.

3

PLATE H-44



EXISTING WALL SECTION
SCALE 1/2"=1'
KNIGHT RESIDENCE/1205 IDAHO AVE N.

4

PLATE N-45

Plate N-45

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE WEST RESIDENCE LOCATED AT 2425 REGENT AVENUE AND THE STREU RESIDENCE LOCATED AT 2445 REGENT AVENUE, GOLDEN VALLEY, MINNESOTA: THE LOW OPENINGS FOR THESE RESIDENCES ARE 842.6 AND 841.78, RESPECTIVELY. THE 10!-YEAR FLOOD LEVEL IS 843.0. WE MUST FLOOD-PROOF TO A LEVEL OF 844.0

WE HAVE DETERMINED THAT A BERM ACROSS THE REAR YARDS OF BOTH HOMES WITHIN THE WILDLIFE PRESERVE WILL BE THE MOST COST EFFECTIVE SOLUTION. WE WILL THEN PROVIDE A SUMP PUMP TO REMOVE INTERIOR DRAINAGE.

SEE PLATES H-46 AND H-47.

SUMMARY OF ELEVATIONS - 2425 REGENT AVENUE

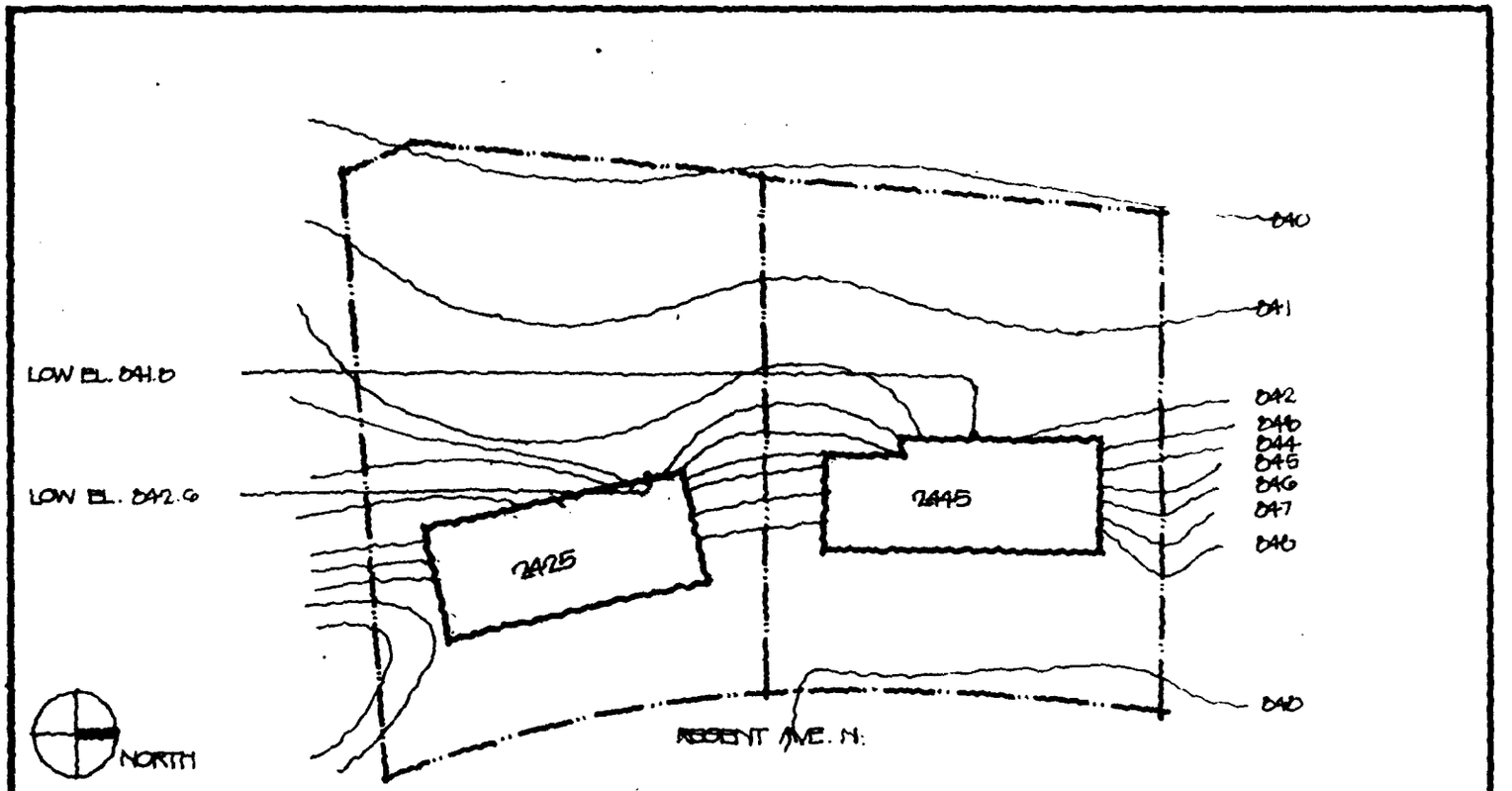
DOOR	842.6
FIRST FLOOR	851.1
100-YEAR FLOOD LEVEL	843.0

SUMMARY OF ELEVATIONS - 2445 REGENT AVENUE

DOOR SILL	841.78
WINDOR SILL	844.13
FIRST FLOOR	849.60
100-YEAR FLOOD LEVEL	843.0

CONSTRUCTION ESTIMATE
2425 AND 2445 REGENT AVENUE NORTH, GOLDEN VALLEY, MINNESOTA

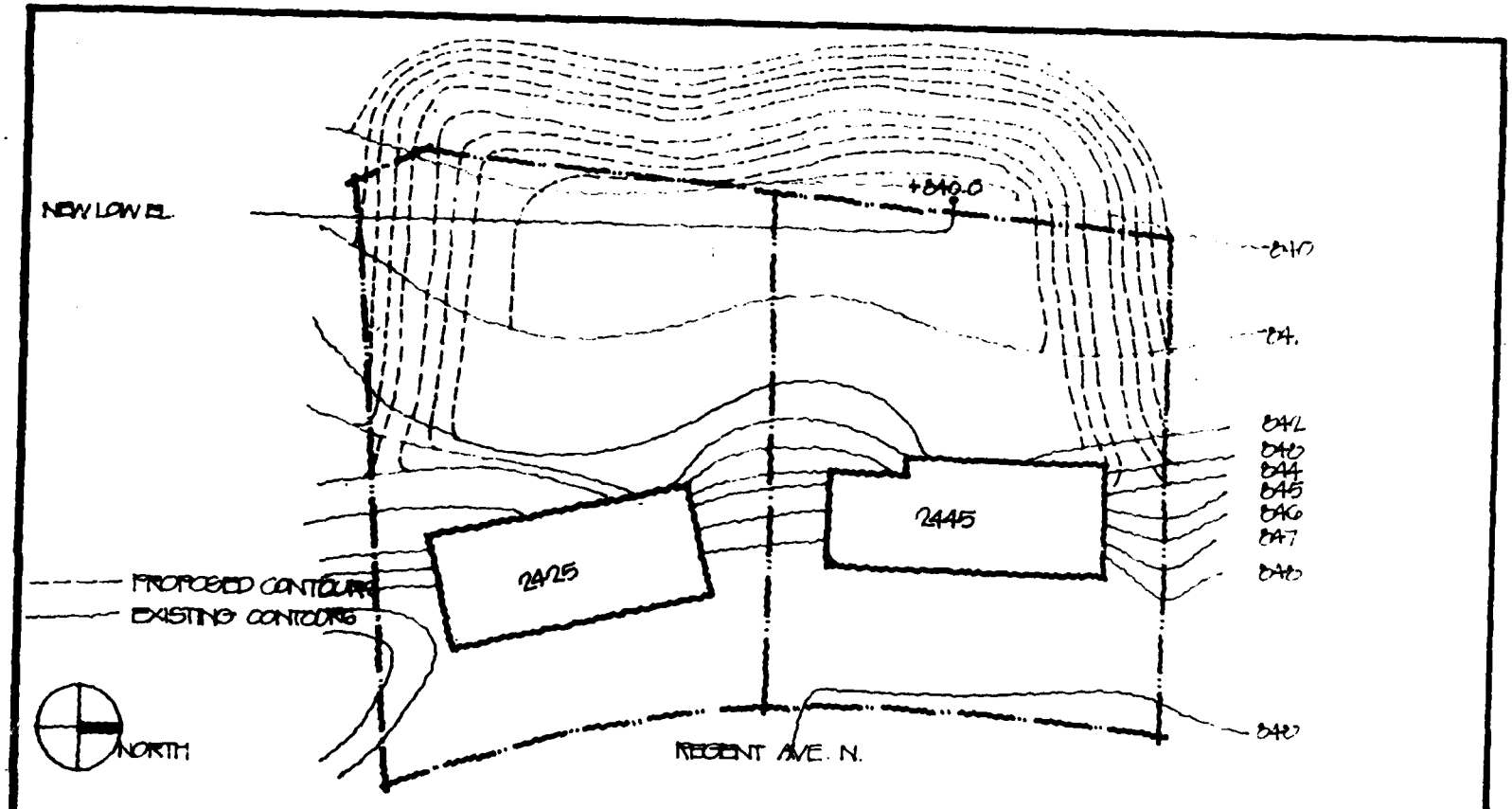
EARTHWORK	
COMMON BORROW	$ 3,550.00
SOD WITH TOPSOIL	2,450.00
SEEDING WITH TOPSOIL	700.00
INTERIOR DRAINAGE	6,050.00
TOTAL ESTIMATE	$12,750.00
OR	$ 6,375.00/HOME



EXISTING SITE PLAN
SCALE: 1"=30'
WEST/STREU RESIDENCE / 2425.2445 REGENT AVE.

1

PLATE H-46



PROPOSED SITE PLAN
SCALE: 1"=30'
WEST/STREU RESIDENCE / 2425, 2445 REGENT AVE.

2

PLATE N-42

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE POOSTER RESIDENCE LOCATED AT 2545 REGENT AVENUE, AND THE NESBITT RESIDENCE LOCATED AT 2505 REGENT AVENUE, GOLDEN VALLEY, MINNESOTA: THE LOW OPENINGS ARE 842.20 AND 842.51, RESPECTIVELY. THE 100-YEAR FLOOD LEVEL IS 843.0. WE MUST FLOOD-PROOF TO A LEVEL OF 844.0.

THE BEST SOLUTION TO THE FOLLOWING PROBLEMS OF THESE TWO HOMES WOULD BE TO CONSTRUCT A BERM OF TOPSOIL AND THEN TO SOD IT. THE SIMILAR CONTOURS AND LARGE REAR YARDS OF THESE TWO HOMES ADAPT VERY NICELY TO THIS SOLUTION. AN INTERIOR DRAINAGE SYSTEM WITH A SUMP PUMP WOULD ALSO HAVE TO BE CONSTRUCTED IN THIS CASE.

SEE PLATES H-48 AND H-49.

SUMMARY OF ELEVATIONS - 2545 REGENT AVENUE

DOOR SILL	842.20
WINDOW SILL	845.00
FIRST FLOOR	850.50

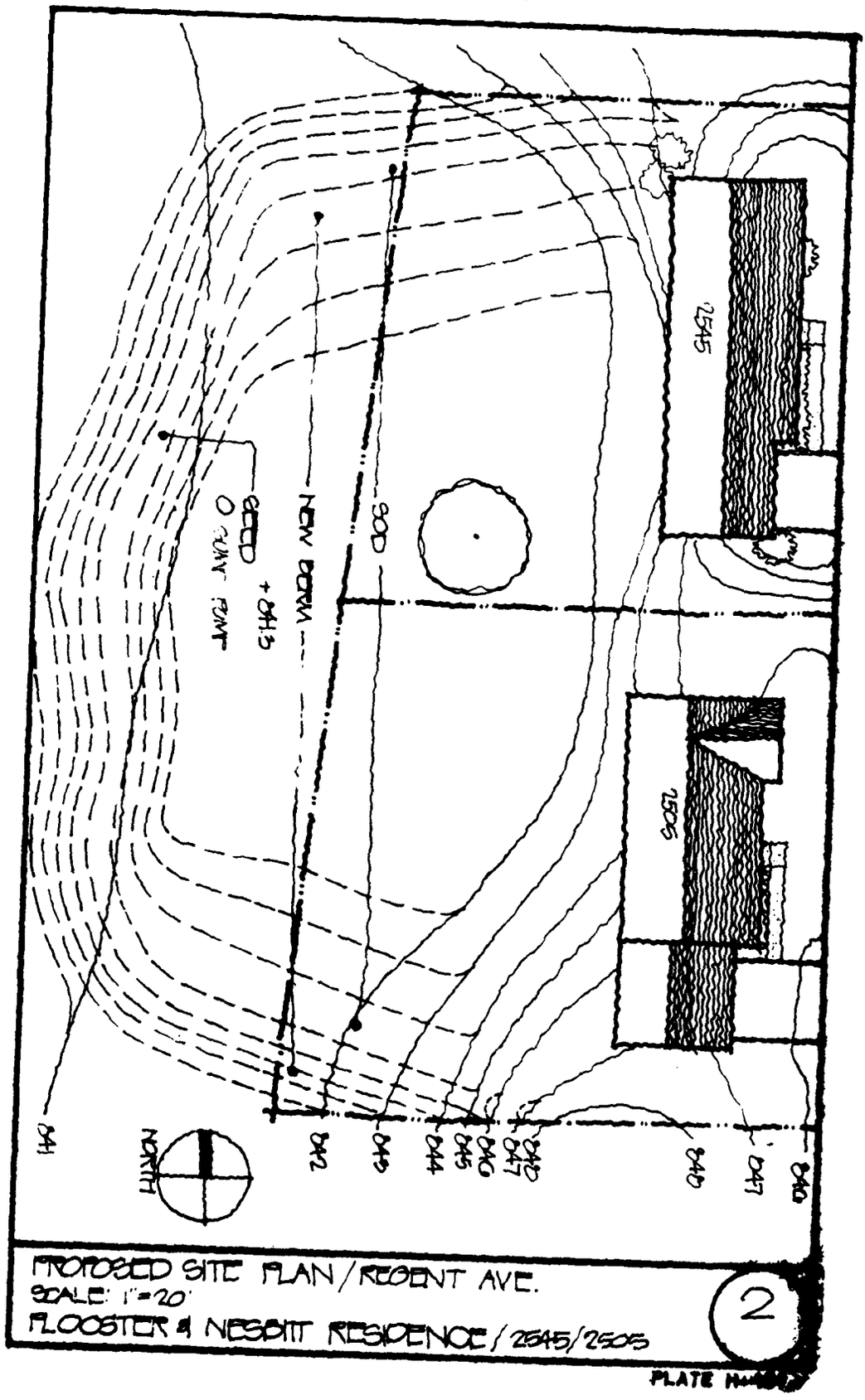
SUMMARY OF ELEVATIONS - 2505 REGENT AVENUE

DOOR SILL (SOUTH DOOR)	842.64
DOOR SILL (NORTH DOOR)	842.51
WINDOW SILL	844.71
FIRST FLOOR	850.60

CONSTRUCTION ESTIMATE
2505 AND 2545 REGENT AVENUE NORTH, GOLDEN VALLEY, MINNESOTA

EARTHWORK	
COMMON BORROW	$ 7,350.00
SOD WITH TOPSOIL	3,600.00
SEEDING WITH TOPSOIL	600.00
INTERIOR DRAINAGE	6,050.00
STORM SEWER	4,250.00
TOTAL ESTIMATE	$21,850.00
OR	$10,925.00/HOME

2545
2505
EXISTING CONTOURS
PROPOSED CONTOURS
NORTH
EXISTING SITE PLAN
SCALE: 1"=20'
FLOOSTER & NESBITT RESIDENCE / 2545/2505
PLATE H-468



PROPOSED SITE PLAN / REGENT AVE.
SCALE: 1"=20'
FLOOSTER & NESBITT RESIDENCE / 2545/2505

2

PLATE NUMBER

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE QUINN RESIDENCE LOCATED AT 3810 BASSETT CREEK DRIVE, GOLDEN VALLEY, MINNESOTA: THE EXISTING ONE HUNDRED YEAR FLOOD LEVEL IS 831.4. THE PROPOSED ONE HUNDRED YEAR FLOOD LEVEL IS 830.7. THE LOW OPENING FOR THE RESIDENCE IS 829.44. WE MUST FLOOD-PROOF TO A LEVEL OF 831.7, WHICH IS ONE FOOT ABOVE THE ONE HUNDRED YEAR FLOOD LEVEL.

THE SOIL CONDITIONS UPON WHICH THE QUINN RESIDENCE RESTS ARE NOT SUITABLE FOR THE CONSTRUCTION OF A CONCRETE BLOCK WALL SECTION BUT PROVIDE SUITABLE CHARACTERISTICS FOR CONSTRUCTION OF A TIMBER WALL. THE HOME IS CONSTRUCTED ON PILINGS. THE GRADE AT THE REAR OF THE SITE SLOPES AWAY FROM THE RESIDENCE TOWARD THE CREEK. THE DISTANCE FROM THE HOME TO THE CREEK IS SHORT.

THE POOR SOIL CONDITIONS COMBINED WITH THE RELATIVELY SHORT DISTANCE FROM THE HOME TO THE CREEK ELIMINATES THE USE OF A BERM TO SOLVE THE FLOODING PROBLEM. WE RECOMMEND THE USE OF A TIMBER WALL AROUND THE REAR OF THIS HOME AND 3820 BASSETT CREEK DRIVE TYING INTO THE HIGH GROUND AT THE ENDS OF THE COMMON WALL. WE WILL PROVIDE A COMMON SUMP PUMP PIT TO REMOVE THE INTERNAL DRAINAGE WATER FOR 3810 AND 3820 BASSETT CREEK DRIVE.

SEE PLATES H-50 THROUGH H-53 AND PLATE H-57.

THE BERKSETH/KEMPF RESIDENCE LOCATED AT 3820 BASSETT CREEK DRIVE, GOLDEN VALLEY, MINNESOTA: THE EXISTING ONE HUNDRED YEAR FLOOD LEVEL IS 831.4. THE PROPOSED ONE HUNDRED YEAR FLOOD LEVEL IS 830.7. THE LOW OPENING ELEVATION OF THE RESIDENCE IS 829.94. WE MUST FLOOD-PROOF TO A LEVEL OF 831.7, WHICH 1.76 FEET ABOVE THE LOW OPENING.

THE SOIL CONDITIONS AT THE BERKSETH/KEMPF RESIDENCE ARE NOT SUITABLE FOR THE CONSTRUCTION OF A CONCRETE BLOCK WALL, BUT PROVIDE SUITABLE CHARACTERISTICS FOR THE CONSTRUCTION OF A TIMBER WALL. THE HOME IS BUILT UPON PILINGS. THE SITE HAS MANY LARGE TREES AND IS RELATIVELY CLOSE TO THE CREEK IN THE REAR YARD. WE FEEL THAT THE POOR SOIL, THE NARROW REAR YARD, AND THE TREES ELIMINATE THE USE OF A BERM.

WE PROPOSED TO SOLVE THE PROBLEM WITH THE CONSTRUCTION OF A TIMBER WALL. ONE SET OF STEPS WOULD BE CONSTRUCTED TO PROVIDE ACCESS OVER THE WALL. A SUMP PUMP PIT WITH A SUBMERSIBLE SUMP PUMP WILL BE CONSTRUCTED BEHIND THE WALL TO REMOVE ANY INTERNAL DRAINAGE WATER FOR 3810 AND 3820 BASSETT CREEK DRIVE.

SEE PLATES H-54 THROUGH H-57.

SUMMARY OF ELEVATIONS - 3810 BASSETT CREEK DRIVE

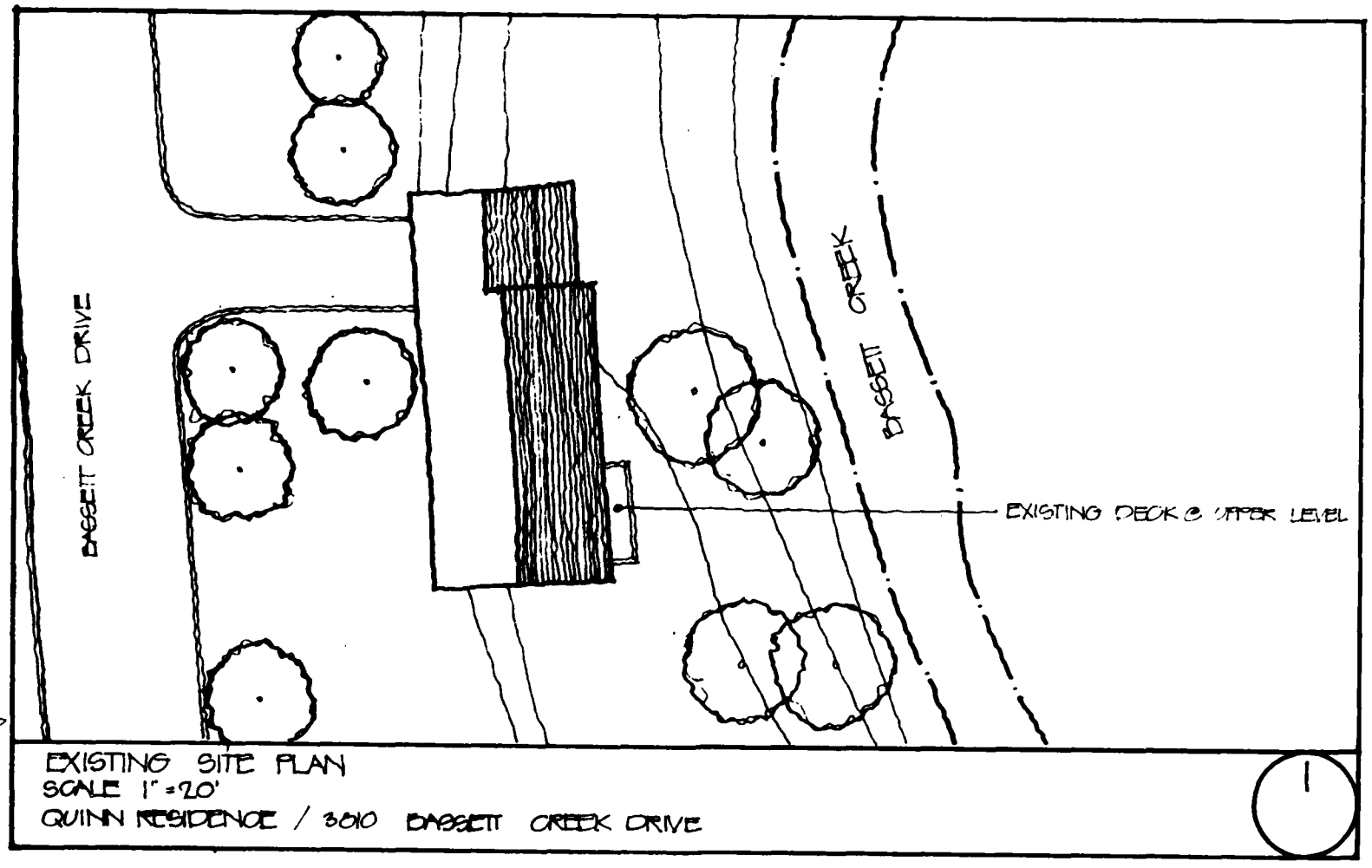
LOWER DOOR	829.53
WINDOW	831.75
DECK	837.94
LOWER FLOOR	829.44
100 YEAR FLOOD LEVEL	830.4
FLOOD-PROOFING LEVEL	831.4

SUMMARY OF ELEVATIONS - 3820 BASSETT CREEK DRIVE

LOWER LEVEL FLOOR	829.94
LOWER LEVEL DOOR	829.99
WINDOW SILL	833.00
UPPER LEVEL	838.41
100 YEAR FLOOD LEVEL (PROPOSED)	830.7
100 YEAR FLOOD LEVEL (EXISTING)	831.4

CONSTRUCTION ESTIMATE:
3810 AND 3820 BASSETT CREEK DRIVE

EARTHWORK	
EXCAVATION AND BACKFILL	$ 4,400.00
SOD WITH TOPSOIL	2,000.00
CARPENTRY	
TIMBER WALL	17,550.00
STEPS	675.00
DRAINAGE	
SUMP PUMP WITH SUMP	6,050.00
DRAINTILE WITH FILTER	2,200.00
TOTAL ESTIMATE	$32,875.00
OR	$16,437.50/HOME



EXISTING SITE PLAN
SCALE 1" = 20'
QUINN RESIDENCE / 3010 BASSETT CREEK DRIVE

1

PLATE H-50

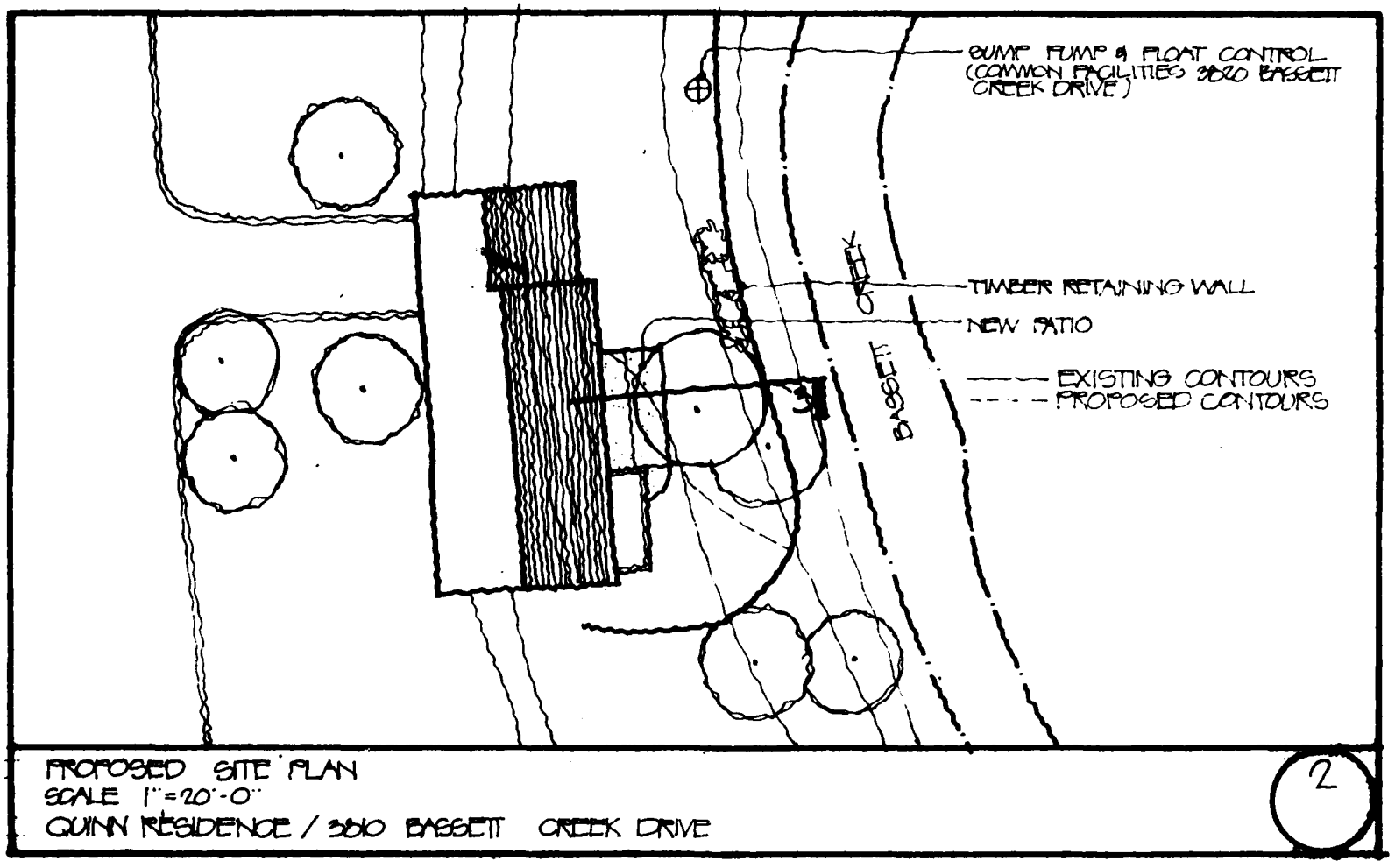
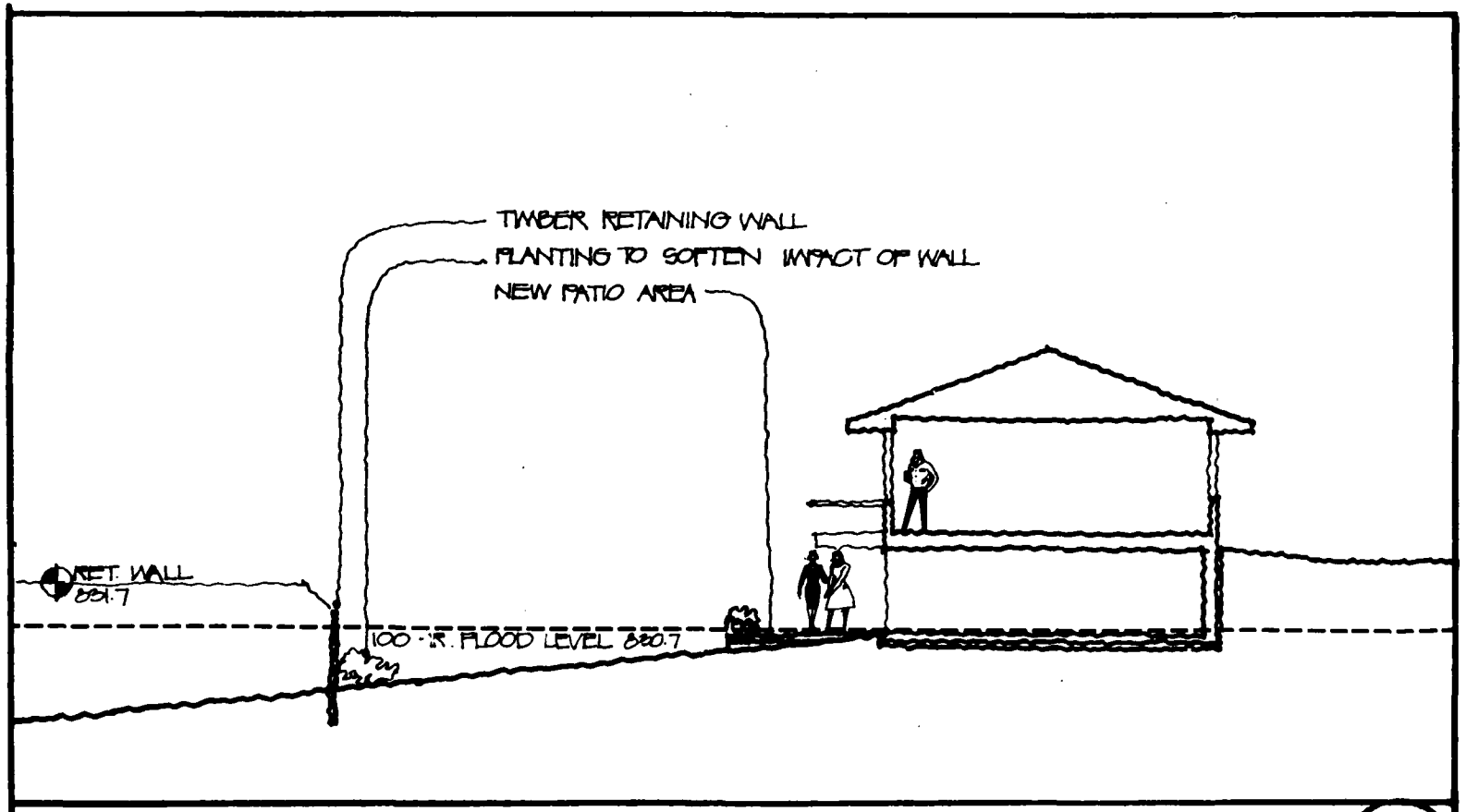

SUMP PUMP & FLOAT CONTROL
(COMMON FACILITIES 3820 BASSETT
CREEK DRIVE)
TIMBER RETAINING WALL
NEW PATIO
EXISTING CONTOURS
PROPOSED CONTOURS
BASSETT CREEK
PROPOSED SITE PLAN
SCALE 1"=20'-0"
QUINN RESIDENCE / 3810 BASSETT CREEK DRIVE
2

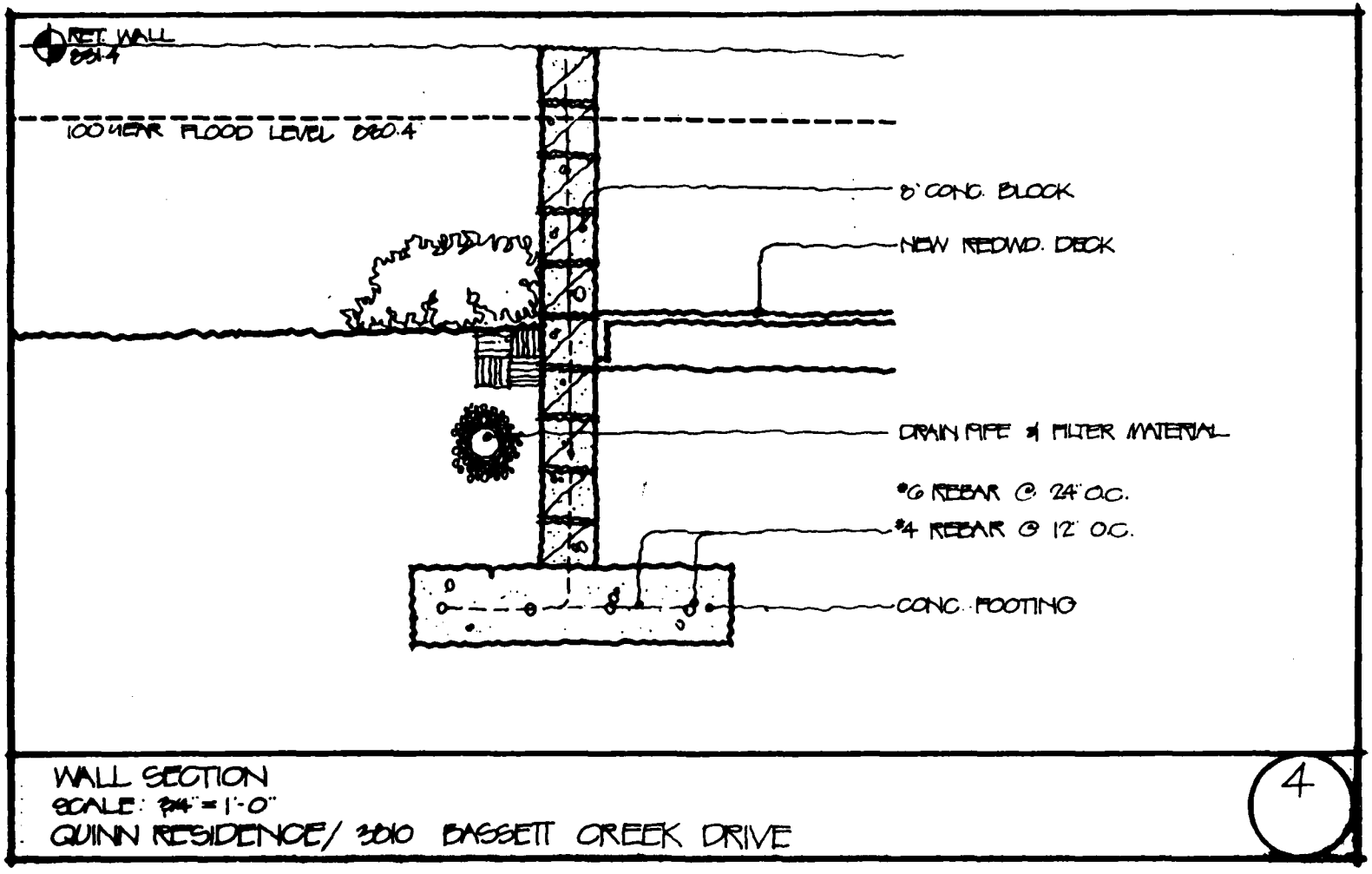
PLATE H-5I



SECTION
SCALE: 1/8" - 1'-0"
QUINN RESIDENCE / 5010 BASSETT CREEK DRIVE

3

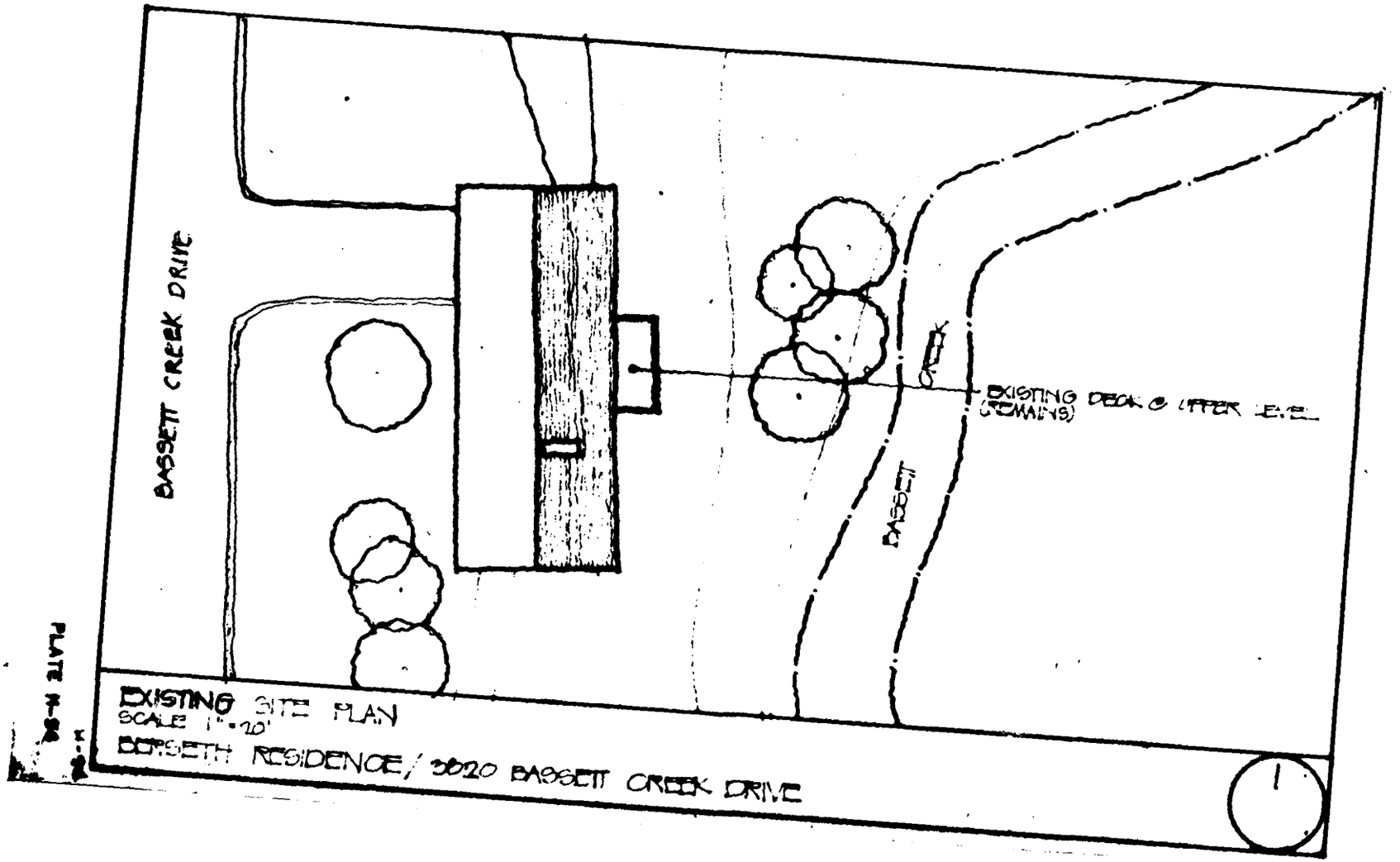
PLATE H-32
H-32



WALL SECTION
SCALE: 3/4" = 1'-0"
QUINN RESIDENCE/ 3010 BASSETT CREEK DRIVE

4

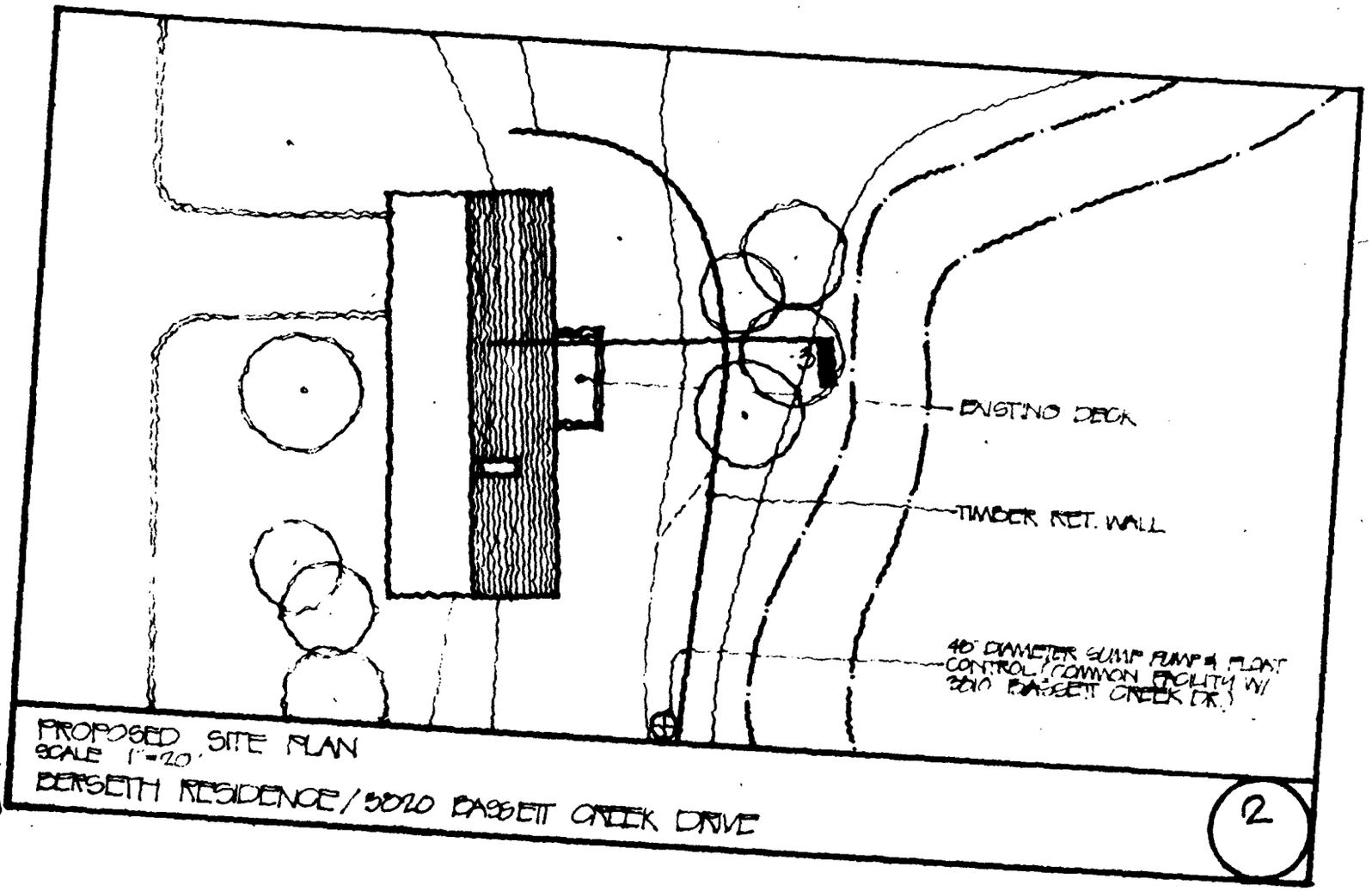
PLATE H-53



EXISTING SITE PLAN
SCALE 1"=20'

BERSETH RESIDENCE / 3020 BASSETT CREEK DRIVE

1

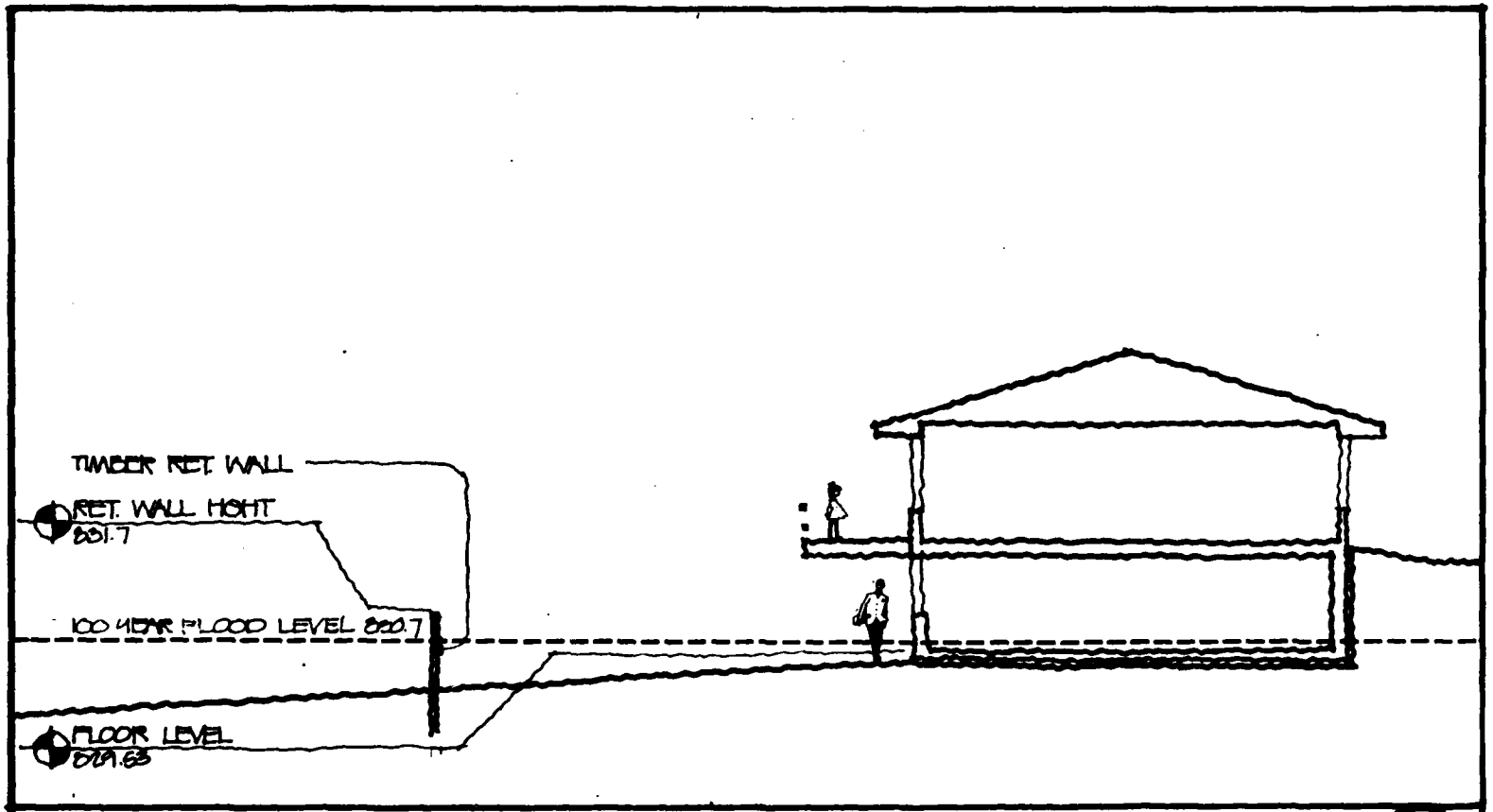


PROPOSED SITE PLAN
SCALE 1"=20'

BERSETH RESIDENCE / 5020 BASSETT CREEK DRIVE

2

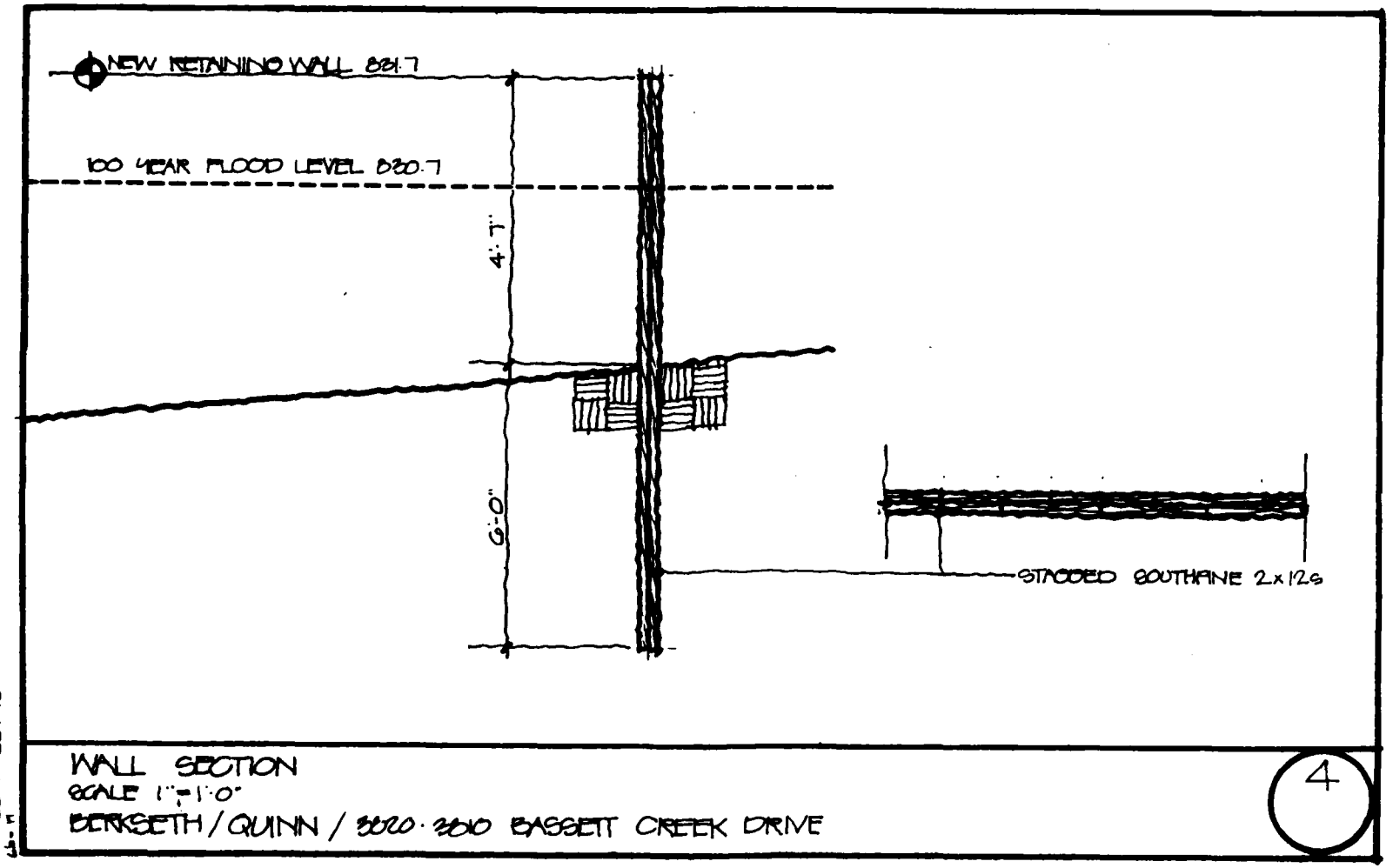
PLATE NO. 3



SECTION
SCALE : 1/8" = 1'-0"
BERKSETH RESIDENCE / 3020 BASSETT CREEK DRIVE

3

PLATE H-98



WALL SECTION
SCALE 1"=1'-0"
BERKSETH/QUINN / 3020-3010 BASSETT CREEK DRIVE

4

PLATE H-97

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE MOLLER RESIDENCE LOCATED AT 4975 BASSETT CREEK DRIVE, GOLDEN VALLEY, MINNESOTA: THE LOW OPENING OF THIS HOME IS 841.14. THE 100-YEAR FLOOD LEVEL IS 841.7. WE MUST FLOOD-PROOF TO A LEVEL OF 842.7.

THE REAR YARD OF THE RESIDENCE IS NOT DEEP ENOUGH TO ALLOW FOR THE CONSTRUCTION OF A BERM. THE CITY SANITARY SEWER ALSO RUNS THROUGH THE REAR YARD. THEREFORE, THE BEST SOLUTION TO THE PROBLEM IS TO CONSTRUCT A RETAINING WALL OF CONCRETE BLOCK AROUND THE REAR OF THE HOME, TYING IT INTO THE EXISTING TOPOGRAPHY AT THE SIDES. WE MUST THEN CONSTRUCT AN INTERIOR DRAINAGE SYSTEM WITH A SUMP PUMP TO HANDLE INTERIOR DRAINAGE.

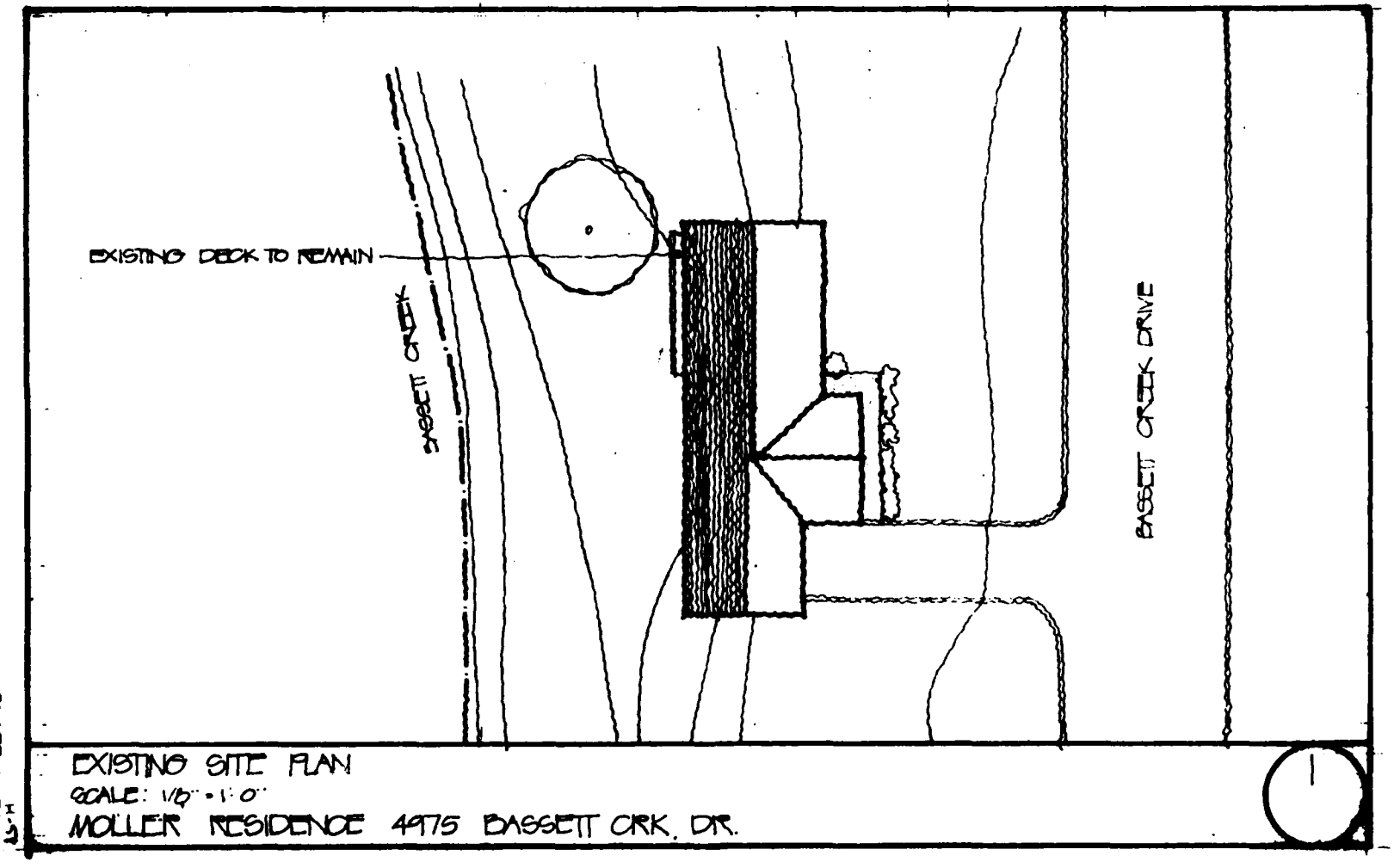
SEE PLATES H-58 THROUGH H-61.

SUMMARY OF ELEVATIONS

DOOR SILL	841.14
WINDOW SILL	844.02
TOP OF DECK	848.88
100-YEAR FLOOD LEVEL	841.7
FLOOD-PROOF LEVEL	843.1

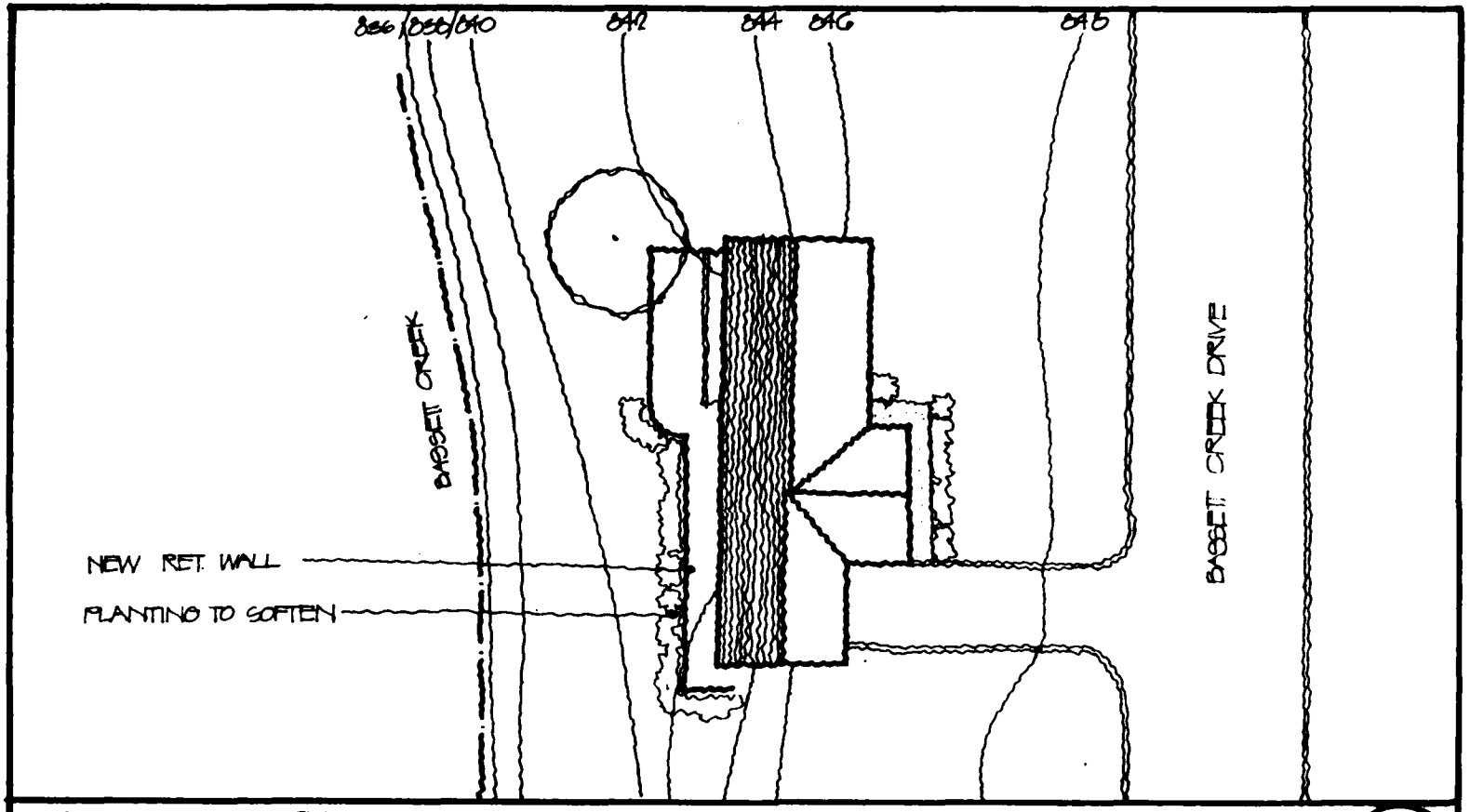
CONSTRUCTION ESTIMATE:
4975 BASSETT CREEK DRIVE, GOLDEN VALLEY, MINNESOTA

EARTHWORK	
EXCAVATION AND BACKFILL	$ 1,000.00
SOD WITH TOPSOIL	675.00
CONCRETE AND MASONRY	
REINFORCED CONCRETE	3,600.00
REINFORCED MASONRY	2,000.00
DRAINAGE	
SUMP PUMP WITH SUMP	6,050.00
DRAINTILE WITH FILTER	800.00
TOTAL ESTIMATE	$14,125.00



EXISTING SITE PLAN
SCALE: 1/8" = 1'-0"
MOLLER RESIDENCE 4975 BASSETT CRK. DR.

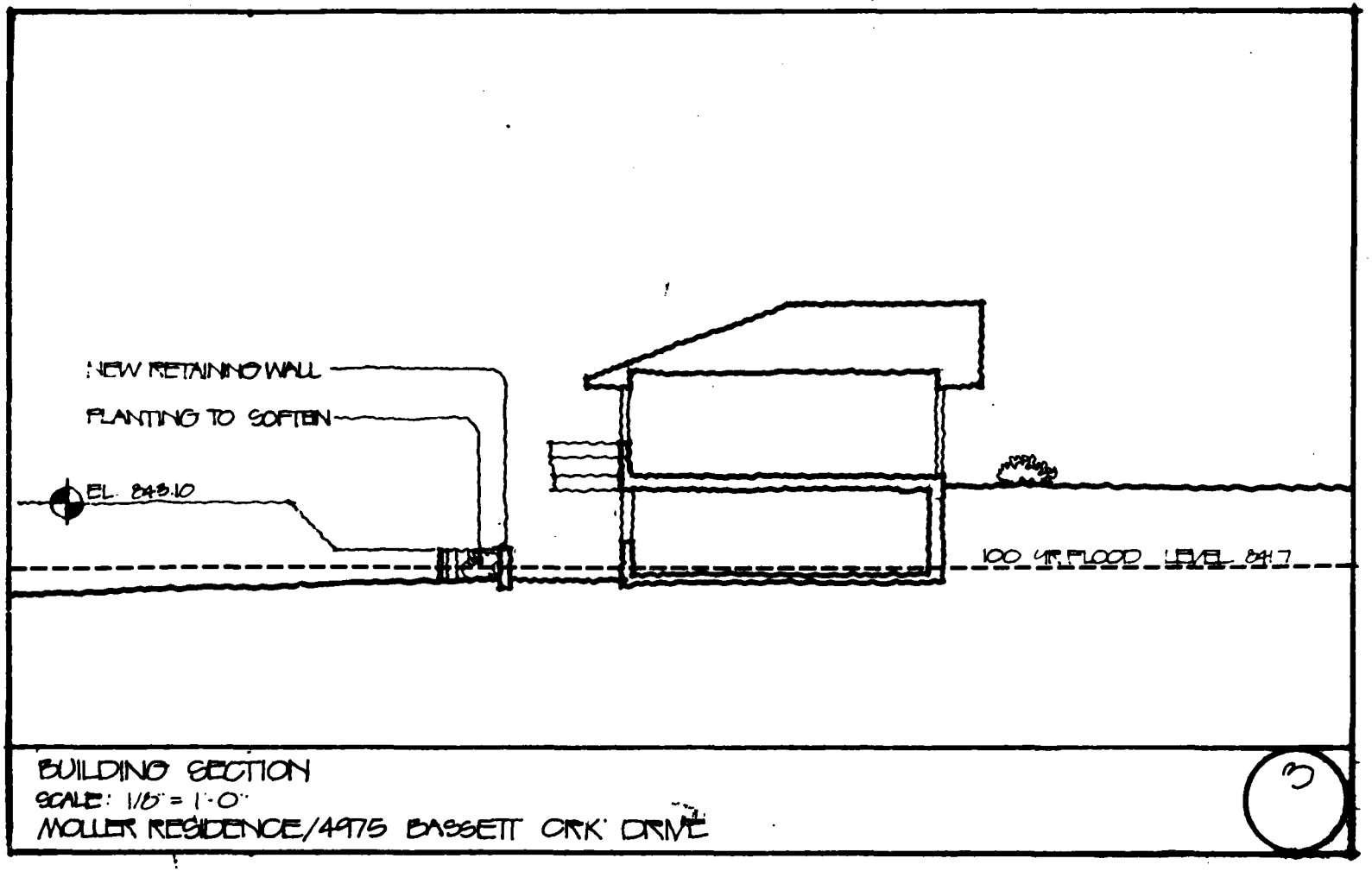
PLATE H-58



PROPOSED SITE PLAN
SCALE: 1/8" = 1'-0"
MOLLER RESIDENCE / 4975 BASSETT CRK. DR.

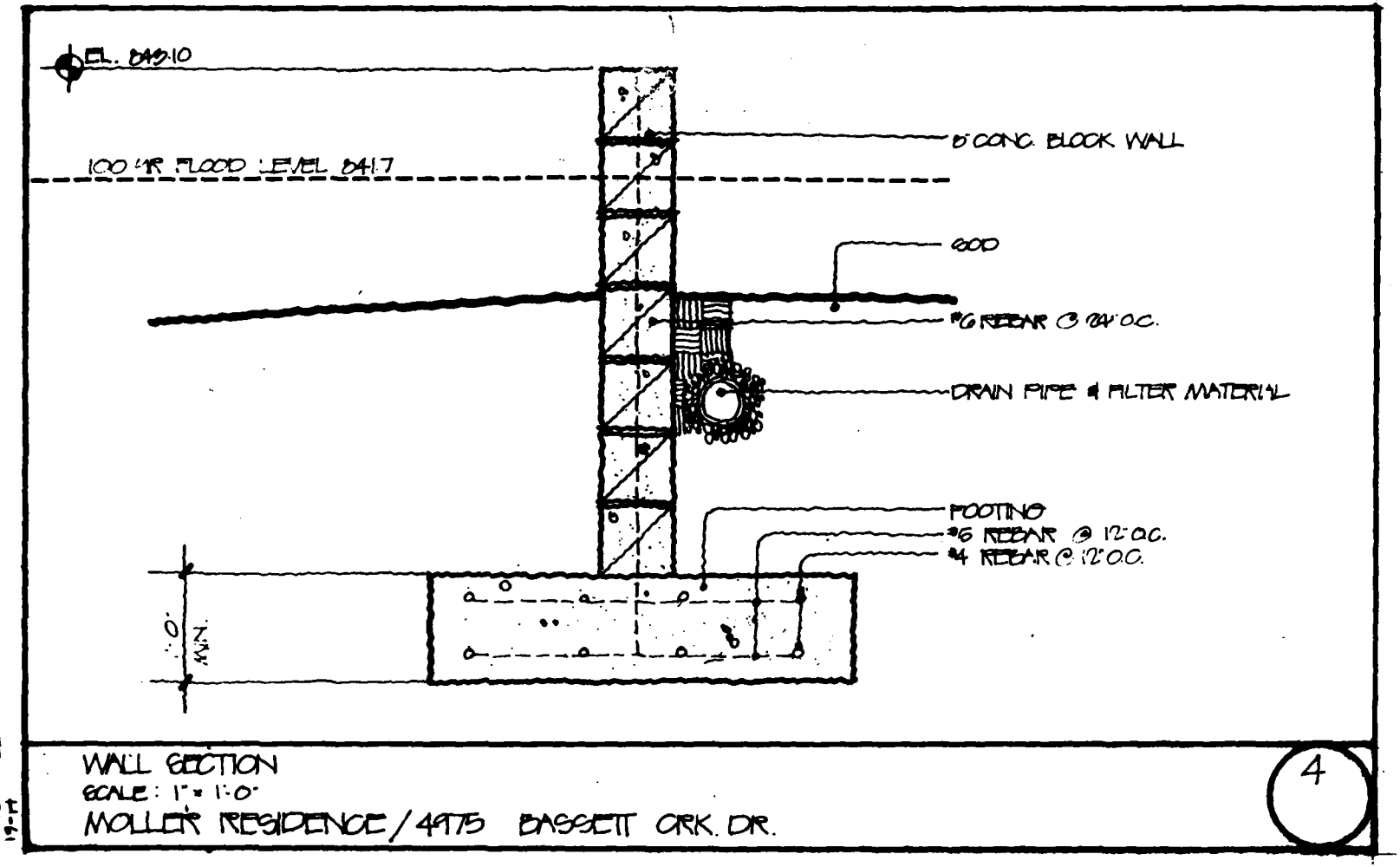
2

PLATE H-59



BUILDING SECTION
SCALE: 1/8" = 1'-0"
MOLLER RESIDENCE/4975 BASSETT CRK. DRIVE

PLATE H-60



WALL SECTION
SCALE: 1" = 1'-0"
MOLLER RESIDENCE / 4975 BASSETT CRK. DR.

4

PLATE H-61

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE GUENTHER RESIDENCE LOCATED AT 5005 BASSETT CREEK DRIVE, GOLDEN VALLEY, MINNESOTA: THE LOW OPENING FOR THE HOME IS 840.75. THE 100-YEAR FLOOD LEVEL IS 841.7. WE MUST FLOOD-PROOF TO A LEVEL AT LEAST 1 FOOT ABOVE THE 100-YEAR LEVEL, WHICH WOULD BE TWO COURSES OF BLOCK, OR AN ELEVATION OF 842.8.

THE REAR YARD OF THE RESIDENCE IS DEEP ENOUGH TO ALLOW THE CONSTRUCTION OF A BERM, HOWEVER, THE SLOPE FROM THE HOUSE WOULD REQUIRE SUCH A BERM TO BE QUITE HIGH. ALSO, THE LOT HAS A NUMBER OF LARGE TREES WHICH WOULD COMPLICATE THE CONSTRUCTION OF A BERM. ALSO, THE CITY SANITARY SEWER RUNS THROUGH THE REAR YARD AND WOULD NOT WITHSTAND THE WEIGHT OF A BERM WITHOUT MODIFICATION. AS A RESULT, WE ARE PROPOSING TO CONSTRUCT A CONCRETE BLOCK RETAINING WALL ALONG THE REAR OF THE HOME. WE WOULD HAVE TO VARY THE WALL TO SAVE ONE LARGE TREE NEAR THE HOUSE AS INDICATED ON THE PLANS.

AN INTERIOR DRAINAGE SYSTEM WILL BE PROVIDED.

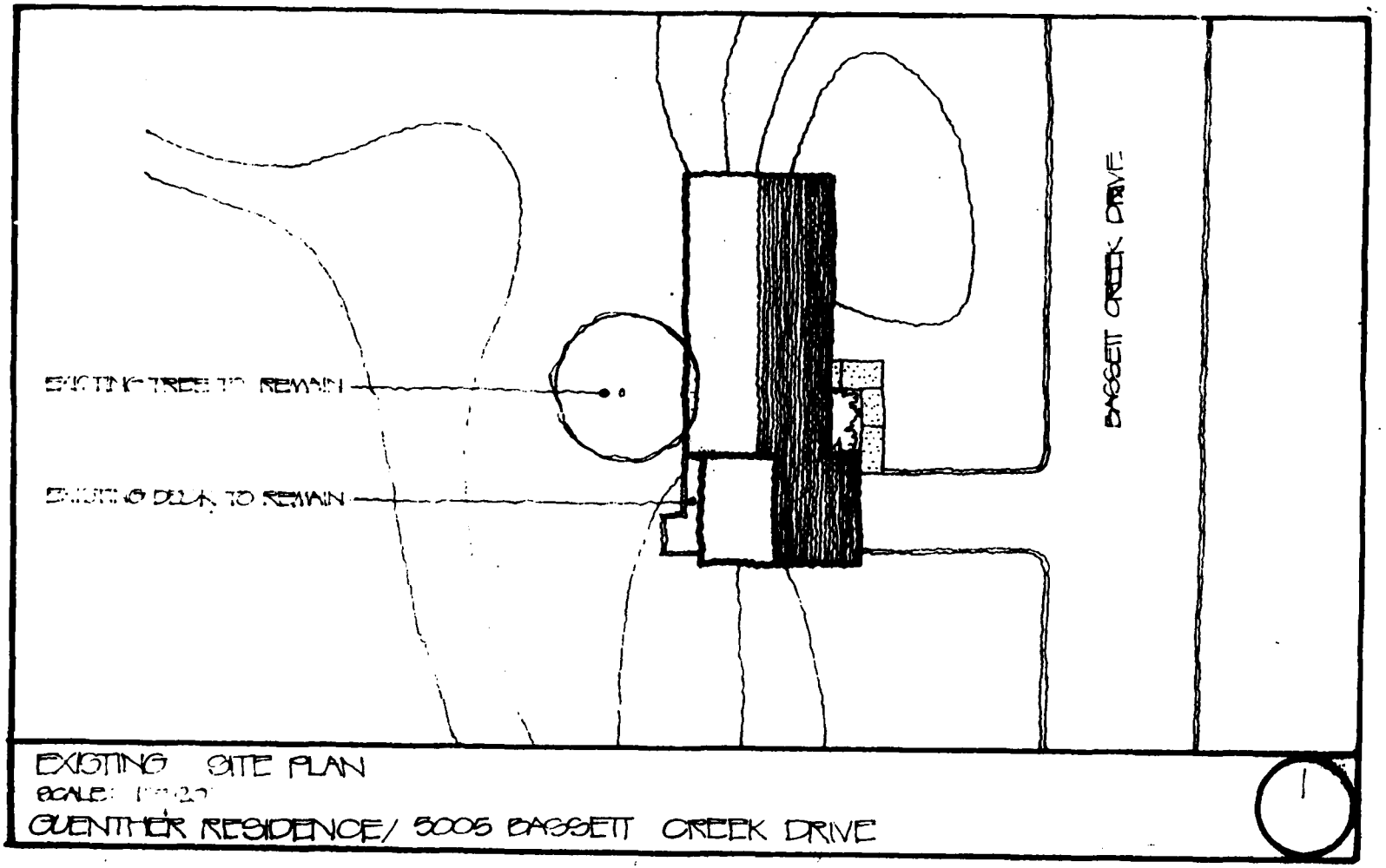
SEE PLATES H-63 THROUGH H-65

SUMMARY OF ELEVATIONS

DOOR SILL	840.75
WINDOW SILL	844.81
TOP OF DECK	848.10
WINDOW SILL	842.83
DOOR SILL	840.81
100-YEAR FLOOD LEVEL	841.7
FLOOD-PROOFING LEVEL	842.8

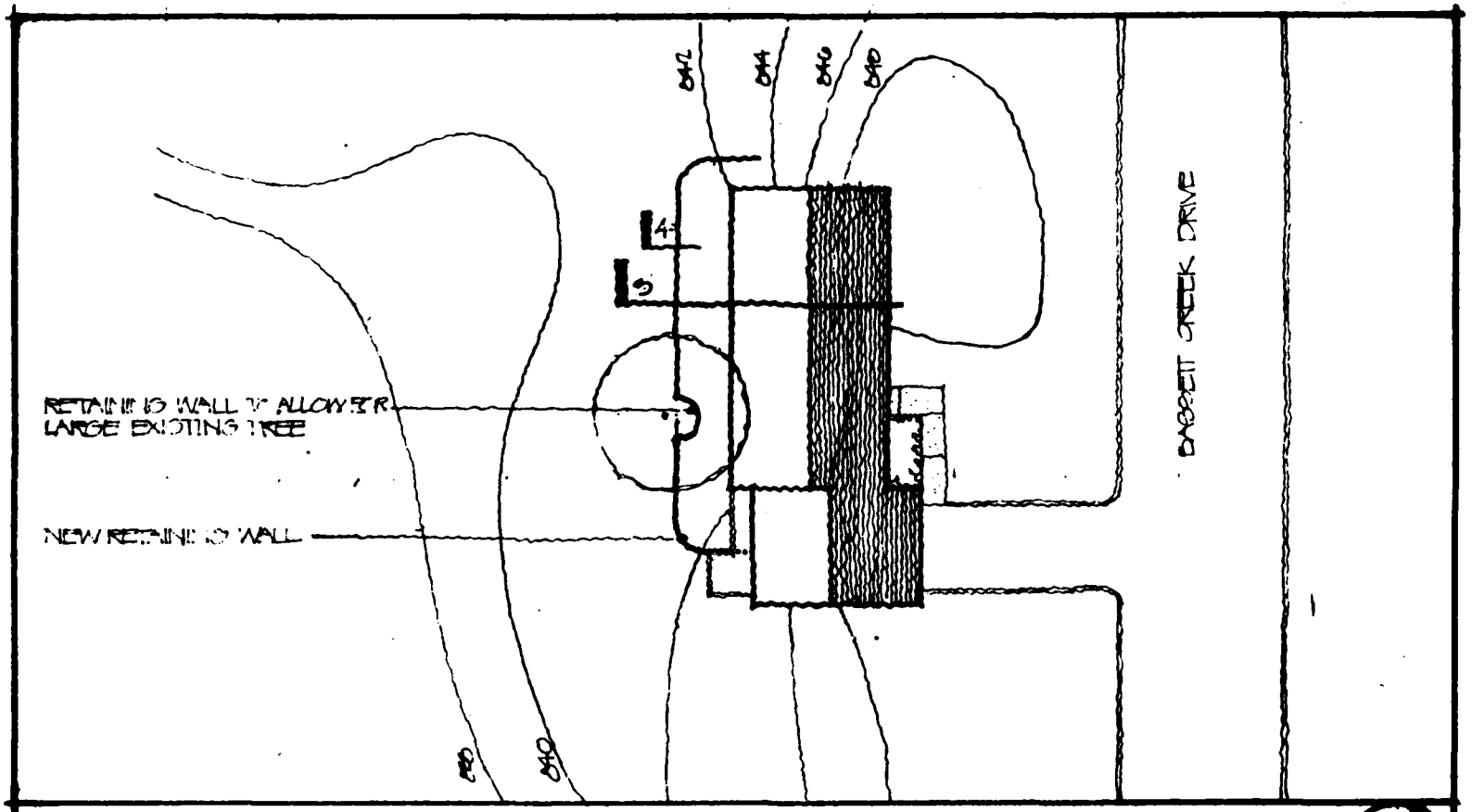
CONSTRUCTION ESTIMATE:
5005 BASSETT CREEK DRIVE, GOLDEN VALLEY, MINNESOTA

EARTHWORK	
EXCAVATION AND BACKFILL	$ 1,000.00
SOD WITH TOPSOIL	675.00
CONCRETE AND MASONRY	
REINFORCED CONCRETE	3,600.00
REINFORCED MASONRY	2,500.00
DRAINAGE	
SUMP PUMP WITH SUMP	6,050.00
DRAINTILE WITH FILTER	800.00
TOTAL ESTIMATE	$14,625.00



EXISTING SITE PLAN
SCALE: 1"=20'
GUENTHER RESIDENCE/ 5005 BASSETT CREEK DRIVE

PLATE H-62



PROPOSED SITE PLAN

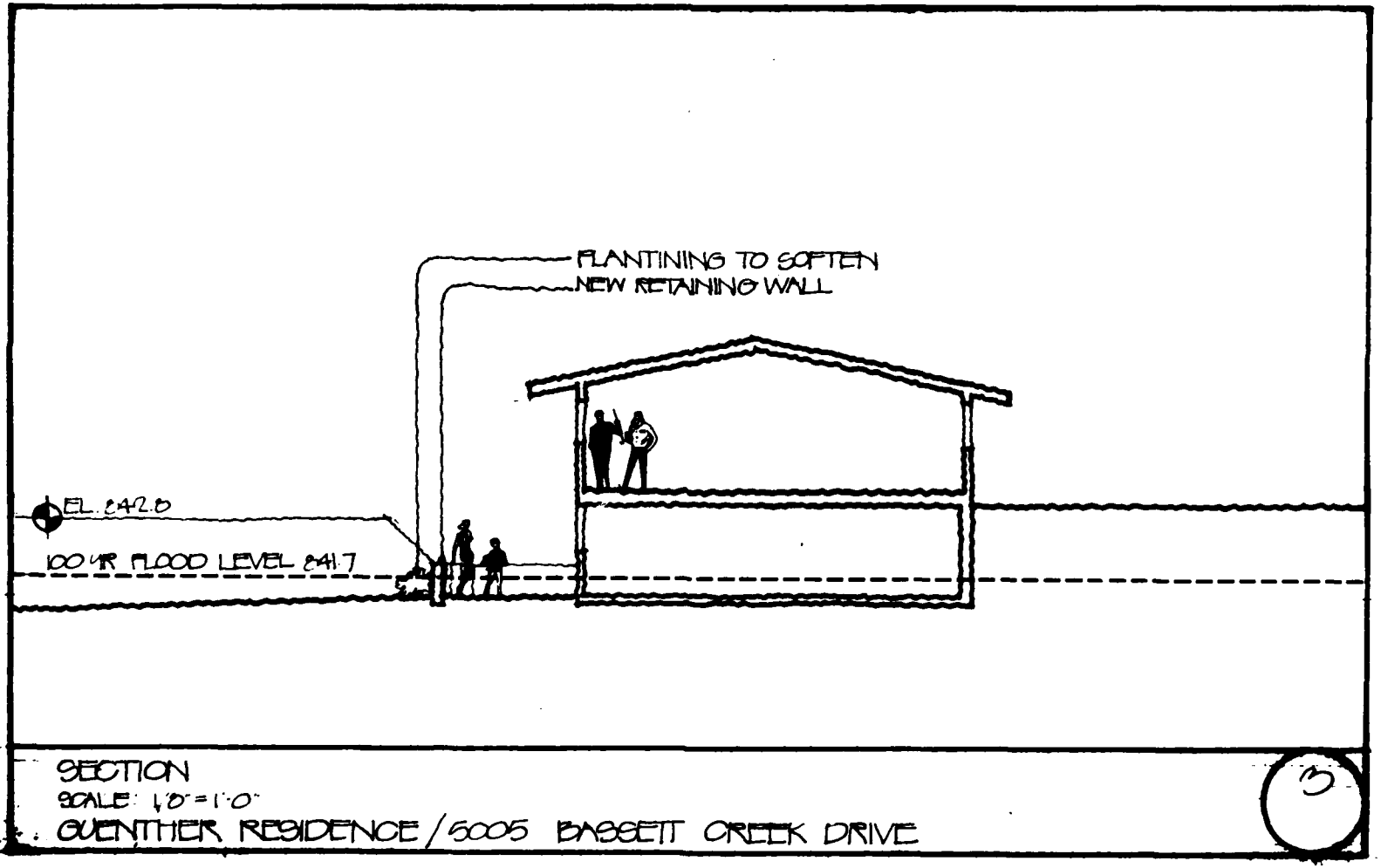
SCALE: 1" = 20'

GUENTHER RESIDENCE/ 5005 BASSETT CREEK DRIVE

2

PLATE H-63

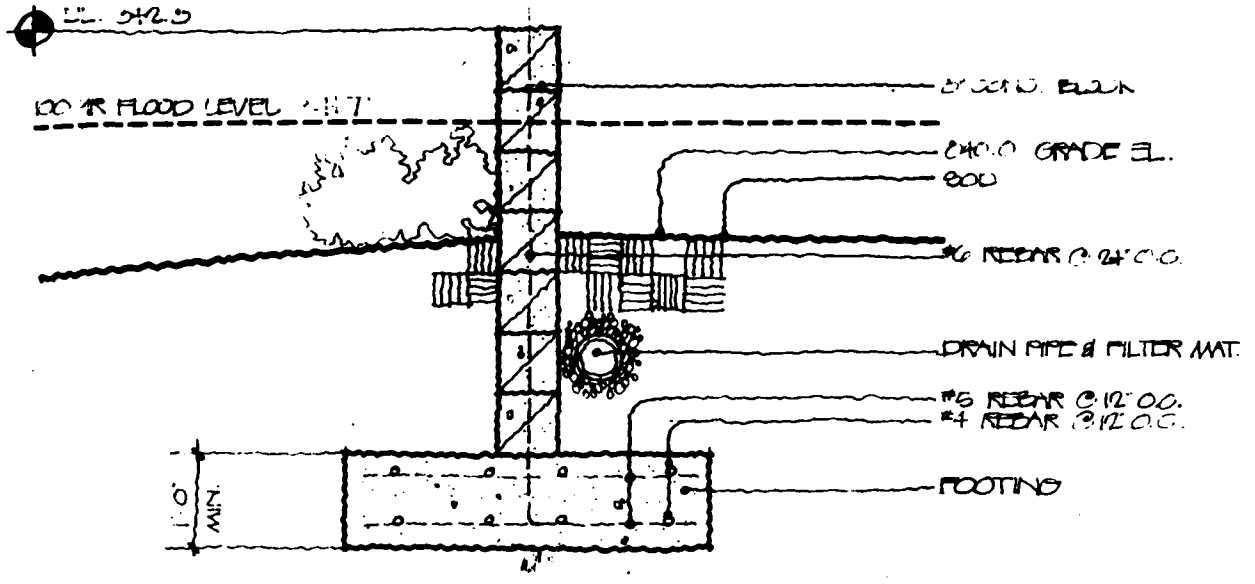
H-63



SECTION
SCALE: 1/8" = 1'-0"
GUENTHER RESIDENCE / 5005 BASSETT CREEK DRIVE

3

PLATE H-64



WALL SECTION
SCALE: 1/4" = 1'-0"
GUENTHER RESIDENCE / 5005 BASSETT CRK. DRIVE

4

PLATE H-65
H-65

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE JACOBSON RESIDENCE LOCATED AT 5126 MINNAQUA AVENUE, THE WILLETS RESIDENCE LOCATED AT 5120 MINNAQUA AVENUE, AND THE PATKA RESIDENCE LOCATED AT 5110 MINNAQUA AVENUE, GOLDEN VALLEY, MINNESOTA: THE EXISTING ONE HUNDRED YEAR FLOOD LEVEL IS 847.5. THE PROPOSED ONE HUNDRED YEAR FLOOD LEVEL IS 842.9. THE PROPOSED LOW OPENINGS ARE 841.98, 841.69, AND 842.01, RESPECTIVELY. WE MUST FLOOD-PROOF TO A LEVEL OF 843.9.

THE SOIL CONDITIONS ARE SUITABLE FOR THE CONSTRUCTION OF A BERM, AND THERE IS HIGH GROUND ADJACENT TO THE STRUCTURES. A BERM COULD BE TIED INTO THIS HIGH GROUND. THE DISTANCES TO THE CREEK IN THE REAR YARDS ARE ALSO COMPATIBLE WITH THE CONSTRUCTION OF A BERM.

WE PROPOSED THAT A BERM BE CONSTRUCTED WHICH WOULD BEGIN AT THE PROPERTY LINE TO THE WEST OF THE JACOBSON RESIDENCE, AND END OF THE PROPERTY LINE TO THE EAST OF MR. PATKA'S PROPERTY, AS INDICATED ON THE SITE PLAN. WE WILL PROVIDE ONE SUMP PUMP PIT TO BE LOCATED IN THE NORTHWEST CORNER OF THE JACOBSON PROPERTY FOR ALL THREE YARDS. THE SIZE OF THE PIT WILL BE 48 INCHES IN DIAMETER. WE WILL REPLACE A LARGE TREE IN THE JACOBSON YARD, AND TWO OR THREE TREES ON BOTH WILLET'S AND PATKA'S PROPERTIES IF THEY ARE DESTROYED. THE YARD WILL BE REGRADED AND SODDED.

SEE PLATES H-66 AND H-67.

SUMMARY OF ELEVATIONS
5126 MINNAQUA AVENUE

DOOR SILL	841.98
TOP DECK	850.06
EXISTING 100 YEAR FLOOD LEVEL	847.5
PROPOSED 100 YEAR FLOOD LEVEL	842.9

SUMMARY OF ELEVATIONS
5120 MINNAQUA AVENUE

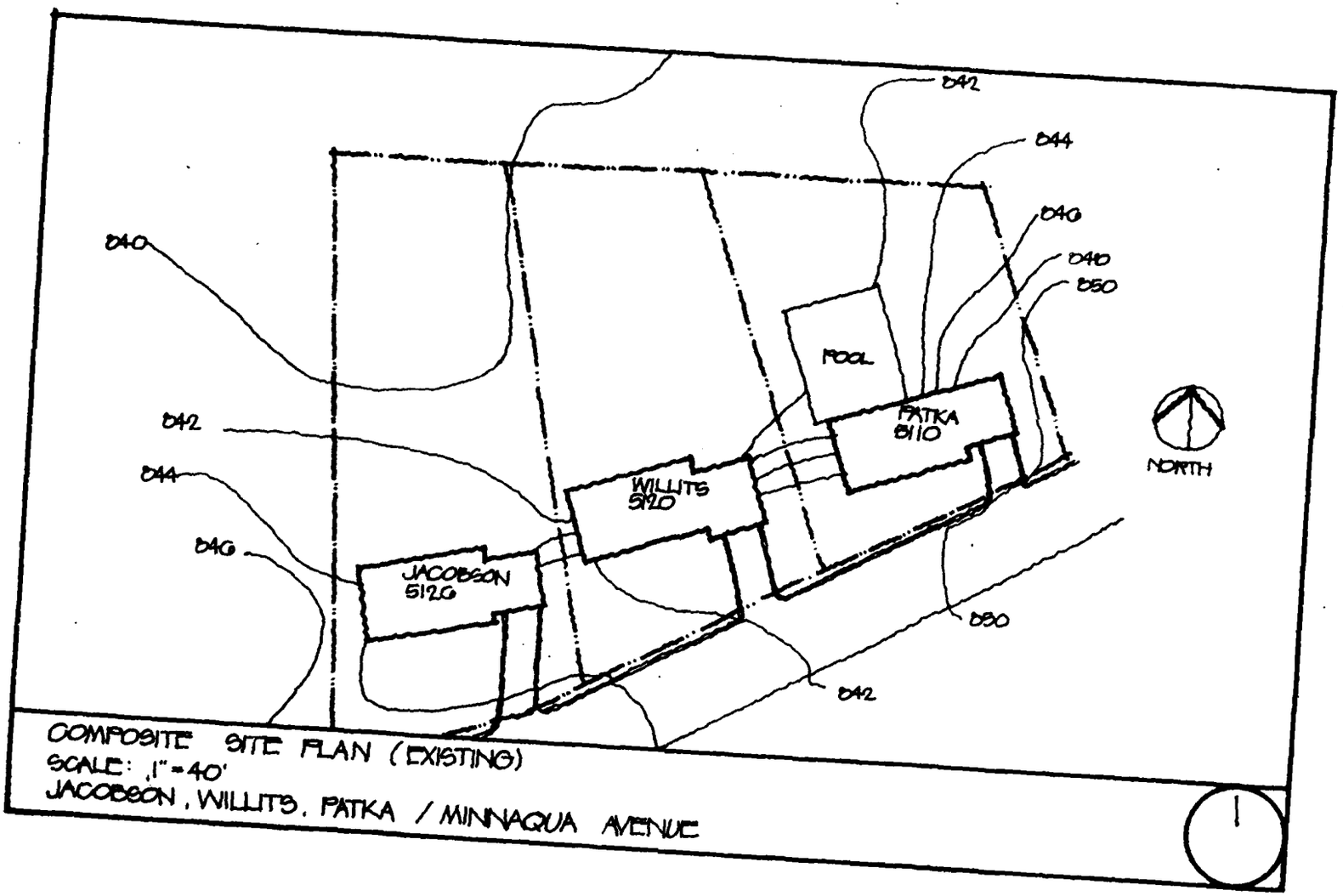
DOOR SILL	841.69
WINDOR SILL	843.93
FIRST FLOOR	849.9
EXISTING 100 YEAR FLOOD LEVEL	847.5
PROPOSED 100 YEAR FLOOD LEVEL	842.9

SUMMARY OF ELEVATIONS
5110 MINNAQUA AVENUE

DOOR SILL	842.01
WINDOW SILL	844.22
FIRST FLOOR	850.0
EXISTING 100 YEAR FLOOD LEVEL	847.5
PROPOSED 100 YEAR FLOOD LEVEL	842.9

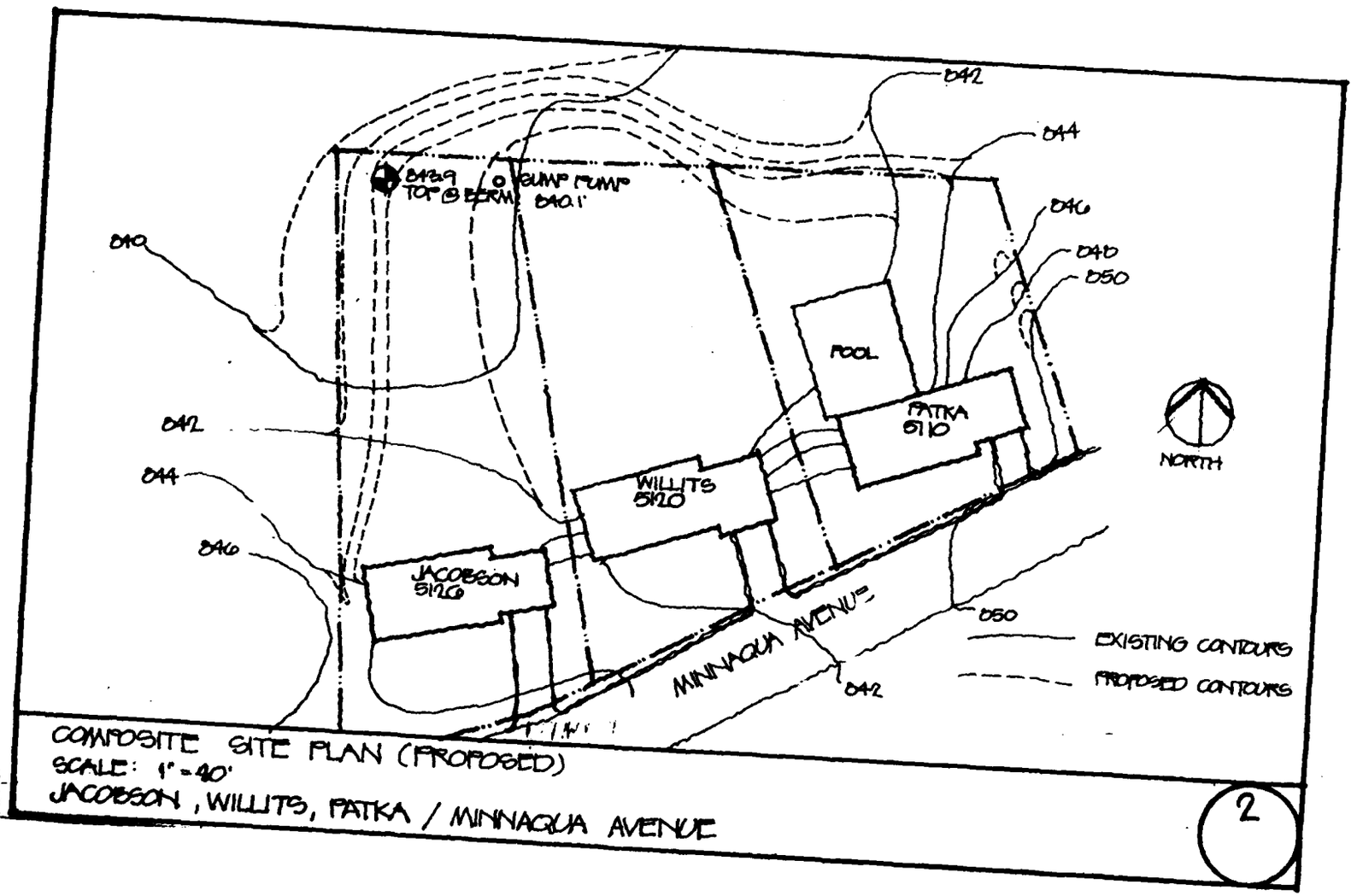
CONSTRUCTION ESTIMATE:
5126, 5120, AND 5110 MINNAQUA AVENUE, GOLDEN VALLEY, MINNESOTA

EARTH WORK	
COMMON BORROW	$ 8,475.00
SOD WITH TOPSOIL	6,200.00
SEEDING WITH TOPSOIL	700.00
INTERIOR DRAINAGE	6,750.00
TOTAL ESTIMATE	$22,125.00
OR	$ 7,375.00/HOME



COMPOSITE SITE PLAN (EXISTING)
SCALE: 1" = 40'
JACOBSON, WILLITS, PATKA / MINNAQUA AVENUE

PLATE H-66



COMPOSITE SITE PLAN (PROPOSED)
SCALE: 1" = 40'
JACOBSON, WILLITS, PATKA / MINNAQUA AVENUE

2

PLATE H-67

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE HEDBERG RESIDENCE LOCATED AT 5125 MINNAQUA AVENUE, GOLDEN VALLEY, MINNESOTA: THE EXISTING ONE HUNDRED YEAR FLOOD LEVEL IS 847.4. THE PROPOSED ONE HUNDRED YEAR FLOOD LEVEL IS 842.9. THE LOW OPENING IN THE RESIDENCE IS 841.41, WHICH IS A DOOR OPENING.

THE SOILS AROUND THE REAR OF THE HOME ARE SUITABLE FOR THE CONSTRUCTION OF A BERM. THE GRADE ON BOTH SIDES OF THE RESIDENCE IS HIGH AND SUITABLE TO BERM AGAINST. THE REAR YARD IS FAR ENOUGH FROM THE CREEK TO ALLOW BERMING. THE INTERIOR OF THE HOME IS COMPLETELY FINISHED.

WE RECOMMEND FLOOD-PROOFING THE HOME BY MEANS OF A BERM AROUND THE RESIDENCE. WE WILL CONSTRUCT THE BERM AS INDICATED ON THE PROPOSED SITE PLAN WHICH ACCOMPANIES THIS SUBMITTAL. WE WILL GRADE THE SITE TO ALLOW FOR THE STORAGE OF A TEN YEAR FREQUENCY RAIN FALL WITHOUT THE OCCURENCE OF FLOOD DAMAGE IN THE EVENT OF A PUMP FAILURE (MECHANICAL OR ELECTRICAL). WE WILL PROVIDE A SUMP PUMP PIT INSIDE THE BERM TO REMOVE THE INTERIOR DRAINAGE WITH ADEQUATE CAPACITY FOR THE CRITICAL 100-YEAR FREQUENCY STORM.

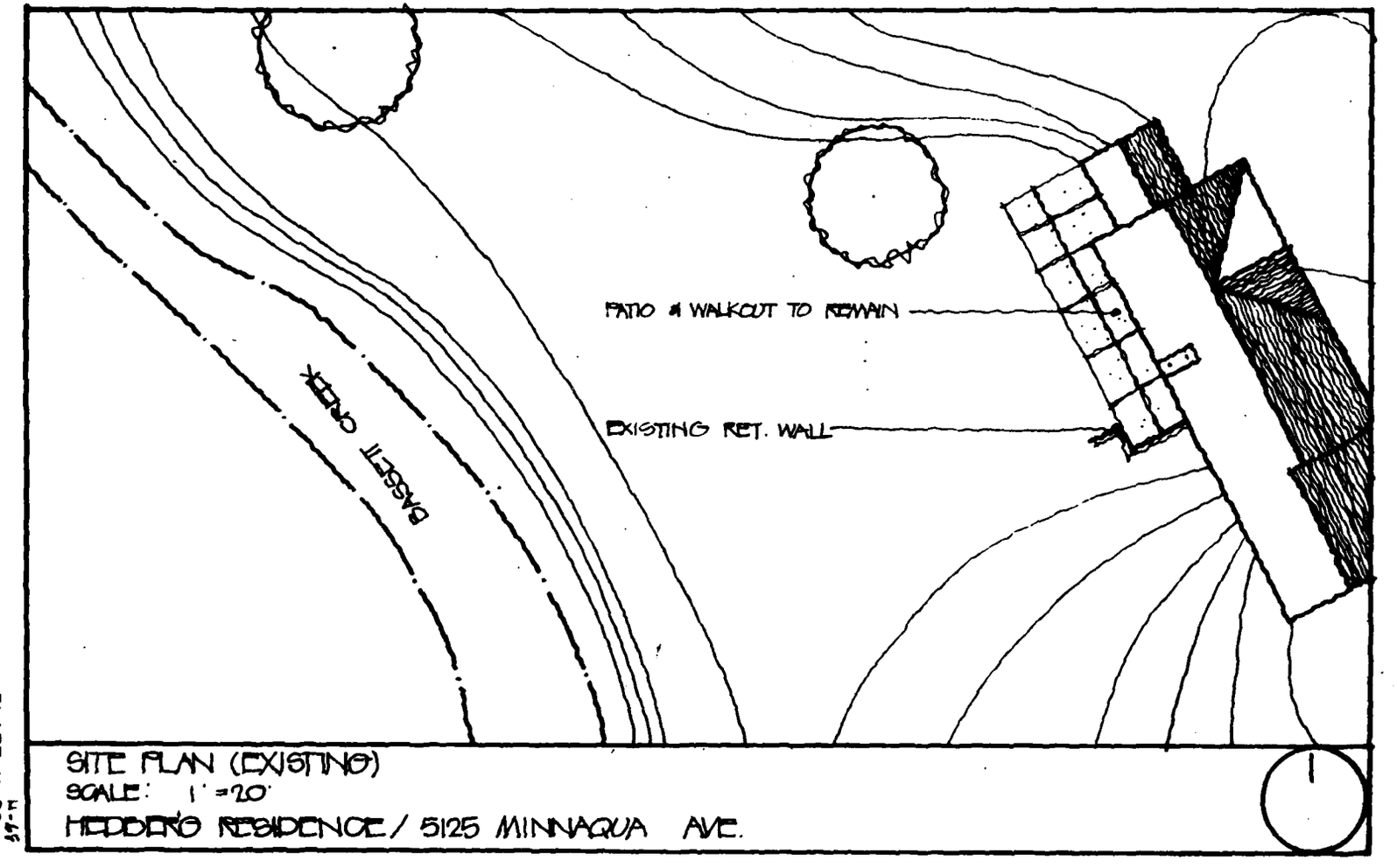
SEE PLATES H-68 THROUGH H-70.

SUMMARY OF ELEVATIONS

DOOR SILL	841.53
WINDOW SILL	844.03
WINDOW SILL	847.11
FIRST FLOOR	850.24
WINDOW SILL	842.68
DOOR SILL	841.41

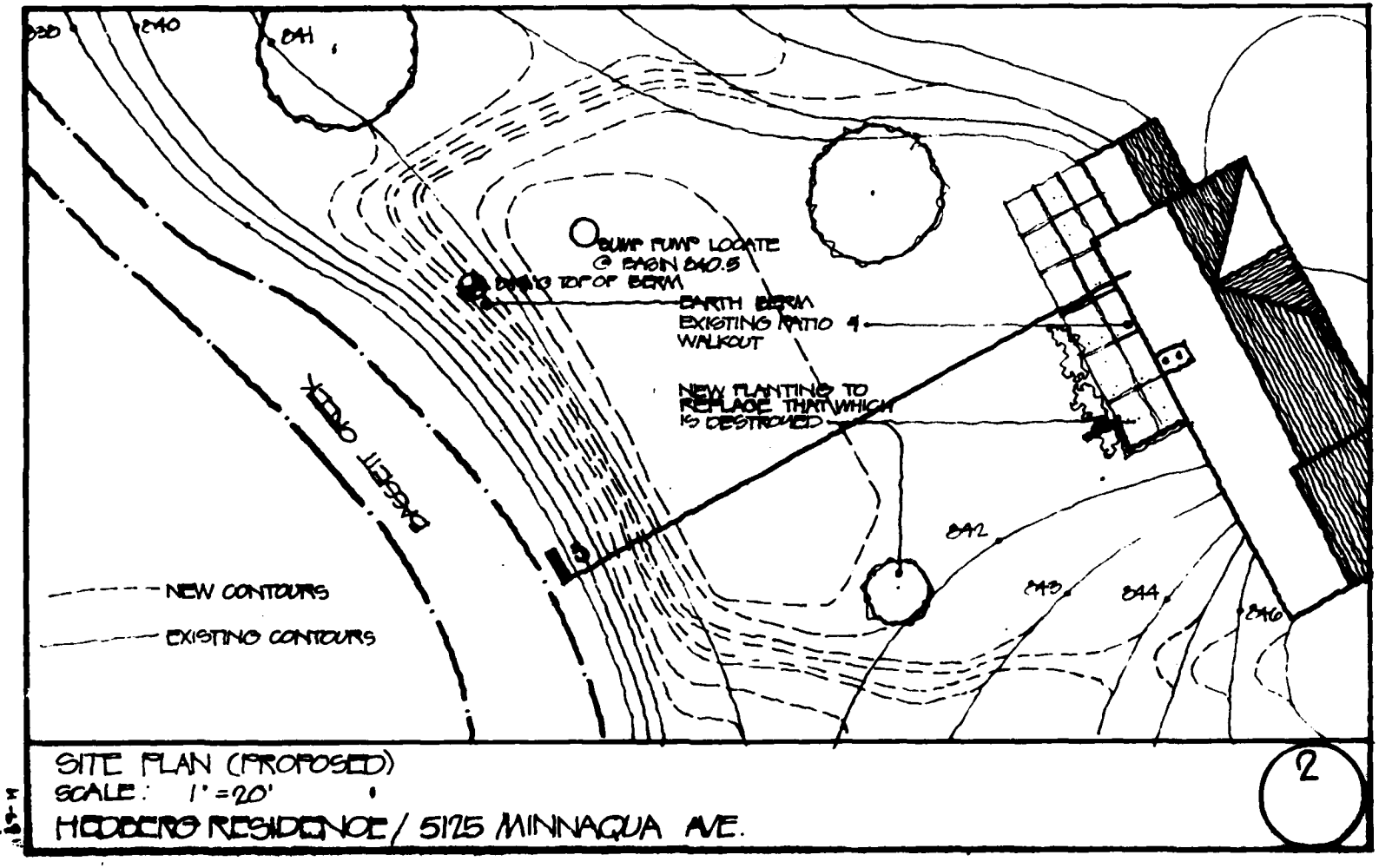
CONSTRUCTION ESTIMATE
5125 MINNAQUA AVENUE, GOLDEN VALLEY, MINNESOTA

EARTHWORK	
COMMON BORROW	$ 550.00
COMMON EXCAVATION	75.00
SOD WITH TOPSOIL	4,300.00
INTERIOR DRAINAGE	6,050.00
TOTAL ESTIMATE	$ 10,925.00



SITE PLAN (EXISTING)
SCALE: 1" = 20'
HEDDEN'S RESIDENCE / 5125 MINNAQUA AVE.

PLATE H-68

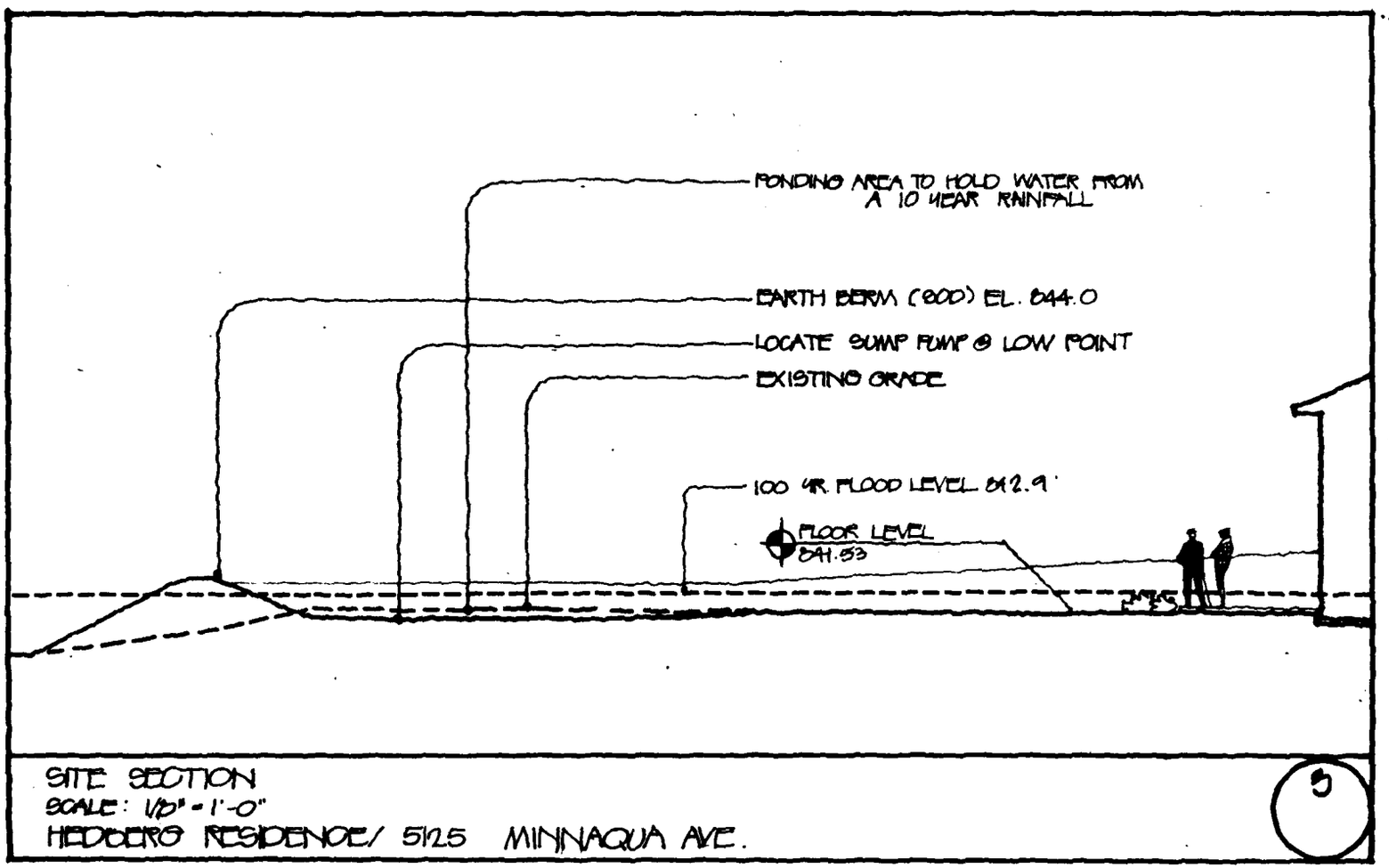


SITE PLAN (PROPOSED)
SCALE: 1" = 20'

HOOBERS RESIDENCE / 5125 MINNAQUA AVE.

2

PLATE NO. 2



SITE SECTION
SCALE: 1/2" = 1'-0"
HEDDERS RESIDENCE/ 5125 MINNAQUA AVE.

5

PLATE H-7.0

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE HANSEN RESIDENCE LOCATED AT 5222 MINNAQUA AVENUE, GOLDEN VALLEY, MINNESOTA: THE LOW OPENING OF THIS HOME IS 841.40. THE ONE HUNDRED YEAR FLOOD LEVEL IS 842.9. WE MUST FLOOD-PROOF TO A LEVEL OF 843.9.

THE HOME IS BUILT ON AN EXISTING SANDBAR WITH PEAT SURROUNDING THE HOUSE. ONE OR TWO INCHES OF FLOOD WATER PENTRATED THE HOME IN THE 1978 FLOOD. THEREFORE, DUE TO THE SOIL CONDITIONS AND THE COMPLEXITY OF THE STRUCTURE ITSELT, WE FEEL THE BEST SOLUTION IS AN EARTH BERM. THE BERM WOULD VIRTUALLY SURROUND THE ENTIRE HOME AND TIE INTO THE EXISTING CONTOURS AT THE REAR OF THE RESIDENCE. AN INTERIOR DRAINAGE SYSTEM WITH SUMP PUMP WOULD ALSO BE INSTALLED.

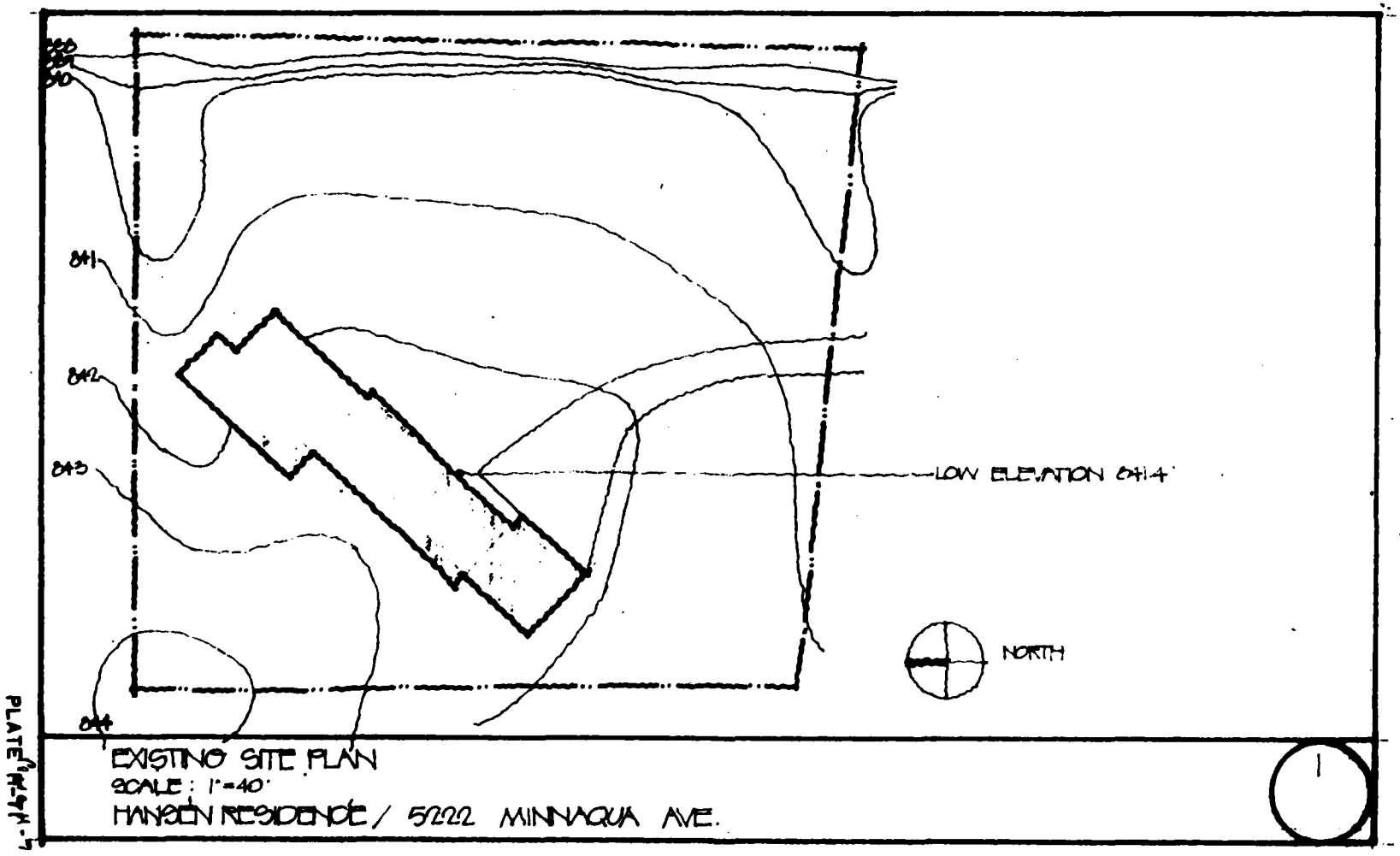
SEE PLATES H-71 AND H-72.

SUMMARY OF ELEVATIONS

DOOR SILL	841.40
DOOR SILL	841.40
WINDOW SILL	843.40
100-YEAR FLOOD LEVEL	842.9
FLOOD PROOF LEVEL	843.9

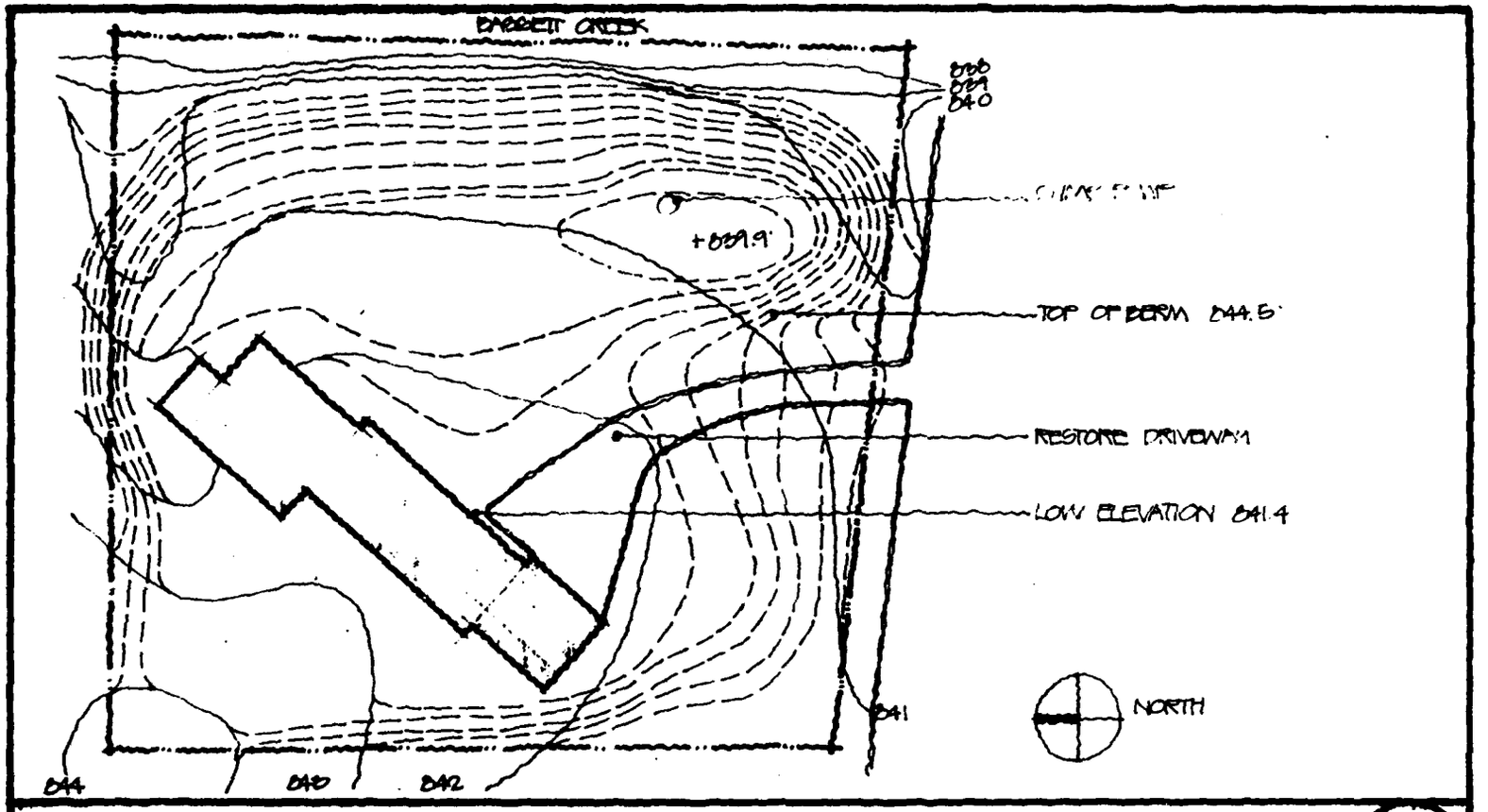
CONSTRUCTION ESTIMATE
5222 MINNAOUA AVENUE, GOLDEN VALLEY, MINNESOTA

EARTHWORK	
COMMON BORROW	$12,575.00
SOD WITH TOPSOIL	425.00
SEEDING WITH TOPSOIL	9,350.00
INTERIOR DRAINAGE	6,050.00
DRIVEWAY	3,850.00
TOTAL ESTIMATE	$34,500.00



EXISTING SITE PLAN
SCALE: 1"=40'
HANSEN RESIDENCE / 5222 MINNAQUA AVE.

1



PROPOSED SITE PLAN
SCALE: 1"=40'
HANSEN RESIDENCE / 5202 MINNAQUA AVE.

2

PLATE [illegible]

BASSETT CREEK FLOOD-PROOFING
SPECIFIC RECOMMENDATIONS

THE HICKS RESIDENCE LOCATED AT 5130 MINNAOUA AVENUE, GOLDEN VALLEY, MINNESOTA: THE ONE HUNDRED YEAR FLOOD LEVEL IS 842.0 AT THIS AREA WITH THE HICKS' LOWER LEVEL AT 840.38. THE HOME EXPERIENCES EXTREME FLOODING DURING ALL MAJOR RAIN STORMS. THE FURNISHINGS IN THE HOME ARE ALL PORTABLE (INCLUDING THE CARPETING) SO THAT THEY CAN BE CARRIED TO HIGHER GROUND IN THE EVENT OF AN IMPENDING RAIN STORM. THE PROBLEM IS FURTHER COMPLICATED BY THE CLOSE PROXIMITY OF THE CREEK TO THE HOME ON TWO SIDES. CONSEQUENTLY, THE ONLY SOLUTION SHORT OF RAISING THE STRUCTURE IS TO BERM AGAINST THE REAR OF THE HOME AND CLOSE UP THE WALK-OUT. THE USE OF A PATIO ON THAT LEVEL WOULD BE LOST, THEREFORE, IT IS RECOMMENDED THAT A REDWOOD DECK BE CONSTRUCTED TO REPLACE THE PATIO WHICH WOULD BE LOST. WE WILL ALSO PROVIDE DRAIN TILE AROUND THE REAR AND SIDES OF THE HOME AND INCORPORATE A SUMP PUMP PIT WITHIN THE DRAIN TILE SYSTEM.

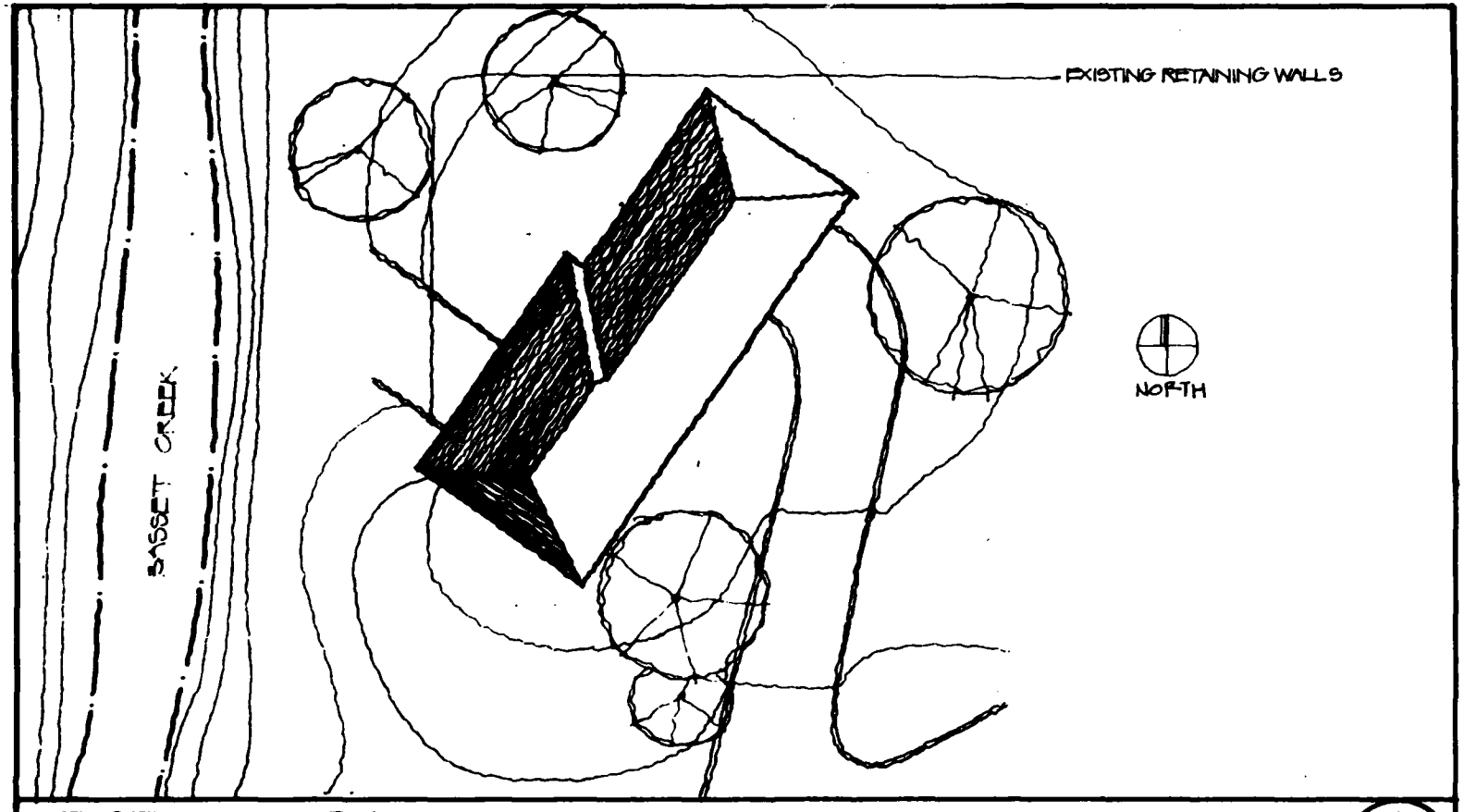
SEE PLATES H-73 THROUGH H-78.

SUMMARY OF ELEVATIONS:

MAIN LEVEL	847.55
WINDOW SILLS	842.46
LOWER LEVEL	840.38
100 YEAR FLOOD LEVEL	842.0
NEW WINDOW WELL LEVEL	843.02
FLOOD PROTECTION ELEVATION	843.0

CONSTRUCTION ESTIMATE
5130 MINNAQUA AVENUE, GOLDEN VALLEY, MINNESOTA

CARPENTRY	
LABOR & MATERIALS	$ 1,350.00
DECK (LABOR & MATERIALS)	1,350.00
MASONRY	
WALL	275.00
SUMP PUMP PIT	5,375.00
DRAIN TILE (130 LINEAL FT)	3,350.00
ELECTRICAL (MISC.)	275.00
TOTAL ESTIMATE:	$11,975.00



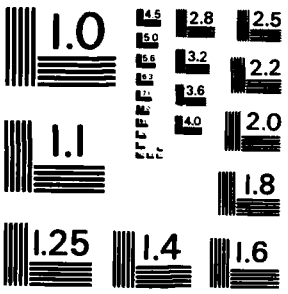
EXISTING SITE PLAN
SCALE 1" = 20'
HICKS RESIDENCE / 5130 MINNAQUA AVENUE

1

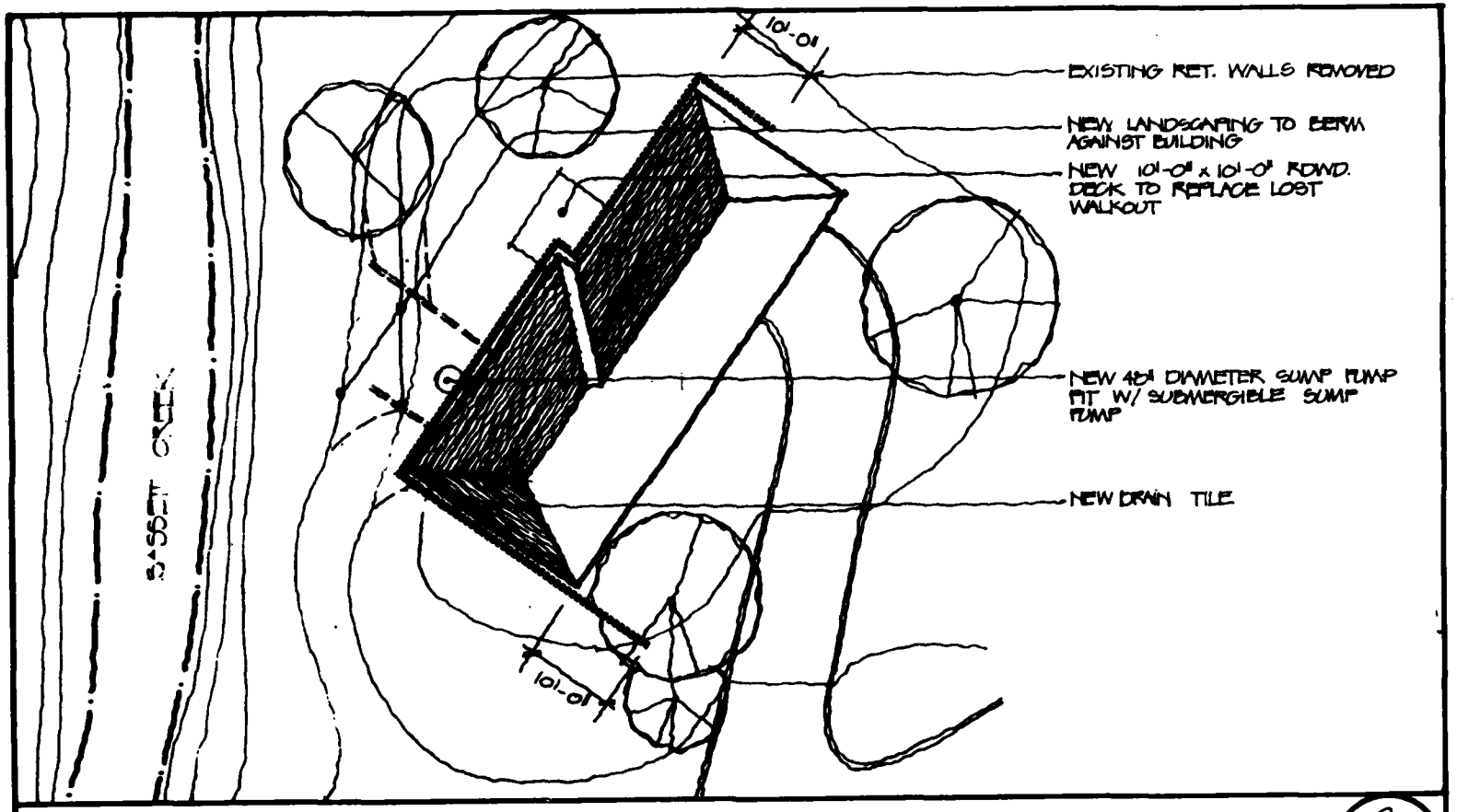
PLATE H-73-'13

AD-A133 795 BASSETT CREEK WATERSHED HENNEPIN COUNTY MINNESOTA 1/7
FEASIBILITY REPORT FOR FLOOD CONTROL MAIN REPORT(U)
CORPS OF ENGINEERS ST PAUL MN ST PAUL DISTRICT SEP 82
UNCLASSIFIED F/G 13/2 NL





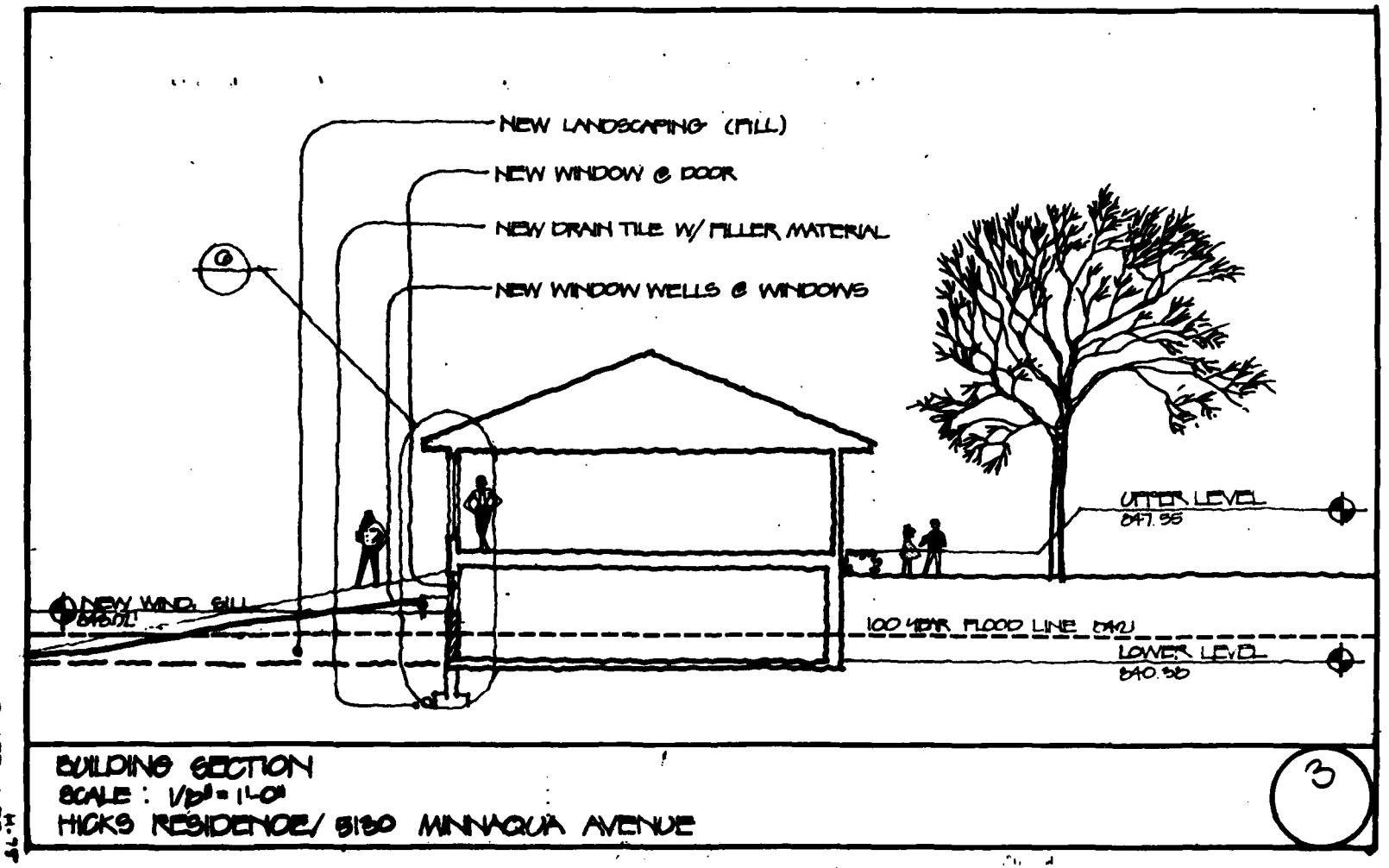
MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS - 1963 - A



PROPOSED SITE PLAN
SCALE 1"= 20'
HICKS RESIDENCE / 5130 MINNAQUA AVENUE

2

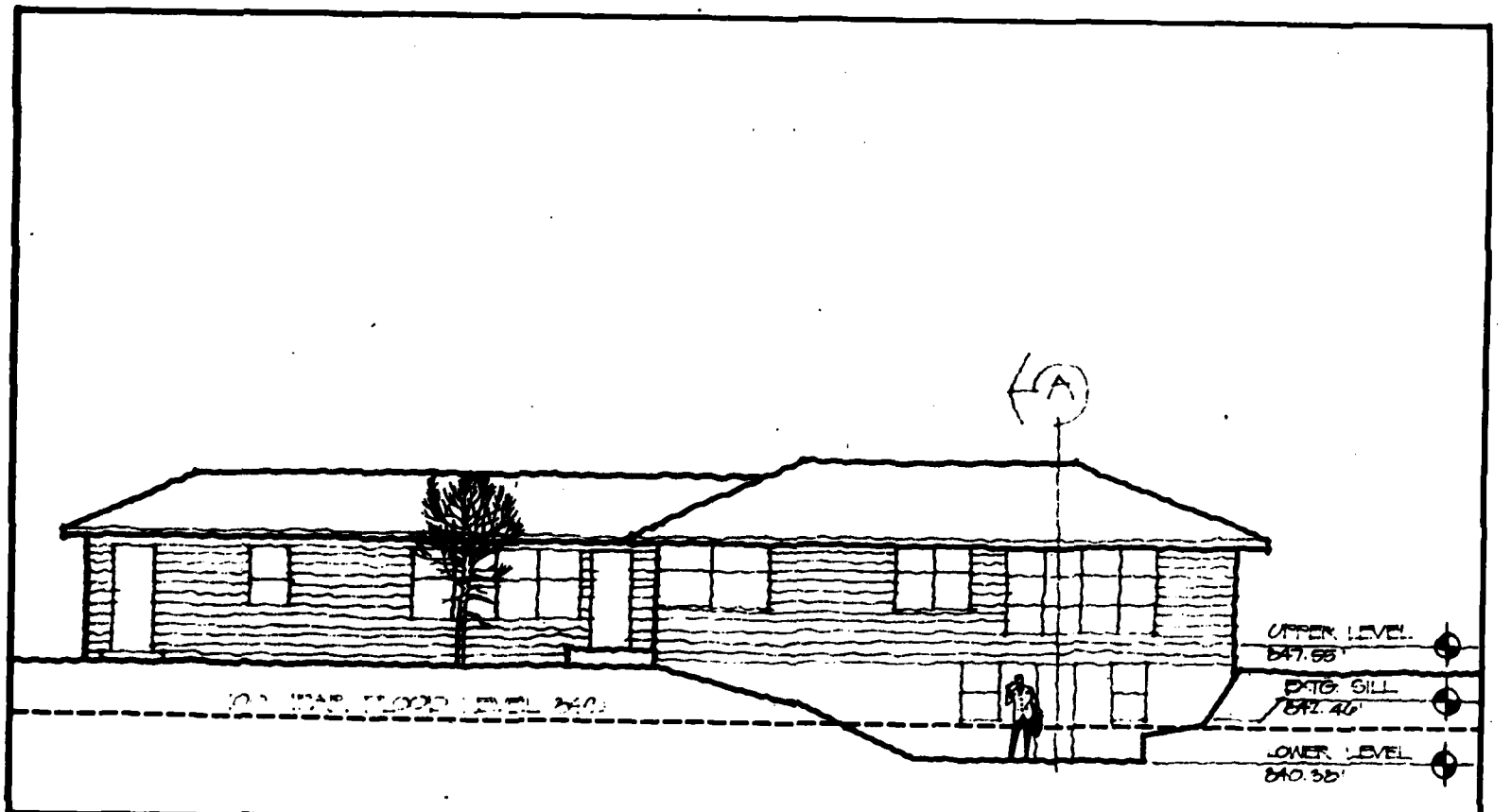
PLATE H-44



BUILDING SECTION
SCALE : 1/2" = 1'-0"
HICKS RESIDENCE/ 5130 MINNAQUA AVENUE

3

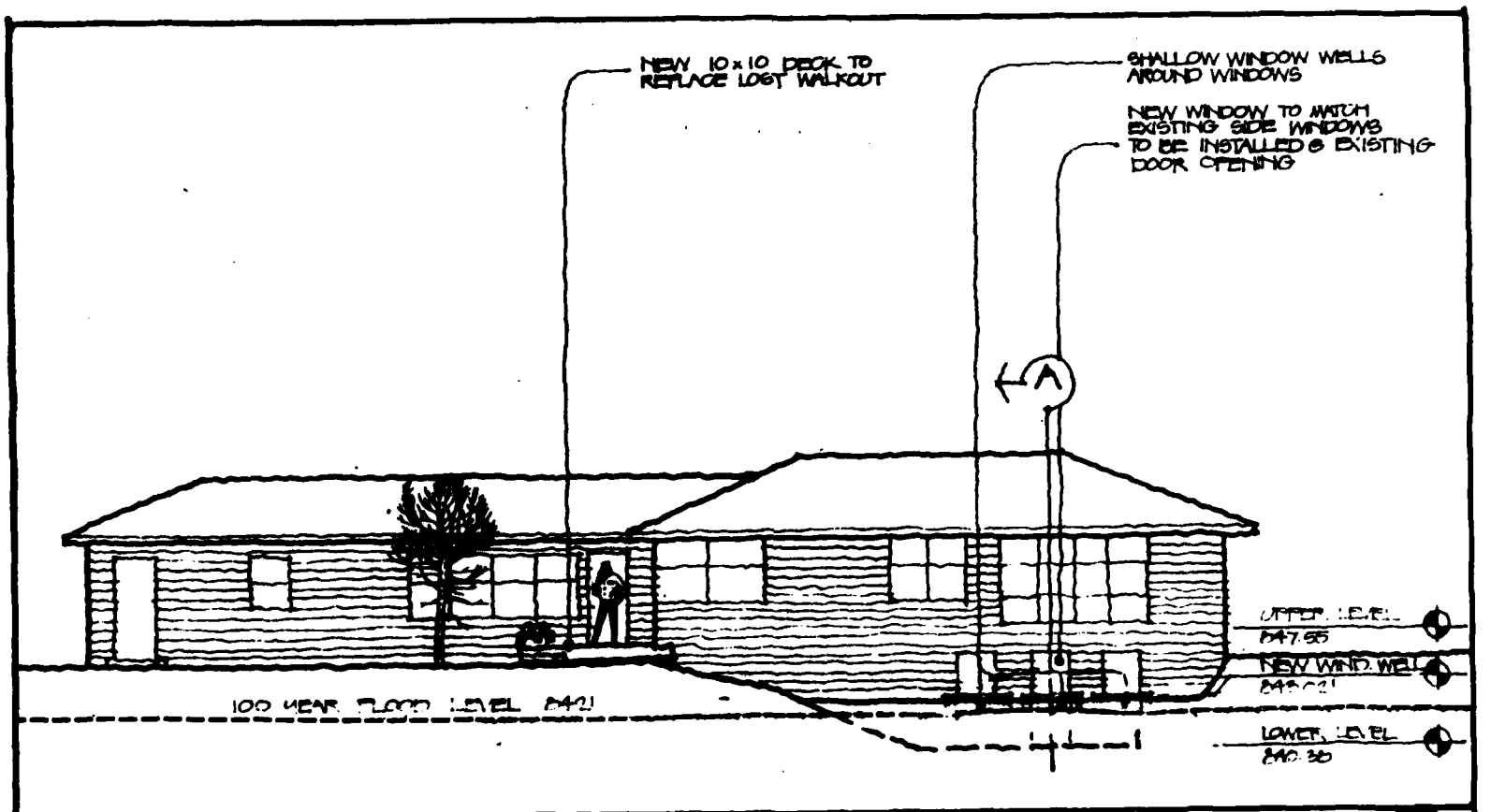
PLATE H-75
H-75



EXISTING REAR ELEVATION
SCALE 1/8" = 1'-0"
HICKS RESIDENCE / 5130 MINNAQUA AVENUE

4

PLATE H-7B
H-7B



PROPOSED REAR ELEVATION
SCALE 1/8" = 1'-0"
HICKS RESIDENCE / 5130 MINNAQUA AVENUE

5

PLATE H-77

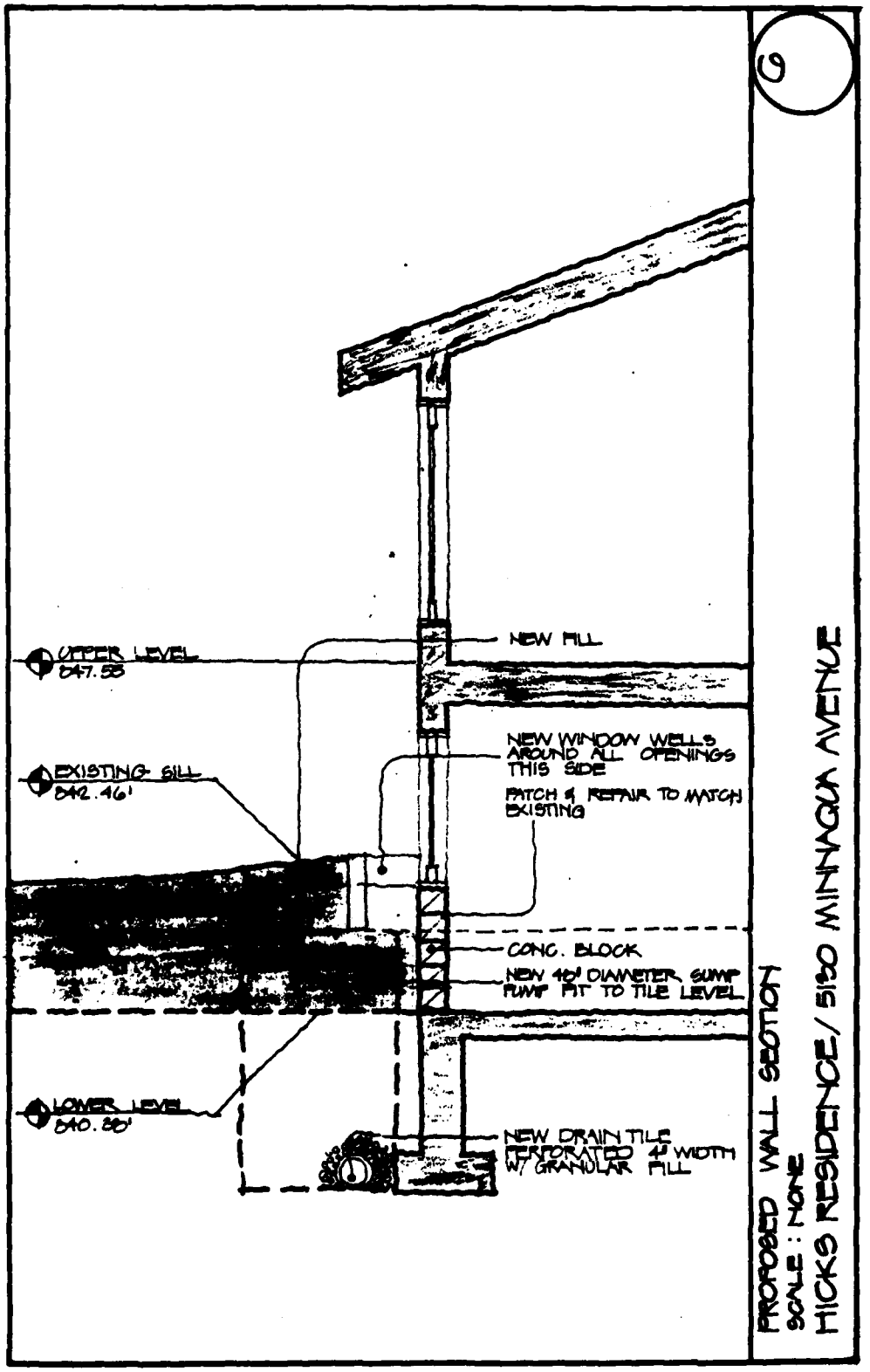

9
UPPER LEVEL
847.55
EXISTING SILL
842.46'
LOWER LEVEL
840.80'
NEW FILL
NEW WINDOW WELLS AROUND ALL OPENINGS THIS SIDE
PATCH & REPAIR TO MATCH EXISTING
CONC. BLOCK
NEW 40" DIAMETER SUMP PUMP PIT TO TILE LEVEL
NEW DRAIN TILE PERFORATED 4" WIDTH W/ GRANULAR FILL
PROPOSED WALL SECTION
SCALE: NONE
HICKS RESIDENCE / 5180 MINNAQUA AVENUE

PLATE H-78

FLOOD PROOFING COST SUMMARY

ADDRESS	COST	
2605 SCOTT AVENUE NORTH	$43,400	
2655 SCOTT AVENUE NORTH	41,500	RAISES
2665 SCOTT AVENUE NORTH	46,800	
2675 SCOTT AVENUE NORTH	44,200	
2685 SCOTT AVENUE NORTH	7,383	1 GROUP
2725 SCOTT AVENUE NORTH	7,383	$22,150
2735 SCOTT AVENUE NORTH	7,383	
2755 SCOTT AVENUE NORTH	5,550	1 GROUP
2775 SCOTT AVENUE NORTH	5,550	$11,100
1205 IDAHO AVENUE NORTH	11,675	
2425 REGENT AVENUE NORTH	7,375	1 GROUP
2445 REGENT AVENUE NORTH	7,375	$14,750
2505 REGENT AVENUE NORTH	10,925	1 GROUP
2545 REGENT AVENUE NORTH	10,925	$21,850
3810 BASSETT CREEK DRIVE	16,450	1 GROUP
3820 BASSETT CREEK DRIVE	16,450	$32,900
4975 BASSETT CREEK DRIVE	14,125	
5005 BASSETT CREEK DRIVE	14,625	
5110 MINNAQUA DRIVE	7,375	1 GROUP
5120 MINNAQUA DRIVE	7,375	$22,125
5126 MINNAQUA DRIVE	7,375	
5125 MINNAQUA DRIVE	10,925	
5222 MINNAQUA DRIVE	34,500	
5130 MINNAQUA AVENUE NORTH	11,975	
TOTAL	$398,600	

DEPARTMENT OF THE ARMY
St. Paul District Corps of Engineers
1135 U.S. Post Office & Custom House
St. Paul, Minnesota 55101

FLOOD CONTROL

BASSETT CREEK

HENNEPIN COUNTY, MINNESOTA

DESIGN MEMORANDUM NO. 2, PHASE II

APPENDIX I

SECTION 215 PROGRAM

Table of Contents

Paragraph		Page
1	GENERAL	I-1
2	CROSSINGS	I-1
3	MINNAQUA AVENUE BRIDGE REMOVAL	I-1
4	WISCONSIN AVENUE EMBANKMENT	I-2
5	TRUNK HIGHWAY 100 EMBANKMENT	I-2
6-7	EDGEWOOD EMBANKMENT	I-2
8	FEDERAL COST	I-2

Plates

Number	
I-1	EDGEWOOD EMBANKMENT, PLAN
I-2	EDGEWOOD EMBANKMENT, PROFILE
I-3	ALTERNATE CROSSING DESIGN FOR NOBLE AND REGENT AVENUES

APPENDIX I

SECTION 215 PROGRAM

GENERAL

1. In August 1979, an agreement under Section 215, Public Law 90-483, was completed to allow advance construction on certain authorized project features by the project sponsors. The agreement between the city of Golden Valley, Minnesota and the Corps of Engineers was also cosigned by the city of Minneapolis, Minnesota. The advance construction was to include new channel crossings, the T.H. 100 embankment, the Edgewood embankment and storage area, the Wisconsin Avenue embankment, the evacuation of one home near 36th Avenue and Florida Avenue in Crystal, channel improvements downstream from T.H. 100 including removal of the Minnaqua Avenue bridge, and floodproofing of 20 residences on the main stem Bassett Creek between Golden Valley Road and T.H. 100 in the city of Golden Valley. In October 1981, a contract was awarded to Shafer Construction Company for construction of certain of the project features. Due to increasing costs, constraints on local funding, the developing federal design program and the identification of required federal design criteria, the Section 215 program was reduced to include only those features which could be constructed independently of other project features and result in significant early flood relief. Several features were totally eliminated and several others were modified or reduced in scale. Some of the latter items perform basically the same function as the designs developed by the District and included in this design memorandum, but do not meet established Corps design criteria. It is anticipated that the Shafer contract will be completed by November 1982. Details on the status of each of the original Section 215 features follow.

CROSSINGS

2. New channel crossings will be constructed at Noble Avenue, Regent Avenue, 32nd Avenue, Brunswick Avenue, 34th Avenue, Florida Avenue, and 36th Avenue. The Douglas Drive crossing was included in the 221 agreement as an advance construction item but was dropped due to high expense, the possibility of future Hennepin County improvements to Douglas Drive and relatively lower priority as a problem area. The two crossings at Noble Avenue and Regent Avenue were bid as alternate designs using a prefab arch-bridge concept. Bids on the alternate designs were attractive enough to have the contract awarded with the alternates. The alternate designs are considered hydraulic equivalents of designs presented on Plate 21. A drawing showing the alternate designs can be found on Plate I-3. Construction of the crossings are considered non-Federal cost items.

MINNAQUA AVENUE BRIDGE REMOVAL

3. As part of the advance construction program, the Minnaqua Avenue bridge and abutments will be removed and the immediate area will be relandscaped to match the surrounding grounds. A new cul-de-sac will be constructed as a terminus to Minnaqua Avenue on the east bank and a curb inlet will be installed for storm drainage. See Plate 22. The bridge and abutment removals, relandscaping and the storm pipe to the creek from the inlet manhole are considered Federal cost items. The cul-de-sac and storm inlet are considered part of a local improvement program and are non-Federal cost items.

WISCONSIN AVENUE EMBANKMENT

4. The Wisconsin Avenue embankment and control structure will not be constructed under the Shafer contract. Indecision on area development details required delaying completion of this item. Embankment fill materials and the control structure are considered Federal cost items. Excess fill required for the proposed roadway, surfacing and other roadway materials are considered non-Federal cost items. The city of Golden Valley may still decide to construct this feature in advance of Federal construction and within the time frame defined in the 215 agreement. A site farther upstream is presently being considered as a better location for this control structure.

TRUNK HIGHWAY 100 EMBANKMENT

5. Under the Shafer contract, a control structure will be attached to the existing box culverts which constitute the existing Bassett creek crossing of T.H. 100. This design cannot be used to complete the project design shown on Plates 23-27. Therefore, this advance work is considered an interim measure not Federally reimbursable. Upon Federal construction of the T.H. 100 embankment, the temporary control will be removed.

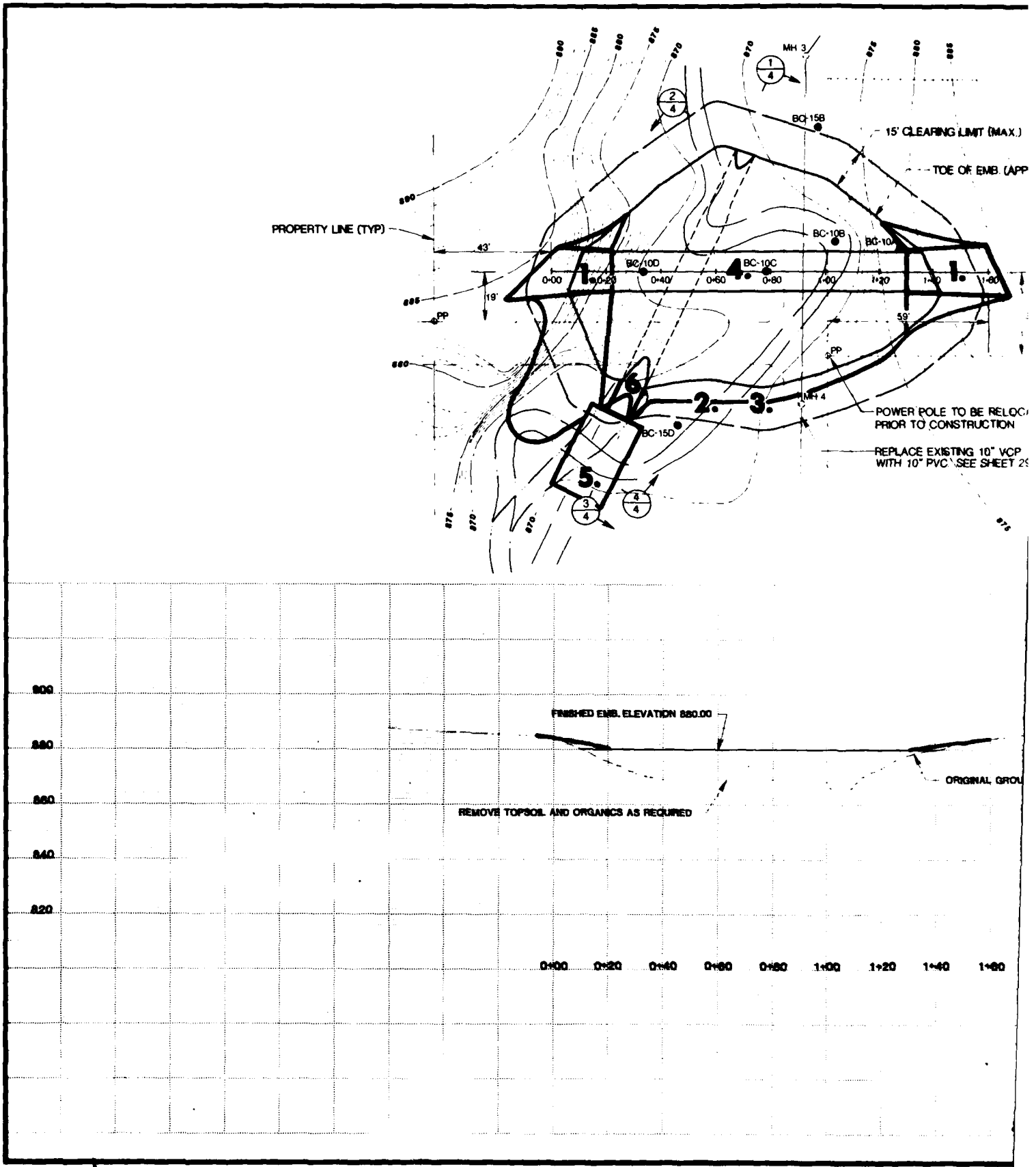
EDGEWOOD EMBANKMENT

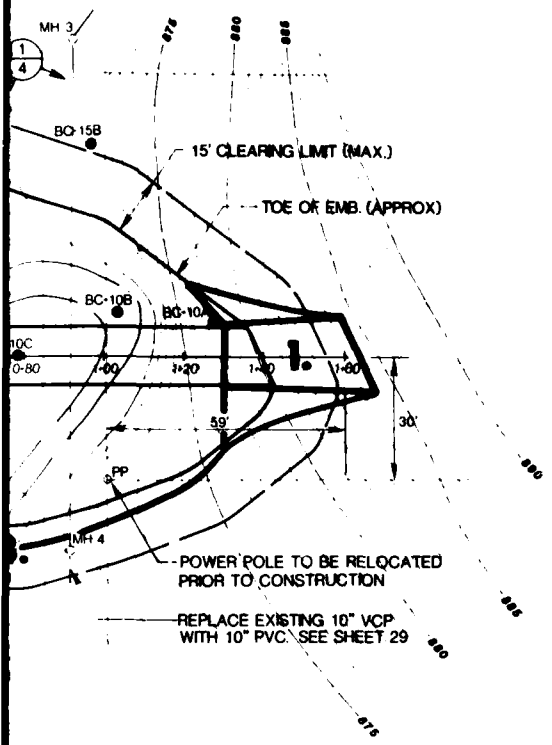
6. The local design proposal for the Edgewood Embankment does not meet design standards required for Corps facilities and has not been indorsed as an acceptable design. However, in June 1982, the Corps agreed to reimburse local interests for the advance construction on the Edgewood embankment which can be incorporated into the final configuration of the Edgewood embankment. This was done with an explicit denial of liability for the performance of the locally constructed Edgewood embankment. It did acknowledge that materials in place upon Federal construction of the final configuration, assuming a failure had not occurred, would have a reimbursable value. Materials which cannot be used in the final embankment design will not be considered reimbursable and disposal of such materials will be considered a non-Federal expense. Some detailed design information on the Edgewood embankment can be found in Appendix A.

7. It is proposed that the Section 215 construction, or first phase work, be supplemented by Federal construction in a second phase, completing the design developed by the St. Paul District. Drawings showing the conceptual modifications to be made to the Section 215 work are shown on plates I-1 and I-2.

FEDERAL COST

8. Presently, a total of $434,342 is estimated to be reimbursable by the Federal government for engineering and construction on project features considered local responsibilities. From this amount, approximately $34,000 would be deducted for Federal review of the advance local work, including engineering, supervision and administration as stated in Article 4(b) of the Section 215 agreement.


MH 3
BC-15B
15' CLEARING LIMIT (MAX.)
TOE OF EMB. (APP
PROPERTY LINE (TYP)
43'
BC-10B
BC-10A
BC-10D
BC-10C
0+00
0+20
0+40
0+60
0+80
1+00
1+20
1+40
1+60
19'
59'
PP
MH 4
BC-15D
POWER POLE TO BE RELOC
PRIOR TO CONSTRUCTION
REPLACE EXISTING 10" VCP
WITH 10" PVC SEE SHEET 2
FINISHED EMB. ELEVATION 880.00
ORIGINAL GROU
REMOVE TOPSOIL AND ORGANICS AS REQUIRED
900
880
860
840
820



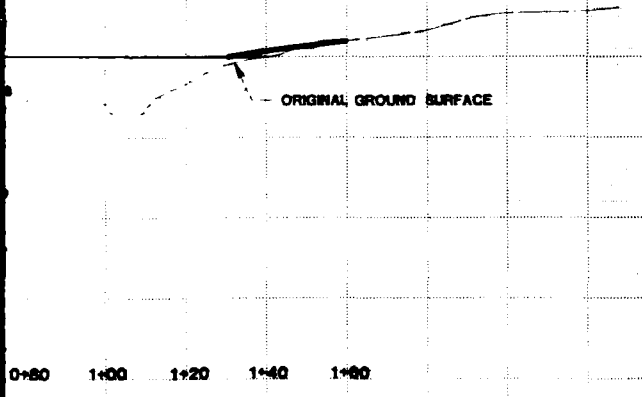
NOTES: 1) BACKFILL FOR 10" PVC BETWEEN MH 3 AND MH 4 SHALL MEET THE GRADATION AND COMPACTION REQUIREMENTS OF IMPERVIOUS CLAY

2) CONTRACTOR SHALL SAVE ALL TREES DESIGNATED BY THE ENGINEER WITHIN THE CLEARING LIMITS



PHASE II MODIFICATIONS

1. PROVIDE ABUTMENT PROTECTION
2. FLATTEN DOWNSTREAM SLOPES
3. KEY IN RIPRAP SLOPE
4. MODIFY EMBANKMENT CREST TO PROVIDE DESIRED HYDRAULIC DESIGN
5. PROVIDE ENERGY DISSIPATOR
6. EXTEND BOX CULVERT



CITY OF GOLDEN VALLEY
BASSETT CREEK FLOOD CONTROL

BARR ENGINEERING CO.
CONSULTING ENGINEERS
MINNEAPOLIS MINNESOTA

EDGEWOOD EMBANKMENT
PLAN AND PROFILE

SHEET NO. OF SHEETS

I HEREBY CERTIFY THAT THIS DRAWING OR PLAN WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY REGISTERED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA

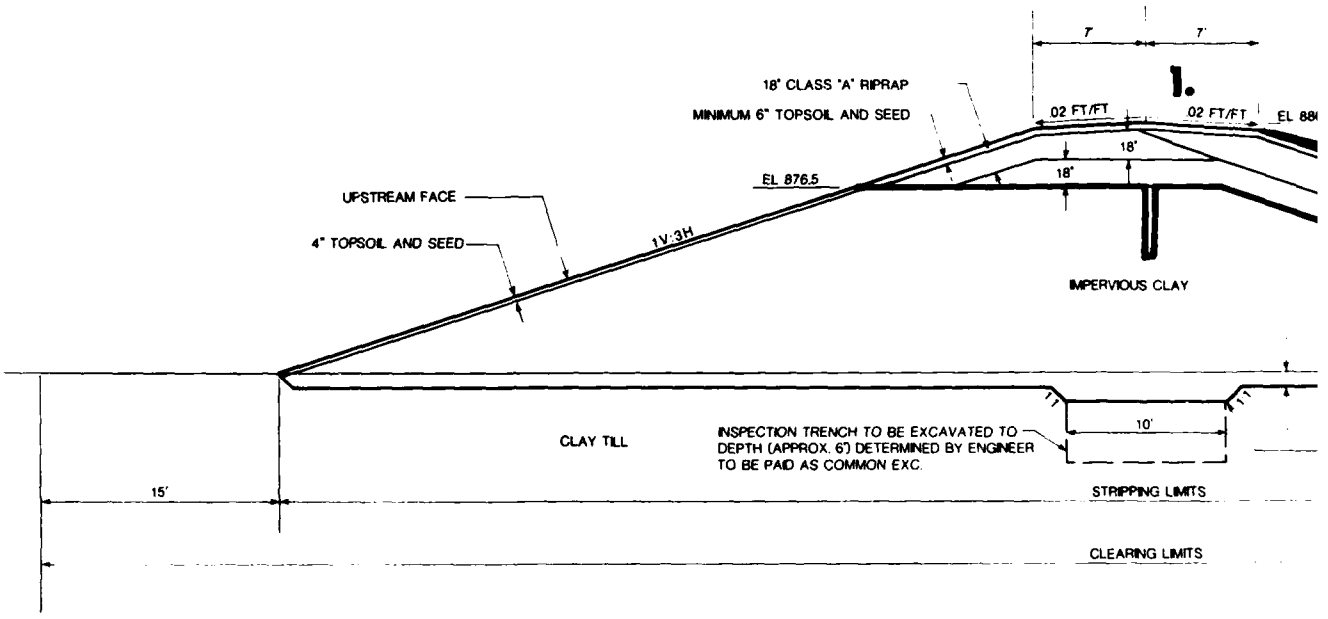
DATE

REG NO

REVISIONS
DETAILS 11/18/81

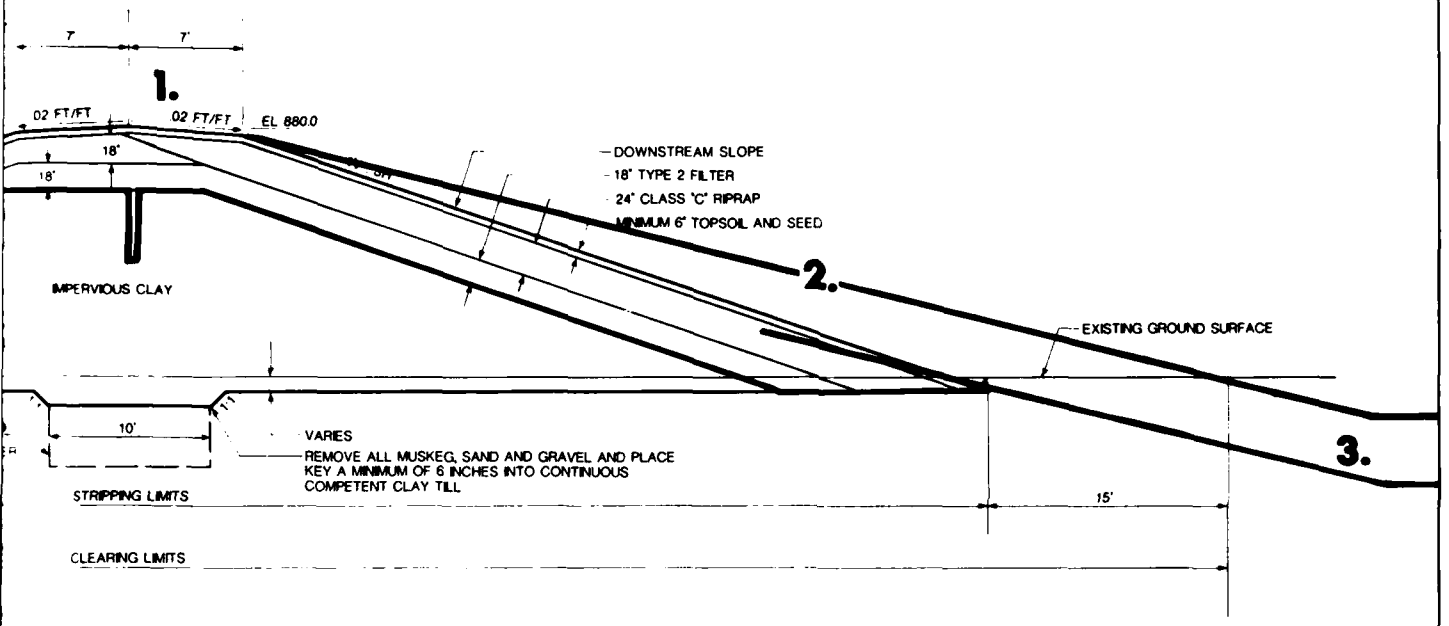
SCALE AS SHOWN
DWN BY
DATE 10/31/80
DWG NO

PLATE I-1



TYPICAL SECTION: EDGEWOOD EMBANKMENT

SCALE 1" 5'



PHASE II MODIFICATIONS

1. MODIFY CREST TO PROVIDE DESIRED HYDRAULIC DESIGN

2. FLATTEN SLOPES (REDESIGN RIPRAP)

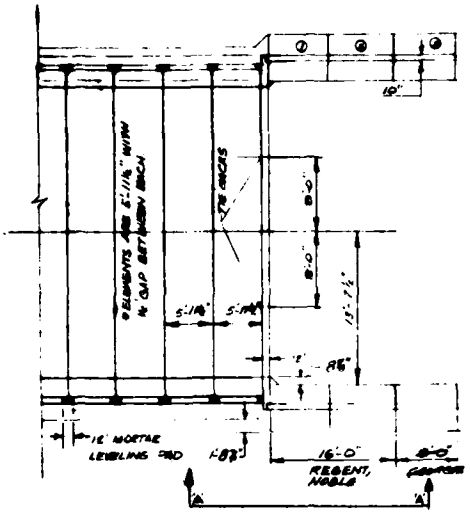
3. KEY IN RIPRAP SLOPE

EDGEWOOD EMBANKMENT

SCALE 1" = 5'

I HEREBY CERTIFY THAT THIS DRAWING OR PLAN WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY REGISTERED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA DATE REG NO	REVISIONS	SCALE: AS SHOWN DWN BY DATE: 10/31/80 DWG NO	CITY OF GOLDEN VALLEY BASSETT CREEK FLOOD CONTROL BARR ENGINEERING CO. CONSULTING ENGINEERS MINNEAPOLIS MINNESOTA EDGEWOOD EMBANKMENT TYPICAL SECTION SHEET NO. OF SHEETS

PLATE I-2



PLAN VIEW

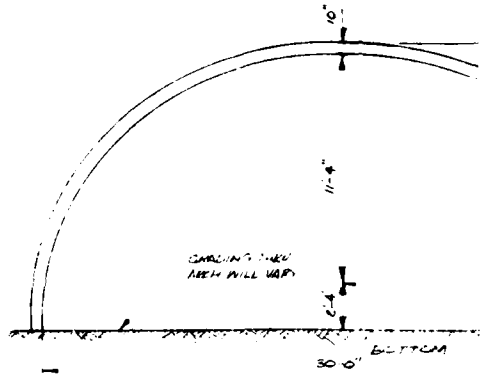
LIST OF WEIGHTS

SPANDREL WALL	31,200 lbs
WING WALLS ①	23,400 lbs
②	18,400 lbs
③	19,900 lbs
STD ELEMENT	36,250 lbs
SHORT ELEMENT	31,750 lbs

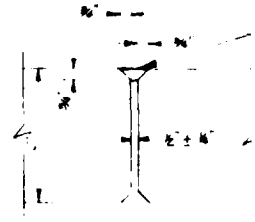
ESTIMATED STRUCTURE LENGTHS

NOBLE AVE.	90'-0" (SHORT)
REGENT AVE.	94'-1" (SHORT)
~~GEORGIAN AVE.~~	~~96'-1"~~ (STD)

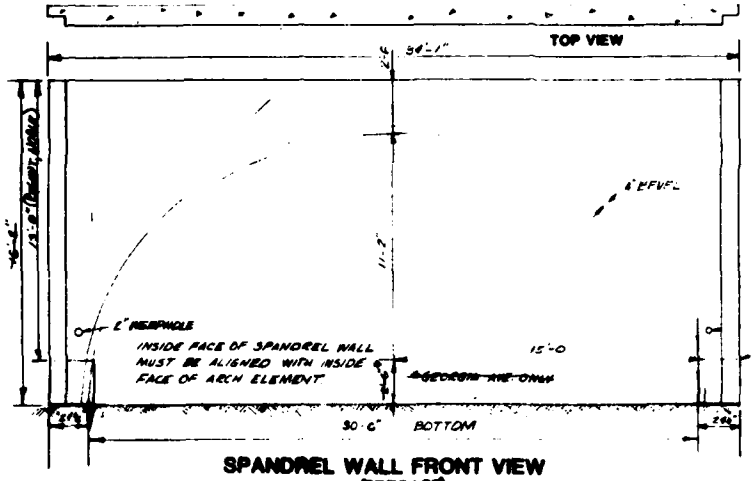
NOTE: LENGTHS SHOWN ARE SPANDREL WALL TO SPANDREL WALL. SPANDREL WALL AND WING WALLS ARE INCIDENTAL.



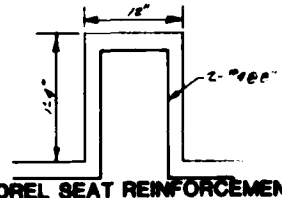
END VIEW - ARCH ELEMENT
NO SCALE



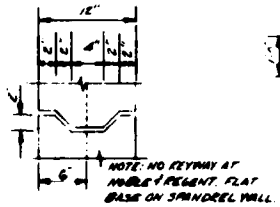
SPACE BETWEEN SECT



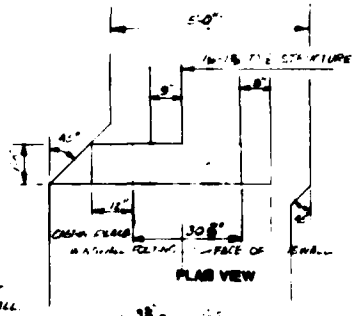
SPANDREL WALL FRONT VIEW
(PRECAST)



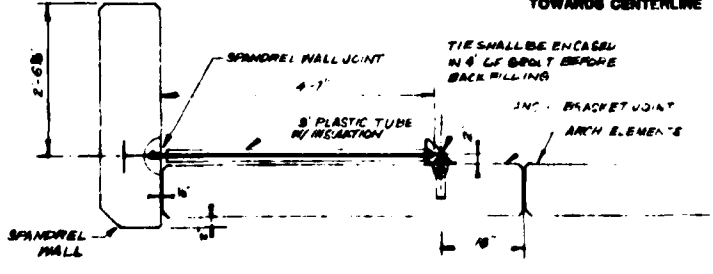
SPANDREL SEAT REINFORCEMENT DETAIL (CIP)



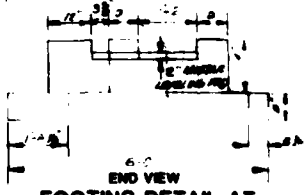
SPANDREL WALL JOINT
TOWARDS CENTERLINE



PLAN VIEW



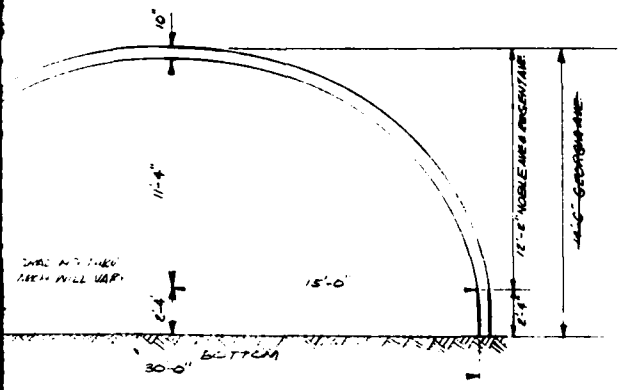
SPANDREL TIE BACK



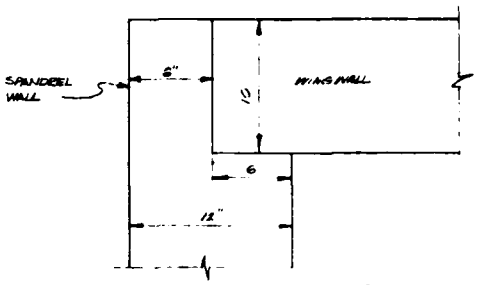
END VIEW

FOOTING DETAIL AT SPANDREL/WINGWALL JOINT
1" = 1'

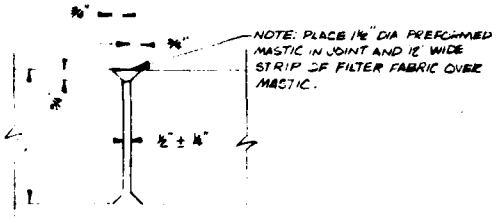
FABRICATION:



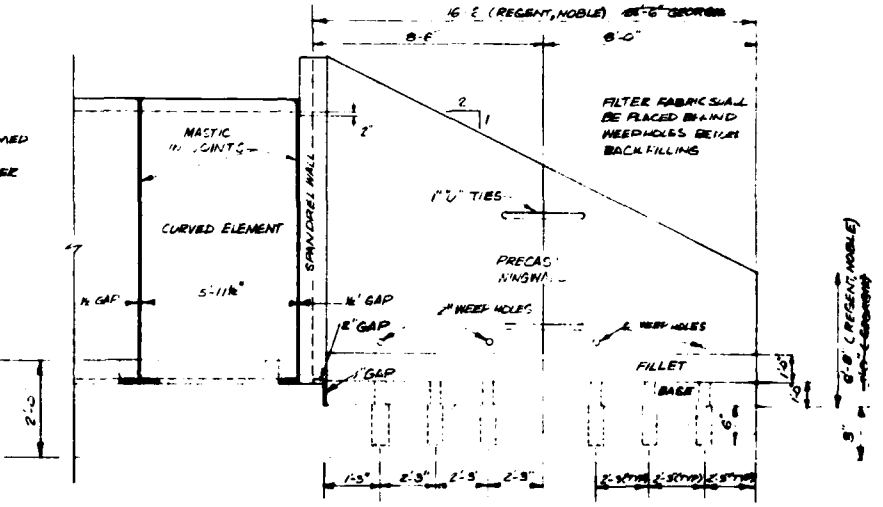
END VIEW - ARCH ELEMENTS (PRECAST)
NO SCALE



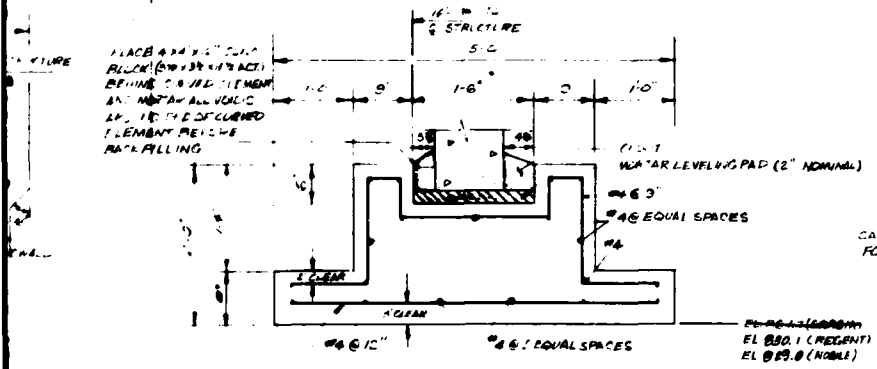
SPANDREL WALL/WINGWALL JOINT
NO SCALE



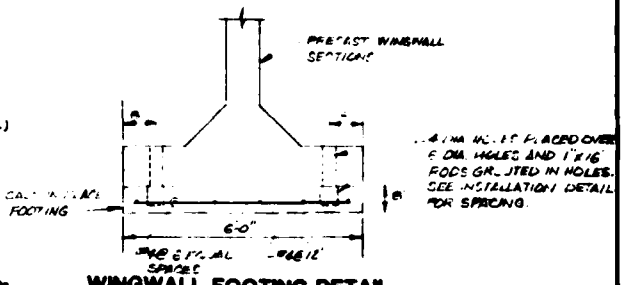
SPACE BETWEEN SECTIONS



INSTALLATION DETAIL
SECTION A-A



FOOTING DETAIL
(TYP)



WINGWALL FOOTING DETAIL

FABRICATION:
FABRICATION AND CONSTRUCTION OF BEBO STRUCTURES AND APPURTENANCES SHALL BE IN ACCORDANCE WITH FABRICATORS RECOMMENDATIONS.

REVISIONS	SCALE:	CITY OF GOLDEN VALLEY
REV. 10/26/81	DRAWN BY: JLS	BASSETT CREEK FLOOD CONTROL
	DATE:	HANCOCK CONCRETE PRODUCTS CO., INC. (612) 838-4044
	DWG. NO.:	ALTERNATE CROSSINGS NOBLE, REGENT, BEBO DETAILS

DATE 10-91
DWG. NO. 6090

SHEET NO. OF SHEETS

PLATE I-3

END

DATE
FILMED

11 83

DTIC