

DEPARTMENT OF THE NAVY

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IN REPLY REFER TO:

Attorney Docket No. 84432 Date: 22 November 2004

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Serial Number 10/779,554

Filing Date 9 February 2004

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20041221 226

Attorney Docket No. 84432 Customer No. 23523

A METHOD FOR ESTIMATING THE PROPERTIES OF A SOLID MATERIAL SUBJECTED TO COMPRESSIONAL FORCES

TO ALL WHOM IT MAY CONCERN:

BE IT KNOWN THAT ANDREW J. HULL, citizen of the United States of America, employee of the United States Government and resident Newport, County of Newport, State of Rhode Island, has invented certain new and useful improvements entitles as set forth above of which the following is a specification:

JAMES M. KASISCHKE ESQ. Reg. No. 36562 Naval Undersea Warfare Center Division Newport Newport, RI 02841-1708 TEL: 401-832-4736 FAX: 401-832-1231

I hereby certify that this correspondence is being deposited with the U.S. Postal Service as U.S. EXPRESS MAIL, Mailing Label No. EV326644788US In envelope addressed to: Commissioner for Patents, Alexandria, VA 20231 on <u>9 Feb</u> 2004

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	1	ATTORNEY DOCKET NO. 84432
	2	
	3	A METHOD FOR ESTIMATING THE PROPERTIES OF A
•	4	SOLID MATERIAL SUBJECTED TO COMPRESSIONAL FORCES
	5	
	6	STATEMENT OF GOVERNMENT INTEREST
	7	The invention described herein may be manufactured and used
	8	by or for the Government of the United States of America for
	9	governmental purposes without the payment of any roughting
	10	thereon or therefore.
	11	
	12	BACKGROUND OF THE INVENTION
	13	(1) Field of the Invention
	14	The present invention relater to a method i
	15	and present invention relates to a method to measure (or
	15	estimate) the complex frequency-dependent dilatational and shear
	16	wavenumbers of a single slab of material subjected to large
	17	static compressional forces. More particularly, this invention
	18	provides a method to determine complex dilatational wavespeed,
	19	complex shear wavespeed, complex Lamé constants, complex Young's
	20	modulus, complex shear modulus, and complex Poisson's ratio.
	21	(2) Description of the Prior Art
	22	Measuring the mechanical properties of slab-shaped materials
	23	are important because these parameters significantly contribute
	24	to the static and dynamic response of structures built with such
	25	materials. One characteristic that most elastomeric solids
÷ .		

poissess is that, when they are subjected to large static forces (or pressure), their rigidity changes. Materials that have one set of mechanical properties at a pressure of one atmosphere can have very different properties when subjected to increased pressure. The ability to determine the pressure dependence of material properties is extremely important for modeling the behavior of systems comprised of these materials.

Resonant techniques have been used to identify and measure 8 longitudinal and shear properties for many years. These methods 9 are based on comparing measured eigenvalues to modeled 10 eigenvalues and calculating the resulting material properties. 11 These methods do not account for static pressure or large 12 compressive forces. Additionally, they typically require long, 13 slender materials to perform the measurement process. Comparison 14 of analytical models to measured frequency response functions are 15 also used to estimate stiffness and loss parameters of a 16 structure. When the analytical model agrees with one or more 17 frequency response functions, the parameters used to calculate 18 the analytical model are considered accurate. If the analytical 19 model is formulated using a numerical method, a comparison of the 20 model to the data can be difficult due to dispersion properties 21 22 of the materials. These methods do not take into account large 23 compressive forces.

In the prior art, some efforts have been made to measure
material properties under large pressures. These methods consist

of placing materials in pressurized settings, insonifying them, ŀ and then measuring their response. These methods are difficult 2 because they have to be conducted under great atmospheric 3 pressure that can adversely affect the instrumentation. Safety 4 issues can also arise in connection with laboratory testing at 5° extreme pressures. Finally, a mass loaded long thin rod has been 6 studied with respect to the bar wavespeed and corresponding 7 Young's modulus; however, this work does not investigate shear 8 9 motion. Accordingly, there is a need for a method of measuring 10 mechanical properties of slab-shaped materials placed under 11 12 pressure. 13 14 SUMMARY OF THE INVENTION Accordingly, in this invention, a method to measure the 15 complex frequency-dependent dilatational and shear wavenumbers of 16 17 a material under a static compressional force is provided. The 18 material is first vibrated in both vertical and horizontal directions while obtaining transfer functions in each direction. 19 The two transfer functions are combined with a theoretical model 20 to estimate a dilatational wavenumber and a shear wavenumber. 21 The wavenumbers can be combined to give the complex dilatational 22 wavespeed, complex shear wavespeed, complex Lamé constants, 23 complex Young's modulus, complex shear modulus, and complex 24 25 Poisson's ratio.

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BRIEF DESCRIPTION OF THE DRAWINGS

2	A more complete understanding of the invention and many of
3	the attendant advantages thereto will be readily appreciated as
4	the same becomes better understood by reference to the following
5	detailed description when considered in conjunction with the
6	accompanying drawings wherein:
7	FIG. 1 shows apparatus for measurement of transfer functions
8	in a vertical direction according to the current invention;
9	FIG. 2 shows apparatus for measurement of transfer functions
10	in a horizontal direction according to the current invention;
11	FIG. 3 is a diagram of the coordinate system of used with a
12	test specimen in the model;
13	FIG. 4A is a plot of the transfer function magnitude versus
14	input frequency for the vertical direction test;
15	FIG. 4B is a plot of the transfer function phase angle
16	versus input frequency for the vertical direction test;
17	FIG. 5A is a plot of the transfer function magnitude versus
18	input frequency for the horizontal direction test;
19	FIG. 5B is a plot of the transfer function phase angle
20	versus input frequency for the horizontal direction test;
21	FIG. 6 is a contour plot of the absolute value of the
22	dilatational wavenumber on an real-imaginary coordinate system of
23	the dilatational wavenumbers at 2000 Hz;

1	FIG. 7 is a contour plot of the absolute value of the
2.	dilatational wavenumber on an real-imaginary coordinate system of
3	the dilatational wavenumbers at 5000 Hz;
4	FIG. 8A is a plot of the real dilatational wavenumber versus
5	frequency;
6	FIG. 8B is a plot of the imaginary dilatational wavenumber
7	versus frequency; and
8	FIG. 9A is a plot of the real shear wavenumber versus
9	frequency;
10	FIG. 9B is a plot of the imaginary shear wavenumber versus
11	frequency;
12	FIG. 10 is a plot of the real and imaginary Young's modulus
13	versus frequency;
14	FIG. 11 is a plot of the real and imaginary shear modulus
15	versus frequency; and
16	FIG. 12 is a plot of the Poisson's ratio versus frequency.
17	
18	DESCRIPTION OF THE PREFERRED EMBODIMENT
19	The test procedure consists of vibrating a mass-loaded,
20	slab-shaped test specimen 10 with a shaker 12 in two different
21	directions, vertical 14A and horizontal 14B, as shown in FIGS. 1
22	and 2, respectively. It is noted that the load mass 16 attached
23	to the top of the test specimen 10 must be sufficiently stiffer
24	than the specimen 10 that it can be modeled as lumped parameter
25	expression rather than a continuous media system. A typical

5.

example would be a steel load mass 16 attached above a rubber-1. like material test specimen 10. This example results in a ratio 2 between the two stiffnesses of greater than 100. Lower ratios 3 result in less accurate estimations. Vibrating the combined 4 specimen 10 and load mass 16 causes different waveforms to 5 propagate in the specimen 10. The inverse method developed here 6 allows for the data from the experiments to be manipulated so 7 that the complex dilatational and shear wavenumbers can be 8 measured for the specimen 10. This test is usually done at 9 multiple frequencies (swept sine) so any frequency dependencies 10 can be identified and measured. Input vibration data is 11 collected from the shaker 12. A sensor 18 is mounted on load 12 mass 16 and another sensor 20 is mounted on shaker 12 for 13 collecting transfer function data. In FIG. 1, the test is set up 14 for monitoring the vertical transfer function. FIG. 2 shows the 15 test as set up for monitoring the horizontal transfer function. 16 17 Sensors 18 and 20 should be oriented properly to capture the motion being measured. Other test configurations using 18 directions other than vertical and horizontal are possible; 19 however, the test setups shown are preferred for ease of set up 20 21 and calculation. These sensors 18 can be either accelerometers that record accelerations, or laser velocimeters that record 22 23 velocities. In the swept sine mode, transfer functions of acceleration divided by acceleration or velocity divided by 24 velocity are both equal to displacement divided by displacement. 25

The time domain data collected from the sensors 18 and 20 are ŀ Fourier transformed into the frequency domain and then recorded 2 3 as complex transfer functions, typically using a spectrum 4 analyzer 22. The motion of the test specimen shown in FIGS. 1 and 2 is 5 6 governed by the equation: 7 $\mu \nabla^2 \mathbf{u} + (\lambda + \mu) \nabla \nabla \bullet \mathbf{u} = \rho \frac{\partial^2 \mathbf{u}}{\partial t^2} ,$ 8 (1)9 where λ and μ are the complex Lamé constants (N/m²); 10 ρ is the density (kg/m^3) ; 11 12 t is time (s); 13 • denotes a vector dot product; and 14 ${f u}$ is the Cartesian coordinate displacement vector of the 15 material. The coordinate system of the test configuration is shown in 16 FIG. 3. Note that using this orientation results in b = 0 and a 17 having a value less than zero. The thickness of the specimen, h, 18 is a positive value. Equation (1) is manipulated by writing the 19 20 displacement vector **u** as 21 $\mathbf{u} = \begin{cases} u_x(x,y,z,t) \\ u_y(x,y,z,t) \\ u_{-}(x,y,z,t) \end{cases} ,$ 22 (2)

1 where x is the location along the plate (m), y is the location 2 into the plate (m), and z is the location normal to the plate 3 (m), as shown in FIG. 3. The symbol ∇ is the gradient vector 4 differential operator written in three-dimensional Cartesian 5 coordinates as

6

7

8

 $\nabla = \frac{\partial}{\partial x} i_x + \frac{\partial}{\partial y} i_y + \frac{\partial}{\partial z} i_z \quad ,$

9 with i_x denoting the unit vector in the x-direction, i_y denoting 10 the unit vector in the y-direction, and i_z denoting the unit 11 vector in the z-direction; ∇^2 is the three-dimensional Laplace 12 operator operating on vector **u** as

(3)

(4)

(6)

13

14

 $\nabla^2 \mathbf{u} = \nabla^2 u_x i_x + \nabla^2 u_v i_v + \nabla^2 u_z i_z \quad \prime$

15

16 and operating on scalar u as

17

18
$$\nabla^2 u_{x,y,z} = \nabla \bullet \nabla u_{x,y,z} = \frac{\partial^2 u_{x,y,z}}{\partial x^2} + \frac{\partial^2 u_{x,y,z}}{\partial y^2} + \frac{\partial^2 u_{x,y,z}}{\partial z^2} \quad ; \qquad (5)$$

19

20 and the term $\nabla \bullet \mathbf{u}$ is called the divergence and is equal to 21

 $\nabla \bullet \mathbf{u} = \frac{\partial u_x}{\partial x} + \frac{\partial u_y}{\partial y} + \frac{\partial u_z}{\partial z} \quad .$ 22

2 The displacement vector \mathbf{u} is written as 3 $\mathbf{u} = \nabla \phi + \nabla \times \vec{\psi} \quad ,$ 4 (7) 5 where ϕ is a dilatational scalar potential, \times denotes a vector 6 cross product, and $ec{\psi}$ is an equivoluminal vector potential 7 8 expressed as 9 $\vec{\psi} = \begin{cases} \psi_x(x,y,z,t) \\ \psi_y(x,y,z,t) \\ \psi_z(x,y,z,t) \end{cases}$ 10 (8) 11 The problem is formulated as a two-dimensional system, thus $y \equiv 0$, 12 $u_v(x,y,z,t) \equiv 0$, and $\partial(\cdot)/\partial y \equiv 0$. Expanding equation (7) and breaking 13 the displacement vector into its individual nonzero terms yields 14

9

15

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16
$$u_x(x,z,t) = \frac{\partial \phi(x,z,t)}{\partial x} - \frac{\partial \psi_y(x,z,t)}{\partial z}$$

17

18 and

19

20

$$u_{z}(x,z,t) = \frac{\partial \phi(x,z,t)}{\partial z} + \frac{\partial \psi_{y}(x,z,t)}{\partial x}$$

÷

(10)

(9)

Equations (9) and (10) are next inserted into equation (1), which results in

 $c_d^2 \nabla^2 \phi(x,z,t) = \frac{\partial^2 \phi(x,z,t)}{\partial t^2}$

- 6 and
- 7

8

9

3

4

5

 $c_s^2 \nabla^2 \psi_y(x,z,t) = \frac{\partial^2 \psi_y(x,z,t)}{\partial^2}$

10 where equation (11) corresponds to the dilatational component and 11 equation (12) corresponds to the shear component of the 12 displacement field. Correspondingly, the constants c_d and c_s are 13 the complex dilatational and shear wave speeds, respectively, and 14 are determined by

15

16

 $c_d = \sqrt{\frac{\lambda + 2\mu}{\rho}}$

 $c_s = \sqrt{\frac{\mu}{\rho}}$.

- 17
- 18 and
- 19

20

(13)

(14)

(11)

(12)

ŀ	The relationship of the Lamé constants to the Young's and shear
2	moduli is shown as
3	
4	$\lambda = \frac{E\upsilon}{(1+\upsilon)(1-2\upsilon)} \tag{15}$
5	
6	and
7	
8	$\mu = G = \frac{E}{2(1+\nu)} , \qquad (16)$
9	
10	where E is the complex Young/a medulus () $V(x^2)$
	, G is the complex loung's modulus (N/m), G is the complex
11	shear modulus (N/m ²), and v is the Poisson's ratio of the
12	material (dimensionless).
13	The conditions of infinite length and steady-state response
14	are now imposed, allowing the scalar and vector potential to be
15	written as
16	
17	$\phi(x,z,t) = \Phi(z) \exp(ikx) \exp(i\omega t) $ (17)
18	
19	and
20	
21	$\Psi_{y}(x,z,t) = \Psi(z) \exp(ikx) \exp(i\omega t) $
	(18)

1 where i is the square root of -1, ω is frequency (rad/s), and k2 is wavenumber with respect to the x axis (rad/m). Inserting 3 equation (17) into equation (11) yields

 $\frac{d^2\Phi(z)}{dz^2} + \alpha^2\Phi(z) = 0 \quad ,$

 $\alpha = \sqrt{k_d^2 - k^2} \quad ,$

 $k_d = \frac{\omega}{c_d} \quad .$

7 where

8

4

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9

10

11

and

12 13

14

16

17

15 Inserting equation (18) into equation (12) yields

 $\frac{d^2\Psi(z)}{dz^2}+\beta^2\Psi(z)=0 \quad ,$

18

19 where

20

21

 $eta = \sqrt{k_s^2 - k^2}$,

(22)

(23)

(19)

(20)

(21)

and
ι. Φ
$\kappa_s = \frac{1}{c_s} . \tag{24}$
The solution to equation (10) is
/ 19/ 18
$\Phi(z) = A(k,\omega) \exp(i\alpha z) + B(k,\omega) \exp(-i\alpha z) , \qquad (25)$
and the solution to equation (22) is
$\Psi(z) = C(k,\omega) \exp(i\beta z) + D(k,\omega) \exp(-i\beta z)$
(26) $(1, 2) (1$
where $A(k,\omega)$, $B(k,\omega)$, $C(k,\omega)$, and $D(k,\omega)$ are wave response
coefficients that are determined below. The displacements can
now be written as functions of the unknown constants using the
expressions in equations (9) and (10). They are
$u(r, \tau, t) - U(r, \tau, c) = u(r, c)$
$u_Z(x,2,t) = O_Z(k,2,\omega) \exp(ikx) \exp(i\omega t)$
$= \{ i\alpha [A(k,\omega)\exp(i\alpha z) - B(k,\omega)\exp(-i\alpha z)] + $ (27)
$ik[C(k,\omega)\exp(i\beta z) + D(k,\omega)\exp(-i\beta z)]$ exp(iar) exp(iar)

 $u_x(x,z,t) = U_x(k,z,\omega) \exp(ikx) \exp(i\omega t)$

= { $ik[A(k,\omega)\exp(i\alpha z) + B(k,\omega)\exp(-i\alpha z)] - i\beta[C(k,\omega)\exp(i\beta z) - D(k,\omega)\exp(-i\beta z)]$ } $\exp(i\alpha z) \exp(i\omega z)$.

(28)

· (29)

3 Specific boundary conditions are now needed to individually solve 4 for the case of vertical and horizontal motion. These are 5 formulated separately below.

For the case of vertical motion, the base at z = a is
vibrated vertically using a shaker, as shown in FIG. 1. Four
boundary conditions are necessary to formulate this problem.
Because the mass is attached to the material, the tangential
(horizontal) motion at the top of the plate (z = b) is zero and
this equation is written as

12

2

13

 $u_x(x,b,t)=0$

14

15 The normal stress at the top of the specimen is equal to and 16 opposite the load created by the mass in the z direction. This 17 expression is

18

 $\tau_{zz}(x,b,t) = (\lambda + 2\mu)\frac{\partial u_z(x,b,t)}{\partial z} + \lambda \frac{\partial u_x(x,b,t)}{\partial x} = -M \frac{\partial^2 u_z(x,b,t)}{\partial x^2}$ (30)

1	where M is mass per unit area (kg/m 2) of the attached m	ass. The
2	tangential motion at the bottom of the plate $(z = a)$ is	s zero and
3	this equation is written as	
4		
5	$u_x(x,a,t)=0 , $	(31)
6	4	
7	and the normal motion at the bottom of the plate is pre	escribed as
8	a system input. This expression is	
9		
10	$u_z(x,a,t) = U_0 \exp(i\omega t)$.	(32)
11		
12	Assembling equations $(1) = (32)$ and letting b	
12	$(1) = (32) \text{ and } \text{fetting } \mathbf{D} = 0$	yields the
12	four-by-four system of linear equations that model the	yields the system.
13	four-by-four system of linear equations that model the They are	yields the system.
12 13 14 15	four-by-four system of linear equations that model the They are	yields the system.
12 13 14 15 16	four-by-four system of linear equations that model the They are $\mathbf{A}\mathbf{x}=\mathbf{b} \ ,$	yields the system. (33)
12 13 14 15 16 17	four-by-four system of linear equations that model the They are $\mathbf{A}\mathbf{x}=\mathbf{b} \ ,$	yields the system. (33)
12 13 14 15 16 17 18	four-by-four system of linear equations that model the They are $\mathbf{A}\mathbf{x} = \mathbf{b} \ ,$ where the entries of equation (33) are	yields the system. (33)
12 13 14 15 16 17 18 19	four-by-four system of linear equations that model the They are Ax = b , where the entries of equation (33) are	yields the system. (33)
12 13 14 15 16 17 18 19 20	four-by-four system of linear equations that model the They are ${\bf A}{\bf x}={\bf b}\ ,$ where the entries of equation (33) are $A_{11}={\bf i}k\ ,$	yields the system. (33) (34)
12 13 14 15 16 17 18 19 20 21	four-by-four system of linear equations that model the They are Ax = b , where the entries of equation (33) are $A_{11} = ik$,	yields the system. (33) (34)
12 13 14 15 16 17 18 19 20 21 21 22	four-by-four system of linear equations that model the They are $A\mathbf{x} = \mathbf{b} ,$ where the entries of equation (33) are $A_{11} = ik ,$	yields the system. (33) (34) (35)
12 13 14 15 16 17 18 19 20 21 22 23	four-by-four system of linear equations that model the They are $A\mathbf{x} = \mathbf{b} ,$ where the entries of equation (33) are $A_{11} = ik ,$ $A_{12} = A_{11} ,$	yields the system. (33) (34) (35)
12 13 14 15 16 17 18 19 20 21 20 21 22 23 24	four-by-four system of linear equations that model the They are $A\mathbf{x} = \mathbf{b} \ ,$ where the entries of equation (33) are $A_{11} = ik \ ,$ $A_{12} = A_{11} \ ,$ $A_{13} = -i\beta \ ,$	yields the system. (33) (34) (35) (36)

1.	$A_{14} = -A_{13} , \tag{3}$	37)
2		
3	$A_{21} = -\alpha^2 \lambda - 2\alpha^2 \mu - \lambda k^2 - iM\omega^2 \alpha , \tag{3}$	88)
4		
5	$A_{22} = -\alpha^2 \lambda - 2\alpha^2 \mu - \lambda k^2 + iM\omega^2 \alpha , \tag{3}$	39)·
6		
7	$A_{23} = -2k\beta\mu - iM\omega^2k , \tag{4}$	Ł0)
8		
9	$A_{24} = 2k\beta\mu - iM\omega^2 k , \tag{4}$	1)
10		
11	$A_{31} = A_{11} \exp(i\alpha a)$, (4)	2)
12		
13	$A_{32} = A_{11} \exp(-i\alpha a)$, (4	3)
14		
15	$A_{33} = A_{13} \exp(i\beta a)$, (4)	4)
16		·
17	$A_{34} = -A_{13} \exp(-i\beta a)$, (4)	5)
18		
19	$A_{41} = i\alpha \exp(i\alpha a) , \tag{4}$	6)
20		
21	$A_{42} = -i\alpha \exp(-i\alpha a) , \qquad (4)$	7)
22		
23	$A_{43} = ik \exp(i\beta a) , \tag{4}$	8)

1	$A_{44} = ik \exp(-i\beta a) ,$	(49)
2		
3	$x_{11} = A(k,\omega) , \qquad$	(50)
4		
5	$x_{21} = B(k,\omega) , \qquad$	(51)
6		
7	$x_{31} = C(k,\omega) , \qquad$	(52)
8		· ·
9	$x_{41} = D(k,\omega) , \qquad$	(53)
10		
11	$b_{11} = 0$,	(54)
12		
13	$b_{21} = 0$,	(55)
14		
15	$b_{31} = 0$,	(56)
16	and	
17		
18	$b_{41} = U_0$.	(57)
19		
20 ·	Using equations (34) - (57) the solution to the constants	$A(k,\omega)$,
21	$B(k,\omega)$, $C(k,\omega)$, and $D(k,\omega)$ can be calculated at each specific	1
22	wavenumber and frequency using	
23		

 $\mathbf{x} = \mathbf{A}^{-1}\mathbf{b}$

(58)

- Noting that for vertical motion, k = 0, and using the
 coefficients from equation (58), the transfer function between
 the vertical base displacement and the vertical mass displacement
 can be written as
- 5

 $T_{1}(\omega) = \frac{1}{R_{1}(\omega)} = \frac{U_{z}(0,b,\omega)}{U_{0}} = \frac{1}{\cos(k_{d}h) - \left(\frac{M}{\rho}\right)k_{d}\sin(k_{d}h)}$ (59)

8 where $T_1(\omega)$ or $R_1(\omega)$ correspond to the data from the vertical 9 motion experiment.

10 The next step is to solve the inverse problem for vertical 11 motion. This involves using the experimental data and equation 12 (59) to estimate the dilatational wavenumber. Equation (59) can 13 be rewritten as

14

15

 $f(k_d) = 0 = \cos(k_d h) - \left(\frac{M}{\rho}\right) k_d \sin(k_d h) - R_1$ (60)

16

17 where the problem now becomes finding the zeros of the right-hand 18 side of equation (60), or, in the presence of actual data that 19 contains noise, finding the relative minima of the right-hand 20 side of equation (60) and determining which relative minimum 21 corresponds to dilatational wave propagation and which relative 22 minima are extraneous. Because equation (60) has a number of 23 relative minima, zero finding algorithms are not applied to this

function, as they typically do not find all of the minima 1 locations and are highly dependent on initial starting locations. 2 The best method to find all of the minima locations is by 3 plotting the absolute value of the right-hand side of equation 4 (60) as a surface with the real part of dilatational wavenumber 5 k_d on one axis and the imaginary part of k_d on the other axis. 6 In order to do this, the user should start at a low frequency 7 where the aliasing minimum has not yet appeared. In the specific 8 example shown herein, this is below 3850 Hz for the dilatational 9 10 wave and below 1550 Hz for the shear wave. At these lower frequencies, the minimum furthest to the left will correspond to 11 dilatational wave propagation. As the frequency increases, 12 extraneous minima will appear to the left of the minimum that 13 14 corresponds to dilatational wave propagation, however, the wave propagation minimum will always be close to the previous test 15 frequency wave propagation minimum provided that the frequency 16 increments are relatively small. At a resolution of 0.5 rad/m 17 for the materials in the example herein, this requires a 18 frequency increment of 37.3 Hz for the dilatational measurement 19 and 14.4 Hz for the shear measurement. Different test specimens 20 and top masses require different increments. Additionally, the 21 real part of the wavenumber is a monotonically increasing 22 23 function with respect to frequency, so at each increase in frequency, the new wavenumber to be estimated has to be greater 24 than the old wavenumber that was previously estimated. 25 This

process is further illustrated as related to the discussion
 concerning FIG. 6 and FIG. 7 below.

For the case of horizontal motion, the base at z = a is vibrated horizontally using a shaker, as shown in FIG. 2. Four boundary conditions are necessary to formulate this problem. Because the mass is attached to the material, the shear (tangential) stress at the top of the plate is equal to opposite the load created by the mass in the x direction. This expression is

10

11
$$\tau_{zx}(x,b,t) = \mu \left[\frac{\partial u_x(x,b,t)}{\partial z} + \frac{\partial u_z(x,b,t)}{\partial x} \right] = -M \frac{\partial^2 u_x(x,b,t)}{\partial t^2} , \qquad (61)$$

12

13 where M is mass per unit area (kg/m^2) of the attached mass. The 14 normal motion at the top of the plate (z = b) is zero and this 15 equation is written as

16

17

18

(62)

19 The tangential motion at the bottom of the plate (z = a) is 20 prescribed as a system input and this equation is written as

21

22

 $u_x(x,a,t) = V_0 \exp(i\omega t)$,

 $u_z(x,b,t)=0$.

(63)

1	and the normal motion at the bottom of the plate is zero.	This
2	expression is	
3		· •.
4	$u_z(x,a,t)=0$	(64)
5		
6	Assembling equations (1) - (28) and (62) - (64) and	letting
7	b = 0 yields the four-by-four system of linear equations t	chat
8	model the system. They are	
9		
10	Ax = b ,	(65)
11.		•
12	where the entries of equation (61) are	
13		
14	$A_{11} = -2\mu k\alpha - i\omega^2 M k ,$	(66)
15		•
16	$A_{12} = 2\mu k\alpha - i\omega^2 M k ,$	(67)
17		
18	$A_{13} = \mu \beta^2 - \mu k^2 + i \omega^2 M \beta ,$	(68)
19		
20	$A_{1A} = \mu \beta^2 - \mu k^2 - i \omega^2 M \beta .$	(69)
21		(0))
22	$A_{21} = i\alpha$,	(70)
23		
24	$A_{22} = -A_{21}$,	(71)
		÷.,

1		•
2	$A_{23} = \mathrm{i}k , \qquad$	(72)
3		
4	$A_{24} = A_{23}$,	(73)
5		
6	$A_{31} = A_{23} \exp(i\alpha a) ,$	(74)
7		
8	$A_{32} = A_{23} \exp(-i\alpha a)$,	(75)
9		
10	$A_{33} = -i\beta \exp(i\beta a)$,	(76)
11		. ·
12	$A_{34} = i\beta \exp(-i\beta a)$,	(77)
13		
14	$A_{41} = A_{21} \exp(i\alpha a) , \qquad \qquad$	(78)
15		
16	$A_{42} = -A_{21} \exp(-i\alpha a)$,	(79)
17		а 1 1
18	$A_{43} = A_{23} \exp(i\beta a) ,$	(80)
19		•
20	$A_{44} = A_{23} \exp(-i\beta a)$,	(81)
21		
22		
23	$x_{11} = A(k, \omega) , \qquad$	(82)
24		
25	$x_{21} = B(k, \omega) ,$	(83)
•	22	

۰. ۱		
1		
2	$x_{31} = C(k,\omega)$, (84)	
- 3		
4	$x_{41} = D(k,\omega)$, (85)	
5		
6		
7	$b_{11} = 0$, (86)	
8		
9	$b_{21} = 0$. (97)	
10		
11	b = V	
10	$v_{31} = v_0$, (88)	
12	and	
13		
14	$b_{41} = 0$ (89)	
15		
16	Using equations (67) - (89) the solution to the constants $A(k,\omega)$,	
17	$B(k,\omega)$, $C(k,\omega)$, and $D(k,\omega)$ can be calculated at each specific	
18	wavenumber and frequency using	
19		
20	$\mathbf{x} = \mathbf{A}^{-1}\mathbf{b} . \tag{90}$	
21		
22	Noting that for horizontal metion is a start of	
22	Recting that for norizontal motion, $\kappa = 0$, and using the	
25	coefficients from equation (90), the transfer function between	
24	the horizontal base displacement and the horizontal mass	,
25	displacement can be written as	

$$T_{2}(\omega) = \frac{1}{R_{2}(\omega)} = \frac{U_{x}(0,b,\omega)}{V_{0}} = \frac{1}{\cos(k_{s}h) - \left(\frac{M}{\rho}\right)k_{s}\sin(k_{s}h)}$$

(91)

(93)

3 where $T_2(\omega)$ or $R_2(\omega)$ correspond to the data from the horizontal 4 motion experiment.

5 The next step is to solve the inverse problem for horizontal 6 motion. This involves using the data and equation (91) to 7 estimate the shear wavenumber. Equation (91) can be rewritten as 8

$$f(k_s) = 0 = \cos(k_s h) - \left(\frac{M}{\rho}\right) k_s \sin(k_s h) - R_2 \quad . \tag{92}$$

10

9

2

It is noted that this equation is identical, except for the subscripts, to equation (60). The shear wavenumber is estimated using the same procedure that was used to estimate the dilatational wavenumber above.

15 The material properties can be determined from the 16 wavenumbers. First, the dilatational and shear wavespeeds are 17 determined using

18

19

 $c_d = \frac{\omega}{k_d}$

20

21 and

1
$$c_s = \frac{\omega}{k_s}$$
, (94)
2
3 respectively. The Lamé constants are calculated using equations
4 (13) and (14) written as
5
6 $\mu = \rho c_s^2$ (95)
7
8 and
9
10 $\lambda = \rho c_s^2 - 2\rho c_s^2$. (96)
11 Poisson's ratio is then calculated using
12
13 $\nu = \frac{\lambda}{2(\mu + \lambda)}$. (97)
14
15 Young's modulus can be calculated with
16
17 $E = \frac{2\mu(2\mu + 3\lambda)}{2(\mu + \lambda)}$ (98)
18
19 and the shear modulus can be determined using
20
21 $G = \mu$. (99)
22 The above measurement method can be simulated by means of an
23 numerical example. Soft rubber-like material properties of the

test specimen are used in this simulation. The material has a 1 Young's modulus E of [(1e8-i2e7)+(5e3f-i3e2f)] N/m² where f is 2 3 frequency in Hz, Poisson's ratio v is equal to 0.40 (dimensionless), density ho is equal to 1200 kg/m³, and a 4 5 The top mass is a 0.0254 m (1 inch) steel thickness h of 0.1 m. plate that has a mass per unit area value M of 199 $\mathrm{kg/m^2}$. FIG. 4 6 is a plot of the transfer function of the system for vertical 7 motion and corresponds to equation (59). FIG. 5 is a plot of the 8 transfer function of the system for horizontal motion and 9 10 corresponds to equation (91). In FIGS. 4 and 5, the top plot is the magnitude in decibels and the bottom plot is the phase angle 11 12 in degrees.

FIG. 6 is a contour plot of the absolute value of equation 13 (60) expressed in decibels versus real dilatational wavenumber on 14 the x axis and imaginary dilatational wavenumber on the y axis at 15 The estimated dilatational wavenumber, read directly 16 2000 Hz. from the plot at the location the minimum value appears and 17 marked with an arrow, is 27.89 + 2.61i rad/m. The actual value 18 of the dilatational wavenumber is 27.99 + 2.60i rad/m, which is 19 slightly different from the estimated value due to the surface [•] 20 discretization of equation (60). FIG. 7 is a contour plot of 21 equation (60) at 5000 Hz. At this frequency, an extraneous 22 minimum appears on the left-hand side of the plot. However, 23 because the real part of the wavenumber must be increasing with 24

1 increasing frequency, the minimum corresponding to dilatational 2 wave propagation is located at the arrow marked spot and is equal 3 to 65.86 + 5.60i rad/m, as compared to an actual value of 65.77 + 4 5.62i rad/m. Again, the difference between the two values can be 5 attributed to the discretization of the surface.

FIG. 8 is plot of actual (solid line) and estimated (x 6 symbols) dilatational wavenumber versus frequency. FIG. 9 is 7 plot of actual (solid line) and estimated (+ symbols) shear 8 9 wavenumber versus frequency. In FIGS. 8 and 9, the top plot is the real part of the wavenumber and the bottom part is the 10 imaginary part of the wavenumber. FIG. 10 is a plot of actual 11 (solid line) and estimated (real part - x symbols, imaginary part 12 - o symbols) Young's modulus versus frequency. FIG. 11 is a plot 13 of actual (solid line) and estimated (real part - x symbols, 14 15 imaginary part - o symbols) shear modulus versus frequency. In FIGS. 10 and 11, the imaginary part of the modulus all have a 16 negative sign but are depicted with positive signs for plotting 17 purposes. FIG. 12 is a plot of actual (solid line) and estimated 18 (square symbols) of the real part of Poisson's ratio versus 19 frequency. Because the numerical example is formulated using a 20 Poisson's ratio that is strictly real, no imaginary component is 21 shown in this plot. Imaginary values of Poisson's ratio are 22 possible and have been shown to theoretically exist. 23

This invention gives the ability to estimate complex dilatational and shear wavespeeds of a material that is slab-

1 shaped and subjected to compressive forces. It also allows 2 estimation of complex Lamé constants of a material that is slabshaped and subjected to compressive forces. Complex Young's and 3 4 shear moduli of a material that is slab-shaped and subjected to 5 compressive forces can be estimated using this invention. The 6 invention also allows estimation of the complex Poisson's ratio of a material that is slab-shaped and subjected to compressive 7 The advantage of this patent is that it does not require 8 forces. 9 a testing configuration that has to be pressurized.

10 Obviously many modifications and variations of the present 11 invention may become apparent in light of the above teachings. 12 In light of the above, it is therefore understood that within the 13 scope of the appended claims, the invention may be practiced 14 otherwise than as specifically described.

1.	ATTORNEY DOCKET NO. 84432
2	
3	A METHOD FOR ESTIMATING THE PROPERTIES OF A
4	SOLID MATERIAL SUBJECTED TO COMPRESSIONAL FORCES
5	
6	ABSTRACT OF THE DISCLOSURE
7	A method to measure the complex frequency-dependent
8	dilatational and shear wavenumbers of a material under a static
. 9	compressional force. The material is first vibrated in a
10	vertical and horizontal directions while obtaining transfer
11	functions in each direction. The two transfer functions are
12	combined with a theoretical model to estimate a dilatational
13	wavenumber and a shear wavenumber. The wavenumbers can be
14	utilized to give the complex dilatational wavespeed, complex
15	shear wavespeed, complex Lamé constants, complex Young's modulus,
16	complex shear modulus, and complex Poisson's ratio.