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No. 896

ROUND HEAT-TREATED CHROMIUM-MOLYBDENUM-STEEL

TUBING UNDER COMBINED LOADS

By William R. Osgood National Bureau of Standards

CLASSIFIED DOCUMENT

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NATIONAL ADVISORY COMMITTEE FOR AERONAUTICS

TECHNICAL NOTE NO. 896

ROUND HEAT-TREATED OHROMIUM-MOLYBDENUM-STEEL

TUBING UNDER COMBINED LOADS

By William R. Osgood

SUMMARY

The results of tests of round heat-treated chromium-molybdenum-steel tubing are presented. Tests were made on tubing under axial load, bending load, torsional load, combined bending and axial load, combined bending and torsional load, and combined axial, bending, and torsional load. Tensile and compressive tests were made to determine the properties of the material. Formulas are given for the evaluation of the maximum strength of this steel tubing under individual or combined loads.

The solution of an example is included to show the procedure to be followed in designing a tubular cantilever member to carry combined loads.

INTRODUCTION

Some of the tubular members, such as the landing gear, used in aircraft are subjected to combined axial, bending, and torsional loads. The design of such members requires a knowledge of their strength under various combinations of these loads. For a given loading any of the stresses within the elastic range may be computed; but these stresses will give little or no information as to the nearness of failure, which occurs in the plastic range.

To predict plastic failures theoretically for any but the simplest of cross sections and the simplest of stress—strain diagrams is an insuperable task. In order, therefore, to estimate the strength at failure under combined loading of practical shapes made of practical materials, such shapes must be tested to failure under the desired conditions of loading.

The interest of the Bureau of Aeronautics, Navy
Department, in the strength of round heat-treated chromium-

molybdenum-steel tubing under combined loads led to the transfer of funds to the National Bureau of Standards for an investigation of the subject. Most of the tests previously made under combined loading have been made on extremely thin sections, so that the results are not applicable to the relatively thick sections that are of primary interest. Moreover, no tests of heat-treated chromium-molybdenum-steel tubing are known to be available.

The plan of the investigation consists of a series of tests under axial load, bending load, and torsional load alone, a series under axial load combined with bending load, a series under bending load combined with torsional load, and finally, a series in which axial load, bending load, and torsional load act simultaneously. It was not possible to make tests under torsional load combined with axial load because the clear distance (6 diam.) between loading points would have been so great in order to get away from local effects that the resulting length of the specimen would have made large bending stresses unavoidable.

The Bureau is indebted to the Summerill Tubing Company for donation of the tubing.

The author wishes to acknowledge the help received from various members of the Engineering Mechanics Section, particularly from Mr. J. Southall Noble who, assisted in the most difficult series of the tests: namely, those under combined axial, bending, and torsional loads. Mr. Noble's care and anticipation of things to be done were of inestimable value in obtaining such success as was obtained.

MATERIAL AND TESTS OF MATERIAL

The material was round, heat-treated, S.A.E. X4130 tubing. The lowest 0.002-offset tensile yield strength of the material tested was 155,000 pounds per square inch, the lowest tensile strength was 164,000 pounds per square inch, and the lowest elongation in 2 inches of the full tube was 5 percent. Thus, the tubing did not all comply with the requirements for tubing in any one physical condition specified in Army-Navy Aeronautical Specification AN-WW-T-850a: Tubing; Steel, Chrome-Molybdenum (X4130), Seamless. Except for the requirement for elongation in 2 inches, however, all the tubing complied with the required

mechanical properties for (HT-150) tubing, minimum yield strength of 135,000 pounds per square inch, minimum tensile strength of 150,000 pounds per square inch.

The nominal cross—sectional properties of the tubes are given in table I. The letter in the first column is the symbol used to identify the size of the tubing - that is, outside diameter (d) and thickness of wall (t).

At least one tensile specimen and one compressive specimen were taken from each tube.

All specimens were marked with symbols that indicated by an initial digit the tube from which the specimen was cut, by a letter (H) the fact that the material was heat treated, by another letter the nominal size of the tube, then by one or more letters following a dash, the type of test to which the particular specimen was to be subjected, and by a final digit different specimens intended for the same type of test. The letters indicating the nominal sizes of the tubes are given in table I. The letters A, B, and T were used to indicate axial, bending, and torsional loading, respectively, except that To was used for pure torsion and T by itself was used for tension. The letter C was used to indicate compressive specimens (axial loading) for which stress-strain data were obtained. For example, specimen 3HA-BT1 was cut from tube 3 of heat-treated chromium-molybdenum steel, H, having the nominal size $1\frac{1}{2}$ inches diameter by 0.120 inch thick, A (table I), and was tested in combined bending and torsion, BT. The final I distinguished this specimen from other specimens cut from tube 3HA, also tested in combined bending and torsion, such as 3HA-BT and 3HA-BT2. Specimens 3HA-T, 3HA-C, and 3HA-To would be tensile, compressive, and torsional specimens, respectively, cut from tube 3HA. Specimen 3HA-A would be a specimen like 3HA-C, for which only the crinkling strength, not stress-strain data, would be obtained.

Measured dimensions of specimens were used in all computations. In general, alternate specimens in a tube were measured. The lengths, minimum and maximum thicknesses at each end, and outside diameters at the middle were measured. Most of the measured specimens were also weighed. The cross-sectional areas were in most cases computed from the weights, the lengths, and the densities; otherwise, the areas were computed from the average measured diameter and the average measured thickness of each specimen. Similarly, the average thicknesses and the section moduli were determined from the computed cross-

sectional areas and the measured outside diameters unless the specimens were not weighed, in which case these quantities were determined directly from the measured dimensions. The thicknesses, cross-sectional areas, and section moduli of specimens adjacent to or between measured specimens were taken equal to those of adjacent specimens or they were interpolated between values for measured specimens.

Tensile tests were made of "full-tube" specimens in suitable testing machines. Compressive tests were made as described in reference 1, except that new hydraulic testing machines were used. The three shortest specimens shown in figure 1(a) are compressive specimens after test. One of these specimens has still attached to its ends the Wood! s-metal castings used to secure somewhat more uniform conditions at the ends of compressive specimens and of crinkling specimens. As in the earlier tests (reference 1) the machines were not particularly well adapted for compressive tests, many of the specimens, although only four diameters long, showing evidence of bending at failure. The bending may have been due to tilting of the platen caused by yielding of the packing between ram and cylinder of the testing machine. It is believed that the ram of a hydraulic machine designed for compressive tests should be a close fit in the cylinder - close enough to dispense with packing. Bending in the specimens may possibly also have been induced by exceeding the critical loads of the testing machines. An excellent discussion of this phenomenon may be found in reference 2.

Strains up to approximately 0.01 were measured with a Ewing extensometer on a 2-inch gage length in all tensile and compressive specimens except one tensile specimen. Two Tuckerman strain gages on 2-inch gage lengths were used on this specimen.

The moduli of elasticity were obtained from the stress-strain data by means of deviation curves (reference 3). Two tensile yield strengths were obtained for each tensile specimen from stress-strain curves: namely, the 0.002-offset yield strength and the 5/7 E-secant yield strength. The offset yield strength is well known, the secant yield strength is discussed in reference 4. It is the stress at the intersection with the stress-strain curve of a straight line through the origin having a slope 5/7 E, where E is Young's modulus. The 5/7 E-secant yield strength was obtained for each compressive specimen from stress-strain curves. Figure 2 shows typical stress-strain curves. The

short cross lines near the tops of the curves indicate the yield strengths, the left—hand lines of the tensile diagrams indicating the offset yield strengths. The mechanical properties are given in table II.

In most (18) of the tubes a compressive specimen - was cut adjacent to a tensile specimen and it was therefore possible to compare the compressive with the tensile properties unaffected by wide variations along the tube. In an attempt to correlate the compressive yield strength with tensile properties, various quantities were plotted against each other. It was found that the compressive yield strength, S, could be predicted from the tensile strength, T, both in pounds per square inch, by the linear relation

$$S = 7/6 (T - 30,000), 174,000 < T < 194,000 (1)$$

Figure 3 shows the plotted values and the line represented by equation (1). The maximum difference between an observed compressive yield strength and the yield strength computed from the formula was 7350 pounds per square inch or 4.2 percent; the observed yield strength was lower by this amount. When the compressive yield strength was plotted against the secant tensile yield strength, a greater scatter was found than in figure 3.

Some wide variations in mechanical properties were found along some of the tubes and may have existed in others. At one end of tube 2HY the secant yield strength and the tensile strength were 156,000 and 164,000 pounds per square inch, respectively, and about 100 inches from this end they were 173,000 and 187,000 pounds per square inch, respectively. The tensile specimen from tube 4HD was cut from an end and had a tensile strength of 171,000 pounds per square inch. The adjacent compressive specimen had a yield strength of 181,000 pounds per square inch. This yield strength was so much higher than would be expected from the results of the other tests, figure 3, that the tensile strength is believed to have been abnormally The values for tube 4HD were not used in obtaining equation (1) and are not plotted in figure 3. Omitting them in predicting compressive yield strength is on the safe side. One other tube, 1HY, had a tensile specimen cut from one end. This specimen had a tensile strength of 168,000 pounds per square inch, and there are indications,

which will be discussed subsequently, that this value is also abnormally low. Two compressive specimens from tube 2HD were about $56\frac{1}{2}$ inches apart, center to center. The compressive yield strengths of these specimens differed by 184 pounds per square inch per inch of distance between them, or over 0.1 percent per inch.

AXIAL TESTS

The axial tests consisted of the compressive tests and a few crinkling tests, the only difference between these two categories being that stress-strain data were not taken on the crinkling specimens. Three compressive specimens after test are shown in figure 1(a).

The results of the axial tests are given in table III. The symbols d and t are the outside diameter and the thickness of the wall, respectively, of the specimen. The crinkling strength, f_A , is the maximum axial load sustained by the specimen divided by its cross-sectional area. The lower group of points in figure 4 shows the relation between d/t and f_A . One point, enclosed in braces (specimen 4HY-C), is in question because the specimen was recut from a specimen tested under combined axial, bending, and torsional loading. Otherwise, the scatter in figure 4 is to be attributed largely to variations in the properties of the material, particularly the compressive yield strength. As discussed in reference 1, the scatter can be much reduced by plotting $\sigma_A = \frac{f_A}{S}$ against $\frac{1}{S} = \frac{Et}{Sd_m}$, where d_m is the mean diameter, d-t,

of the specimen. Values of $1/\delta$ and σ_A are given in table III and are plotted as the lower group of points in figure 5. The compressive yield strengths of 28 of the compressive specimens (all except 4HY-C) were known accurately, and the relation between σ_A and $1/\delta$ for these specimens can be given by

$$\sigma_{A} = 0.67 + 0.112\frac{1}{\delta} - 0.0099\frac{1}{\delta^{2}} + 0.0003\frac{1}{\delta^{3}}, \qquad 4 < \frac{1}{\delta} < 16$$
 (2)

with a maximum deviation in σ_A of 0.020 or less than 2 percent. The data in themselves are not considered to

warrant the refinement implied in equation (2), but the use to be made of the data in subsequent estimates of compressive yield strengths and in the combined tests made it seem desirable to approximate as closely as reasonably possible by means of an analytical expression. The curve represented by equation (2) is shown in figure 5. To one familiar with the type of results depicted, the occurrence of a point of inflection may cause some skepticism, but a ready explanation may be given. The compressive stress-strain curves of the HA material $(1/\delta)$ between 15 and 16) all were appreciably steeper in the plastic range than those of the other material. Consequently, in accordance with Geckeler's theory (references 1 and 5), higher loads at failure would be expected from the HA material than if its stress-strain curves had been less steep in the plastic range, specifically, less steep at the stress at failure.

Specimens 1HL-A and 1HL-Al were taken between two compressive specimens, and their moduli of elasticity and their yield strengths were estimated by linear interpolation between the values found for the compressive specimens. The points representing specimens IHL-A and IHL-Al in figure 5 are enclosed in brackets. The moduli of elasticity of the other crinkling specimens were assumed equal to that of the compressive specimen of the tube from which Their yield strengths were estimated, as explained in appendix A, from equation (2) and the maximum variation in yield strength found between two compressive specimens from the same tube, namely 184 pounds per square inch per inch, all tubes with two compressive specimens being considered. The points representing these specimens in figure 5 are enclosed in parentheses. Finally, the point representing specimen 4HY-C is enclosed in braces, because it is questionable as mentioned previously, and should be left out of consideration. This point fortunately falls above the curve, and its omission is therefore on the safe side.

BENDING TESTS

The bending tests were made as described in reference 1, the specimens being loaded symmetrically at two sections and essentially the same apparatus with a few practical modifications being used. Three bending specimens after test are shown in figure 1(b).

The results of the bending tests are given in table IV. The modulus of rupture, fB, was computed from the familiar flexure formula.

$$f_B = \frac{\overline{M}}{Z}$$

where $\overline{\mathbf{M}}$ is the maximum bending moment sustained, and Z is the section modulus of the cross-sectional area. The upper group of points in figure 4 shows the relation between d/t and f_B . As in the axial tests, the scatter is pronounced and somewhat more marked than in those tests. It seems likely that variation in the mechanical properties circumferentially as well as longitudinally and from tube to tube enter the picture here. If, for example, the compressive yield strength of the part of the specimen in compression happened to be relatively low, the bending strength of that specimen would be lower than if it had been tested with this part in some other position. Without a knowledge of the circumferential variation of the mechanical properties nothing can be done to correct for. their effects, but something can be done about that part of the scatter caused by variation in the average compressive yield strength between specimens, at least to the extent to which the yield strengths of the individual specimens are known. Again, as in reference 1, σ_R =

 $\frac{f_B d_m}{S d} \quad \text{is plotted against} \quad \frac{1}{\delta} = \frac{E t}{S d_m} \qquad \text{Values of -1/8} \quad \text{and} \quad$

 $\sigma_{\rm p}$ are given in table IV and are plotted as the upper group of points in figure 5. Values of the moduli of elasticity and the compressive yield strengths were estimated as explained in appendix A. Two specimens, 2HF-B and 2HF-Bl, were more than 100 inches away from the nearest specimen for which a yield strength could be estimated with some degree of assurance and, in view of the large possible variation in yield strength in this length, about 10 percent, the points representing these specimens in figure 5 have been enclosed in braces. They are considered unreliable. Specimen 2HF-B (the lower point in the figure), moreover, was cut from an end of the tube and may well have had an abnormally low tensile yield strength and possibly a somewhat lowered compressive yield strength. One other point is conspicuous: namely, the one representing specimen 3HY-B. This specimen also was cut from an end of the tube, but because it was less į, ,

than 100 inches from a specimen with a fairly reliably known yield strength, it did not seem justifiable to throw it out, even though its yield strength was not so well known as was to be desired. Other low points in figure 5 similarly represent specimens cut from ends of tubes; but because not all specimens cut from ends give low values of σ_B , a considerable number of additional tests would be necessary to establish a relation between $1/\delta$ and σ_B that could be said to be definitely better than one based on the available data. If only specimens 2HF-B and 2HF-Bl are neglected, the relation between σ_B and $1/\delta$ can be given by

$$\sigma_{\rm B} = 0.7 + 0.16 \frac{1}{\delta} - 0.015 \frac{1}{\delta^2} + 0.000485 \frac{1}{\delta^3}, \ 4 < \frac{1}{\delta} < 16$$
 (3)

with a deviation in $\sigma_{\rm R}$ of 6.5 percent for specimen 3HY-B and otherwise with a maximum deviation of 4.5 percent. The two curves in figure 5 probably should have the same shape: for example, if one of them is markedly concave downward over part of the range of $1/\delta$ and concave upward over the rest of the range, one would expect the other to be so also, although the point of inflection might not come at exactly the same $1/\delta$. If all specimens cut from ends of tubes were neglected and a fair curve to represent the results were drawn, the curve would follow the shape of the curve representing crinkling strength less closely than the curve shown in figure 5. The curve of crinkling strength is well established and the curve of bending strength should follow it. It therefore seems that equation (3) is at least as good as, and probably better than, a relation obtained by neglecting specimens cut from the ends of tubes. Including these specimens is safe because $\sigma_{\rm p}$ is lowered when they are included.

As in the case of the axial tests, the refinement implied by equation (3) is not justifiable on the basis of the data alone. The use to be made of these data in the analysis of the combined tests later, however, made it seem desirable to approximate closely to them. The rise in σ_B at high values of $1/\delta$ is to be explained in the same way as the rise in σ_A at high values of $1/\delta$.

TORSIONAL TESTS

The tersional tests were made as described in reference 6. Three tersional specimens after test are shown in figure 1(c).

The results of the torsional tests are given in table V. The modulus of rupture in torsion, $\,f_{\,\underline{T}}\,,\,$ was computed from the formula

$$f_{\underline{T}} = \frac{M\underline{T}}{2Z}$$

where M_T is the maximum twisting moment. The relation between d/t and f_T is shown in figure 6. The scatter in the points is due primarily to variation in the mechanical properties of the specimens. The most significant property in this respect is probably the compressive yield strength. In order to introduce this quantity in a rational manner, the solution for two-lobed buckling of a long, thin elastic cylindrical shell under torsion is examined. Timoshenko (reference 7) finds the shearing

stress $f_m = f_T \frac{d_m}{d}$ at the instant of buckling to be given by

$$f_{m} = \frac{2E}{3(1-v^{2})^{\frac{3}{4}}} \left(\frac{t}{d_{m}}\right)^{\frac{3}{2}}$$

The theory for plastic buckling is not known to have been worked out, but it is probable that, as in Geckeler's theory (reference 1), the double-modulus would appear on the right side of the equation and that a relation could

be found between the variables $\frac{1}{\delta_s} = \left(\frac{E}{S}\right)^{\frac{8}{3}} \frac{t}{d_m}$ and $\tau_T =$

 $\frac{f_T}{S} \frac{d_m}{d}$ which would be the same for all materials having affinely related stress-strain curves (S necessarily being the secant yield strength). For the elastic case the relation would be

$$\tau_{T} = \frac{2}{3(1-v^{2})^{\frac{3}{4}}} \left(\frac{1}{\delta_{s}}\right)^{\frac{3}{2}} \tag{4}$$

The preceding considerations suggest that $\tau_{\underline{T}}$ be plotted against $\frac{1}{\delta_B}$. Values of these nondimensional

variables are given in table V and have been plotted in figure 7. Values of the moduli of elasticity and the compressive yield strengths were estimated as explained

in appendix A. The relation between $\frac{1}{\delta \, s}$ and τ_T can be given by

$$T_{\rm T} = 0.06(\frac{1}{\delta_{\rm S}} - 0.8) + 0.5, 0.8 < \frac{1}{\delta_{\rm S}} < 3.2$$
 (5)

with a maximum deviation of 5.5 percent. Equation (4) for elastic buckling, with $v=\sqrt{1-\frac{64}{75}\sqrt[4]{\frac{4}{3}}}\approx 0.25$, is also

represented in figure 7 by the dotted curve.

COMBINED AXIAL AND BENDING TESTS

The combined axial and bending tests were made in a horizontal testing machine. Figure 8 shows the setup for a test. The ends of the specimen were plugged with snugfitting plugs, shown at the right in figure 9. The outer ends of the plugs contained hardened steel inserts, shown in the top plug in figure 9, and were turned to fit closely in the ball bearing shown in the pillow-block assembly in figure 9. When a plug was pushed into one of the ball bearings, its hardened insert bore on a half-inch steel ball in the pillow-block assembly, shown on the axis of the ball bearing in figure 9. The point of contact between the hardened insert and the ball was accurately located on the axis of rotation of the pillow blocks. The capacity of one of the pillow-block assemblies was 20,000 pounds.

Tests were made as follows. The pillow-block assemblies were securely fastened to the heads of the testing

machine with the axes of the pillow blocks horizontal and accurately parallel (fig. 8). Loading clamps (reference 1) were applied to symmetrically situated sections of the specimen not less than about 6 diameters apart, and the plugs mentioned previously were pushed into the ends of the specimen. The specimen was then transferred to the testing machine, the outer ends of the plugs being inserted in the ball bearings of the pillow-block assemblies; one plug was inserted first and the head of the machine was then moved up and the other plug inserted. An initial axial load of a few hundred pounds, depending on the size of the specimen, was applied.

A deflectometer (figs. 8 and 10) was attached to the middle of the specimen. Since the middle of the specimen assumed a pronouncedly oval shape at failure, it was thought desirable in the interests of accuracy to measure the deflection of a horizontal diameter. The deflectometer was therefore attached to the specimen by means of two hard conical points in the ends of a clip that snapped into diametrically opposite prick-punched holes in the specimen. The clip was fastened to a steel scale sliding in a slotted frame having a vernier. Deflections could be read directly to 0.002 inch. The deflectometer was eminently satisfactory. It could be removed from the specimen and replaced with a change in reading not greater than 0.001 inch and, with the deflectometer attached, the specimen could be rotated about its axis through several degrees with a change in reading of the deflection not exceeding 0.001 inch.

A dial gage, shown in figure 8, was attached to measure the motion of the pillow-block assemblies toward each other as axial load was applied.

With the deflectometer and the dial gage attached, initial readings of these two instruments were taken, and then transverse load was applied to the specimen by means. of hangers, an equalizer, and dead weights (fig. 8). The axial load was then increased gradually until a maximum was passed, and frequent readings were taken of the axial load, the deflectometer, the dial gage, and the distance between loaded sections. Three specimens after failure under combined axial and bending load are shown in figure 1(d).

The axial stress, $f_a = P/A$, was computed as the axial

load, P, at failure divided by the cross-sectional area, A, of the specimen. The bending stress was computed from the flexure formula

$$f_b = \frac{\overline{M}}{z}$$

where M is the maximum bending moment produced by the transverse load and the axial load. The moment arm of the reaction for determining the bending moment produced by the transverse load was computed from the initial distance between the axis of a set of pillow blocks and the adjacent loaded section, corrected for shortening from the reading of the dial gage and the distance between the loaded sections. The bending moment produced by the axial load was the product of the axial load and the deflection of the middle of the specimen.

Values of f_a and f_b are given in table VI. If f_a and f_b were to be plotted against each other, the points would be found to scatter about a series of curves representing different values of d/t. The scatter could

be reduced by plotting $\sigma_a=\frac{f_a}{S}$ and $\sigma_b=\frac{f_b}{S}$ $\frac{d_m}{d}$ against each other. It was found that the effect of different values of d/t could be much reduced, and perhaps elimi-

nated within the experimental error, by plotting $\frac{\sigma_b}{\overline{\sigma_B}}$ against $\frac{\sigma_a}{\sigma_A}$, where σ_B and σ_A are computed from equations (3) and (2), respectively. Values of $\frac{1}{\delta}$ for computing σ_A and σ_B , and values of σ_a , σ_b , $\frac{\sigma_a}{\overline{\sigma_A}}$, and

are given in table VI. The moduli of elasticity and the compressive yield strengths of the individual specimens were estimated as explained in appendix A. The ratio $\frac{Ch}{CR}$ plot-

ted against the ratio $\frac{\sigma_a}{\sigma_A}$ is shown in figure 11. This

figure still shows appreciable scatter, some of which is no doubt due to the uncertainty in the yield strengths, almost surely so for the 2HY- specimens. The order of these

specimens in the tube was 2HY-T1, -AB, -AB1, -AB2, -AB3, -AB4, -T. Specimen 2HY-T was an end specimen with an. abnormally low tensile strength. The compressive yield strength estimated from it (see appendix A) would consequently also be abnormally low, so that, in interpolating the compressive yield strengths of the specimens between 2HY-Tl and 2HY-T, abnormally low values would be obtained at the 2HY-T end of the interval. The relative positions of the points in figure 11 representing the 2HY- specimens reflect this situation, the points for specimens 2HY-AB2, -A33, and -AB4 being quite out of line with the points for specimens 2HY-AB and 2HY-ABl. Again, specimen 1HD-AB, taken next to a crinkling specimen for which the yield strength is fairly well known, falls in line fairly well in figure 11; whereas specimens 1HD-AB1 and 1HD-AB2 with questionable yield strengths are out of line. Some of the scatter in figure 11 is also due to the uncertainty in the moment of the axial load. As the maximum axial load is approached, the deflection increases rapidly relative to the axial load, and it is difficult to say just what the deflection is at the maximum axial load. Frequently no change in axial load is indicated by the testing machine for a considerable variation in deflection (several tenths of an in.), particularly in the case of the relatively thick specimens. It seems quite possible, almost probable, that the d/t-effect still showing in figure 11 is due to the uncertainty in the moment of the axial load. The tendency in the laboratory was to underestimate this moment (on the side of safety), and, as mentioned, the error in the estimate was likely to be greater for the thick speci-If this is so, the points representing the HA- and the HL-tubes should be moved up. This difficulty of determining the maximum axial load and the accompanying deflection is inherent in any testing machine that does not follow through with the load. Dead-weight loading would be required to overcome it satisfactorily. The points in figure 11 have been approximated to by the equation

$$\left(\frac{\sigma_{\mathbf{a}}}{\overline{\sigma_{\mathbf{A}}}}\right)^{\frac{8}{10}} + \left(\frac{\sigma_{\mathbf{b}}}{\overline{\sigma_{\mathbf{B}}}}\right)^{\frac{9}{10}} = 1, \qquad \frac{\sigma_{\mathbf{a}}}{\overline{\sigma_{\mathbf{A}}}} < 1, \tag{6}$$

and the curve represented by the equation is shown in the figure. The curve favors low values of σ_a/σ_A and tends to be conservative in general. In most practical cases in which combined axial and bending loads occur, it is

believed that the axial stress will be smaller than the maximum bending stress, so that the upper half of the curve is perhaps more important than the lower half.

COMBINED BENDING AND TORSIONAL TESTS

The combined bending and torsional tests were made as shown in figures 12 and 13. The tests were made essentially in the same way as the bending tests, except that torsion was introduced by clamping split pulleys to the specimen at the loaded sections and loading through wire ropes leaving the pulleys on opposite sides of the specimen. Pulleys with diameters of 12 and 30 inches were used (the 30-in. pulleys are shown in the figures) and, by varying the span, various combinations of bending moment and twisting moment could be secured. Great difficulty was experienced in gripping the specimen in torsion. The grips shown in figure 14 were finally designed and worked satisfactorily. Snug-fitting plugs each 11 inches long were pushed into the specimen to the positions at which the grips were to be applied. The specimen was placed in one of the grips, and three cylindrical segments, shown at the extreme right and left of the figure, were inserted in the outer end of the grip symmetrically around the circumference of the specimen. Three serrated hard steel wedges I inch wide, shown in the middle of the figure, were then inserted between the specimen and the inner end of the grip at the position of one of the plugs previously pushed into the specimen. These wedges slid on the three plane surfaces shown at the inner end of the left-hand grip in the figure. They were pushed tightly against the specimen by means of fine-thread setscrews. By also tightening the cylindrical segments against the specimen by means of setscrews, the grip was securely held on the specimen with its axis and that of the specimen colinear. The other grip was attached in the same way, opposite hand. These grips replaced the grips first used, shown in figures 12 and 13, and are shown in figure 15 attached to a specimen. Three combined bending and torsional specimens after failure are shown in figure 1(s). It is interesting to compare the orientations of the buckles in two of the specimens with those of the bending and torsional specimens. In the bending and the torsional specimens the buckles, when they appeared, formed at right angles and at approximately 45°, respectively, to the axis

of the specimen. In the combined bending and torsional specimens the buckles appeared at an intermediate inclination, as would be expected.

The bending stress, $f_b,$ and the torsional stress, $f_t,$ were computed from the formulas $f_b=\frac{\overline{M}}{Z}$ and $f_t=\frac{M_t}{2Z},$ respectively, where \overline{M} and M_t are the maximum bending moment and maximum twisting moment. Values of f_b and f_t are given in table VII. When f_b and f_t are plotted against each other, the points are found to scatter about a series of curves representing different values of d/t. As in the combined axial and bending tests the scatter could be reduced by plotting

$$\sigma_b = \frac{f_b d_m}{Sd}$$
 and $\tau = \frac{f_t d_m}{Sd}$

against each other. As found previously, the effect of the ratio $\,d/t\,$ can be eliminated within the experimental error by plotting τ/τ_{m} against σ_{b}/σ_{B} , where τ_{T} and $\sigma_{\rm R}$ are computed from equations (5) and (3), respectively. Values of $1/\delta$ and $1/\delta_S$ for computing σ_B and τ_T , and values of σ_b , τ , σ_b/σ_B , and τ/τ_m are given in The moduli of elasticity and the compressive table VII. yield strengths of the individual specimens were estimated as explained in appendix A. The values of τ/τ_T plotted against σ_h/σ_R are shown in figure 16. Although this figure still shows appreciable scatter, most of it can be explained by variations in yield strength along the length of the tubing. The specimens of tube 3HY were cut in the order 3HY-BT, -BT1, -BT2, -BT3, -T, -C, -BT4, -BT5, -B. The order of specimens in figure 14 arranged according to increasing values of σ_h/σ_B is SHY-BT1, -BT, -BT2, -BT3, -BT4, -BT5. It was necessary in the absence of other information to assume the compressive yield strength of all of these specimens to be that of the compressive specimen, 3HY-0. It seems rather evident,

however, from the low value of $\frac{\sigma_b}{\sigma_B}$ (0.123) and the accompanying high value of $\frac{\tau}{\tau_T}$ (1.072) for specimen 3HY-BT1

that the yield strength of this specimen was greater than that of the compressive specimen. It is hardly conceivable that $\tau/\tau_{\rm T}$ could exceed 1. Since specimen

3HY-BTl was near the end of the tube and probably had a relatively high yield strength, the yield strength may be assumed to have decreased from this end of the tube to the other end. If this assumption is made, specimens 3HY-BT4 and 3HY-BT5 which now appear to have low values of σ_b/σ_B and τ/τ_T and were on the other side

of the compressive specimen 34Y-0, would have lower yield strengths than assumed, and the points representing them would be raised and moved to the right in figure 16. The other values for the 3HY-specimens would also lie nearer the curve. Another tube exhibiting anomalous behavior in figure 16 is 2HD. Specimen 2HD-BT3 was cut adjacent to a compressive specimen and between two such specimens; its compressive yield strength should therefore be known rather accurately, and it appears to be satisfactorily represented in figure 16 (the point (0.099, 1.006)). The other three 2HD specimens, however, were cut between a compressive specimen and the end of the tube, and none of them was adjacent to the compressive specimen. Their yield strengths were all assumed to be that of the compressive specimen. It seems

probable, from the high value of $\frac{\tau}{\tau_{\rm T}}$ (1.047) that this

assumption was in error and that their yield strengths were actually higher than assumed. Finally, values for specimens 3HF-BT and 3HF-BT1 gave rather high values. These two specimens were cut from the same relative position in the tube as the three 2HD specimens just discussed, and their yield strengths similarly may not have been reliable. The compressive yield strengths of the remaining specimens could, in general, be estimated more reliably than those of the specimens that have been mentioned. All the errors in the previous discussion have tacitly been attributed to errors in T and $\sigma_{\rm b}.$ It should be remembered, however, that $\tau_{\rm T}$ and $\sigma_{\rm B}$ also contain errors and that these errors also enter the problem, although in an unknown way.

When consideration is taken of the anomalous points in figure 16, the relation between σ_b/σ_B and $^T/\tau_T$ can be given reasonably by

$$\left(\frac{\sigma_b}{\sigma_B}\right)^{\frac{5}{2}} + \left(\frac{\tau}{\tau_T}\right)^{\frac{5}{2}} = 1 \tag{7}$$

The curve represented by this equation is shown in figure 16. With the apparatus at hand combinations of smaller values of τ/τ_T and larger values of σ_b/σ_B than those shown could not be obtained, but, since any curve representing the results of tests must go through the point (1.0, 0) the uncertainty in the lower portion of the curve adopted is not considered to be excessive.

COMBINED AXIAL, BENDING, AND TORSIONAL TESTS

The final series of tests, under combined axial, bending, and torsional loads, were made as shown in figure 15. The setup is similar to that used in the combined axial and bending tests, but the transverse loads were applied by dead weights suspended from opposite sides of two pulleys, thus producing torsion as well as bending. The pulleys, 36 inches in diameter for these tests, played the same role as the pulleys in the combined bending and torsional tests. The procedure for making a test was practically the same as that for making a combined axial and bending test. The vernier on the deflectometer was read with a telescope. Three specimens after failure are shown in figure 1(f).

The axial stress, $f_a = \frac{P}{A}$, was computed as the axial load, P, at failure divided by the cross-sectional area, A, of the specimen. The bending stress f_b and the torsional stress f_t were computed from the formulas $f_b = \frac{R}{Z}$ and $f_t = \frac{M_t}{2Z}$, respectively, where \overline{M} was the maximum bending moment and M_t was the (constant) twisting moment. Values of f_a , f_b , and f_t are given in table VIII. The previous two series of tests under combined load indicated that the significant variables to be correlated were σ_a/σ_A , σ_b/σ_B , and τ/τ_T . These quantities were therefore computed for this last series of tests. Values of $1/\delta$ and $1/\delta_B$ for computing σ_A , σ_B , τ_T from equations (2), (3), and (5);

values of σ_a , σ_b , and τ ; and values of σ_a/σ_A , σ_b/σ_B , and τ/τ_T are given in table VIII. If the last three vari—

ables are assumed to form a consistent set for all the conditions of the tests, a surface which adequately represents them must be found. The surface must cut the σ_a/σ_A , σ_b/σ_B —plane and the σ_b/σ_B , τ/τ_T -plane in the curves of equations (6) and (7), respectively. Several surfaces were tried and, since the uncertainty in σ_b/σ_B was far greater than that in σ_a/σ_A and τ/τ_T , observed and computed values of σ_b/σ_B were compared. The surface that was finally selected as most nearly representative of the test data was

$$\frac{\left(\frac{\sigma_b}{\sigma_B}\right)^{\frac{5}{2}}}{\left[1-\left(\frac{\sigma_a}{\sigma_A}\right)^{\frac{9}{10}}\right]^{\frac{25}{9}}} + \frac{\left(\frac{\tau}{\tau_T}\right)^{\frac{5}{2}}}{\left(1-\frac{\sigma_a}{\sigma_A}\right)^{\frac{5}{2}}} = 1, \quad \frac{\sigma_a}{\sigma_A} < 1 \quad (8)$$

The traces of this surface on the coordinate planes are shown in figure 17. Of the 21 sets of observed values of σ_a/σ_A , σ_b/σ_B , and τ/τ_T nine sets fell inside the surface and twelve fell outside. There was a distinct tendency for the specimens with low values of $\,\mathrm{d}/\mathrm{t}\,$ to fall inside the surface. This tendency may represent a real d/t -effect but, as explained in the discussion of the combined axial and bending tests, it may merely represent the greater uncertainty in estimating the bending moment arising from the axial load. In the case of the thick specimens the moment was estimated low rather than high. An attempt was made to improve the estimate by plotting readings of axial load against deflection and picking the maximum load from a smooth curve drawn through the points, but near the maximum load the curve was so flat for a large range of deflection that a reliable maximum and corresponding deflection could not be read off. The only answer to this problem would seem to be deadweight application of axial load, an expensive procedure.

APPLICATION TO (HT-180) MATERIAL

All the results of the investigation have been given in nondimensional form. This form, which is best suited for presenting test data, is not convenient for practical use, and it remains to apply the information obtained in terms of the actual material and its dimensions. Although, as stated at the beginning of this report, except for the requirement for elongation, all the material complied with the required mechanical properties for HT-150 tubing, nevertheless the material is more nearly representative of the HT-180 tubing, minimum required yield strength of 165,000 pounds per square inch and minimum required tensile strength of 180,000 pounds per square inch. It will therefore be assumed that it does represent HT-180 tub-In order to apply the various equations representing the results, the modulus of elasticity and the compressive yield strength to be expected of material just complying with the specification must be known. average modulus found for the compressive specimens was $E = 29.5 \times 10^6$ pounds per square inch. The compressive yield strength, S, is obtained from equation (1) (T = 180,000) as 175,000 pounds per square inch. With these values of E and S the several equations of the report may be transformed.

The crinkling strength becomes from equation (2)

$$f_{A} = 117,200 \left[1 + \frac{28.18}{\frac{d}{t} - 1} - \frac{419.9}{\left(\frac{d}{t} - 1\right)^{2}} + \frac{2145}{\left(\frac{d}{t} - 1\right)^{3}} \right], 11.54 < \frac{d}{t} < 43.14$$
 (9)

The curve represented by this equation is shown in figure 4.

The modulus of rupture becomes from equation (3)

$$f_{B} = \frac{122500\frac{d}{t}}{\frac{d}{t}-1} \left[1 + \frac{38.53}{\frac{d}{t}-1} - \frac{608.9}{\left(\frac{d}{t}-1\right)^{2}} + \frac{3319}{\left(\frac{d}{t}-1\right)^{3}} \right], 11.54 < \frac{d}{t} < 43.14 \quad (10)$$

The curve represented by this equation is shown in figure 4.

The modulus of rupture in torsion becomes from equation (5). $f_{T} = \frac{79100\frac{d}{t}}{\frac{d}{t}-1} \left(1 + \frac{4.051}{\frac{d}{t}-1}\right), \quad 10.54 < \frac{d}{t} < 39.14 \quad (11)$

The curve represented by this equation is shown in figure 6.

Equations (6), (7), and (8) for combined loads become simply

$$\left(\frac{f_a}{f_A}\right)^{\frac{9}{10}} + \left(\frac{f_b}{f_B}\right)^{\frac{9}{10}} = 1, \qquad \frac{f_a}{f_A} < 1, \quad (12)$$

$$\left(\frac{f_b}{f_B}\right)^{\frac{5}{2}} + \left(\frac{f_t}{f_T}\right)^{\frac{5}{2}} = 1, \qquad (13)$$

$$\frac{\left(\frac{f_b}{f_B}\right)^g}{\left[1-\left(\frac{f_a}{f_A}\right)^{\frac{9}{10}}\right]^{\frac{25}{9}}} + \frac{\left(\frac{f_t}{f_T}\right)^{\frac{5}{2}}}{\left(1-\frac{f_a}{f_A}\right)^{\frac{5}{2}}} = 1, \frac{f_a}{f_A} < 1, '14$$

respectively, where fa, fb, and ft are the components of stress arising from the actual loading, and fA, fB, and fr have the values given by equations (9), (10), and (11) for the particular d/t of the tube being considered. should be noted that all the equations represent maximum conditions, that is, conditions at failure. In the presence of axial load, fo must therefore include the bending stress produced by this load since this stress may be considerable at failure. To estimate it requires a knowledge of deflection at failure; use of the deflection under the assumption of elastic conditions will not be The actual deflection is known only for the particular conditions of the tests. Many additional tests would be required to determine this deflection for an appreciable number of other types of loading. It is suggested, therefore, that for other types of loading the actual deflection be considered to bear the same relation to the elastic deflection as the actual deflection of the tested specimens bore to the elastic deflection. Figure 18 shows, in effect, this relation for the specimens tested under combined axial and bending loads. The abscissas are the ratios of the maximum total bending moment M produced

by the transverse load and the axial load, computed by elastic theory (see appendix B), to the maximum bending moment $M_{\rm D}$ produced by the transverse load alone. The ordinates are the ratios of the observed maximum total bending moment \overline{M} to $M_{\rm D}$. A straight line

$$\frac{\overline{M}}{M_b} = 1 + \frac{17}{13} \left(\frac{M}{M_b} - 1 \right), 1 < \frac{M}{M_b} < 3.6$$
 (15)

expressing the relation for low values of $\rm M/M_b$ is shown in the figure. The relation is inadequately defined for high values of $\rm M/M_b$ but the equation

$$\frac{\overline{M}}{M_b} = 4.4 + \frac{15}{8} \left(\frac{M}{M_b} - 3.6 \right), \frac{M}{M_b} > 3.6$$
 (15a)

may be used if necessary in the absence of other information. If the specimens tested under combined axial, bending, and torsional loads were treated in the same way the points representing them would in general fall below those shown in figure 18. If the data were sufficiently good, it would be possible to draw a series of curves between the lines represented by equations (15) and (15a) and the (dotted) line for the elastic case, $\overline{M}/M_b = M/M_b$ each of these curves representing some condition of torsion; but the data are not good enough.

It should be emphasized that equations (15) and (15a), applied to other conditions of loading than those of the tests or to tubing with stress-strain curves differing widely in shape from those of the tests, are nothing more than arbitrary expressions for reducing Young's modulus to make it possible to estimate the effect of plastic deflections by elastic theory. equations are substantiated by only a limited number of tests made under special conditions of loading (symmetrical two-point loading) and represent conditions at failure for this type of load. If torsional loads are added, the equations become conservative. Nothing is known as to how good the relation is for other types of bending load. On the other hand, consideration of the whole picture combined with engineering judgment makes it seem that equation (15) may be exploited without too much trepidation.

To design a tubular member subjected to axial load combined with bending load or bending load and torsional load, proceed as follows. Select a trial section, compute $M/M_{\rm b}$ from the known load and dimensions, and then compute $\overline{M}/M_{\rm b}$ and thus \overline{M} from equation (15). From this value of \overline{M} , compute $f_{\rm b}$,

$$f_b = \frac{\overline{M}}{Z}$$

Compare the value so obtained with the allowable value from equation (13) or equation (14). It may be noted that tables of the three-halves powers and the five-halves powers of numbers, such as reference 8, are of material assistance in solving equations (12), (13), and (14). Charts may also be constructed to facilitate the solution of these equations.

Example

It is desired to design a tubular cantilever member to carry an end thrust P=16,000 pounds, a bending moment $N_b=54,000$ pound—inches applied at the free end, and a twisting moment $N_t=25,000$ pound—inches applied at the free end. The length 1 of the member is 52 inches. The modulus of elasticity may be taken as 29,500,000 pounds per square inch.

Try a section 3 inches in outside diameter by 0.120 inch thick. For this section d/t=25.0, A=1.086 square inches, I=1.128 inches, Z=0.7518 inch. Then

$$f_a = \frac{16000}{1.086} = 14,730.$$

and, from equation (9),

$$f_A = 187,600$$

so that

$$\frac{f_a}{f_A} = 0.0785$$

Also

$$f_t = \frac{25000}{2 \times 0.7518} = 16,630$$

and, from equation (11),

$$f_T = 96,330$$

. so that

$$\frac{\mathbf{f}_{\mathbf{t}}}{\mathbf{f}_{\mathbf{T}}} = 0.1726$$

The allowable value of $\rm f_b/f_B$ may now be obtained from equation (14). Substituting in the values just found for $\rm f_a/f_A$ and $\rm f_t/f_T$ and solving for $\rm f_b/f_B$ gives

$$\frac{f_b}{f_B} = 0.893$$

In order to obtain the actual value of $\,f_{\,b},\,\,$ the maximum bending moment $\,\overline{\rm M}\,\,$ must be known. By elastic theory (see appendix B, equation (33)) for the given conditions of loading

$$\frac{M}{M_b}$$
 = sec 52 $\sqrt{\frac{16000}{29500000 \times 1.128}}$ = 2.395

and from equation (15)

$$\frac{\overline{\mathbf{M}}}{\mathbf{M}\mathbf{b}} = 2.824$$

Consequently.

$$f_b = \frac{2.824 \times 54000}{0.7518} = 202,800$$

and, from equation (10).

$$f_{R} = 228,200$$

so that

$$\frac{f_b}{f_B} = 0.889$$

Since this value is less than the allowable value of 0.893, the 3-inch by 0.120-inch tube is satisfactory.

National Bureau of Standards, Washington, D. C., October 2, 1942.

APPENDIX A

ESTIMATION OF THE COMPRESSIVE YIELD STRENGTH

AND YOUNG'S MODULUS

After the beginning of the investigation, the mechanical properties of a single tube were found to be quite variable along its length and the compressive yield strength, for example, of a single compressive specimen could not be taken as that of other specimens at some distance from it. It was therefore found necessary to do what could be done to estimate the compressive yield strengths of these other specimens.

After the variability was discovered, two compressive specimens were taken from each tube; but unfortunately some tubes had been used up by this time, from each of which only one compressive specimen had been cut. When two compressive specimens existed for a tube, the

moduli of elasticity and the yield strengths of intermediate specimens were obtained by linear interpolation between the values for the two compressive specimens.

When a tube had only one compressive specimen but also one or more crinkling specimens or a tensile specimen not adjacent to a compressive specimen, the compressive yield strength of the crinkling specimen (or specimens) or of the tensile specimen was estimated by the procedure to be explained. The compressive yield strengths of intermediate specimens were then obtained by linear interpolation, as mentioned in the last paragraph.

The moduli of elasticity and the compressive yield strengths of all specimens between an end of a tube and the nearest specimen for which these quantities were known, or had been estimated, were taken equal to the values for the nearest specimen.

The following procedure was adopted for estimating the compressive yield strengths of crinkling specimens and of tensile specimens not adjacent to compressive specimens. If there were no variation in properties along the length of a tube, the compressive yield strength of such a specimen would be that of the compressive specimen. Denote this yield strength by X. In the case of a crinkling specimen the crinkling strength, fA, and the ratio of diameter to thickness, d/t, are known. modulus of elasticity, E, is assumed to be the same as that of the compressive specimen from the same tube, equation (2) may be solved for the compressive yield strength that the crinkling specimen would have had to have to satisfy equation (2). Denote this yield strength by X1. The "true" yield strength, S, for specimens between the crinkling specimen and the compressive specimen will presumably lie between X and X:

$$S = X \pm x_2 = X' \mp x_2' \tag{16}$$

Here x_2 and x_2 ! are corrections to be applied to X and X!, respectively, to bring about the equality (16). The values of x_2 and x_2 ! were determined as follows. Light sets of observations, on tubes having two compressive specimens each, indicated a maximum variation in compressive yield strength of 184 pounds per square inch per inch. Denote by x_1 the product of 184 and the distance between the middle of the compressive specimen and the middle of the crinkling specimen. The value of $\mathbf{x_1}$ is therefore the maximum value by which the yield strength of the crinkling specimen may be expected to depart from that of the compressive specimen. On the other hand, 28 observations on compressive specimens indicated a maximum departure of 0.020 in $\sigma_{\mathbf{A}}$ from the value given by equa-

tion (2). Denote by x_1 , the quantity $\frac{0.020}{\sigma_A}$ X¹. That is, x_1 is approximately the maximum value by which the yield strength of the crankling specimen may be expected to differ from the yield strength determined for it from equation (2). Now, assume the corrections x_2 and x_2 to be determined from equation (16) and the relation

$$\frac{x_2!}{x_2} = \frac{n_1 x_1!}{n_1! x_1} \tag{17}$$

where n_1 and n_1 are the numbers of observations in the two sets discussed: namely, 8 and 28, respectively. It is reasonable that $\frac{x_2!}{x_2}$ should vary, roughly, inversely as the numbers of observations in the two sets and

The procedure for estimating the compressive yield strength of a tensile specimen not adjacent to a compressive specimen is essentially the same as that just explained. In this case, X^{\dagger} is the yield strength from equation (1), and x_1^{\dagger} is 7350 pounds per square inch,

directly as the maximum errors in the two sets.

the maximum departure of an observed value from that given by equation (1). There were 17 observations for determining equation (1), so that $n_1^{-1} = 17$.

Elimination of x_2 and x_2 from equations (16) and (17) gives

$$S = X \frac{1 + \frac{n_1! x_1 X!}{n_1 x_1! X}}{1 + \frac{n_1! x_1}{n_1 x_1!}}$$
(18)

The application of the procedure to a specific case will be illustrated. The modulus of elasticity and the compressive yield strength of specimen lHD-C were 29.040.000 and 183,700 pounds per square inch, respectively. The crinkling strength of specimen lHD-A was 179.700 pounds per square inch and its ratio of diameter to thickness was 30.80. If $f_A = 179,700$, E = 29,040,000, and d/t = 30.80 are substituted in equation (2) and the equation is solved for the compressive yield strength, it is found to be 172,700 pounds per square inch. The corresponding value of \mathcal{O}_A is 1.041. Put

X = 183,700

 $X^1 = 172.700$

The distance between specimens 1HD-C and 1HD-A was 44.75 inches and therefore $x_1 = 184 \times 44.75 = 8234$. The value

of x_1 is $x_1' = \frac{0.020}{1.041} \times 172,700 = 3318$; $n_1 = 8$, and for the crinkling specimen $n_1' = 28$. Substitution of all these quantities in equation (18) and simplifying gives S = 173,800.

APPENDIX B

ELASTIC DEFLECTION OF BEAMS UNDER COMBINED LOAD

Suppose that a beam is loaded axially by equal and opposite compressive forces P and further that other forces, such as transverse forces, are applied to bend the beam. Then, the bending moment M at any cross section may be written as

$$M = M_{x} - P_{y} \tag{19}$$

where $M_{\mathbf{x}}$ is the portion of the moment produced by other than the axial load, and \mathbf{y} is the deflection. The equation of the elastic curve of the beam is

$$EI \frac{d^2y}{dx^2} = M \tag{20}$$

where I is the least moment of inertia of the crosssectional area about an axis normal to the plane of bending, and x is measured in the longitudinal direction of the beam. From equations (19) and (20)

$$\frac{d^2y}{dx^2} + \frac{P}{EI}y = \frac{M_X}{EI}$$

or, with $\frac{P}{EI} = \alpha$?

$$\frac{\mathrm{d}^2 y}{\mathrm{d}^2 x} + \alpha^2 y = \frac{\mathrm{EI}}{\mathrm{M}^2} \tag{S1}$$

If the beam is of uniform cross section, the integral of equation (21) may be written in the form

$$y = \frac{M_x}{p} + C_1 \sin \alpha x + C_2 \cos \alpha x$$

$$-\frac{1}{P}\left(\sin \alpha x \int \sin \alpha x \frac{dM_X}{dx} dx + \cos \alpha x \int \cos \alpha x \frac{dM_X}{dx} dx\right)$$
 (22)

from which

$$\frac{dy}{dx} = \frac{1}{P} \frac{dM_X}{dx} + C_1 \alpha \cos \alpha x - C_2 \alpha \sin \alpha x$$

$$-\frac{1}{P}\left(\cos \alpha x \int \cos \alpha x \frac{d^{2}M_{x}}{dx^{2}} dx + \sin \alpha x \int \sin \alpha x \frac{d^{2}M_{x}}{dx^{2}} dx\right) \quad (23)$$

where the arbitrary constants of integration, C_1 and C_2 , are to be determined from the boundary conditions. Equation (22) may also be written

$$M = -G_1$$
 Psin $\alpha x - G_2$ Pcos αx

+
$$\sin \alpha x \int \sin \alpha x \frac{dM_X}{dx} dx + c \cdot s \alpha x \int \cos \alpha x \frac{dM_X}{dx} dx$$
 (24)

It may be noted that the parentheses in equations (22) and (23) and the sum of the last two terms of equation (24) become zero if $M_{\rm X}$ is a linear function of x.

The constants of integration for a few conditions of loading will be determined. First take the symmetrical case of the tests: namely, two equal transverse loads each situated a distance $k\frac{1}{2}$ from the nearer support, where $0 \le k \le 1$, and 1 is the length of the beam. M_x is everywhere linear in x, and from equations (22) and (23) if M_b denotes the maximum moment produced by the transverse loads and the origin is taken at one end of the beam,

for
$$0 \le x \le k \frac{1}{2}$$
,

$$y = \frac{2M_bx}{Pk1} + C_1^{\dagger} \sin \alpha x - C_2^{\dagger} \cos \alpha x \qquad (25)$$

$$\frac{dy}{dx} = \frac{2M_b}{Pk1} + C_1! \alpha \cos \alpha x - C_2! \alpha \sin \alpha x \qquad (26)$$

for
$$k\frac{1}{2} \leq x \leq \left(1 - \frac{k}{2}\right)1$$
,

$$y = \frac{M_b}{P} + C_1 \sin \alpha x + C_2 \cos \alpha x \qquad (27)$$

$$\frac{dy}{dx} = C_1 \alpha \cos \alpha x - C_2 \alpha \sin \alpha x \qquad (28)$$

When x = 0, y = 0; and therefore $C_2^{l} = 0$. When $\dot{x} = k\frac{l}{2}$, $y_{equation(25)} = y_{equation(27)}$, and $\left(\frac{dy}{dx}\right)$ equation(26) $= \left(\frac{dy}{dx}\right)$ equation(28). Moreover, when $x = \frac{l}{2}$, $\frac{dy}{dx} = 0$. These conditions give

$$C_1$$
 sin $k\frac{1}{2}\alpha = C_1 \sin k\frac{1}{2}\alpha + C_2 \cos k\frac{1}{2}\alpha$

$$\frac{2M_{b}}{Pkl} + C_{1}^{t}\alpha \cos k\frac{l}{2}\alpha = C_{1}\alpha \cos k\frac{l}{2}\alpha - C_{2}\alpha \sin k\frac{l}{2}\alpha$$

$$0 = C_{1}\cos \frac{l}{2}\alpha - C_{2}\sin \frac{l}{2}\alpha$$

Elimination of C_1 ! from these equations, and solution for C_1 and C_2 give

$$C_1 = -\frac{2M_b}{Pk l\alpha} \sin k \frac{l}{2}\alpha \tan \frac{l}{2}\alpha$$

$$C_2 = -\frac{2M_b}{Pk l\alpha} \sin k \frac{l}{2}\alpha$$

The maximum bending moment occurs at the middle of the beam, and when the values just found for C_1 and C_2 , together with $x=\frac{1}{2}$, are substituted in equation (24), the maximum value for $\frac{M}{M_b}$ assuming elastic action, is

$$\frac{M}{M_b} = \frac{\sin \frac{1}{2} \sqrt{\frac{P}{EI}}}{k \frac{1}{2} \sqrt{\frac{P}{EI}} \cos \frac{1}{2} \sqrt{\frac{P}{EI}}}$$
(29)

It is from this equation that the abscissas of the points of figure 18 were computed.

For the case of a beam carrying a single concentrated load at the middle, k = 1:

$$\frac{M}{H_b} = \frac{\tan \frac{1}{2} \sqrt{\frac{P}{EI}}}{\frac{1}{2} \sqrt{\frac{P}{EI}}}$$
 (30)

For the case of a beam loaded by couples at the ends, k = 0:

$$\frac{M}{M_{\rm b}} = \sec \frac{l}{2} \sqrt{\frac{P}{EI}} \tag{31}$$

If l is replaced by 2l in equation (30), the case of a cantilever beam loaded by a concentrated force at the free end is obtained:

$$\frac{M}{M_{b}} = \frac{\tan l \sqrt{\frac{P}{EI}}}{l \sqrt{\frac{P}{EI}}}$$
 (32)

If l is replaced by 2l in equation (31), the case of a cantilever beam loaded by a couple at the free end is obtained:

$$\frac{M}{M_{\rm b}} = \sec l \sqrt{\frac{P}{EI}} \tag{33}$$

Finally, the maximum value of $M/M_{\rm b}$ for a uniformly loaded beam will be determined. In terms of the maximum moment $N_{\rm b}$ produced by the transverse load

$$M_{x} = 4M_{b} \frac{x}{l} \left(1 - \frac{x}{l}\right)$$

Equation (22) becomes

$$y = \frac{4Nb}{P} \frac{x}{i} \left(1 - \frac{x}{i} \right) + C_1 \sin \alpha x + C_2 \cos \alpha x + \frac{8Mb}{P l^2 \alpha^2}$$

and for the determination of C_1 and C_2 when x = 0 or l, y = 0:

$$C_{a} = -\frac{8Mb}{P l^{2}\alpha^{2}}.$$

$$C_1 = -\frac{8Mb}{Pl^2\alpha^2} \frac{1 - \cos \alpha l}{\sin \alpha l}$$

The maximum value of M/M_D is at the middle of the beam, and substitution in equation (24) of the values of C_1 and C_2 just found, together with $x=\frac{1}{2}$, gives

$$\frac{M}{Mh} = \frac{8EI}{Pls} \left(\sec \frac{l}{2} \sqrt{\frac{P}{EI}} - 1 \right)$$
 (34)

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TABLE I .- NOMINAL CROSS-SECTIONAL PROPERTIES OF TUBES

Symbol	Outside diameter, d (in.)	Thickness of wall, t (in.)	<u>ā</u> t	Cross-sectional area, A (sq in.)	Section modulus, Z (in.3)
L Y D F	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0.065 .035 .049 .065	15.4 35.7 30.6 23.1 12.5	0.1909 .1336 .2234 .2930 .5202	0.0419 .0395 .0785 .1008 .1664

TABLE II.- NECHANICAL PROPERTIES OF TUBES

	TENSION									
Speci- men	Youngts modulus, E (lb/sq in.)	0.002-off- set yield strength (lb/sq in.)	5/7 E-secant yield strength (lb/sq in.)	Tensile strength, Tensile of Tens	Elonga- tion in 2 in. (percent)					
1HL-T 2HL-T 3HL-T 3HL-T 5HL-T 5HL-T 1HY-T 2HY-T 2HY-T 1HD-T 2HD-T 2HD-T 2HD-T 2HF-T 2HF-T 1HA-T	29,400 x 10 ³ 28,700 29,200 29,000 29,100 30,000 29,300 29,400 29,200 28,800 28,500 28,500 28,500 28,500 28,500 28,500 28,500 28,500 28,500 28,500 28,500 28,500 28,500 28,500	168,000 165,000 168,000 168,000 171,000 176,000 158,000 156,000 172,000 181,000 175,000 167,000 167,000 167,000 167,000 169,000 167,000	168,000 165,000 168,000 169,000 171,000 176,000 158,000 156,000 173,000 181,000 175,000 167,000 167,000 162,000 174,000 170,000 168,000 156,000	181,000 179,000 183,000 183,000 185,000 192,000 164,000 187,000 185,000 178,000 174,000 183,000 171,000 187,000 187,000 187,000	9998655755-67551767					
T-AHS	29,300 29,400	159,000 161,000	160,000 162,000	178,000 181,000	8 8					

TABLE II.- MECHANICAL PROPERTIES OF TUBES (Concluded)

Specimen modulus, E (1b/sq in.) y str (1b/sq in.) 1HL-C 29,800 x 10 ³ 173 1HL-C1 29,800 173 2HL-C1 29,800 173 2HL-C1 29,800 173 3HL-C1 29,800 173 3HL-C1 29,500 174 4HL-C 29,700 183 5HL-C 30,000 173 6HL-C 29,800 186 1HY-C 30,000 183 2HY-C 29,900 183 3HY-C 29,900 183 3HY-C 29,200 173 1HD-C 29,200 173 2HD-C1 29,300 173 3HD-C1 29,300 183 3HD-C1 29,200 183 4HD-C 29,200 183 4HD-C1 29,200 183 4HD-C 29,200 183 2HF-C 29,800 184 2HF-C 29,800 184 2HF-C 29,800 184 2HF-C		COMPRESSION	•
1HL-01 29,800 173 2HL-C1 29,800 173 3HL-C1 29,200 174 3HL-C1 29,500 174 4HL-C 29,700 183 5HL-C 30,000 173 6HL-C 29,800 186 1HY-C 30,000 186 2HY-C 29,900 186 3HY-C 29,300 196 4HY-C 29,300 173 1HD-C 29,300 173 2HD-C1 28,700 186 3HD-C1 29,200 186 4HD-C 29,200 186 4HD-C1 29,200 186 1HF-C 29,400 186 2HF-C 29,800 187 3HF-C 29,200 173	Specimen	modulus, E	5/7 E-secant yield strength (lb/sq in.)
1HA-C 29,900 176 1HA-C1 29,800 166	HL-C1	29,800 29,800 29,800 29,500 29,500 29,700 30,000 29,800 29,900 29,300 29,300 29,300 29,300 28,700 28,900 28,900 29,800 29,800 29,800 29,800 29,800 29,800 29,800	179,000 173,000 175,000 179,000 176,000 176,000 178,000 186,000 186,000 186,000 190,000 196,000 173,000 184,000 177,000 185,000 181,000 182,000 184,000 181,000

TABLE III.- AXIAL TESTS

	 		,	,
Specimen	<u>đ</u>	Crinkling strength, fA (1b/sq in.)	l = Et Sdm	$ \sigma_{\underline{A}} = \frac{\underline{f}_{\underline{A}}}{\underline{S}} $
3HA-C 1HA-C1 1HA-C 2HA-A2 2HA-A3	12.11 12.25 12.29 12.33 12.33	194,600 191,000 194,700 198,800 200,800	15.64 15.96 15.56 15.49 15.14	1.154 1.151 1.144 1.160 1.145
2HL-C 5HL-C 3HL-C 4HL-C 1HL-A 1HL-C 1HL-C1 3HL-C1 2HL-C1 2HL-C1 6HL-C	15.23 15.26 15.27 15.28 15.30 15.31 15.31 15.32 15.35 15.40 15.40	194,100 200,100 194,300 204,300 198,900 195,200 198,300 192,300 192,300 194,700 195,300 204,300	11.92 11.77 11.61 11.40 11.85 11.71 11.65 12.06 11.82 11.57	1.112 1.119 1.102 1.120 1.129 1.096 1.108 1.114 1.118 1.093 1.101
SHF-C 1HF-A1 1HF-C 3HF-C1 3HF-A	22.97 23.15 23.17 23.23 23.23 23.23	198,800 197,700 200,100 183,800 187,900	7.36 7.21 7.04 7.68 7.60 7.38	1.079 1.074 1.062 1.074 1.085 1.068
1HD-A 3HD-C 1HD-C 4HD-C1 3HD-C1 2HD-C 2HD-C1 4HD-C	30.80 30.86 30.86 30.97 31.03 31.05 31.10	179,700 181,000 192,000 189,700 194,000 177,700 190,600 186,700	5.61 5.48 5.29 5.37 5.24 5.67 5.23 5.31	1.034 1.024 1.045 1.044 1.048 1.034 1.046
5HY-C 1HY-A 2HY-C 4HY-C 1HY-C 3HY-C	34.18 34.30 34.33 34.75 34.90 36.20	174,600 174,700 188,400 202,000 185,900 189,600	5.09 5.31 4.78 4.43 4.77 4.40	1.009 1.028 1.004 1.031 1.001

TABLE IV .- BENDING TESTS

			<u>i</u>	
Specimen	d t	Modulus of rupture fB (lb/sq in.)	l = Et Sdm	σ _B = <u>fgd_m</u> Sd
lHA-B lHA-Bl lHA-B2	12.29 12.30 12.31	256,000 256,900 265,600	15.56 15.70 15.82	1.382 1.402 1.410
1HL-B2 1HL-B 5HL-B 6HL-B	15.28 15.31 15.31 15.40 15.40	248,100 241,700 248,600 256,300 247,400	11.89 11.74 11.65 11.65 11.15	1.320 1.272 1.297 1.340 1.246
SHF-B1 SHF-B2 SHF-B2 1HF-B 3HF-B 3HF-B2 3HF-B1 1HF-B1	23.06 23.06 23.17 23.18 23.23 23.23 23.23	234,200 232,500 220,200 241,100 246,400 236,400 228,800 228,200 235,300	7.33 7.33 7.20 7.03 7.64 7.60 7.60	1.216 1.207 1.144 1.253 1.251 1.314 1.264 1.261
1HD-B 1HD-B1 . 4HD-B 2HD-B 2HD-B1	30.90 30.97 30.97 31.05 31.10	226,200 218,500 220,900 228,600 220,500	5.29 5.67 5.37 5.37 5.23	1.191 1.217 1.176 1.236 1.170
2HY-B 5HY-B 5HY-B	34.21 34.25 34.53 36.10	236,000 230,200 217,000 211,100	4.83 4.87 5.04 4.41	1.231 1.211 1.217 1.080

TABLE V .- TORSIONAL TESTS

Specimen	<u>đ</u> t	Modulus of rupture fr (1b/sq in.)	$\frac{\frac{1}{\delta_s}}{\frac{1}{\delta_s}} = \frac{\frac{1}{\delta_s}}{\frac{1}{\delta_m}}$	$\frac{\tau_{T} = \frac{f_{T}d_{m}}{Sd}}{Sd}$
lHA-Tol	12.29	109,600	2.787	0,595
lEA-To2	12.31	116,800	2.799	.641
lHA-To	12.31	111,900	2.773	.604
5HL-To	15.26	110,700	2.134	.579
1HL-To2	15.28	114,200	2.155	.611
1HL-To1	15.30	108,600	2.136	.574
1HL-To	15.31	112,700	2.117	.588
lHF-Tol	23.15	106,600	1.334	.558
lHF-To	23.16	102,900	1.308	.523
lHF-To2	23.17	101,800	1.327	.529
SHF-To	23.32	91,700	1.369	.507
1HD-Tol	30.80	95,800	1.007	.525
3ED-To	30.90	98,300	.975	.514
1HD-To	31.00	92,200	.975	.486
5HY-To	34.18	92,900	.921	.521
2HY-To	34.25	101,800		.533

TABLE VI .- COMBINED AXIAL AND BENDING TESTS

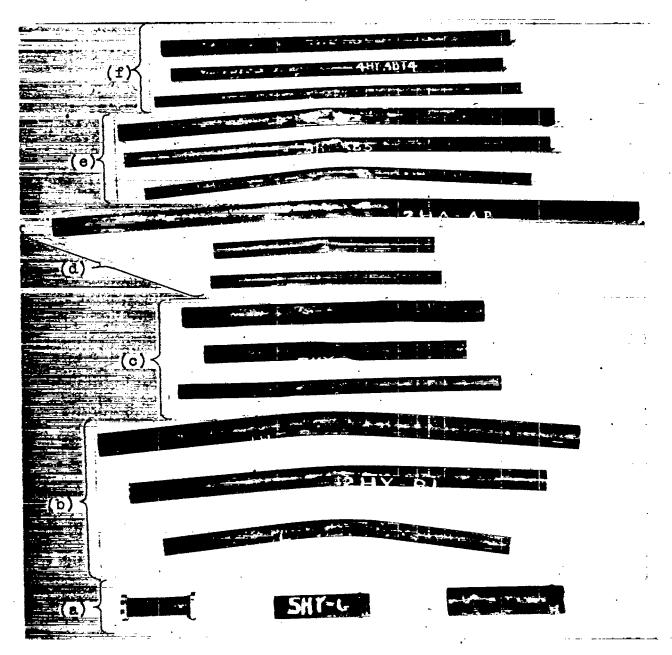
Specimen	<u>d</u> t	Axial stress, fa (1b/sq in.)	Bending stress, fb (Ib/sq in.)	1 8 Et Sd _m	σ _a = <u>f_a</u> S	$\sigma_b = \frac{\mathbf{f}_b \mathbf{d}_m}{\mathbf{S} \mathbf{d}}$	σ _a σ _A	σ <u>h</u> σ _B
2HA-AB3 2HA-AB2 2HA-AB 1HL-AB1 1HL-AB 2HF-AB1 1HD-AB1 1HD-AB1 1HD-AB2 2HY-AB1 2HY-AB3 2HY-AB3 2HY-AB2 2HY-AB2	12.30 12.31 12.31 15.32 15.32 15.32 22.88 22.94 30.74 30.86 30.97 34.30 34.30 34.30 34.30	12,500 39,700 4,800 84,500 43,600 52,600 58,600 63,500 23,900 48,100 142,800 125,400 120,800 117,100 90,500	227,800 191,400 234,500 90,700 187,400 163,500 157,700 132,000 209,300 171,600 35,800 52,200 73,100 71,800 92,000	15.15 15.14 15.14 12.03 11.98 7.37 5.60 5.150 5.150 5.25 5.01	.071 .226 .028 .488 .251 .286 .318 .366 .138 .277 .817 .768 .723	1.191 1.001 1.226 .490 1.008 .849 .819 .735 1.165 .955 .199 .311 .425 .408	.062 .199 .025 .441 .227 .265 .295 .352 .133 .267 .797 .741 .701 .666 .495	.871 .732 .897 .378 .777 .674 .651 .606 .962 .789 .167 .258 .394 .394

40

Specimen	đ t	Bending stress, f _b (lb/sq in.)	Torsional stress, ft (lb/sq in.)	$\frac{1}{\delta} = \frac{\mathbf{E}\mathbf{t}}{\mathbf{S}\mathbf{d}_{\mathbf{m}}}$	ES dm	σ _b = f _b d _m Sd	τ ₌ f _t d _m Sd.	$\frac{\sigma_{\mathbf{b}}}{\sigma_{\mathbf{B}}}$	<u> </u>
3HA-BT 3HA-BT1 3HA-BT2 2HA-BT 2HI-BT3 2HI-BT2 2HI-BT1 3HI-BT1	12.10 12.10 12.11 12.30 15.23 15.25 15.27 15.30 15.30 23.06 23.06 23.06 23.27 31.05 31.05 31.05 31.18 36.10 36.10 36.49 36.78	218,100 163,300 39,700 217,000 30,700 74,900 107,700 203,800 173,100 51,100 77,300 100,900 205,800 172,700 94,200 72,400 48,100 21,700 181,100 181,200 153,800 101,900 80,500 27,600	70,800 99,200 111,400 76,200 111,000 106,900 103,100 74,200 89,600 102,700 100,500 96,500 66,200 77,600 90,100 93,000 93,000 95,300 93,100 58,900 69,200 93,700 99,600 104,700	15.66 15.66 15.66 15.99 11.98 11.58	2.806 2.806 2.803 2.729 2.154 2.150 2.137 2.109 2.109 1.346 1.346 1.381 1.023 1.023 1.023 1.023 1.023 1.023	1.186 .888 .216 1.135 .164 .401 .571 1.080 .918 .265 .401 .524 1.150 .965 .531 .408 .271 .120 .992 .886 .927 .787 .522 .412	0.385 .536 .594 .594 .597 .597 .597 .597 .598 .598 .598 .598 .598 .598 .598 .598	0.853 .639 .156 .830 .126 .309 .441 .835 .709 .211 .319 .438 .336 .223 .099 .824 .736 .803 .453 .358 .123	0.621 .869 .977 .646 1.022 .985 .943 .679 .820 1.000 .979 .990 1.021 1.047 1.006 .634 .783 .603 .707 .958 1.020 1.072

TABLE VIII .- COMBINED AXIAL, BENDING, AND TORSIONAL TESTS

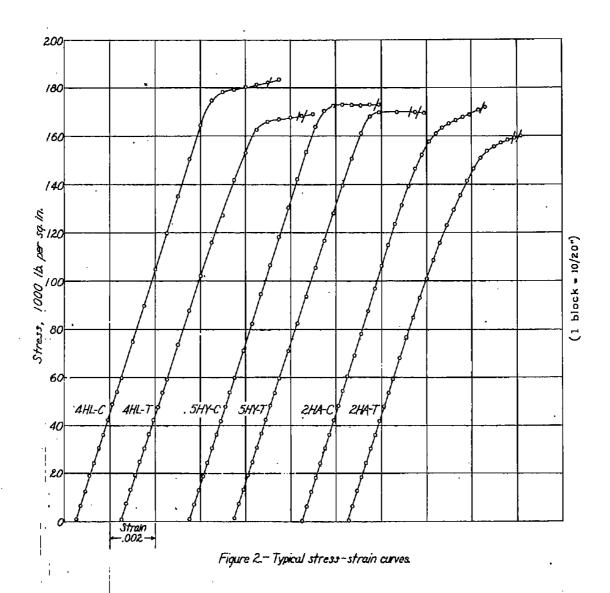
Specimen	d. t	Axial stress, fa (lb/sq in.)	Bending stress, fb (1b/sq in.)	Torsional stress ft (lb/sq in.)	1 = δ Et Sd _m	$\frac{\frac{1}{\delta_8}}{\left(\frac{E}{S}\right)^3} \frac{e}{d_m}$	σ _{a,} =	σ _b = f _b d _m Sd	τ _n f _t d _m Sd	σ _a .	$\frac{\sigma_b}{\sigma_B}$	<u>τ</u>
3HA-ABT1	12.20	29,400.	197,100	20,200	15.52	2.781	0.174	1.073	0.110	0.152	0.776	0.178
3HA-ABT	12.21	32,700	169,500	40,500	15.50	2.779	.194	.922	.220	.169	667	
-HHL-ABT1	15.26	13,800	177,300	43,000	11.41	2.091	.076	.908	.220	.069	.702	1 1
4HL_ABT	15.28	4,700	217,600	47,300	11.40	2.088	.026	1.115	243	.024	.862	
2HL-ABT	15.33	18,300	188,900	20,600	11.67	. 2.118	103	-993	108	.093	.767	186
3HL-ABT	15.33	12,900	197,800	20,800	11.61	2.111	.073	1.051	.1,11	.066	812	192
3HL-ABT2	15.40	1,300	238,600	31,000	11.72	2.121	.007	1.279	.166	.006	.987	287
3HL-ABT1	15.40	10,600	200,700	31,100	11.64	2.110	.060	1.071	166	.054	.827	287
3EF-ABT	23.23	19,100	180,100	39,800	7.60	1.375	.111	-995	.220	.103	.788	
4HD-ABT1	30.91	14,800	198,400	30,700	5.36	987	.082	1.059	.164	.079	882	
JED-ABT	30.93	53,400	144,600	40,600	5.41	•993	.298	.781	.220	.288	.649	
3HD-ABT1	30.99	38,500	203,300	40,200	5.31	979	21.1	1.075	213	.205	.897	.417
STEA-CH!	31.03	12,200	202,300	20,500	5.34	.983	067	1.079	109	- 065	.898	.213
TELA_CIEP!	31.03	25,200	194,000	30,900	5.32	.981	.140	1.037	.165	.136	.864	.323
4HD-ABT3	31.16	36,100	183,000	20,600	5.33	. 980	.199	975	.110	.193	.812	.215
THA-YH	34.71	29,100	171,200	52,500	4.43	.835	.148	.848	.260	148	•733	.518
4HY-ABI5	34.75	15,000	222,600	22,300	4.43	.835	.077	1.103	.111	.077	953	.221
fEA-YBL?	34.75	9,600	209,500	144,700	4.43	.835	-049	1.038	.221	.049	.897	· ##0
HHY-ABIL	34.78	4,300	201,600	22,300	4.43	.834	.ozz	•999	.110	.022	.863	-
HHY-ABT3	34.85	24,100	196,200	33,600	4.42	.832	.123	.972	.166	.123	•8#I	.331
STEA-AH	34.97	5,000	221,600	33,600	4.40	.829	.025	1.098	.167	.025	-951	-333



(a) Compressive specimens.
(b) Bending specimens.
(c) Torsional specimens.
(d) Combined axial and bending specimens.
(e) Combined torsional and bending specimens.
(f) Combined axial, bending, and torsional specimens.

Figure 1. Specimens after testing.





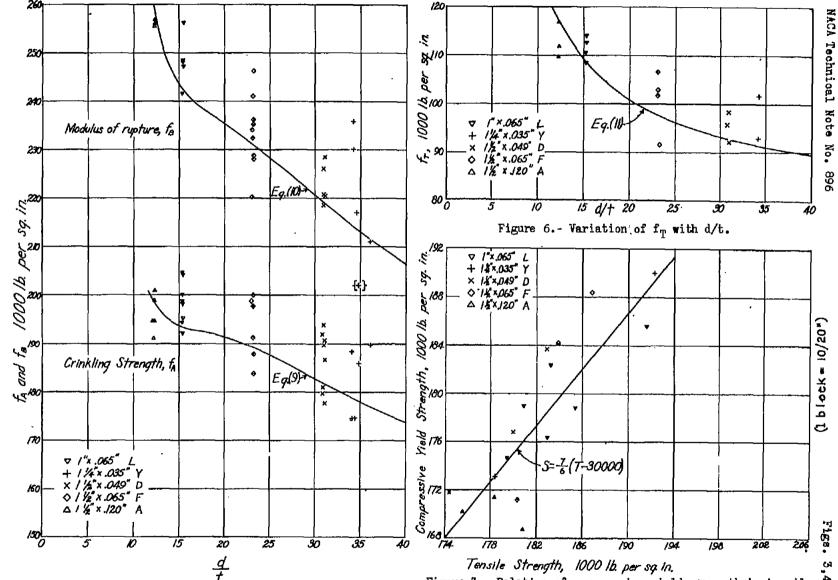
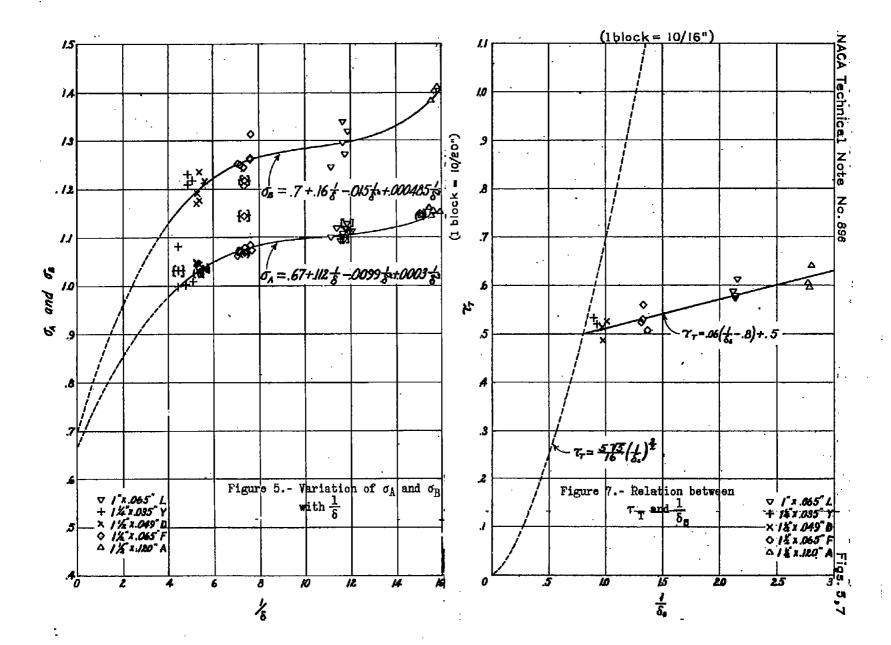


Figure 4.- Variation of \mathbf{f}_{A} and \mathbf{f}_{B} with d/t.

Figure 3.- Relation of compressive yield strength to tensile strength.



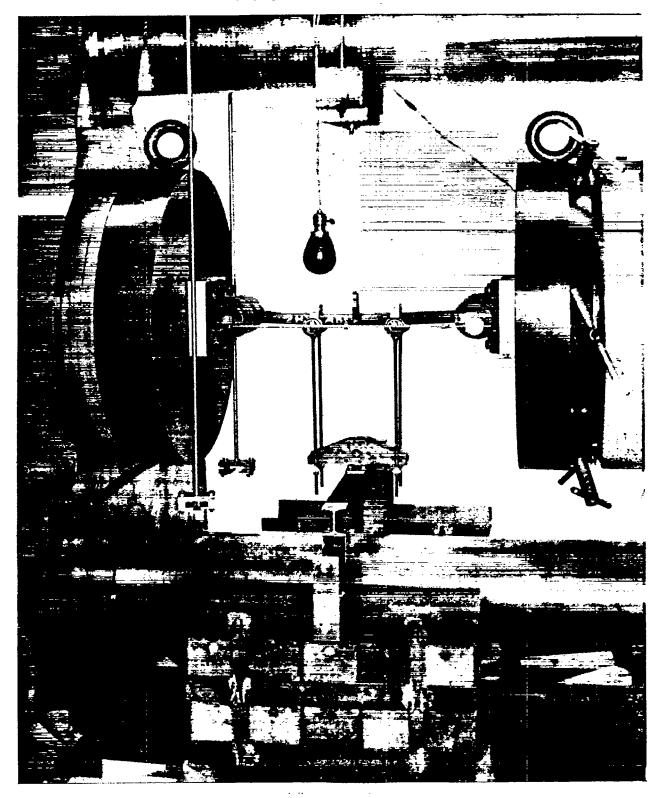


Figure 8.— Test setup for combined axial and bending tests.

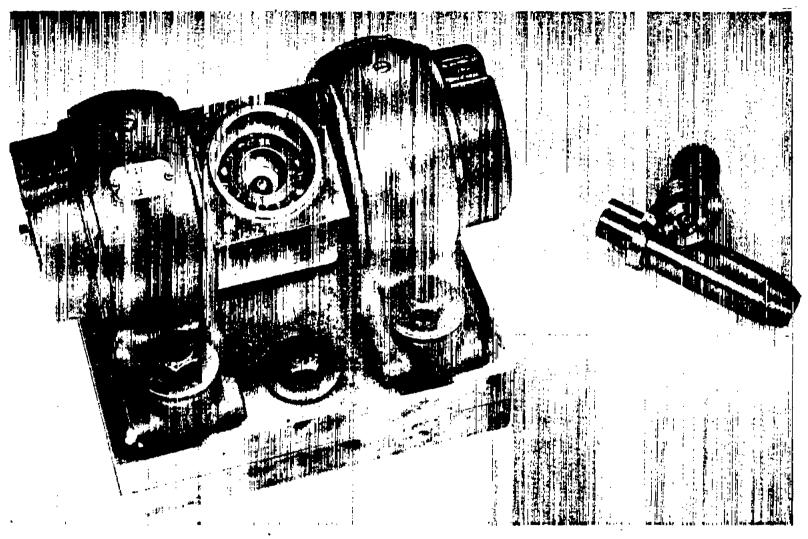


Figure 9. Pillow block assembly and plugs for combined axial and bending tests.





Figure 14.— Grips and plugs used for tests in combined bending and torsion.

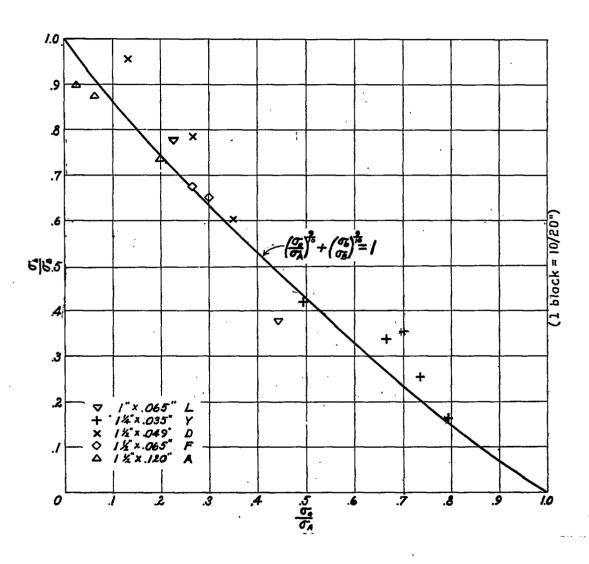


Figure 11.- Relation between $\frac{\sigma_b}{\sigma_B}$ and $\frac{\sigma_a}{\sigma_A}$.

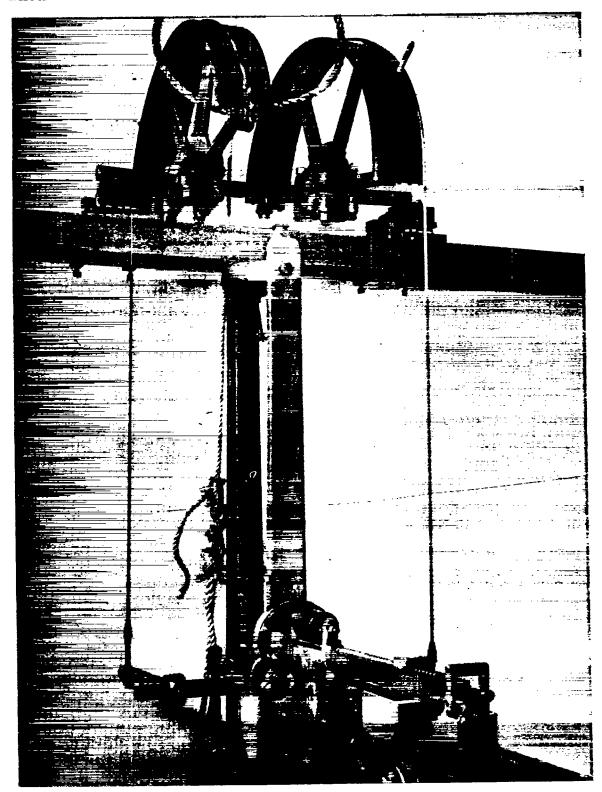


Figure 12. Setup for combined bending and torsional tests.

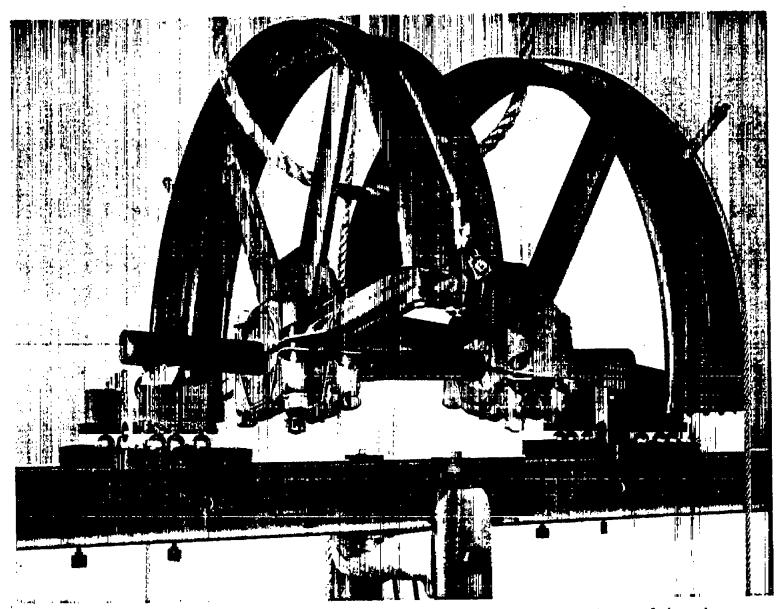


Figure 13.— Detail of test setup for tests in combined bending and torsion.

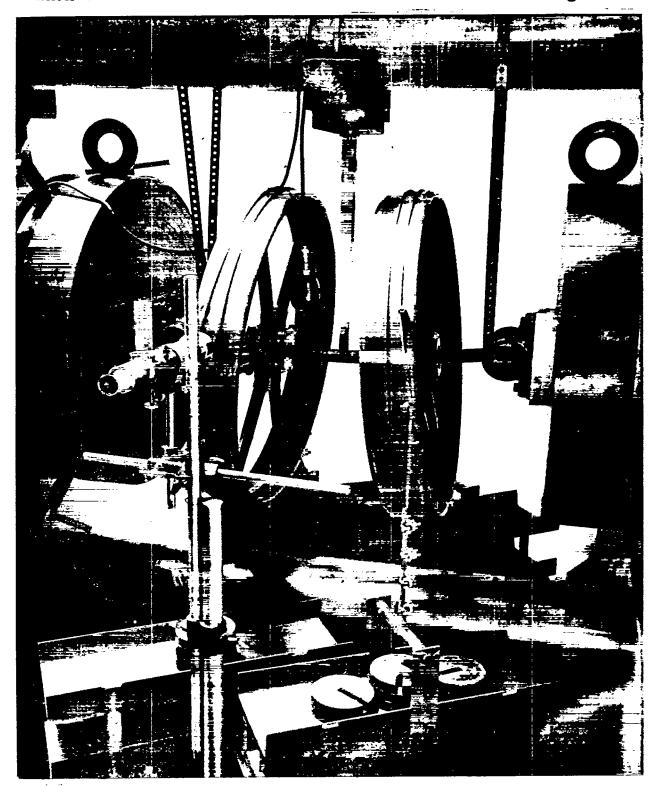


Figure 15.— Setup used in combined axial, bending, and torsional tests.

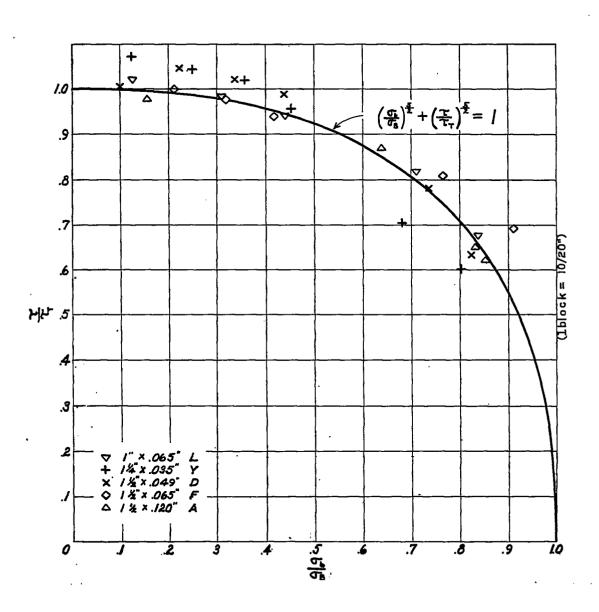
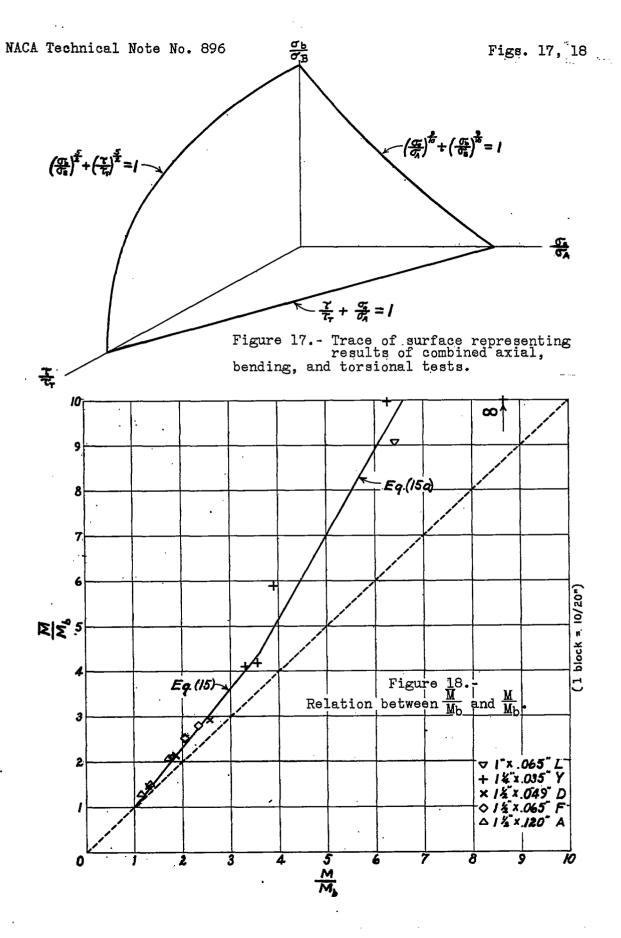


Figure 16.- Variation of $\frac{\tau}{\tau_T}$ with $\frac{\sigma_b}{\sigma_B}$.

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_	EDIH FOR 60 A (19 KG 47)	P-7-4-12 ATI- 8647				
	Obgood, W. R. Rac/// Author(s)	DIVISION: Stress Analysis and Structures (7) SECTION: Structural Testing (4) CROSS REFERENCES: Tubing, Structural - Strength (95267); Steel alloys - Strength (90408) REVISION				
	FORG'N. TITLE: A X	hest-treated chromium-molybdenum-ateel tubing under comined loads al Loads, Tubez, Stell National Advisory Committee for Aeronautics, Washington D. C.				
		GE FORG'N.CLASS U. S.CLASS. DATE PAGES ILLUS. FEATURES Restr. Jul'43 55 26 photos, tables, graphs				
	U. S. Eng. Restr. Jul'43 55 26 photos, tables, graphs ABSTRACT Round heat-treated Cr-Mo-Steel tubing was tested under axial load, bending load, torsional load, combined bending-axial load, combined bending-torsional load, and combined axial-bending-torsional load. Properties of material were determined by tensile and compressive tests. Practical example is solved showing procedure in designing tubular cantilever member to carry combined loads. Formulas are derived for evaluation of maximum strength of steel tubing under individual or combined loads.					
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TADIN FORM 69 A (13 Km2 47) DREADICATED P-7-4-12 ATT 8647 DIVISION: Stress Analysis and Structures (7) Osgood, W. R. ORIG AGENCY NUMBER SECTION: Structural Testing (4) TN-896 CROSS REFERENCES: Tubing, Structural - Strength (95267); Steel elloys - Strength (90L08) REVISION AUTHOR(S) AMER. TITLE: Round heat-treated chromium-molybdenum-steel tubing under combined loads FORG'N. TITLE. ORIGINATING AGENCY: National Advisory Committee for Aeronautics, Washington, D. C. TRANSLATION: COUNTRY LANGUAGE FORG'N.CLASS U. S.CLASS. | DATE | PAGES | ILLUS. FFATURES U. S. Eng. Restr. Jul'43 55 26 photos, tables, graphs ABSTRACY Round heat-treated Cr-Mo-Steel tubing was tested under exial load, bending load, torsional load, combined bending-axial load, combined bending-torsional load, and combined axial-bending-torsional loed. Properties of material were determined by tensile and compressive tests. Practical example is solved showing procedure in designing tubular cantilever member to carry combined loads. Formulas are derived for evaluation of maximum strength of steel tubing under individual or combined loads. NOTE: Requests for copies of this report must be addressed to: N.A.C.A.. Washington.

D. C.

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