DIGITAL IMAGE PHOTOGRAMMETRY

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FINAL REPORT

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DIGITAL IMAGE PHOTOGRAMMETRY

FINAL REPORT

by

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MARCH 1996

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ABSTRACT

A problem in modular shipbuilding is the lack of a reliable <u>and quick</u> method of obtaining and utilizing dimensional control in 3D. Photogrammetry has been successfully used as a tool for this application, but because of the large number of systematic errors associated with filmbased cameras only very large shipyards have attempted this. Recently, developments in Charge Coupled Device (CCD) imaging arrays for cameras have allowed some success in applying photogrammetric techniques *without film* in dimensional control. The software and hardware configurations have been expensive and complicated. Digital camera systems and computers were purchased and programmed to tie existing *inexpensive* software packages with Geometric Dilution of Control (GDOP) error propagation analysis originally designed for topographic mapping into a tool for production shipyard fabrication dimensional control.

Five separate digital photogrammetry applications were initiated (the first three were at Avondale Shipyards) consisting of:

1.) <u>Shell Bolster Model.</u> Photographs were taken of a scale model at Avondale and checked at the University of New Orleans (UNO). Images were imported to the Desktop Mapping System (DMS[®]) mensuration software. Photogrammetric analysis was performed using PC GIANT[©] software. The GDOP error analysis results appeared good, but initial reaction by Avondale personnel indicated that discrepancies existed. It was discovered that the discrepancies were due to the poor identification of the pin-prick targets utilized.

2.) <u>Double Hull Mid-body Tanker Section</u>. Plans were made to use the digital camera system in providing dimensional control after an existing ship stern was cut for later mating to a new mid-body section and bow. Results appear promising. Large (1¹/₄-inch diameter) "day glow" targets were used in daylight at a distance of approximately 88 feet with complete success.

3.) <u>Plate Shop / Factory.</u> There was concern at Avondale Shipyards with their numericallycontrolled flame cutting tables with respect to differential movement of large steel plates (1-inch thick x 20ft x 60ft) being cut. The remote control three-camera system was ideally suited for such an investigation to determine how much movement exists and when and where it occurs. Three cameras were set up and exposures were shot at 10-minute intervals for 2 hours; the period required to cut the subject steel plate. The results were inconclusive because of camera exposures of the target points. The electronic flashes were quite adequate for the distances (less than 200 feet), but the orientation of the target points (flat retro-reflective tape stickers) were at too shallow an angle to permit sufficient light to return to all of the cameras. Initial results of target design research can be improved upon by using magnets and ball-bearings painted with various retro-reflective materials.

4.) <u>"As Built" Industrial Site</u>. Wink Engineering collaborated with us with respect to an industrial "As Built" experiment which demonstrated ¼ - inch accuracy easily achieved over 30 feet. Retro-reflective targets (Scotchlite® tape) were used indoors with a electronic flash. The GDOP indicates that 30 feet is not a limiting size.

5.) <u>Tugboat Hull Offsets</u>. Collaborative efforts were facilitated through the assistance of A.K. Suda & Associates, Inc. for a project to determine "as built" hull offsets of a tugboat inside of a dry dock. The project was a success with only one-half day of field work. Retro-reflective targets (Scotchlite® tape) were used in daylight with a electronic flash. Accuracy achieved was 1/3 - inch accuracy in the X-Y plane (more or less parallel to the deck) and $\frac{1}{4}$ - inch accuracy in the Z component (vertical) for a vessel over 100 feet long.

The UNO results demonstrated that existing *inexpensive* topographic mapping software with GDOP error propagation analysis can be used with high-resolution CCD cameras for shipbuilding and industrial "3D As-Built" applications. It is recommended that work continue for target design, software to easily connect applications, and to develop a training package to facilitate technology transfer of inexpensive terrestrial photogrammetry software & techniques to the U.S. Shipbuilding Industry.



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INTRODUCTION

A major difficulty in the shipbuilding industry is the lack of a reliable <u>and quick</u> method of obtaining three-dimensional measurements of complex parts during fabrication and fitting to other parts. Photogrammetry has been successfully used as a tool for this application, but because of the large number of systematic errors associated with film-based cameras, only very large shipyards have attempted this because of the complexity of the film-based problem. The requirements have been for expensive and exotic photogrammetric instruments, expensive proprietary special-purpose software packages, heavy training requirements for a multidisciplinary staff, etc. Furthermore, film-based photogrammetric systems have tended to be on the slow end of the spectrum of dimensional-control systems. For <u>quick</u> turn-around time for results back to the workers in the shipyard, film-based photogrammetry has not been effective.

Recently, developments in Charge Coupled Device (CCD) imaging arrays for cameras have allowed some success in applying photogrammetric techniques without film in dimensional control. Previously classified technology for high-resolution CCD arrays has become available on the open market, but the existing film-based software has still been quite expensive. Digital camera systems (Kodak DCS 460) and computers were purchased and configured to tie existing inexpensive software packages with Geometric Dilution of Precision (GDOP) error propagation originally designed for topographic mapping into a tool for production shipyard fabrication dimensional control. The availability of GDOP is a critical distinction for photogrammetric software. Most photogrammetry packages, both in the public domain (free) as well as commercial, have only rudimentary indicators of adjustment quality (errors) and commonly give only root-mean-square (rms) values for the fit of object space control. PC GIANT® performs an error propagation analysis of the geometric dilution of precision for every point in the adjustment including the unknown points being solved. The presentation of the GDOP results in the form of eigenvectors/eigenvalues allows the shipyard analyst to inspect the accuracy of each and every individual point identified for fitting. Graphical screen plots of positional errors

presented as ellipses are an easy check to verify consistency of results; blunders and large errors become instantly evident. GDOP allows for a constant and consistent quality check for accuracy control.

The Kodak DCS 460 cameras are the most expensive component of the system developed. Presently, the DCS 460 costs approximately \$29,000 each, plus an additional \$10,000 to include all the requisite accessories (multiple lenses, radio remote-control, tripod, case, etc.). This total is equivalent to the price of a luxury sedan automobile in 1996. The reliability of the three cameras has been flawless except for one faulty battery that was replaced within 24 hours. The cameras seem to be completely acceptable for heavy day-to-day use in a shipyard environment.

However, the software will probably cost *less than \$3,000* per seat.



Figure 1

Kodak DCS 460 Camera

OBJECTIVE

The shipyard system will be capable of being used in production demonstrations as well as serving as a model configuration of components easily assembled by individual shipyards throughout the United States. The primary objective is to provide a demonstrable system that consists of standard (state-of-the-art) hardware components, standard (state-of-the-art) software components, and a minimum of customizing. Nothing in this research is especially new in concept except that technology has progressed in PC-based image processing, PC-based photogrammetry and digital camera design to the extent that old ideas that were extremely difficult to implement - are now well within reach of any shipyard in need of reliable, highvolume dimensional control. The system is intended to demonstrate that a single technician (with one or two helpers) can provide near real-time 3D dimensional control in a production shipyard environment. By minimizing the use of "dry dock" time, the competitiveness of U.S. shipyards will be enhanced with the most advanced CCD cameras available for unclassified applications.

SCOPE

The system is intended to be used in as many different application areas as possible that are pertinent to the U.S. shipbuilding industry within the time constraints of the project (1 year). Critical components to this scope were:

- 1. Research current work & present results of that research.
- 2. Purchase & acquire hardware/software.
- 3. Integrate complete system to operational status.
- 4. Collaborate with industry partners for possible projects.
- 5. Produce results.

METHOD OF PROCEDURE (METHODOLOGY)

The accuracies stated herein are as reported by the photogrammetric solution through the rigorous least squares adjustment of observed parameters and the computation of the geometric dilution of precision (GDOP). A variance-covariance matrix for each set of parameters is determined from the inverse of the normal equation. This is then multiplied by the estimate of variance of unit weight. The standard deviation for each element is the square root of the diagonal terms of that matrix.

The Variance of Unit Weight (σ_o^2) may be estimated by the equation:

$$\sigma_o^2 = \frac{\sum (v_j \ w_j \ v_j)}{(n-u)}$$

where

 v_i is the residual of the ith observation

w_i is the weight

n is the number of observations

u is the number of 'unknowns' or 'solvable parameters'

(n-u) is the degrees of freedom

In the photogrammetric problem the number (n) of observations is equal to the number of plate components; one for x and one for y, or two times the number of image points measured. Add to this the number of measurements for object space coordinates , one for each of the known components (X,Y,Z). Depending on the external source of information, camera station position (X_c,Y_c,Z_c) and orientation elements azimuth, elevation, swing (α ,h,s) as well; they can be added to the number of observations as six times the number of camera stations. Although these are considered as solvable parameters, they can also be treated as weighted observations if sufficient information is available. The unknowns or solvable parameters (u) are the object space control positions. For each unique point in the adjustment, three unknowns are counted. Camera station position (X_c, Y_c, Z_c) and orientation elements (α ,h,s) are commonly considered 'unknowns', giving rise to additional numbers of unknowns equal to six times the number of camera stations.

To summarize, let:

- v = the output residual for each observation.
- w = input weight which may be thought of as $1/\sigma^2$ for each observation.

(Note that 'weight' is the reciprocal of sigma squared.)

n = total number of observations.

m = 2 * number of plate measurements.

- c = 1 for each object space component.
- s = 6 * number of camera stations.

Factor 6 represents the camera parameters: the position coordinates (X_c, Y_c, Z_c) and orientation elements (α, h, s) . These parameters are always treated as unknowns; however, depending on the external source of information, these may also be treated as weighted observations contributing to the number of direct weighted observation equations. When the weights of the direct observations are small, the camera parameters may be treated as completely free and no contribution is then made to the direct weighted observations.

p = 3 * number of points (X_G,Y_G,Z_G). Note: one, two or three of these components may have also been counted as observations under 'c'.

Again simplistically, the estimate of variance of unit weight is defined as the summation of the input weights $(1/\sigma^2)$ multiplied by the output residuals squared (v^2) . If all is perfect,

$$\frac{\sum v^2}{\sigma^2} = (n - u)$$

for all observations. This summation, when divided by the <u>degrees of freedom</u> (the number of observations minus the number of parameters) results in a value close to 1.00.

For a two-dimensional case, we consider the bivariate normal distribution then for random error components only:

$$\left(\frac{x}{\sigma_x}\right) - 2\varrho\left(\frac{x}{\sigma_x}\right)\left(\frac{y}{\sigma_y}\right) + \left(\frac{y}{\sigma_y}\right)^2 = (1 - \varrho^2) c^2$$

This represents a family of error ellipses centered on the origin of the X,Y coordinate system. When c = 1, this is the *standard error ellipse*. The size, shape and orientation of the standard error ellipse are governed by the distribution parameters σ_x , σ_y , and ϱ .



Six examples illustrating the effects of different combinations of error distribution parameters are shown in the figures to the left. Note that these figures represent the various effects of a bias as the result of the least squares adjustment of random error. What we desire is a result equivalent to ellipse (a) - no bias such that the error figure is equal in all directions - a circle. The further we depart from a circle, the less desirable the result in that a significant bias is displayed. Ellipse (f), then, is the least desirable for a position determination. The Shipbuilder is given a quality check tool that on the surface can be viewed as a subjective criterion. In fact, the choice of the appropriate math model for the photogrammetric

adjustment offers a solid mathematical foundation for the graphical review of "goodness of fit". In surveying, we acknowledge that all measurements are made with some degree of error. With an error propagation of the geometric dilution of precision (GDOP) in a 3D analytical photogrammetric adjustment of observations, we obtain a realistic estimate of the reliability of our measurements. There is less reliance on "experience" and a greater assurance of an objective estimator of the quality of the observations, quality of dimensions and quality of the fabrication accuracy control.

A typical standard error ellipse in the X-Y plane is shown below.



Since c = 1, the imaginary box (broken line) that encloses the ellipse has half-dimensions σ_x and σ_y . In general, the principal axes of the ellipse, x' and y' do not coincide with the coordinate axes X and Y; the major axis of the ellipse, x' makes an angle θ with the X-axis. A positional error is expressed in the x,y coordinate system by random vector [X',Y']. The orthogonal (rotational) transformation which relates the two vectors is

$$\begin{bmatrix} X' \\ Y' \end{bmatrix} = \begin{bmatrix} \cos \theta & \sin \theta \\ -\sin \theta & \cos \theta \end{bmatrix} \begin{bmatrix} X \\ Y \end{bmatrix}$$

where θ is the angle of rotation.

Now the covariance matrices for random vectors

$$\begin{bmatrix} X \\ Y \end{bmatrix}, \begin{bmatrix} X' \\ Y' \end{bmatrix}$$
$$\begin{bmatrix} \sigma_x^2 & \sigma_{xy} \\ \sigma_{xy} & \sigma_y^2 \end{bmatrix}, \begin{bmatrix} \sigma_x^2 & 0 \\ 0 & \sigma_y^2 \end{bmatrix}$$

are

respectively. The off-diagonal terms in the covariance matrix for $\begin{bmatrix} X' \\ Y' \end{bmatrix}$ are zero because X' and Y' are uncorrelated (x' and y' are the principal axes of the ellipse).

Applying the general law of propagation of variances and covariances to the vector relationship given above, we get:

$$\begin{bmatrix} \sigma_{\dot{x}}^2 & 0 \\ 0 & \sigma_{\dot{y}}^2 \end{bmatrix} = \begin{bmatrix} \cos \theta & \sin \theta \\ -\sin \theta & \cos \theta \end{bmatrix} \begin{bmatrix} \sigma_x^2 & \sigma_{xy} \\ \sigma_{xy} & \sigma_y^2 \end{bmatrix} \begin{bmatrix} \cos \theta & -\sin \theta \\ \sin \theta & \cos \theta \end{bmatrix}$$

Multiplying the matrices and equating corresponding elements we obtain

$$\sigma_{\dot{x}}^{2} = \sigma_{x}^{2} \cos^{2} \theta + 2\sigma_{xy} \sin \theta \cos \theta + \sigma_{y}^{2} \sin^{2} \theta$$
$$\sigma_{\dot{y}}^{2} = \sigma_{x}^{2} \sin^{2} \theta - 2\sigma_{xy} \sin \theta \cos \theta + \sigma_{y}^{2} \cos^{2} \theta$$
$$0 = (\sigma_{y}^{2} - \sigma_{x}^{2}) \sin \theta \cos \theta + \sigma_{xy} (\cos^{2} \theta - \sin^{2} \theta)$$

Substituting (1/2) sin 2 θ for sin θ cos θ , and cos 2 θ for (cos² θ - sin² θ), we obtain

$$\frac{1}{2} (\sigma_y^2 - \sigma_x^2) \sin 2\theta + \sigma_{xy} \cos 2\theta = 0$$

from which we get

$$\tan 2\theta = \frac{2\sigma_{xy}}{\sigma_x^2 - \sigma_y^2}$$

The quadrant of 2θ is determined in the usual way from the signs of the numerator $2\theta_{xy}$ and denominator $(\sigma_x^2 - \sigma_y^2)$. Eliminating θ results in the following expressions for the variances of X' and Y':

$$\sigma_{\dot{x}}^{2} = \frac{\sigma_{x}^{2} + \sigma_{y}^{2}}{2} + \left[\frac{(\sigma_{x}^{2} - \sigma_{y}^{2})^{2}}{4} + \sigma_{xy}^{2}\right]^{\frac{1}{2}}$$
$$\sigma_{\dot{y}}^{2} = \frac{\sigma_{x}^{2} + \sigma_{y}^{2}}{2} - \left[\frac{(\sigma_{x}^{2} - \sigma_{y}^{2})^{2}}{4} + \sigma_{xy}^{2}\right]^{\frac{1}{2}}$$

The standard deviations σ_{x} , and σ_{y} are the *semimajor axis* and *semiminor axis*, respectively, of the standard error ellipse.

It can be demonstrated that the variances $\sigma_{x'}^2$ and $\sigma_{y'}^2$ are the *eigenvalues* of the covariance

matrix of the random vector $\begin{bmatrix} X \\ Y \end{bmatrix}$.

For the three-dimensional case as provided by **PC GIANT**[©], the eigenvectors are provided in the form of a 3X3 matrix of direction cosines for each point and the eigenvalues are provided for each component ($\sigma_{x'}, \sigma_{y'}, \sigma_{z'}$). Graphics software provides 2-D views for the X-Y plane, X-Z plane and the Y-Z plane. Research results of the industry survey indicated the following equipment used by shipyards:

EQUIPMENT	SHIPYARD
THEODOLITE	AVONDALE
	NORFOLK NAVAL SHIPYARD
	NEWPORT NEWS
	BATH IRON WORKS, CORP.
TRANSIT	NEWPORT NEWS
	BATH IRON WORKS, CORP.
LEVEL	NEWPORT NEWS
OPTICAL MICROMETER	BATH IRON WORKS, CORP.
ACMAN	INGALLS
	NASSCO
LASER INTERFEROMETER	AVONDALE
	NEWPORT NEWS
	NASSCO
TOTAL STATION	NEWPORT NEWS
DIGITAL PHOTOGRAMMETRY	NORFOLK NAVAL SHIPYARD
	NEWPORT NEWS
	BATH IRON WORKS, CORP.
HARDCOPY PHOTOGRAMMETRY	NORFOLK NAVAL SHIPYARD
	NEWPORT NEWS
	BATH IRON WORKS, CORP.

Active participation with Avondale Shipyards commenced the last week of the third Quarter. Three (3) separate Digital Photogrammetry applications have been initiated at the request of Avondale, consisting of:

Shell Bolster Model.

(Figure 2).

Photographs were taken of a scale model at Avondale and checked at UNO. Photos were overexposed (photographer error), and the model was later delivered to UNO for a second photography session. The exposures were correctly done the second time, with good geometry and good tonal range



Figure 2 Shell Bolster Model

imported to the Desktop Mapping System (DMS[®]) software, and the majority of the point selection was done by Mr. Shannon Doliese, Director of Accuracy Control at Avondale Shipyards. Photogrammetric analysis was later performed by the Principal Investigator and Dr. Michael E. Pittman, Consultant. The analysis results appeared good, but initial reaction by Avondale personnel indicated that discrepancies existed.

The actual targets were holes made in the surface of the model by a drafting compass needle. The sizes of the holes varied under magnification, the material around many of the holes were craterous (Figure 3), and when the results of the photogrammetric analysis were perused, the

Images were



Figure 3 Example of Marked Points

units were expressed at full scale. We believe that whatever discrepancies do exist are due to the difficulty in the identification of the photogrammetric targets available. The preparation of the model *was intended* for mechanical 3D digitization which was used with acceptable results. Although a different method of marking targets might be used in the future for such models, the use of digital photogrammetry is probably inappropriate when mechanical 3D digitizers are accessible.

<u>Double Hull Mid-Body Tanker Section.</u> Informational photographs were taken of a mid-body section under fabrication at Avondale Shipyards (Figure 4).

Plans have been made to use the digital camera system in providing dimensional control after an existing ship is cut for later mating to the new mid-body section. As of the end of the period of funded research for this project, the existing ship stern has just been photographed in the dry dock at Avondale Shipyards (Figure 5). Tests were made for target visibility with



Figure 4 Double Hull Mid-Body Section



Figure 5 Stern Section in Dry Dock - General View

excellent results. Camera distance was about 88 feet from the mating surface of the stern section, and a 28mm wide-angle lens was used. This particular focal length of lens was chosen because of the physical constraints imposed by the size of the interior of the dry dock. Targets were Avery Color Coding Labels (1¼"

Round). The beige ship color required "Red Glow" (Avery 05497-2020RG) for contrast.

A return trip was made to the dry dock and Avondale Shipyards made a cherry picker available for the photography session (Figure 6). Avondale personnel placed the "Red Glow" target stickers (one hour) at the locations where coordinates were desired by the Accuracy Control Section. Photos were taken at nine locations with 100% overlap such that



Figure 6 Stern Section - Survey Photo

practically every control point and unknown point ("pass point") appeared in each of the nine convergent photos (one hour). Resulting accuracies were $X = \pm 0.016$ inches, $Y = \pm 0.433$ inches, $Z = \pm 0.014$ inches (four hours for analysis) and were deemed acceptable (Appendix A).

<u>Plate Shop / Factory.</u> There is some concern at Avondale Shipyards with the numericallycontrolled flame cutting tables with respect to differential movement of large steel plates (1 inch thick x 20 ft x 60 ft) being cut. Sometimes these steel plates move during cutting, othertimes they don't. These plates may move a lot or a little during cutting, then move back to where they were. The three-camera system with simultaneous remote control is ideally suited for such an investigation to determine how much movement exists and when & where it occurs. A visit to the Plate Shop / Factory was made and control was established by Avondale Accuracy Control Department.

Three cameras were set up, and 3 simultaneous exposures were shot at 10minute intervals for 2 hours, the period required to cut the subject steel plate (Figure 7). The results were inconclusive because of camera exposures of the target points. The Nikon "SB-26" electronic flash units were quite adequate for the distances (less than 200 feet), but the



Figure 7 Plate Shop / Factory

orientation of the target points (flat retro-reflective tape stickers) were at too shallow an angle to permit sufficient light to return to the camera. (Stickers that were oriented perpendicular to the camera & strobe lights showed up with spectacular light returns at distances exceeding 200 feet.) Experiments were initiated to develop retro-reflector targets that would be adequate for such distances and for any angle of incidence. Initial results of target design research can be improved upon by using magnets and ball-bearings painted with various retro-reflective materials (Figure 8).



Figure 8 Spherical Targets with Magnets

Initially, ball bearings were painted with 3M Scotchlite[™] "7216 White Reflective Liquid". The quality of the targets was considered poor because of the viscous nature of the liquid coating. On recommendation from a professional sign painter's store the targets were then painted with Zinsser[®] Bulls Eye 1-2-3 white primer. In an attempt to replicate the aluminum layer of Scotchlite[™] reflective tape, the targets were then spray painted with a splattered aluminum paint. The targets were then spray painted with 3M Photo Mount[™] spray adhesive and coated with spherical glass beads. The resultant targets appear promising.

In addition to the three projects initiated with the collaboration with Avondale, two additional projects were completed with potential for shipbuilding applications: one in collaboration with Wink, Inc. (industrial or "as built"), the other with A.K. Suda, Inc. (ship repair hull offsets).

<u>Industrial "As-Built 3D CAD</u> <u>Model."</u> An industrial facility under construction was chosen for a pilot project, and was targeted and surveyed (two hours) by two Wink surveyors (Figure 9). The target points were flat retro-reflective tape stickers (circular) with r e c t a n g u l a r t a b s attached for ID notes (one



Figure 9 Industrial Site Demonstration

hour) (Figure 10). The Wink control consisted of approximately 12 points surveyed to an accuracy of better than 1/16 inch in X-Y-Z. The photogrammetric solution included 19



Figure 10 Industrial Site - Target Resolutions

photographs with 2 different focal length lenses. Results were satisfactory and were generally within the requisite accuracy of ¼ inch in X-Y-Z. The computed coordinates were delivered in the form of a final report. The photogrammetric solution took 16 hours. Retrofitting new equipment into an existing engine room is an obvious application of this

easily-implemented technique. The site survey requires only the technician and the camera.

5.) <u>"As-Built" Tugboat Hull Offsets.</u> A.K. Suda, Inc., *Consulting Naval Architects*, needed to determine the "as-built" dimensions of an existing tugboat (M/V J.K. McLean) in order to compute the stability characteristics of the vessel. Desired overall accuracy was $\pm \frac{1}{2}$ inch for all three components (X-Y-Z), and speed of measurement was a major concern in order to *minimize the charges for dry dock rental time* (Figure 11).



Figure 11 M/V J.K. McLean (tugboat)

The vessel was available at 12:30 pm, and three men started targeting the bulkhead locations with 13/32 inch diameter Scotchlite^m reflective tape. The targeting operation took a total of $4\frac{1}{2}$ hours. Four object space control points were surveyed with the aid of a 100 foot steel tape and an automatic level. The X-Y-Z control was completed in 15 minutes. A total of 52 photographs were taken with electronic flash in 15 minutes. Total dry dock time was 5 hours. Of the 52 photos taken, 26 were actually used in the photogrammetric analysis. Photogrammetric analysis time totalled 48 hours because of two blunders - one blunder in the reduction of the object space control points (approximately one foot), one blunder because of duplicate point identifications assigned during the measurement phase. Thirty seven hours were wasted because of human error; actual productive work *would have taken* about 12 hours *if* there were no blunders. Final accuracy was ±0.33 inches in X (lengthwise along the keel), ±0.0.35 inches in Y (width offsets perpendicular to the keel) and ±0.20 inches in Z (vertical). Note that the blunders were made in the office and were corrected in the office.

CONCLUSIONS

Digital Image Photogrammetry is a system that is reliable and easily implemented with "offthe-shelf" equipment and inexpensive topographic mapping software. Higher accuracies can be obtained by modeling more sources of systematic error such as lens distortion. Greater functionality can be obtained from the system by customizing the topographic mapping software to a more specific shipbuilding context; specifically with respect to units of measurement and reference conventions. A phototriangulation software package that computes the error propagation of the Geometric Dilution of Precision is a necessity for reliable production Quality Checks.

RECOMMENDATIONS

The Digital Image Photogrammetry system should be promoted as a 'Low-Cost' investment to change current methodology in dimensional control.

Software should be developed that will easily adapt inexpensive topographic software applications to industrial applications in use in shipyard production environments.

A documented series of "successes" with industrial collaborators should serve as instructional material useful in spreading the gospel of easy, low-cost dimensional control using digital image photogrammetry.

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21

APPENDIX A

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Stern Section of Double-Hull Tanker

PC GIANT[®] Solution Output

PC-Giant (c)Copyright 1990-1994

05/16/96 16:44:26.33

GPA Associates UNO Box 1200 New Orleans, LA 70148 (504) 286-1200

AHLK Stern - AVONDALE SHIPYARDS, INC.

Object Space Reference System is Rectangular (Close-Range) Rotation angles are Terrestrial Object-to-Photo Complete Triangulation process is requested Error Propagation is requested [Eigenvector/Eigenvalue output] Unit Variance will be based on constrained camera parameters All Image Residuals will be listed Triangulated Object Coordinates will be saved Adjusted Camera Station Parameters will be saved Images are in millimeters. Object space is in feet



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PAGE 1

FRAME lft_bot

PRINCIPAL DISTANCE = -28.0000 mm Std. Dev. of X = 0.0060 mm Std. Dev. of Y = 0.0060 mm

CAMERA STATION PARAMETERS

	P	Ο S Ι Τ Ι Ο Ν	Std. Dev.	A T T I T U D E (Terrestrial->Ph)	Std. Dev.
x	=	-45.8370 ft	10.0000 ft	Azim. = 224840.42700	1 00 00.0000
Y	=	-88.8940 ft	10.0000 ft	Elev. = 09 37 33.5650 0	1 00 00.0000
Z	=	7.2940 ft	10.0000 ft	Swing = - 01 59 53.6020 0	1 00 00.0000

PLATE COORDINATES in millimeters

ID	x	Y	ID	x	Y
31	-5.4810	-5.1510	32	-7.2370	-2.8060
33	-7.1980	0.3850	34	-7.1570	3.5010
35	-7.0850	6.9400	. 25	-10.9280	7.7080
21	-9.0260	-5.1290	22	-9.9060	-2.8110
23	-10.4720	0.5040	10	2.4860	-6.8020
11	2.4080	-5.0560	51	10.8130	-4.4220
52	11.1470	-2.6870	53	11.2720	-0.2960
54	11.2670	2.0410	43	9.3980	-0.2120
42	9.6450	-2.6980			

FRAME lft_mid

PRINCIPAL DISTANCE = -28.0000 mm Std. Dev. of X = 0.0060 mm Std. Dev. of Y = 0.0060 mm



CAMERA STATION PARAMETERS

	PO	SITION	ſ	Std. Dev.	; (*	A T Ferr	T I esti	T (ria	JDE 1->Ph)		Stċ	l. Dev.
x	=	-41.2960	ft	10.0000 f	t Azim.	=	20	03	27.0900	01	00	00.0000
Y	=	-91.3140	ft	10.0000 f	t Elev.	=	00	46	42.5660	01	00	00.0000
z	=	22.6930	ft	10.0000 f	t Swing	= -	01	37	30.0630	01	00	00.0000

PLATE COORDINATES in millimeters

ID	x	Y	ID	x	Y
31	-5.5840	-5.5080	32	-7.3810	-3.3280
33	-7.4450	-0.1560	34	-7.5110	3.0670
35	-7.5710	6.7480	25	-11.6630	7.0610
21	-9.0470	-5.6670	22	-10.0080	-3.4810
23	-10.7320	-0.2280	24	-11.2650	3.1040
10	2.2930	-6.6980	11	2.2510	-5.0210
41	8.6650	-4.2400	42	9.7550	-2.2770
43	9.6730	0.2500	44	9.5900	2.7980
55	12.5500	5.6030	52	11.3340	-2.1790
53	11.6310	0.2830	54	11.8000	2.7540

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FRAME lft_top

PRINCIPAL DISTANCE = -28.0000 mm Std. Dev. of X = 0.0060 mm Std. Dev. of Y = 0.0060 mm



CAMERA STATION PARAMETERS

	P	05	ΙΤΙΟΙ	N	Std. Dev.	A T T (Terre:	I T (strial	JDE L->Ph)		Std.	Dev.
x	=		-40.0240	ft	10.0000 ft	Azim. =	20 21	54.5760	01	00 0	0.0000
Y	=		-87.7250	ft	10.0000 ft	Elev. $= -$	11 24	37.7720	01	00 0	0.0000
Z	=		42.0950	ft	10.0000 ft	Swing = (00 18	05.5000	01	00 0	0.0000

PLATE COORDINATES in millimeters

.

TD	X	Y	ID	x	Y
10	1.8730	-6.0740	11	1.9120	-4.5260
31	-5.8000	-5.3200	32	-7.6190	-3.3490
33	-7.7640	-0.2450	34	-7.9060	3.0570
21	-9.2120	-5.6160	22	-10.2470	-3.6110
23	-11.1360	-0.4660	24	-11.8710	2.9400
25	-12.5440	6.8990	42	9.5540	-1.5970
43	9.7380	0.8890	44	9.9350	3.4910
41	8.2870	-3.5220	52	11.1320	-1.4400
53	11.7240	1.0000	54	12.2060	3.5380

FRAME cen_bot

PRINCIPAL DISTANCE = -28.0000 mmStd. Dev. of X = 0.0060 mm Std. Dev. of Y = 0.0060 mm



CAMERA STATION PARAMETERS

POSITION		Std. Dev.	A T T I T U D E (Terrestrial->Ph)	Std. Dev.
X =	0.0790 ft	10.0000 ft	Azim. = 359 18 51.1420	01 00 00.0000
Y =	-104.0160 ft	10.0000 ft	Elev. = 08 32 20.2960	01 00 00.0000
Z =	7.9370 ft	10.0000 ft	Swing = - 00 04 57.9490	01 00 00.0000

	P	LATE COORDINATE	S in millim	eters	
ID	x	Y	ID	x	Y
11	0.3330	-4.7540	31	-7.1690	-4.5640
32	-8.5600	-2.5600	33	-8.4410	0.1050
34	-8.3180	2.7470	35	-8.1890	5.6860
25	-11.6470	5.8350	· 21	-10.0380	-4.4460
22	-10.6500	-2.5630	23	-10.9990	0.0970
24	-11.1960	2.7190	41	7.8600	-4.5340
42	9.2510	-2.5090	43	9.1190	0.1770
44	8.9750	2.8140	45	8.8180	5.7630
55	12.3060	5.9520	51	10.7580	-4.4220
52	11.3720	-2.4960	53	11.7140	0.1810
54	11.8900	2.8160			

FRAME cen_mid

PRINCIPAL DISTANCE = -28.0000 mm Std. Dev. of X = 0.0060 mm Std. Dev. of Y = 0.0060 mm



CAMERA STATION PARAMETERS

	ΡO	SITION	N	Std. Dev.	A T (Ter	TITU restria	JDE l->Ph)		Std. Dev.
x	=	0.3390	ft	10.0000 ft	Azim. =	359 04	49.9800	01	00 00.0000
Y	=	-103.6610	ft	10.0000 ft	Elev. =	01 39	11.9560	01	00 00.0000
\mathbf{Z}	=	23.3170	ft	10.0000 ft	Swing =	00 18	59.7010	01	00 00.0000

PLATE COORDINATES in millimeters v ID Y

ID	x	Y	ID	x	Y
11	0.3400	-5.5330	31	-7.1210	-5.2680
32	-8.5610	-3.2600	33	-8.5230	-0.5580
34	-8.4790	2.1800	35	-8.4230	5.3090
21	-9.9710	-5.1200	· 22	-10.6550	-3.2450
23	-11.1100	-0.5410	24	-11.4130	2.1780
25	-12.0350	5.3360	41	7.8330	-5.3790
42	9.3250	-3.3800	43	9.3130	-0.6530
44	9.2930	2.1010	45	9.2630	5.2510
51	10.7420	-5.2930	52	11.4620	-3.3910
53	11.9580	-0.6700	54	12.2900	2.0780
55	12.9260	5.2640			

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FRAME cen_top

PRINCIPAL DISTANCE = -28.0000 mm Std. Dev. of X = 0.0060 mm Std. Dev. of Y = 0.0060 mm



CAMERA STATION PARAMETERS

	P	οςιτιοι	N	Std. Dev.	A T T I T U D E (Terrestrial->Ph)	Std. Dev.
x	=	0.2820	ft	10.0000 ft	Azim. = 359 11 24.0670 01	00 00.0000
Y	=	-103.6540	ft	10.0000 ft	Elev. = - 06 32 33.9820 01	00 00.0000
z	=	37.6500	ft	10.0000 ft	Swing = - 00 25 26.8810 01	00 00.0000

PLATE COORDINATES in millimeters

ID	x	Y	ID	x	Y
11	0.3620	-5.1690	31	-6.8810	-4.9950
32	-8.3720	-3.1140	33	-8.4870	-0.4910
34	-8.5950	2.2330	35	-8.7320	5.4450
21	-9.6420	-4.8890	22	-10.4270	-3.1190
23	-11.0550	-0.5050	24	-11.5550	2.2080
25	-12.4080	5.2730	41	7.6160	-4.9440
42	9.1160	-3.0350	43	9.1970	-0.3920
44	9.2680	2.3450	52	11.2050	-3.0200
53	11.8130	-0.3880	54	12.2780	2.3510
55	13.0850	5.4690	51	10.4280	-4.8470

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FRAME rht_bot

PRINCIPAL DISTANCE = -28.0000 mm Std. Dev. of X = 0.0060 mmStd. Dev. of Y = 0.0060 mm



CAMERA STATION PARAMETERS

	ΡO	SITIOI	N	Std. Dev.	A I (Ter	TI: restr:	FUDE ial->Ph)	4	Std. Dev.
x	=	30.5290	ft	10.0000 ft	Azim. =	344 (09 39.7560	01	00 00.0000
Y	=	-94.6690	ft	10.0000 ft	Elev. =	07 :	16 20.1220	01	00 00.0000
Z	=	10.2950	ft	10.0000 ft	Swing =	01 (04 00.4480	01	00 00.0000

PLATE COORDINATES in millimeters х v тΠ Y

x	Y	ID	x	Y
-1.1300	-6.5520	11	-1.0950	-4.8070
-8.0350	-4.3960	32	-9.1830	-2.4260
-9.0330	0.1520	34	-8.8750	2.7160
-8.7170	5.6000	21	-10.4490	-4.2110
-10.9210	-2.4150	23	-11.1570	0.1050
-11.2670	2.5940	25	-11.8910	5.5050
7.0040	-4.8300	42	8.7140	-2.5550
8.6470	0.5320	44	8.5660	3.5650
8.4780	6.9600	51	10.4740	-4.7770
11.3020	-2.5650	53	11.8190	0.5890
12.1480	3.7230	55	12.4410	7.4800
	X -1.1300 -8.0350 -9.0330 -8.7170 -10.9210 -11.2670 7.0040 8.6470 8.4780 11.3020 12.1480	XY-1.1300-6.5520-8.0350-4.3960-9.03300.1520-8.71705.6000-10.9210-2.4150-11.26702.59407.0040-4.83008.64700.53208.47806.960011.3020-2.565012.14803.7230	XYID-1.1300-6.552011-8.0350-4.396032-9.03300.152034-8.71705.600021-10.9210-2.415023-11.26702.5940257.0040-4.8300428.64700.5320448.47806.96005111.3020-2.56505312.14803.723055	X Y ID X -1.1300 -6.5520 11 -1.0950 -8.0350 -4.3960 32 -9.1830 -9.0330 0.1520 34 -8.8750 -8.7170 5.6000 21 -10.4490 -10.9210 -2.4150 23 -11.1570 -11.2670 2.5940 25 -11.8910 7.0040 -4.8300 42 8.7140 8.6470 0.5320 44 8.5660 8.4780 6.9600 51 10.4740 11.3020 -2.5650 53 11.8190 12.1480 3.7230 55 12.4410

FRAME rht_mid

PRINCIPAL DISTANCE = -28.0000 mm Std. Dev. of X = 0.0060 mm Std. Dev. of Y = 0.0060 mm



CAMERA STATION PARAMETERS

POS	ΙΤΙΟΝ	Std. I	ev.	AT (Terre	T I T U estrial	JDE L->Ph)	S	td. Dev.
X =	30.2710 f	t 10.000	0 ft Az	im. = 1	343 38	25.6440	01 0	0 00.0000
Y =	-92.5230 f	t 10.000	0 ft El	lev. =	00 30	11.2300	01 0	
Z =	22.9930 f	t 10.000	0 ft Sp	ding =	00 05	56.3430	01 0	

PLATE COORDINATES in millimetersIDXYIDXY

10	-0.8600	-6.8910	11	-0.8620	-5.1620
31	-7.8410	-4.5810	32	-9.1010	-2.5800
33	-9.0980	0.0570	34	-9.0870	2.7380
35	-9.0720	5.8130	21	-10.2690	-4.3460
22	-10.8520	-2.5300	23	-11.2530	0.0530
24	-11.5290	2.6460	25	-12.3950	5.6330
41	7.3410	-5.3630	42	9.1230	-3.1100
43	9.1170	0.0910	44	9.0980	3.3240
45	9.0630	7.0170	51	10.8780	-5.4020
52	11.7930	-3.1790	53	12.4370	0.0870
54	12.8910	3.4200	55	13.3620	7.3790

55

13.5360 6.2730

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FRAME rht_top

PRINCIPAL DISTANCE = -28.0000 mm Std. Dev. of X = 0.0060 mm Std. Dev. of Y = 0.0060 mm



CAMERA STATION PARAMETERS

POSITIO	ON Std. De	IV. A 1 (Tej	T I T U I rrestrial-:	D E >Ph)	Std. Dev.
X = 30.03'	70 ft 10.0000) ft Azim. =	343 43 23	3.4 760 01	00 00.0000
Y = -93.07'	00 ft 10.0000) ft Elev. =	- 09 23 10	0.0310 01	
Z = 41.42'	30 ft 10.0000) ft Swing =	- 01 19 49	5.1580 01	

	P	LATE COORDINA	ATES in millim	eters	
ID	x	Y	ID	, X	Y
10	0 5300	6 8080			
TO	-0.5/00	-6.7870	11	-0.6200	-5.2330
31	-7.3350	-4.5170	32	-8.6920	-2.6250
33	-8.8880	-0.1110	34	-9.0880	2.5240
21	-9.6840	-4.2350	22	-10.4000	-2.5270
23	-11.0240	-0.0500	24	-11.5380	2.5140
25	-12.6920	5.3670	41	7.1640	-5.6310
42	8.9110	-3.6270	43	8.9930	-0.6230
44	9.0690	2.5350	45	9.1490	6.3000
51	10.4980	-5.7700	52	11.4560	-3.7610
53	12.2270	-0.7260	54	12.8470	2.5200

	OBJE Po	CT CC sition	NTR	OLDA Std. Dev.	T A
10	X = Y = Z =	0.0000 0.0000 0.0000	ft ft ft	0.0050 0.0050 0.0050	TYPE = 0
11	X = Y = Z =	0.0000 0.0000 6.0000	ft ft ft	0.0050 0.0050 0.0050	TYPE = 0
34	X = Y = Z =	-33.0000 0.0000 34.2344	ft ft ft	0.0050 0.0050 0.0050	TYPE = 0
44	X = Y = Z =	33.0000 0.0000 34.2344	ft ft ft	0.0050 0.0050 0.0050	TYPE = 0

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	CAME	RA ST	TATIONS	CORRE	стіом	S
	P	оѕіті	0 N	A	ттгтт	D E
	x	Y	Z	Azim.	Elev.	Swing
			Iterat:	ion 1		
lft_bot	-0.0194	0.0181	-0.0187 ft	-0.000227	0.000230	0 000051
lft_mid	-0.0204	0.0139	-0.0241 ft	-0.000235	0 000260	0 0000000
lft_top	-0.0201	0.0089	-0.0243 ft	-0.000238	0 000200	-0 000015
cen bot	-0.0308	0.0039	-0.0258 ft		0.000229	-0.000013
cen mid	-0.0320	0.0002	-0.0298 ft		0.000240	0.000037
cen top	-0.0324	-0 0045	_0 0310 ft	-0.000297	0.000282	0.000022
rht bot	-0 0210		-0.0318 IL	-0.000293	0.000296	-0.000001
mhe mid	0.0210	-0.0072	-0.01/9 Ht	-0.000221	0.000172	0.000031
rnc_mia	-0.0206	-0.0092	-0.0197 ft	-0.000224	0.000208	0.000012
rht_top	-0.0208	-0.0131	-0.0223 ft	-0.000219	0.000249	-0.000016

Provisional Weighted Sum of Squares = 346.296

			I	terat:	ion 2		
lft_bot	-0.0002	0.0001	0.0000	ft	-0.000003	0.000000	0.00000
lft_mid	-0.0003	0.0002	0.0002	ft	-0.000003	-0.000002	0.000000
lft_top	-0.0003	0.0002	0.0000	ft	-0.000003	0.000000	-0.000001
cen_bot	-0.0004	0.0000	0.0000	ft	-0.000004	0.000000	0.000000
cen_mid	-0.0005	0.0000	0.0000	ft	-0.000005	0.000000	0.000000
cen_top	-0.0006	0.0000	0.0000	ft	-0.000005	0.000000	0.000000
rht_bot	-0.0004	-0.0001	-0.0001	ft	-0.000004	0.000001	0.000000
rht_mid	-0.0004	-0.0002	-0.0001	ft	-0.000004	0.000001	0.000000
rht_top	-0.0005	-0.0002	-0.0001	ft	-0.000005	0.000001	0.000000

Provisional Weighted Sum of Squares = 341.811

			Iteration	. 3		
lft_bot	0.0000	0.0000	0.0000 ft	0.000000	0.000000	0.00000
lft_mid	0.0000	0.0000	0.0000 ft	0.000000	0.000000	0.000000
lft_top	0.0000	0.0000	0.0000 ft	0.000000	0.000000	0.000000
cen_bot	0.0000	0.0000	0.0000 ft	0.000000	0.000000	0.000000
cen_mid	0.0000	0.0000	0.0000 ft	0.000000	0.000000	0.000000
cen_top	0.0000	0.0000	0.0000 ft	0.000000	0.000000	0.000000
rht_bot	0.0000	0.0000	0.0000 ft	0.000000	0.000000	0.000000
rht_mid	0.0000	0.0000	0.0000 ft	0.000000	0.000000	0.000000
rht_top	0.0000	0.0000	0.0000 ft	0.000000	0.000000	0.000000

Provisional Weighted Sum of Squares = 341.794

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b

TRIANGULATED IMAGE POINTS RESIDUALS (in thousandths of millimeters)

31	lft_bot	lft_miđ	lft_top	cen_bot	cen_mid	cen_top	rht_bot	rht_mid	rht_top
	0	-2	1	0	-1	7	1	-5	0
	1	0	3	-2	1	-1	0	-2	0
32	lft_bot	lft_mid	lft_top	cen_bot	cen_mid	cen_top	rht_bot	rht_mid	rht_top
	0	0	1	1	0	-3	1	-1	0
	3	0	2	-2	0	1	-1	-1	-1
33	lft_mid	lft_bot	lft_top	cen_bot	cen_mid	cen_top	rht_bot	rht_mid	rht_top
	-3	0	0	0	1	0	0	1	0
	2	-1	-1	1	2	0	0	-1	-2
34*0*	1ft_mid	lft_top	lft_bot	cen_bot	cen_mid	cen_top	rht_bot	rht_mid	rht_top
	1	-5	6	-3	1	0	-9	-1	-2
	-12	-12	-8	-9	-8	-7	-7	-13	-12
35	lft_mid 0 4	cen_bot 5 4	lft_bot -5 4	cen_mid -1 -1	cen_top 9 -1	rht_bot 1 -6	rht_mid -6 -5		
25	lft_top	cen_bot	lft_mid	cen_mid	lft_bot	cen_top	rht_bot	rht_mid	rht_top
	-1	-3	2	0	2	0	-4	1	2
	7	-1	2	-4	1	2	-3	-4	-4
21	cen_bot	cen_mid	lft_top	lft_mid	cen_top	lft_bot	rht_bot	rht_mid	rht_top
	1	-1	7	0	-3	-3	0	0	0
	-1	-1	0	-2	-2	-2	3	3	5
22	lft_top	cen_bot	cen_mid	cen_top	lft_mid	rht_bot	lft_bot	rht_mid	rht_top
	2	-1	0	-1	0	1	-1	0	0
	0	0	1	0	-3	3	-2	1	0
23	lft_top	cen_mid	cen_bot	cen_top	rht_bot	lft_mid	rht_mid	lft_bot	rht_top
	2	-2	-1	-3	0	0	1	0	2
	2	0	0	0	3	-1	-1	0	-2
10*0*	rht_bot -6 11	lft_mid 0 0	rht_mid -11 14	rht_top -17 7	lft_bot 0 0	lft_top 5 -6			
11*0*	cen_top	cen_bot	rht_bot	lft_bot	rht_mid	lft_mid	cen_mid	rht_top	lft_top
	-10	-4	0	-4	-5	-5	-8	-9	-1
	9	11	12	8	15	9	13	6	-4
51	lft_bot 0 3	cen_bot 4 7	cen_top 0 11	rht_bot 0 -11	rht_mid -1 -5	cen_mid 1 0	rht_top -2 0		
52	cen_bot	cen_top	lft_bot	rht_bot	lft_mid	rht_mid	cen_mid	lft_top	rht_top
	1	-2	1	1	0	1	-2	-1	0
	-3	-5	4	0	2	5	2	-3	-1

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TRIANGULATED IMAGE POINTS RESIDUALS (in thousandths of millimeters)

53	cen_mid	cen_top	cen_bot	rht_bot	rht_mid	lft_top	lft_mid	lft_bot	rht_top
	-2	-2	-2	1	1	3	0	-4	0
	0	-2	-1	2	~1	0	2	2	-1
54	lft_mid	cen_mid	rht_bot	cen_bot	lft top	cen top	rht mid	lft bot	rht top
	-1	0	-2	-4	- 7	2	2	-6	0
	-1	-2	2	1	4	-2	1	-2	-2
43	lft_top	rht_bot	lft_bot	cen_mid	cen_bot	rht mid	lft mid	cen top	rht top
	-3	0	16	-2	-3	0	0	-2	2
	0	-3	-11	6	-3	0	-1	2	8
42	cen_mid	cen_bot	lft_bot	lft_mid	rht mid	rht bot	lft top	rht top	cen top
	0	2	-1	3	- 1	0	-4	-2	1
	0	-2	0	0	6	0	-9	4	-3
24	rht_mid	cen_mid	cen_top	cen_bot	lft_mid	rht_top	lft_top	rht bot	
	0	-3	0	4	0	Ō	-1	0	
	-2	3	-2	2	4	-5	2	-4	
41	lft_mid	cen_bot	rht_bot	cen_top	rht_mid	lft_top	rht_top	cen_mid	
	4	0	4	0	-1	-10	-5	7	
	1	-2	5	-4	3	-1	0	-3	
44*0*	rht_bot	lft_top	cen_bot	rht_mid	lft_mid	cen_top	rht_top	cen_mid	
	18	-3	8	16	0	10	22	- 7	
>30	-8	12	-4	-12	-3	0	1	-2	
55	lft_mid	rht_mid	cen_mid	cen_bot	rht_bot	cen_top	rht_top		
	0	3	-1	0	1	-2	-2		
	-1	-1	-1	0	3	5	-3		
45	rht_mid	cen_bot	cen_mid	rht_bot	rht_top				
	0	-1	5	-9	6				
	v	•	-5	-4	4				
	Weig	hted Sum	of Square	s (Camera) =	0.	0		
	Weig	hted Sum	of Smiare	s (UDJECT s (Plates) =) =	41.	ь 7		
			párare	- (**4668	, -	439.	,		
	Weig	hted Sum	of Square	s (Total)	=	281.	3		
	Degr	ees oi Fr	eedom	• • • • • • • • • •	=	31	0		
	a po	steriori	Variance	of Unit W	eight =		0.907		

TRIANGULATED CAMERA STATIONS (Terrestrial->Ph) PAGE 14

Ident	Position		Error El	lipsoid>	Length
lft_bot	X = -45. Y = -88. Z = 7.	8566 ft + 8758 ft - 2754 ft +	+0.0102 -0.8226 +0.5685	-0.1717 +0.9851 +0.5586 +0.1059 +0.8115 +0.1356	0.1153 ft > 0.0695 ft > 0.0238 ft
	Attitude:	Azim. = 2 Elev. = 0 Swing =- 0	22 49 31. 09 38 14. 02 00 09.	4017 0472 Std Dev: 7240	00 02 15.3811 00 04 20.2418 00 00 52.1031
lft_mid	X = -41. Y = -91. Z = 22.	3167 ft - 2999 ft + 6691 ft +	-0.2916 +0.7840 +0.5481	+0.1814 +0.9392 -0.5172 +0.3433 +0.8364 +0.0086	2> 0.1161 ft 3> 0.0694 ft 5> 0.0218 ft
	Attitude:	Azim. = 2 Elev. = 0 Swing =- 0	20 04 16. 00 47 32. 01 37 44.	5059 4810 Std Dev: 3851	00 02 26.5482 00 04 06.2165 00 00 44.5085
lft_top	X = -40. Y = -87. Z = 42.	0444 ft - 7159 ft + 0707 ft +	-0.4722 +0.6653 +0.5783	+0.5295 +0.7048 -0.3105 +0.6790 +0.7894 -0.2057	8> 0.1388 ft)> 0.0708 ft /> 0.0222 ft
	Attitude:	Azim. = 2 Elev. =- 1 Swing = 0	20 22 4 1. 11 23 4 9. 00 17 55.	8904 8395 Std Dev: 5141	00 03 08.5308 00 04 26.8838 00 00 51.3082
cen_bot	X = 0. Y = -104. Z = 7.	0478 ft - 0121 ft - 9112 ft -	+0.0989 -0.9951 -0.0034	-0.1813 +0.9784 -0.0213 +0.0966 +0.9832 +0.1825	1>0.1366 ft5>0.0972 ft5>0.0205 ft
	Attitude:	Azim. = 35 Elev. = (Swing =- (59 19 50. 08 33 09. 00 04 58.	6801 7663 Std Dev: 5523	00 02 55.9239 00 04 22.8336 00 00 41.8080
cen_mid	X = 0. Y = -103. Z = 23.	3065 ft 6608 ft 2872 ft	+0.1575 +0.9875 -0.0051	-0.0275 +0.9872 +0.0095 -0.1573 +0.9996 +0.0280	L> 0.1327 ft 3> 0.1053 ft 5> 0.0201 ft
	Attitude:	Azim. = 3 Elev. = 0 Swing = 0	59 05 52. 01 40 10. 00 19 03.	1563 0777 Std Dev: 4294	00 03 13.1696 00 04 18.1911 00 00 39.8244
cen_top	X = 0. Y = -103. Z = 37.	.2490 ft .6585 ft .6182 ft	+0.3419 +0.9397 -0.0020	+0.1126 +0.9330 -0.0388 -0.339 +0.9929 -0.119	0.1274 ft 7> 0.1274 ft 1> 0.1198 ft 0.0205 ft
	Attitude:	Azim. = 3 Elev. =- 4 Swing =- 4	59 12 26. 06 31 33. 00 25 19.	1008 0861 Std Dev: 1743	00 03 37.5073 00 04 04.8738 00 00 41.8463
rht_bot	X = 30 Y = -94 Z = 10	.5076 ft .6763 ft .2770 ft	+0.3111 +0.8517 -0.4217	-0.0240 +0.950 +0.4506 -0.267 +0.8924 +0.160	1> 0.1197 ft 5> 0.0714 ft 6> 0.0209 ft
	Attitude:	Azim. = 3 Elev. = Swing =	44 10 25. 07 16 56. 01 04 10.	2940 1461 Std Dev: 6991	00 02 28.3635 00 04 04.5495 00 00 42.7132

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TRIANGULATED CAMERA STATIONS (Terrestrial->Ph) PAGE 15

Ident	Position	Erro	or Ellipsoid	> L	ength
rht_mid	X = 30 Y = -92 Z = 22	.2500 ft +0.40 .5323 ft -0.80 .9733 ft +0.43	012 +0.1647 - 077 -0.4004 - 021 -0.9014 -	⊦0.9010> ⊦0.4328> -0.0276>	0.1200 ft 0.0700 ft 0.0201 ft
	Attitude:	Azim. = 343 39 Elev. = 00 30 Swing = 00 06) 12.7349) 52.7467 Std ; 10.4941	00 02 Dev: 00 04 00 00	2 39.1122 4 04.5073 9 41.2954
rht_top	X = 30 Y = -93 Z = 41	.0157 ft +0.54 .0833 ft -0.70 .4006 ft +0.44	62 +0.4155 + 97 -0.2317 + 50 -0.8796 +	+0.7273> +0.6653> +0.1683>	0.1313 ft 0.0748 ft 0.0211 ft
	Attitude:	Azim. = 343 44 Elev. =- 09 22 Swing =- 01 19	12.6986 222.7541 Std 26.2052	00 03 Dev: 00 04 00 00	07.2612 00.7174 45.7613
SUMMARY	STAT:	ISTICS	FOR CAN	IERA S	TATIONS
	1	RMS For Standar	d Deviations		
Count	t = 9	$\begin{array}{rcl} X &=& 0.0873\\ Y &=& 0.0461\\ Z &=& 0.1184 \end{array}$	ft Azim. ft Elev. ft Swing	$\begin{array}{rrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrr$	2656 22820 27674

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		ANGULA			UIAID	
Ident	Posit:	ion (feet)	E	ror Ellipso	oid>	Length (ft)
	X =	0.0058	-3.345E-02	-9.884E-01	+1.478E-01	0.0047
10 *0*	Y =	0.0017	+9.990E-01	-3.726E-02	-2.310E-02	0.0044
	Z =	-0.0053	-2.834E-02	-1.469E-01	-9.887E-01	0.0044
	X =	0.0094	-4.395E-02	-9.864E-01	+1.584E-01	0.0047
11 *0*	Y =	-0.0021	+9.989E-01	-4.126E-02	+2.023E-02	0.0043
•	- Z =	5,9840	+1.342E-02	-1.591E-01	-9.872E-01	0.0042
		0.0000			510/22 02	
	X =	-38,2552	+1.286E-01	-9.882E-01	+8.260E-02	0.0344
21	Y =	0.2242	+8.501E-01	+1.528E-01	+5.040E-01	0.0117
	7. =	7,1021	+5.107E-01	-5.381E-03	-8.597E-01	0.0106
	-	/				
	X =	-40,9036	+1.638E-01	-9.854E-01	+4.547E-02	0.0330
22	Y =	0.1737	+9.157E-01	+1.690E-01	+3.647E-01	0.0116
	7. =	14.0425	+3.671E-01	+1.810E-02	-9.300E-01	0.0106
	-					
	X =	-42.7607	-2.057E-01	+9.785E-01	+1.319E-02	0.0314
23	Y =	0.1147	-9.756E-01	-2.040E-01	-8.145E-02	0.0114
	- 7 =	24.0802	+7.701E-02	+2.962E-02	-9.966E-01	0.0106
		21.0002	.,.,		515002 02	010100
	X =	-44.0511	-2.669E-01	+9.619E-01	+5.970E-02	0.0346
24	v =	0.0438	-9.3998-01	-2.735E-01	+2.042E-01	0.0123
~ ~	 7 =	34 2495	-2.128E-01	+1.5958-03	-9.771E-01	0.0112
	4 -	54.2455	-2.1200-01	+1.5556-05	-9.7710-01	0.0112
	X =	-44.7532	-2.211E-01	+9.571E-01	+1.872E-01	0.0350
25	Y =	-4.0226	-2.757E-01	-2.455E-01	+9.294E-01	0.0123
	7 =	45,1988	+9.354E-01	+1.539E-01	+3.182E-01	0.0117
						•••===
	X =	-27.6186	+1.068E-01	-9.885E-01	+1.075E-01	0.0297
31	Y =	0.2071	-9.217E-01	-1.389E-01	-3.621E-01	0.0101
	Z =	6.6549	-3.729E-01	+6.039E-02	+9.259E-01	0.0098
					<i>'</i>	
	X =	-33.0732	+1.544E-01	-9.864E-01	+5.609E-02	0.0296
32	Y =	0.1813	-9.604E-01	-1.632E-01	-2.260E-01	0.0102
	Z =	14.0304	-2.321E-01	+1.897E-02	+9.725E-01	0.0100
	X =	-33.0460	-1.924E-01	+9.812E-01	+1.414E-02	0.0281
33	Y =	0.0966	+7.206E-01	+1.510E-01	-6.767E-01	0.0098
	Z =	24.0802	+6.661E-01	+1.200E-01	+7.361E-01	0.0097
	X =	-32.9980	+1.176E-03	-1.000E+00	+2.985E-03	. 0.0048
34 *0*	Y =	0.0002	+7.324E-01	-1.172E-03	-6.808E-01	0.0046
	Z =	34.2521	-6.808E-01	-2.987E-03	-7.324E-01	0.0046
. –	X =	-32.9207	-1.749E-01	+9.643E-01	+1.987E-01	0.0382
35	Y =	-0.1454	-4.802E-01	-2.597E-01	+8.379E-01	0.0120
	Z =	45.9864	+8.596E-01	+5.112E-02	+5.085E-01	0.0116
		08 5444		0 8455 65		
	X =	27.5610	-1.7/5E-01	-9.745E-01	+1.371E-01	0.0350
41	¥ =	0.0709	+7.547E-01	-4.539E-02	+0.545E-01	0.0104
	Z =	6.6936	+0.316E-01	-2.197E-01	-7.435E-01	0.0101

TRIANGULATED OBJECT POINTS

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PAGE 17

	TRIANGULATED OBJECT POINTS
Ident	Position (feet) Error Ellipsoid> Length (ft)
	X = 32.9914 - 2.405E - 01 - 9.689E - 01 + 5.874E - 02 0.0327
42	Y = 0.0085 - 5.631E - 01 + 8.995E - 02 - 8.215E - 01 0.0102
	Z = 14.0684 + 7.907E - 01 - 2.306E - 01 - 5.672E - 01 0.0101
	X = 32.9785 + 2.455E - 01 + 9.693E - 01 + 1.305E - 02 0.0319
43	Y = 0.0039 + 2.992E - 01 - 8.857E - 02 + 9.501E - 01 0.0100
	Z = 24.1032 +9.221E-01 -2.294E-01 -3.117E-01 0.0098
	X = 32.9828 + 4.776E - 03 + 1.000E + 00 - 1.389E - 03 0.0048
44 *0*	$\mathbf{Y} = 0.0002 - 7.490E - 01 + 2.656E - 03 - 6.625E - 01 0.0046$
	Z = 34.2380 + 6.626E - 01 - 4.205E - 03 - 7.490E - 01 0.0046
	X = 32.8321 + 1.103E - 01 + 9.745E - 01 + 1.955E - 01 0.0615
45	Y = -0.2546 + 2.464E - 02 - 1.994E - 01 + 9.796E - 01 0.0135
	$\mathbf{Z} = 45.8741 + 9.936\mathbf{E} - 01 - 1.032\mathbf{E} - 01 - 4.600\mathbf{E} - 02 0.0119$
	X = 38.1270 -2.151E-01 -9.698E-01 +1.147E-01 0.0416
51	Y = -0.0052 + 5.472E - 01 - 2.170E - 01 - 8.084E - 01 0.0118
	Z = 7.1186 + 8.089E - 01 - 1.111E - 01 + 5.774E - 01 0.0115
50	x = 40.7659 - 2.663E - 01 - 9.624E - 01 + 5.327E - 02 0.0368
52	Y = -0.0353 + 8.662E - 01 - 2.632E - 01 - 4.247E - 01 0.0115
	Z = 14.0930 - 4.228E - 01 + 6.696E - 02 - 9.038E - 01 0.0109
	X = 42.5834 +2.782E-01 +9.605E-01 +1.213E-02 0.0364
53	Y = -0.1776 -9.460E - 01 + 2.718E - 01 + 1.765E - 01 0.0115
	Z = 24.0662 - 1.662E - 01 + 6.057E - 02 - 9.842E - 01 0.0109
54	X = -0.1805 - 0.2347 01 + 2.052 - 01 + 1.3352 - 02 - 0.03/9
54	1 = -0.1006 - 9.354E - 01 + 2.601E - 01 - 2.16/E - 01 0.0123
	2 = 34.1766 + 2.288E - 01 + 9.102E - 03 - 9.734E - 01 0.0111
	X = 44.5966 +2.460E-01 +9.517E-01 +1.838E-01 0.0474
55	Y = -3.9806 +5.592E - 01 - 2.942E - 01 + 7.751E - 01 0.0135
	Z = 45.1475 +7.917E-01 -8.788E-02 -6.046E-01 0.0124
SUMMA	RY STATISTICS FOR OBJECT POINTS

RMS For Standard Deviations

Count	=	18	X =	0.0134	feet
Count	=	18	Y =	0.0361	feet
Count	=	18	Z =	0.0120	feet

C ;	-Gi AHL	an K	t(d Ste	:)] er:	199 1 ·	₹0- - Z	- 4 , AV(, ()NI	gp/ DAJ	A 2 Le	Ass Sh	OC: IP		RDS	s 5,	II	NC	•														Pł	lGE	. 1	8.8	
(c 0	R	R	E	с	т	I	0	N	s		A	P	₽	L	I	E	D		т	0		o	в	J	Е	с	т		с	0	N	т	R	0	L
										x	=			(00!	58	ft							x	=			C).(009	94	ft			
								10	D	Y	=			().(00:	17	ft					11	-	Y	Ξ			-0).(002	21	ft			
										Z	=			- ().(00!	53	ft	•						Z	=			-().(010	50	ft	:		
										х	=			() .(00:	20	ft							х	=			-0	۰. د	017	72	ft			
								34	4	Y	=			(. c	00	02	ft					44	ļ.	Y	=			(۰.	000	20	ft	:		
										Z	=			(5.	01	77	ft	•						Z	=			().	00:	36	ft	:		
				x	•	•••	. 1	Nu	nbo nbo	er er	of	Co				nt:	5 : 5 :	=		4		RMS RMS	5 =	-				0	0.01	L0:	3 1 3 1	Eee	et et			
				Ż	:	•••	. 1	งาน	nb	er	of	C	omj	poi	1e	nt	5	=		4		RM:	5 =	•				Ö	. 01	L2	4 :	Eee	et			

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Error Ellipsoids - ZX plane