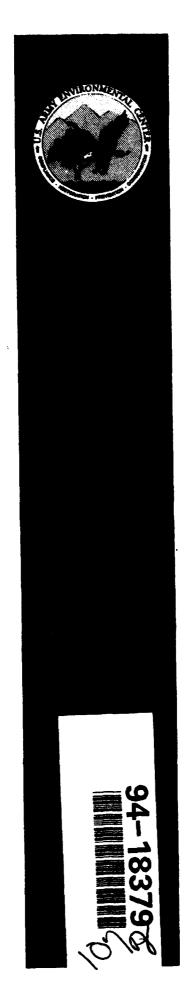
Report No. SFIM-AEC-BC-CR+2312C



AD-A280 331

FINAL

INTERMEDIATE AND ALLUVIAL WELL INSTALLATION REPORT

SITE 4, EXPLOSIVE WASHOUT LAGOONS

SUPPLEMENTARY REMEDIAL INVESTIGATION/ FEASIBILITY STUDY OF UMATILLA DEPOT ACTIVITY HERMISTON, OREGON

Contract No. DAAA15-90-D-0015 Delivery Order No.10

Prepared for:

U.S. ARMY ENVIRONMENTAL CENTER Aberdeen Proving Ground, Maryland 21010

076

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1. AGENCY USE ONLY (Leave blank)		E AND DATES COVERED - Feb 94)				
4. TITLE AND SUBTITLE Intermediate and Aluvial Well Install Site 4, Explosive Washout Lagoons Supplementary Remedial Investigati Umatilla Depot Activity, Hermiston,	ion/Feasibility Study of	<u>_</u>		DING NUMBERS AAA15-90-D-0015 0		
6. AUTHOR(S) Ritchie, E., T. Llewellyn, M. Ochsner, S. Lemont, R. Tucker						
7. PERFORMING ORGANIZATION NA Dames & Moore 849 International Drive, Suite 320 Linthicum, Maryland 21090		FORMING ORGANIZATION ORT NUMBER				
9. SPONSORING/MONITORING AGEN		10. SPONSORING/MONITORING AGENCY REPORT NUMBER				
U.S. Army Environmental Center Aberdeen Proving Ground, Marylan	SFIN	SFIM-AEC-BC-CR-93120				
11. SUPPLEMENTARY NOTES			J			
12a. DISTRIBUT, ON/AVAILABILITY ST	ATEMENT		12b. DI	STRIBUTION CODE		
Distribution Unlimited Approved for Public Release						
13. ABSTRACT (Maximum 200 words)					
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14. SUBJECT TERMS	geology Investigation, Remedial Invest	insting /Fersibility Cauda		15. NUMBER OF PAGES 106		
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17. SECURITY CLASSIFICATION OF REPORT	18. SECURITY CLASSIFICATION OF THIS PAGE	19. SECURITY CLAS OF ABSTRACT	SIFICATION	20. LIMITATION OF ABSTRACT		
NSN 7540-01-280-5500		L		Standard form 298 (Rev. 2-89)		

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Prescribed by ANSI Std. 239-18

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LIST OF ACRONYMS AND ABBREVIATIONS

bls	Below land surface
BRAC	Base Realignment and Closure
1,3-DNB	1,3-Dinitrobenzene
2,4-DNT	2,4-Dinitrotoluene
DOT	U.S. Department of Transportation
FSP	Field Sampling Plan
HMX	High Melting Explosive
ID	Inside diameter
NGVD	National Geodetic Vertical Datum
PVC	Polyvinyl chloride
RDX	Royal Demolition Explosive
RI	Remedial Investigation
1,3,5-TNB	1,3,5-Trinitrobenzene
2,4,6-TNT	2,4,6-Trinitrotoluene
UMDA	Umatilla Depot Activity
USAEC	U.S. Army Environmental Center, formerly USATHAMA
USATHAMA	U.S. Army Toxic and Hazardous Materials Agency
XRF	X-ray fluorescence

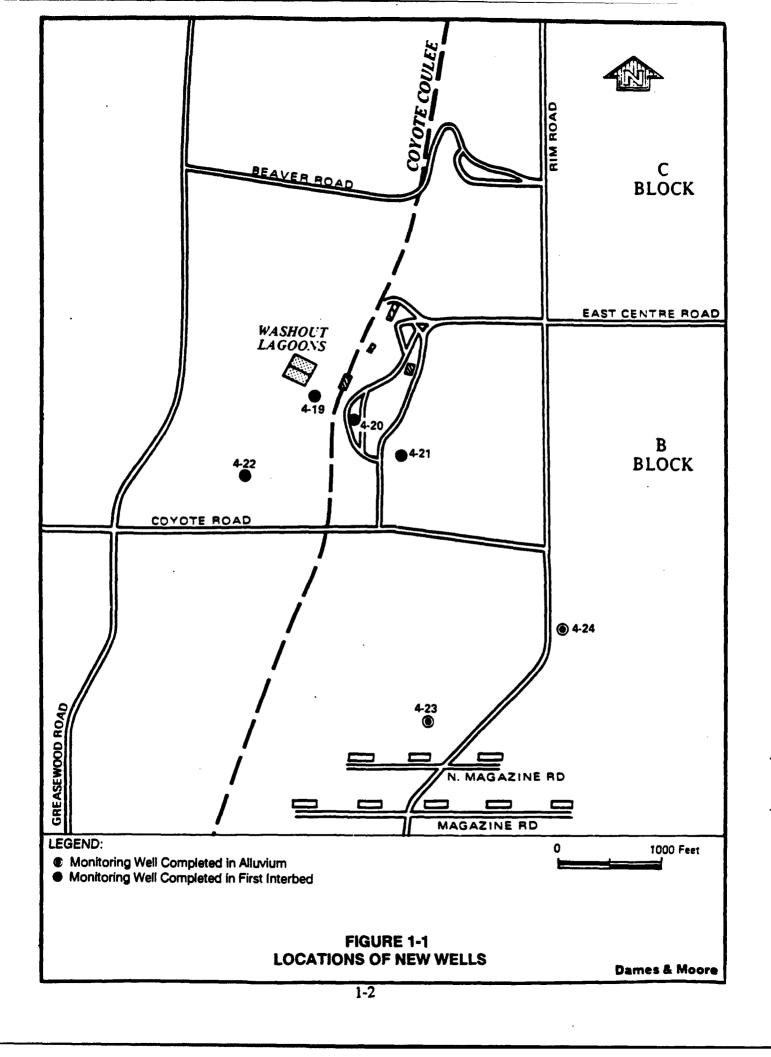
1.0 INTRODUCTION

This report presents the well installation methods and details for four intermediate wells and two alluvial wells installed at Site 4, Explosive Washout Lagoons, Umatilla Depot Activity (UMDA), Hermiston, Oregon. It has been prepared for the U.S. Army Environmental Center (USAEC) formerly the U.S. Army Toxic and Hazardous Materials Agency (USATHAMA), under the Base Realignment and Closure (BRAC) Program, Contract No. DAAA15-90-D-0015, Delivery Order No. 10. The purpose of the report is to document the methods used for the well installations and the hydrogeologic conditions encountered.

As discussed in detail in the Remedial Investigation (RI) report for UMDA (Dames & Moore, 1992b), during previous investigations at Site 4 contamination detected in the unconfined alluvial aquifer was attributed to historical activities at the Explosive Washout Lagoons. Contaminants consist primarily of Royal Demolition Explosive (RDX), High Melting Explosive (HMX), 1,3,5,-trinitrobenzene (TNB), 1,3-dinitrobenzene (DNB), 2,4,6-trinitrotoluene (TNT), 2,4-dinitrotoluene (DNT), nitrobenzene, and nitrate/nitrite. Explosives (primarily RDX) and nitrate/nitrite were also detected in deep basalt wells installed into the Columbia River Basalts (4-8, 4-9, 4-10, and 4-17). The nature of the migration pathways for contaminant transport between the alluvial and basalt aquifers was unclear. It was surmised that the migration was facilitated by either natural fracturing within the basalt flows or manmade pathways created during the installation of the four deep wells.

To assist in further evaluating conditions at Site 4, four intermediate monitoring wells--4-19, 4-20, 4-21, and 4-22--were installed as part of the current investigation. The locations of these wells are shown in Figure 1-1. The objectives of the investigation were to:

- Further assess the stratigraphy beneath Site 4.
- Assess the degree of hydraulic interconnection between the basalt and the overlying alluvial sediments.



- Document water quality within the intermediate zone between the existing shallow alluvial and deep basalt wells.
- Document horizontal and vertical groundwater flow patterns within the intermediate zone.

In additior, two new alluvial wells--4-23 and 4-24--were installed to the south and southeast of the Explosive Washout Lagoons (see Figure 1-1) to further delineate the extent of explosives and nitrate/nitrite contamination within the alluvial aquifer. Note that the chemical analysis results for the alluvial (as well as intermediate) wells and contamination assessment are presented in an addendum to the RI report.

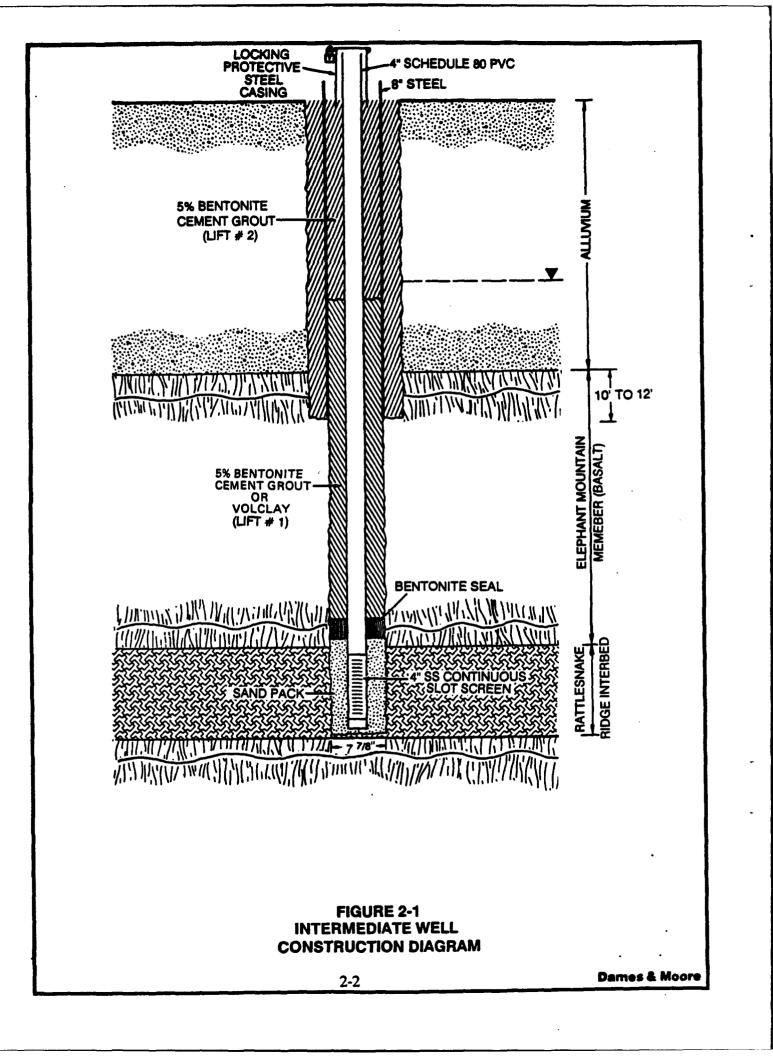
2.0 INTERMEDIATE WELL INSTALLATION METHODS

2.1 GENERAL WELL INSTALLATION METHODS

The four intermediate wells (4-19, 4-20, 4-21, and 4-22) were installed in accordance with the specifications of the Field Sampling Plan (FSP) Addendum (Dames & Moore, 1992a). The generalized intermediate well construction method is described below and is illustrated in Figure 2-1. Any deviations from the FSP Addendum are also discussed. Boring logs and well construction diagrams for each well are presented in Appendices A and B, respectively.

To minimize the potential of introducing contamination from the alluvial aquifer into the intermediate zone, the boreholes were telescoped at the bedrock contact. A Drill Systems AP-1000 reverse circulation percussion hammer rig was used to penetrate the upper alluvial deposits. An Ingersoll-Rand TH-60 air rotary rig was used to advance the hole in bedrock. In general, the following procedures were used during the drilling and well installation process:

- Triple wall drill casing was driven to refusal with the AP-1000 percussion hammer drill rig. The depth of refusal was between 54.5 and 155 feet below land surface (bls), depending on the well location. The inner dual-wall drill pipe was removed, leaving the 12-inch inside-diameter (ID) steel drill casing in place to prevent the borehole from collapsing. Split-spoon samples were collected at a 5-foot interval for the first well installed (4-19) and a 10-foot interval for the other three wells. As discussed with and agreed to by USAEC, the change from a 5-foot to a 10-foot interval was made primarily because of the homogenous subsurface conditions encountered in the first well and the difficulty in recovery of samples from the cobble-rich matrix of the alluvium.
- The TH-60 air rotary drill rig was moved onto the borehole to advance it into the basalt and to spot core the basalt to verify its nature and composition (see Section 4.3 for a description of this unit, the Elephant Mountain Basalt). An NW size core barrel (2.985 inches by 10 feet in length) was used for spot



coring. When core samples were not being collected, a 5 1/2-inch downhole hammer drill bit with reverse air circulation was used to advance the borehole.

- The borehole into the upper basalt was advanced 10 to 12 feet into the unit and then reamed to 10 inches in diameter. An 8-inch steel casing was installed from the surface to 10 to 12 feet into the basalt. This casing was grouted in place using bentonite/cement grout tremied to the bottom of the borehole. The entire annulus was filled with bentonite/cement grout, and a substantial grout plug was tremied inside the 8-inch casing.
- The 12-inch triple wall casing was withdrawn, maintaining a head on the grout in the borehole to preclude collapse of the formation.
- The grout was allowed to cure for at least 12 hours.
- Following the curing period, the grout plug was drilled out and the borehole was advanced to the bottom of the first interbed by direct air rotary methods (see Section 4.3 for a description of this unit, the Rattlesnake Ridge Interbed). During advancement of the borehole, the basalt was spot cored with an NW size core barrel to verify its nature and composition.
- Packer tests were conducted during installation of two of the intermediate wells--4-20 and 4-22. Although not included in the FSP Addendum, as discussed with and agreed to by USAEC, the packer tests were conducted to assess the horizontal permeability of the upper basalt, for which no data were available from previous investigations. In both tests, a single packer was used to isolate a 6-foot interval of basalt within a borehole. Water loss at three pressures was recorded, and horizontal permeabilities were calculated. Data are presented in Appendix C.
- Following the packer test (if it was performed) and prior to installation of the monitoring well casing, the borehole was opened to 7 7/8-inch diameter with the TH-60 air rotary drill rig using a downhole hammer drill bit.

- At the completion of drilling, the drill rig was used to condition the borehole prior to well installation. Borehole conditioning is an important part of the drilling process, because it helps keep the borehole stable and free of obstructions. This was accomplished by slowly moving the drill bit up and down in the borehole while circulating air to remove any dislodged material.
- Monitoring well construction consisted of installing a 4-inch Schedule 80 polyvinyl chloride (PVC) well casing in the borehole. A 10-foot continuously wound stainless-steel well screen (slot size 0.02 inch) was set in the first interbed (150 to 200 feet bls). The continuous slot stainless-steel screen will allow use of any of the intermediate wells as recovery wells if remediation of this interbed is necessary, and if the well locations are consistent with remedial design objectives. A sand pack was placed to 3 to 5 feet above the top of the screen. The filter pack was a 10-20 Colorado Silica sand. A bentonite seal, 3- to 5-feet thick, was placed above the filter sand (Peltonite). Volclay was tremied (in one lift) to the water table in wells 4-19 and 4-21. A 5 percent bentonite/cement grout was tremied to the water table in wells 4-20 and 4-22. This first lift was then allowed to cure. Bentonite/cement grout was tremied from the top of the volclay or grout to the surface in a second lift. The use of volclay was discontinued after wells 4-19 and 4-21 because of the length of time required for it to cure. The long curing time necessitated a 4- to 6-day delay between the first and second grout lifts. While an adequate seal appeared to become established with the volclay, USAEC and Dames & Moore agreed that the 5 percent bentonite/cement grout would be used exclusively in the remaining two wells (4-20 and 4-22), because the faster curing time permitted the second grout lift to be conducted within 12 hours of the first.

Surface completions for all the wells consisted of a protective steel casing over the PVC well casing, four steel pickets, and a gravel blanket. The steel protective casing was 8 inches in diameter and extended approximately 2.5 feet below and 2.5 feet above the ground surface. It was fitted with a sliding steel cap that can be locked to the protective

casing. The steel pickets are 3 inches in diameter and were placed approximately 4 feet from the protective casing. The pickets extend approximately 3 feet below and 3 feet above the ground surface. A 6-inch gravel blanket covered the area between the pickets and the protective casing. A mortar collar was placed inside the protective casing to approximately 6 inches above the ground surface, and a 1/4-inch drainage hole was drilled through the protective casing 1/8-inch above the mortar collar. The protective casing and pickets were painted orange, and the monitoring well's identifying number was painted in white on the protective casing.

All cuttings generated during the drilling activities were placed in new 17H U.S. Department of Transportation (DOT)-certified drums and labeled with well number, date, depth, and contents. Water generated during drilling was contained and transferred to the evaporation basins. The drums of cuttings are being stored in a central area warehouse at UMDA (Building 412) until the start of full-scale remediation of the contaminated soil at the Explosive Washout Lagoons. At that time, the remediation contractor will add the drummed cuttings from the Site 4 wells to the contaminated soil from the lagoons and the material will be composted.

The monitoring wells were developed for approximately 2 hours, between 2 to 7 days after emplacement of the internal mortar collar. Well development was performed using a 5-foot stainless-steel bailer and a submersible pump. The wells were first bailed to help remove any sediment that may have collected in the bottom of the casings. In addition, inserting and removing the bailer created a surging action that helped to clear the well screens. After bailing, the wells were pumped using a submersible pump to complete the development process. Temperature, pH, and conductivity were measured in the water removed from each well at least five times throughout development, always including the first and last water removed from the casings. Field readings collected during well development are provided in Appendix D. Development generally continued until five well volumes--including both the casing and saturated annulus--were removed, the physical parameters were reasonably stable, and the water appeared to be clear. The bailer and submersible pump were steam cleaned between wells; the pump discharge line for each well consisted of dedicated polyethylene tubing.

2.2 WELL-SPECIFIC CONSTRUCTION DETAILS

Table 2-1 summarizes the intermediate well construction. Appendices A and B provide the boring logs and well construction diagrams, respectively. Specific details on the construction history of each of the intermediate wells are as follows:

- <u>Well 4-19</u> was initiated on 11/11/92. The well was installed on 11/16/92, including the first lift of the volclay. Final surface completion was conducted on 11/20/92. The well was developed on 12/15/92, slug tested on 12/19/92, and groundwater samples collected on 1/8/93.
- <u>Well 4-20</u> was initiated on 11/30/92. The well was installed on 12/7/92, including the first lift of bentonite/cement grout. Final surface completion was conducted on 12/8/92. The well was developed on 12/18/92, slug tested on 12/19/92, and groundwater samples collected on 1/6/93.
- <u>Well 4-21</u> was initiated on 11/15/92. The well was installed on 11/19/92, including the first lift of the volclay. Final surface completion was conducted on 11/21/92. The well was developed on 12/16/92, slug tested on 12/19/92, and groundwater samples collected on 1/7/93.
- <u>Well 4-22</u> was initiated on 12/3/92. The well was installed on 12/9/92, including the first lift of bentonite/cement grout, with final surface completion on 12/10/92. The well was developed on 12/15/92, slug tested on 12/19/92, and groundwater samples collected on 1/9/93.

TABLE 2-1

INTERMEDIATE WELL CONSTRUCTION SUMMARY

Grout Type (first Lift)	Volciay	Volclay	Bentonite/ Cement Grout	Bentonite/ Cement Grout
Static Water Elevation ¹ (msl)	445.23	470.90	460.68	456.45
Static Water Level 1.3 (ft)	112.57	145.50	153.92	58.35
Screened Interval (msl)	407.80397.80	418.40408.40	412.60-402.60	414.80-404.80
Screened Interval (ft)	150-160	198-208	202-212	100-110
Depth of 8" Casing (ft)	105.0	162.0	167.0	67.0
Depth to Refusal (ft)	94.5	151.0	154.5	55.0
Total Depth (ft)	168.0	218.0	215.0	119.0
Measuring 7 Point I (msi)	559.59	617.45	616.57	516.58
Surface Elevation (ft)	557.80	616.40	614.60	514.80
Well No.	4-19	4-20	4-21	4-22

Measured December 16, 1992.
 Depth below measuring point (top of casing)

:

2-7

3.0 ALLUVIAL WELL INSTALLATION METHODS

3.1 GENERAL WELL INSTALLATION METHODS

Two alluvial wells (4-23 and 4-24) were installed in accordance with the specifications of the FSP Addendum (Dames & Moore, 1992a). Boring logs and well construction diagrams are presented in Appendices A and B, respectively. A Drill Systems AP-100reverse circulation percussion hammer rig was used to penetrate the upper alluvial deposits. The following procedures were employed during the drilling and well installation process:

- Dual-wall drill casing was driven at least 10 feet into the unconfined alluvial aquifer with the AP-1000 percussion hammer drill rig. The depth to the unconfined aquifer was approximately 100 to 103 feet bls, depending on the well locations. Split-spoon samples were collected at selected intervals to verify subsurface lithology.
- Monitoring well construction consisted of installing a 4-inch Schedule 40 PVC well casing in the borehole. A 15-foot cut slot screen (slot size 0.01 inch) was set approximately 10 feet into the water table. A 15-foot screen was used to accommodate seasonal fluctuations of 3 to 5 feet in water table elevations in the unconfined aquifer. The filter pack--a 10-20 CSSI grade Colorado Silica sand--was placed to 5 feet above the top of the screen. A 5-foot-thick bentonite seal was placed above the filter sand (Peltonite). A bentonite/cement grout was tremied from the top of the bentonite grout to the surface.

Surface completions, cuttings disposal, and well development for the two alluvial wells were performed according to the methods described for the intermediate wells in Section 2.1. Figure 3-1 shows the general alluvial well construction details.

3.2 WELL-SPECIFIC CONSTRUCTION DETAILS

Table 3-1 summarizes the alluvial well construction. Appendices A and B provide the well logs and well construction diagrams, respectively. Specific details regarding the construction history of each of the alluvial wells are as follows:

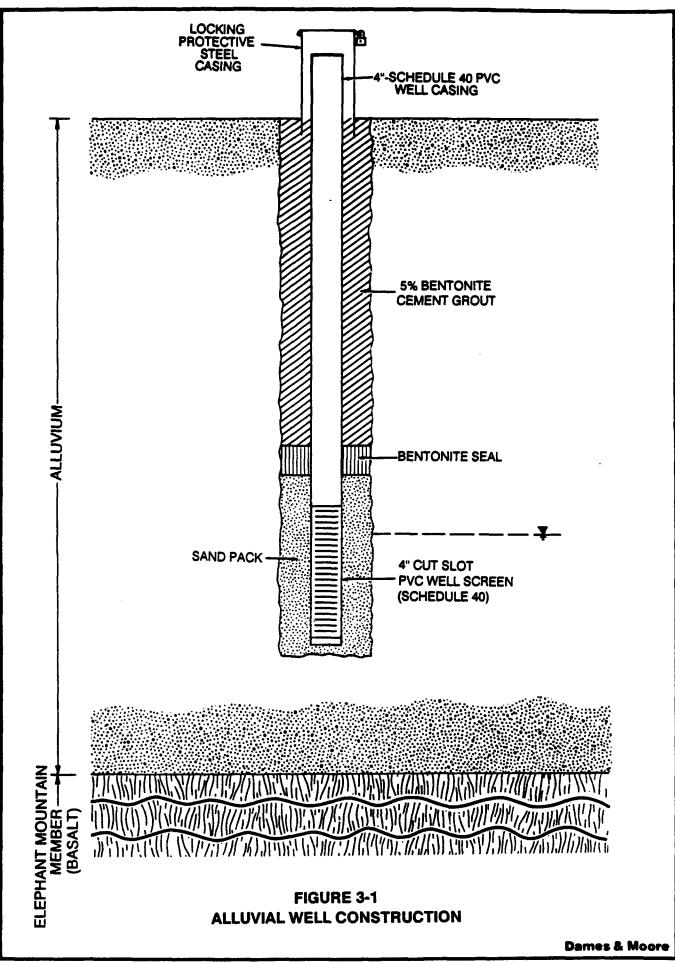


TABLE 3-1

Weil No.	Surface Elevation (ft)	Measuring Point (msl)	Total Dèpth (ft)	Screened Interval (ft)	Screened Interval (msl)	Static Water Level ^{1,2} (ft)	Static Water Elevation ¹ (msl)
4-23	593.4	595.34	111	94-109	499.4484.4	100.45	492.95
4-24	596.5	598.15	113.5	96.5-111.5	500.0485.0	103.45	493.05

.

ALLUVIAL WELL CONSTRUCTION SUMMARY

¹ Measured December 16, 1992. ² Depth below measuring point (top of casing).

- <u>Well 4-23</u> was initiated on 11/14/92. The well was installed and completed on 11/15/92. The well was developed on 11/19/92 for a period of 1 hour. Slug testing was conducted on 11/23/92, and groundwater samples were collected on 12/15/92.
- <u>Well 4-24</u> was initiated on 11/14/92. The well was installed and completed on 11/15/92. The well was developed on 11/19/92 for a period of 1 to 2 hours. Slug testing was conducted on 11/23/92, and groundwater samples were collected on 12/15/92.

4.0 HYDROGEOLOGIC SETTING

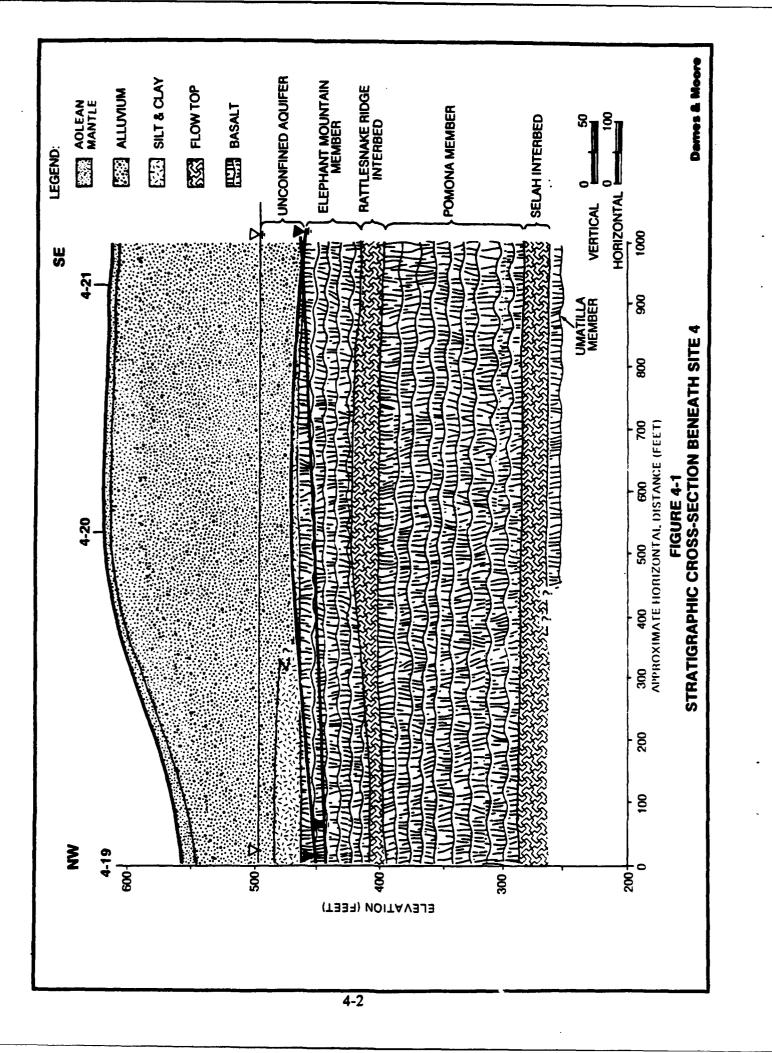
This section describes of the hydrogeologic conditions encountered beneath Site 4 during installation of the four new intermediate wells (4-19, 4-20, 4-21, and 4-22) and the two new alluvial wells (4-23 and 4-24). Also discussed are the hydrogeologic conditions encountered during previous installation of existing wells. Based on the additional data collected during this phase of work, the interpretation of Site 4 geology presented in the 1992 RI report (Dames & Moore, 1992b) has been somewhat revised. Generally, the geologic conditions encountered beneath Site 4 consist of a relatively thin aolean mantle at the surface, Pleistocene alluvial flood deposits, and Miocene basalt flows of the Columbia River Basalts. Figure 4-1 presents a northwest/southeast-trending geologic cross-section through Site 4, based on information from the new and existing wells.

4.1 AOLEAN DEPOSITS

The aolean deposits (windblown sediments) encountered in all new wells varied in thickness from approximately 2 feet (east of Coyote Coulee; see below for description) to 15 feet (west of the Coulee). The aolean mantle consists of a well sorted, fine-to-medium angular sand, tan-to-grey in color. These deposits are similar to those recorded in the boring logs of other Site 4 wells previously installed.

4.2 ALLUVIAL DEPOSITS

The Pleistocene alluvial flood deposits beneath the aolean deposits at Site 4 were deposited by a series of catastrophic flood events, which were precipitated by the cyclic failure and re-building of the ice dams that impounded Glacial Lake Missoula, in western Montana, approximately 17,200 to 11,000 years ago (Breckenridge <u>et al.</u>, 1989). Each of these ice dam failures caused extremely high energy flows down the Columbia River. In the area of UMDA, these floodwaters expanded across the Umatilla Basin. Partial constriction of the Columbia River downstream of the Umatilla Basin caused hydrodynamic damming of the floodwaters; formation of glacial Lake Condon; and deposition of significant thicknesses of rounded-to-subrounded sand-, gravel-, and cobble-sized clasts in the Umatilla Basin (Breckenridge <u>et al.</u>, 1989). Coyote Coulee, which runs through Site 4 on an approximate northeast/southwest trend, is a 50-foot escarpment interpreted to be formed



by standing waves in the floodwaters. Numerous smaller (10- to 20-foot) bottom ripples are present to the northwest of Site 4 and trend in the same direction.

Geologic samples collected from intermediate wells 4-20 and 4-21, and alluvial wells 4-23 and 4-24, suggest that--to the east of Coyote Coulee--the alluvial flood deposits consist of coarse sands, gravels, and cobbles composed primarily of basalt with lesser amounts of quartz, feldspar, and mica. These deposits extend from the surface (approximately 600 feet above National Geodetic Vertical Datum (NGVD) of 1929) to the basalt contact at approximately 460 feet above the NGVD. Directly above the basalt contact, the sediments are composed of slightly finer saturated silty sands.

Geologic samples from intermediate wells 4-19 and 4-22 suggest that the alluvial flood deposits west of the coulee are typically finer than those to the east, and consist of fine-to-coarse sand and gravel composed primarily of basalt with lesser amounts of quartz, feldspar, and mica. The alluvium extends from the base of the aolean mantle to an approximate elevation of 480 feet above the NGVD, where a sequence of fine sediments approximately 20-feet thick caps the basalt. Sediments are composed of silts, clays, and fine sand, and tend to be dry. At well 4-22, lake varves or rythmites were observed in these sediments, suggesting that they are lacustrine in origin. This unit may represent residual deposits associated with glacial Lake Condon. These lacustrine deposits--which extend to the top of the basalt--remained undisturbed in the lower energy environment on the downstream side of the standing wave associated with later flooding.

The alluvial flood deposits become saturated at approximately 495 feet above the NGVD. The saturated alluvium forms the uppermost aquifer beneath Site 4. It is unconfined, highly permeable, and ranges in saturated thickness from 35 feet (east of the coulee) to 10 to 15 feet (west of the coulee, where the lacustrine deposits thin the alluvium). Previous slug testing and permeability testing suggest a permeability range of 200 to 1,000 feet/day for the alluvium and 3 to 5 feet/day for the silt and clay of the lacustrine deposits (Dames & Moore, 1992b). Slug testing was conducted on the two new alluvial wells (4-23 and 4-24) and analyzed by methods proposed by Bouwer and Rice (1976). Permeability values of 1,571 and 1,944 feet/day, respectively, were obtained (Appendix E).

4.3 BASALT FLOWS

The uppermost basalt flows present beneath Site 4 are those of the Saddle Mountain Formation. Like all the Columbia River Basalts, they erupted during the Miocene Epoch. Flows and interbeds within the Saddle Mountain Formation consist of-from youngest to oldest-- the Elephant Mountain Member, the Rattlesnake Ridge Interbed, the Pomona Member, the Selah Interbed, the Umatilla Member, and the Mabton Interbed (Oberlander and Miller, 1981).

The uppermost basalt flow beneath Site 4 occurs at an approximate elevation of 460 feet above NGVD, or between 50 to 150 feet bls, depending on the location of the well. The topographic surface of the basalt at this area is relatively flat, with a variable depth to basalt due to the topographic expression of Coyote Coulee. Based on installation of the four new intermediate wells, this uppermost flow was found to be approximately 50 to 60 feet in thickness and composed of slightly-to-highly weathered aphanitic light grey basalt. Locally, alteration to kaolanitic clays was observed; however, overall, the rock was only slightly weathered and fractured. X-ray fluorescence (XRF) of core samples collected from this flow during the installation of the intermediate wells was used to evaluate the rock chemistry and identified the flow as the Elephant Mountain Member of the Saddle Mountain Basalts (Beeson, 1993). Results of this analysis are presented in Appendix F.

A flowtop interbed at an elevation of approximately 400 feet was encountered beneath the Elephant Mountain Member; this has been interpreted to be the Rattlesnake Ridge Interbed. The unit is approximately 20-feet thick and is composed of highly chloritized, highly vesicular basalt and interbed sediments. All four intermediate wells (4-19, 4-20, 4-21, and 4-22) were installed into this interbed.

Logs of the previously installed deep basalt wells (4-8, 4-9, 4-10, and 4-17) suggest that a second basalt flow, approximately 100-feet thick, is present beneath the Rattlesnake Ridge Interbed. This flow is interpreted to be the Pomona Member of the Saddle Mountain Basalts. A 20- to 30-foot-thick interbed present beneath this flow is interpreted to be the Selah Interbed. The deep basalt wells are all screened within this interbed. The Rattlesnake Ridge and Selah Interbeds represent confined or leaky confined aquifers, with the Elephant Mountain and Pomona Members acting as the poorly permeable confining layers that separate them. Packer testing conducted at intermediate wells 4-20 and 4-22 suggest that the horizontal permeabilities in the Elephant Mountain Member range from 0.01 to 0.16 feet/day (Appendix C). Slug testing of the new intermediate wells was conducted and analyzed by methods proposed by Cooper <u>et al.</u> (1967). These data suggest that the permeability of the Rattlesnake Ridge Interbed is between 277 and 624 feet/day (Appendix E). Earlier slug testing suggests that the formation permeability of the Selah Interbed is between 2 and 16 feet/day (Dames & Moore, 1992b).

5.0 SUMMARY

Four intermediate and two alluvial wells were installed at Site 4, Explosive Washout Lagoons, at UMDA. Although the geologic conditions encountered were somewhat different from those anticipated, well construction methods were largely in accordance with those presented in the FSP Addendum (Dames & Moore, 1992a). As discussed with and agreed to by USAEC, deviations from this plan included the addition of two packer tests at intermediate wells 4-20 and 4-22; collection of split-spoon samples at 10-foot (rather than 5-foot) intervals at intermediate wells 4-20, 4-21, and 4-22; and use of a bentonite/cement grout mix in place of volclay to seal the borehole annulus beneath the water table at intermediate wells 4-20 and 4-22. Based on information from the installation of the new wells and existing wells, the geology encountered beneath Site 4 consists of an aolean mantle, alluvial flood deposits, lacustrine sediments in some areas, and basalt flows with

associated interbeds. The alluvial flood deposits form a highly permeable but relatively thin unconfined aquifer. The basalt interbeds form a sequence of confined or leaky confined aquifers separated by the basalt flows. The intermediate wells were installed into the uppermost interbed aquifer (Rattlesnake Ridge Interbed).

As discussed in Section 1.0, data were also collected to assess the degree of hydraulic connection between the aquifers, to document water quality within the unconfined alluvial aquifer and Rattlesnake Ridge Interbed, and to document horizontal and vertical groundwater flow patterns. These data are included in an addendum to the RI report.

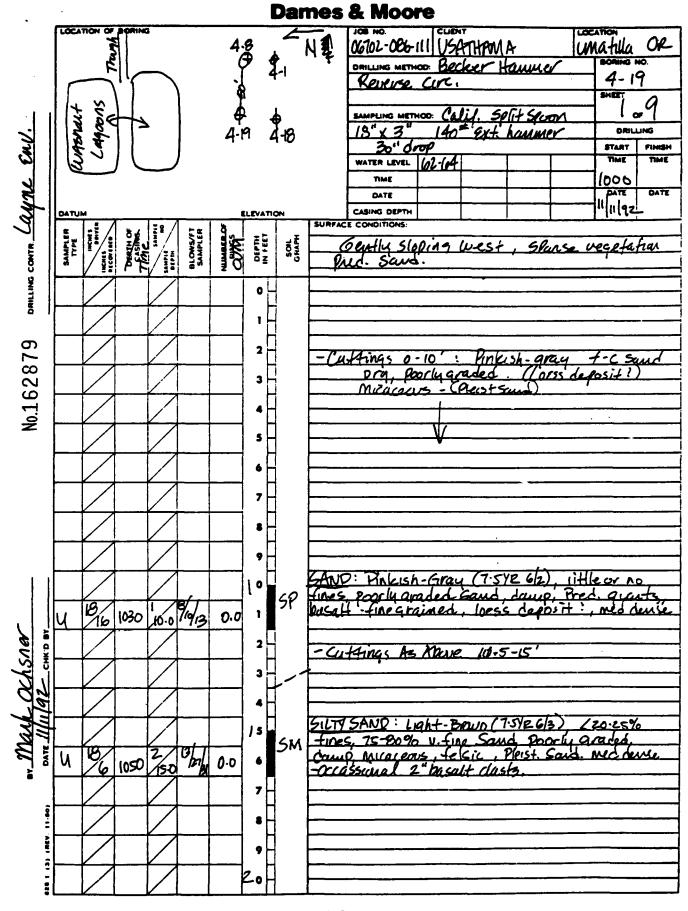
6.0 REFERENCES

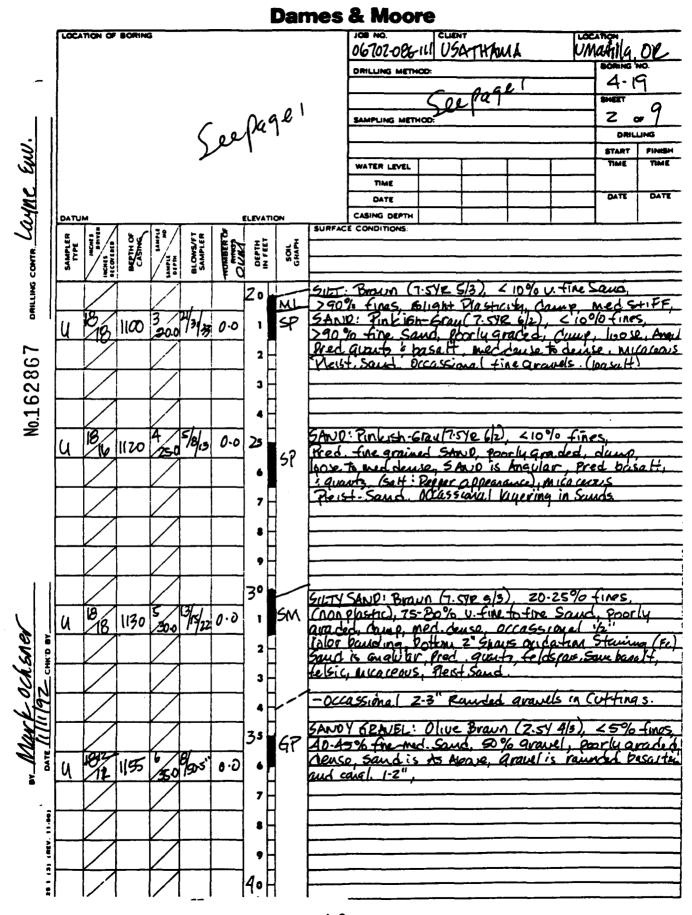
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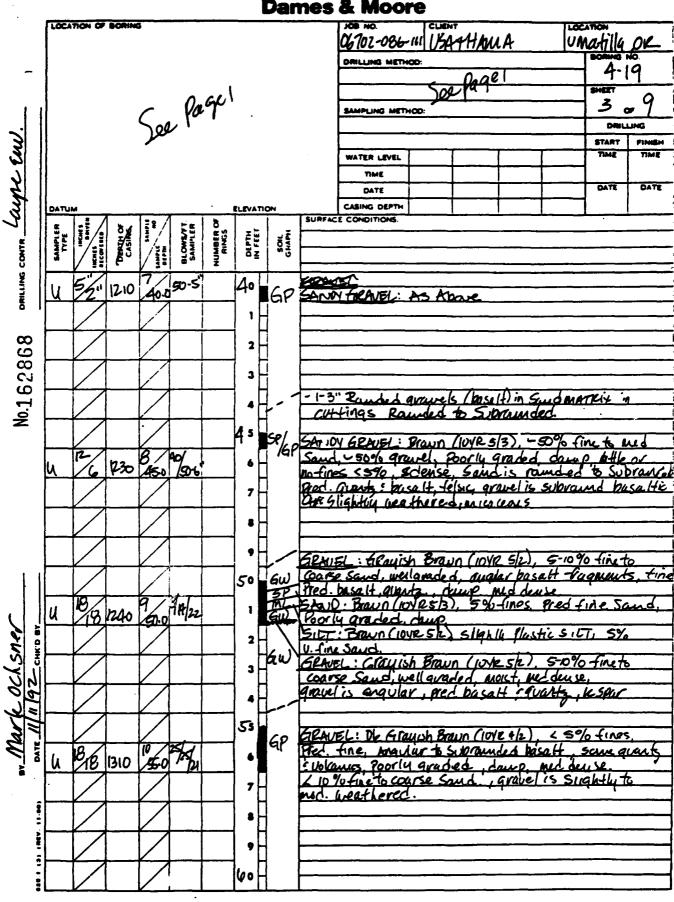
APPENDIX A

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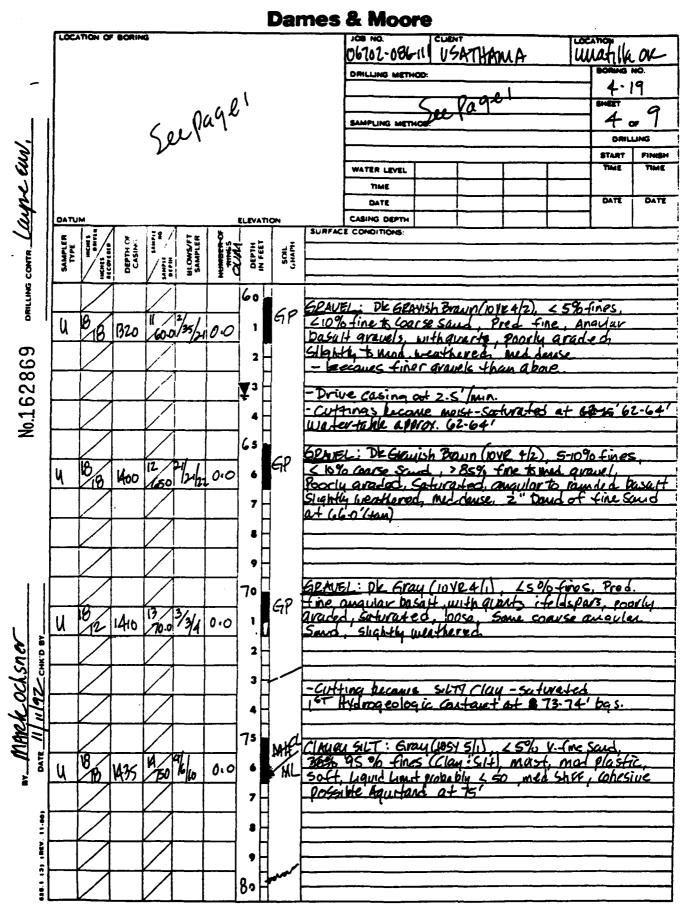
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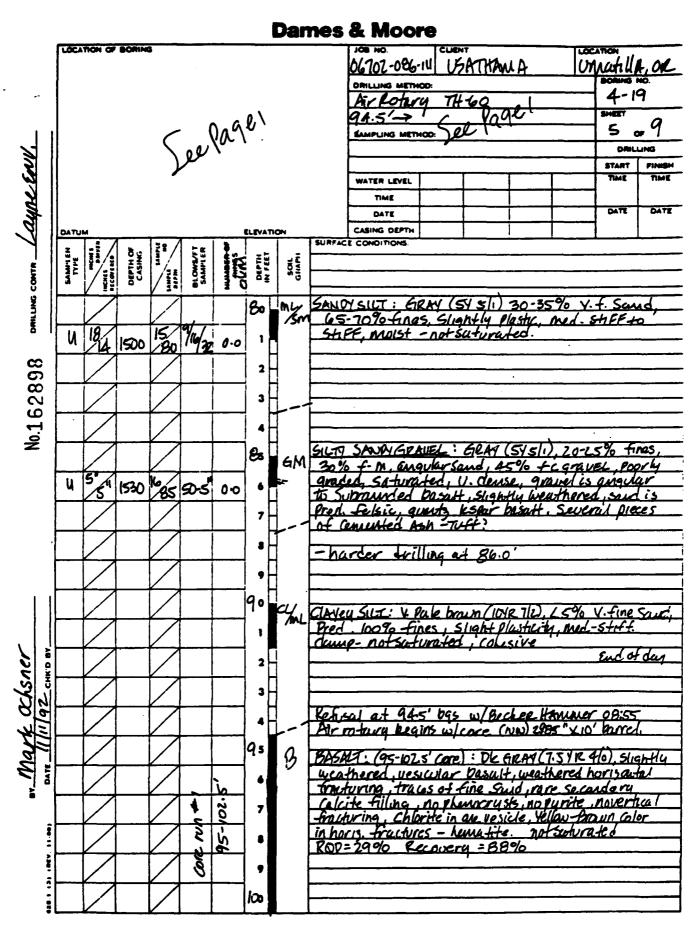




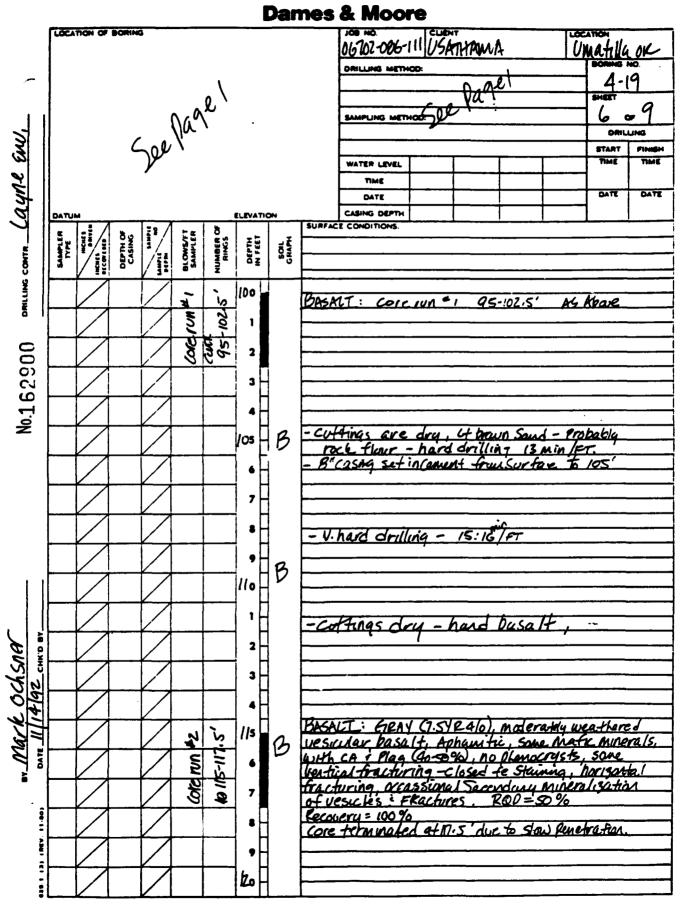


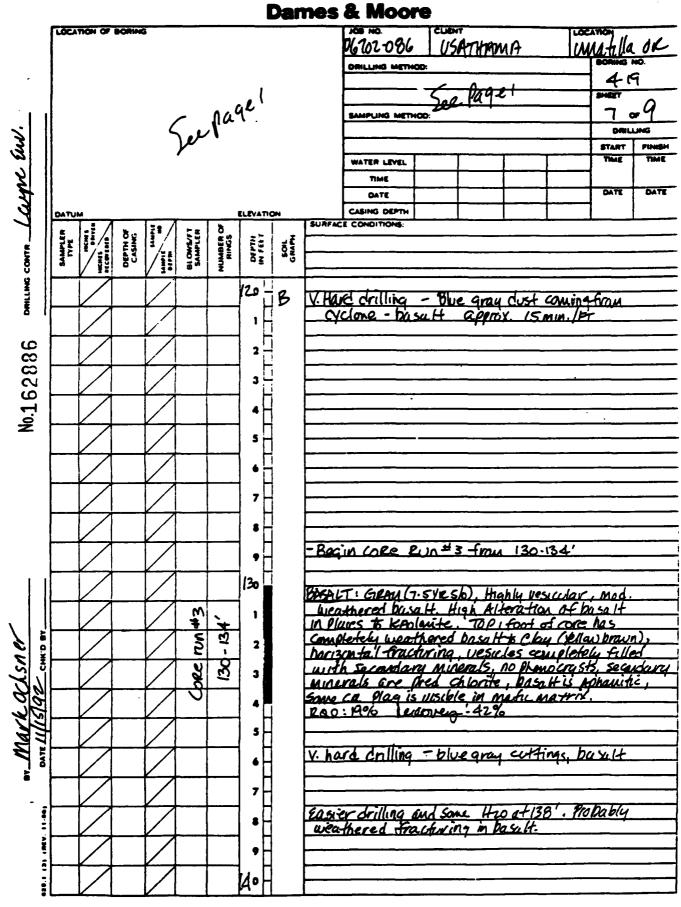
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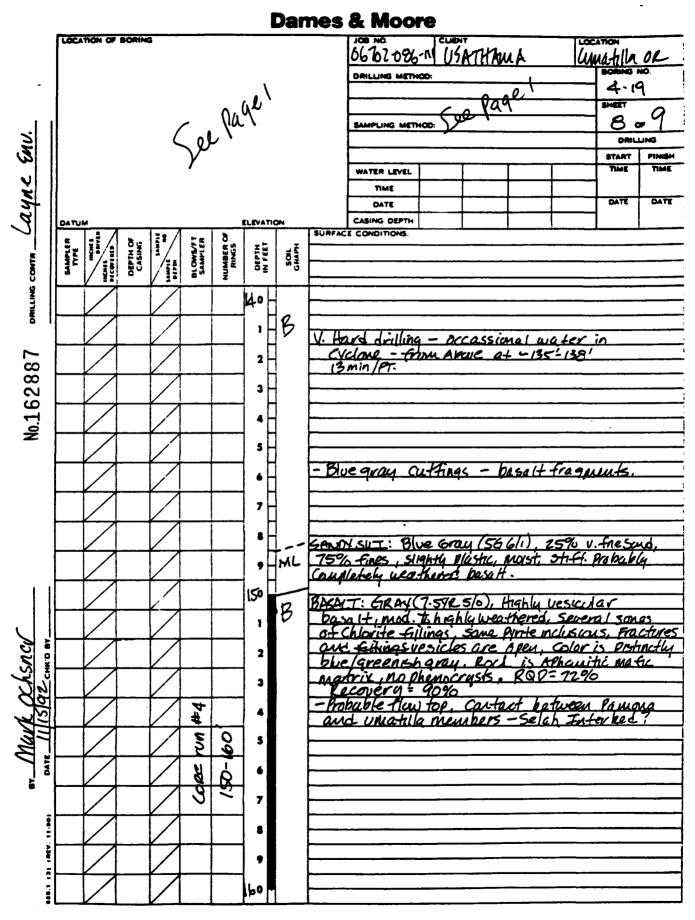


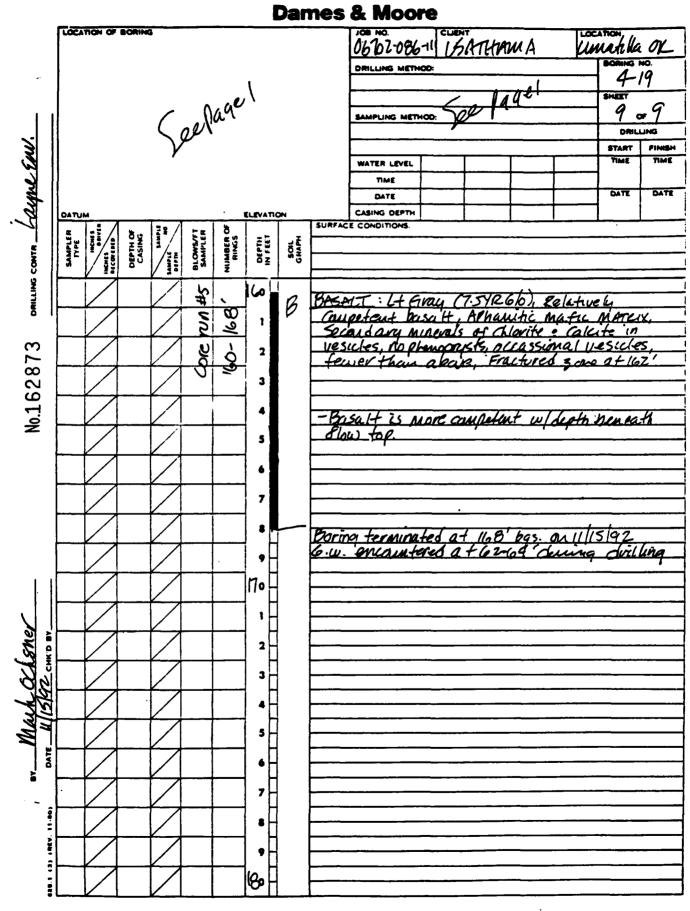


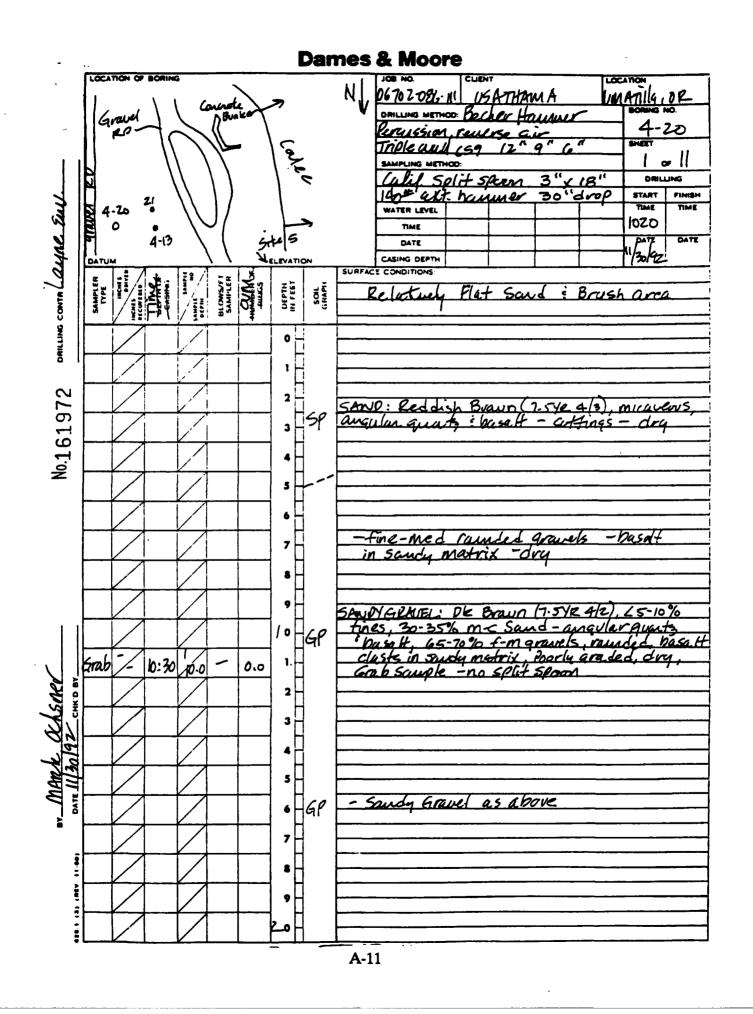
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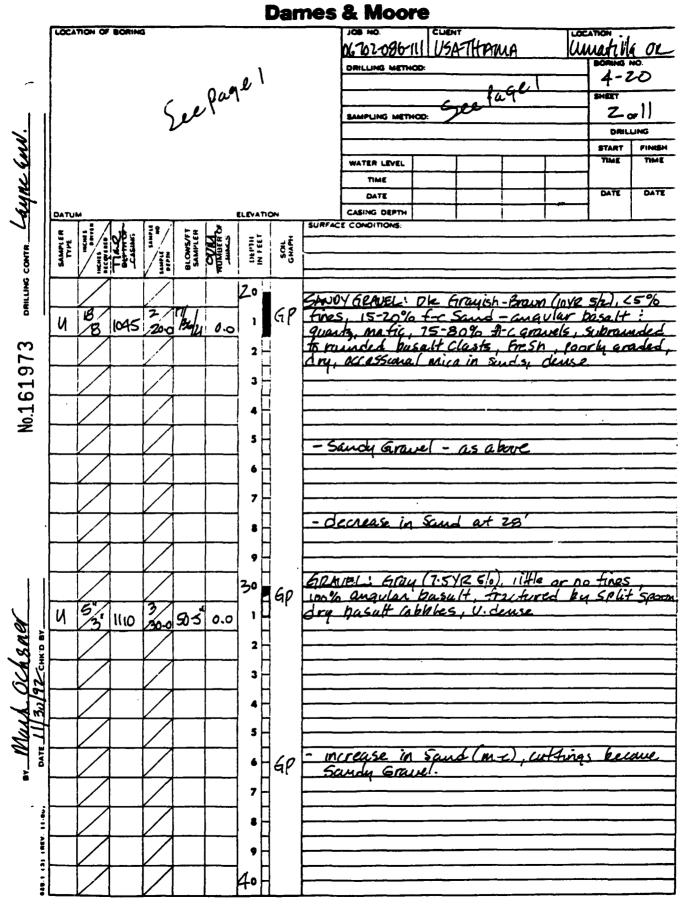


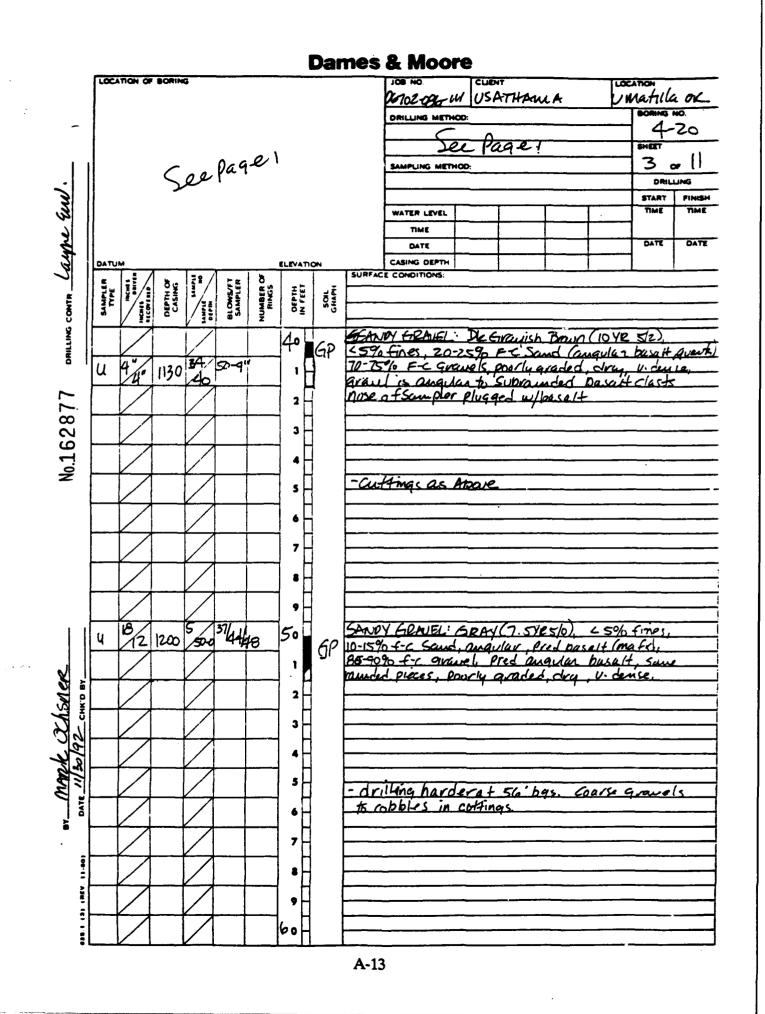


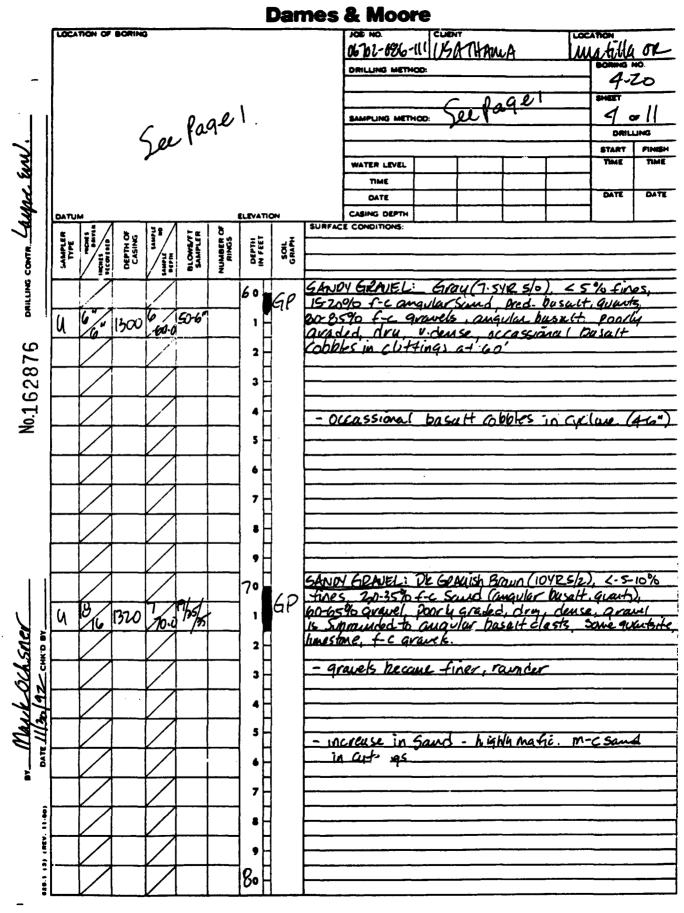


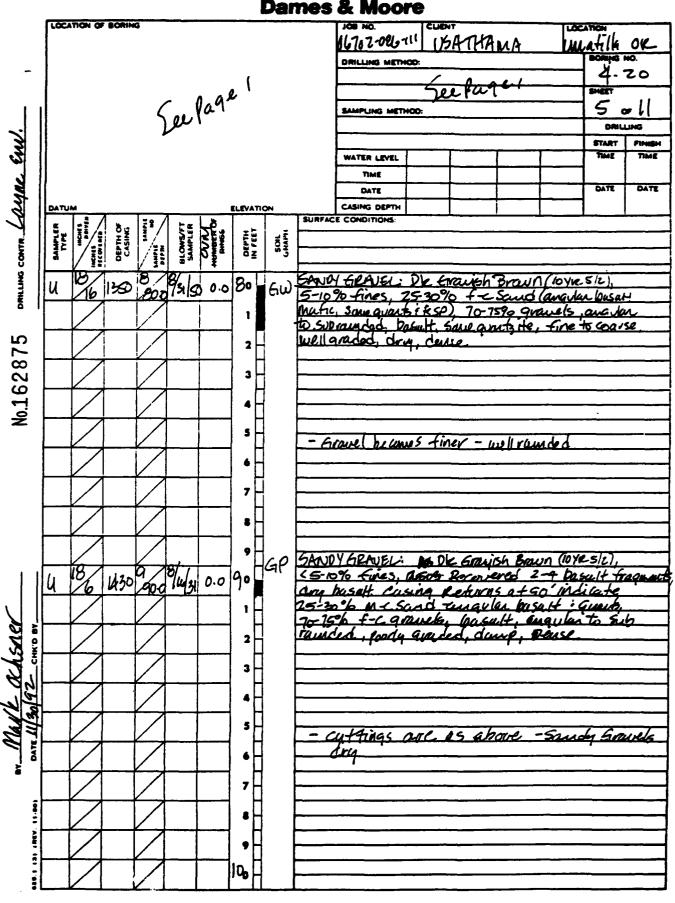




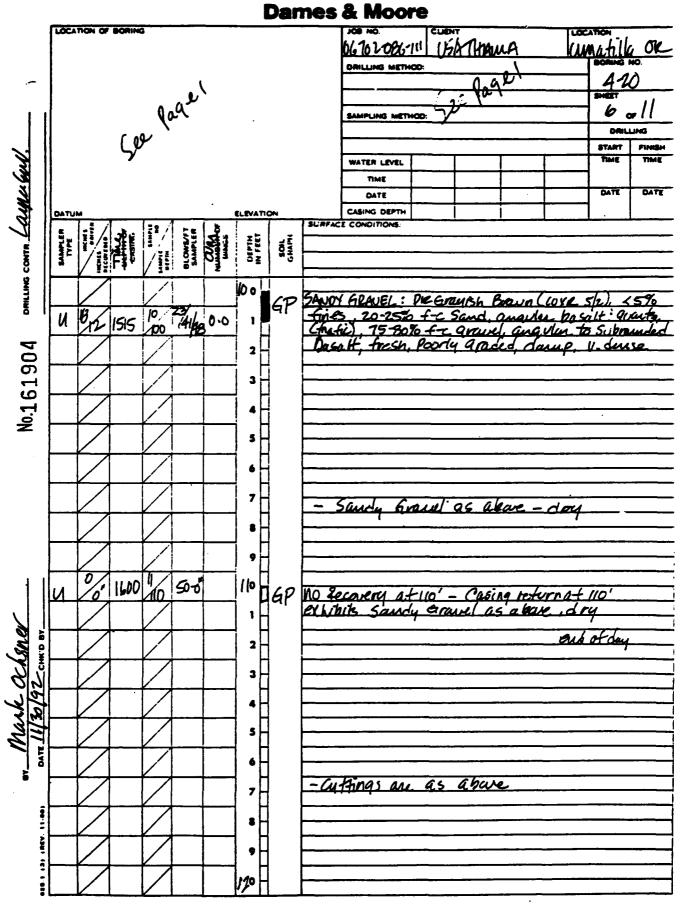


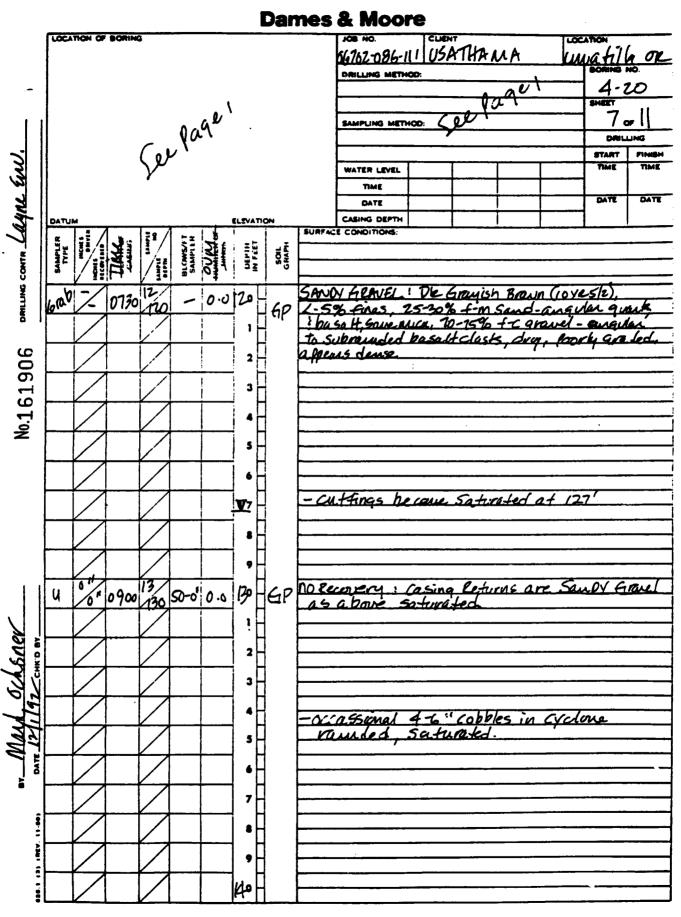


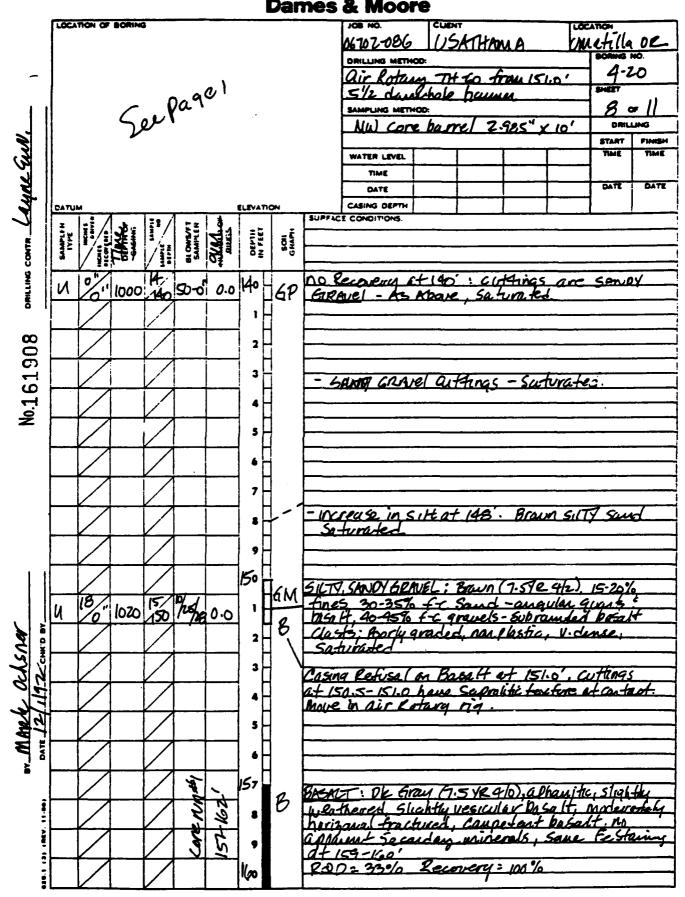




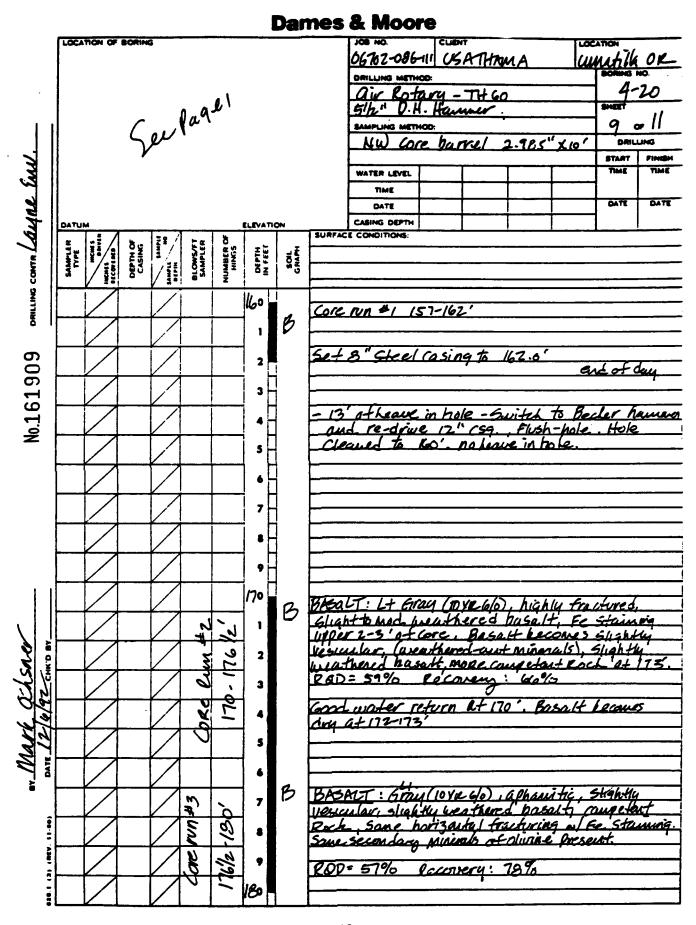
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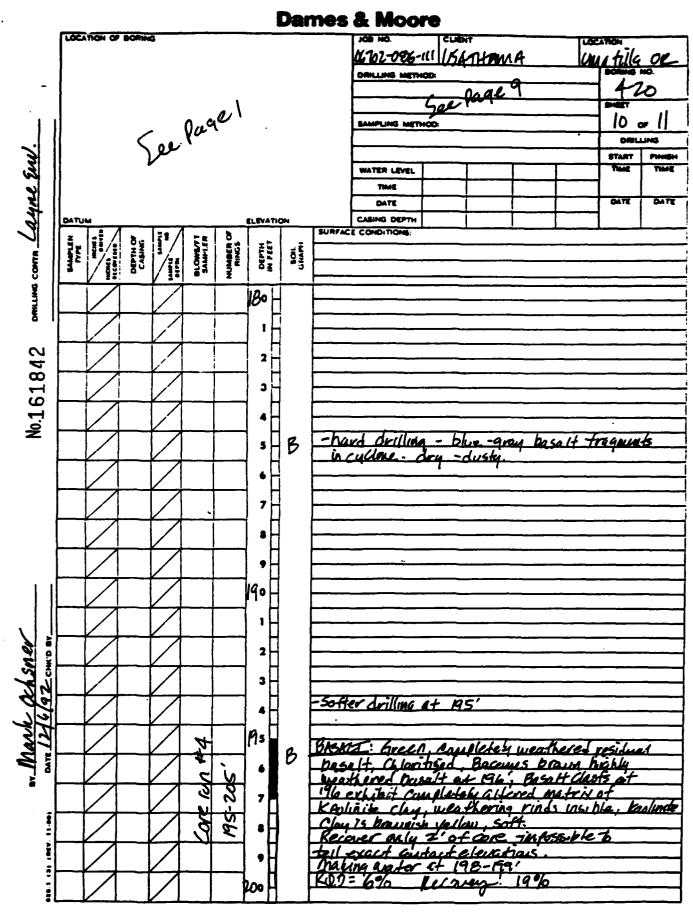


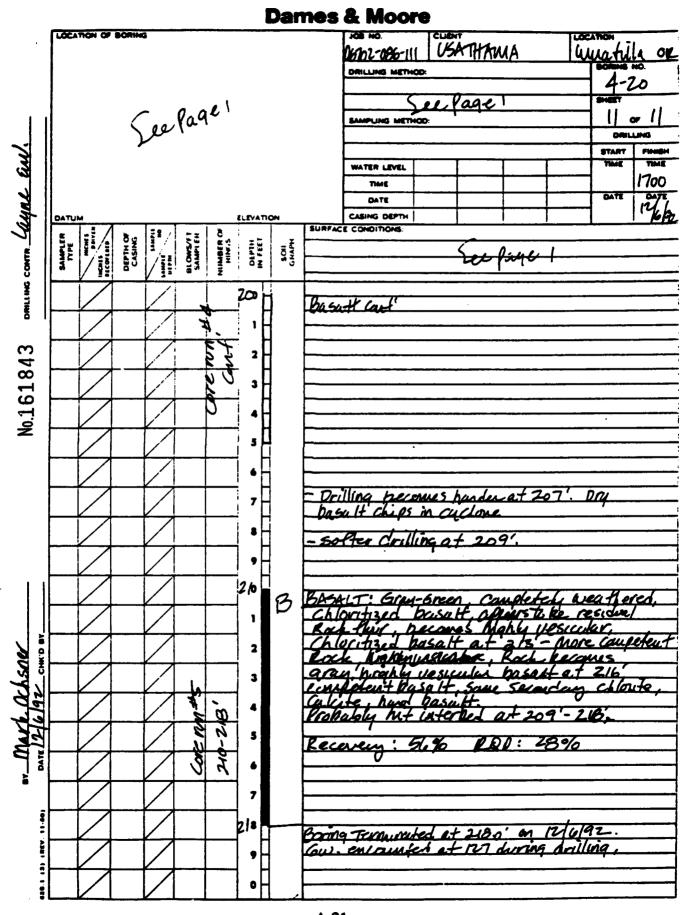


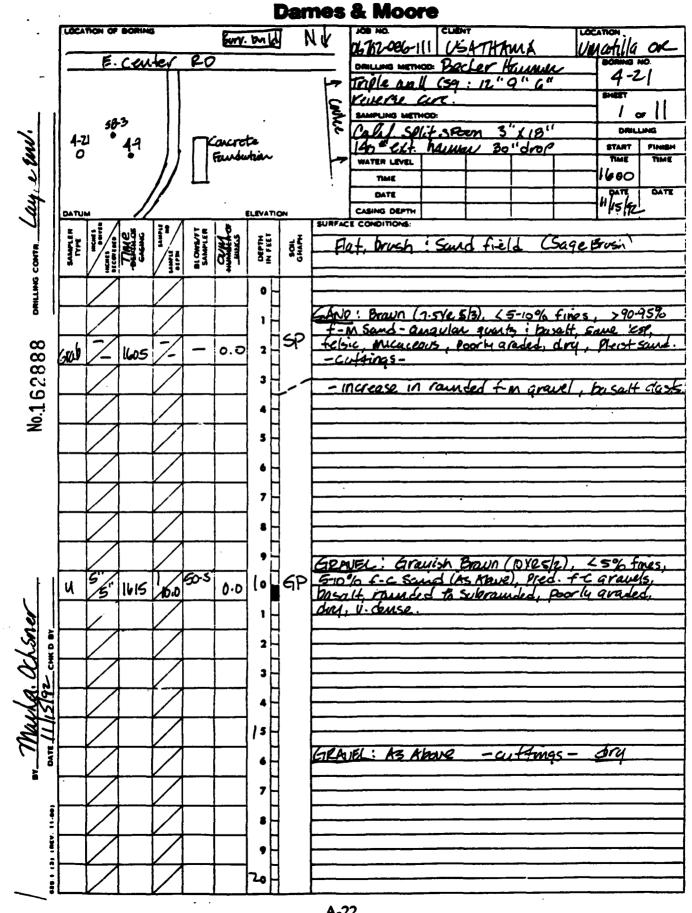


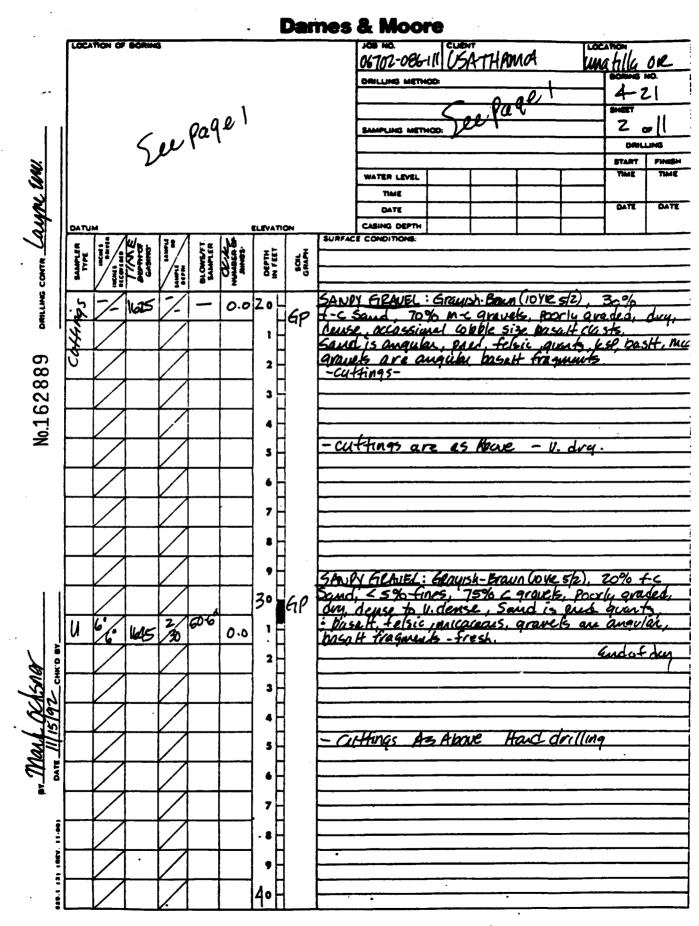
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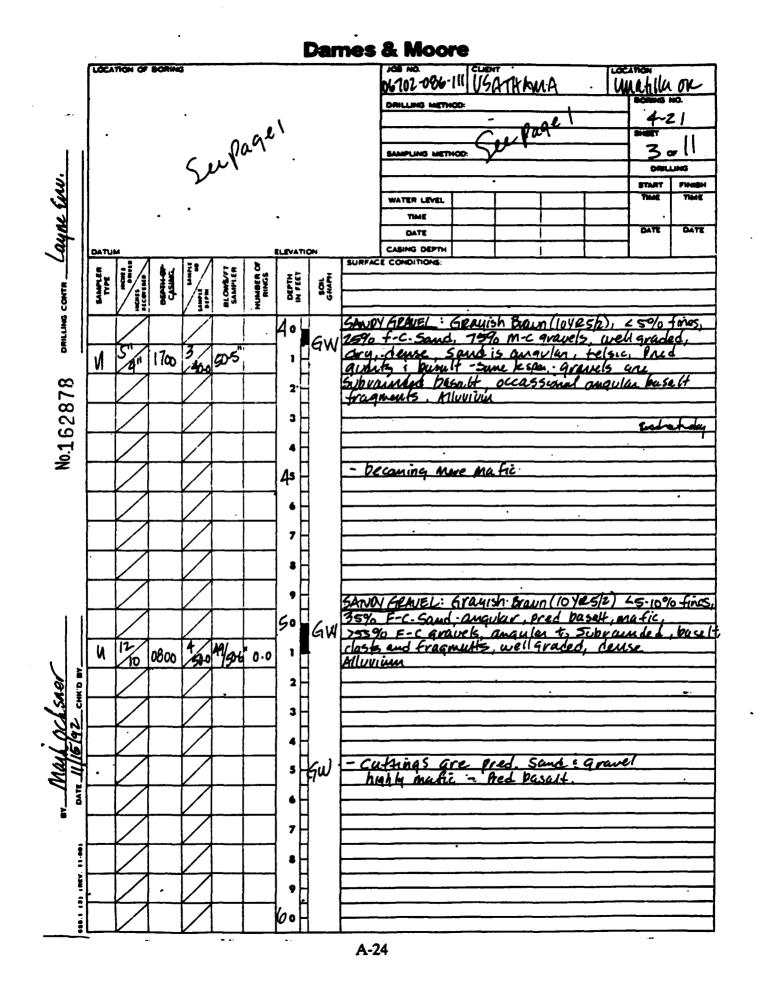


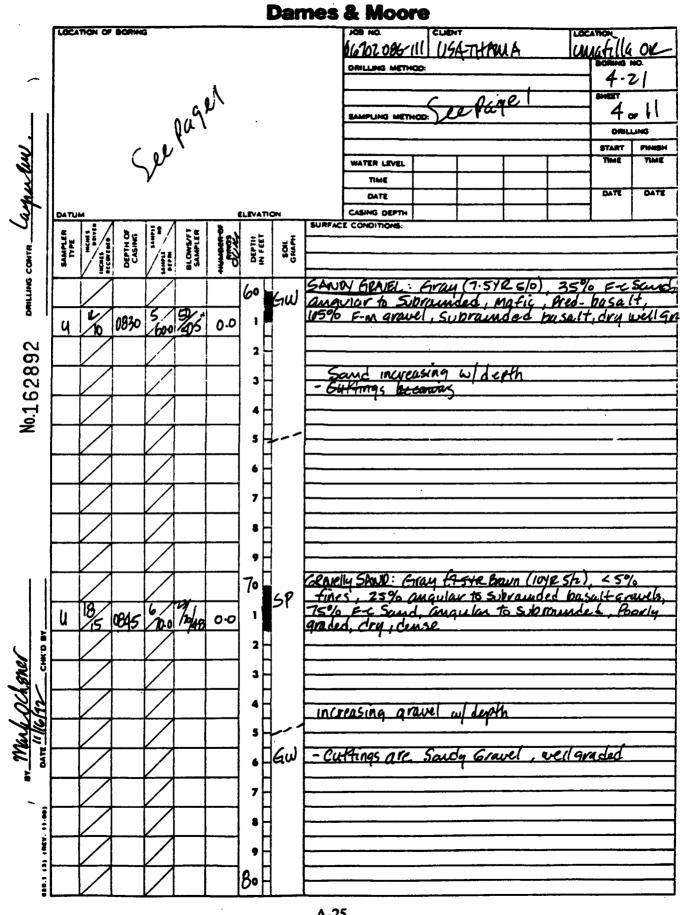


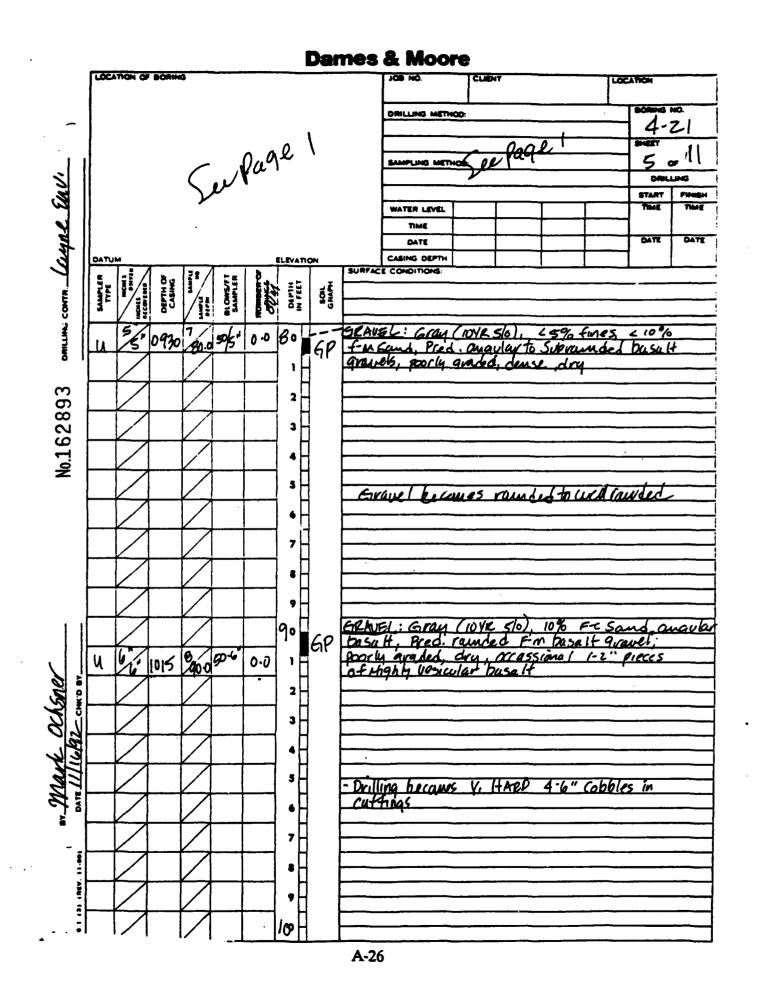


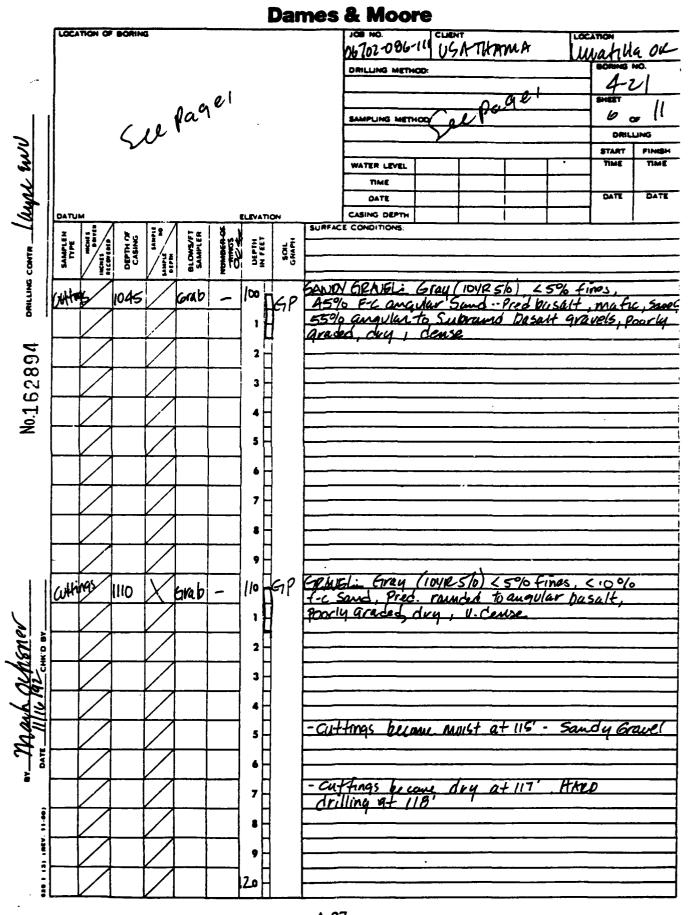


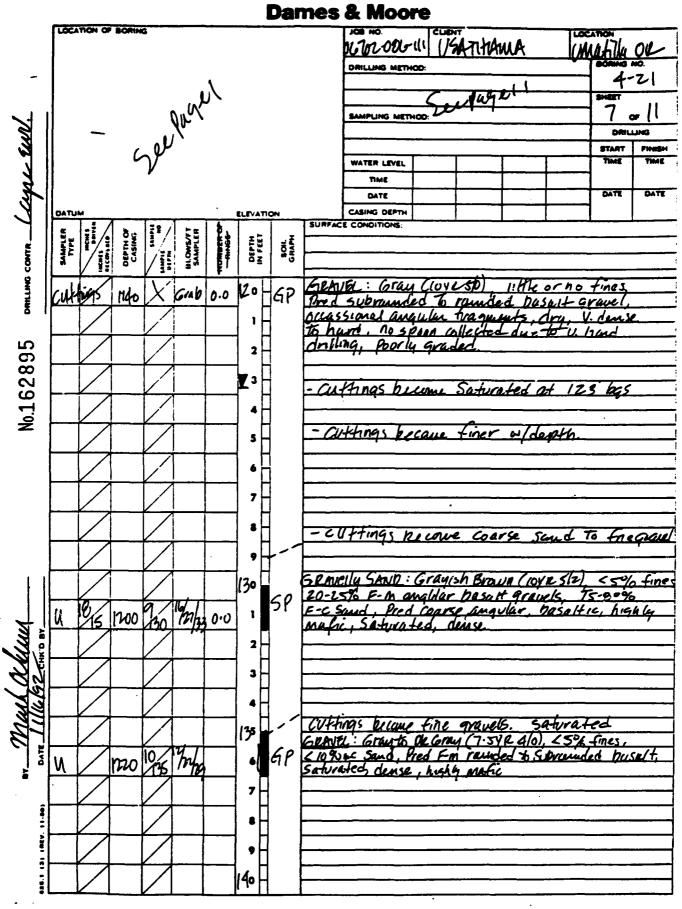


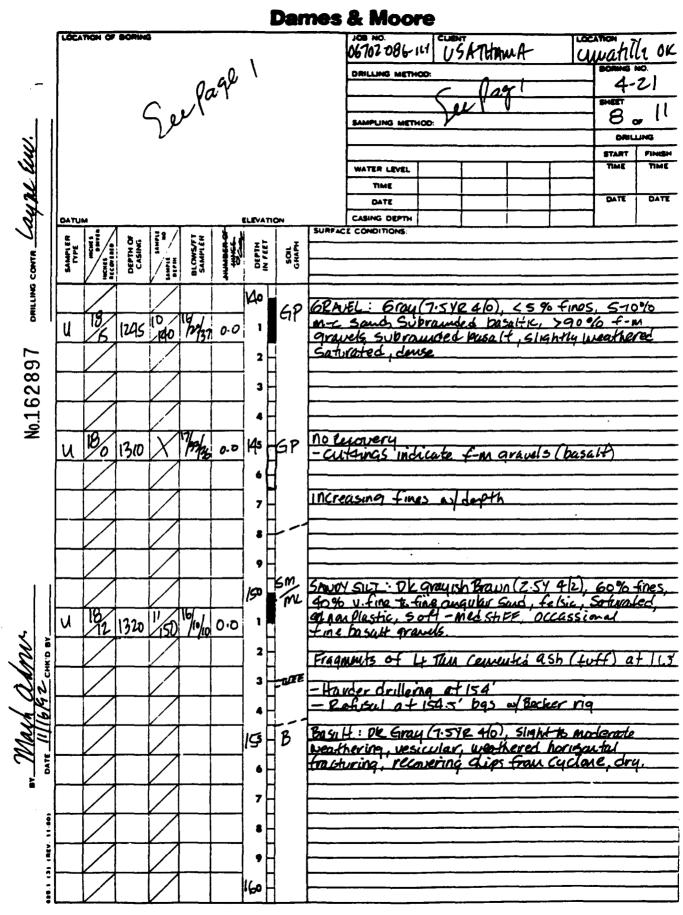


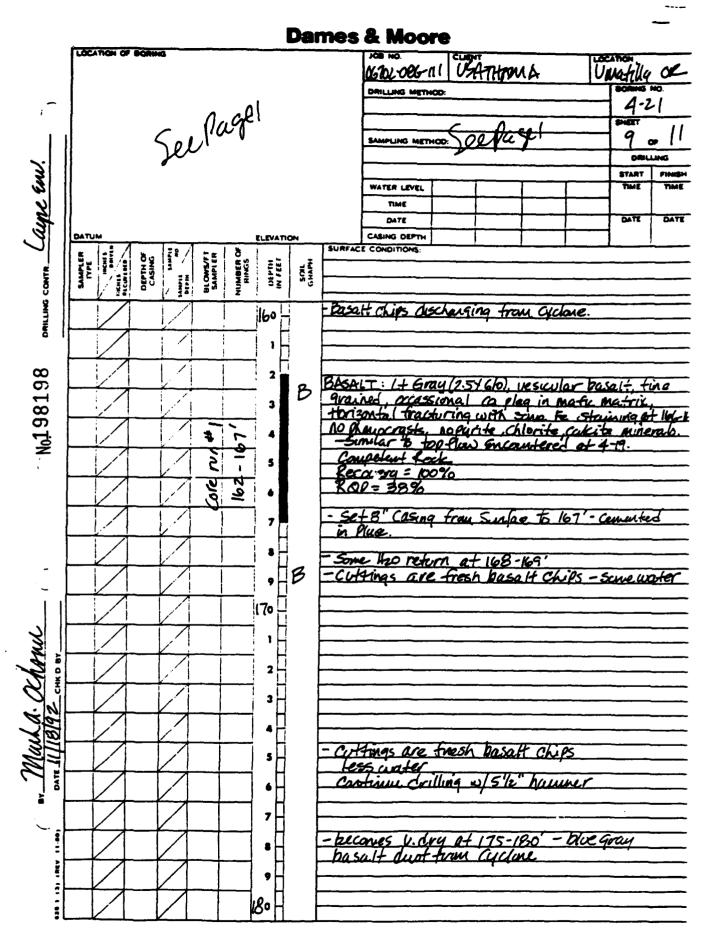


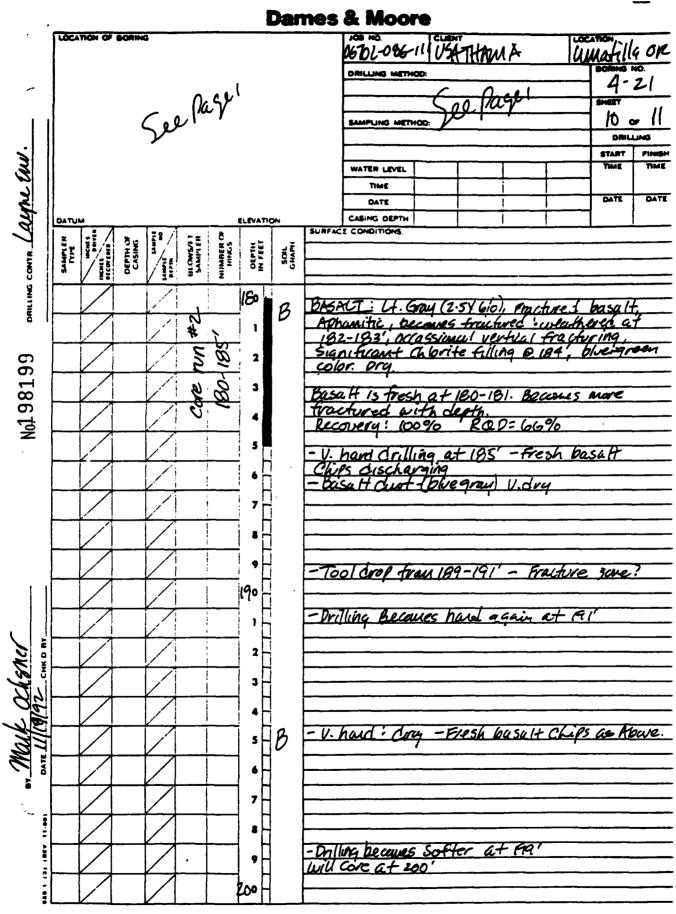


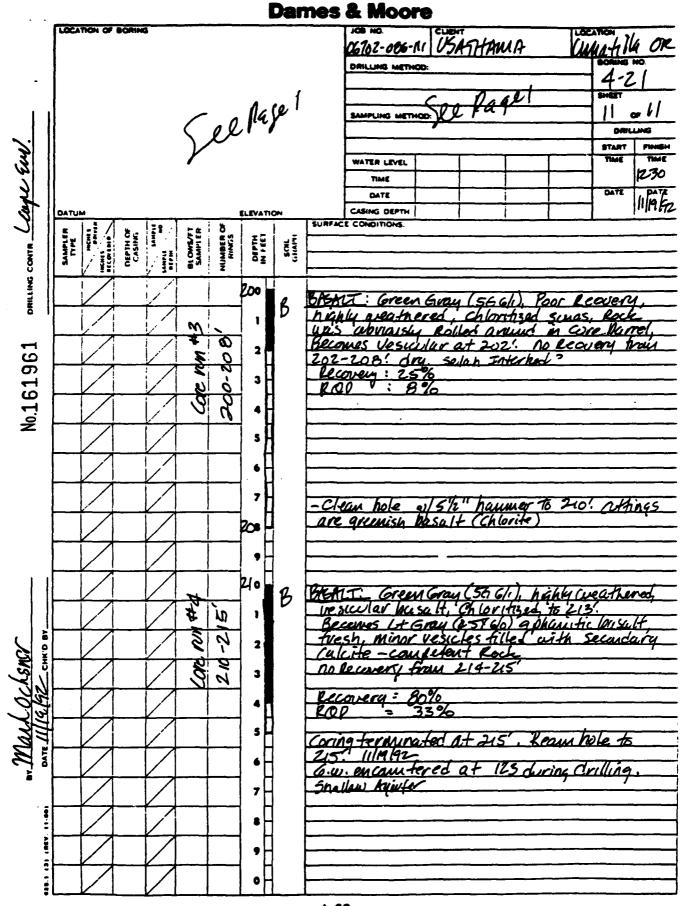


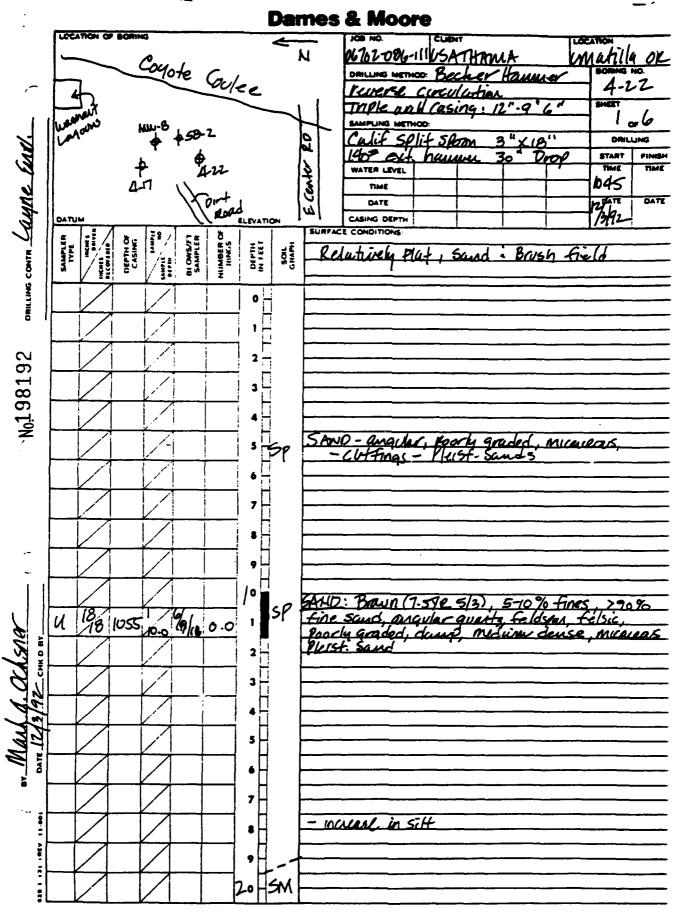


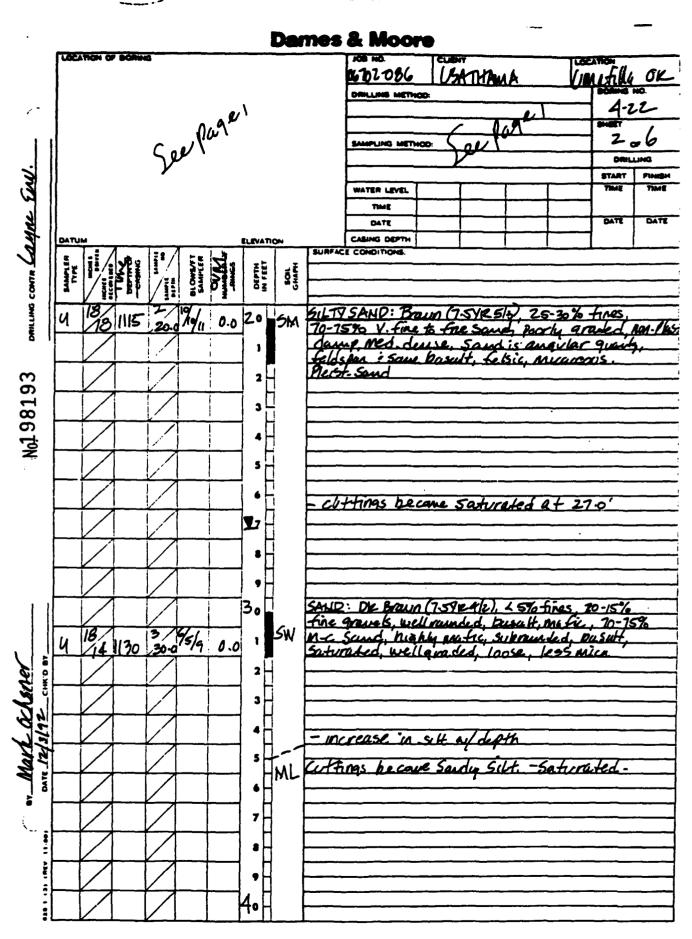


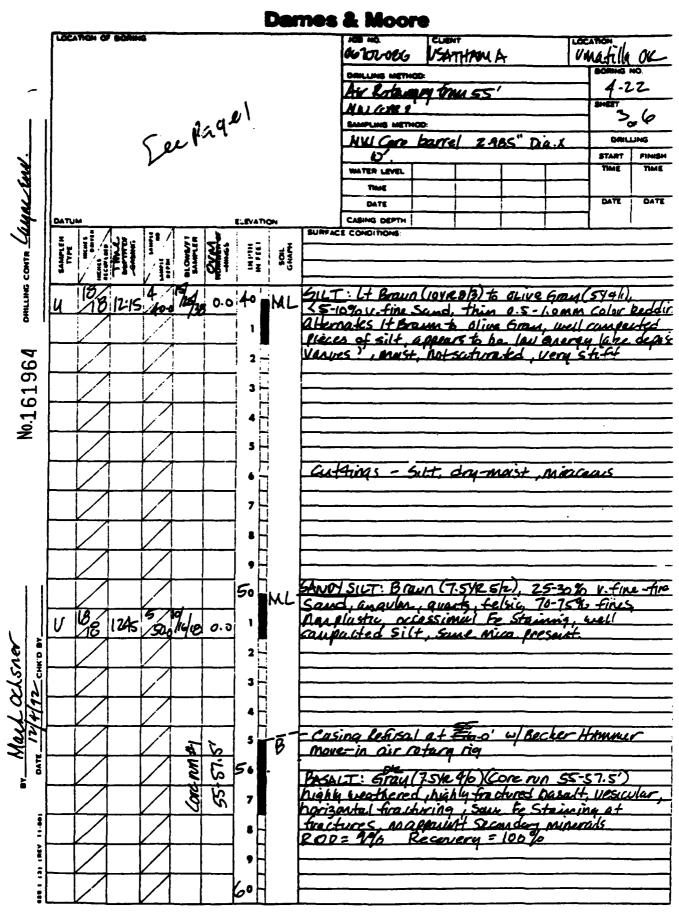


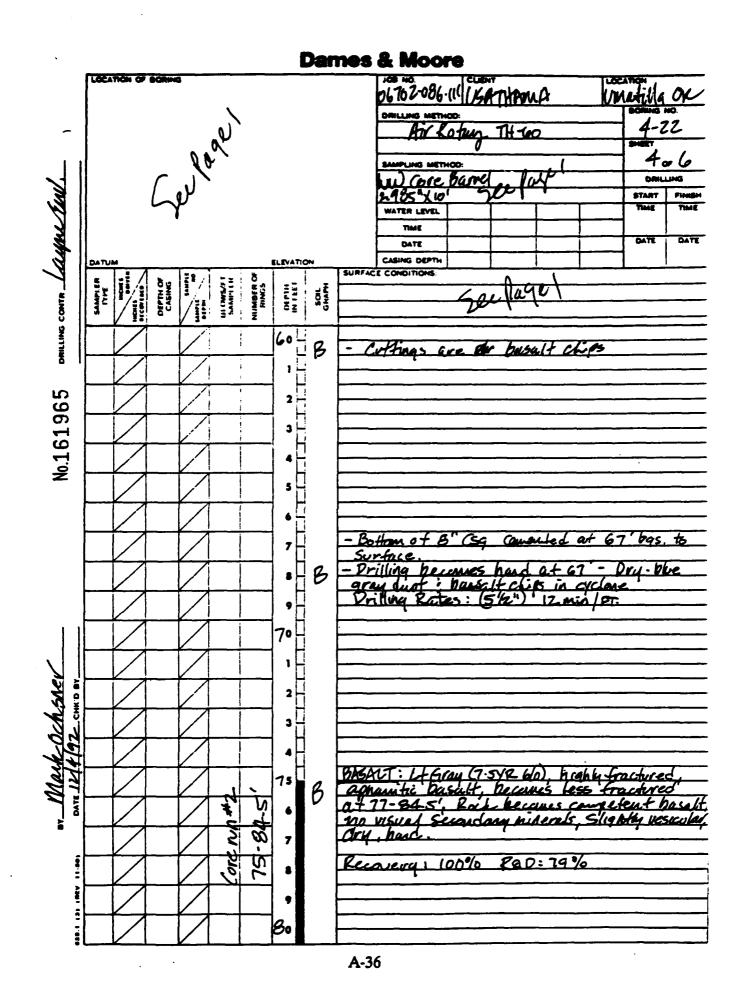


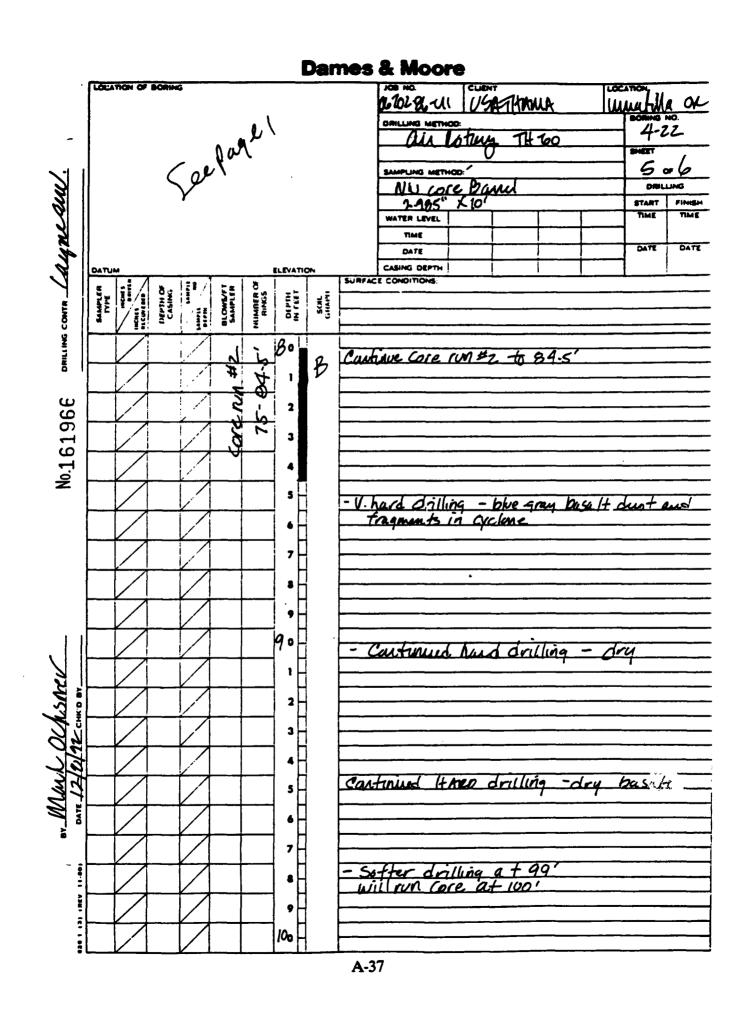


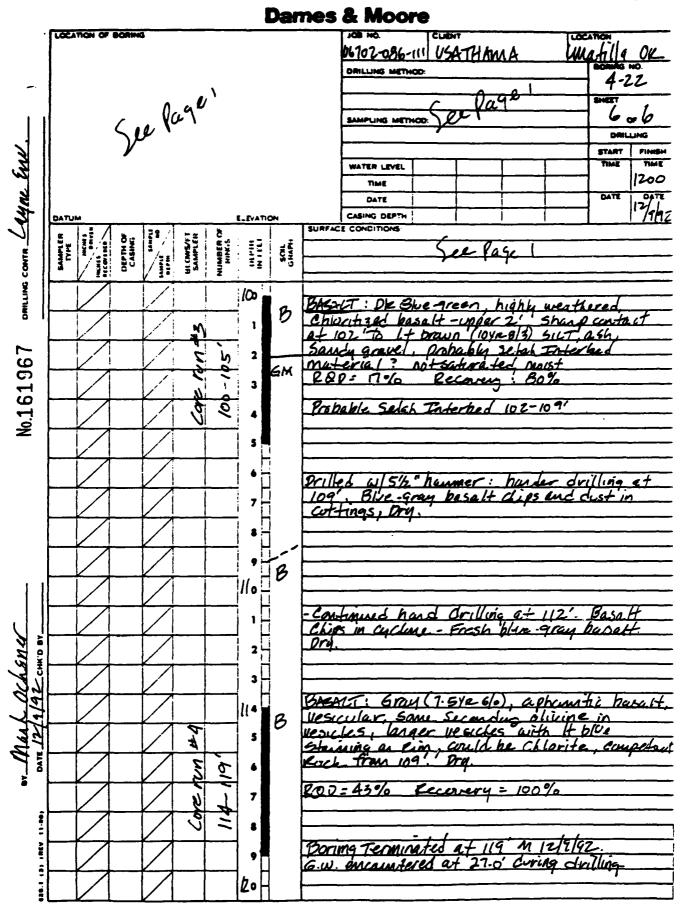


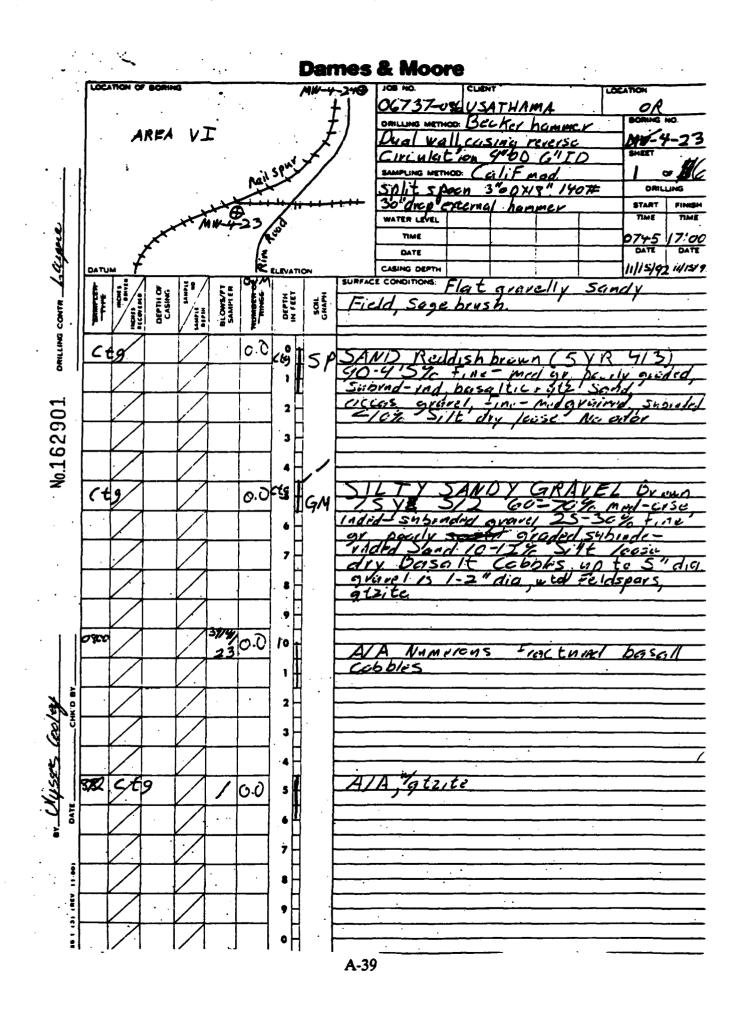


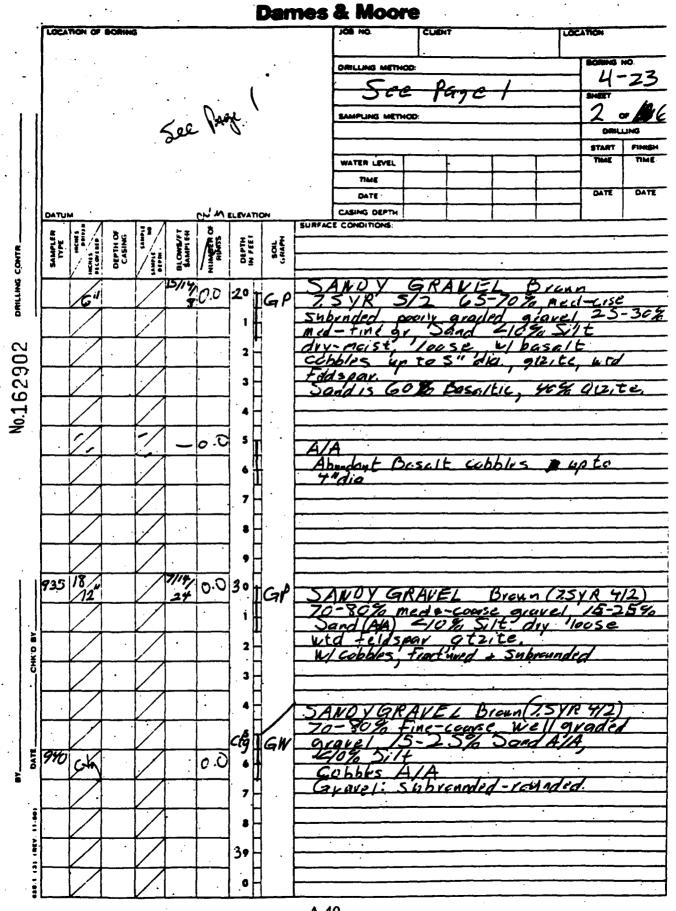


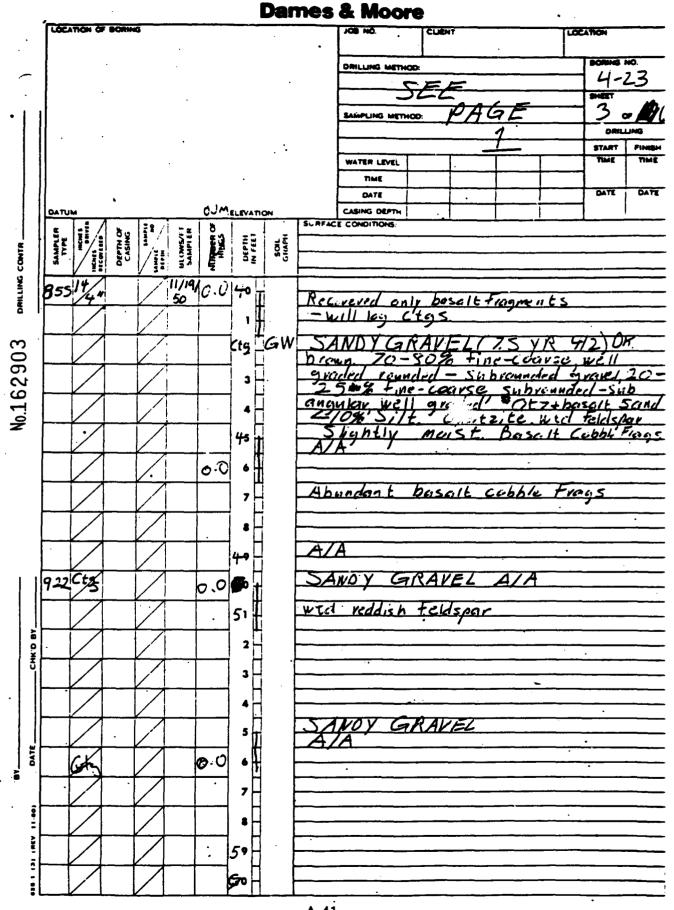


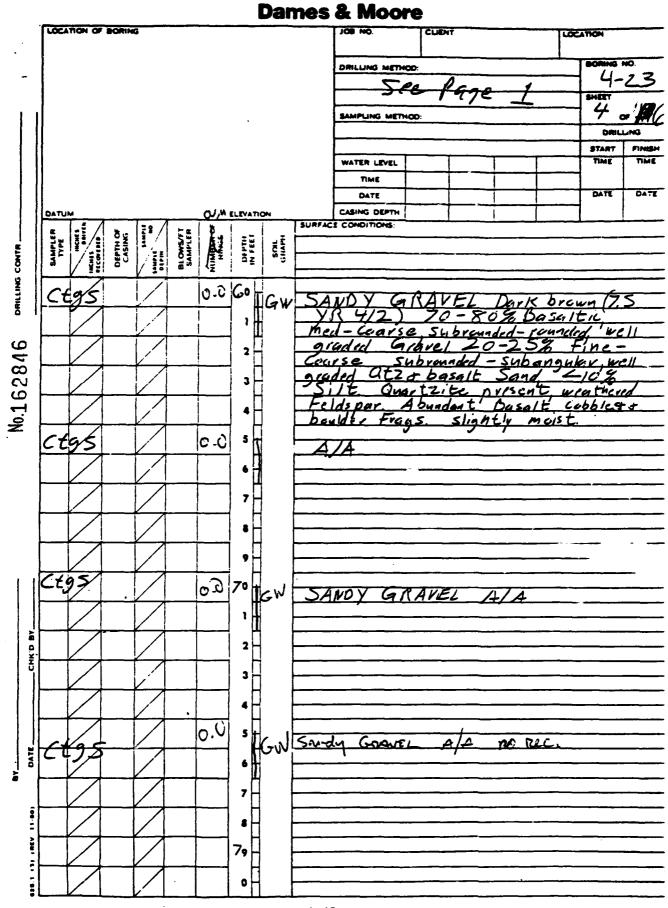


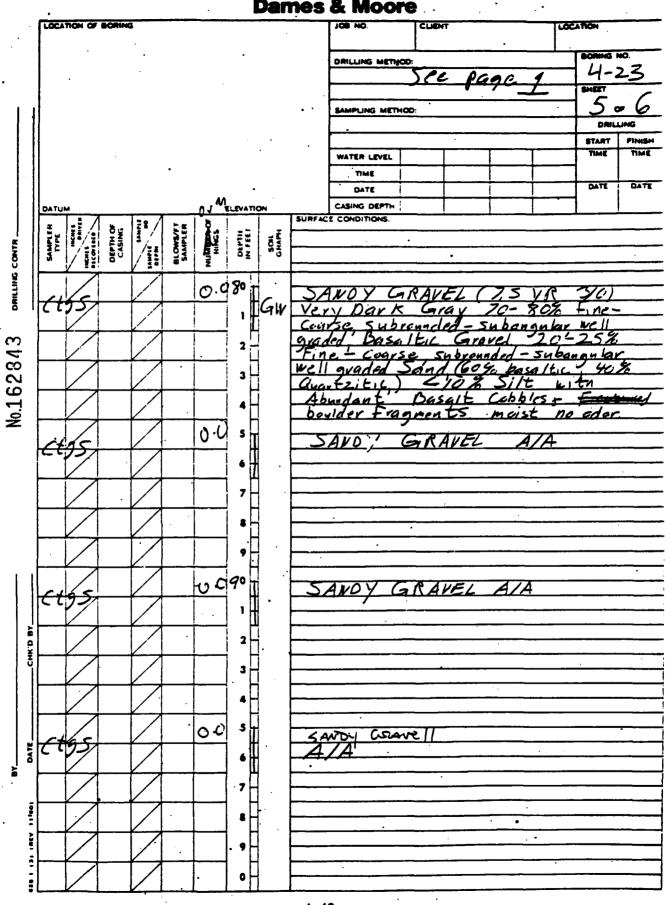




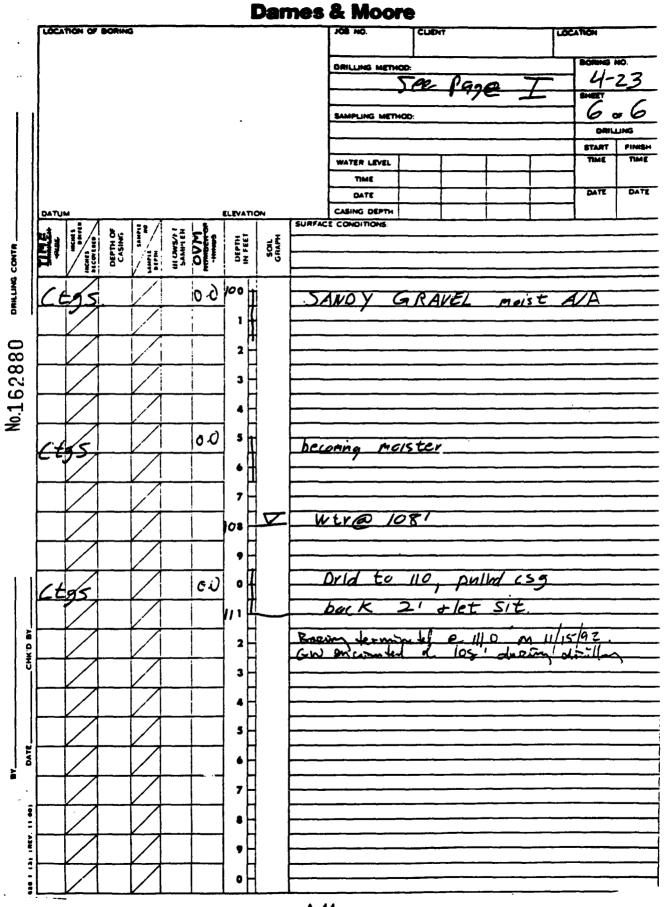


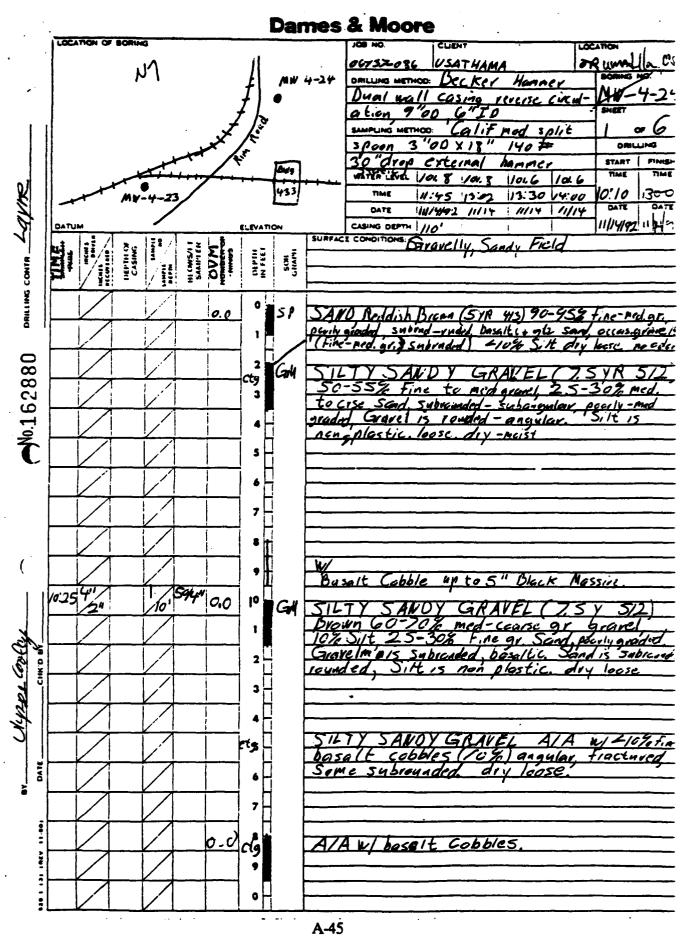




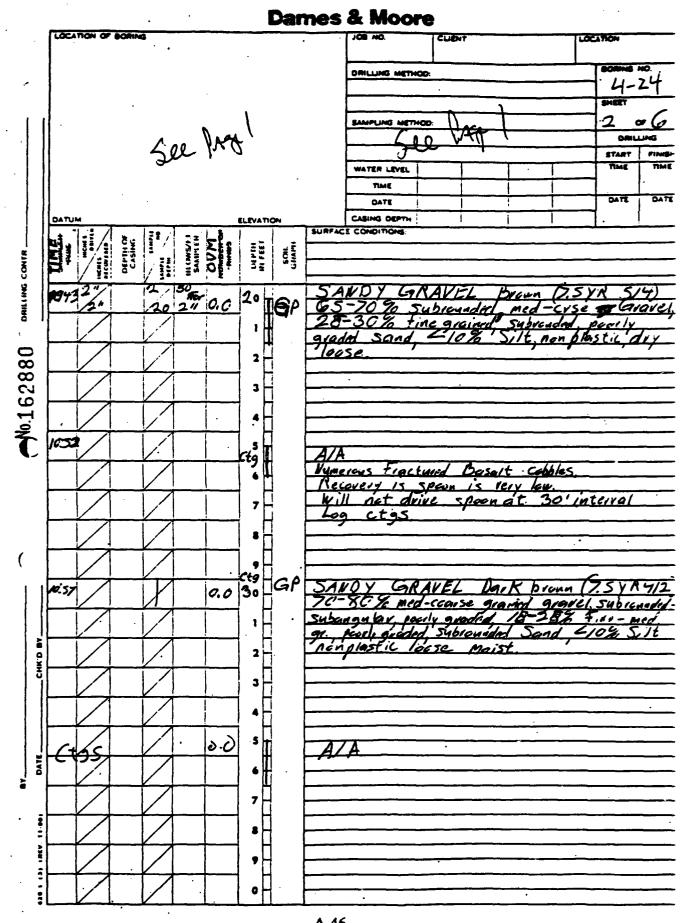


Dames & Moore

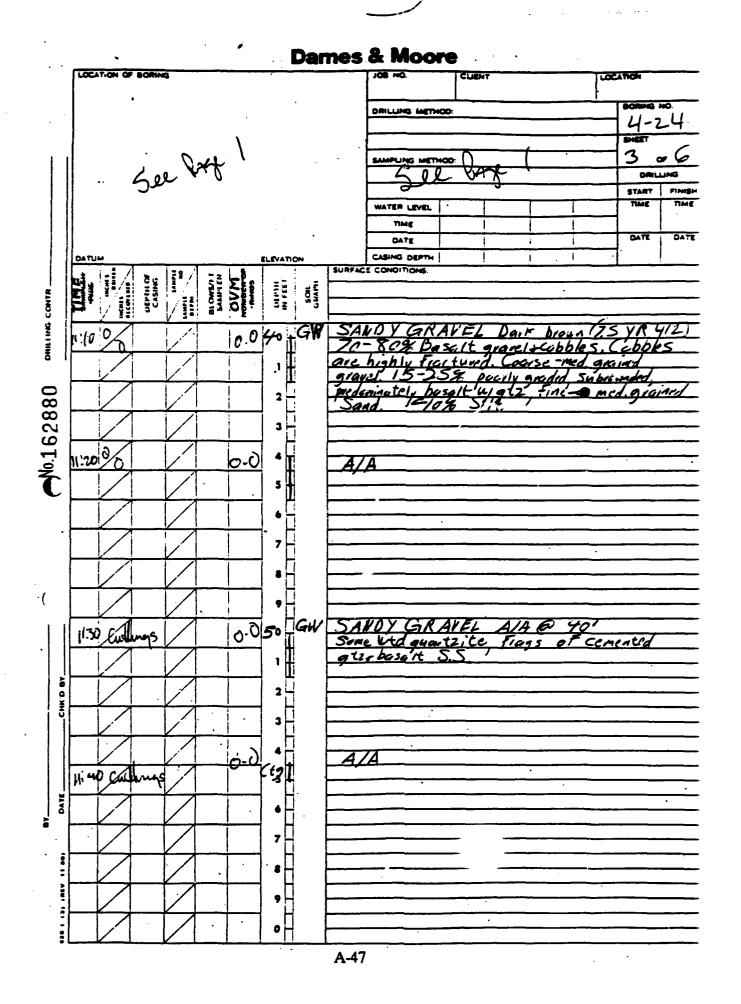


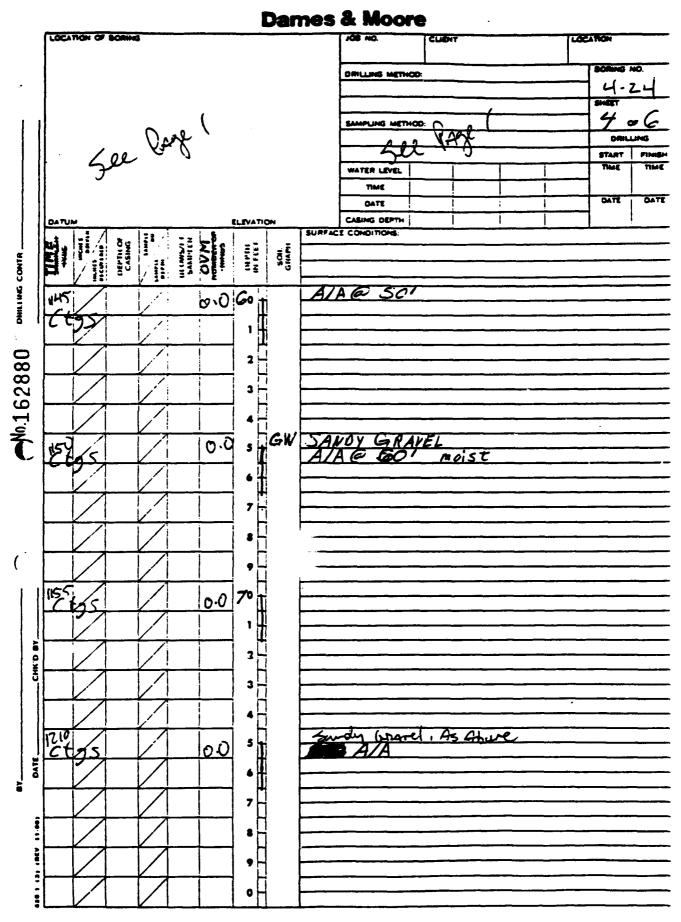


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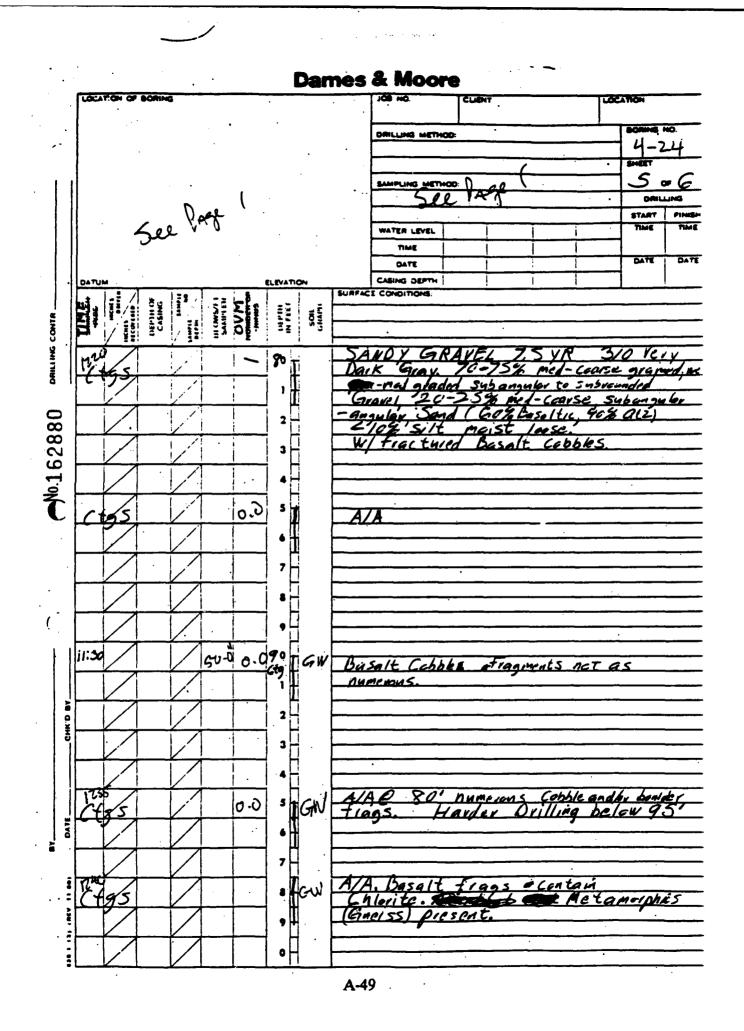
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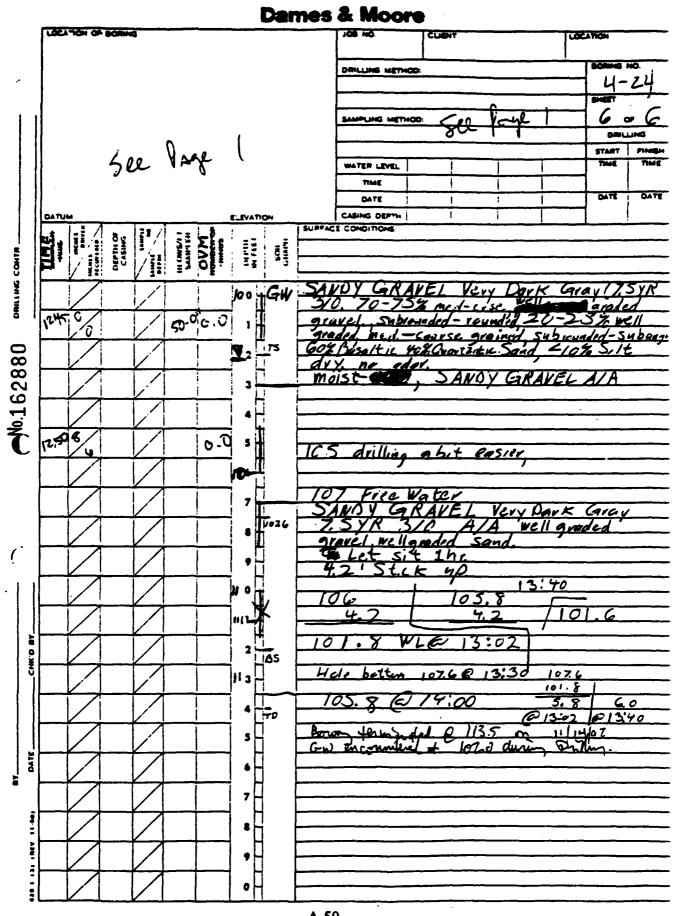




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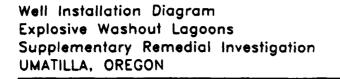




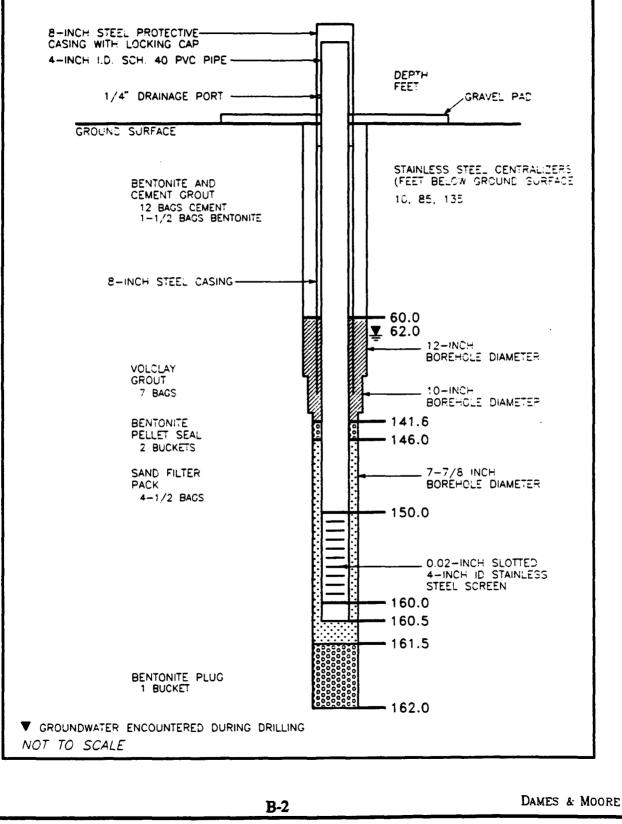
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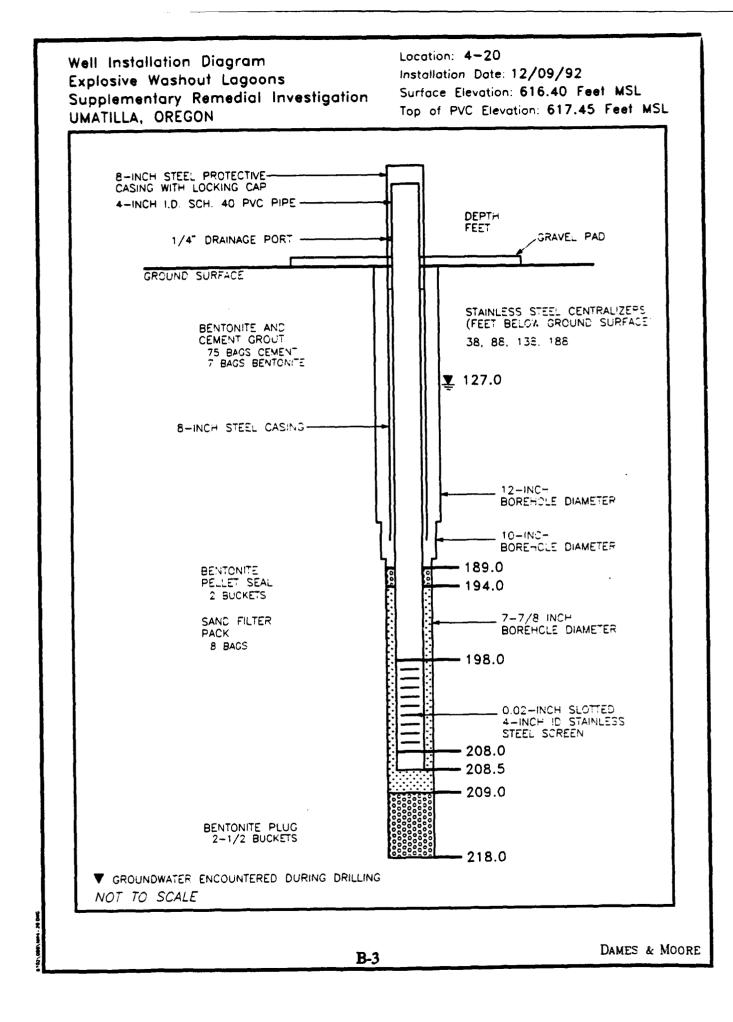
APPENDIX B

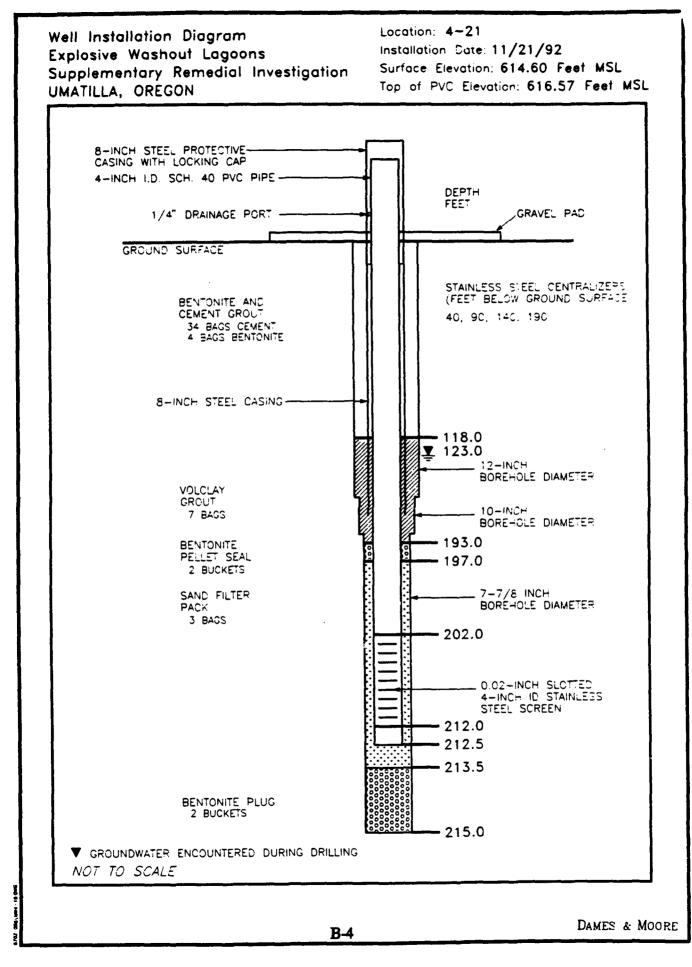
Well Construction Diagrams

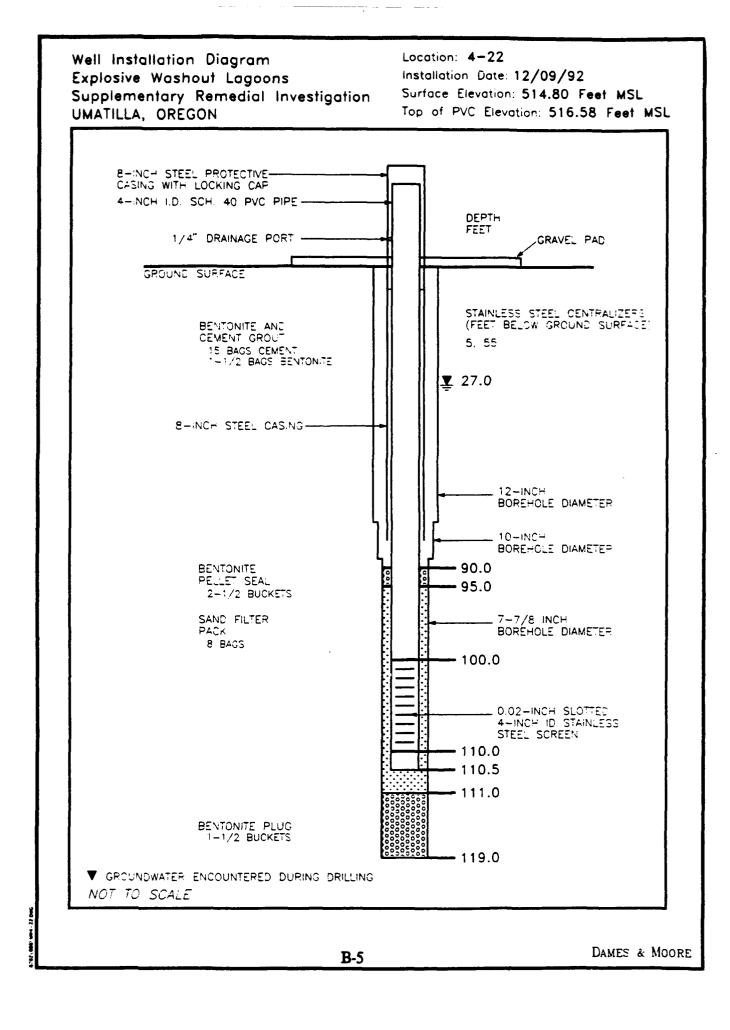


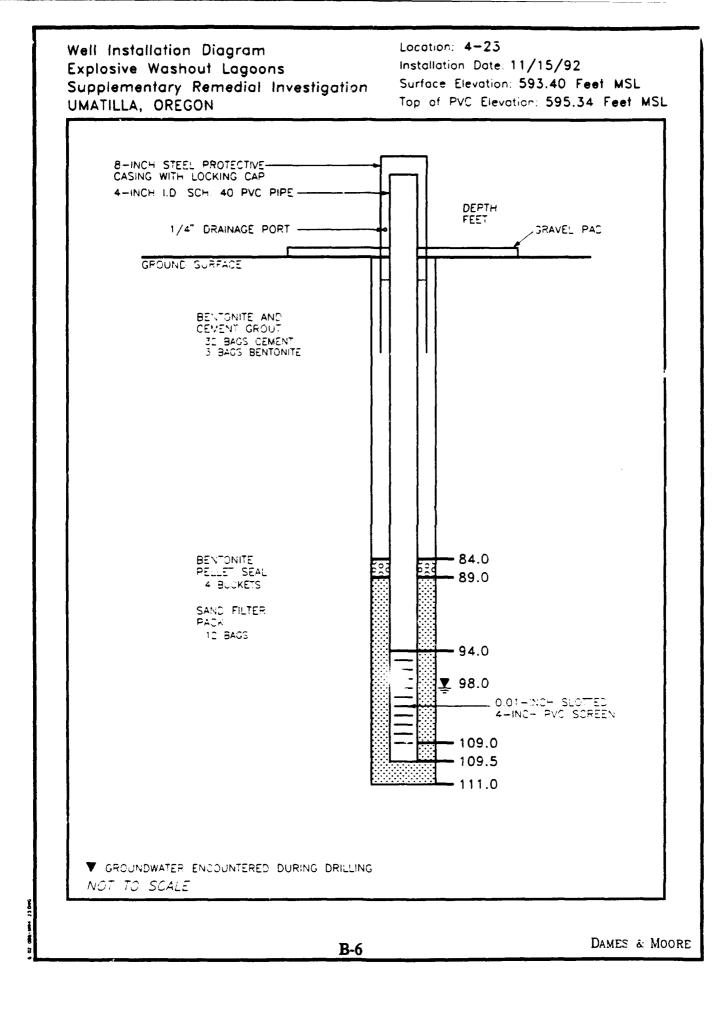
Location: 4-19 Installation Date: 11/17/92 Surface Elevation: 557.80 Feet MSL Top of PVC Elevation; 559,59 Feet MSL

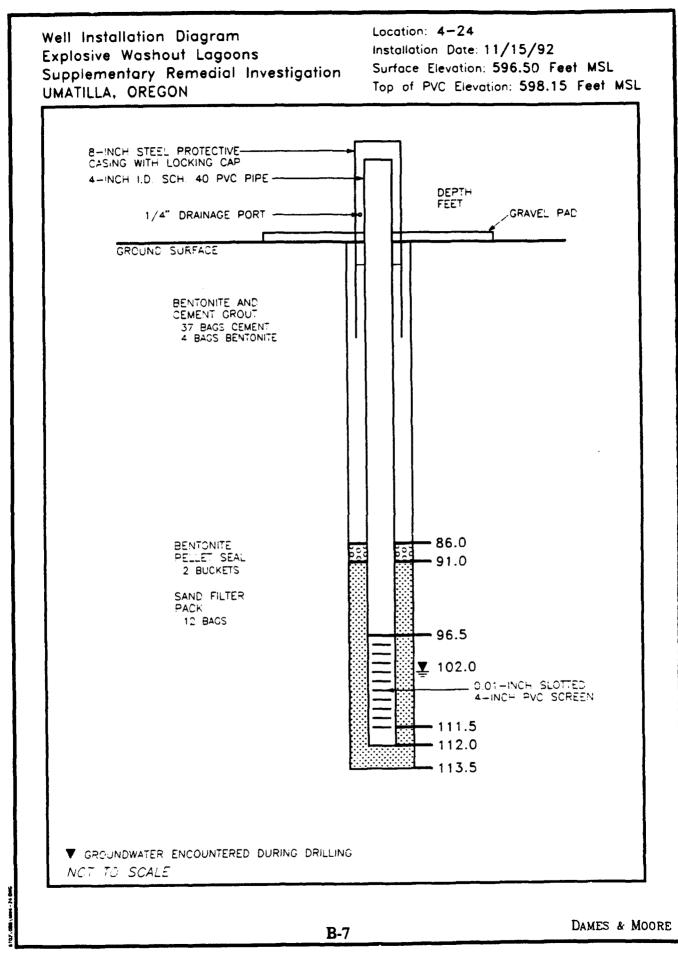








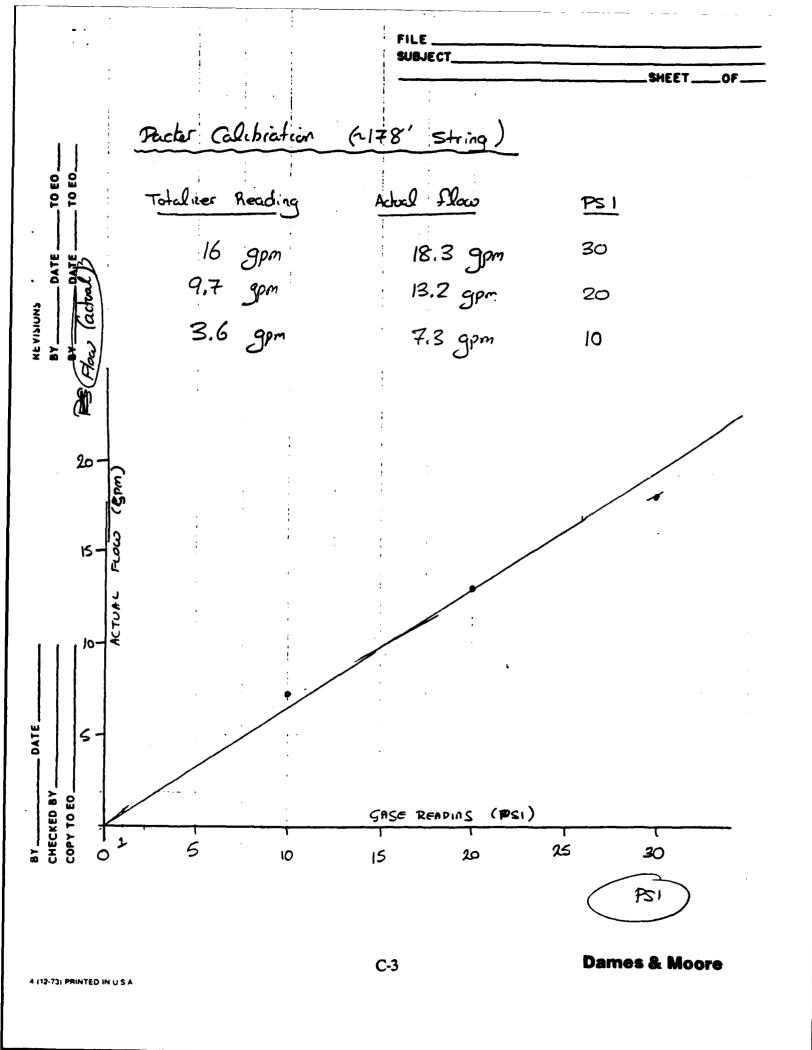




APPENDIX C

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* Column pressure * (depth to unper parker or depth to groundwater, while haven is small of $\int_{X} Geore he all$ Q = flow (A³/a.i.)Conversion forms:eu.ft X 7.68 gallionsH = Headr = hale reduces (119)Horizon Hu.

 $K = \frac{G}{2\pi LH} l_n \frac{L}{r} \quad (L > 10.7)$

C-4

FILE Uillid SUBJECT Packer Test @ 4-22

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APPENDIX D

Field Parameters (Temperature, Specific Conductance, pH)

Weil Development Development Development Volume (gal) (Target Purge Heasure Measure 4-19 12-15-92 112.57 325 T1 7.76 17.2 4-19 12-15-92 112.57 325 T1 7.78 17.2 4-20 12-18-92 112.57 350 T1 6.27 16.9 4-20 12-18-92 145.50 350 T1 6.27 16.9 4-20 12-18-92 145.50 350 T1 6.27 16.9 4-20 12-18-92 145.50 350 T1 6.27 16.9 17.2 T3 7.97 16.1 17.2 13 7.97 16.1 4-20 12-18-92 145.50 350 T1 6.27 16.9 13 7.97 16.1 17.2 13 7.97 16.1 14 7.66 16.7 15 7.55 16.7 16.7 14 12-16-92		
(252.40) T2 7.43 17.8 T3 7.57 17.8 T4 7.53 17.2 T5 7.49 17.2 T6 7.46 16.7 4-20 12-18-92 145.50 350 T1 6.27 18.9 (332.85) T2 8.96 17.2 T3 7.87 16.1 T4 7.66 16.7 T5 7.55 16.7 T6 7.54 16.7 T6 7.54 16.7 T7 7.56 16.7	Conductivity (uMHOS)	
(252.40) T2 7.43 17.8 T3 7.57 17.8 T4 7.53 17.2 T5 7.49 17.2 T6 7.46 16.7 4-20 12-18-92 145.50 350 T1 6.27 18.9 (332.85) T2 8.96 17.2 T3 7.87 16.1 T4 7.66 16.7 T5 7.55 16.7 T6 7.54 16.7 T6 7.54 16.7 T7 7.56 16.7	420	
T3 7.57 17.8 T4 7.53 17.2 T5 7.49 17.2 T6 7.46 16.7 4-20 12-18-92 145.50 350 T1 8.27 18.9 (332.85) T2 8.96 17.2 T3 7.87 16.1 T4 7.66 16.7 T5 7.55 16.7 T6 7.54 16.7 T6 7.54 16.7 T6 7.54 16.7 T6 7.54 16.7 T7 7.56 16.7 T6 7.54 16.7 T7 7.56 16.7 T6 7.54 16.7 T7 7.56 16.7	400	
4-20 12-18-92 145.50 350 T1 6.27 18.9 (332.85) T2 8.96 17.2 T3 7.87 16.1 T4 7.66 16.7 T5 7.55 16.7 T6 7.54 16.7 T7 7.56 16.7 T6 7.54 16.7 T7 7.56 16.7	440	
4-20 12-18-92 145.50 \$\$50 T1 8.27 18.9 (332.85) T2 8.96 17.2 T3 7.87 16.1 T4 7.66 16.7 T5 7.55 16.7 T6 7.54 16.7 T7 7.56 16.7 T6 7.54 16.7 T6 7.54 16.7 T7 7.56 16.7 T6 7.54 16.7 T7 7.56 16.7 T7 7.56 16.7	400	
4-20 12-18-92 145.50 350 T1 8.27 18.9 (332.85) T2 8.96 17.2 T3 7.87 16.1 T4 7.66 16.7 T5 7.55 16.7 T6 7.54 16.7 T7 7.56 16.7	380	
(332.85) T2 8.96 17.2 T3 7.87 16.1 T4 7.66 16.7 T5 7.55 16.7 T6 7.54 16.7 T7 7.56 16.7 T7 7.56 16.7	370	
T3 7.87 16.1 T4 7.66 16.7 T5 7.55 16.7 T6 7.54 16.7 T7 7.56 16.7 T7 7.56 16.7	495	
T4 7.66 16.7 T5 7.55 16.7 T8 7.54 16.7 T7 7.56 16.7	375	
T5 7.55 16.7 T6 7.54 16.7 T7 7.56 16.7 T7 7.56 16.7	320	
T6 7.54 16.7 T7 7.56 16.7	300	
T7 7.56 16.7	300	
	300	
4 01 12-18-02 153 02 290 T1 7.92 18.9	310	
4-21 12-10-92 100.92 200 11 100	440	
(283.90) T2 7.71 16.7	- 400	
T3 7.30 16.1	340	
T4 7.29 16.7	330	
T5 7.31 16.7	330	
4-22 12-15-92 58.35 260 T1 7.79 17.2	240	
(253.80) T2 7.40 16.7	300	
T3 7.12 16.7	320	
T4 7.56 16.7	300	
T5 7.55 16.7	310	
4-23 11-19-92 100.62 160 T1 7.37 13.4	412	
(102.35) T2 7.14 12.9	342	
T3 7.20 12.9	312	
T4 7.17 13.1	332	
T5 7.20 13.1	• 330	
. T6 7.21 13.2	327	
4-24 11-19-92 103.60 125 T1 7.72 12.9	439	

WELL DEVELOPMENT RECORDS FOR SITE 4 WELLS

(a) Target purge volume calculated is five pore volumes.

(b) Measurement T1 was taken from the first water that was removed from the well. The last measurement listed was taken within approximately fifteen minutes of the end of purging (i.e., from some of the last water removed from the well).

(84)

366

327 333

332

7.57

7.50

7.51

7.52

T2 T3

T4

T5

12.8

12.6

12.7

12.6

APPENDIX E

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Slug Test Data

SLUG TEST CALCULATIONS INTERMEDIATE WELLS 4-19, 4-20, 4-21, AND 4-22

Cooper Slug Test Calculations**

<u>Well No.</u>	<u>K (ft/day)</u>	<u>D (ft)</u>	<u>rw (ft)</u>	<u>rc (ft)</u>	<u>t (sec)</u>	<u>B</u>	A
4-19	277	10	0.33	0.17	0.9	1.0	0.1
4-20	416	10	0.33	0.17	0.6	1.0	0.1
4-21	624	10	0.33	0.17	0.4	1.0	0.001
4-22	357	10	0.33	0.17	0.7	1.0	0.1

^{**} Input quantities are as follows:

D = Length of screen

rw = Redius from center of well to undisturbed aquifer material

rc = Casing diameter

t = Time from plot of drawdown vs. time corresponding to B=1 on type curve

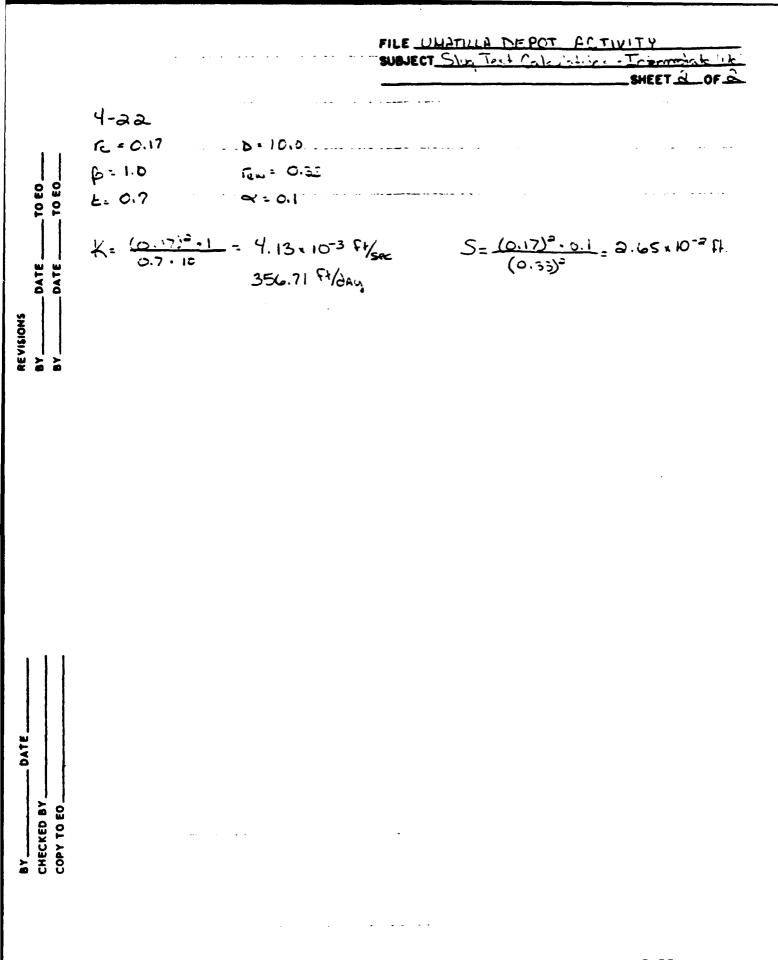
B = Type curve value for KDt/r²c

A = Type curve value for r²wS/r²c

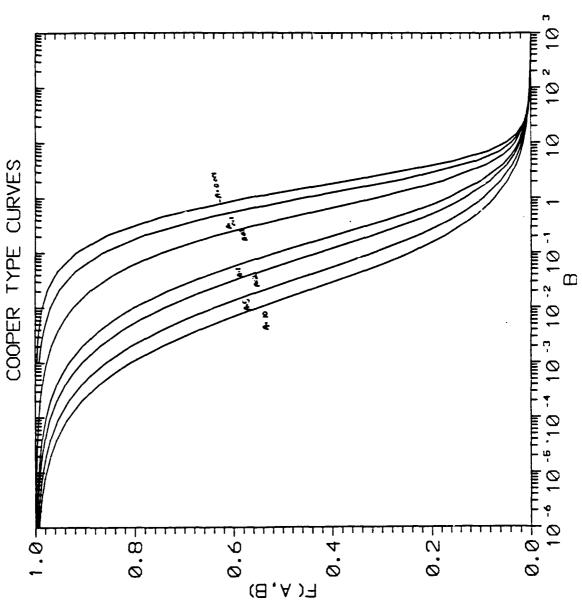
$$\frac{PILE UMBTULG. DePot Activity
Subset Slug Test Calculation: Tates intermediate the
Subset Slug Test Calculation:
KD = $\frac{c^2 p}{c} - \cdots - S_1 - \frac{c^2 a}{r_{co}^2}$
 $4 - 19$
 $c_1 = 0.17$ $b_1 = 10.0$
 $p_1 = 1.0$ $c_{w_1} = 5.52$
 $c_2 = 0.9$ $a_1 = 0.1$
 $k_2 = \frac{(0.17)^{L-1}}{c^{2} + 10} = 3.21 \times 10^{-3} P/sec$ $S = \frac{(0.15)^2 \cdot a_1}{(0.25)^2} = 3.45 \cdot b^{-1}R$
 $q_1 = \frac{1}{2}$
 $4 - 20$
 $R = \frac{10}{c^{2} + 10} = \frac{1}{2} \cdot \frac{10^{-3}}{c^{2} + 10} = \frac{1}{2} \cdot \frac$$$

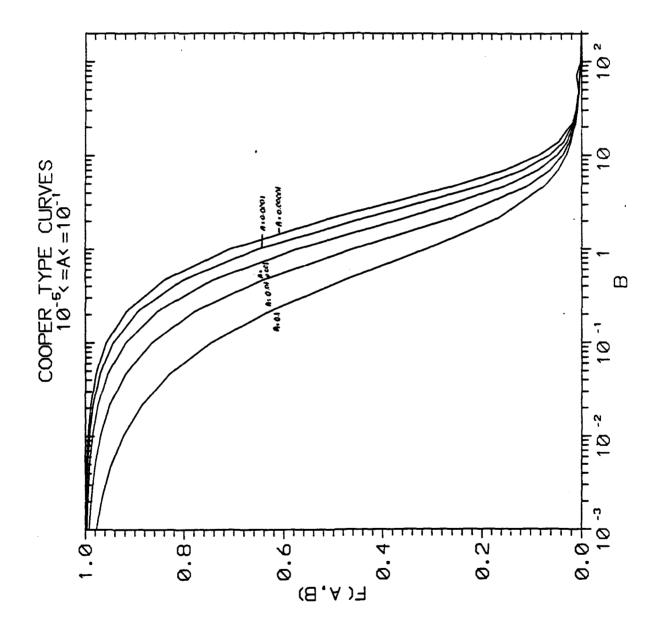
Dames & Moore

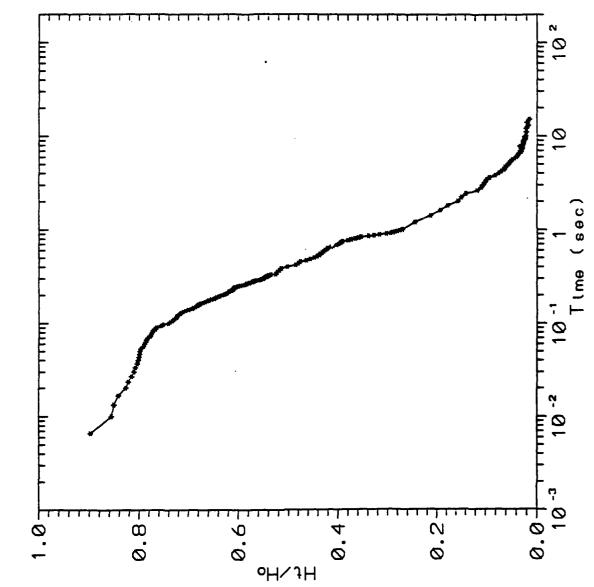
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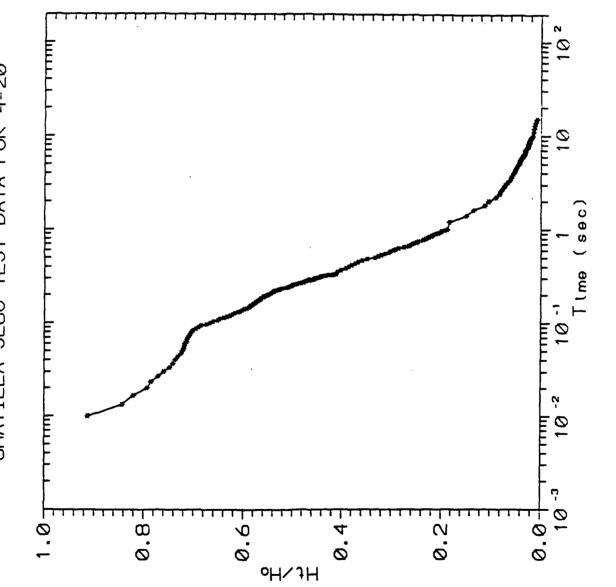
Dames & Moore







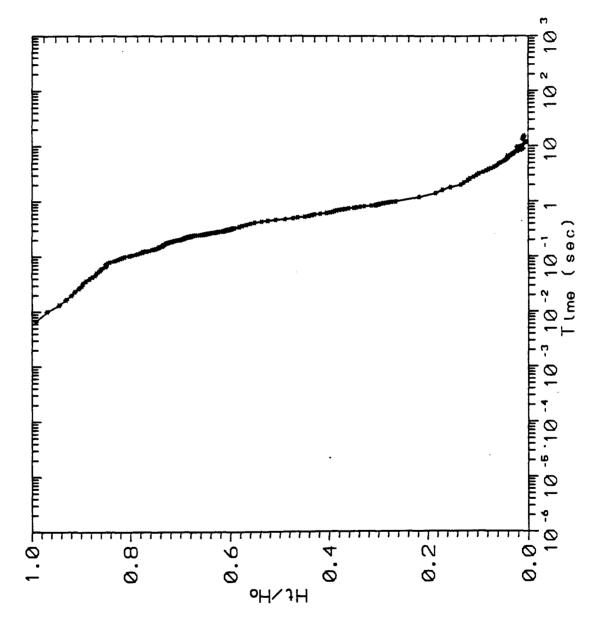
UMATILLA SLUG TEST DATA FOR 4-19

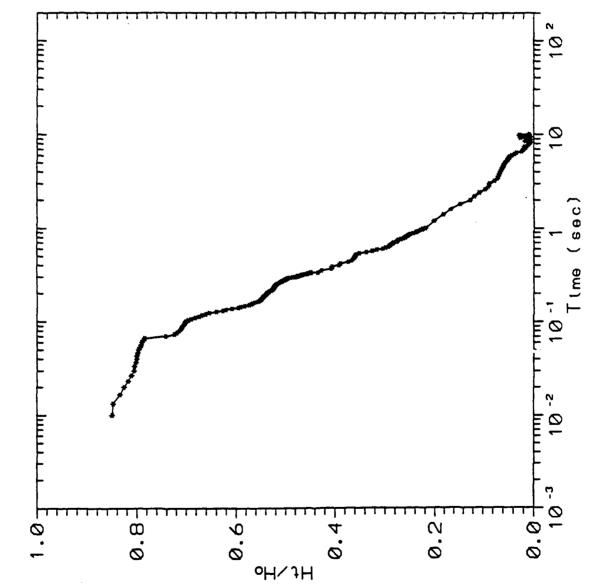












UMATILLA SLUG TEST DATA FOR 4-22

E-10

SLUG TEST CALCULATIONS ALLUVIAL WELLS 4-23 AND 4-24

Bouwer and Rice Slug Test Model Calculations*

<u>Weil No.</u>	<u>K (ft/day)</u>	<u>D (ft)</u>	<u>H (ft)</u>	<u>L (ft)</u>	<u>rw (ft)</u>	<u>rc (ft)</u>	<u>t (sec)</u>	<u>y0 (ft)</u>	<u>yt (ft)</u>
4-23	1571	8.55	8.55	15	0.38	0.24	0.2	0.07	0.032
4-24	1944	8.05	8.05	15	0.38	0.24	0.45	0.055	0.006

H = Depth from water table to bottom of screen

- t = Arbitrary time selected to pick value of yt from straight-line portion of plot of drawdown vs. time
- y0 = Drawdown at point where the extended straight-line portion of the graph intersects the y-axis at zero time
- yt = Drawdown at time t on the straight-line portion of the graph

^{*} Input quantities are as follows:

D = Depth from water table to base of aquifer (saturated thickness)

L = Length of screen

rw = Radius from center of well to undisturbed aquifer material

rc = Casing diameter plus a correction for the presence of a gravel or sand pack

FILE UMATILLA DEPOT ACTIVITY SUBJECT Slug Test Calculations - Allyvial Well-SHEET ... Bouwer and fice Metinod Calculations $K = \frac{c^{2} \ln (2k_{w})}{2k} \cdot \frac{1}{2} \cdot \ln (2k_{w}) - \ln 2k_{w} = \left[\frac{1 \cdot 1}{2k_{w}} + \frac{c}{2k_{w}}\right]^{-1}$ Correction for $r_c = \left[r_c^2 + n\left(r_w^2 - r_c^2\right)\right]^2$ 4-23 t= 0.2 H= 8,55 ra=0.17 11: - 2.37 n - 25% L = 15.0 ್ಷ ು.ುಕ್ಕ F. = 0.32 r. = 0.24 C = 2.39 \ \ \ \ $r_{c} = \left[0.17^{2} + 0.25 \left(0.28^{2} - 0.17^{2} \right) \right]^{1/2} = 0.24$ K = 0.24². 2.42. 1 . 10. (0.07) = 1.82 × 10⁻² = 1/sec 1571.19 504.

4-24 H = 8.05G= 0.17 t - 0.45 1 - 25% L= 15.0 40 = 0.055 4 = 0.006 C = 2.39 Fw=0.38 1 = J. 24 re= [0.17=+ aas (0.38= - 0.17=)] = 0.24 $ln^{3}r_{1} = \left[\frac{1.1}{ln(1.5)} + \frac{a\cdot 39}{15}\right]^{-1} = 2.38$ $K = 0.24^{2} \cdot 2.38 \cdot \frac{1}{0.45} \cdot \ln(\frac{0.055}{0.006}) = 2.25 \times 10^{-2} \, \text{fl}_{\text{Sc}}$ $3 \cdot 15 \cdot 0.45 \cdot \ln(\frac{0.055}{0.006}) = 1943.86 \, \text{fl}_{\text{Sc}}$

Dames & Moore

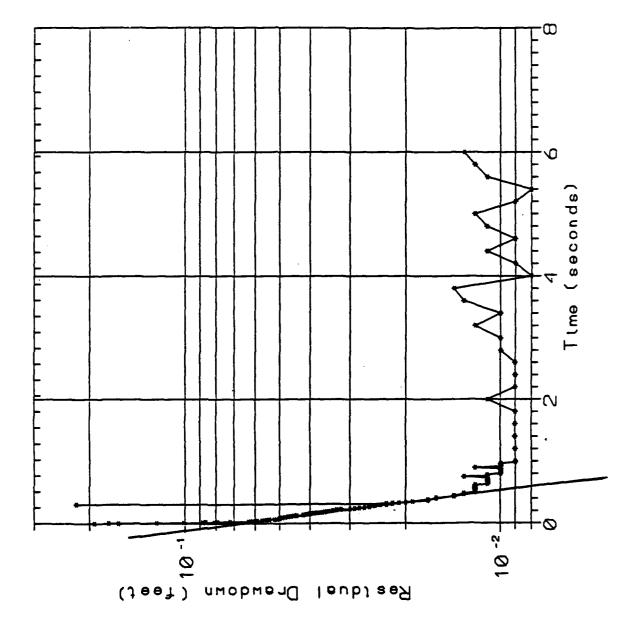
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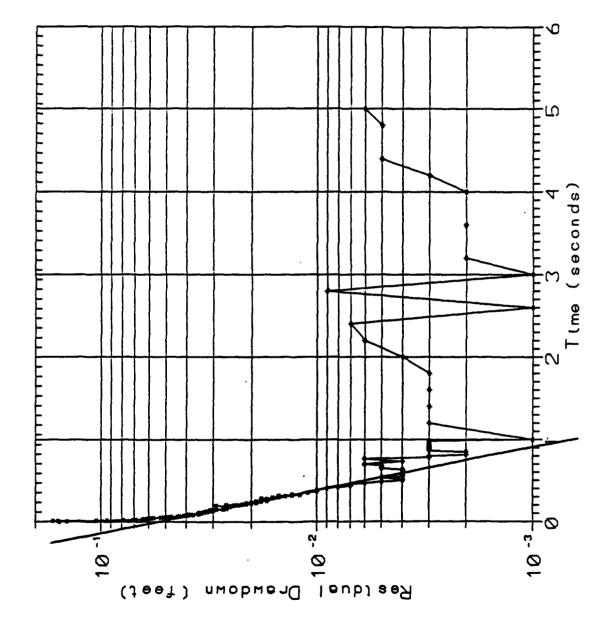
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APPENDIX F

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XRF Analytical Results of Basalt Core Samples

February 22, 1993

Mark Ochsner Dames and Moore 1750 S.W. Harbor Way, Suite 400 Portland, OR 97201

Dear Mark:

The X-ray fluorescence (XRF) analyses for the Umatilla-Ordinance Depot samples 4-20 (170-176.5') and 4-22 (75-80') just received from Washington State University's GeoAnalytical Laboratory are enclosed. Both of these samples are Elephant Mountain Member of the Saddle Mountains Basalt. The chemistry is an excellent match for Elephant Mountain and the lithology is also consistent, being abundantly microphyric with larger phenocrysts rare. Chemically the lower SiO₂, intermediate P_2O_5 , and higher TiO₂ abundances are distinctive. This unit is usually underlain by the Rattlesnake Ridge interbed.

I checked the magnetic polarity using top and bottom of the core samples with a flux-gate magnetometer and found the sample from 4-22 to read "reversed" while the sample from 4-20 read "normal". Perhaps one of the cores had been inverted in handling, but Elephant Mountain is considered to be "reversed" to "transitional" paleomagnetic polarity.

Elephant Mountain Member comprises two flows in some of its distributional area, but the Umatilla location is very near to the mapped edge of this unit. The other reversed polarity unit that also pinches out in this locality is the Pomona Member, but these chemical analyses are nothing like the Pomona composition. Therefore, I can confidently conclude that these two core samples are from the Elephant Mountain Member of the Saddle Mountains Basalt.

Thank you for the opportunity to consult with you on this project! If you have and questions or need further consultation, please give me a call.

Sincerely,

-N Beer anun

(Marvin H. Beeson, Ph.D. Geologist (OR G493)

Table of XRF Data

UMATILLA-ORDINANCE DEPOT

Drill Hole Depth XRF Date	4-20 170-175' 17-Feb-93	4-22 75-80' 17-Feb-93
	Normalized F	Results (Weight %)
SiO2	51.16	50.33
AI2O3	12.86	12.75
TiO2	3.55	3.49
FeO*	14.91	14.34
MnO	0.215	0.30
CaO	8.60	9.95
MgO	4.41	4.37
K20	1.15	1.14
Na2O	2.60	2.79
P205	0.538	0.538
	Trace Eleme	nts (ppm)
Ni	4	6
Cr	23	20
Sc	39	35
V	412	424
Ba	467	459
Rb	28	28
Sr	235	240
Zr	239	236
Y	51	50
Nb	26.4	27.3
Ga	22	19
Cu	15	11 ·
Zn	148	
Pb	6	3
La	29	34
Ce	78	72
Th	6	8
CRBG	Elephant	Elephant
UNIT	Mountain	Mountain

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Major elements are normalized on a volatile-free basis "Total Fe is expressed as FeO

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XRF Analyses by WSU GeoAnalytical Laboratory