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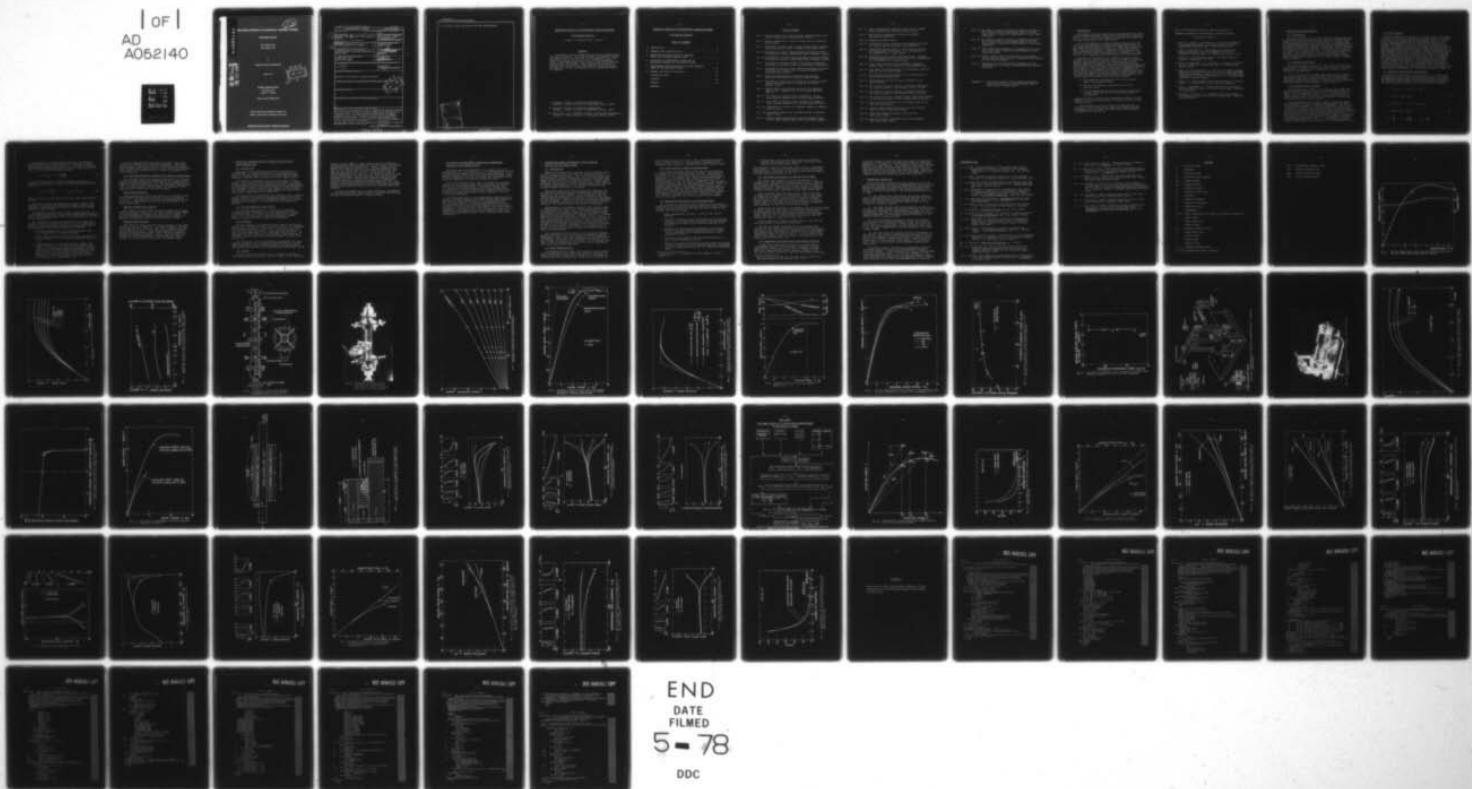
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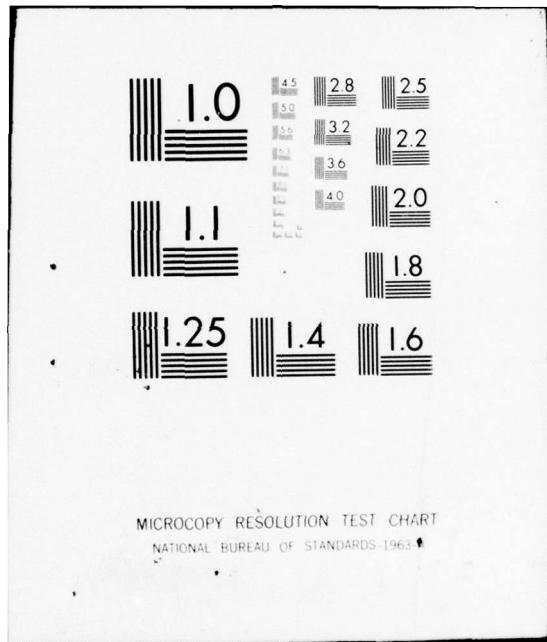
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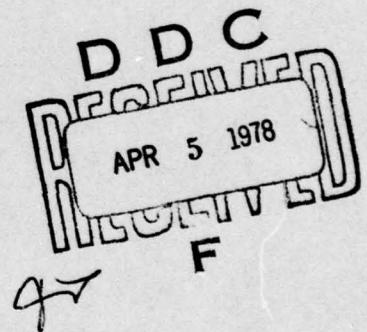
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Ori Ishai, Dan Peretz and Shlomit Gall

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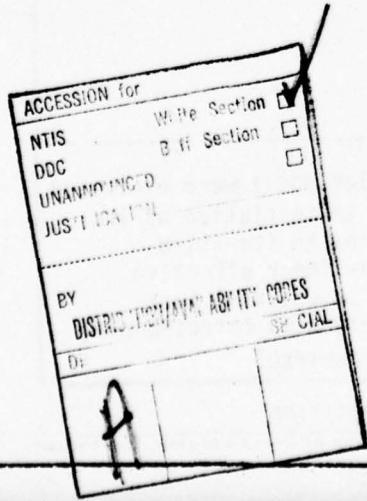
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MECHANICAL BEHAVIOR OF MULTIMATERIAL COMPOSITE SYSTEMS

Interlaminar Behavior

O. Ishai*, D. Peretz** and S. Gali***

ABSTRACT

Interlaminar stress characteristics within a bonded doubler model were evaluated. The study covered the inelastic range of the adhesive up to initiation of its visco-plastic flow. The analytical solution was derived by an iterative procedure, using a linear FEM program which follows a non-linear effective stress-strain relationship. This relationship was based on uniaxial test results of the bulk adhesive, which were found to correlate with the corresponding in-situ characteristics of the adhesive.

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MECHANICAL BEHAVIOR OF MULTIMATERIAL COMPOSITE SYSTEMS

Interlaminar Behavior

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1. INTRODUCTION

Most modern structural adhesives are designed for high toughness and therefore exhibit highly non-linear inelastic characteristics. Hence, a non-linear program is essential for predicting structural performance of a bonded system.

Due to the complex visco-elastic-plastic nature of the IAL even at low stress levels, rigorous analytical formulation of its mechanical behavior up to failure is impossible, especially when time- and environmental factors are taken into account. Thus, comprehensive empirical information is necessary on the inelastic stress-strain relationship of the adhesive as a function of time, temperature and such factors as moisture content.

Bulk data on the adhesive may suffice so long as stress distribution and cohesive failure within the bond are concerned. However, when failure initiates or propagates close to the adhesive-adherend interfacial zone, in-situ characteristics must be taken into account and correlated with the bulk properties. Accordingly, a series of tests on the in-situ mechanical behavior of an adhesive, for different loading modes, geometries and strain rates, was conducted with a view to correlating the findings with the behavior of the bulk adhesive under uniaxial load. This goal was partly achieved and results are described in the present report which deals with three main subjects:

- (a) The mechanical characteristics of the bulk epoxy used in the adhesive layer.
- (b) The in-situ mechanical characteristics of the adhesive layer.
- (c) The two-dimensional stress distribution (analytically solved) within the IAL at the non-linear range of the adhesive.

Preliminary tests on the in-situ hygrothermal behavior of the adhesive will be reported briefly in conjunction with a future research program.

The following is a list of reports related directly or indirectly to the output of the three-year research activity sponsored by the U.S. Army; they have either been published or submitted for publication.

List of Publications Related to Research Project on
MECHANICAL BEHAVIOR OF MULTIMATERIAL COMPOSITE SYSTEMS

Interlaminar Behavior

1. Ishai, O., Peretz, D. and Gali, S., "Direct Determination of Interlaminar Stresses in Polymeric Adhesive Layer," *Experimental Mechanics*, 17(7), 265-270, July 1977.
2. Ishai, O. and Gali, S., "Two-Dimensional Interlaminar Stress Distribution within the Adhesive Layer of a Symmetrical Doubler Model," *J. Adhesion*, 8, 301-312, 1977.
3. Peretz, D., "Interlaminar Behavior of Bonded Bimaterial Systems," *Composites*, July 1977.
4. Steg, I.D. and Ishai, O., "The Effect of Lateral Constraint on the Strength of a Single Lap Joint," *J. Adhesion*, 8, 263-273, 1977.
5. Ishai, O. and Girshengorn, T., "Strength of Bonded Aluminum-CFRP Single Lap Joint," Presented at the 9th National SAMPE Technical Conference, Atlanta, GA, Oct 4-6, 1977. Materials & Processes - In Service Performance, Vol. 9, National SAMPE Technical Conference Series.
6. Peretz, D., "Shear Stress-Strain Characteristics of Adhesive Layers," to be published in the *Journal of Adhesion*.
7. Gali, S. and Ishai, O., "Interlaminar Stress Distribution within an Adhesive Layer in the Non-Linear Range," submitted for publication.
8. Peretz, D. and Ishai, O., "Mechanical Characterization of Thin Adhesive Layer in-situ Under Combined Load," submitted for publication.

2. ADHESIVE BULK CHARACTERISTICS

2.1 Introduction

Bulk epoxy characteristics under different loading modes and testing conditions are currently obtained using standard experimental facilities such as the Instron tester. A ductile type of epoxy-versamid system, which was previously investigated [1-4], was selected to serve as representative for structurally tough adhesives throughout the present investigation. Most of the data is available for uniaxial tension. The purpose of the present test series was to relate this data to other loading modes such as shear. This relation will enable the prediction of the general stress-strain behavior of bulk epoxy from the elastic stage through the inelastic one up to failure, under a combined state of stress.

2.2 Specimen Fabrication

The material tested consisted of Shell epoxy epon 815 and General Mills versamid V-140 in the ratio of 70:30. The mixture was cured for 24 hours at room temperature followed by post-curing of 6 hours at 80°C.

For the tensile test, dog-bone specimens were cut from the cast plates according to ASTM D638-64T. For the shear test, thin-walled tubes were prepared and machined to have thinner thickness along the gauge length. Special metal grips were prepared and bonded to the specimens as shown in Fig. 4.

2.3 Uniaxial Tension Loading

The specimens were loaded in the Instron tester under uniaxial tension at various strain rates ranging from $15 \times 10^{-4} \text{ min}^{-1}$ to $38 \times 10^{-2} \text{ min}^{-1}$ at room temperature (about 22°C). A typical stress-strain curve is shown in Figure 1. The general trend for all curves shows a short linear portion beyond which a non-linear curve terminates in a stress plateau.

Other specimens were similarly loaded in uniaxial tension, and transverse strain was recorded simultaneously with longitudinal strain by means of a special extensometer. The ratio of transverse to longitudinal strains provides the Poisson ratio, ν , which is plotted as a function of uniaxial strain in Figure 1. The strain-rate effect can be evaluated from the family of stress-strain curves obtained under different strain rates (Fig. 2). The major effect is on the maximum stress plateau, S_f , which can be defined as yield stress, and shows an increase in linearity with log strain-rates [1] (Fig. 3). The effect on initial Young's modulus and proportional limit is less pronounced (Fig. 3).

2.4 Shear Loading

Tubular specimens were mounted on an Instron tester and loaded in torsion. A device was designed to measure change of torsional angle, using the regular Instron extensometer (Fig. 4). The overall torsional deformation of the specimens consists of an unknown displacement at the grip region which is difficult to estimate. In order to obtain the net shear deformation along the uniform gauge length, the following procedure was carried out: Tubular specimens of different lengths, ranging from 40 to 110 mm, were loaded in torsion up to failure. Torsional displacement at each torsion moment level was plotted as a function of specimen length. These plots show a series of lines (Fig. 5) which can be extrapolated to intersect the zero length axis at different torsional displacement levels. The extrapolated shear strain at zero length, as a function of the respective shear stress (calculated from the respective torsional moment) provides the correction curve (Fig. 6). By subtraction of the correction curve from the overall shear stress-strain curve, the "true" shear stress-strain relationship can be obtained as shown in Figure 6.

2.5 Effective Stress-Strain Relationship

An effective stress-strain relationship (Fig. 7, solid curve) was derived from the uniaxial-tensile stress (Fig. 1) for the representative epoxy resin. The effective stresses and strains are related to their respective stress components by using the von Mises' deviatoric-energy yield criterion [5] as follows:

$$\sigma = C_1 [\sigma_x - \sigma_y]^2 + (\sigma_y - \sigma_z)^2 + (\sigma_z - \sigma_x)^2 + 6(\tau_{xy}^2 + \tau_{yz}^2 + \tau_{zx}^2)]^{1/2} \quad (1)$$

$$\epsilon = C_2 [\epsilon_x - \epsilon_y]^2 + (\epsilon_y - \epsilon_z)^2 + (\epsilon_z - \epsilon_x)^2 + 6(\epsilon_{xy}^2 + \epsilon_{yz}^2 + \epsilon_{zx}^2)]^{1/2} \quad (2)$$

$$C_1 = \frac{\sqrt{2}}{2} \quad C_2 = \frac{\sqrt{2}}{2(1+\nu)} \quad \epsilon_{ij} = \frac{\gamma_{ij}}{2}$$

In the case of uniaxial tension test data, the effective stress-strain curve is identical to the respective uniaxial tensile stress-strain curve of Figure 1. In order to calculate the effective stress-strain relation from the shear tests data (Fig. 6) the following relations are used:

$$s = \sqrt{3} \tau_{xz} \quad e = \frac{\sqrt{3}}{2} \frac{\dot{\gamma}_{xz}}{1+\nu} \quad (3)$$

It has to be noted that in order to compare shear data with respective tensile data, one must use the same effective strain rate, i.e.,

$$\dot{\gamma}_{xz} = (1+\nu) \frac{2}{\sqrt{3}} \dot{\varepsilon}_x \quad \dot{\gamma}_{xz} \sim \sqrt{3} \dot{\varepsilon}_{xy} \quad (4)$$

where $\dot{\gamma}_{xz}$ and $\dot{\varepsilon}_x$ are the shear and tensile strain rates respectively.

The plots of shear and tensile tests data in terms of the effective stress-strain relationship in Figure 7 exhibit good agreement. This supports the effective stress-strain concept used in the present study.

The analytical solution for inelastic stress distribution in the adhesive within the doubler model necessitates the derivation of the adhesive moduli as well as Poisson Ratio at the different strain levels.

The variations of effective tangent (E_t) and secant (E_s) moduli, as well as effective Poisson ratio, ν , as functions of effective strain, are shown in Figure 8. Three regions may be distinguished along the stress-strain curve, namely:

1. an almost linear region up to the proportional limit (S_ℓ) through which ν is almost constant;
2. a non-linear, probably viscoelastic range, characterized by an increase in ν , up to the "yield plateau" level (S_f);
3. a macro-plastic flow range beyond which stresses are almost constant and even tend to drop at higher strain levels. ν is almost invariant at this range. By subtraction of the elastic components of axial and transverse strains, the "non-elastic Poisson ratio" may be derived. At the "yield plateau" its value tends to approach 0.5, which is the theoretical limit based on the assumption of incompressible flow.

All the characteristics described in Figures 7 and 8 are functions of loading strain rate and temperature. The present study, however, will be confined to the reference strain rate and temperature specified in Figures 1 and 7 which are considered to be typical and representative for similar "non-linear" adhesive materials.

2.6 Bulk Characteristics Compared with Adhesive In-Situ Data

In a previous report and subsequent publication [6] an investigation of shear stress-strain relationship of a similar epoxy adhesive in-situ was reported. The study was concerned with the effect of adhesive layer thickness on its mechanical characteristics. The main objective was to compare the behavior of the adhesive layer in-situ with its bulk reference.

Stress-Strain Behavior

Shear stress-strain curves for adhesives of different thickness compared with the bulk references are shown in Figures 6 and 9. The similarity of bulk to in-situ behavior is evident at the linear elastic range up to the initiation of the yield plateau level.

Adhesive Shear Elastic Modulus

The effect of in-situ adhesive layer thickness on its shear modulus is shown in Figure 10. The level of bulk modulus seems to be approximately the same as the thick adhesive layer. It may be concluded that no significant variation in modulus is apparent for adhesive layers of thicknesses above 0.3mm.

Adhesive Shear Strength

The effect of in-situ adhesive layer thickness on its shear strength is shown in Figure 11. The shear strength of the bulk epoxy seems to be on a slightly lower level compared with that of the adhesive in-situ. It may be concluded that for epoxy adhesive thickness above 0.15mm, shear strength is almost invariant with adhesive thickness. Hence, bulk shear data can be used for a first estimation of bond strength as long as failure is of the cohesive type within the adhesive layer.

3. MECHANICAL CHARACTERIZATION OF ADHESIVE LAYER IN-SITU UNDER COMBINED LOAD

3.1 Introduction

Knowledge of the mechanical properties of adhesives in a combined state of stress is essential in engineering applications of structural bonded joints in which the adhesive layer undergoes a complex state of stress even under simple loading.

Unfortunately, experimental data in this content is very scanty. Early studies of the single-lap joint model [7] tended to oversimplify the problem by assuming pure shear; subsequently, the more sophisticated closed-form solution of Goland and Reissner [8] envisaged a lateral normal stress component, the so-called "peel stress", while more recent experimental work deals mostly with cases of pure shear [6] or tension [9].

The investigation reported here was conducted using a torsion-tension apparatus to measure the mechanical properties of thin adhesive layers in-situ. Some preliminary results are presented, indicating the suitability of the method for the purpose in question.

3.2 Testing Apparatus (Figs. 12a,b)

The test specimen consists of a pair of 2025 aluminum flanged cylinders (adherends) a. The loading device consists of a torsion arm, b, operated from any conventional loading tester (e.g. Instron), and a spring operated axial loading post, c, capable of applying tension or compression.

3.3 Procedure

The test adhesive was composed of Shell epoxy resin 815 and General Mills versamid V-140 in the ratio of 70:30. The adherends were aligned on a special fixture with a controlled gap of 1mm, into which the resin was poured. The joint was cured for 24 hours at room temperature, plus 6 additional hours at 60°C. The specimens were then tested on a conventional Instron machine at different constant shear-strain rates and under different axial load levels.

For calibration, shear displacements of specimens with "zero adhesive thickness" were recorded and deducted from the overall shear displacement of the bonded specimens. The shear strains and stresses are computed from recorded moment-displacement curves.

3.4 Results

Typical shear stress-strain curves in Figure 13 indicate that normal uniaxial load has a clear-cut effect on the inelastic

behavior of the adhesive. Axial tension tends to reduce the initial modulus, shear strength and ultimate strain, while axial compression tends to increase them. However, because of the lower in-situ tensile strength of the adhesive-adherend interface and its brittle mode of failure under tension load, the range of applicability is limited. Stiffness and level of yield plateau seem to be lower with the reduction in strain rate. In-situ stress-strain relationship shows fair agreement with reference bulk data. Failure under combined load may be described by means of the apparent ultimate-stress combination plotted in the shear-normal axial stress plane (Fig. 14). The failure envelope thus obtained also shows reduction of the shear strength with increasing tensile load, and increase under compression.

It may be concluded that the above methodology for adhesive in-situ characterization under combined loading can provide useful data for failure analysis of bonded systems.

4. THE EFFECT OF HYGROTHERMAL CONDITIONS ON MECHANICAL BEHAVIOR OF THE ADHESIVE LAYER.

The above topic was scheduled to be initiated only at an advanced stage of the research due to its complexity and the need for adequate experimental and analytical basic substrate. It is aimed to achieve a more realistic picture of IAL behavior under external conditions, and also to provide some information on the long-term durability of structural adhesive joints.

At the present preliminary stage, the effect of hot water immersion on the adhesive layer was investigated and compared with an oven dried counterpart. Preliminary results (Fig. 15) indicate a significant change in the shear stress-strain characteristics. The presoaked adhesive specimen showed lower stiffness, strength and ultimate strain compared to its previously dried reference.

The next step will be a more comprehensive investigation of adhesive specimens exposed to different hygrothermal conditions and tested in shear and tension in wet and dry conditions. The long range scheme is to measure hygrothermal effects on interlaminar stress and strain distributions within a doubler specimen by utilizing the method described in the Final Technical Report of the second year. The experimental results will be compared with the analytical FEM solution and will take into account the hygrothermal effects on the adhesive stress-strain relationship.

5. INTERLAMINAR STRESS DISTRIBUTION WITHIN ADHESIVE LAYER AT THE NON-LINEAR RANGE.

5.1 Introduction

Numerous publications are available on the analytical solution for stress distribution within the adhesive layer of a structural bonded joint model. Most of the earlier works [8,10-13] are based on certain assumptions such as uniform shear and normal stress distribution through the adhesive thickness which enable the derivation of a closed form solution. These simplifications, which provide only the average stress data, lead also to certain inconsistencies in the equilibrium equations and violate boundary conditions at the edges.

More recent works deal with the analytical complexity of the problem by applying numerical methods such as finite difference [14] and finite element [15-18] and provide the two-dimensional stress distribution within the adhesive layer for various bonded joint models of isotropic [15,16] and orthotropic [17,18] adherends. These solutions are in agreement with equilibrium and boundary conditions of the structural model but are limited to the elastic-linear stress-strain relationship.

Unfortunately, the polymeric adhesive layer commonly used is characterized by a non-linear stress-strain behavior, even at a relatively low stress level. This non-linearity is more pronounced at high stresses, reaching a stress plateau which may be defined as "macro-plastic" yielding (see Fig. 7). Another approach to represent this mode of behavior is by a simplified elastic-plastic model [18-20] which may provide an approximate upper bound for the real solution. Such a solution is insufficient considering the fact that under service load, the material is not allowed to approach this "yield point" and that at the critical region of the adhesive, the stress level is mainly at the non-linear range, beyond the elastic limit but below the yield plateau.

The viscoelastic-plastic nature of the polymeric adhesive, which is reflected in its sensitivity to temperature and its dependence on time [3,4], also contributes to the complexity of the problem. The present work attempts to provide the first step towards the solution of the problem. It is confined, however, to the two-dimensional case and to constant temperature and strain rate loading conditions.

5.2 Model Representation

Interlaminar adhesive layer (IAL) behavior is most conveniently represented by the symmetrical doubler model shown in Figure 16 and fully described in Reference 16. Reference 16 also provides the two-dimensional linear elastic solution for

stress distribution of this model. The corresponding FEM network is shown in Figure 17. The analysis is focused on the "boundary" edge zone where stresses attain their critical value.

5.3 Plane Stress vs. Plane Strain Solutions

Plane stress and plane strain states may be considered as two bounds for the three-dimensional solution. They represent the situation close to the free edges ($y = \pm b$) and along the midsection ($y = 0$) respectively. Stress distributions based on assumptions of plane stress and plane strain for the linear range are shown in Figs. 18 and 19. Comparison of the τ_{xz} and σ_y distributions in the two cases indicated only minor variations. It may be concluded that the two-dimensional solution is not affected significantly by the unknown stresses acting along the third direction. The σ_y distribution along y derived from the plane strain solution (Fig. 20), (which may provide the upper bound for the actual three-dimensional state of stress) attains its maximum at the center line ($y = 0$). Generally it seems that the worst condition for failure would be at the corners ($y = \pm b$, $x = \pm c$).

5.4 Analytical Procedure at the Non-linear Range

The procedure for determining the stresses at the different locations within the adhesive boundary zone under a given external uniaxial load will follow the flow chart given in Figure 21 and includes the following steps:*

1. Apply predetermined external σ_c level at the central adherend.
2. Calculate effective strains and stresses of the different elements in the FEM network using the linear FEM program and assuming the same initial effective modulus for all elements.
3. Determine the specific secant modulus at each element based on the experimental relationship given in Figures 7 and 8 according to the respective effective strain calculated in step 2.
4. Rerun the linear program with the modified modulus at each element according to step 3.
5. Compare the calculated stress S'_k at each element as derived in step 4 with the stress S_k obtained from the experimental effective stress-strain curve for the respective calculated strain.

* A full printout of a representative FEM program is given in Appendix 1.

6. Repeat steps 3-5 up to the stage where the difference between the calculated and "empirical" stresses is less than 2% of the final stress value.

The different steps are illustrated in Fig. 22 for a representative element k. The level of convergence was achieved by significantly fewer iterations in the case of plane strain compared with the plane stress solution (Fig. 23).

5.5 IAL Stress Distribution at the Non-linear Range

The results were focused on stress distribution at the boundary zone. The effect of axial external stress σ_c (applied to the central adherend) on the effective stress and strain at the critical point (located close to the IAL edge) is shown in Figures 24 and 25. The difference between linear and nonlinear solutions is pronounced, especially in the plane stress case.

Lower effective stress and strain were found in the case of plane strain compared with plane stress. The reverse trend was found for the shear (τ_{zx}) and lateral normal (σ_z) stresses as a function of axial external stress (Figure 26): namely, higher stresses in the case of non-linear solution of the plane strain state. In the case of the linear solution, no significant difference is shown for the two plane states.

Shear stress distribution at the boundary zone for $\sigma_c=53.3 \text{ kg/mm}^2$ is shown in Fig. 27. Here again no significant difference can be distinguished between plane stress and plane strain solutions, except at the critical location close to the lower edge corner (see Fig. 17). Similar trends were found for the two-dimensional lateral normal stress (Fig. 28).

In most cases, the effective stress tends to level off and even drop faster in the plane stress state.

The effective-stress distribution at the boundary zone (Figures 29, 30) permit evaluation of the ductile failure process of the IAL. The region of "viscoplastic flow" may comprise the elements where the effective stress or strain exceed their prespecified limit* as shown in Fig. 8. For the specific load level of the present case, this region is located close to the lower edge of the IAL (Fig. 17).

5.6 Comparison with Simplified Elasto-Plastic Solution

A simplified elasto-plastic model of the stress-strain relationship (Figure 7) has substantial advantages over the more realistic non-linear one, in that it reduces the parameters for describing the complex inelastic process to two - the initial elastic modulus E_0 , and the yield-stress plateau S_f .

* This limit may be defined as the level where initiation of residual deformation was detected (Ref. [21]).

It may also permit inclusion of the strain-rate and temperature-dependence in the analysis, provided their effect on the above parameters is available [3]. The respective stress distributions based on this model (Figures 31-34) show that while there is no significant deviation from the exact solution at the critical location, there is some farther away from the edges. More iterations are needed for convergence of the elasto-plastic solution compared with the more exact non-linear one (Figure 35).

6. SUMMARY AND CONCLUSIONS

The present report comprises two main parts, the first of which deals with bulk and in-situ characterization of an adhesive material and provides the empirical basis for the second part, devoted to analytical solution of the stress distribution in the IAL at the non-linear range. The following conclusions may be drawn:

- (a) The stress-strain relationship of the bulk adhesive under a combined state of stress may be derived from the stress-strain relationship obtained under uniaxial loading. This follows the "effective" stress-strain approach and von Mises' criteria for inelastic behavior.
- (b) The shear stress-strain relationship of the adhesive in-situ, its Young's modulus and ultimate strength, may be roughly considered as invariant for thicknesses about 0.2mm; in these circumstances, bulk data provide the basic parameters for a preliminary assessment of the relevant mechanical behavior of the adhesive within a bonded structure.
- (c) The finite-element method was found to be adequate for determining the stress distribution of the IAL at the non-linear range. At high external loading levels, non-linear behavior was found to predominate in narrow boundary zones close to the IAL edge, whereas most of the IAL remains at the elastic (linear) range.
- (d) The non-linear FEM solution serves for assessment of IAL failure. The latter is manifested by initiation of plastic flow of the adhesive, which can be related to a specific limit point on the effective stress-strain curve. Under a given external load, a "visco-plastic" state prevails in the boundary zone of the IAL with stresses in all elements exceeding the above limit.

The present study is the first step towards more general non-linear analysis and empirical investigation of bonded structural systems. Future research will involve time-dependent non-linear stress analysis of the IAL with reference to the viscoelastic-plastic nature of the polymeric adhesive; another phase will cover the influence of environmental factors (temperature, moisture) on the above time-dependent mode of behavior.

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GLOSSARY

- b - half IAL width
c - constant
c - half IAL length
 E_0 - initial Young's modulus
 E_s - secant modulus
 E_t - tangent modulus
 e - effective strain
 \dot{e} - effective strain rate
h - thickness
 h_0 - adhesive thickness
n - number of iterations
s - effective stress
T - temperature
 x, y, z - axial, transverse and lateral coordinates respectively
 γ - shear strain
 $\dot{\gamma}$ - shear strain rate
 ϵ - normal strain
 ϵ_c - uniaxial external strain
 $\dot{\epsilon}$ - axial strain rate
 ν - Poisson ratio
 τ - shear stress
 σ - normal stress
 σ_c - uniaxial external stress
 $x = \frac{x}{c}$ - nondimensional axial coordinate

IAL - interlaminar adhesive layer
CAL - central adherend layer
EAL - external adherend layer
FEM - finite element method

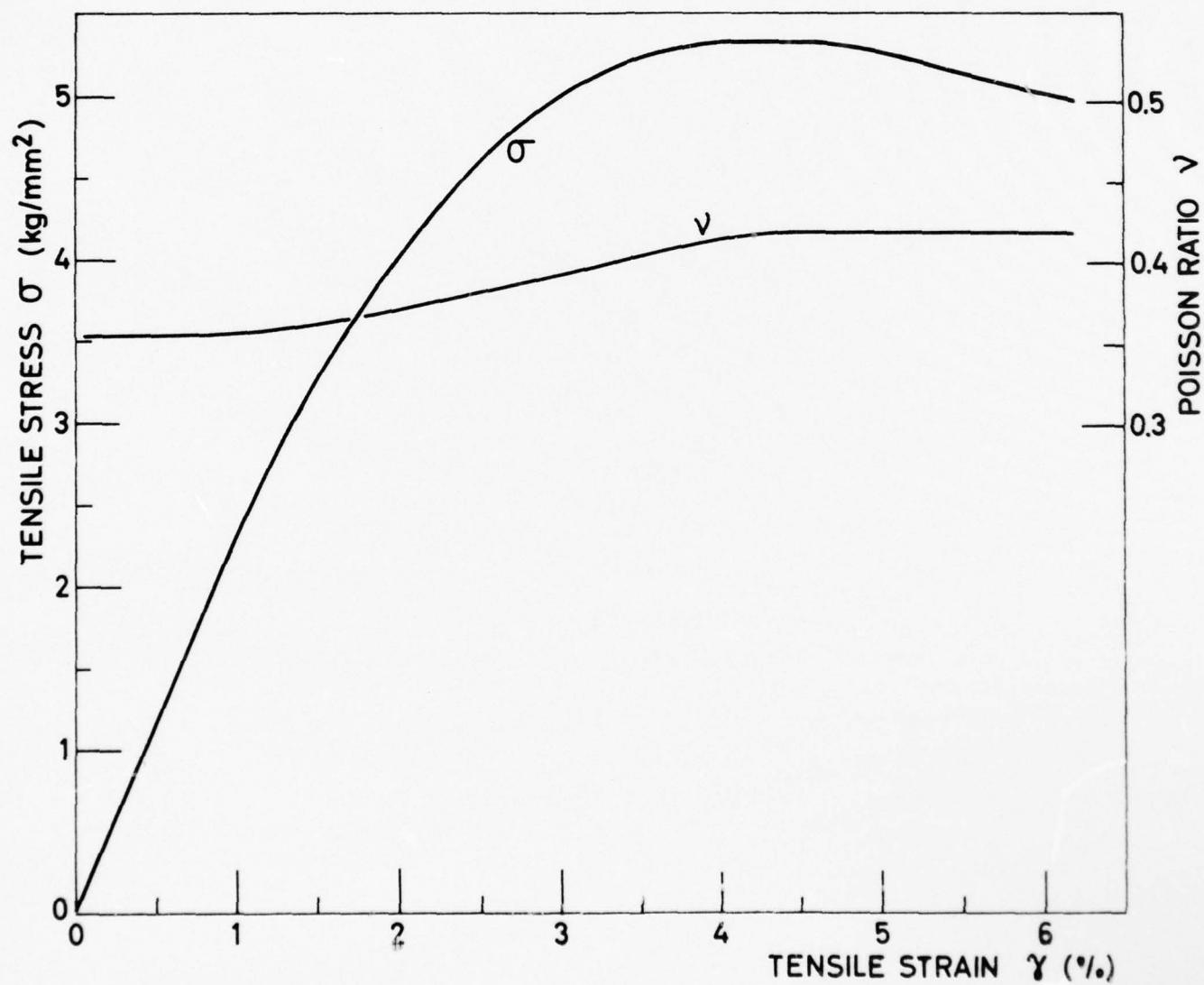


FIG. 1 TYPICAL STRESS-STRAIN CURVE AND THE RELATED POISSON RATIO FOR BULK EPOXY RESIN UNDER UNIAXIAL TENSION.

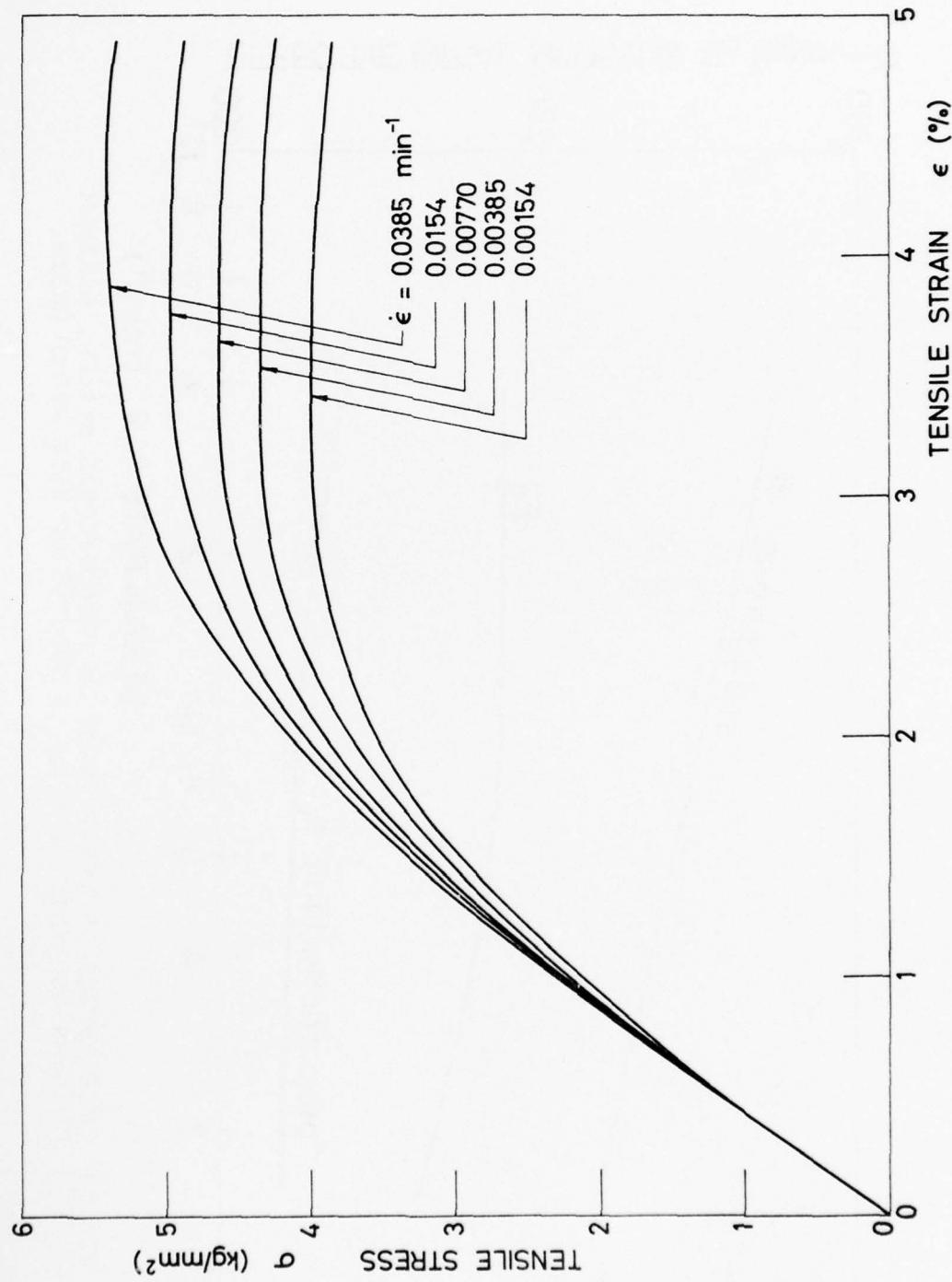


FIG. 2 TENSILE STRESS-STRAIN CURVES OF EPOXY RESIN AT DIFFERENT STRAIN RATES.

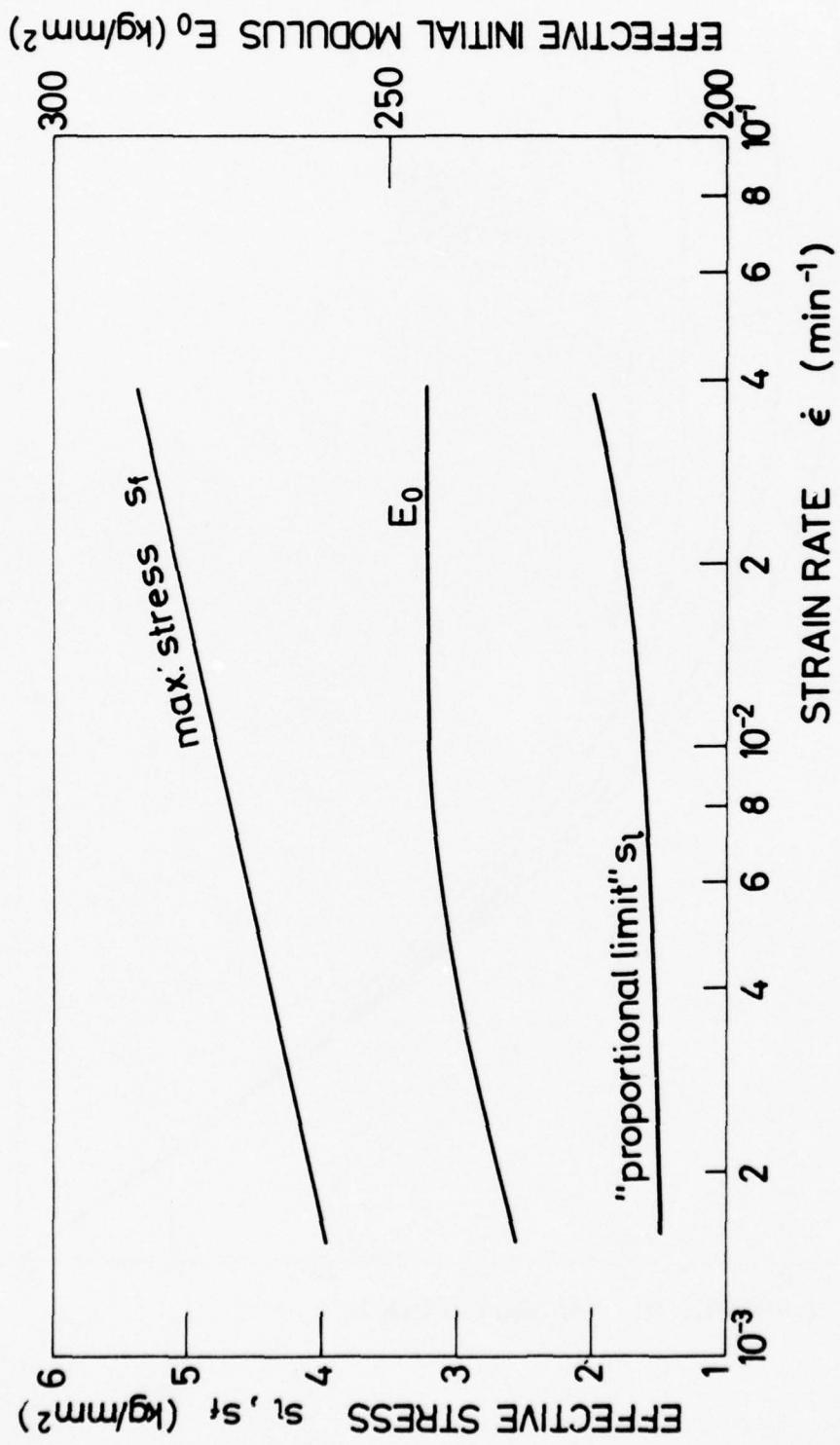


FIG. 3 THE EFFECT OF STRAIN RATE ON PROPORTIONAL STRESS, MAXIMUM STRESS AND INITIAL YOUNG'S MODULUS OF BULK EPOXY RESIN.

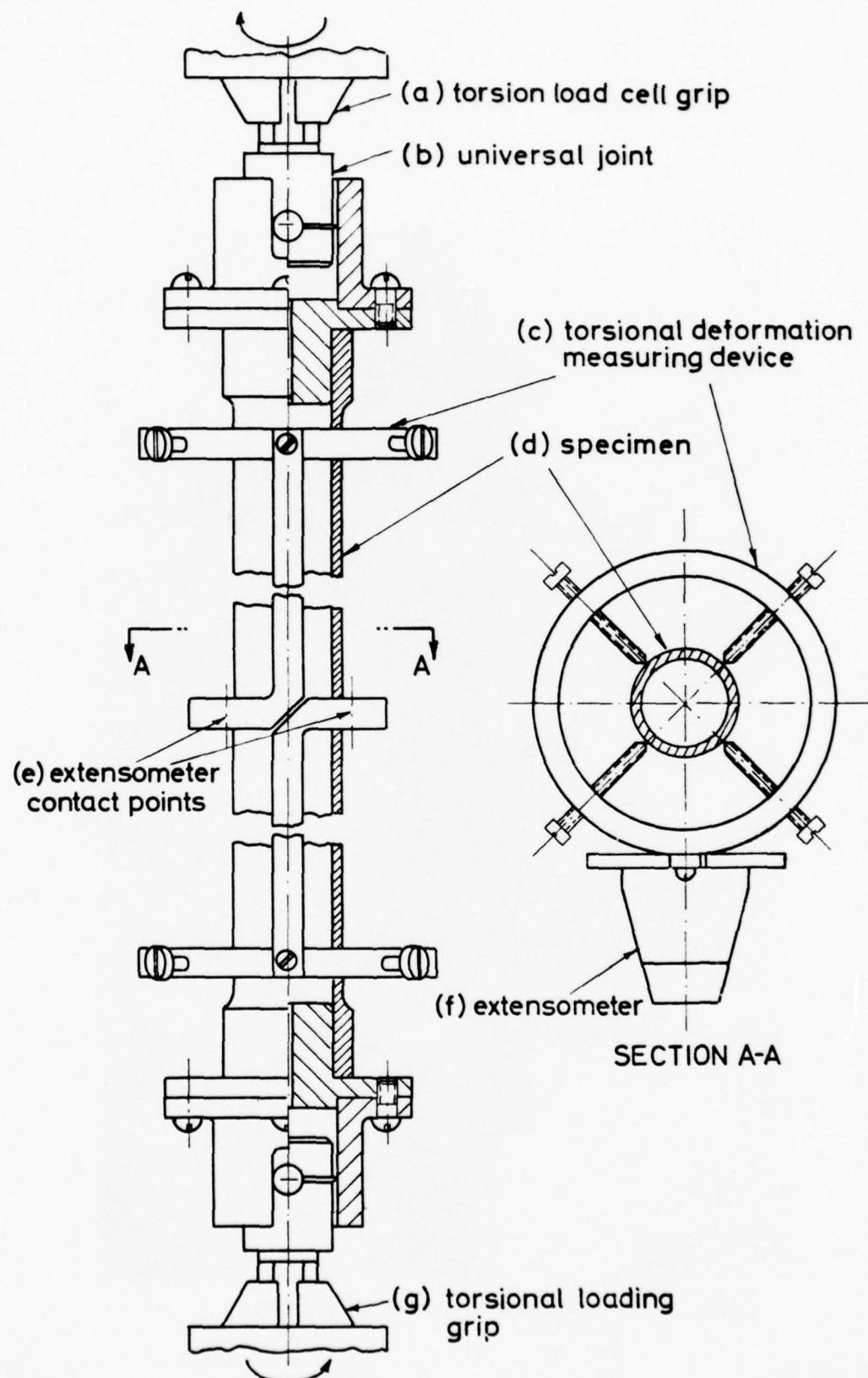


FIG. 4a ILLUSTRATION OF TUBULAR EPOXY SPECIMEN
WITH SPECIAL DEVICE FOR MEASURING ANGULAR
DISPLACEMENT UNDER TORSIONAL LOADING

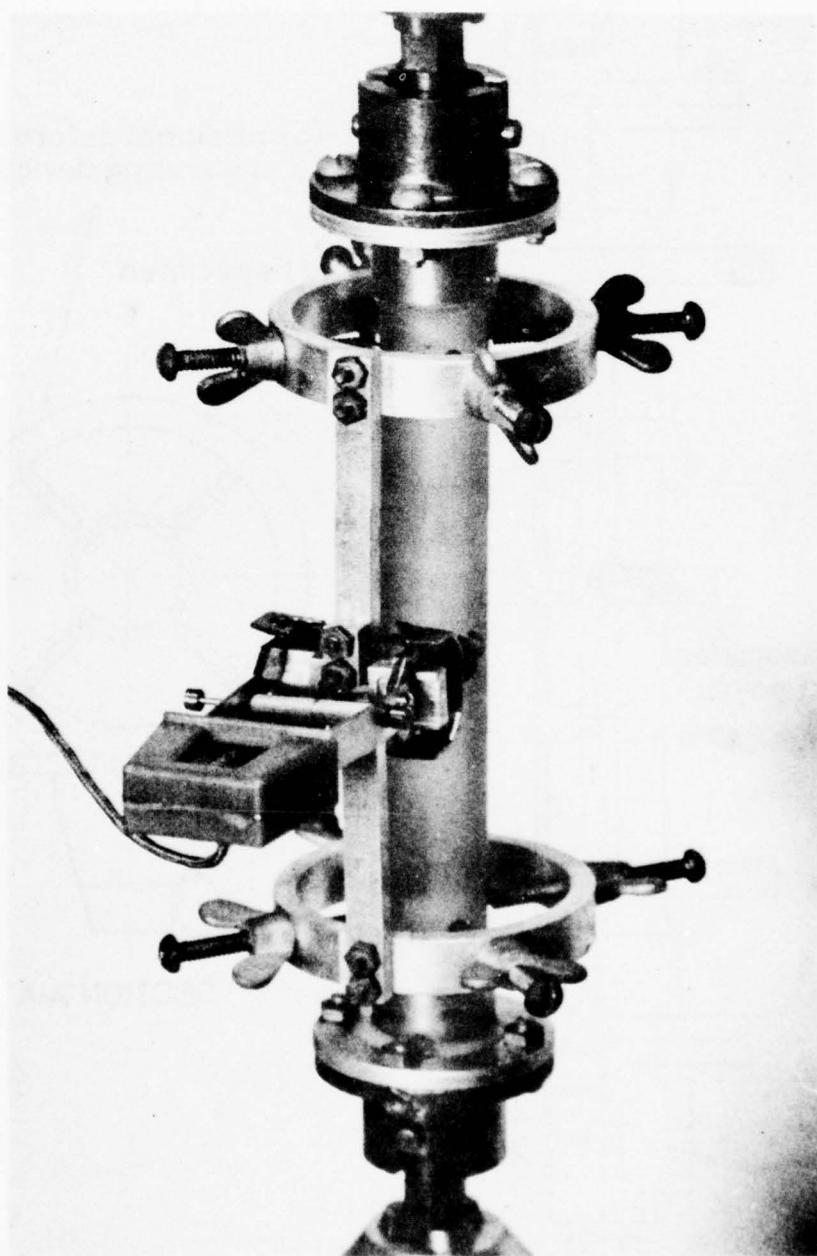


FIG. 4b PHOTOGRAPH OF TUBULAR EPOXY SPECIMEN WITH SPECIAL DEVICE FOR MEASURING ANGULAR DISPLACEMENT UNDER TORSIONAL LOADING.

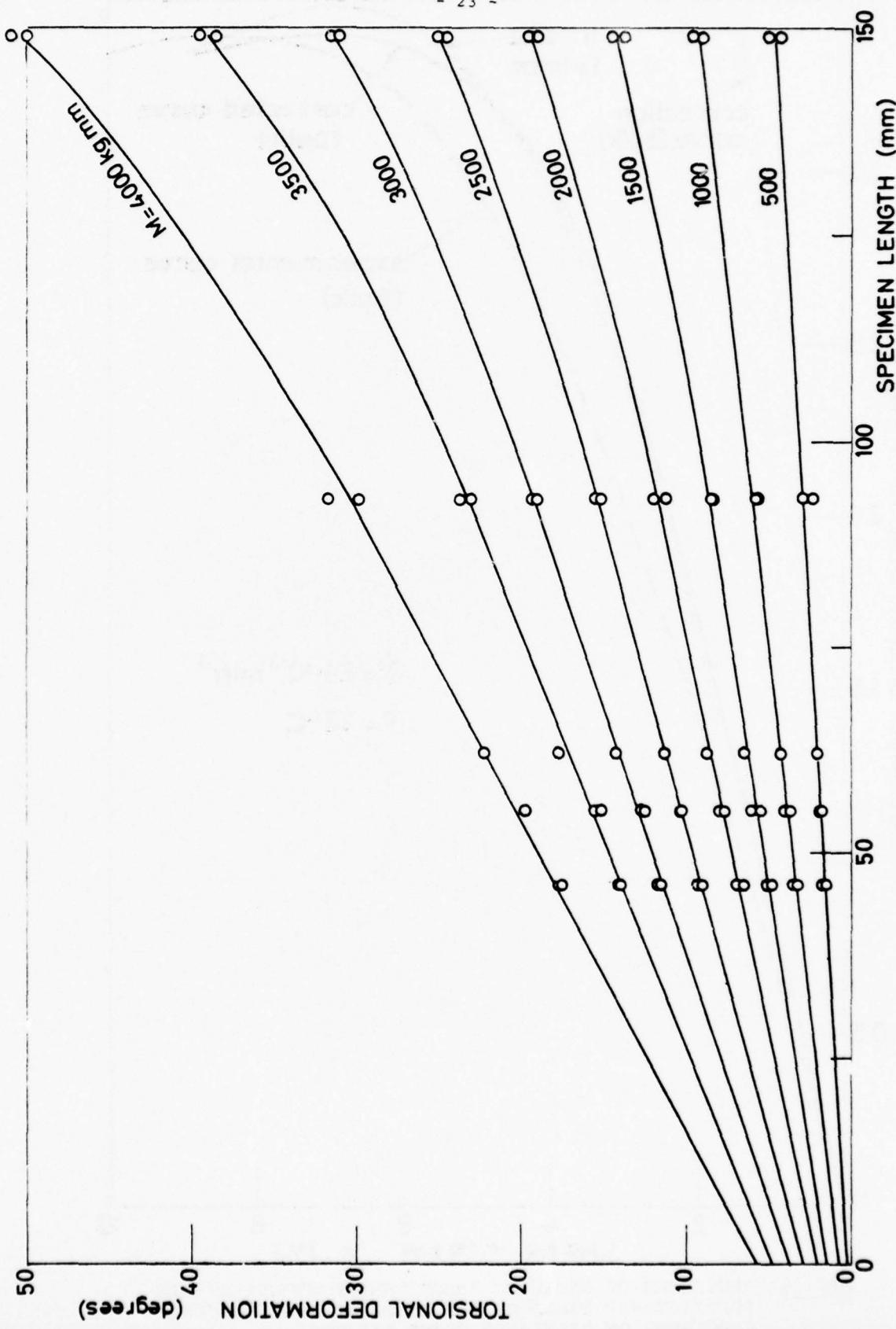


FIG. 5 THE EFFECT OF SPECIMEN LENGTH ON TORSIONAL DEFORMATION
PROCEDURE FOR CORRECTING SHEAR STRESS-STRAIN RELATIONSHIP.

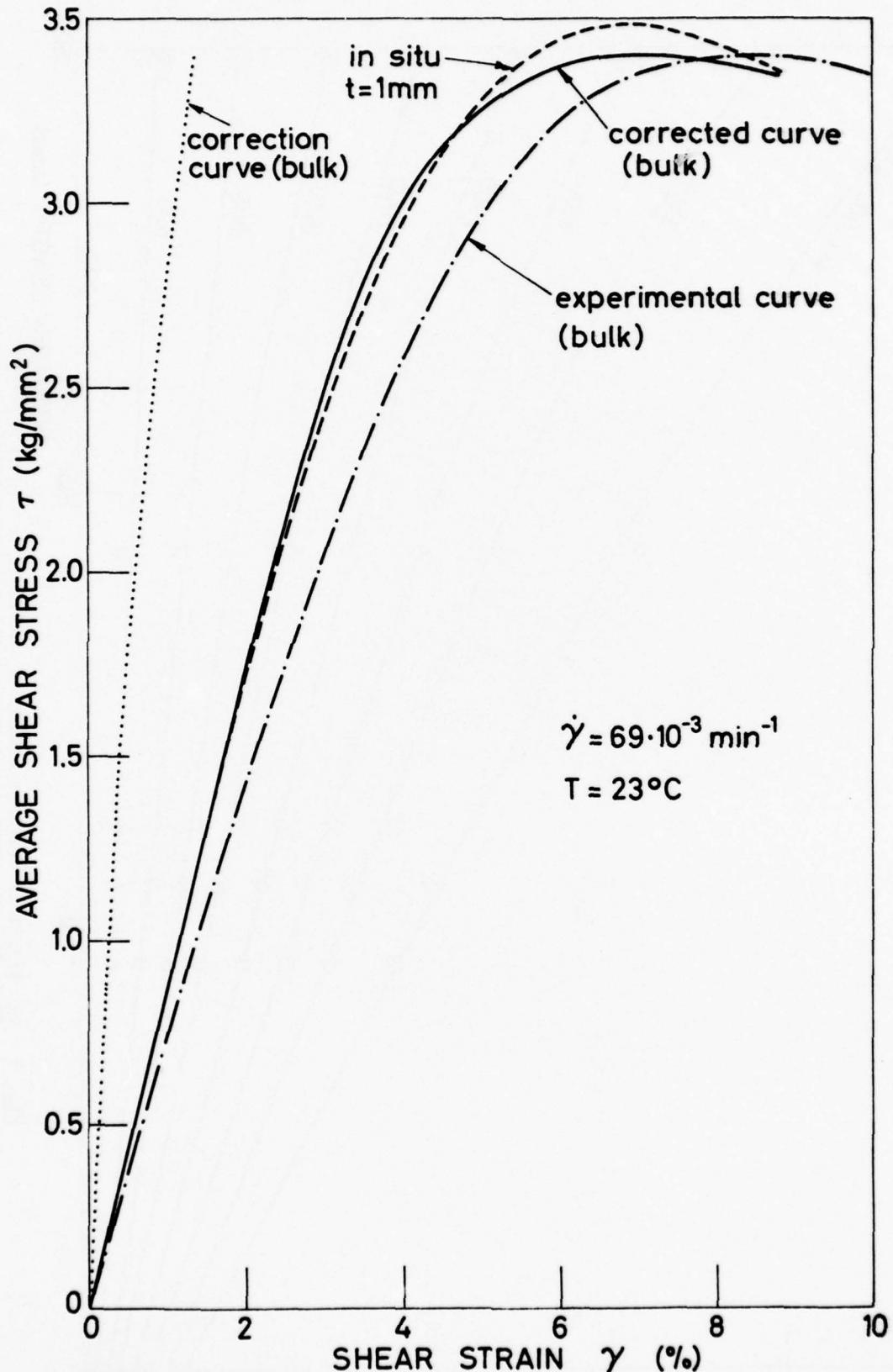


FIG. 6 PROCEDURE OF DERIVING "TRUE" SHEAR STRESS-STRAIN RELATIONSHIP BASED ON TORSIONAL TEST OF TUBULAR SPECIMENS OF DIFFERENT GAUGE LENGTHS.

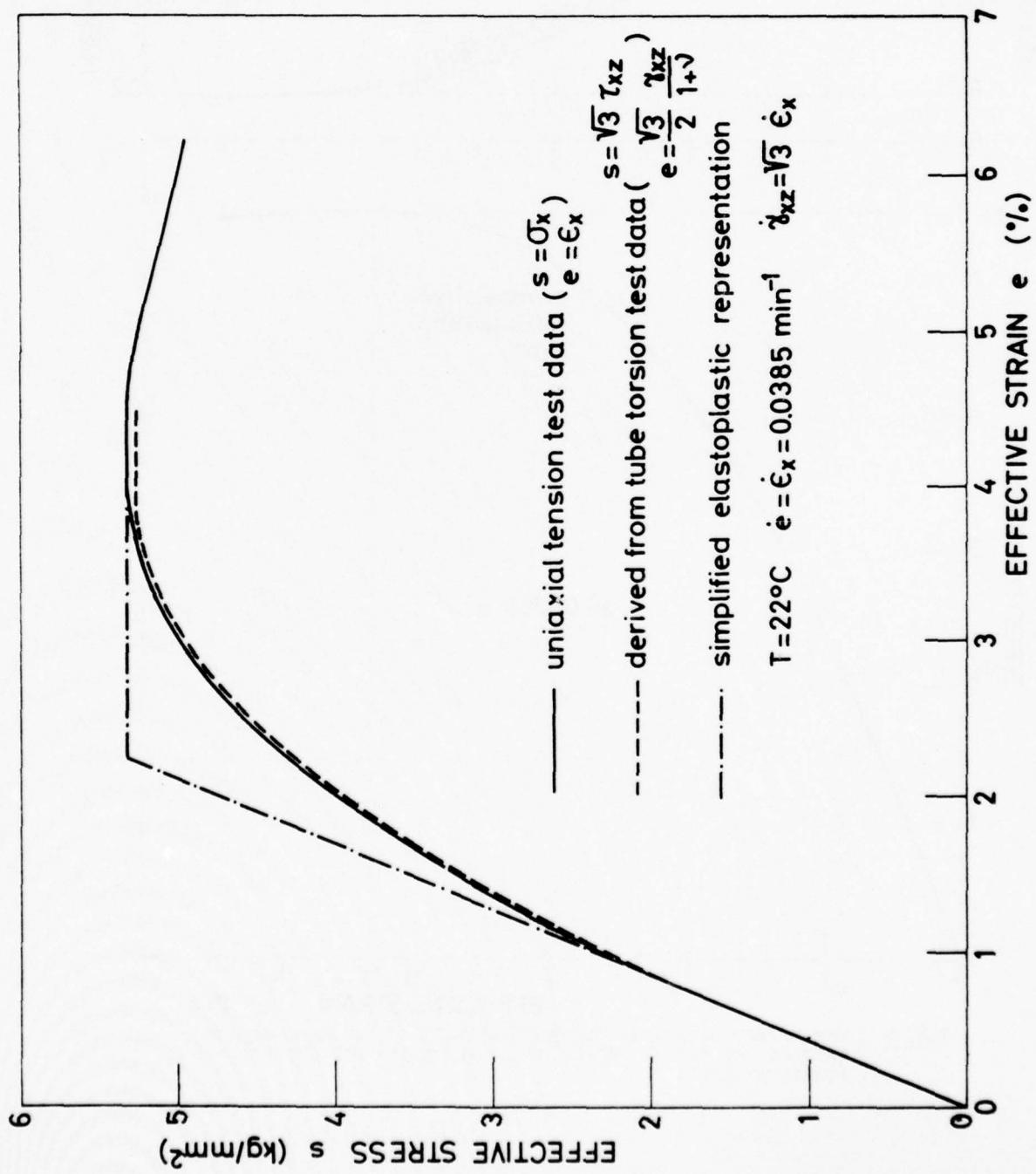


FIG. 7 EFFECTIVE STRESS-STRAIN RELATIONSHIP AND RELATED PROPERTIES FOR EPOXY RESIN USED AS ADHESIVE LAYER.

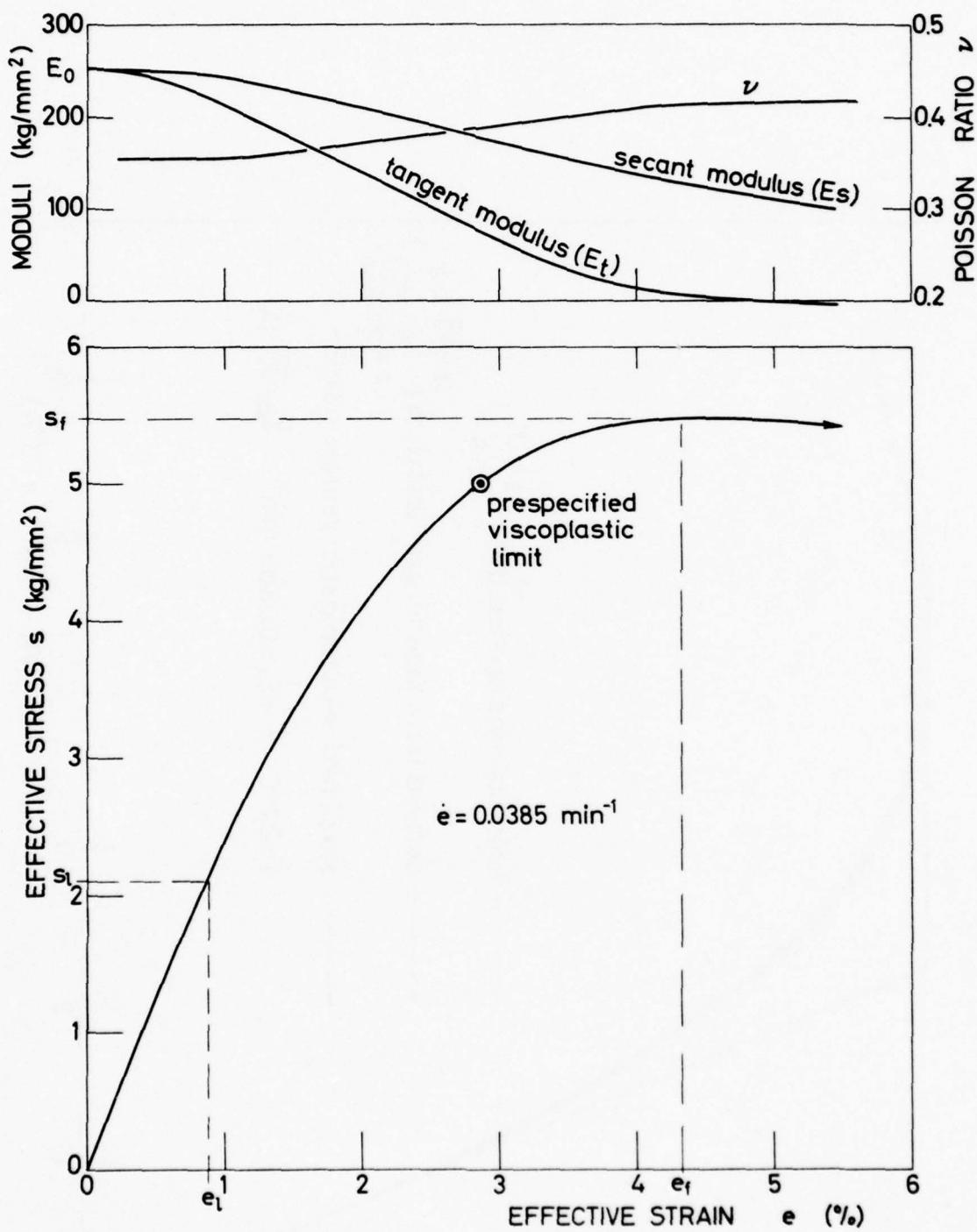


FIG. 8 THE EFFECT OF STRAIN ON THE VARIATION OF POISSON'S RATIO,
TANGENT AND SECANT MODULI OF EPOXY RESIN USED FOR
ADHESIVE LAYER.

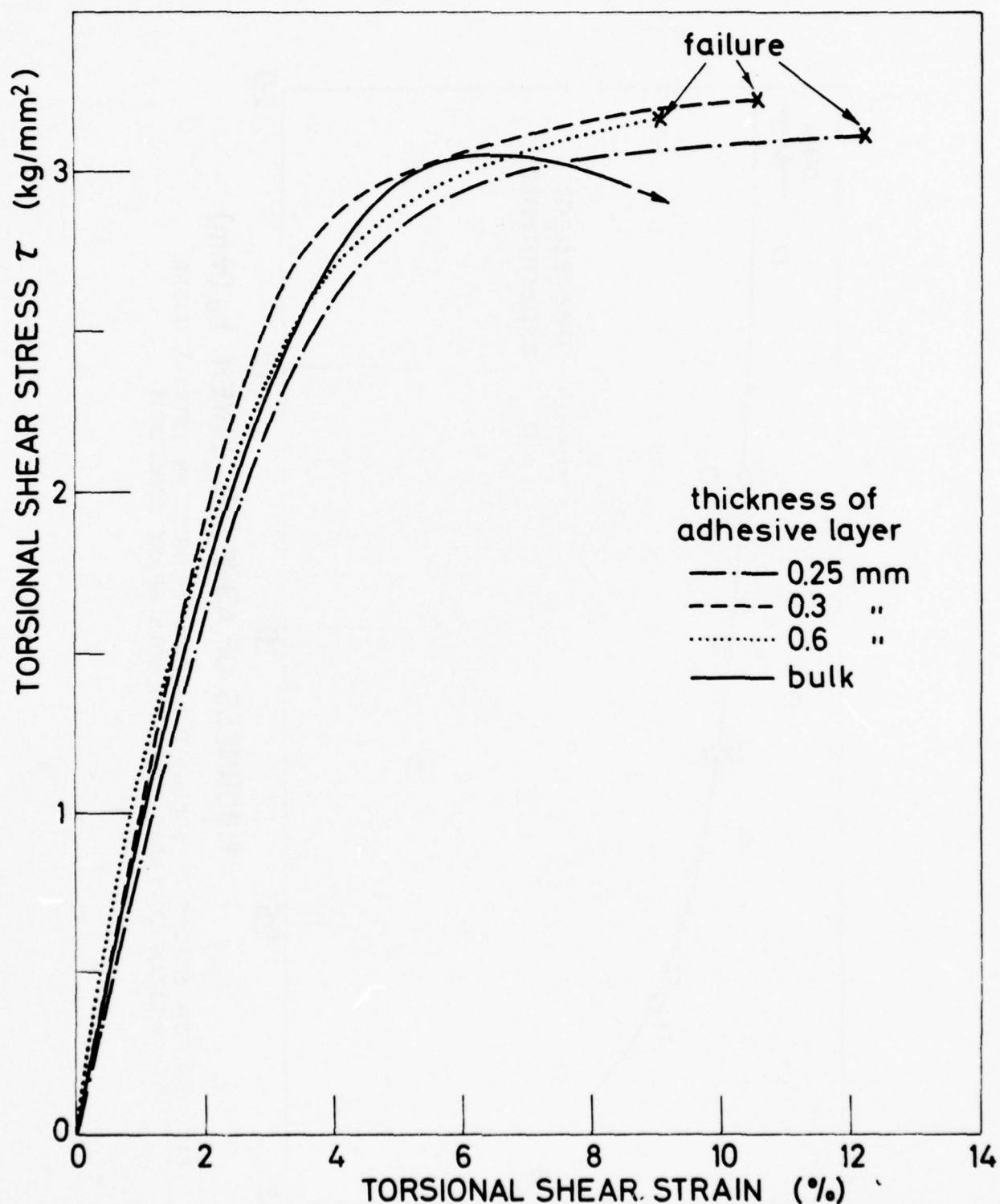


FIG. 9 TYPICAL SHEAR STRESS-STRAIN CURVES FOR THE ADHESIVE LAYER IN-SITU COMPARED WITH ITS BULK EPOXY REFERENCE (FIG. 6).

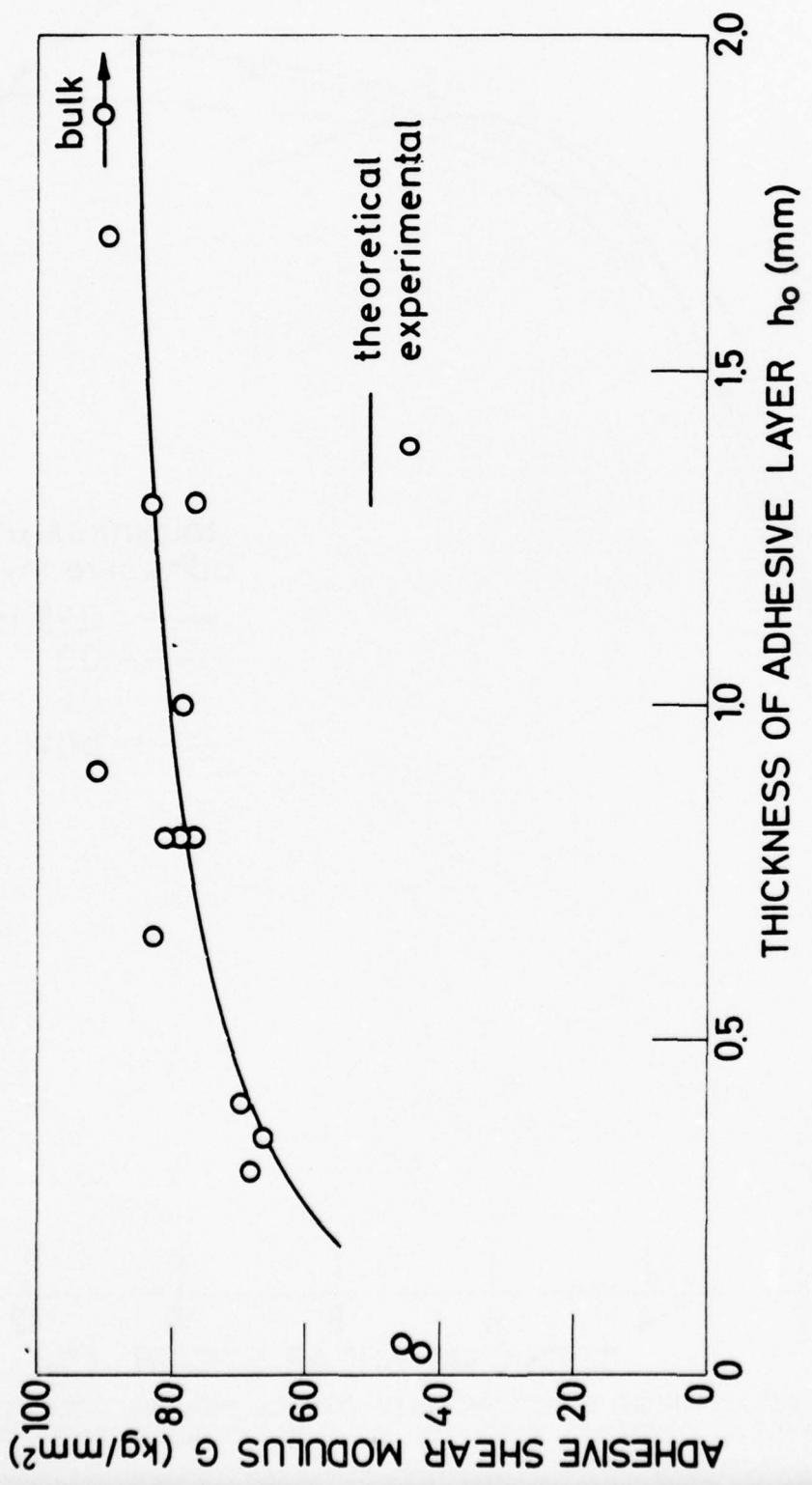


FIG. 10 THE EFFECT OF ADHESIVE LAYER THICKNESS ON INITIAL SHEAR MODULUS COMPARED WITH ITS BULK EPOXY REFERENCE.

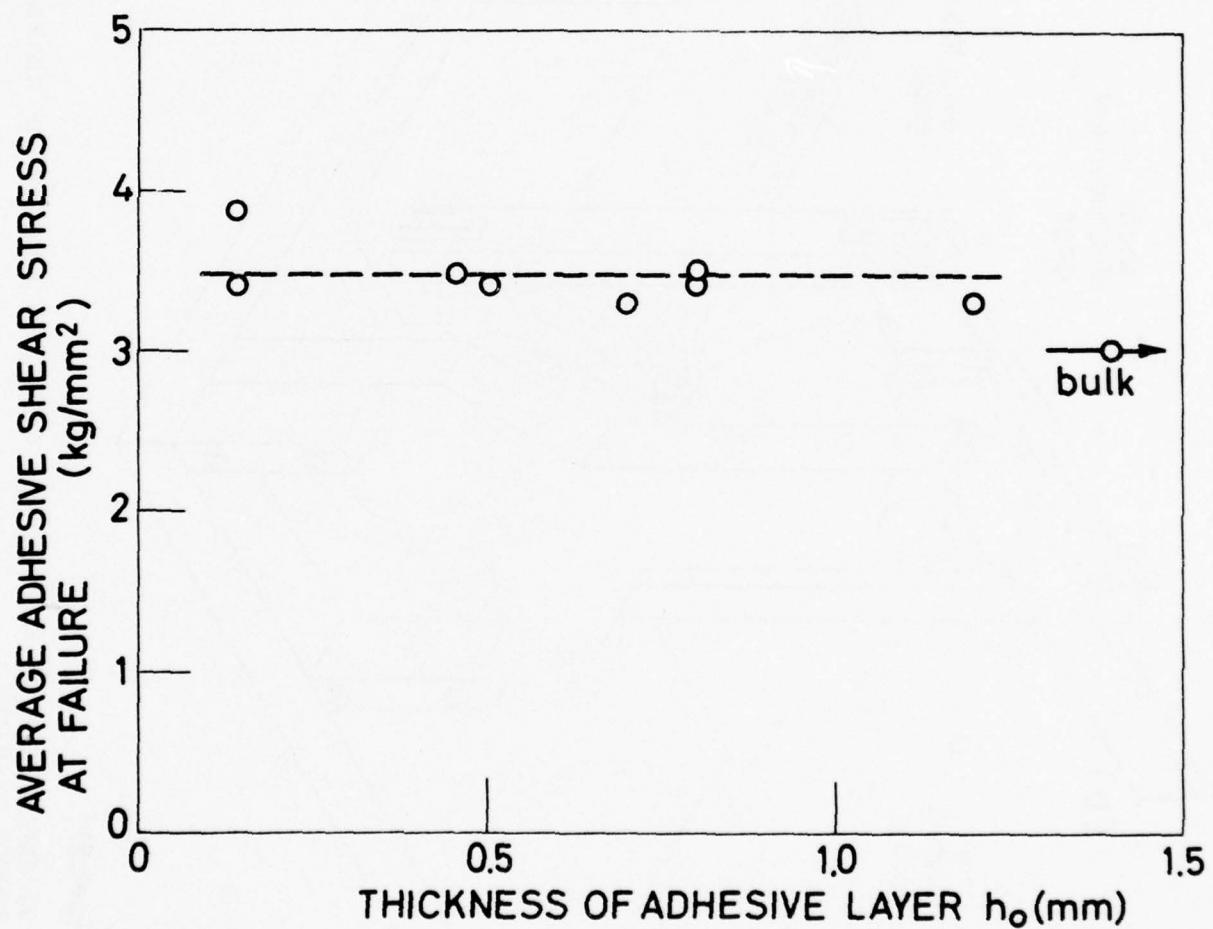


FIG. 11 THE EFFECT OF ADHESIVE LAYER THICKNESS ON ULTIMATE SHEAR STRENGTH COMPARED WITH ITS BULK EPOXY REFERENCE.

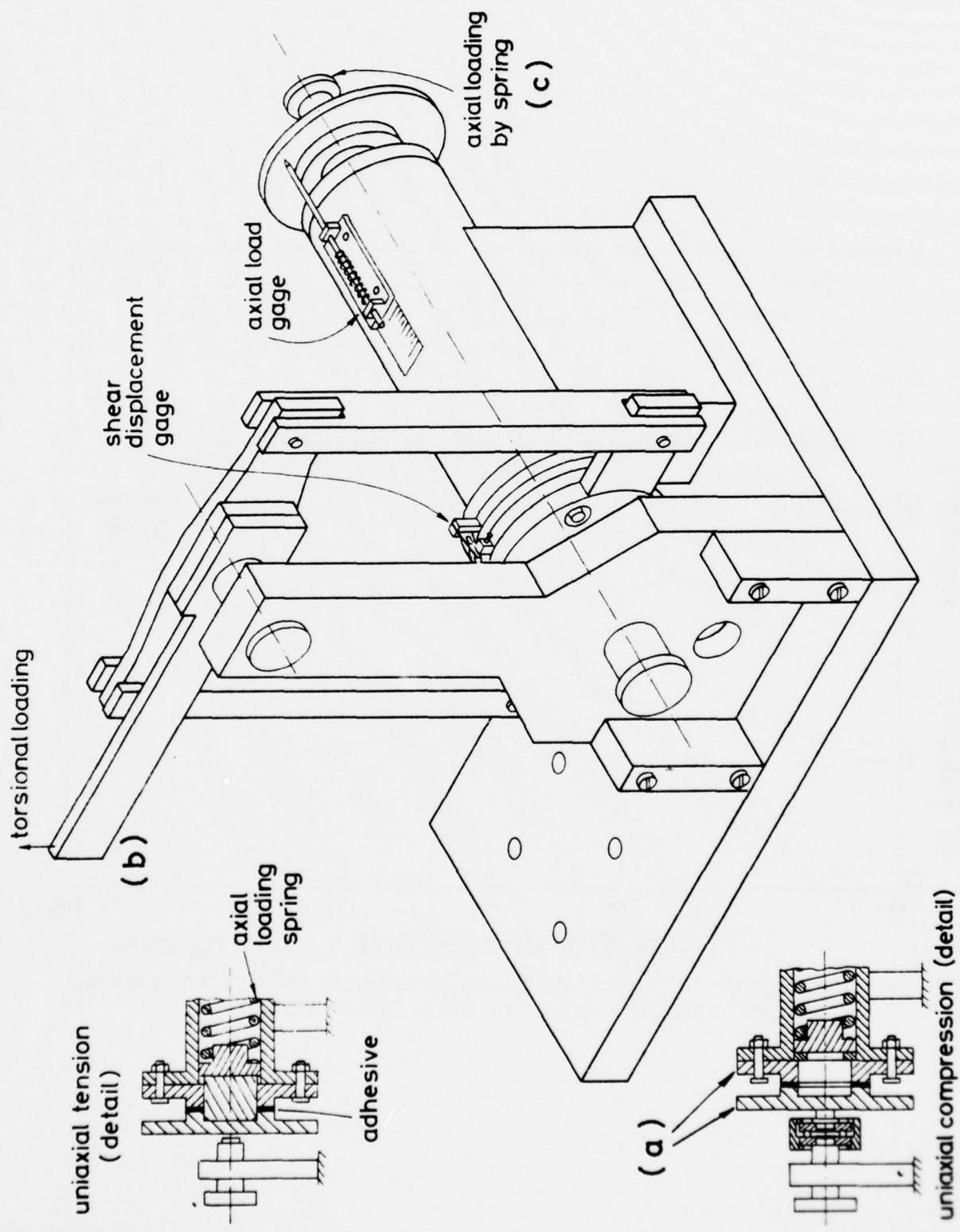


FIG. 12a ILLUSTRATION OF DEVICE FOR COMBINED LOADING OF ADHESIVE LAYER IN-SITU.

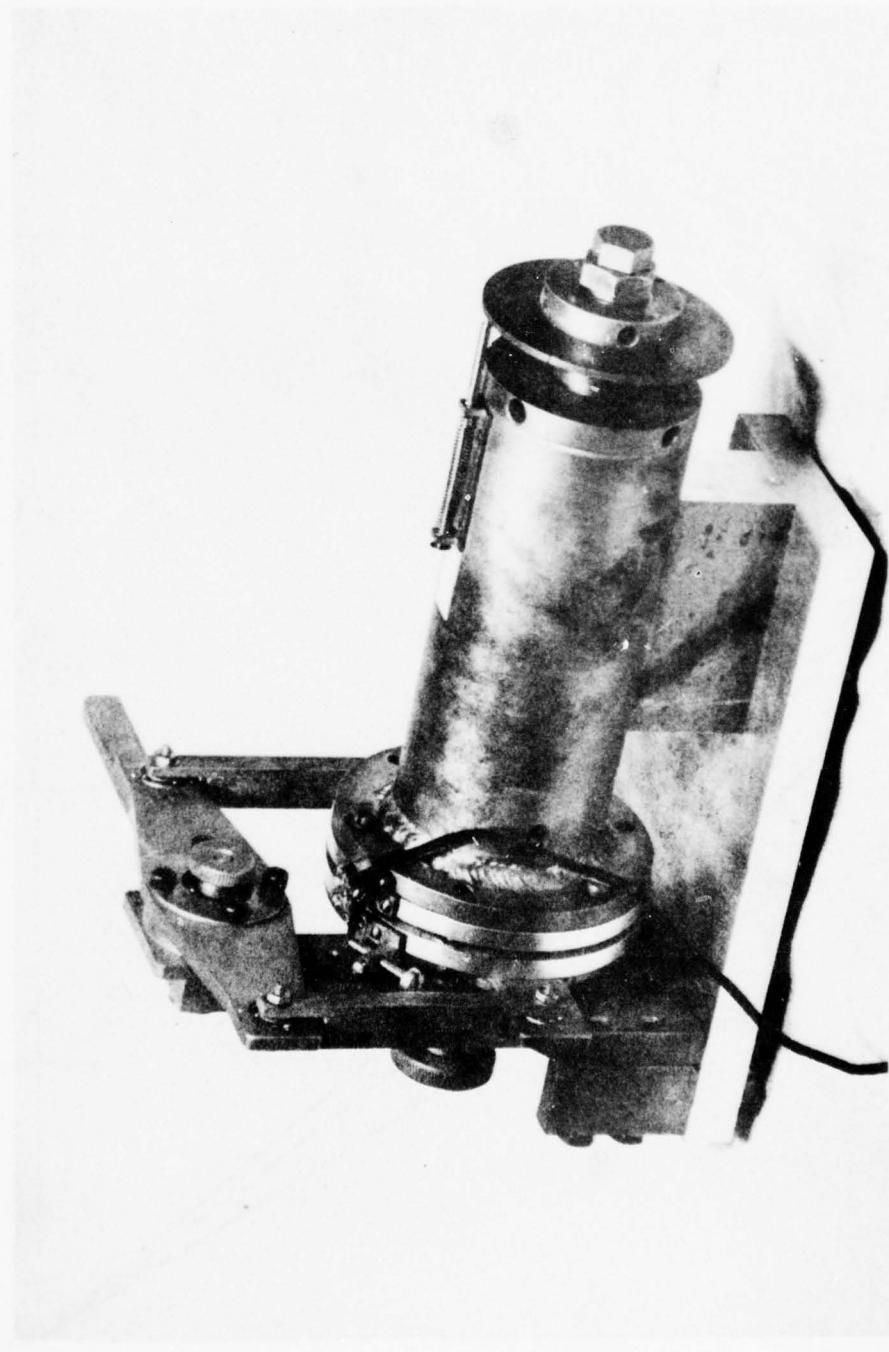


FIG. 12b PHOTOGRAPH OF DEVICE FOR COMBINED LOADING OF ADHESIVE LAYER IN-SITU.

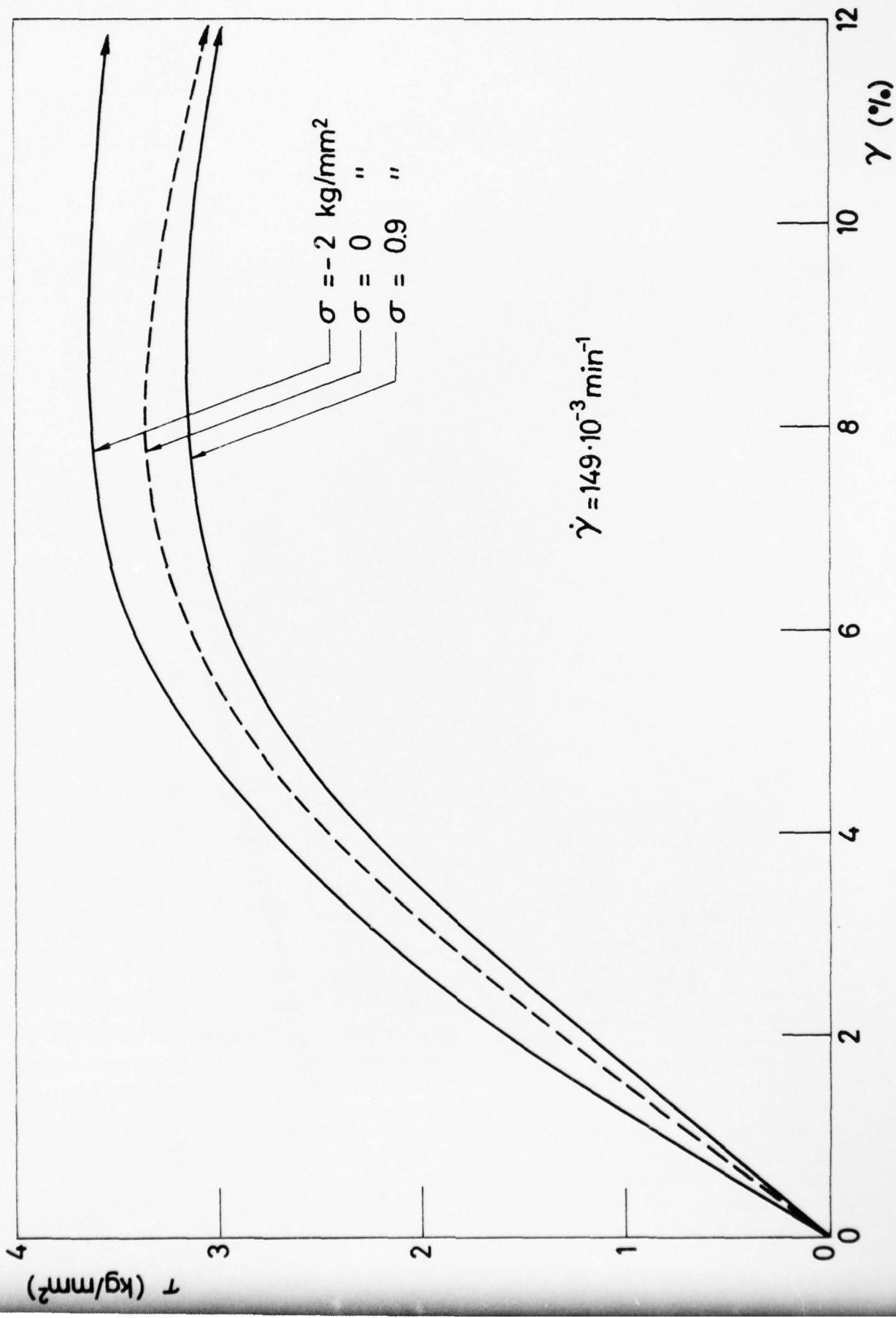


FIG. 13 TYPICAL SHEAR STRESS-STRAIN CURVES OF ADHESIVE LAYER IN-SITU UNDER COMBINED SHEAR AND AXIAL NORMAL LOADING.

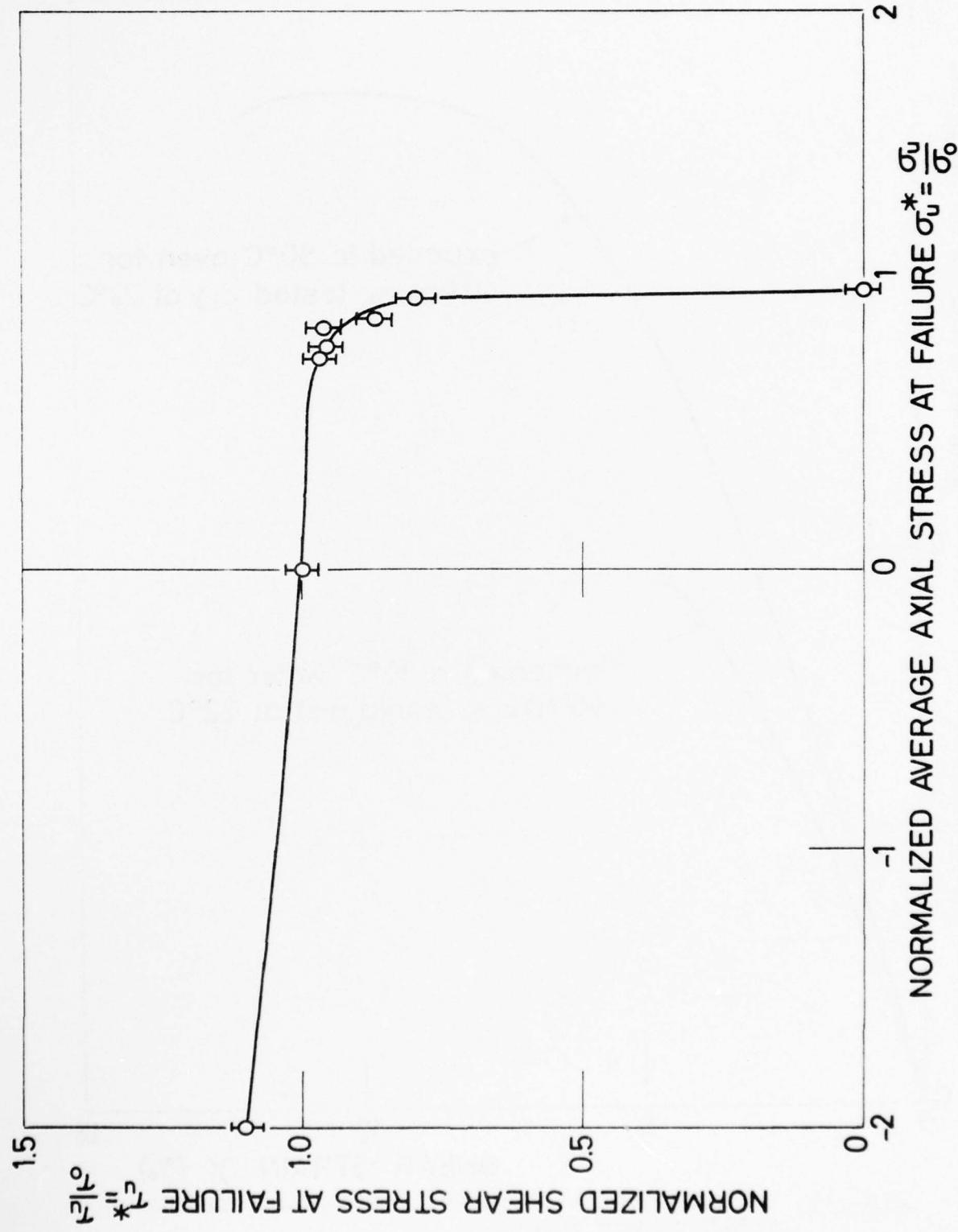


FIG. 14 FAILURE ENVELOPE FOR ADHESIVE LAYER IN-SITU (LOADED UNDER COMBINED SHEAR AND AXIAL NORMAL STRESS).

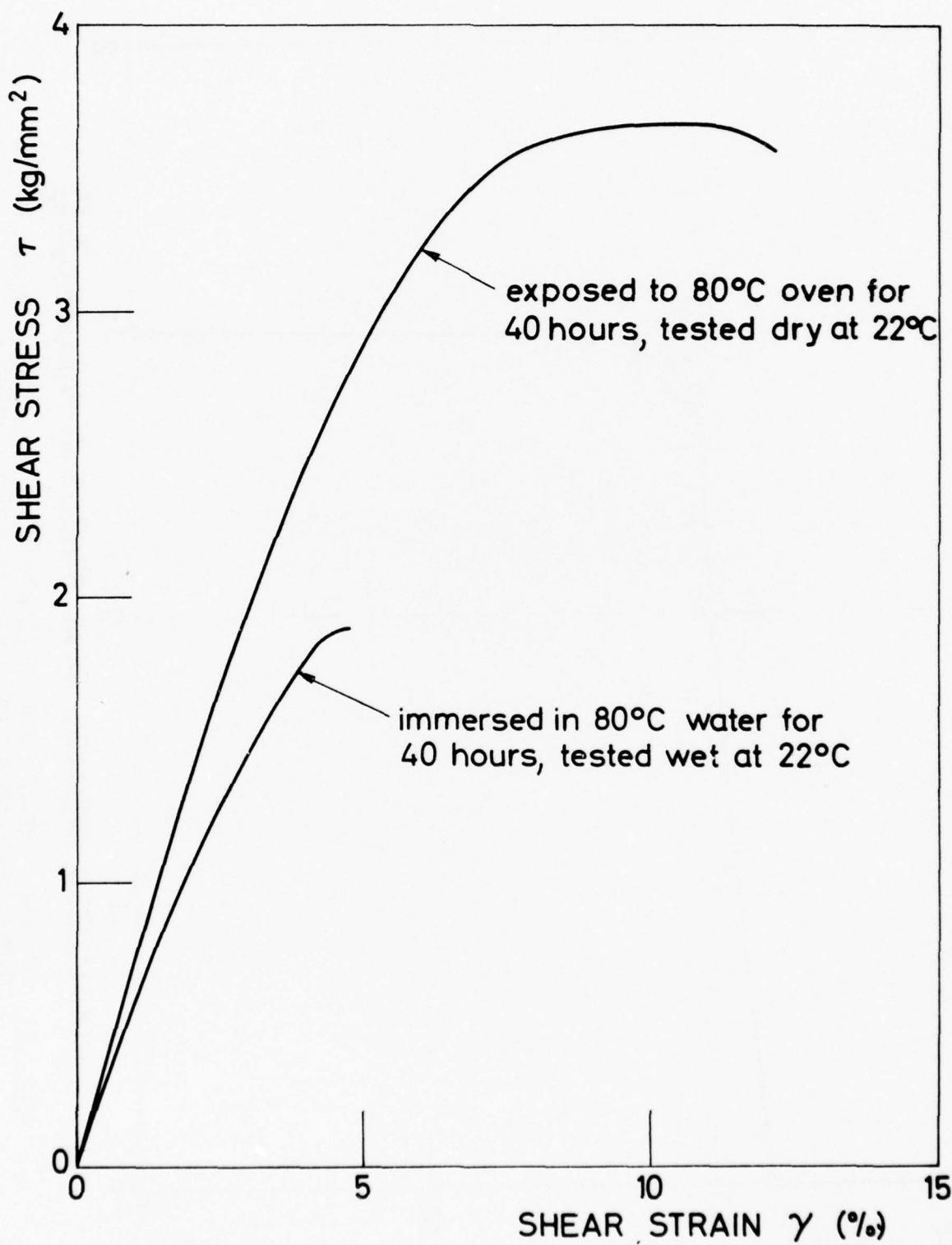


FIG. 15 THE EFFECT OF HYGROTHERMAL HISTORY ON ADHESIVE STRESS-STRAIN BEHAVIOR.

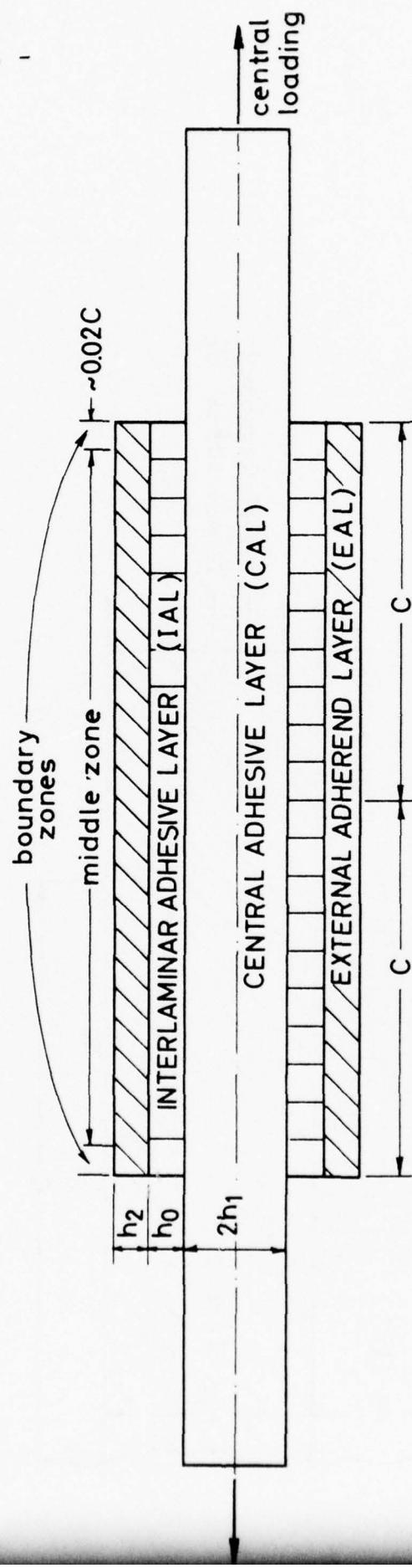


FIG. 16 SYMMETRICAL DOUBLER MODEL.

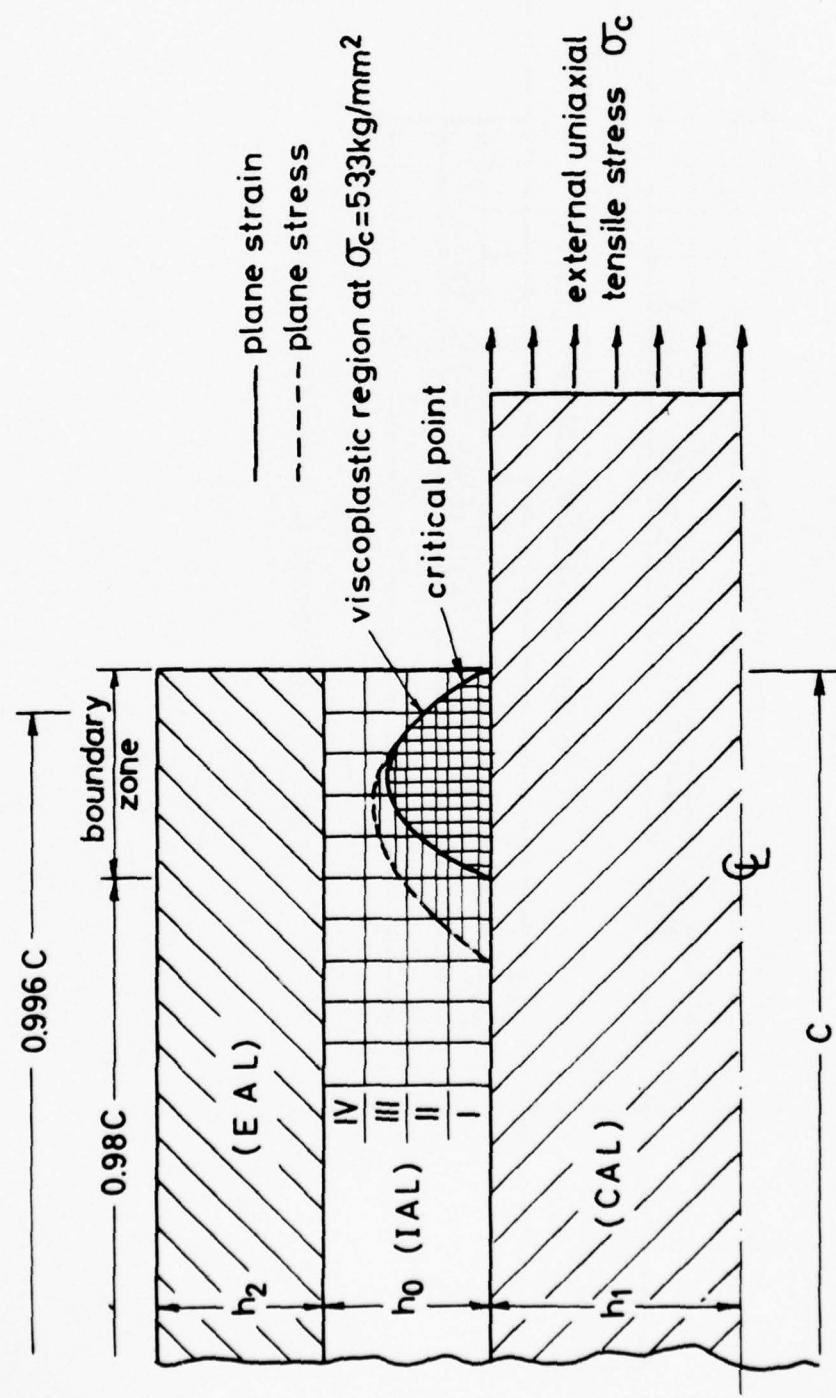


FIG. 17 ILLUSTRATION OF IAL NETWORK AT THE BOUNDARY ZONE USED FOR FINITE ELEMENT NON-LINEAR STRESS ANALYSIS.

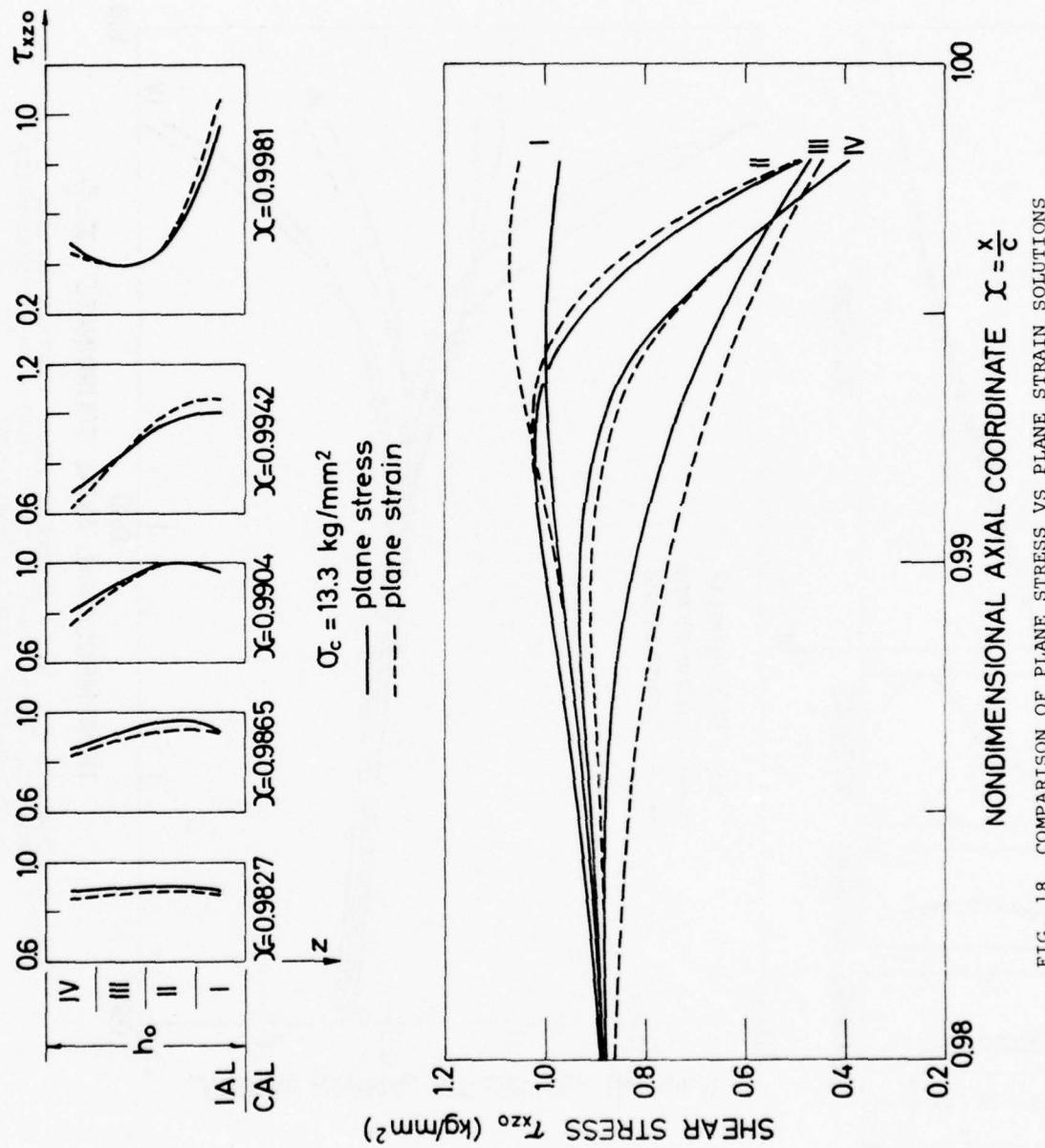


FIG. 18 COMPARISON OF PLANE STRESS VS PLANE STRAIN SOLUTIONS FOR INTERLAMINAR SHEAR-STRESS (τ_{xz}) DISTRIBUTION AT THE BOUNDARY ZONE (LINEAR RANGE).

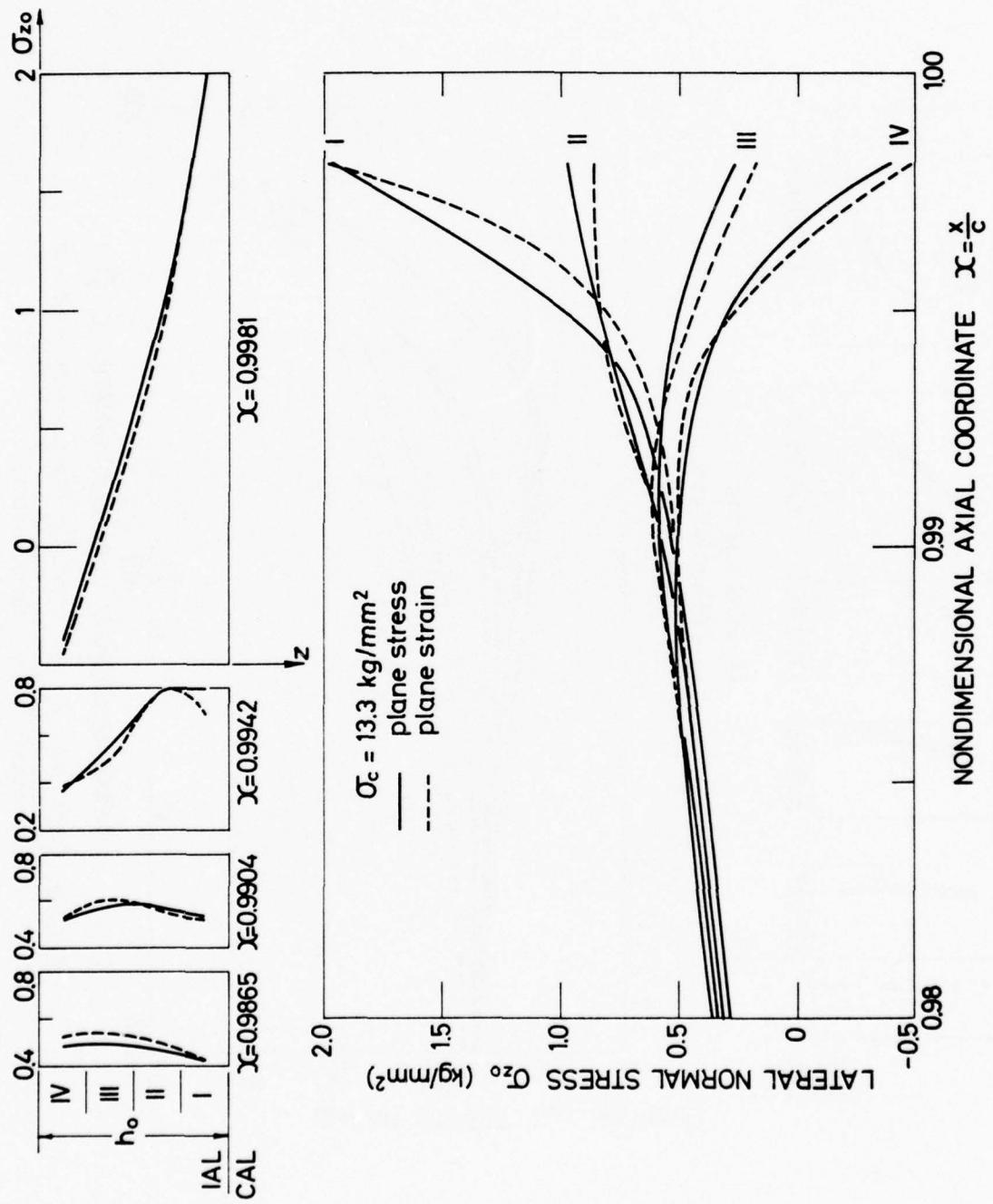


FIG. 19 COMPARISON OF PLANE STRESS VS. PLANE STRAIN SOLUTIONS FOR INTERLAMINAR LATERAL NORMAL STRESS (σ_z) DISTRIBUTION AT THE BOUNDARY ZONE (LINEAR RANGE).

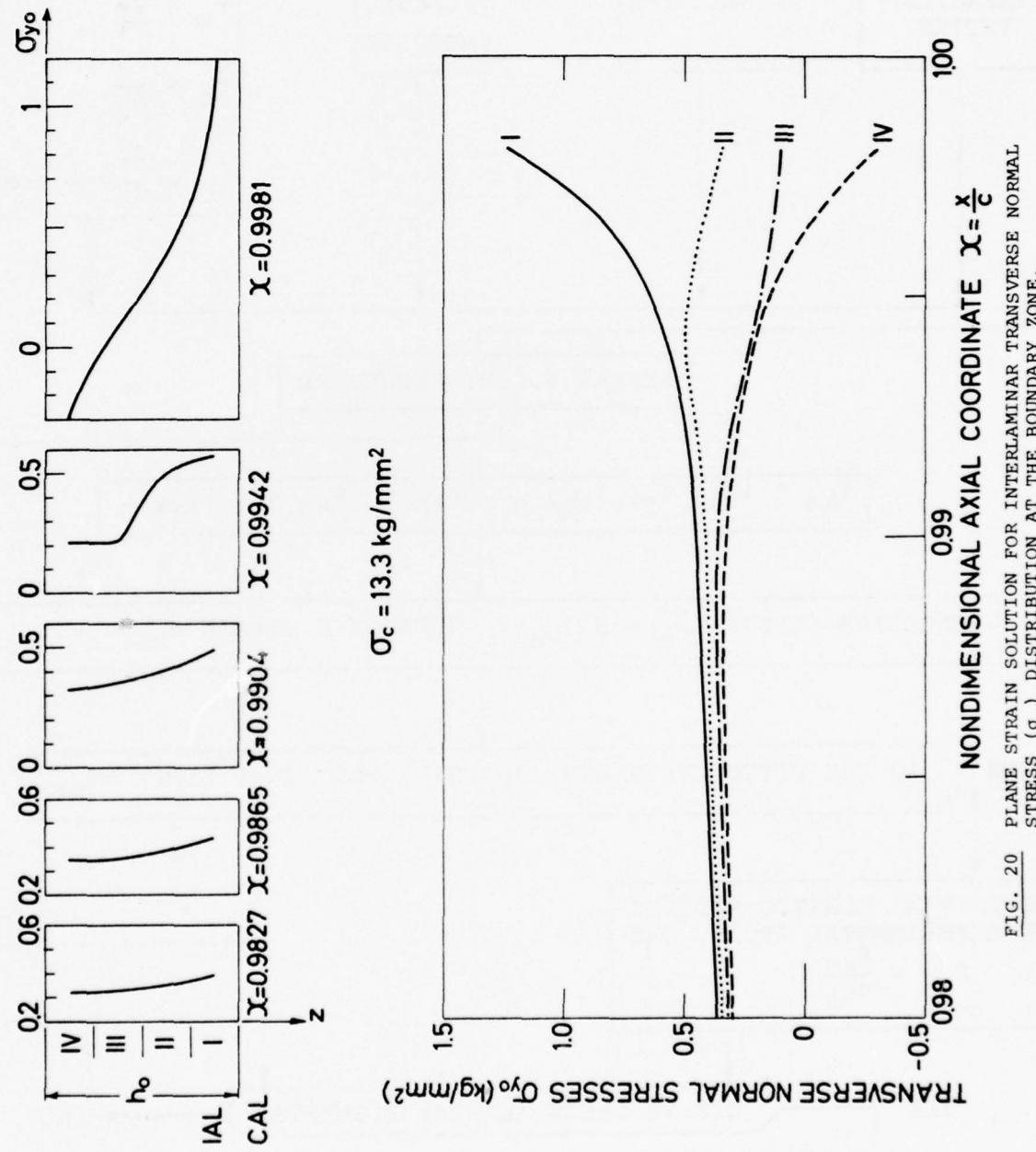


FIG. 20 PLANE STRAIN SOLUTION FOR INTERLAMINAR TRANSVERSE NORMAL STRESS (σ_y) DISTRIBUTION AT THE BOUNDARY ZONE.

FLOW CHART

FOR STRESS ANALYSIS OF INTERLAMINAR ADHESIVE LAYER
AT THE NONLINEAR RANGE.

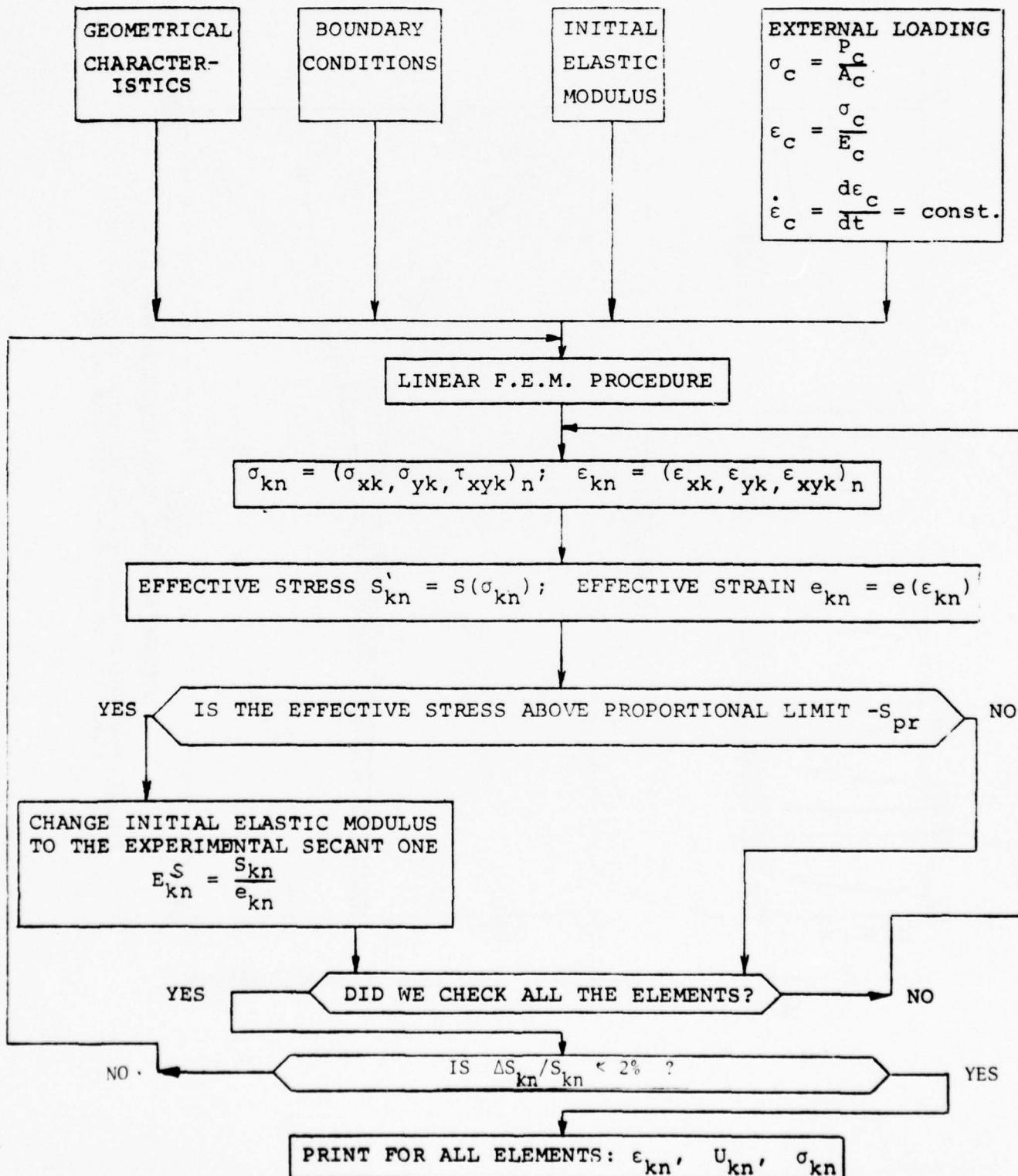


FIG. 21 FLOW CHART FOR STRESS ANALYSIS OF INTERLAMINAR ADHESIVE LAYER AT THE NON-LINEAR RANGE.

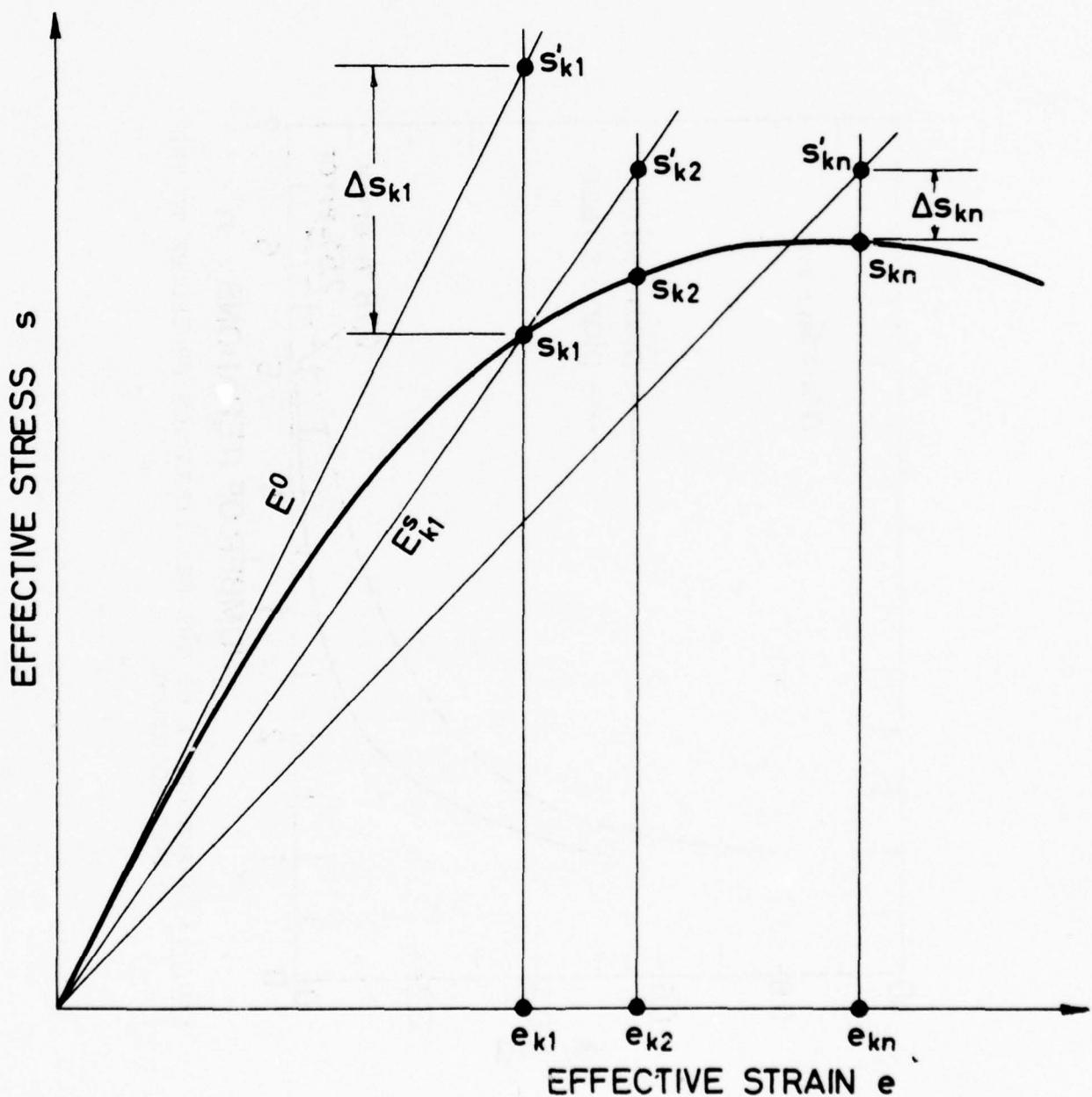


FIG. 22 ILLUSTRATION OF FEM PROCEDURE FOR STRESS ANALYSIS OF TAL AT THE NONLINEAR RANGE.

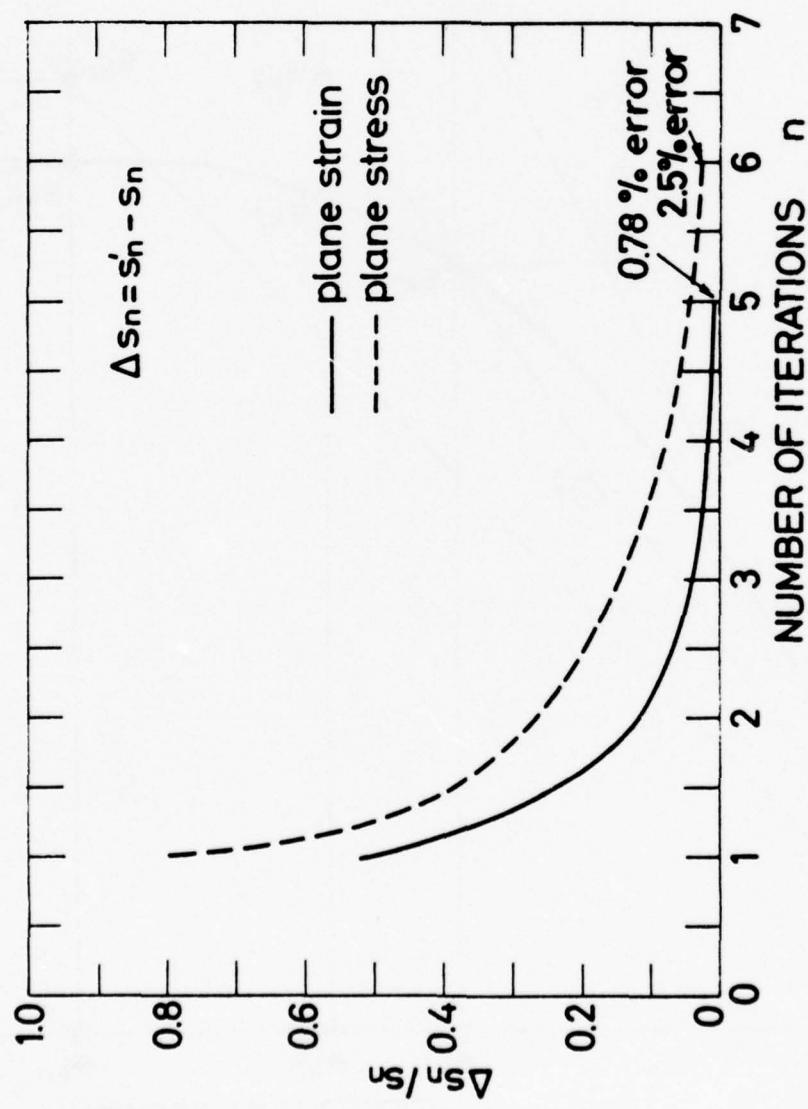


FIG. 2.3 CONVERGENCE OF THE FEM ITERATION PROCEDURE TO THE EXACT SOLUTION.

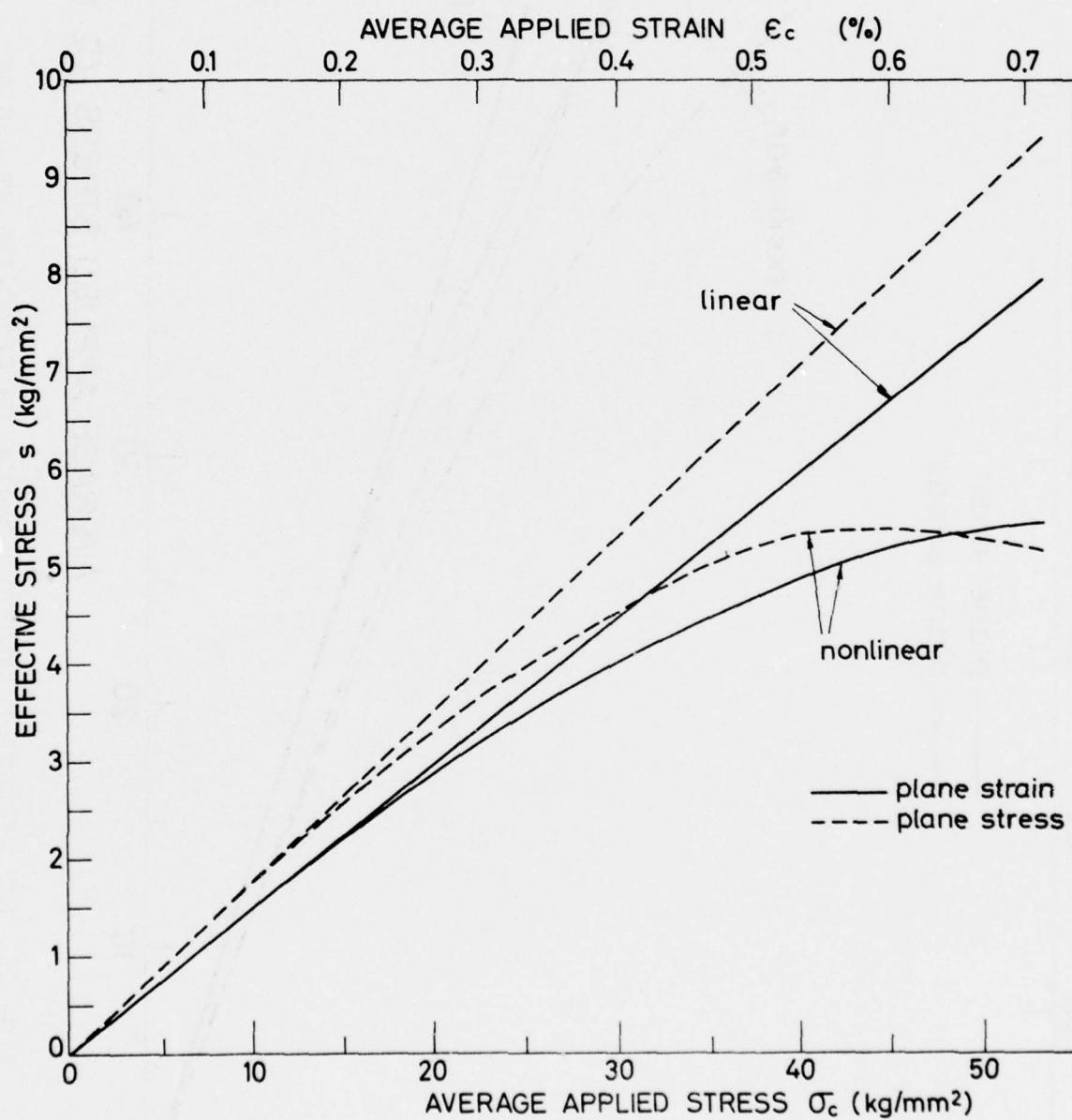


FIG. 24 THE EFFECT OF CENTRAL LOADING ON EFFECTIVE STRESSES AT THE CRITICAL POINTS IN THE BOUNDARY ZONE OF THE IAL.

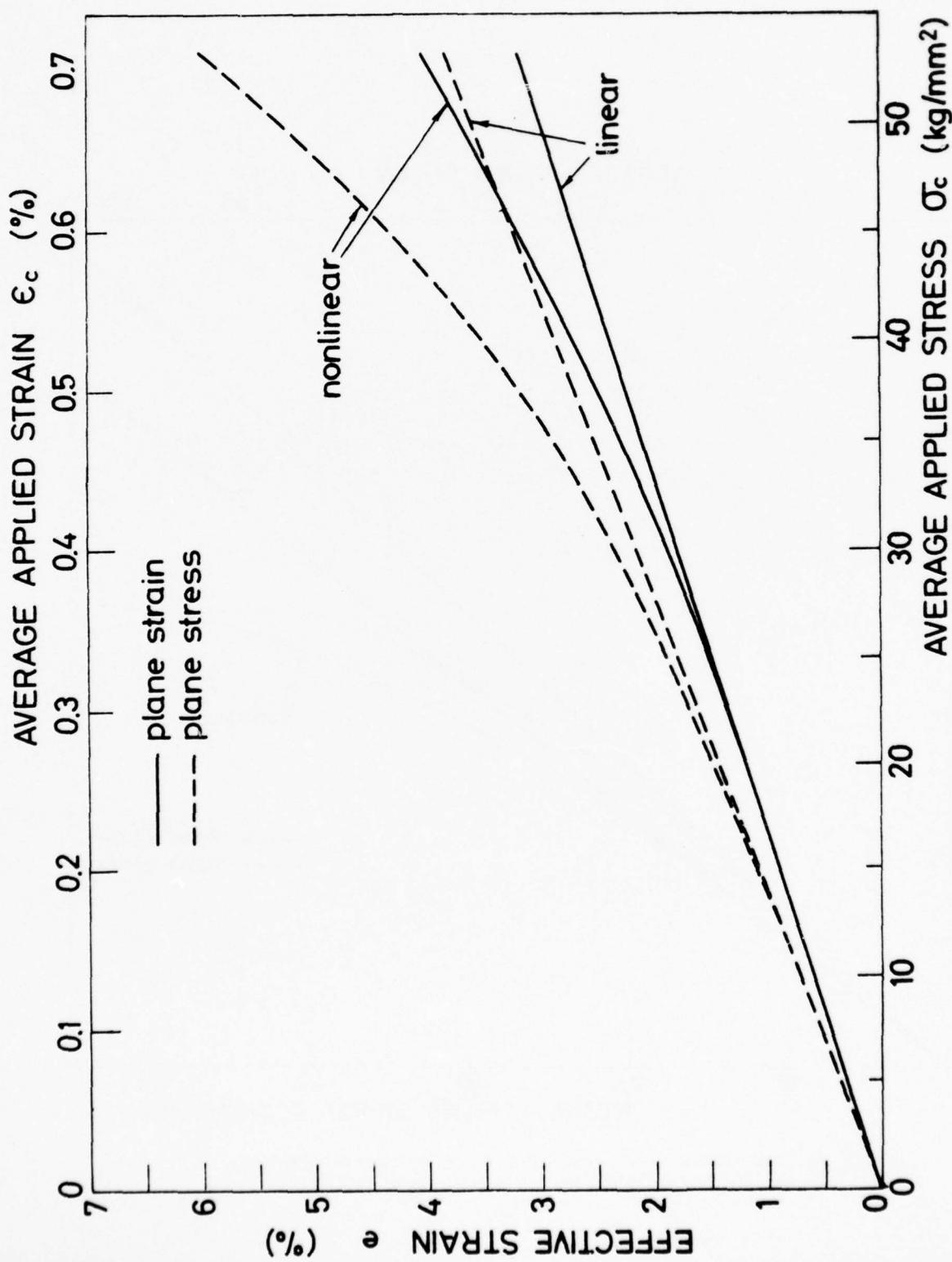


FIG. 25 THE EFFECT OF CENTRAL LOADING ON EFFECTIVE STRAIN AT THE CRITICAL POINTS IN THE BOUNDARY ZONE OF THE IAL.

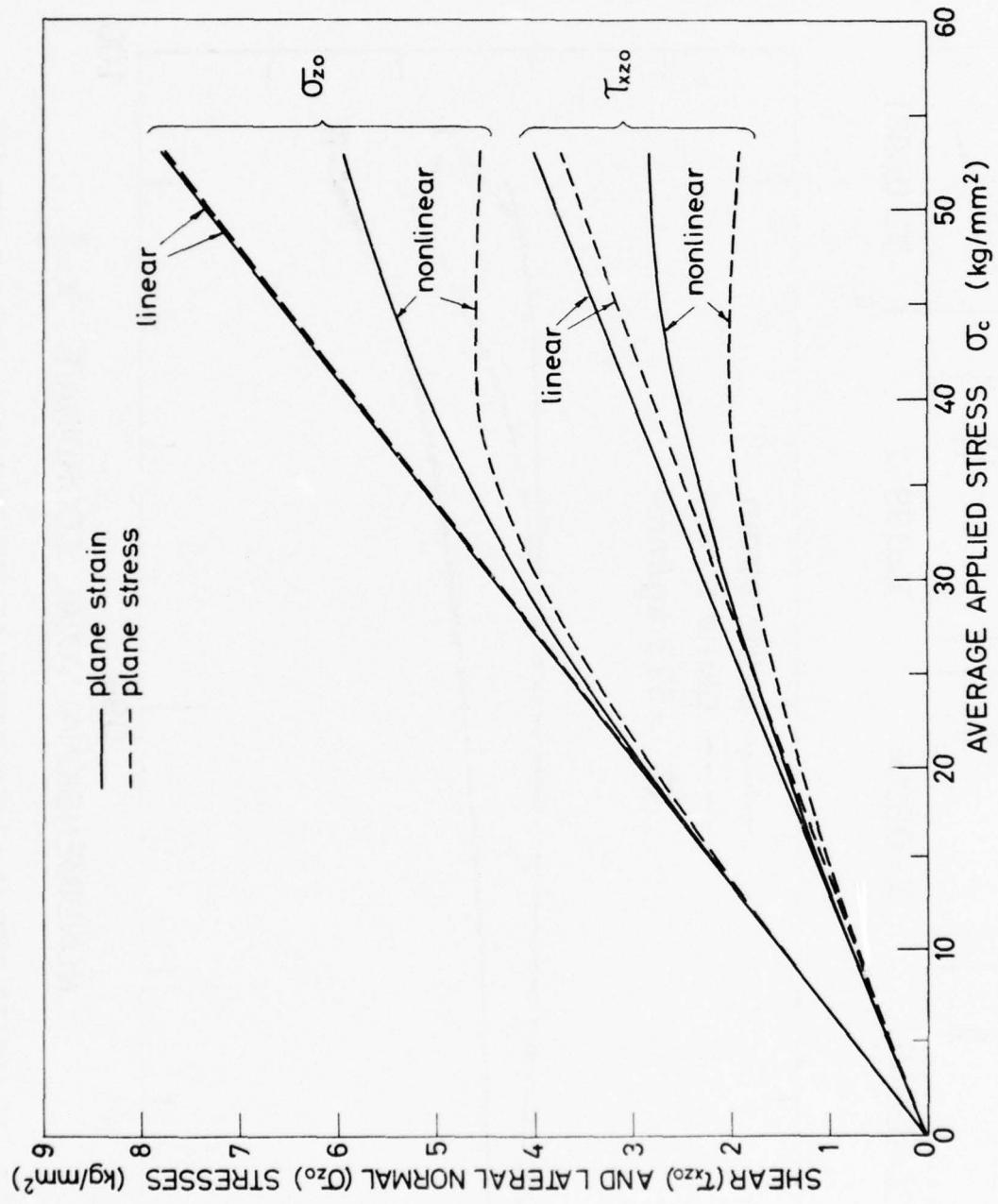


FIG. 26 THE EFFECT OF CENTRAL LOADING ON SHEAR AND NORMAL STRESSES AT THE CRITICAL POINT IN THE BOUNDARY ZONE OF THE IAL.

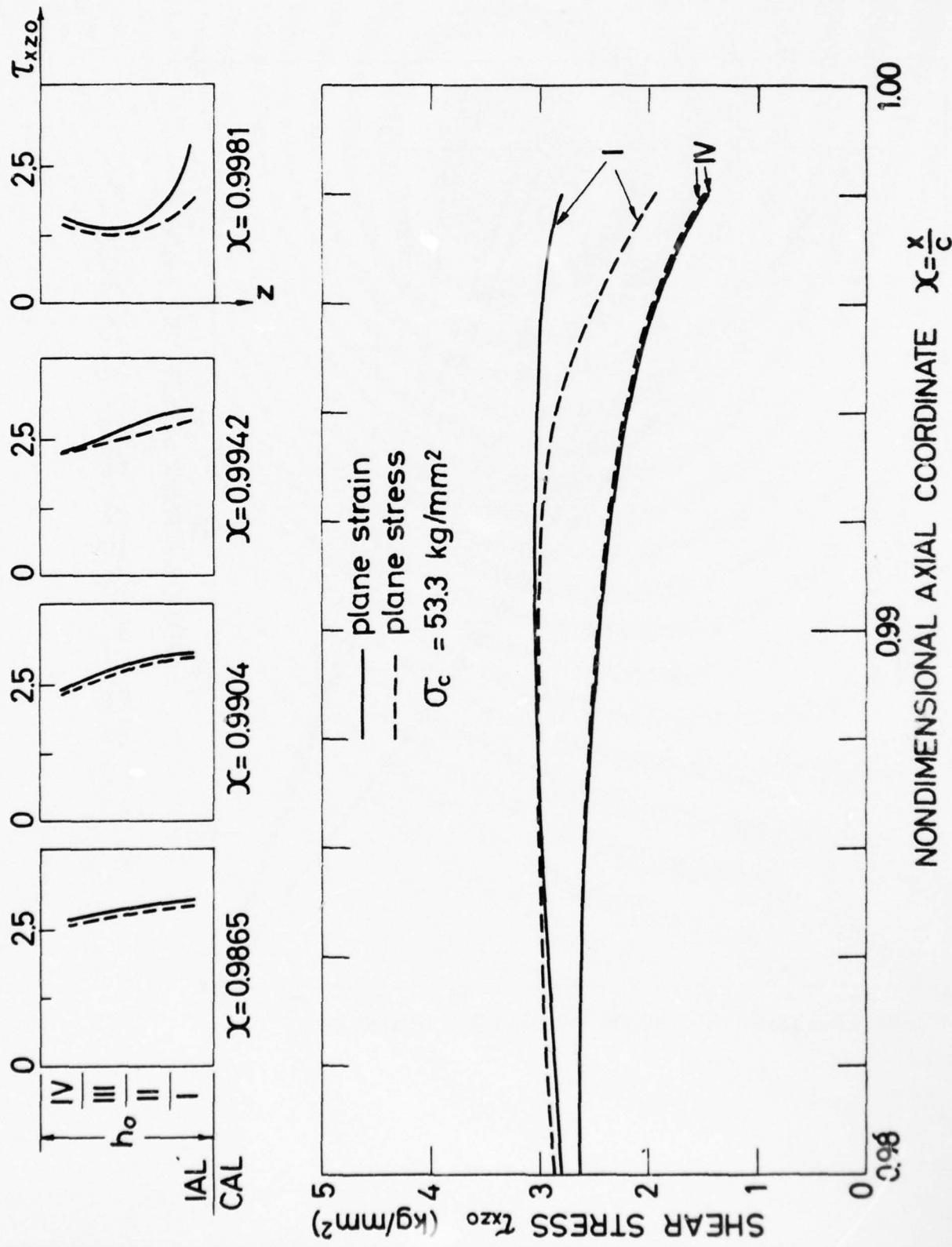


FIG. 27 SHEAR STRESS DISTRIBUTION AT THE BOUNDARY ZONE OF THE IAL
(NON-LINEAR RANGE).

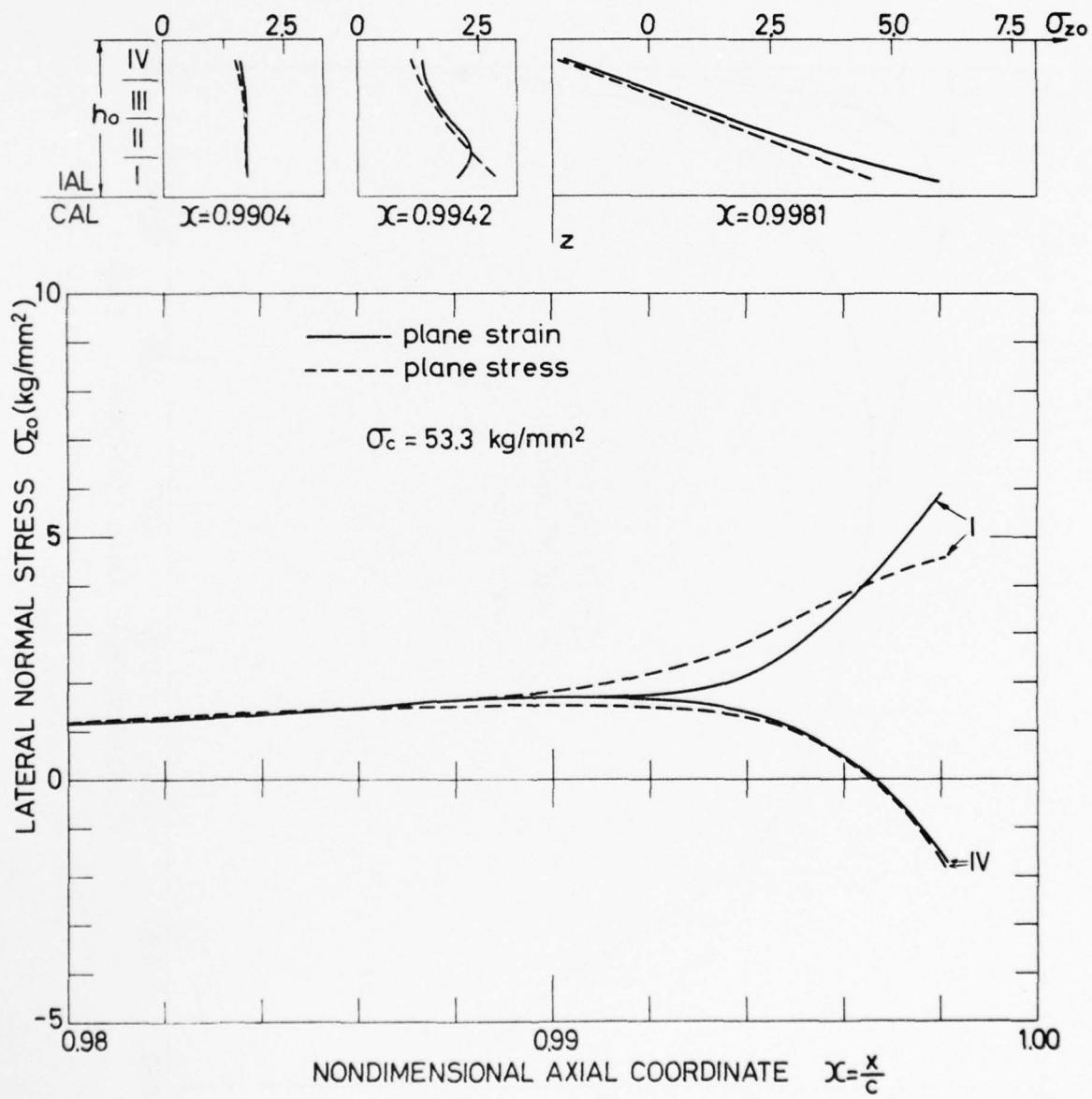


FIG. 28 LATERAL NORMAL STRESS DISTRIBUTION AT THE BOUNDARY ZONE OF THE IAL (NON-LINEAR RANGE).

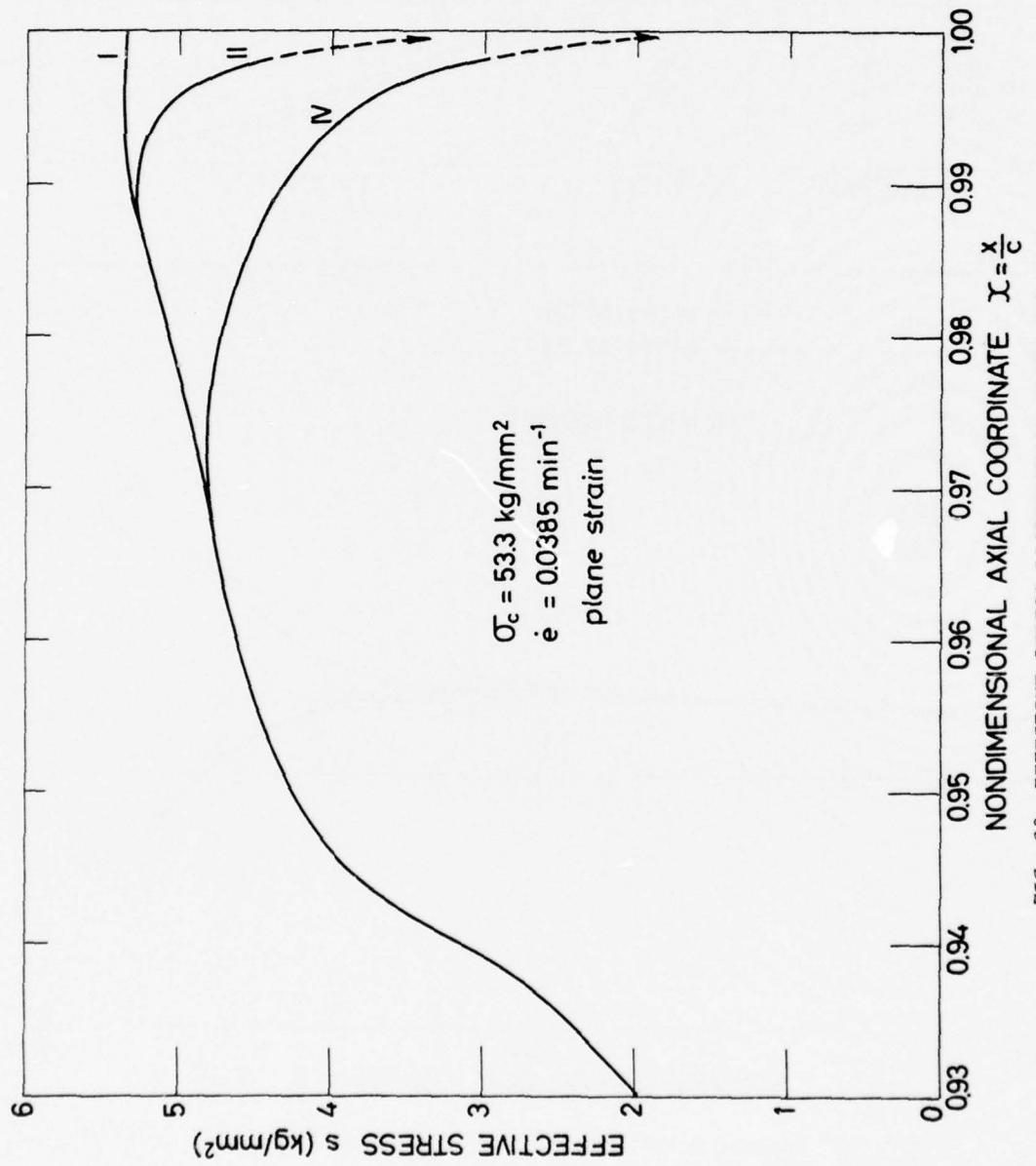


FIG. 29 EFFECTIVE STRESS DISTRIBUTION ALONG THE IAI
(NON-LINEAR RANGE).

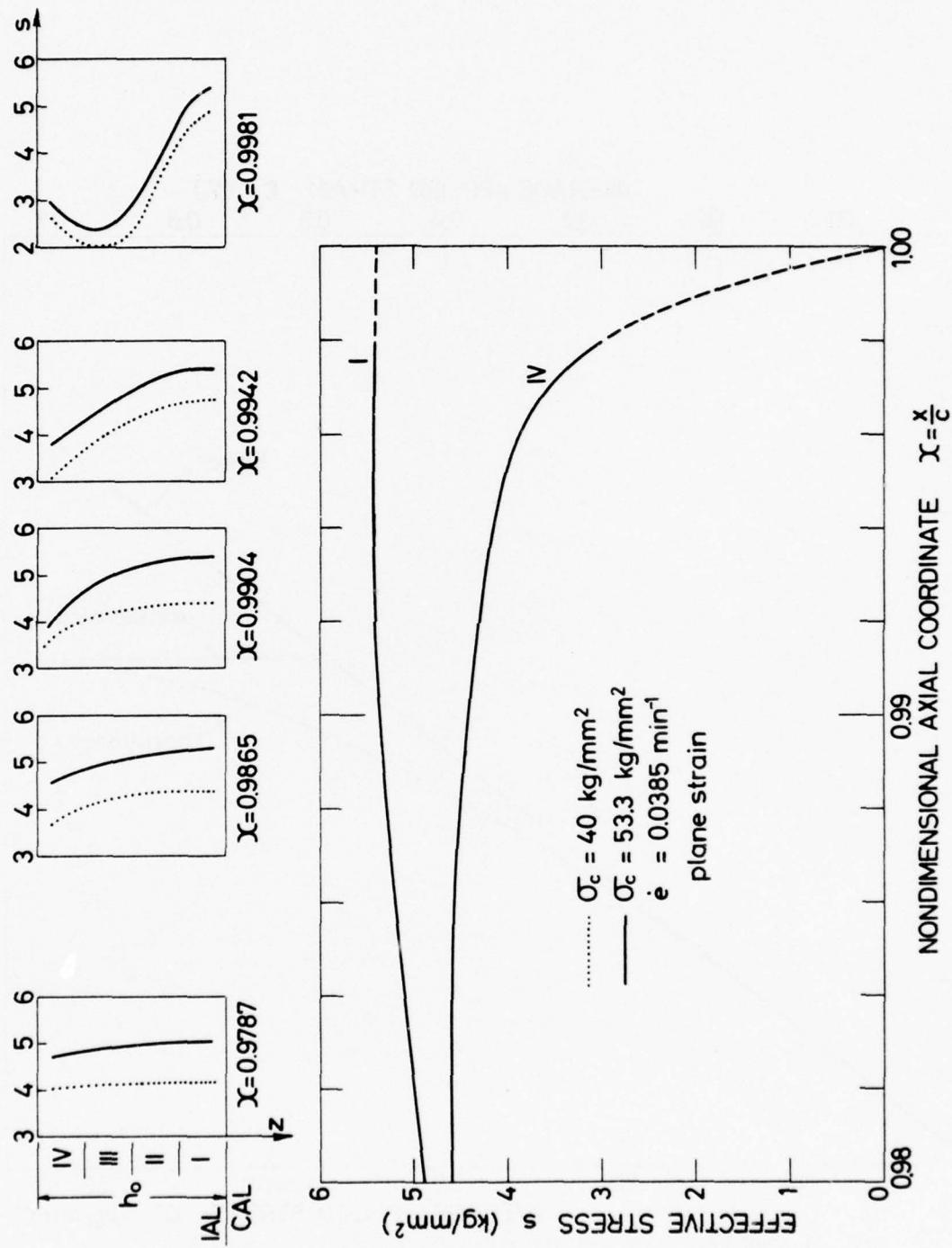


FIG. 30 EFFECTIVE STRESS DISTRIBUTION AT THE IAL BOUNDARY ZONE (NON-LINEAR RANGE).

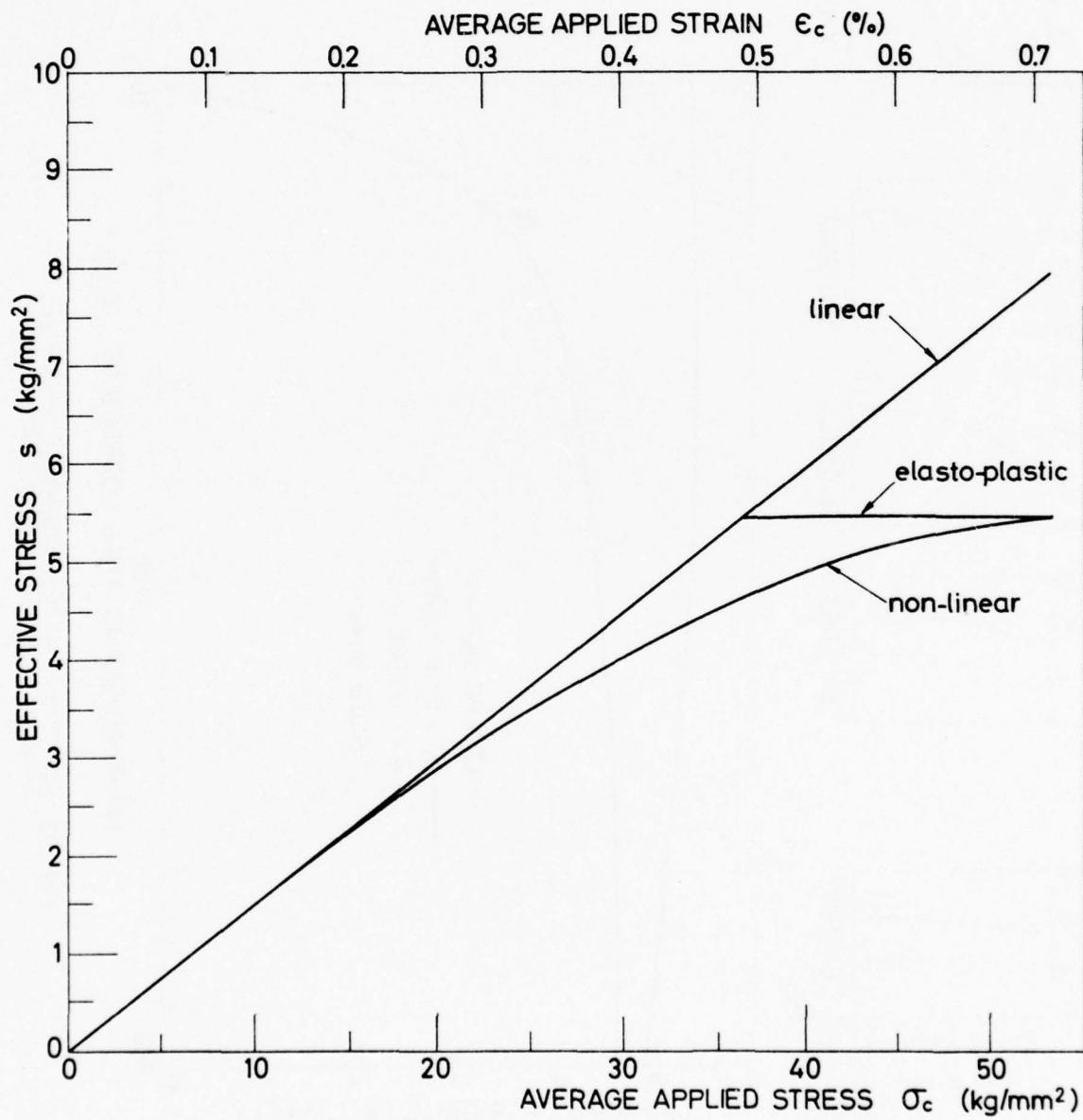


FIG. 31 THE EFFECT OF CENTRAL LOADING ON EFFECTIVE STRESSES AT THE CRITICAL POINT IN THE BOUNDARY ZONE OF THE IAL (PLANE STRAIN, SIMPLIFIED ELASTO-PLASTIC VS NON-LINEAR SOLUTIONS).

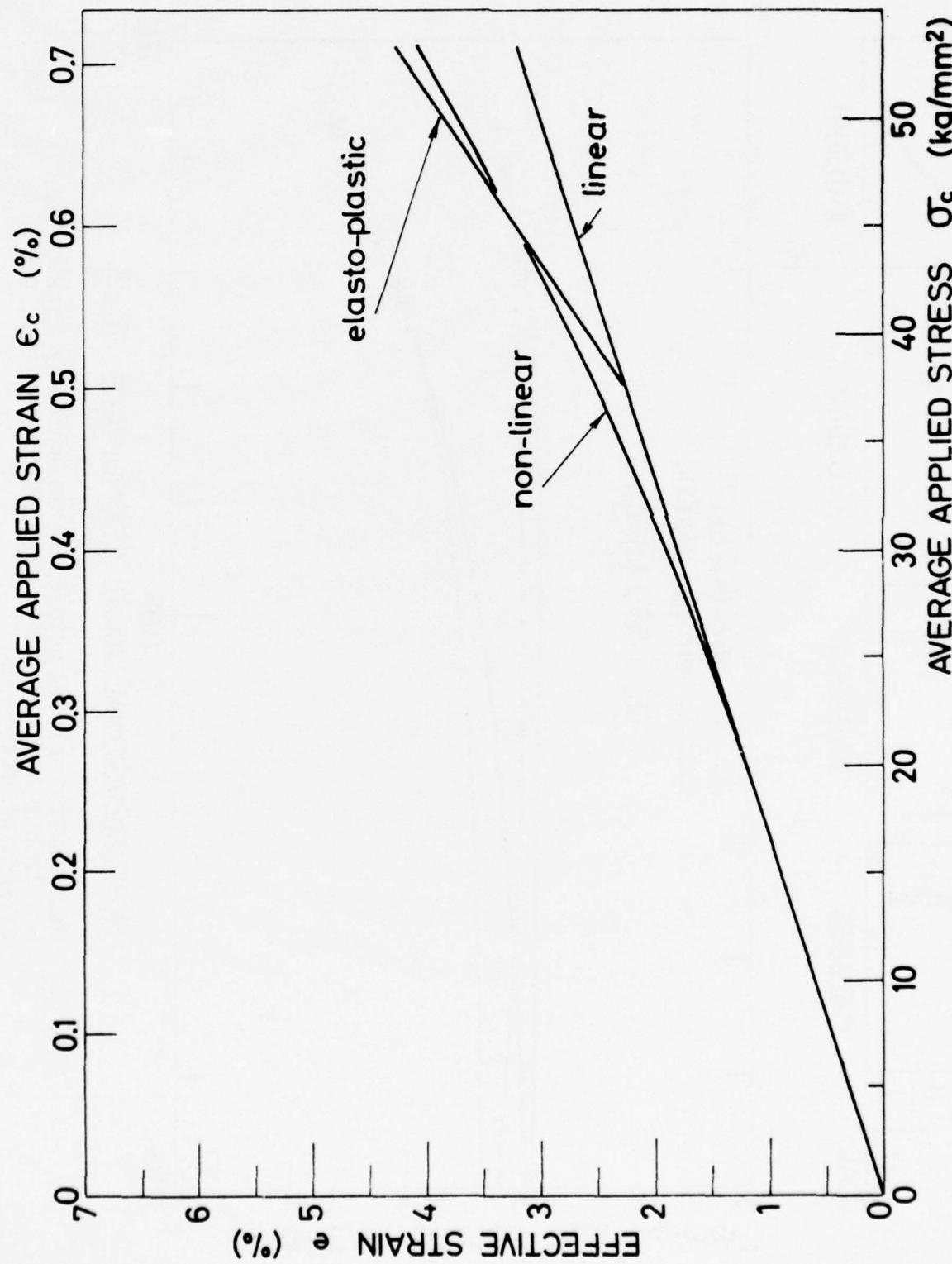


FIG. 32 THE EFFECT OF CENTRAL LOADING ON EFFECTIVE STRAINS AT THE CRITICAL POINT IN THE BOUNDARY ZONE OF THE IAL (PLANE STRAIN, SIMPLIFIED ELASTO-PLASTIC VS NON-LINEAR SOLUTIONS).

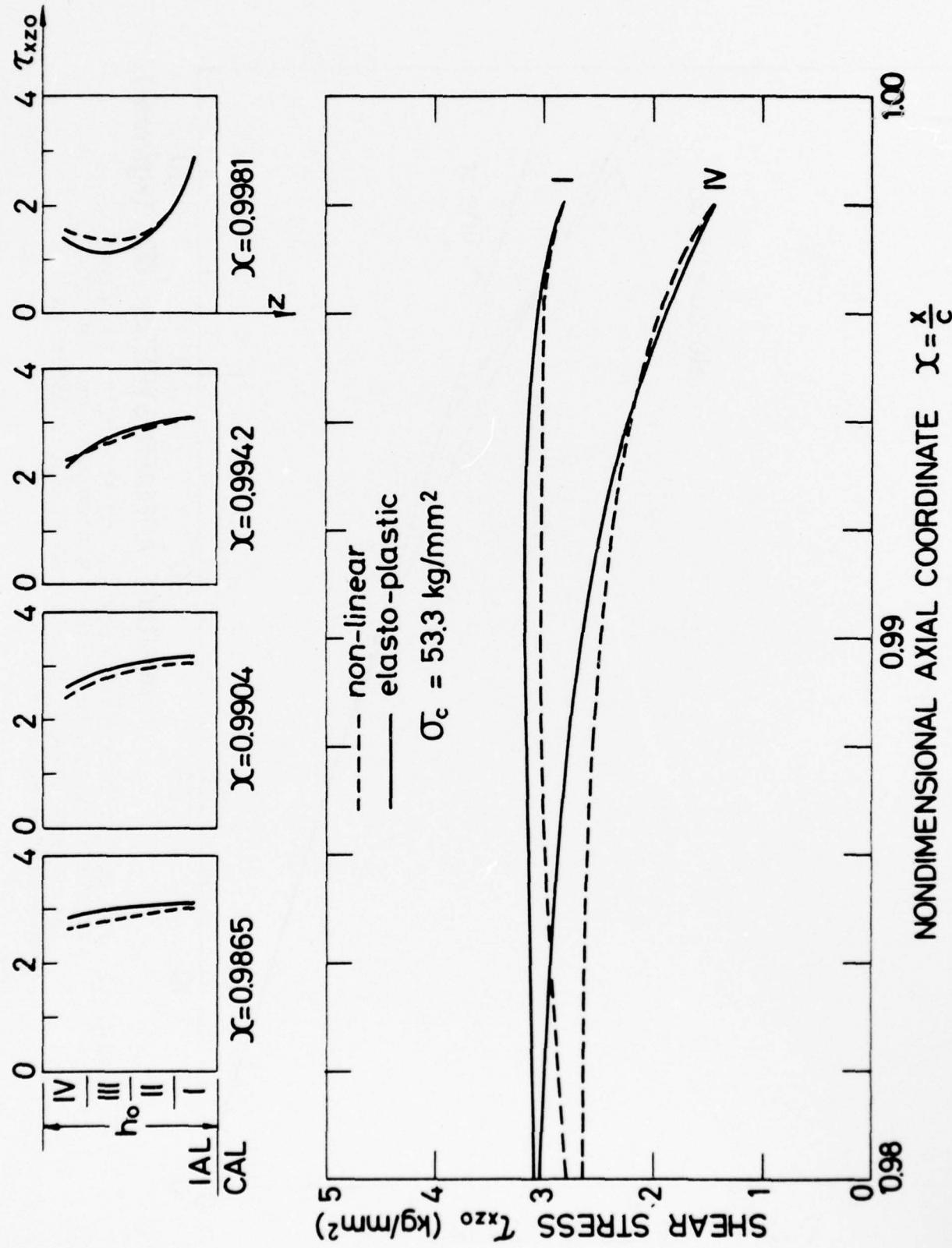


FIG. 33 SHEAR STRESS DISTRIBUTION AT THE BOUNDARY ZONE OF THE IAL (PLANE STRAIN, SIMPLIFIED ELASTO-PLASTIC VS NON-LINEAR SOLUTIONS).

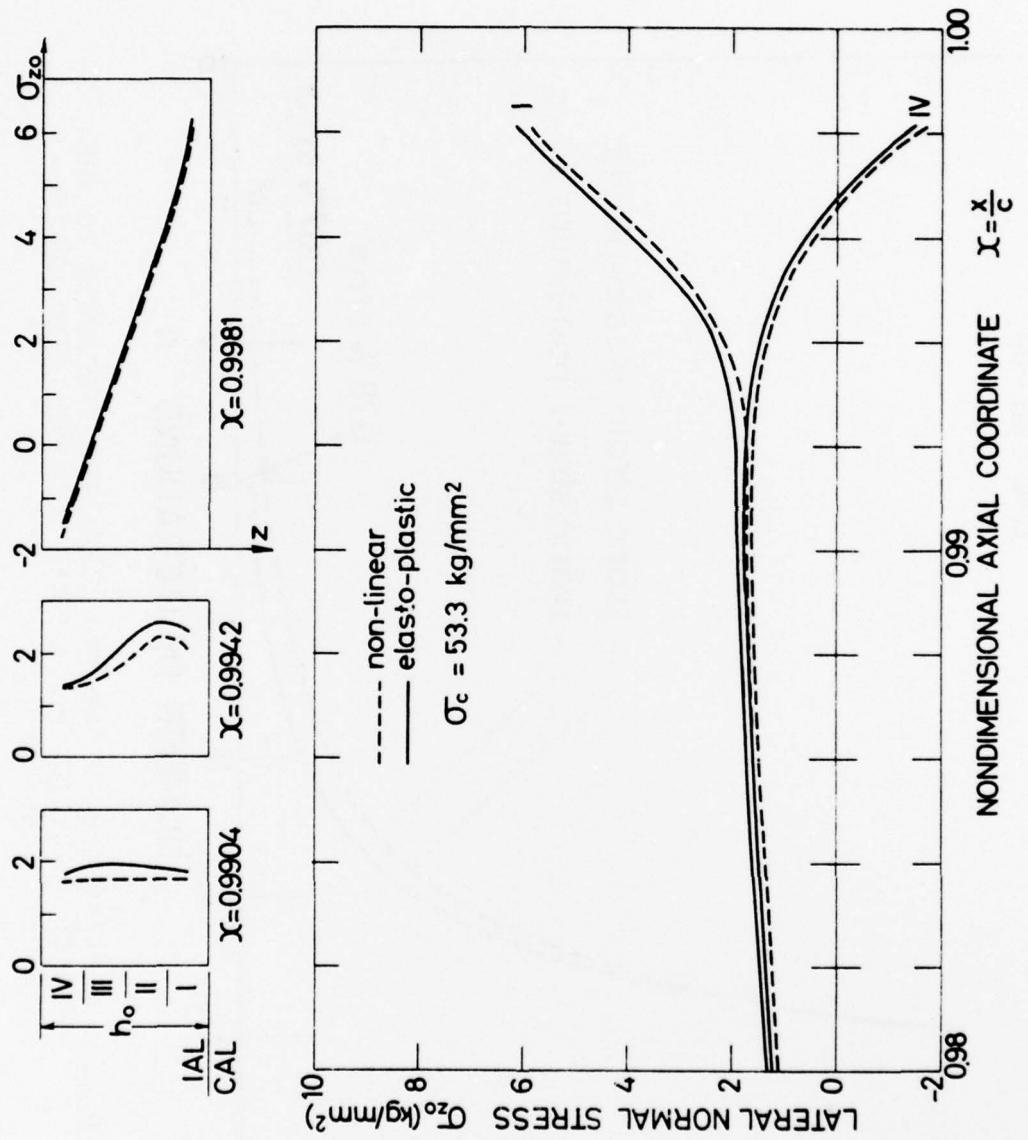


FIG. 34 LATERAL NORMAL STRESS DISTRIBUTION AT THE BOUNDARY ZONE
OF THE IAL (PLANE STRAIN, SIMPLIFIED ELASTO-PLASTIC
SOLUTION VS NON-LINEAR SOLUTIONS).

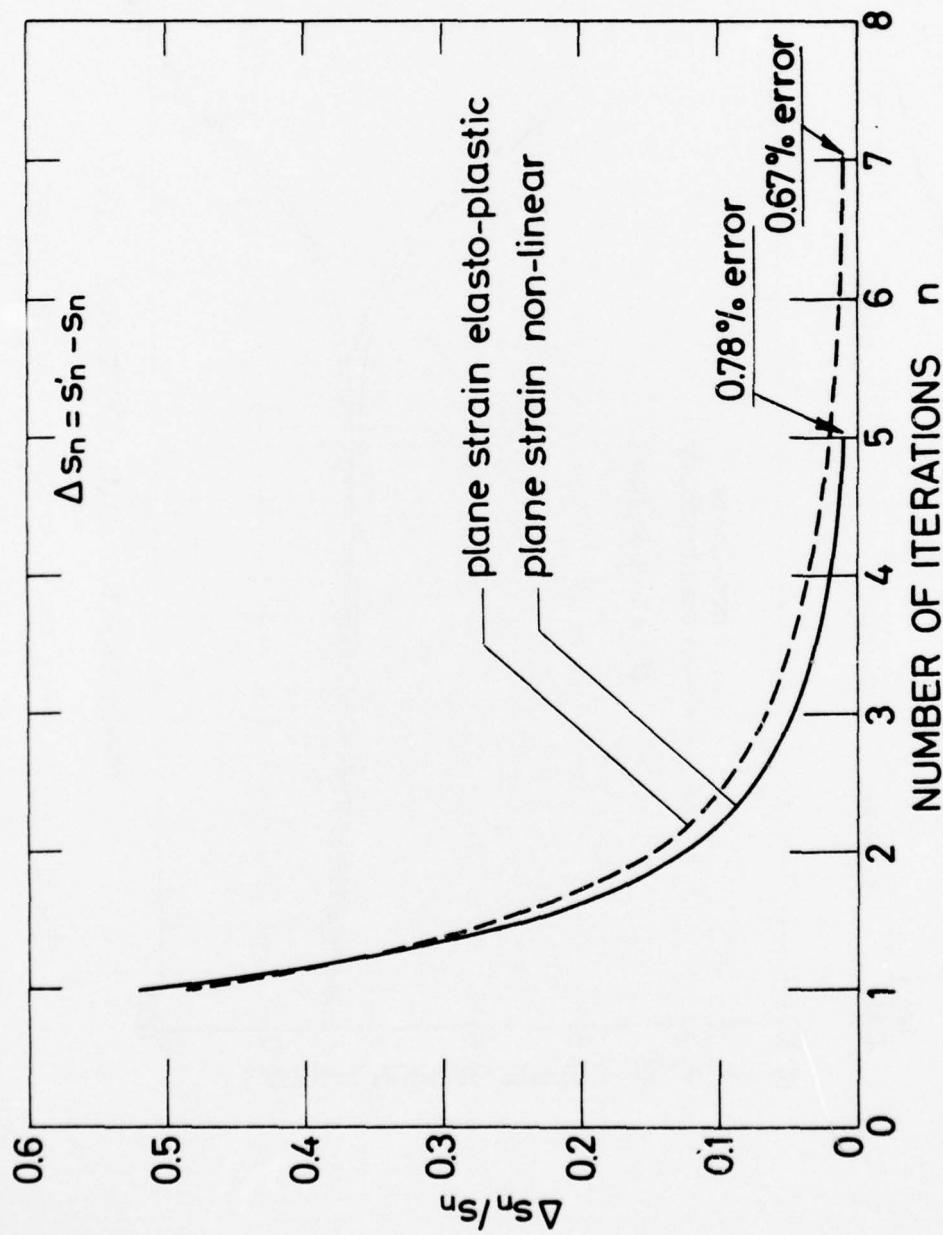


FIG. 35 CONVERGENCE OF THE FEM ITERATION PROCEDURE TO THE EXACT SOLUTION (NON-LINEAR VS SIMPLIFIED ELASTO-PLASTIC SOLUTIONS).

APPENDIX 1

PRINT-OUT OF TYPICAL FINITE ELEMENT PROGRAM FOR STRESS
DISTRIBUTION IN AN INTERLAMINAR ADHESIVE LAYER WITHIN
A DOUBLER MODEL.

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CS/360 FORTRAN H

PILER OPTIONS - NAME= MATN,CPT=02,LTNFCNT=60,SIZE=0000K,
SOURCE,FBFCITC,NOLIST,NOFECK,LCAD,NCMAP,NCFCIT,IP,NCXREF
C
C TWO DIMENSION PLAIN STRAIN/STRESS FINITE ELEMENT PROGRAM
C FCR GENERAL MATERIAL
C
REAL*8 TITLE(5)
COMMON /NNP,NFL,NMAT,NSLC,NCFT,NBODY,NTYP,IE(200,E),RO(10),TH(10),
1FT(10),UT(10),GGT(10),F1T(10),E2T(10),U12T(10),L21T(10),G12T(10),
2GC(3,3),OM(3,3,10),E,U,GG,E1,E2,U12,L21,G12,GOT(3,3,10),
3X(200),Y(200),ULX(200),VLY(200),KODE(200),JSC(20),JSC(20),
4SURTRX(20,2),SURTRY(20,2),EF(10)
COMMON /ONE/OK(10,10),G(10),B(3,10),C(3,3),PT(3,6),XG(5),YG(5)
COMMON /TWO/ IRANC,NEQ, R(400), AK(400,50)
DATA MAXEL, MAXNP, MAXMAT, MAXBW, MAXSLC
1 / 200, 200, 10, 50, 20/
9999 READ 100,NPROF, (TITLE(I),I=1,9)
IF(NPROF.LE.0) GO TO 999
1020 PRINT 200,NPRCB,(TITLE(I),I=1,9)
CALL DATTIN (MAXEL,MAXNP,MAXMAT,MAXSLC,TSTOP)
MAXDF = 2*MAXNP
MAXDIF = 0
DO 1 I=1,NEL
DO 1 J=1,4
DO 1 K=1,4
LL= IABS(IE(I,J)- IE(I,K))
IF(LL.GT.MAXDIF) MAXDIF = LL
1 CONTINUE
IRANE = 2*(MAXDIF + 1)
NEQ = 2*NNP
IF(IBAND.GT.MAXBW) GO TO 900
IF(ISTOP.GT.0) GO TO 999
CALL ASEML(ISTOP)
IF(ISTOP.GT.0) GO TO 999
CALL BANSCL(1,AK,R,NEQ,IRANE,MAXDF,MAXBW)
CALL BANSCL(2,AK,R,NEQ,IRANE,MAXDF,MAXBW)
PRINT 300, (T,R(2*I-1),R(2*I),I=1,NNP)
CALL STRESS
GO TO 9999
900 PRINT 301, TRAND, MAXBW
GO TO 9999
100 FORMAT(15,3X,9A8)
200 FORMAT(/8H1PROBLEM,15,3H.. ,9A8)
300 FORMAT(37H1OUTPUT TABLE 1.. NCAL DISPLACEMENTS //
1 13X,4HNDE, 9X, 11FL = X-DISP.,9X,11HV = Y-DISP./
2 (5X,I12,2E20.8))
901 FORMAT(//12H BANDWIDTH =,I4,25H EXCEEDS MAX ALLOWABLE =,I4//
1 30H GO ON TO NEXT PROBLEM)
999 STOP
END

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- 57 -

JAN 73)

CS/360 FORTRAN H

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:ILER CPTTCNS - NAME= MATN,CPT=02,LINECNT=60,SIZE=0000K,
      SOURCE,FRCFC,FOLST,NODECK,LOAD,NOMAP,NOEDIT,TD,NCXREF
      SUBROUTINE MATERP
      COMMON NNP,NFL,NMAT,NSLC,NCFT,NBODY,NTYP,TF(200,5),R0(10),TH(10),
     1ET(10),UT(10),GGT(10),E1T(10),E2T(10),U12T(10),U21T(10),C12T(10),
     2GG(3,3),QM(3,3,10),F,U,GG,E1,F2,U12,L21,G12,QGT(3,3,10),
     3X(200),Y(200),ULX(200),VLY(200),KODE(200),TSC(20),LSC(20),
     4SURTRY(20,2),SURTRY(20,2),EF(10)
      DO 250 IMAT=1,NMAT
      E=E+ET(IMAT)
      L=L+LT(IMAT)
      GG=GGT(IMAT)
      E1=E1T(IMAT)
      E2=E2T(IMAT)
      U12=U12T(IMAT)
      U21=U21T(IMAT)
      G12=G12T(IMAT)
      DO 350 I=1,3
      DO 350 J=1,3
      350 GG(I,J)=QGT(I,J,IMAT)
      IF ( E .EQ. 0. .AND. GG .EQ. 0. ) GO TO 500
      IF ( E .EQ. 0. ) GO TO 300
      IF ( U .EQ. 0. ) GO TO 200
      IF ( GG .EQ. 0. ) GG = E*0.5/(1.+U)
      GO TO 400
      200 IF ( GG .NE. 0. ) U = 1. - E*0.5/GG
      GO TO 400
      300 IF ( U .EQ. 0. ) GO TO 400
      E = 2.*(1.+U)*GG
      400 IF (NOPT.EQ.1)GO TO 450
      GG(1,1) = E/(1.-U*U)
      GG(2,1) = U*GG(1,1)
      GG(3,3) = GG
      GG(2,2) = GG(1,1)
      GO TO 1000
      450 GG(1,1)=E*(1-L)/((1+U)*(1-2*U))
      GG(2,1)=E*U/((1+U)*(1-2*U))
      GG(2,2)=GG(1,1)
      GG(3,3)=GG
      GO TO 1000
      500 IF ( E1 .EQ. 0. .AND. E2 .EQ. 0. ) GO TO 1000
      IF ( U12 .EQ. 0. ) GO TO 700
      IF ( L21 .EQ. 0. ) U21 = U12*E2/E1
      GO TO 800
      700 U12 = U21*E1/F2
      800 GG(1,1) = E1/(1.-U12*U21)
      GG(2,1) = U21*GG(1,1)
      GG(2,2) = E2/(1.-U12*U21)
      GG(3,3) = G12
      1000 DO 1200 I=1,3
      DO 1200 J=1,1
      GM(I,J,IMAT)=GO(I,J)
      GM(J,I,IMAT)=GO(I,J)
      1200 CONTINUE
      250 CONTINUE
      RETURN
      END
      00001000
      00002000
      00003000
      00004000
      00005000
      00006000
      00007000
      00008000
      00009000
      00010000
      00011000
      00012000
      00013000
      00014000
      00015000
      00016000
      00017000
      00018000
      00019000
      00020000
      00021000
      00022000
      00023000
      00024000
      00025000
      00026000
      00027000
      00028000
      00029000
      00030000
      00031000
      00032000
      00033000
      00034000
      00035000
      00036000
      00037000
      00038000
      00039000
      00040000
      00041000
      00042000
      00043000
      00044000
      00045000
      00046000
      00047000
      00048000
      00049000
      00050000
      00051000
      00052000
      00053000
      00054000
      00055000

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JAN 73)

CS/260 FORTRAN H

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FILER OPTIONS - NAME= MAIN,CPT=02,LTAECNT=60,SIZE=0000K,
      SOURCE,ERCCIC,LOLTST,NODECK,LCDR,NOMAP,NOEDIT,LD,NCXREF
      SUBROUTINE DATAIN(      MAXEL,MAXNP,MAXMAT,MAXSLC,ISTCP)      000C1000
      COMMON NNP,NFL,NMAT,NSLC,NCFT,NBODY,NTYP,IE(200,5),RO(10),TH(10),
      1ET(10),UT(10),GGT(10),E1T(10),E2T(10),U12T(10),U21T(10),G12T(10),
      2GG(3,3),QM(3,3,10),E,U,GG,E1,E2,U12,U21,G12,GQT(3,3,10),
      3X(200),Y(200),ULX(200),VLY(200),KODE(200),ISC(20),JSC(20),
      4SURTRX(20,2),SURTRY(20,2),EF(10)

C      ISTOP = 0
      READ 1,NNP,NFL,NMAT,NSLC,NCFT,NBODY
C      PRINT 100,NNP,NEL,NMAT,NSLC,NCPT,NBODY
      IF(NNP.LE.MAXNP) GO TO 201
      ISTOP = ISTCP + 1
      PRINT 251, MAXNP
201     IF(NEL.LE.MAXEL) GO TO 202
      ISTOP = ISTCP + 1
      PRINT 252, MAXEL
202     IF(NMAT.LE.MAXMAT) GO TO 203
      ISTOP = ISTCP + 1
      PRINT 253, MAXMAT
203     IF(NSLC.LE.MAXSLC) GO TO 204
      ISTOP = ISTCP + 1
      PRINT 254, MAXSLC
204     IF(ISTOP.EQ.0) GO TO 205
      PRINT 255, ISTOP
      STOP
C      205 READ 2, (RO(I),TH(I),I=1,NMAT)
      PRINT 101
      PRINT 51, (I,RO(I),TH(I),I=1,NMAT)
      READ 10,(ET(I),UT(I),GCT(I),E1T(I),E2T(I),U12T(I),U21T(I),
      1G12T(I),I=1,NMAT)
      PRINT 80, (I,ET(I),UT(I),GGT(I),E1T(I),E2T(I),U12T(I),U21T(I),
      1G12T(I),I=1,NMAT)
      DO 150 IMAT=1,NMAT
      DO 150 I=1,3
      READ 11, (GQT(I,J,IMAT),J=1,3)
      PRINT 81, (GQT(I,J,IMAT),J=1,3)
150    CONTINUE
      CALL MATERP
      DO 550 IMAT=1,NMAT
      DO 550 I=1,3
      PRINT 82, (QM(I,J,IMAT),J=1,3)
550    CONTINUE
      PRINT 103
      N=1
5     READ 3, M,KODE(M),X(M),Y(M),ULX(M),VLY(M)
      IF(M-N)4,6,7
4     PRINT 105, M
      PRINT 52,M,KODE(M), X(M),Y(M),ULX(M),VLY(M)
      ISTOP= ISTOP + 1
      GO TO 5
7     DF = M + 1 - N
      RX=(X(M)-X(N-1))/DF
      RY=(Y(M)-Y(N-1))/DF
      KODE(N)=0
      000C2000
      000C3000
      000C2500
      000C4000
      000C5000
      000C6000
      000C7000
      000C8000
      000C9000
      00010000
      00013000
      00014000
      00015000
      00016000
      00017000
      00018000
      00019000
      00020000
      00021000
      00022000
      00023000
      00024000
      00025000
      00026000
      00027000
      00028000
      00029000
      00031010
      00031020
      00031040
      00031050
      00031110
      00031120
      00031130
      00031140
      00031150
      00031500
      00031510
      00031520
      00031530
      00031540
      00034000
      00035000
      00036000
      00037000
      00038000
      00039000
      00040000
      00041000
      00042000
      00043000
      00044000
      00045000

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X(N)=X(N-1)+RX          0004E000
Y(N)=Y(N-1)+RY          00047000
ULX(N)=0.0               0004F000
VLY(N)=0.0               00045000
6 PRINT 52,N,KODE(N),X(N),Y(N),ULX(N),VLY(N)           0005C000
N=N+1                   00051000
IF(M-N)9,E,A             00052000
9   IF(N.LE.NNP) GO TO 5           00053000
      PRINT 106              0005E000
13      L=0                  00057000
14 READ 15, M,(TE(M,I),I=1,5)        0005F000
16      L=L+1                00059000
      IF(M-L)117,17,19          0006E000
117 PRINT 118,M              00061000
      PRINT 53,M, (IE(M,I), I=1,5)  00062000
      ISTOP=ISTOP+1            00063000
      GO TO 14                00064000
18      IE(L,1)= IF(L-1,1)+1       00065000
      IE(L,2)= IF(L-1,2)+1       00066000
      IE(L,3)=IE(L-1,3)+1       00067000
      IE(L,4)=IE(L-1,4)+1       00068000
      IE(L,5)=IE(L-1,5)         00069000
17 PRINT 53, L,(TE(L,I),I=1,5)       0007C000
      IF(M-L)20,20,16          00071000
20      IF(NEL-L)21,21,14          00072000
21      CONTINUE               0007Z000
      IF(NSLC.EQ.0) GO TO 31     0007E000
30 PRINT 108                 00077000
      DC 40 L=1,NSLC           00078000
      READ 41,ISC(L),JSC(L),SURTRX(L,1),SURTRX(L,2),SURTRY(L,1),
1 SURTRY(L,2)               0008E000
40 PRINT 42,ISC(L),JSC(L),SURTRY(L,1),SLRTRX(L,2),SURTRY(L,1),
1 SURTRY(L,2)               00081000
31      IF(ISTOP.EQ.0) GO TO 999  00082000
      PRINT 900, ISTOP          00084000
00085000
1 FORMAT(6I5)                0008E000
100 FORMAT(35H0INPUT TABLE 1.. BASIC PARAMETERS //      00087000
1      5X, 40H NUMBER OF NODEL POINTS. . . . . . . . . ,IE/ 00088000
2      5X, 40H NUMBER OF ELEMENTS. . . . . . . . . . ,IE/ 00089000
3      5X, 40H NUMBER OF DIFFERENT MATERIALS . . . . . . . ,IE/ 00090000
4      5X, 40H NUMBER OF SURFACE LOAD CARDS. . . . . . . ,IE/ 00091000
5      5X, 40H 1 = PLANE STRAIN, 2 = PLANE STRESS. . . . ,IE/ 00092000
6      5X, 40H BODY FORCES(1 = IN -Y DIREC., 0 = NCNE),IE) 00093000
251 FORMAT(//32H TOO MANY NODEL POINTS, MAXIMUM = ,IE) 00094000
252 FORMAT(//32H TOO MANY ELEMENTS, MAXIMUM = ,IE) 00095000
253 FORMAT(//32H TOO MANY MATERIALS, MAXIMUM = ,IE) 00096000
254 FORMAT(//40H TOO MANY SURFACE LOAD CARES, MAXIMUM = ,IE) 00097000
255 FORMAT(//28H EXECUTION HALTED BECAUSE OF,T5,13H FATAL ERRORS/) 00098000
2 FORMAT(2E10.3)             00099000
101 FORMAT(36H0INPUT TABLE 2.. MATERIAL PROPERTIES //    00100000
1      10H MATERIAL,5X,10HNUDELIUS OF,6X,9HPOISSON'S,7X, 001C1000
28HMATERIAL,7X, 8HMATERIAL /
34X,6HNUMBER,5X,10HELASTICITY,5X,7H RATIC,8X,7HDENSITY,6X, 001C2000
45HTHICKNESS )               001C3000
51 FORMAT(I10,2E15.4)          001C4000
10 FORMAT(BE10.3)              001C5000
80 FORMAT(T10,BE15.4)           001C6010
001CE020

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11 FORMAT(3E10.3)	001C5030
81 FORMAT(3E10.3)	001C5040
82 FORMAT(3E10.3)	001C5050
103 FORMAT(34H1INPUT TABLE 3.. NOCAL POINT DATA //	001C6000
1 5X,5HNCAL,4BX,7HX-CISF.,8X,7HY-DISP./	001C7000
25X,5HPOINT,6X,4HTYPE,14X,1HX,14X,1HY,8X,7HOP LOAD,8X,7HOP LOAD)	001C8000
3 FORMAT(2I5,4E10.3)	001C9000
105 FORMAT(5X,17HERROR IN CARD NO.,I5/)	00110000
52 FORMAT(2I10,4E15.4)	00111000
106 FORMAT(34H1INPUT TABLE 4.. ELEMENT DATA //	00112000
1 11X,31HGLOBAL INDICES OF ELEMENT NODES/2X,7HELEMENT,	00113000
27X,1H1,7X,1H2,7X,1H3,7X,1H4,2X,8HMATERIAL)	00114000
118 FORMAT(5X, 25HFRRCR IN ELEMENT CARD NO.,I5)	00115000
15 FORMAT(6I5)	00116000
53 FORMAT(T10,4T8,I10)	00117000
108 FORMAT(37H1INPUT TABLE 5.. SURFACE LOADING DATA //	00118000
117X, 33HSURFACE LOAD INTENSITIES AT NODES/	00119000
24X,6HNODE I,4X,6HNODE J,10X,2FXI,10X,2HXJ,10X,2HYI,10X,2HYJ)	00120000
41 FORMAT(2I5,4E10.3)	00121000
42 FORMAT(2I10,4E12.4)	00122000
900 FORMAT(//45H ASSEMBLY AND SOLUTION WILL NOT BE PERFORMED.,I5,	00123000
121H FATAL CARD ERRORS)	00124000
999 RETURN	00125000
END	00126000

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CS/360 FORTRAN H

PILER OPTIONS - NAME= MATN,CPT=02,LINFCNT=60,SIZE=000OCK,	
SOURCE,FBCDTC,NOLIST,NOECK,LOAD,NOMAP,NOEDIT,IO,NOXREF	
SUBROUTINE GFCMRC(L,N)	000C1000
COMMON/TWO/ IFAND,NFG,B(400), AK(400,50)	000C2000
C THIS SUBROUTINE MODIFIES THE ASSEMBLAGE STIFFNESS AND LOADS FOR THE	000C3000
C FRESCRIBED DISPLACEMENT U AT DEGREE OF FREEDOM N, EQ.(6-18B). (REF.1)	000C4000
DO 100 M=2,JBAND	000C5000
K = N - M + 1	000C6000
IF(K.LE.0) GO TO 50	000C7000
R(K) = R(K) - AK(K,M)*U	000C8000
AK(K,M) = 0.0	000C9000
50 K = N + M - 1	00010000
IF(K.GT.NFG) GO TO 100	00011000
R(K) = R(K) - AK(N,M)*U	00012000
AK(N,M) = 0.0	00013000
100 CONTINUE	00014000
AK(N,1) = 1.0	00015000
R(N) = U	00016000
RETURN	00017000
END	00018000

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CS/360 FORTRAN H

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ILER OPTIONS - NAME= MAIN,CPT=02,LINECNT=60,SIZE=0000K,
   SOURCE,FBCCDC,NOLST,NODECK,LOAD,NOMAP,NOEDIT,IO,NCXREF
   SUBROUTINE ASEMBL(ISTOP)                                     000C1000
   COMMON NNP,NFL,NMAT,NSLC,NCFT,NBODY,NTYP,IF(200,5),R0(10),TH(10),
   1ET(10),UT(10),GGT(10),E1T(10),E2T(10),U12T(10),U21T(10),G12T(10),
   2GG(3,3),QM(3,3+10),E,U,GG,E1+E2,U12,L21,G12,QGT(3,3+10),
   3X(200),Y(200),ULX(200),VLY(200),KODE(200),ISC(20),JSC(20),
   4SURTRX(20,2),SURTRY(20,2),EF(10)                         000C2000
   COMMON/ONE/ QK(10,10),Q(10),R(3,10),C(3,3),BT(3,6),XG(5),YG(5)
   COMMON/TWO/ IEAND,NEQ,R(400),AK(400,50)                     000C3000
   DIMENSION LP(8)                                         000C4000
C
   REWIND 1                                         000C5000
   TSTOPF = 0                                         000C6000
   BT(1,4) = 0.0                                       000C7000
   BT(1,5) = 0.0                                       000C8000
   BT(1,6) = 0.0                                       000C9000
   BT(2,1) = 0.0                                       00010000
   BT(2,2) = 0.0                                       00011000
   BT(2,3) = 0.0                                       00012000
   DO 2 I=1,NEQ                                       00013000
      R(I)=0.0                                         00014000
   DO 2 J =1,IBAND                                     00015000
      AK(I,J) = 0.0                                     00016000
   2 DO 10 M=1,NEL                                     00017000
      IF(IE(M,5).GT.0) GO TO 11
      ISTOF = ISTOP + 1                               00018000
      GO TO 10                                         00019000
   11 CALL QUAD(M,AREA)                                00020000
      IF(AREA.GT.0.0) GO TO 16
      ISTOF = ISTOP + 1                               00021000
   PRINT 20,M                                         00022000
   16 IF(IE(M,3).EQ.TE(M,4)) GO TO 26
      DO 31 J = 1+2                                     00023000
         IJ= 10-J                                      00024000
         IK= IJ+1                                      00025000
         PIVOT = GK(IK,IK)                            00026000
      DO 32 K= 1,IJ                                     00027000
         F = GK(IK,K)/PIVOT                           00028000
         * GK(IK,K)=F                                 00029000
      DO 33 T=K,IJ                                     00030000
         QK(T,K)=QK(T,K)- F*QK(T,IK)                00031000
   33         GK(K+1) = QK(T,K)                         00032000
   32         Q(K) = Q(K)-GK(IK,K)*G(IK)              00033000
   31         Q(IK) = Q(IK)/PIVOT                      00034000
   26 WRITE (1) ((OK(I,J),J=1,10),T=q,10), Q(9), Q(10),
      1((B(I,J),J=1+10)+I=1,3),((C(I,J),J=1+3)+I=1,3),XQ(5),YG(5)
      LIM=8                                         00035000
      IF(TE(M,3).EQ.TE(M,4)) LTM = 6
      DO 40 I=2,LIM,2                                00036000
         IJ = I/2                                      00037000
         LP(I-1) = 2*TE(M,IJ) - 1                   00038000
         LP(I) = 2*TE(M,IJ)                          00039000
   40 DO 50 LL=1,LTM                                  00040000
         I = LP(LL)                                    00041000
         R(I) = R(I) + Q(LL)                         00042000
   50 DO 50 MM=1,LIM                                00043000
         J = LP(MM) - I + 1                         00044000

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      IF (J.LE. 0 ) GO TO 50          00077000
      AK(T,J)= AK(T,J)+ GK(LL,MM)  00078000
50    CONTINUE                      00079000
10    CONTINUE                      00080000
DO 55 N=1,NNP                      00083000
      IF(KODE(N).EQ.3) GO TO 55     00084000
      K=2*N
      IF( KODE(N).EQ.1) GO TO 57     00085000
      R(K-1) = R(K-1) + ILX(N)      00086000
      IF(KODE(N).NE.0) GO TO 55     00087000
      R(K) = R(K) + VLY(N)         00088000
57    CONTINUEF                     00089000
      IF(NSLC.FG.0) GO TO 60         00090000
      DO 61 L = 1,NSLC             00091000
      I = ISC(L)                  00092000
      J = USC(L)                  00093000
      TI=2*I
      JJ=2*j
      DX = X(JJ) - X(I)           00100000
      DY = Y(JJ) - Y(I)           00101000
      FL = SQRT(DX*DX + DY*DY)   00102000
      PXT=SURTRX(L,1)*FL        00103000
      PXJ=SURTRY(L,1)*FL        00104000
      PYT=SURTRY(L,2)*FL        00105000
      PYJ=SURTRY(L,2)*FL        00106000
      R(TI-1) = R(TI-1) + FXI/3.0 + PXJ/6.0  00107000
      R(JJ-1) = R(JJ-1) + FXI/6.0 + PXJ/6.0  00108000
      R(TI)=R(TI)+ PYI/3.0 + PYJ/6.0  00109000
      R(JJ)=R(JJ) + PYI/6.0 + PYJ/3.0  00110000
      E1    CCONTINUF                   00111000
      E0    DC 70 M=1,NNP              00112000
      IF(KODE(M).GE.0.AND.KODE(N).LE.3) GO TO 72  00117000
      ISTOP = ISTOP + 1            00118000
      GO TO 70                      00119000
72    IF(KODE(M).EQ.0) GO TO 70     00120000
      IF(KODE(M).EQ.2) GO TO 71     00121000
      CALL GEOME(M,ULX(M),2*M-1)   00122000
      IF(KODE(M).EQ.1) GO TO 70     00123000
71    CALL GEOMFC(VLY(M),2*M)     00124000
70    CONTINUE                      00125000
      ENDFILE 1                    00126000
      IF(ISTOP.EQ.0) GO TO 81       00127000
      PRINT 100, ISTOP              00128000
20    FORMAT(1/5X,17H AREA OF ELEMENT ,15,14H IS NEGATIVE /)  00129000
100   FORMAT(1//42H SOLUTION WILL NOT BE PERFORMED BECAUSE OF ,15,  00130000
      1 15H DATA ERRORS          /)  00131000
      P1 RETURN                     00132000
      END                          00133000

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(IN 73)

CS/360 FORTRAN H

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    LER OPTIONS - NAME= MAIN,OPT=02,LINECNT=60,SIZE=0000K,
      SOURCE,FBCDTC,NOLIST,NOECK,LOAD,NOMAP,NOEDIT,ID,NCXREF
      SUBROUTINE QUAD(M,TOTALA)                               000C1000
      COMMON NNP,NFL,NMAT,NSLC,NCFT,NBODY,NTYP,IF(200,5),R0(10),TF(10), 000C2000
      1FT(10),UT(10),GGT(10),E1T(10),F2T(10),U12T(10),U21T(10),G12T(10), 000C3000
      2GG(3,3),QM(3,2,10),E,U,GG,E1,E2,U12,L21,G12,QQT(3,3,10), 000C3500
      3X(200),Y(200),ULX(200),VLY(200),KODE(200),ISC(20),JSC(20), 000C4000
      4SURTRX(20,2),EURTRY(20,2),EF(10) 000C5000
      COMMON/ONE/ QM(10,10),Q(10),P(3,10),C(3,3),PT(3,6),XQ(5),YG(5) 000C6000
      COMMON/TWO/ IEAND,NEG,R(400),AK(400,50) 000C7000
      000C8000

C
      I= IE(M,1) 000C9000
      J= IE(M,2) 00010000
      K= IE(M,3) 00011000
      L= IE(M,4) 00012000
      MTYP = IE(M,5) 00013000
      TOTALA = 0.0 00014000
      IF(INMAT.EQ.1.AND.M.GT.1) GO TO 5 00015000
      C(1,1)=QM(1,1,MTYP) 00020000
      C(1,2)=QM(1,2,MTYP) 00021000
      C(1,3)=QM(1,3,MTYP) 00022000
      C(2,1)=QM(2,1,MTYP) 00023000
      C(2,2)=QM(2,2,MTYP) 00024000
      C(2,3)=QM(2,3,MTYP) 00025000
      C(3,1)=QM(3,1,MTYP) 00026000
      C(3,2)=QM(3,2,MTYP) 00027000
      C(3,3)=QM(3,3,MTYP) 00028000
      5      LIM = 4 00029000
      IF(K.EQ.L) LIM = 3 00024000
      XQ(5) = 0.0 00025000
      YQ(5) = 0.0 00026000
      DO 10 N=1,LIM 00027000
      NN = IE(M,N) 00028000
      XQ(N) = X(NN) 00029000
      YQ(N) = Y(NN) 00030000
      XQ(5) = XQ(5) + X(NN)/FLOAT(LIM) 00041000
      10     YQ(5) = YG(5) + Y(NN)/FLOAT(LIM) 00042000
      DO 13 IT=1,10 00045000
      Q(TI)=0.0 00046000
      DO 12 JJ=1,10 00047000
      12     GK(II,JJ)=0.0 00048000
      DO 13 JJ=1,3 00049000
      13     B(JJ,II) = 0.0 00050000
      IF(K.NE.L) GO TO 15 00051000
      CALL CST(1,2,3,TOTALA) 00052000
      GO TO 999 00053000
      15 CALL CST(1,2,5,AREA) 00054000
      TOTALA = TOTALA + AREA 00055000
      CALL CST(2,3,5,AREA) 00056000
      TOTALA = TOTALA + AREA 00057000
      CALL CST(3,4,5,AREA) 00058000
      TOTALA = TOTALA + AREA 00059000
      CALL CST(4,1,5,AREA) 00060000
      TOTALA = TOTALA + AREA 00061000
      999 RETURN 00062000
      END 00063000

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IN 73)

CS/360 FORTRAN H

ILER OPTIONS - NAME= MAIN,OPT=02,LINECNT=60,SIZE=0000K,
SOURCE,ERCCTC,NOLSTT,NOECK,LOAD,NCMAP,NOECC,TD,NCXREF
SUBROUTINE CST(I,J,K,AREA)
COMMON NNP,NFL,NMAT,NSLC,NCFT,NRODY,MTYP,TF(200,5),RC(10),TH(10),
1ET(10),UT(10),GGT(10),E1T(10),E2T(10),U12T(10),U21T(10),C12T(10),
2GG(3,3),QM(3,3,10),E,U,GG,E1,E2,U12,L21,G12,QQT(3,3,10),
3X(200),Y(200),ULX(200),VLY(200),KODE(200),ISC(20),JSC(20),
4SURTRY(20,2),SURTRY(20,2),EP(10)
COMMON/ONE/ QK(10,10),G(10),R(3,10),C(3,3),PT(3,6),XQ(5),YC(5)
COMMON/TWO/ IPAND,NEQ,R(400),AK(400,50)
DIMENSION CR(3,6),LC(6),LT(3),TK(6,6)
C
LT(1)= I
LT(2)= J
LT(3)= K
BT(1,1)= YQ(J)-YC(K) 000C1000
BT(1,2)= YQ(K)-YC(I) 000C2000
BT(1,3)= YG(I)-YG(J) 000C3000
BT(2,4)= XG(K)-XQ(J) 000C4000
BT(2,5)= XG(I)-XG(K) 000C5000
BT(2,6)= XG(J)-XG(I) 000C6000
BT(3,1)= PT(2,4) 000C7000
BT(3,2)= RT(2,5) 000C8000
BT(3,3)= RT(2,6) 000C9000
BT(3,4)= RT(1,1) 000CA000
BT(3,5)= RT(1,2) 000CB000
BT(3,6)= RT(1,3) 000CC000
AREA = (BT(2,4)*BT(1,3) - BT(2,6)*BT(1,1))/2.0 000CD000
DO 10 IT=1,3 000CE000
DO 10 JJ = 1,6 000CF000
CR(II,JJ) = 0.0 000D000
DO 10 KK = 1,3 000D1000
10 CR(II,JJ) = CR(II,JJ) + C (II,KK)*RT(KK,JJ) 000D2000
DO 12 II = 1,6 000D3000
DO 12 JJ = 1,6 000D4000
TK(II,JJ)=0.0 000D5000
DO 12 KK=1,3 000D6000
12 TK(II,JJ)= TK(II,JJ)+RT(KK,IT)*CR(KK,JJ) 000D7000
DO 15 II=1,3 000D8000
15 LC(II) = 2*LT(II) - 1 000D9000
LC(II+3) = 2*LT(II) 000DA000
DO 30 IT=1,6 000DB000
30 LL = LC(II) 000DC000
FK = 1.0/(4.0*AREA) 000DD000
FR = 2.0*FK 000DE000
DO 20 JJ=1,6 000DF000
20 MM = LC(JJ) 000E000
MM = LC(JJ) 000E1000
QK(LL,MM) = QK(LL,MM) + TK(II,JJ)*TH(MTYP)*FK 000E2000
DO 30 JJ = 1,3 000E3000
30 B(JJ,LL) = B(JJ,LL) + RT(JJ,II)*FR 000E4000
IF(NBODY.EQ.0) GO TO 555 000E5000
TBCDYF = AREA* RC(MTYP)* TH(MTYP) 000E6000
RDYF = -TBCDYF/3.0 000E7000
DO 35 IT=1,3 000E8000
35 JJ= 2* LT(II) 000E9000
Q(JJ)= Q(JJ)+ RDYF 000EA000
999 RETURN 000EB000
ENC 000EC000

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AN 72)

CS/260 FORTRAN H

```

ILER OPTIONS - NAME= MAIN,CPT=02,LINECNT=60,SIZE=0000K,
    SOURCE,EBCCDIC,NOLIST,NOECK,LOAD,NOMAP,NOEDIT,IO,NCXREF
    SUBROUTINE STRESS
    COMMON NNP,NEL,NMAT,NSLC,NOPT,NBODY,NTYP,IE(200,5),R0(10),TR(10),
    1ET(10),UT(10),GGT(10),E1T(10),E2T(10),U12T(10),U21T(10),G12T(10),
    2QQ(3,3),QM(3,2,10),E,U,GG,E1,E2,U12,L21,G12,QQT(3,3,10),
    3X(200),Y(200),ULX(200),VLY(200),KODE(200),ISC(20),JSC(20),
    4SURTRX(20,2),SURTRY(20,2),EP(10)
    COMMON/ONE/ QK(10,10),Q(10),B(3,10),C(3,3),BT(3,6),XQ(5),YG(5)
    COMMON/TWO/ IEAND,NEG,R(400), AK(400,50)
    DIMENSION SIG(6)

C
    REWIND 1
    PRINT 300
        NOLINE = 47
        DO 5 M=1,NEL
        READ(1) ((QK(I,J),J=1,10),I=1,2), Q(9), Q(10),
    1 ((B(I,J),J=1,10),I=1,3), ((C(I,J),J=1,3),I=1,3), XC,YC
        LIM = 4
        IF(IE(M,3).EQ.IE(M,4)) LIM = 3
        DO 10 I=1,LIM
            II = 2*I
            JJ = 2*IE(M,I)
            Q(II+1) = R(JJ-1)
    10   Q(II) = R(JJ)
        IF(LIM.EQ.3) GO TO 16
        DO 15 K=1,2
            JK = K + 8
            IK = JK - 1
            DO 15 L=1,TK
                Q(JK) = Q(JK) - GK(K,L)*Q(L)
                LIM = 10
                FAC = 0.25
                GC TO 17
    15   LIM = 6
                FAC = 1.0
    16   DO 20 I=1,3
                EP(I)=0.0
                DO 20 J=1, LIM
                    EP(I)=EP(I)+B(I,J)*Q(J)*FAC
    20   DO 30 I=1,3
                    SIG(I) = 0.0
    30   DO 30 J=1,3
                    SIG(I)=SIG(I)+C(I,J)*EP(J)
                    SP = (SIG(1)+SIG(2))/2.0
                    SM = (SIG(1)-SIG(2))/2.0
                    DS = SQRT(SM*SM+SIG(3)*SIG(3))
                    SIG(4) = SP + DS
                    SIG(5) = SP - DS
                    SIG(6) = 0.0
                    IF(SIG(3).NE.0.0.AND.SM.NE.0.0) SIG(6) = 2*648*ATAN2(SIG(3),
    1                                         SM)
    1   IF(NOLINE.GT.0) GO TO 54
        PRINT 1000
            NOLINE = 49
            NOLINE = NOLINE - 1
    54   PRINT 1010, M,XC,YC,(SIG(I),I=1,6)
        ENDFILE 1

```

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```
300 FORMAT(47H10OUTPUT TABLE 2.. STRESSES AT ELEMENT CENTROIDS //  
11X,7HELEMENT,5X,1HX,9X,1HY,4X,BHSIGMA(X),4X,BHSIGMA(Y),4X,  
2BHTAU(X,Y),4X,BHSIGMA(1),4X,BHSIGMA(2), 7X,5HANGLE )  
000E5000  
00070000  
00071000  
1000 FORMAT(1H1, 7HELEMENT,9X,1HX,9X,1HY,4X,BHSIGMA(X),4X,BHSIGMA(Y),  
14X,BHTAU(X,Y),4X,BHSIGMA(1),4X,BHSIGMA(2), 7X,5HANGLE )  
00072000  
00073000  
1010 FORMAT(1B, 2F10.2,1P6E12.4)  
00074000  
RETURN  
00075000  
END  
00076000
```

JAN 72)

CS/360 FORTRAN H

```
PILER OPTIONS - NAME= MAIN,CPT=02,LINECNT=60,SIZE=00000K,  
SOURCE,FBCIC,NOLTST, NODECK,LOAD,NCMAP,NOFCIT,IO,NCXREF  
SUBROUTINE BANSOL(KKK,AK,R,NEQ,IBAND,MDIM)  
C SYMMETRIC BAND MATRIX EQUATION SOLVER. (REF. 2)  
C  
C KKK = 1 TRIANGULARIZES THE BAND MATRIX AK. EG. (2-2)  
C KKK = 2 SOLVES FOR RIGHT HAND SIDE R. SOLUTION RETURNS IN R. EG.(2-3)  
C  
DIMENSION AK(MDIM,MDIM), R(1)  
NRS = NEQ - 1  
NR = NEQ  
IF(KKK.EQ.2) GO TO 200  
DO 120 N= 1,NRS  
M= N-1  
MR = MIN0(IPAND,NR-M)  
PIVOT = AK(N+1)  
DO 120 L=2,MR  
CP= AK(N,L)/PIVOT  
I = M+L  
J = C  
DO 110 K=L,MR  
J = J + 1  
110 AK(I,J)= AK(I,J) - CP*AK(N,K)  
120 AK(N,L) = CP  
GO TO 400  
200 DO 220 N=1,NRS  
M= N-1  
MR = MIN0(IPAND,NR-M)  
CP= R(N)  
R(N)=CP/AK(N,1)  
DO 220 I=2,MR  
I = M + L  
220 R(I)= R(I) - AK(N,L)*CP  
R(NR) = R(NR)/AK(NR,1)  
DO 320 I = 1,NRS  
N= NR- I  
M= N-1  
MR = MIN0(IPAND,NR-M)  
DO 320 K = 2,MR  
L = M+K  
C STORE COMPUTED DISPLACEMENTS IN LOAD VECTOR R  
320 R(N)= R(N)- AK(N,K)*R(L)  
400 RETURN  
END
```