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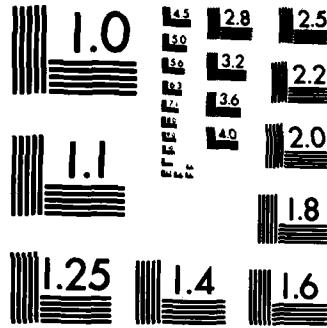
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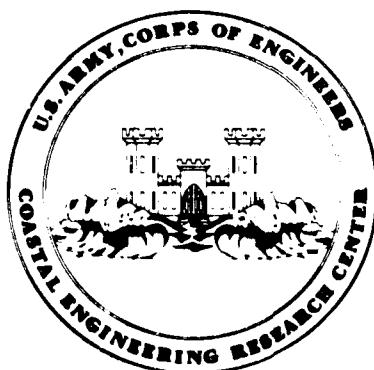
# The Elevation and Duration of Wave Crests

by

William N. Seelig, John P. Ahrens,  
and  
William G. Grosskopf

MISCELLANEOUS REPORT NO. 83-1

JANUARY 1983



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REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
1. REPORT NUMBER MR 83-1	2. GOVT ACCESSION NO. AD-A127872	3. RECIPIENT'S CATALOG NUMBER
4. TITLE (and Subtitle)  THE ELEVATION AND DURATION OF WAVE CRESTS		5. TYPE OF REPORT & PERIOD COVERED Miscellaneous Report
7. AUTHOR(s) William N. Seelig John P. Ahrens William G. Grosskopf		6. PERFORMING ORG. REPORT NUMBER
9. PERFORMING ORGANIZATION NAME AND ADDRESS Department of the Army Coastal Engineering Research Center (CERRE-CS) Kingman Building, Fort Belvoir, VA 22060		10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS A31592
11. CONTROLLING OFFICE NAME AND ADDRESS Department of the Army Coastal Engineering Research Center Kingman Building, Fort Belvoir, VA 22060		12. REPORT DATE January 1983
14. MONITORING AGENCY NAME & ADDRESS(if different from Controlling Office)		13. NUMBER OF PAGES 73
		15. SECURITY CLASS. (of this report)  UNCLASSIFIED
		15a. DECLASSIFICATION/DOWNGRADING SCHEDULE
16. DISTRIBUTION STATEMENT (of this Report)  Approved for public release; distribution unlimited.		
17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from Report)		
18. SUPPLEMENTARY NOTES		
19. KEY WORDS (Continue on reverse side if necessary and identify by block number)  Irregular waves                                  Wave breaking Monochromatic waves                              Wave crests		
20. ABSTRACT (Continue on reverse side if necessary and identify by block number)  The stream-function wave theory of Dean (1974) is used together with monochromatic and irregular laboratory wave data to develop methods for estimating the elevation and duration of wave crests. The resulting prediction techniques are applied to a wide range of wave conditions measured at CERC's Field Research Facility in Duck, North Carolina, and are shown to give reliable and often conservative estimates of crest elevation. The techniques presented in this report can be used for both nonbreaking and breaking wave conditions.		

## PREFACE

As waves move toward the shoreline they become increasingly nonlinear with the elevation of the wave crests becoming relatively large and short in duration. This report is published to provide coastal engineers with techniques for estimating crest elevation and duration. The techniques were developed using laboratory data from a number of sources covering a wide range of conditions for both monochromatic and irregular waves under nonbreaking and breaking conditions. Predictions compare favorably with wave observations made at the U.S. Army Coastal Engineering Research Center's (CERC) Field Research Facility at Duck, North Carolina. The work was carried out under CERC's Wave Estimation for Design work unit, Coastal Flooding and Storm Protection Program, Coastal Engineering Area of Civil Works Research and Development.

This report was prepared by William N. Seelig, Research Hydraulic Engineer, John P. Ahrens, Oceanographer, and William G. Grosskopf, Hydraulic Engineer, under the general supervision of Dr. R.M. Sorenson, Chief, Coastal Processes and Structures Branch and Mr. R.P. Savage, Chief, Research Division.

Technical Director of CERC was Dr. Robert W. Whalin, P.E.

Approved for publication in accordance with Public Law 166, 79th Congress, approved 31 July 1945, as supplemented by Public Law 172, 88th Congress, approved 7 November 1963.

  
TED E. BISHOP  
Colonel, Corps of Engineers  
Commander and Director

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**CONVERSION FACTORS, U.S. CUSTOMARY TO METRIC (SI) UNITS OF MEASUREMENT**

U.S. customary units of measurement used in this report can be converted to metric (SI) units as follows:

Multiply	by	To obtain
inches	25.4	millimeters
	2.54	centimeters
square inches	6.452	square centimeters
cubic inches	16.39	cubic centimeters
feet	30.48	centimeters
	0.3048	meters
square feet	0.0929	square meters
cubic feet	0.0283	cubic meters
yards	0.9144	meters
square yards	0.836	square meters
cubic yards	0.7646	cubic meters
miles	1.6093	kilometers
square miles	259.0	hectares
knots	1.852	kilometers per hour
acres	0.4047	hectares
foot-pounds	1.3558	newton meters
millibars	$1.0197 \times 10^{-3}$	kilograms per square centimeter
ounces	28.35	grams
pounds	453.6 0.4536	grams kilograms
ton, long	1.0160	metric tons
ton, short	0.9072	metric tons
degrees (angle)	0.01745	radians
Fahrenheit degrees	5/9	Celsius degrees or Kelvins <sup>1</sup>

<sup>1</sup> To obtain Celsius (C) temperature readings from Fahrenheit (F) readings, use formula:  $C = (5/9)(F - 32)$ .

To obtain Kelvin (K) readings, use formula:  $K = (5/9)(F - 32) + 273.15$ .

## SYMBOLS AND DEFINITIONS

$A_1, A_1^*, A_2, A_3$	empirical coefficients
$C_1, C_2, C_3$	empirical coefficients
$d$	water depth
$d_b$	water depth at the breaking point
$H$	wave height
$H_b$	breaking wave height
$H'_o$	unrefracted deepwater significant wave height
$H_s$	significant wave height for irregular waves
$L_A$	wavelength given by Airy wave theory
$L_o$	deepwater wavelength given by Airy theory
$m$	beach slope (tangent of the angle between the slope and the horizontal)
$p$	probability level of exceedance
$T$	wave period
$T_c$	duration of wave crest
$T_p$	period of peak energy density for irregular waves
$T_t$	duration of the wave trough
$(\overline{T_c}/T)_p$	mean dimensionless crest duration for all crests above the probability of exceedance level, $p$
$U_R$	Ursell number = $HL_A^2/d^3$
$\eta$	water level
$\eta_b$	crest elevation at breaking
$\eta_c$	elevation of the wave crest above the mean water level (MWL)
$(\eta_c)_p$	elevation of wave crest associated with a probability of exceedance, $p$
$\eta_{rms}$	root-mean-square (rms) water level
$\eta_t$	elevation of wave trough (negative)

## THE ELEVATION AND DURATION OF WAVE CRESTS

by

William N. Seelig, John P. Ahrens, and William G. Grosskopf

### I. INTRODUCTION

Coastal structures are generally built to withstand extreme waves that occur infrequently. One of the important aspects of these waves is that they may be highly nonlinear if the wave steepness is large or the relative water depth is small. Two of the important engineering characteristics of nonlinear waves are that the crest elevations are more than half of the wave height and the duration of the crests are less than half of the wave period.

Crest elevation is important because the forces on a structure may be more strongly related to crest elevation above the water level than the trough distance below the water level. Uplift forces on piers and fixed structures are particularly sensitive to wave crest elevation, which is one of the reasons why most offshore platforms and piers are built well above the water level. Crest elevation also influences wave runup and overtopping (Weggel, 1976a).

The relatively short duration of nonlinear wave crests is important, because the combination of high elevation and short duration produces large water particle motions and accelerations. These hydraulic characteristics have the potential for suspending sediment, lifting armor units, and causing surprisingly high drag forces on coastal structures.

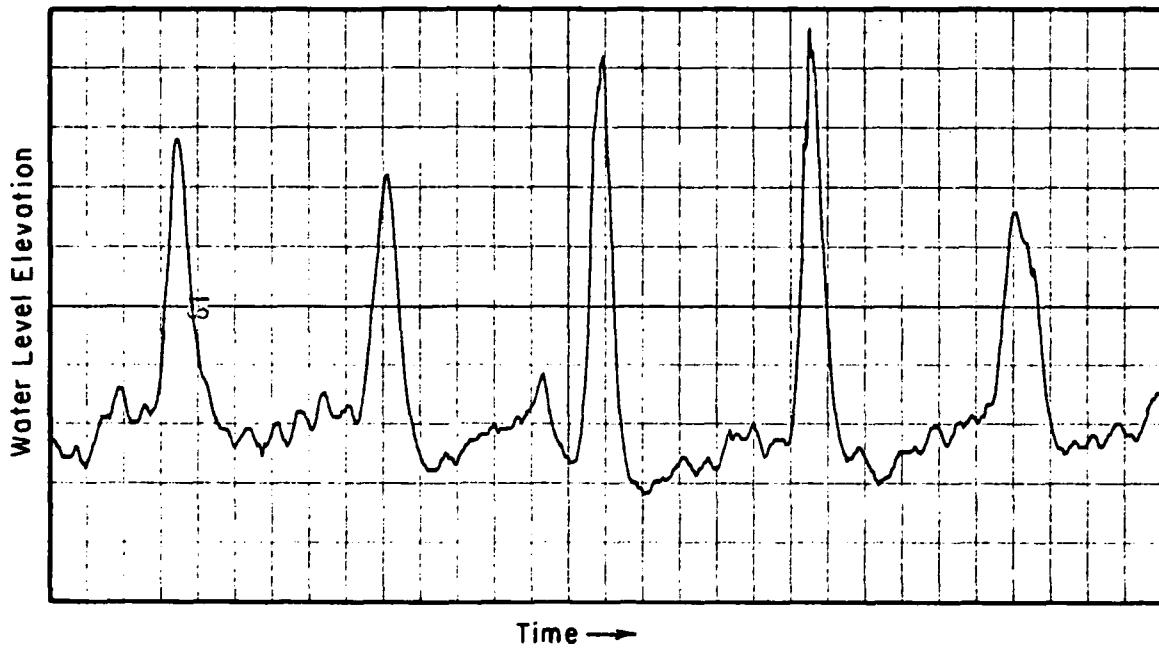
Figure 1 is a sample wave record made as part of the Coastal Engineering Research Center's (CERC) coastal wave collection program (Thompson, 1977). This example clearly shows the high crest elevations and short crest durations that may occur in coastal waters.

This study develops methods of predicting crest elevations and duration for monochromatic and irregular waves. Laboratory waves are used to minimize the influence of refraction, diffraction, irregular topography, directional spread of wave energy, and multiple wave trains on the characteristics of wave crests. Assumed independent variables used in the prediction methods are water depth, wave period or period of peak energy density for irregular waves, wave height (significant wave height for irregular waves), and beach slope. Predictions are compared to selected prototype wave measurements taken at the CERC Field Research Facility (FRF) at Duck, North Carolina.

### II. LITERATURE REVIEW

Goda (1964) performed some basic research on the elevation of monochromatic laboratory wave crests. Using his own data and the data of Bretschneider (1958), Goda found that the ratio of wave height to water depth played a dominant role in determining the relative crest elevation. However, there are considerable scatter in the results.

Jahns and Wheeler (1973) and others have performed theoretical derivations of expected crest heights for irregular wave conditions. Without including nonlinear effects they conclude that a Rayleigh-type distribution should



Significant wave height = 1.96 m  
 Period of peak energy density = 14 s  
 Water depth (approximately) = 6 m

**Figure 1.** Example of an extremely nonlinear wave condition  
 (recorded by a CERC continuous-wire staff at Lake  
 Worth, Florida, 28 March 1971 at 0820).

provide an upper limit or conservative estimate of crest elevations corresponding to various probabilities of exceedance. Actual irregular wave data have crests that are higher than given by this theory, so Jahns and Wheeler (1973) suggest an empirical correction factor. One limitation of the correction factor is that the observed probability of exceedance may deviate several or many orders of magnitude from the Rayleigh distribution.

Dean (1974) developed one of the most comprehensive theories to date for predicting properties of monochromatic waves traveling over a flat bottom. This higher order stream-function theory can be used to predict crest elevation and duration for a wide range of wave and water level conditions. The theory indicates that in the Airy limit, where the wave amplitude approaches zero, the wave crest elevation is approximately equal to one-half the wave height and the duration of the crest is half the wave period. In the cnoidal wave limit the crest elevation approaches the wave height and the duration of the crest becomes small compared to the wave period. Figure 2 is an example predicted water level time history for a highly nonlinear wave. In this extreme condition the ratio of crest elevation,  $\eta_c$ , to wave height,  $H$ , is  $\eta_c/H = 0.90$ . The duration of the crest,  $T_c$ , is one-fourth the wave period,  $T$ , or  $T_c/T = 0.25$ .

Singamsetti and Wind (1980) presented wave crest characteristics for monochromatic laboratory wave conditions at the point of breaking for constant beach slopes,  $m$ , of 1 on 40, 1 on 20, 1 on 10, and 1 on 5.

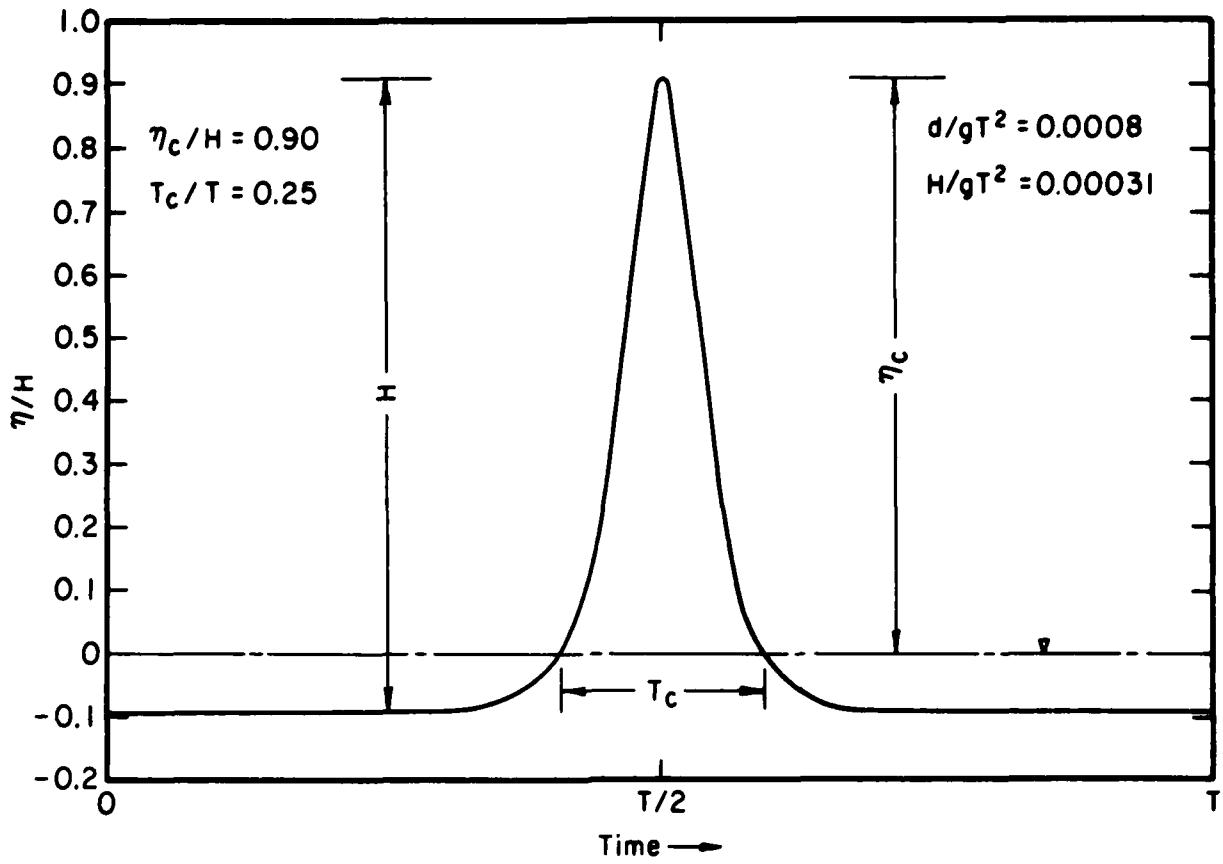


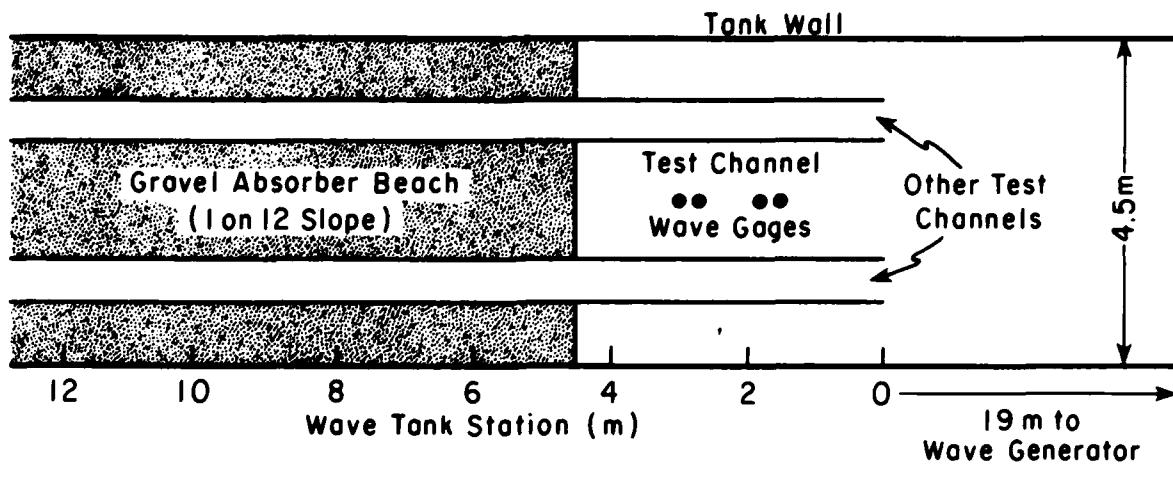
Figure 2. Predicted monochromatic wave profile (after Dean, 1974).

### III. SOURCES OF DATA

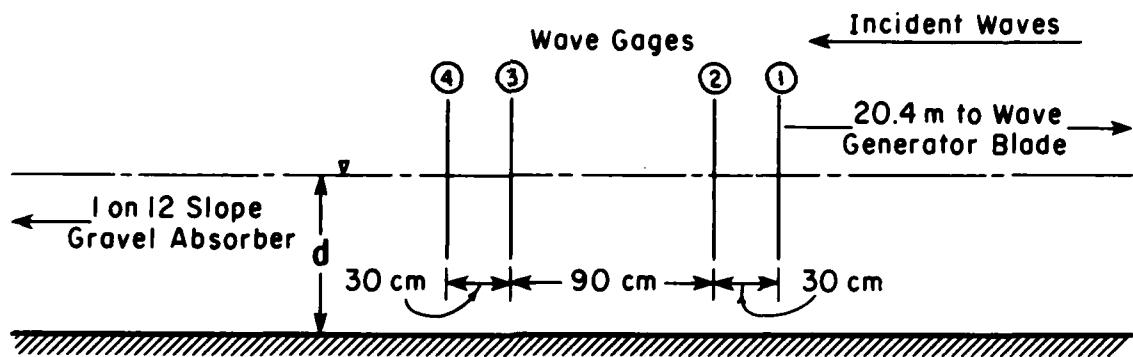
Sources of data upon which the predictive techniques are based include laboratory data collected by the authors for a wide range of conditions, laboratory data collected by other authors, and field data from the FRF at Duck, North Carolina. Conditions under which these data were collected and methods of analysis are described below.

#### 1. Laboratory Tests Conducted by Authors.

a. Wave Conditions, Data, and Test Setups. Most of the monochromatic wave data and all the irregular wave data collected over a horizontal bottom were obtained in a 105.7-centimeter-wide channel within CERC's 1.2-meter-high by 4.5-meter-wide by 42.6-meter-long wave tank. The monochromatic wave data are tabulated in Appendix A and the irregular wave data are tabulated in Appendices B and C. A gravel absorber beach with a slope 1 on 12 was used at the end of the channel to reduce wave reflection to a low level. Four paralleled wire resistance-type wave gages were used in the channel to record the wave conditions. A plan view and a profile view of the test setup with gage locations are shown in Figure 3. Figure 4 shows a short section of typical irregular wave records taken approximately simultaneously at each of the four gages. The time axis of the records has been shifted slightly to place the highest crest in the center of each section to show the differences in the time



a. Plan View



b. Profile View

Figure 3. Experimental setup in CERC's 4.5-meter-wide tank.

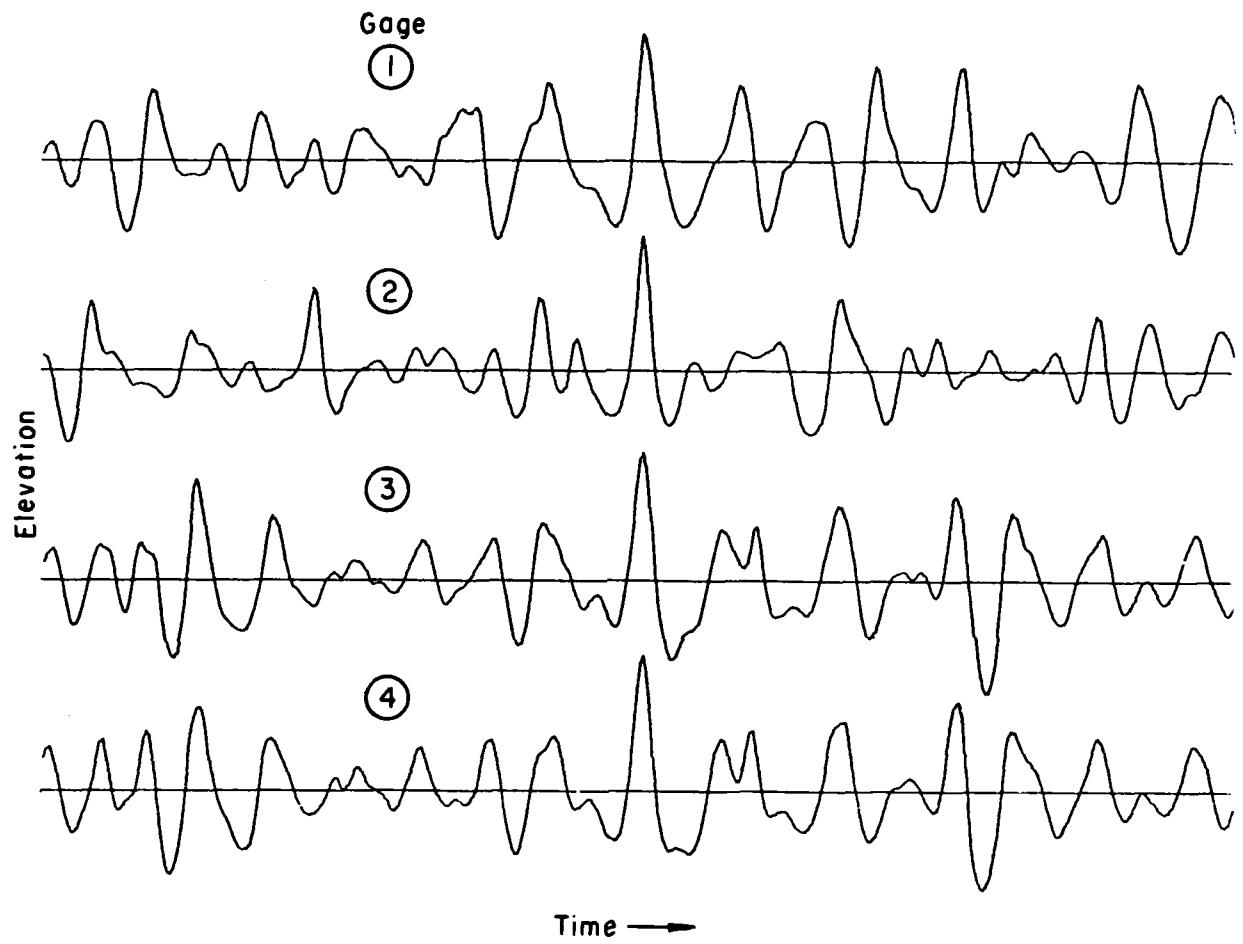


Figure 4. Sample of water level time histories taken at four gages during an irregular wave test (time axis was shifted for the highest crest to appear at the center of each plot).

sequences of the records. Since gages 1 and 4 are the farthest apart they show the greatest differences. The data from gages 1 and 4 are tabulated in Appendixes A, B, and C.

Some additional monochromatic wave data over a horizontal bottom were collected in CERC's 0.9-meter-high by 0.9-meter-wide by 45.7-meter-long wave tank. This tank has a 1 on 30 hogs hair absorber beach to keep reflected wave energy to very low levels. Three parallel wire resistance-type wave gages were used to record the wave conditions. A profile view of the test setup is shown in Figure 5; the data are tabulated in Appendix A.

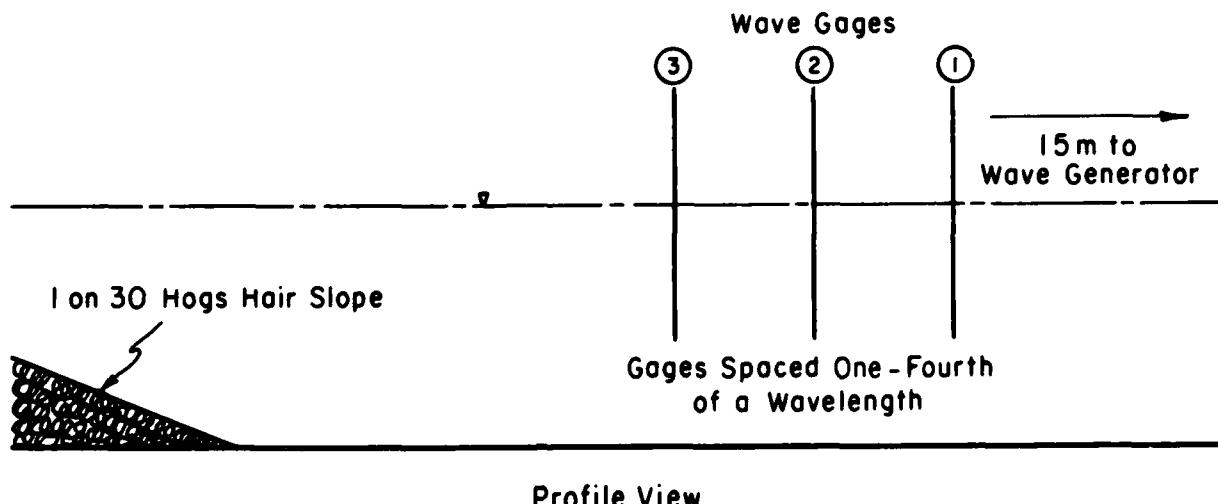


Figure 5. Experimental setup in CERC's 0.9-meter-wide tank.

Wave data were also collected in CERC's 0.9-meter-high by 0.4-meter-wide by 45.7-meter-long wave tank. The setup in this tank was designed to obtain data on the shoaling and breaking of irregular waves. Ten parallel wire wave gages were used: three gages over the horizontal tank bottom and seven over a 1 on 3 concrete slope. Figure 6 shows a profile view of the test setup and gage locations; the data are tabulated in Appendix D.

Table 1 provides a summary of the wave conditions, data and test setups used in the laboratory tests.

b. Procedures and Analysis. All the waves were generated using hydraulically actuated piston-type wave makers. The monochromatic waves studied included waves with sinusoidal blade motion with Ursell numbers less than 25 and cnoidal waves with Ursell numbers greater than 25. The Ursell number,  $U_R$ , is defined

$$U_R = \frac{HL_A^2}{d^3} \quad (1)$$

where  $H$  is the average wave height over the flat bottom part of the wave tank and  $L_A$  the local wavelength, calculated using linear theory and defined by

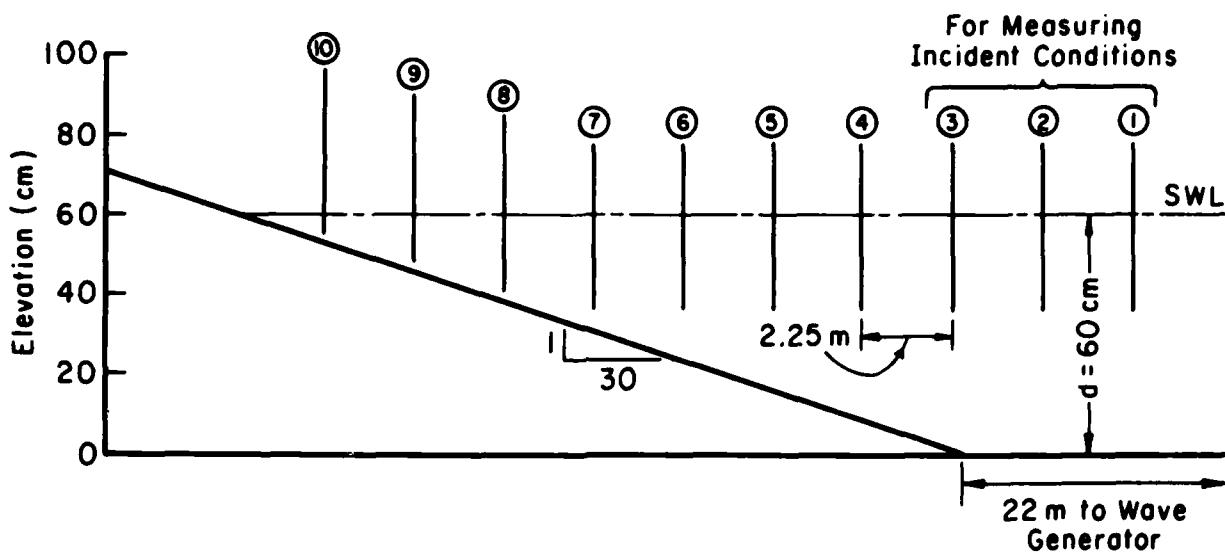


Figure 6. Experimental setup in CERC's 0.4-meter-wide tank

Table 1. Summary of CERC laboratory test data.

Wave tank	Wave conditions	Bottom slope	Test setup, Fig. No.	Data in App.
1.2 by 4.5 by 42.6 m	Monochromatic	Horizontal	3	A
1.2 by 4.5 by 42.6 m	Irregular	Horizontal	3	B, C
0.9 by 0.9 by 45.7 m	Monochromatic	Horizontal	5	A
0.9 by 0.4 by 45.7 m	Irregular	Horizontal and 1 on 30	6	D

$$L_A = \frac{gT^2}{2\pi} \tanh \left( \frac{2\pi d}{L_A} \right) \quad (2)$$

where  $T$  is the wave period and  $d$  the water depth. For sinusoidal blade motion the restriction on the Ursell number was to eliminate the influence of secondary waves; the cnoidal wave data were collected by Weggel (1976b) and are discussed in Section 2.

The irregular waves used in this study included a wide range of spectral types and relative depths. Spectra were simulated by taking various theoretical spectra, such as Pierson-Moskowitz, Joint North Sea Wave Project (JONSWAP), Bretschneider, and the six-parameter spectra of Ochi and Hubble (1976), and slicing the spectra into 60 equal area segments with each segment represented by a sinusoidal component at the frequency of the midpoint of the segment. Each component was transformed into a piston blade motion using Biesel's equation (Biesel, 1951) at the given frequency and assuming a random-phase relation among the components. The time histories of the 60 components were

linearly superimposed to determine the generator blade motion. Figure 7 shows a sample of observed and predicted wave spectra.

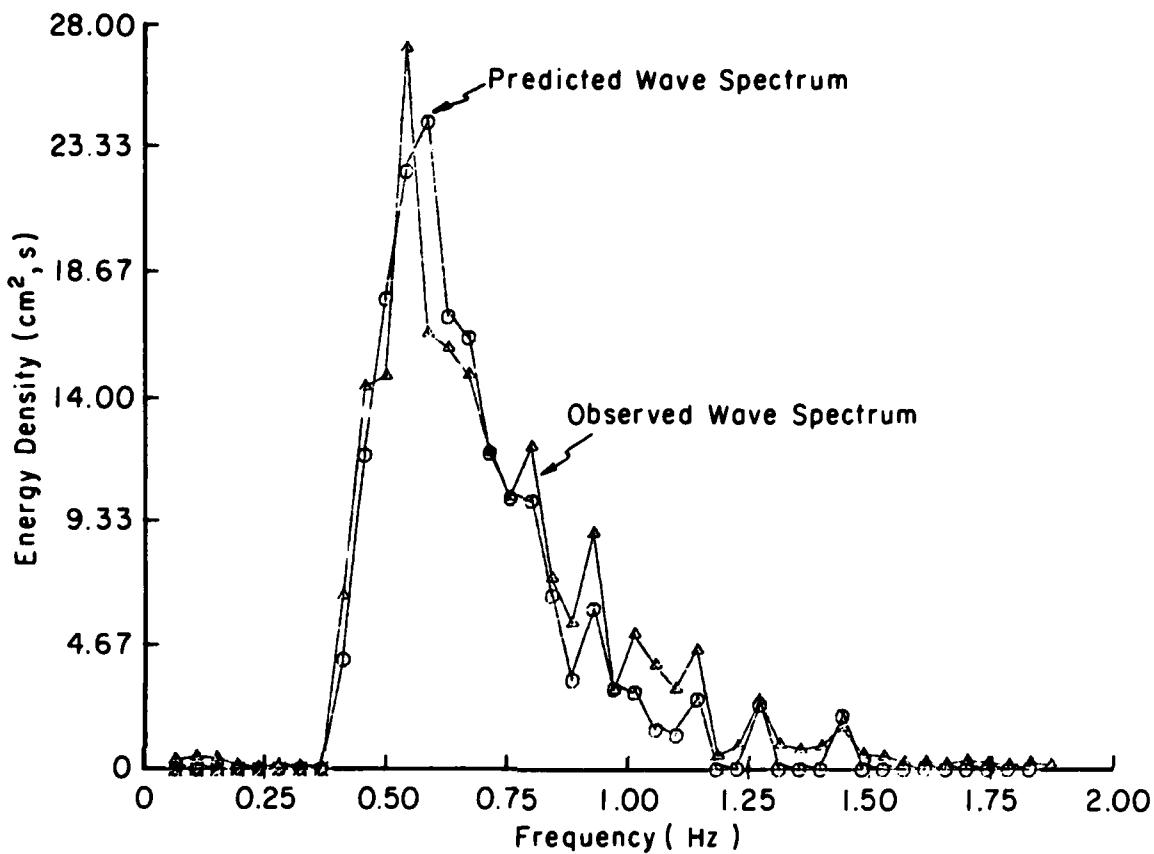


Figure 7. Observed and predicted wave spectra (sample of Pierson-Moskowitz spectra).

In all tests the water surface time history was recorded at each wave gage for 4,096 data points. A sampling rate of 16 times per second was used for waves with periods less than 3 seconds, and a rate of 8 times per second was used for waves with periods greater than 3 seconds. The data were digitized and recorded on magnetic tape through the use of a Data Acquisition System (DAS). An analysis of the tapes was performed later on a general-purpose computer.

Two distinct types of data analysis were performed on the digitized wave records: a fast Fourier transform (FFT) and a zero up-crossing analysis. The FFT was used to determine the spectrum of the wave record, such as shown in Figure 7, which in turn was used to determine the period of peak energy density. The period of peak energy density,  $T_p$ , is the reciprocal of the frequency of the midpoint of the 11 consecutive spectral lines with the most wave energy. The zero up-crossing method of analysis is used to determine the wave heights, crest heights, wave periods, and crest durations of individual waves in irregular wave trains or average values for monochromatic wave trains. Crest height,  $n_c$ , is measured relative to the mean water level (MWL) and

crest duration,  $T_c$ , is the time difference between an adjacent zero up-cross and a zero down-cross; zero indicates the MWL of the record. The significant wave height,  $H_s$ , for irregular waves was computed by multiplying the root-mean-square (rms) water surface displacement by four. All the analysis was based on the assumption that there was no wave reflection in any of the channels where data were collected.

For irregular wave conditions the elevations of the wave crests were ranked from the highest to the lowest and the probability of exceedance,  $p$ , for various levels of probability were calculated. For example,  $\eta_{0.01}$  indicates a crest height with a probability of being exceeded by 1 percent of the crest heights, i.e.,  $p = 0.01$ . Since the larger waves were of the most interest the following probability levels for crest heights were calculated:  $p = 0.005, 0.01, 0.02, 0.05, 0.10, 0.135, 0.15$ . Dimensionless wave crest height probability levels,  $(\eta_c)p/H_s$ , for wave gages 1 and 4 of the test setup shown in Figure 3 are tabulated in Appendix B. The mean dimensionless crest duration,  $(T_c/T)_p$ , was also determined for the highest 5, 10, 15, 20, 25, 33, and 100 percent of the wave crests for gages 1 and 4 and is tabulated in Appendix C.

## 2. Other Laboratory Tests Used.

Weggel (1976b) conducted tests using nonsinusoidal blade motion to generate cnoidal waves. These waves have Ursell numbers between 27 and 184 and are relatively free of secondary waves. The cnoidal wave conditions are given in Appendix E.

Singamsetti and Wind (1980) studied the breaking of monochromatic waves on a number of smooth laboratory slopes. The relative crest elevations at breaking  $\eta_b/H_b$  and the relative crest durations at breaking,  $(T_c/T)_b$  are tabulated in Appendix A, where  $\eta_b$  is the height of the crest above MWL at breaking,  $H_b$  is the wave height at breaking, and  $T$  is the period of a monochromatic wave.

## 3. Prototype Data Used.

Prototype wave data collected at FRF, Duck, North Carolina, are used in this study, particularly the data measured during a major storm in October 1980 in which a large variety of wave steepnesses were observed. The gages were Waverider buoys located 0.5, 3, 6, and 12 kilometers from shore in an area where the bottom contours were generally straight and parallel. The dominant wave direction was approximately along the line of the instruments. The data were analyzed in the same manner as that previously described for the laboratory data.

## IV. TEST RESULTS AND PREDICTION TECHNIQUES FOR MONOCHROMATIC WAVES

Monochromatic laboratory wave test results are compared with Dean's streamfunction wave theory (1974) and the resulting prediction techniques are discussed.

## 1. Monochromatic Wave Crest Elevations.

a. Waves Traveling Over a Horizontal Bottom. Dean (1974) developed a stream-function wave theory to predict a number of wave characteristics for waves propagating over a flat horizontal bottom for a number of relative depths and wave heights equal to 0.25, 0.50, 0.75, and 1.00 of the breaking limit. An examination of the tabular values of  $\eta_c/H$  (Dean, 1974, Vol. II) shows that relative crest height can be approximated using the empirical relation:

$$\frac{\eta_c}{H} = \frac{A_1}{1.0 + \tanh \left[ -A_2 \ln \left( A_3 \frac{H}{L_o} \right) \right]} \quad (3)$$

where  $L_o$  is the deepwater wavelength given by Airy theory as

$$L_o = \frac{gT^2}{2\pi} \quad (4)$$

Wave trough elevation,  $\eta_t$ , is equal to wave height minus crest elevation. Dean's tabular results are compared with equation (3) and  $A_1$  is found to be 1.0 for wave traveling over a flat bottom ( $m = 0.0$ ). Values of  $A_2$  and  $A_3$  are functions of relative depth ( $d/L_o$  or  $d/(gT^2)$ ), as shown in Figure 8 and given in Table 2.

Laboratory data in this study show excellent agreement with Dean's stream-function theory and give a value  $A_1 = 0.992 \pm 0.0393$  (Table 3). Cnoidal waves ( $U_R > 25$ ) have slightly smaller values of this empirical parameter with  $A_1 = 0.95 \pm 0.038$  (Table 3).

b. Waves on a Slope. Singamsetti and Wind's (1980) data are used to evaluate  $A_1$  for monochromatic waves at the breaking point on various beach slopes. Wave crest elevations, as indicated by  $A_1$ , decrease approximately linearly with beach slope,  $m$  (Table 3, Fig. 9). For example, waves at the breaking point on a 1 on 5 slope ( $m = 0.20$ ) have only 67 percent of the crest elevations of a wave with same  $H/(gT^2)$  and  $d/(gT^2)$  traveling over a flat bottom.

Figure 10 illustrates the predicted influence of beach slope on the breaking crest elevation for a sample wave tank condition. In this case waves in the incident flat part of the tank have  $d/(gT^2) = 0.019$  and  $H/d = 0.37$ . The figure shows the predicted crest elevation at the breaking point using the method described in Appendix F for estimating  $\eta_c$ . In this example slopes flatter than 1 on 20 have little influence on the crest elevation, while slopes steeper than 1 on 20 produce lower crest elevations.

## 2. Crest Elevation Prediction Aids.

Equation (3), which is used for predicting monochromatic wave crest elevations, can be presented in a number of graphical forms useful for predicting and understanding trends in relative crest elevation. Figure 11 shows  $\eta_c/H$  versus relative depth for various wave steepnesses. Relative crest elevation,

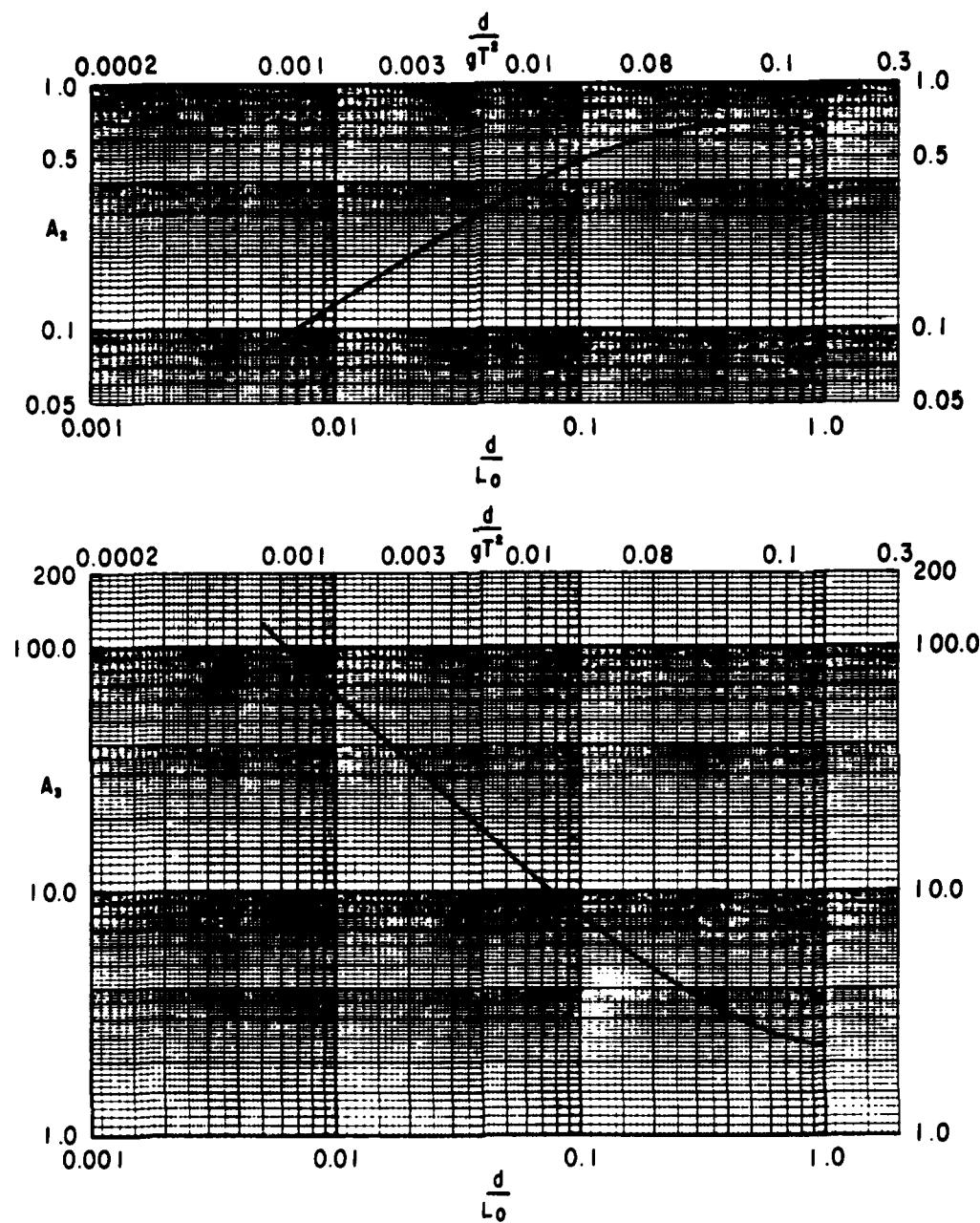


Figure 8. Empirical coefficients  $A_2$  and  $A_3$  for estimating  $n_c/H$ .

Table 2. Values of the empirical coefficients  $A_2$  and  $A_3$  for predicting relative crest.<sup>1</sup>

$d/L_o$	$d/gT^2$	$A_2$	$A_3$
0.005	0.000796	0.0791	125.9
0.01	0.00159	0.124	67.6
0.02	0.00318	0.197	35.5
0.05	0.00796	0.353	15.07
0.10	0.0159	0.498	7.71
0.20	0.0318	0.609	4.11
0.50	0.0796	0.698	2.82
1.0	0.159	0.624	2.37

<sup>1</sup>Elevation for horizontal bottoms, using equation (3) and  $A_1 = 1.0$ .

Table 3. Observed versus predicted relative crest elevations for monochromatic waves.

Type of wave generator motion	$1/m$	$A_1$	Std. dev.	Coefficient of variation (pct)	No. of tests	$U_R$ range	Source
Sine	00	0.992	0.0393	4.0	53	0.3 to 25	This study
Cnoidal	00	0.95	0.038	4.0	13	27 to 184	Weggel (1976b)
Sine	40	0.909 <sup>1</sup>	0.0375	4.1	26	3.7 to 24	Singamsetti and Wind (1980)
Sine	20	0.88 <sup>1</sup>	0.035	4.0	12	1.8 to 24	Singamsetti and Wind (1980)
Sine	10	0.818 <sup>1</sup>	0.0752	9.2	22	2.5 to 23	Singamsetti and Wind (1980)
Sine	5	0.672 <sup>1</sup>	0.0370	5.5	21	2.0 to 22	Singamsetti and Wind (1980)

<sup>1</sup>At the breaking point.

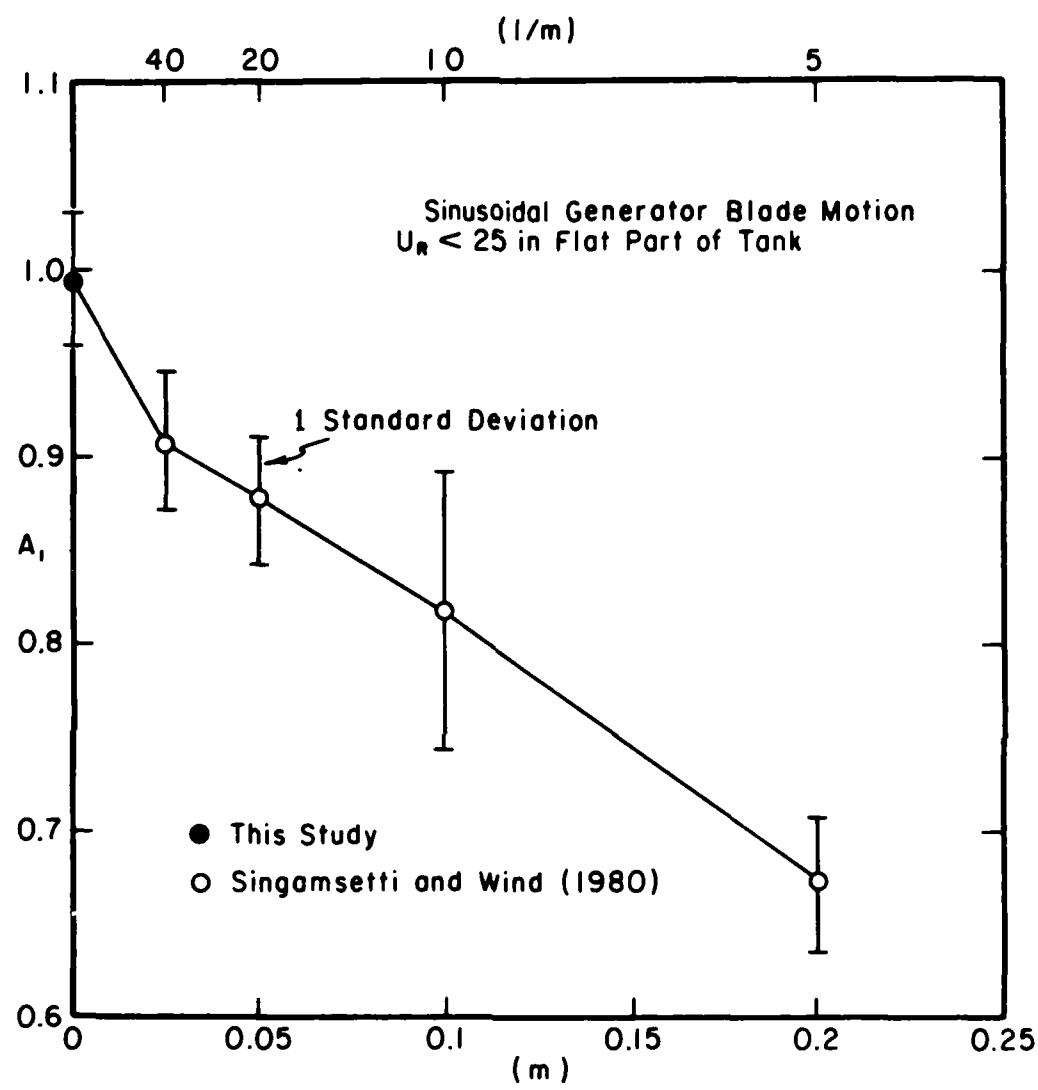


Figure 9. Effect of beach slope on relative wave crest elevation.

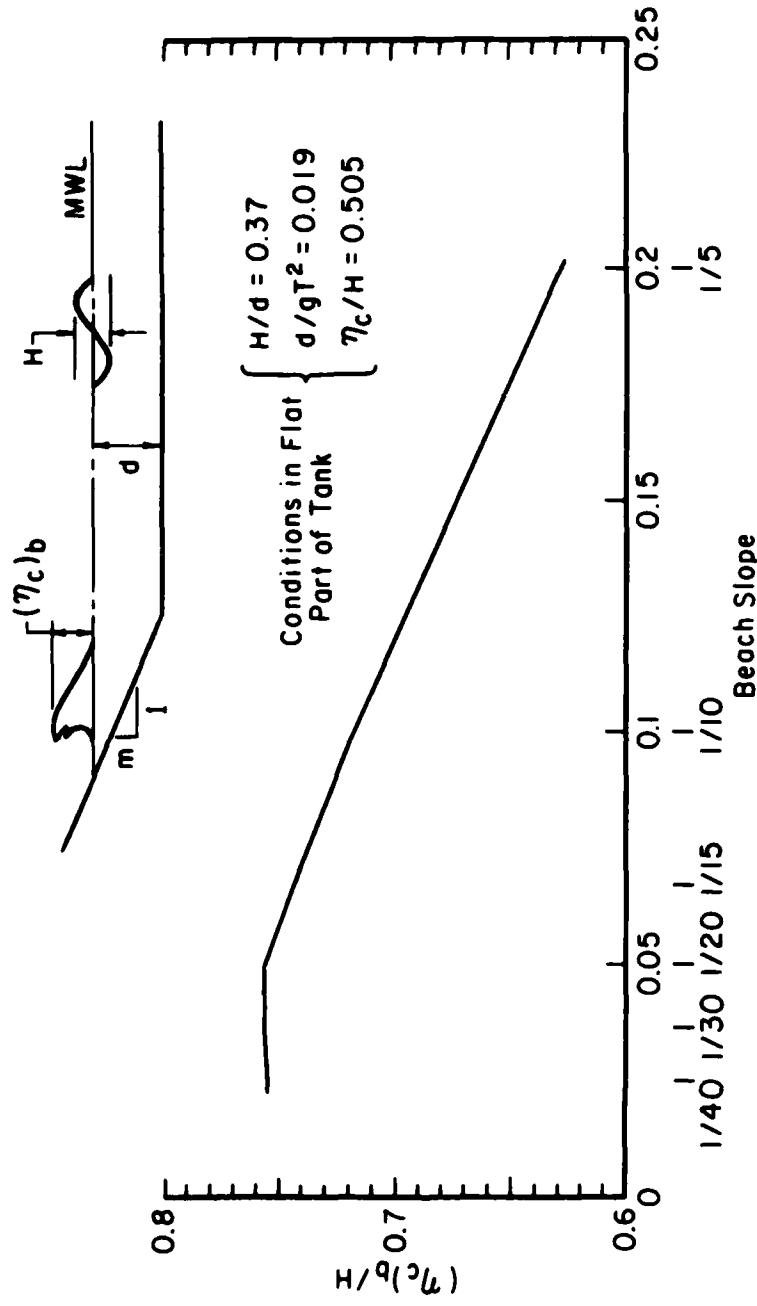


Figure 10. Effect of beach slope on breaker crest elevation.

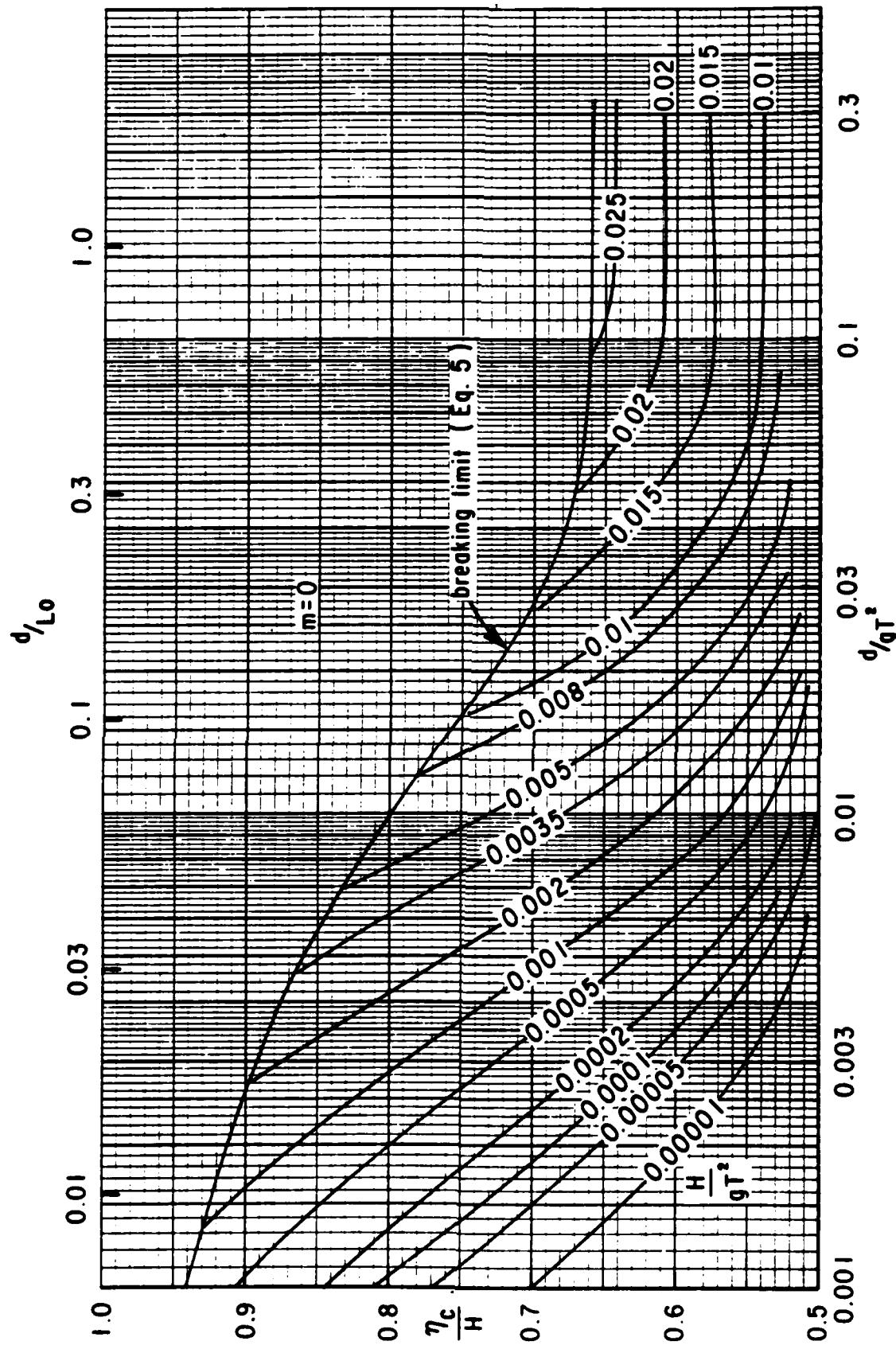


Figure 11. Wave crest elevation for constant values of wave steepness.

$\eta_c/H$ , is strongly influenced by  $d/(gT^2)$  for a given value of wave steepness for  $d/(gT^2) > 0.05$ . The breaking limit,  $H_b$ , given in this figure is from Goda (1975):

$$H_b = 0.17 L_o \left\{ 1.0 - \exp \left( -4.712 \frac{d}{L_o} \left[ 1.0 + 15m^{1.333} \right] \right) \right\} \quad (5)$$

Figure 12 presents  $\eta_c/H$  for constant values of the ratio of wave height to breaker height,  $H/H_b$ . The figure shows that in relatively deep water,  $\eta_c/H$  only deviates from the Airy condition of  $\eta_c/H = 0.5$  when the wave height becomes a significant fraction of the breaking wave height. Relatively shallower water causes a wave to become more nonlinear, even when the wave height is small compared to the maximum possible breaker height. For example,  $\eta_c/H = 0.75$  at  $d/(gT^2) = 0.0015$  for  $H/H_b = 0.1$  (Fig. 12).

The parameter  $\eta_c/d$  is presented in Figure 13 for constant values of  $H/d$  at various values of  $d/(gT^2)$ . This figure shows the combined influence of wave steepness and water depth on crest elevation.  $\eta_c/d$  reaches a minimum for constant values of  $H/d$  at  $d/(gT^2)$  in the neighborhood of 0.03. At values of  $d/(gT^2)$  larger than 0.03 the increasing wave steepness causes  $\eta_c/d$  to increase until the breaking point is reached. For smaller values of  $d/(gT^2)$  the decreased depth or increased period produces greater wave non-linearity and increased  $\eta_c/d$ .

### 3. Relative Crest Duration for Monochromatic Waves.

As waves become increasingly nonlinear the duration of the crest,  $T_c$ , decreases to less than half of the wave period. Values of relative crest duration,  $T_c/T$ , predicted by stream-function theory (Dean, 1974) can be approximated by the empirical relation:

$$\frac{T_c}{T} = C_1 \tanh \left[ C_2 \ln \left( C_3 \frac{L_o}{H} \right) \right] \quad (6)$$

where  $C_1$ ,  $C_2$ , and  $C_3$  are empirical coefficients with  $C_1 = 0.5$  for waves on a flat bottom ( $m = 0.0$ ). Values of  $C_2$  and  $C_3$  are given in Figure 14 and Table 4. Good correlation is found between equation (6) and experimental data for small beach slopes as shown in Table 5. However, the equation does not apply to beach slopes steeper than 1 on 20 ( $m = 0.05$ ).

Figure 15 presents predicted values of  $T_c/T$  as functions of  $d/(gT^2)$  and  $H/(gT^2)$ . Note that the duration of the wave trough,  $T_t$  is given by

$$\frac{T_t}{T} = 1.0 - \frac{T_c}{T} \quad (7)$$

## V. TEST RESULTS AND PREDICTION TECHNIQUES FOR IRREGULAR WAVES

Irregular wave crest elevations and durations are more variable than monochromatic wave conditions because wave energy is distributed over a range of frequencies. This energy distribution produces waves with a variety of heights and periods, which have varying amounts of nonlinearity. Larger waves are of

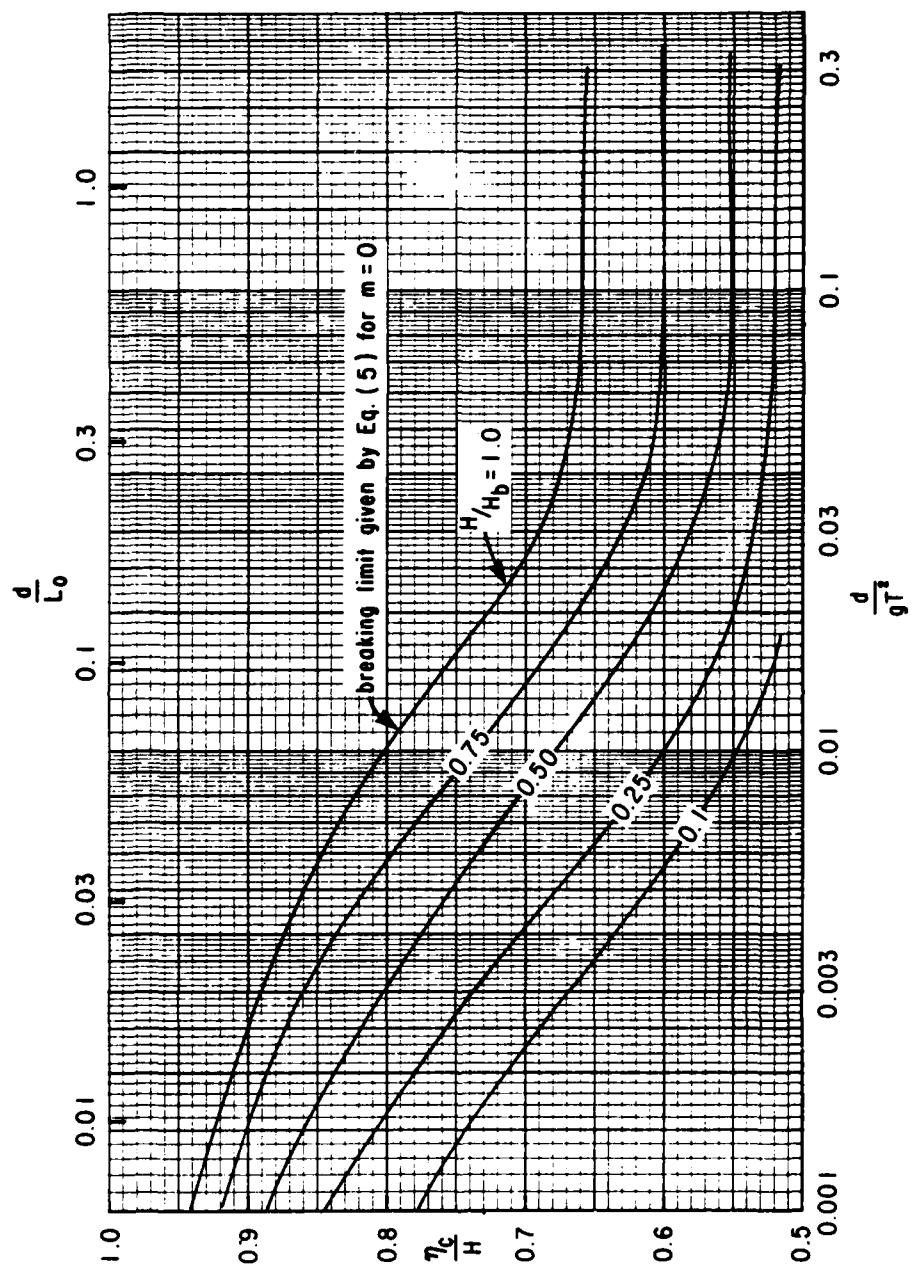


Figure 12. Wave crest elevation for constant values of  $H/H_b$ .

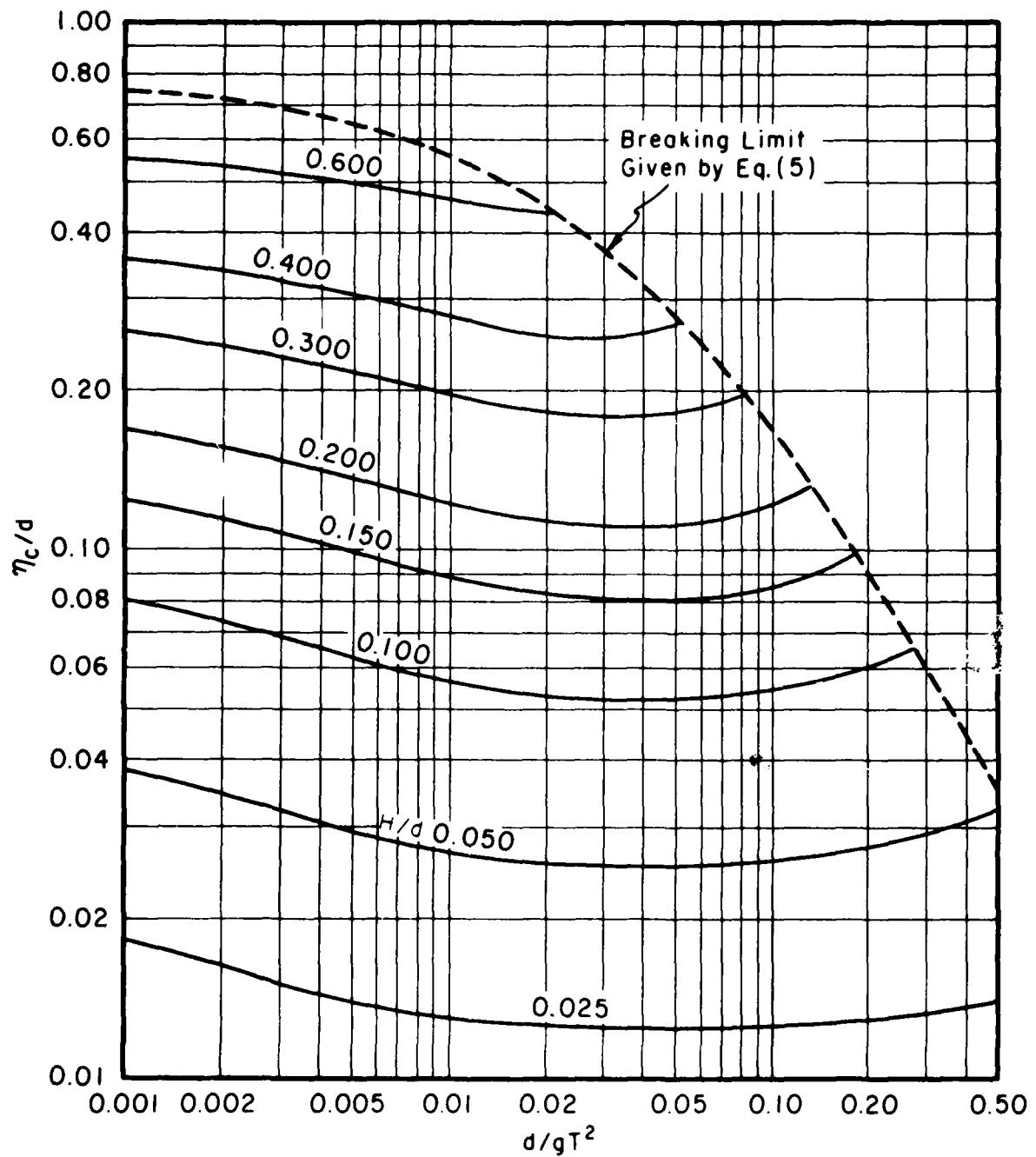


Figure 13.  $\eta_c/d$  for constant values of wave height to depth.

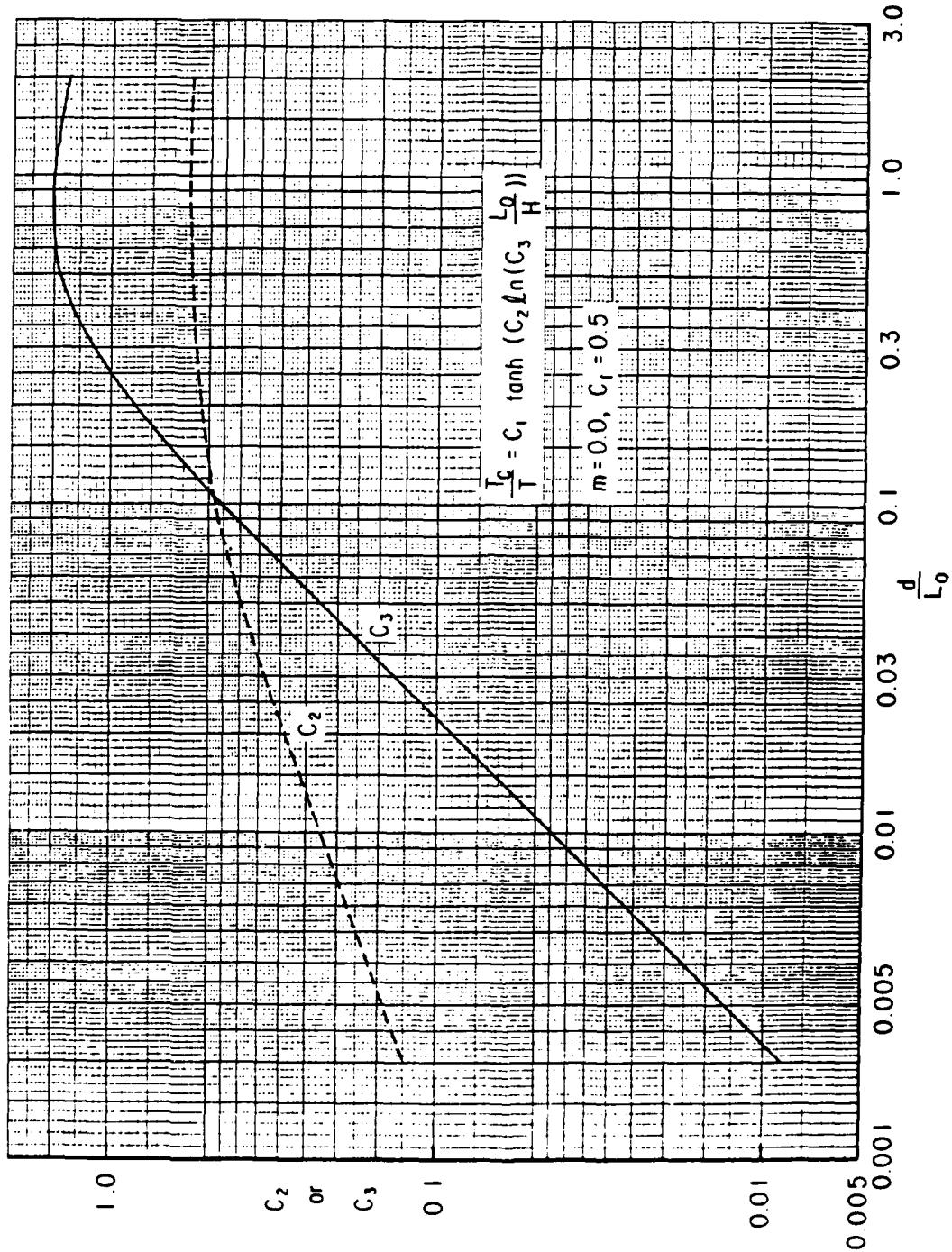


Figure 14. Empirical parameters for estimating wave crest durations ( $m = 0.0$ ).

Table 4. Coefficients for estimating the duration of wave crests.<sup>1</sup>

$d/L_o$	$d/gT^2$	$C_2$	$C_3$
0.002	0.00032	0.1242	0.00788
0.005	0.00080	0.1784	0.0205
0.01	0.0016	0.2025	0.044
0.02	0.0032	0.2903	0.0852
0.05	0.0080	0.4036	0.2058
0.1	0.016	0.4702	0.4529
0.2	0.032	0.5442	0.8268
0.5	0.080	0.5518	1.448
1.0	0.16	0.5542	1.4601
2.0	0.32	0.5691	1.363

<sup>1</sup> $C_1 = 0.5$  for waves on a flat bottom  
( $m = 0.0$ ).

Table 5. Observed versus predicted relative crest durations (monochromatic waves-sinusoidal generator blade motion).

$1/m$	Mean value of $C_1$	Std. dev. of $C_1$	Coefficient of variation of $C_1$ (pct)	No. of tests	Source
$\infty$	0.498	0.042	8.4	23	This study
$40^1$	0.53	0.104	19.6	26	Singamsetti and Wind (1980)
$20^1$	0.56	0.11	19.6	24	Singamsetti and Wind (1980)
$10^1$	0.70	0.42	60.0	22	Singamsetti and Wind (1980)
$5^1$	0.91	0.32	35.2	21	Singamsetti and Wind (1980)

<sup>1</sup>At the breaking point.

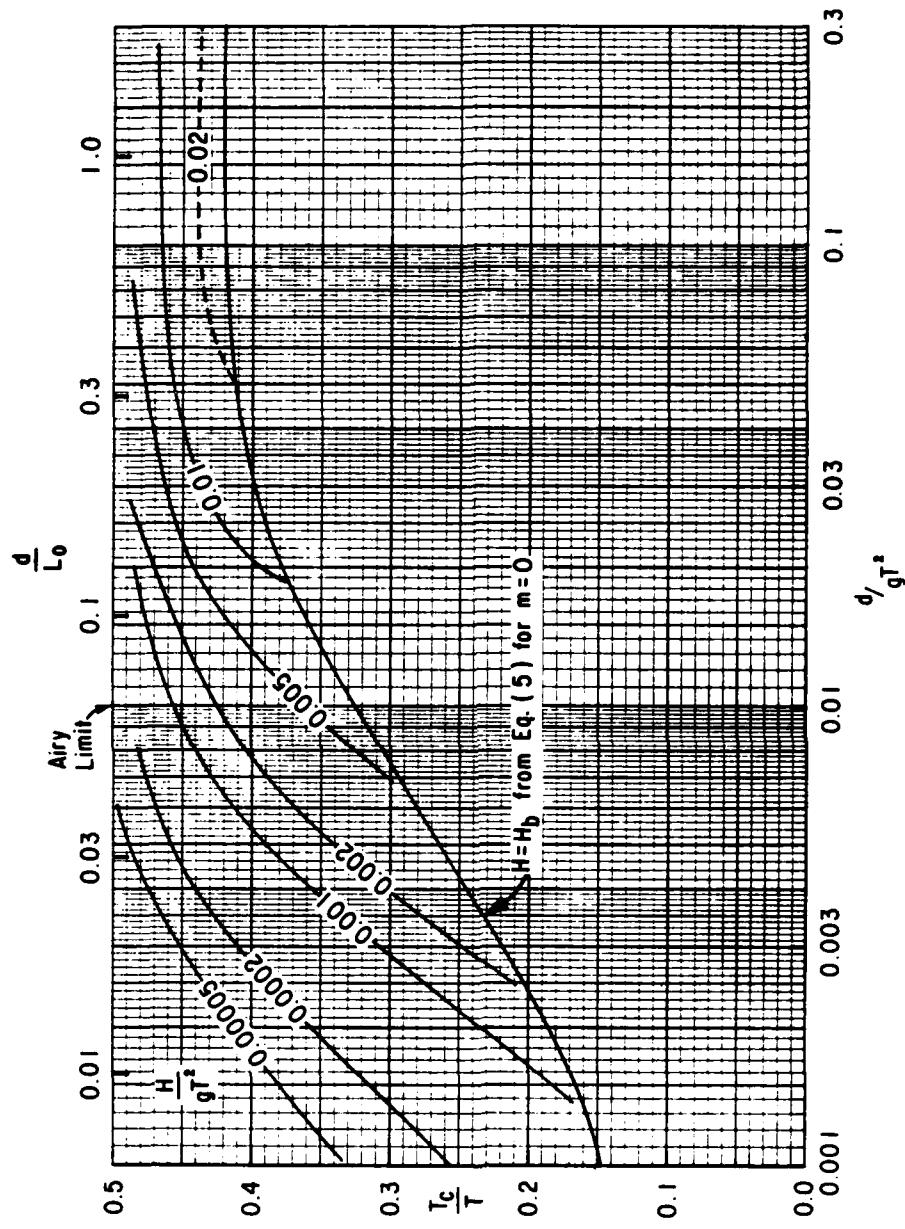


Figure 15. Values of dimensionless wave crest duration.

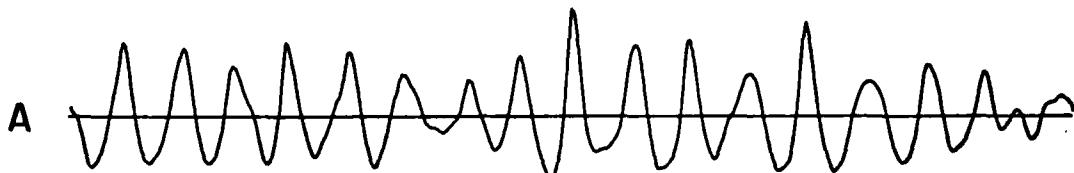
greatest interest, so the primary emphasis of this analysis is on the highest 15 percent of wave crests in a wave train. The variable  $p$  is used to designate the probability level of exceedance of interest. For example,  $(\eta_c)_{p=0.05}$ , where the subscript  $p = 0.05$ , indicates that 5 percent of the crest elevations exceed the value of  $\eta_c$ .

Assumed independent variables are significant wave height,  $H_s$  (defined as four times the rms water level,  $\eta_{rms}$ ), period of peak energy density,  $T_p$ , water depth,  $d$ , and beach slope,  $m$ .

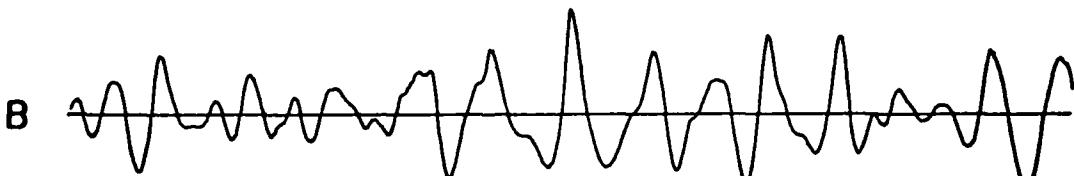
### 1. Irregular Wave Crest Elevations.

Figure 16 presents several irregular wave records illustrating various amounts of nonlinearity. Records A and B have crests which are slightly larger than half of their zero up-crossing wave heights; record C has crest elevations which are a large fraction of wave height. Table 6, which is a compilation of selected wave record results, indicates the elevation of maximum crests for various water depth, significant wave height, and period of peak energy density

$$d/gT_p^2 = 0.025 \quad H_s/gT_p^2 = 0.0063 \quad H_s/d = 0.25$$



$$d/gT_p^2 = 0.017 \quad H_s/gT_p^2 = 0.0035 \quad H_s/d = 0.21$$



$$d/gT_p^2 = 0.0041 \quad H_s/gT_p^2 = 0.00106 \quad H_s/d = 0.26$$



Figure 16. Sample of irregular wave time histories.

Table 6. Selected irregular wave crest elevation data.

Location	Date	Time	$H_s/d$	$H_s/gT_p^2$	$d/gT_p^2$	No. of waves	Highest crest observed		Source
							$\eta_c/H_s$	$\eta_c/d$	
Field Data									
Atlantic City, N.J.	2 Mar. 69	2001	0.47	0.0036	0.0078	60	1.088	0.51	Thompson (1980)
Atlantic City, N.J.	21 Jan. 69	1202	0.54	0.0032	0.0058	52	1.31	0.71	Thompson (1980)
Nags Head, N.C.	19 Oct. 72	1840	0.45	0.0055	0.012	106	0.897	0.40	Thompson (1980)
Lake Worth, Fla.	28 Mar. 71	0820	0.32	0.001	0.0031	72	1.65	0.53	Thompson (1980)
Gulf of Mexico			(Small?)	?	?	22,194	1.308	0.41	Jahns and Wheeler (1973)
Laboratory Data ( $m = 0$ )									
	17 Oct. 80	1404	0.084	0.00035	0.0042	288	1.71	0.14	This study
	17 Oct. 80	1241	0.285	0.00061	0.0021	300	1.63	0.47	This study
	17 Oct. 80	1427	0.27	0.00057	0.0021	300	1.406	0.38	This study
	6 Oct. 80	1257	0.122	0.0055	0.045	224	1.40	0.17	This study
	6 Oct. 80	1306	0.158	0.0053	0.0337	205	1.449	0.23	This study
	6 Oct. 80	1417	0.117	0.0057	0.049	236	1.28	0.15	This study

combinations. Thompson (1980) discussed several coastal gage records with large ratios of  $H_s/d$  that have  $\eta_c/H_s$  ratios on the order of one for the highest crest observed, while  $\eta_c/d$  for the largest crest is somewhat less than one. Jahns and Wheeler (1973) found similar extremes for hurricane and storm waves in the Gulf of Mexico. Large values of  $\eta_c/H_s$  can be obtained if the significant wave height is small compared to the water depth and the wave steepness is low. Individual wave crests with elevations greater than 1.5 times the significant wave height were observed in a number of the laboratory experiments, where  $H_s/d < 0.3$  and  $d/(gT_p^2) < 0.005$  for  $m = 0.0$ .

Similar values of maximum wave crest elevation were found for irregular waves shoaling on a 1 on 30 laboratory smooth slope. An examination of a typical set of results showed that crest elevation increases as the wave shoals, then decreases as the wave breaks (Fig. 17). Note that in this figure both the crest elevation and the stillwater depth have been normalized by the deepwater equivalent significant wave height,  $H_o'$ . Typical values of the single highest crest observed during an irregular wave test of 260 seconds for model time are presented in Table 7. Water level time histories at several gages at various depths along the 1 on 30 slope are given in Figure 18 for an irregular wave condition.

Jahns and Wheeler (1973) suggested a method for predicting irregular wave crest statistics, using the dimensionless parameter  $(\eta_c)/H_s$ . A Rayleigh-type distribution is assumed, then an empirical correction factor is recommended to increase the probability of occurrence of crests. One of the disadvantages of this approach is that the data often dramatically differ from a Rayleigh distribution because actual probabilities are orders of magnitude larger than predicted. For example, Figure 19 compares crest height distributions for the wave records A, B, and C shown in Figure 16 with the Rayleigh distribution.

The approach taken in this report is to predict crest elevations in terms of local parameters for a given probability level in terms of  $H_s$ ,  $d$ , and  $T_p$ . Figure 20 shows values of  $\eta_c/d$  for wave records A, B, and C illustrated in Figure 16.

The suggested method for predicting irregular wave crest elevations is to use the stream-function results to determine the general form of the equation and laboratory results to calibrate the prediction technique for various probability levels. The equation used is

$$\frac{(\eta_c)_p}{H_s} = \frac{A_1^*}{1.0 + \tanh \left( -A_2 \ln \left( A_3 \frac{H_s}{L_o} \right) \right)} \quad (8)$$

where the parameters  $A_2$  and  $A_3$  are taken from stream-function results (Fig. 8 and Table 2), and  $A_1^*$  is an empirical parameter determined from the experimental data. The laboratory data (App. C) indicate that  $A_1^*$  is a function of the probability of exceedance, with  $A_1^*$  increasing as the probability,  $p$ , decreases (Fig. 21, Table 8). Equation (8) shows good correlation with the laboratory data as indicated by the small standard deviation of  $A_1^*$  about the mean for values of  $p = 0.01$  and greater (Tables 8 and 9).  $A_1^*$  at  $p = 0.005$  has poor resolution because the data collection runs had only a few crests more than 200 crests per run. Figure 22 gives observed and predicted crest elevations at the 2-percent probability level of exceedance.

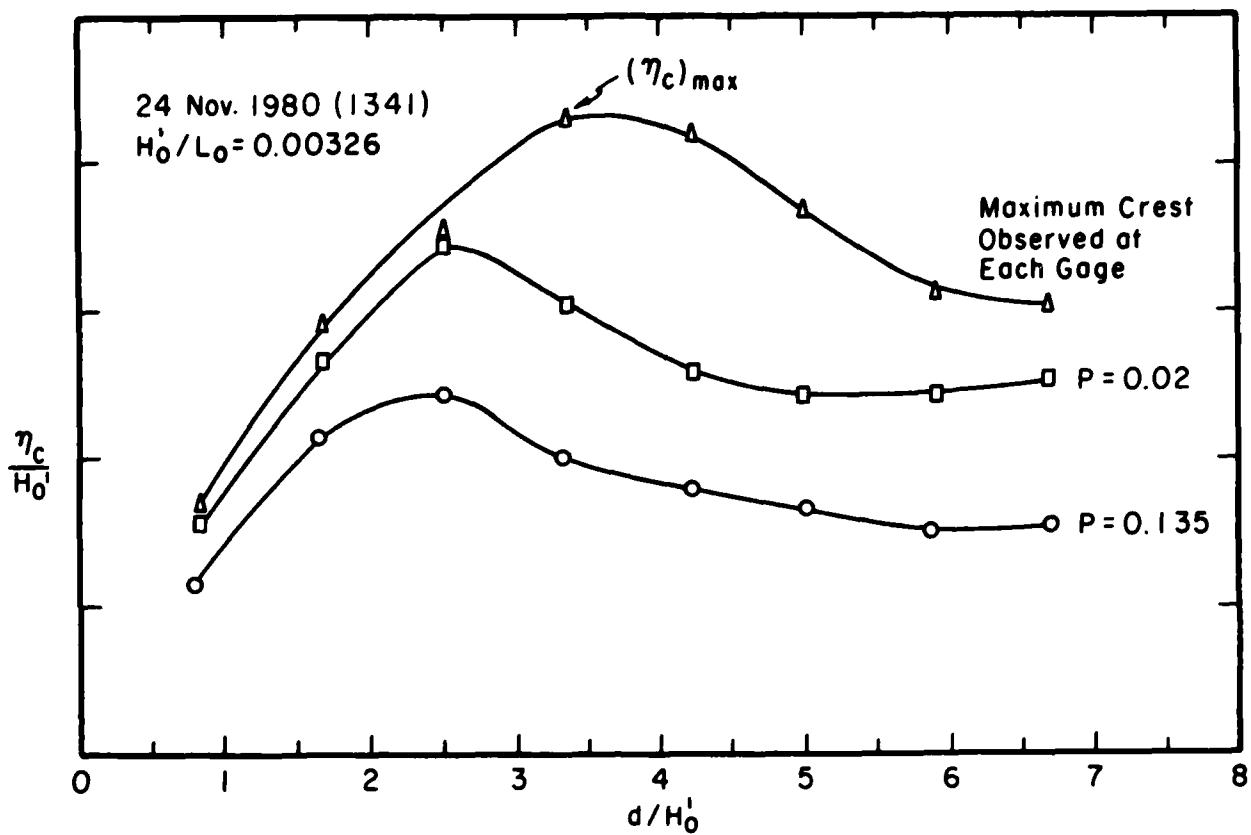


Figure 17. Variation in wave crest elevation for a 1 on 30 slope.

Table 7. Selected irregular wave crest data for waves breaking on a 1 on 30 slope.

Date	Time	$H_o^I/L_o$	Wave and crest data at the occurrence of the highest single crest				
			$H_s/d$	$d/gT_p^2$	$(\eta_c)_{max}/H_s$	$(\eta_c)_{max}/d$	$(\eta_c)_{max}/H_o^I$
25 Nov. 80	1223	0.040	0.27	0.12	1.23	0.33	1.03
25 Nov. 80	1110	0.032	0.35	0.067	1.25	0.44	1.08
24 Nov. 80	1517	0.024	0.41	0.051	1.29	0.53	1.15
24 Nov. 80	1310	0.021	0.33	0.057	1.27	0.42	1.15
24 Nov. 80	1431	0.0115	0.38	0.031	1.60	0.60	1.60
24 Nov. 80	1259	0.0050	0.27	0.021	1.86	0.50	2.13
24 Nov. 80	1332	0.0033	0.29	0.014	1.75	0.51	2.16

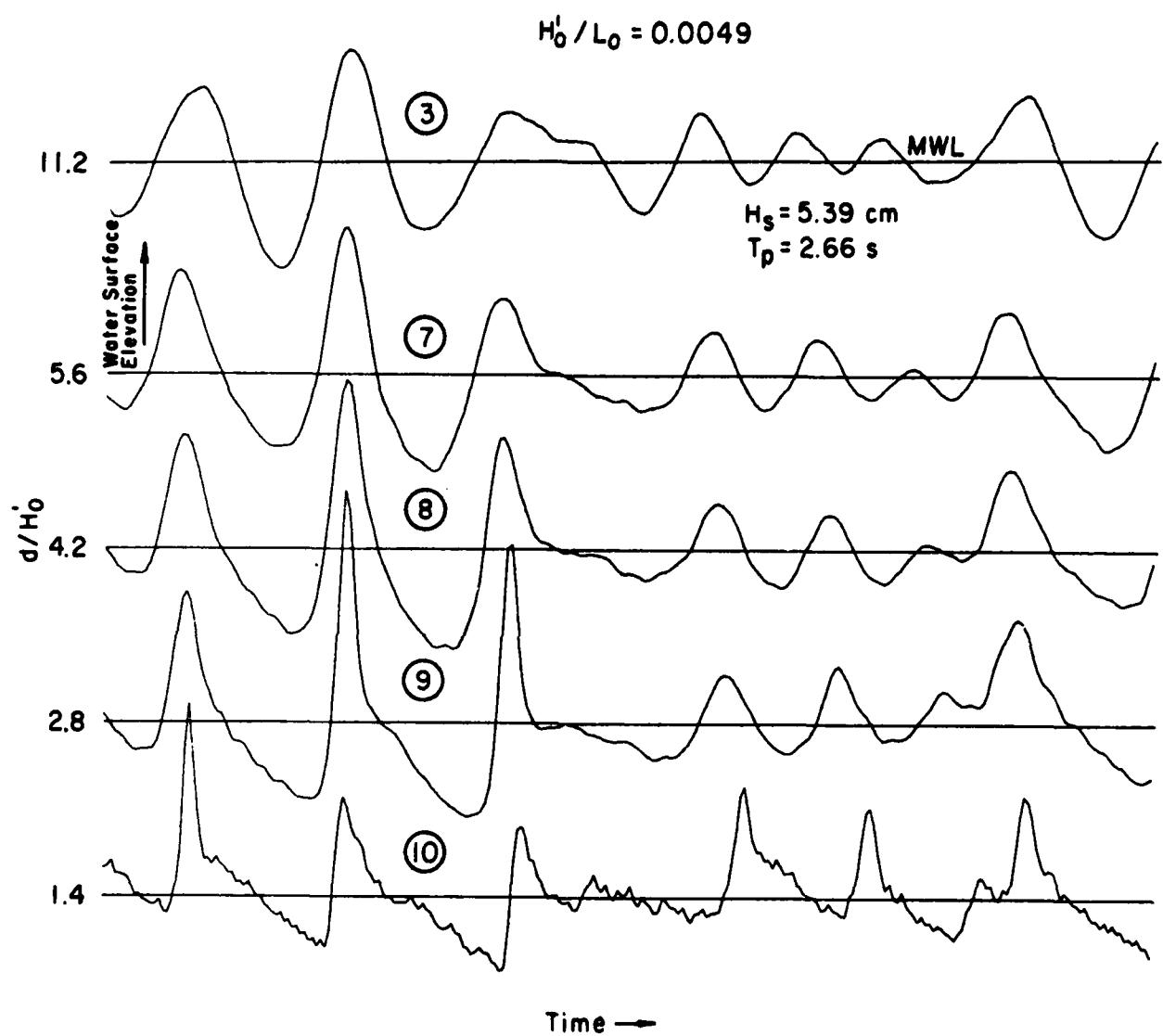


Figure 18. Sample of water level time histories for irregular waves at selected locations along a 1 on 30 slope.

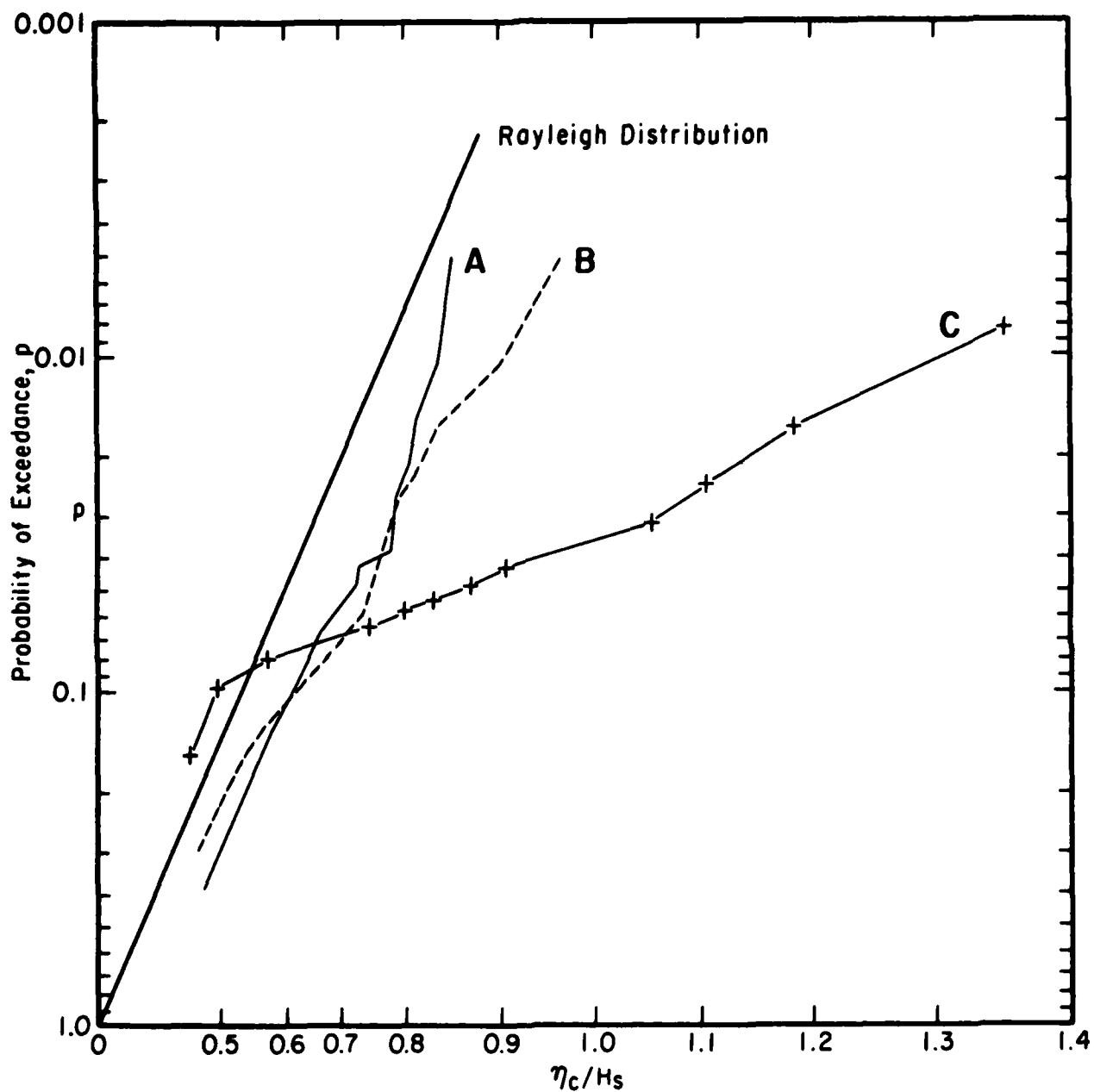


Figure 19. Sample of irregular wave crest elevation distributions  
(from wave records shown in Fig. 16).

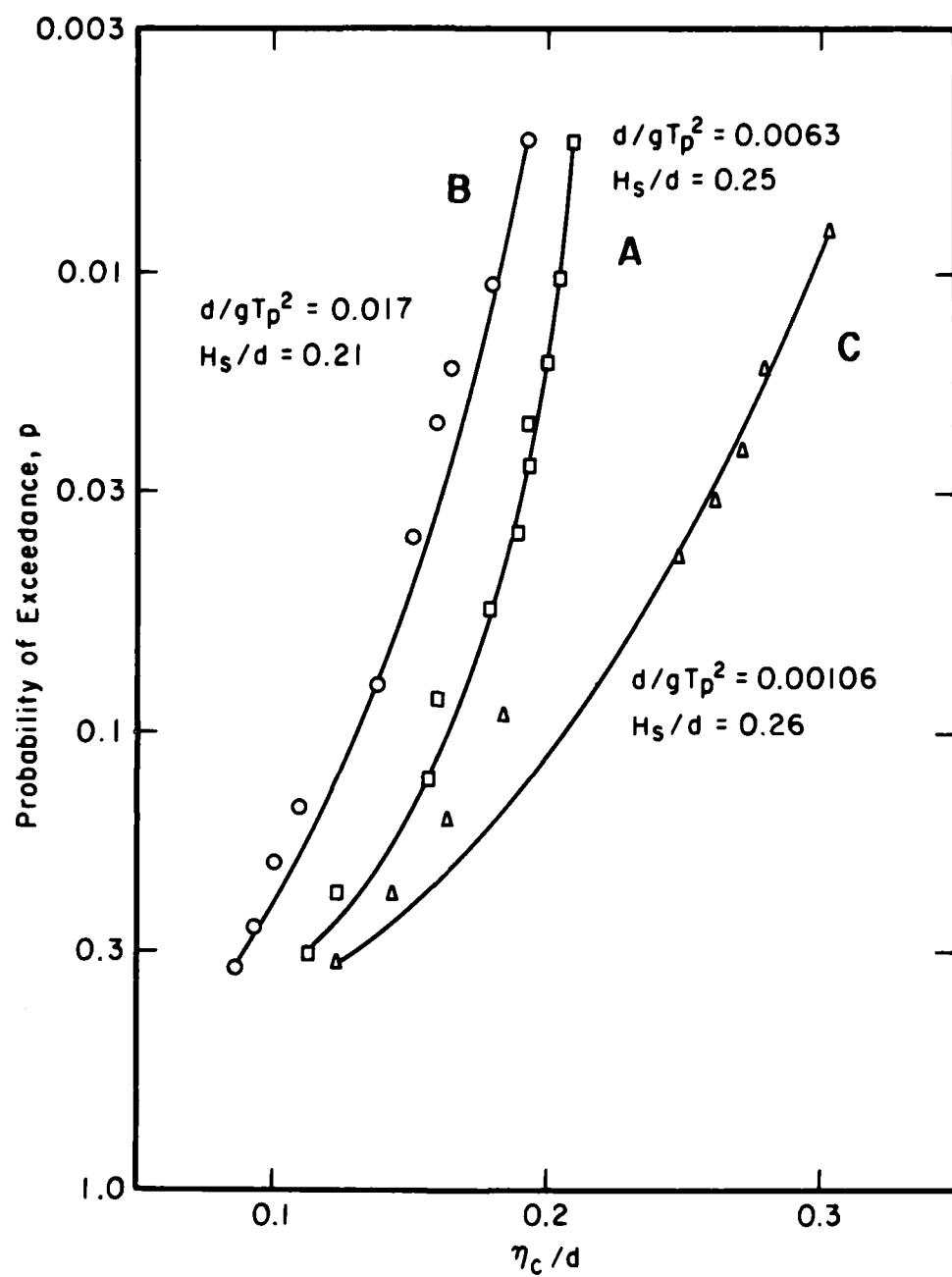


Figure 20. Sample of wave crest distributions (from wave records shown in Fig. 16).

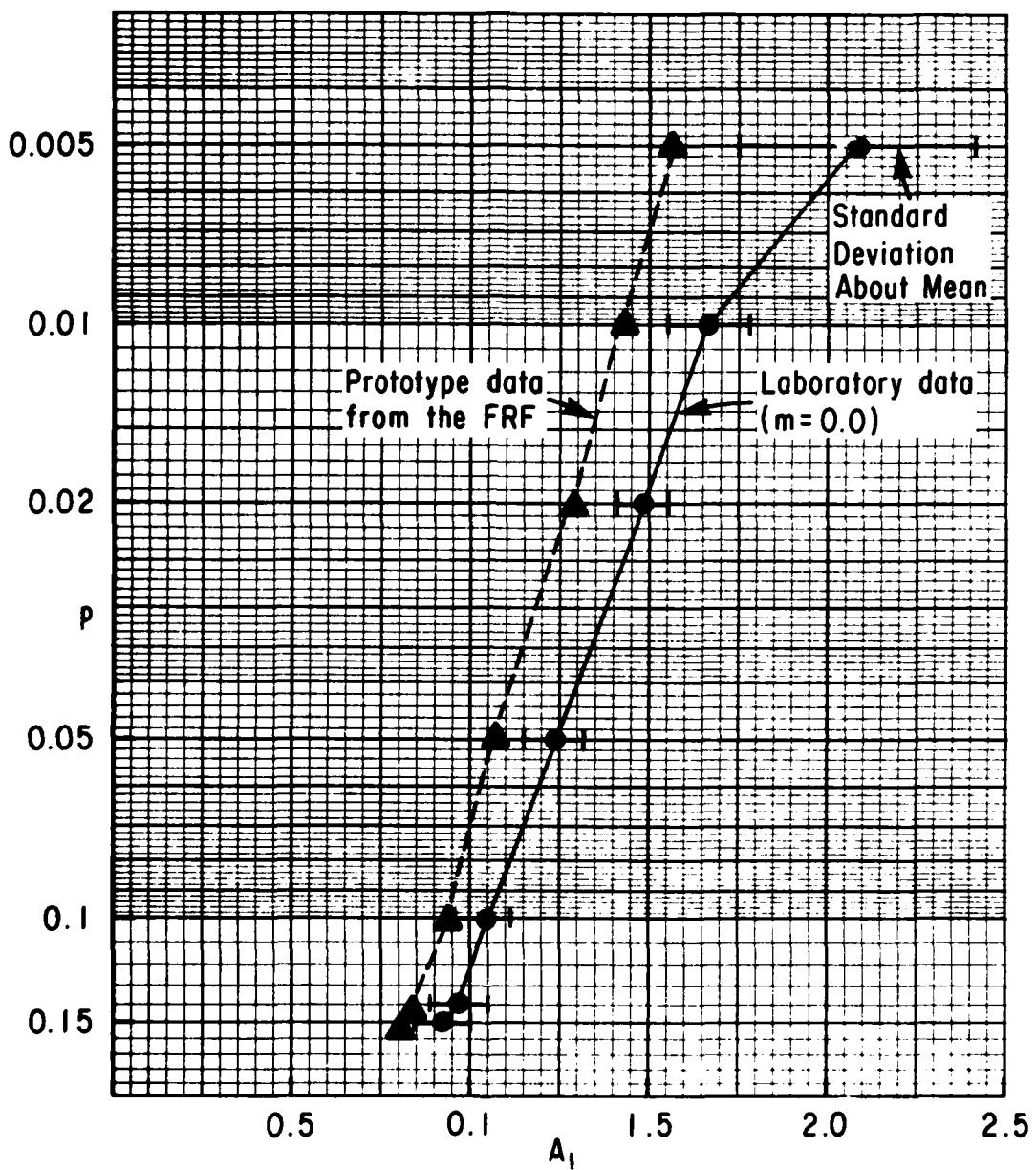


Figure 21. Values of  $A_1^*$  for probability of exceedance for irregular waves.

Table 8. Values of  $A_1^*$  for levels of probability of exceedance for irregular waves ( $m = 0.0$ ), derived from 122 laboratory experiments.

Probability of exceedance, p	$A_1^*$	Std. dev. of $A_1^*$	Coefficient of variation (pct)
0.150	0.929	0.082	8.8
0.135	0.960	0.083	8.6
0.100	1.048	0.082	7.8
0.050	1.240	0.085	6.9
0.020	1.488	0.152	10.2
0.010	1.669	0.209	12.5
0.005	2.119	0.33	15.6

Table 9. Values of  $A_1^*$  for levels of probability of exceedance for irregular waves on a 1 on 30 laboratory slope ( $m = 0.033$ ), derived from 192 laboratory measurements.

Probability of exceedance, p	$A_1^*$	Std. dev. of $A_1^*$	Coefficient of variation (pct)
0.150	0.919	0.058	6.3
0.135	0.948	0.061	6.4
0.100	1.029	0.075	7.3
0.050	1.20	0.099	8.2
0.020	1.46	0.176	12.1
0.010	1.70	0.285	16.8

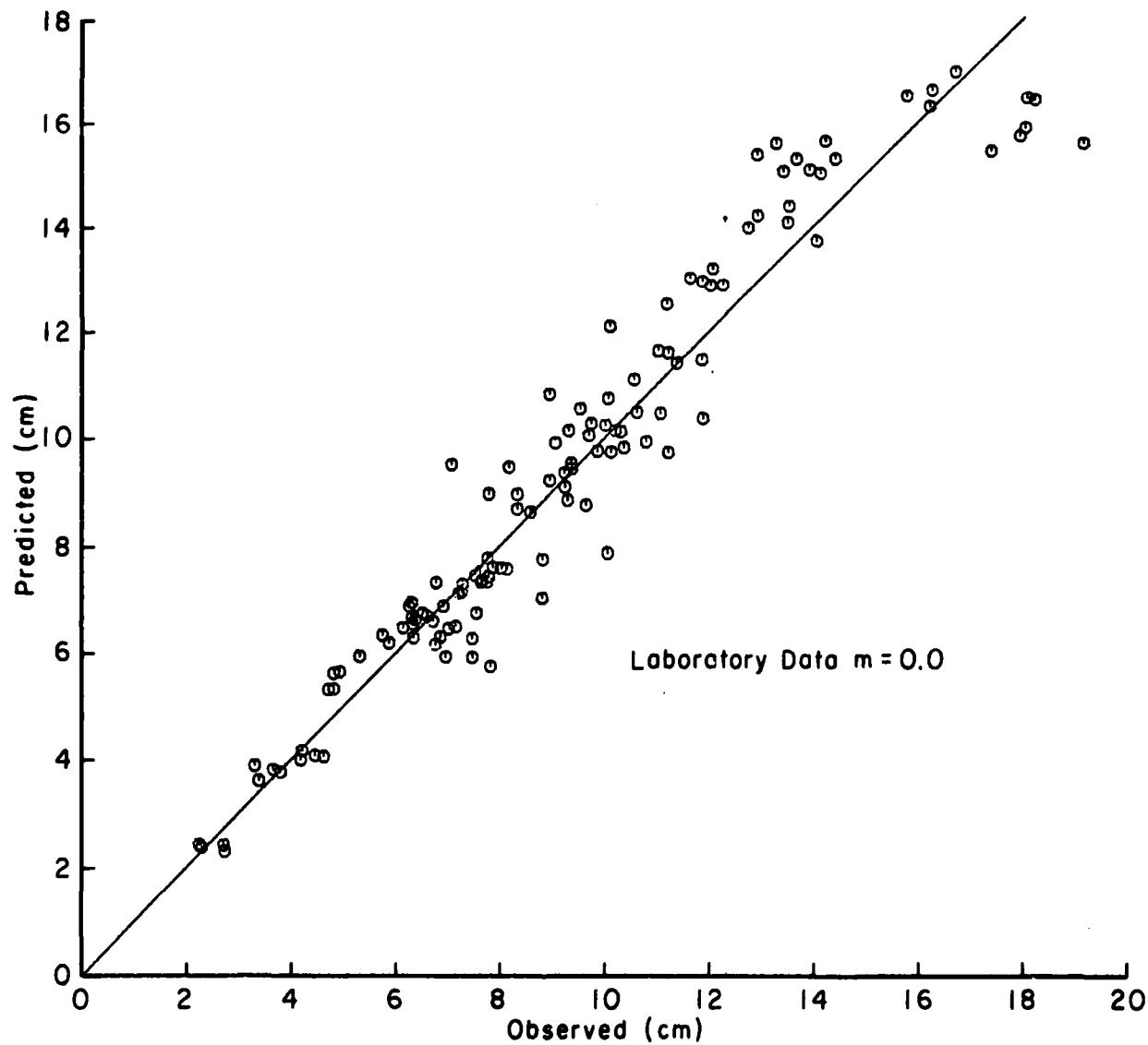


Figure 22. Observed and predicted crest elevations exceeded by 2 percent of the wave crests.

The wave crest elevation of irregular waves on a 1 on 30 slope may also be described in terms of deepwater wave characteristics. An examination of the data indicates that deepwater wave steepness and local stillwater depth control the magnitude of crest elevation at a given probability level. For example, Figure 23 presents the crest elevation exceeded by 13.5 percent of the wave crests for the deepwater wave steepness range  $0.0035 < H_o'/L_o < 0.0075$  for  $m = 0.0333$  (a 1 on 30 slope with the experimental setup shown in Fig. 6). The irregular wave data show a consistent trend with crest elevation increasing in the shoaling region and decreasing after breaking.

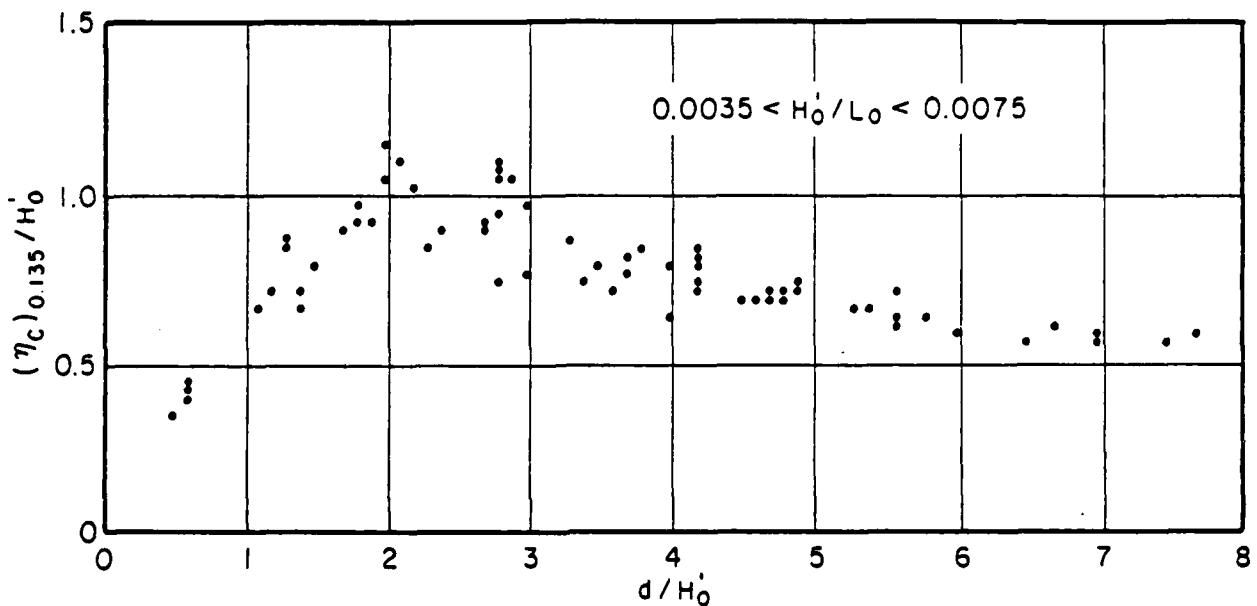


Figure 23. Wave crest elevation exceeded by 13.5 percent of the wave crests.

## 2. Crest Elevation Prediction Aids.

Irregular wave crest elevations may be predicted in terms of local wave conditions using equation (8). Figure 24 gives crest elevation exceeded by 2 percent of the wave crests ( $p = 0.02$ ) normalized by the water depth. Curves of constant  $H_s/d$  are plotted with  $d/(gT^2)$  on the abscissa. As a first estimate the upper limit of wave height is assumed to be given by equation (5) and the crest elevation of this condition is shown as a dashline. Figure 25 presents crest elevations exceeded by 13.5 percent of the wave crests. Figures 24 and 25 may be used for waves traveling over a flat bottom and for plane slopes as steep as 1 on 30.

Crest elevations are presented in terms of deepwater wave characteristics for incident waves normal to a 1 on 30 slope for the 2- and 13.5-percent probability of exceedance levels in Figure 26. These curves were determined using the laboratory data presented in Appendix D. A dashline is used to indicate a region of uncertainty.

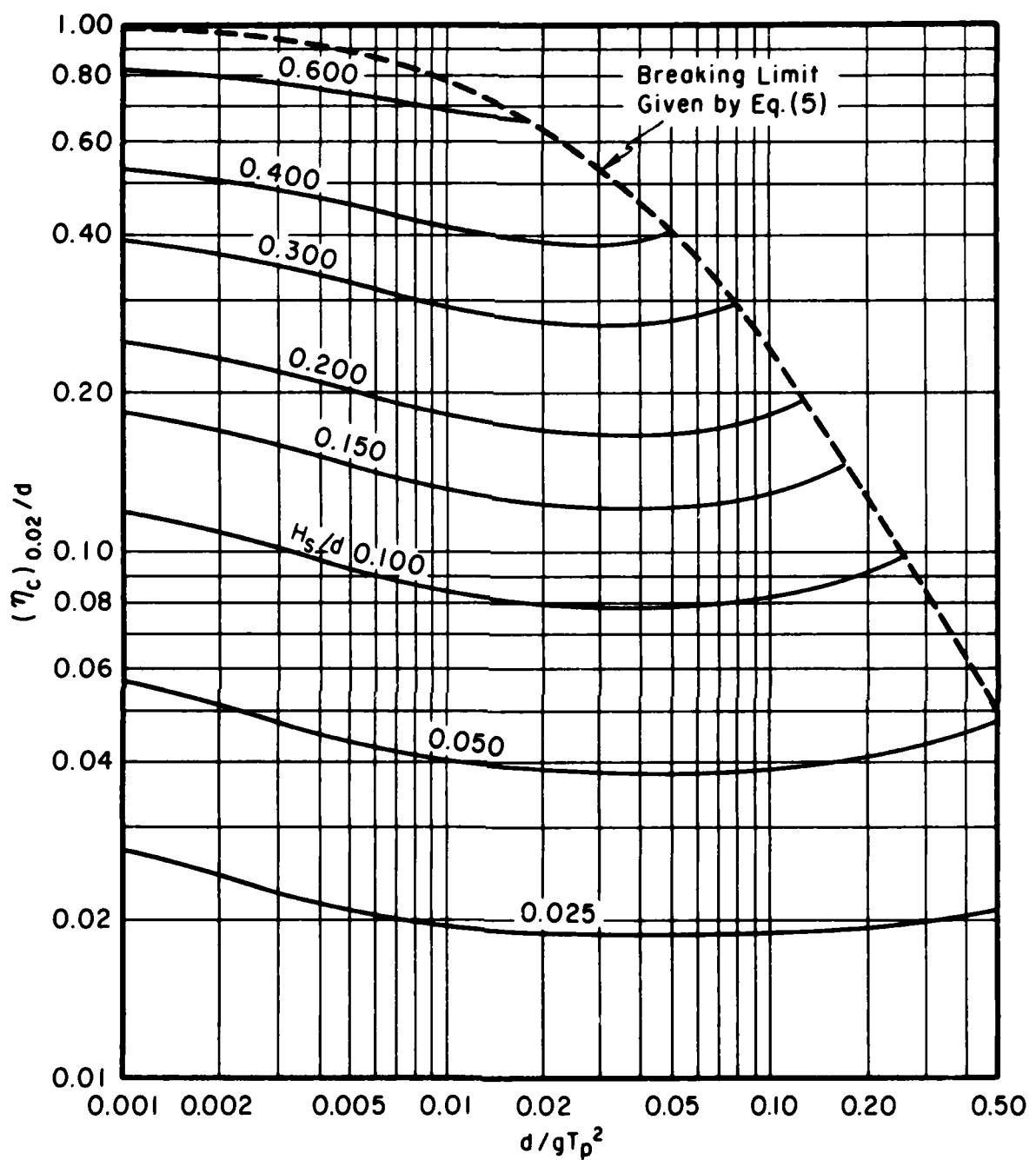


Figure 24. Crest elevation at the 2-percent probability level of exceedance for irregular waves.

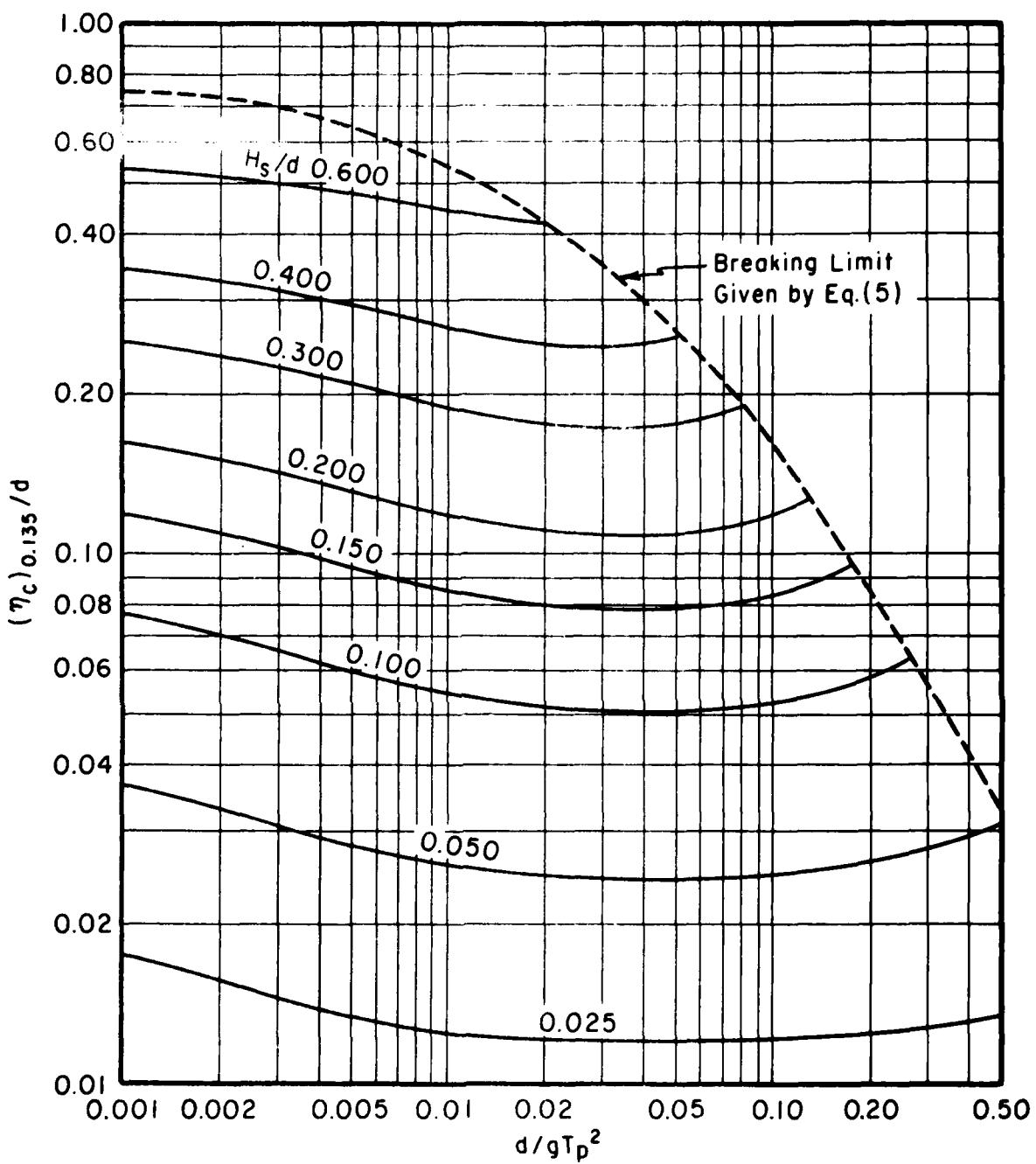


Figure 25. Crest elevation at the 13.5-percent probability level of exceedance for irregular waves.

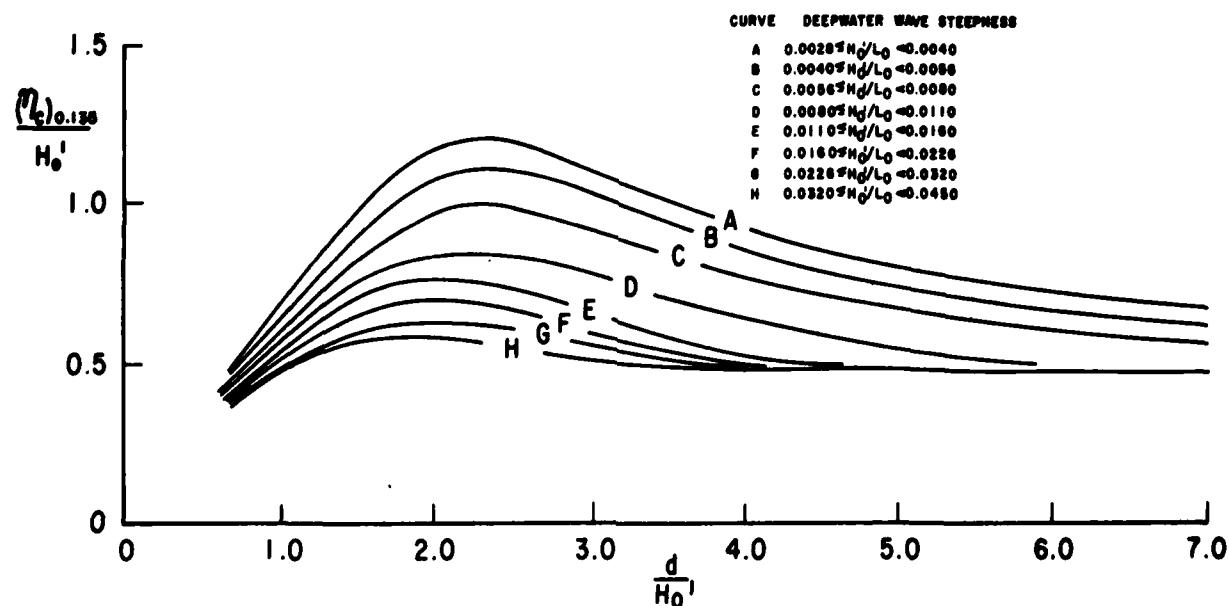
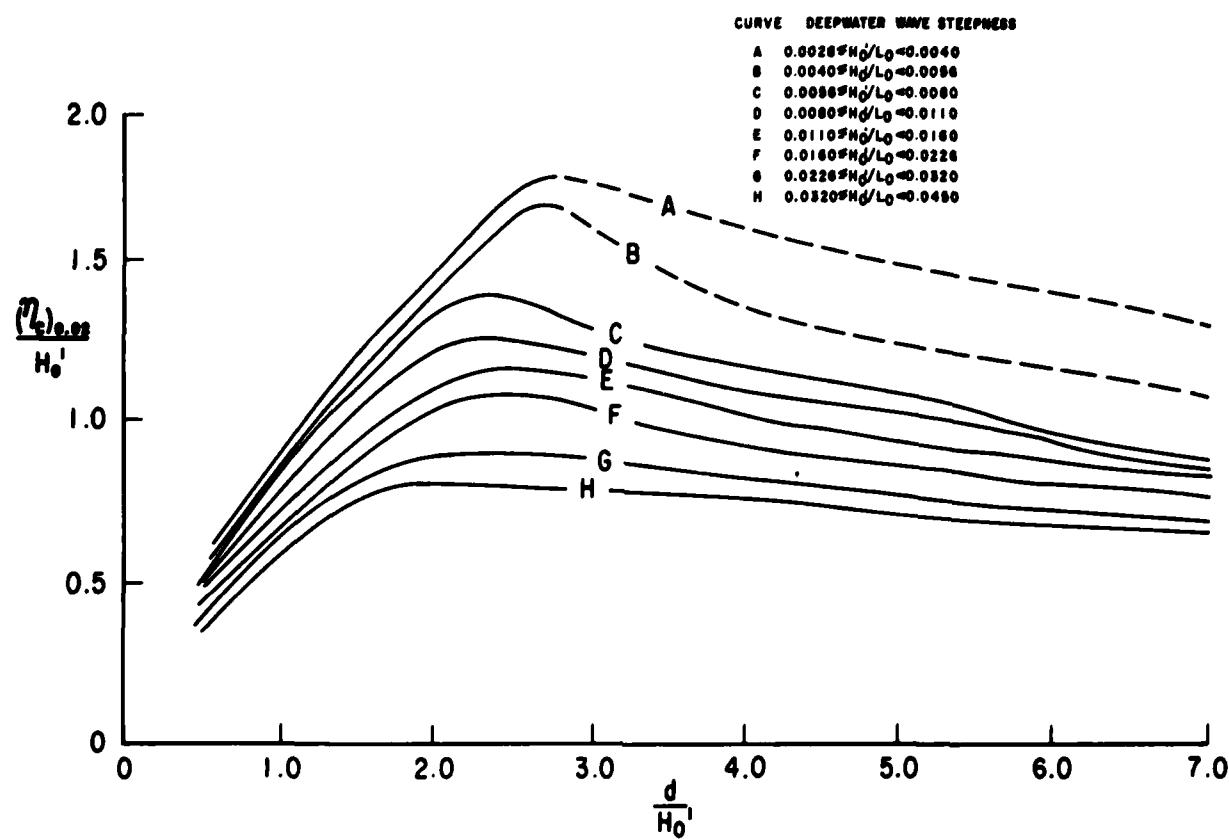


Figure 26. Wave crest elevations along a 1 on 30 slope for 2- and 13.5-percent probability of exceedance levels.

### 3. The Duration of Irregular Wave Crests.

The larger waves in an irregular wave train have the greatest nonlinearity, so the duration of the crests of these waves tends to be small relative to wave period. Figure 27 illustrates this trend by presenting the mean relative crest duration for various probability levels of exceedance for waves traveling over a flat bottom. For example, 5 percent of the largest crests have a mean dimensionless crest duration  $T_c/T = 0.406$ , while the average value for all of the wave crests (at the 100-percent level) have  $T_c/T = 0.473$ .

## VI. COMPARISON OF RESULTS FOR PROTOTYPE DATA

The techniques presented in the previous section are to be used for the future prediction of wave characteristics in field situation. In this section, field data collected at the FRF in Duck, North Carolina, are analyzed in the same manner as the laboratory data previously described and comparisons are presented to demonstrate the applicability of the predictive techniques.

### 1. Field Conditions.

Four Waverider buoys were used to obtain the prototype data in an area where the bottom contours are essentially straight and parallel. Table 10 gives the locations of the gages and the mean water depths. The data were collected in the third week of October 1980 during a severe storm that produced wave heights exceeding 4 meters. The data set has widely varying significant wave height and period of peak energy density combinations resulting as the storm moved into and through the study area. Table 10 summarizes the range of wave conditions used in this report.

### 2. Prototype Verification.

The field data were analyzed in the same manner as the laboratory data used in formulating the predictive parameters. The observed crest elevations were compared with those predicted by equation (8), using values of  $A_1^*$  for  $m = 0.0$  (Table 8), since the bottom slope was very gentle. Table 11 shows the results of this comparison for decreasing exceedance probabilities. It should be noted that the predicted crest elevations are slightly conservative, with the coefficient of variation approximately 8.5 percent. Figures 28 and 29 illustrate the predictive ability of equation (8); the diagonal line indicates perfect agreement. The scatter decreases for higher probability of exceedance levels, in part, because the data for higher probability levels are more statistically reliable. These figures also show that the scatter from the prediction line increases slightly as  $H_s/d$  increases. This increase in conservatism with increasing wave height-to-depth ratio may be due to the fact that the waves are interacting with a bottom that is slightly sloping and comprised of a porous sandy material (rather than a rigid floor used in the laboratory experiments). The directional characteristics of the prototype waves could also be influencing these results because a spectrum of wave directions was present, even though the primary wave direction was along the line of the Waveriders.

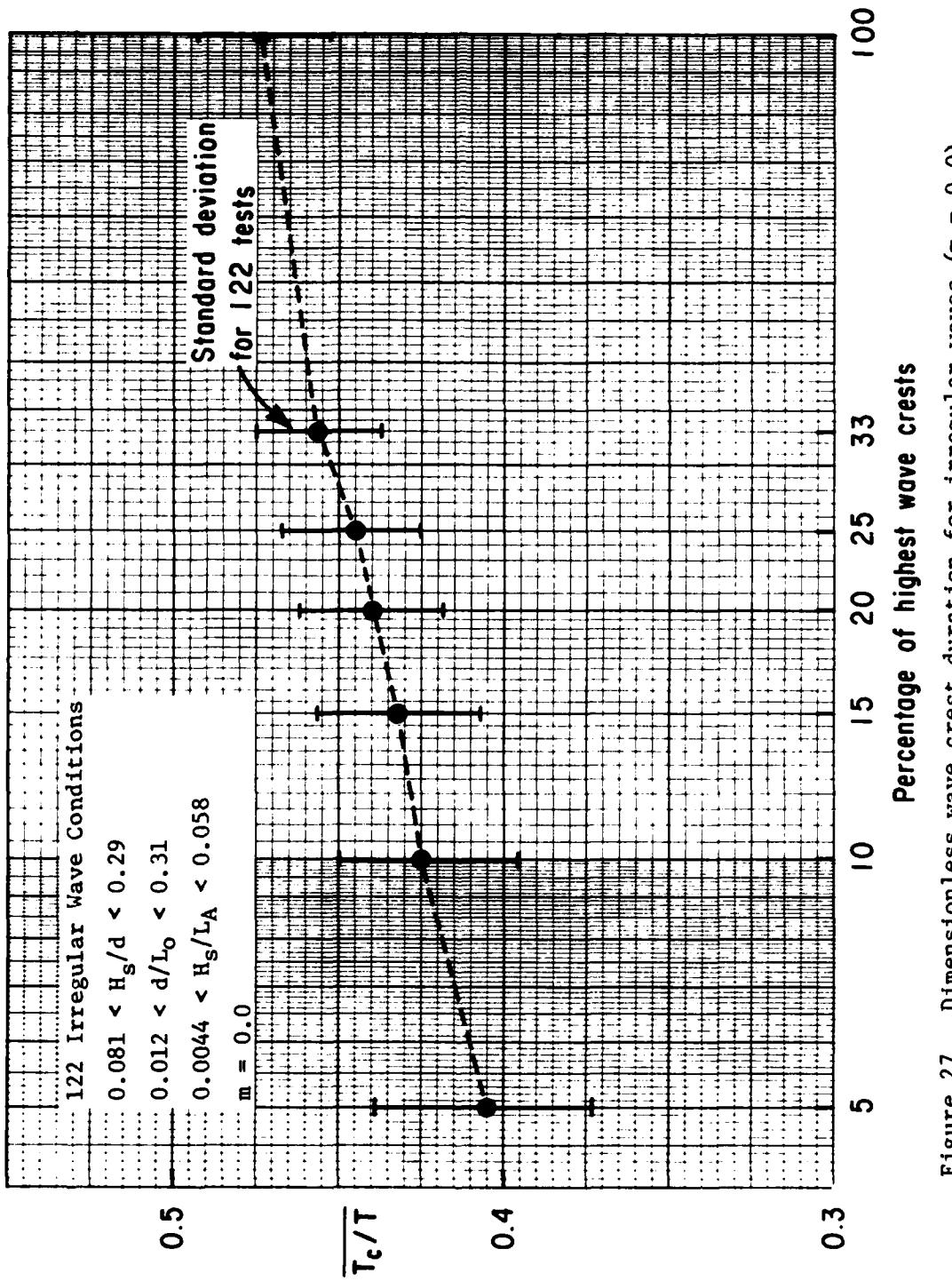


Figure 27. Dimensionless wave crest duration for irregular waves ( $m = 0.0$ ).

Table 10. Summary of prototype wave conditions.

Waverider distance from shore (km)	Water depth (m)	$H_s$ (m)		$T_p$ (s)		Wave steepness	
		Max.	Min.	Max.	Min.	Max.	Min.
0.5	7	3.6	0.5	14	3	0.0641	0.0016
3.0	17	3.8	0.5	14	3	0.0570	0.0016
6.0	18	4.3	0.4	14	3	0.0570	0.0013
12.0	25	4.4	0.4	14	3	0.0570	0.0013

Table 11. Values of crest heights observed and predicted for waves at Duck, North Carolina.<sup>1</sup>

Probability of exceedance	Mean ratio of observed and predicted crest elevations <sup>2</sup>	$A_1^*$ Observed	Coefficient of variation (pct)
0.150	0.912	0.847	8.5
0.135	0.909	0.873	8.3
0.100	0.902	0.945	8.6
0.050	0.877	1.087	8.7
0.020	0.861	1.281	8.6
0.010	0.848	1.415	9.0
0.005	0.729	1.545	11.6

<sup>1</sup>Values derived from 100 20-minute wave records taken in October 1980.

<sup>2</sup>Values of  $A_1^*$  from Table 8 used to make predictions.

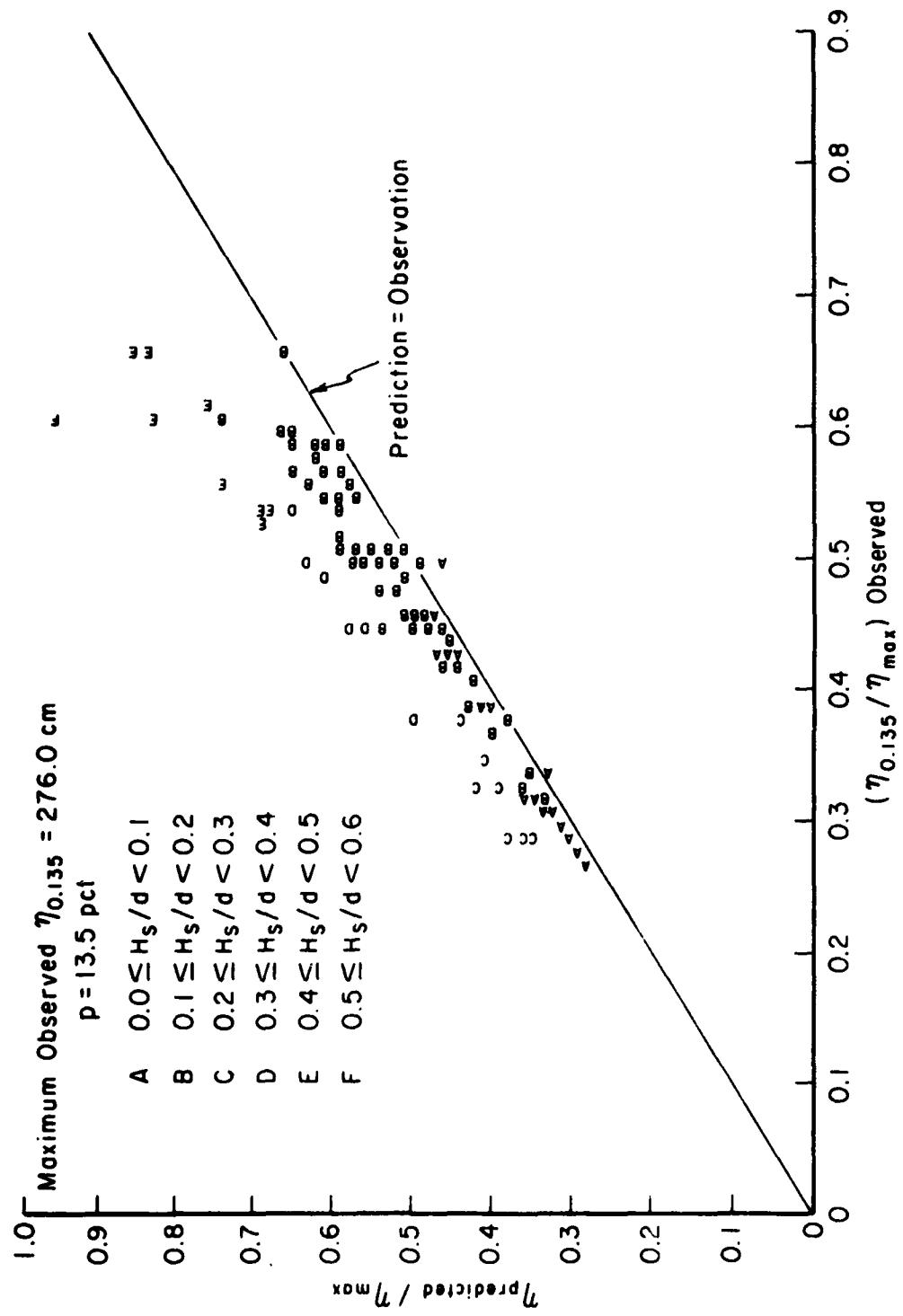


Figure 28. Observed versus predicted crest elevations ( $p = 0.135$ ).

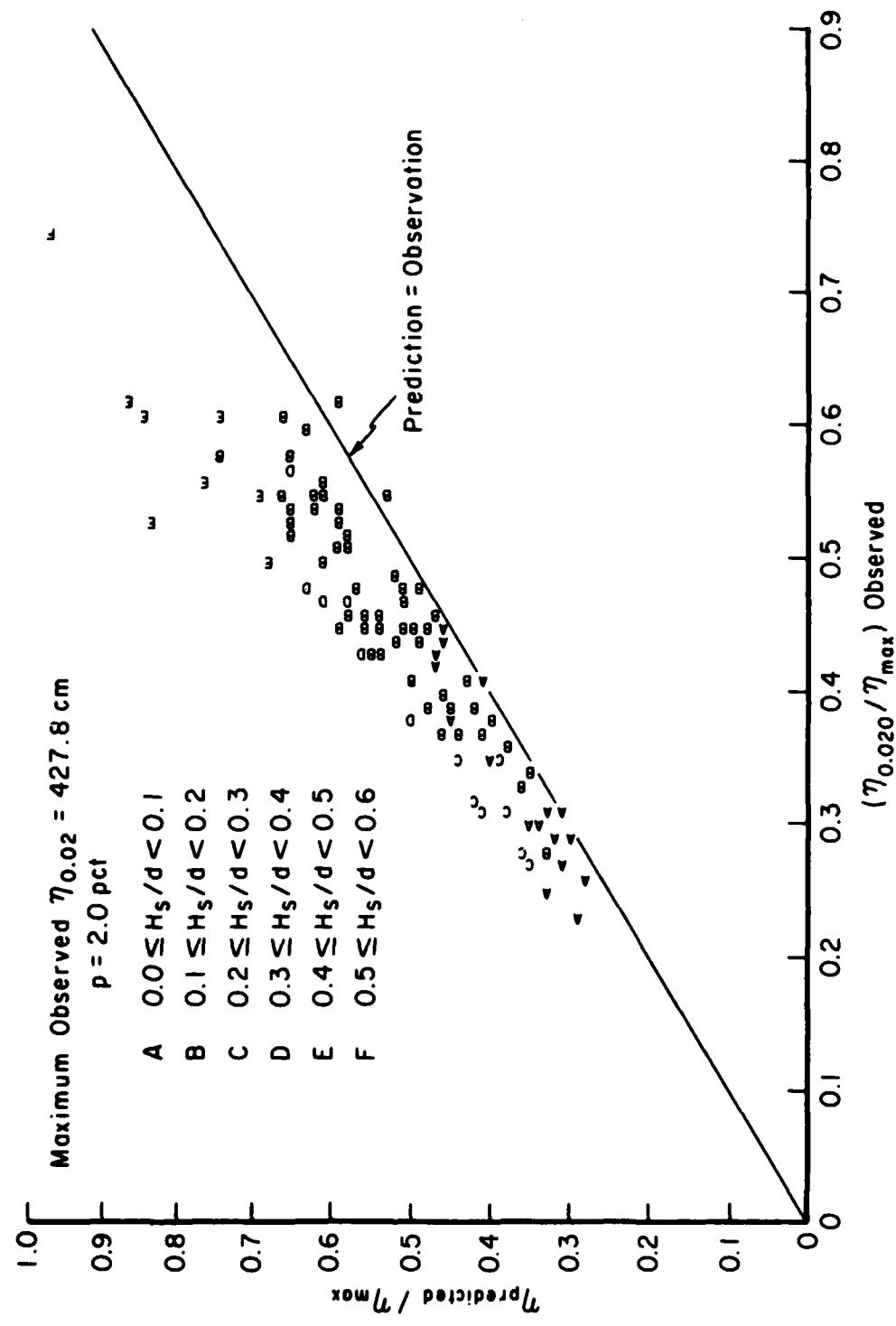


Figure 29. Observed versus predicted crest elevations ( $p = 0.020$ ).

## VII. SUMMARY AND CONCLUSIONS

The stream-function tables of Dean (1974) are used to develop empirical formulas for estimating monochromatic wave crest elevations and durations. The resulting formulas are calibrated with laboratory data. The method for predicting crest elevations was found to work well for waves traveling over a flat bottom or at the breaker point on plane smooth slopes. The equation for predicting crest elevations was calibrated for irregular wave conditions traveling over a flat bottom and shoaling on a 1 on 30 plane slope. The irregular wave data taken with the 1 on 30 slope were also expressed in terms of deepwater conditions and it was found that deepwater steepness and local water depth could be used to predict crest elevations.

Prediction equations developed from two-dimensional irregular laboratory waves were found to be conservative in predicting crest elevations when compared to field measurements taken at CERC's Field Research Facility, Duck, North Carolina. The conservatism is slight and increases with increasing  $H_s/d$ .

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**APPENDIX A**

**MONOCHROMATIC TESTS RESULTS**

BREAKING MONOCHROMATIC WAVE DATA  
 COT(M)= 5  
 DATA BY SINGAMSETTI AND WIND(1980)

HB(CM)	T(SEC)	DB(CM)	DB/LO	HB/LO	ETA C/H	TC/T
11.7	1.55	12.7	0.034	0.0311	0.568	0.411
19.3	1.55	15.6	0.041	0.0513	0.625	0.422
15.5	1.28	12.2	0.048	0.0606	0.663	0.408
9.6	1.28	10.2	0.040	0.0376	0.608	0.453
9.5	1.55	7.2	0.019	0.0252	0.654	0.412
10.5	1.04	8.0	0.048	0.0628	0.653	0.373
15.9	1.28	13.0	0.051	0.0621	0.645	0.407
9.0	1.04	7.7	0.046	0.0538	0.639	0.408
11.7	1.04	9.6	0.057	0.0696	0.675	0.419
18.3	1.72	19.3	0.042	0.0398	0.581	0.392
12.4	1.72	11.3	0.024	0.0270	0.628	0.424
16.2	1.28	13.6	0.053	0.0631	0.648	0.397
12.1	1.28	10.1	0.039	0.0471	0.674	0.440
8.9	1.28	8.0	0.031	0.0347	0.657	0.445
9.2	1.55	9.6	0.025	0.0246	0.619	0.402
8.7	1.04	8.0	0.047	0.0518	0.638	0.398
15.0	1.28	13.0	0.051	0.0582	0.615	0.370
7.7	1.04	9.7	0.058	0.0458	0.635	0.436
11.7	1.04	9.6	0.057	0.0699	0.638	0.400
18.3	1.72	19.2	0.042	0.0398	0.610	0.464
12.3	1.72	17.5	0.038	0.0268	0.586	0.498

SUM, SUM1, NO= 0.6720 1.0858 21  
 SD, SD1 = 0.036970.09162  
 TC/T N, AVERAGE SD= 21 1.8236 0.3165

BREAKING MONOCHROMATIC WAVE DATA  
 COT(M)= 10  
 DATA BY SINGAMSETTI AND WIND(1980)

HB(CM)	T(SEC)	DB(CM)	DB/LO	HB/LO	ETA C/H	TC/T
13.7	1.55	12.7	0.034	0.0366	0.673	0.422
16.9	1.55	19.9	0.053	0.0451	0.740	0.292
11.7	1.28	12.8	0.050	0.0460	0.745	0.350
8.6	1.28	10.6	0.042	0.0336	0.727	0.367
11.1	1.55	10.1	0.027	0.0296	0.722	0.430
7.1	1.03	11.2	0.067	0.0545	0.739	0.326
13.5	1.28	14.4	0.056	0.0526	0.725	0.366
7.2	1.03	9.6	0.058	0.0434	0.678	0.342
10.5	1.03	12.7	0.076	0.0631	0.738	0.342
16.9	1.72	18.4	0.040	0.0367	0.729	0.323
14.0	1.72	11.4	0.025	0.0305	0.741	0.419
15.0	1.28	18.2	0.071	0.0587	0.755	0.331
14.1	1.55	12.8	0.034	0.0376	0.674	0.431
17.0	1.55	18.7	0.050	0.0454	0.000	0.000
11.8	1.28	13.0	0.051	0.0462	0.741	0.348
10.1	1.28	8.8	0.034	0.0395	0.728	0.306
11.9	1.55	10.5	0.028	0.0318	0.000	0.000
8.6	1.03	8.8	0.053	0.0520	0.743	0.339
14.3	1.28	17.1	0.067	0.0559	0.748	0.345
7.7	1.03	7.7	0.047	0.0468	0.719	0.361
11.2	1.03	13.3	0.080	0.0677	0.686	0.390
17.4	1.71	18.2	0.040	0.0383	0.752	0.373
14.0	1.71	12.2	0.027	0.0307	0.732	0.332
15.6	1.28	18.6	0.073	0.0610	0.734	0.314

SUM, SUM1, NO= 0.8181 1.2420 22  
 SD, SD1 = 0.075200.08541  
 TC/T N, AVERAGE SD= 22 1.4007 0.4204

BREAKING MONOCHROMATIC WAVE DATA  
 COT(M)= 20  
 DATA BY SINGAMSETTI AND WIND(1980)

HB(CM)	T(SEC)	DB(CM)	DB/LO	HB/LO	ETA C/H	TC/T
14.0	1.55	17.0	0.045	0.0374	0.751	0.307
17.4	1.55	20.0	0.053	0.0464	0.745	0.309
11.4	1.28	12.5	0.049	0.0449	0.770	0.273
9.6	1.28	10.0	0.039	0.0379	0.782	0.274
10.5	1.55	10.1	0.027	0.0281	0.779	0.295
8.8	1.04	10.7	0.064	0.0521	0.756	0.325
13.5	1.28	17.2	0.068	0.0530	0.767	0.333
7.8	1.04	9.2	0.055	0.0467	0.778	0.330
10.0	1.04	12.8	0.076	0.0598	0.752	0.360
17.6	1.73	20.0	0.043	0.0377	0.756	0.322
13.2	1.73	12.4	0.027	0.0284	0.792	0.269
16.2	1.28	20.2	0.079	0.0638	0.741	0.331
14.2	1.55	15.2	0.040	0.0379	0.789	0.289
18.0	1.55	20.7	0.055	0.0482	0.758	0.293
11.8	1.28	13.4	0.053	0.0465	0.768	0.300
10.0	1.28	10.8	0.043	0.0394	0.804	0.277
10.5	1.55	10.2	0.027	0.0281	0.772	0.290
9.1	1.04	11.0	0.066	0.0544	0.747	0.298
13.2	1.28	15.7	0.061	0.0520	0.766	0.320
7.7	1.04	8.9	0.053	0.0458	0.751	0.314
10.1	1.04	12.6	0.075	0.0601	0.770	0.336
17.1	1.73	17.9	0.038	0.0366	0.805	0.286
13.1	1.73	12.5	0.027	0.0282	0.774	0.265
16.5	1.28	20.8	0.082	0.0648	0.784	0.328

SUM, SUM1, ND= 0.8806 1.3052 24  
 SD, SD1 =0.032740.08863  
 TC/T N, AVERAGE SD= 24 1.1256 0.1118

BREAKING MONOCHROMATIC WAVE DATA  
 COT(M)= 40  
 DATA BY SINGAMSETTI AND WIND(1980)

HB(CM)	T(SEC)	DB(CM)	DB/LO	HB/LO	ETA C/H	TC/T
13.6	1.55	14.5	0.039	0.0363	0.805	0.292
17.0	1.55	20.2	0.054	0.0453	0.783	0.302
11.9	1.28	13.9	0.055	0.0468	0.793	0.301
9.2	1.28	11.0	0.043	0.0364	0.779	0.240
11.1	1.55	11.7	0.031	0.0297	0.822	0.259
9.6	1.04	11.6	0.069	0.0571	0.751	0.328
7.9	1.04	9.2	0.055	0.0470	0.766	0.316
15.9	1.28	21.9	0.086	0.0625	0.758	0.311
13.6	1.28	15.0	0.059	0.0537	0.747	0.356
11.0	1.04	15.2	0.090	0.0654	0.750	0.366
12.4	1.04	16.8	0.100	0.0738	0.767	0.335
13.3	1.04	20.4	0.122	0.0791	0.718	0.349
13.1	1.72	14.9	0.032	0.0285	0.824	0.266
13.6	1.55	14.5	0.039	0.0363	0.798	0.277
16.8	1.55	21.2	0.056	0.0450	0.775	0.302
11.8	1.28	12.9	0.051	0.0464	0.741	0.354
9.1	1.28	10.1	0.040	0.0360	0.752	0.286
11.0	1.55	11.7	0.031	0.0295	0.823	0.274
9.5	1.04	12.6	0.075	0.0565	0.761	0.324
7.7	1.04	9.2	0.055	0.0461	0.790	0.301
15.9	1.28	19.4	0.076	0.0627	0.722	0.375
13.2	1.28	16.4	0.065	0.0519	0.772	0.309
10.9	1.04	14.2	0.084	0.0648	0.697	0.316
12.5	1.04	15.3	0.097	0.0744	0.759	0.328
13.7	1.04	15.4	0.116	0.0815	0.738	0.337
14.0	1.72	15.4	0.033	0.0303	0.822	0.270

MONOCHROMATIC WAVE TEST (M=0, 0)  
SINUSOIDAL BLADE MOTION

H(CM)	T(SEC)	D(CM)	D/LD	H/LD	ETA C/H	TC/T
5. 47	1. 79	50.	0. 100	0. 0110	0. 539	
9. 07	1. 79	50.	0. 100	0. 0181	0. 557	
12. 66	1. 79	50.	0. 100	0. 0253	0. 587	
16. 27	1. 79	50.	0. 100	0. 0326	0. 600	
19. 88	1. 79	50.	0. 100	0. 0394	0. 601	
3. 33	0. 96	50.	0. 348	0. 0232	0. 517	
6. 57	0. 96	50.	0. 348	0. 0457	0. 534	
8. 11	0. 96	50.	0. 348	0. 0564	0. 545	
9. 63	0. 96	50.	0. 348	0. 0670	0. 540	
10. 96	0. 96	50.	0. 348	0. 0762	0. 554	
12. 23	0. 96	50.	0. 348	0. 0851	0. 570	
13. 44	0. 96	50.	0. 348	0. 0935	0. 576	
14. 65	0. 96	50.	0. 348	0. 1019	0. 579	
6. 05	3. 03	50.	0. 035	0. 0042	0. 626	
7. 41	3. 03	50.	0. 035	0. 0052	0. 647	
8. 85	3. 03	50.	0. 035	0. 0062	0. 665	
10. 34	3. 03	50.	0. 035	0. 0072	0. 680	
11. 83	3. 03	50.	0. 035	0. 0083	0. 694	
13. 31	3. 03	50.	0. 035	0. 0093	0. 706	
14. 88	3. 03	50.	0. 035	0. 0104	0. 716	
16. 53	3. 03	50.	0. 035	0. 0115	0. 728	
18. 14	3. 03	50.	0. 035	0. 0127	0. 734	
3. 36	0. 80	50.	0. 501	0. 0337	0. 521	
4. 99	0. 80	50.	0. 501	0. 0500	0. 546	
6. 65	0. 80	50.	0. 501	0. 0666	0. 555	
8. 06	0. 80	50.	0. 501	0. 0807	0. 565	
9. 34	0. 80	50.	0. 501	0. 0936	0. 567	
10. 51	0. 80	50.	0. 501	0. 1052	0. 580	
5. 31	2. 53	50.	0. 050	0. 0053	0. 590	
6. 71	2. 53	50.	0. 050	0. 0067	0. 602	
8. 16	2. 53	50.	0. 050	0. 0082	0. 613	
10. 40	2. 53	50.	0. 050	0. 0104	0. 625	
12. 67	2. 53	50.	0. 050	0. 0127	0. 629	
14. 20	2. 53	50.	0. 050	0. 0142	0. 637	
16. 57	2. 53	50.	0. 050	0. 0166	0. 639	
19. 00	2. 53	50.	0. 050	0. 0190	0. 645	
5. 88	4. 00	50.	0. 020	0. 0024	0. 688	
7. 63	4. 00	50.	0. 020	0. 0031	0. 609	
9. 44	4. 00	50.	0. 020	0. 0038	0. 636	
11. 29	4. 00	50.	0. 020	0. 0045	0. 657	
13. 23	4. 00	50.	0. 020	0. 0053	0. 673	
12. 38	4. 00	50.	0. 020	0. 0050	0. 683	
11. 32	4. 00	50.	0. 020	0. 0045	0. 697	
12. 44	4. 00	50.	0. 020	0. 0050	0. 693	
8. 26	1. 27	50.	0. 199	0. 0328	0. 545	
13. 41	1. 27	50.	0. 199	0. 0533	0. 580	
17. 00	1. 27	50.	0. 199	0. 0676	0. 562	
20. 61	1. 27	50.	0. 199	0. 0319	0. 621	
6. 89	1. 27	50.	0. 199	0. 0274	0. 538	
5. 61	5. 66	50.	0. 010	0. 0011	0. 527	

H(CM)	T(SEC)	D(CM)	D/LD	H/LD	ETA	C/H	TC/T
6. 27	5. 66	50.	0. 010	0. 0013	0. 539		
7. 27	5. 66	50.	0. 010	0. 0015	0. 537		
8. 34	5. 66	50.	0. 010	0. 0017	0. 547		
9. 45	5. 66	50.	0. 010	0. 0019	0. 569		
11. 90	5. 66	50.	0. 010	0. 0024	0. 599		
13. 77	5. 66	50.	0. 010	0. 0028	0. 616		
15. 60	5. 66	50.	0. 010	0. 0031	0. 631		
17. 11	5. 66	50.	0. 010	0. 0034	0. 641		
1. 64	1. 60	60.	0. 150	0. 0041	0. 520	0. 481	
7. 16	1. 60	60.	0. 150	0. 0179	0. 543	0. 475	
10. 90	1. 60	60.	0. 150	0. 0273	0. 551	0. 476	
15. 37	1. 60	60.	0. 150	0. 0385	0. 585	0. 449	
20. 05	1. 60	60.	0. 150	0. 0502	0. 592	0. 448	
1. 81	1. 75	60.	0. 126	0. 0038	0. 511	0. 494	
6. 28	1. 75	60.	0. 126	0. 0131	0. 542	0. 472	
8. 57	1. 75	60.	0. 126	0. 0179	0. 563	0. 460	
11. 38	1. 75	60.	0. 126	0. 0238	0. 581	0. 454	
16. 24	1. 75	60.	0. 126	0. 0340	0. 603	0. 432	
2. 80	1. 96	60.	0. 100	0. 0047	0. 524	0. 489	
10. 52	1. 96	60.	0. 100	0. 0176	0. 599	0. 438	
14. 93	1. 96	60.	0. 100	0. 0249	0. 629	0. 411	
20. 67	1. 96	60.	0. 100	0. 0345	0. 673	0. 395	
26. 08	1. 96	60.	0. 100	0. 0435	0. 716	0. 373	
1. 43	2. 27	60.	0. 075	0. 0020	0. 518	0. 490	
7. 90	2. 27	60.	0. 075	0. 0093	0. 578	0. 455	
10. 99	2. 27	60.	0. 075	0. 0137	0. 601	0. 442	
15. 27	2. 27	60.	0. 075	0. 0190	0. 629	0. 418	
21. 73	2. 27	60.	0. 075	0. 0270	0. 655	0. 380	
2. 72	2. 77	60.	0. 050	0. 0023	0. 523	0. 488	
7. 79	2. 77	60.	0. 050	0. 0065	0. 575	0. 450	
11. 18	2. 77	60.	0. 050	0. 0093	0. 617	0. 414	
16. 45	2. 77	60.	0. 050	0. 0137	0. 648	0. 375	
23. 16	2. 77	60.	0. 050	0. 0193	0. 716	0. 336	
29. 31	2. 77	60.	0. 050	0. 0245	0. 761	0. 322	
1. 79	5. 06	60.	0. 015	0. 0004	0. 523	0. 485	

**APPENDIX B**

**IRREGULAR WAVE CREST ELEVATIONS**

IRREGULAR WAVE CREST ELEVATIONS (LAM DATA)

D/(G\*T2) Hs/(G\*T2) n(cm) Hs(cm) Tp(sec) ETA L/Hs FOR PROBABILITY LEVELS

P=0.15 0.135 0.10 0.05 0.02 0.01 0.005

.00251	.000827	60.0	14.47	1.56	.563	.578	.631	.720	.792	.828
.00251	.000624	60.0	14.90	1.56	.547	.557	.614	.654	.822	.879
.00257	.000691	60.0	16.10	1.54	.589	.616	.664	.756	.837	.847
.00257	.000696	60.0	16.00	1.54	.542	.561	.594	.685	.796	.826
.00066	.000059	60.0	6.24	3.05	.533	.545	.584	.716	.767	.779
.00066	.000056	60.0	5.98	3.05	.524	.536	.588	.721	.789	.806
.00061	.001102	60.0	9.94	3.16	.535	.541	.591	.689	.710	.871
.00061	.000046	60.0	9.45	3.16	.551	.566	.606	.733	.824	1.045
.00175	.000346	60.0	12.01	1.88	.548	.586	.626	.745	.811	.907
.00175	.000341	60.0	11.84	1.88	.547	.558	.601	.724	.869	.906
.00044	.000049	60.0	12.22	3.56	.579	.598	.680	.794	.915	1.036
.00044	.000046	60.0	11.80	3.56	.598	.616	.666	.727	.851	.919
.00041	.001106	60.0	15.54	3.86	.642	.687	.707	.829	1.071	1.139
.00041	.001104	60.0	15.29	3.86	.640	.658	.762	.873	1.062	1.090
.00270	.000687	60.0	15.26	1.51	.557	.575	.636	.708	.790	.807
.00270	.000673	60.0	14.95	1.51	.564	.570	.599	.704	.804	1.024
.00216	.000585	60.0	16.25	1.66	.579	.589	.650	.729	.831	.887
.00216	.000578	60.0	16.07	1.66	.561	.582	.604	.713	.803	.884
.00057	.000058	60.0	6.17	3.24	.523	.533	.563	.743	.794	.811
.00057	.000045	60.0	5.86	3.24	.531	.539	.589	.754	.824	.848
.00175	.000345	60.0	11.94	1.88	.558	.570	.611	.745	.836	.919
.00175	.000342	60.0	11.87	1.88	.528	.557	.610	.715	.859	.865
.00200	.000522	45.0	11.74	1.51	.578	.598	.648	.731	1.011	1.132
.00200	.000503	45.0	11.31	1.51	.545	.550	.588	.663	.801	.885
.00147	.000349	45.0	12.19	1.77	.593	.607	.656	.747	.866	.956
.00147	.000341	45.0	11.94	1.77	.529	.542	.597	.650	.750	.888
.00065	.000062	45.0	4.30	2.67	.518	.543	.567	.792	.853	.888
.00065	.000059	45.0	4.09	2.67	.516	.539	.583	.756	.832	.893
.00099	.001145	45.0	8.41	2.15	.535	.554	.585	.664	.952	1.090
.00099	.001179	45.0	8.14	2.15	.526	.548	.601	.693	.832	.881
.00035	.000071	45.0	9.60	3.71	.653	.688	.764	.905	1.152	1.190
.00035	.000071	45.0	9.07	3.61	.672	.678	.709	.875	1.115	1.126
.00035	.000046	45.0	12.26	3.61	.650	.693	.761	.905	1.145	1.185
.00035	.000042	45.0	11.70	3.61	.688	.702	.734	.878	.994	1.123
.00045	.000095	45.0	9.54	3.20	.605	.619	.661	.828	1.132	1.172
.00020	.000040	45.0	9.19	4.83	.584	.610	.717	.872	1.095	1.234
.00062	.000092	45.0	6.72	2.72	.577	.581	.640	.723	.856	.869
.00062	.000087	45.0	6.35	2.72	.535	.556	.592	.721	.835	.850
.0113	.000343	75.0	17.49	2.17	.530	.572	.644	.602	1.082	1.105
.0117	.001307	75.0	17.50	2.21	.543	.573	.643	.772	.993	1.111

.0354	.00845	75.0	17.94	1.47	.590	.596	.624	.691	.761	.928
.0354	.00843	75.0	17.88	1.46	.594	.596	.634	.718	.750	.901
.0273	.00645	75.0	19.07	1.87	.617	.635	.694	.709	.947	1.050
.0273	.00646	75.0	19.11	1.87	.560	.577	.600	.716	.825	.928
.00846	.00843	75.0	8.05	2.98	.544	.555	.580	.655	.764	.834
.00846	.00849	75.0	7.76	2.98	.556	.572	.622	.673	.814	.842
.0200	.00370	75.0	13.85	1.95	.553	.576	.590	.672	.795	.948
.0210	.00383	75.0	13.70	1.91	.518	.538	.593	.718	.865	.947
.00556	.00115	75.0	15.57	3.71	.601	.640	.652	.603	.911	1.093
.00556	.00115	75.0	15.53	3.71	.605	.610	.654	.764	.850	1.013
.0157	.000376	75.0	17.95	2.21	.545	.568	.665	.752	1.005	1.084
.0152	.000559	75.0	17.72	2.25	.536	.573	.652	.627	1.012	1.078
.0354	.00846	75.0	17.95	1.47	.595	.601	.624	.696	.802	.842
.0354	.00842	75.0	17.86	1.47	.576	.587	.634	.698	.799	.853
.0273	.001493	75.0	19.02	1.67	.601	.644	.696	.795	.957	1.007
.0273	.001690	75.0	18.91	1.67	.562	.577	.617	.735	.857	.980
.0066	.000682	75.0	7.09	2.98	.532	.542	.611	.694	.784	.845
.00103	.000106	75.0	7.76	2.72	.551	.559	.620	.692	.797	.851
.00846	.00155	75.0	11.74	2.98	.580	.590	.622	.764	.812	.893
.00846	.000151	75.0	11.33	2.98	.537	.549	.612	.698	.821	.885
.0200	.00369	75.0	13.81	1.95	.544	.558	.587	.689	.811	.899
.0200	.00364	75.0	13.61	1.95	.511	.527	.581	.734	.835	.939
.00846	.00151	75.0	15.39	3.46	.643	.658	.682	.799	.902	1.098
.00846	.00114	75.0	15.33	3.71	.596	.604	.658	.771	.841	1.015
.0374	.00420	75.0	8.01	1.43	.550	.582	.611	.640	.759	.821
.0374	.00415	75.0	8.32	1.43	.492	.503	.564	.637	.760	.816
.0432	.00045	75.0	8.23	1.32	.541	.553	.595	.695	.864	1.010
.0432	.000467	75.0	7.97	1.32	.529	.542	.561	.662	.934	.982
.0354	.000445	75.0	9.60	1.47	.524	.535	.586	.686	.802	.879
.0349	.000434	75.0	9.32	1.48	.544	.551	.598	.685	.822	1.040
.0435	.00555	75.0	9.75	1.33	.545	.569	.621	.701	.796	.893
.0434	.00557	75.0	9.61	1.32	.530	.565	.560	.666	.855	1.075
.0354	.00520	75.0	10.01	1.46	.525	.536	.598	.733	.883	.968
.0354	.00514	75.0	10.77	1.46	.544	.553	.596	.704	.796	.905
.0491	.00557	75.0	8.41	1.25	.560	.596	.630	.721	.896	1.009
.0491	.00546	75.0	8.94	1.25	.553	.561	.620	.702	.804	.947
.0453	.00555	75.0	9.23	1.30	.528	.535	.567	.643	.839	1.037
.0453	.00544	75.0	9.01	1.30	.511	.523	.565	.663	.877	.922
.0354	.00525	75.0	11.81	1.51	.541	.563	.600	.660	.793	.840
.0354	.00516	75.0	11.61	1.51	.533	.553	.578	.639	.795	.945
.0341	.00552	75.0	12.13	1.50	.537	.573	.628	.706	.854	1.163
.0341	.00548	75.0	12.03	1.50	.514	.527	.583	.672	.932	1.235
.0354	.00552	75.0	8.02	1.46	.544	.567	.614	.683	.790	.857
.0354	.00546	75.0	7.90	1.43	.507	.523	.568	.692	.744	.875

.0439	.000480	75.0	8.19	1.32	.546	.562	.620	.720	.856	1.004	1.170
.0439	.000469	75.0	8.01	1.32	.513	.521	.544	.713	.855	.952	1.070
.0349	.000438	75.0	9.39	1.48	.524	.548	.606	.761	.828	.926	1.038
.0349	.000430	75.0	9.25	1.48	.508	.528	.567	.704	.740	.848	1.134
.0439	.000554	75.0	9.55	1.33	.591	.578	.624	.884	.823	.919	.999
.0439	.000569	75.0	9.17	1.32	.522	.534	.557	.671	.803	1.041	1.223
.0358	.000525	75.0	11.02	1.46	.549	.566	.606	.745	.843	.992	1.245
.0358	.000517	75.0	10.84	1.46	.548	.554	.647	.684	.769	.874	
.0441	.000578	75.0	8.84	1.25	.544	.591	.640	.732	.996	1.100	1.288
.0441	.000567	75.0	8.67	1.25	.548	.564	.596	.666	.797	1.099	1.242
.0453	.000558	75.0	9.23	1.30	.527	.534	.570	.633	.827	1.027	1.269
.0453	.000543	75.0	8.98	1.30	.514	.524	.564	.656	.803	.835	1.360
.0334	.000520	75.0	11.70	1.51	.564	.573	.623	.663	.801	.877	1.397
.0334	.000509	75.0	11.45	1.51	.514	.536	.584	.664	.782	1.028	1.473
.0341	.000515	75.0	11.30	1.50	.539	.565	.636	.730	.818	.902	1.342
.0341	.000508	75.0	11.16	1.50	.540	.570	.597	.665	.746	1.043	1.406
.0034	.000443	30.0	3.82	2.99	.573	.584	.651	.726	.870	.966	1.014
.0032	.000439	30.0	3.68	2.98	.565	.616	.678	.777	1.035	1.066	1.092
.0035	.000058	30.0	6.16	3.05	.570	.585	.676	.856	1.070	1.184	1.332
.0028	.000056	30.0	6.06	3.02	.566	.603	.696	.815	1.075	1.238	1.354
.0020	.000059	30.0	8.68	3.88	.606	.627	.703	.941	1.223	1.309	1.500
.0023	.000064	30.0	8.44	3.68	.633	.683	.756	.976	1.149	1.431	1.606
.0042	.000035	30.0	2.53	2.71	.492	.532	.579	.700	.913	1.064	1.172
.0024	.000023	30.0	2.43	2.28	.545	.556	.627	.781	.928	1.120	1.149
.0042	.00056	30.0	4.04	2.71	.590	.606	.663	.874	1.037	1.191	1.323
.0028	.00037	30.0	3.95	2.30	.545	.599	.717	.858	1.069	1.266	1.366
.0077	.00160	30.0	6.24	1.99	.518	.542	.652	.899	1.196	1.290	1.396
.0077	.00156	30.0	6.04	1.99	.586	.598	.717	.872	1.285	1.465	1.583
.0022	.00057	30.0	7.96	3.76	.613	.634	.695	.902	1.026	1.292	1.487
.0075	.00195	30.0	7.83	2.02	.650	.692	.759	.937	1.125	1.238	1.377
.0039	.00053	30.0	4.05	2.80	.503	.532	.641	.828	1.143	1.266	1.349
.0031	.00041	30.0	3.94	2.12	.544	.554	.664	.845	1.133	1.183	1.267
.0041	.00035	30.0	2.57	2.72	.451	.476	.539	.739	1.063	1.330	1.626
.0042	.00034	30.0	2.46	2.71	.481	.491	.550	.794	1.115	1.274	1.534
.0078	.00169	30.0	6.47	1.98	.617	.645	.722	.877	1.044	1.101	1.217

NBS 119  
NHS 771

NS 400GT1,DGT2,FMS .10000E+02 .20000F+02 .30273E+00  
B1,B2,RMS EM .80000E+01 .80000E+00 .80561E+03

NS 1010GT1,DGT2,FMS .20000E+02 .40000F+02 .45000E+01  
B1,B2,RMS EM .80000E+01 .80000E+00 .45697E+00

NS 1700GT1,DGT2,FMS .40000E+02 .80000F+02 .85638E+02  
B1,B2,RMS EM .80000E+01 .80000F+00 .62273E+00

NS 790GT1,DGT2,FMS .80000E+02 .16000E+01 .31639E+01

**APPENDIX C**

**IRREGULAR WAVE CREST DURATIONS**

IRREGULAR WAVE AVERAGE DIMENSIONLESS CREST DURATIONS											
D/(GWT2)	Hs/(GWT2)	D(cm)	Hs(cm)	Tp(sec)	AVERAGE TC/I						
					P=1.00	0.33	0.25	0.20	0.15	0.10	0.05
.00251	.000627	60.0	14.97	1.56	.434	.432	.424	.423	.415	.400	.401
.00251	.000624	60.0	14.90	1.56	.435	.426	.426	.427	.424	.416	.400
.00257	.000641	60.0	14.10	1.54	.436	.422	.414	.407	.394	.383	.369
.00257	.000666	60.0	14.00	1.54	.434	.438	.424	.417	.413	.405	.387
.00065	.000669	60.0	6.29	3.05	.452	.450	.448	.449	.440	.433	.427
.00066	.000666	60.0	5.94	3.05	.454	.458	.457	.455	.468	.460	.428
.00061	.00102	60.0	9.94	3.16	.450	.450	.449	.452	.461	.446	.439
.00061	.000696	60.0	9.45	3.16	.442	.442	.439	.434	.425	.437	.444
.00173	.00346	60.0	12.01	1.88	.457	.444	.436	.425	.407	.379	.383
.00173	.000341	60.0	11.86	1.88	.460	.466	.462	.463	.458	.436	.400
.00044	.00044	60.0	12.22	3.56	.465	.452	.445	.432	.428	.433	.428
.00044	.00049	60.0	11.86	3.56	.455	.456	.427	.432	.425	.386	.380
.00041	.00106	60.0	15.58	3.88	.443	.440	.434	.426	.423	.379	.298
.00041	.00104	60.0	15.24	3.88	.460	.460	.421	.416	.389	.372	.325
.00270	.01647	60.0	15.76	1.51	.453	.424	.411	.407	.406	.399	.391
.00270	.000673	60.0	14.95	1.51	.433	.431	.426	.421	.416	.411	.382
.00216	.00545	60.0	16.25	1.68	.431	.416	.412	.404	.396	.383	.374
.00216	.000574	60.0	16.07	1.68	.430	.426	.415	.411	.407	.393	.384
.00057	.00058	60.0	8.17	3.28	.463	.461	.456	.450	.447	.440	.419
.00057	.00055	60.0	5.94	3.28	.460	.460	.454	.462	.445	.439	.423
.00173	.00345	60.0	11.96	1.98	.450	.445	.428	.420	.411	.388	.367
.00173	.00342	60.0	11.87	1.98	.450	.456	.448	.441	.427	.416	.422
.00200	.000422	45.0	11.74	1.51	.435	.421	.404	.407	.390	.380	.376
.00200	.000603	45.0	11.61	1.51	.447	.443	.434	.430	.417	.401	.386
.00147	.00349	45.0	12.19	1.77	.438	.436	.422	.413	.393	.376	.349
.00147	.000391	45.0	11.94	1.77	.450	.450	.419	.420	.418	.422	.396
.00065	.00062	45.0	4.30	2.67	.460	.460	.450	.451	.450	.438	.430
.00065	.00059	45.0	4.09	2.67	.453	.455	.453	.444	.445	.427	.424
.00094	.00105	45.0	8.41	2.15	.472	.471	.475	.485	.472	.463	.408
.00094	.00179	45.0	8.14	2.15	.460	.460	.468	.468	.456	.458	.465
.00035	.00071	45.0	9.60	3.71	.421	.421	.412	.395	.371	.366	.378
.00035	.00071	45.0	9.07	3.61	.447	.451	.424	.421	.401	.350	.367
.00035	.00096	45.0	12.26	3.61	.451	.444	.430	.426	.427	.412	.376
.00035	.00092	45.0	11.70	3.61	.449	.452	.449	.457	.452	.461	.406
.00045	.00095	45.0	9.54	3.20	.462	.462	.457	.451	.443	.423	.379
.00029	.00040	45.0	9.19	4.83	.466	.466	.468	.460	.429	.443	.346
.00062	.00092	45.0	6.72	2.72	.439	.441	.435	.420	.429	.435	.433
.00062	.00067	45.0	6.35	2.72	.437	.438	.472	.474	.431	.419	.421
.00153	.00383	75.0	17.69	2.17	.480	.446	.438	.417	.408	.393	.372
.00157	.00387	75.0	17.50	2.21	.487	.456	.425	.414	.407	.390	.370

.0354	.00845	75.0	17.94	1.07	.476	.431	.426	.422	.414	.406	.394
.0358	.00643	75.0	17.68	1.06	.491	.440	.426	.424	.412	.395	.375
.0273	.00645	75.0	19.07	1.07	.463	.422	.401	.391	.387	.376	.359
.0273	.00646	75.0	19.11	1.07	.499	.446	.430	.423	.429	.416	.396
.0286	.00643	75.0	8.05	2.08	.448	.468	.466	.468	.446	.449	.459
.0286	.00649	75.0	7.76	2.08	.501	.469	.464	.448	.450	.448	.430
.0200	.006370	75.0	13.65	1.95	.478	.450	.438	.437	.425	.414	.417
.0210	.006383	75.0	13.71	1.91	.503	.468	.462	.453	.438	.417	.395
.0056	.001115	75.0	15.97	3.71	.472	.450	.412	.403	.373	.376	.354
.0056	.001115	75.0	15.93	3.71	.480	.459	.420	.420	.397	.392	.379
.0157	.006376	75.0	17.95	2.01	.479	.438	.427	.423	.395	.391	.370
.0158	.006359	75.0	17.72	2.05	.486	.453	.441	.435	.433	.404	.384
.0354	.006446	75.0	17.95	1.07	.473	.433	.425	.418	.408	.406	.403
.0354	.006432	75.0	17.66	1.07	.487	.442	.436	.431	.422	.410	.414
.0273	.006443	75.0	19.02	1.07	.460	.421	.411	.397	.386	.373	.358
.0273	.006464	75.0	18.91	1.07	.494	.440	.433	.426	.423	.414	.381
.0084	.000492	75.0	7.99	2.08	.494	.470	.464	.466	.450	.450	.453
.0103	.00116	75.0	7.76	2.02	.497	.461	.459	.441	.442	.445	.433
.0086	.001335	75.0	11.74	2.08	.485	.444	.436	.432	.440	.424	.419
.0086	.001311	75.0	11.33	2.08	.496	.456	.451	.427	.427	.421	.416
.0200	.00569	75.0	13.81	1.95	.470	.463	.457	.457	.426	.403	.407
.0200	.00368	75.0	13.61	1.95	.506	.474	.463	.450	.443	.403	.391
.0064	.00131	75.0	15.34	3.46	.464	.448	.422	.404	.396	.373	.351
.0056	.001114	75.0	15.33	3.71	.492	.436	.412	.415	.418	.407	.391
.0474	.006420	75.0	8.41	1.43	.485	.466	.457	.461	.451	.442	.440
.0374	.006415	75.0	8.32	1.43	.498	.478	.470	.469	.467	.454	.449
.00439	.006465	75.0	8.25	1.52	.470	.467	.456	.442	.432	.432	.432
.00439	.006467	75.0	7.97	1.52	.480	.462	.451	.452	.445	.437	.425
.0354	.006448	75.0	9.50	1.07	.483	.480	.477	.475	.475	.453	.458
.0354	.006434	75.0	9.32	1.08	.480	.457	.447	.445	.431	.414	.410
.00438	.006565	75.0	9.75	1.33	.480	.460	.447	.447	.440	.437	.436
.00439	.006557	75.0	9.51	1.32	.495	.465	.452	.452	.453	.430	.409
.0154	.006570	75.0	10.91	1.46	.481	.470	.471	.469	.455	.452	.425
.0154	.006514	75.0	10.77	1.46	.496	.460	.454	.452	.445	.442	.421
.00491	.006557	75.0	8.51	1.25	.482	.453	.447	.441	.444	.441	.433
.00491	.006546	75.0	8.34	1.25	.488	.450	.442	.437	.439	.431	.432
.00453	.006558	75.0	9.25	1.30	.497	.477	.461	.472	.463	.448	.442
.00453	.006546	75.0	9.01	1.30	.494	.468	.460	.459	.454	.451	.423
.0334	.006525	75.0	11.81	1.51	.475	.456	.450	.447	.445	.439	.424
.0334	.006516	75.0	11.61	1.51	.495	.471	.459	.449	.446	.435	.411
.0341	.006552	75.0	12.13	1.50	.476	.454	.442	.434	.432	.424	.417
.0341	.006544	75.0	12.03	1.50	.514	.464	.460	.455	.442	.427	.408
.0354	.006562	75.0	8.02	1.46	.484	.473	.461	.453	.444	.441	.429
.0374	.006594	75.0	7.90	1.46	.493	.483	.469	.455	.457	.443	.414

.00439	.000480	75.0	8.14	1.12	.0491	.0485	.0487	.0490	.0492	.0491	.0481
.00439	.000469	75.0	8.01	1.12	.0490	.0486	.0487	.0488	.0489	.0490	.0480
.00449	.000438	75.0	9.19	1.48	.0491	.0482	.0489	.0494	.0494	.0493	.0483
.00449	.000430	75.0	9.23	1.48	.0493	.0481	.0488	.0490	.0490	.0494	.0414
.00439	.000554	75.0	9.55	1.33	.0491	.0487	.0480	.0489	.0486	.0482	.0424
.00439	.000549	75.0	9.37	1.32	.0497	.0481	.0480	.0488	.0482	.0486	.0402
.00358	.000525	75.0	11.02	1.46	.0475	.0464	.0459	.0464	.0455	.0443	.0431
.00358	.000517	75.0	10.84	1.46	.0495	.0467	.0451	.0453	.0450	.0452	.0439
.00491	.000578	75.0	8.84	1.25	.0481	.0451	.0440	.0434	.0435	.0431	.0421
.00491	.000567	75.0	8.67	1.25	.0492	.0460	.0450	.0443	.0444	.0440	.0430
.00453	.000558	75.0	9.25	1.30	.0490	.0472	.0468	.0464	.0458	.0457	.0437
.00453	.000543	75.0	8.94	1.30	.0504	.0467	.0461	.0461	.0454	.0443	.0424
.00334	.000520	75.0	11.70	1.51	.0474	.0458	.0449	.0445	.0442	.0438	.0437
.00334	.000509	75.0	11.45	1.51	.0494	.0472	.0457	.0452	.0447	.0442	.0418
.00341	.000515	75.0	11.30	1.50	.0476	.0439	.0430	.0427	.0424	.0435	.0409
.00341	.000508	75.0	11.16	1.50	.0502	.0467	.0450	.0450	.0446	.0440	.0423
.00034	.000043	30.0	3.82	2.99	.0494	.0474	.0477	.0423	.0404	.0394	.0387
.00032	.000039	30.0	3.08	3.08	.0458	.0428	.0419	.0422	.0417	.0409	.0382
.00033	.000068	30.0	6.16	3.05	.0484	.0470	.0473	.0497	.0395	.0373	.0365
.00028	.000056	30.0	6.06	3.12	.0470	.0466	.0430	.0437	.0414	.0390	.0358
.00020	.000059	30.0	8.64	3.88	.0452	.0515	.0499	.0466	.0447	.0433	.0378
.00023	.000064	30.0	8.44	3.68	.0444	.0465	.0454	.0441	.0418	.0420	.0326
.00042	.000035	30.0	2.53	2.71	.0495	.0481	.0460	.0447	.0433	.0444	.0407
.00028	.000023	30.0	2.43	3.26	.0464	.0492	.0450	.0441	.0439	.0444	.0424
.00042	.000056	30.0	4.64	2.71	.0494	.0469	.0429	.0416	.0400	.0381	.0361
.00028	.000037	30.0	3.45	3.30	.0465	.0437	.0410	.0409	.0374	.0370	.0346
.00077	.000160	30.0	6.24	1.99	.0495	.0545	.0430	.0472	.0460	.0438	.0391
.00077	.000156	30.0	6.08	1.99	.0467	.0474	.0447	.0474	.0449	.0423	.0379
.00022	.000057	30.0	7.96	3.76	.0465	.0490	.0484	.0471	.0457	.0441	.0402
.00075	.000145	30.0	7.83	2.62	.0451	.0454	.0446	.0439	.0441	.0424	.0375
.00049	.000053	30.0	4.05	2.80	.0442	.0467	.0454	.0445	.0435	.0410	.0417
.00031	.000041	30.0	3.94	3.12	.0479	.0445	.0430	.0415	.0415	.0404	.0400
.00041	.000035	30.0	2.57	2.72	.0490	.0483	.0472	.0465	.0463	.0460	.0460
.00042	.000034	30.0	2.46	2.71	.0471	.0471	.0465	.0454	.0444	.0431	.0426
.00078	.000169	30.0	6.47	1.98	.0468	.0490	.0472	.0481	.0479	.0490	.0529
.00078	.000163	30.0	6.26	1.98	.0461	.0496	.0485	.0466	.0443	.0443	.0416
.00021	.000058	30.0	8.17	3.79	.0458	.0520	.0497	.0503	.0494	.0471	.0429
.00080	.000214	30.0	8.01	3.95	.0482	.0494	.0498	.0501	.0495	.0473	.0477

APPENDIX D

IRREGULAR WAVE DATA FOR SHOALING CONDITIONS

## WAVE CREST ELEVATION WITH A 1/30 PLANE SLOPE

ID	SENSOR	TP(S)	HS(CH)	D(CH)	CREST ELEVATION/HS						
					P=0.15	0.135	0.10	0.05	0.02	0.01	0.005
<b>R011251110 157 10</b>											
2	1.30	8.62	60.0	.535	.551	.604	.679	.757	.795	.818	
3	1.30	8.49	60.0	.510	.528	.567	.669	.773	.808	1.026	
4	1.46	8.35	60.0	.524	.543	.580	.647	.723	.759	.945	
5	1.40	8.29	52.5	.533	.555	.570	.665	.731	.803	.842	
6	1.43	8.04	45.0	.534	.546	.587	.682	.796	.874	1.012	
7	1.43	7.67	37.5	.523	.535	.555	.670	.845	.882	.954	
8	1.43	7.60	30.0	.501	.577	.613	.715	.791	.880	.937	
9	1.43	7.95	22.5	.569	.604	.693	.748	.939	1.011	1.226	
10	1.55	7.60	15.0	.691	.703	.763	.874	.948	1.044	1.056	
11	1.58	5.22	7.5	.740	.745	.790	.887	.998	1.060	1.135	
<b>R011251118 257 10</b>											
2	1.32	8.93	60.0	.526	.552	.575	.629	.794	.886	.976	
3	1.32	8.80	60.0	.507	.518	.557	.654	.853	.998	1.060	
4	1.12	8.67	60.0	.518	.547	.584	.686	.858	.879	1.004	
5	1.32	8.59	52.5	.554	.582	.607	.697	.871	.971	.992	
6	1.32	8.38	45.0	.530	.546	.593	.722	.841	.926	1.043	
7	1.32	8.21	37.5	.546	.574	.649	.739	.917	.941	1.058	
8	1.32	8.00	30.0	.506	.624	.655	.752	.881	.918	.980	
9	1.32	7.96	22.5	.575	.604	.712	.790	.983	1.001	1.010	
10	1.32	7.41	15.0	.693	.711	.741	.812	.920	.943	1.011	
11	1.48	5.11	7.5	.721	.729	.765	.826	.944	1.039	1.104	
<b>R011251129 357 10</b>											
2	1.48	9.53	60.0	.570	.581	.597	.653	.749	.815	.881	
3	1.37	9.40	60.0	.542	.547	.581	.722	.765	.837	.942	
4	1.41	9.20	60.0	.537	.553	.581	.712	.795	.839	.961	
5	1.41	9.25	52.5	.554	.561	.592	.709	.778	.884	.912	
6	1.01	9.06	45.0	.545	.553	.610	.733	.870	.938	.944	
7	1.57	9.93	37.5	.547	.555	.604	.726	.783	.830	1.047	
8	1.53	9.47	30.0	.597	.614	.660	.735	.845	.880		
9	1.57	9.10	22.5	.636	.657	.732	.864	.950	.983		
10	1.75	8.36	15.0	.745	.757	.791	.865	.954	1.076		
11	1.86	5.36	7.5	.726	.733	.758	.860	.934	1.001		
<b>R011251138 457 5</b>											
2	1.21	5.16	60.0	.490	.510	.548	.656	.768	.808	.845	
3	1.32	5.06	60.0	.513	.520	.563	.646	.762	.862	.908	
4	1.32	5.01	60.0	.510	.524	.547	.606	.742	.918	.977	
5	1.32	4.95	52.5	.499	.511	.564	.664	.777	.935	1.071	
6	1.32	4.80	45.0	.494	.516	.557	.648	.750	.940	.914	
7	1.32	4.66	37.5	.528	.557	.600	.699	.768	.832	.874	
8	1.32	4.70	30.0	.500	.518	.552	.632	.731	.790	.980	
9	1.32	4.76	22.5	.546	.553	.572	.704	.805	.849	1.013	
10	1.32	4.74	15.0	.613	.620	.692	.751	.933	.981	1.073	
11	1.42	4.44	7.5	.747	.773	.803	.883	.937	.994	1.013	
<b>R011251147 457 5</b>											
2	1.32	9.68	60.0	.515	.551	.590	.696	.803	.861	.903	
3	1.32	9.57	60.0	.540	.555	.601	.704	.860	.911	.921	
4	1.32	9.37	60.0	.547	.555	.597	.687	.832	1.006	1.095	
5	1.32	9.28	52.5	.542	.552	.590	.679	.734	.998	1.197	
7	1.32	9.16	45.0	.556	.571	.601	.704	.899	.934	1.154	
8	1.32	9.00	37.5	.532	.554	.595	.731	.956	1.145	1.192	
9	1.32	8.99	30.0	.554	.583	.627	.766	.905	1.053	1.097	
10	1.32	8.78	22.5	.606	.627	.681	.758	.911	1.030	1.097	
11	1.33	7.91	15.0	.695	.730	.781	.838	.923	.987	1.002	
11	1.60	5.25	7.5	.705	.716	.708	.872	1.031	1.087	1.123	
<b>R011251156 557 5</b>											
2	1.46	5.42	60.0	.501	.520	.563	.645	.726	.777	.790	
3	1.46	5.39	60.0	.503	.500	.548	.657	.715	.790	.836	
4	1.46	5.30	60.0	.508	.523	.590	.706	.766	.822	.939	
5	1.46	5.27	52.5	.524	.537	.570	.634	.744	.855	1.082	
6	1.41	5.17	45.0	.514	.519	.570	.629	.808	.934		
7	1.41	5.08	37.5	.497	.514	.596	.647	.797	.834	.975	
8	1.41	5.07	30.0	.523	.540	.634	.710	.814	.873	.878	
9	1.41	5.21	22.5	.552	.562	.602	.701	.925	1.008	1.033	
10	1.70	5.24	15.0	.652	.666	.727	.806	1.023	1.045		
11	8.53	4.59	7.5	.693	.612	.633	.693	1.006	1.055	1.184	

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R0112512 S 557 S

2	1.46	10.79	60.0	.554	.500	.028	.766	.034	.874	1.021
3	1.46	10.54	60.0	.521	.547	.030	.711	.027	.944	1.187
4	1.46	10.39	60.0	.548	.584	.014	.722	.062	1.042	1.126
5	1.46	10.39	52.5	.532	.551	.004	.752	.040	.853	
6	1.46	10.15	45.0	.571	.581	.014	.646	.059	.934	.985
7	1.46	10.13	37.5	.554	.574	.004	.747	.000	1.040	
8	1.46	10.05	30.0	.584	.610	.000	.492	.049	1.012	
9	1.46	9.93	22.5	.052	.060	.020	.422	.054	.477	
10	1.52	8.69	15.0	.743	.755	.016	.452	.089	.961	
11	1.52	5.45	7.5	.722	.727	.077	.467	.051	1.040	

R011251223 657 S

2	1.25	9.07	60.0	.526	.530	.560	.621	.693	1.018	1.114
3	1.25	6.93	60.0	.515	.514	.552	.044	.413	1.068	1.086
4	1.25	6.83	40.0	.526	.544	.595	.672	.796	.871	.989
5	1.25	6.72	52.5	.550	.560	.002	.731	.416	.876	.886
6	1.25	8.40	45.0	.510	.533	.580	.077	.428	1.092	1.124
7	1.25	8.25	37.5	.552	.559	.002	.724	.422	1.081	1.105
8	1.25	8.09	30.0	.579	.591	.045	.705	.482	.942	1.179
9	1.23	7.42	22.5	.074	.087	.725	.764	.924	.976	.999
10	1.23	7.32	15.0	.715	.720	.050	.450	.966	.987	.995
11	1.53	4.92	7.5	.725	.736	.761	.839	.877	.938	1.044

R011251233 757 S

2	1.30	9.33	60.0	.511	.522	.568	.710	.834	1.072	1.196
3	1.30	9.26	60.0	.543	.548	.574	.644	.786	.888	1.175
4	1.30	9.04	60.0	.561	.574	.011	.077	.017	.961	1.165
5	1.30	8.97	52.5	.561	.572	.004	.066	.901	1.033	1.236
6	1.30	8.76	45.0	.539	.556	.610	.706	.793	1.064	1.146
7	1.30	8.59	37.5	.550	.572	.045	.729	.852	1.070	1.244
8	1.30	8.47	30.0	.577	.584	.039	.751	.908	1.043	1.145
9	1.30	8.41	22.5	.019	.040	.707	.646	.967	1.020	1.237
10	1.30	7.62	15.0	.714	.722	.045	.819	.899	.966	1.005
11	1.14	5.05	7.5	.715	.735	.770	.849	.932	.944	1.057

R011251245 657 S

2	1.46	0.21	60.0	.471	.484	.517	.603	.749	1.231	1.359
3	1.46	0.16	60.0	.473	.486	.510	.576	.674	1.116	1.354
4	1.46	0.10	60.0	.467	.489	.524	.573	.704	1.136	1.376
5	1.46	0.06	52.5	.514	.511	.547	.580	.820	1.200	1.211
6	1.46	5.92	45.0	.479	.486	.544	.658	.968	1.171	1.585
7	1.46	5.87	37.5	.476	.494	.533	.642	1.227	1.491	1.576
8	1.46	5.80	30.0	.445	.514	.543	.615	1.275	1.519	
9	1.52	5.92	22.5	.523	.539	.571	.686	1.244	1.591	
10	1.54	5.52	15.0	.660	.690	.735	.614	.957	1.165	
11	1.54	4.78	7.5	.787	.796	.836	.873	.989	1.081	

R0112513 6 657 0

2	1.52	9.56	60.0	.513	.520	.584	.725	.834	1.044	1.284
3	1.52	9.48	60.0	.524	.550	.04	.681	.793	1.104	1.374
4	1.52	9.38	60.0	.525	.535	.595	.666	.911	.993	1.260
5	1.52	9.32	52.5	.510	.524	.550	.623	.930	1.512	
6	1.52	9.14	45.0	.526	.534	.572	.651	.820	1.226	
7	1.52	9.04	37.5	.523	.529	.575	.680	.889	1.177	
8	1.52	8.67	30.0	.553	.564	.014	.694	.974	1.170	
9	1.52	8.74	22.5	.617	.630	.676	.774	.921	1.023	
10	1.54	8.11	15.0	.724	.760	.740	.820	.874	.955	
11	1.54	5.24	7.5	.675	.698	.747	.841	1.032	1.142	

R0112510 1 645 10

2	1.77	12.81	60.0	.581	.590	.027	.725	.833	.900	
3	1.77	12.73	60.0	.526	.563	.614	.661	.798	.895	
4	1.77	12.62	60.0	.526	.536	.566	.660	.726	.816	
5	1.77	12.64	52.5	.551	.561	.630	.705	.784	.844	
6	1.77	12.55	45.0	.600	.620	.635	.702	.746	.837	
7	1.77	12.52	37.5	.611	.627	.661	.730	.845	1.039	
8	1.77	12.31	30.0	.654	.662	.713	.767	.907	.968	
9	1.77	11.96	22.5	.751	.763	.790	.884	.909	.939	
10	2.03	9.83	15.0	.792	.806	.825	.871	.959	1.014	
11	2.39	0.03	7.5	.665	.672	.721	.886	1.007	1.041	

R011251010 645 S

2	1.79	0.26	60.0	.516	.537	.568	.622	.709	.752	
3	1.79	0.18	60.0	.526	.554	.582	.642	.799	.808	
4	1.79	0.13	60.0	.473	.482	.551	.653	.775	.847	
5	1.78	0.13	52.5	.508	.516	.564	.675	.751	.798	
6	1.79	0.09	45.0	.530	.546	.566	.641	.844	.829	
7	1.78	0.09	37.5	.531	.550	.570	.645	.761	.833	
8	1.67	0.12	30.0	.534	.554	.621	.711	.794	.855	
9	1.67	0.35	22.5	.627	.640	.690	.746	.786	.909	
10	2.03	0.39	15.0	.731	.753	.631	.918	1.012	1.048	
11	2.03	4.91	7.5	.815	.824	.672	.964	1.024	1.126	

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8011251020 745 S

2	3.51	0.68	60.0	.560	.615	.664	.772	.949	.994
3	3.46	0.58	60.0	.590	.604	.623	.754	.912	.999
4	3.51	6.57	60.0	.520	.530	.597	.748	.934	1.094
5	3.40	0.66	52.5	.547	.576	.625	.745	.869	1.091
6	3.51	6.79	45.0	.557	.564	.585	.703	.832	1.073
7	3.46	0.90	37.5	.546	.550	.575	.736	.856	1.047
8	3.66	7.10	30.0	.603	.626	.660	.722	1.013	1.161
9	3.71	7.64	22.5	.705	.715	.780	.457	1.099	1.292
10	4.27	7.73	15.0	.660	.684	1.040	1.160	1.396	1.474
11	3.88	5.02	7.5	.797	.811	.869	.966	1.055	1.148

8011251024 745 T0

2	3.71	14.63	60.0	.659	.688	.748	.904	1.154	1.202
3	3.46	13.91	60.0	.640	.656	.758	.881	1.099	1.179
4	3.46	13.83	60.0	.571	.630	.590	.857	.945	1.274
5	3.46	13.92	52.5	.650	.661	.712	.803	.981	1.302
6	3.60	14.12	45.0	.596	.657	.705	.804	.881	1.172
7	3.41	14.32	37.5	.665	.673	.688	.916	1.102	1.213
8	3.41	14.69	30.0	.745	.740	.841	1.020	1.213	1.240
9	3.71	14.09	22.5	.804	.817	.893	1.069	1.257	1.294
10	3.88	11.18	15.0	.741	.815	.848	.963	1.129	1.135
11	11.13	8.19	7.5	.625	.630	.697	.778	.870	.925

8011241517 345 S

2	1.67	9.54	60.0	.532	.552	.575	.681	.730	.860
3	1.67	9.35	60.0	.534	.540	.591	.677	.838	.900
4	1.67	9.29	60.0	.520	.546	.596	.647	.833	.946
5	1.67	9.27	52.5	.505	.527	.575	.673	.814	.975
6	1.67	9.15	45.0	.549	.559	.599	.706	.797	.921
7	1.67	9.03	37.5	.546	.610	.648	.721	.843	1.105
8	1.67	9.10	30.0	.571	.576	.614	.715	.858	1.234
9	1.67	9.15	22.5	.713	.722	.765	.900	1.019	1.117
10	1.67	8.47	15.0	.611	.623	.666	.942	1.050	1.091
11	1.83	5.48	7.5	.730	.744	.800	.905	.981	1.064

801125 942 545 T0

2	3.12	10.56	60.0	.524	.546	.638	.864	.989	1.156
3	3.12	10.59	60.0	.500	.512	.664	.654	1.059	
4	3.12	10.49	60.0	.573	.576	.654	.798	1.006	1.092
5	3.12	10.70	52.5	.600	.624	.671	.832	1.000	
6	3.12	10.83	45.0	.615	.603	.680	.793	1.070	
7	3.12	11.18	37.5	.681	.700	.746	.891	1.221	1.229
8	3.12	11.33	30.0	.731	.672	.975	1.060	1.387	1.473
9	3.12	10.95	22.5	.904	.603	1.031	1.104	1.251	
10	3.33	9.27	15.0	.654	.670	.963	1.102	1.209	1.239
11	15.06	6.83	7.5	.613	.647	.732	.672	1.028	1.300

801125 451 545 8

2	3.16	8.33	60.0	.512	.526	.607	.775	.918	1.091
3	3.16	8.33	60.0	.500	.533	.634	.600	.985	1.010
4	3.16	8.28	60.0	.540	.544	.637	.773	.925	
5	3.16	8.44	52.5	.573	.580	.640	.814	.940	
6	3.16	8.55	45.0	.585	.605	.629	.804	1.049	
7	3.16	8.76	37.5	.635	.645	.768	.785	1.148	
8	3.16	8.93	30.0	.692	.757	.679	.972	1.340	
9	3.16	9.14	22.5	.651	.910	1.043	1.291	1.402	1.567
10	3.37	8.01	15.0	.694	.918	1.022	1.149	1.236	1.267
11	15.06	6.03	7.5	.682	.707	.605	.956	1.036	1.265

8011241439 145 S

2	2.17	8.20	60.0	.468	.471	.524	.671	.825	1.012
3	2.17	8.16	60.0	.505	.504	.557	.621	.755	.908
4	2.17	8.07	60.0	.536	.542	.623	.691	.752	.945
5	2.21	8.11	52.5	.490	.507	.558	.646	.750	1.004
6	2.21	8.05	45.0	.476	.485	.555	.711	.871	1.165
7	2.21	8.06	37.5	.511	.531	.622	.748	.926	1.079
8	2.21	8.30	30.0	.581	.607	.634	.760	.880	1.140
9	1.68	8.46	22.5	.726	.751	.639	.961	1.075	1.360
10	2.31	7.94	15.0	.791	.818	.671	1.078	1.113	1.171
11	2.37	5.46	7.5	.704	.706	.743	.834	1.042	1.148

8011241440 245 S

2	1.47	9.17	60.0	.513	.525	.582	.628	.712	.762
3	1.46	9.04	60.0	.514	.525	.539	.611	.678	.753
4	1.47	8.90	60.0	.534	.542	.570	.637	.767	.860
5	1.47	8.85	52.5	.524	.545	.558	.673	.724	.761
6	1.47	8.75	45.0	.527	.545	.584	.646	.737	.817
7	1.47	8.72	37.5	.545	.560	.610	.689	.748	.740
8	1.47	8.62	30.0	.564	.587	.612	.684	.791	.834
9	1.47	9.04	22.5	.652	.664	.714	.820	.915	.987
10	1.47	8.52	15.0	.743	.770	.620	.847	.945	.984
11	1.52	5.30	7.5	.700	.717	.765	.893	.924	1.074

8011241457 245 5

2	1.46	9.18	60.0	.516	.520	.579	.624	.721	.737
3	1.46	9.04	60.0	.523	.529	.546	.625	.679	.722
4	1.46	8.42	60.0	.551	.562	.584	.635	.748	.833
5	1.46	8.46	52.5	.542	.546	.554	.638	.727	.741
6	1.46	8.72	45.0	.525	.558	.600	.667	.744	.641
7	1.47	8.69	37.5	.554	.570	.608	.690	.742	.603
8	1.47	8.81	30.0	.571	.583	.614	.687	.785	.925
9	1.47	8.04	22.5	.631	.671	.714	.815	.925	.945
10	1.47	8.51	15.0	.754	.764	.799	.887	.957	.971
11	1.59	5.32	7.5	.694	.694	.787	.872	.975	1.035 1.062

801124157 345 5

2	1.47	9.54	60.0	.533	.552	.573	.689	.740	.857
3	1.47	9.34	60.0	.535	.544	.590	.701	.637	.400
4	1.47	9.28	60.0	.523	.542	.594	.644	.879	.942
5	1.47	9.27	52.5	.517	.532	.574	.685	.816	.969
6	1.47	9.15	45.0	.545	.560	.587	.731	.801	.938
7	1.47	9.02	37.5	.546	.582	.657	.721	.854	1.104
8	1.47	9.09	30.0	.568	.576	.600	.714	.863	1.245
9	1.47	9.17	22.5	.700	.714	.769	.916	1.003	1.074
10	1.47	9.46	15.0	.800	.832	.867	.965	1.016	1.026
11	1.47	5.47	7.5	.707	.733	.804	.902	1.012	1.074

8011241439 145 5

2	2.17	8.20	60.0	.468	.471	.524	.671	.825	1.012
3	2.17	8.16	60.0	.505	.504	.557	.621	.755	.908
4	2.17	8.07	60.0	.536	.542	.623	.691	.752	.945
5	2.21	8.11	52.5	.490	.517	.558	.646	.750	1.004
6	2.21	8.05	45.0	.476	.485	.555	.711	.871	1.165
7	2.21	8.06	37.5	.511	.531	.622	.788	.926	1.079
8	2.21	8.30	30.0	.581	.607	.635	.760	.880	1.140
9	1.68	8.46	22.5	.726	.751	.839	.961	1.075	1.360
10	2.31	7.94	15.0	.741	.818	.971	1.078	1.113	1.171
11	2.37	5.46	7.5	.784	.716	.743	.834	1.042	1.148

8011241448 245 5

2	1.47	9.17	60.0	.513	.527	.582	.628	.732	.762
3	1.46	9.04	60.0	.514	.525	.539	.611	.678	.753
4	1.47	8.90	60.0	.534	.542	.576	.637	.767	.860
5	1.47	8.85	52.5	.524	.545	.558	.623	.724	.761
6	1.47	8.75	45.0	.527	.545	.583	.648	.737	.817
7	1.47	8.72	37.5	.545	.560	.619	.684	.748	.786
8	1.47	8.82	30.0	.569	.587	.612	.684	.791	.934
9	1.47	9.04	22.5	.652	.669	.714	.820	.915	.987
10	1.47	8.52	15.0	.743	.770	.829	.897	.945	.984
11	1.52	5.30	7.5	.700	.717	.765	.893	.924	1.074

8011241457 245 5

2	1.46	9.18	60.0	.517	.520	.579	.624	.721	.737
3	1.46	9.04	60.0	.523	.524	.546	.625	.679	.722
4	1.46	8.42	60.0	.551	.562	.584	.635	.748	.833
5	1.46	8.86	52.5	.541	.542	.554	.638	.727	.741
6	1.46	8.72	45.0	.525	.555	.600	.657	.744	.641
7	1.47	8.69	37.5	.554	.570	.608	.690	.742	.803
8	1.47	8.81	30.0	.571	.583	.614	.687	.785	.925
9	1.47	9.04	22.5	.630	.671	.714	.805	.925	.985
10	1.47	8.51	15.0	.754	.770	.794	.887	.957	.971
11	1.59	5.32	7.5	.694	.694	.787	.872	.975	1.035 1.062

801124157 345 5

2	1.67	9.54	60.0	.533	.552	.573	.689	.740	.857
3	1.67	9.34	60.0	.535	.541	.590	.701	.637	.400
4	1.67	9.28	60.0	.523	.542	.594	.644	.879	.942
5	1.67	9.27	52.5	.517	.532	.574	.685	.816	.969
6	1.67	9.15	45.0	.545	.566	.587	.731	.801	.938
7	1.67	9.02	37.5	.550	.589	.657	.721	.854	1.104
8	1.67	9.09	30.0	.568	.572	.606	.719	.863	1.245
9	1.67	9.17	22.5	.700	.714	.760	.916	1.003	1.074
10	1.67	8.46	15.0	.804	.832	.867	.965	1.016	1.026
11	1.67	5.47	7.5	.707	.733	.804	.902	1.012	1.078

8011241321 675 10

2	1.84	10.11	60.0	.529	.539	.573	.641	.768	.869
3	1.74	10.04	60.0	.537	.546	.603	.748	.919	.996
4	1.84	9.96	60.0	.505	.512	.574	.665	.757	.778
5	1.84	9.43	52.5	.536	.548	.564	.625	.917	.960
6	1.84	9.90	45.0	.544	.570	.589	.656	.798	.939
7	1.84	9.80	37.5	.572	.579	.606	.750	.868	1.098
8	1.86	9.83	30.0	.587	.605	.666	.785	.967	1.135
9	1.84	9.43	22.5	.645	.712	.760	.876	1.052	1.109
10	1.86	8.77	15.0	.759	.767	.817	.876	.998	1.068
11	2.10	5.57	7.5	.691	.722	.781	.862	.973	1.106

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R011241332 775 10

2	4.20	10.90	60.0	.567	.575	.035	.771	1.008	1.051
3	4.20	10.64	60.0	.583	.597	.027	.761	1.044	1.160
4	4.20	10.66	60.0	.631	.638	.728	.775	1.105	1.202
5	4.20	10.73	52.5	.612	.638	.691	.736	1.043	1.290
6	4.20	10.80	45.0	.672	.677	.713	.818	.999	1.457
7	4.20	11.04	37.5	.692	.713	.766	.833	1.069	1.723
8	4.20	11.34	30.0	.784	.820	.879	.940	1.280	1.050
9	4.20	11.62	22.5	.923	.937	.949	1.048	1.280	1.334
10	4.20	10.31	15.0	.888	.915	.949	1.043	1.177	1.287
11	4.41	7.04	7.5	.642	.668	.725	.792	.895	.916

R011241341 775 10

2	4.20	10.85	60.0	.574	.603	.047	.802	1.022	1.067
3	4.20	10.73	60.0	.566	.573	.024	.758	1.049	1.168
4	4.20	10.73	60.0	.608	.641	.720	.765	1.114	1.231
5	4.27	10.85	52.5	.603	.623	.678	.729	1.028	1.263
6	4.27	10.88	45.0	.680	.684	.703	.813	1.014	1.477
7	4.27	11.06	37.5	.698	.715	.754	.827	1.044	1.688
8	4.27	11.32	30.0	.764	.791	.873	.972	1.197	1.666
9	4.20	11.63	22.5	.941	.946	.993	1.127	1.515	1.362
10	4.20	10.36	15.0	.980	.923	.974	1.059	1.189	1.264
11	4.66	7.14	7.5	.675	.696	.735	.793	.985	1.026

R011241431 145 5

2	2.17	8.12	60.0	.477	.481	.525	.682	.838	1.030
3	2.17	8.04	60.0	.500	.511	.565	.624	.767	.908
4	2.17	7.95	60.0	.541	.553	.618	.705	.781	.958
5	2.17	7.99	52.5	.495	.505	.546	.648	.746	1.014
6	2.17	8.04	45.0	.483	.493	.550	.719	.897	1.158
7	2.17	8.01	37.5	.508	.527	.591	.785	.940	1.061
8	2.17	8.21	30.0	.550	.588	.634	.773	.909	1.101
9	1.64	8.44	22.5	.720	.724	.820	.971	1.084	1.327
10	2.31	7.97	15.0	.742	.800	.856	1.016	1.111	1.138
11	2.37	5.45	7.5	.737	.746	.775	.842	1.001	1.143

R011241156 475 10

2	2.72	8.11	60.0	.571	.577	.049	.776	.835	.852
3	2.72	8.13	60.0	.556	.573	.004	.740	.770	.811
4	2.72	8.07	60.0	.540	.575	.018	.741	.809	.892
5	2.72	8.22	52.5	.552	.576	.034	.723	.815	.979
6	2.70	8.42	45.0	.588	.593	.034	.797	.899	1.145
7	2.70	8.50	37.5	.605	.681	.712	.942	1.083	1.278
8	2.70	8.85	30.0	.740	.763	.875	1.135	1.235	1.488
9	2.67	9.10	22.5	.935	.964	1.158	1.320	1.492	1.505
10	3.51	8.06	15.0	.948	.946	1.027	1.171	1.231	1.256
11	9.14	5.71	7.5	.751	.763	.798	.852	1.176	1.305

R011241249 575 10

2	2.61	5.36	60.0	.509	.516	.574	.733	.941	
3	2.61	5.35	60.0	.470	.488	.534	.712	.874	.950
4	2.61	5.30	60.0	.491	.503	.570	.729	.916	.971
5	2.61	5.37	52.5	.492	.503	.578	.725	.925	.994
6	2.61	5.45	45.0	.524	.559	.661	.763	.937	1.164
7	2.61	5.57	37.5	.565	.581	.730	.825	1.020	1.318
8	2.61	5.75	30.0	.593	.622	.777	.950	1.313	1.537
9	2.61	6.06	22.5	.661	.705	.825	1.141	1.436	1.826
10	2.59	5.91	15.0	.692	.950	1.028	1.052	1.570	1.712
11	3.51	4.53	7.5	.790	.814	.914	1.021	1.192	1.248

R011241259 575 10

2	2.61	5.36	60.0	.503	.509	.577	.723	.945	1.067
3	2.61	5.36	60.0	.476	.492	.537	.713	.884	.961
4	2.61	5.32	60.0	.492	.500	.584	.722	.904	.968
5	2.61	5.30	52.5	.493	.517	.573	.732	.934	1.010
6	2.61	5.47	45.0	.530	.558	.671	.766	.939	1.167
7	2.61	5.58	37.5	.560	.572	.717	.899	1.017	1.323
8	2.61	5.78	30.0	.590	.617	.768	.959	1.341	1.554
9	2.61	6.09	22.5	.712	.749	.846	1.232	1.639	1.858
10	2.59	5.91	15.0	.896	.975	1.040	1.042	1.572	1.765
11	3.51	4.58	7.5	.743	.871	.965	1.004	1.165	1.173

R011241510 675 10

2	1.84	10.17	60.0	.522	.536	.585	.657	.783	.840
3	1.74	10.05	60.0	.521	.540	.604	.751	.900	1.002
4	1.84	9.98	60.0	.510	.519	.564	.675	.744	.780
5	1.84	9.47	52.5	.533	.515	.582	.612	.929	.955
6	1.84	9.49	45.0	.541	.566	.598	.688	.861	.937
7	1.84	9.75	37.5	.570	.584	.624	.771	.884	1.130
8	1.84	9.83	30.0	.594	.613	.665	.801	.968	1.141
9	1.84	9.81	22.5	.646	.704	.759	.876	1.061	1.114
10	2.00	8.75	15.0	.790	.804	.835	.887	.969	1.080
11	2.10	5.55	7.5	.602	.720	.744	.833	1.030	1.071

80112411 2 275 10

2	1.47	13.84	60.0	.537	.540	.582	.673	.752	.838
3	1.46	13.65	60.0	.559	.563	.610	.667	.759	.828
4	1.46	13.47	60.0	.556	.565	.593	.688	.799	.833
5	1.46	13.48	52.5	.575	.598	.626	.673	.839	.916
6	1.46	13.32	45.0	.603	.609	.642	.715	.869	.906
7	1.47	13.11	37.5	.595	.602	.624	.752	.856	.920
8	1.47	13.01	30.0	.655	.660	.737	.786	.835	.895
9	1.47	12.30	22.5	.709	.717	.742	.793	.838	.873
10	1.47	9.65	15.0	.723	.727	.757	.813	.943	1.052
11	8.63	5.68	7.5	.682	.684	.745	.889	.954	1.028

8011241111 275 10

2	1.46	13.86	60.0	.530	.537	.587	.679	.752	.878
3	1.46	13.69	60.0	.558	.569	.602	.644	.756	.893
4	1.46	13.60	60.0	.560	.571	.593	.691	.806	.852
5	1.46	13.52	52.5	.573	.593	.618	.672	.851	.908
6	1.46	13.33	45.0	.586	.597	.651	.709	.895	.941
7	1.47	13.16	37.5	.580	.599	.639	.734	.844	.884
8	1.47	13.06	30.0	.678	.686	.723	.795	.854	.880
9	1.47	12.30	22.5	.697	.708	.753	.832	.874	.889
10	1.47	9.76	15.0	.720	.743	.770	.829	.936	1.016
11	8.63	5.70	7.5	.639	.653	.712	.877	.957	1.059

8011241121 375 10

2	1.67	13.87	60.0	.565	.605	.636	.722	.833	.929
3	1.67	13.76	60.0	.533	.542	.577	.687	.800	1.050
4	1.67	13.65	60.0	.562	.564	.589	.686	.769	1.116
5	1.67	13.66	52.5	.570	.582	.598	.738	.871	1.091
6	1.67	13.40	45.0	.012	.027	.070	.741	.837	.941
7	1.67	13.16	37.5	.029	.051	.710	.820	.940	.954
8	1.67	12.99	30.0	.091	.722	.750	.856	.920	.955
9	1.67	12.43	22.5	.708	.717	.764	.856	.954	1.042
10	1.67	9.66	15.0	.744	.752	.789	.871	.954	1.032
11	8.26	5.03	7.5	.624	.652	.713	.797	.914	1.088

8011241130 375 10

2	1.67	14.05	60.0	.578	.602	.621	.717	.803	.918
3	1.67	13.96	60.0	.532	.546	.577	.680	.814	.884
4	1.67	13.79	60.0	.540	.554	.591	.677	.764	1.059
5	1.67	13.74	52.5	.532	.537	.580	.700	.841	1.097
6	1.67	13.30	45.0	.000	.026	.070	.751	.833	.950
7	1.67	13.09	37.5	.041	.051	.708	.805	.925	1.014
8	1.67	12.97	30.0	.086	.705	.700	.858	.926	.983
9	1.67	12.39	22.5	.738	.752	.787	.868	.912	.945
10	1.67	9.71	15.0	.794	.794	.760	.852	.904	.975
11	8.26	5.05	7.5	.622	.637	.641	.800	.924	1.140

8011211416 760 10

2	3.56	12.17	59.9	.587	.618	.736	.912	1.151	1.205
3	3.56	11.94	59.9	.634	.642	.673	.899	1.096	1.158
4	3.56	11.84	59.9	.591	.605	.731	.847	1.115	1.132
5	3.56	12.02	52.4	.632	.657	.735	.818	1.064	1.090
6	3.56	12.11	44.9	.019	.665	.760	.842	1.061	1.226
7	3.56	12.39	37.4	.077	.700	.747	.933	1.148	1.221
8	3.56	12.67	29.9	.740	.768	.857	.968	1.365	1.418
9	4.06	12.81	22.4	.880	.894	.992	1.072	1.217	1.340
10	4.06	10.65	14.9	.820	.851	.891	1.026	1.121	1.262
11	4.49	7.44	7.4	.576	.591	.704	.791	.886	.948

8011211425 860 10

2	3.61	12.18	59.9	.600	.629	.739	.930	1.175	1.209
3	3.61	12.00	59.9	.632	.645	.694	.902	1.100	1.150
4	3.61	11.90	59.9	.601	.660	.716	.844	1.097	1.114
5	3.61	12.16	52.4	.659	.672	.732	.829	1.057	1.044
6	3.61	12.24	44.9	.061	.716	.776	.857	1.049	1.203
7	3.61	12.57	37.4	.075	.687	.764	.934	1.131	1.170
8	3.61	12.86	29.9	.760	.805	.846	.976	1.340	
9	3.61	12.98	22.4	.501	.499	.455	1.091	1.165	1.256
10	4.06	10.79	14.9	.850	.869	.940	1.030	1.172	1.274
11	4.49	7.64	7.4	.587	.597	.683	.791	.906	1.012

8011211445 860 10

2	3.41	14.54	59.9	.617	.653	.735	.814	.984	1.194
3	3.41	14.28	59.9	.541	.617	.685	.793	.944	1.105
4	3.41	14.22	59.9	.637	.655	.690	.869	1.015	1.260
5	3.41	14.28	52.4	.565	.597	.679	.876	.957	1.353
6	3.41	14.26	44.9	.001	.702	.755	.864	1.076	1.434
7	3.41	14.48	37.4	.065	.678	.721	.957	1.102	1.269
8	3.41	14.95	29.9	.719	.754	.878	1.089	1.183	1.400
9	4.43	13.98	22.4	.872	.874	.887	.970	1.079	1.217
10	4.92	11.40	14.9	.762	.780	.854	.893	1.018	1.107
11	5.02	7.73	7.4	.600	.621	.689	.808	.937	1.052

A011211456 260 4

2	1.52	7.68	59.9	.471	.487	.521	.611	.803	1.096	1.601
3	1.52	7.50	59.9	.534	.544	.582	.630	.874	1.258	1.300
4	1.54	7.40	59.9	.504	.513	.580	.643	.831	1.379	1.478
5	1.54	7.34	52.4	.476	.490	.538	.673	.916	1.145	1.257
6	1.54	7.27	44.9	.511	.516	.546	.633	.824	1.007	1.530
7	1.54	7.12	37.4	.522	.533	.570	.639	.951	1.366	1.406
8	1.54	7.40	24.9	.518	.532	.595	.701	1.014	1.403	1.535
9	1.54	7.14	22.4	.575	.581	.620	.620	.952	1.539	
10	1.56	6.74	14.9	.692	.730	.803	.983	1.104	1.152	
11	1.56	5.13	7.4	.710	.724	.754	.874	.953	1.051	1.115

B0112115 5 260 4

2	1.50	8.71	59.9	.452	.465	.541	.665	1.014	1.254	1.448
3	1.50	8.49	59.9	.490	.502	.520	.725	1.153	1.186	1.610
4	1.50	8.46	59.9	.486	.514	.537	.691	.951	1.275	1.323
5	1.50	8.44	52.4	.497	.506	.543	.700	.915	1.146	1.406
6	1.50	8.34	44.9	.494	.512	.535	.639	.886	1.213	
7	1.50	8.16	37.4	.500	.512	.526	.700	1.039	1.234	
8	1.50	8.36	29.9	.528	.542	.572	.717	1.185	1.241	
9	1.52	7.85	22.4	.575	.608	.664	.797	.993	1.336	
10	1.52	7.77	14.9	.733	.767	.818	.870	.994	1.057	
11	1.52	5.26	7.4	.712	.740	.811	.907	.989	1.047	1.119

A011211514 360 4

2	1.67	8.00	59.9	.461	.483	.532	.635	1.044	1.298
3	1.67	7.86	59.9	.472	.483	.500	.582	.834	1.215
4	1.67	7.77	59.9	.494	.511	.541	.601	.879	1.219
5	1.67	7.78	52.4	.477	.502	.561	.656	.845	1.228
6	1.67	7.69	44.9	.494	.501	.545	.654	.927	1.263
7	1.67	7.66	37.4	.522	.542	.575	.657	1.107	1.229
8	1.67	7.94	29.9	.546	.567	.625	.741	1.234	1.536
9	1.67	7.57	22.4	.641	.649	.721	.821	1.056	1.308
10	1.71	7.20	14.9	.799	.818	.878	.968	1.098	1.178
11	1.73	5.16	7.4	.700	.707	.770	.896	1.018	1.151

B011211523 360 4

2	1.66	8.30	59.9	.476	.494	.538	.635	1.007	1.253
3	1.66	8.10	59.9	.470	.480	.511	.578	.821	1.169
4	1.66	7.99	59.9	.516	.518	.544	.605	.805	1.204
5	1.66	7.98	52.4	.495	.527	.577	.686	.859	1.278
6	1.66	7.97	44.9	.484	.493	.578	.674	.946	1.250
7	1.66	7.91	37.4	.542	.557	.590	.640	1.079	1.192
8	1.66	8.23	29.9	.564	.592	.664	.754	1.177	1.505
9	1.66	7.87	22.4	.644	.688	.738	.805	1.065	1.255
10	1.67	7.47	14.9	.812	.832	.887	.945	1.104	1.143
11	1.67	5.20	7.4	.724	.732	.770	.893	.983	1.214

C:

Percent full

APPENDIX E  
CNOIDAL WAVE TESTS RESULTS

Wave height (cm)	Wave period (s)	Water depth (cm)	Relative crest height $\eta/H$
11.278	2.30	44.05	0.6260
11.583	2.80	44.05	0.6658
11.433	3.40	44.05	0.7526
11.468	4.10	44.05	0.7678
11.590	4.80	44.05	0.7767
11.464	5.70	44.05	0.8118
21.58	1.60	44.05	0.6330
21.67	2.00	44.05	0.6470
23.38	2.40	44.05	0.6895
23.52	2.40	44.05	0.7258
21.88	2.90	44.05	0.7724
23.104	3.39	44.05	0.7228
22.25	4.00	44.05	0.8036

## APPENDIX F

### ESTIMATING THE WAVE CREST ELEVATION AT BREAKING ON A PLANE SLOPE

The equations of Singamsetti and Wind (1980) can be used with the methods developed in this report to predict the crest elevation at the breaking point for monochromatic waves normally incident to a slope. This technique uses as input water depth,  $d$ , wave height,  $H$ , and period,  $T$ , in a constant depth region offshore of a plane slope,  $m$ . Deepwater wave height is found from

$$H'_o = \frac{H}{\sqrt{\tanh\left(\frac{2\pi d}{L_A}\right) \left[1.0 + \frac{4\pi d/L_A}{\sinh(4\pi d/L_A)}\right]}} \quad (F-1)$$

where  $L_A$  is the wavelength from Airy theory

$$L_A = \frac{gT^2}{2\pi} \tanh\left(\frac{2\pi d}{L_A}\right) \quad (F-2)$$

Breaking wave height may be determined from

$$H_b = H'_o 0.575(m)^{0.031} (H'_o/L_o)^{-0.254} \quad (F-3)$$

where  $L_o$  is deepwater Airy wavelength

$$L_o = \frac{gT^2}{2\pi} \quad (F-4)$$

Depth at breaking,  $d_b$ , is found from

$$d_b = H_b (1.067)(m)^{-0.155} (H'_o/L_o)^{0.130} \quad (F-5)$$

$$\text{for } \frac{1}{40} < m < \frac{1}{5}$$

$$d_b = H_b (1.761)(m)^{-0.107} (H'_o/L_o)^{0.237}$$

$$\text{for } m = \frac{1}{5}$$

The value of  $d_b/L_o$  is used to determine  $A_2$  and  $A_3$  from Figure 6 in the test of this report;  $A_1$  is determined from the value of the beach slope using Figure 7. Crest height at breaking,  $(n_c)_b$ , is then given by

$$(n_c)_b = \frac{A_1 H_b}{1.0 + \tanh\left[-A_2 \ln\left(A_3 \frac{H_b}{L_o}\right)\right]} \quad (F-6)$$

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2. Monochromatic waves.
3. Wave breaking.
4. Wave crests. I. Title. II. Ahrens, John P. III. Grosskopf, William G. IV. Coastal Engineering Research Center (U.S.).  
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