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NATIONAL DAM SAFETY PROGRAM. OTISCO LAKE DAM (INVENTORY NUMBER --ETC(U)
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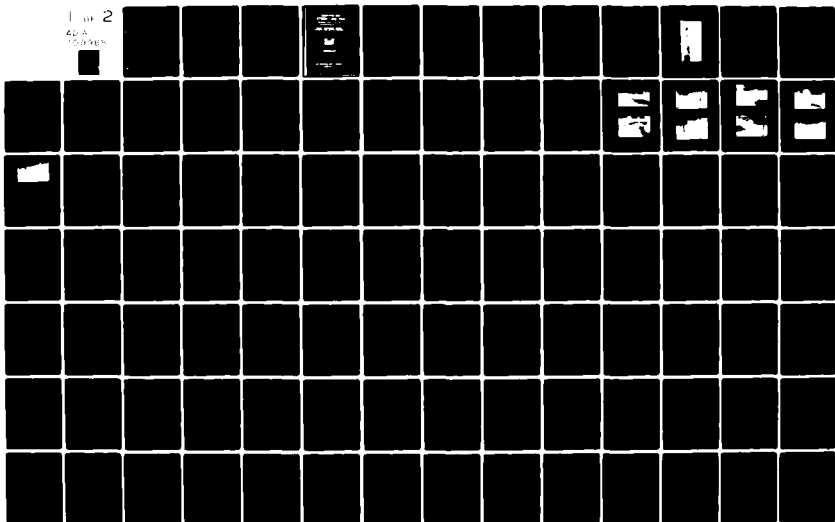
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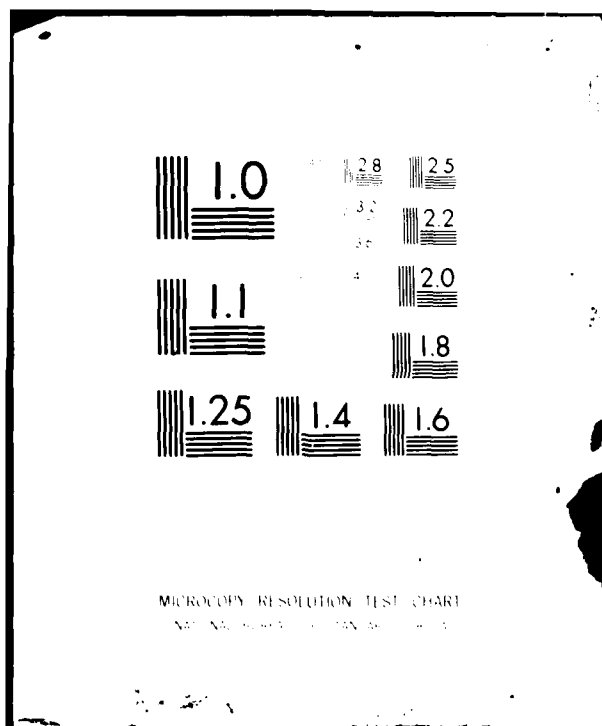
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20. ABSTRACT (Continue on reverse side if necessary and identify by block number) This report provides information and analysis on the physical condition of the dam as of the report date. Information and analysis are based on visual inspection of the dam by the performing organization. The examination of documents and the visual inspection of Otisco Lake Dam did not reveal conditions which constitute an immediate hazard to human life or property. However, the dam has some deficiencies which require further investigation and remedial action.		

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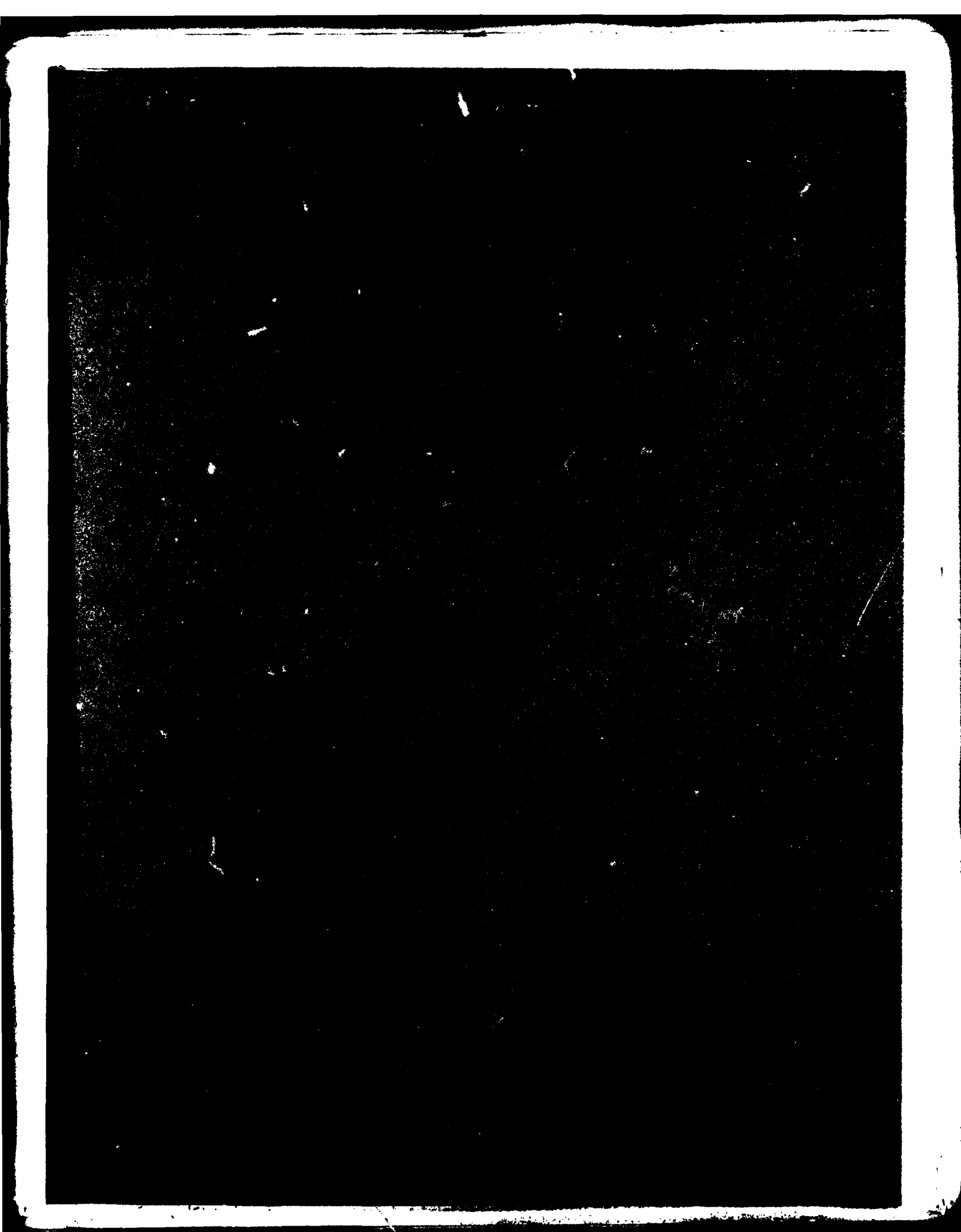
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Further investigations are required to assess the stability of the spillway section. Analysis performed indicated that the structure is only marginally stable under normal loading conditions and is unstable when subjected to severe loading conditions (such as flood flows or ice loading).

The spillway has sufficient capacity to discharge the Probable Maximum Flood (PMF) therefore, it has been assessed as "Adequate" according to the Corps of Engineers screening criteria.

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PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation, and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I Investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through frequent inspections can unsafe conditions be detected and only through continued care and maintenance can these conditions be prevented or corrected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aide in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

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PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM
OTISCO LAKE DAM
I.D. NO. NY-753
DEC #73B-2751A
OSWEGO RIVER BASIN
ONONDAGA COUNTY

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PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM

Name of Dam:	Otisco Lake Dam I.D. No. NY 753
State Located:	New York
County:	Onandaga
Watershed:	Oswego River Basin
Stream:	Ninemile Creek; tributary to Onandaga Lake
Date of Inspection:	June 12, 1981

ASSESSMENT:

The examination of documents and the visual inspection of Otisco Lake Dam did not reveal conditions which constitute an immediate hazard to human life or property. However, the dam has some deficiencies which require further investigation and remedial action.

Further investigations are required to assess the stability of the spillway section. Analysis performed indicated that the structure is only marginally stable under normal loading conditions and is unstable when subjected to severe loading conditions, (such as flood flows or ice loading).

The spillway has sufficient capacity to discharge the Probable Maximum Flood (PMF) therefore, it has been assessed as "Adequate" according to the Corps of Engineers screening criteria.

In addition, the dam has several problem areas which if left uncorrected, have the potential for the development of hazardous conditions and must be corrected within 1 year. These areas are:

1. Repair the deteriorated portions of the concrete on the downstream edges of the concrete paving.
2. Repair the undermining of the concrete paving.
3. Provide a program of periodic inspection and maintenance of the dam and appurtenances, Document this information for future reference.
4. An emergency action plan must be developed and maintained during the life of the structure.

DATE
P.D.

George Koch

George Koch
Chief, Dam Safety Section
New York State Department
of Environmental Conservation
NY License No. 45937

member
tribe

Reviewed by:

Richard E. Mayo, Jr., DFC
Col. W. M. Smith, Jr.
New York District Engineer

DATE
TIME
BY
LOCATION

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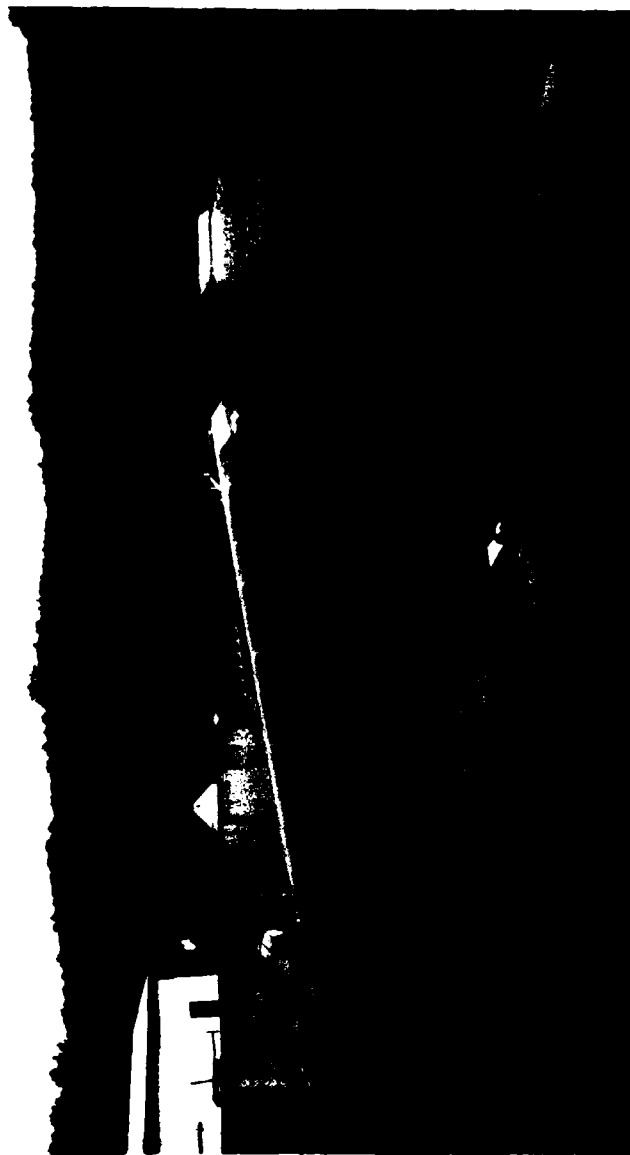
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OTISCO LAKE DAM OVERVIEW

PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM
OTISCO LAKE DAM I.D. NO. NY753
DEC # 73B-2751A OSWEGO RIVER BASIN
ONONDAGA COUNTY

SECTION 1: PROJECT INFORMATION

1.1 GENERAL

a. Authority

The Phase I inspection reported herein was authorized by the Department of the Army, New York District, Corps of Engineers, to fulfill the requirements of the National Dam Inspection Act, Public Law 92-367.

b. Purpose of Inspection

This inspection was conducted to evaluate the existing conditions of the dam, to identify deficiencies and hazardous conditions, to determine if these deficiencies constitute hazards to life and property, and to recommend remedial measures where required.

1.2 DESCRIPTION OF PROJECT

a. Description of Dam and Appurtenances

The Otisco Lake Dam is a 300 feet long earth dam which has an ogee masonry/concrete primary spillway and a concrete paved secondary spillway abutted by reinforced concrete training walls. The masonry/concrete spillway is 18 feet high and has 55 feet of ogee weir length. The secondary spillway has a total length of 336 feet of uncontrolled overflow section. The secondary spillway channel within the training walls is directed into the natural channel by the training walls. There is a concrete core wall located under the overflow section and around the abutment walls. There are three 4' x 4' low level outlets located adjacent to the primary spillway section.

b. Location

The dam is located on Ninemile Creek, which is tributary to Onondaga Lake, Seneca River and finally the Oswego River. The village of Marietta, New York is downstream of the dam, within one mile.

c. Size

The dam is 18 feet high and impounds 26,000 acre feet at normal pool elevation. The dam is classified as "intermediate" in size.

d. Hazard Classification

The dam is classified as high hazard due to its location, above several low lying homes in the area between the dam and the village of Marietta, New York.

e. Ownership

The dam is owned by Onondaga County Water Authority, P.O. Box 9, Northern

Concourse, Syracuse, New York 13211. The person responsible for operation and maintenance, who was contacted to make the inspection of the dam was Mr. Joseph DeVoldre, (315) 455-7061.

f. Purpose of Dam

The dam was built to increase storage for water supply.

g. Design and Construction History

In 1857, a feeder dam for the Erie Canal was constructed at the site of the present dam. By 1872 the dam had been raised another ten feet for more storage for canal use. In 1907, a new dam of masonry construction (which now makes up the lower portion of the primary spillway) was completed adding another four feet of storage and providing a domestic water source. The dam as it now exists was completed in 1962.

h. Normal Operating Conditions

All flows in excess of the Onondaga County Water Authority requirements are passed over the uncontrolled spillway. The three low level outlets are operable and in good working condition.

1.3 PERTINENT DATA

a. Drainage Area (sq.mi.)

39.15

b. Elevations (ft., USGS datum)

Top of Dam	798.1
Secondary Spillway	790.1
Primary Spillway	786.6
Low Level Outlets	768.6
Original Stream Channel	768. ±

c. Reservoir

Surface Area @ spillway crest (acres)	2291.
Storage @ Top of dam (acre-feet)	47,890.
Storage @ Spillway Crest (acre-feet)	26,067.

d. Dam

Type: Earth fill with reinforced concrete paving on the downstream slope and concrete core wall, an earth embankment makes up the right portion of the dam.

Length (ft)

Concrete paved overflow section	336.
Earth embankment	300.

Height (ft)

Upstream Slope, Earth embankment	3H:1V
Downstream Slope, Earth Embankment	3H:1V
Crest Width, Earth Embankment	10.

e. Spillway

Type: Masonry/concrete ogee section

Weir length (ft) 55.

Spillway Capacity (cfs)

@ Secondary Spillway Crest 1,369.

@ Top of Dam 31,345.

f. Reservoir Drain

Type: Three - 4' x 4' sluiceways with manually controlled gate.

Capacity @ Normal Pool Elevation (cfs) 750.

SECTION 2: ENGINEERING DATA

2.1 GEOTECHNICAL DATA

a. Geology

The Otisco Lake Dam is located in the "Alleghany Plateau" physiographic province of New York State. The hills are smoothly sloping and generally well drained. The deep soils of the area are of the Honeoye-Lima Association.

b. Subsurface Investigation

No information about foundation conditions or borings could be found other than some description from the 1958 Dam Reconstruction Application. This stated the character of the stream bed and banks as "silty sand with traces of gravel, compact sand and gravel, and silty clay in various parts".

c. Design Records

There are no design records for the original dam constructed at the site. The only records available on the most recent reconstruction are plans which are included in App.F. Drawings or can be obtained from O'Brien and Gere, Engineers at 1304 Buckley Road, Syracuse, N.Y. (315) 451-4700.

2.2 CONSTRUCTION RECORDS

There are no construction records available for the original construction or reconstruction of Otisco Lake Dam.

2.3 OPERATION RECORDS

Operating records are available at the water station located adjacent to the dam. Any other data available regarding water levels, usage, or quality can be obtained from Mr. Joseph DeVoldre, Water Plant Manager, (315) 455-7061.

2.4 EVALUATION OF DATA

The data presented in this report is compiled from information contained in the files of Department of Environmental Conservation, drawings from O'Brien and Gere, Engineers, and data gained from the visual inspection. This information appears to be adequate and reliable for Phase I Inspection purposes.

SECTION 3: VISUAL INSPECTION

3.1 FINDINGS

a. General

Visual inspection of the Otisco Lake Dam and surrounding watershed was conducted on June 12, 1981. The weather was cloudy and the temperature ranged in the seventies. The reservoir water surface was at the primary spillway crest.

b. Dam

Both the earth embankments and the concrete paved section of the embankment appear to be in very good condition and well maintained. The vertical and horizontal alignments of the embankments are good. Some minor deterioration of concrete and minor undermining were found at the downstream edge of the secondary spillway (Photos 6 & 7). There is a roadway that passes between the spillway training wall and the right earth embankment. (See Photo #5). The road elevation is approximately 2.5 feet above the secondary spillway crest, therefore, under extreme flooding conditions, it would be possible for flow to pass over the dam on the roadway before overtopping occurs. There are stop logs available at the dam to block off the road, preventing erosion and hazard due to extreme floods. However, there is no emergency action plan detailing how or when to implement a plan.

c. Seepage

There was no seepage or sloughing found at the toe or any of the slopes.

d. Spillway

The overflow spillway is in good condition. Both the masonry and concrete are well maintained. The secondary spillway concrete paving is also in good condition.

e. Reservoir Drain

There are three 4' x 4' sluiceways located adjacent to the primary spillway. The manually operated valves are in good working condition, located at the secondary spillway elevation. A 36 inch and 24 inch pipe are also available to draw off water into the supply system.

f. Reservoir

Originally a naturally occurring lake, the area around it appears very stable. Sedimentation is not a problem with the dam at present.

3.2 EVALUATION OF OBSERVATIONS

The only deficiencies that could be found with the visual inspection was the small amount of concrete deterioration and slight undermining of the concrete paving on the left side of the spillway channel.

SECTION 4: OPERATION AND MAINTENANCE PROCEDURES

4.1 PROCEDURES

The normal water surface is approximated by the uncontrolled overflow primary spillway. The other draw from the reservoir is through a 36 inch or 24 inch pipeline to the Onondaga County Water Authority distribution system.

4.2 MAINTENANCE OF THE DAM

The dam is maintained by the Onondaga County Water Authority. Maintenance is considered satisfactory. All valves and gates are operated and lubricated annually.

4.3 WARNING SYSTEM

There is no warning system in effect.

4.4 EVALUATION

The dam and appurtenances have been maintained in a satisfactory condition, except for the minor maintenance noted in this report.

SECTION 5: HYDROLOGIC/HYDRAULIC

5.1 DRAINAGE AREA CHARACTERISTICS

Otisco Lake Dam is located about 3/4 mile southeast of the Village of Marietta. The lake is fed at its upper end by Spafford Creek, while downstream of the dam, the excess flow is discharged into Ninemile Creek. The total drainage area of the basin is 39.15 square miles and the surface area of the lake at normal pool is 3.71 square miles. The basin drains generally in a northerly direction. Its slope ranges from moderate to steep and, except for some marshy areas south of the lake, is fairly well drained. It was analysed as a single basin.

5.2 ANALYSIS CRITERIA

The analysis of the spillway capacity of the dam and storage of the reservoir was performed using the Corps of Engineers HEC-1 computer program incorporating the "Snyder Synthetic Unit Hydrograph" method and the "Modified Puls" flood routing procedure. The floods selected for analysis were the PMF and 1/2 the PMF in accordance with the recommended guidelines of the Corps of Engineers.

5.3 SPILLWAY CAPACITY

The spillway has a capacity of 31,345 cfs at the top of the dam. An inflow of 17,706 cfs generated by a storm equal to 1/2 the PMF will produce a maximum outflow of 8901 cfs. An inflow of 35,412 cfs resulting from the PMF will produce a maximum outflow of 22,719 cfs which is well below the spillway capacity of 31,345 cfs at top of dam.

5.4 RESERVOIR CAPACITY

The reservoir capacity to normal pool elevation is 26,067 acre-feet. Surge storage to top of dam is an additional 21,823 acre-feet, creating a total storage of 47,890 acre-feet. The surge storage between spillway and dam crests is equivalent to 10.5 inches of runoff.

5.5 FLOODS OF RECORD

The maximum known flood of record in Ninemile Creek occurred on June 23, 1972 at a point 1.8 miles downstream from Otisco Lake Dam. Ratiod by drainage area the estimated inflow at Otisco Lake Dam was 913 cfs. The resulting depth of flow over the spillway crest would be about 2.2 feet.

5.6 OVERTOPPING POTENTIAL

The PMF analysis indicates that the dam will not be overtopped by a storm equal to 1/2 the PMF nor by the PMF.

5.7 EVALUATION

The spillway is adequate to pass the flows produced by 1/2 the PMF as well as the PMF without overtopping of the dam. The structure is, therefore, assessed as "Adequate".

SECTION 6: STRUCTURAL STABILITY

6.1 EVALUATION OF STRUCTURAL STABILITY

a. Visual Observation

Both the earth embankment and the masonry/concrete spillway section appeared stable. No sloughing or subsidence was found on the dam.

b. Design and Construction Data

No information regarding structural stability of the dam or spillway section was located.

c. Operating Records

Any data or information on operations of the Otisco Lake dam can be found at the pumping station located at the dam.

d. Post Construction Changes

The dam as it exists is a reconstruction of a masonry dam that was built in 1907. The overflow spillway was raised and the embankments were paved with concrete.

6.2 STRUCTURAL STABILITY ANALYSIS

A structural stability analysis was conducted for the masonry/concrete gravity spillway portion of the dam. The results of the analysis are as follows:

<u>Case</u>	<u>Overturning Safety Factor</u>	<u>Resultant in Middle Third</u>	<u>Sliding Safety Factor</u>
a. Normal conditions; water surface at spillway crest	1.73	Yes	1.03
b. Case a. plus ice load of 5,000 lb/ft	1.30	No	0.76
c. 1/2 PMF flows; water surface 6.4 ft above spillway crest	1.33	No	0.64
d. PMF flows; water surface 9.6 feet above spillway crest	1.19	No	0.54
e. Seismic loading; water surface at spillway crest	1.66	Yes	0.73

The analysis indicates that this portion of the dam is marginally stable under normal loading conditions and would be unstable under severe loading conditions (ice loading, flood flows). The analysis was based on available information and was done in accordance with Corps of Engineers "Recommended Guidelines", assuming full uplift pressure under the upstream toe decreasing to tailwater pressure under the downstream toe.

Further investigations are required to better assess the stability of the spillway section. Subsurface explorations and concrete cores are required to obtain information about the uplift forces acting on the dam. An accurate cross section of the spillway should be developed for the analysis. A revised stability analysis should then be performed using this data. Based on the results of these analyses, the need for modifications to the structure should be determined.

This structure is located in Seismic Zone 2. A seismic stability analysis was performed assuming a seismic coefficient of 0.1. The results of this analysis (shown on page 8) indicate that the safety factor against sliding fall below 1.0 when seismic considerations are included.

SECTION 7: ASSESSMENT/RECOMMENDATIONS

7.1 ASSESSMENT

a. Safety

The Phase I Inspection of Otisco Lake Dam revealed that the spillway is adequate to pass the Probable Maximum Flood according to the Corps of Engineers screening criteria.

The inspection also revealed that the stability of this structure is questionable. Analysis performed indicated that the structure is only marginally stable under normal loading conditions and is unstable when subjected to severe loading conditions (such as flood flows or ice loading).

b. Adequacy of Information

The information which was available for the preparation of this report presented a fairly complete history of the structure. Final plans of the most recent modification did not reveal the actual configuration of the foundation of the spillway. Therefore, some assumptions had to be made on the section of the spillway. Overall, the information is considered adequate for Phase I Inspection purposes.

c. Need for Additional Investigations

Further investigations are required to assess the stability of the spillway section. Subsurface explorations and concrete cores are required to obtain information about the uplift forces acting on the dam. An accurate cross section of the spillway should be developed for the analysis. A revised stability analysis should then be performed using this data. Based on the results of these analyses, the need for modifications to the structure should be determined.

d. Urgency

The additional stability investigation must be initiated within 6 months from the date of notification. Within 1 year of notification, remedial measures as a result of these investigations must be initiated, with completion of the measures during the following year. In the interim, develop an emergency action plan for the notification of downstream residents and proper governmental authorities in the event of overtopping and provide round-the-clock surveillance of the dam during periods of extreme run-off.

7.2 RECOMMENDED MEASURES

1. The results of the stability investigation will determine the appropriate remedial actions for the spillway section.
2. Repair the deterioration of the concrete on the downstream edges of the concrete pavement.
3. Repair the undermining of the concrete paving.
4. Provide a program of periodic inspection and maintenance of the dam and appurtenances. Document this information for future reference.
5. An emergency action plan must be developed and maintained during the life of the structure.

APPENDIX A
PHOTOGRAPHS

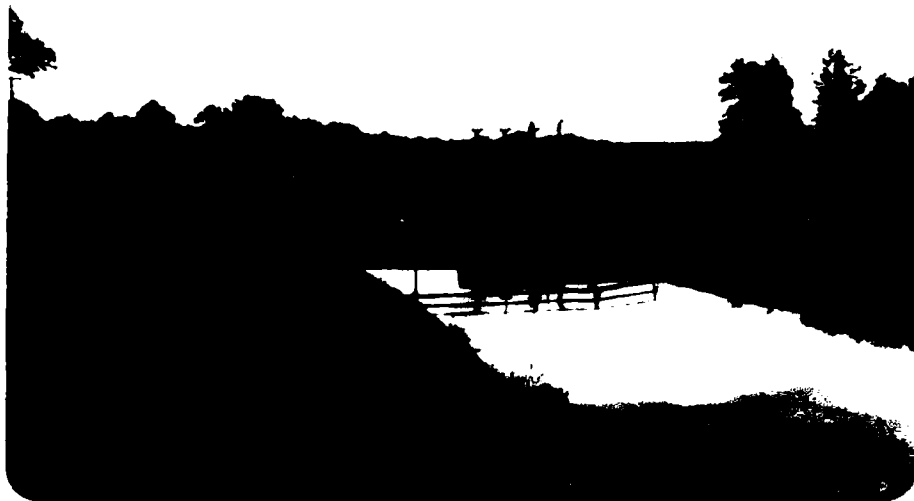


PHOTO #2 DOWNSTREAM VIEW OF DAM



PHOTO # 3 SPILLWAY CREST



PHOTO # 4 RESERVOIR DRAINS, VALVE CONTROLS



PHOTO # 5. ROADWAY LOCATED BETWEEN SPILLWAY ABUTMENT
AND LEFT EARTH EMBANKMENT

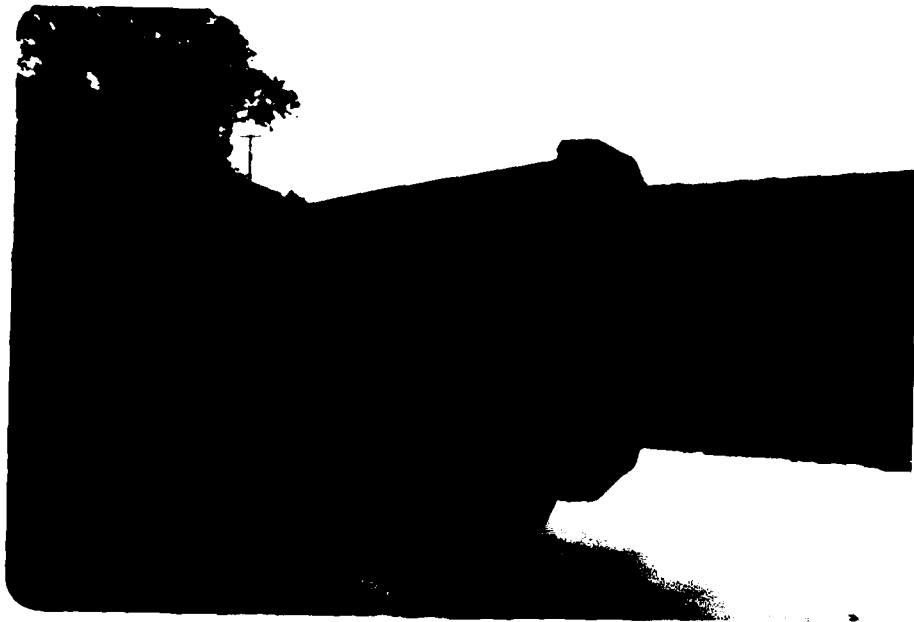


PHOTO # 6 MINOR DETERIORATION OF CONCRETE



PHOTO # 7 MINOR UNDERMINING OF CONCRETE PAVEMENT



PHOTO # 8 DOWNSTREAM VIEW OF SPILLWAY
AND NORMAL TAILWATER



PHOTO # 9 CONCRETE PAVING ON LEFT EMBANKMENT



PHOTO # 10 DOWNSTREAM CHANNEL

APPENDIX B
VISUAL INSPECTION CHECKLIST

VISUAL INSPECTION CHECKLIST1) Basic Data

a. General

Name of Dam OTISCO LAKE DAM
Fed. I.D. # 753 DEC Dam No. 73B-2751
River Basin OSWEGO RIVER BASIN
Location: Town MARCELLUS County ONONDAGA
Stream Name NINE MILE CREEK
Tributary of ONONDAGA
Latitude (N) 42° 54.3' Longitude (W) 76° 18.8'
Type of Dam EARTH w/ concrete paving
Hazard Category high
Date(s) of Inspection JUNE 12, 1981
Weather Conditions cloudy, 70's
Reservoir Level at Time of Inspection AT PRIMARY SPILLWAY CREST

b. Inspection Personnel KEN HARMER, JAMIE VEITCH

c. Persons Contacted (Including Address & Phone No.)

MR. JOSEPH J. DeVoldre
WATER PLANT MANAGER, ONONDAGA CITY WATER AUTHORITY
P.O. BOX 9, NORTHERN CONCOURSE, SYRACUSE NY 13211
(315) 455-7061

d. History:

Date Constructed 1857 Date(s) Reconstructed 1872
1907
Designer (1962) O'Brien & Gere Engrs. Syracuse NY 1962
Constructed By ~ UNKNOWN ~
Owner ONONDAGA COUNTY WATER AUTHORITY

2) Embankment

a. Characteristics

- (1) Embankment Material EARTH EMBANKMENTS, CONCRETE
PAVED EARTH ABUTMENTS FORMING AUX. SPILLWAY
- (2) Cutoff Type CONCRETE
- (3) Impervious Core —
- (4) Internal Drainage System PIPED TO OUTLET CHANNEL - NO
FLOW @ TIME OF INSPECTION
- (5) Miscellaneous —

b. Crest

- (1) Vertical Alignment GOOD
- (2) Horizontal Alignment GOOD
- (3) Surface Cracks NONE
- (4) Miscellaneous —

c. Upstream Slope

- (1) Slope (Estimate) (V:H) 1:3 EARTH PORTION
- (2) Undesirable Growth or Debris, Animal Burrows NONE
- (3) Sloughing, Subsidence or Depressions NONE

(4) Slope Protection Vegetation

(5) Surface Cracks or Movement at Toe None

d. Downstream Slope

(1) Slope (Estimate - V:H) 1:3

(2) Undesirable Growth or Debris, Animal Burrows None

(3) Sloughing, Subsidence or Depressions None

(4) Surface Cracks or Movement at Toe None

(5) Seepage None

(6) External Drainage System (Ditches, Trenches; Blanket) NOT
Flowing

(7) Condition Around Outlet Structure excellent

(8) Seepage Beyond Toe None

e. Abutments - Embankment Contact

good no signs of erosion, seepage

(1) Erosion at Contact NONE

(2) Seepage Along Contact NONE

3) Drainage System

a. Description of System

b. Condition of System good

c. Discharge from Drainage System NONE

4) Instrumentation (Monumentation/Surveys, Observation Wells, Weirs, Piezometers, Etc.)

NONE

93-15-3(9/80)

5) Reservoir

- a. Slopes STABLE
- b. Sedimentation NOT A PROBLEM AROUND DAM
- c. Unusual Conditions Which Affect Dam NONE

6) Area Downstream of Dam

- a. Downstream Hazard (No. of Homes, Highways, etc.) Highway runs between embankment and abutment
- b. Seepage, Unusual Growth NONE
- c. Evidence of Movement Beyond Toe of Dam NONE
- d. Condition of Downstream Channel GOOD

7) Spillway(s) (Including Discharge Conveyance Channel)

- a. General masonry/concrete ogee section
- b. Condition of Service Spillway good condition

c. Condition of Auxiliary Spillway concrete pavement - very
good condition

d. Condition of Discharge Conveyance Channel good

8) Reservoir Drain/Outlet

Type: Pipe _____ Conduit _____ Other sluiceway

Material: Concrete ☒ Metal _____ Other _____

Size: 4' x 4' Length 17'

Invert Elevations: Entrance _____ Exit _____

Physical Condition (Describe): good Unobservable _____

Material: concrete

Joints: could not see Alignment _____

Structural Integrity: apparently sound

Hydraulic Capability: total 750 cfs at normal pool

Means of Control: Gate ☒ Valve _____ Uncontrolled _____

Operation: Operable ☒ Inoperable _____ Other _____

Present Condition (Describe): good

9) Structural

- a. Concrete Surfaces good
- b. Structural Cracking None
- c. Movement - Horizontal & Vertical Alignment (Settlement) None
- d. Junctions with Abutments or Embankments good
- e. Drains - Foundation, Joint, Face Not Flowing
- f. Water Passages, Conduits, Sluices good condition
- g. Seepage or Leakage NONE FOUND.

h. Joints - Construction, etc.

GOOD

i. Foundation APPARENTLY GOOD - NO SIGNS

SETTLEMENT OR MOVEMENT

j. Abutments GOOD

k. Control Gates OPERABLE

l. Approach & Outlet Channels GOOD

m. Energy Dissipators (Plunge Pool, etc.) GOOD CONDITION

n. Intake Structures GOOD

o. Stability GOOD

p. Miscellaneous WELL MAINTAINED STRUCTURE

10) Appurtenant Structures (Power House, Lock, Gatehouse, Other)a. Description and Condition PUMP STATION LOCATEDIMMEDIATELY DOWNSTREAM WELL
MAINTAINED.11) Operation Procedures (Lake Level Regulation):NO SET PROCEDURES, EXCEPT TO DRAW
WATER SUPPLY AND KEEP LAKE LEVEL
AT CREST.

APPENDIX C
HYDROLOGIC/HYDRAULIC
ENGINEERING DATA AND COMPUTATIONS

Otisco Lake

1 of 3

Drainage area = 39.15 mi²
(Planimetric from Aerial) = 25,056 acres

Lake Area (@ El. 788) = 3.71 mi²
(Planimetric from Aerial) = 2374 acres

Shoreline = 13.4 mi. (Gazetteer)
Max. length of Lake = 5.75 " } Feasibility Report for
Max. width " = 0.75 " } Oswego River Watershed,
Max. depth " = 70 feet } May 1978.
Normal pool elev. = 786.6" (From Plans as revised)
1-5-81

Stage - Capacity (From Fig. A-30, Feasibility Report May 1978)

<u>Elev.</u>	<u>Vol. (Acre-feet)</u>
722	0
780	13,600
785	23,000
788	28,750
790	32,500
795	42,000

otisco LakePrecipitation : $\leq \text{PMP} = 21.0''$ (H.M. No. 33)

<u>DUR.</u>	<u>6</u>	<u>12</u>	<u>24</u>	<u>48</u>
%	94	108	118	126

Drainage area = 39.15 mi^2

$$L = 12.5 \times \frac{62,500}{12 \times 5280} = 12.33 \text{ mi.}$$

$$L_{ca} = 4.8 \times \frac{62,500}{12 \times 5280} = 4.73 \text{ mi.}$$

Assume $C_t = 2.2$ $C_p = 0.625$

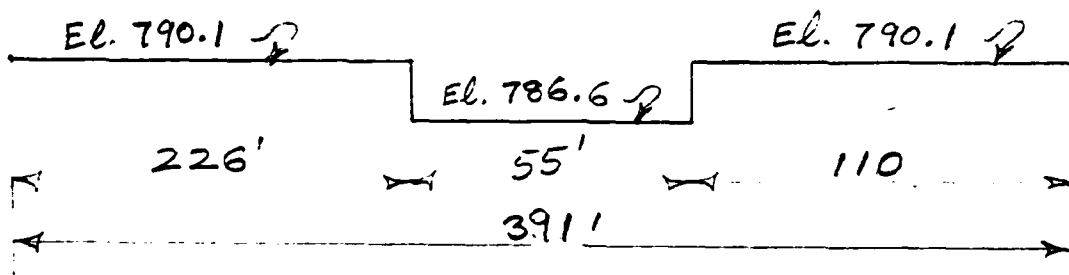
$$\begin{aligned} t_p &= C_t (L \times L_{ca})^{0.3} \\ &= 2.2 (12.33 \times 4.73)^{0.3} = 7.45 \text{ hr.} \end{aligned}$$

$$\begin{aligned} t_+ &= \frac{t_p}{5.5} = \frac{7.45}{5.5} = 1.35 \text{ hrs} \\ &= 1 \text{ hr } 21 \text{ min.} \quad \text{Use } \underline{1 \text{ hr } 30 \text{ min.}} \end{aligned}$$

$$\begin{aligned} T_p &= t_p + 0.25(t_R - t_+) \\ &= 7.45 + 0.25(1.50 - 1.35) \\ &= 7.45 + 0.04 = 7.49 \text{ hr.} \end{aligned}$$

Piseco Lake

3 of 3



Total spillway Length = 391'

Spillway crest elev. = 786.6 (From Plans as revised 1-5-81)

Assume $C = 3.8$ (Ogee Section)

$C = 3.4$ (Dam)

EL	L_1	C_1	H_1	L_2	C_2	H_2	Q_1	Q_2	Q
786.6	55	3.8	0	-	-	-	0	-	0
788	55	3.8	1.4	-	-	-	347	-	347
789	55	3.8	2.4	-	-	-	777	-	777
790.1	55	3.8	3.5	-	-	-	1369	-	1369
791	55	3.8	4.4	336	3.4	0.9	1929	975	2904
792	55	3.8	5.4	336	3.4	1.9	2623	2992	5615
793	55	3.8	6.4	336	3.4	2.9	3384	5642	9026
794	55	3.8	7.4	336	3.4	3.9	4207	8799	13006
795	55	3.8	8.4	336	3.4	4.9	5088	12391	17479

CHECK LIST FOR DAMS
HYDROLOGIC AND HYDRAULIC
ENGINEERING DATA

1

AREA-CAPACITY DATA:

	<u>Elevation</u> (ft.)	<u>Surface Area</u> (acres)	<u>Storage Capacity</u> (acre-ft.)
1) Top of Dam	<u>798.1</u>	<u>2973</u>	<u>47890</u>
2) Design High Water (Max. Design Pool)	<u>-</u>	<u>-</u>	<u>-</u>
3) Auxiliary Spillway Crest	<u>790.1</u>	<u>-</u>	<u>-</u>
4) Pool Level with Flashboards	<u>-</u>	<u>-</u>	<u>-</u>
5) Service Spillway Crest	<u>786.6</u>	<u>2291</u>	<u>26067</u>

DISCHARGES

	<u>Volume</u> (cfs)
1) Average Daily	<u>-</u>
2) Spillway @ Maximum High Water	<u>31345</u>
3) Spillway @ Design High Water	<u>-</u>
4) Spillway @ Auxiliary Spillway Crest Elevation	<u>-</u>
5) Low Level Outlet	<u>750</u>
6) Total (of all facilities) @ Maximum High Water	<u>32096</u>
7) Maximum Known Flood	<u>913</u>
8) At Time of Inspection	<u>0</u>

CREST:

ELEVATION: 798.1Type: Masonry, broad crestedWidth: _____ Length: 336Spillover —Location —

SPILLWAY:

SERVICE

AUXILIARY

786.6 Elevation NoneOgee Type —— Width —Type of Control✓ Uncontrolled —

Controlled:

— Type —
(Flashboards; gate)— Number —— Size/Length —— Invert Material —— Anticipated Length
of operating service —— Chute Length —— Height Between Spillway Crest
& Approach Channel Invert
(Weir Flow) —

HYDROMETEROLOGICAL GAGES:

Type : Water-Stage recorder # 04240180Location: Ninemile Creek, 1.8 mi downstream from
Otisco Lake Dam.

Records:

Date - June 23, 1972Max. Reading - 1030 cfs (gage height-8.65 ft.)

FLOOD WATER CONTROL SYSTEM:

Warning System: _____

Method of Controlled Releases (mechanisms):

DRAINAGE AREA: 39.15 Sq. mi

DRAINAGE BASIN RUNOFF CHARACTERISTICS:

Land Use - Type: Woods, open fields, substantial residential devel.

Terrain - Relief: Moderate to steep slopes, generally well drained.

Surface - Soil: High to medium-fine, moderately well drained, soils of glacial till origin.

Runoff Potential (existing or planned extensive alterations to existing (surface or subsurface conditions)

No alterations planned or anticipated

Potential Sedimentation problem areas (natural or man-made; present or future)

Potential Backwater problem areas for levels at maximum storage capacity including surcharge storage:

Significant number of homes and cottages along and/or near the lake shore.

Dikes - Floodwalls (overflow & non-overflow) - Low reaches along the Reservoir perimeter:

Location: _____

Elevation: _____

Reservoir:

Length @ Maximum Pool 5.75 (Miles)

Length of Shoreline (@ Spillway Crest) 13.4 (Miles)

PREVIEW OF SEQUENCE OF STREAM NETWORK CALCULATIONS

RUNOFF HYDROGRAPH AT 1

ROUTE HYDROGRAPH TO 1

END OF NETWORK

```

*****
FLOOD HYDROGRAPH PACKAGE (HEC-1)
DAM SAFETY VERSION JULY 1978
LAST MODIFICATION 26 FEB 79
MODIFIED FOR HONEYWELL APR 79
*****
NEW YORK STATE
DEPT OF ENVIRONMENTAL CONSERVATION
FLOOD PROTECTION BUREAU
*****

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RUN DATE 07/21/81
OTISCO LAKE
PHASE 1
PMF

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```

JOB SPECIFICATION
NQ      MHR      NMIN      IDAY      IHR      IMIN      METRC      IPLT      IPRT      NSTAN
200      1          30          0          0          0          0          0          0          0
JOPER      5          0          0          0          0          0          0          0          0

```

```

MULTI-PLAN ANALYSES TO BE PERFORMED
NPLAN= 1 NRTIO= 6 LRTIO= 1
RTIOS= 0.20 0.40 0.50 0.60 0.80 1.00

```

```

*****
SUB-AREA RUNOFF COMPUTATION
*****

```

```

INFLOW FROM BASIN
ISTAQ      ICOMP      IECON      ITAPE      JPLT      JPRT      INAME      ISTAGE      IAUTO
1          0          0          0          2          2          1          0          0

```

```

HYDROGRAPH DATA
IMYDG      IUHG      TAQEA      SNAP      TRSDA      TRSPC      RATIO      ISHOW      ISAME      LOCAL
1          1      39.15      0.      39.15      0.      0.      0          0          0

```

```

PRECIP DATA
SPEF      PMS      R6      R12      R24      R48      R72      R96
0.      21.00      94.00      108.00      118.00      126.00      0.      0.

```

TRSPC COMPUTED BY THE PROGRAM IS 0.843

```

LOSS DATA
LROPT      STRKR      DLTKR      RTIOL      ERAIN      STRKS      RTIOK      STRTL      CNSTL      ALSMX      RTIMP
0          0          0.      1.00      0.      0.      1.00      1.00      0.10      0.      0.

```

```

UNIT HYDROGRAPH DATA
TP= 7.49 CP=0.63 NTA= 0

```

```

RECESSION DATA
START2= -2.00 GRCSN= -0.10 RTICR= 2.50
APPROXIMATE CLARK COEFFICIENTS FROM GIVEN SNYDER CP AND TP ARE TC= 5.79 AND R= 4.47 INTERVALS
EXP UNDERFLO AT LOCATION 230246
EXP UNDERFLO AT LOCATION 230246

```

```

UNIT HYDROGRAPH 27 END-OF-PERIOD ORDINATES, LAG= 7.45 HOURS, CP= 0.62 VOL= 1.00
172.      623.      1218.      1769.      2094.      2085.      1786.      1427.      1140.      910.
727.      581.      464.      370.      296.      236.      189.      151.      120.      96.
77.      61.      49.      39.      31.      25.      20.

```

```

END-OF-PERIOD FLOW
MO.DA      HR.MH      PERIOD      RAIN      EXCS      LOSS      COMP      Q      MO.DA      HR.MH      PERIOD      RAIN      EXCS      LOSS      COMP      Q
0

```

1.01	1.30	1	0.01	0.01	71.	1.07	7.30	101	0.	0.	16.
1.01	3.00	2	0.01	0.01	65.	1.07	9.00	102	0.	0.	14.
1.01	4.30	3	0.01	0.01	59.	1.07	10.30	103	0.	0.	13.
1.01	6.00	4	0.01	0.	54.	1.07	12.00	104	0.	0.	12.
1.01	7.30	5	0.04	0.	50.	1.07	13.30	105	0.	0.	11.
1.01	9.00	6	0.04	0.	45.	1.07	15.00	106	0.	0.	10.
1.01	10.30	7	0.04	0.	41.	1.07	16.30	107	0.	0.	9.
1.01	12.00	8	0.04	0.	38.	1.07	18.00	108	0.	0.	8.
1.01	13.30	9	0.29	0.	34.	1.07	19.30	109	0.	0.	7.
1.01	15.00	10	0.52	0.08	45.	1.07	21.00	110	0.	0.	6.
1.01	16.30	11	0.24	0.09	94.	1.07	22.30	111	0.	0.	5.
1.01	18.00	12	0.00	0.00	178.	1.08	0.	112	0.	0.	4.
1.01	19.30	13	0.02	0.	272.	1.08	1.30	113	0.	0.	3.
1.01	21.00	14	0.02	0.	344.	1.08	3.00	114	0.	0.	2.
1.01	22.30	15	0.02	0.	369.	1.08	4.30	115	0.	0.	1.
1.02	0.	16	0.02	0.	343.	1.08	6.00	116	0.	0.	0.
1.02	1.30	17	0.18	0.03	291.	1.08	7.30	117	0.	0.	0.
1.02	3.00	18	0.18	0.03	252.	1.08	9.00	118	0.	0.	0.
1.02	4.30	19	0.18	0.03	240.	1.08	10.30	119	0.	0.	0.
1.02	6.00	20	0.18	0.03	252.	1.08	12.00	120	0.	0.	0.
1.02	7.30	21	0.52	0.47	356.	1.08	13.30	121	0.	0.	0.
1.02	9.00	22	0.62	0.47	665.	1.08	15.00	122	0.	0.	0.
1.02	10.30	23	0.62	0.47	1234.	1.08	16.30	123	0.	0.	0.
1.02	12.00	24	0.62	0.47	2040.	1.08	18.00	124	0.	0.	0.
1.02	13.30	25	4.33	4.18	3623.	1.08	19.30	125	0.	0.	0.
1.02	15.00	26	8.82	8.67	7643.	1.08	21.00	126	0.	0.	0.
1.02	16.30	27	3.49	3.34	14847.	1.08	22.30	127	0.	0.	0.
1.02	18.00	28	0.30	0.00	23624.	1.09	0.	128	0.	0.	0.
1.02	19.30	29	0.27	0.12	31290.	1.09	1.30	129	0.	0.	0.
1.02	21.00	30	0.27	0.12	35412.	1.09	3.00	130	0.	0.	0.
1.02	22.30	31	0.27	0.12	34804.	1.09	4.30	131	0.	0.	0.
1.03	0.	32	0.27	0.12	30482.	1.09	6.00	132	0.	0.	0.
1.03	1.30	33	0.	0.	25062.	1.09	7.30	133	0.	0.	0.
1.03	3.00	34	0.	0.	20318.	1.09	9.00	134	0.	0.	0.
1.03	4.30	35	0.	0.	16460.	1.09	10.30	135	0.	0.	0.
1.03	6.00	36	0.	0.	13287.	1.09	12.00	136	0.	0.	0.
1.03	7.30	37	0.	0.	10673.	1.09	13.30	137	0.	0.	0.
1.03	9.00	38	0.	0.	8537.	1.09	15.00	138	0.	0.	0.
1.03	10.30	39	0.	0.	6819.	1.09	16.30	139	0.	0.	0.
1.03	12.00	40	0.	0.	5446.	1.09	18.00	140	0.	0.	0.
1.03	13.30	41	0.	0.	4350.	1.09	19.30	141	0.	0.	0.
1.03	15.00	42	0.	0.	3518.	1.09	21.00	142	0.	0.	0.
1.03	16.30	43	0.	0.	3210.	1.09	22.30	143	0.	0.	0.
1.03	18.00	44	0.	0.	2929.	1.10	0.	144	0.	0.	0.
1.03	19.30	45	0.	0.	2672.	1.10	1.30	145	0.	0.	0.
1.03	21.00	46	0.	0.	2438.	1.10	3.00	146	0.	0.	0.
1.03	22.30	47	0.	0.	2225.	1.10	4.30	147	0.	0.	0.
1.04	0.	48	0.	0.	2030.	1.10	6.00	148	0.	0.	0.
1.04	1.30	49	0.	0.	1852.	1.10	7.30	149	0.	0.	0.
1.04	3.00	50	0.	0.	1690.	1.10	9.00	150	0.	0.	0.
1.04	4.30	51	0.	0.	1542.	1.10	10.30	151	0.	0.	0.
1.04	6.00	52	0.	0.	1407.	1.10	12.00	152	0.	0.	0.
1.04	7.30	53	0.	0.	1284.	1.10	13.30	153	0.	0.	0.
1.04	9.00	54	0.	0.	1171.	1.10	15.00	154	0.	0.	0.
1.04	10.30	55	0.	0.	1069.	1.10	16.30	155	0.	0.	0.
1.04	12.00	56	0.	0.	975.	1.10	18.00	156	0.	0.	0.
1.04	13.30	57	0.	0.	890.	1.10	19.30	157	0.	0.	0.
1.04	15.00	58	0.	0.	812.	1.10	21.00	158	0.	0.	0.
1.04	16.30	59	0.	0.	741.	1.10	22.30	159	0.	0.	0.
1.04	18.00	60	0.	0.	674.	1.10	24.00	160	0.	0.	0.
1.04	19.30	61	0.	0.	607.	1.10	25.30	161	0.	0.	0.
1.04	21.00	62	0.	0.	547.	1.10	27.00	162	0.	0.	0.

STATION 1

	0.	4000.	8000.	12000.	16000.	20000.	24000.	28000.	32000.	36000.	PRECIP(L) AND EXCESS(X)	0.	2.	4.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.	12.		8.			0.
1.30	11														L
3.00	21														L
4.30	31														L
6.00	41														L
7.30	51														L
9.00	61														L
10.30	71														L
12.00	81														L
13.30	91														L
15.00	101														LX
16.30	111														LX
18.00	121														L
19.30	131														L
21.00	141														L
22.30	151														L
0.	161														L
1.30	171														L
3.00	181														L
4.30	191														L
6.00	201														L
7.30	211														L
9.00	221														LXX
10.30	231														LXX
12.00	241														LXX
13.30	251														LXX
15.00	261														LXXXXXXXXXXXX
16.30	271														LXXXXXXXXXXXX
18.00	281														LXXXXXXXXXXXX
19.30	291														LXXXXXXXXXXXX
21.00	301														L
22.30	311														LX
0.	321														LX
1.30	331														L
3.00	341														L
4.30	351														L
6.00	361														L
7.30	371														L
9.00	381														L
10.30	391														L
12.00	401														L
13.30	411														L
15.00	421														L
16.30	431														L
18.00	441														L
19.30	451														L
21.00	461														L
22.30	471														L
0.	481														L
1.30	491														L
3.00	501														L
4.30	511														L
6.00	521														L
7.30	531														L
9.00	541														L
10.30	551														L

12.00 56.1
13.30 57.1
15.00 58.1
16.30 59.1
18.00 60.1
19.30 61.1
21.00 62.1
22.30 63.1
0. 64.1
1.30 65.1
3.00 66.1
4.30 67.1
5.00 68.1
7.30 69.1
9.30 70.1
10.30 71.1
12.00 72.1
13.30 73.1
15.30 74.1
16.30 75.1
18.00 76.1
19.30 77.1
21.00 78.1
22.30 79.1
0. 80.1
1.30 81.1
3.00 82.1
4.30 83.1
6.00 84.1
7.30 85.1
9.00 86.1
10.30 87.1
12.30 88.1
13.30 89.1
15.00 90.1
16.30 91.1
18.00 92.1
19.30 93.1
21.00 94.1
22.30 95.1
0. 96.1
1.30 97.1
3.00 98.1
4.30 99.1
6.00 100.1
7.30 101.1
9.00 102.1
10.30 103.1
12.30 104.1
13.30 105.1
15.00 106.1
16.30 107.1
18.00 108.1
19.30 109.1
21.00 110.1
22.30 111.1
0. 112.1
1.30 113.1
3.00 114.1
4.30 115.1
6.00 116.1
7.30 117.1

9.001191
10.301191
12.001201
13.301211
15.301221
16.301231
18.301241
19.301251
21.001261
22.301271
0. 1281
1.301291
3.001301
4.301311
6.001321
7.301331
9.301341
10.301351
12.001361
13.301371
15.001381
16.301391
18.301401
19.301411
21.001421
22.301431
0. 1441
1.301451
3.001461
4.301471
6.001481
7.301491
9.001501
10.301511
12.301521
13.301531
15.001541
16.301551
18.001561
19.301571
21.001581
22.301591
0. 1601
1.301611
3.001621
4.301631
6.301641
7.301651
9.001661
10.301671
12.001681
13.301691
15.301701
16.301711
18.301721
19.301731
21.001741
22.301751
0. 1761
1.301771
3.001781
4.301791

6.001801
7.301811
9.001821
10.301831
12.001841
13.301851
15.001861
16.301871
18.001881
19.301891
21.001901
22.301911
0. 1921
1.301931
3.001941
4.301951
6.001961
7.301971
9.001981
10.301991
12.002001

HYDROGRAPH AT STA 1 FOR PLAY 1, RTIO 1

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 2[illegible]

HYDROGRAPH ROUTING

ROUTE THROUGH RESERVOIR

ISTAQ	ICOMP	IECON	ITAPE	JPLT	JPRT	INAME	ISTAGE	IAUTO
1	1	0	0	2	2	1	0	0

ROUTING DATA			
QLOSS	CLOSS	AVG	LAG
0.	0.	0.	0.

ROUTING DATA			
IPMP	IPMP	IPMP	LSTR
0	0	0	0

ROUTING DATA			
NSIPS	NSTOL	LAG	AMSKK
1	0	0	0.

ROUTING DATA			
TSK	STORA	ISPRAT	
0.	-788.	-1	

STAGE	785.60	788.00	789.00	790.10	791.00	792.00	793.00	794.00	795.00
FLOW	0.	347.00	777.00	1369.00	2906.00	5615.00	9026.00	13006.00	17479.00
CAPACITY	0.	13600.	23000.	28750.	32500.	42000.			
ELEVATION	722.	780.	785.	788.	790.	795.			

CREL	SPWID	COQW	EXPW	ELEV	COQL	CAREA	EXPL
786.6	0.	0.	0.	0.	0.	0.	0.

DAM DATA

TOPEL	COOD	EXPD	DAMWID
798.1	3.8	1.5	50.

WARNING *** TOP OF DAM, BOTTOM OF BREACH, OR LOW-LEVEL OUTLET IS NOT WITHIN RANGE OF GIVEN ELEVATIONS IN STORAGE-ELEVATION DATA
 BOTTOM OF RESERVOIR ASSUMED TO BE AT 722.00
 STORAGE-ELEVATION DATA WILL BE EXTRAPOLATED ABOVE ELEVATION 795.00

STATION 1, PLAN 1, RATIO 1

END-OF-PERIOD HYDROGRAPH ORDINATES

OUTFLOW

342.	336.	331.	326.	321.	316.	311.	307.	302.	297.
293.	288.	285.	281.	278.	274.	271.	268.	264.	261.
258.	255.	254.	255.	260.	274.	305.	373.	516.	689.
888.	1086.	1242.	1356.	1575.	1723.	1794.	1807.	1779.	1720.
1642.	1552.	1459.	1370.	1342.	1313.	1283.	1253.	1223.	1193.
1162.	1132.	1102.	1072.	1042.	1013.	984.	956.	928.	900.
873.	847.	821.	796.	772.	753.	734.	715.	697.	679.
661.	644.	627.	611.	595.	579.	564.	549.	534.	520.
506.	492.	479.	466.	453.	441.	429.	417.	406.	394.
384.	373.	363.	353.	345.	334.	324.	319.	324.	319.
314.	309.	304.	293.	294.	290.	285.	281.	276.	272.
267.	263.	259.	255.	251.	247.	243.	239.	235.	232.
228.	224.	221.	217.	214.	210.	207.	204.	201.	197.
194.	191.	188.	185.	182.	179.	176.	174.	171.	168.
165.	163.	160.	158.	155.	153.	150.	148.	146.	143.
141.	139.	137.	134.	132.	130.	128.	126.	124.	122.
123.	118.	115.	114.	113.	111.	109.	107.	106.	104.
102.	101.	99.	97.	96.	94.	93.	91.	90.	89.
87.	86.	84.	83.	82.	80.	79.	78.	77.	75.
74.	73.	72.	71.	70.	69.	67.	66.	65.	64.

STORAGE

28709.	28669.	28529.	29590.	28551.	28475.	28437.	28400.	28364.
28330.	28297.	28267.	28240.	28214.	28188.	28136.	28109.	28083.
28058.	28039.	28031.	28040.	28078.	28185.	28428.	29488.	30240.
31013.	31700.	32244.	32646.	32920.	33084.	33163.	33178.	33081.
32994.	32894.	32791.	32691.	32593.	32492.	32285.	32179.	32074.
31968.	31862.	31757.	31653.	31550.	31448.	31347.	31151.	31055.
30961.	30969.	30779.	30691.	30605.	30520.	30437.	30276.	30198.
30121.	30046.	29973.	29901.	29830.	29762.	29695.	29565.	29503.
29442.	29382.	29324.	29267.	29212.	29159.	29106.	29005.	28957.
28910.	28864.	28819.	28775.	28733.	28691.	28650.	28569.	28530.
28491.	28453.	28416.	28379.	28342.	28306.	28271.	28236.	28188.
28135.	28102.	28070.	28033.	27997.	27976.	27946.	27886.	27858.
27829.	27801.	27774.	27747.	27720.	27694.	27668.	27617.	27593.
27569.	27545.	27521.	27498.	27475.	27453.	27431.	27388.	27367.
27346.	27326.	27306.	27286.	27267.	27248.	27229.	27192.	27174.
27157.	27139.	27122.	27106.	27089.	27073.	27057.	27026.	27010.
26995.	26980.	26966.	26952.	26938.	26924.	26910.	26884.	26871.
26858.	26845.	26833.	26821.	26809.	26797.	26785.	26774.	26751.
26741.	26730.	26719.	26709.	26699.	26689.	26679.	26659.	26650.
26641.	26632.	26623.	26614.	26605.	26597.	26588.	26572.	26564.

STAGE

788.0	788.0	787.9	787.9	787.9	787.9	787.8	787.8	787.8
787.8	787.8	787.7	787.7	787.7	787.7	787.7	787.7	787.7
787.6	787.6	787.6	787.6	787.6	787.6	787.6	787.6	787.6
789.2	789.6	789.9	790.1	790.2	790.3	790.4	790.3	790.3
790.3	790.2	790.2	790.1	790.0	790.0	789.9	789.8	789.8
789.7	789.7	789.6	789.5	789.5	789.4	789.3	789.2	789.2
789.2	789.1	789.0	789.0	788.9	788.9	788.9	788.8	788.8
788.7	788.7	788.6	788.6	788.5	788.5	788.5	788.4	788.4
788.4	788.3	788.3	788.3	788.2	788.2	788.2	788.1	788.1
788.1	788.1	788.0	788.0	788.0	787.9	787.9	787.9	787.9
787.9	787.8	787.8	787.8	787.8	787.7	787.7	787.7	787.7
787.7	787.7	787.6	787.6	787.6	787.6	787.6	787.5	787.5
787.5	787.5	787.5	787.5	787.4	787.4	787.4	787.4	787.4
787.4	787.4	787.4	787.3	787.3	787.3	787.3	787.3	787.3
787.3	787.3	787.2	787.2	787.2	787.2	787.2	787.2	787.2
787.2	787.2	787.2	787.1	787.1	787.1	787.1	787.1	787.1
787.1	787.1	787.1	787.1	787.0	787.0	787.0	787.0	787.0
787.0	787.0	787.0	787.0	787.0	787.0	787.0	787.0	787.0
787.0	786.9	786.9	786.9	786.9	786.9	786.9	786.9	786.9
786.9	786.9	786.9	786.9	786.9	786.9	786.9	786.9	786.9

PEAK OUTFLOW IS 1807. AT TIME 57.00 HOURS

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
1807.	1775.	1513.	1074.	85371.
51.	50.	43.	30.	2417.
CFS	CM	CM	CM	
INCHES	INCHES	INCHES	INCHES	
MM	MM	MM	MM	
AC-FT	AC-FT	AC-FT	AC-FT	
THOUS CU M	THOUS CU M	THOUS CU M	THOUS CU M	
	1086.	3701.	7879.	33054.

[illegible]

15.00 58.1 0
16.30 59.1 0
18.00 60.1 0
19.30 61.1 0
21.00 62.1 0
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12.30 72.1 0
13.30 73.1 0
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7.30 613.1 0
9.00 614.1 0
10.30 615.1 0
12.00 616.1 0
13.30 617.1 0
15.00 618.1 0
16.30 619.1 0
18.00 620.1 0
19.30 621.1 0
21.00 622.1 0
22.30 623.1 0
0. 624.1 0
1.30 625.1 0
3.00 626.1 0
4.30 627.1 0
6.00 628.1 0
7.30 629.1 0
9.00 630.1 0
10.30 631.1 0
12.00 632.1 0
13.30 633.1 0
15.00 634.1 0
16.30 635.1 0
18.00 636.1 0
19.30 637.1 0
21.00 638.1 0
22.30 639.1 0
0. 640.1 0
1.30 641.1 0
3.00 642.1 0
4.30 643.1 0
6.00 644.1 0
7.30 645.1 0
9.00 646.1 0
10.30 647.1 0
12.00 648.1 0
13.30 649.1 0
15.00 650.1 0
16.30 651.1 0
18.00 652.1 0
19.30 653.1 0
21.00 654.1 0
22.30 655.1 0
0. 656.1 0
1.30 657.1 0
3.00 658.1 0
4.30 659.1 0
6.00 660.1 0
7.30 661.1 0
9.00 662.1 0
10.30 663.1 0
12.00 664.1 0
13.30 665.1 0
15.00 666.1 0
16.30 667.1 0
18.00 668.1 0
19.30 669.1 0
21.00 670.1 0
22.30 671.1 0
0. 672.1 0
1.30 673.1 0
3.00 674.1 0
4.30 675.1 0
6.00 676.1 0
7.30 677.1 0
9.00 678.1 0
10.30 679.1 0
12.00 680.1 0
13.30 681.1 0
15.00 682.1 0
16.30 683.1 0
18.00 684.1 0
19.30 685.1 0
21.00 686.1 0
22.30 687.1 0
0. 688.1 0
1.30 689.1 0
3.00 690.1 0
4.30 691.1 0
6.00 692.1 0
7.30 693.1 0
9.00 694.1 0
10.30 695.1 0
12.00 696.1 0
13.30 697.1 0
15.00 698.1 0
16.30 699.1 0
18.00 700.1 0
19.30 701.1 0
21.00 702.1 0
22.30 703.1 0
0. 704.1 0
1.30 705.1 0
3.00 706.1 0
4.30 707.1 0
6.00 708.1 0
7.30 709.1 0
9.00 710.1 0
10.30 711.1 0
12.00 712.1 0
13.30 713.1 0
15.00 714.1 0
16.30 715.1 0
18.00 716.1 0
19.30 717.1 0
21.00 718.1 0
22.30 719.1 0
0. 720.1 0
1.30 721.1 0
3.00 722.1 0
4.30 723.1 0
6.00 724.1 0
7.30 725.1 0
9.00 726.1 0
10.30 727.1 0
12.00 728.1 0
13.30 729.1 0
15.00 730.1 0
16.30 731.1 0
18.00 732.1 0
19.30 733.1 0
21.00 734.1 0
22.30 735.1 0
0. 736.1 0
1.30 737.1 0
3.00 738.1 0
4.30 739.1 0
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7.30 741.1 0
9.00 742.1 0
10.30 743.1 0
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13.30 745.1 0
15.00 746.1 0
16.30 747.1 0
18.00 748.1 0
19.30 749.1 0
21.00 750.1 0
22.30 751.1 0
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1.30 753.1 0
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4.30 755.1 0
6.00 756.1 0
7.30 757.1 0
9.00 758.1 0
10.30 759.1 0
12.00 760.1 0
13.30 761.1 0
15.00 762.1 0
16.30 763.1 0
18.00 764.1 0
19.30 765.1 0
21.00 766.1 0
22.30 767.1 0
0. 768.1 0
1.30 769.1 0
3.00 770.1 0
4.30 771.1 0
6.00 772.1 0
7.30 773.1 0
9.00 774.1 0
10.30 775.1 0
12.00 776.1 0
13.30 777.1 0
15.00 778.1 0
16.30 779.1 0
18.00 780.1 0
19.30 781.1 0
21.00 782.1 0
22.30 783.1 0
0. 784.1 0
1.30 785.1 0
3.00 786.1 0
4.30 787.1 0
6.00 788.1 0
7.30 789.1 0
9.00 790.1 0
10.30 791.1 0
12.00 792.1 0
13.30 793.1 0
15.00 794.1 0
16.30 795.1 0
18.00 796.1 0
19.30 797.1 0
21.00 798.1 0
22.30 799.1 0
0. 800.1 0
1.30 801.1 0
3.00 802.1 0
4.30 803.1 0
6.00 804.1 0
7.30 805.1 0
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10.30 807.1 0
12.00 808.1 0
13.30 809.1 0
15.00 810.1 0
16.30 811.1 0
18.00 812.1 0
19.30 813.1 0
21.00 814.1 0
22.30 815.1 0
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1.30 817.1 0
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4.30 819.1 0
6.00 820.1 0
7.30 821.1 0
9.00 822.1 0
10.30 823.1 0
12.00 824.1 0
13.30 825.1 0
15.00 826.1 0
16.30 827.1 0
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19.30 829.1 0
21.00 830.1 0
22.30 831.1 0
0. 832.1 0
1.30 833.1 0
3.00 834.1 0
4.30 835.1 0
6.00 836.1 0
7.30 837.1 0
9.00 838.1 0
10.30 839.1 0
12.00 840.1 0
13.30 841.1 0
15.00 842.1 0
16.30 843.1 0
18.00 844.1 0
19.30 845.1 0
21.00 846.1 0
22.30 847.1 0
0. 848.1 0
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3.00 850.1 0
4.30 851.1 0
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10.30 855.1 0
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13.30 857.1 0
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7.30 885.1 0
9.00 886.1 0
10.30 887.1 0
12.00 888.1 0
13.30 889.1 0
15.00 890.1 0
16.30 891.1 0
18.00 892.1 0
19.30 893.1 0
21.00 894.1 0
22.30 895.1 0
0. 896.1 0
1.30 897.1 0
3.00 898.1 0
4.30 899.1 0
6.00 900.1 0
7.30 901.1 0
9.00 902.1 0
10.30 903.1 0
12.00 904.1 0
13.30 905.1 0
15.00 906.1 0
16.30 907.1 0
18.00 908.1 0
19.30 909.1 0
21.00 910.1 0
22.30 911.1 0
0. 912.1 0
1.30 913.1 0
3.00 914.1 0
4.30 915.1 0
6.00 916.1 0

12.001201.0
13.301211.0
15.301221.0
16.301231.0
18.301241.0
19.301251.0
21.001261.0
22.301271.0
0.1281.0
1.301291.0
3.001301.0
4.301311.0
6.001321.0
7.301331.0
9.001341.0
10.301351.0
12.001361.0
13.301371.0
15.001381.0
16.301391.0
18.001401.0
19.301411.0
21.001421.0
22.301431.0
0.1441.0
1.301451.0
3.001461.0
4.301471.0
6.001481.0
7.301491.0
9.001501.0
10.301511.0
12.001521.0
13.301531.0
15.001541.0
16.301551.0
18.001561.0
19.301571.0
21.001581.0
22.301591.0
0.1601.0
1.301611.0
3.001621.0
4.301631.0
6.001641.0
7.301651.0
9.001661.0
10.301671.0
12.001681.0
13.301691.0
15.001701.0
16.301711.0
18.001721.0
19.301731.0
21.001741.0
22.301751.0
0.1761.0
1.301771.0
3.001781.0
4.301791.0
6.001801.0
7.301811.0

9.0018210
10.3013310
12.0018410
13.3018510
15.0018610
16.3018710
18.0018810
19.3018910
21.0019010
22.3019110
0. 19210
1.3019310
3.0019410
4.3019510
6.0019610
7.3019710
9.0019810
10.3019910
12.0020010

•OVN•

WARNING *** TOP OF DAM, BOTTOM OF BREACH, OR LOW-LEVEL OUTLET IS NOT WITHIN RANGE OF GIVEN ELEVATIONS IN STORAGE-ELEVATION DATA
 BOTTOM OF RESERVOIR ASSUMED TO BE AT 722.00
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STATION 1, PLAN 1, RATIO 2
 END-OF-PERIOD HYDROGRAPH ORDINATES

		OUTFLOW			
342.	332.	327.	317.	313.	308.
294.	297.	295.	280.	278.	275.
267.	268.	288.	319.	415.	619.
2721.	5408.	6347.	6267.	5972.	5558.
3830.	3426.	3069.	2618.	2440.	2273.
1828.	1575.	1461.	1331.	1298.	1265.
1165.	1101.	1069.	1008.	978.	948.
864.	810.	785.	744.	725.	706.
652.	618.	601.	570.	554.	539.
497.	470.	457.	433.	421.	409.
376.	366.	346.	341.	330.	325.
310.	300.	296.	286.	282.	277.
264.	256.	252.	244.	240.	236.
225.	218.	215.	208.	205.	201.
192.	186.	183.	177.	174.	172.
164.	158.	156.	151.	149.	146.
139.	135.	133.	129.	127.	125.
101.	98.	96.	93.	92.	90.
86.	83.	82.	80.	78.	77.

		STORAGE			
28711.	28672.	28596.	28521.	28484.	28447.
28343.	28313.	28268.	28234.	28215.	28194.
28133.	28125.	28187.	28234.	29046.	29936.
34196.	35379.	36567.	36663.	36490.	36260.
35356.	35049.	34766.	34516.	34081.	33697.
33201.	33056.	32792.	32673.	32440.	32324.
31978.	31865.	31753.	31643.	31428.	31221.
30927.	30833.	30742.	30653.	30480.	30315.
30079.	30004.	29859.	29789.	29721.	29589.
29403.	29344.	29287.	29176.	29123.	29021.
28878.	28833.	28789.	28704.	28622.	28581.
28464.	28426.	28389.	28316.	28246.	28211.
28111.	28079.	28047.	27985.	27924.	27895.
27809.	27781.	27754.	27701.	27649.	27624.
27551.	27528.	27504.	27459.	27415.	27394.
27331.	27311.	27292.	27253.	27215.	27197.
27144.	27127.	27094.	27077.	27045.	27030.
26985.	26970.	26956.	26926.	26900.	26887.
26849.	26836.	26824.	26800.	26777.	26766.
26733.	26722.	26702.	26681.	26672.	26662.

		STAGE			
788.0	788.0	787.9	787.9	787.9	787.8
787.8	787.8	787.7	787.7	787.7	787.7
787.7	787.7	787.7	787.7	787.7	787.7
790.9	791.5	791.9	792.1	792.2	792.0
791.5	791.3	791.2	791.1	790.8	790.6
790.4	790.3	790.2	790.1	790.0	789.9
789.5	789.5	789.5	789.5	789.4	789.3

789.2	789.1	789.1	789.0	789.0	788.9	788.9	788.8	788.8	788.7
788.7	788.7	783.6	788.6	788.6	788.5	788.5	788.4	788.4	788.4
788.3	788.3	783.3	788.2	788.2	788.2	788.2	788.1	788.1	788.1
788.1	788.0	783.0	788.0	788.0	788.0	787.9	787.9	787.9	787.9
787.9	787.8	787.8	787.8	787.8	787.8	787.7	787.7	787.7	787.7
787.7	787.6	787.6	787.6	787.6	787.6	787.6	787.6	787.5	787.5
787.5	787.5	787.5	787.5	787.5	787.4	787.4	787.4	787.4	787.4
787.4	787.4	787.4	787.3	787.3	787.3	787.3	787.3	787.3	787.3
787.3	787.2	787.2	787.2	787.2	787.2	787.2	787.2	787.2	787.2
787.2	787.2	787.1	787.1	787.1	787.1	787.1	787.1	787.1	787.1
787.1	787.1	787.1	787.0	787.0	787.0	787.0	787.0	787.0	787.0
787.0	787.0	787.0	787.0	787.0	787.0	787.0	787.0	787.0	787.0
786.9	786.9	786.9	786.9	786.9	786.9	786.9	786.9	786.9	786.9

PEAK OUTFLOW IS 6347. AT TIME 52.50 HOURS

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
CFS	6347.	6103.	4527.	2352.	152798.
CMS	180.	173.	128.	67.	4327.
INCHES		1.45	4.30	6.71	9.08
MM		36.83	109.26	170.37	230.54
AC-FT		3026.	8977.	13998.	18942.
THOUS CU M		3733.	11073.	17266.	25364.

STATION 1

[illegible]

15.00 58.1 0
16.30 59.1 0
18.30 60.1 0
19.30 61.1 0
21.30 62.1 0
22.30 63.1 0
0. 64.1 0
1.30 65.1 0
3.00 66.1 0
4.30 67.1 0
5.30 68.1 0
7.30 69.1 0
9.00 70.1 0
10.30 71.1 0
12.00 72.1 0
13.30 73.1 0
15.30 74.1 0
16.30 75.1 0
18.00 76.1 0
19.30 77.1 0
21.00 78.1 0
22.30 79.1 0
0. 80.1 0
1.30 81.1 0
3.00 82.1 0
4.30 83.1 0
6.00 84.1 0
7.30 85.1 0
9.00 86.1 0
10.30 87.1 0
12.00 88.1 0
13.30 89.1 0
15.00 90.1 0
16.30 91.1 0
18.00 92.1 0
19.30 93.1 0
21.30 94.1 0
22.30 95.1 0
0. 96.1 0
1.30 97.1 0
3.00 98.1 0
4.30 99.1 0
6.00 100.1 0
7.30 101.1 0
9.00 102.1 0
10.30 103.1 0
12.00 104.1 0
13.30 105.1 0
15.00 106.1 0
16.30 107.1 0
18.00 108.1 0
19.30 109.1 0
21.00 110.1 0
22.30 111.1 0
0. 112.1 0
1.30 113.1 0
3.00 114.1 0
4.30 115.1 0
6.00 116.1 0
7.30 117.1 0
9.00 118.1 0
10.30 119.1 0

12.0012010.....
13.3012110.....
15.0012210.....
16.3012310.....
18.0012410.....
19.3012510.....
21.0012610.....
22.3012710.....
0. 12810.....
1.3012910.....
3.0013010.....
4.3013110.....
6.0013210.....
7.3013310.....
9.0013410.....
10.3013510.....
12.0013610.....
13.3013710.....
15.0013810.....
16.3013910.....
18.0014010.....
19.3014110.....
21.0014210.....
22.3014310.....
0. 14410.....
1.3014510.....
3.0014610.....
4.3014710.....
6.0014810.....
7.3014910.....
9.0015010.....
10.3015110.....
12.0015210.....
13.3015310.....
15.0015410.....
16.3015510.....
18.0015610.....
19.3015710.....
21.0015810.....
22.3015910.....
0. 16010.....
1.3016110.....
3.0016210.....
4.3016310.....
6.0016410.....
7.3016510.....
9.0016610.....
10.3016710.....
12.0016810.....
13.3016910.....
15.0017010.....
16.3017110.....
18.0017210.....
19.3017310.....
21.0017410.....
22.3017510.....
0. 17610.....
1.3017710.....
3.0017810.....
4.3017910.....
6.0018010.....
7.3018110.....

9.001821
10.301931
12.301841
13.301851
15.001861
16.301371
18.001881
19.301991
21.301901
22.301911
0. 1921
1.301931
3.001941
4.301951
5.001961
7.301971
9.001981
10.301991
12.002001

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END-OF-PERIOD HYDROGRAPH ORDINATES

OUTFLOW							
337.	332.	327.	318.	313.	309.	304.	299.
292.	289.	287.	283.	281.	279.	276.	274.
272.	275.	284.	342.	486.	742.	1187.	2450.
6772.	8197.	8827.	8608.	8084.	7426.	6708.	5979.
4739.	4284.	3837.	3090.	2822.	2637.	2461.	2295.
1991.	1852.	1721.	1484.	1377.	1339.	1306.	1273.
1207.	1174.	1141.	1078.	1047.	1016.	986.	956.
899.	872.	845.	792.	769.	750.	730.	711.
675.	640.	623.	606.	590.	574.	559.	544.
515.	501.	487.	461.	449.	436.	424.	413.
390.	380.	369.	349.	343.	337.	332.	327.
317.	312.	307.	297.	292.	288.	283.	279.
270.	266.	261.	253.	249.	245.	241.	238.
230.	226.	223.	216.	212.	209.	206.	202.
196.	193.	190.	184.	181.	178.	175.	172.
167.	164.	162.	157.	154.	152.	149.	147.
142.	140.	138.	133.	131.	129.	127.	125.
121.	119.	117.	116.	112.	110.	108.	107.
103.	102.	100.	97.	95.	94.	92.	91.
89.	87.	85.	83.	81.	80.	79.	77.

28712.	28674.	28536.	28599.	28552.	28489.	28453.	28417.	28382.
28713.	28675.	28537.	28600.	28553.	28490.	28454.	28418.	28383.
28714.	28676.	28538.	28601.	28554.	28491.	28455.	28419.	28384.
28715.	28677.	28539.	28602.	28555.	28492.	28456.	28420.	28385.
28716.	28678.	28540.	28603.	28556.	28493.	28457.	28421.	28386.
28717.	28679.	28541.	28604.	28557.	28494.	28458.	28422.	28387.
28718.	28680.	28542.	28605.	28558.	28495.	28459.	28423.	28388.
28719.	28681.	28543.	28606.	28559.	28496.	28460.	28424.	28389.
28720.	28682.	28544.	28607.	28560.	28497.	28461.	28425.	28390.
28721.	28683.	28545.	28608.	28561.	28498.	28462.	28426.	28391.
28722.	28684.	28546.	28609.	28562.	28499.	28463.	28427.	28392.
28723.	28685.	28547.	28610.	28563.	28500.	28464.	28428.	28393.
28724.	28686.	28548.	28611.	28564.	28501.	28465.	28429.	28394.
28725.	28687.	28549.	28612.	28565.	28502.	28466.	28430.	28395.
28726.	28688.	28550.	28613.	28566.	28503.	28467.	28431.	28396.
28727.	28689.	28551.	28614.	28567.	28504.	28468.	28432.	28397.
28728.	28690.	28552.	28615.	28568.	28505.	28469.	28433.	28398.
28729.	28691.	28553.	28616.	28569.	28506.	28470.	28434.	28399.
28730.	28692.	28554.	28617.	28570.	28507.	28471.	28435.	28400.
28731.	28693.	28555.	28618.	28571.	28508.	28472.	28436.	28401.
28732.	28694.	28556.	28619.	28572.	28509.	28473.	28437.	28402.
28733.	28695.	28557.	28620.	28573.	28510.	28474.	28438.	28403.
28734.	28696.	28558.	28621.	28574.	28511.	28475.	28439.	28404.
28735.	28697.	28559.	28622.	28575.	28512.	28476.	28440.	28405.
28736.	28698.	28560.	28623.	28576.	28513.	28477.	28441.	28406.
28737.	28699.	28561.	28624.	28577.	28514.	28478.	28442.	28407.
28738.	28700.	28562.	28625.	28578.	28515.	28479.	28443.	28408.
28739.	28701.	28563.	28626.	28579.	28516.	28480.	28444.	28409.
28740.	28702.	28564.	28627.	28580.	28517.	28481.	28445.	28410.
28741.	28703.	28565.	28628.	28581.	28518.	28482.	28446.	28411.
28742.	28704.	28566.	28629.	28582.	28519.	28483.	28447.	28412.
28743.	28705.	28567.	28630.	28583.	28520.	28484.	28448.	28413.
28744.	28706.	28568.	28631.	28584.	28521.	28485.	28449.	28414.
28745.	28707.	28569.	28632.	28585.	28522.	28486.	28450.	28415.
28746.	28708.	28570.	28633.	28586.	28523.	28487.	28451.	28416.
28747.	28709.	28571.	28634.	28587.	28524.	28488.	28452.	28417.
28748.	28710.	28572.	28635.	28588.	28525.	28489.	28453.	28418.
28749.	28711.	28573.	28636.	28589.	28526.	28490.	28454.	28419.
28750.	28712.	28574.	28637.	28590.	28527.	28491.	28455.	28420.
28751.	28713.	28575.	28638.	28591.	28528.	28492.	28456.	28421.
28752.	28714.	28576.	28639.	28592.	28529.	28493.	28457.	28422.
28753.	28715.	28577.	28640.	28593.	28530.	28494.	28458.	28423.
28754.	28716.	28578.	28641.	28594.	28531.	28495.	28459.	28424.
28755.	28717.	28579.	28642.	28595.				

[illegible]

789.3	789.2	789.1	789.0	788.9	788.8
788.8	788.7	788.6	788.5	788.4	788.3
788.4	788.3	788.2	788.1	788.0	787.9
788.1	788.0	787.9	787.8	787.7	787.6
787.9	787.8	787.7	787.6	787.5	787.4
787.7	787.6	787.5	787.4	787.3	787.2
787.5	787.4	787.3	787.2	787.1	787.0
787.4	787.3	787.2	787.1	787.0	786.9
787.3	787.2	787.1	787.0	786.9	786.8
787.2	787.1	787.0	786.9	786.8	786.7
787.1	787.0	786.9	786.8	786.7	786.6
787.0	786.9	786.8	786.7	786.6	786.5
787.0	786.9	786.8	786.7	786.6	786.5

PEAK OUTFLOW IS 8901. AT TIME 52.50 HOURS

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
CFS	8901.	8619.	6164.	3029.	186657.
CMS	252.	244.	175.	86.	5286.
INCHES		2.05	5.86	8.61	11.09
MM		52.02	148.80	218.70	281.63
AC-FT		4274.	12226.	17969.	23139.
THOUS CU M		5272.	15080.	22165.	28542.

• QvF •

STATION 1

INFLOW(I), OUTFLOW(O) AND OBSERVED FLOW(*)

[illegible]

15.00 58.1 0
16.30 59.1 0
18.00 60.1 0
19.30 61.1 0
21.00 62.1 0
22.30 63.1 0
0. 64.1 0
1.30 65.1 0
3.00 66.1 0
4.30 67.1 0
6.00 68.1 0
7.30 69.1 0
9.00 70.1 0
10.30 71.1 0
12.00 72.1 0
13.30 73.1 0
15.00 74.1 0
16.30 75.1 0
18.00 76.1 0
19.30 77.1 0
21.00 78.1 0
22.30 79.1 0
0. 80.1 0
1.30 81.1 0
3.00 82.1 0
4.30 83.1 0
6.00 84.1 0
7.30 85.1 0
9.00 86.1 0
10.30 87.1 0
12.00 88.1 0
13.30 89.1 0
15.00 90.1 0
16.30 91.1 0
18.00 92.1 0
19.30 93.1 0
21.00 94.1 0
22.30 95.1 0
0. 96.1 0
1.30 97.1 0
3.00 98.1 0
4.30 99.1 0
6.00 100.1 0
7.30 101.1 0
9.00 102.1 0
10.30 103.1 0
12.00 104.1 0
13.30 105.1 0
15.00 106.1 0
16.30 107.1 0
18.00 108.1 0
19.30 109.1 0
21.00 110.1 0
22.30 111.1 0
0. 112.1 0
1.30 113.1 0
3.00 114.1 0
4.30 115.1 0
6.00 116.1 0
7.30 117.1 0
9.00 118.1 0
10.30 119.1 0
12.00 120.1 0

12.0012010.....
13.3012110.....
15.0012210.....
16.3012310.....
18.0012410.....
19.3012510.....
21.0012610.....
22.3012710.....
0. 12810.....
1.3012910.....
3.0013010.....
4.3013110.....
6.0013210.....
7.3013310.....
9.0013410.....
10.3013510.....
12.0013610.....
13.3013710.....
15.0013810.....
16.3013910.....
18.0014010.....
19.3014110.....
21.0014210.....
22.3014310.....
0. 14410.....
1.3014510.....
3.0014610.....
4.3014710.....
6.0014810.....
7.3014910.....
9.0015010.....
10.3015110.....
12.0015210.....
13.3015310.....
15.0015410.....
16.3015510.....
18.0015610.....
19.3015710.....
21.0015810.....
22.3015910.....
0. 16010.....
1.3016110.....
3.0016210.....
4.3016310.....
6.0016410.....
7.3016510.....
9.0016610.....
10.3016710.....
12.0016810.....
13.3016910.....
15.0017010.....
16.3017110.....
18.0017210.....
19.3017310.....
21.0017410.....
22.3017510.....
0. 17610.....
1.3017710.....
3.0017810.....
4.3017910.....
6.0018010.....
7.3018110.....

9.0018210
10.3018310
12.0015410
13.301851
15.001861
16.301871
18.001881
19.301891
21.001901
22.301911
0.1921
1.301931
3.001941
4.301951
6.001961
7.301971
9.001981
10.301991
12.002001

789.4	789.3	789.3	789.2	789.2	789.1	789.0	789.0	788.9
788.9	788.8	788.8	788.8	788.7	788.7	788.6	788.6	788.5
788.5	788.5	788.4	788.4	788.4	788.3	788.3	788.2	788.2
788.2	788.1	788.1	788.1	788.1	788.0	788.0	788.0	788.0
787.9	787.9	787.9	787.9	787.9	787.8	787.8	787.8	787.8
787.7	787.7	787.7	787.7	787.7	787.6	787.6	787.6	787.6
787.6	787.6	787.5	787.5	787.5	787.5	787.5	787.5	787.4
787.4	787.4	787.4	787.4	787.4	787.4	787.3	787.3	787.3
787.3	787.3	787.3	787.3	787.3	787.2	787.2	787.2	787.2
787.2	787.2	787.2	787.2	787.2	787.1	787.1	787.1	787.1
787.1	787.1	787.1	787.1	787.1	787.1	787.1	787.1	787.0
787.0	787.0	787.0	787.0	787.0	787.0	787.0	787.0	787.0
787.0	787.0	787.0	787.0	786.9	786.9	786.9	786.9	786.9

PEAK OUTFLOW IS 11690. AT TIME 51.00 HOURS

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
CFS	11690.	11191.	7846.	3692.	220545.
CMS	331.	317.	222.	105.	6245.
INCHES		2.66	7.46	10.53	13.10
MM		67.54	189.40	267.41	332.76
AC-FT		5549.	15561.	21971.	27340.
THOUS CU M		6845.	19195.	27101.	33724.

STATION 1

INFLOW(I), OUTFLOW(O) AND OBSERVED FLOW(+)

[illegible]

15.30 58.1 0
16.30 59.1 0
18.00 60.1 0
19.30 51.1 0
21.00 42.1 0
22.30 53.1 0
0. 64.1 0
1.30 65.1 0
3.00 66.1 0
4.30 67.1 0
6.00 68.1 0
7.30 69.1 0
9.00 70.1 0
10.30 71.1 0
12.00 72.1 0
13.30 73.1 0
15.00 74.1 0
16.30 75.1 0
18.00 76.1 0
19.30 77.1 0
21.00 78.1 0
22.30 79.1 0
0. 80.1 0
1.30 81.1 0
3.00 82.1 0
4.30 83.1 0
6.00 84.1 0
7.30 85.1 0
9.00 86.1 0
10.30 87.1 0
12.00 88.1 0
13.30 89.1 0
15.00 90.1 0
16.30 91.1 0
18.00 92.1 0
19.30 93.1 0
21.00 94.1 0
22.30 95.1 0
0. 96.1 0
1.30 97.1 0
3.00 98.1 0
4.30 99.1 0
6.00 100.1 0
7.30 101.1 0
9.00 102.1 0
10.30 103.1 0
12.00 104.1 0
13.30 105.1 0
15.00 106.1 0
16.30 107.1 0
18.00 108.1 0
19.30 109.1 0
21.00 110.1 0
22.30 111.1 0

12-0012010
13-3012110
15-3012210
16-3012310
18-3012410
19-3012510
21-0012610
22-3012710
0-12810
1-3012910
3-0013010
4-3013110
6-0013210
7-3013310
9-3013410
10-3013510
12-0013610
13-3013710
15-0013810
16-3013910
18-0014010
19-3014110
21-3014210
22-3014310
0-1441
1-301451
3-001461
4-301471
6-301481
7-301491
9-001501
10-301511
12-001521
13-301531
15-301541
16-301551
18-001561
19-301571
21-001581
22-301591
0-1601
1-301611
3-001621
4-301631
6-001641
7-301651
9-001661
10-301671
12-001681
13-301691
15-001701
16-301711
18-001721
19-301731
21-301741
22-301751
0-1761
1-301771
3-001781
4-301791
6-001801

9.001821
10.301831
12.001841
13.301851
15.001861
16.301871
18.001881
19.301891
21.001901
22.301911
0. 1921
1.301931
3.001941
4.301951
6.001961
7.301971
9.001981
10.301991
12.002001

•OVN•

WARNING *** TOP OF DAM, BOTTOM OF REACH, OR LOW-LEVEL OUTLET IS NOT WITHIN RANGE OF GIVEN ELEVATIONS IN STORAGE-ELEVATION DATA
 BOTTOM OF RESERVOIR ASSUMED TO BE AT 722.00
 STORAGE-ELEVATION DATA WILL BE EXTRAPOLATED ABOVE ELEVATION 795.00

STATION 1, PLAN 1, RATIO 5

END-OF-PERIOD HYDROGRAPH ORDINATES

OUTFLOW	
342.	338.
298.	295.
287.	288.
11745.	15269.
8376.	7329.
2821.	2648.
1426.	1355.
1066.	1036.
785.	763.
602.	586.
458.	446.
347.	341.
295.	291.
252.	243.
215.	212.
183.	180.
156.	154.
133.	131.
113.	111.
96.	95.
329.	333.
292.	293.
312.	296.
17322.	17338.
5611.	6400.
2324.	2482.
1290.	1323.
976.	1006.
725.	744.
555.	570.
421.	433.
331.	336.
282.	287.
240.	244.
205.	208.
175.	177.
149.	151.
127.	129.
108.	110.
92.	93.
320.	315.
292.	291.
457.	696.
15445.	13952.
4573.	4133.
2031.	1895.
1225.	1193.
918.	890.
706.	670.
540.	526.
410.	399.
326.	320.
273.	269.
237.	229.
202.	198.
172.	169.
146.	144.
125.	123.
106.	105.
90.	89.
311.	315.
290.	291.
1190.	12420.
10977.	3738.
3383.	1767.
1647.	1161.
1129.	836.
836.	652.
635.	484.
471.	377.
367.	310.
306.	260.
222.	226.
189.	192.
161.	164.
137.	140.
117.	119.
100.	101.
85.	86.
306.	311.
289.	290.
3078.	10977.
9582.	3383.
3063.	1647.
1533.	1098.
810.	836.
618.	652.
471.	484.
357.	377.
301.	310.
256.	260.
218.	222.
186.	189.
159.	161.
135.	137.
115.	117.
98.	100.
84.	85.

STORAGE

28714.	28679.	28644.	28608.	28573.	28538.	28503.	28468.	28433.	28399.
28369.	28346.	28332.	28326.	28325.	28324.	28320.	28310.	28299.	28288.
28292.	28297.	28355.	28480.	28720.	29229.	30273.	32064.	34522.	37192.
39498.	41061.	41933.	41933.	41651.	41136.	40502.	39820.	39131.	38465.
37838.	37255.	36737.	36297.	35914.	35570.	35261.	34984.	34736.	34512.
34307.	34115.	33930.	33754.	33587.	33427.	33276.	33134.	32999.	32872.
32753.	32639.	32527.	32414.	32301.	32187.	32075.	31963.	31852.	31742.
31533.	31526.	31421.	31318.	31217.	31117.	31020.	30925.	30832.	30741.
30652.	30566.	30481.	30398.	30316.	30236.	30158.	30081.	30006.	29933.
29862.	29792.	29724.	29657.	29592.	29528.	29467.	29406.	29347.	29290.
29234.	29180.	29126.	29075.	29024.	28975.	28927.	28881.	28836.	28791.
29747.	29706.	28665.	28624.	28584.	2854.	28505.	28467.	28429.	28392.
28355.	28319.	28283.	28248.	28214.	28180.	28146.	28113.	28081.	28049.
29018.	27987.	27956.	27926.	27897.	27868.	27839.	27811.	27783.	27756.
27729.	27703.	27677.	27651.	27626.	27601.	27577.	27553.	27529.	27506.
27483.	27461.	27439.	27417.	27395.	27374.	27353.	27333.	27313.	27293.
27274.	27254.	27235.	27217.	27199.	27181.	27163.	27145.	27128.	27111.
27095.	27078.	27062.	27047.	27031.	27016.	27001.	26986.	26971.	26957.
26943.	26929.	26915.	26901.	26888.	26875.	26862.	26850.	26837.	26825.
26813.	26801.	26789.	26778.	26766.	26755.	26744.	26734.	26723.	26713.
793.0	788.0	787.9	787.9	787.9	787.9	787.9	787.9	787.8	787.8
737.3	787.8	787.8	787.8	787.8	787.8	787.8	787.8	787.8	787.8
797.9	787.8	787.8	787.9	788.0	788.3	788.8	789.8	791.1	792.5
793.7	794.5	794.9	795.0	794.8	794.5	794.2	793.5	793.1	793.1
792.8	792.5	792.2	792.0	791.8	791.6	791.5	791.3	791.2	791.1
791.0	790.8	790.8	790.7	790.6	790.5	790.4	790.3	790.3	790.2
790.1	790.1	790.0	790.0	789.9	789.8	789.8	789.7	789.7	789.6

STAGE

793.0	788.0	787.9	787.9	787.9	787.9	787.9	787.9	787.8	787.8
737.3	787.8	787.8	787.8	787.8	787.8	787.8	787.8	787.8	787.8
797.9	787.8	787.8	787.9	788.0	788.3	788.8	789.8	791.1	792.5
793.7	794.5	794.9	795.0	794.8	794.5	794.2	793.5	793.1	793.1
792.8	792.5	792.2	792.0	791.8	791.6	791.5	791.3	791.2	791.1
791.0	790.8	790.8	790.7	790.6	790.5	790.4	790.3	790.3	790.2
790.1	790.1	790.0	790.0	789.9	789.8	789.8	789.7	789.7	789.6

739.5	739.5	739.4	789.4	789.3	789.3	789.2	789.1	789.1
739.0	739.0	738.9	788.9	788.8	788.8	788.7	788.7	788.6
738.6	738.6	738.5	788.5	788.4	788.4	788.3	788.3	788.3
738.3	738.2	738.2	788.2	788.1	788.1	788.1	788.0	788.0
738.0	738.0	738.0	787.9	787.9	787.9	787.9	787.8	787.8
737.8	737.8	737.8	787.7	787.7	787.7	787.7	787.7	787.6
737.6	737.6	737.6	787.6	787.6	787.5	787.5	787.5	787.5
737.5	737.5	737.4	787.4	787.4	787.4	787.4	787.4	787.4
737.3	737.3	737.3	787.3	787.3	787.3	787.3	787.3	787.2
737.2	737.2	737.2	787.2	787.2	787.2	787.2	787.2	787.1
737.1	737.1	737.1	787.1	787.1	787.1	787.1	787.1	787.1
737.0	737.0	737.0	787.0	787.0	787.0	787.0	787.0	787.0
737.0	737.0	737.0	787.0	787.0	787.0	786.9	786.9	786.9

PEAK OUTFLOW IS 17322. AT TIME 51.00 HOURS

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
CFS	17322.	16594.	11213.	5047.	288373.
CMS	491.	470.	319.	143.	8166.
INCHES		3.94	10.66	14.39	17.13
AC-FT		100.15	270.69	365.50	435.10
THOUS CU M		8228.	22241.	30031.	35749.
		10149.	27434.	37042.	44095.

STATION 1

[illegible]

12.0012010
13.3012110
15.0012210
16.3012310
18.0012410
19.3012510
21.0012610
22.3012710
0. 12810
1.3012910
3.0013010
4.3013110
6.0013210
7.3013310
9.0013410
10.3013510
12.0013610
13.3013710
15.0013810
16.3013910
18.0014010
19.3014110
21.0014210
22.3014310
0. 14410
1.3014510
3.0014610
4.3014710
6.0014810
7.3014910
9.0015010
10.3015110
12.0015210
13.3015310
15.0015410
16.3015510
18.0015610
19.3015710
21.0015810
22.3015910
0. 16010
1.3016110
3.0016210
4.3016310
6.0016410
7.3016510
9.0016610
10.3016710
12.0016810
13.3016910
15.0017010
16.3017110
18.0017210
19.3017310
21.0017410
22.3017510
0. 17610
1.3017710
3.0017810
4.3017910
6.0018010
7.3018110

9.001821
10.301831
12.001841
13.301851
15.001861
16.301871
18.001881
19.301891
21.001901
22.301911
0. 1921
1.301931
3.001941
4.301951
6.001961
7.301971
9.001981
10.301991
12.002001

•OVN•

WARNING *** TOP OF DAM, BOTTOM OF DREACH, OR LOW-LEVEL OUTLET IS NOT WITHIN RANGE OF GIVEN ELEVATIONS IN STORAGE-ELEVATION DATA
 BOTTOM OF RESERVOIR ASSIGNED TO BE AT 722.00
 STORAGE-ELEVATION DATA WILL BE EXTRAPOLATED ABOVE ELEVATION 795.00

STATION 1, PLAN 1, RATIO 6

END-OF-PERIOD HYDROGRAPH ORDINATES

OUTFLOW									
343.	338.	334.	329.	325.	321.	316.	312.	307.	303.
299.	297.	296.	296.	297.	298.	298.	298.	297.	296.
296.	300.	310.	331.	339.	336.	851.	1742.	5292.	11104.
17011.	20993.	22719.	22712.	21611.	19895.	17879.	15772.	13710.	11900.
10291.	8855.	7756.	6817.	6013.	5376.	4881.	4434.	4029.	3662.
3329.	3229.	2807.	2641.	2481.	2327.	2180.	2040.	1907.	1780.
1661.	1548.	1442.	1360.	1329.	1297.	1264.	1232.	1200.	1168.
1136.	1105.	1074.	1043.	1013.	983.	954.	925.	897.	870.
843.	817.	791.	769.	749.	730.	711.	693.	675.	657.
640.	623.	606.	590.	575.	559.	544.	530.	515.	501.
438.	474.	462.	449.	437.	425.	413.	402.	391.	380.
370.	359.	349.	343.	338.	332.	327.	322.	317.	312.
307.	302.	297.	293.	289.	283.	279.	275.	270.	266.
262.	258.	253.	249.	245.	242.	238.	234.	230.	227.
223.	219.	216.	213.	209.	206.	203.	199.	196.	193.
190.	187.	184.	181.	178.	175.	173.	170.	167.	165.
162.	159.	157.	154.	152.	149.	147.	145.	142.	140.
138.	136.	134.	131.	129.	127.	125.	123.	121.	119.
117.	116.	114.	112.	110.	108.	107.	105.	103.	102.
100.	99.	97.	95.	94.	92.	91.	89.	88.	87.

STORAGE

28716.	28682.	28648.	28614.	28580.	28546.	28512.	28478.	28444.	28411.
28352.	28362.	28353.	28355.	28362.	28370.	28372.	28369.	28362.	28356.
28357.	28383.	28463.	28427.	28433.	28474.	30982.	33106.	36074.	39192.
41801.	43492.	44226.	44223.	43755.	43026.	42170.	41275.	40399.	39572.
38804.	38105.	37492.	36970.	36522.	36132.	35786.	35472.	35188.	34931.
34698.	34487.	34292.	34107.	33928.	33757.	33593.	33437.	33289.	33148.
33015.	32889.	32771.	32653.	32547.	32435.	32323.	32211.	32099.	31987.
31876.	31767.	31659.	31552.	31447.	31343.	31242.	31142.	31044.	30949.
30856.	30764.	30675.	30588.	30503.	30420.	30338.	30258.	30179.	30102.
30027.	29953.	29881.	29811.	29742.	29675.	29610.	29546.	29484.	29423.
29364.	29306.	29250.	29195.	29141.	29089.	29038.	28989.	28941.	28894.
28348.	28306.	28261.	28218.	28177.	28136.	28095.	28056.	28016.	27978.
28440.	28402.	28365.	28329.	28293.	28258.	28224.	28189.	28156.	28123.
28090.	28058.	28027.	27996.	27965.	27935.	27905.	27876.	27847.	27819.
27791.	27764.	27737.	27710.	27684.	27659.	27633.	27608.	27584.	27560.
27536.	27513.	27490.	27467.	27445.	27423.	27401.	27380.	27359.	27339.
27319.	27299.	27279.	27260.	27241.	27222.	27204.	27186.	27168.	27150.
27133.	27116.	27100.	27083.	27067.	27051.	27035.	27020.	27005.	26990.
26975.	26961.	26947.	26933.	26919.	26905.	26892.	26879.	26866.	26853.
26828.	26828.	26816.	26804.	26793.	26781.	26770.	26759.	26748.	26737.

STAGE

788.0	788.0	787.9	787.9	787.9	787.9	787.9	787.9	787.8	787.8
787.8	787.8	787.8	787.8	787.8	787.8	787.8	787.8	787.8	787.8
787.8	787.8	787.9	787.9	788.1	788.4	789.1	790.3	791.9	793.5
794.9	795.8	796.2	796.2	795.9	795.5	795.1	794.6	794.2	793.7
793.3	792.9	792.6	792.4	792.1	791.9	791.7	791.6	791.4	791.3
791.2	791.0	790.9	790.8	790.8	790.7	790.6	790.5	790.4	790.3
790.3	790.2	790.1	790.1	790.0	790.0	789.9	789.8	789.8	789.7

789.7	789.6	789.6	789.5	789.4	789.4	789.3	789.3	789.2	789.2
789.1	789.1	789.0	789.0	788.9	788.9	788.8	788.8	788.7	788.7
788.7	788.6	788.6	788.6	788.5	788.5	788.5	788.4	788.4	788.4
788.3	788.3	788.3	788.2	788.2	788.2	788.2	788.1	788.1	788.1
788.1	788.0	788.0	788.0	788.0	787.9	787.9	787.9	787.9	787.9
787.8	787.8	787.8	787.8	787.8	787.7	787.7	787.7	787.7	787.7
787.7	787.6	787.6	787.6	787.6	787.6	787.6	787.5	787.5	787.5
787.5	787.5	787.5	787.5	787.4	787.4	787.4	787.4	787.4	787.4
787.4	787.4	787.3	787.3	787.3	787.3	787.3	787.3	787.3	787.3
787.3	787.2	787.2	787.2	787.2	787.2	787.2	787.2	787.2	787.2
787.2	787.1	787.1	787.1	787.1	787.1	787.1	787.1	787.1	787.1
787.1	787.1	787.1	787.1	787.0	787.0	787.0	787.0	787.0	787.0
787.0	787.0	787.0	787.0	787.0	787.0	787.0	787.0	787.0	786.9

PEAK OUTFLOW IS 22719. AT TIME 49.50 HOURS

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
CFS	22719.	21871.	14667.	6412.	356242.
CMS	643.	619.	415.	182.	10088.
INCHES		5.20	13.94	18.28	21.16
AC-FT		132.00	354.08	464.35	537.50
THOUS CU M		10845.	29092.	38152.	44162.
		13377.	35885.	47060.	54473.

STATION 1

INFLOW(I), OUTFLOW(O) AND OBSERVED FLOW(*)

[illegible]

15.00 58. 1 0
16.30 59. 1 0
18.03 60. 1 0
19.30 61. 1 0
21.00 62. 1 0
22.33 63. 1 0
0. 64. 1 0
1.33 65. 1 0
3.00 66. 1 0
4.30 67. 1 0
6.00 68. 1 0
7.33 69. 1 0
9.00 70. 1 0
10.30 71. 1 0
12.00 72. 1 0
13.30 73. 1 0
15.00 74. 1 0
16.33 75. 1 0
18.00 76. 1 0
19.30 77. 1 0
21.03 78. 1 0
22.30 79. 1 0
0. 80. 1 0
1.33 81. 1 0
3.00 82. 1 0
4.33 83. 1 0
6.00 84. 1 0
7.33 85. 1 0
9.03 86. 1 0
10.30 87. 1 0
12.00 88. 1 0
13.30 89. 1 0
15.00 90. 1 0
16.30 91. 1 0
18.00 92. 1 0
19.30 93. 1 0
21.00 94. 1 0
22.33 95. 1 0
0. 96. 1 0
1.30 97. 1 0
3.00 98. 1 0
4.30 99. 1 0
6.00 100. 1 0
7.30 101. 1 0
9.00 102. 1 0
10.30 103. 1 0
12.00 104. 1 0
13.30 105. 1 0
15.00 106. 1 0
16.30 107. 1 0
18.00 108. 1 0
19.30 109. 1 0
21.00 110. 1 0
22.30 111. 1 0
0. 112. 1 0
1.30 113. 1 0
3.00 114. 1 0
4.30 115. 1 0
6.00 116. 1 0
7.30 117. 1 0
9.00 118. 1 0
10.30 119. 1 0

12.0012010.....
13.3012110.....
15.0012210.....
16.3012310.....
18.0012410.....
19.3012510.....
21.0012610.....
22.3012710.....
0. 12810.....
1.3012910.....
3.0013010.....
4.3013110.....
6.0013210.....
7.3013310.....
9.0013410.....
10.3013510.....
12.0013610.....
13.3013710.....
15.0013810.....
16.3013910.....
18.0014010.....
19.3014110.....
21.0014210.....
22.3014310.....
0. 14410.....
1.3014510.....
3.0014610.....
4.3014710.....
6.001481.....
7.301491.....
9.001501.....
10.301511.....
12.001521.....
13.301531.....
15.001541.....
16.301551.....
18.001561.....
19.301571.....
21.001581.....
22.301591.....
0. 1601.....
1.301611.....
3.001621.....
4.301631.....
6.001641.....
7.301651.....
9.001661.....
10.301671.....
12.001681.....
13.301691.....
15.001701.....
16.301711.....
18.001721.....
19.301731.....
21.001741.....
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0. 1761.....
1.301771.....
3.001781.....
4.301791.....
6.001801.....
7.301811.....

9.001821
10.301831
12.001841
13.301851
15.001861
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18.001881
19.301891
21.001901
22.301911
0.1921
1.301921
3.001941
4.301951
6.001961
7.301971
9.001981
10.301991
12.002001.

PEAK FLOW AND STORAGE (END OF PERIOD) SUMMARY FORMULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)
 AREA IN SQUARE MILES (SQUARE KILOMETERS)

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO FLOWS					
				RATIO 1	RATIO 2	RATIO 3	RATIO 4	RATIO 5	RATIO 6
				0.20	0.40	0.50	0.60	0.80	1.00
HYDROGRAPH AT	1	39.15 (58842.09)	1	7082. (200.55)	14165. (401.10)	17706. (501.38)	21247. (601.66)	28330. (802.21)	35412. (1002.76)
ROUTED TO	1	39.15 (58842.09)	1	1807. (51.17)	6347. (179.72)	8901. (252.04)	11690. (331.03)	17322. (490.50)	22719. (643.33)

SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1

ELEVATION STORAGE OUTFLOW	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM	RATIO OF PMF	MAXIMUM RESERVOIR W.S. ELEV	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
	788.00	786.60	798.10	0.20	33178.	1807.	0.	0.	57.00	0.
	28750.	26067.	47890.	0.40	36708.	6347.	0.	0.	52.50	0.
	347.	0.	31345.	0.50	38130.	8901.	0.	0.	52.50	0.
				0.60	39472.	11690.	0.	0.	51.00	0.
				0.80	41933.	17322.	0.	0.	51.00	0.
				1.00	44226.	22719.	0.	0.	49.50	0.

APPENDIX D
STABILITY ANALYSIS

AD-A109 965

NEW YORK STATE DEPT OF ENVIRONMENTAL CONSERVATION ALBANY F/G 13/13
NATIONAL DAM SAFETY PROGRAM. OTISCO LAKE DAM (INVENTORY NUMBER --ETC(U)
SEP 81 G KOCH DACW51-79-C-0001

UNCLASSIFIED

NL

2 of 2

ALL INFORMATION CONTAINED HEREIN IS UNCLASSIFIED



END

DATE

TIME

02:22

DTIC



MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS-1963-A

STRUCTURAL STABILITY ANALYSIS

The analysis of the spillway section was based on information shown on the plans and on measurements made at the time of the inspection. A normal analysis was performed including both overturning and sliding analyses. Due to unknown foundation conditions, full uplift was assumed at the upstream toe, decreasing to the tailwater pressure at the downstream toe. It was assumed that the base of the dam is at the same level as the bottom of the spillway apron (which is 3 feet thick).

ANALYSIS CONDITIONS

1. Normal conditions; water surface at spillway crest
2. Same as #1 plus ice load of 5,000 pounds per linear foot
3. 1/2 PMF flow; water surface 6.4 feet above the spillway crest
4. PMF flow; water surface 9.6 feet above spillway crest
5. Seismic Conditions - Water at Spillway Crest with seismic coefficient of 0.1.



STABILITY ANALYSIS PROGRAM - WORK SHEET

INPUT ENTRY

ANALYSIS CONDITION

		1	2	3	4	5
Unit Weight of Dam (K/ft ³)	0	0.15	0.15	0.15	0.15	0.15
Area of Segment No. 1 (ft ²)	1	30.75	30.75	30.75	30.75	30.75
Distance from Center of Gravity of Segment No. 1 to Downstream Toe (ft)	2	20.25	20.25	20.25	20.25	20.25
Area of Segment No. 2 (ft ²)	3	52.5	52.5	52.5	52.5	52.5
Distance from Center of Gravity of Segment No. 2 to Downstream Toe (ft)	4	18.25	18.25	18.25	18.25	18.25
Area of Segment No. 3 (ft ²)	5	90	90	90	90	90
Distance from Center of Gravity of Segment No. 3 to Downstream Toe (ft)	6	13	13	13	13	13
Base Width of Dam (Total) (ft)	7	21	21	21	21	21
Height of Dam (ft)	8	21	21	21	21	21
Ice Loading (K/L ft.)	9	—	5.0	—	—	—
Coefficient of Sliding	10	0.55	0.55	0.55	0.55	0.55
Unit Weight of Soil (K/ft ³) (does not include 18)	11	0.055	0.055	0.055	0.055	0.055
Active Soil Coefficient - Ka	12	0.33	0.33	0.33	0.33	0.33
Passive Soil Coefficient - Kp	13	3.00	3.0	3.0	3.0	3.0
Height of Water over Top of Dam or Spillway (ft)	14	—	—	6.36	9.57	—
Height of Soil for Active Pressure (ft)	15	3.0	3.0	3.0	3.0	3.0
Height of Soil for Passive Pressure (ft)	16	3.0	3.0	3.0	3.0	3.0
Height of Water in Tailrace Channel (ft)	17	5.0	5.0	7.0	7.0	5.0
Weight of Water (K/ft ³)	18	0.0624	0.0624	0.0624	0.0624	0.0624
Area of Segment No. 4 (ft ²)	19	72	72	72	72	72
Distance from Center of Gravity of Segment No. 4 to Downstream Toe (ft)	20	11	11	11	11	11
Height of Ice Load or Active Water (ft) (does not include 14)	46	21	21	21	21	21
Seismic Coefficient (g)	50	—	—	—	—	0.1
AREA OF SEG. 5 →	21	22.5	22.5	22.5	22.5	22.5
DIST. TO CENT. OF GRAVITY TO D.S. TOE SEG. 5 →	22	2.25	2.25	2.25	2.25	2.25
<u>RESULTS OF ANALYSIS</u>						
Factor of Safety vs. Overturning		1.73	1.30	1.33	1.19	1.66
Distance From Toe to Resultant		9.91	5.37	6.12	4.15	9.33
Factor of Safety vs. Sliding		1.03	0.76	0.64	0.54	0.73

APPENDIX E
REFERENCES

E

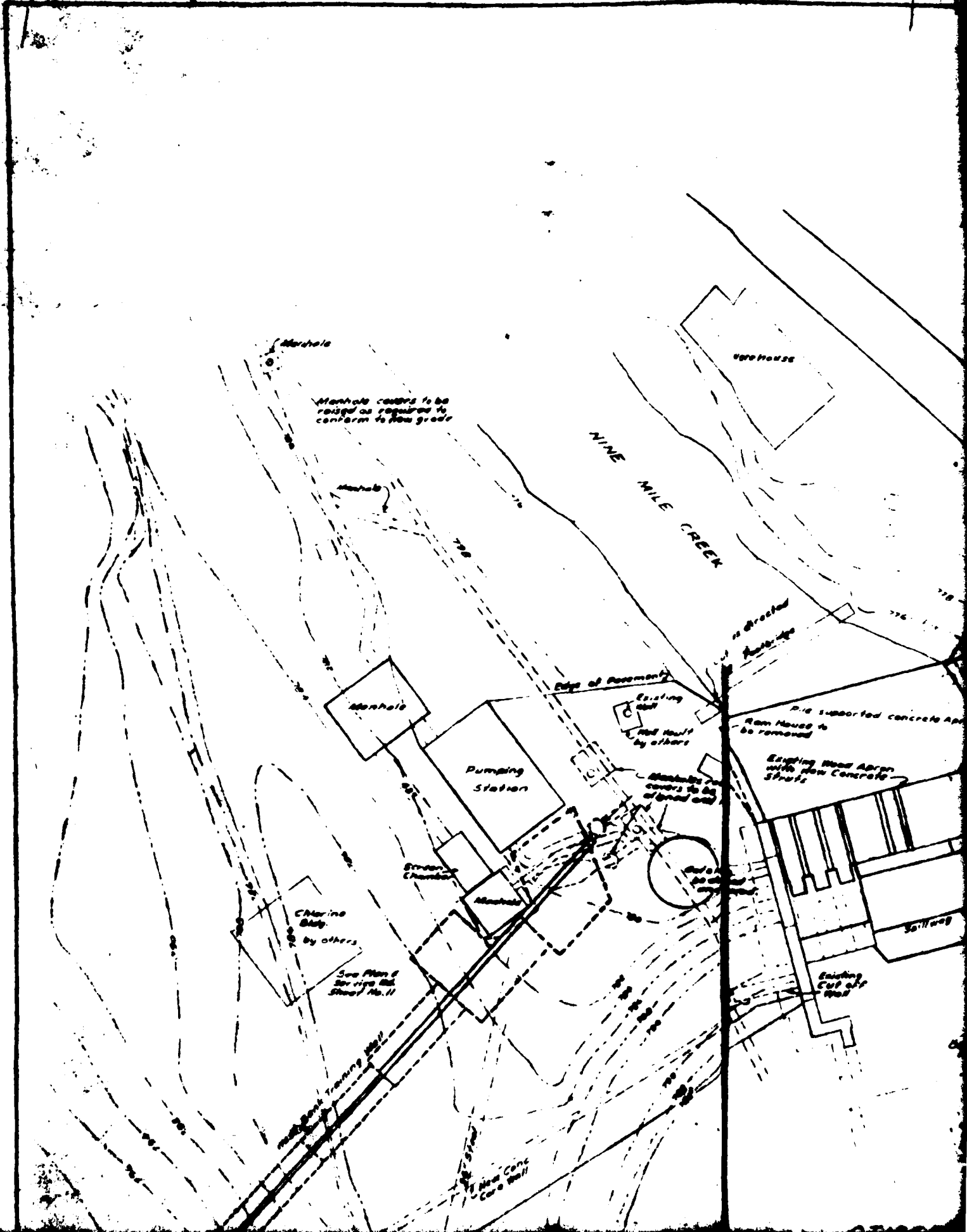
APPENDIX E

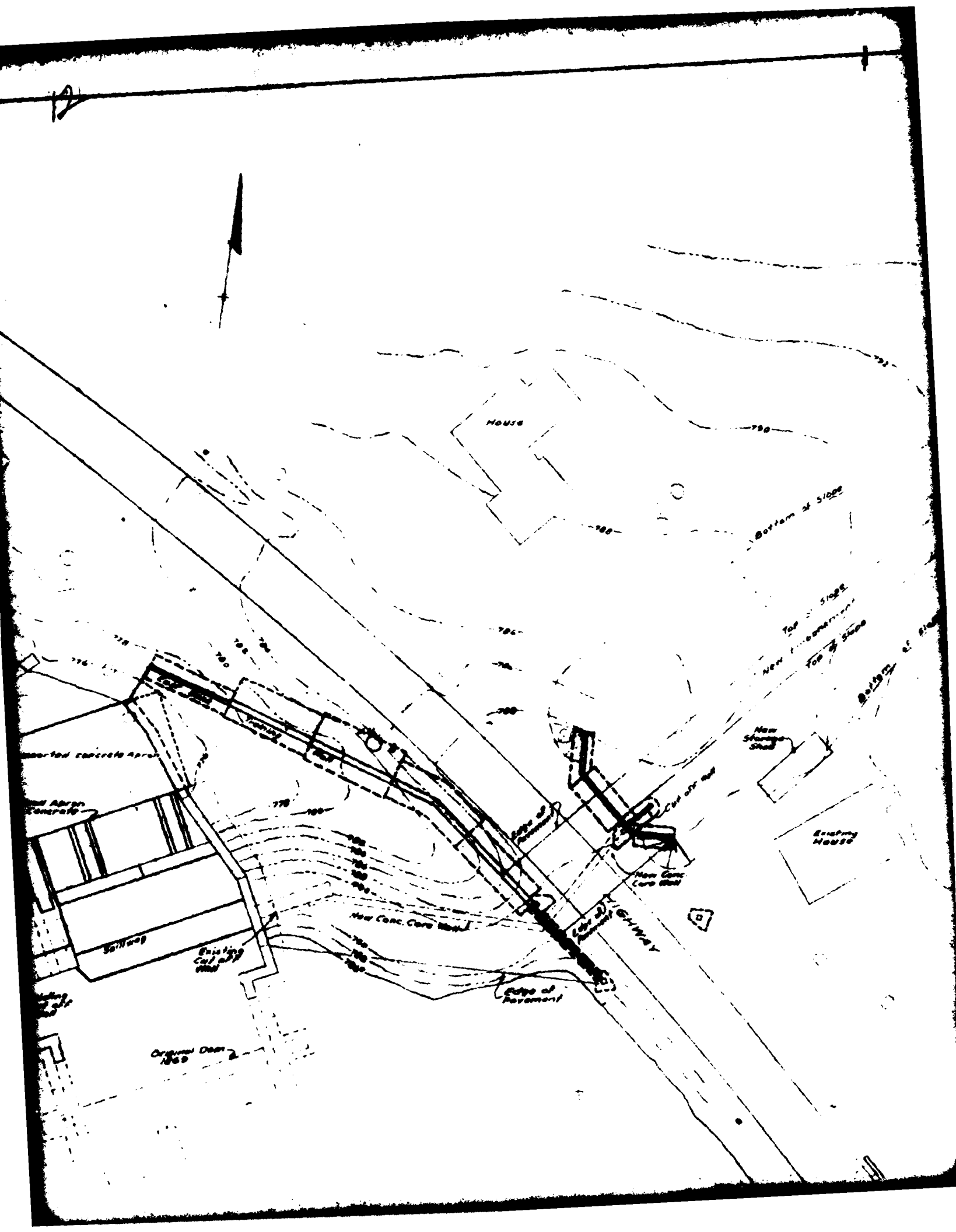
REFERENCES

- 1) U.S. Department of Commerce, Technical Paper No. 40, Rainfall Frequency Atlas of the United States, May 1961,
- 2) U.S. Department of Commerce, Hydrometeorological Report No. 33, Seasonal Variation of the Probable Maximum Precipitation East of the 105th Meridian for Areas from 10 to 1,000 Square Miles and Durations of 6, 12, 24, and 48 Hours; April 1956.
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- 4) H.W. King and E.F. Brater, Handbook of Hydraulics, 5th edition, McGraw-Hill, 1963.
- 5) T.W. Lambe and R.V. Whitman, Soil Mechanics, John Wiley and Sons, 1965.
- 6) W.D. Thornbury, Principles of Geomorphology, John Wiley and Sons, 1969.
- 7) University of the State of New York; Geology of New York, Education Leaflet 20, Reprinted 1973.
- 8) Cornell University Agriculture Experiment Station (compiled by M.G. Cline and R.L. Marshall), General Soil Map of New York State and Soils of New York Landscapes, Information Bulletin 119, 1977,

APPENDIX F

DRAWINGS





13

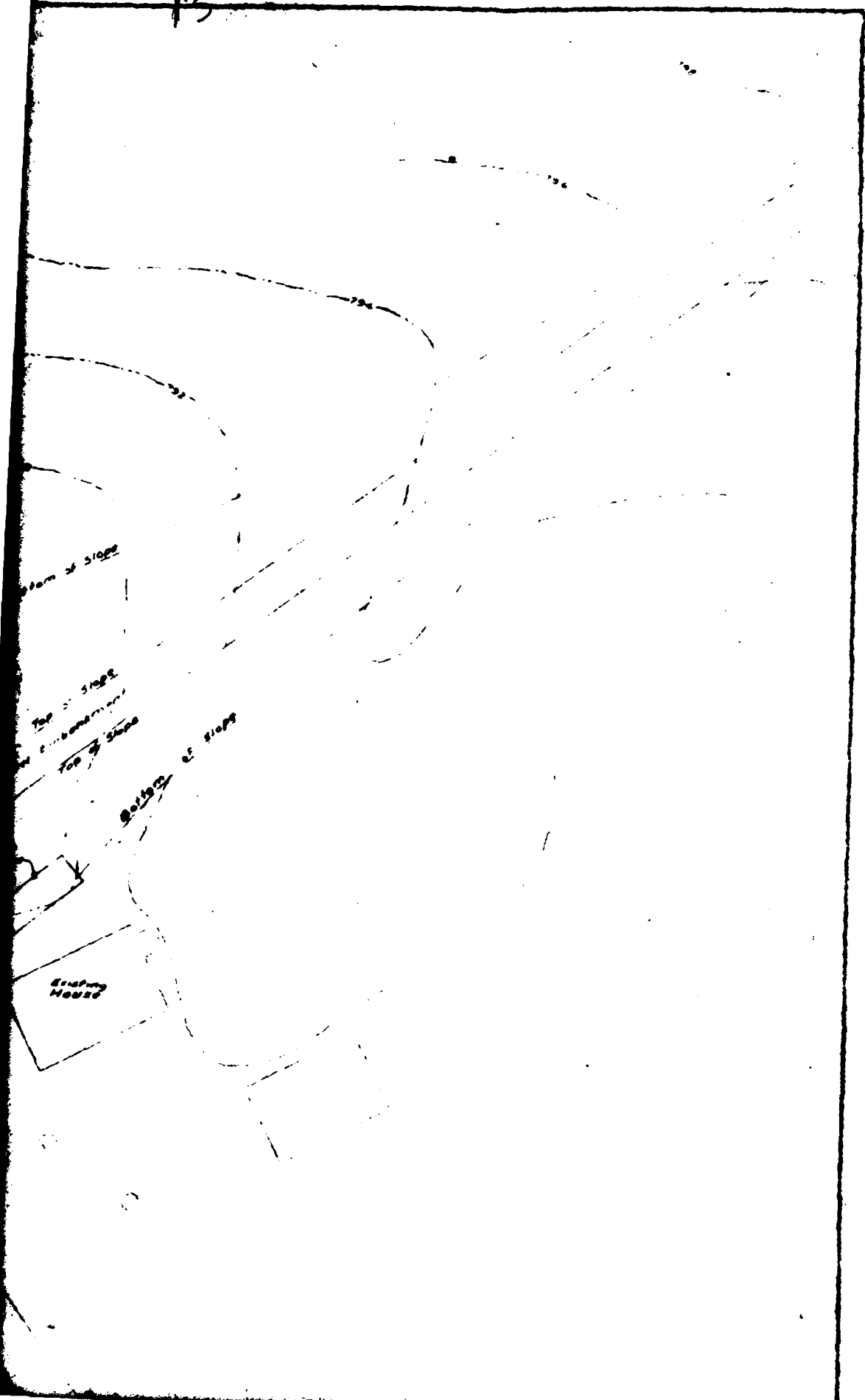
Bottom of Slope

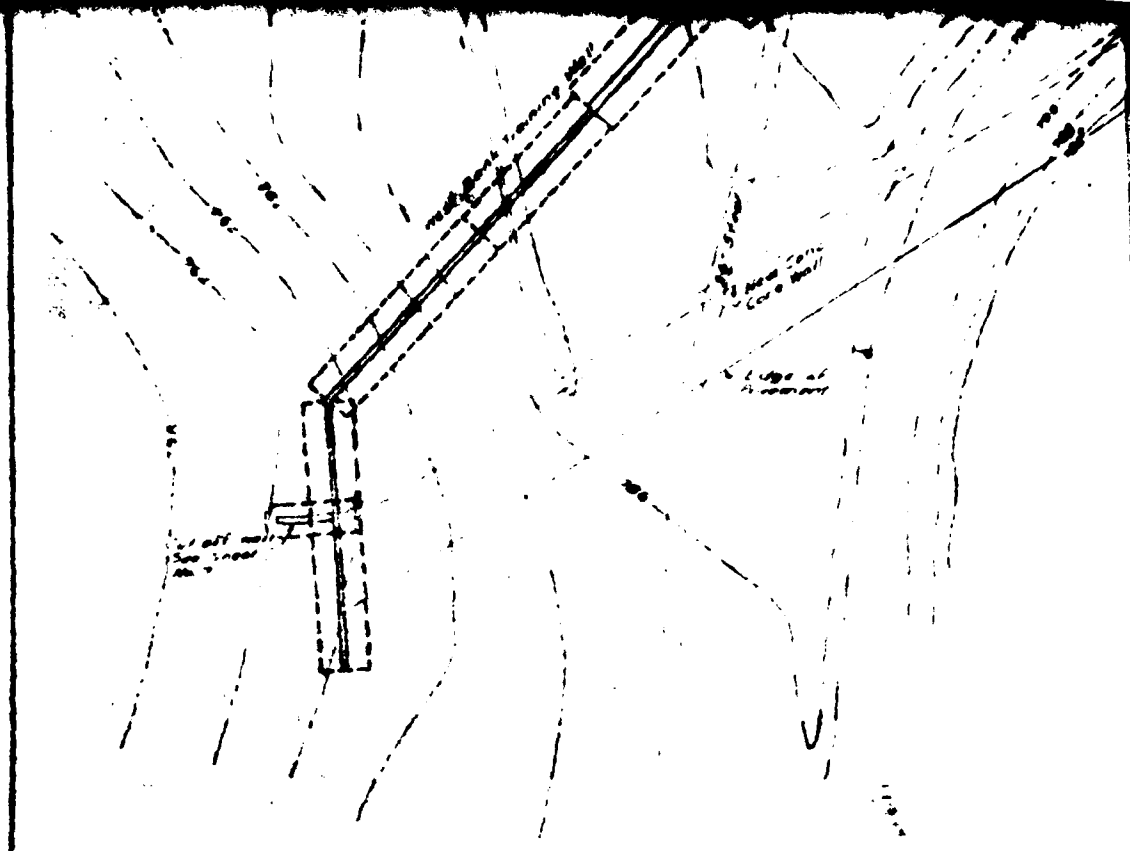
Top of Slope

Top of Slope

Bottom of Slope

Grading House

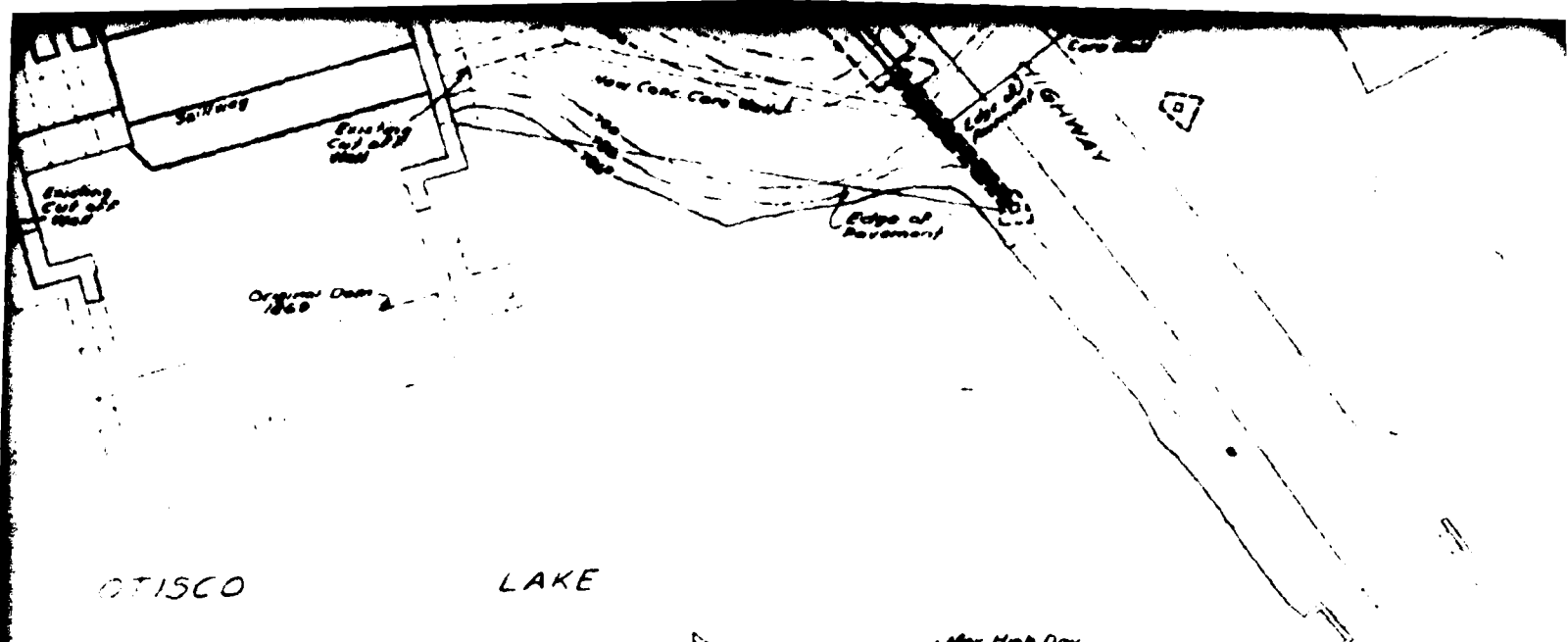




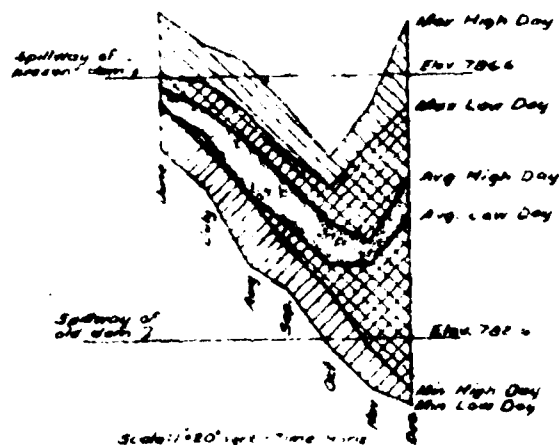
OTISCO

IN CHARGE OF W.E.N.
DESIGNED BY J.J.H.
MADE BY J.E.G.E.D.
DRAWN BY

4



OTISCO LAKE



Scale: 1" = 80' vert. Time: 1 hr
OTISCO LAKE LEVELS
 Maximum + Minimum Daily Levels
 By Months
 1948 - 1959

15

Erasmus
House

ONONDAGA COUNTY WATER AUTHORITY
OTISCO LAKE DAM MODIFICATIONS
GENERAL PLAN

O'BRIEN & GERE
CONSULTING ENGINEERS & LAND SURVEYORS
SYRACUSE, NEW YORK

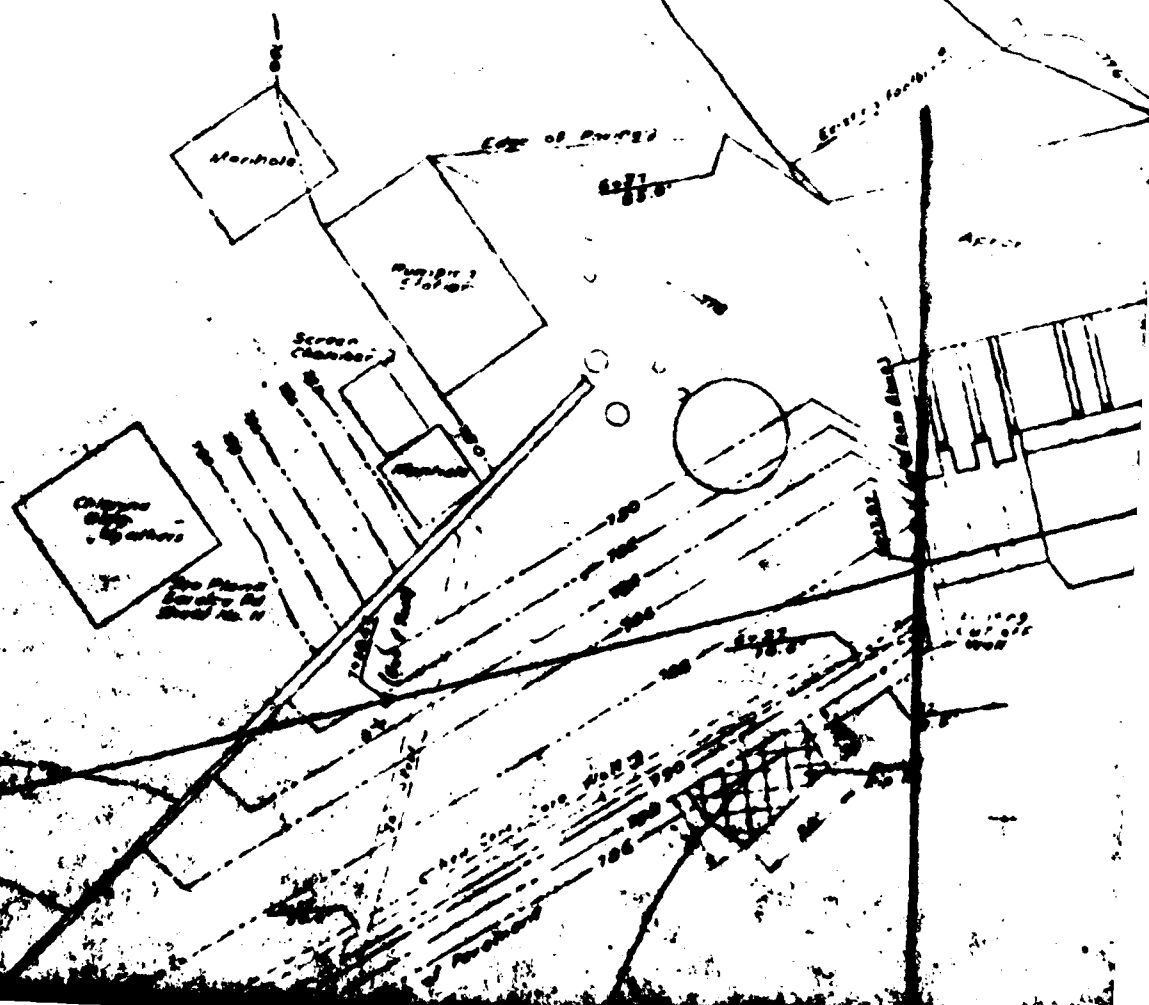
DATE: MAY 5, 1981

SCALE: 1" = 20'

FILE NO: 2095 15-081

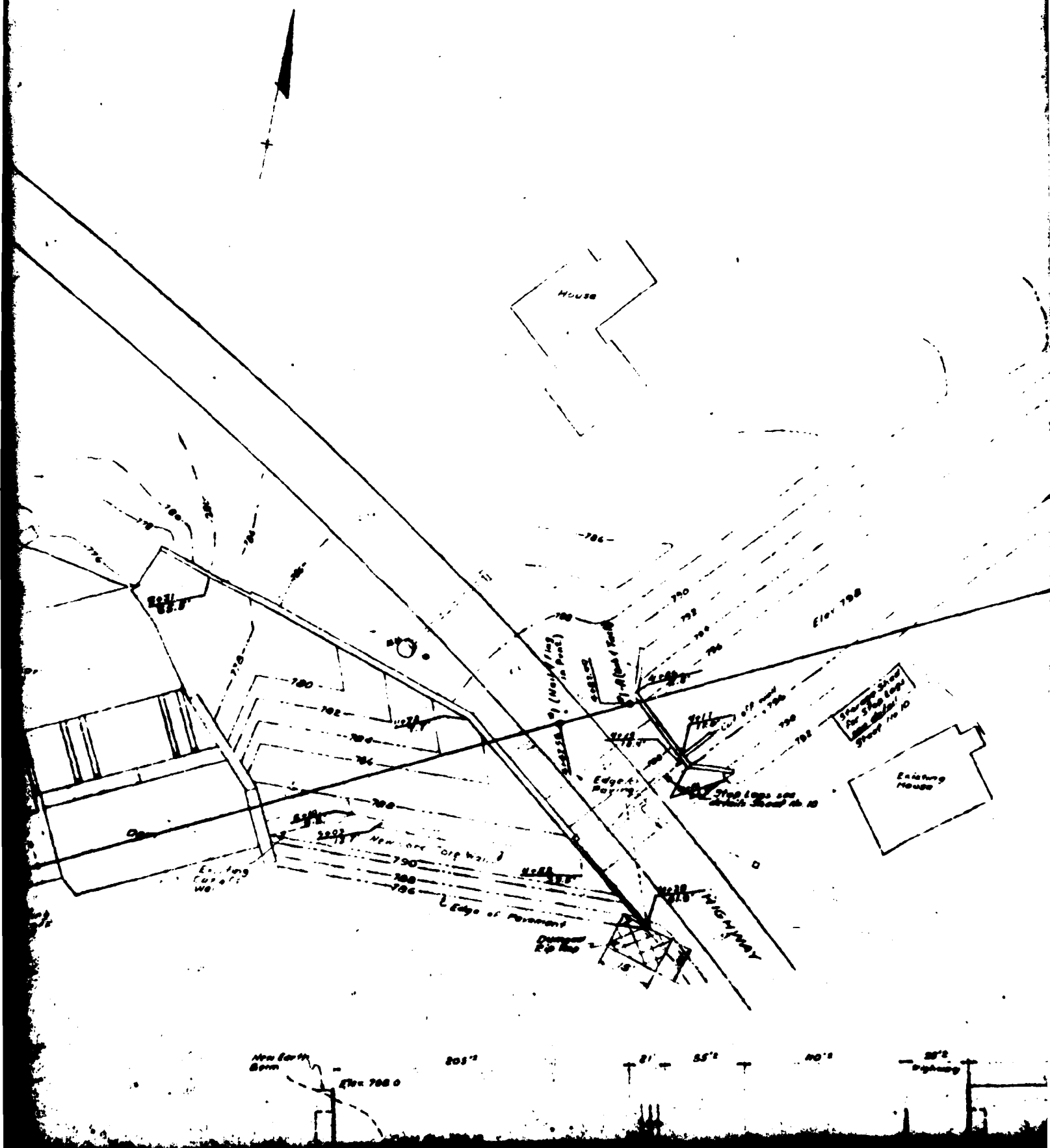
B 1

16

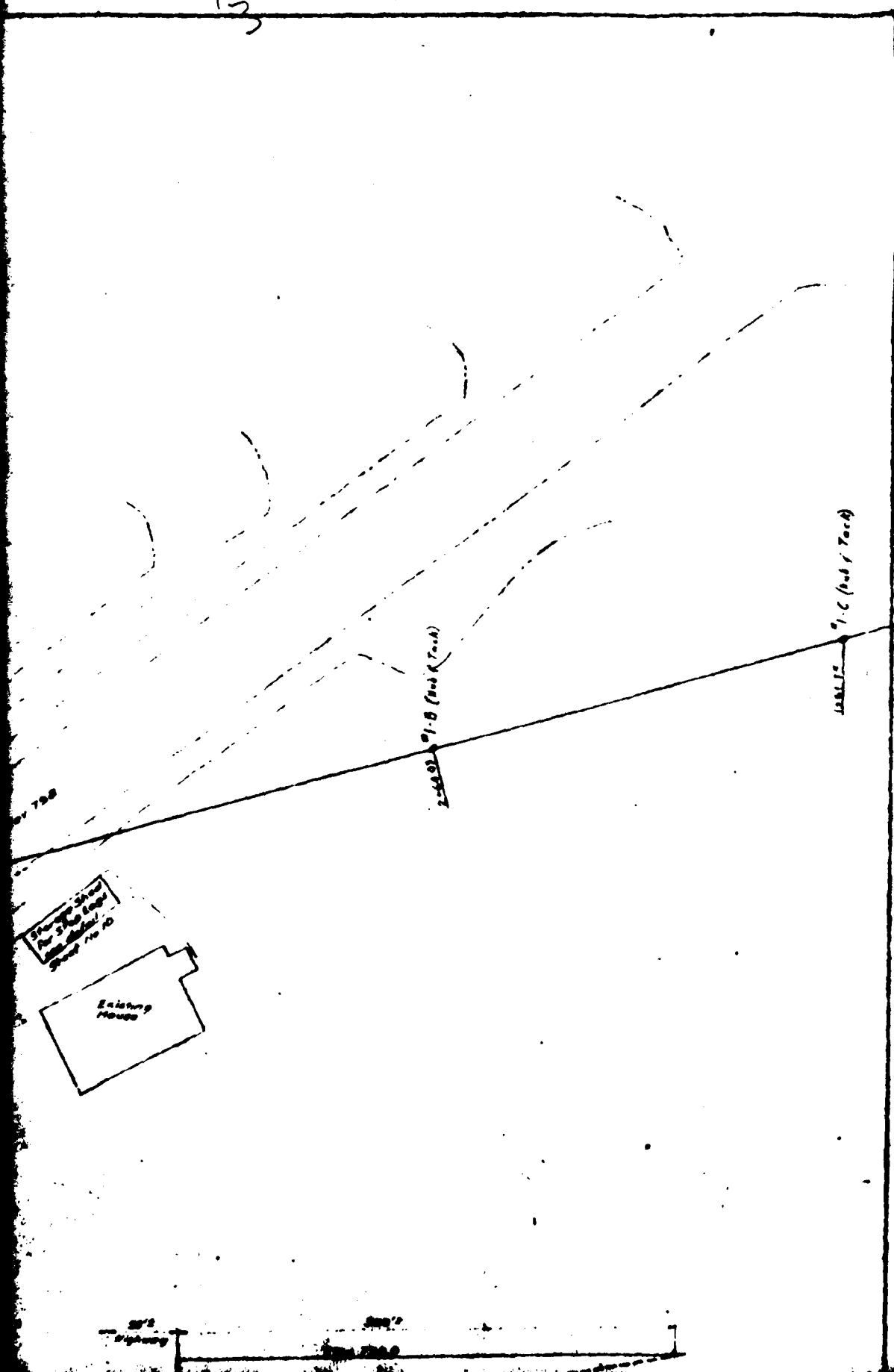


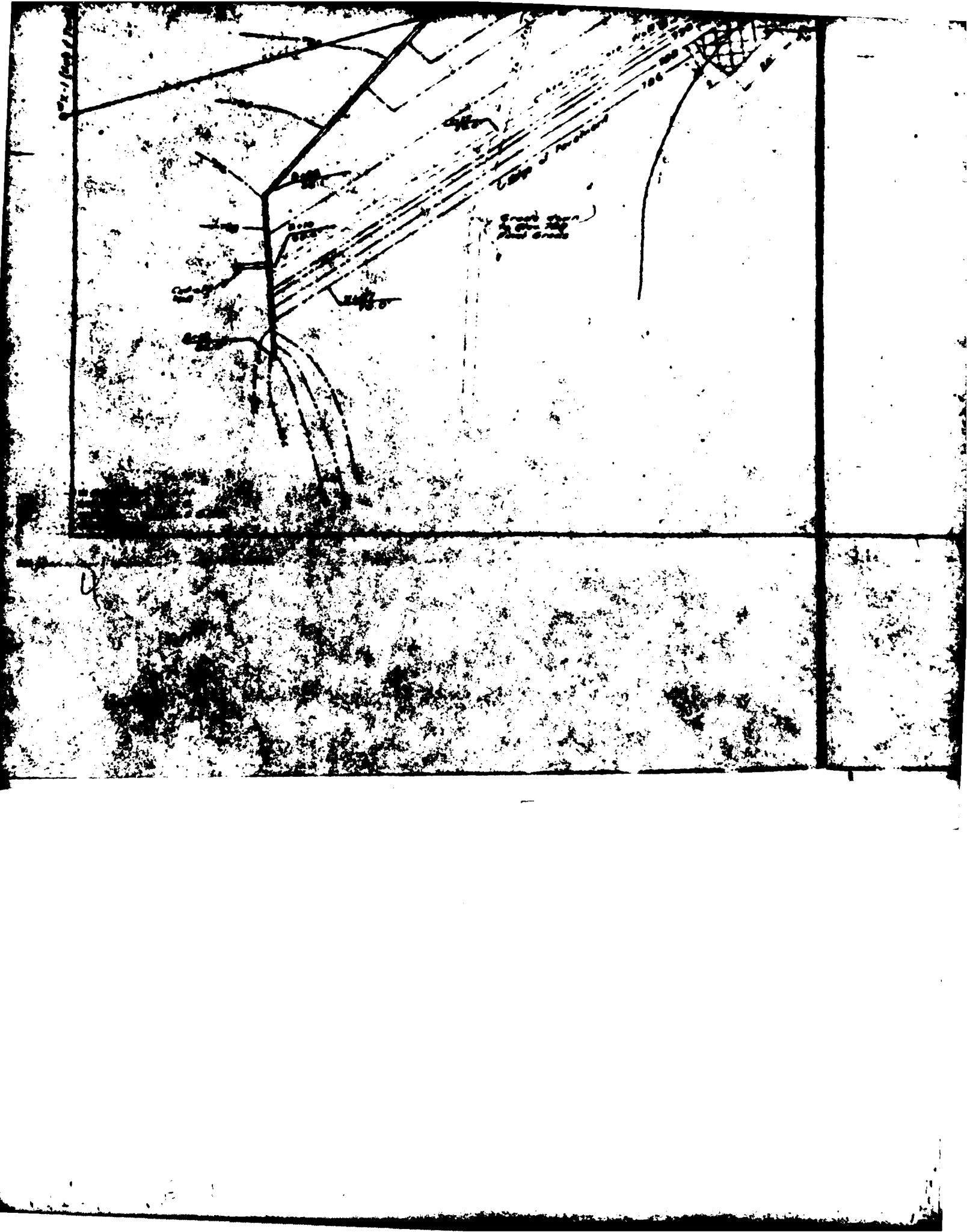
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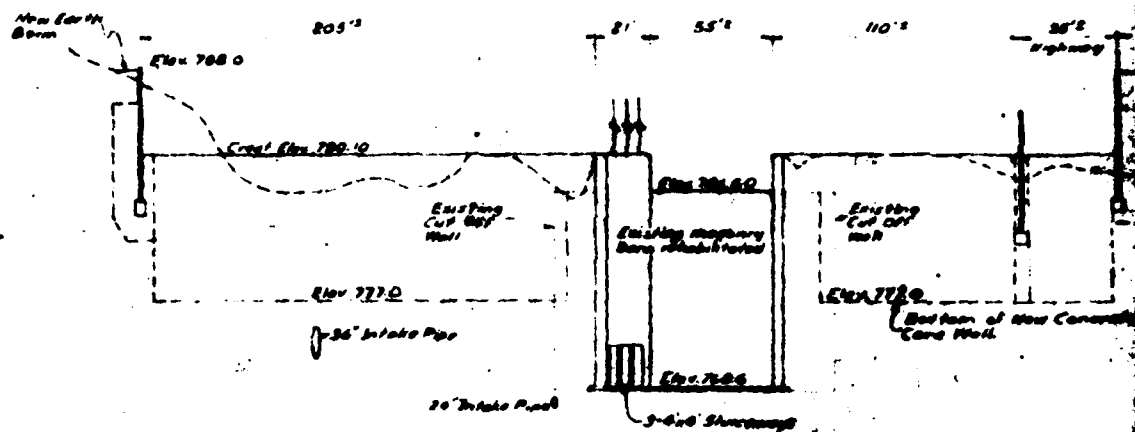
1



13







ELEVATION OF DAM & EMBANKMENTS ALONG CRESTS

(All sections viewed from a perpendicular)

Scale: Horiz. 1" = 50'
Vert. 1" = 10'

NO LONGER
IN USE

Existing
House

110'±

35'±

300'±

Elev 728.0

Existing
Cut Off
Wall

Elev 722.0

Bottom of New Concrete
Core Wall

EMBANKMENTS

1975

(unpublished)

CHONDAGA COUNTY WATER AUTHORITY
OTISCO LAKE DAM MODIFICATIONS
FINAL CONTOUR PLAN

DATE MAY 15, 1988

BY [illegible]

FILE NO. 10-1-10-10

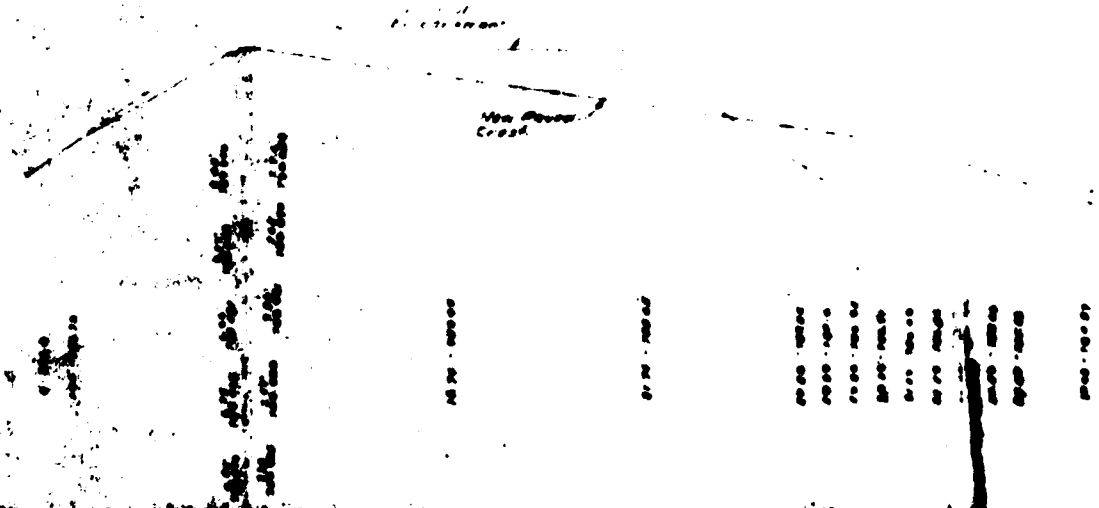
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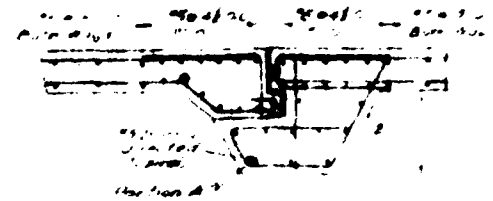
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6

[illegible]

**TYPICAL TRANSVERSE JOINT
BETWEEN PAVEMENT SLABS**





TYPICAL LONGITUDINAL JOINT BETWEEN PAVEMENT SLABS

20'-0"

See also Section A-1
for details of joint
construction



TYPICAL SECTION THRU EMBANKMENT



TYPICAL CREST VENT Not to scale

DETAIL OF NEW

18" x 8" Dia Plate
for 8 Struts
18" x 8" Dia Plate
for 6 Struts
8" long out

Location of
Jacks



Reinforced Concrete

**JOINT
SLABS**

AIR VENT DETAIL

**TYPICAL JOINT
WITH NEW MASONRY**

TAIL OF NEW EMBANKMENT

Not To Scale

Top view of "Joint Slab"

CREST ELEVATIONS

James M. ...
June 1968

Paraphrase
John 1:1-18

1. What are the main points of the article?
 2. What are the author's conclusions?
 3. What are the author's recommendations?

Approved:
5 June 6 1100

TYPICAL JOINT SEALS
W/ 1 WALLS
OF 70 S.I.W.O.

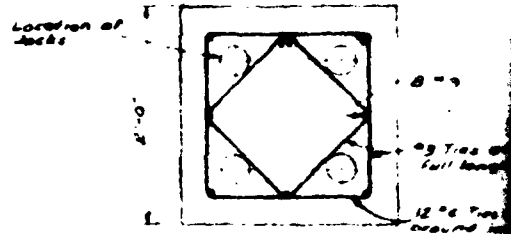
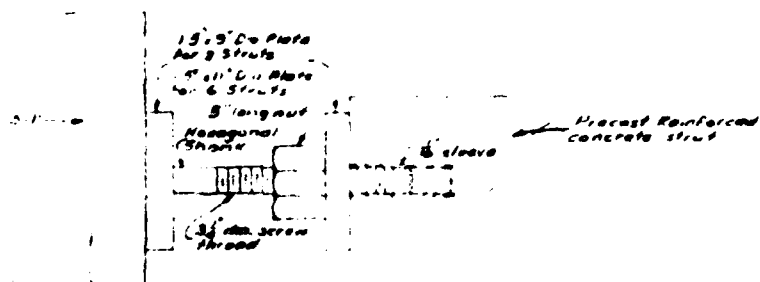
NOT A SCALD

IT WAS THE FIRST TIME
THAT THE
COUNTRY
WAS

See drawing
see notes
Sheet # 9

DETAIL OF NE

✓✓ ✓✓ ✓✓ ✓✓



**TYPICAL DETAIL OF
REINFORCED STRUTS**

Use one joint filler strip
Grades 4099-3rd or approved
equal (cut into pos. then.

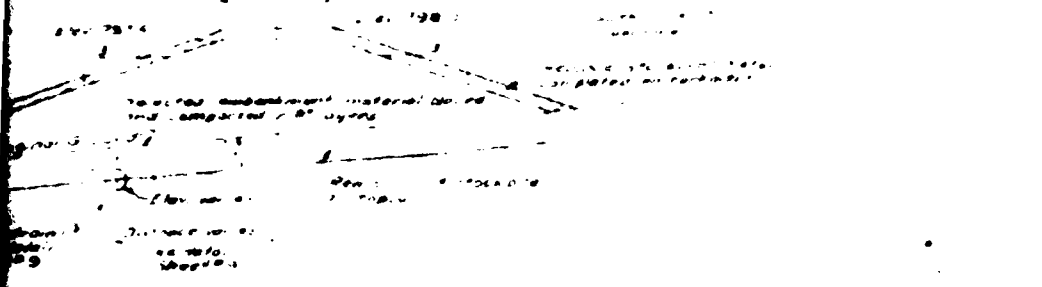
TYPICAL STRUT & JACK SCREW

**JOINT SEAL
WALLS**

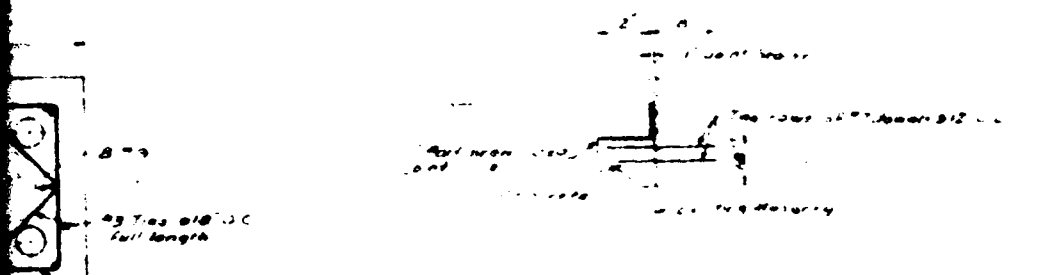
to Score

REVISION

REVISED



DETAIL OF NEW EMBANKMENT

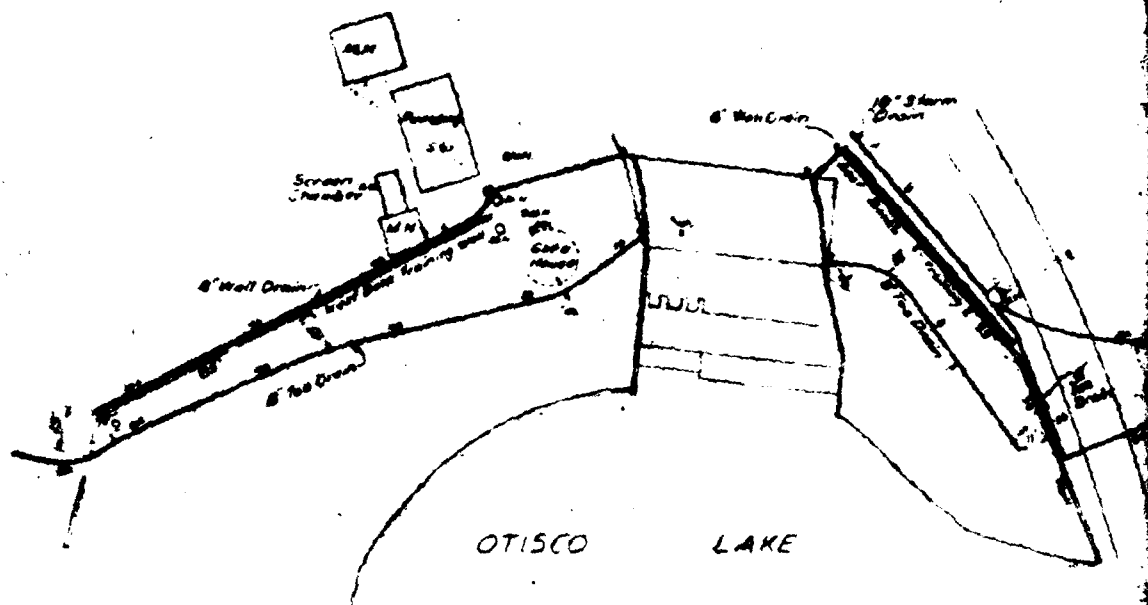


TYPICAL JOINT WITH EXIST. MASONRY

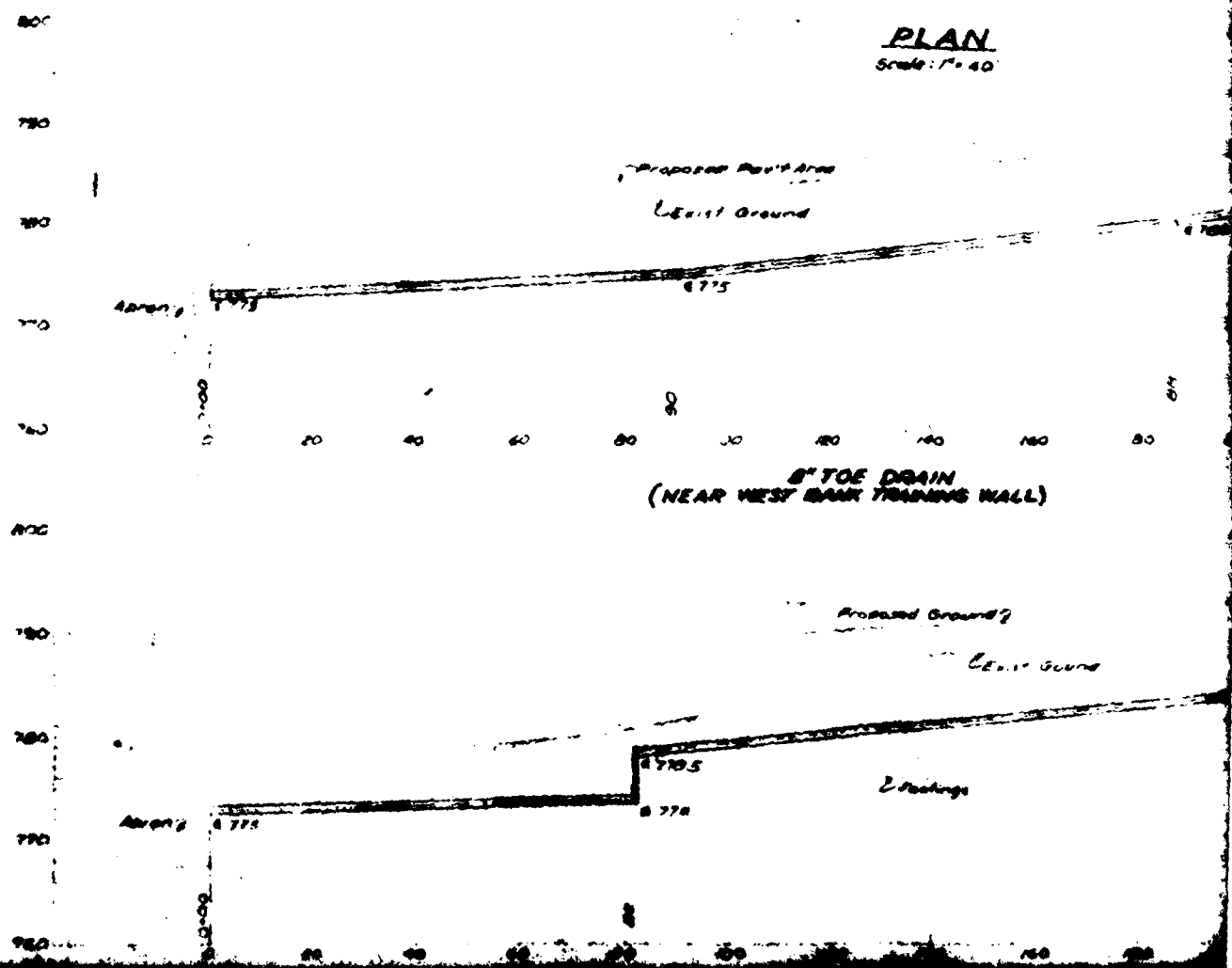
DETAIL OF STRUTS

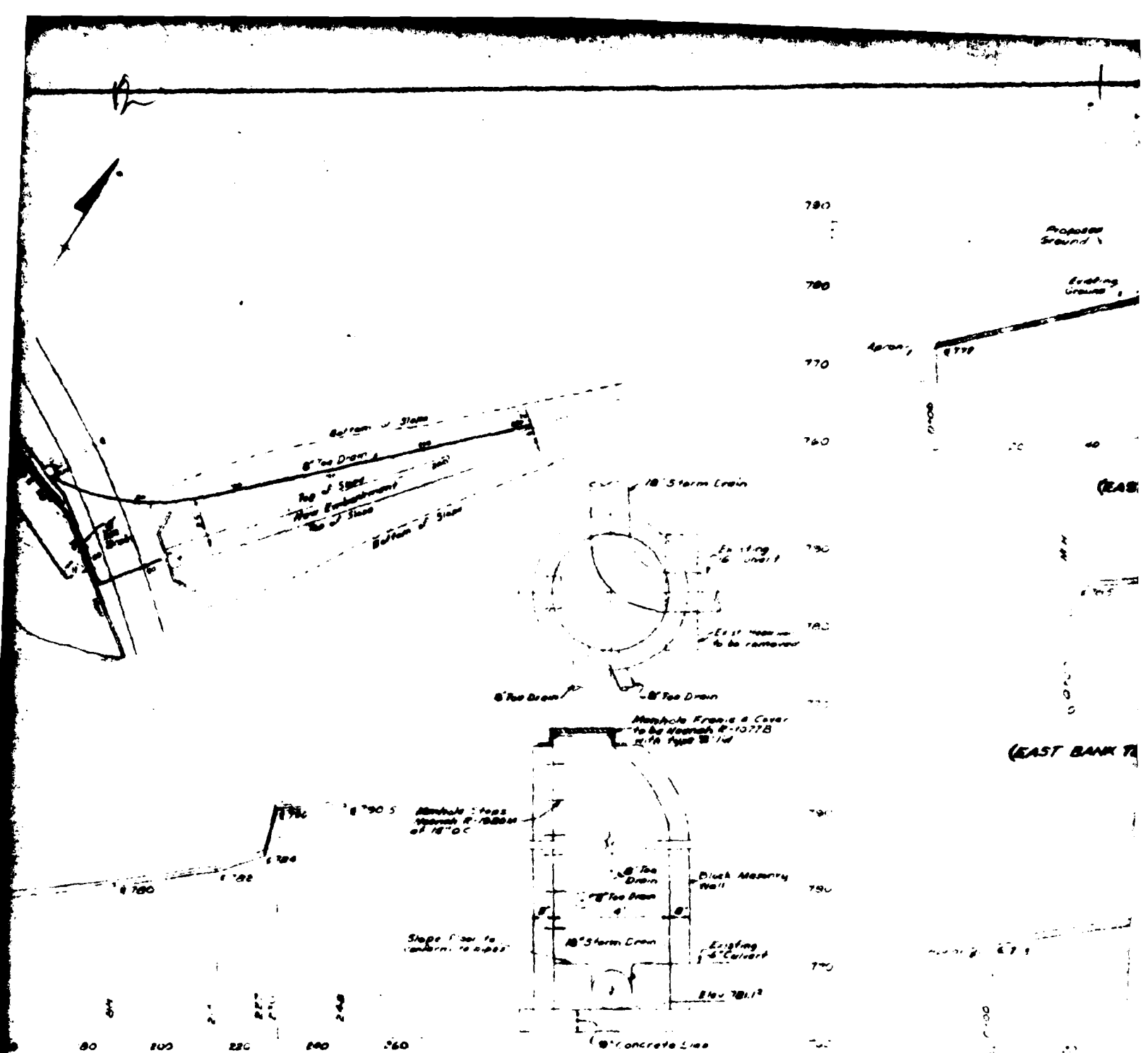
REVISIONS 1 6	ONONDAGA COUNTY WATER AUTHORITY OTISCO LAKE DAM MODIFICATIONS TYPICAL EMBANKMENT & PAVING DETAILS		DATE MAY 15, 1961 SCALE AS SHOWN P.L.N. 209 5-15-18F	3
	O'BRIEN & GORE CONSULTING ENGINEERS & LAND SURVEYORS SYRACUSE, NEW YORK			
	<i>[Signature]</i>			
	16			

A

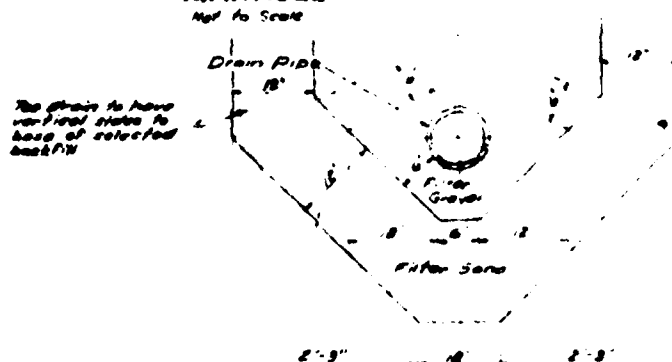


PLAN
Scale: 1" = 40'

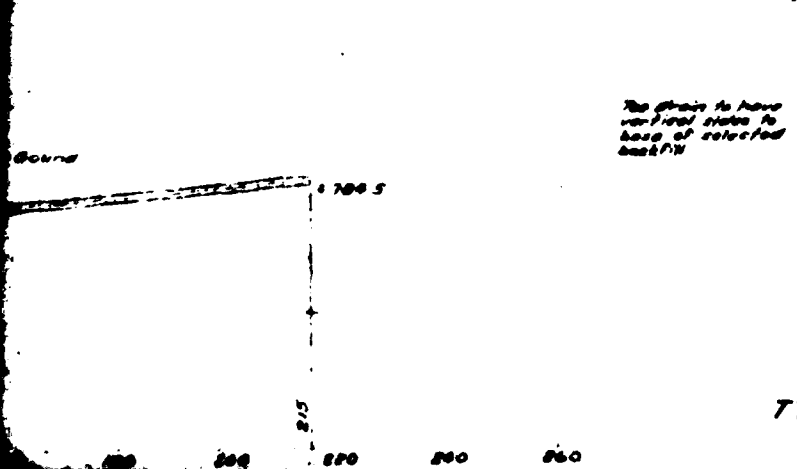




**TYPICAL SECTION THRU
MANHOLE**
Not To Scale



TYPICAL SECTION OF TOE DRAIN
Not To Scale



13

Proposed
Ground

Existing
Ground

2 feet

8" WALL DRAIN
(EAST BANK TRAINING WALL)

20 40 60 80 100 120 140 160 180

8" TOE DRAIN
(EAST BANK TRAINING WALL & UNDER HIGHWAY)

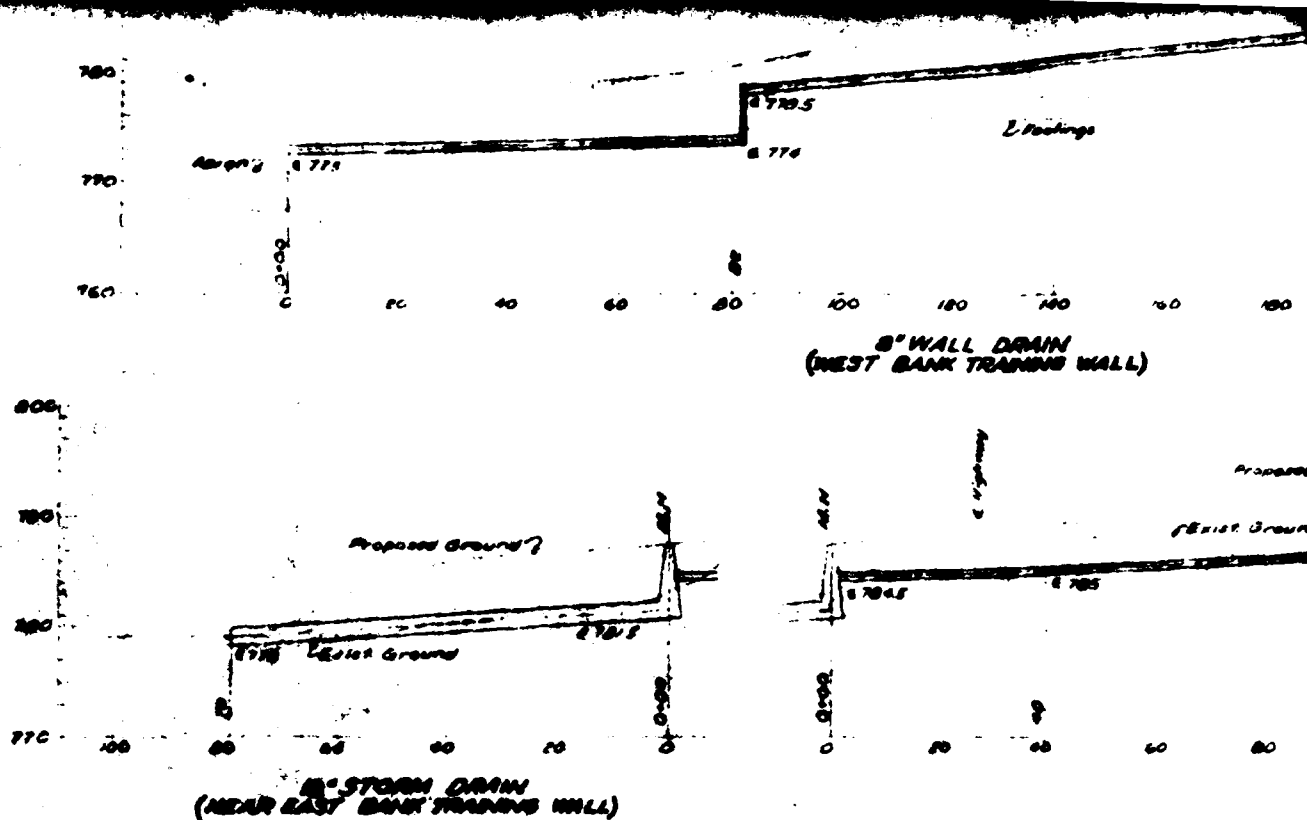
8" TOE DRAIN
(BETWEEN DAM & EAST BANK TRAINING WALL)

8" TOE DRAIN
(BETWEEN DAM & EAST BANK TRAINING WALL)

Top of toe drain
for embankment
east of highway

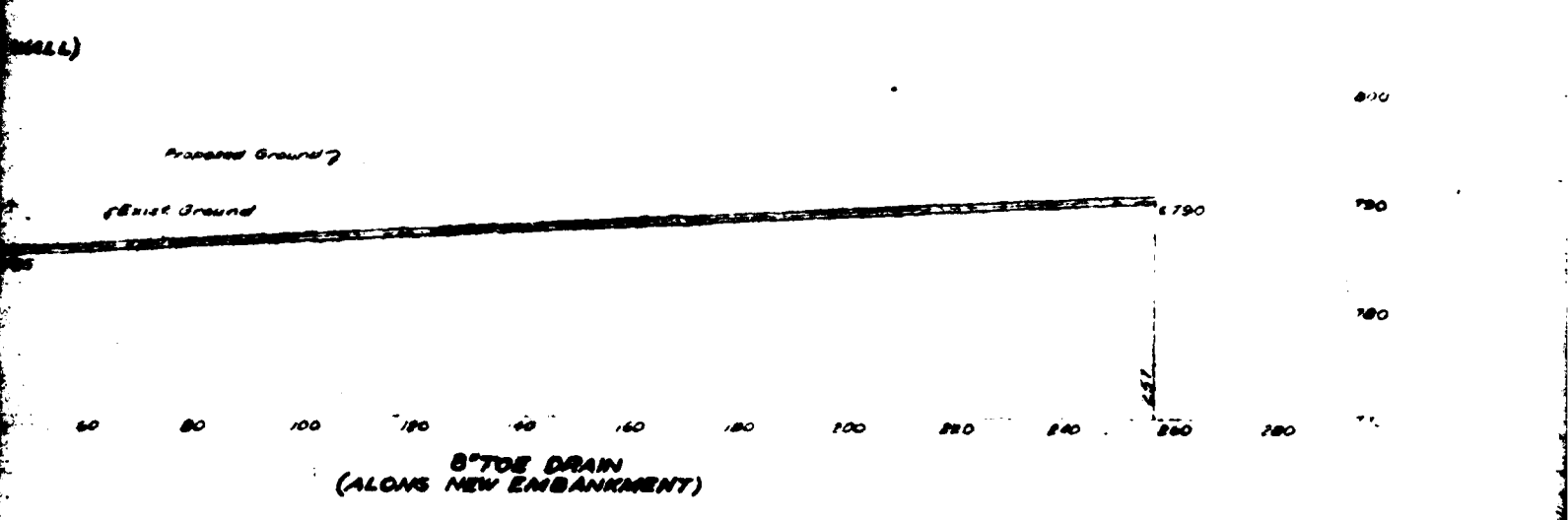
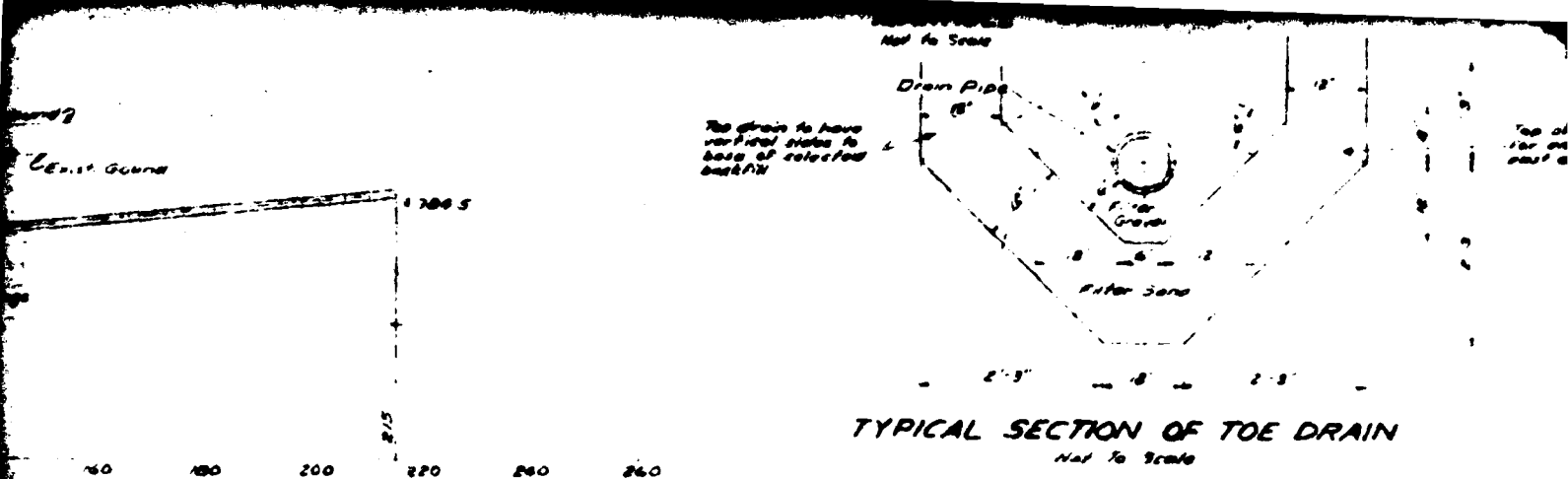
20' Horiz. to 1710' vert.

5-2
20' Horiz. to 1710' vert.



DESIGNED BY: J.E.H.
 CHECKED BY: J.E.H.
 DATE: 10/1/50
 DRAWN BY: J.E.H.

14



OF THE DRAIN
(BETWEEN DAM & EAST BANK TRAINING WALL)

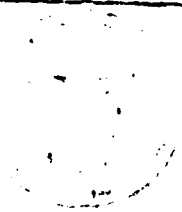
Top of toe drain
for embankment
out of highway

Top of toe drain
for embankment
out of highway

DRAIN

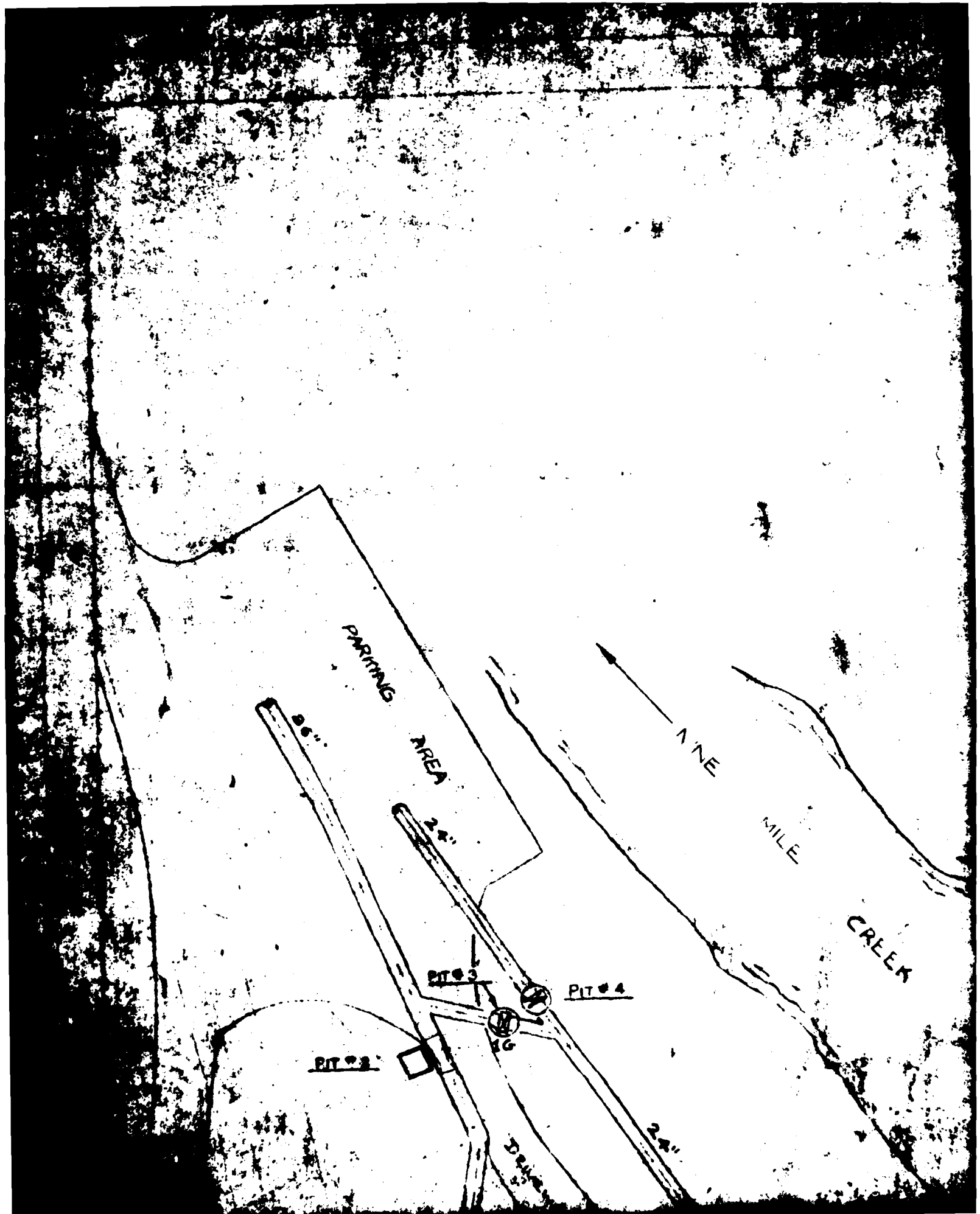
TYPICAL SECTION OF WALL DRAIN

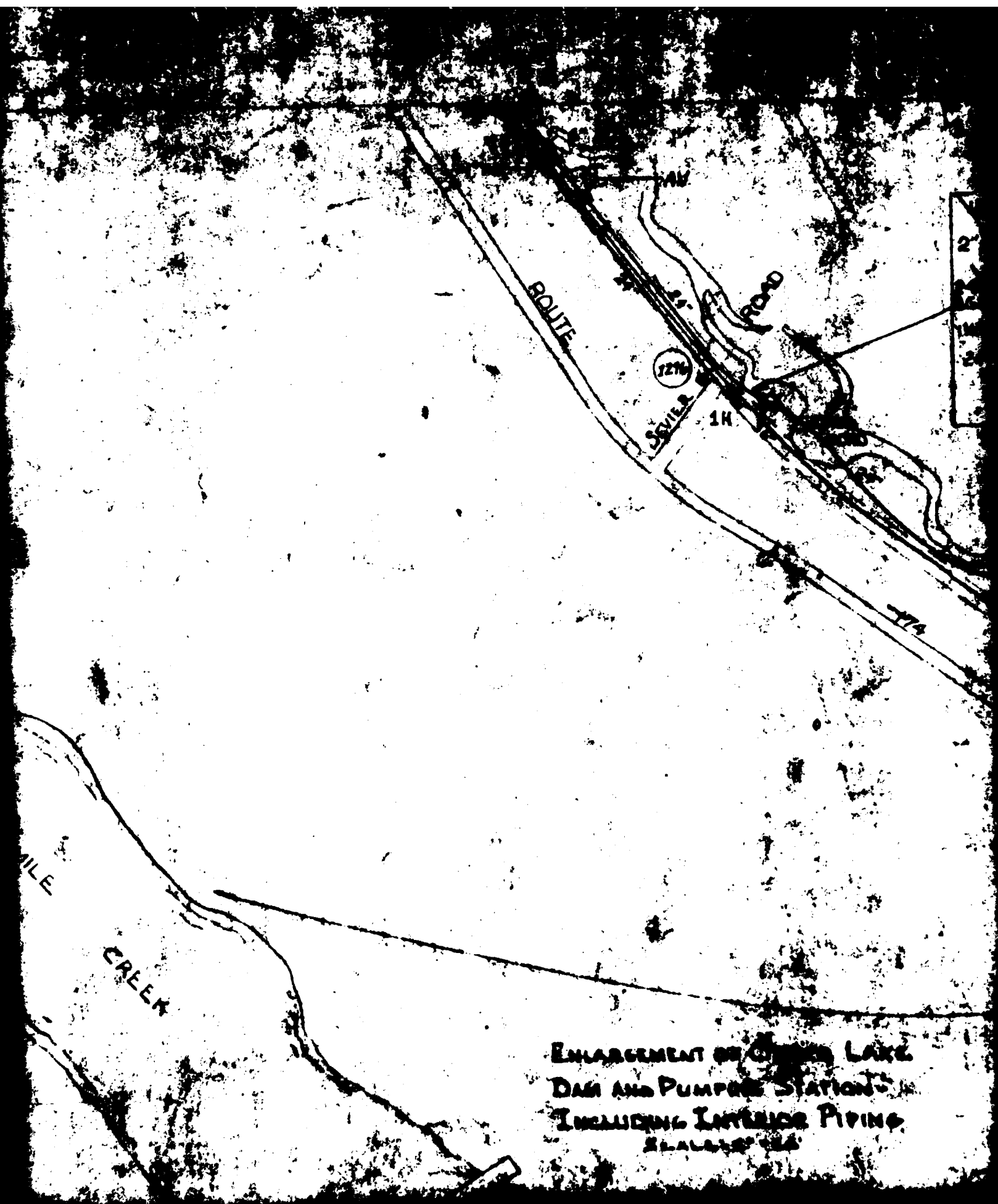
NOT TO SCALE

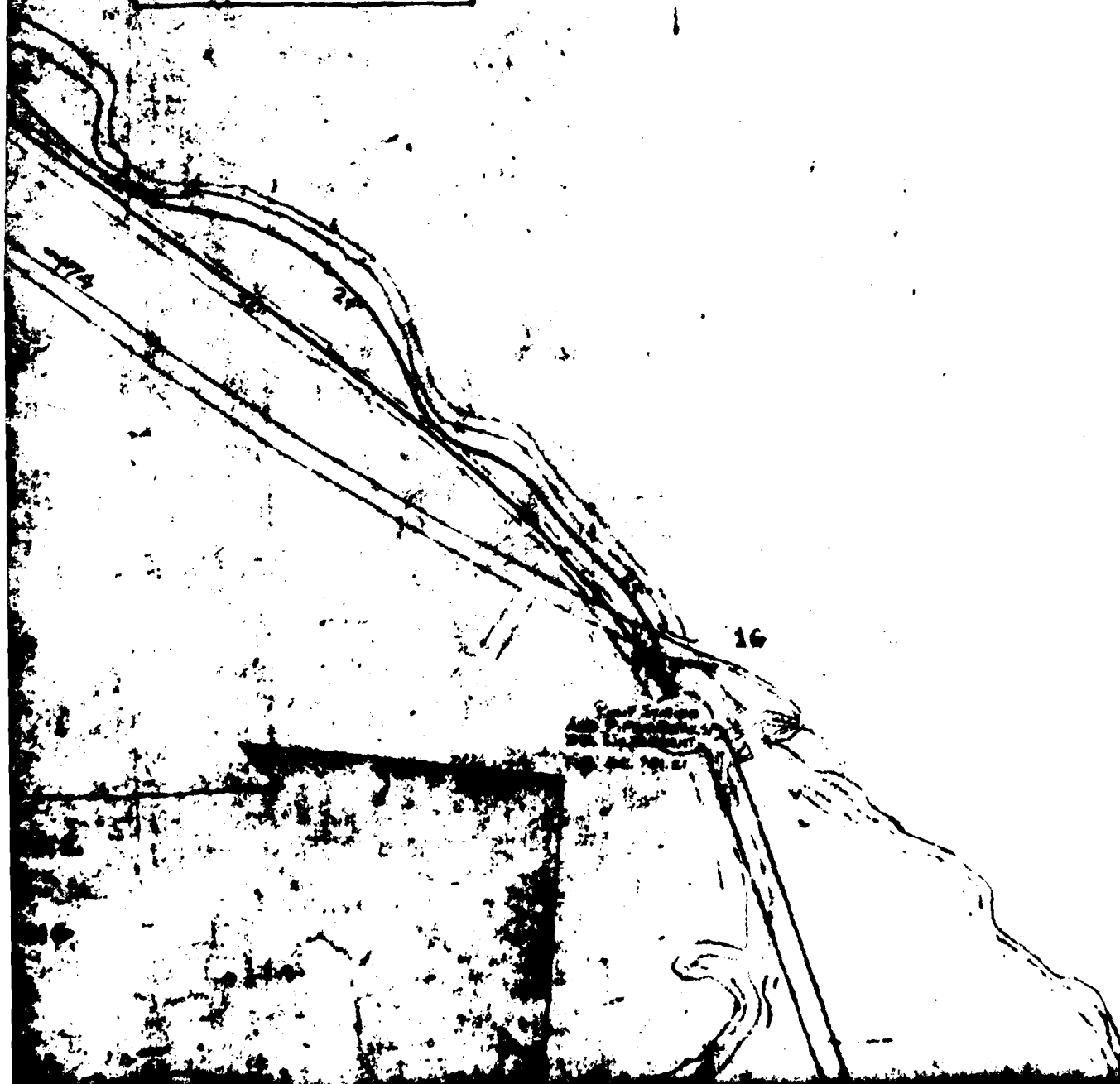
REVISIONS 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 51 52 53 54 55 56 57 58 59 60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80 81 82 83 84 85 86 87 88 89 90 91 92 93 94 95 96 97 98 99 100	ONONDAGA COUNTY WATER AUTHORITY OTISCO LAKE DAM MODIFICATIONS DRAINS		
	O'BRIEN & GERE CONSULTING ENGINEERS & LAND SURVEYORS 100 N. 1ST ST. SYRACUSE, N.Y. 13201		
	DATE MAY 15, 1967 SCALE AS SHOWN FILE NO. 200.0-15-80F		
	5		

A 2

6







New Earth
Berm

PIT No.	Sample Room	PIT TYPE	Notes
1	YES	DISCHARGE PIT	24" DIA
			24" DIA
			24" DIA
2	YES	TRANSMITTER PIT	FLOW
3	NO	VALVE PIT	24" DIA
4	NO	VALVE PIT	24" DIA
5	YES	WELL PIT	AUXILIARY
			TENANT
6	NO	VENTURI PIT	24" DIA
7	NO	VALVE PIT	24" DIA
			24" DIA
			REF
8	NO	CHLORINE SOLUTION PIT	3 CORN
			11.5
9	NO	OLD VALVE HOUSE PIT	24" DIA
			4 CORN
			2 SCREEN
10	NO	VALVE PIT	24" DIA
11	NO	VALVE PIT	24" DIA
12	YES	JUNCTION MANHOLE	24" DIA
			BUTTER
			24" DIA
13	NO	CHLORINE SOLUTION LINE PIT	CHLORINE
			CON
			SOL
14	ABOVE GROUND	SCREEN CHAMBER	SCREEN
			3

New Earth
Berm

Elev. 798.0

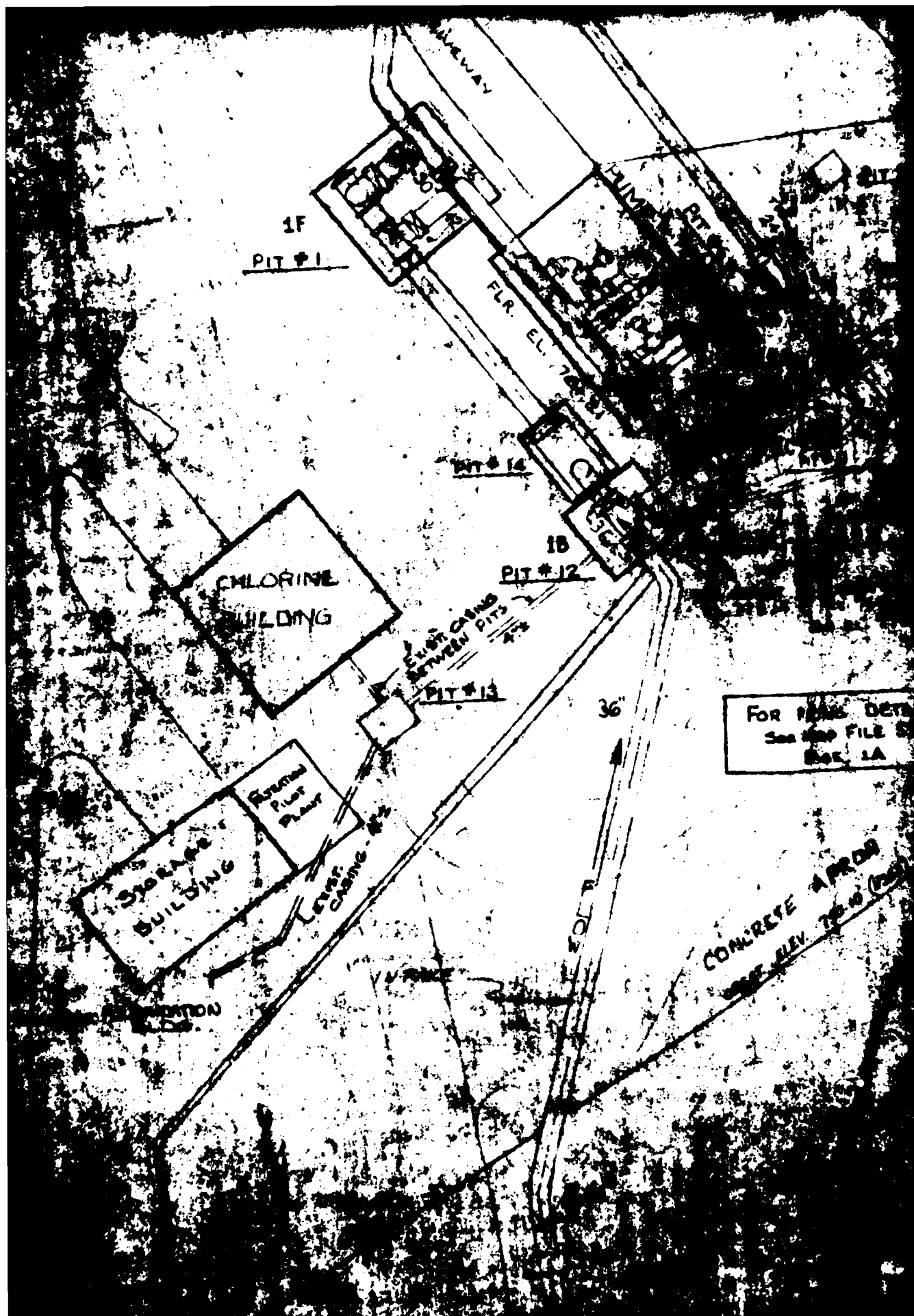
205' ±

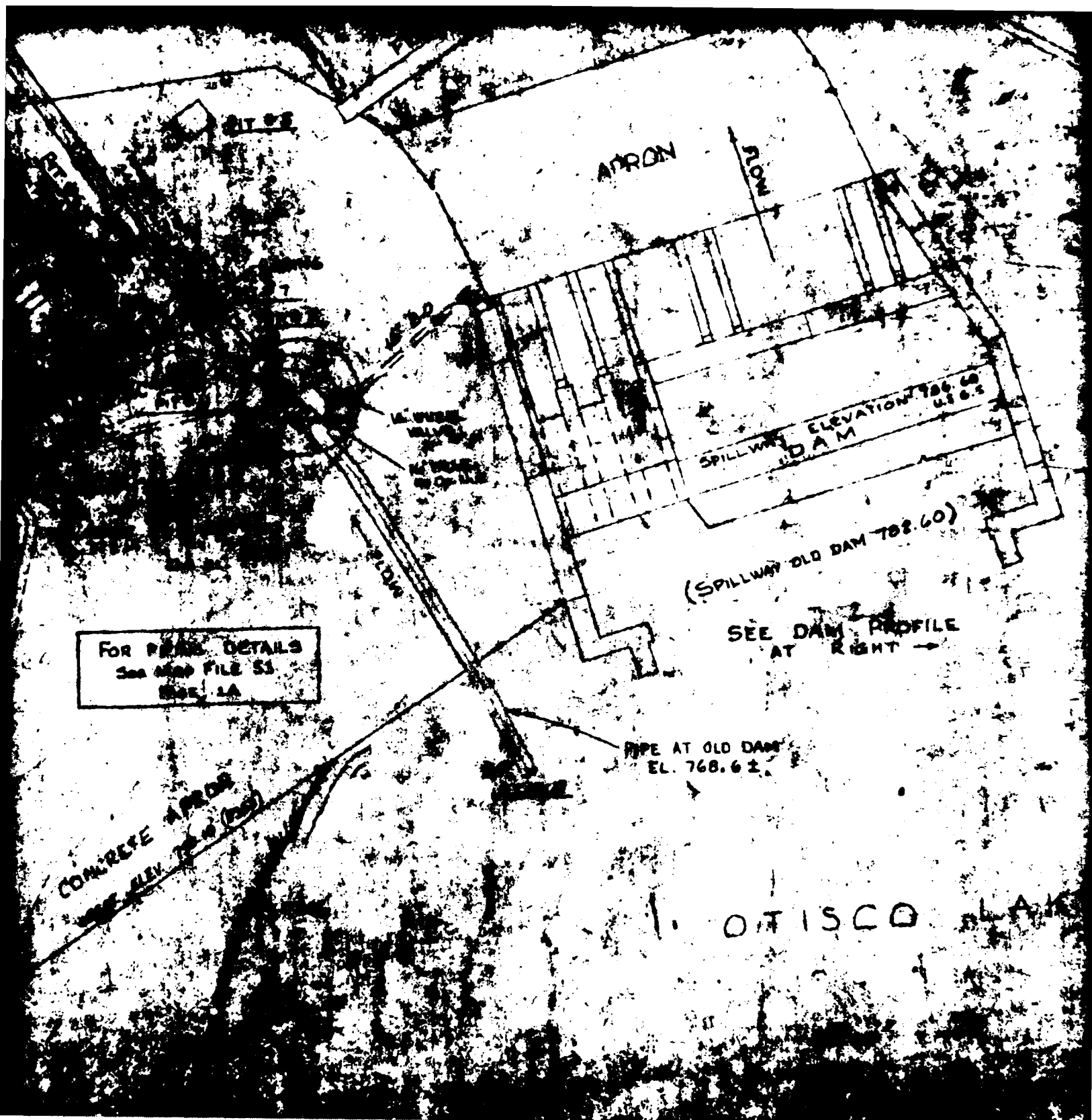
21'

55'

110'

PIT TITLE	CONTENTS
CHARGE W	42" SUCTION VALVE, HYDRAULIC BUTTERFLY
	36" DISCHARGE VALVE, HYD. BUTTERFLY
	80" BY-PASS VALVE, HYD. BUTTERFLY-CLOSED
	90° CHECK VALVE
TRANSMITTER PIT	FLOW METER TRANSMITTER
VALVE PIT	24" DISCHARGE VALVE, MANUAL GATE-OPEN
VALVE PIT	24" DISCHARGE VALVE, MANUAL GATE-OPEN
WELL PIT	AUXILIARY CHLORINATOR SUPPLY WELL PUMP
	TENANT HOUSE WATER PUMP & TANK
VENTURI PIT	24" x 16" VENTURI, 1/2" SAMPLE PUMP
VALVE PIT	24" BY-PASS VALVE, MAN. GATE-CLOSED
	24" CHECK VALVE, CLAPPER ASSEMBLY
	REMOVED.
CHLORINE INJECTION PIT	3 CORPORATIONS & PIPING USED AS
	INJECTORS.
24 VALVE HOUSE PIT	8-24" SUCTION VALVES, MANUAL GATES
	4 OPEN, 1 CLOSED.
	2 SCREEN POTS, SCREENS REMOVED
VALVE PIT	24" DISCHARGE VALVE, MAN. GATE-OPEN
VALVE PIT	24" SUCTION VALVE, MAN. GATE-CLOSED
JUNCTION MANHOLE	2-36" SUCTION VALVES, MANUAL
	BUTTERFLIES, OPEN-CHLORINE INJECTION
	24" SUCTION VALVE, MAN. BUTTERFLY-OPEN
CHLORINE SOLUTION LINE PIT	CHLORINE SOLUTION LINES WITH
	CONNECTIONS FOR FLUORIDE
	SOLUTION INJECTION.
SCREEN CHAMBER	SCREEN & MIXING CHAMBER WITH
	3 SCREENS AND ELECTRIC HOIST





OTISCO LAKE

36"

NARROW

CONCRETE

CREST ROAD 250' 10" (USGS)

42" INTAKE

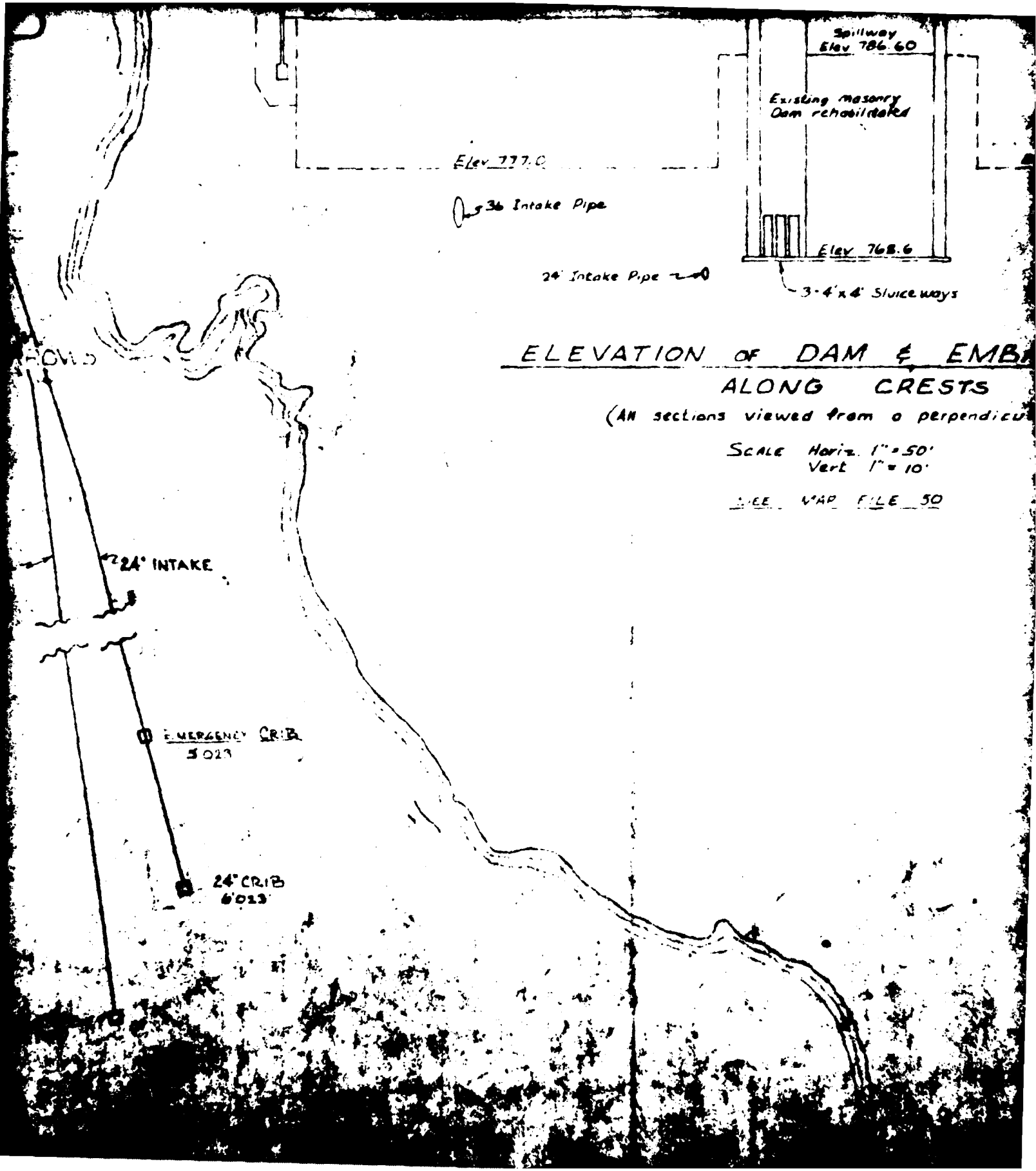
24" INTAKE

100' 60"

60'

FILE

LAKE



Elev 777.0

36 Intake Pipe

24 Intake Pipe

Spillway
Elev 786.60

Existing masonry
Dam rehabilitated

Elev 768.6

3-4' x 4' Sluiceways

ELEVATION OF DAM & EMB ALONG CRESTS

(All sections viewed from a perpendicular)

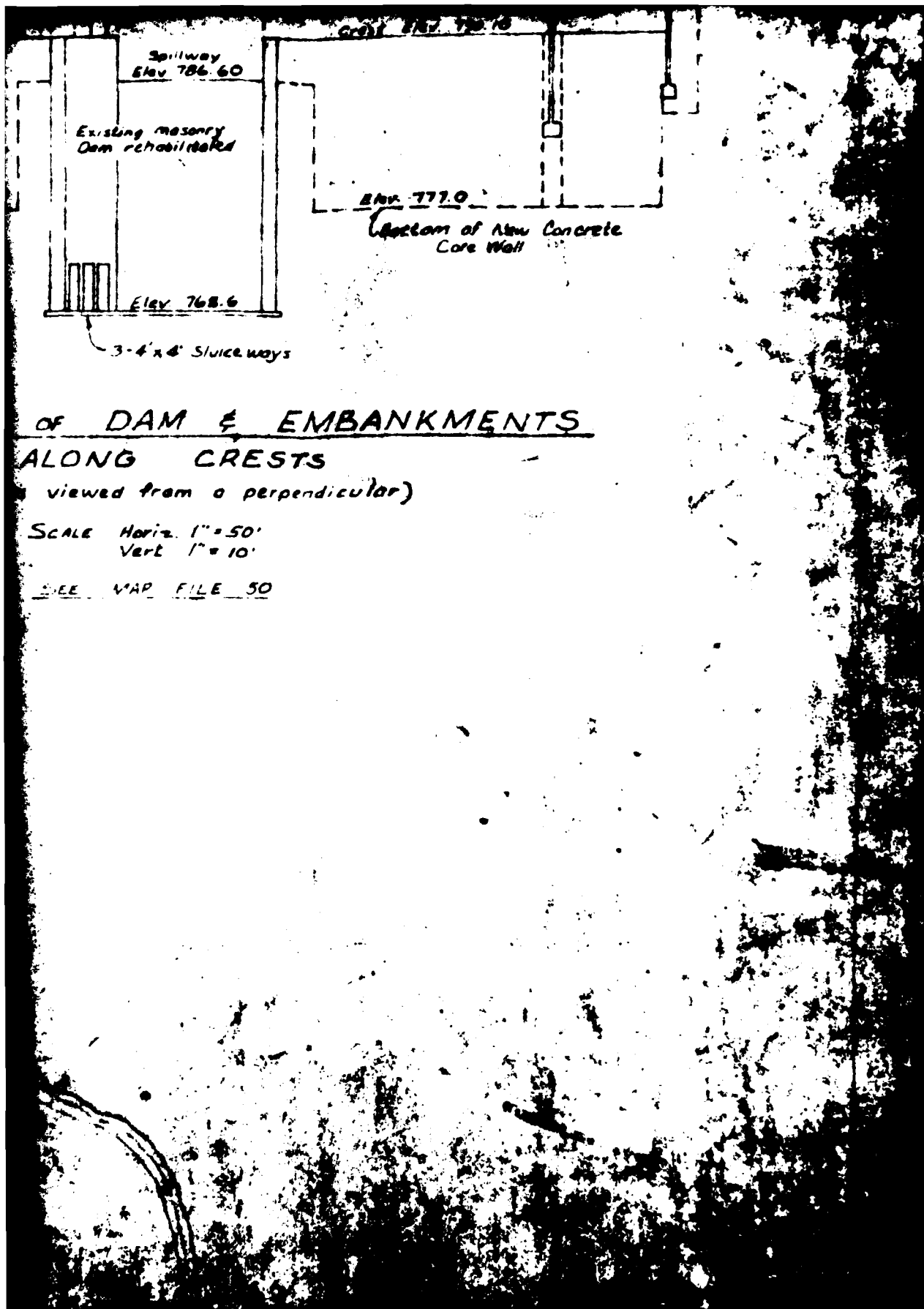
Scale Horiz. 1" = 50'
Vert. 1" = 10'

SEE MAP FILE 50

24" INTAKE

EMERGENCY CRIB
5023

24" CRIB
6013



02