Naval Research Laboratory

Washington, DC 20375-5320



NRL/MR/6384--94-7641

Addressing Tension Instability in SPH Methods

C.T. DYKA

Geo-Centers, Inc. Fort Washington, MD

R.P. INGEL

Composites and Ceramics Branch Materials Sciences and Technology Division



December 30, 1994

19950106 077

Approved for public release; distribution unlimited.

REPOR	DOCUMENTATIO	N PAGE	Form Approved OMB No. 0704-0188
Public reporting burden for this collection of in gathering and maintaining the data needed, an collection of information, including suggestion: Davis Highway, Suite 1204, Arlington, VA 22	formation is estimated to average 1 hour per of d completing and reviewing the collection of in for reducing this burden, to Washington Heac 202-4302, and to the Office of Management a	esponse, including the time for reviewing ins formation. Send comments regarding this bu quarters Services, Directorate for Informatic and Budget. Paperwork Reduction Project (0	tructions, searching existing data sources, urden estimate or any other aspect of this on Operations and Reports, 1215 Jefferson 704-0188), Washington, DC 20503.
I. AGENCY USE ONLY (Leave Blank)	2. REPORT DATE	3. REPORT TYPE AND DATES COV	/ERED
	December 30, 1994	Interim	
. TITLE AND SUBTITLE			5. FUNDING NUMBERS
Addressing Tension Instability	in SPH Methods		
. AUTHOR(S)			-
C.T. Dyka* and R.P. Ingel			
. PERFORMING ORGANIZATION NAM	E(S) AND ADDRESS(ES)	ter and an and the second s	8. PERFORMING ORGANIZATION
Geo-Centers, Inc.Naval Research Laboratory10903 Indian Head HighwayWashington, DC 20375-5320Fort Washington, MD 20744Varian Statement Sta			REPORT NUMBER NRL/MR/638494-7641
. SPONSORING/MONITORING AGENC	Y NAME(S) AND ADDRESS(ES)		10. SPONSORING/MONITORING
ARPA			AGENCY REPORT NUMBER
Arlington, VA 22209			
*C - C - t - 10002 L			
*Geo-Centers, Inc., 10903 Inc	lian Head Highway, Fort Washing	ton, MD 20744	
*Geo-Centers, Inc., 10903 Inc 2a. DISTRIBUTION/AVAILABILITY ST/	lian Head Highway, Fort Washing	gton, MD 20744	12b. DISTRIBUTION CODE
*Geo-Centers, Inc., 10903 Inc 2a. DISTRIBUTION/AVAILABILITY ST/ Approved for public release; c	lian Head Highway, Fort Washing NTEMENT istribution unlimited.	gton, MD 20744	12b. DISTRIBUTION CODE
*Geo-Centers, Inc., 10903 Inc 2a. DISTRIBUTION/AVAILABILITY ST/ Approved for public release; c	lian Head Highway, Fort Washing TEMENT istribution unlimited.	gton, MD 20744	12b. DISTRIBUTION CODE
*Geo-Centers, Inc., 10903 Inc 2a. DISTRIBUTION/AVAILABILITY ST/ Approved for public release; c 3. ABSTRACT (<i>Maximum 200 words</i>)	lian Head Highway, Fort Washing TEMENT istribution unlimited.	gton, MD 20744	12b. DISTRIBUTION CODE
*Geo-Centers, Inc., 10903 Inc 2a. DISTRIBUTION/AVAILABILITY ST/ Approved for public release; c 3. ABSTRACT (<i>Maximum 200 words</i>) Smoothed particle hydrodyr SPH method is currently plague stresses are calculated at points dimensional (1D) program calle applications. The results from a together in tension and compare	lian Head Highway, Fort Washing TEMENT istribution unlimited. amics (SPH) has the potential to I d by tension instability. In this we other than the SPH nodes, to add d SPH1D. In addition, objective s pplying the unconventional approx- s quite well to a finite element an	pton, MD 20744 be an important method for structork, a new unconventional appro- ress this difficulty. This algorith stress rate calculations are discus- ach to a simple bar are very enc- alysis.	12b. DISTRIBUTION CODE ctural analysis. However, the bach is introduced, in which th um is implemented into a one ssed and specialized to 1D couraging. The bar holds
*Geo-Centers, Inc., 10903 Inc 2a. DISTRIBUTION/AVAILABILITY ST/ Approved for public release; c 3. ABSTRACT (<i>Maximum 200 words</i>) Smoothed particle hydrodyr SPH method is currently plague stresses are calculated at points dimensional (1D) program calle applications. The results from a together in tension and compare	lian Head Highway, Fort Washing TEMENT istribution unlimited. amics (SPH) has the potential to I d by tension instability. In this we other than the SPH nodes, to add d SPH1D. In addition, objective s pplying the unconventional approx- is quite well to a finite element an	pton, MD 20744 be an important method for structork, a new unconventional appro- ress this difficulty. This algorithe stress rate calculations are discuss ach to a simple bar are very enc- alysis.	12b. DISTRIBUTION CODE
*Geo-Centers, Inc., 10903 Inc *Geo-Centers, Inc., 10903 Inc 2a. DISTRIBUTION/AVAILABILITY ST/ Approved for public release; c 3. ABSTRACT (<i>Maximum 200 words</i>) Smoothed particle hydrodyn SPH method is currently plague stresses are calculated at points dimensional (1D) program calle applications. The results from a together in tension and compare 4. SUBJECT TERMS	lian Head Highway, Fort Washing TEMENT istribution unlimited. amics (SPH) has the potential to l d by tension instability. In this we other than the SPH nodes, to add d SPH1D. In addition, objective s pplying the unconventional approx s quite well to a finite element an	ton, MD 20744 be an important method for structork, a new unconventional approrress this difficulty. This algorith stress rate calculations are discuss ach to a simple bar are very ence alysis.	12b. DISTRIBUTION CODE ctural analysis. However, the bach is introduced, in which the m is implemented into a one ssed and specialized to 1D couraging. The bar holds
 *Geo-Centers, Inc., 10903 Inc. *Geo-Centers, Inc., 10903 Inc. 2a. DISTRIBUTION/AVAILABILITY ST/ Approved for public release; of 3. ABSTRACT (<i>Maximum 200 words</i>) Smoothed particle hydrodyr SPH method is currently plagues stresses are calculated at points dimensional (1D) program calle applications. The results from a together in tension and compare b. SUBJECT TERMS Explicit analysis 	Ian Head Highway, Fort Washing TEMENT istribution unlimited. amics (SPH) has the potential to l d by tension instability. In this we other than the SPH nodes, to add d SPH1D. In addition, objective s pplying the unconventional appro- rs quite well to a finite element an SPH	ton, MD 20744 be an important method for structork, a new unconventional appro ress this difficulty. This algorith stress rate calculations are discus ach to a simple bar are very enc alysis.	12b. DISTRIBUTION CODE ctural analysis. However, the bach is introduced, in which th um is implemented into a one ssed and specialized to 1D bouraging. The bar holds
*Geo-Centers, Inc., 10903 Inc *Geo-Centers, Inc., 10903 Inc Pa. DISTRIBUTION/AVAILABILITY ST/ Approved for public release; c 3. ABSTRACT (<i>Maximum 200 words</i>) Smoothed particle hydrodyr SPH method is currently plague stresses are calculated at points dimensional (1D) program calle applications. The results from a together in tension and compare 8. SUBJECT TERMS Explicit analysis Fracture Impact	lian Head Highway, Fort Washing TEMENT istribution unlimited. amics (SPH) has the potential to I d by tension instability. In this we other than the SPH nodes, to add d SPH1D. In addition, objective a pplying the unconventional approx- is quite well to a finite element an SPH Transient Wave propagation	ton, MD 20744 be an important method for structork, a new unconventional approrress this difficulty. This algorithestress rate calculations are discussed to a simple bar are very encallysis.	12b. DISTRIBUTION CODE ctural analysis. However, the bach is introduced, in which the units implemented into a one seed and specialized to 1D bouraging. The bar holds ctural analysis. However, the bach is introduced, in which the units implemented into a one seed and specialized to 1D bouraging. The bar holds ctural analysis. However, the bach is introduced, in which the units implemented into a one seed and specialized to 1D bouraging. The bar holds ctural analysis. However, the bach is implemented into a one seed and specialized to 1D bouraging. The bar holds ctural analysis. However, the bach is implemented into a one seed and specialized to 1D bouraging. The bar holds ctural analysis. However, the bach is implemented into a one seed and specialized to 1D bouraging. The bar holds ctural analysis. However, the bach is implemented into a one seed and specialized to 1D bouraging. The bar holds ctural analysis. However, the bach is implemented into a one seed and specialized to 1D bouraging. The bar holds ctural analysis. However, the bach is implemented into a one seed and specialized to 1D bouraging. The bar holds ctural analysis. However, the bach is implemented into a one seed and specialized to 1D bouraging. The bach is implemented into a one seed and specialized to 1D bouraging. The bach is implemented into a one seed and specialized to 1D bouraging. The bach is implemented into a one seed and specialized to 1D bouraging. The bach is implemented into a one seed and specialized to 1D bouraging. The bach is implemented into a one seed and specialized to 1D bouraging. The bach is implemented into a one seed and specialized to 1D bouraging. The b
 *Geo-Centers, Inc., 10903 Inc. *Geo-Centers, Inc., 10903 Inc. 2a. DISTRIBUTION/AVAILABILITY ST/ Approved for public release; c 3. ABSTRACT (<i>Maximum 200 words</i>) Smoothed particle hydrodyn SPH method is currently plague stresses are calculated at points dimensional (1D) program calle applications. The results from a together in tension and compare b. SUBJECT TERMS Explicit analysis Fracture Impact 7. SECURITY CLASSIFICATION OF REPORT 	Ian Head Highway, Fort Washing TEMENT istribution unlimited. amics (SPH) has the potential to I d by tension instability. In this we other than the SPH nodes, to add d SPH1D. In addition, objective s pplying the unconventional approx s quite well to a finite element an SPH Transient Wave propagation 18. SECURITY CLASSIFICATION OF THIS PAGE	ton, MD 20744 be an important method for structork, a new unconventional appro- ress this difficulty. This algorith stress rate calculations are discuss ach to a simple bar are very enc- alysis.	12b. DISTRIBUTION CODE ctural analysis. However, the bach is introduced, in which the mis implemented into a one seed and specialized to 1D couraging. The bar holds ctural analysis. However, the bach is introduced, in which the mis implemented into a one seed and specialized to 1D couraging. The bar holds ctural analysis. However, the bach is introduced, in which the mis implemented into a one seed and specialized to 1D couraging. The bar holds ctural analysis. However, the bach is introduced, in which the mis implemented into a one seed and specialized to 1D couraging. The bar holds ctural analysis. However, the bach is introduced, in which the mis implemented into a one seed and specialized to 1D couraging. The bar holds ctural analysis. However, the bach is introduced, in which the mis implemented into a one seed and specialized to 1D couraging. The bar holds ctural analysis. However, the bach is introduced, in which the mis implemented into a one seed and specialized to 1D couraging. The bar holds ctural analysis. However, the bach is introduced, in which the mis implemented into a one seed and specialized to 1D couraging. The bar holds ctural analysis. However, the bach is introduced into a one seed and specialized to 1D couraging. The bach is introduced into a one seed and specialized to 1D couraging. The bach is introduced into a one seed and specialized to 1D couraging. The bach is introduced into a one seed and specialized to 1D couraging. The bach is introduced into a one seed and specialized to 1D couraging. The bach is introduced into a one seed and specialized to 1D couraging. The bach is introduced intreduced inte seed and to 1D couraging. The bach is introduced intr

.

.

•

•

CONTENTS

.

1.	INTRODUCTION	1
2.	THE SPH METHODE	2
3.	THE SPH EQUATIONS (BRIEFLY)	3
4.	CONSTITUTIVE EQUATIONS AND STRESS RATES	5
5.	ADDRESSING TENSION INSTABILITY	7
6.	NUMERICAL IMPLEMENTATION-SPH1D	9
7.	APPLICATION	10
8.	CONCLUSIONS	12
9.	ACKNOWLEDGMENTS	13
10.	REFERENCES	14



ADDRESSING TENSION INSTABILITY IN SPH METHODS

1. INTRODUCTION

Smoothed particle hydrodynamics (SPH) is a Lagrangian method that requires no spatial mesh. SPD is really an interpolation method in which particles can be employed as part of the approximation [1]. (Note that in this paper, the words element and particle will be used interchangeably when referring to SPH methods). The calculation of interactions among the particles is based upon their separation alone. This aspect along with the absence of a grid allows very large deformations to be computed in a straightforward fashion. In addition, arbitrary fracture surfaces can be opened without requiring special considerations such as an a priori knowledge of the fracture location as in finite element methods (FEM). SPH is thus an appealing and valuable computational tool, especially for high deformation events such as impact.

However, SPH is a maturing method and still has a few technical barriers to overcome before becoming a widely used tool in computational mechanics. Its biggest drawback is the well know instability in tension - premature fragmentation of the SPH grid in tension. Swegle *et al.* [2] have done a formal stability analysis that clearly discusses the roots of tension instability. Hicks *et al.* [3] have applied a conservative smoothing approach with some success, but tension instability remains a serious obstacle in general. Other difficulties with SPH methods include the accurate calculation of finite strains, inhomogeneous media, stress oscillations or "sawtooth" behavior (see [4]), and the application of natural boundary conditions.

In this paper, we present a new unconventional (UC) approach to address tension instability (and also oscillatory stresses) in which the stresses are calculated at points other than the SPH nodes. In addition, stress rate calculations are briefly addressed and specialized to one dimension (1D) applications.

1

Manuscript approved on November 8, 1994

2. THE SPH METHOD

The SPH method is non-local in nature and based upon interpolation - see [1], [4-6]. Consider the function $f(\mathbf{x})$ and its kernel average $\langle f(\mathbf{x}) \rangle$ expressed as:

$$\langle f(\mathbf{x}) \rangle = \int f(\mathbf{x}') W(\mathbf{x} - \mathbf{x}', h) dV'$$
 (1)

where V is the volume, $\underline{\mathbf{x}}$ is the current position vector, and W is the kernel with a width measured by the parameter h. Various possibilities exist for the choice of $W(\underline{\mathbf{x}} - \underline{\mathbf{x}}', h)$ with the most popular being the cubic-b-spline [4]. The kernel W should satisfy the following condition:

$$\lim_{h \to 0} W(\underline{\mathbf{x}} - \underline{\mathbf{x}}', h) = \delta(\underline{\mathbf{x}} - \underline{\mathbf{x}}')$$
(2)

where δ is Dirac's delta function. Normalization of W and compact support (W is zero outside a limited domain) are expressed as:

$$\int W(\mathbf{x},h)dV = 1 \tag{3a}$$

and

$$W(\underline{\mathbf{x}},h) = 0 \qquad (|\underline{\mathbf{x}}| > 2h) \qquad (3b)$$

Associating with a particle j, a volume

$$dV^j = \frac{M^j}{\rho^j} \tag{4}$$

introduces the concept of particle mass, M^{j} , and density, ρ^{j} . The integral in (1) can then be replaced by the approximation:

$$\langle f(\mathbf{x}) \rangle \approx \sum_{j=1}^{N} f(\mathbf{x}^{j}) W(\mathbf{x} - \mathbf{x}^{j}, h) \frac{M^{j}}{\rho^{j}}$$
 (5)

where N is the total number of particles.

Dropping the $\langle \rangle$ sign, gradients of $f(\mathbf{x})$ can be expressed as [4]:

$$\nabla (f(\mathbf{x})) = -\sum_{j=1}^{N} \frac{M^{j}}{\rho^{j}} f(\mathbf{x}^{j}) \frac{\partial W}{\partial x_{l}^{j}} e_{l}^{j}$$
(6)

in which the subscripts denote component and \mathbf{e}_l^j is the *l* component of the unit vector at *j*. Another form for the gradient of $f(\mathbf{x})$ that is widely used but which introduces additional approximation is [4]:

$$\nabla (f(\mathbf{x})) = -\rho (\mathbf{x}) \sum_{j=1}^{N} M^{j} \left[\frac{f(\mathbf{x})}{\rho^{2}(\mathbf{x})} + \frac{f(\mathbf{x}^{j})}{\rho^{2}(\mathbf{x}^{j})} \right] \frac{\partial W}{\partial x_{l}^{j}} e_{l}^{j}$$
(7)

3. THE SPH EQUATIONS (BRIEFLY)

In subscript notation, the conservation of linear momentun is expressed as:

$$\frac{dV_m}{dt} = -\frac{1}{\rho} \frac{\partial \sigma_{mn}}{\partial x_n} \tag{8}$$

in which V_m is the velocity and σ_{mn} is the Cauchy stress tensor, and the summation convention is implied by the repeated index n. Applying (6), (8) becomes at particle i:

$$\frac{dV_m^i}{dt} = -\frac{1}{\rho^i} \sum_{j=1}^N \frac{M^j}{\rho^i} \frac{\sigma_{mn}^j}{\rho^j} \frac{\partial W^{ij}}{\partial x_n^j}$$
(9)

This form does not conserve momentum since the force on particle i due to j is not the same as j due to i. A widely used alternative to (9) that conserves momentum but introduces further approximation is obtained by substituting (7) into (8) to produce (no summation on i):

$$\frac{dV_m^i}{dt} = -\sum_{j=1}^N M^j \left[\frac{\sigma_{mn}^j}{\left(\rho^j\right)^2} + \frac{\sigma_{mn}^i}{\left(\rho^i\right)^2} \right] \frac{\partial W^{ij}}{\partial x_n^j} \tag{10}$$

Artifical viscosity is usually included in the linear momentun as an artifical viscous pressure. Benz[1], Libersky and Petschek [2] and Libersky *et al.* [7] give details for including it into (10).

Mass conservation is of the form:

$$\frac{d\rho}{dt} = -\rho \frac{\partial V_m}{\partial x_m} \tag{11}$$

Introducing (7) into (11) produces:

$$\frac{d\rho^{i}}{dt} = -\rho^{i} \sum_{j=1}^{N} \frac{M^{j}}{\rho^{j}} V_{m}^{j} \frac{\partial W^{ij}}{\partial x_{m}^{j}}$$
(12)

A widely used alternative to (12) is [4]:

$$\frac{d\rho^{i}}{dt} = -\rho^{i} \sum_{j=1}^{N} \frac{M^{j}}{\rho^{j}} \left(V_{m}^{i} - V_{m}^{j}\right) \frac{\partial W^{ij}}{\partial x_{m}^{j}}$$
(13)

The density can also be directly determined from (5) and is of the form:

$$\rho^i = \sum_{j=1}^N M^j W^{ij} \tag{14}$$

Equation (14) will be employed in this paper.

In addition to momentum and mass conservation, energy conservation can also be included in the governing equations (see [1] and [2] for instance). In this work, it has not been introduced since a linear elastic material will be assumed.

4. CONSTITUTIVE EQUATIONS AND STRESS RATES

The current position $\underline{x}(t)$ of the material point \overline{x} can be expressed as:

$$\underline{x}(t) = \overline{\underline{x}} + \underline{U}(t)$$
(15)

where $\underline{U}(t)$ is the current displacement as measured from \overline{x} . In index notation, the rate of deformation $d_{mn}(t)$ is of the form:

$$d_{mn}(t) = \frac{1}{2} \left(\frac{\partial V_m}{\partial x_n} + \frac{\partial V_n}{\partial x_m} \right)$$
(16)

Note in (16), that d_{mn} is an Eulerian variable and the derivatives are with respect to x_n (not \bar{x}_n). The rate of deformation can be put in SPH form by directly applying either equation (6) or (7). In [2], further manipulations and approximations are introduced to produce the correct trace and to express d in terms of velocity differences. For 1D applications, the use of (6) or (7) will suffice.

Next consider the velocity gradient L_{mn} which is defined by:

$$L_{mn}(t) = \frac{\partial V_m}{\partial x_n} \tag{17}$$

Using (16), L_{mn} can be divided into symmetric and skew-symmetric parts:

$$L_{mn} = d_{mn} + \theta_{mn} \tag{18}$$

where θ_{mn} is the rotation and given by

$$\theta_{mn} = \frac{1}{2} \left(\frac{\partial V_m}{\partial x_n} - \frac{\partial V_n}{\partial x_m} \right)$$
(19)

and d_{mn} is determined from (16).

Having defined the rate of deformation, we next seek an objective stress rate. The Jauman stress rate $\dot{\sigma}_{mn}^{jau}$ is widely used [8], and is expressed as:

$$\dot{\sigma}_{mn}^{jau} = \dot{\sigma}_{mn} - \theta_{lj} \sigma_{ml} - \theta_{lm} \sigma_{nl}$$
(20)

where σ_{ml} and $\dot{\sigma}_{mn}$ are components of the Cauchy stress and the rate of Cauchy stress (which is not objective). For 1D applications, $\theta_{mn} = 0$ and

$$\dot{\sigma}_{11}^{jau} = \dot{\sigma}_{11} \tag{21}$$

The Jauman stress rate can be related to the rate of deformation via the constitutive tensor $\boldsymbol{\mathcal{L}}$. In component form this relation is:

$$\dot{\sigma}_{mn} = C_{mnlq} d_{lq} \tag{22}$$

For 1D applications and a linear material, (22) reduces to:

$$\dot{\sigma}_{11} = C_{1111} d_{11} \tag{23}$$

where $C_{1111} = E$ (the modulus of elasticity and assumed to be constant), and

$$d_{11} = \frac{\partial V_1}{\partial x_1} \tag{24}$$

Explicit integration of $\dot{\sigma}_{11}$ to obtain $\sigma_{11}(t)$ is in the form:

$$\sigma_{11}(t) = \sigma_{11}(t - dt) + \dot{\sigma}_{11}(t - dt) dt$$
(25)

where dt is the current time step and from (23)

$$\dot{\sigma}_{11}(t-dt) = E \cdot d_{11}(t-dt) dt$$
 (26)

Because the displacement U_1 is the time derivative of V_1 and E is constant, (26) can be directly integrated in time to yield:

$$\sigma_{11} = E\bar{\varepsilon}_{11} \tag{27}$$

where through (24)

$$\bar{\varepsilon}_{11}(t) = \frac{\partial}{\partial x_1} U_1(t)$$
 (28)

Equations (6) or (7) are then introduced into (28) to express $\bar{\epsilon}_{11}(t)$ in a SPH form. We note that $\bar{\epsilon}_{11}$, which is determined by integrating d_{11} in time, is not the well known linear strain. Rather it is an Eulerian variable because the derivative in (28) is with respect to $x_1(t)$ not \bar{x}_1 .

5. ADDRESSING TENSION INSTABILITY

Tension instability is a problem that has longed troubled conventional SPH methods and greatly limited its application. In [2], a stability analysis for 1D is presented. That analysis shows SPH calculations to be unstable when:

$$\left(\frac{\partial^2 W}{\partial x^2}\right)\sigma_{11} > 0 \tag{29}$$

This means that the standard SPH method, in which the node and the stress point are both located at the centroid of the element (particle), will be unstable in tension. Swegle *et al.* in [2] also demonstrate that this tension instability can not be corrected by artifical viscosity.

The approach in this work to overcome tension instability is indicated in Fig. 1, in which the stress points (denoted by x) are computed at points away from the SPH nodes (denoted by 0 and located at the center) as measured by the distance R. The limits of R are:

$$0 \le R \le 0.5 \tag{30}$$

R = 0.5 corresponds to the conventional form in which both stress points are located at the centroid of the element, while R = 0 places the two stress points at the left and right edges of the element. In conventional SPH because the stress points coincide with the SPH node, particle i does not enter into the calculation of σ_{11}^{i} , since $\frac{\partial W^{ij}}{\partial x^{j}} = 0$ for a symmetric kernel when i = j. In this new approach, which we refer to as unconventional or UC, for $0 \le R < 0.5$ node *i* will be included in the stress calculations for SPH element *i*. Thus stress calculations from the unconventional approach should be more accurate since node *i* will be included at the stress points i_a and i_b (see Figure 1). Also the UC method should help to lessen or eliminate oscillation or the "sawtooth" effect evident in standard SPH stress calculations [4].

In additon and more importantly, however, the UC approach should also help to address the tension instability present in the conventional (R = 0.5) SPH. The reason for this is as follows. Assume a uniform 1D grid, as shown in Fig. 1, with the smoothing length 2h = 2L, where L is the length of each SPH element. For the conventional approach, all nodes for the SPH gradient calculations are at least h (=L)distance away, and node i is not included in any of the calculations for stress and linear momentum at i. In [2], it has been shown that instability in tension is governed by (29), and for the standard cubic-b-spline kernel this is satisfied in tension whenever $x_{ii} > 0.6h$ (approximately). Unfortunately for conventional SPH methods, (29) is satisfied for all the SPH nodes included in the gradient calculations at node i. As mentioned above for the UC approach, node i is included in the stress calculations at the stress points i_a and i_b (R < 0.5) as shown in Fig. 1. Also, the stress points i_a and i_b will be included in the linear momentum calculated at node *i*. Thus, (29) will not be satisfied at these two stress points. In addition, the two SPH elements adjacent to i, i-1 and i+1, may also contain stress points (depending on the value of R) which do not satisfy (29). Therefore, the UC method should have a strong stabilizing effect on the SPH mesh in tension, not allowing it to prematurely fragment.

8

6. NUMERICAL IMPLEMENTATION - SPH1D

For the unconventional (UC) formulation in our 1D code SPH1D, four options have been programmed for the calculation of stress and linear momentum. These options are specified by the parameter JSP. Gradient calculations used to determine the stress (σ_{11}) employ either (6) or (7) in this work. Linear momentum calculations are based on either (9) or (10). Recall that (9) does not conserve momentum, but for the UC formulation (6) and (9) are more accurate than (7) and (10). Equations (7) and (10) represent the standard SPH approach in which additional approximations have been introduced [4,5].

JSP = 1 corresponds to the use of (7) and (10) or the standard SPH particle equations, but with the UC formulation in which the stress points are not located at the SPH nodes ($0 \le R < 0.5$). For JSP = 2, (6) is employed for the stresses while (9) is used for linear momentum calculations. JSP = 3 uses (6) for the stresses and (10) for linear momentum. Thus, options JSP =1 and 3 introduce additional approximations for the UC, but (10) does enforce the conservation of momentum. Also it is noted that for the linear momentum calculations using either (9) (JSP =2) or (10) (JSP = 1 or 3), each of the two stress points within a particular SPH element (particle) is assumed to possess one half of the total mass of that element - this allows retention of the particle concept. In addition, when using (10) (JSP = 1 or 3), the stress at the typical SPH node *i*, since it is not calculated, is assumed to simply be the average of the two stress points i_a and i_b (see Fig. 1) in that element.

The fourth option (JSP = 4) in SPH1D uses the same equations as JSP =2, or (6) for stress calculations and (9) for linear momentum. However, with JSP = 4 the stress σ_{11} is calculated through (6) and (27) and (28) only at the element centroid (as if R = 0.5 in Figure 1) and it is assumed that:

$$\sigma_{11}^{i} = \sigma_{11}^{i_{a}} = \sigma_{11}^{i_{b}}$$
(31)

So σ_{11} is assumed to be constant in SPH element *i*. Linear momentum is then calculated using (9) but with the stresses applied not at *i* (the centroid) but rather at i_a and i_b as specified in the UC approach when $0 \le R < 0.5$. The option JSP = 4, thus represents a compromise between the conventional and unconventional SPH approaches, that may at least help to stabilize the mesh in tension, although not addressing stress oscillation or "sawtooth" behavior.

Ghost particles are employed for the application of essential boundary conditions. See [5] for the enforcement of free and fixed end conditions. Also, explicit time integration in the form of a modified central difference (MCD) method is applied to the linear momentum equations. Taylor and Flanagan [9] briefly discuss the MCD method which consists of a forward difference to compute the velocities followed by a backward difference to calculate the displacements. In equation form the MCD is:

$$\mathbf{Y}(t) = \mathbf{Y}(t-dt) + dt \cdot \mathbf{A}(t-dt)$$
(32a)

$$\underline{U}(t) = \underline{U}(t - dt) + dt \cdot \underline{V}(t)$$
(32b)

where \mathbf{A} is the acceleration and dt is the time increment.

Overall, the SPH1D program is a relatively simple 1D code, but it will be a useful platform to demonstrate our unconventional approach. Finally, in SPH1D only a linear elastic material model has been implemented at this point.

7. APPLICATION

In this section as test of our unconventional approach, the SPH1D code will be applied to the elastic 1D bar described in Fig. 2. The bar is fixed at the right end B and the left one quarter of the bar is given an initial velocity of V_0 =-5 m/sec, thus putting the bar in tension initially. Standard SPH methods (R = 0.5) can not solve this problem due to the tension instability that will immediately develop. As indicated in Fig. 2a, the SPH grid is very coarse with only 40 uniform SPH elements (particles) used in the model. A comparable finite element model using the ABAQUS [10,11] program that consists of 40, 2D solid elements is described in Fig. 2b. Fig. 3 presents the displacement time history of the left end A (SPH node 1 actually) for the UC results with R = 0.25 (so the stress points are at the quarter points of the SPH elements). Also included are the finite element method (FEM) results using ABAQUS with implicit time integration, which is unconditionally stable. The time step used for the explicit SPH calculations as well as for the implicit FEM model is dt= 0.4 E-6 sec. This time step is based upon an estimate of the Courant stability limit of approximately 0.66 E-6 sec., which is determined by dividing the length of the typical SPH element by the wave speed C.

J

As indicated in Fig. 3, the SPH1D results (solid line) with R = 0.25 are very close to the ABAQUS predictions (dashed line) for the displacement history at the left end of the bar, point A. Some slight phase difference between the two analyses develops later but this is probably due to the use of an explicit solver for SPH1D and an implicit solver for ABAQUS, as well as basic differences between the SPH and FEM forms of discretization.

The JSP = 2 option was used to generate Fig. 3. This option in SPH1D employs (6) for stress calculations and (9) for linear momentum. The use of the other options, JSP =1,3 which employ (10) and do conserve momentum as well as JSP = 4, produced results that eventually went unstable. JSP = 4 went unstable the quickest in the analysis. Reducing the timestep dt and increasing the artifical viscosity did not stabilize the SPH1D calculations for JSP =1, 3 or 4. Only JSP =2 remained stable and all the results in this section used that option.

Various values for R in the approximate range of $0 \le R \le 0.40$ produced SPH results very similar to Fig. 3 in which R = 0.25. For the range $0.4 < R \le 0.5$, the response of the bar tended to be become unstable as the two stress points approached the centroid of the SPH element (R = 0.5).

Fig. 4 indicates the predicted time history for the velocity of the left end A for SPH1D and ABAQUS. In general, the agreement is excellent with again some slight phase differences developing as the analysis goes on.

In Figs. 5 and 6, the stress σ_{11} at SPH node 11 (stress point 11_a) is compared to σ_{11} at the centroid of finite element 11 - point C in Figs. 2a and 2b. Fig. 5 indicates the extended time history of σ_{11} . Fig. 6 plots the results only up to .0002 sec. so that the differences in the two analyses are more evident. As indicated in Fig. 6, early in the analysis especially the SPH1D stress results do tend to fluctuate more than the ABAQUS results . In general, the SPH1D predictions for σ_{11} compare reasonably well to ABAQUS, but not as well as did the displacement and velocity at A as indicated in Figures 3 and 4. This is not too surprising since stress is determined by differentiating the displacement, and the mesh is very coarse with only 40 SPH and 40 FEM elements used in the analyses.

٩

The 1D bar is next given an initial velocity of $V_0 = 5$ m/sec, thus putting the bar in compression initally. This is done to determine the effect of the UC formulation on the stress oscillation or "sawtooth" behavior encountered in the standard or conventional SPH approach (R = 0.5). Putting the bar initially in compression allows a comparison between the UC and the conventional SPH, since the latter is not stable in tension. Fig. 7 shows a snapshot of the stress σ_{11} along the entire bar at t = 1.0 E-5 sec. The UC results with R = 0.25 are indicated by the solid line, and the conventional SPH results in which R = 0.5 are depicted by the dashed line. In general, we see that the UC results are much smoother indicating a lessening of the "sawtooth" behavior.

8. CONCLUSIONS

In this work, tension instability has been addressed for SPH methods. A new unconventional (UC) approach has been presented in which the stresses are computed at points away from the SPH nodes. The location of these two stress points within a 1D SPH element (particle) is controlled by the parameter R. The UC approach removed the tension instability in the bar considered for R in the approximate range of $0 \le R < 0.45$. No optimum value of R was found in general. UC displacement and velocity results were in excellent agreement with a comparable ABAQUS finite element model. Axial stress comparison to the ABAQUS model were not quite as good, but that probably is to be expected given the coarseness of the meshes and the non-local nature of the SPH method, which interpolates outside the SPH element (unlike FEM). In a comparison to standard SPH (R = 0.5) for the bar in compression initially, the UC seemed to alleviate the stress oscillation or "sawtooth" effect.

The UC approach was successful in removing tension instability, but only for the option JSP = 2, in which (6) and (9) were used for the stress and momentum calculations. Equations (6) and (9) are not widely used in the SPH literature, and (9) does not conserve momentum. These are important points to note with the UC approach.

Unfortunately, the JSP =4 option (like JSP =1 and 3) failed to remain stable for the UC method. If this option had been successful, modifications to existing SPH codes would have been easier and less expensive since only one stress point (the centroid) within each particle would have to be tracked.

Overall, based on the results from the simple 1Dbar, the use of the unconventional approach for SPH calculations is very encouraging. This approach can be extended to 2D and 3D problems, but this will be computationally expensive since additional stress points within each SPH element have to be tracked in the analysis besides just the centroid. Also, extension to 2D and 3D may require a rethinking of the overall particle concept. Finally, perhaps a method analogous to hourglass control for reduced integration in finite element techniques [11] may be possible with the UC method.

9. ACKNOWLEDGMENTS

This work was supported in part by ARPA, and that support is gratefully acknowledged. The authors would also like to thank Dr. R. Balaliance, Head of the Mechanics of Materials Branch at NRL for his encouragement and support in general. In addition, the encouragement and enthusiasm from Dr. E. Oran, Senior Scientist for Reactive Flow at NRL, was very helpful and greatly appreciated.

13

10. REFERENCES

- W. Benz, Smooth particle hydrodynamics: a review. Numerical Modeling of Nonlinear Stellar Pulsation: Problems and Prospects, Kluwer Academic, Boston, (1990).
- 2. J. W. Swegle, D. L. Hicks, and S. W. Attaway, SPH stability analysis. A Colloquium on Advances in Smooth Particle Hydrodynamics, Albuquerque (1993).
- 3. D. L. Hicks, J. W. Swegle, and S W. Attaway, Smoothing and SPH. A Colloquium on Advances in Smooth Particle Hydrodynamics, Albuquerque (1993).
- J. W. Swegle, S. W. Attaway, M. W. Heinstein, F. J. Mello, and D. L. Hicks, An Analysis of Smoothed Particle Hydrodynamics. Sandia National Lab, Report No. SAND93-2513UC-705, Albuquerque, March (1994).
- L. D. Libersky and A. G. Petschek, Smooth particle hydrodynamics. Proc., The Next Free Lagrange Conference, Jackson Hole, WY, 1990 (Edited by H. E. Trease, J. W. Fritts, and W. P. Crowley), Springer-Verlag, New York (1991).
- J. J. Monaghan, Smoothed particle hydrodynamics. Annu. Rev. Astron. Astrophys. 30, 543-574 (1992).
- L. D. Libersky, A. G. Petschek, T. C. Carney, J. R. Hipp and F. A. Allahdadi, High strain Lagrangian hydrodynamics: a three dimensional SPH code for dynamic material response. *J. Comput. Phys.* 109, 67-75 (1993).
- D. J. Benson, Computational methods in Lagrangian and Eulerian hydrocodes. Comp. Meth. App. Mech. Engin. 99, 235-394 (1992).
- L. M. Taylor and D. P. Flanagan, *PRONTO2D*, A Two Dimensional Transient Solids Dynamics Program. Sandia National Lab, Report No. SAND 86-0594UC-32, Albuquerque, March (1987).
- ABAQUS Theory Manual, Version 5.2. Hibbitt, Karlsson and Sorensen, Inc., Pawtucket, RI (1992).
- 11. ABAQUS User's Manual, Volumes 1 and 2, Version 5.2. Hibbitt, Karlsson and

Sorensen, Inc., Pawtucket, RI (1992).

•

.

٠

12. R. D. Cook, D. S. Malkus, and M. E. Plesha, *Concepts and Applications of Finite Element Analysis*, 3rd Edn. Wiley, New York (1989).



Figure 1. Typical 1D SPH elements for the unconventional approach. When R = 0.5 have the conventional SPH method.



Figure 2. A simple bar given an initial velocity: (a) SPH1D grid, (b) ABAQUS FEM grid.



¥

5

Figure 3. Displacement history for the left end of the bar (point A).



Figure 4. Velocity history for the left end of the bar (point A).



Figure 5. Extended stress history at node 11 (point C).

1

4

ż



Figure 6. Early stress history at node 11 (point C).



1

ź.

Figure 7. SPH1D UC and conventional stress predictions for the bar at t = 1.0E-5 sec (V₀ = .5 m/sec).