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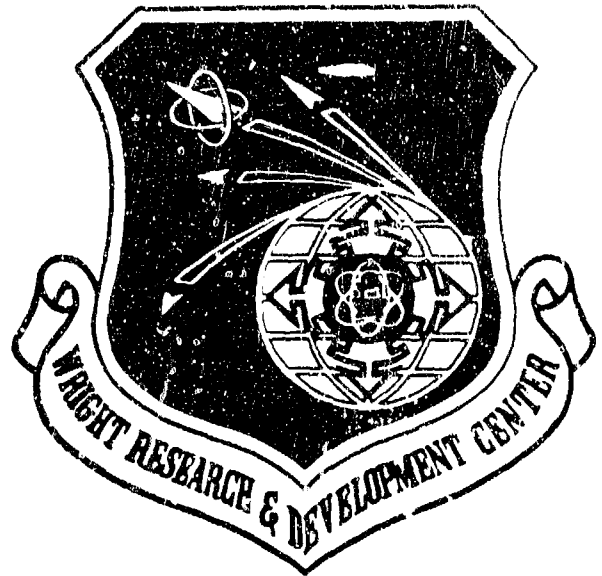
Vipperla B. Venkayya

Victoria A. Tischler

Analysis and Optimization Branch
Structures Division

April 1989

(Revised April 1993)



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WRIGHT RESEARCH and DEVELOPMENT CENTER
AIR FORCE SYSTEMS COMMAND
WRIGHT-PATTERSON AIR FORCE BASE, OHIO 45433-6523

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QUAD4 SEMINAR

Vipperla B. Venkayya

Victoria A. Tischler

Analysis and Optimization Branch
Structures Division

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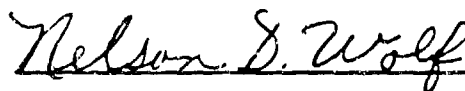
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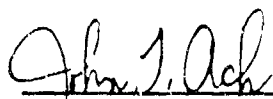


VIPPERLA B. VENKAYYA
Aerospace Engineer
Design & Analysis Methods Group



NELSON D. WOLF, Technical Manager
Design & Analysis Methods Group
Analysis & Optimization Branch

FOR THE COMMANDER



JOHN T. ACH, Chief
Analysis & Optimization Branch
Structures Division

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FOREWORD

This technical report is prepared for presentation at training seminars modeling plate elements in NASTRAN (NASA STRuctural ANalysis Program). This training seminar is usually given at NASTRAN User Colloquiums or NASTRAN Applications Short Courses.

INTRODUCTION

NASTRAN (NASA STRuctural ANalysis Program) is the most widely used general purpose structural analysis program in the world. This program was originally developed in the mid-sixties, and the first version was released in 1968. The program developed was sponsored by the National Aeronautics and Space Administration (NASA). This development in the form of enhancements and maintenance was continued until 1972 as a single Government version under the sponsorship of NASA. Since then the Government version is being developed as COSMIC-NASTRAN, while a commercial version is being marketed by McNeal and Swendler Corporation (MSC) as MSC-NASTRAN. Even though the basic structure of these two versions remains the same, there are significant differences in capabilities and efficiencies. Around 1980 the MSC introduced a new plate bending element called the QUAD4 to its element library. It is one of the most versatile plate bending elements, even though its theoretical basis is somewhat controversial. The basic strength of this element is that it was subjected to exclusive numerical testing, and a number of empirical adjustments were made to conform the results to known solutions. The QUAD4 element embodies a number of improvements over the earlier elements (QDMEM, QDMEM1, QDMEM2, QDELT, QAD1 and QAD2).

- a. It is an isoparametric formulation.
- b. It models inplane (membrane) behavior more realistically.
- c. The layered composites modeling capability is extensive.
- d. Membrane-bending coupling can be modeled realistically.
- e. It is the only plate element with an offset feature.
- f. It is a convenient element for modeling laminate plates.
- g. The same element can be used in modeling sandwich plates; even though it is not as simple to model sandwich plates with composite face sheets.
- h. A single plate element replaced all other elements.

Until around 1987, COSMIC-NASTRAN did not have a similar (QUAD4) capability. The absence of this capability represented a significant inconvenience, in particular, for modeling layered composites. Between 1983-1986 a QUAD4 element was developed under the sponsorship of the Air Force (Flight Dynamics Laboratory) for use in the program ASTROS (Automated Structural Optimization System). At the same time this element was incorporated into COSMIC-NASTRAN. The ASTROS-QUAD4 element is similar to the MSC-QUAD4, but there are significant differences in the theoretical formulation between the two elements.

This report provides an informal background for modeling with the QUAD4 element. It contains over 15 problems to illustrate various options of the element. However, the report was intended only as background material to be used in conjunction with a one-day short course. The section on the theoretical formulation was

reproduced verbatim from one of the interim reports of the ASTROS
contract.

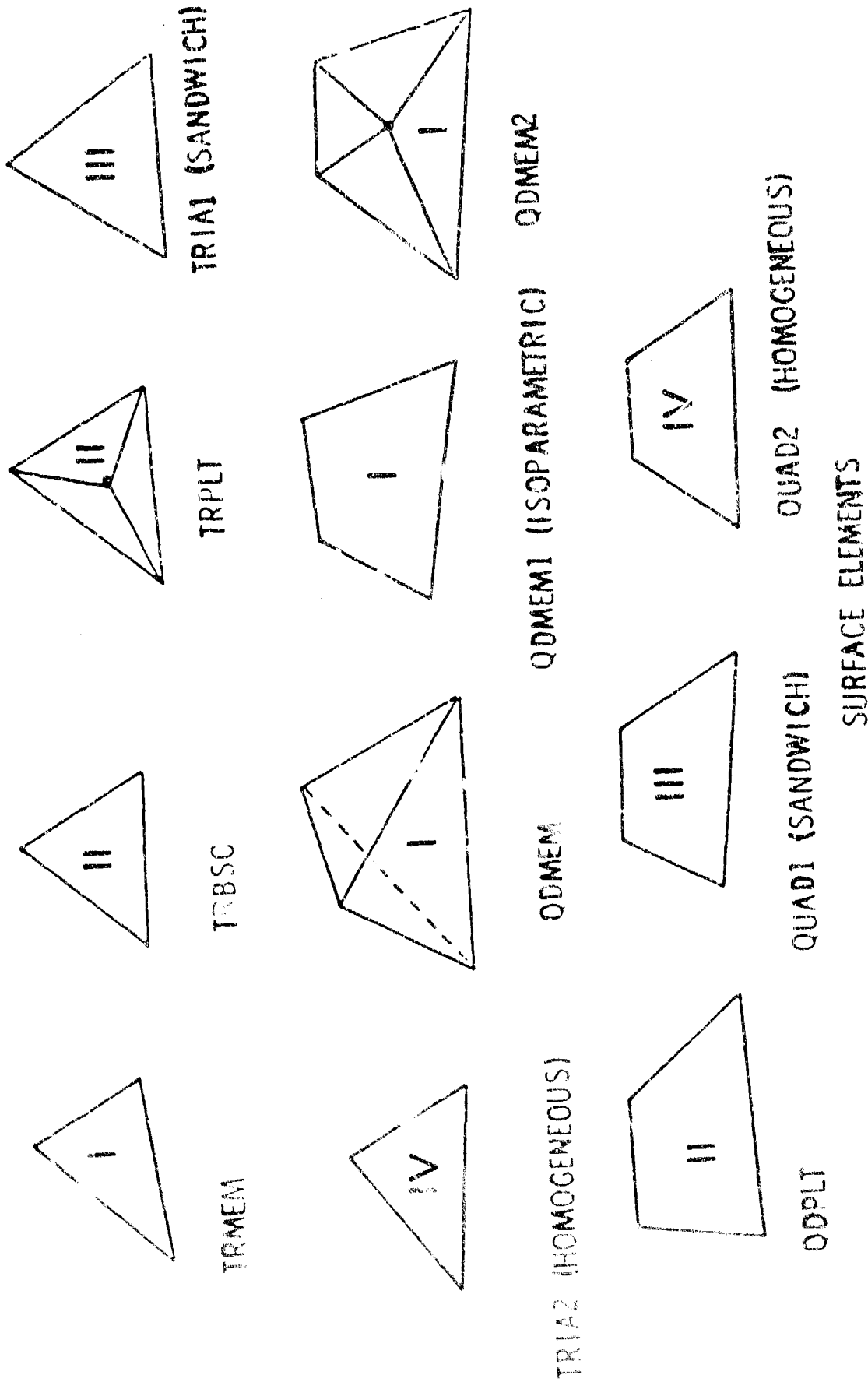
2.0 SEMINAR VIEWGRAPHS

QUAD4 ELEMENT - PURPOSE

- MODEL MEMBRANE BENDING PROBLEMS
- MODEL COMPOSITE PLATES
- MODEL MEMBRANE BENDING COUPLING
- MODEL SANDWICH PLATES IN MEMBRANE BENDING
- REPLACES QDMEM, QDMEM1, QDMEM2, QDPLT,
QUAD1, QUAD2, SHEAR

NASTRAN OVERVIEW

STRUCTURAL ELEMENTS IN NASTRAN PLATE ELEMENTS (III)



• • • • • QUAD4 ELEMENT - FEATURES • • • • •

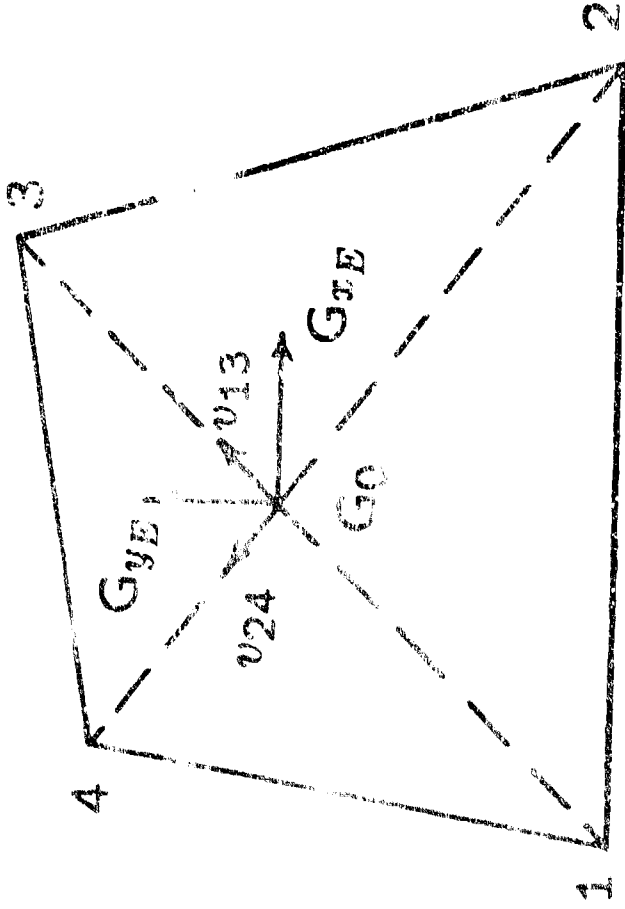
- ISOPARAMETRIC ELEMENT
- BILINEAR VARIATION OF
 - GEOMETRY
 - DEFORMATION
- PROVIDES STIFFNESS FOR 5 DEGREES OF FREEDOM
- TRANSVERSE SHEAR FLEXIBILITY
- INTERLAMINAR SHEAR STRESSES
- FIVE FAILURE THEORIES

QUAD4 ELEMENT - FEATURES

- ALLOWS MODELING
 - THIN PLATES
 - THICK PLATES
 - HIGH ASPECT RATIO ELEMENTS
 - SKEWED ELEMENTS
 - OFF SET ELEMENTS
- NUMERICAL MODELING OF OUT-OF-PLANE SHEAR STRAINS
 - LUMPED MASS MODELS
 - CONSISTENT MASS MODELS (PLANNED)
 - DIFFERENTIAL STIFFNESS (PLANNED)

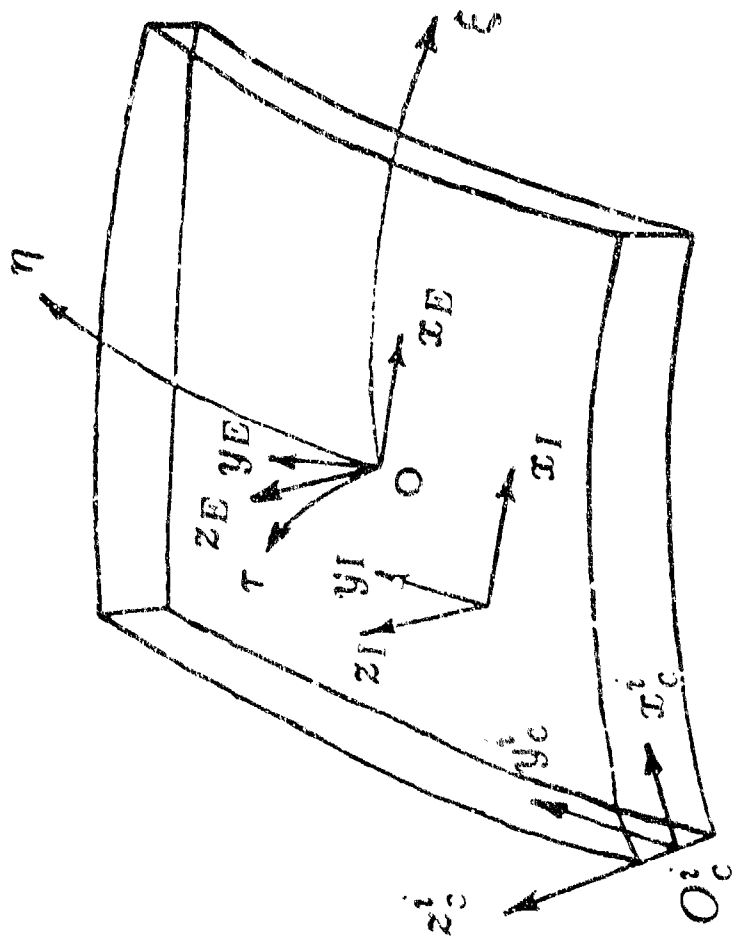
QUAD4 ELEMENT - THEORETICAL OUTLINE

- TWO ELEMENT COORDINATE SYSTEMS
 - DEFINED BY ELEMENT CONNECTIVITY
 - INTERNAL ELEMENT COORDINATE SYSTEM
- FOR STIFFNESS FORMULATION



Internal Element Coordinate System

QUAD4 ELEMENT - THEORETICAL OUTLINE



Isoparametric Quadrilateral 4-Node Plate & Shell Element

QUAD4 ELEMENT - SHAPE FUNCTIONS

$$\tilde{x}_E(\xi, \eta) = N_1 \tilde{x}^1 + N_2 \tilde{x}^2 + N_3 \tilde{x}^3 + N_4 \tilde{x}^4$$

$$\tilde{u}_E(\xi, \eta) = N_1 \tilde{u}^1 + N_2 \tilde{u}^2 + N_3 \tilde{u}^3 + N_4 \tilde{u}^4$$

$N_i \Rightarrow N_i(\xi, \eta) \Rightarrow$ SHAPE FUNCTIONS

$\tilde{x}^i \Rightarrow$ GRID POINT COORDINATES

$$[x_i, y_i, z_i]^t$$

$\tilde{u}^i \Rightarrow$ GRID POINT DISPLACEMENTS

$$[u_i, v_i, w_i, \theta_{xi}, \theta_{yi}, \theta_{zi}]^t$$

QUAD4 ELEMENT - SHAPE FUNCTIONS

$$N_i = \frac{1}{4}(1 + \xi\xi_i)(1 + \eta\eta_i)$$

$$\frac{\partial N_i}{\partial \xi} = \frac{1}{4}\xi_i(1 + \eta\eta_i)$$

$$\frac{\partial N_i}{\partial \eta} = \frac{1}{4}\eta_i(1 + \xi\xi_i)$$

QJAD4 - STRAIN DISPLACEMENT RELATIONS

• LINEAR STRAIN - DISPLACEMENT RELATIONS

$$\{e\} = \begin{pmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \\ \gamma_{yz} \\ \gamma_{zx} \end{pmatrix} = \begin{pmatrix} \frac{\partial u}{\partial x} \\ \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \\ \frac{\partial w}{\partial y} + \frac{\partial v}{\partial z} \\ \frac{\partial w}{\partial x} + \frac{\partial u}{\partial z} \end{pmatrix}$$

QUAD4 - STRAIN DISPLACEMENT RELATIONS

MEMBRANE - BENDING STRAIN - DISPLACEMENT RELATIONS

$$\begin{pmatrix} \epsilon_M \\ \gamma_{TS} \\ \epsilon_B \end{pmatrix} = \begin{pmatrix} \frac{\partial N_i}{\partial x} & 0 & 0 & 0 \\ 0 & \frac{\partial N_i}{\partial y} & \frac{\partial N_i}{\partial x} & 0 \\ \frac{\partial N_i}{\partial y} & \frac{\partial N_i}{\partial x} & \frac{\partial N_i}{\partial y} & \frac{\partial N_i}{\partial x} \\ 0 & \frac{\partial N_i}{\partial x} & 0 & 0 \\ 0 & \frac{\partial N_i}{\partial y} & \frac{\partial N_i}{\partial y} & 0 \\ 0 & 0 & \frac{\partial N_i}{\partial x} & 0 \\ 0 & 0 & \frac{\partial N_i}{\partial y} & 0 \\ 0 & \frac{\partial N_i}{\partial x} & 0 & 0 \\ 0 & \frac{\partial N_i}{\partial y} & \frac{\partial N_i}{\partial x} & 0 \\ 0 & 0 & \frac{\partial N_i}{\partial y} & 0 \\ 0 & 0 & \frac{\partial N_i}{\partial x} & 0 \\ 0 & 0 & \frac{\partial N_i}{\partial y} & 0 \end{pmatrix} \begin{pmatrix} u_M \\ v_M \\ w_M \\ u_B \\ v_B \\ w_B \end{pmatrix}$$

QUAD4 - STRAIN DISPLACEMENT RELATIONS

• RELATION BETWEEN:

$[u_M \ v_M \ w_M \ u_B \ v_B \ w_B]$ and $[u \ v \ w \ \theta_x \ \theta_y \ \theta_z]$

$$\begin{pmatrix} u_M \\ v_M \\ w_M \\ u_B \\ v_B \\ w_B \end{pmatrix} = \begin{pmatrix} T & | & 0 \\ \hline 0 & | & \frac{JtA}{2} \end{pmatrix} \begin{pmatrix} u \\ v \\ w \\ \theta_x \\ \theta_y \\ \theta_z \end{pmatrix} E$$

• JACOBIAN (TRANSFORMATION FROM \tilde{x} TO ξ)

$$\begin{pmatrix} \frac{\partial N_i}{\partial \xi} \\ \frac{\partial N_i}{\partial \eta} \\ \frac{\partial N_i}{\partial \zeta} \end{pmatrix} = \begin{pmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial y}{\partial \xi} & \frac{\partial z}{\partial \xi} \\ \frac{\partial x}{\partial \eta} & \frac{\partial y}{\partial \eta} & \frac{\partial z}{\partial \eta} \\ \frac{\partial x}{\partial \zeta} & \frac{\partial y}{\partial \zeta} & \frac{\partial z}{\partial \zeta} \end{pmatrix} \begin{pmatrix} \frac{\partial N_i}{\partial x} \\ \frac{\partial N_i}{\partial y} \\ \frac{\partial N_i}{\partial z} \end{pmatrix}$$

QUAD4 ELEMENT - STRESS - STRAIN RELATIONS

* STRESS AND STRAIN ARE RELATED BY:

$$\begin{pmatrix} \sigma_M \\ \sigma_B \\ \tau_{TS} \end{pmatrix} = \begin{pmatrix} G_1 & 0 & 0 \\ 0 & G_2 & 0 \\ 0 & 0 & G_3 \end{pmatrix} \begin{pmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS, MEC} \end{pmatrix} = \begin{pmatrix} \epsilon_M \\ \epsilon_B \\ 0 \end{pmatrix}^T$$

or

$$[\sigma]_i = [G]_i (\{\epsilon\}_{MEC} - \{\epsilon\}_T)_i$$

QUAD4 ELEMENT - STRESS - STRAIN RELATIONS

where

$\{\sigma_M\}$ - Membrane stress vector

$\{\sigma_B\}$ - Bending stress vector

$\{\tau_{TS}\}$ - Transverse shear stress vector

$\{G_1\}$ - Membrane moduli matrix

$\{G_2\}$ - Bending moduli matrix

$\{G_3\}$ - Transverse shear moduli matrix

$\{e_M\}$ - Membrane strain vector

$\{e_B\}$ - Bending strain vector

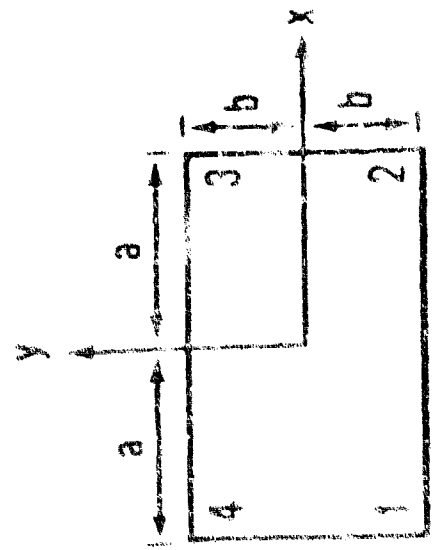
$\{\gamma_{TS}\}$ - Transverse shear strain vector

QUAD4 ELEMENT - STRESS - STRAIN RELATIONS

- MATERIAL CONSTANTS TRANSFORMATION

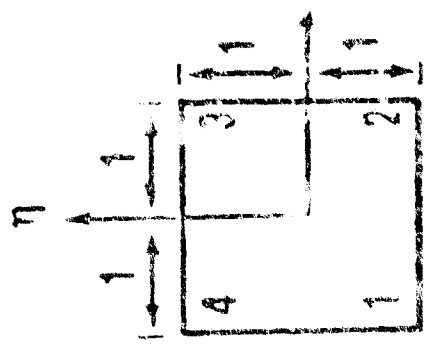
$$[G]_I = [U]^T [G]_M [U]$$

ISOPARAMETRIC MAPPING



$$\xi = \frac{x}{a}$$

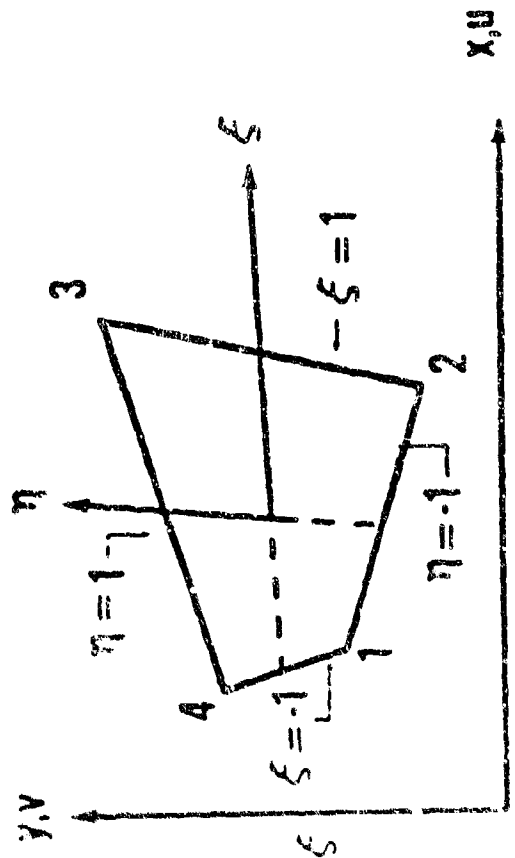
$$\eta = \frac{y}{b}$$



$$x = F_1(\xi, \eta)$$

$$y = F_2(\xi, \eta)$$

$$\underline{x} = \phi \underline{e} \underline{x} \underline{e}$$



ISOPARAMETRIC ELEMENTS: SAME POLYNOMIAL APPROXIMATION FOR DISPLACEMENTS AND COORDINATE TRANSFORMATION

FAILURE THEORIES

- 1) HILL
- 2) HOFFMAN
- 3) TSAI-WU
- 4) MAXIMUM STRESS
- 5) MAXIMUM STRAIN

FAILURE THEORIES

σ_t = Ultimate uniaxial tensile strength in the fiber direction

σ_c = Ultimate uniaxial compressive strength in the fiber direction

σ_t = Ultimate uniaxial tensile strength perpendicular to the fiber direction

σ_c = Ultimate uniaxial compressive strength perpendicular to the fiber direction

τ = Ultimate planar shear strength under pure shear loading

E_t = Ultimate uniaxial tensile strain in the fiber direction

E_c = Ultimate uniaxial compressive strain in the fiber direction

E_t = Ultimate uniaxial tensile strain perpendicular to the fiber direction

E_c = Ultimate uniaxial compressive strain perpendicular to the fiber direction

E_s = Ultimate planar shear strain under pure shear loading

FAILURE THEORIES

HILLS'S THEORY

$$\frac{\sigma_1^2}{x_2} + \frac{\sigma_2^2}{y_2} - \frac{\sigma_1\sigma_2}{x_2} + \frac{\tau_{12}^2}{s^2} = \text{FAILURE INDEX (FI)}$$

$x = x_t$ if $\sigma_1 > 0$, x_c if $\sigma_1 < 0$; similarly for y

For the interaction term:

$$\frac{\sigma_1\sigma_2}{s^2} , \quad x = x_t \quad \text{if } \sigma_1\sigma_2 > 0 \\ x = x_c \quad \text{otherwise}$$

FAILURE THEORIES

HOFFMAN'S THEORY

$$\left(\frac{1}{x_t} - \frac{1}{x_c}\right)\sigma_1 + \left(\frac{1}{y_t} - \frac{1}{y_c}\right)\sigma_2 + \frac{\sigma_1^2}{x_t x_c} + \frac{\sigma_2^2}{y_t y_c} + \frac{\tau_{12}^2}{s^2} + \frac{\sigma_1 \sigma_2}{x_t x_c} = FI$$

Note that this theory takes into account the difference in the tensile and compressive allowable stresses by using linear terms in the failure equation.

FAILURE THEORIES

TSAI-WU THEORY

$$F_{101} + F_{202} + F_{11}\sigma_1^2 + F_{22}\sigma_2^2 + 2F_{12}\sigma_1\sigma_2 + F_{66}\tau_{12}^2 = FI$$

$$F_1 = \frac{1}{x_t} - \frac{1}{x_c}, \quad F_2 = \frac{1}{y_t} - \frac{1}{y_c},$$

$$F_{11} = \frac{1}{x_t x_c}, \quad F_{22} = \frac{1}{y_t y_c}, \quad F_{66} = \frac{1}{s^2}$$

F_{12} needs to be determined experimentally from a biaxial test. However, satisfactory results may be obtained by setting it to zero.

FAILURE THEORIES

MAXIMUM STRESS

$$\sigma_1 \geq x_t, \sigma_1 > 0; \sigma_1 \leq -x_c, \sigma_1 < 0$$

$$\sigma_2 \geq y_t, \sigma_2 > 0; \sigma_2 \leq -y_c, \sigma_2 < 0$$

$$\tau_{12} \geq s, \tau_{12} > 0; \tau_{12} \leq s, \tau_{12} < 0$$

FAILURE THEORIES

MAXIMUM STRAIN

$$\epsilon_1 \geq E_t, \quad \epsilon_1 > 0; \quad \epsilon_1 \leq E_c, \quad \epsilon_1 < 0$$

$$\epsilon_2 \geq F_t, \quad \epsilon_2 > 0; \quad \epsilon_2 \leq F_c, \quad \epsilon_2 < 0$$

$$\gamma_{12} \geq E_s, \quad \gamma_{12} > 0; \quad \gamma_{12} \leq E_s, \quad \gamma_{12} < 0$$

SHEAR REDUCTION FACTORS

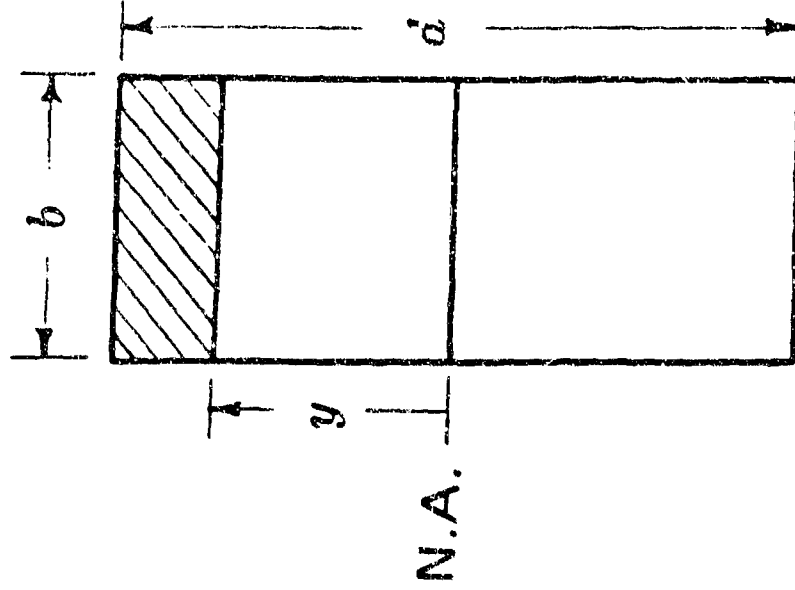
1. RECTANGULAR SECTION

$$I = \frac{bd^3}{12} \quad Q_y = \frac{b}{2} \left(\frac{d^2}{4} - y^2 \right)$$

$$\tau_y = \frac{vQ_y}{Ib} = \frac{v}{2I} \left(\frac{d^2}{4} - y^2 \right)$$

$v \Rightarrow$ Shear

$$\gamma_y = \frac{v}{2GI} \left(\frac{d^2}{4} - y^2 \right)$$



SHEAR REDUCTION FACTORS

Strain energy density at a distance y

$$= \frac{1}{2} \tau_y \gamma_y = \frac{1}{24GI^2} \left(\frac{d^2}{4} - y^2 \right)^2$$

$$\text{TOTAL STRAIN ENERGY} = \frac{v^2}{8GI^2} \int_{-d/2}^{d/2} \left(\frac{d^4}{16} - \frac{d^2 y^2}{2} + y^4 \right) dy$$

Carrying out the integration

$$\text{TOTAL STRAIN ENERGY} = \frac{1}{24} \frac{v^2}{G(5/6)A} = \frac{1}{24} \frac{v^2}{2G(\alpha A)}$$

$$\text{where } \alpha = \frac{5}{6} = 0.8333333$$

3.0 THEORETICAL DEVELOPMENT OF THE QUAD4 ELEMENT

APPENDIX A

THE QUAD4 ELEMENT

This appendix provides the theoretical development for the QUAD4 element that has been installed into ASTROS. An overview of this element is given in Subsection 5.3.3, while this appendix provides detailed information on the element. This detail is necessary because, unlike the other elements, the ASTROS QUAD4 element has not been documented elsewhere.

A.1 DISPLACEMENT FUNCTIONS

The QUAD4 element has two distinct element coordinate systems. These are the "user defined" element coordinate system as defined by the element connectivity data and the "internal element" coordinate system, which is defined as having its origin at $G_0 (X^0_E, Y^0_E, Z^0_E)$. This origin is computed by taking the average of the grid point coordinates. The positive X- and Y-axes of the internal element coordinate system are defined with the aid of two points, G_{xE} and G_{yE} described below.

\vec{V}_{13} and \vec{V}_{24} are defined as the unit diagonal vectors as illustrated in Figure A-1. Thus, the coordinates of points G_{xE} and G_{yE} are given by the following:

$$\begin{aligned} G_{xE} &= \left(\frac{X^0_E + X'_E}{2}, \frac{Y^0_E + Y'_E}{2}, Z^0_E \right) \\ G_{yE} &= \left(\frac{X^0_E - Y'_E}{2}, \frac{Y^0_E + X'_E}{2}, Z^0_E \right) \end{aligned} \tag{A-1}$$

where, X^0_E , Y^0_E and Z^0_E are the coordinates of the origins of the internal coordinate system and X'_E and Y'_E are the components of the bisector vector of the unit diagonals V_{13} and V_{24} .

The coordinates of points G_0 , G_{xE} and G_{yE} , are used to define the transformation from the internal element coordinate system to the coordinate system in which the grid points are defined. The internal element coordinate system is necessary to correctly handle irregular-shaped and non-planar elements and is henceforth referred to as the "element" (E) coordinate system.

Using 2-D interpolation functions, the geometry field at any point (ξ, η) in the element cross-section (see Figure A-2) is defined, where the nodal curvilinear coordinates are related to the nodal cartesian coordinates system in the element coordinate system by the following relationship:

$$\{X_E(\xi, \eta)\} = \sum_{i=1}^4 N_i(\xi, \eta) \{X_E^i\}$$

where i refers to grid point i , and

$$\{X_E^i\} = (X_E, Y_E, Z_E) \text{ at node } i,$$

$N(\xi, \eta)$ are the interpolation (shape) functions which define the contribution of each node at a given point with the element. These functions and their derivatives are:

$$N_i = 1/4(1 + \xi\xi_i)(1 + \eta\eta_i)$$

$$\frac{\partial N_i}{\partial \eta} = \frac{1}{4}\xi_i(1 + \eta\eta_i) \quad (A-2)$$

The deformations of the element are also represented with the identical interpolation functions:

$$\{U_E(\xi, \eta)\} = \sum_{i=1}^4 N_i(\xi, \eta) \{U_E^i\} \quad (A-3)$$

where $\{U_E^i\} = (U_E, V_E, W_E, \theta_{x_E}, \theta_{y_E}, \theta_{z_E})^T$ represents the vector of displacements at grid point i in the element coordinate system.

A.2 STRAIN-DISPLACEMENT RELATIONSHIP

The QUAD4 element incorporates a reduced solid theory for thick shells. According to this theory, the element has five dof at each grid, defined in a coordinate system whose X-Y plane is tangent to the mid-surface of the shell at the given grid point. The z-axis, therefore, is the normal to mid-surface at that point. In our nomenclature, this is called the "C" system (Figure A-2 and A-3).

A generalization of the "C" system, called "I" system, incorporates the characteristics of the "C" system at a general point on the mid-surface of the shell element, normally the integration point (Figure A-2).

In order to establish a common definition for "I" and "C" systems, consider the following steps:

(A) The tangents to mid-surface at a given point (ξ, η) are:

$$\{V_{t_1}\} = \frac{\partial \begin{Bmatrix} x \\ y \\ z \end{Bmatrix}_E}{\partial \xi} = \sum_{i=1}^4 \frac{\partial N_i}{\partial \xi} \begin{Bmatrix} x \\ y \\ z \end{Bmatrix}_E^i \quad (A-4)$$

$$\{V_{t_2}\} = \frac{\partial \begin{Bmatrix} x \\ y \\ z \end{Bmatrix}_E}{\partial \eta} = \sum_{i=1}^4 \frac{\partial N_i}{\partial \eta} \begin{Bmatrix} x \\ y \\ z \end{Bmatrix}_E^i \quad (A-5)$$

where

$\begin{Bmatrix} x \\ y \\ z \end{Bmatrix}_E^i$ are the coordinates of grid points in "E" system.

(B) The axes of the new system then follow:

$$\{Z\}_{I/C} = \{V_n\} = \frac{\{V_{t_1}\} \times \{V_{t_2}\}}{|\{V_{t_1}\} \times \{V_{t_2}\}|}$$

$$\{X\}_{I/C} = \frac{\{Y\}_E \times \{Z\}_{I/C}}{|\{Y\}_E \times \{Z\}_{I/C}|} \quad (A-6)$$

$$\{Y\}_{I/C} = \{Z\}_{I/C} \times \{X\}_{I/C}$$

(C) Finally:

$$[TIE] = [(\{X\}_I \{Y\}_I \{Z\}_I)^T] \quad (A-7)$$

$$[TCE]^i = [(\{X\}_c^i \{Y\}_c^i \{Z\}_c^i)^T] \quad (A-8)$$

Note that the "C" system is not necessarily invariant when we go from one grid to the next. This is due to the possible warping of the element.

Since the ultimate goal of this discussion is to establish a relationship between the element strains (which are defined in the "I" system), and the nodal displacements (defined in the "E" system), it is necessary to develop a series of transformations along with the strain-displacement relationships.

Consider the five dof's in the "C" system at each grid point "i" to be arranged in the following manner (Figure A-3):

$$(U)_c^i = \begin{Bmatrix} u \\ v \\ w \end{Bmatrix}_c^i ; \quad (\theta)_c^i = \begin{Bmatrix} \alpha \\ \beta \end{Bmatrix}_c^i \quad (A-9)$$

In order to be compatible with the other dof's in the model, these are related to the six dof at that grid point, defined in the "E" system, by the relationship:

$$(U)_c^i = [TCE]^i (U)_E^i$$

$$(\theta)_c^i = \begin{bmatrix} 0 & 1 & 0 \\ -1 & 0 & 0 \end{bmatrix} [TCE]^i (\theta)_E^i \quad (A-10)$$

The extra transformation in the rotational case is a result of the difference in the definition of rotations for "C" and "E" systems (Figures A-3 and A-4).

The same five dof's are related to six dof's in the "I" system by using the transformations developed in Equations A-7 and A-8. Considering Equation A-10 and A-3:

$$(U)_I = [TIE] \sum_{i=1}^4 N_i [(TCE)^i]^T (U)_c^i = [TIE] \sum_{i=1}^4 N_i (U)_E^i$$

$$= \sum_{i=1}^4 N_i [T] (U)_E^i \quad (A-11)$$

and

$$(\theta)_I = [TIE] \sum_{i=1}^4 N_i [(TCE)^i]^T (\theta)_c^i = [TIE] \sum_{i=1}^4 N_i [(TCE)^i]^T$$

$$\begin{bmatrix} 0 & 1 & 0 \\ -1 & 0 & 0 \end{bmatrix} [TCE]^i (\theta)_E^i = \sum_{i=1}^4 N_i [A]^i (\theta)_E^i \quad (A-12)$$

Note that while [T] is invariant, [A] depends on the direction of the normal to mid-surface at each grid point.

At a point along the Z-axis of "I" system, at a level of $Z = \zeta t_I/2$, where,

$$t_I = \sum_{i=1}^4 N_i t_i$$

is the thickness of the element evaluated at this particular integration point, the dof's in "I" system may be written in the following form:

$$(u_M)_I = (u)_I \quad ; \quad (u_B)_I = \zeta t_I/2 (\theta)_I \quad (A-13)$$

The strain-displacement relationships can now be developed, using these rearranged dof's:

$$(\epsilon_M)_I = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}_M - \begin{Bmatrix} \partial u / \partial x \\ \partial v / \partial y \\ \partial u / \partial y + \partial v / \partial x \end{Bmatrix}_I - \begin{bmatrix} \partial / \partial x & 0 & 0 \\ 0 & \partial / \partial y & 0 \\ \partial / \partial y & \partial / \partial x & 0 \end{bmatrix} (u_M)_I \quad (A-14)$$

$$(\epsilon_B)_I = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}_B - \begin{Bmatrix} \partial u / \partial x \\ \partial v / \partial y \\ \partial u / \partial y + \partial v / \partial x \end{Bmatrix}_I - \begin{bmatrix} \partial / \partial x & 0 & 0 \\ 0 & \partial / \partial y & 0 \\ \partial / \partial y & \partial / \partial x & 0 \end{bmatrix} (u_B)_I \quad (A-15)$$

$$(\gamma_S)_I = \begin{Bmatrix} \gamma_{yz} \\ \gamma_{zx} \end{Bmatrix}_I - \begin{Bmatrix} \partial w / \partial y + \partial v / \partial z \\ \partial w / \partial x + \partial u / \partial z \end{Bmatrix} - \begin{bmatrix} 0 & 0 & \partial / \partial y & 0 & \partial / \partial z & 0 \\ 0 & 0 & \partial / \partial x & \partial / \partial z & 0 & 0 \end{bmatrix} \begin{Bmatrix} (u_M)_I \\ (u_B)_I \end{Bmatrix} \quad (A-16)$$

Inserting Equations A-11 through A-13 into Equations A-14 through A-16, and considering the following:

$$\frac{\partial}{\partial z} (u_B)_I = \frac{\partial}{\partial z} z (\theta)_I = (\theta)_I$$

and (A-17)

$$\begin{Bmatrix} \partial / \partial x \\ \partial / \partial y \\ 1 \end{Bmatrix} = \sum_{i=1}^4 \begin{Bmatrix} \partial N_i / \partial x \\ \partial N_i / \partial y \\ N_i \end{Bmatrix}$$

we arrive at the following general relationships:

$$(\epsilon_M)_I = \sum_{i=1}^4 \begin{bmatrix} \partial N_i / \partial x & 0 & 0 \\ 0 & \partial N_i / \partial y & 0 \\ \partial N_i / \partial y & \partial N_i / \partial x & 0 \end{bmatrix} [T] (U) \begin{matrix} i \\ E \end{matrix} \quad (A-18)$$

$$(\epsilon_B)_I = \frac{5t_I}{2} \sum_{i=1}^4 \begin{bmatrix} \partial N_i / \partial x & 0 & 0 \\ 0 & \partial N_i / \partial y & 0 \\ \partial N_i / \partial y & \partial N_i / \partial x & 0 \end{bmatrix} [A]^i \{\theta\}_E^i \quad (A-19)$$

$$(\gamma_S)_I = \sum_{i=1}^4 \begin{bmatrix} 0 & 0 & \partial N_i / \partial y & | & 0 & N_i & 0 \\ 0 & 0 & \partial N_i / \partial x & | & N_i & 0 & 0 \end{bmatrix} \begin{bmatrix} [T] & | & 0 \\ \hline 0 & | & [A]^i \end{bmatrix} \left\{ \begin{array}{l} U \\ \theta \end{array} \right\}_E^i \quad (A-20)$$

or, collectively:

$$(\epsilon)_I = \begin{bmatrix} \epsilon_M \\ \vdots \\ \epsilon_B \\ \vdots \\ \gamma_S \end{bmatrix}_I = \begin{bmatrix} \partial N_1 / \partial x & 0 & 0 & | & & & \\ 0 & \partial N_1 / \partial y & 0 & | & & & \\ \partial N_1 / \partial y & \partial N_1 / \partial x & 0 & | & & & \\ \hline & & & & \frac{5t_I}{2} \begin{bmatrix} \partial N_i / \partial x & 0 & 0 \\ 0 & \partial N_i / \partial y & 0 \\ \partial N_i / \partial y & \partial N_i / \partial x & 0 \end{bmatrix} & & \\ \hline 0 & 0 & \partial N_1 / \partial y & | & 0 & N_1 & 0 \\ 0 & 0 & \partial N_1 / \partial x & | & N_1 & 0 & 0 \end{bmatrix} \begin{bmatrix} [T] & | & 0 \\ \hline 0 & | & [A]^i \end{bmatrix} \left\{ \begin{array}{l} u \\ v \\ w \\ \theta_x \\ \theta_y \\ \theta_z \end{array} \right\}_E^i \quad (A-21)$$

Since the shape functions N_i are defined in terms of the curvilinear coordinates (ξ, η) , the shape function derivatives are related to the corresponding Cartesian derivatives in the element [E] coordinate system, by using the rules of partial differentiation, as:

$$\begin{Bmatrix} \partial N_i / \partial \xi \\ \partial N_i / \partial \eta \\ \partial N_i / \partial \zeta \end{Bmatrix} = \begin{bmatrix} \partial x / \partial \xi & \partial y / \partial \xi & \partial z / \partial \xi \\ \partial x / \partial \eta & \partial y / \partial \eta & \partial z / \partial \eta \\ \partial x / \partial \zeta & \partial y / \partial \zeta & \partial z / \partial \zeta \end{bmatrix} \begin{Bmatrix} \partial N_i / \partial x \\ \partial N_i / \partial y \\ \partial N_i / \partial z \end{Bmatrix} \quad (A-22)$$

The first and second rows of the transformation matrix (or Jacobian matrix [J]) are the tangent vectors to the surface $r = \text{constant}$ and the third row is the interpolated values of the nodal normals. (Note the nodal normals are evaluated by carrying out the cross product of the two tangent vectors at the node point.)

From Equation A-7 the coordinates in the "I" system are related to the coordinates in the "E" system by the following:

$$\{U\}_I = [TIE]\{U\}_E$$

Therefore, the derivatives are given by:

$$\begin{Bmatrix} \partial N_1 / \partial x \\ \partial N_1 / \partial y \\ \partial N_1 / \partial z \end{Bmatrix} = [\phi] \begin{Bmatrix} \partial N_1 / \partial \xi \\ \partial N_1 / \partial \eta \\ \partial N_1 / \partial \zeta \end{Bmatrix}$$

where

(A-23)

$$[\phi] = [TIE] [J]^{-1} = \begin{bmatrix} \phi_{11} & \phi_{12} & 0 \\ \phi_{21} & \phi_{22} & 0 \\ \phi_{31} & \phi_{32} & \phi_{33} \end{bmatrix}$$

Note that $\partial N_1 / \partial \zeta$ and $\partial N_1 / \partial z$ will be zero when the interpolated normal at the integration point coincides with the normal to the mid-surface; e.g., in the case of the flat plate (ϕ_{31} and ϕ_{32} are zero). The zero terms in $[\phi]$, i.e., ϕ_{13} and ϕ_{23} , result from dot products of perpendicular vectors.

A.3 STRESS-STRAIN RELATIONSHIPS

Stresses are related to the previously defined strains by the elasticity matrix $[G]$ (where $[G]$ is partitioned to give separate membrane stresses).

$$\begin{Bmatrix} \sigma_M \\ \sigma_B \\ \tau_{TS} \end{Bmatrix} = \begin{bmatrix} G_1 & 0 & 0 \\ 0 & G_2 & 0 \\ 0 & 0 & G_3 \end{bmatrix} \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{Bmatrix}_{MEC} = \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \epsilon_T \end{Bmatrix}_T$$

or

(A-24)

$$[\sigma]_I = [G]_I ((\epsilon)_{MEC} - (\epsilon)_T)_I$$

where

(σ_M) Membrane stress vector

(σ_B) Bending stress vector

(τ_{TS}) Transverse shear stress vector

$[G_1]$ Membrane moduli matrix

$[G_2]$ Bending moduli matrix

$[G_3]$ Transverse shear moduli matrix

and subscripts "MEC" and "T" refer to mechanical and thermal, respectively.

The membrane-bending coupling moduli matrix [G4] will be incorporated into the [G] matrix following this discussion of the uncoupled matrices.

All anisotropic, orthotropic and isotropic material properties are supported. The elastic modulus matrix [G]_M is defined in the material coordinate system and transformed into the user defined element coordinate system by means of a transformation angle, θ_M, which references the user defined element X-AXIS or the material coordinate system ID (MCSID) specified by the user. θ_M is in the X-Y plane of the element as shown in Figure A-5.

The elastic modulus matrix in the element coordinate system is:

$$[G]_I = [U]^T [G]_M [U] \quad (A-25)$$

(Note that since the projection of X_I onto the X_E-Y_E plane is parallel to X_E, no extra transformations are required between the "E" and "I" systems.)

The transformation matrix for [G₁], [G₂] and [G₄] is:

$$[U_1] = \begin{bmatrix} \cos^2 \theta_M & \sin^2 \theta_M & \cos \theta_M \sin \theta_M \\ \sin^2 \theta_M & \cos^2 \theta_M & -\cos \theta_M \sin \theta_M \\ -2 \sin \theta_M \cos \theta_M & 2 \sin \theta_M \cos \theta_M & \cos^2 \theta_M - \sin^2 \theta_M \end{bmatrix} \quad (A-26)$$

and the transformation matrix for [G₃] is:

$$[U_2] = \begin{bmatrix} \cos \theta_M & \sin \theta_M \\ -\sin \theta_M & \cos \theta_M \end{bmatrix} \quad (A-27)$$

For isotropic materials:

(A) Membrane

$$[G_1] = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ & \nu & 0 \\ \text{SYM} & & \frac{1-\nu}{2} \end{bmatrix} \quad (A-28)$$

(B) Bending

$$[G_2] = \frac{t^3}{12I} [G_1] \quad (A-29)$$

(C) Transverse Shear

$$[G_3] = \frac{t}{E} \begin{bmatrix} \beta_1 \frac{(1-\nu)}{2K} & 0 \\ 0 & \beta_2 \frac{(1-\nu)}{2K} \end{bmatrix} \quad (A-30)$$

where E is the Young's modulus; t is the element thickness at the corresponding integration point, ν is the Poisson's ratio and t_s/t is the transverse shear factor.

Note that in matrix $[G_3]$, the factor "K" is introduced to compensate for the difference in shear distribution through the thickness, which is parabolic and not constant as indicated by the displacement function. The value of $K=1.2$ is the ratio of the relevant strain energies. The β_i factors, which are derived numerically, are introduced to compensate for the "locking" of the element due to excessive shear stiffness.

For anisotropic materials:

(A) Membrane

$$[G_1] = \begin{bmatrix} G_{11} & G_{12} & G_{13} \\ & G_{22} & G_{23} \\ \text{SYM} & & G_{33} \end{bmatrix} \quad (A-31)$$

(B) Bending

$$[G_2] = \frac{t^3}{12I} [G_1] \quad (A-32)$$

(C) Transverse Shear

$$[G_3] = \frac{t}{E} \begin{bmatrix} G_{11} & G_{12} \\ G_{12} & G_{22} \end{bmatrix} \quad (A-33)$$

For orthotropic materials:

(A) Membrane

$$[G_1] = \frac{1}{1-\nu_{12}\nu_{21}} \begin{bmatrix} E_1 & \nu_{12}E_2 & 0 \\ & E_2 & 0 \\ \text{SYM} & & G_{12}(1-\nu_{12}\nu_{21}) \end{bmatrix} \quad (A-34)$$

(B) Bending

$$[G_2] = \frac{t^3}{12I} [G_1] \quad (A-35)$$

(C) Transverse shear

$$[G_3] = \frac{t_s}{t} \begin{bmatrix} G_{1z} & 0 \\ 0 & G_{2z} \end{bmatrix} \quad (A-36)$$

where E_1 and E_2 are the Young's moduli in the principal material axes, ν_{12} is the major Poisson's ratio; G_{1z} is the in-plane shear modulus, G_{1z} and G_{2z} are the out-of-plane shear moduli and t_s/t is the transverse shear factor.

The derivation of the $[G_4]$ membrane-bending coupling matrix begins by denoting the strains at the mid-surface as:

$$(\epsilon_M^0) = \begin{Bmatrix} \epsilon_x^0 \\ \epsilon_y^0 \\ \gamma_{xy} \end{Bmatrix} \quad (A-37)$$

and the out of plane curvatures as:

$$(K) = \begin{Bmatrix} K_x \\ K_y \\ K_{xy} \end{Bmatrix} \quad (A-38)$$

Therefore, the strains at a distance z above the mid-surface of the element are:

$$(\epsilon) = (\epsilon_M^0) - z(K) \quad (A-39)$$

The corresponding 2-D stresses are:

$$(\sigma) = [G]_I ((\epsilon_M^0) - z(K)) \quad (A-40)$$

where $[G]_I$ is a (3x3) matrix of elastic moduli.

The forces and moments per unit length are therefore given by:

$$(F) = \int_{z_a}^{z_b} (\sigma) dz = \int_{z_a}^{z_b} [G]_I ((\epsilon^0) - z(K)) dz \quad (A-41)$$

$$(F) = t[G_1](\epsilon^0) + t^2[G_4](K)$$

$$(M) = \int_{z_a}^{z_b} (\sigma) z dz = \int_{z_a}^{z_b} [G]_I (-z(\epsilon^0) + z^2(K)) dz \quad (A-42)$$

$$(M) = t^2[G_4](\epsilon^0) + I[G_2](K)$$

where t is the plate thickness and I is the bending inertia. Assuming a linear variation of elastic properties between top and bottom surface.

$$[G_1] = \frac{1}{t} \int_{-t/2}^{t/2} G dz = \frac{G_T + G_B}{2} \quad (A-43)$$

$$[G_2] = \frac{1}{I} \int_{-t/2}^{t/2} G dz = \left(\frac{t}{12I}\right)[G_1] \quad (A-44)$$

$$[G_4] = \frac{1}{t^3} \int_{-t/2}^{t/2} (-z)G dz = -\left[\frac{G_T - G_B}{12}\right] \quad (A-45)$$

Note that the membrane-bending stiffness coupling terms vanish for a element whose elastic properties are symmetric relative to the mean plane of the element.

By assuming that the elastic modulus has a linear variation between the top and bottom surfaces, define:

$$G = G_1 + \zeta/2(G_T - G_B) \quad (A-46)$$

Therefore, from Equations A-31 and A-32:

(A) Membrane

$$G = G_1 + \zeta/2(-12G_4) \quad (A-47)$$

$$G = G_1 - 6\zeta G_4 \quad (A-48)$$

(B) Bending

$$G_2 = \frac{G_1 t^3}{12I} \quad (A-49)$$

$$G = \frac{12I}{t^3} G_2 - 6\zeta G_4$$

Matrix $[G_3]$ is not affected since transverse shears are assumed to have no coupling action.

Therefore, the stress-strain relationship, allowing for membrane, bending, transverse shear and membrane-bending coupling is:

$$\begin{Bmatrix} \sigma_M \\ \sigma_{TOT} \\ \tau_{TS} \end{Bmatrix} = \begin{bmatrix} G_1 & G_1 - 6\delta G_4 & 0 \\ G_1 - 6\delta G_4 & G_2 & 0 \\ 0 & 0 & G_3 \end{bmatrix} \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{Bmatrix} \quad (A-50)$$

where

$$(\sigma_M) = \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix} \quad , \quad \text{Membrane stresses}$$

$$(\sigma_{TOT}) = \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}_{TOT} \quad , \quad \text{Total membrane and bending stresses}$$

$$(\sigma_{TS}) = \begin{Bmatrix} \tau_{yz} \\ \tau_{xz} \end{Bmatrix} \quad , \quad \text{Transverse shear stresses}$$

$$(\epsilon_M) = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} \quad , \quad \text{Membrane strain}$$

$$(\epsilon_B) = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}_B \quad , \quad \text{Bending strains}$$

$$(\gamma_{TS}) = \begin{Bmatrix} \gamma_{yz} \\ \gamma_{xz} \end{Bmatrix} \quad , \quad \text{Transverse shear strains}$$

A.4 STIFFNESS MATRIX

The element stiffness matrix is derived by minimizing the total potential energy and is given in numerical form by employing the Gauss-quadrature integration method:

$$[K]_E = \sum \xi \sum \eta \sum \zeta [B]^T [G] [B] W_\xi W_\eta W_\zeta \det [J] \quad (A-51)$$

where (ξ, η, ζ) are the Gaussian integration point coordinates and $W_\xi, W_\eta,$ and W_ζ are the associated weight factors. $\det [J]$ represents the physical volume of the element as calculated at this point, B is the strain displacement relationship of Equation A-21 and G is the stress strain relationship of Equation A-50.

Each element stiffness matrix partition in the element coordinate system, $[K_{ij}]_{EE}$, is transformed to the global coordinate system by the following transformation:

$$[K_{ij}]_G = [TEG]_i^T [K_{ij}]_{EE} [TEG]_i \quad (A-52)$$

where $[TEG]_i$ is determined by relating the element coordinate system to the global coordinate system for grid i through the basic coordinate system:

$$[TEG]_i = [TEB]_i [TBG]_i \quad (A-53)$$

A.5 CONSISTENT AND LUMPED MASS MATRICES

The consistent mass matrix terms are evaluated, neglecting the rotational inertias associated with the α and β degrees of freedom, by the following expression:

$$M_{ij} = \sum_{n=1}^4 N_i N_j \rho |J| t_n \quad (A-54)$$

where N_i is the shape function for node i , ρ is the mass per unit volume, $|J|$ is the physical area of the element and t_n is the element thickness at the integration point.

The lumped mass matrix, which is calculated at the pseudo center (i.e., the average of the element grid coordinates), is prorated to the edges based on the distance of the pseudo center from each edge.

The terms of the lumped mass matrix are evaluated using:

$$M_{ij} = \sum_{i=1}^4 N_i \rho |J| t_n \quad (A-55)$$

The transformation of the mass matrix to the global coordinate system is carried out using the same transformation matrices as used for the stiffness matrix in Equation A-52.

A.6 STRESS RECOVERY

The element stresses in partitioned form from Equation A-50 are

$$\begin{Bmatrix} \sigma_M \\ \sigma_{TOT} \\ \tau_{TS} \end{Bmatrix} = \begin{bmatrix} G_1 & G_1 - 6\zeta G_4 & 0 \\ G_1 - 6\zeta G_4 & G_2 & 0 \\ 0 & 0 & G_3 \end{bmatrix} \left\{ \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{Bmatrix}_{MEC} - \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{Bmatrix} \right\}$$

or

$$\begin{Bmatrix} \sigma_M \\ \sigma_{TOT} \\ \tau_{TS} \end{Bmatrix} = \begin{bmatrix} G_1 & G_4' & 0 \\ G_4' & G_2 & 0 \\ 0 & 0 & G_3 \end{bmatrix} \left\{ \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{Bmatrix}_{MEC} - \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{Bmatrix}_T \right\} \quad (A-56)$$

For a specified grid point temperature, the thermal strain vector is:

$$(\epsilon_M)_T = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}_T = (\alpha_I)(T_I - T_0) \quad (A-57)$$

where $(\alpha_I) = [U]^{-1}(\alpha_M)$ is a vector of thermal expansion coefficients in the element coordinate system. $[U]$ is the strain transformation matrix given in Equation A-26 and (α_M) is the vector of thermal expansion coefficients in the material axes. T_I and T_0 are the specified grid point temperature and mid-surface (stress-free) temperature, respectively.

For a thermal gradient T' , the thermal strain vector $(\epsilon_B)_T$ is:

$$(\epsilon_B)_T = (\alpha_I) \left(\frac{\zeta t}{2} T' \right) \quad (A-58)$$

For thermal moments $(M)_T$, the thermal strain vector $(\epsilon_B)_T$ is:

$$(\epsilon_B)_T = \frac{-\zeta t}{2I} [G_2] (M)_T \quad (A-59)$$

NOTE: ASTROS does not support thermal gradient or moments so that the above equations are provided for completeness only.

The in-plane stress vector $(\sigma)_z$ at fiber distance z from the mid-surface is:

$$(\sigma)_z = \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}_z = \left(\frac{1-z}{2} \right) \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}_1 + \left(\frac{1+z}{2} \right) \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}_2 \quad (A-60)$$

where the stress vectors $(\sigma_x, \sigma_y, \tau_{xy})_1^T$ and $(\sigma_x, \sigma_y, \tau_{xy})_2^T$ are the bottom and top fiber stress vectors, respectively.

If a temperature T_1 is specified at the point where outer fiber stresses are to be calculated, the additional thermal stress due to the difference between the specified temperature and a temperature that would be produced by a uniform thermal gradient T' or thermal moments $(M)_T$ is calculated using:

$$(\Delta\sigma)_T = [G_2](\alpha_1)(T_1 - T_0 - T'z) \quad (A-61)$$

for a thermal gradient T' , and

$$(\Delta\sigma)_T = -z \frac{(M)_T}{I} + [G_2](\alpha)T_1 \quad (A-52)$$

A.7 FORCE RESULTANTS

The forces at the mid-surface are evaluated by taking the average stress values over the element thickness:

(A) Forces

$$(F) = \begin{Bmatrix} F_x \\ F_y \\ F_{xy} \end{Bmatrix} = ((\sigma)_{z1} + (\sigma)_{z2}) \frac{I}{2} \quad (A-63)$$

(B) Moments

$$(P) = \begin{Bmatrix} M_x \\ M_y \\ M_{xy} \end{Bmatrix} = ((\sigma)_{z1} - (\sigma)_{z2}) \frac{I}{2} \quad (A-64)$$

(C) Transverse Shear Forces

$$(Q) = \begin{Bmatrix} Q_x \\ Q_y \end{Bmatrix} = ((\tau)_{z1} + (\tau)_{z2}) \frac{I}{2} \quad (A-65)$$

where stress vectors $(\sigma)_{z1}$, $(\sigma)_{z2}$ are stresses at the integration points (default option) or at grid points (if requested) and, similarly, $(\tau)_{z1}$ and $(\tau)_{z2}$ are the transverse shear stresses.

A.8 THERMAL LOAD VECTOR

The thermal load vector is computed as:

$$(F_T) = \int_V [B][G](\alpha)_T dv \quad (A-66)$$

where the load vector (F_T) is defined as:

$$\{P_T\} = \begin{Bmatrix} F_T \\ \dots \\ M_T \end{Bmatrix} \quad (A-67)$$

where $\{F_T\}$ and $\{M_T\}$ are the thermal forces and moments, respectively.

The thermal strain vector is:

$$\{\epsilon_T\} = \begin{Bmatrix} \epsilon_{M_T} \\ \dots \\ \epsilon_{B_T} \end{Bmatrix} = \begin{Bmatrix} \alpha_M \\ \dots \\ \alpha_B \end{Bmatrix} \Delta T \quad (A-68)$$

where $\{\epsilon_{M_T}\}$ and $\{\epsilon_{B_T}\}$ are the thermal membrane and bending strains, and correspondingly α_M and α_B are the thermal coefficients of expansion for membrane and bending. ΔT is dependent on the temperature loading being specified.

- (A) For a specified grid point temperature the thermal membrane strain vector, $\{\epsilon_{M_T}\}$, is:

$$\{\epsilon_M\} = \alpha_M (T_1 - T_0) \quad (A-69)$$

T_1 = Grid point temperature

T_0 = Reference (stress-free) temperature

- (B) For a thermal gradient, the thermal bending strain vector, $\{\epsilon_{B_T}\}$, is:

$$\{\epsilon_B\} = \alpha_B \left(-\frac{z}{2} T' \right) \quad (A-70)$$

- (C) For thermal moments, the thermal bending strain vector, $\{\epsilon_{B_T}\}$, is:

$$\{\epsilon_B\} = [G_2] \{M\}_T \frac{z}{2I} \quad (A-71)$$

NOTE: ASTROS does not support thermal gradients or moments so that the above equations are provided for completeness only.

A.9 LAMINATED COMPOSITE MATERIALS

The capability to model a stack of layers with a single QUAD4 element is detailed including the computation of equivalent "single layer" properties, i.e., membrane, bending transverse shear and membrane-bending coupling. The recovery of element forces, layer and interlaminar shear stresses and the computation of ply failure indices is also described in the following overview of theory.

A.9.1 Overview of Theory

The calculation of the "overall" properties for the laminated composite elements is based on the classical lamination theory with the following assumptions:

- (A) Each of the lamina is in a state of plane stress.
- (B) The laminate is presumed to consist of perfectly bonded lamina.
- (C) The bonds are presumed to be infinitesimally thin and non-shear deformable. That is, the displacements are continuous across the lamina boundaries so that lamina can not slip relative to one another. Thus, the laminate behaves as a single layer with "special" properties.

The material properties of laminated composite materials are reflected in the following force-strain relationship:

$$\begin{Bmatrix} F \\ M \\ V \end{Bmatrix} = \begin{bmatrix} t G_1 & t^2 G_4 & 0 \\ t^2 G_4 & I G_2 & 0 \\ 0 & 0 & t_s G_3 \end{bmatrix} \begin{Bmatrix} \epsilon_M - \epsilon_M^T \\ \kappa - \kappa^T \\ \gamma \end{Bmatrix} \quad (A-72)$$

where

$$(F) = \begin{Bmatrix} F_x \\ F_y \\ F_{xy} \end{Bmatrix}, \quad \text{Membrane forces per unit length.}$$

$$(M) = \begin{Bmatrix} M_x \\ M_y \\ M_{xy} \end{Bmatrix}, \quad \text{Bending moments per unit length.}$$

$$(V) = \begin{Bmatrix} V_x \\ V_y \end{Bmatrix}, \quad \text{Transverse shear forces per unit length.}$$

and the remaining terms have been defined previously.

The G_1 , G_2 , and G_4 terms are defined by the following:

$$\begin{aligned} G_1 &= \frac{1}{t} \int [G_E] dz \\ G_2 &= \frac{1}{I} \int z^2 [G_E] dz \\ G_4 &= \frac{1}{t} \int -z [G_E] dz \end{aligned} \quad (A-73)$$

The limit on the integration are from the bottom surface to the top surface of the laminated composite. The elasticity matrix $[G_E]$ has the following form for isotropic materials:

$$[G_E] = \begin{bmatrix} \frac{E}{1-\nu^2} & \frac{\nu E}{1-\nu^2} & 0 \\ \text{SYM} & \frac{E}{1-\nu^2} & 0 \\ & & G \end{bmatrix} \quad (\text{A-74})$$

$$G = \frac{E}{2(1+\nu)} \quad (\text{A-75})$$

For orthotropic materials, matrix $[G_E]$ is:

$$[G_E] = \begin{bmatrix} \frac{E_1}{1-\nu_1\nu_2} & \frac{\nu_1 E_2}{1-\nu_1\nu_2} & 0 \\ \text{SYM} & \frac{E_2}{1-\nu_1\nu_2} & 0 \\ & & G_{12} \end{bmatrix} \quad (\text{A-76})$$

Equation A-73 may be rewritten as:

$$[G_{ij}]_1 = \frac{1}{t} \sum_{K=1}^N [\bar{G}_{ij}]^K (z_K - z_{K-1})$$

$$[G_{ij}]_2 = \frac{1}{3I} \sum_{K=1}^N [\bar{G}_{ij}]^K (z_K^3 - z_{K-1}^3) \quad (\text{A-77})$$

$$[G_{ij}]_3 = \frac{-1}{2t^2} \sum_{K=1}^N [\bar{G}_{ij}]^K (z_K^2 - z_{K-1}^2)$$

where $[\bar{G}_{ij}]^K$ is the reduced moduli matrix evaluated for each lamina K after transforming the lamina property matrix from the fiber to the element material axes.

z_K and z_{K-1} are the top and bottom distances of lamina K from the geometric middle plane of the laminate, as illustrated in Figure A-6, and N is the number of laminae (or plies). Note that the plies are numbered serially starting with 1 at the bottom layer. The bottom layer is defined as the surface with the largest -z value in the element coordinate system. If the

option to model membrane-only elements is exercised, matrices $[G_2]$, $[G_3]$, and $[G_4]$ are set to zero.

If the user defined element axis is not coincident with the element material axis, the user specified transformation angle θ_M , which references the element X-axis, is added to the layer orientation angle. The property matrices $[G_1]$, $[G_2]$, and $[G_4]$ are then transformed to the user defined element axis using the following equation:

$$[G_E] = [U]^T [G_M] [U] \quad (A-78)$$

where

$$[U] = \begin{bmatrix} \cos^2\theta & \sin^2\theta & \cos\theta\sin\theta \\ \sin^2\theta & \cos^2\theta & -\cos\theta\sin\theta \\ -2\sin\theta\cos\theta & 2\sin\theta\cos\theta & \cos^2\theta - \sin^2\theta \end{bmatrix} \quad (A-79)$$

The transverse shear flexibility (G_3) matrix is defined by:

$$[G_3] = \begin{bmatrix} G_{11} & G_{12} \\ G_{12} & G_{22} \end{bmatrix} \quad (A-80)$$

and the corresponding matrix transformed into the user-defined element coordinate system is given by:

$$[G] = [W]^T [G_M] [W] \quad (A-81)$$

where

$$[W] = \begin{bmatrix} \cos\theta & \sin\theta \\ -\sin\theta & \cos\theta \end{bmatrix} \quad (A-82)$$

The derivation of the transverse shear flexibility matrix $[G_3]$ for the laminate is considered next.

The mean value of the transverse shear modulus, G , for the laminated composite is defined in terms of the transverse shear strain energy, U , through the depth as:

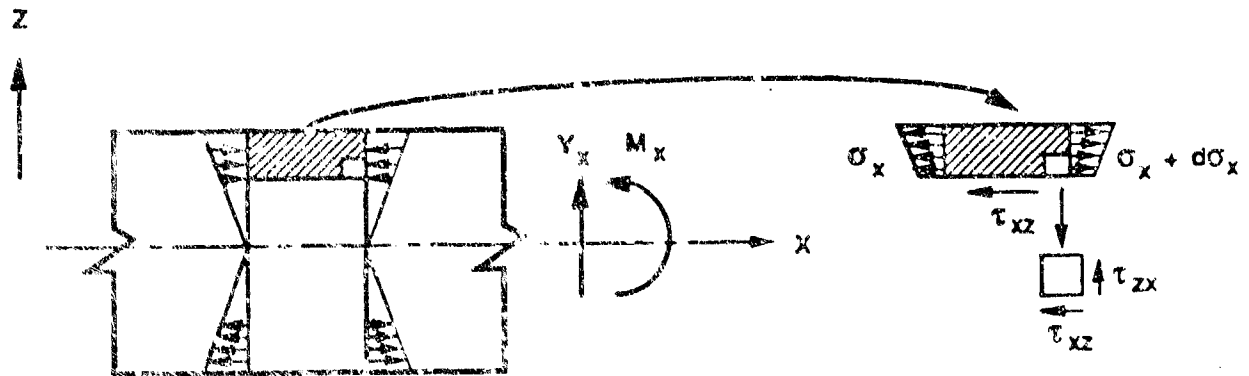
$$U = \frac{V^2}{2Gt} = \frac{1}{2} \int \frac{[r(z)]^2}{G(z)} dz \quad (A-83)$$

A unique mean value of transverse shear strain is assumed to exist for both the x- and y-components of the element coordinate system, but for ease of

discussion, only the evaluation of an uncoupled x-component of the shear moduli will be illustrated here. From Equation A-83, the mean value of transverse shear modulus is written in the following form:

$$\frac{1}{G_x} = \frac{E}{V^2} \sum_{i=1}^N \int_{z_{i-1}}^{z_i} \frac{(v_{zx}(z))^2}{(G_x)_i} dz \quad (\text{A-84})$$

where G is an "average" transverse shear coefficient used by the element code and $(G_x)_i$ is the local shear coefficient for layer i . To evaluate Equation A-84, it is necessary to obtain an expression for $[v_{zx}(z)]$. This is accomplished by assuming that the x- and y-components of stress are decoupled from one another. This assumption allows the desired equation to be deduced through an examination of a beam of unit cross-sectional width.



The equilibrium conditions in the horizontal direction and for total moment are:

$$\frac{\partial \tau_{xz}}{\partial z} - \frac{\partial \sigma_x}{\partial x} = 0 \quad (\text{A-85})$$

$$V_x + \frac{\partial M_x}{\partial x} = 0 \quad (\text{A-86})$$

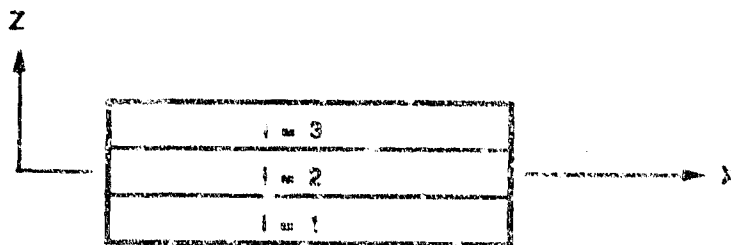
If the location of the neutral surface is denoted by z_N and ρ is the radius of curvature of the beam, the axial stress, σ_x , is expressed in the form:

$$\sigma_x = \frac{E(z - z_N)}{(EI)_x} M_x \quad (\text{A-87})$$

Equation A-87 is differentiated with respect to x and combined with Equations A-85 and A-86. For constant E_x , the result is integrated to yield the following expression:

$$\tau_{xz} = C_1 + \frac{V}{(EI)_x} \left[z_{xz} - \frac{z^2}{2} \right] E_{x1} \quad z_{1-1} < z < z_1 \quad (A-88)$$

Equation A-88 is used in the analysis of n -ply laminates because sufficient conditions exist to determine the constants C_i ($i=1, 2, \dots, n$) and the "directional bending center," z_x . For example, consider the following laminated configuration:



At the bottom surface ($i=1$, $z=z_0$, and $\tau_{xz}=0$), therefore:

$$C_1 = \frac{V}{(EI)_x} \left[z_x z_0 - \frac{z_0^2}{2} \right] E_{x1} \quad (A-89)$$

and for the first ply at the interface between plies $i=1$ and $i=2$ ($z=z_1$):

$$(\tau_{xz})_1 = \frac{V}{(EI)} \left[z_x(z_1 - z_0) - \frac{1}{2} (z_1^2 - z_0^2) \right] E_{x1} \quad (A-90a)$$

At this interface between plies $i=1$ and $i=2$:

$$(\tau_{xz})_2 = C_2 + \frac{V}{(EI)_x} \left[z_x z_1 - \frac{z_1^2}{2} \right] E_{x2} \quad (A-90b)$$

and since $(\tau_{xz})_2 = (\tau_{xz})_1$ at $z=z_1$:

$$C_2 = (\tau_{xz})_1 - \frac{V}{(EI)_x} \left[z_x z_1 - \frac{z_1^2}{2} \right] E_{x2} \quad (A-91)$$

Then, in the ply, $Z_1 < Z < Z_2$, the shear is:

$$\tau_{xz}(z) = (\tau_{xz})_1 + \frac{V E_x X_2^2}{(EI)_x} [\bar{z}_x(z-z_1) - \frac{1}{2}(z^2-z_1^2)] \quad (A-92)$$

In general, for any ply $z_{i-1} < z < z_i$, the shear is:

$$\tau_{xz}(z)_i = (\tau_{xz})_{i-1} + \frac{V E_x X_i^2}{(EI)_x} [\bar{z}_x(z-z_{i-1}) - \frac{1}{2}(z^2-z_{i-1}^2)] \quad (A-93)$$

At any ply interface, z_i , the shear is therefore:

$$(\tau_{xz})_i = \frac{V X_i}{(EI)_x} \sum_{j=1}^i E_{xj} T_j [\bar{z} - \frac{1}{2}(z_j + z_{j-1})] \quad (A-94)$$

where $T_j = z_j - z_{j-1}$.

Note that the shear at the top face, $(\tau_{xz})_n$, is zero and therefore:

$$(\tau_{xz})_n = \frac{V X_n}{(EI)_x} \left[\bar{z}_x \sum_{j=1}^n E_{xj} T_j - \sum_{j=1}^n E_{xj} T_j \frac{(z_j + z_{j-1})}{2} \right] = 0 \quad (A-95)$$

Equation A-95 proves that if Z_x is the bending center, the shear at the top surface must be zero.

A better form of Equation A-93, for this purpose, is:

$$[\tau_{xz}(z)]_i = \frac{V E_x X_i^2}{(EI)_x} \left[f_{x_i} + \bar{z}(z-z_{i-1}) - \frac{1}{2}(z^2-z_{i-1}^2) \right] \quad (A-96)$$

where

$$f_{x_i} = \frac{1}{E_{x_i}} \sum_{j=1}^{i-1} E_{x_j} T_j [\bar{z}_x - \frac{1}{2}(z_j + z_{j-1})] \quad (A-97)$$

Substituting Equation A-96 into Equation A-84 yields:

$$\frac{1}{G_x} = \frac{T}{(EI)_x^2} \sum_{i=1}^n \frac{1}{G_{x_i}} R_{x_i} \quad (A-98a)$$

where

$$R_{x_1} = (E_{x_1})^2 T_1 \left[(fx_1 + (\bar{z}_x - z_{i-1})T_1 - \frac{1}{3} T_1^2) fx_1 + (\frac{1}{3} (\bar{z}_x - 2z_{i-1}) - \frac{1}{4} T_1) \bar{z}_x T_1^3 + (\frac{1}{3} z_{i-1}^3 + \frac{1}{4} z_{i-1} T_1 + \frac{1}{20} T_1^3) T_1^3 \right] \quad (A-98b)$$

This expression for the inverse shear modulus for the x-direction is generalized to provide for the calculation of each term in the two-by-two matrix of shear moduli as:

$$[\bar{G}_{ki}] = \left[\frac{T}{(EI)_{kk}^2} \sum_{i=1}^n [G_{ki}]^{-1} R_{ki} \right]^{-1} \quad (A-99)$$

where

$$k = 1, 2$$

$$l = 1, 2$$

Note that if no shear is given, $[G^i]^{-1} = 0$, and also that, in Equation A-99:

$$(\bar{EI})_{11} = 1,1 \text{ term of } I \times [G_2]^*$$

$$(\bar{EI})_{22} = 2,2 \text{ term of } I \times [G_2]^*$$

where $[G_2]^*$ is calculated in the same manner as $[G_3]$ except that Poisson's ratio is set to zero. The moduli for individual plies are provided through user input. Because $G_{12} \neq G_{21}$, in general, an average value is used for the coupling terms.

$$[G_3] = \begin{bmatrix} G_{11} & (\bar{G}_{12})_{AVG} \\ (\bar{G}_{12})_{AVG} & G_{22} \end{bmatrix} \quad (A-100)$$

A.9.2 Element Layer Stress Recovery

The linear strain variation is given by:

$$(\epsilon_x) = (\epsilon_M) - z(K) \quad (A-101)$$

where

- (ϵ_x) - Layer strain vector in the element coordinate system.
- (ϵ_M) - Reference surface strain in the element coordinate system.
- (K) - Reference surface curvatures in the element coordinate system.
- Z - Distance of the mid-surface of the layer k from the laminate reference surface.

The individual layer stress vector in the fiber coordinate system is:

$$(\sigma_L) = [G_L] [T] (\epsilon_x) \quad (A-102)$$

where

- (σ_L) - Layer stress vector in the fiber coordinate system.
- $[G_L]$ - Stress-strain matrix in the fiber coordinate system.
- $[T]$ - Transformation matrix to transform strains from element coordinate system to fiber coordinate system.
- (ϵ_x) - Layer strain vector in the element coordinate system.

For element temperature and/or thermal gradients, the strain vector has to be corrected for thermal effects before applying Equation A-103:

$$(\epsilon_x) = (\epsilon'_x) - (\alpha) (T + zT') \quad (A-103)$$

and for thermal moments

$$(\epsilon_x) = (\epsilon_x) - (\epsilon_x)^T \quad (A-104)$$

where

- (ϵ'_x) - Mechanical strains.
- (α) - Thermal coefficients of expansion in the element coordinate system.
- T - Element temperature.
- T' - Element thermal gradient.
- z - Distance from the middle of the layer to the laminate reference surface.
- $(\epsilon_x)^T$ - Layer strains due to thermal moments in the element coordinate system.

The thermal strain vector due to applied thermal moments is determined by substituting for (M) in Equation A-73 and solving for the reference surface strains and curvatures, $\{\epsilon_M^T\}$ and $\{K^T\}$, respectively.

A.9.3 Interlaminar Shear Stresses

The interlaminar shear stress τ_{yz} , τ_{xz} can be computed at any ply interface from Equation A-96.

A.9.4 Force Resultants

Forces and moments for the element are computed using:

$$(F) = \sum_{i=1}^N (\sigma_x) T_i$$

$i=1, N$ (No. of layers) (A-105)

$$(M) = \sum_{i=1}^N -z_i T_i (\sigma_x)$$

where

(F) - In-plane force resultants.

(M) - Out-of-plane moments.

(σ_x) - Stresses in the element coordinate system.

T_i - Layer thickness.

z_i - Distance from the middle of the layer to the laminate reference surface.

A.9.5 Failure Indices

Failure indices assume a value of one on the periphery of a failure surface in stress space. If the failure index is less than one, the lamina stress is interior to the periphery of the failure surface and the lamina is assumed "safe" and if it is greater than one the lamina is assumed to have "failed." The failure indices represent a phenomenological failure criterion, because only the occurrence of failure is predicted.

The analytical definition of a failure surface in stress space for a lamina subjected to biaxial (planar) states of stress is provided via the following failure theories.

- (1) HILL
- (2) HOFFMAN
- (3) TSAI-WU
- (4) MAXIMUM STRESS
- (5) MAXIMUM STRAIN

In the analysis of laminated composites, which are typically orthotropic materials (possibly exhibiting unequal properties in tension and compression), the strength of orthotropic lamina is a function of body orientation relative to the imposed stress. In order to determine the structural integrity of the lamina, a set of intrinsic strength properties (allowable stresses or allowable strains) in the principal material directions are defined as:

- X_t - Ultimate uniaxial tensile strength in the fiber direction,
- X_c - Ultimate uniaxial compressive strength in the fiber direction,
- Y_t - Ultimate uniaxial tensile strength perpendicular to the fiber direction,
- Y_c - Ultimate uniaxial compressive strength perpendicular to the fiber direction,
- S - Ultimate planar shear strength under pure shear loading,
- E_t - Ultimate uniaxial tensile strain in the fiber direction,
- E_c - Ultimate uniaxial compressive strain in the fiber direction,
- F_t - Ultimate uniaxial tensile strain perpendicular to the fiber direction,
- F_c - Ultimate uniaxial compressive strain perpendicular to the fiber direction, and
- E_s - Ultimate planar shear strain under pure shear loading.

For most composite materials, the planar shear strengths and strains are equal for positive and negative shear loadings.

The five failure theories and a bonding failure index are now described:

HILL'S THEORY

$$\frac{\sigma_1^2}{X^2} + \frac{\sigma_2^2}{Y^2} - \frac{\sigma_1\sigma_2}{X^2} + \frac{r_{12}^2}{S^2} = \text{FAILURE INDEX (FI)} \quad (\text{A-106})$$

and $X-X_t$ if σ_1 is positive, and X_c if σ_1 is negative; similarly for y . For the interaction term, $(\sigma_1\sigma_2)/X^2$, $X-X_t$ if $\sigma_1\sigma_2$ is positive $X-X_c$ otherwise.

HOFFMAN'S THEORY

$$\left[\frac{1}{X_t} - \frac{1}{X_c} \right] \sigma_1 + \left[\frac{1}{Y_t} - \frac{1}{Y_c} \right] \sigma_2 + \frac{\sigma_1^2}{X_t X_c} + \frac{\sigma_2^2}{Y_t Y_c} + \frac{r_{12}^2}{S^2} + \frac{\sigma_1 \sigma_2}{X_t X_c} = \text{FI} \quad (\text{A-107})$$

Note that this theory takes into account the difference in the tensile and compressive allowable stresses by using linear terms in the failure equation.

TSAL-WU THEORY

This quadratic interaction theory allows for the strength predictions wherein interaction among stress components can be considered in determining strengths in a biaxial field. Thus, in the case of an orthotropic lamina in a general state of planar stress:

$$F_1 \sigma_1 + F_2 \sigma_2 + F_{11} \sigma_1^2 + F_{22} \sigma_2^2 + 2F_{12} \sigma_1 \sigma_2 + F_{66} r_{12}^2 = \text{FI} \quad (\text{A-108})$$

$$F_1 = \frac{1}{X_t} - \frac{1}{X_c}, \quad F_2 = \frac{1}{Y_t} - \frac{1}{Y_c}, \quad (\text{A-109})$$
$$F_{11} = \frac{1}{X_t X_c}, \quad F_{22} = \frac{1}{Y_t Y_c}, \quad F_{66} = \frac{1}{S^2}$$

and F_{12} needs to be determined experimentally, from a biaxial test. However, satisfactory results may be obtained by setting it to zero.

MAXIMUM STRESS

Failure is assumed to occur when any one of the stress components is equal to its corresponding intrinsic strength property. In mathematical form, the Maximum Stress theory is given by:

$$\begin{aligned}
\sigma_1 \geq X_T, \sigma_1 > 0 & ; \sigma_1 \leq -X_C, \sigma_1 < 0 \\
\sigma_2 \geq Y_T, \sigma_2 > 0 & ; \sigma_2 \leq -Y_C, \sigma_2 < 0 \\
\tau_{12} \geq S, \tau_{12} > 0 & ; \tau_{12} \leq -S, \tau_{12} < 0
\end{aligned}
\tag{A-110}$$

where the intrinsic strength properties are as defined previously.

MAXIMUM STRAIN

The Maximum Strain theory is analogous to the Maximum Stress theory. Failure is assumed to result when any one of the strain components is equal to its corresponding intrinsic ultimate strain. In mathematical form the Maximum Strain theory is given by:

$$\begin{aligned}
\epsilon_1 \geq E_T, \epsilon_1 > 0 & ; \epsilon_1 \leq E_C, \epsilon_1 < 0 \\
\epsilon_2 \geq F_T, \epsilon_2 > 0 & ; \epsilon_2 \leq F_C, \epsilon_2 < 0 \\
\gamma_{12} \geq E_S, \gamma_{12} > 0 & ; \gamma_{12} \leq E_S, \gamma_{12} < 0
\end{aligned}
\tag{A-111}$$

where the intrinsic ultimate strains are as defined previously.

FAILURE INDEX OF BONDING

The failure index of bonding material is calculated as the maximum interlaminar shear stress divided by the allowable bonding stress.

A.10 CORRECTION OF OUT-OF-PLANE SHEAR STRAIN

The typical formulation for a QUAD4 type finite element follows a standard bilinear isoparametric theory, with directional reduced integration for out-of-plane shear strain. However, this formulation has been found to be inadequate when the geometry of the element is irregular, and a correction defined herein has been implemented in ASTROS to correct this problem.

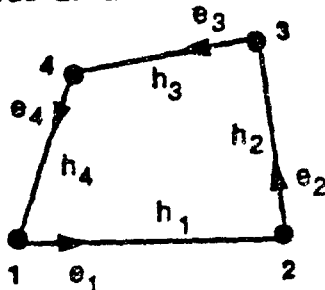
The modification is based upon the theory presented by Hughes and Tezdayar (Reference A-1), but is generalized to include non-planarity of the element, and special features to accommodate ASTROS's structure. The formulation enforces constant shear along each edge of the element, eliminating the need to perform reduced integration.

The formulation of this modification consists of establishing strain-displacement relationships in the element coordinate system. It involves six degrees of freedom (dof), the rotational part of which will be modified later to include the singularity about the normal to the mid-surface.

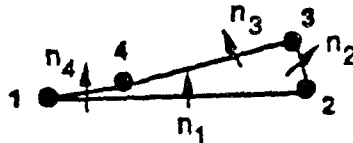
A.10.1 Geometric Variables

The following terms are defined for each edge of an irregular-shaped, non-planar element:

A Unit Normal Vector (\vec{e}) in the direction of the next node as illustrated;



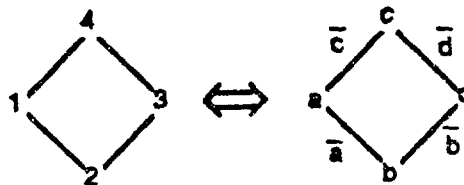
A Unit Normal Vector (\vec{n}) which is a normalized average of the nodal normals to the mid-surface along that edge;



Length of each edge (h_i); and cosine of the internal angle at each corner (α_i).

A.10.2 Edge Shears and Shear Vectors

Given the following numbering sequence:



At the middle of each edge, the constant shears parallel to edges \bar{a} , \bar{b} , \bar{c} and \bar{d} , respectively, are:

$$g_a = \frac{1}{h_a} \vec{n}_a \cdot (\vec{U}_b - \vec{U}_a) - \frac{1}{2} \vec{e}_a (\vec{\theta}_b + \vec{\theta}_a)$$

$$g_b = \frac{1}{h_b} \vec{n}_b \cdot (\vec{U}_d - \vec{U}_b) - \frac{1}{2} \vec{e}_b (\vec{\theta}_d + \vec{\theta}_b)$$

(A-112)

$$g_c = \frac{1}{h_c} \vec{n}_c \cdot (\vec{U}_a - \vec{U}_c) - \frac{1}{2} \vec{e}_c (\vec{\theta}_a + \vec{\theta}_c)$$

$$g_d = \frac{1}{h_d} \vec{n}_d \cdot (\vec{U}_c - \vec{U}_d) - \frac{1}{2} \vec{e}_d (\vec{\theta}_c + \vec{\theta}_d)$$

where \vec{U} and $\vec{\theta}$ are the vectors of translations and rotations at each node, respectively, in the element coordinate system.

The shear vector (\vec{v}_b) at node (b) is given by:

$$\vec{v}_b = \frac{1}{1-\alpha_b^2} (g_b + g_a \alpha_b) \vec{e}_b + \frac{1}{1-\alpha_b^2} (g_a + g_b \alpha_b) \vec{e}_a \quad (A-113)$$

or

$$\begin{aligned} \vec{v}_b = & \left[\frac{1}{(1-\alpha_b^2)h_a} (\vec{e}_a + \alpha_b \vec{e}_b) (\vec{n}_a \cdot \vec{U}_a) \right] + \left[\frac{1}{(1-\alpha_b^2)h_a} (\vec{e}_a + \alpha_b \vec{e}_b) (\vec{n}_a \cdot \vec{U}_b) \right] \\ & - \left[\frac{1}{(1-\alpha_b^2)h_b} (\vec{e}_b + \alpha_b \vec{e}_a) (\vec{n}_c \cdot \vec{U}_b) \right] + \left[\frac{1}{(1-\alpha_b^2)h_b} (\vec{e}_b + \alpha_b \vec{e}_a) (\vec{n}_b \cdot \vec{U}_d) \right] \\ & - \left[\frac{1}{2(1-\alpha_b^2)} (\vec{e}_a + \alpha_b \vec{e}_b) (\vec{e}_a \cdot \vec{\theta}_a) \right] \\ & - \left[\frac{1}{2(1-\alpha_b^2)} \left[(\vec{e}_a + \alpha_b \vec{e}_b) (\vec{e}_a \cdot \vec{\theta}_b) + (\vec{e}_b + \alpha_b \vec{e}_a) (\vec{e}_b \cdot \vec{\theta}_b) \right] \right] \\ & - \left[\frac{1}{2(1-\alpha_b^2)} (\vec{e}_b + \alpha_b \vec{e}_a) (\vec{e}_b \cdot \vec{\theta}_c) \right] \end{aligned} \quad (A-114)$$

and similarly for the other nodes, by permutations of the a, b, c and d subscripts.

A.10.3 Nodal Contributions of Shear Strain

The contribution of each node to the total shear strain ($\vec{\gamma}_T$) evaluated at an integration point is:

$$\vec{\gamma}_T = \sum_{i=1}^4 N_i \vec{\gamma}_i \quad (\text{A-115})$$

The "pseudo-contribution" of each edge to the total shear strain (G) has the following form:

$$\begin{aligned} \vec{G}_a &= \frac{N_a}{1-\alpha^2} (\vec{e}_a + \alpha_a \vec{e}_c) + \frac{N_b}{1-\alpha^2} (\vec{e}_a + \alpha_b \vec{e}_b) \\ \vec{G}_b &= \frac{N_b}{1-\alpha^2} (\vec{e}_b + \alpha_b \vec{e}_a) + \frac{N_d}{1-\alpha^2} (\vec{e}_b + \alpha_d \vec{e}_d) \\ \vec{G}_c &= \frac{N_c}{1-\alpha^2} (\vec{e}_c + \alpha_c \vec{e}_d) + \frac{N_a}{1-\alpha^2} (\vec{e}_c + \alpha_a \vec{e}_a) \\ \vec{G}_d &= \frac{N_d}{1-\alpha^2} (\vec{e}_d + \alpha_d \vec{e}_b) + \frac{N_c}{1-\alpha^2} (\vec{e}_d + \alpha_c \vec{e}_c) \end{aligned} \quad (\text{A-116})$$

Hence, the columns of the [B] matrix partition for shear, corresponding to node b, $[BS_{bj}]$, are:

$$\left. \begin{aligned} (BS_{b1}) &= \frac{n_a^i}{h_a} \vec{G}_a - \frac{n_b^i}{h_b} \vec{G}_b \\ (BS_{bj}) &= \frac{e_a^i}{2} \vec{G}_a - \frac{e_b^i}{2} \vec{G}_b \end{aligned} \right\} \begin{array}{l} i=1,2,3 \\ i=4,5,6 \end{array} \quad (\text{A-117})$$

A.10.4 Transformations

The following transformations have to be performed before the preceding formulation can replace the existing [B] matrix generation for out-of-plane shear.

$$[BS_b]_{(3 \times 6)} = [TIE]_{(3 \times 3)} \overline{[BS_b]}_{(3 \times 6)} \begin{bmatrix} [I] & | & 0 \\ \hline & & [TEE] \end{bmatrix}_{(6 \times 6)} \quad (A-118)$$

where

[TIE] - Is the orthogonal transformation between integration points and the element coordinate system, required since all the strains are calculated in the 1 system.

[I] - Is a 3x3 identity matrix.

[TEE] - Is the 3x3 transformation which takes into account the following facts

(A) Hughes' convention for rotations is different than the one implemented in STROS; and,

(B) The rotation about the normal to the mid-surface at each grid point is singular.

If NV is the normal vector at a given grid point, then.

$$[TEE] = \begin{bmatrix} 0 & -NV^3 & NV^2 \\ NV^3 & 0 & -NV^1 \\ -NV^1 & NV^1 & 0 \end{bmatrix} \quad (A-119)$$

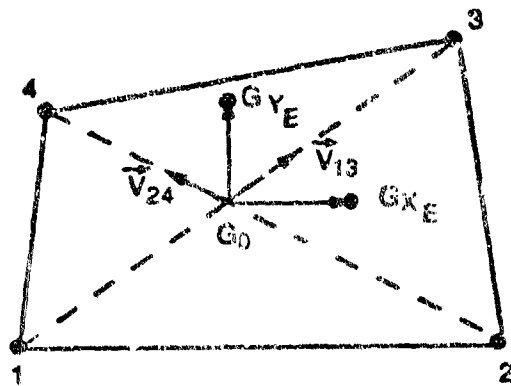


Figure A-1. Internal Element Coordinate System

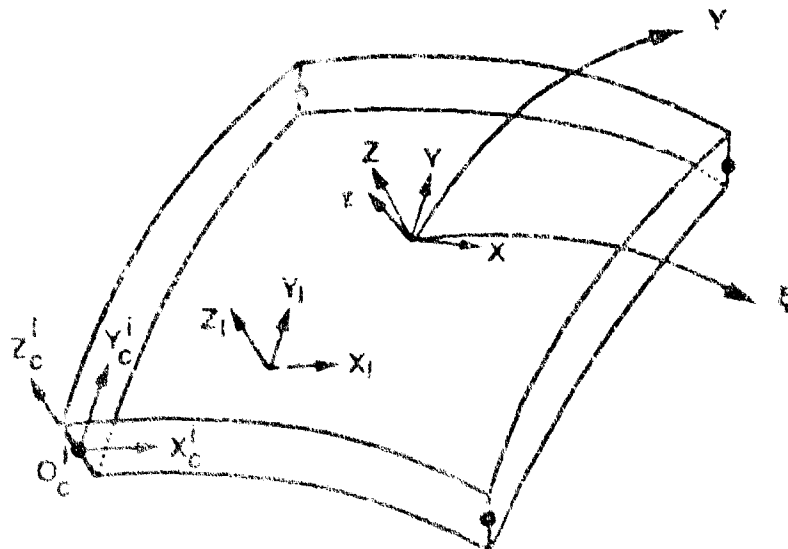


Figure A-2. Isoparametric Quadrilateral 4-Node Plate and Shell Element

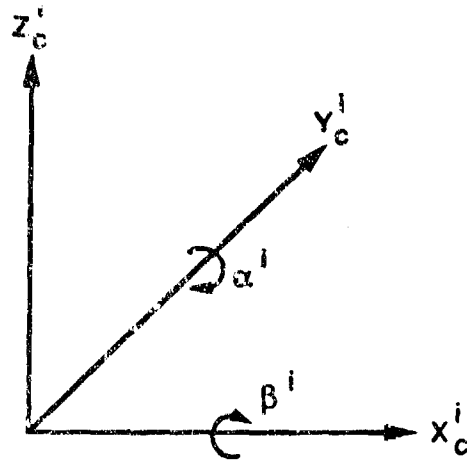


Figure A-3. Deformations at Grid Point i

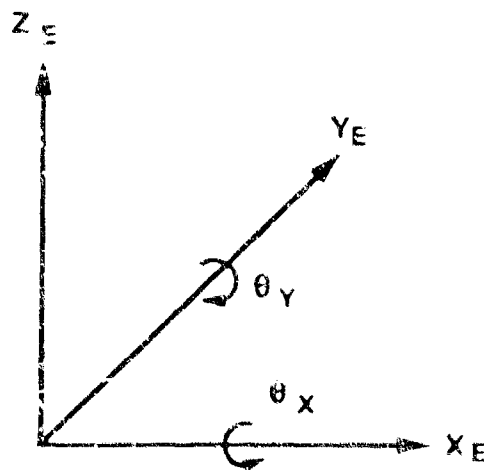


Figure A-4. Deformations in the Global Direction

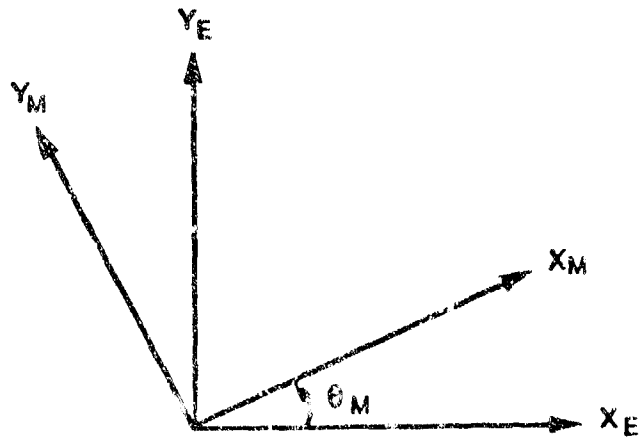


Figure A-5. Material and User Defined Element Axes

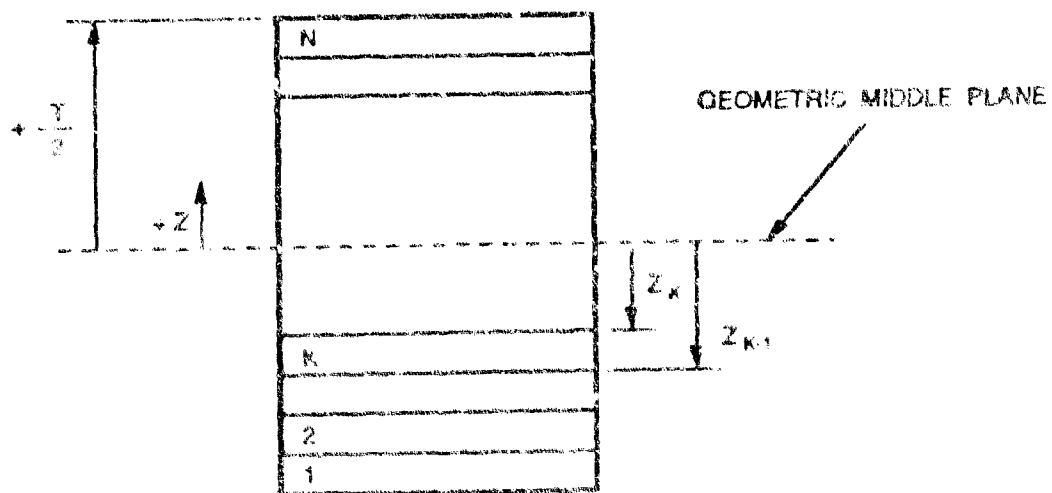


Figure A-6. Geometry of a N-Layered Element

REFERENCES

- A-1. Hughes, T. J. R. and Tezduyar, T. F., "Finite Elements Based Upon Mindlin Plate Theory with Particular Reference to the Four-Node Bilinear Isoparametric Element," Transactions of ASME, Journal of Applied Mechanics, Volume 48, No. 3, September, 1981, pp 587-596.

APPENDIX A: GUIDELINES FOR MODELING WITH QUAD4 (TRIAB) ELEMENTS

GUIDELINES FOR MODELING WITH QUAD4 (TRIA3) ELEMENTS

ASTROS (an Automated Structural Optimization System) and four versions of NASTRAN (COSMIC NASTRAN, UAI NASTRAN, CSA NASTRAN and MSC NASTRAN) provide QUAD4 and TRIA3 elements for the analysis of plates. They are basically flat plate elements, but they are often used for shell structures as well as approximations. These approximations become closer to reality with a finer mesh size. Much of the discussion in these guidelines is with reference to the QUAD4 element, but comments are equally applicable to the TRIA3 element. The original QUAD4 formulation in programs ASTROS, COSMIC NASTRAN and UAI NASTRAN is just about identical, and differences (if any) are the result of subsequent revisions. MSC NASTRAN and CSA NASTRAN are different, but nevertheless all five programs give comparable results for most well posed problems. Also the input requirements and the card structure are quite compatible in all five cases with minor differences in interpretation. The purpose of these seminar notes is to explain various QUAD4 modeling options and to point-out the differences in the five programs. An explanation of the input parameters on the various cards is considered the easiest way to accomplish this objective.

The QUAD4 is one of the most extensively used elements in NASTRAN as well as ASTROS. It is a very versatile element and can be used to model a variety of plate simulations such as (see Fig. 1).

- a. Membrane (inplane loading) behavior
- b. Bending (out of plane loading) behavior
- c. Membrane-bending (uncoupled)
- d. Membrane-bending (coupled-linear)
- e. Laminated plates
- f. Layered composites
- g. Sandwich plates with metal face sheets
- h. Sandwich plates with layered composite face sheets
- i. Isotropic materials
- j. Anisotropic (including orthotropic) materials

Application of the QUAD4 is often confusing because of the many options available for its use.

There are five cards which describe the input parameters for the QUAD4. They describe its geometry and properties along with some auxiliary information.

Geometry and Property Cards

CQUAD4 - Connection card
PSHELL - Property card for homogeneous and sandwich plates

PCOMP
or
PCOMP1* - Property cards for laminated or layered plates
or
PCOMP2*

*Not applicable to MSC NASTRAN

Material Cards

MAT1 - Isotropic materials
MAT2 - Anisotropic materials
MAT8 - Orthotropic materials
FLOAD4 - Pressure load definition on the QUAD4 element

The property cards PCOMP1 and PCOMP2 are not applicable to MSC NASTRAN. For a given element either the PSHELL or PCOMP card is applicable but not both. PSHELL cards are for homogeneous (nonlaminated) and sandwich plates with nonlayered face sheets. PCOMP cards are for laminated (layered) plates. In the case of sandwich plates with layered face sheets the honeycomb (sandwich) core will be treated as a laminate or layer.

A supplementary explanation of the parameters on each of these cards should aid in understanding the modeling nuances of the element.

Notes on the CQUAD4

The format of the CQUAD4 is the same for all the NASTRANS and ASTROS with the exception of an additional parameter "TMAX" (field 3 on the continuation card) in ASTROS. "TMAX" is the maximum allowable thickness of the plate, applicable only in optimization.

The definitions of the parameters in fields 2 to 7 are self explanatory and need no further clarification. Similarly no additional explanation is necessary for the thickness parameters specified in fields 4 to 7 on the continuation card. However, the parameters TM (THETA) and ZOFF need a supplementary explanation or caution.

Parameter TM (THETA)

The parameter TM defines the material property orientation. There are two options for this definition.

Option 1:

Define the angle between the side of the element (connecting G1 and G2) and the material axis. This is the least desirable option. It is prone to errors, because every time the sequence of the element connection changes, the angle must be changed. Also in a complex three dimensional model it is not easy to determine this angle without writing a preprocessor.

Option 2:

The integer option is preferable. An integer in field 8 refers to a separate coordinate system for defining the orientation of the material axis of the element. The material property definition is now independent of the connection sequence. The new coordinate system can be defined with a CORDZR card.

Offset Parameter ZO (ZOFFS, ZOFF)

The offset parameter provision in the QUAD4 element constitutes a significant enhancement for plate elements. Before the QUAD4 the grid points of the structure could only be defined on the mid-surface of the plate elements. The Bar (beam or bend) was the only other element with an offset capability. However, some of the mass elements have the offset capability.

The offset, ZO, is shown for various cases in Fig. 3. Note the distinction between the grid point surface and mid-surface of the element. The definition of the offset ZO on the CQUAD4 is the same in all four NASTRAN and ASTROS. The offset implementation in COSMIC NASTRAN and ASTROS appears to give more consistent results than MSC and CSA NASTRAN. See Reference A1 for results on the benchmarking of offsets.

Notes on PSHELL

PSHELL or PCOMP are the property cards referenced on the CQUAD4 (in field 3). The PSHELL is to be used when the plate is not laminated (or layered), while the PCOMP is for laminated plates. Only one of these is applicable for a given element. A preprocessor in NASTRAN (ASTROS) generates equivalent PSHELL cards from the PCOMP cards before proceeding to the solution. NASTRAN (ASTROS) provides versatility to the QUAD4 element through the PSHELL card. It allows the modeling of membrane (inplane), bending, shear and membrane-bending coupling behavior. Fig. 2 illustrates key features of the elements described on the PSHELL card. It is a sandwich plate with two face sheets separated by a honeycomb core.

The first two fields of the PSHELL card are for the name and property identification called from the CQUAD4. The third field, MID1, is the material identification number for the face sheets in membrane behavior. The parameter T is the total thickness of the two face sheets. MID2 is the material identification number for bending behavior, MID3 for shear and MID4 for membrane-bending coupling. There are two types of membrane bending coupling. The coupling resulting from asymmetry in plate construction (non-symmetric laminates) is called linear coupling. Nonlinear coupling, on the other hand, is a result of the interaction of internal forces such as inplane and out of plane (beam-column effect) forces. The latter coupling can be accounted for only in differential stiffness and/or buckling analysis. The parameter $12I/T^3$ (field 6) can be calculated by using the following definition for I.

$$I=2\left[\frac{1}{12}\left(\frac{T}{2}\right)^3+\frac{T}{2}\left(\frac{TS}{2}+\frac{T}{4}\right)^2\right]$$

I is basically the moment of inertia of the face sheets about the neutral axis (centroidal). It is assumed that the face sheets are symmetric about the neutral axis. If they are not, the moment of inertia about the neutral axis can be calculated. For solid plates this parameter is simply 1.0

The definition of the parameter TS/T is obvious from the figure.

No further explanation is necessary for the parameters in the next three fields, NSM, Z1 and Z2.

The parameters MCSID and SCSID (not available in MSC and CSA NASTRAN) refer to the material coordinate system. As stated in the description of the card CQUAD4 there are two options for this definition. By leaving the field blank or a real value the first option is invoked. In this option the parameter represents the angle between the side of the element connecting the grid points G1 and G2 and the material axis. The second option is an integer which refers to a coordinate system defined on a COORD-card. The second option is the most desirable because the grid point sequence on the CQUAD4 card does not affect the material axis.

The offset parameter Z0 is the same as defined on the CQUAD4 (See Fig. 3). An important point to note is that there is no provision for the offset definition on the PSHELL card in MSC and CSA NASTRAN. There is some advantage in having this option on the PSHELL, because when the number of PSHELL cards is significantly fewer than the number of CQUAD4 cards, this parameter need not be repeated on all the CQUAD4 cards. The entry on CQUAD4, however, overrides that on the PSHELL card.

The PSHELL card provides the facility to model homogeneous as well as sandwich plates. However, the face sheets of the sandwich plates are assumed to be homogeneous (isotropic, orthotropic or anisotropic) plates. A discussion of sandwich plates with face sheets made of layered composites is deferred until the PCOMP cards description. The minimum thickness parameter TMIN is applicable only in ASTROS optimization.

Before leaving this discussion, it is worth pointing out some anomalies (idiosyncrasies) (as they exist at present, Feb 1993) in the application of the PSHELL card in various versions of NASTRAN (ASTROS).

Specifying MID1 and leaving the remaining three material identifications blank simulates the membrane behavior in all five programs with little or no differences to point out.

Specifying MID2 only invokes bending behavior only (as it should be) in MSC NASTRAN. It does not compute shear deformation. CSA NASTRAN on the other hand includes shear deformation, ostensibly with the material properties invoked by MID2. ASTROS and COSMIC NASTRAN go even further by including membrane, bending and shear deformations even though only MID2 is specified. Membrane and bending behavior are computed by the material properties called from MID2. This is not so bad, because these two behaviors are generally uncoupled. The most important point to note is that ASTROS (COSMIC NASTRAN) computes the shear deformation by assuming a material infinitely stiff in transverse shear when the MID3 field is blank. These results are, in general, not acceptable. The easiest way to avoid shear stiffness overestimation at present is not to leave MID3 blank when MID2 is specified. See results of the examples at the end of this Appendix. Future versions of ASTROS and COSMIC NASTRAN will uncouple these computations.

Notes on PCOMP, PCOMP1, PCOMP2

The purpose of PCOMP, PCOMP1 and PCOMP2 is to define element property parameters in modeling laminated plates including layered (fiber reinforced) composites. All three cards serve the same purpose except the options are different. If the layers are made of different materials and the thicknesses of the layers are all different, then the PCOMP card is appropriate. If all the layers are made of the same material and thickness, then PCOMP1 is appropriate. If the material is the same, but the thicknesses are different, then PCOMP2 is appropriate. The first two fields on the PCOMP cards need no further explanation.

Parameter Z0

The parameter Z0 refers to the distance from the grid point surface to the bottom of the plate. The plate bottom surface is defined in Fig. 4. It is the reference surface from which the stacking sequence of the laminates is defined. The parameters ZOFF defined on the (QUAD4 and PSHELL) are a source of confusion sometimes. However, figures 3 and 4 should provide better clarification.

Parameter SBOND

The bonding material shear stress is indirectly related to the interlaminar shear and its value is generally empirical. A value of 400 to 500 psi for SBOND appears to be reasonable in the absence of a value obtained from experiments. Any approximation of this parameter will not affect the analysis results. It affects only the failure theory which is basically a postprocessing function.

Parameters FT (Field 6)

This parameter simply identifies the desired failure theory. The five failure theories are discussed in detail in the theoretical section of the QUAD4 seminar notes. The possible values of FT are:

- "HILL" for the Hill Theory
- "HOFF" for the Hoffman Theory
- "TSAI" for the Tsai-Wu Theory
- "STRESS" for the Maximum Stress Theory
- "STRAIN" for the Maximum Strain Theory

Parameter LOPT

This parameter description needs no further explanation than what is given in the PCOMP card descriptions. It should be pointed out, however, that no parameters are in fields 7 and 8 in COSMIC NASTRAN, while MSC and CSA NASTRAN define "TREF," a reference temperature for thermal stress analysis and "GE" the material damping coefficient for dynamic analysis.

Parameters MID_i, T_i, TH_i, and SOUT_i

These parameters pertain to the *i*th layer. MID₁ is the material identification number of the first layer. The layer count goes up from the bottom surface of the plate. For the definition of the bottom surface see Fig. 4 for the definition of Z₀. The MID_{*i*} refers to one of three material cards: MAT1 for isotropic materials, MAT2 for anisotropic materials and MAT8 for orthotropic materials. The parameter T_{*i*} defines the thickness of the first layer and TH_{*i*} refers to the orientation of the material axis with reference to the material axis defined on the CQUAD4. SOUT_{*i*} is the stress output parameter. Then the parameters are repeated for all the layers unless the symmetry option is used under the parameter LOPT. If any MID_{*i*}, T_{*i*} or TH_{*i*} are blank, then the last non-blank values specified for each will be used.

Material Cards

Isotropic & Anisotropic

The parameters on MAT1 and MAT2 are self explanatory from the card descriptions.

Orthotropic Material MAT8

Most of the parameters on MAT8 are self explanatory with the exceptions of G1Z and G2Z (fields 7 and 8). When these parameters are left blank, MSC NASTRAN and CSA NASTRAN do not calculate the transverse shear deformation in plate bending problems. ASTROS and COSMIC NASTRAN, on the other hand, assume that the material is infinitely stiff in transverse shear, and thus overestimate the stiffness of the element. To avoid such overestimation and to obtain comparable results to the other NASTRANS, transverse shear values have to be provided. Values of about two or three orders of magnitude less than the modulus of elasticity of the material are recommended.

These guidelines and the accompanying examples should clarify most of the questions arising in the use of the QUAD4 and TRIA3 elements.

REFERENCE

1. Pitrof, Stephen M. and Venkayya, Vipperla B., Benchmarking the QUAD4/TRIA3 Element, Twenty-first NASTLAN User's Colloquium, Tampa FL, April 1993.

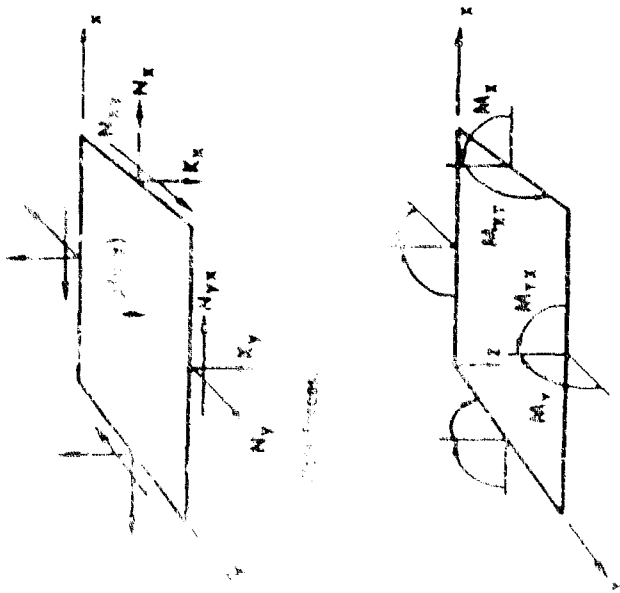
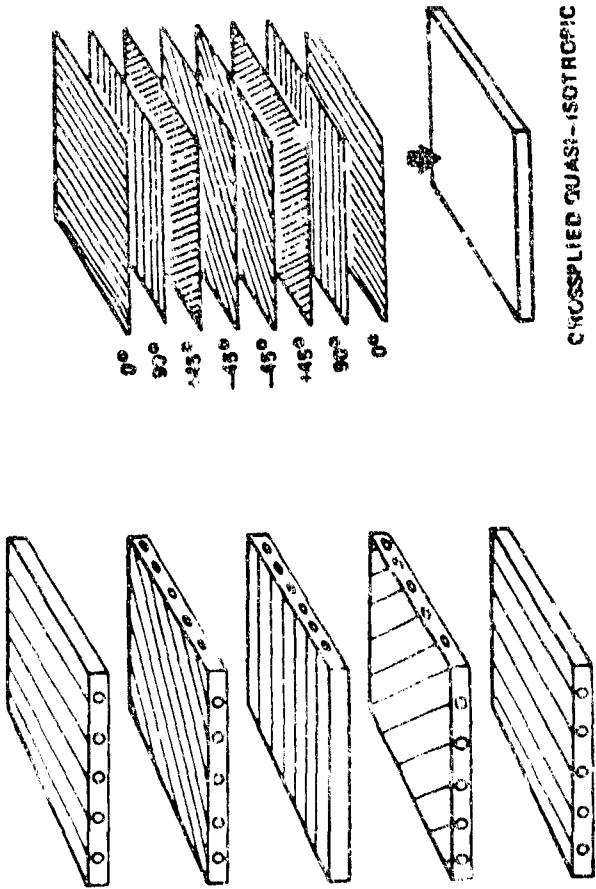


Plate moments.



CROSSPLYED QUASI-ISOTROPIC

Laminate construction.

Figure 1

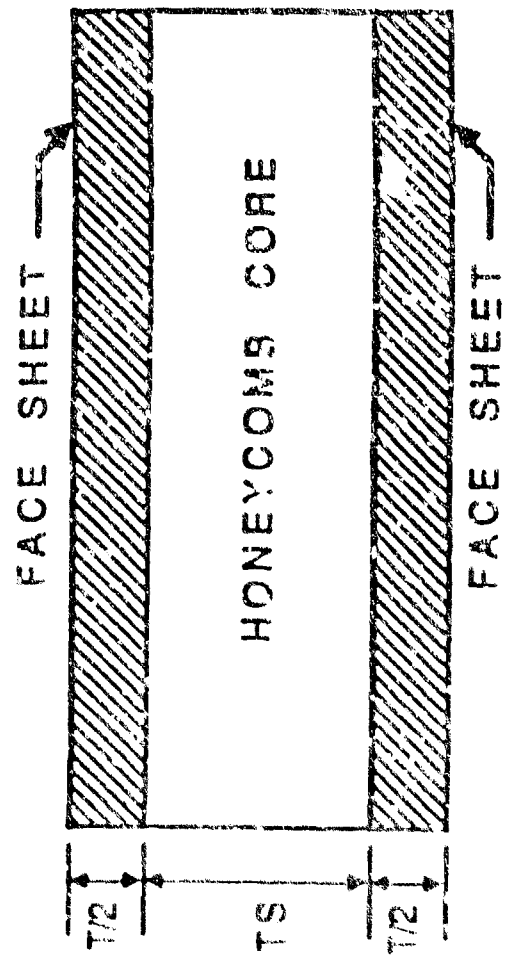


Figure 2

QUAD4 - OFFSET DEFINITION

(CQUAD4 PSHELL)

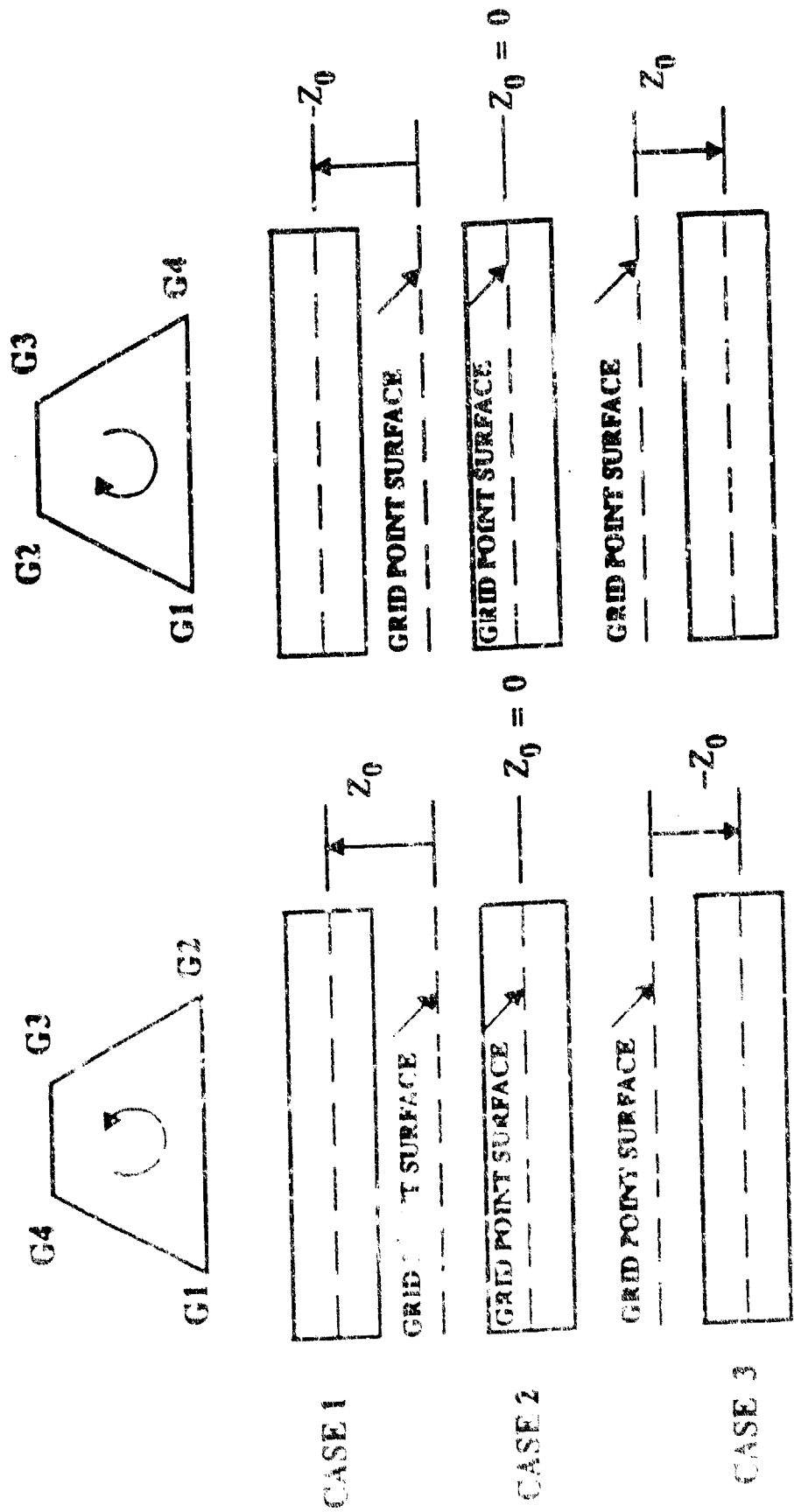


Figure 3

QUAD4 - OFFSET DEFINITION

(PCOMP PCOMPI PCOMP2)

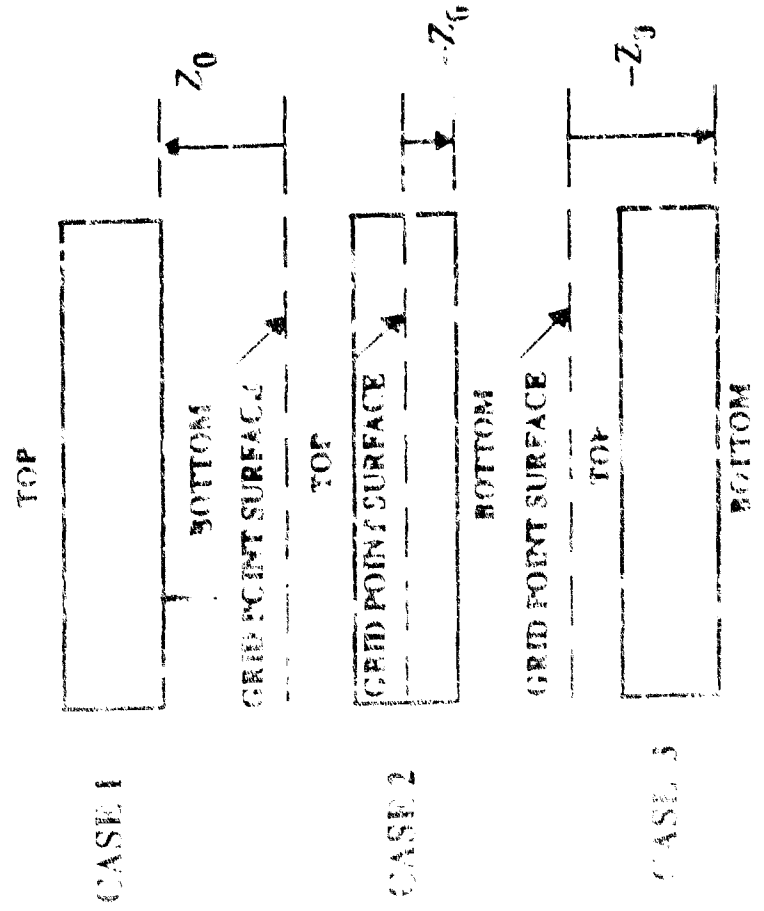
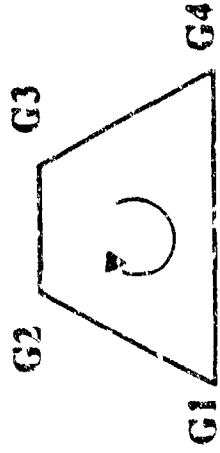
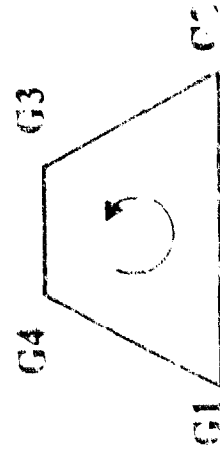


Figure 4

PROBLEM #5

RECTANGULAR PLATE

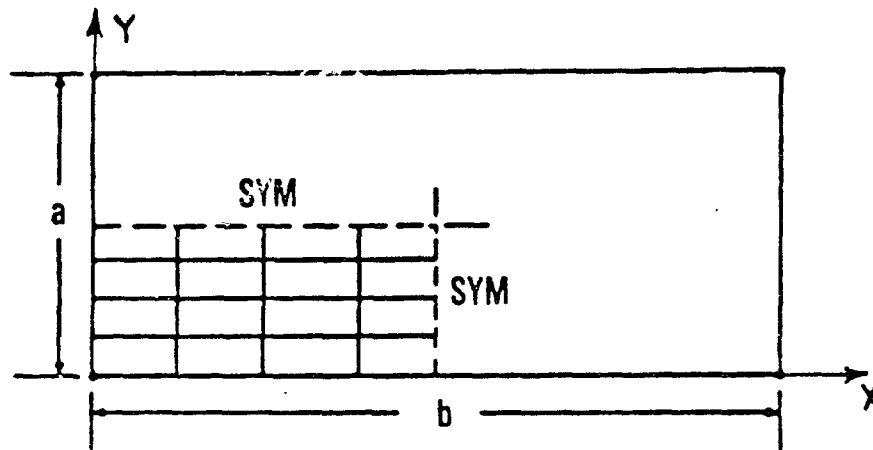


FIGURE 5

A finite element model of 1/4 of a rectangular plate is shown in Fig. 5. The length of the plate is 2 or 10, the width is 2, i.e. Aspect Ratios (AR) 1.0 and 5.0, and the thickness is .001. The material properties are given as $E=1.7472 \times 10^6$ and $\nu=0.3$. The plate is subjected to two loading conditions and boundary conditions for each aspect ratio as follows:

CASE 1: Clamped Supports. Concentrated load of $P = -4.0 \times 10^{-4}$ at the center of the plate.

CASE 2: Simple Supports. Uniform pressure load of $q=10^{-4}$ over the plate.

Convergence is studied by varying the mesh size. Input data is given for a 6x6 mesh. Theoretical results for the lateral displacement at the center of the plate are given in Table 2.

CASE 1		CASE 2	
AR = 1.0	AR = 5.0	AR = 1.0	AR = 5.0
5.60	7.23	4.062	12.97

TABLE 2

Example A4: The plate is modeled as a sandwich plate with composite face sheets and a honeycomb core. The thickness of the core is 2in. Two cases are considered. In Case 1 the MATS entries G1Z and G2Z are not specified. In Case 2 G1Z and G2Z are given the value 4.0×10^6 psi.

Z DISPLACEMENT ($\times 10^{-3}$) AT THE CENTER OF THE PLATE

NASTRAN	MID2 ONLY	MID1, MID2 AND MID3 SPECIFIED
COSMIC	-7.324	-7.687
MSC	-7.643	-7.683
CSA	-7.630	-7.677
ASTROS	-7.322	-7.680

EXAMPLE A1: QUAD4 - MID SPECIFICATIONS
Metal Homogeneous Aluminum

Z DISPLACEMENT AT THE CENTER OF THE PLATE

NASTRAN	FACE SHEETS AND CORE HAVE THE SAME MATERIAL PROP	CORE MATERIAL $E=2.5 \times 10^4, G=1.0 \times 10^4$	CORE MATERIAL $G=1.0 \times 10^4$
COSMIC	-0.00194	-0.0150	-0.0150
MSC	-0.00193	-0.0150	-0.0150
CSA	-0.00192	-0.0150	-0.0150
ASTROS	-0.00193	-0.0150	-0.0150

EXAMPLE A2: QUAD4 SANDWICH - HONEYCOMB PROPERTIES
Sandwich Plate Isotropic Face Sheets

Z DISPLACEMENT ($\times 10^{-2}$) AT THE CENTER OF THE PLATE

NASTRAN	NO G1Z, G2Z	G1Z = G2Z = 4.0×10^5
COSMIC	-.803	-1.071
MSC	-.838	-1.074
CSA	-.945	-1.029
ASTROS	-.831	-1.071

EXAMPLE A3: QUAD4 - SHEAR - G1Z, G2Z SPECIFIED
Composite Solid Plate

Z DISPLACEMENT ($\times 10^{-3}$) AT THE CENTER OF THE PLATE

NASTRAN	NO G1Z, G2Z	G1Z = G2Z = 4.0×10^5
COSMIC	-.199	-2.691
MSC	-6.464	-6.526
CSA	-5.067	-5.109
ASTROS	-.173	-6.531

EXAMPLE A4: QUAD4 - SANDWICH - COMPOSITE FACE SHEETS
Composite Sandwich Plate with Honeycomb Core

APPENDIX B: SAMPLE PROBLEMS

QUAD4 SEMINAR

APPENDIX B

SAMPLE PROBLEMS

<u>PROBLEM NUMBER</u>	<u>DESCRIPTION</u>	<u>PAGE NUMBER</u>
1*	PATCH TEST FOR PLATES	83
2	STRAIGHT CANTILEVER BEAM	85
3	CURVED BEAM	86
4	TWISTED BEAM	87
5	RECTANGULAR PLATE	88
6	SCORDELIS-LO ROOF	89
7	SPHERICAL SHELL	90
8*	COMPOSITE PLATE - PURE TWIST LOADING	91
9	COMPOSITE PLATE - UNIFORM PRESSURE LOADING	92
10	OPEN COMPOSITE TUBE	93
11	STRAIGHT TENSILE TEST - SIMULATION OF EQUIVALENT ISOTROPIC PROPERTIES. LAMINATE CONFIGURATION 0/0/0/0.	94
12	COMPOSITE SHELL ROOF MODEL	96
13	COMPOSITE RECTANGULAR PLATE	97
14	COMPOSITE SANDWICH PLATE	99

* Problems 1 through 7 reference isotropic material and are defined in the MSC/NASTRAN APPLICATION MANUAL, SECTION 5.

+ Problems 8 through 12 are provided by UAI/NASTRAN.

B.1 PROBLEM DESCRIPTIONS

PROBLEM #1

PATCH TEST

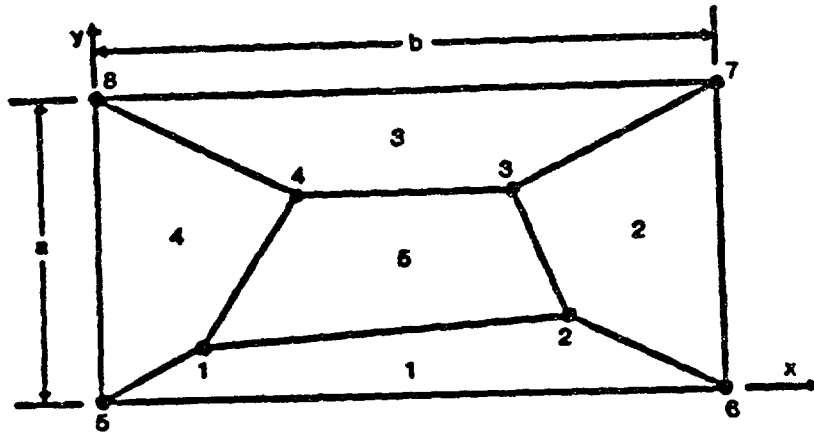


FIGURE 1

A model of a patch test for plates is shown in Fig. 1. The length of the plate is .24, the width is .12, and the thickness is .001. The location of the inner nodes is given in Table 1. The material properties are given as $E=1.0 \times 10^6$ and $\nu=0.25$. Displacement boundary conditions are specified for the following cases:

CASE 1. Membrane Plate

$$u = 10^{-3}(x+y/2)$$

$$v = 10^{-3}(y+x/2)$$

CASE 2. Bending Plate

$$w = 10^{-3}(x^2+xy+y^2)/2$$

$$\theta_x = \frac{\partial w}{\partial y} = 10^{-3}(y+x/2)$$

$$\theta_y = -\frac{\partial w}{\partial x} = 10^{-3}(-x-y/2)$$

The theoretical solution for Case 1 is given by

$$\epsilon_x = \epsilon_y = \gamma = 10^{-3}; \sigma_x = \sigma_y = 1333.; \tau_{xy} = 400.$$

The theoretical solution for Case 2 is given by

Bending moments per unit length:

$$m_x = m_y = 1.111 \times 10^{-7}, m_{xy} = 10^{-7}$$

Surface stresses:

$$\sigma_x = \sigma_y = \pm .667; \tau_{xy} = \pm .200$$

LOCATION OF INNER NODES		
NODE	X	Y
1	.04	.02
2	.18	.03
3	.16	.08
4	.08	.08

TABLE 1

PROBLEM #2

STRAIGHT CANTILEVERED BEAM

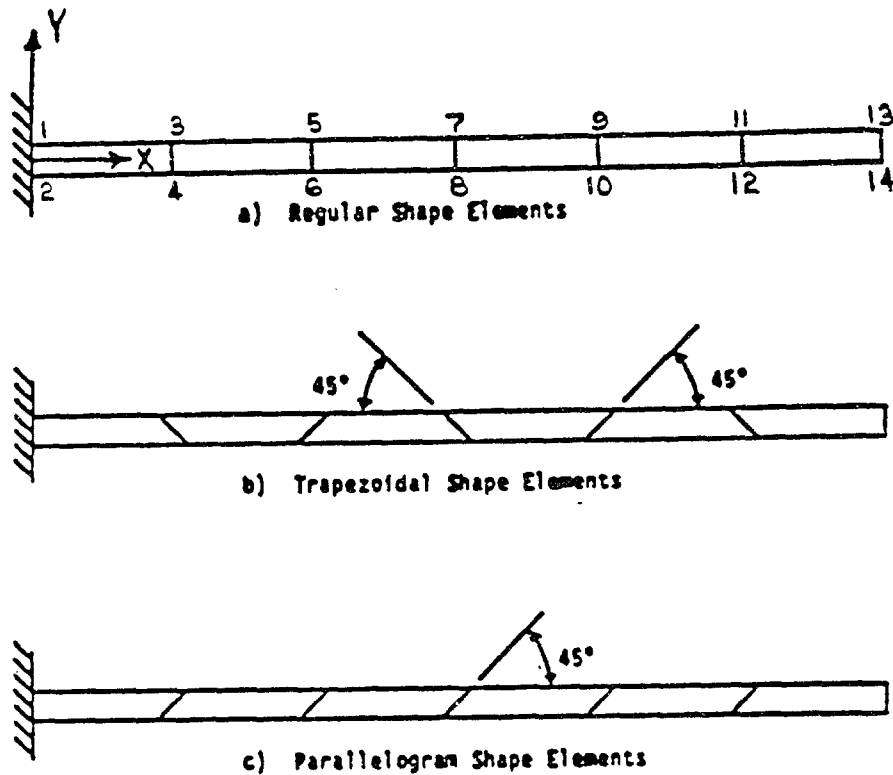


FIGURE 2

A finite element model of a straight cantilevered beam with rectangular, trapezoidal and parallelogram shape elements is shown in Fig. 2 (plane view). The length of the beam is 6, the width is 0.2 and the depth is 0.1. The beam has material properties $E=1.0 \times 10^7$ and $\nu=.30$. All the elements have equal volume. The beam is subjected to 4 loading conditions as follows:

CASE 1: A unit force is applied at the tip in the X direction. (Extension)

CASE 2: A unit force is applied at the tip in the Y direction. (In-plane shear)

CASE 3: A unit force is applied at the tip in the Z direction. (Out-of-plane shear)

CASE 4: A unit force is applied at the tip in the $+Z$ direction. (Twist)

The theoretical solution for the displacement at the tip in the direction of the load for Case 1 is 3.0×10^{-5} , for Case 2 0.1081, for Case 3 0.4321 and for Case 4 $.03208 \times 10^{-2}$.

PROBLEM #3

CURVED BEAM

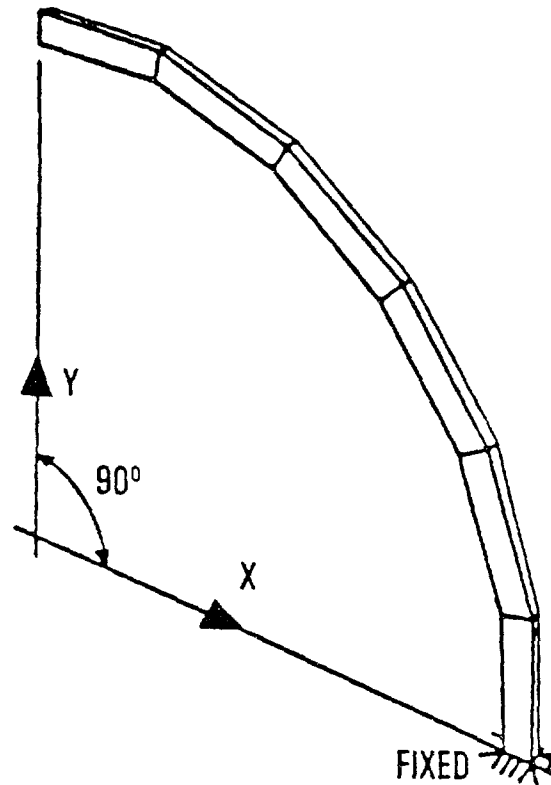


FIGURE 3

A finite element model of a curved cantilevered beam is shown in Fig. 3. The inner radius is 4.12, the outer radius is 4.32, the arc is 90° , and the thickness is 0.1. The beam has material properties $E=1.0 \times 10^7$ and $\nu=0.25$. The beam is subjected to two loading conditions as follows:

CASE 1: A unit force is applied at the tip in the +Y direction.
(In-plane shear)

CASE 2: A unit force is applied at the tip in the +Z direction.
(Out-of-plane shear)

The theoretical solution for the deflection at the tip in the direction of the load for Case 1 is .08734 and for Case 2 is .5022.

PROBLEM #4

TWISTED BEAM

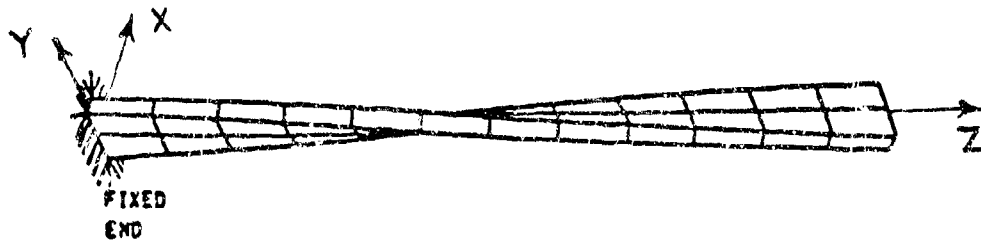


FIGURE 4

A finite element model of a twisted beam is shown in Fig. 4. The length of the beam is 12, the width is 1.1 and the depth is .32. The beam is twisted 90° from root to tip. The material properties are given as $E=29.0 \times 10^6$ and $\nu=0.22$, and the beam is subjected to two loading conditions as follows:

CASE 1: A unit force is applied at the tip in the +X direction.
(In-plane shear)

CASE 2: A unit force is applied at the tip in the -Y direction.
(Out-of-plane shear)

The theoretical solution for the deflection at the tip in the direction of the load for Case 1 is .005424 and for Case 2 is .001754.

PROBLEM #5

RECTANGULAR PLATE

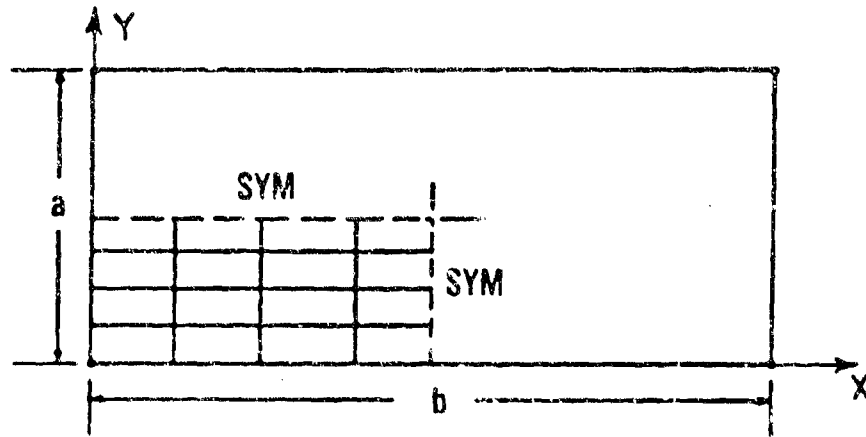


FIGURE 5

A finite element model of 1/4 of a rectangular plate is shown in Fig. 5. The length of the plate is 2 or 10, the width is 2, i.e. Aspect Ratios (AR) 1.0 and 5.0, and the thickness is .001. The material properties are given as $E=1.7472 \times 10^{10}$ and $\nu=0.3$. The plate is subjected to two loading conditions and boundary conditions for each aspect ratio as follows:

CASE 1: Clamped Supports. Concentrated load of $P = -4.0 \times 10^{-4}$ at the center of the plate.

CASE 2: Simple Supports. Uniform pressure load of $q=10^{-4}$ over the plate.

Convergence is studied by varying the mesh size. Input data is given for a 6x6 mesh. Theoretical results for the lateral displacement at the center of the plate are given in Table 2.

CASE 1		CASE 2	
AR = 1.0	AR = 5.0	AR = 1.0	AR = 5.0
5.60	7.23	4.062	12.97

TABLE 2

PROBLEM 6

SCORDELIS - LO ROOF

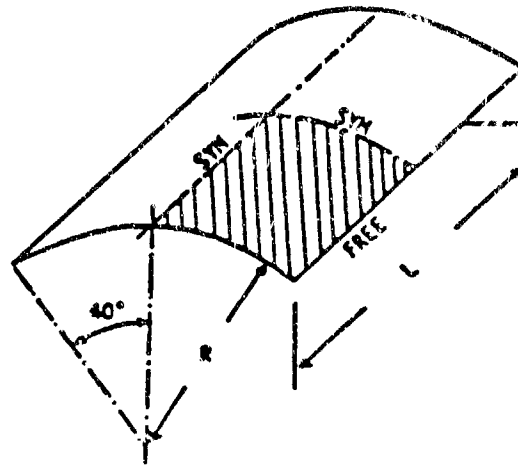


FIGURE 6

A model of a Scordelis-Lo Roof i.e. a singly curved shell, is shown in Fig. 6. The roof has a radius of 25', a length of 50', and a thickness of 0.25'. The material properties are given as $E=4.32 \times 10^8$ lbs/ft² and $\nu=0.0$. The roof is loaded by its own weight, and the weight of the roof is 90 lbs/ft². Only 1/4 of the roof is modeled. Symmetry boundary conditions are imposed on the interior edges and $U_x=U_y=0$ on the curved edges. Convergence is studied by varying the mesh size. Input data is given for an 8x8 mesh. The theoretical solution for the midside vertical displacement is .3086'. A value of .3024' was used for normalization of the results.

Reference: Zienkiewicz, O.C., The Finite Element Method, Third Edition, McGraw-Hill Book Co, (UK) Limited, pp 349-351.

PROBLEM #7

SPHERICAL SHELL

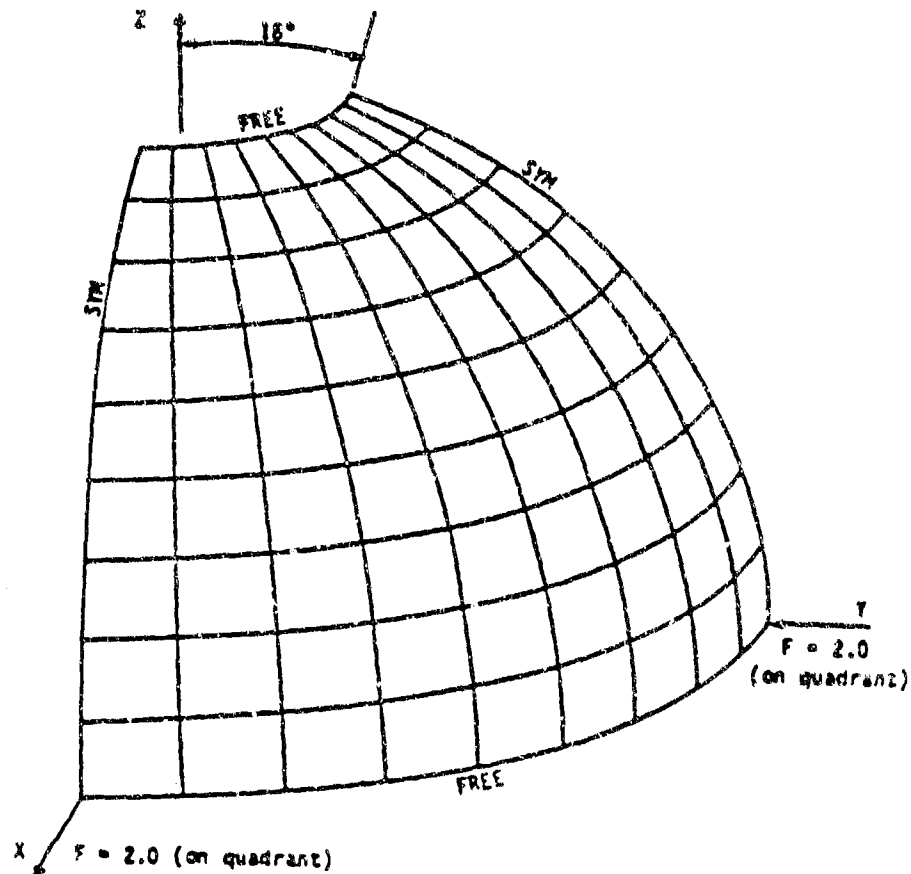


FIGURE 7

A model of a spherical shell, i.e. a doubly-curved shell, is shown in Fig. 7. The shell has a radius of 10 and a thickness of 0.4. The material properties are given as $E=6.825 \times 10^7$ and $\nu=0.3$. The shell is subjected to concentrated forces as shown in Fig. 7. Only 1/4 of the shell is modeled and symmetry boundary conditions are imposed on two sides. In addition the * node is constrained in the Z direction. Convergence is studied by varying the mesh size. Input and output are given for an 8x8 mesh. A theoretical lower bound for the radial deflection at the center in the case where the hole at the center is not present is .0924. A value of .0940 was used for normalization of the results.

Reference: Morley, L.S.D., and A.J. Morris, Conflict Between Finite Elements and Shell Theory, Royal Aircraft Establishment Report, London, 1978.

PROBLEM #8

REGULAR SYMMETRIC CROSS-PLY LAMINATE

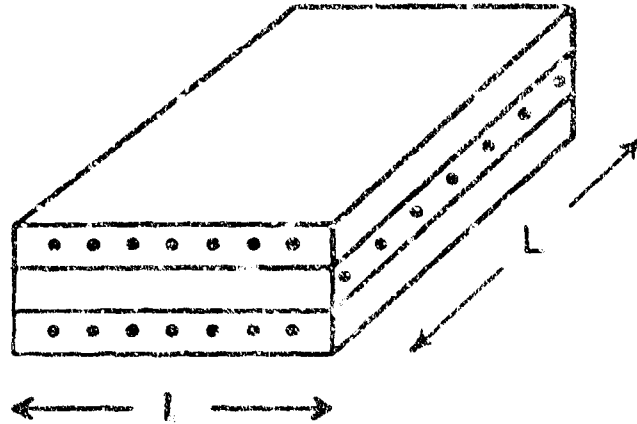


FIGURE 8

A composite square plate of length 5 modeled as a symmetric cross-ply laminate $[0, 90, 0]$ with four elements is shown in Fig. 8. The thickness of each ply is .02222. The material properties are given as $E_1=2.0 \times 10^7$, $E_2=5.0 \times 10^5$, $\nu_{12}=0.25$ and $G_{12}=2.5 \times 10^5$ and are the same for each ply. Three corners of the plate are pinned and a unit force in the $-Z$ direction is applied at the free corner to simulate a pure twist loading. The theoretical solution for the Z deflection at the free corner is -3.750×10^{-3} . Theoretical results are also given for τ for the element containing the free corner in Table 3.

LAYER	τ
1	-50.0
2	0.0
3	50.0

TABLE 3

PROBLEM #9

REGULAR SYMMETRIC CROSS-PLY LAMINATE

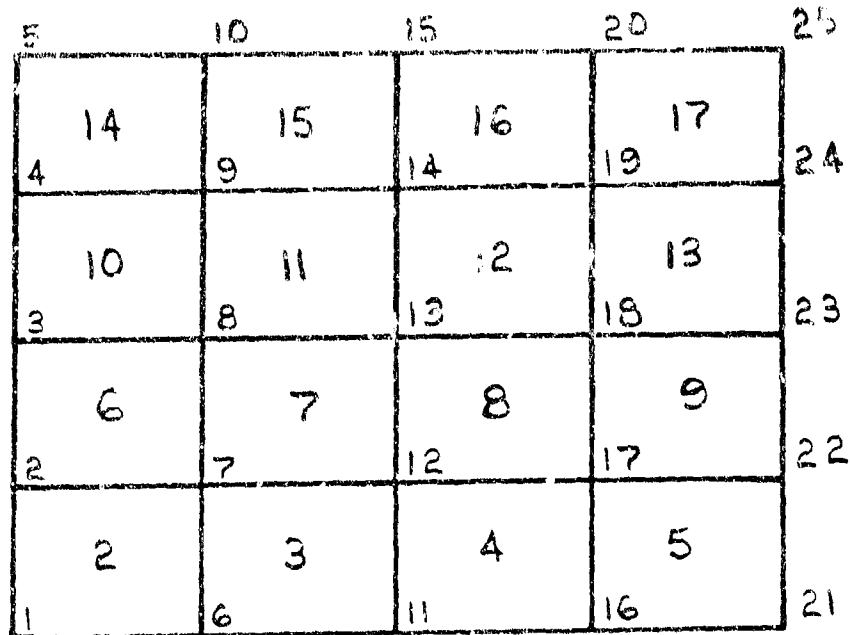


FIGURE 9

A quarter model of a composite square plate modeled as a symmetric cross-ply laminate is shown in Fig. 9. The length of each side is 1.0, and the thickness of each ply is .000666. The material properties are given as $E_1=2.0 \times 10^7$, $E_2=5.0 \times 10^5$, $\nu_{12}=0.25$ and $G_{12}=2.5 \times 10^5$ and are the same for each ply. The full plate is simply supported and is subjected to a uniform pressure load of -1.0×10^{-4} . The finite element model contains 25 nodes and 16 elements. The theoretical solution for the Z deflection at Grid 25 (center of the plate) is -1.836×10^{-3} . Theoretical results are also given for the stresses in layers 1 and 3 in element 17 in Table 4.

σ_1	σ_2	τ_{12}
59.6	1.8	-.06

TABLE 4

PROBLEM #10

COMPOSITE OPEN TUBE

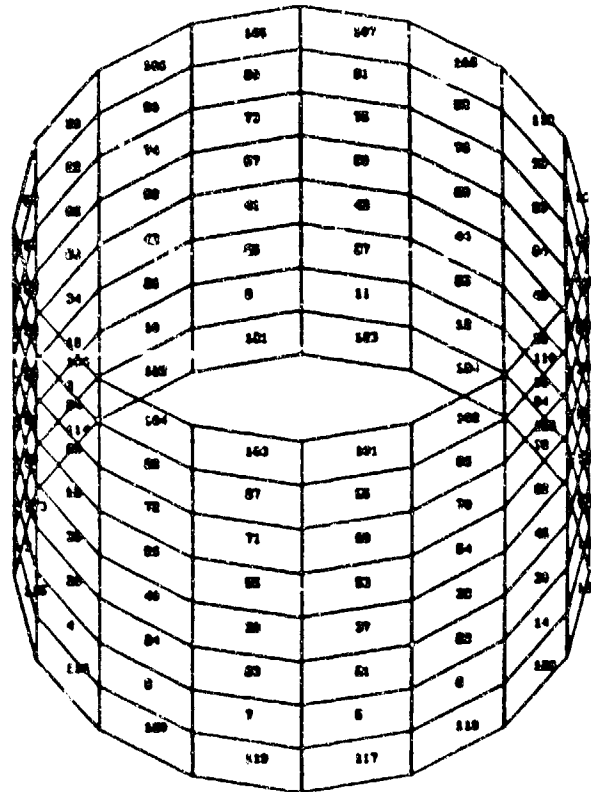


FIGURE 10

A finite element model of a composite open tube modeled with the symmetric lay-up [45, -45, 0, 90, 90, 0, -45, 45] is shown in Fig. 10. The model contains 144 nodes and 128 elements. The length of the tube is 80, the radius is 50, and the thickness of each layer is .24. The material properties are given as $E_1=73.8 \times 10^3$, $E_2=3.75 \times 10^3$, $\nu_{12}=0.4$, and $G_{12}=1.74 \times 10^3$. The tube is subjected to a uniform pressure load of 10.5. The theoretical solution for the hoop loading, F_Y , for element 8 is 525. Results from MSC/NASTRAN are given in Table 5 for the layer stresses in element 8.

LAYER	σ_1	σ_2	τ_{12}
1	2.519E2	1.740E1	2.273E1
2	2.494E2	1.748E1	-2.273E1
3	-2.257E2	3.288E1	1.108E-2
4	7.268E2	2.651E0	2.221E-2
5	7.269E2	2.657E0	5.551E-2
6	-2.255E2	3.231E1	-8.881E-2
7	2.535E2	1.741E1	-2.274E1
8	2.478E2	1.760E1	2.274E1

TABLE 5

PROBLEM #11

STRAIGHT BEAM TEST

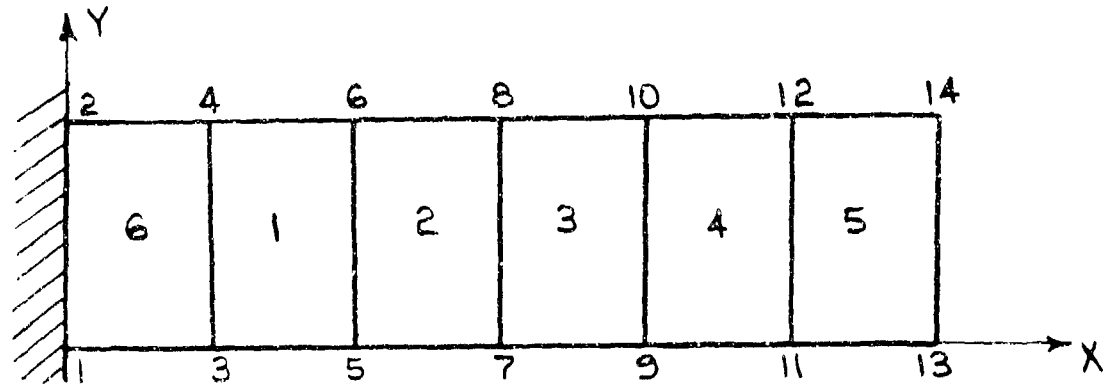


FIGURE 11

A finite element model of a cantilevered beam modeled as a laminate configuration [0, 0, 0, 0] in a simulation of equivalent isotropic properties is shown in Fig. 11. The length of the beam is 6, and the thickness of each layer is .25. The material properties are given as $E=10.0 \times 10^6$ and $\nu=.3$. The beam is subjected to two loading conditions as follows:

CASE 1: A unit force is applied at the tip in the X direction. (Extension)

CASE 2: A unit force is applied at the tip in the Z direction.

For Case 1 the theoretical solution for the X deflection at Grids 13 and 14 is 3.0×10^{-5} . For Case 2 the theoretical solution for the Z deflection at Grids 13 and 14 is .4320. Theoretical results are given for the bending moment distribution from the free end to the cantilevered end in Table 6.

ELEMENT NO.	M_x
5	2.50
4	7.50
3	12.50
2	17.50
1	22.50
6	27.50

TABLE 6

For Case 2 results from MSC/NASTRAN are given in Table 7 for the direct layer bending stress in element 6.

LAYER NO.	σ_1
1	1.238E4
2	4.126E3
3	-4.126E3
4	-1.238E4

TABLE 7

PROBLEM #12

COMPOSITE SHELL ROOF

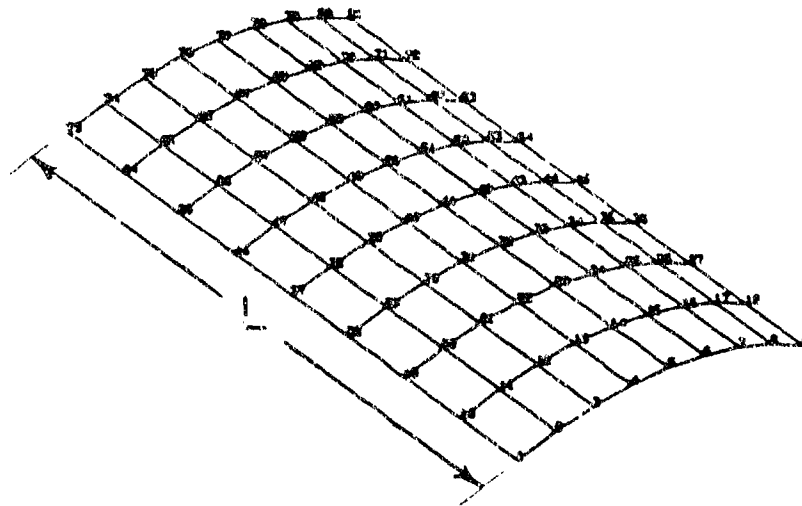


FIGURE 12

A finite element model of a composite shell roof modeled with the symmetric lay-up [45, -45, 15, -15, -15, 15, -45, 45] is shown in Fig. 12. The length and radius of the shell are 25, and the thickness of each layer is .03125. The material properties are given as $E_1=2.0 \times 10^8$, $E_2=0.5 \times 10^7$, $\nu_{12}=0.25$, $G_{12}=2.5 \times 10^6$, and $G_{17}=G_{27}=2.5 \times 10^6$. The shell is subjected to a uniform pressure of 90.0. Results from MSC/NASTRAN are given in Table 8 for the radial deflection at selected nodes.

GRID	T1
34	-1.0662
35	-1.3441
36	-1.6074
43	-1.3267
44	-1.6739
45	-2.0079

TABLE 8

PROBLEM #13

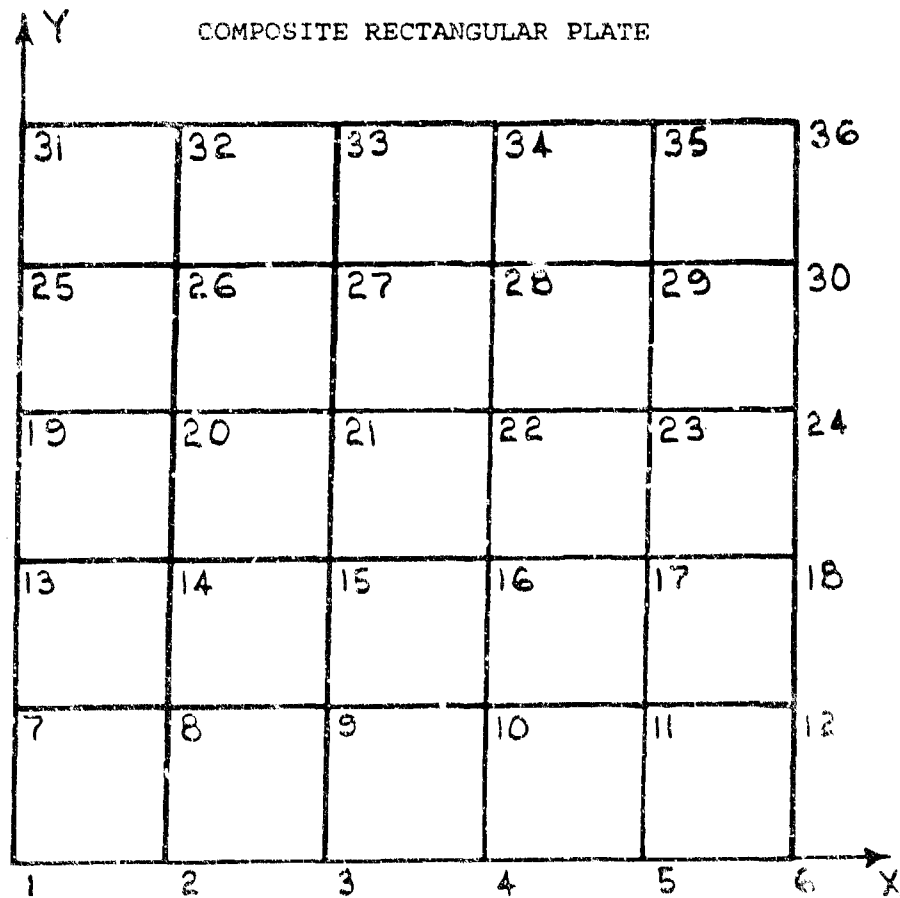


FIGURE 13

A composite rectangular plate modeled with the symmetric lay-up $[45, -45, 90, 0, 0, 90, -45, 45]$ is shown in Fig. 13. The model contains 36 nodes and 25 elements. The length of the plate is 20, the width is 15, and the thickness of each layer is .0236. The material properties are given as $E_1=18.5 \times 10^6$, $E_2=1.6 \times 10^6$, $\nu_{12}=0.25$, $G_{12}=.65 \times 10^8$ and $G_{13}=G_{23}=4.0 \times 10^8$. Four variations of the plate are considered.

CASE 1: The plate is modeled as a membrane subjected to a compression load in the $-X$ direction at the $X=15.0$ edge.

CASE 2: The plate is modeled with bending properties and subjected to a uniform pressure load of 2 in the $-Z$ direction.

CASE 3: The plate is modeled in membrane and bending and is subject to the loads defined in Case 1 and Case 2.

CASES 1 through 3 are defined for a static analysis, RF #1.

CASE 4: Normal modes analysis using RF #3. The first ten natural frequencies are found.

PROBLEM #14

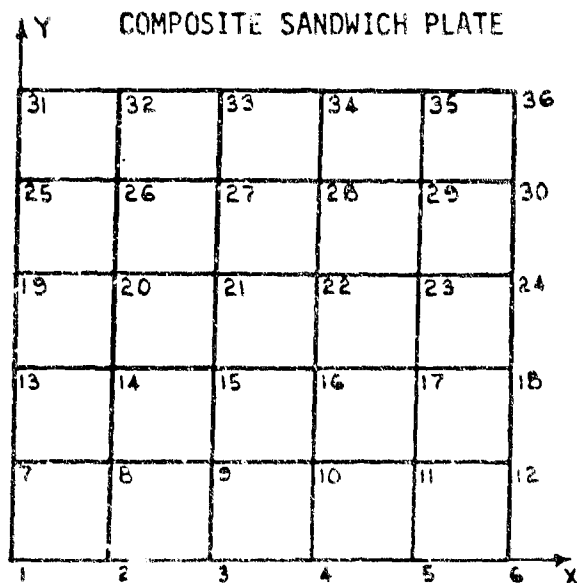
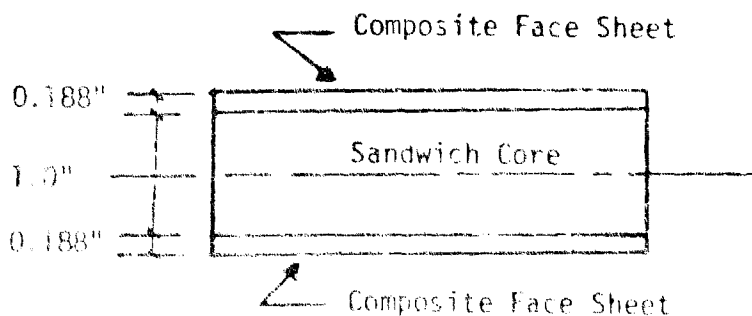


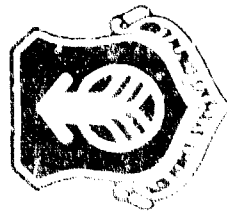
FIGURE 14

A sandwich plate with an isotropic core and composite skins modeled with the symmetric lay-up [45, -45, 90, 0, 0, 90, -45, 45] is shown in Fig 14. The model contains 35 nodes and 25 elements. The length of the plate is 20, the width is 15, the thickness of each layer is .0236, and the thickness of the core is 1. The material properties of the composite skin are given as $E_1=18.5 \times 10^6$, $E_2=1.6 \times 10^6$, $\nu=0.25$ and $G_{12}=.65 \times 10^6$, $G_{13}=G_{23}=4.0 \times 10^5$. The material properties of the core are given as $G=3.0 \times 10^4$. The plate is subjected to a uniform pressure load of 2.

ELEMENT DEFINITION

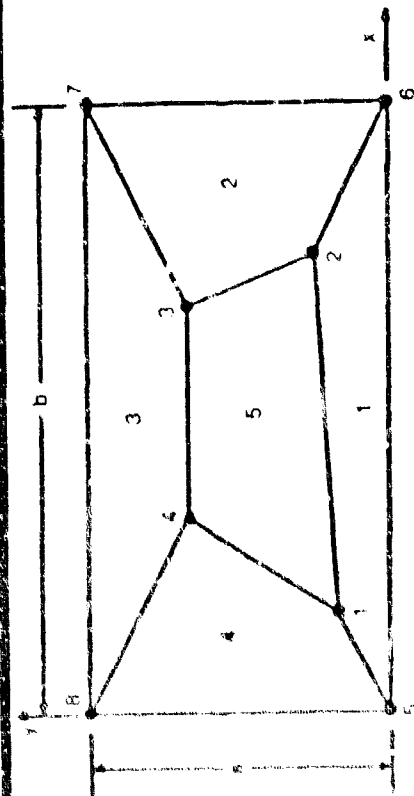


B.2 RESULTS AND COMPARISONS



PATCH TEST

PROBLEM #1



$a = .12$ $b = .24$ $t = .001$
 $E = 1.0E+6$
 $\nu = 0.25$

MEMBRANE PLATE

$$B.C. \begin{cases} u = 10^{-3}(x + y/2) \\ v = 10^{-3}(y + x/2) \end{cases}$$

LOADING

CONSTANT STRESS
CONSTANT CURVATURE

BENDING PLATE

$$B.C. \begin{cases} W = 10^{-3}(x^2 + xy + y^2)/2 \\ \theta_x = 10^{-3}(y + x/2) \\ \theta_y = 10^{-3}(-x - y/2) \end{cases}$$

MAXIMUM ERROR IN σ_x

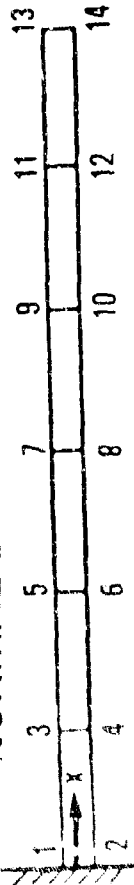
QUAD2	COSMIC	QUAD4	MSC	THEORY
0.0	0.0	0.0	0.0	1333.
33.1%	0.0	0.0	0.0	$\pm .667$



STRAIGHT CANTILEVER BEAM

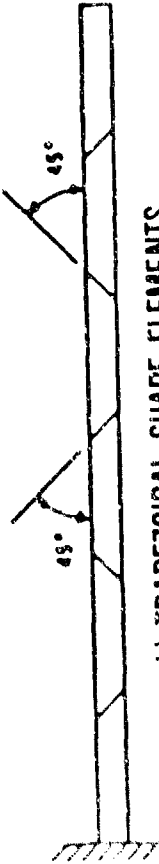
PROBLEM #2

NORMALIZED TIP DEFLECTION IN THE DIRECTION OF THE LOAD

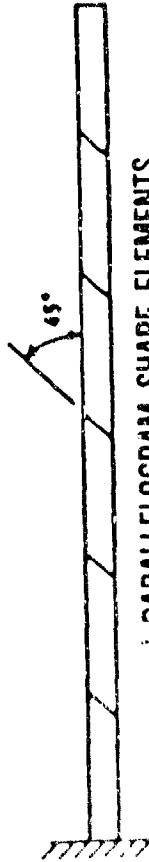


$L=6.0$ $W=0.2$ $D=0.1$
 $E=1.0E7$ $\nu = .30$
 UNIT FORCES AT THE TIP

a) RECTANGULAR SHAPE ELEMENTS



b) TRAPEZOIDAL SHAPE ELEMENTS



c) PARALLELOGRAM SHAPE ELEMENTS

RECTANGULAR

QUAD2	COSMIC	QUAD4	MSC
.992	.995	.995	.995
.032	.904	.904	.904
.971	.985	.985	.986
.566	.959	.959	.902

EXTENSION
 IN PLANE SHEAR
 OUT-OF-PLANE SHEAR
 TWIST

TRAPEZOIDAL

QUAD2	COSMIC	QUAD4	MSC
.992	.996	.996	.996
.017	.070	.071	.071
.965	.981	.981	.968
.631	.916	.916	.913

EXTENSION
 IN PLANE SHEAR
 OUT-OF-PLANE SHEAR
 TWIST

PARALLELOGRAM

QUAD2	COSMIC	QUAD4	MSC
.992	.991	.996	.996
.013	.078	.078	.078
.960	.987	.976	.976
.598	.881	.904	.904

EXTENSION
 IN-PLANE SHEAR
 OUT-OF-PLANE SHEAR
 TWIST

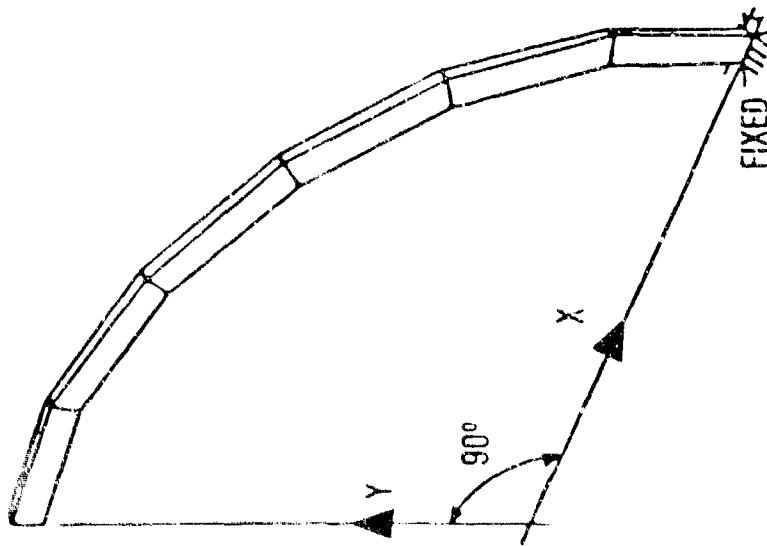


CURVED BEAM

PROBLEM #3

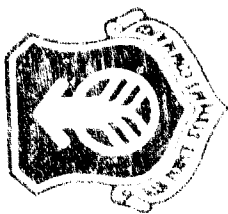
NORMALIZED TIP DISPLACEMENT IN THE DIRECTION OF THE LOAD

INNER RADIUS = 4.12
 OUTER RADIUS = 4.32
 THICKNESS = .1
 $E = 1.0E7$
 $\nu = .25$



IN-PLANE (VERTICAL)
 OUT-OF-PLANE

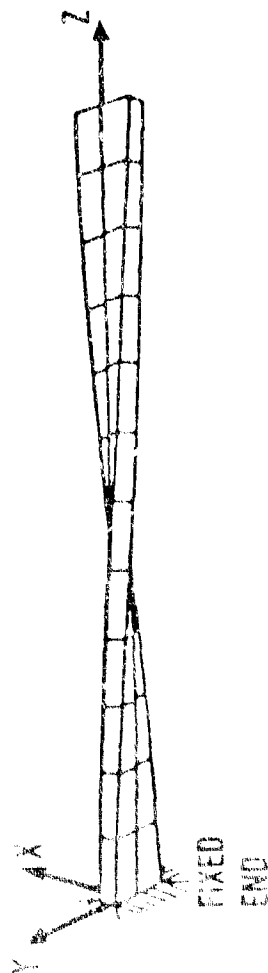
QUAD2	QUAD4	COSMIC	MISC
.625	.833	.833	.833
.596	.950	.950	.923



TWISTED BEAM

PROBLEM #A

NORMALIZED TIP DEFLECTION IN THE DIRECTION OF THE LOAD



$L = 12.0$ $W = 1.1$ $D = .32$
 $TWIST = 90^\circ$ (ROOT TO TIP)
 $E = 29.0E6$
 $\nu = 0.22$

UNIT FORCES AT THE TIP

QUAD2	QUAD4	MSC
98.46	COSMIC	.993
226.4		.985

IN-PLANE

OUT-OF-PLANE



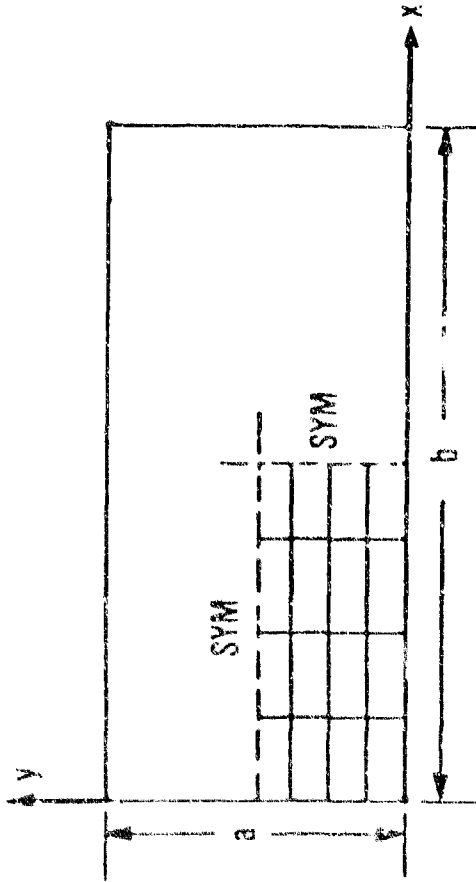
RECTANGULAR PLATE CLAMPED SUPPORTS

PROBLEM #5

NORMALIZED LATERAL DEFLECT

THE RESULTS

$a = 2.0$ $b = 2.0$ OR 10.0
 THICKNESS = 0.001
 $E = 1.7472E7$
 $\nu = 0.3$
 CENTRAL LOAD = $4.0E4$



ASPECT RATIO = 1.0

MESH	QUAD2	QUAD4 COSMIC	QUAD4 MSC
2x2	.983	.997	.938
4x4	1.012	1.034	1.014
6x6	1.010	1.026	1.016
8x8	1.009	1.019	1.014

ASPECT RATIO = 5.0

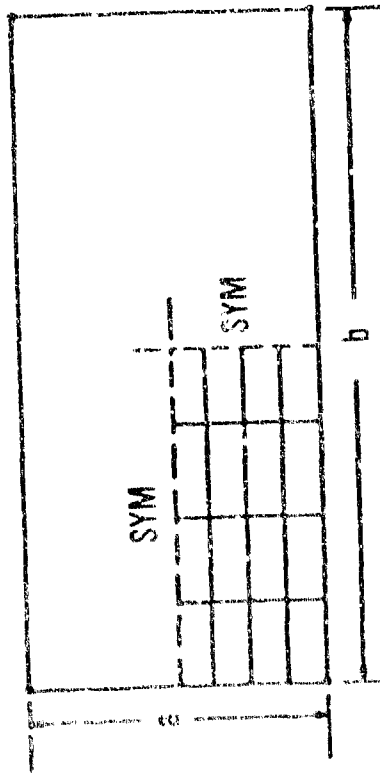
MESH	QUAD2	QUAD4 COSMIC	QUAD4 MSC
2x2	.333	.773	.519
4x4	.513	1.028	.863
6x6	.639	1.058	.940
8x8	.725	1.057	.972



RECTANGULAR PLATE SIMPLE SUPPORTS

PROBLEM #5

NORMALIZED LATERAL DEFLECTION AT THE CENTER



$a = 2.0$ $b = 2.0$ OR 10.0
 THICKNESS = 0.001
 $E = 1.7472E7$
 $\nu = 0.3$
 UNIFORM PRESSURE $q = 1.0E-4$

ASPECT RATIO = 1.0

MESH	QUAD4		QUAD4	
	QUAD2	COSMIC	COSMIC	MSC
2x2	.375	1.030	1.030	.981
4x4	.998	1.013	1.013	1.004
6x6	1.001	1.009	1.009	1.003
8x8	1.002	1.006	1.006	1.002

ASPECT RATIO = 5.0

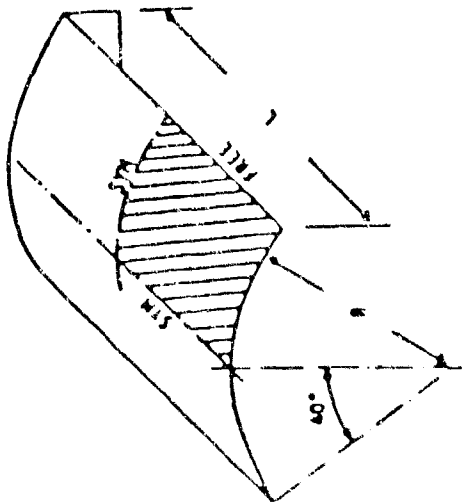
MESH	QUAD2		QUAD4	
	QUAD2	COSMIC	COSMIC	MSC
2x2	1.005	.913	.913	.999
4x4	1.014	.991	.991	.995
6x6	1.007	.996	.996	1.000
8x6	1.005	.998	.998	1.001



SCORDELIS - LO - ROOF

PROBLEM #6

NORMALIZED VERTICAL DEFLECTION AT THE MIDPOINT OF THE FREE EDGE



RADIUS = 25.0 FT
 LENGTH = 50.0 FT
 THICKNESS = 0.25 FT
 $E = 4.32E8$ LBS / FT²
 $\nu = 0.0$

LOADED BY ITS OWN WEIGHT - 90 LBS / FT²
 $U_x = U_z = 0$ ON CURVED EDGES

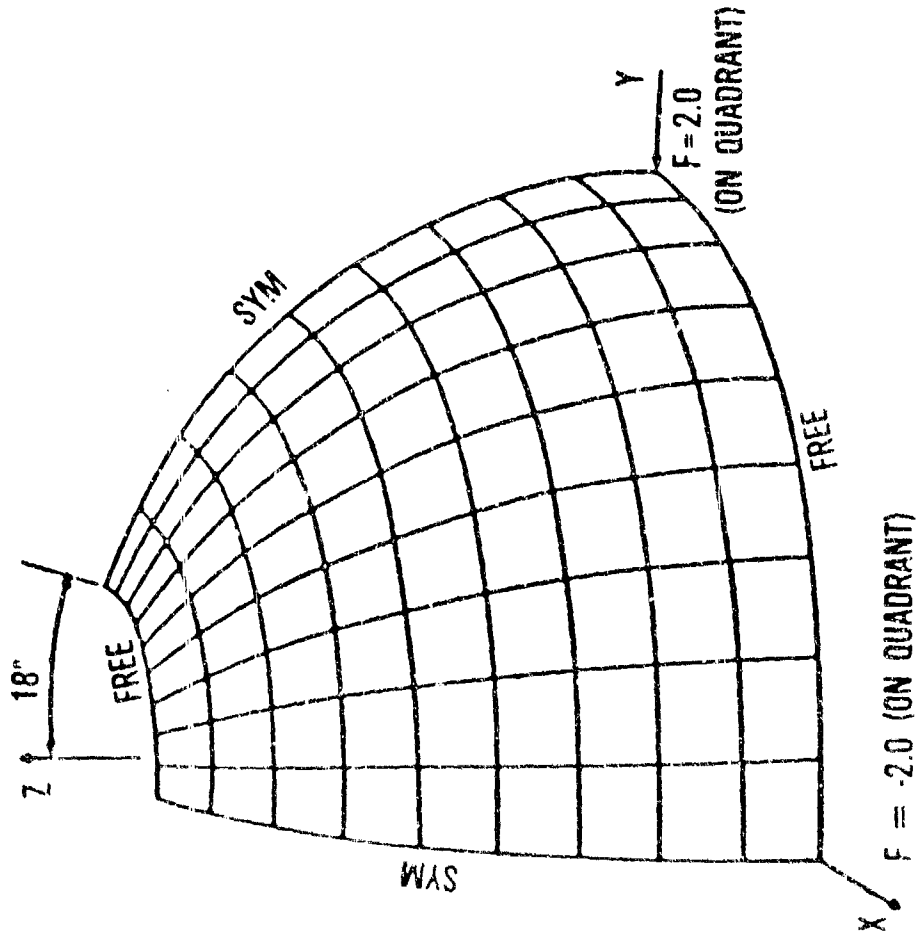
MESH	QUAD2	COSMIC	QUAD4	MSC
2x2	.900	1.419	1.376	
4x4	.691	1.047	1.050	
6x6	.791	.999	1.018	
8x8	.859	.978	1.008	
10x10	.896	.966	1.004	



SPHERICAL SHELL

PROBLEM #7

NORMALIZED RADIAL DEFLECTION AT THE CENTER



RADIUS = 10.0
 THICKNESS = .04
 $E = 6.825E7$
 $\nu = 0.3$

MESH	QUAD2	QUAD4 COSMIC	MSC
2x2	.928	1.051	.972
4x4	.990	1.054	1.024
6x6	.990	1.030	1.013
8x8	.986	1.015	1.005
10x10	.984	1.007	1.001
12x12	.982	1.003	.998



SUMMARY OF TEST RESULTS

TEST DESCRIPTION	ELEMENT IN-PLANE	LOADING OUT-OF-PLANE	ELEMENT SHAPE	COSMIC QUAD 2	COSMIC QUAD 4	MSCI QUAD 4
1. PATCH TEST	X		IRREGULAR	A	A	A
2. PATCH TEST	X	X	IRREGULAR	D	A	A
3. STRAIGHT BEAM, EXTENSION	X		ALL	A	A	A
4. STRAIGHT BEAM, BENDING	X		REGULAR	F	B	B
5. STRAIGHT BEAM, BENDING	X		IRREGULAR	F	F	F
6. STRAIGHT BEAM, BENDING		X	REGULAR	B	A	A
7. STRAIGHT BEAM, BENDING		X	IRREGULAR	B	A	B
8. STRAIGHT BEAM, TWIST			ALL	D	B	B
9. CURVED BEAM	X		REGULAR	F	C	C
10. CURVED BEAM		X	REGULAR	D	B	B
11. TWISTED BEAM	X	X	REGULAR	F	A	A
12. RECTANGULAR PLAT (5x5)		X	REGULAR	A	A	A
13. SCORDELIS-LO ROOF (5x5)	X	X	REGULAR	D	B	B
14. SPHERICAL SHELL (9x9)	X	X	REGULAR	A	A	A

MSCINASTRAM IS A SERVICE AND TRADEMARK OF MACNEAL-SCHWENDER CORP
 GRADE SCORES ARE DEFINED BY:

- A. ERROR LESS THAN 2%
- B. ERROR BETWEEN 2% AND 10%
- C. ERROR BETWEEN 10% AND 20%
- D. ERROR BETWEEN 20% AND 50%
- F. ERROR EXCEEDS 50%



LAMINATED COMPOSITE PLATE — PURE TWIST LOADING

PROBLEM #8

$$I_1 = I_2 = I_3 = .022222$$

$$E_1 = 2.0E7$$

$$E_2 = 5.0E5$$

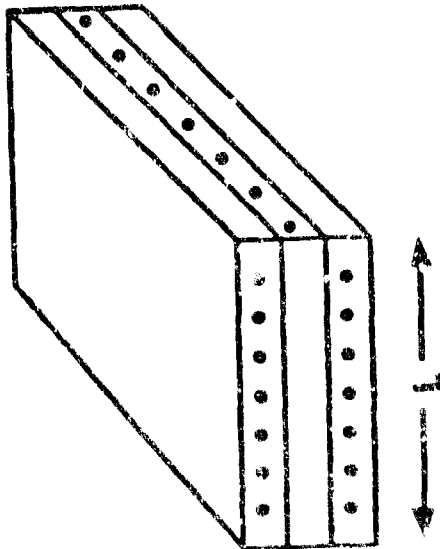
$$\nu_{12} = 0.25$$

$$G_{12} = 2.5E5$$

$$L = 5.0$$

$$\theta_1 = \theta_3 = 0.0$$

$$\theta_2 = 90.0$$



REGULAR SYMMETRIC
CROSS-PLY LAMINATE

A SQUARE PLATE OF A 4X4 MESH WITH THREE
CORNERS PINNED AND A TRANSVERSE POINT
LOAD AT THE FREE CORNER TO SIMULATE A
PURE TWIST LOADING

COMPARISON OF T3 DEFLECTION AT GRID 1

COSMICINASTRAN	MSCINASTRAN	THEORETICAL
-3.758E-2	-3.769E-2	-3.750E-3
(1.002)*	(1.005)	

COMPARISON OF TAU FOR ELEMENT 1, ALL LAYERS

COSMICINASTRAN	MSCINASTRAN	THEORETICAL
PLY 1 -5.0E1	-5.0E1	-5.0E1
PLY 2 0.0E1	0.0	0.0
PLY 3 5.0E1	5.0E1	5.0E1

*NORMALIZED VALUE

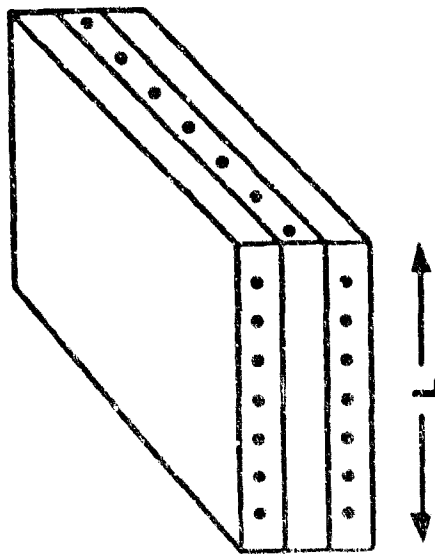


LAMINATED COMPOSITE PLATE

PROBLEM #9

$$\begin{aligned}
 \nu_1 = \nu_2 = \nu_3 &= .000666 \\
 E_1 &= 2.0E7 \\
 E_2 &= 5.0E5 \\
 \nu_{12} &= 0.25 \\
 G_{12} &= 2.5E5
 \end{aligned}$$

$$\begin{aligned}
 L &= 1.0 \\
 P &= 1.0 \text{ E-4} \\
 \theta_1 &= \theta_2 = 0.0 \\
 \theta_3 &= 90.0
 \end{aligned}$$



REGULAR SYMMETRIC
CROSS-PLY LAMINATE
SIMPLY SUPPORTED
UNIFORM PRESSURE LOAD

COMPARISON OF T3 DEFLECTION AT GRID 25

COSMIC/NASTRAN	MSC/NASTRAN	THEORETICAL
-1.852E-3	-1.739E-3	-1.836E-3
(1.009)*	(.947)	

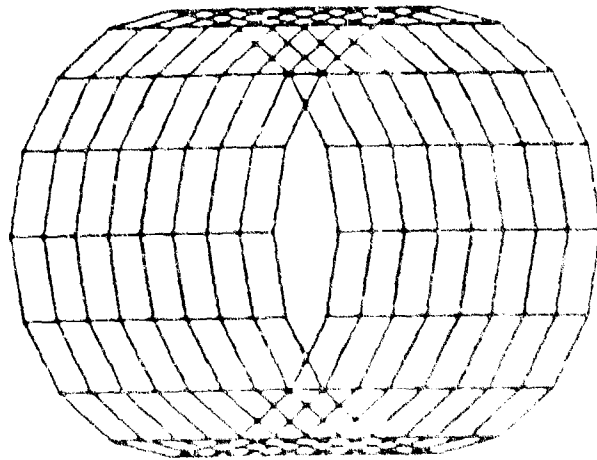
STRESS RESULTS		
LAYER 1&3	COSMIC 04	THEORY
σ_1	53.6	56.6
σ_2	1.4	1.8
τ_{12}	-.04	-.08

*NORMALIZED VALUE



LAMINATED COMPOSITE OPEN TUBE OF RADIUS 50.0

PROBLEM #10



SYMMETRIC
8 LAYERS

[45, -45, 0, 90, 90, 0, -45, 45]

CONSTANT PRESSURE

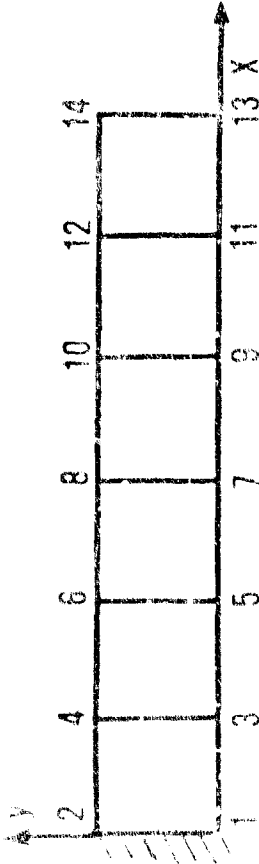
$T_i = .24 \quad i = 1, \dots, 8$
 $E_1 = 73.8E3 \quad L = 80.0$
 $E_2 = 3.75E3 \quad P = 10.5$
 $\nu_{12} = 0.4$
 $G_{12} = 1.74E3$

COMPARISON OF σ_1 LAYER STRESS FOR ELEMENT ID 9		
LAYER	COSMOS/NASTRAN	MSC/NASTRAN
1	2.425E2	2.519E2
2	2.543E2	2.494E2
3	-2.268E2	-2.257E2
4	7.252E2	7.268E2
5	7.252E2	7.269E2
6	-2.238E2	-2.255E2
7	2.560E2	2.535E2
8	2.472E2	2.478E2



STRAIGHT BEAM TEST

PROBLEM #11



$\nu = .25$ $i = 1$ 4
 $E_1 = 10.0E6$ $l = 6.0$
 $\nu = .3$

COMPARISON OF T1 DEFLECTION AT GRIDS 13 AND 14
(EXTENSIONAL)

	COSMIC/NASTRAN	MSC/NASTRAN	THEORETICAL
GRID 13	2.980E-5	2.986E-5	3.0E-5
GRID 14	2.981E-5	2.989E-5	3.0E-5

	(BENDING)
GRID 13	4.224E-1
GRID 14	4.224E-1

CANTILEVERED BEAM MODEL UNDER A)
 EXTENSIONAL AND B) BENDING LOADINGS
 SIMULATION OF EQUIVALENT ISOTROPIC
 PROPERTIES. LAMINATE CONFIGURATION

{0/0/0/0}

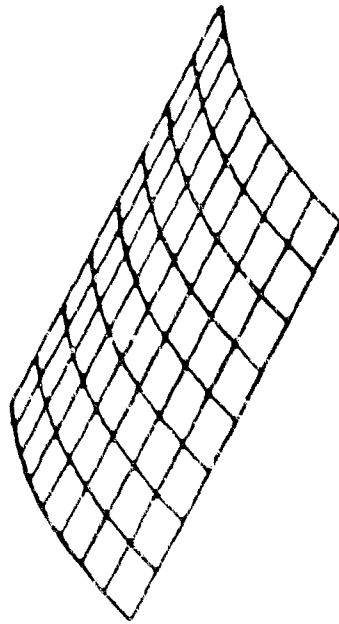
COMPARISON OF DIRECT LAYER BENDING STRESS ELEMENT 6 (LARGEST BENDING MOMENT)		
LAYER	COSMIC/NASTRAN	MSC/NASTRAN
1	1.238E4	1.238E4
2	4.125E3	4.126E3
3	-4.125E3	-4.126E3
4	-1.238E4	-1.238E4



LAMINATED COMPOSITE SHELL ROOF MODEL

PROBLEM #12

$T_i = .03125$ $i = 1, \dots, 8$ **RADIUS = 25.0**
 $E_1 = 2.0E8$ $L = 25.0$
 $E_2 = 0.5E7$ $P = 90.0$
 $NU_{12} = 0.25$ $G_{12} = 2.5E6$
 $G_{12} = 2.5E6$ $b_{22} = 2.5E6$



SYMMETRIC ANGLE PLY

8 LAYERS

[45, 45, 15, -15, -15, 15, -45, 45]

UNIFORM PRESSURE

COMPARISON OF T1 DEFLECTION AT SELECTED GRIDS		
GRID	COSMIC/NASTRAN	MSC/ASTRAN
34	-1.0735	-1.0662
35	-1.3545	-1.3441
36	-1.6213	-1.6074
43	-1.3563	-1.3287
44	-1.6876	-1.6739
45	-2.0267	-2.0079

APPENDIX C: CASE CONTROL AND BULK DATA CARDS
PERTAINING TO THE QUAD4 ELEMENT

Case Control Data Card ELSTRESS - Element Stress Output Request

Description: Requests form and type of element stress output.

Format and Example(s):

ELSTRESS $\left[\begin{array}{cccc} \text{SØRT1} & \text{PRINT} & \text{EXTREME} & \text{REAL} \\ \text{SØRT2} & \text{PUNCH} & \text{LAYER} & \text{IMAG} \\ & \text{NØPRINT} & & \text{PHASE} \end{array} \right] = \left\{ \begin{array}{c} \text{ALL} \\ n \\ \text{NØNE} \end{array} \right\}$

ELSTRESS = 5

ELSTRESS = ALL

ELSTRESS(SØRT1, PRINT, PUNCH, PHASE) = 15

<u>Option</u>	<u>Meaning</u>
SØRT1	Output will be presented as a tabular listing of elements for each load, frequency, eigenvalue, or time, depending on the rigid format. SØRT1 is not available in Transient problems (where the default is SØRT2).
SØRT2	Output will be presented as a tabular listing of load, frequency, or time for each element type. SØRT2 is available only in Static Analysis, Transient and Frequency Response problems.
PRINT	The printer will be the output media.
PUNCH	The card punch will be the output media.
EXTREME or LAYER	Requests stresses to be calculated at the extreme (top and bottom) fibers of a plate element or, for composites, the stresses for each layer. (See Remark B)
REAL or IMAG	Requests real and imaginary output on Complex Eigenvalue or Frequency Response problems.
PHASE	Requests magnitude and phase ($0.0^\circ \leq \text{phase} < 360.0^\circ$) on Complex Eigenvalue or Frequency Response problems.
ALL	Stresses for all elements will be output.
n	Set identification of a previously appearing SET card (Integer > 0). Only stresses for elements whose identification numbers appear on this SET card will be output.
NØNE	Stresses for no elements will be output.

- Remarks:
- Both PRINT and PUNCH may be requested.
 - An output request for ALL in Transient and Frequency response problems generally produces large amounts of printout. An alternative to this would be to define a SET of interest.
 - In Static Analysis or Frequency Response problems, any request for SØRT2 output causes all output to be SØRT2.
 - ELSTRESS is an alternate form and is entirely equivalent to STRESS.
 - ELSTRESS = NØNE allows overriding an overall request.

(Continued)

2.3-18 (8/10/87)

ELSTRESS (Continued)

6. If element stresses in material coordinate system are desired (only for TRIA1, TRIA2, QUAD1 and QUAD2 elements and only in Rigid Format 1), the parameter STRESS (see the description of the PARAM bulk data card in Section 2.4.2) should be set to be a positive integer. If, in addition to element stresses in material coordinate system, stresses at the connected grid points are also desired, the parameter STRESS should be set to 0.
7. When LAYER is selected, individual layer stresses and/or failure indices will be output.
8. The option EXTREME and LAYER is only applicable for the QUAD4 element.

Case Control Data Card STRESS - Element Stress Output Request

Description: Requests form and type of element stress output.

Format and Example(s):

STRESS $\left[\begin{array}{c} \text{SORT1} \\ \text{SORT2} \end{array} , \begin{array}{c} \text{PRINT} \\ \text{PUNCH} \\ \text{NOPRINT} \end{array} , \begin{array}{c} \text{EXTREME} \\ \text{LAYER} \end{array} , \begin{array}{c} \text{REAL} \\ \text{IMAG} \\ \text{PHASE} \end{array} \right] \cdot \left\{ \begin{array}{c} \text{ALL} \\ n \\ \text{NONE} \end{array} \right\}$

STRESS = 5

STRESS = ALL

STRESS(SORT1, PRINT, PUNCH, PHASE) = 15

<u>Option</u>	<u>Meaning</u>
SORT1	Output will be presented as a tabular listing of elements for each load, frequency, eigenvalue, or time, depending on the rigid format. SORT1 is not available in transient problems (where the default is SORT2).
SORT2	Output will be presented as a tabular listing of load, frequency, or time for each element type. SORT2 is available only in Static Analysis, Transient and Frequency Response problems.
PRINT	The printer will be the output media.
PUNCH	The card punch will be the output media.
EXTREME or LAYER	Requests stresses to be calculated at the extreme (top and bottom) fibers of a plate element or, for composites, the stresses for each layer. (See Remark 8)
REAL or IMAG	Requests real and imaginary output on Complex Eigenvalue or Frequency Response problems.
PHASE	Requests magnitude and phase ($0.0^\circ \leq \text{phase} < 360.0^\circ$) on Complex Eigenvalue or Frequency Response problems.
ALL	Stresses for all elements will be output.
n	Set identification of a previously appearing SET card (integer > 0). Only stresses for elements whose identification numbers appear on this SET card will be output.
NONE	Stresses for no elements will be output.

- Remarks:
1. Both PRINT and PUNCH may be requested.
 2. An output request for ALL in Transient and Frequency response problems generally produces large amounts of printout. An alternative to this would be to define a SET of interest.
 3. In Static Analysis or Frequency Response problems, any request for SORT2 output causes all output to be SORT2.
 4. STRESS is an alternate form and is entirely equivalent to ELSTRESS.
 5. STRESS = NONE allows overriding an overall request.

(Continued)

2.3-47 (8/10/87)

STRESS (Continued)

6. If element stresses in material coordinate system are desired (only for TRIA1, TRIA2, QUAD1 and QUAD2 elements and only in Rigid Format 1), the parameter STRESS (see the description of the PARAM bulk data card in Section 2.4.2) should be set to be a positive integer. If, in addition to element stresses in material coordinate system, stresses at the connected grid points are also desired, the parameter STRESS should be set to 0.
7. When LAYER is selected, individual layer stresses and/or failure indices will be output.
8. The option EXTREME and LAYER is only applicable for the QUAD4 element.

2.3-47a (8/10/67)

Input Data Card CQUAD4

Quadrilateral Element Connection

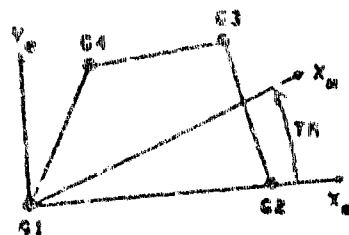
Description: Defines a quadrilateral plate element (QUAD4) of the structural model. This is an isoparametric membrane-bending element, with variable element thickness, layered composite material and thermal analysis capabilities.

Format and Example:

1	2	3	4	5	6	7	8	9	10
CQUAD4	EID	PID	G1	G2	G3	G4	TM	Z0	abc
CQUAD4	101	17	1001	1005	1010	1024	45.0	0.02	ABC
+bc			T1	T2	T3	T4			
+BC			0.03	0.125	0.05	0.04			

<u>Field</u>	<u>Contents</u>
EID	Element identification number (Integer > 0)
PID	Identification number of a PSHELL entry (Default is EID) (Integer > 0) For composites, see Remark 5.
G1	Grid point identification numbers of connection points (Integer > 0)
Z0	Offset of the element reference plane from the plane of grid points (Real or blank, see Remark 3 for default)
TM	Material property orientation specification (Real or blank; or 0 ≤ Integer < 1,000,000). If Real or blank, specifies the material property orientation angle in degrees. If Integer, the orientation of the material x-axis is along the projection onto the plane of the element of the x-axis of the coordinate system specified by the integer value.
TS	Membrane thickness of element at grid points G1 (Real or blank, see Remark 4 for default).

Remarks: 1. The QUAD4 geometry, coordinate systems and numbering are shown in the figure below:



2. Element identification numbers must be unique with respect to all other element identification numbers.

(Continued)

2.4-E7a (8/10/87)

CQUAD4 (Continued)

3. The material coordinate system (TM) and the offset (ZO) may also be provided on the PSHELL entry. The PSHELL data will be used if the corresponding field on the CQUAD4 entry is blank.
4. The T1 are optional, if not supplied they will be set to the value of T specified on the PSHELL entry. In such cases, the continuation entry is not required.
5. For composites, a PCOMP, PCOMP1, PCOMP2 card can be used instead of a PSHELL card.

BULK DATA DECK

Input Data Card MAT2 Material Property Definition

Description: Defines the material properties for linear, temperature-independent, anisotropic materials.

Format and Example:

1	2	3	4	5	6	7	8	9	10
MAT2	MID	G11	G12	G13	G22	G23	G33	RHO	+abc
MAT2	13	6.2+3			6.2+3		5.1+3	0.056	ABC
+abc	A1	A2	A12	T0	GE	ST	SC	SS	+def
+BC	0.15			-500.0	0.002	20.+5			DEF
+def	MCSID								
+EF	1008								

<u>Field</u>	<u>Contents</u>
MID	Material identification number (Integer > 0)
G _{ij}	The material property matrix (Real)
RHO	Mass density (Real)
A _i	Thermal expansion coefficient vector (Real)
T0	Thermal expansion reference temperature (Real)
GE	Structural element damping coefficient (Real)
ST, SC, SS	Stress limits for tension, compression and shear (Real) (Used only to compute margins of safety in certain elements; they have no effect on the computational procedures)
MCSID	Material coordinate system identification number (integer ≥ 0 or blank)

- Remarks:
1. The material identification numbers must be unique for all MAT1, MAT2 and MAT3 cards.
 2. MAT2 materials may be made temperature dependent by use of the MAT2 card.
 3. The mass density, RHO, will be used to automatically compute mass for all structural elements except the two-dimensional bending only elements TRBSC, TRPLT and QOPLT.
 4. The convention for the G_{ij} in fields 3 through 8 is represented by the matrix relationship.

$$\begin{pmatrix} \sigma_1 \\ \sigma_2 \\ \tau_{12} \end{pmatrix} = \begin{bmatrix} G_{11} & G_{12} & G_{13} \\ G_{12} & G_{22} & G_{23} \\ G_{13} & G_{23} & G_{33} \end{bmatrix} \begin{pmatrix} \epsilon_1 \\ \epsilon_2 \\ \gamma_{12} \end{pmatrix}$$

5. MCSID (> 0) is required if stresses or strains/curvatures are to be computed in a material coordinate system. This is applicable only for TRIA1, TRIA2, QUAD1, and QUAD2 elements.

Input Data Card MATB

Orthotropic Plate Material Property Definition

Description: Defines the material property for an orthotropic material for plate elements.

Format and Example:

1	2	3	4	5	6	7	8	9	10
MATB	MID	E1	E2	NU12	G12	G1Z	G2Z	RHO	abc
MATB	299	32.+6	4.2+5	0.33	2.9+6			0.042	ABC
+bc	A1	A2	TREF	XT	XC	YT	YC	S	def
+BC	14.-6	2.3-6	175.						DEF
+ef	GE	F12							
+EF	2.5-4								

Field

Contents

MID	Material identification number (Integer > 0)
E1,E2	Modulus of elasticity in the material x and y directions (Real ≠ 0.0)
NU12	Poisson's Ratio (Real) (See Remark 5)
G12	Linear in-plane shear modulus (Real > 0.0)
G1Z	Transverse shear modulus for shear in X-Z plane (Real)
G2Z	Transverse shear modulus for shear in Y-Z plane (Real)
RHO	Mass density (Real)
A1,A2	Thermal expansion coefficients in the material x and y directions (T, Real > 0.0)
TREF	Thermal expansion reference temperature (XC, Real)
XT,XC	Allowable stresses/strains in tension and compression, respectively, in the material x direction. Required if failure index calculation is desired. (XT, Real > 0.0) (XC, Real) (Default value for XC is XT) (See Remark 3)
YT,YC	Allowable stresses/strains in tension and compression, respectively, in the material y direction. Required if failure index calculation is desired. (YT, Real > 0.0) (YC, Real) (Default value for YC is YT) (See Remark 3)
S	Allowable stress/strain for in-plane shear (Real > 0.0) (See Remark 3)
GE	Structural damping coefficient (Real)
F12	Tsai-Wu interaction term (Real) (See Remark 4)

(Continued)

2.4-173a (8/10/87)

MATB (Continued)

- Remarks:
1. Material coordinate systems are defined by the plate element connection entries on the CQUAD4 card.
 2. The stress-strain relationship defined by this data is:

$$\begin{bmatrix} \epsilon_1 \\ \epsilon_2 \\ \gamma_{12} \end{bmatrix} = \begin{bmatrix} 1/E1 & -\text{NU12}/E1 & 0 \\ -\text{NU12}/E1 & 1/E2 & 0 \\ 0 & 0 & 1/G12 \end{bmatrix} \begin{bmatrix} \sigma_1 \\ \sigma_2 \\ \tau_{12} \end{bmatrix} + (T-TREF) \begin{bmatrix} \alpha 1 \\ \alpha 2 \\ 0 \end{bmatrix}$$

$$\begin{bmatrix} \tau_{23} \\ \tau_{31} \end{bmatrix} = \begin{bmatrix} G12 & 0 \\ 0 & G23 \end{bmatrix} \begin{bmatrix} \gamma_{23} \\ \gamma_{31} \end{bmatrix}$$

3. Fields XT, XC, YT, YC and S are used only for composite materials when failure calculations are requested with PCOMP, PCOMP1 or PCOMP2 Bulk Data entries. Allowables represent stresses except when the maximum strain failure theory is used.
4. The F12 field is used only for composite materials when the Tsai-Wu failure theory is used and failure calculations are requested.
5. NU12 is Poisson's Ratio (ϵ_1/ϵ_2 for uniaxial loading in 1-direction). Note that NU21 = ϵ_2/ϵ_1 uniaxial loading in 2-direction, is related to NU12, E1 and E2 by the relationship, (NU12) (E2) = (NU21) (E1).

Input Data Card PCOMP Layered Composite Element Property

Description: Defines the properties of a n-ply laminated composite material.

Format and Example:

1	2	3	4	5	6	7	8	9	10
PCOMP	PID	Z0	NSM	SBOND	FT			LOPT	abc
PCOMP	100	-0.5	1.5	5.+3	NOFF			SYMMEM	ABC
+bc	MID1	T1	TH1	SOUT1	MID2	T2	TH2	SOUT2	def
+BC	150	0.05	90.	YES			-45.		DEF
+ef	MID3	T3	TH3	SOUT3	ETC. . .				
+EF			45.0					

<u>Field</u>	<u>Contents</u>
PID	Property identification number (1,000,000 > Integer > 0)
Z0	Offset of the element reference plane from the plane of grid points (Real or blank, see Remark 2)
NSM	Non-structural mass per unit area. (Real)
SBOND	Allowable shear stress of the bonding material. (Real > 0.0 or blank) Required if failure theory is used.
FT	Failure theory, one of the strings "HILL", "HOFF", "TSAI", "STRESS", or "STRAIN". See Remark 4. (BCD or blank)
LOPT	Lamination generation option, one of the strings, "ALL", "SYM", "MEM", or "SYMMEM". See Remark 5. (BCD or blank) Default is all.
MID1	Material identification number of the 1 th layer. (Integer > 0 or blank)
T1	Thickness of the 1 th layer (Real, > 0.0 or blank)
TH1	Angle between the longitudinal direction of the fibers of the 1 th layer and the material X-axis. (Real or blank)
SOUT1	Stress output request for 1 th layer, one of the strings "YES" or "NO". (Default is "NO")

- Remarks:
1. The plies are numbered from 1 to n beginning with the bottom layer.
 2. The offset (Z0) is used only when the corresponding field on the CQXAD4 Bulk Data entry referencing this property are blank.

(Continued)

2.4-222a (8/10/87)

PCOMP (Continued)

3. SBOND is required if bonding material failure index calculations are desired.
4. The failure theory is used to determine the element failure on a ply-by-ply basis. The available theories are:

HILL	- Hill Theory
HOFF	- Hoffman Theory
TSAI	- Tsai-Wu Theory
STRESS	- For Maximum Stress Theory
STRAIN	- For Maximum Strain Theory

5. To minimize input requirements several lamination options (LOPT) are available. All indicates that every ply is specified, SYM indicates that ply layup is symmetric about the center ply and that the plies on one side of the center line are specified. SYMMEM indicates a symmetric layup of membrane only plies.
6. The material properties, MID_i, may reference only MAT1, MAT2 and MAT8 Bulk Data entries.
7. If any of the MID_i, T_i or TH_i are blank, then the last non-blank values specified for each will be used to define the values for the ply.

Input Data Card PCOMP1 Layered Composite Element Property

Description: Defines the properties of a n-ply laminated composite material where all plies are composed of the same material and are of equal thickness.

Format and Example:

1	2	3	4	5	6	7	8	9	10
PCOMP1	PID	Z0	NSM	SBOND	FT	MID	TPLY	LOPT	abc
PCOMP1	100	-0.5	1.7	5.+3	STRAIN	200	0.25	SYM	ABC
+bc	TH1	TH2	TH3	TH4	TH5	ETC. . .			
+BC	-45.0	45.0	90.0	90.0	45.0			

<u>Field</u>	<u>Contents</u>
PID	Property identification number (1,000,000 > Integer > 0)
Z0	Offset of the element reference plane from the plane of grid points (Real or blank, see Remark 2)
NSM	Non-structural mass per unit area. (Real)
SBOND	Allowable shear stress of the bonding material. (Real > 0.0)
FT	Failure theory, one of the strings "HILL", "HOFF", "TSAI", "STRESS", or "STRAIN". See Remark 4.
MID	Material identification number for all layers. (Integer > 0)
LOPT	Lamination generation option, one of the strings, "ALL", "SYM", "MEM", or "SYMMEM". See Remark 5.
TPLY	Thickness of all layers (Real, > 0.0 or blank)
THi	Angle between the longitudinal direction of the fibers of the i th layer and the material X-axis. (Real or blank)

- Remarks:
1. The plies are numbered from 1 to n beginning with the bottom layer.
 2. The offset (Z0) is used only when the corresponding field on the CQUAD4 Bulk Data entry referencing this property is blank.
 3. SBOND is required if bonding material failure index calculations are desired.
 4. The failure theory is used to determine the element failure on a ply-by-ply basis. The available theories are:
 - HILL - Hill Theory
 - HOFF - Hoffman Theory
 - TSAI - Tsai-Wu Theory
 - STRESS - For Maximum Stress Theory
 - STRAIN - For Maximum Strain Theory

(Continued)

PCCOMP (Continued)

5. To minimize input requirements several lamination options (LOPT) are available. ALL indicates that every ply is specified, SYM indicates that ply layup is symmetric about the center ply and that the plies on one side of the center line are specified. SYMMEM indicates a symmetric layup of membrane only plies.
6. The material property, MID, may reference only MAT1, MAT2 and MAT8 Bulk Data entries.

2.4.21-9 (8-10-87)

Input Data Card PCOMP2

Layered Composite Element Property

Description: Defines the properties of a n-ply laminated composite material where all plies are composed of the same material.

Format and Example:

1	2	3	4	5	6	7	8	9	10
PCOMP2	PID	Z0	NSM	SBOND	FT	MID	X	LOPT	abc
PCOMP2	100	-0.5	1.7	5.+3	TSAI	200		SYM	ABC
+bc	T1	TH1	T2	TH2	T3	TH3	ETC. . .		
+BC	0.25	-45.0	0.5	90.0	0.25	45.0		

<u>Field</u>	<u>Contents</u>
PID	Property identification number (1,000,000 > Integer > 0)
Z0	Offset of the element reference plane from the plane of grid points (Real or blank, see Remark 2)
NSM	Non-structural mass per unit area. (Real)
SBOND	Allowable shear stress of the bonding material. (Real > 0.0)
FT	Failure theory, one of the strings "HILL", "HOFF", "TSAI", "STRESS", or "STRAIN". See Remark 4.
MID	Material identification number for all layers. (Integer > 0 or blank)
LOPT	Lamination generation option, one of the strings, "ALL", "SYM", "MEM", or "SYMMEM". See Remark 5.
T _i	Thickness of the i th layer (Real, > 0.0 or blank)
TH _i	Angle between the longitudinal direction of the fibers of the i th layer and the material X-axis. (Real or blank)

- Remarks:
1. The plies are numbered from 1 to n beginning with the bottom layer.
 2. The offset (Z0) is used only when the corresponding field on the EQUAD4 Bulk Data entry referencing this property is blank.
 3. SBOND is required if bonding material failure index calculations are desired.
 4. The failure theory is used to determine the element failure on a ply-by-ply basis. The available theories are:
 - HILL - Hill Theory
 - HOFF - Hoffman Theory
 - TSAI - Tsai-Wu Theory
 - STRESS - For Maximum Stress Theory
 - STRAIN - For Maximum Strain Theory

(Continued)

2.4-223e (8/10/87)

PCOMP2 (Continued)

5. To minimize input requirements several lamination options (LOPT) are available. All indicates that every ply is specified, SYM indicates that ply layup is symmetric about the center ply and that the plies on one side of the center line are specified. SYMMEM indicates a symmetric layup of membrane only plies.
6. The material property, MID, may reference only MAT1, MAT2 and MAT8 Bulk Data entries.
7. If any of the T1 or TH1 are blank, then the last non-blank values specified for each will be used to define the values for the ply.

2.4-223f (8/10/87)

Input Data Card PLOAD4 Pressure Loads on Face of Structural Elements

Description: Defines a load on a face of a QUAD4 element.

Format and Example:

1	2	3	4	5	6	7	8	9	10
PLOAD4	SID	EID	P1	P2	P3	P4			abc
PLOAD4	101	2043	15.	18.	23.6				ABC

+bc	CID	N1	N2	N3					
+BC	52	1.0	0.	0.					

Alternate Form:

PLOAD4	SID	E1	P1	P2	P3	P4	"THRU"	E2	ghi
PLOAD4	1001	452	105.				THRU	568	GHI

+h1	CID	N1	N2	N3					
+HI	2375	0.	1.	1.					

<u>Field</u>	<u>Contents</u>
SID	Load set identification number (Integer > 0)
EID,E1,E2	Element identification number (Integer > 0, E1 < E2)
P1	Pressure at the grid point defining the element face (Real or blank)
CID	Coordinate system identification number (Integer > 0)
N1	Components of a vector in system CID that defines the direction of the pressure (Real)

- Remarks:
1. For the plate element QUAD4, if the continuation entry is not given, the direction of the pressure is normal to the element in the element Z direction. If only P1 is given, the pressure is assumed to be uniform over the element surface.
 2. If the loaded surface of an element is curved, and a direction vector is not specified, the direction of the pressure may vary over the surface. The pressure intensity is the load per unit surface area.
 3. Equivalent grid point loads are computed. A uniform pressure need not result in equal grid point loads.

Bulk Data Entry PSHELL Shell Element Property

Description: Defines the membrane, bending, transverse shear, and coupling properties of the QUAD4 shell element.

Format and Example:

1	2	3	4	5	6	7	8	9	10
PSHELL	PID	MID1	T	MID2	12I/T ³	MID3	TS/T	NSM	abc
PSHELL	203	204	1.90	205	1.2	206	0.8	6.32	ABC
+bc	Z1	Z2	MID4	MCSID	SCSID	Z0			
+BC	+ .95	- .95		0	0	0.01			

<u>Field</u>	<u>Contents</u>
PID	Property identification number (Integer > 0)
MID1	Material identification number for membrane (Integer > 0 or blank)
T	Default value for membrane thickness (Real > 0.0)
MID2	Material identification number for bending (Integer > 0 or blank)
12I/T ³	Bending stiffness parameter (Real or blank, default = 1.0)
MID3	Material identification number for transverse shear (Integer > 0 or blank, must be blank unless MID2 > 0)
TS/T	Transverse shear thickness divided by membrane thickness (Real or blank, default = .833333)
NSM	Nonstructural mass per unit area (Real)
Z1,Z2	Fiber distances for stress computation. The positive direction is determined by the righthand rule and the order in which the grid points are listed on the connection entry. (Real or blank, defaults are -T/2 for Z1 and +T/2 for Z2).
MID4	Material identification number for membrane-bending coupling (Integer > 0 or blank, must be blank unless MID1 > 0 and MID2 > 0, may not equal MID1 or MID2)
MCSID	Identification number of material coordinate system (Real or blank, or Integer \geq 0) (See Remark 11)
SCSID	Identification number of stress coordinate system (Real or blank, or Integer \geq 0) (See Remark 11)
Z0	Offset of the element reference plane from the plane of grid points. (Real or blank, default = 0.0) (See Remark 12)

(Continued)

2.4 255 (8/10/87)

PSHELL (Continued)

- Remarks:
1. All PSHELL property entries must have unique identification numbers.
 2. The structural mass is computed from the density using the membrane thickness and membrane material properties.
 3. The results of leaving any MID field blank are:

MID1	No membrane or coupling stiffness
MID2	No bending, coupling, or transverse shear stiffness
MID3	No transverse shear flexibility
MID4	No membrane-bending coupling
 4. The continuation entry is not required.
 5. Structural damping, when needed, is obtained from the MID1 material.
 6. The MID4 field should be left blank if the material properties are symmetric with the middle surface of the shell.
 7. For structural problems, PSHELL entries may reference MAT1, MAT2 or MAT8 material property data.
 8. If the transverse shear material, MID3, references MAT2 data, then G33 must be zero. If MID3 references MAT8 data, then G1,Z and G2,Z must not be zero.
 9. For heat transfer problems PSHELL entries may reference MAT4 or MAT5 material property data.
 10. If MCSID/SCSID is left blank (0.0) or is real, it is considered to be the angle of rotation of the X axis of the material/stress coordinate system with respect to the X axis of the element coordinate system in the XY plane of the latter. If Integer, the orientation of the material/stress x-axis is along the projection of the x-axis of the specified coordinate system onto the x-y plane of the element system. The value of MCSID is the default value for the TM field on the CQUAD4 Bulk Data entries.
 11. The value of ZO is the default value for the corresponding field on the CQUAD4 Bulk Data entries.

APPENDIX D: NASTRAN INPUT DATA FOR SAMPLE
PROBLEMS 1-6, 8, 9, 11, 13c, 14

INFORMATION DISSEMINATION MESSAGE TEST,
 REMAINING PLATE MATCH TEST

CASE CONTROL DECK ECHO

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 000100

000001 INFORMATION MESSAGE TEST, MAIN DATA NOT SORTED, XSORT WILL RE-SORT RECORDS.
 000002 REMAINING PLATE MATCH TEST
 000003
 000004

COMP	COMP	1	2	3	4	5	6	7	8	9	10
1-	1	1	1	1	1	1	1	1	1	1	1
2-	2	1	1	1	1	1	1	1	1	1	1
3-	3	1	1	1	1	1	1	1	1	1	1
4-	4	1	1	1	1	1	1	1	1	1	1
5-	5	1	1	1	1	1	1	1	1	1	1
6-	6	1	1	1	1	1	1	1	1	1	1
7-	7	1	1	1	1	1	1	1	1	1	1
8-	8	1	1	1	1	1	1	1	1	1	1
9-	9	1	1	1	1	1	1	1	1	1	1
10-	10	1	1	1	1	1	1	1	1	1	1
11-	11	1	1	1	1	1	1	1	1	1	1
12-	12	1	1	1	1	1	1	1	1	1	1
13-	13	1	1	1	1	1	1	1	1	1	1
14-	14	1	1	1	1	1	1	1	1	1	1
15-	15	1	1	1	1	1	1	1	1	1	1
16-	16	1	1	1	1	1	1	1	1	1	1
17-	17	1	1	1	1	1	1	1	1	1	1
18-	18	1	1	1	1	1	1	1	1	1	1
19-	19	1	1	1	1	1	1	1	1	1	1
20-	20	1	1	1	1	1	1	1	1	1	1

ID MASTROM, FISCHLER
APP DISP
SOL 1.0
TIME 540
CARD
BENDING PLATE PATCH TEST

APRIL 12, 1988 RELEASE 1807 CSC PAGE 2

CASE CONTROL DECK ECHO

CARD
COUNT
1
2
3
4
5
6
TITLE - BENDING PLATE PATCH TEST
SPC - 1
SPCFORCES - ALL
DISP - ALL
STRESS - ALL
BEGIN BULK

USER INFORMATION MESSAGE 007, BULK DATA NOT SORTED, ISORT WILL RE-ORDER DECK.
APRIL 12, 1988 RELEASE 1807 CSC PAGE 3

CARD	COUNT	1	2	3	4	5	6	7	8	9	10
1	1	COLUM4									
2	1	COLUM4									
3	1	COLUM4									
4	1	COLUM4									
5	1	COLUM4									
6	1	GRID									
7	1	GRID									
8	1	GRID									
9	1	GRID									
10	1	GRID									
11	1	GRID									
12	1	GRID									
13	1	GRID									
14	1	PSHELL									
15	1	PSHELL									
16	1	SPC									
17	1	SPC									
18	1	SPC									
19	1	SPC									
20	1	SPC									
21	1	SPC									
22	1	SPC									
23	1	SPC									
24	1	ENDDATA									

IP MASTMAN, TISCHLER
 APP DISP
 SOL 1.4
 TIME 5
 CEND

PROBLEM #2

1 STRAIGHT CANTILEVER BEAM - REGULAR SHAPE ELEMENTS

APRIL 11, 1983 RELEASE 1987 CMC PAGE 2

CASE CONTROL DECK ECHO

CARD
 COUNT

1 TITLE = STRAIGHT CANTILEVER BEAM - REGULAR SHAPE ELEMENTS
 2 DISP = ALL
 3 STRESS = ALL
 4 ELFORCES = ALL
 5 STRAIN = ALL
 6 SPC = 1
 7 SPCFORCES = ALL
 8 OLOAD = ALL
 9 BLOAD IN THE X DIRECTION
 10 SURFACE 1
 11 LOAD = 1
 12 BLOAD IN THE Z DIRECTION
 13 SURFACE 2
 14 LOAD = 2
 15 BLOAD IN THE Y DIRECTION
 16 SURFACE 3
 17 LOAD = 3
 18 STUIFY LOAB
 19 SURFACE 4
 20 LOAD = 4
 21 BEGIN BULK

6 USER INFORMATION MESSAGE '07' BULK DATA NOT SORTED. XSORT WILL RE-ORDER DECK.
 1 STRAIGHT CANTILEVER BEAM - REGULAR SHAPE ELEMENTS

APRIL 11, 1986 RELEASE 1987 CDC PAGE 3

SORTED BULK DATA ECHO

CARD	COUNT	1	2	3	4	5	6	7	8	9	10
1	1	1	1	1	1	1	1	1	1	1	1
2	1	1	1	1	1	1	1	1	1	1	1
3	1	1	1	1	1	1	1	1	1	1	1
4	1	1	1	1	1	1	1	1	1	1	1
5	1	1	1	1	1	1	1	1	1	1	1
6	1	1	1	1	1	1	1	1	1	1	1
7	1	1	1	1	1	1	1	1	1	1	1
8	1	1	1	1	1	1	1	1	1	1	1
9	1	1	1	1	1	1	1	1	1	1	1
10	1	1	1	1	1	1	1	1	1	1	1
11	1	1	1	1	1	1	1	1	1	1	1
12	1	1	1	1	1	1	1	1	1	1	1
13	1	1	1	1	1	1	1	1	1	1	1
14	1	1	1	1	1	1	1	1	1	1	1
15	1	1	1	1	1	1	1	1	1	1	1
16	1	1	1	1	1	1	1	1	1	1	1
17	1	1	1	1	1	1	1	1	1	1	1
18	1	1	1	1	1	1	1	1	1	1	1
19	1	1	1	1	1	1	1	1	1	1	1
20	1	1	1	1	1	1	1	1	1	1	1
21	1	1	1	1	1	1	1	1	1	1	1

PROBLEM #2

12345	9	4.0	-0.1	0
67890	11	5.0	0.1	0
12345	12	5.0	0.1	0
67890	13	6.0	0.1	0
12345	14	1.0E+07	0	1
67890	15	1.0E+07	1	14
12345	16	123456	THRU	12
67890	17	123456	THRU	2
12345	18			
67890	19			
12345	20			
67890	21			
12345	22			
67890	23			

PROBLEM #2

26-
27-
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29-
30-
31-
32-

0010 10714
0010 14
0010 14
PART 1
PCELL 1
SOCI 1
SOCI 1
ENDATA

5.0
6.0
6.0
1.0 E+07
6 1
123452 1

-0.1
-0.1
-0.1
1.3
THRU
THRU

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14
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PROBLEM #2

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27-
28-
29-
30-
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32-

GRID 11
GRID 12
GRID 13
GRID 14
PART 1
PSHELL 1
SPCL 1
SPCL 2
ENDDATA

4.8
5.0
6.0
6.0
1.0 E+07
123456 1

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-0.1
-0.1
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THRU
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12. WESTMAN, FISCHLER

APR 12 1968
504 1.0
TIME 5.09
COMB
CUMULATED BEAM

CASE CONTROL DECK ECHO

CASE
COUNT
1 TITLE - CUMULATED BEAM
2 DISP - ALL
3 SPOFCRCS - ALL
4 STAFFCS - ALL
5 QLOAD - ALL
6 SPC - 1
7 BLOAD IN THE Y DIRECTION
8 SURCASE 1
9 LOAD 1
10 BLOAD IN THE Z DIRECTION
11 SURCASE 2
12 LOAD 2
13 RECTIBUILD

EVERY USER LINE MESSAGE 207. BULK DATA NOT SORTED. SORT WILL BE ORDER RECD.

CASE	COUNT	1	2	3	4	5	6	7	8	9	10
CUMULATED BEAM	1	1	1	1	1	1	1	1	1	1	1
DISP - ALL	2	1	1	1	1	1	1	1	1	1	1
SPOFCRCS - ALL	3	1	1	1	1	1	1	1	1	1	1
STAFFCS - ALL	4	1	1	1	1	1	1	1	1	1	1
QLOAD - ALL	5	1	1	1	1	1	1	1	1	1	1
SPC - 1	6	1	1	1	1	1	1	1	1	1	1
BLOAD IN THE Y DIRECTION	7	1	1	1	1	1	1	1	1	1	1
SURCASE 1	8	1	1	1	1	1	1	1	1	1	1
LOAD 1	9	1	1	1	1	1	1	1	1	1	1
BLOAD IN THE Z DIRECTION	10	1	1	1	1	1	1	1	1	1	1
SURCASE 2	11	1	1	1	1	1	1	1	1	1	1
LOAD 2	12	1	1	1	1	1	1	1	1	1	1
RECTIBUILD	13	1	1	1	1	1	1	1	1	1	1
	14	1	1	1	1	1	1	1	1	1	1

ID MASTRAM, TISCHLER
APP DISP
SOL 1.0
TIME 5.00
CEND
TUISTED BEAM

CASE CONTROL DECK ECHO

TITLE - TUISTED BEAM
SFC - 5
SFCFORCES - ALL
OLOAD - ALL
DISP - ALL
STRESS - ALL
SUNCASE 1
LOAD - 1
SUBCASE 2 - 2
LOAD - 2
BEGIN BULK

1
0
0

1
0
0

SORTED BULK DATA ECHO

CARD COUNT	1	2	3	4	5	6	7	8	9	10
1-	1	1	20	39	38	36	35	34	33	32
2-	2	20	38	37	35	34	33	32	31	30
3-	3	20	36	35	34	33	32	31	30	29
4-	4	20	35	34	33	32	31	30	29	28
5-	5	20	33	32	31	30	29	28	27	26
6-	6	20	32	31	30	29	28	27	26	25
7-	7	20	30	29	28	27	26	25	24	23
8-	8	20	27	26	25	24	23	22	21	20
9-	9	20	26	25	24	23	22	21	20	19
10-	10	20	24	23	22	21	20	19	18	17
11-	11	20	24	23	22	21	20	19	18	17
12-	12	20	21	20	19	18	17	16	15	14
13-	13	20	20	19	18	17	16	15	14	13
14-	14	20	20	19	18	17	16	15	14	13
15-	15	20	18	17	16	15	14	13	12	11
16-	16	20	15	14	13	12	11	10	9	8
17-	17	20	14	13	12	11	10	9	8	7
18-	18	20	14	13	12	11	10	9	8	7
19-	19	20	11	10	9	8	7	6	5	4
20-	20	20	11	10	9	8	7	6	5	4
21-	21	20	9	8	7	6	5	4	3	2
22-	22	20	8	7	6	5	4	3	2	1
23-	23	20	6	5	4	3	2	1	0	0
24-	24	20	5	4	3	2	1	0	0	0
25-	25	1	5	4	3	2	1	0	0	0
26-	26	1	5	4	3	2	1	0	0	0
27-	27	1	5	4	3	2	1	0	0	0
28-	28	1	5	4	3	2	1	0	0	0
29-	29	1	5	4	3	2	1	0	0	0
30-	30	1	5	4	3	2	1	0	0	0
31-	31	1	5	4	3	2	1	0	0	0
32-	32	1	5	4	3	2	1	0	0	0
33-	33	1	5	4	3	2	1	0	0	0

34- TWISTED BEAM

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CARD

COUNT

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 GRID 6
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SORTED BULK DATA ECHO

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34- TWISTED BEAM

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CARD

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SORTED BULK DATA ECHO

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APPLICATION OF SINGLE POINT CONSTRAINTS. REFER TO PRINTOUT OF AUTOMATICALLY GENERATED SPC1 CARDS FOR DETAILS.
TUISHED BEAM

1

•

AUTOMATICALLY GENERATED SPC1 CARDS

CARD	1	2	3	4	5	6	7	8	9	10
COUNT	5	5	4	23	26	29	32	35		
1-	5	5	4	23	26	29	32	35		
2-	5	5	5	2	5	8	11	14		
3-	5	5	5	20				17		

•

ID NASTRAN, TISSOMER
APP DISP
SOL 1.0
TIME 549
CEND

1 RECTANGULAR PLATE - CLAMPED AR-1 N-6 APRIL 13, 1988 RELEASE 1987 CDC PAGE 2

CASE CONTROL DECK ECHO
CARD COUNT
1 TITLE - RECTANGULAR PLATE - CLAMPED AR-1 N-6
2 DISP - ALL
3 STRESS - ALL
4 LOAD - ALL
5 SPCFORCES - ALL
6 SPC - 5
7 SUBCASE 1
8 UNIFORM PRESSURE
9 LOAD - 1
10 SUBCASE 2
11 B CENTRAL LOAD
12 LOAD - 2
13 BEGIN BULK

0 USER INFORMATION MESSAGE 207, BULK DATA NOT SORTED, XSORT WILL RE-ORDER DECK.
1 RECTANGULAR PLATE - CLAMPED AR-1 N-6 APRIL 13, 1988 RELEASE 1987 CDC PAGE 3

CARD COUNT	1	2	3	4	5	6	7	8	9	10
1	1	1	1	1	1	1	1	1	1	1
2	1	1	1	1	1	1	1	1	1	1
3	1	1	1	1	1	1	1	1	1	1
4	1	1	1	1	1	1	1	1	1	1
5	1	1	1	1	1	1	1	1	1	1
6	1	1	1	1	1	1	1	1	1	1
7	1	1	1	1	1	1	1	1	1	1
8	1	1	1	1	1	1	1	1	1	1
9	1	1	1	1	1	1	1	1	1	1
10	1	1	1	1	1	1	1	1	1	1
11	1	1	1	1	1	1	1	1	1	1
12	1	1	1	1	1	1	1	1	1	1
13	1	1	1	1	1	1	1	1	1	1
14	1	1	1	1	1	1	1	1	1	1
15	1	1	1	1	1	1	1	1	1	1
16	1	1	1	1	1	1	1	1	1	1
17	1	1	1	1	1	1	1	1	1	1
18	1	1	1	1	1	1	1	1	1	1
19	1	1	1	1	1	1	1	1	1	1
20	1	1	1	1	1	1	1	1	1	1
21	1	1	1	1	1	1	1	1	1	1
22	1	1	1	1	1	1	1	1	1	1
23	1	1	1	1	1	1	1	1	1	1
24	1	1	1	1	1	1	1	1	1	1
25	1	1	1	1	1	1	1	1	1	1
26	1	1	1	1	1	1	1	1	1	1
27	1	1	1	1	1	1	1	1	1	1
28	1	1	1	1	1	1	1	1	1	1
29	1	1	1	1	1	1	1	1	1	1
30	1	1	1	1	1	1	1	1	1	1
31	1	1	1	1	1	1	1	1	1	1

32- 32- COUADA 32 37 38 45 44
 33- 33- COUADA 33 38 39 46 45
 34- 34- COUADA 34 39 40 47 46
 1 RECTANGULAR PLATE - CLAMPED AR-1 N-6

1
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SORTED BULK DATA ECHO

CARD COUNT	1	2	3	4	5	6	7	8	9	10
35- COUADA 35	1	35	1	40	41	42	47	48	49	48
36- COUADA 36	1	36	1	41	42	43	48	49	50	49
37- FORCE	2	40	40	-1.0E-04.0	1.0					
38- GRID	1	1	.0000	.0000	.0000	.0000	.0000	.0000	.0000	.0000
39- GRID	2	2	.0000	.0000	.0000	.0000	.0000	.0000	.0000	.0000
40- GRID	3	3	.0000	.0000	.0000	.0000	.0000	.0000	.0000	.0000
41- GRID	4	4	.0000	.0000	.0000	.0000	.0000	.0000	.0000	.0000
42- GRID	5	5	.0000	.0000	.0000	.0000	.0000	.0000	.0000	.0000
43- GRID	6	6	.0000	.0000	.0000	.0000	.0000	.0000	.0000	.0000
44- GRID	7	7	.0000	.0000	.0000	.0000	.0000	.0000	.0000	.0000
45- GRID	8	8	.1667	.1667	.1667	.1667	.1667	.1667	.1667	.1667
46- GRID	9	9	.1667	.1667	.1667	.1667	.1667	.1667	.1667	.1667
47- GRID	10	10	.1667	.1667	.1667	.1667	.1667	.1667	.1667	.1667
48- GRID	11	11	.1667	.1667	.1667	.1667	.1667	.1667	.1667	.1667
49- GRID	12	12	.1667	.1667	.1667	.1667	.1667	.1667	.1667	.1667
50- GRID	13	13	.1667	.1667	.1667	.1667	.1667	.1667	.1667	.1667
51- GRID	14	14	.1667	.1667	.1667	.1667	.1667	.1667	.1667	.1667
52- GRID	15	15	.3333	.3333	.3333	.3333	.3333	.3333	.3333	.3333
53- GRID	16	16	.3333	.3333	.3333	.3333	.3333	.3333	.3333	.3333
54- GRID	17	17	.3333	.3333	.3333	.3333	.3333	.3333	.3333	.3333
55- GRID	18	18	.3333	.3333	.3333	.3333	.3333	.3333	.3333	.3333
56- GRID	19	19	.3333	.3333	.3333	.3333	.3333	.3333	.3333	.3333
57- GRID	20	20	.3333	.3333	.3333	.3333	.3333	.3333	.3333	.3333
58- GRID	21	21	.3333	.3333	.3333	.3333	.3333	.3333	.3333	.3333
59- GRID	22	22	.5000	.5000	.5000	.5000	.5000	.5000	.5000	.5000
60- GRID	23	23	.5000	.5000	.5000	.5000	.5000	.5000	.5000	.5000
61- GRID	24	24	.5000	.5000	.5000	.5000	.5000	.5000	.5000	.5000
62- GRID	25	25	.5000	.5000	.5000	.5000	.5000	.5000	.5000	.5000
63- GRID	26	26	.5000	.5000	.5000	.5000	.5000	.5000	.5000	.5000
64- GRID	27	27	.5000	.5000	.5000	.5000	.5000	.5000	.5000	.5000
65- GRID	28	28	.5000	.5000	.5000	.5000	.5000	.5000	.5000	.5000
66- GRID	29	29	.6667	.6667	.6667	.6667	.6667	.6667	.6667	.6667
67- GRID	30	30	.6667	.6667	.6667	.6667	.6667	.6667	.6667	.6667
68- GRID	31	31	.6667	.6667	.6667	.6667	.6667	.6667	.6667	.6667

1 RECTANGULAR PLATE - CLAMPED AR-1 N-6
 0
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SORTED BULK DATA ECHO

CARD COUNT	1	2	3	4	5	6	7	8	9	10
69- GRID	1	32	3	.0007	.0000	.0000	.0000	.0000	.0000	.0000
70- GRID	2	33	3	.0007	.0000	.0000	.0000	.0000	.0000	.0000
71- GRID	3	34	3	.6667	.3333	.0000	.0000	.0000	.0000	.0000
72- GRID	4	35	3	.6667	1.0000	.0000	.0000	.0000	.0000	.0000
73- GRID	5	36	3	.0000	.0000	.0000	.0000	.0000	.0000	.0000
74- GRID	6	37	3	.0000	.0000	.0000	.0000	.0000	.0000	.0000
75- GRID	7	38	3	.0000	.0000	.0000	.0000	.0000	.0000	.0000
76- GRID	8	39	3	.0000	.0000	.0000	.0000	.0000	.0000	.0000
77- GRID	9	40	3	.0000	.0000	.0000	.0000	.0000	.0000	.0000
78- GRID	10	41	3	.0000	.0000	.0000	.0000	.0000	.0000	.0000
79- GRID	11	42	3	.0000	.0000	.0000	.0000	.0000	.0000	.0000
80- GRID	12	43	3	.0000	.0000	.0000	.0000	.0000	.0000	.0000
81- GRID	13	44	3	1.0000	.0000	.0000	.0000	.0000	.0000	.0000
82- GRID	14	45	3	1.0000	.3333	.0000	.0000	.0000	.0000	.0000

PROBLEM #5

83-	GRID	46	1.0000	.5000	.0000					
84-	GRID	47	1.0000	.6667	.0000					
85-	GRID	48	1.0000	.8333	.0000					
86-	GRID	49	1.0000	1.0000	.0000					
87-	MAT1	1	1.747227	3						
88-	PLORDE	1	1.4E-04	1	THRU	36	1	1.0	.0	
89-	PSHELL	1	1.001	1	THRU	1.0				
90-	SPC1	5	9	14	THRU	14				
91-	SPC1	5	16	21	THRU	21				
92-	SPC1	5	23	28	THRU	28				
93-	SPC1	5	30	35	THRU	35				
94-	SPC1	5	37	42	THRU	42				
95-	SPC1	5	44	49	THRU	49				
96-	SPC1	5	44	45		48	47	48	49	
97-	SPC1	5	15	14		42	42	42	49	
98-	SPC1	5	24	21		5	5	5	6	
99-	SPC1	5	123456	1		28	28	28	36	
100-	SPC1	5	123456	7		3	4	4		
	SPC1	5	123456	43		15	22	22		
	EMDDATA									

IB INSTRUM, TIGHELLER
APP DISP
SOL 1.0
TIME 5.00
CEND

APRIL 13, 1988 RELEASE 1987 CDC PAGE 2

1 RECTANGULAR PLATE - CLAMPED AB-S N=6

CASE CONTROL DECK ECHO

CMP#
COUNT

TITLE - RECTANGULAR PLATE - CLAMPED AB-S N=6
DISP - ALL
STRESS - ALL
LOAD - ALL
SUPPORTS - ALL
SFC - 5
SUBCASE 1
8 UNIFORM PRESSURE
LOAD - 1
SUBCASE 2
8 CENTRAL LOAD
LOAD - 2
BEGIN BULK

882 USER INFORMATION MESSAGE 287, BULK DATA NOT SORTED. XSORT WILL RE-ORDER DECK.
APRIL 13, 1988 RELEASE 1987 CDC PAGE 3

CMP#
COUNT

CMP#	COUNT	1	2	3	4	5	6	7	8	9	10
1-	1	1	1	1	1	1	1	1	1	1	1
2-	2	1	1	1	1	1	1	1	1	1	1
3-	3	1	1	1	1	1	1	1	1	1	1
4-	4	1	1	1	1	1	1	1	1	1	1
5-	5	1	1	1	1	1	1	1	1	1	1
6-	6	1	1	1	1	1	1	1	1	1	1
7-	7	1	1	1	1	1	1	1	1	1	1
8-	8	1	1	1	1	1	1	1	1	1	1
9-	9	1	1	1	1	1	1	1	1	1	1
10-	10	1	1	1	1	1	1	1	1	1	1
11-	11	1	1	1	1	1	1	1	1	1	1
12-	12	1	1	1	1	1	1	1	1	1	1
13-	13	1	1	1	1	1	1	1	1	1	1
14-	14	1	1	1	1	1	1	1	1	1	1
15-	15	1	1	1	1	1	1	1	1	1	1
16-	16	1	1	1	1	1	1	1	1	1	1
17-	17	1	1	1	1	1	1	1	1	1	1
18-	18	1	1	1	1	1	1	1	1	1	1
19-	19	1	1	1	1	1	1	1	1	1	1
20-	20	1	1	1	1	1	1	1	1	1	1
21-	21	1	1	1	1	1	1	1	1	1	1
22-	22	1	1	1	1	1	1	1	1	1	1
23-	23	1	1	1	1	1	1	1	1	1	1
24-	24	1	1	1	1	1	1	1	1	1	1
25-	25	1	1	1	1	1	1	1	1	1	1
26-	26	1	1	1	1	1	1	1	1	1	1
27-	27	1	1	1	1	1	1	1	1	1	1
28-	28	1	1	1	1	1	1	1	1	1	1
29-	29	1	1	1	1	1	1	1	1	1	1
30-	30	1	1	1	1	1	1	1	1	1	1
31-	31	1	1	1	1	1	1	1	1	1	1

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22- COLUMBIA 38
31- COLUMBIA 33
34- COLUMBIA 34
1 RECTANGULAR PLATE - CLAMPED 60-S No.6

37 38 39 40 41 42 43 44 45 46
35 36 37 38 39 40 41 42 43 44 45 46

SORTED BULK DATA ECHO

COUNT	1	2	3	4	5	6	7	8	9	10
32-	COLUMBIA 38	38	1	40	41	42	43	44	45	46
33-	COLUMBIA 33	33	1	41	42	43	44	45	46	
34-	COLUMBIA 34	34	1							
35-	FORCE 2	2	49							
36-	CALD	1								
37-	CALD	2								
38-	CALD	3								
39-	CALD	4								
40-	CALD	5								
41-	CALD	6								
42-	CALD	7								
43-	CALD	8								
44-	CALD	9								
45-	CALD	10								
46-	CALD	11								
47-	CALD	12								
48-	CALD	13								
49-	CALD	14								
50-	CALD	15								
51-	CALD	16								
52-	CALD	17								
53-	CALD	18								
54-	CALD	19								
55-	CALD	20								
56-	CALD	21								
57-	CALD	22								
58-	CALD	23								
59-	CALD	24								
60-	CALD	25								
61-	CALD	26								
62-	CALD	27								
63-	CALD	28								
64-	CALD	29								
65-	CALD	30								
66-	CALD	31								
67-	CALD									
68-	CALD									

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RECTANGULAR PLATE - CLAMPED 60-S No.6

SORTED BULK DATA ECHO

COUNT	1	2	3	4	5	6	7	8	9	10
69-	CALD	32	1	40	41	42	43	44	45	46
70-	CALD	33	1	41	42	43	44	45	46	
71-	CALD	34	1							
72-	CALD	35	49							
73-	CALD	36								
74-	CALD	37								
75-	CALD	38								
76-	CALD	39								
77-	CALD	40								
78-	CALD	41								
79-	CALD	42								
80-	CALD	43								
81-	CALD	44								
82-	CALD	45								

PROBLEM #5

83-	46	1.247287	5.0000	5.0000	1	1	1	36	47	48	49
84-	47	1.65164	5.0000	5.0000	1	1	1.9	14	35	42	46
85-	48	1	5.0000	5.0000	1	1	14	21	4	5	8
86-	49	1	5.0000	5.0000	1	1	23	28	4	5	8
87-	1	1	1.0001	1	1	1	16	21	4	5	8
88-	1	1	1	1	1	1	23	28	4	5	8
89-	1	1	1	1	1	1	30	35	4	5	8
90-	1	1	1	1	1	1	37	42	4	5	8
91-	1	1	1	1	1	1	44	49	4	5	8
92-	1	1	1	1	1	1	44	49	4	5	8
93-	1	1	1	1	1	1	15	20	4	5	8
94-	1	1	1	1	1	1	24	29	4	5	8
95-	1	1	1	1	1	1	123456	1	4	5	8
96-	1	1	1	1	1	1	123456	7	4	5	8
97-	1	1	1	1	1	1	123456	43	4	5	8
98-	1	1	1	1	1	1	123456	7	4	5	8
99-	1	1	1	1	1	1	123456	43	4	5	8
100-	1	1	1	1	1	1	123456	43	4	5	8

18 MASTRAM, TIMONDEL
 AFS DISC
 SOL 1.6
 TIME 1500
 CEND

1 RECTANGULAR PLATE - SIMPLY SUPPORTED AB-1 M-6

APRIL 13, 1968 RELEASE 1987 CBC PAGE 2

CASE CONTROL DECK ECHO

CARD
 COUNT
 1
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TITLE - RECTANGULAR PLATE - SIMPLY SUPPORTED AB-1 M-6
 DIM - ALL
 STRESS - ALL
 LOAD - ALL
 SPC POINTS - ALL
 SPC - 5
 SUBCASE 1
 UNIFORM PRESSURE
 LOAD - 1
 SUBCASE 2
 CENTRAL LOAD
 LOAD - 2
 BEGIN BULK

8 8775 USED INCORPORATION MESSAGE 207, BULK DATA NOT SORTED, NSORT WILL RE-ORDER BULK.
 9 1 RECTANGULAR PLATE - SIMPLY SUPPORTED AB-1 M-6
 5 8

APRIL 13, 1968 RELEASE 1987 CBC PAGE 3

CARD
 COUNT
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32- 32- 33- 34- 1
 33- 33- 34- 1
 34- 34- 1
 1 RECTANGULAR PLATE - SIMPLY SUPPORTED AR=1 N=6

APRIL 13, 1988 RELEASE 1987 CDC PAGE 4

0
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SORTED BULK DATA ECHO

CARD	COUNT	1	2	3	4	5	6	7	8	9	10
35-	35-	1	1	1	40	41	48	47	47	47	10
36-	36-	1	1	1	41	42	49	48	48	48	
37-	37-	2	49		-1.0E-04,0					1.0	
38-	38-	1			.0000	.0000	.0000				
39-	39-	2			.0000	.1667	.0000				
40-	40-	3			.0000	.3333	.0000				
41-	41-	4			.0000	.5000	.0000				
42-	42-	5			.0000	.6667	.0000				
43-	43-	6			.0000	.8333	.0000				
44-	44-	7			.0000	1.0000	.0000				
45-	45-	8			.1667	.0000	.0000				
46-	46-	9			.1667	.1667	.0000				
47-	47-	10			.1667	.3333	.0000				
48-	48-	11			.1667	.5000	.0000				
49-	49-	12			.1667	.6667	.0000				
50-	50-	13			.1667	.8333	.0000				
51-	51-	14			.1667	1.0000	.0000				
52-	52-	15			.3333	.0000	.0000				
53-	53-	16			.3333	.1667	.0000				
54-	54-	17			.3333	.3333	.0000				
55-	55-	18			.3333	.5000	.0000				
56-	56-	19			.3333	.6667	.0000				
57-	57-	20			.3333	.8333	.0000				
58-	58-	21			.3333	1.0000	.0000				
59-	59-	22			.5000	.0000	.0000				
60-	60-	23			.5000	.1667	.0000				
61-	61-	24			.5000	.3333	.0000				
62-	62-	25			.5000	.5000	.0000				
63-	63-	26			.5000	.6667	.0000				
64-	64-	27			.5000	.8333	.0000				
65-	65-	28			.5000	1.0000	.0000				
66-	66-	29			.6667	.0000	.0000				
67-	67-	30			.6667	.1667	.0000				
68-	68-	31			.6667	.3333	.0000				

1 RECTANGULAR PLATE - SIMPLY SUPPORTED AR=1 N=6

APRIL 13, 1988 RELEASE 1987 CDC PAGE 5

0
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SORTED BULK DATA ECHO

CARD	COUNT	1	2	3	4	5	6	7	8	9	10
69-	69-	32			.0000	.0000	.0000				
70-	70-	33			.0000	.0000	.0000				
71-	71-	34			.6667	.8333	.0000				
72-	72-	35			.6667	1.0000	.0000				
73-	73-	36			.8333	.0000	.0000				
74-	74-	37			.8333	.1667	.0000				
75-	75-	38			.8333	.3333	.0000				
76-	76-	39			.8333	.5000	.0000				
77-	77-	40			.8333	.6667	.0000				
78-	78-	41			.8333	.8333	.0000				
79-	79-	42			.8333	1.0000	.0000				
80-	80-	43			1.0000	.0000	.0000				
81-	81-	44			1.0000	.1667	.0000				
82-	82-	45			1.0000	.3333	.0000				

1 RECTANGULAR PLATE - SIMPLY SUPPORTED AR=1 N=6

APRIL 13, 1988 RELEASE 1987 CDC PAGE 5

PROBLEM #5

83-	GRID	46	1.0000	.5000	.0000																				
84-	GRID	47	1.0000	.5557	.0000																				
85-	GRID	48	1.0000	.8333	.0000																				
86-	GRID	49	1.0000	1.0000	.0000																				
87-	MAVL	1	1.7472E7	.3																					
88-	PLONDR	1	1.0E-04	1																					
89-	PSHELL	1	1	.001																					
90-	SPCI	5	6	9	THRU	1	1.0																		
91-	SPCI	5	6	18	THRU	21	14																		
92-	SPCI	5	6	23	THRU	28	22																		
93-	SPCI	5	6	30	THRU	35	26																		
94-	SPCI	5	6	37	THRU	42	32																		
95-	SPCI	5	6	44	THRU	49	38																		
96-	SPCI	5	6	44	THRU	49	46																		
97-	SPCI	5	15	15	THRU	48	45																		
98-	SPCI	5	24	24	THRU	28	21																		
99-	SPCI	5	1236	1	THRU	3	2																		
100-	SPCI	5	1236	7	THRU	8	8																		
	ENDDATA			43																					

10 ANSTROM TISONLER
APR 13 1988
SOL 118
TIME 5:00
CEND

1 RECTANGULAR PLATE - SIMPLY SUPPORTED AP-5 M-6

APRIL 13, 1988

RELEASE 1987 CDC

PAGE 2

CASE CONTROL BECR ECHO

CASE CONTROL BECR ECHO

TITLE - RECTANGULAR PLATE - SIMPLY SUPPORTED AP-5 M-6

SIZE - ALL

STRESS - ALL

LOAD - ALL

SUPPORTS - ALL

SPEC - 5

SUBCASE 1

\$ UNIFORM PRESSURE

LOAD 1

SUBCASE 2

\$ CENTRAL LOAD

LOAD 2

BECEM BULK

1 USE BULK MESSAGE 207, BULK DATA NOT SORTED, NSORT WILL RE-ORDER DEC.

2 RECTANGULAR PLATE - SIMPLY SUPPORTED AP-5 M-6

APRIL 13, 1988

RELEASE 1987 CDC

PAGE 3

BECEM BULK

CASE	CONTROL	BECEM	ECHO
1	1	1	1
1	2	2	2
1	3	3	3
1	4	4	4
1	5	5	5
1	6	6	6
1	7	7	7
1	8	8	8
1	9	9	9
1	10	10	10
1	11	11	11
1	12	12	12
1	13	13	13
1	14	14	14
1	15	15	15
1	16	16	16
1	17	17	17
1	18	18	18
1	19	19	19
1	20	20	20
1	21	21	21
1	22	22	22
1	23	23	23
1	24	24	24
1	25	25	25
1	26	26	26
1	27	27	27
1	28	28	28
1	29	29	29
1	30	30	30
1	31	31	31
1	32	32	32
1	33	33	33
1	34	34	34
1	35	35	35
1	36	36	36
1	37	37	37
1	38	38	38
1	39	39	39
1	40	40	40
1	41	41	41
1	42	42	42
1	43	43	43

32- 32- COUNDA 32 1 37 38 45 46
 33- 33- COUNDA 33 1 38 39 46 47
 34- 34- COUNDA 34 1 39 40 47 48
 1 RECTANGULAR PLATE - SIMPLY SUPPORTED AR-5 N-6 APRIL 13, 1968 RELEASE 1987 CDC PAGE 4

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SORTED BULK DATA ECHO

CARD	COUNT	1	2	3	4	5	6	7	8	9	10
COUNDA	35	1	1	1	41	48	47	47	7	9	10
COUNDA	36	1	1	1	42	49	48	48			
FORCE	2	1	49		-1.0E-04.0		1.0				
CAID	1				.0000	.0000					
CAID	2				.0000	.1667	.0000				
CAID	3				.0000	.3333	.0000				
CAID	4				.0000	.5000	.0000				
CAID	5				.0000	.6667	.0000				
CAID	6				.0000	.8333	.0000				
CAID	7				.0000	1.0000	.0000				
CAID	8				.8333	.1667	.0000				
CAID	9				.8333	.3333	.0000				
CAID	10				.8333	.5000	.0000				
CAID	11				.8333	.6667	.0000				
CAID	12				.8333	.8333	.0000				
CAID	13				.8333	1.0000	.0000				
CAID	14				1.6667	.1667	.0000				
CAID	15				1.6667	.3333	.0000				
CAID	16				1.6667	.5000	.0000				
CAID	17				1.6667	.6667	.0000				
CAID	18				1.6667	.8333	.0000				
CAID	19				2.5000	.0000	.0000				
CAID	20				2.5000	.1667	.0000				
CAID	21				2.5000	.3333	.0000				
CAID	22				2.5000	.5000	.0000				
CAID	23				2.5000	.6667	.0000				
CAID	24				2.5000	.8333	.0000				
CAID	25				2.5000	1.0000	.0000				
CAID	26				3.3333	.0000	.0000				
CAID	27				3.3333	.1667	.0000				
CAID	28				3.3333	.3333	.0000				
CAID	29				3.3333	.5000	.0000				
CAID	30				3.3333	.6667	.0000				
CAID	31				3.3333	.8333	.0000				

1 RECTANGULAR PLATE - SIMPLY SUPPORTED AR-5 N-6 APRIL 13, 1968 RELEASE 1987 CDC PAGE 5

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SORTED BULK DATA ECHO

CARD	COUNT	1	2	3	4	5	6	7	8	9	10
CAID	32	1	1	1	3.3333	.0000	.0000				
CAID	33	1	1	1	3.3333	.1667	.0000				
CAID	34	1	1	1	3.3333	.3333	.0000				
CAID	35	1	1	1	3.3333	.5000	.0000				
CAID	36	1	1	1	4.1667	.0000	.0000				
CAID	37	1	1	1	4.1667	.1667	.0000				
CAID	38	1	1	1	4.1667	.3333	.0000				
CAID	39	1	1	1	4.1667	.5000	.0000				
CAID	40	1	1	1	4.1667	.6667	.0000				
CAID	41	1	1	1	4.1667	.8333	.0000				
CAID	42	1	1	1	5.0000	.0000	.0000				
CAID	43	1	1	1	5.0000	.1667	.0000				
CAID	44	1	1	1	5.0000	.3333	.0000				

PROBLEM #5

83-	CAF ID	46	5.9900	5.9900	.6886			
84-	CAF ID	42	5.9900	5.9900	.6886			
85-	CAF ID	42	5.9900	5.9900	.6886			
86-	CAF ID	42	5.9900	5.9900	.6886			
87-	MATL	1	1.74787	1	1	1.0	1.0	.0
88-	PLUMB	1	1.92-07	1	1	1.0	1.0	.0
89-	PLUMB	1	1.92-07	1	1	1.0	1.0	.0
90-	PLUMB	1	1.92-07	1	1	1.0	1.0	.0
91-	SPC	1	16	1	1	1.0	1.0	.0
92-	SPC	1	16	1	1	1.0	1.0	.0
93-	SPC	1	16	1	1	1.0	1.0	.0
94-	SPC	1	16	1	1	1.0	1.0	.0
95-	SPC	1	16	1	1	1.0	1.0	.0
96-	SPC	1	16	1	1	1.0	1.0	.0
97-	SPC	1	16	1	1	1.0	1.0	.0
98-	SPC	1	16	1	1	1.0	1.0	.0
99-	SPC	1	16	1	1	1.0	1.0	.0
100-	SPC	1	16	1	1	1.0	1.0	.0
	END OF							

18 MASTRANI, TISCHLER

APR 31 54
VOL 1 6
PAGE 504
COLUMBIA

1 SCORRELLIS - LO ROOF N = 8

CASE CONTROL BECK ECHO

CARD
COUNT
1
2
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TITLE - SCORRELLIS - LO ROOF N = 8
DISP - ALL
SYNDS - ALL
LOAD - ALL
SPACES - ALL
SER - 8
SUBCASE 1
CARD 1
BEAM BUILD

8 BEAM USER INFORMATION MESSAGE 887, BEAM DATA NOT SORTED. MSORT WILL RE-CORNER BECK.
1 SCORRELLIS - LO ROOF N = 8

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SORTED 3 1 K DATA ECHO

CARD	COUNT	1	2	3	4	5	6	7	8	9	10
1	1	1	1	1	1	1	1	1	1	1	1
2	1	1	1	1	1	1	1	1	1	1	1
3	1	1	1	1	1	1	1	1	1	1	1
4	1	1	1	1	1	1	1	1	1	1	1
5	1	1	1	1	1	1	1	1	1	1	1
6	1	1	1	1	1	1	1	1	1	1	1
7	1	1	1	1	1	1	1	1	1	1	1
8	1	1	1	1	1	1	1	1	1	1	1
9	1	1	1	1	1	1	1	1	1	1	1
10	1	1	1	1	1	1	1	1	1	1	1
11	1	1	1	1	1	1	1	1	1	1	1
12	1	1	1	1	1	1	1	1	1	1	1
13	1	1	1	1	1	1	1	1	1	1	1
14	1	1	1	1	1	1	1	1	1	1	1
15	1	1	1	1	1	1	1	1	1	1	1
16	1	1	1	1	1	1	1	1	1	1	1
17	1	1	1	1	1	1	1	1	1	1	1
18	1	1	1	1	1	1	1	1	1	1	1
19	1	1	1	1	1	1	1	1	1	1	1
20	1	1	1	1	1	1	1	1	1	1	1
21	1	1	1	1	1	1	1	1	1	1	1
22	1	1	1	1	1	1	1	1	1	1	1
23	1	1	1	1	1	1	1	1	1	1	1
24	1	1	1	1	1	1	1	1	1	1	1
25	1	1	1	1	1	1	1	1	1	1	1
26	1	1	1	1	1	1	1	1	1	1	1
27	1	1	1	1	1	1	1	1	1	1	1
28	1	1	1	1	1	1	1	1	1	1	1
29	1	1	1	1	1	1	1	1	1	1	1
30	1	1	1	1	1	1	1	1	1	1	1
31	1	1	1	1	1	1	1	1	1	1	1
32	1	1	1	1	1	1	1	1	1	1	1
33	1	1	1	1	1	1	1	1	1	1	1
34	1	1	1	1	1	1	1	1	1	1	1
35	1	1	1	1	1	1	1	1	1	1	1
36	1	1	1	1	1	1	1	1	1	1	1
37	1	1	1	1	1	1	1	1	1	1	1
38	1	1	1	1	1	1	1	1	1	1	1
39	1	1	1	1	1	1	1	1	1	1	1
40	1	1	1	1	1	1	1	1	1	1	1
41	1	1	1	1	1	1	1	1	1	1	1
42	1	1	1	1	1	1	1	1	1	1	1
43	1	1	1	1	1	1	1	1	1	1	1
44	1	1	1	1	1	1	1	1	1	1	1
45	1	1	1	1	1	1	1	1	1	1	1
46	1	1	1	1	1	1	1	1	1	1	1
47	1	1	1	1	1	1	1	1	1	1	1
50	1	1	1	1	1	1	1	1	1	1	1
51	1	1	1	1	1	1	1	1	1	1	1
52	1	1	1	1	1	1	1	1	1	1	1
53	1	1	1	1	1	1	1	1	1	1	1
54	1	1	1	1	1	1	1	1	1	1	1
55	1	1	1	1	1	1	1	1	1	1	1
56	1	1	1	1	1	1	1	1	1	1	1
57	1	1	1	1	1	1	1	1	1	1	1
58	1	1	1	1	1	1	1	1	1	1	1
59	1	1	1	1	1	1	1	1	1	1	1
60	1	1	1	1	1	1	1	1	1	1	1
61	1	1	1	1	1	1	1	1	1	1	1
62	1	1	1	1	1	1	1	1	1	1	1
63	1	1	1	1	1	1	1	1	1	1	1
64	1	1	1	1	1	1	1	1	1	1	1
65	1	1	1	1	1	1	1	1	1	1	1
66	1	1	1	1	1	1	1	1	1	1	1
67	1	1	1	1	1	1	1	1	1	1	1
68	1	1	1	1	1	1	1	1	1	1	1
69	1	1	1	1	1	1	1	1	1	1	1
70	1	1	1	1	1	1	1	1	1	1	1
71	1	1	1	1	1	1	1	1	1	1	1
72	1	1	1	1	1	1	1	1	1	1	1
73	1	1	1	1	1	1	1	1	1	1	1
74	1	1	1	1	1	1	1	1	1	1	1
75	1	1	1	1	1	1	1	1	1	1	1
76	1	1	1	1	1	1	1	1	1	1	1
77	1	1	1	1	1	1	1	1	1	1	1
78	1	1	1	1	1	1	1	1	1	1	1
79	1	1	1	1	1	1	1	1	1	1	1
80	1	1	1	1	1	1	1	1	1	1	1
81	1	1	1	1	1	1	1	1	1	1	1
82	1	1	1	1	1	1	1	1	1	1	1
83	1	1	1	1	1	1	1	1	1	1	1
84	1	1	1	1	1	1	1	1	1	1	1
85	1	1	1	1	1	1	1	1	1	1	1
86	1	1	1	1	1	1	1	1	1	1	1
87	1	1	1	1	1	1	1	1	1	1	1
88	1	1	1	1	1	1	1	1	1	1	1
89	1	1	1	1	1	1	1	1	1	1	1
90	1	1	1	1	1	1	1	1	1	1	1
91	1	1	1	1	1	1	1	1	1	1	1
92	1	1	1	1	1	1	1	1	1	1	1
93	1	1	1	1	1	1	1	1	1	1	1
94	1	1	1	1	1	1	1	1	1	1	1
95	1	1	1	1	1	1	1	1	1	1	1
96	1	1	1	1	1	1	1	1	1	1	1
97	1	1	1	1	1	1	1	1	1	1	1
98	1	1	1	1	1	1	1	1	1	1	1
99	1	1	1	1	1	1	1	1	1	1	1
100	1	1	1	1	1	1	1	1	1	1	1

85-	CARD	20	14.3394	6.2500	20.4788
86-	CARD	21	12.5000	6.2500	21.6506
87-	CARD	22	10.5655	6.2500	22.6577
88-	CARD	23	8.5505	6.2500	23.4923
89-	CARD	24	6.4705	6.2500	24.1481
90-	CARD	25	4.3412	6.2500	24.6202
91-	CARD	26	2.1789	6.2500	24.9049
92-	CARD	27	-.0000	6.2500	25.0000
93-	CARD	28	16.0697	9.3750	19.1511
94-	CARD	29	14.3394	9.3750	20.4788
95-	CARD	30	12.5000	9.3750	21.6506
96-	CARD	31	10.5655	9.3750	22.6577
97-	CARD	32	8.5505	9.3750	23.4923
98-	CARD	33	6.4705	9.3750	24.1481
99-	CARD	34	4.3412	9.3750	24.6202
100-	CARD	35	2.1789	9.3750	24.9049
101-	CARD	36	-.0000	9.3750	25.0000
102-	CARD	37	16.0697	12.5000	19.1511

1 SCORDELIS - LO ROOF M - 8 MAY 12, 1968 RELEASE 1987 CDC PAGE 6

CARD COUNT SORTED BULK DATA ECHO

103-	CARD	38	14.3394	12.5000	20.4788
104-	CARD	39	12.5000	12.5000	21.6506
105-	CARD	40	10.5655	12.5000	22.6577
106-	CARD	41	8.5505	12.5000	23.4923
107-	CARD	42	6.4705	12.5000	24.1481
108-	CARD	43	4.3412	12.5000	24.6202
109-	CARD	44	2.1789	12.5000	24.9049
110-	CARD	45	-.0000	12.5000	25.0000
111-	CARD	46	16.0697	15.6250	19.1511
112-	CARD	47	14.3394	15.6250	20.4788
113-	CARD	48	12.5000	15.6250	21.6506
114-	CARD	49	10.5655	15.6250	22.6577
115-	CARD	50	8.5505	15.6250	23.4923
116-	CARD	51	6.4705	15.6250	24.1481
117-	CARD	52	4.3412	15.6250	24.6202
118-	CARD	53	2.1789	15.6250	24.9049
119-	CARD	54	-.0000	15.6250	25.0000
120-	CARD	55	16.0697	18.7500	19.1511
121-	CARD	56	14.3394	18.7500	20.4788
122-	CARD	57	12.5000	18.7500	21.6506
123-	CARD	58	10.5655	18.7500	22.6577
124-	CARD	59	8.5505	18.7500	23.4923
125-	CARD	60	6.4705	18.7500	24.1481
126-	CARD	61	4.3412	18.7500	24.6202
127-	CARD	62	2.1789	18.7500	24.9049
128-	CARD	63	-.0000	18.7500	25.0000
129-	CARD	64	16.0697	21.8750	19.1511
130-	CARD	65	14.3394	21.8750	20.4788
131-	CARD	66	12.5000	21.8750	21.6506
132-	CARD	67	10.5655	21.8750	22.6577
133-	CARD	68	8.5505	21.8750	23.4923
134-	CARD	69	6.4705	21.8750	24.1481
135-	CARD	70	4.3412	21.8750	24.6202
136-	CARD	71	2.1789	21.8750	24.9049

1 SCORDELIS - LO ROOF M - 8 MAY 12, 1968 RELEASE 1987 CDC PAGE 7

SORTED BULK DATA ECHO

14	3724	6.2500	20.4788
15	3608	6.2500	21.6586
16	5655	6.2500	22.5577
17	5785	6.2500	23.4923
18	4785	6.2500	24.1481
19	3412	6.2500	24.6282
20	1789	6.2500	24.9749
21	9209	6.2500	25.0000
22	1324	6.2500	25.0000
23	9487	6.2500	25.0000
24	8200	6.2500	25.0000
25	6455	6.2500	25.0000
26	4798	6.2500	25.0000
27	3412	6.2500	25.0000
28	1789	6.2500	25.0000
29	9209	6.2500	25.0000
30	1324	6.2500	25.0000
31	9487	6.2500	25.0000
32	8200	6.2500	25.0000
33	6455	6.2500	25.0000
34	4798	6.2500	25.0000
35	3412	6.2500	25.0000
36	1789	6.2500	25.0000
37	9209	6.2500	25.0000
38	1324	6.2500	25.0000
39	9487	6.2500	25.0000
40	8200	6.2500	25.0000
41	6455	6.2500	25.0000
42	4798	6.2500	25.0000
43	3412	6.2500	25.0000
44	1789	6.2500	25.0000
45	9209	6.2500	25.0000
46	1324	6.2500	25.0000
47	9487	6.2500	25.0000
48	8200	6.2500	25.0000
49	6455	6.2500	25.0000
50	4798	6.2500	25.0000
51	3412	6.2500	25.0000
52	1789	6.2500	25.0000
53	9209	6.2500	25.0000
54	1324	6.2500	25.0000
55	9487	6.2500	25.0000
56	8200	6.2500	25.0000
57	6455	6.2500	25.0000
58	4798	6.2500	25.0000
59	3412	6.2500	25.0000
60	1789	6.2500	25.0000
61	9209	6.2500	25.0000
62	1324	6.2500	25.0000
63	9487	6.2500	25.0000
64	8200	6.2500	25.0000
65	6455	6.2500	25.0000
66	4798	6.2500	25.0000
67	3412	6.2500	25.0000
68	1789	6.2500	25.0000
69	9209	6.2500	25.0000
70	1324	6.2500	25.0000
71	9487	6.2500	25.0000

SCORPION - LC 6008 N. 8

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SORTED BULK DATA ECHO

1	14	3724	6.2500	20.4788
2	15	3608	6.2500	21.6586
3	16	5655	6.2500	22.5577
4	17	5785	6.2500	23.4923
5	18	4785	6.2500	24.1481
6	19	3412	6.2500	24.6282
7	20	1789	6.2500	24.9749
8	21	9209	6.2500	25.0000
9	22	1324	6.2500	25.0000
10	23	9487	6.2500	25.0000
11	24	8200	6.2500	25.0000
12	25	6455	6.2500	25.0000
13	26	4798	6.2500	25.0000
14	27	3412	6.2500	25.0000
15	28	1789	6.2500	25.0000
16	29	9209	6.2500	25.0000
17	30	1324	6.2500	25.0000
18	31	9487	6.2500	25.0000
19	32	8200	6.2500	25.0000
20	33	6455	6.2500	25.0000
21	34	4798	6.2500	25.0000
22	35	3412	6.2500	25.0000
23	36	1789	6.2500	25.0000
24	37	9209	6.2500	25.0000
25	38	1324	6.2500	25.0000
26	39	9487	6.2500	25.0000
27	40	8200	6.2500	25.0000
28	41	6455	6.2500	25.0000
29	42	4798	6.2500	25.0000
30	43	3412	6.2500	25.0000
31	44	1789	6.2500	25.0000
32	45	9209	6.2500	25.0000
33	46	1324	6.2500	25.0000
34	47	9487	6.2500	25.0000
35	48	8200	6.2500	25.0000
36	49	6455	6.2500	25.0000
37	50	4798	6.2500	25.0000
38	51	3412	6.2500	25.0000
39	52	1789	6.2500	25.0000
40	53	9209	6.2500	25.0000
41	54	1324	6.2500	25.0000
42	55	9487	6.2500	25.0000
43	56	8200	6.2500	25.0000
44	57	6455	6.2500	25.0000
45	58	4798	6.2500	25.0000
46	59	3412	6.2500	25.0000
47	60	1789	6.2500	25.0000
48	61	9209	6.2500	25.0000
49	62	1324	6.2500	25.0000
50	63	9487	6.2500	25.0000
51	64	8200	6.2500	25.0000
52	65	6455	6.2500	25.0000
53	66	4798	6.2500	25.0000
54	67	3412	6.2500	25.0000
55	68	1789	6.2500	25.0000
56	69	9209	6.2500	25.0000
57	70	1324	6.2500	25.0000
58	71	9487	6.2500	25.0000

SCORPION - LC 6008 N. 8

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SCORPION - LC 6008 N. 8

PROBLEM #6

CARD	COUNT	1	2	3	4	5	6	7	8	9	10
137-		GRID	72		4	21.8750	25.0000				
138-		GRID	73	16.0697	25.0000	19.1511					
139-		GRID	74	14.3394	25.0000	20.4788					
140-		GRID	75	12.5000	25.0000	21.6506					
141-		GRID	76	10.5658	25.0000	22.8577					
142-		GRID	77	8.5506	25.0000	23.4823					
143-		GRID	78	6.4705	25.0000	24.1481					
144-		GRID	79	4.3412	25.0000	24.8302					
145-		GRID	80	2.1789	25.0000	24.5249					
146-		GRID	81	-.0000	25.0000	23.0000					
147-		DATA	1	4.38 E+0	1	11.18012	1	1.0	.9		
148-		PSHELL	1	1	THRU	81	4	5	6	+BCD	
149-		SPC1	5	6	2	3	4	5	6	+CDE	
150-		SPC1	5	13	1	1	4	5	6	+ABC	
151-		+BCD	5	8	9	18	36	45	54		
152-		SPC1	5	15	9	27	76	77	78		
153-		+CDE	63	72	81	75	76	77	78		
154-		SPC1	5	24	73	74	75	76	77		
155-		+ABC	79	80	81	81	81	81	81		
		ENDDATA									

0 77

10 UAI, NASTRAN PROBLEM 1

SOL 110
MFR DISP
TIME 30
CEND
LAMINATED COMPOSITE PLATE
PURE TUIST LOADING

MARCH 22, 1989 RELEASE 1988 CDC PAGE 2

CASE CONTROL DECK ECHO

CARD
COUNT

1 TITLE = LAMINATED COMPOSITE PLATE
2 SUBTITLE = PURE TUIST LOADING

3
4
5 MODEL = A SQUARE PLATE OF A 4X4 MESH WITH THREE CORNERS
6 PINNED AND A TRANSVERSE POINT LOAD AT THE FREE
7 CORNER TO SIMULATE A PURE TUIST LOADING. THE
8 LAMINATE LAYUP IS OF A CROSS-PLY CONFIGURATION
9 [0/90/0].

10 1 1 COMPARISON OF T3 DEFLECTION AT GRID 1 1 1

11 UAI/NASTRAN MSC/NASTRAN THEORETICAL
12 -3.758E-2 -3.763E-2 -3.750E-2

13 1 2 COMPARISON OF TAU FOR ELEMENT 1, ALL LAYERS 1 1

14 UAI/NASTRAN MSC/NASTRAN THEORETICAL
15 -5.0E1 -5.0E1 -5.0E1
16 0.0 0.0 0.0
17 5.0E1 5.0E1 5.0E1

18 REFERENCES: JONES R. M., MECHANICS OF COMPOSITE MATERIALS.
19 H. GRAU-HILL BOOK COMPANY. (PAGE 181)

20 SPC = 1
21 SURCHSE 1
22 LABEL = LAYER STRESS REQUEST

23 LAMINATED COMPOSITE PLATE
24 PURE TUIST LOADING

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CASE CONTROL DECK ECHO

CARD
COUNT

25 DISP = ALL
26 STRESS(LAYER) = ALL
27 FORCE = ALL
28 LOAD = 1
29 BEGIN BULK
30 LAMINATED COMPOSITE PLATE
31 PURE TUIST LOADING

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32 USER INFORMATION MESSAGE 207, BULK DATA NOT SORTED, XSORT WILL RE-ORDER DECK.
33 LAMINATED COMPOSITE PLATE

MARCH 22, 1989 RELEASE 1988 CDC PAGE 5


```

13- COUADA 12 1 13 18 19 21 14 1
14- COUADA 13 1 18 23 21 19 1
15- COUADA 14 1 4 9 10 5 1
16- COUADA 15 1 9 14 15 10 1
17- COUADA 16 1 1 19 20 15 1
18- COUADA 17 1 19 24 25 20 1
19- GRID 1 0.000 0.000 0.000 0.000
20- GRID 2 0.000 0.250 0.000 0.000
21- GRID 3 0.000 0.000 0.000 0.000
22- GRID 4 0.000 0.750 0.000 0.000
23- GRID 5 0.000 1.000 0.000 0.000
24- GRID 6 0.250 0.000 0.000 0.000
25- GRID 7 0.250 0.250 0.000 0.000
26- GRID 8 0.250 0.500 0.000 0.000
27- GRID 9 0.250 0.750 0.000 0.000
28- GRID 10 0.250 1.000 0.000 0.000
29- GRID 11 0.500 0.000 0.000 0.000
30- GRID 12 0.500 0.250 0.000 0.000
31- GRID 13 0.500 0.500 0.000 0.000
32- GRID 14 0.500 0.750 0.000 0.000
33- GRID 15 0.500 1.000 0.000 0.000
34- GRID 16 0.750 0.000 0.000 0.000

```

1 COMP01 *** UAI *** QUADA4 FLAT PLATE TEST

0 MESH 4X4 , ASPECT RATIO 1.0 SYMM CROSS-PLY [0/90/0]

0

CARD	COUNT	---1---	++2++	---3---	+++4+++	---5---	+++6+++	---7---	+++8+++	---9---	+++10+++
35-	GRID 17				.750	.250	0.000				
36-	GRID 18				.750	.500	0.000				
37-	GRID 19				.750	.750	0.000				
38-	GRID 20				.750	1.000	0.000				
39-	GRID 21				1.000	0.000	0.000				
40-	GRID 22				1.000	.250	0.000				
41-	GRID 23				1.000	.500	0.000				
42-	GRID 24				1.000	.750	0.000				
43-	GRID 25				1.000	1.000	0.000				
44-	MAT8 1				20.0E+06.50	E+6.25	.250 E+6				
45-	PCOMP 1				-.001						
46-	+PC1 1				.000666	0.0	YES			.000666	90.0
47-	+PC2 1				.000666	0.0	YES				
48-	PLOAD4 1				2	-1.0E-04					
49-	SPC1 1				6	1	THRU				17
50-	SPC1 1				15	22	23				
51-	SPC1 1				24	10	15				
52-	SPC1 1				1234	2	3				5
53-	SPC1 1				1235	6	11				
54-	SPC1 1				1245	6	16				
55-	SPC1 1				12345	25	21				
	ENDDATA										

ID UAI,MASTRAN PROBLEM 4
 SOL 1.0
 APP DISP
 TIME 30
 CEND

1 COMP04 *** UAI *** QUAD4 4-NODE STRAIGHT BEAM TEST MARCH 22, 1989 RELEASE 1988 CDC PAGE 2

0 REGULAR SHAPE ELEMENTS (ISOTROPIC PROPERTIES)
 0 CASE CONTROL DECK ECHO

CARD
 COUNT

1 TITLE * COMP04 *** UAI *** QUAD4 4-NODE STRAIGHT BEAM TEST
 2 LABEL * REGULAR SHAPE ELEMENTS (ISOTROPIC PROPERTIES)
 3
 4
 5 MODEL: CANTILEVERED BEAM MODEL UNDER A) EXTENSIONAL AND
 6 B) BENDING LOADINGS. SIMULATION OF EQUIVALENT
 7 ISOTROPIC PROPERTIES. LAMINATE CONFIGURATION
 8 [0/0/0/0]
 9
 10 ** COMPARISON OF T1 DEFLECTION AT GRIDS 13 AND 14 **

UAI/MASTRAN	MSC/MASTRAN	THEORETICAL

SUBCASE 1 (EXTENSIONAL)		
GRID 13	2.986E-5	3.0E-5
GRID 14	2.986E-5	3.0E-5

11
 12
 13
 14
 15
 16
 17
 18
 19
 20
 21
 22
 23
 24
 25
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 27
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 29
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 31
 32
 33
 34

1 ** COMPARISON OF T3 DEFLECTION AT GRIDS 13 AND 14 **

UAI/MASTRAN	MSC/MASTRAN	THEORETICAL

SUBCASE 2 (BENDING)		
GRID 13	4.253E-1	4.320E-1
GRID 14	4.253E-1	4.320E-1

35
 36
 37
 38
 39
 40
 41
 42
 43
 44
 45
 46

1 ** COMPARISON OF BENDING MOMENT DISTRIBUTION FROM **
 2 ** THE FREE END TO THE CANTILEVERED END **
 3 NOTE: THE BENDING MOMENTS ARE AT THE ELEMENT CENTER

UAI/MASTRAN	MSC/MASTRAN	THEORETICAL

SUBCASE 3 (BENDING)		
2.500E0		
2.500E0		

1 COMP04 *** UAI *** QUAD4 4-NODE STRAIGHT BEAM TEST MARCH 22, 1989 RELEASE 1988 CDC PAGE 3

0 REGULAR SHAPE ELEMENTS (ISOTROPIC PROPERTIES)
 0 CASE CONTROL DECK ECHO

CARD
 COUNT

1 TITLE * COMP04 *** UAI *** QUAD4 4-NODE STRAIGHT BEAM TEST
 2 LABEL * REGULAR SHAPE ELEMENTS (ISOTROPIC PROPERTIES)
 3
 4
 5 MODEL: CANTILEVERED BEAM MODEL UNDER A) EXTENSIONAL AND
 6 B) BENDING LOADINGS. SIMULATION OF EQUIVALENT
 7 ISOTROPIC PROPERTIES. LAMINATE CONFIGURATION
 8 [0/0/0/0]
 9
 10 ** COMPARISON OF DIRECT LAYER BENDING STRESS **
 11 ** ELEMENT 6 (LARGEST BENDING MOMENT) **

UAI/MASTRAN	MSC/MASTRAN

SUBCASE 4 (DIRECT LAYER BENDING STRESS)	
ELEMENT 6	7.500E0
ELEMENT 6	1.250E1
ELEMENT 6	1.750E1
ELEMENT 6	2.250E1
ELEMENT 6	2.750E1

12
 13
 14
 15
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 17
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 19
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 41
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 45
 46

1 ** COMPARISON OF DIRECT LAYER BENDING STRESS **
 2 ** ELEMENT 6 (LARGEST BENDING MOMENT) **

UAI/MASTRAN	MSC/MASTRAN

SUBCASE 4 (DIRECT LAYER BENDING STRESS)	
ELEMENT 6	7.500E0
ELEMENT 6	1.250E1
ELEMENT 6	1.750E1
ELEMENT 6	2.250E1
ELEMENT 6	2.750E1

35
 36
 37
 38
 39
 40
 41
 42
 43
 44
 45
 46

46 1.238E4
 47 1.238E4
 48 4.125E3
 49 4.125E3
 50 -4.125E3
 51 -1.238E4
 52
 53
 54 STRESS(LAYER) = ALL
 55 DISP = ALL
 56 FORCE = ALL
 57 SPC = 1
 58 SUBCASE 1
 59 SUBTITLE = EXTENSION
 60 LOAD = 1
 61 SUBCASE 2
 62 SUBTITLE = OUT-OF-PLANE SHEAR
 63 LOAD = 2
 64 BEGIN BULK
 65 COMP04 *** QUAD4 4-NODE STRAIGHT BEAM TEST

1 MARCH 22, 1989 RELEASE 1988 CDC PAGE 4
 *** USER INFORMATION MESSAGE 207, BULK DATA NOT SORTED, XSORT WILL RE-ORDER DECK.
 1 COMP04 *** QUAD4 4-NODE STRAIGHT BEAM TEST MARCH 22, 1989 RELEASE 1988 CDC PAGE 5
 0 REGULAR SHAPE ELEMENTS (ISOTROPIC PROPERTIES)

S O R T E D B U L K D A T A E C H O

CARD	COUNT	---	1---	++2+++	---3---	0.0	++4+++	---5---	0.0	++6+++	---7---	++8+++	---9---	++10+++
1-	CORD2R	1	0.0	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2-	+ABC	1	0.0	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3-	QUAD4	1	1	3	5	7	9	10	12	14	2	0.0	0.0	0.0
4-	QUAD4	2	1	5	7	9	11	13	14	4	1.0	0.0	0.0	0.0
5-	QUAD4	3	1	7	9	11	13	14	3	0.5	1.0	0.0	0.0	0.0
6-	QUAD4	4	1	9	11	13	14	1	0.5	1.0	0.0	0.0	0.0	0.0
7-	QUAD4	5	1	11	13	14	1	0.5	0.5	0.0	0.0	0.0	0.0	0.0
8-	QUAD4	6	1	13	14	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
9-	FORCE	1	13	14	14	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
10-	FORCE	2	13	14	14	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
11-	FORCE	3	13	14	14	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
12-	FORCE	4	13	14	14	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
13-	GRID	1	2	2	2	2	2	2	2	2	2	2	2	2
14-	GRID	2	2	2	2	2	2	2	2	2	2	2	2	2
15-	GRID	3	2	2	2	2	2	2	2	2	2	2	2	2
16-	GRID	4	2	2	2	2	2	2	2	2	2	2	2	2
17-	GRID	5	2	2	2	2	2	2	2	2	2	2	2	2
18-	GRID	6	2	2	2	2	2	2	2	2	2	2	2	2
19-	GRID	7	2	2	2	2	2	2	2	2	2	2	2	2
20-	GRID	8	2	2	2	2	2	2	2	2	2	2	2	2
21-	GRID	9	2	2	2	2	2	2	2	2	2	2	2	2
22-	GRID	10	2	2	2	2	2	2	2	2	2	2	2	2
23-	GRID	11	2	2	2	2	2	2	2	2	2	2	2	2
24-	GRID	12	2	2	2	2	2	2	2	2	2	2	2	2
25-	GRID	13	2	2	2	2	2	2	2	2	2	2	2	2
26-	GRID	14	2	2	2	2	2	2	2	2	2	2	2	2
27-	MAT1	1	.100E+08	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
28-	PCOMP2	1	0.0	0.025	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
29-	+PC1	1	0.0	0.025	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
30-	SPC1	1	6	1	6	1	6	1	6	1	6	1	6	1
31-	SPC1	1	123456	1	123456	1	123456	1	123456	1	123456	1	123456	1
	ENDDATA													

NASTRAN EXECUTIVE CONTROL DECK ECHO

15 COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1990
APP DISPLACEMENT
SOL 110
TYPE 10
TIME 10
SENO

CASE CONTROL DECK ECHO

CARD
COUNT
1 TITLE = PROBLEM 13 CASE 3 COMBINED LOADING
2 LOAD = 1
3 SPC = 1
4 OUTPUT
5 QUAD=ALL
6 STRESS=ALL
7 DISPLACEMENTS=ALL
8 STRAIN=ALL
9 BEGIN BULK

CARD COUNT	---	1----	+++2+++	---3---	+++4+++	---5---	+++6+++	---7---	+++8+++	---9---	+++10+++
1	CORP2R	1	1.0	0.0	0.0	0.0	0.0	0.0	0.0	1.0	+XYP
2	+XYP	1	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
3	QUAD4	1	1	1	2	3	4	5	6	7	8
4	QUAD4	2	1	1	3	4	5	6	7	8	9
5	QUAD4	3	1	1	4	5	6	7	8	9	10
6	QUAD4	4	1	1	5	6	7	8	9	10	11
7	QUAD4	5	1	1	6	7	8	9	10	11	12
8	QUAD4	6	1	1	7	8	9	10	11	12	13
9	QUAD4	7	1	1	8	9	10	11	12	13	14
10	QUAD4	8	1	1	9	10	11	12	13	14	15
11	QUAD4	9	1	1	10	11	12	13	14	15	16
12	QUAD4	10	1	1	11	12	13	14	15	16	17
13	QUAD4	11	1	1	12	13	14	15	16	17	18
14	QUAD4	12	1	1	13	14	15	16	17	18	19
15	QUAD4	13	1	1	14	15	16	17	18	19	20
16	QUAD4	14	1	1	15	16	17	18	19	20	21
17	QUAD4	15	1	1	16	17	18	19	20	21	22
18	QUAD4	16	1	1	17	18	19	20	21	22	23
19	QUAD4	17	1	1	18	19	20	21	22	23	24
20	QUAD4	18	1	1	19	20	21	22	23	24	25
21	QUAD4	19	1	1	20	21	22	23	24	25	26
22	QUAD4	20	1	1	21	22	23	24	25	26	27
23	QUAD4	21	1	1	22	23	24	25	26	27	28
24	QUAD4	22	1	1	23	24	25	26	27	28	29
25	QUAD4	23	1	1	24	25	26	27	28	29	30
26	QUAD4	24	1	1	25	26	27	28	29	30	31
27	QUAD4	25	1	1	26	27	28	29	30	31	32
28	QUAD4	26	1	1	27	28	29	30	31	32	33
29	QUAD4	27	1	1	28	29	30	31	32	33	34
30	QUAD4	28	1	1	29	30	31	32	33	34	35
31	QUAD4	29	1	1	30	31	32	33	34	35	
32	QUAD4	30	1	1	31	32	33	34	35		
33	QUAD4	31	1	1	32	33	34	35			
34	QUAD4	32	1	1	33	34	35				
35	QUAD4	33	1	1	34	35					
36	QUAD4	34	1	1	35						
37	QUAD4	35	1	1							
38	QUAD4	36	1	1							
39	QUAD4	37	1	1							
40	QUAD4	38	1	1							
41	QUAD4	39	1	1							
42	QUAD4	40	1	1							
43	QUAD4	41	1	1							
44	QUAD4	42	1	1							
45	QUAD4	43	1	1							
46	QUAD4	44	1	1							
47	QUAD4	45	1	1							
48	QUAD4	46	1	1							
49	QUAD4	47	1	1							
50	QUAD4	48	1	1							
51	QUAD4	49	1	1							
52	QUAD4	50	1	1							
53	QUAD4	51	1	1							
54	QUAD4	52	1	1							
55	QUAD4	53	1	1							
56	QUAD4	54	1	1							
57	QUAD4	55	1	1							
58	QUAD4	56	1	1							
59	QUAD4	57	1	1							
60	QUAD4	58	1	1							
61	QUAD4	59	1	1							
62	QUAD4	60	1	1							

APPENDIX E: NASTRAN INPUT AND OUTPUT FOR SAMPLE
PROBLEMS 7, 10, 12, 13a, 13b, 13d, 14, and 9 and 12 with TRIA3

1 NASTRAM EXECUTIVE CONTROL DECK ECHO

PROBLEM 47

18 NASTRAM, FISCHLER
APP DISK
VOL 1
LINE 9
CDC

CASE CONTROL DECK E C M G

CARD
COUNT

TITLE - SPHERICAL SHELL - N=8
DISP - ALL
STRESS - ALL
DLOAD - ALL
SUPPORTS - ALL
SRC - 5
SUBCASE 1
LOAD - 1
BEGIN BULK

Sorted Bulk Data Echo

Count	1	2	3	4	5	6	7	8	9
10000	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10001	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10002	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10003	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10004	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10005	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10006	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10007	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10008	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10009	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10010	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10011	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10012	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10013	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10014	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10015	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10016	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10017	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10018	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10019	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10020	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10021	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10022	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10023	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10024	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10025	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10026	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10027	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10028	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10029	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10030	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10031	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10032	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10033	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10034	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10035	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10036	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10037	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10038	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10039	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10040	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10041	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10042	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10043	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0
10044	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0

CARD COUNT	1	2	3	4	5	6	7	8	9	10
69-	CARD	1	10.00	90.00	.00	.00	.00	.00	.00	.00
70-	CARD	2	10.00	81.00	.00	.00	.00	.00	.00	.00
71-	CARD	3	10.00	72.00	.00	.00	.00	.00	.00	.00
72-	CARD	4	10.00	63.00	.00	.00	.00	.00	.00	.00
73-	CARD	5	10.00	54.00	.00	.00	.00	.00	.00	.00
74-	CARD	6	10.00	45.00	.00	.00	.00	.00	.00	.00
75-	CARD	7	10.00	36.00	.00	.00	.00	.00	.00	.00
76-	CARD	8	10.00	27.00	.00	.00	.00	.00	.00	.00
77-	CARD	9	10.00	18.00	.00	.00	.00	.00	.00	.00
78-	CARD	10	10.00	9.00	.00	.00	.00	.00	.00	.00
79-	CARD	11	10.00	11.25	11.25	.00	.00	.00	.00	.00
80-	CARD	12	10.00	22.50	22.50	11.25	.00	.00	.00	.00
81-	CARD	13	10.00	33.75	33.75	22.50	11.25	.00	.00	.00
82-	CARD	14	10.00	45.00	45.00	33.75	22.50	11.25	.00	.00
83-	CARD	15	10.00	56.25	56.25	45.00	33.75	22.50	11.25	.00
84-	CARD	16	10.00	67.50	67.50	56.25	45.00	33.75	22.50	11.25
85-	CARD	17	10.00	78.75	78.75	67.50	56.25	45.00	33.75	22.50
86-	CARD	18	10.00	90.00	90.00	78.75	67.50	56.25	45.00	33.75
87-	CARD	19	10.00	101.25	101.25	90.00	78.75	67.50	56.25	45.00
88-	CARD	20	10.00	112.50	112.50	101.25	90.00	78.75	67.50	56.25
89-	CARD	21	10.00	123.75	123.75	112.50	101.25	90.00	78.75	67.50
90-	CARD	22	10.00	135.00	135.00	123.75	112.50	101.25	90.00	78.75
91-	CARD	23	10.00	146.25	146.25	135.00	123.75	112.50	101.25	90.00
92-	CARD	24	10.00	157.50	157.50	146.25	135.00	123.75	112.50	90.00
93-	CARD	25	10.00	168.75	168.75	157.50	146.25	135.00	123.75	112.50
94-	CARD	26	10.00	180.00	180.00	168.75	157.50	146.25	135.00	123.75
95-	CARD	27	10.00	191.25	191.25	180.00	168.75	157.50	146.25	135.00
96-	CARD	28	10.00	202.50	202.50	191.25	180.00	168.75	157.50	146.25
97-	CARD	29	10.00	213.75	213.75	202.50	191.25	180.00	168.75	157.50
98-	CARD	30	10.00	225.00	225.00	213.75	202.50	180.00	168.75	157.50
99-	CARD	31	10.00	236.25	236.25	225.00	213.75	202.50	180.00	168.75
100-	CARD	32	10.00	247.50	247.50	236.25	225.00	213.75	202.50	180.00
101-	CARD	33	10.00	258.75	258.75	247.50	236.25	225.00	213.75	202.50
102-	CARD	34	10.00	270.00	270.00	258.75	247.50	236.25	225.00	213.75

1 SPHERICAL SHELL - N=8
 0
 0

CARD COUNT	1	2	3	4	5	6	7	8	9	10
137-	GRID	69	10.00	46.00	78.75	78.75	78.75	78.75	78.75	78.75
138-	GRID	70	10.00	36.00	78.75	78.75	78.75	78.75	78.75	78.75
139-	GRID	71	10.00	27.00	78.75	78.75	78.75	78.75	78.75	78.75
140-	GRID	72	10.00	18.00	90.00	90.00	90.00	90.00	90.00	90.00
141-	GRID	73	10.00	10.00	90.00	90.00	90.00	90.00	90.00	90.00
142-	GRID	74	10.00	10.00	81.00	90.00	90.00	90.00	90.00	90.00
143-	GRID	75	10.00	10.00	72.00	90.00	90.00	90.00	90.00	90.00
144-	GRID	76	10.00	10.00	63.00	90.00	90.00	90.00	90.00	90.00
145-	GRID	77	10.00	10.00	54.00	90.00	90.00	90.00	90.00	90.00
146-	GRID	78	10.00	10.00	45.00	90.00	90.00	90.00	90.00	90.00
147-	GRID	79	10.00	10.00	36.00	90.00	90.00	90.00	90.00	90.00
148-	GRID	80	10.00	10.00	27.00	90.00	90.00	90.00	90.00	90.00
149-	GRID	81	10.00	10.00	18.00	90.00	90.00	90.00	90.00	90.00
150-	MATL	1	6.88E07	1.3	1.0	1.0	1.0	1.0	1.0	0.0
151-	PSHELL	1	1	1	1	1	1	1	1	0.0
152-	SPCI	5	9	74	75	76	77	78	78	+XYZ
153-	SPCI	5	156	156	156	156	156	156	156	+BCD
154-	+XYZ	79	80	81	81	81	81	81	81	
155-	SPCI	5	246	246	246	246	246	246	246	
156-	+BCD	7	8	8	8	8	8	8	8	
156-	ENDDATA	7	8	8	8	8	8	8	8	

SUBCASE 1

POINT ID.	TYPE	DISPLACEMENT VECTOR								
		T1	T2	T3	R1	R2	R3			
1	G	9.544737E-02	.0	4.349715E-02	.0	.0	.0	-1.965434E-02	.0	.0
2	G	6.78819E-02	.0	4.13094E-02	.0	.0	.0	-1.563841E-02	.0	.0
3	G	4.695509E-02	.0	3.630303E-02	.0	.0	.0	-1.284450E-02	.0	.0
4	G	3.12333E-02	.0	2.979189E-02	.0	.0	.0	-9.686679E-03	.0	.0
5	G	1.969743E-02	.0	2.246792E-02	.0	.0	.0	-7.789540E-03	.0	.0
6	G	1.137281E-02	.0	1.633621E-02	.0	.0	.0	-6.106912E-03	.0	.0
7	G	5.802311E-03	.0	3.818661E-03	.0	.0	.0	-4.663406E-03	.0	.0
8	G	2.818199E-03	.0	3.875984E-03	.0	.0	.0	-3.311765E-03	.0	.0
9	G	1.012649E-03	.0	.0	.0	.0	.0	-2.302308E-03	.0	.0
10	G	8.770363E-02	-7.785390E-04	3.949185E-02	1.878509E-05	3.949185E-02	1.878509E-05	-1.662371E-02	7.472394E-03	4.415313E-03
11	G	6.319187E-02	-4.582560E-04	3.769480E-02	-1.690150E-03	3.769480E-02	-1.690150E-03	-1.448311E-02	4.415313E-03	2.913439E-03
12	G	4.398692E-02	-8.955133E-04	3.340055E-02	-1.553613E-03	3.340055E-02	-1.553613E-03	-1.144960E-02	2.913439E-03	2.036608E-03
13	G	2.835179E-02	-1.843323E-04	2.715280E-02	-1.868348E-03	2.715280E-02	-1.868348E-03	-9.284242E-03	2.036608E-03	1.338199E-03
14	G	1.845959E-02	-1.136186E-04	2.001461E-02	-1.216408E-03	2.001461E-02	-1.216408E-03	-7.483871E-03	1.338199E-03	9.338001E-04
15	G	1.071653E-02	-6.453034E-05	1.413663E-02	-9.106048E-04	1.413663E-02	-9.106048E-04	-6.989461E-03	9.338001E-04	5.441539E-04
16	G	5.554975E-03	-3.358496E-05	8.216670E-03	-8.663939E-04	8.216670E-03	-8.663939E-04	-4.543639E-03	5.441539E-04	3.890113E-04
17	G	2.468774E-03	-1.413968E-05	3.283310E-03	-6.839354E-04	3.283310E-03	-6.839354E-04	-3.248951E-03	3.890113E-04	-5.332016E-04
18	G	9.551497E-04	-6.387433E-06	-2.948631E-04	-8.317373E-04	-2.948631E-04	-8.317373E-04	-8.335243E-03	-5.332016E-04	1.029095E-02
19	G	7.068362E-02	-5.943434E-03	2.879180E-02	-1.672673E-03	2.879180E-02	-1.672673E-03	-1.272369E-02	1.029095E-02	7.533821E-03
20	G	5.169261E-02	-3.937679E-03	2.747165E-02	-2.072313E-03	2.747165E-02	-2.072313E-03	-1.151334E-02	7.533821E-03	5.334102E-03
21	G	3.831221E-02	-8.626221E-03	2.410071E-02	-2.594324E-03	2.410071E-02	-2.594324E-03	-9.836334E-03	5.334102E-03	3.845292E-03
22	G	2.35236E-02	-1.709279E-03	1.978111E-02	-2.317123E-03	1.978111E-02	-2.317123E-03	-8.16242E-03	3.845292E-03	2.553011E-03
23	G	1.536196E-02	-1.053468E-03	1.481305E-02	-2.308811E-03	1.481305E-02	-2.308811E-03	-6.787670E-03	2.553011E-03	1.753933E-03
24	G	8.932318E-03	-6.057056E-04	9.88145E-03	-1.947311E-03	9.88145E-03	-1.947311E-03	-5.443695E-03	1.753933E-03	1.024766E-03
25	G	4.834611E-03	-3.131132E-04	5.368509E-03	-1.692643E-03	5.368509E-03	-1.692643E-03	-4.264298E-03	1.024766E-03	-9.709962E-04
26	G	2.060113E-03	-1.376029E-04	1.596790E-03	-1.297075E-03	1.596790E-03	-1.297075E-03	-3.067222E-03	-9.709962E-04	1.818583E-02
27	G	7.977308E-04	-5.477712E-05	-1.142203E-03	-1.541375E-03	-1.142203E-03	-1.541375E-03	-2.401565E-03	1.818583E-02	8.99065E-03
28	G	5.104025E-02	-1.644782E-02	1.360612E-02	-3.706443E-03	1.360612E-02	-3.706443E-03	-9.954810E-03	8.99065E-03	6.595649E-03
29	G	3.735272E-02	-1.159913E-02	1.292242E-02	-4.684048E-03	1.292242E-02	-4.684048E-03	-8.947507E-03	6.595649E-03	4.855648E-03
30	G	2.629284E-02	-7.982799E-03	1.119666E-02	-4.194769E-03	1.119666E-02	-4.194769E-03	-7.969223E-03	4.855648E-03	3.395106E-03
31	G	1.765761E-02	-5.278470E-03	8.84045E-03	-3.713324E-03	8.84045E-03	-3.713324E-03	-6.756233E-03	3.395106E-03	2.268499E-03
32	G	1.115223E-02	-3.303756E-03	6.202541E-03	-3.48526E-03	6.202541E-03	-3.48526E-03	-5.890237E-03	2.268499E-03	1.343873E-03
33	G	6.488834E-03	-1.911786E-03	3.591389E-03	-2.984866E-03	3.591389E-03	-2.984866E-03	-4.736257E-03	1.343873E-03	.0
34	G	3.368579E-03	-9.900306E-04	1.113423E-03	-2.472434E-03	1.113423E-03	-2.472434E-03	-3.792770E-03	.0	.0

SUBCASE 1

DISPLACEMENT VECTOR

POINT ID.	TYPE	T1	T2	T3	R1	R2	R3
36	0	1.497948E-03	-4.382182E-04	-9.254679E-04	-1.874798E-03	-2.771893E-03	9.367054E-04
37	0	5.865490E-04	-1.713210E-04	-2.407279E-03	-2.049751E-03	-2.439508E-03	-1.258584E-03
38	0	3.204137E-03	-3.204137E-02	-3.899490E-03	-6.398147E-03	-6.398147E-03	1.257742E-02
39	0	2.389766E-02	-2.329766E-02	-3.899490E-03	-6.197170E-03	-6.197170E-03	9.628137E-03
40	0	1.623622E-02	-1.623622E-02	-3.899490E-03	-6.066810E-03	-6.066810E-03	7.041978E-03
41	0	1.065837E-02	-1.065837E-02	-3.899490E-03	-5.285623E-03	-5.285623E-03	5.188747E-03
42	0	6.841495E-03	-6.841495E-03	-3.899490E-03	-4.851496E-03	-4.851496E-03	3.568677E-03
43	0	3.974918E-03	-3.974918E-03	-3.899490E-03	-3.887494E-03	-3.887494E-03	2.446879E-03
44	0	2.068214E-03	-2.068214E-03	-3.899490E-03	-3.186160E-03	-3.186160E-03	1.469518E-03
45	0	9.154102E-04	-9.154102E-04	-3.899490E-03	-2.376913E-03	-2.376913E-03	1.014390E-03
46	0	3.558834E-04	-3.558834E-04	-3.899490E-03	-2.336398E-03	-2.336398E-03	1.1347534E-03
47	0	1.644782E-04	-1.644782E-04	-3.899490E-03	-9.464210E-03	-9.464210E-03	1.218703E-03
48	0	1.159913E-04	-1.159913E-04	-3.899490E-03	-8.947607E-03	-8.947607E-03	8.998655E-03
49	0	7.982798E-03	-7.982798E-03	-1.899548E-02	-7.958883E-03	-7.958883E-03	8.5856448E-03
50	0	5.278479E-03	-1.785781E-02	-1.664394E-02	-6.780233E-03	-3.713324E-03	4.855542E-03
51	0	3.303766E-03	-1.115223E-02	-1.400154E-02	-5.800287E-03	-4.004049E-03	3.305100E-03
52	0	1.911786E-03	-8.488834E-03	-1.136039E-02	-4.730257E-03	-2.924866E-03	2.268499E-03
53	0	9.980366E-04	-3.368579E-03	-8.912421E-03	-3.792770E-03	-2.472434E-03	1.343273E-03
54	0	4.382122E-04	-1.497948E-03	-6.873796E-03	-2.771893E-03	-1.874798E-03	9.367854E-04
55	0	1.713210E-04	-5.865490E-04	-5.301718E-03	-2.439508E-03	-2.049751E-03	1.258584E-03
56	0	5.943434E-03	-7.068323E-02	-3.659086E-02	-1.278289E-02	-1.578583E-02	1.629886E-02
57	0	3.937679E-03	-5.169281E-02	-3.197971E-02	-1.161336E-02	-2.072313E-03	7.573821E-03
58	0	1.700279E-03	-2.435236E-02	-2.755010E-02	-9.162462E-03	-2.504324E-03	5.334102E-03
59	0	1.653468E-04	-1.536196E-02	-2.261206E-02	-6.787676E-03	-2.317133E-03	3.845292E-03
60	0	6.057056E-04	-8.932311E-03	-1.769614E-02	-5.443096E-03	-2.308881E-03	2.553011E-03
61	0	3.131132E-04	-2.060113E-03	-1.316751E-02	-4.264295E-03	-1.947311E-03	1.753932E-03
62	0	1.376899E-04	-2.060113E-03	-9.395787E-03	-3.067222E-03	-1.297079E-03	7.199098E-04
63	0	5.477712E-05	-7.977398E-04	-6.666795E-03	-2.401555E-03	-1.541379E-03	9.769962E-04
64	0	7.726308E-04	-8.770363E-02	-4.729052E-02	-1.662371E-02	-1.541379E-03	7.472394E-03
65	0	4.552569E-04	-6.319187E-02	-4.540366E-02	-1.448311E-02	-1.899490E-06	4.415313E-03
66	0	2.955313E-04	-4.398689E-02	-4.088654E-02	-1.144960E-02	-1.853613E-03	2.913439E-03
67	0	1.843323E-04	-2.976179E-02	-3.496196E-02	-9.284242E-03	-1.206348E-03	2.630088E-03
68	0	1.135186E-04	-1.845969E-02	-2.841361E-02	-7.483871E-03	-1.216568E-03	1.308100E-03

SUBCASE 1

DISPLACEMENT VECTOR

POINT ID.	TYPE	T1	T2	T3	R1	R2	R3
69	Q	6.452034E-05	-1.071653E-02	-2.193663E-02	-5.920461E-03	-9.990048E-04	9.338091E-04
70	Q	3.358406E-06	-5.555075E-03	-1.601567E-02	-4.563686E-03	-8.668938E-04	5.441538E-04
71	Q	1.413006E-06	-8.468774E-03	-1.108231E-02	-3.248951E-03	-8.639364E-04	3.890113E-04
72	Q	6.387433E-06	-9.551497E-04	-7.502144E-03	-8.336243E-03	-8.317373E-04	-5.338016E-04
73	Q	.0	-9.544737E-02	-5.120615E-02	-1.966434E-02	.0	.0
74	Q	.0	-6.788819E-02	-4.918783E-02	-1.563841E-02	.0	.0
75	Q	.0	-4.695506E-02	-4.410093E-02	-1.294459E-02	.0	.0
76	Q	.0	-3.183308E-02	-3.759028E-02	-9.896879E-03	.0	.0
77	Q	.0	-1.960743E-02	-3.046699E-02	-7.780640E-03	.0	.0
78	Q	.0	-1.137281E-02	-2.343421E-02	-6.100912E-03	.0	.0
79	Q	.0	-6.892211E-03	-1.791756E-02	-4.643486E-03	.0	.0
80	Q	.0	-2.618199E-03	-1.167496E-02	-3.311705E-03	.0	.0
81	Q	.0	-1.012620E-03	-7.790907E-03	-2.368206E-03	.0	.0

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SUBJECT :

LOAD VECTOR

01	00	00	00	00	00
02	00	00	00	00	00
03	00	00	00	00	00
04	00	00	00	00	00
05	00	00	00	00	00
06	00	00	00	00	00
07	00	00	00	00	00
08	00	00	00	00	00
09	00	00	00	00	00
10	00	00	00	00	00
11	00	00	00	00	00
12	00	00	00	00	00
13	00	00	00	00	00
14	00	00	00	00	00
15	00	00	00	00	00
16	00	00	00	00	00
17	00	00	00	00	00
18	00	00	00	00	00
19	00	00	00	00	00
20	00	00	00	00	00

01	00	00	00	00	00
02	00	00	00	00	00
03	00	00	00	00	00
04	00	00	00	00	00
05	00	00	00	00	00
06	00	00	00	00	00
07	00	00	00	00	00
08	00	00	00	00	00
09	00	00	00	00	00
10	00	00	00	00	00
11	00	00	00	00	00
12	00	00	00	00	00
13	00	00	00	00	00
14	00	00	00	00	00
15	00	00	00	00	00
16	00	00	00	00	00
17	00	00	00	00	00
18	00	00	00	00	00
19	00	00	00	00	00
20	00	00	00	00	00

01	00	00	00	00	00
02	00	00	00	00	00
03	00	00	00	00	00
04	00	00	00	00	00
05	00	00	00	00	00
06	00	00	00	00	00
07	00	00	00	00	00
08	00	00	00	00	00
09	00	00	00	00	00
10	00	00	00	00	00
11	00	00	00	00	00
12	00	00	00	00	00
13	00	00	00	00	00
14	00	00	00	00	00
15	00	00	00	00	00
16	00	00	00	00	00
17	00	00	00	00	00
18	00	00	00	00	00
19	00	00	00	00	00
20	00	00	00	00	00

01	00	00	00	00	00
02	00	00	00	00	00
03	00	00	00	00	00
04	00	00	00	00	00
05	00	00	00	00	00
06	00	00	00	00	00
07	00	00	00	00	00
08	00	00	00	00	00
09	00	00	00	00	00
10	00	00	00	00	00
11	00	00	00	00	00
12	00	00	00	00	00
13	00	00	00	00	00
14	00	00	00	00	00
15	00	00	00	00	00
16	00	00	00	00	00
17	00	00	00	00	00
18	00	00	00	00	00
19	00	00	00	00	00
20	00	00	00	00	00

SUBCASE 1
(CQUAD4)

RESULTS IN GENERAL QUADRILATERAL ELEMENTS (CQUAD4)

ELEMENT	I	J	K	L	STRESSES IN STRESS COORDINATE SYSTEM		SHELL STRESSES (ZERO SHEAR)		ANGLE	PRINCIPAL STRESSES (ZERO SHEAR)	MINOR	MAX
					NORMAL - X	NORMAL - Y	MINOR	MAX				
1	1	2	3	4	1.25145E+03	3.753104E+03	-9.54372E+02	3.932077E+03	-79.8574	-1.57725E+03	8.72875E+02	8.72875E+02
2	1	2	3	4	1.25145E+03	3.753104E+03	8.54230E+02	1.23235E+03	18.8508	-4.886287E+03	8.77684E+02	8.77684E+02
3	1	2	3	4	1.25145E+03	3.753104E+03	4.52750E+02	8.43472E+03	-94.1491	-8.251843E+03	8.87826E+02	8.87826E+02
4	1	2	3	4	1.25145E+03	3.753104E+03	4.82337E+02	4.82337E+02	6.8868	-5.154734E+02	1.081501E+03	1.081501E+03
5	1	2	3	4	1.25145E+03	3.753104E+03	2.48878E+02	1.48878E+03	-25.4874	-1.47884E+02	1.552752E+02	1.552752E+02
6	1	2	3	4	1.25145E+03	3.753104E+03	1.38340E+02	1.38340E+02	4.3846	-1.757743E+02	1.38340E+02	1.38340E+02
7	1	2	3	4	1.25145E+03	3.753104E+03	1.48984E+02	1.48984E+02	-98.8434	-1.85819E+03	1.38340E+02	1.38340E+02
8	1	2	3	4	1.25145E+03	3.753104E+03	8.69331E+01	1.864289E+03	3.8334	-1.34894E+03	1.38340E+02	1.38340E+02
9	1	2	3	4	1.25145E+03	3.753104E+03	2.15436E+01	1.12878E+03	-87.8883	-1.64788E+02	1.081501E+03	1.081501E+03
10	1	2	3	4	1.25145E+03	3.753104E+03	4.87617E+01	8.62818E+02	8.1705	-1.152687E+03	1.081501E+03	1.081501E+03
11	1	2	3	4	1.25145E+03	3.753104E+03	4.26748E+01	8.26748E+01	-88.7218	-8.26748E+02	8.62818E+02	8.62818E+02
12	1	2	3	4	1.25145E+03	3.753104E+03	8.64863E+01	8.64863E+01	-5.7315	-1.66872E+03	8.62818E+02	8.62818E+02
13	1	2	3	4	1.25145E+03	3.753104E+03	8.25489E+01	8.25489E+01	8.2548	-8.80657E+02	8.13479E+02	8.13479E+02
14	1	2	3	4	1.25145E+03	3.753104E+03	1.67241E+01	1.67241E+01	83.2382	-8.26524E+02	8.13479E+02	8.13479E+02
15	1	2	3	4	1.25145E+03	3.753104E+03	1.77651E+01	1.77651E+01	-1.6265	-5.41414E+02	8.13479E+02	8.13479E+02
16	1	2	3	4	1.25145E+03	3.753104E+03	1.87619E+01	1.87619E+01	-63.7877	-7.88723E+02	7.88723E+02	7.88723E+02
17	1	2	3	4	1.25145E+03	3.753104E+03	1.85294E+01	1.85294E+01	25.8218	-8.884401E+02	8.26524E+02	8.26524E+02
18	1	2	3	4	1.25145E+03	3.753104E+03	1.21049E+01	1.21049E+01	-71.6563	-8.659817E+02	8.26524E+02	8.26524E+02
19	1	2	3	4	1.25145E+03	3.753104E+03	1.24361E+01	1.24361E+01	19.1837	-8.86844E+02	8.26524E+02	8.26524E+02
20	1	2	3	4	1.25145E+03	3.753104E+03	8.48039E+01	8.48039E+01	-73.1835	-1.58178E+03	8.26524E+02	8.26524E+02
21	1	2	3	4	1.25145E+03	3.753104E+03	1.44024E+01	1.44024E+01	16.4453	-1.45178E+02	1.554287E+02	1.554287E+02

GENERAL STRESS COORDINATE SYSTEM (G.C.S.) SUBCASE 1

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 1-10.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 11-20.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 21-30.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 31-40.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 41-50.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 51-60.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 61-70.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 71-80.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 81-90.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 91-100.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 101-110.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 111-120.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 121-130.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 131-140.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 141-150.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 151-160.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 161-170.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 171-180.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 181-190.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 191-200.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 201-210.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 211-220.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 221-230.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 231-240.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 241-250.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 251-260.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 261-270.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 271-280.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 281-290.

Table with columns: NODAL POINT, STRESS, X, Y, Z, U, V, W, X, Y, Z, U, V, W, X, Y, Z, U, V, W. Rows 291-300.

SUBCASE 1

STRESSES IN SHEAR QUADRILATERAL ELEMENTS (C O U M 5 4)

ELEMENT	STRESSES IN SHEAR COORD. SYSTEM			PRINCIPAL STRESSES (ZERO SHEAR)			MAX MINOR
	MEMBER X	MEMBER Y	MEMBER Z	MAJOR	MINOR	MINOR	
1	1.26676E+02	3.56723E+02	5.35204E+02	8.69953E+02	-8.26097E+02	8.45235E+02	8.45235E+02
2	1.84433E+02	1.27115E+02	3.75828E+02	5.23467E+02	-5.65237E+02	2.49225E+02	2.49225E+02
3	1.57224E+02	3.75828E+02	-1.26676E+02	3.21819E+02	-2.37854E+02	5.55287E+02	5.55287E+02
4	1.26676E+02	5.35204E+02	4.48853E+02	3.48948E+02	-8.11883E+02	7.89278E+02	7.89278E+02
5	1.84433E+02	3.56723E+02	-3.75828E+02	2.12543E+02	-2.89979E+02	2.64255E+02	2.64255E+02
6	1.57224E+02	1.27115E+02	1.26676E+02	1.71881E+02	-1.78358E+02	1.75289E+02	1.75289E+02
7	1.26676E+02	3.56723E+02	1.26676E+02	1.71881E+02	-1.78358E+02	1.75289E+02	1.75289E+02
8	1.84433E+02	1.27115E+02	3.75828E+02	1.41452E+02	-1.41672E+02	1.41555E+02	1.41555E+02
9	1.57224E+02	3.75828E+02	-1.26676E+02	1.21453E+02	-1.29512E+02	1.28882E+02	1.28882E+02
10	1.26676E+02	5.35204E+02	4.48853E+02	1.26689E+02	-1.01134E+02	1.01113E+02	1.01113E+02
11	1.84433E+02	3.56723E+02	-3.75828E+02	1.84594E+02	-1.63123E+02	1.64757E+02	1.64757E+02
12	1.57224E+02	1.27115E+02	1.26676E+02	1.27983E+02	-1.32829E+02	1.31810E+02	1.31810E+02
13	1.26676E+02	3.56723E+02	1.26676E+02	3.29588E+02	-3.10485E+02	3.24817E+02	3.24817E+02
14	1.84433E+02	1.27115E+02	3.75828E+02	3.19322E+02	-3.11521E+02	3.16654E+02	3.16654E+02
15	1.57224E+02	3.75828E+02	-1.26676E+02	3.08241E+02	-2.78297E+02	3.11979E+02	3.11979E+02
16	1.26676E+02	5.35204E+02	4.48853E+02	4.60784E+02	-7.87529E+02	6.29724E+02	6.29724E+02
17	1.84433E+02	3.56723E+02	-3.75828E+02	2.86878E+02	-2.12343E+02	2.54125E+02	2.54125E+02
18	1.57224E+02	1.27115E+02	1.26676E+02	2.16846E+02	-2.16846E+02	2.16846E+02	2.16846E+02

STRESSES IN GENERAL QUADRILATERAL ELEMENTS (IN STRESS COORDINATE SYSTEM)

SUBCASE 1

ELEMENT	I	J	K	L	STRESS IN STRESS COORD. SYSTEM			PRINCIPAL STRESSES (ZERO SHEAR)			MAX SCALE
					NORMAL X	NORMAL Y	NORMAL Z	MAJOR	MINOR	SHEAR	
1	1	2	3	4	2.125715E+02	-8.285471E+01	1.894033E+03	1.768953E+03	-1.715214E+03	1.768953E+03	1.768953E+03
2	1	2	3	4	-2.493135E+02	2.867447E+02	1.764403E+03	1.715214E+03	-1.715214E+03	1.715214E+03	1.715214E+03
3	1	2	3	4	1.432303E+02	-1.414805E+02	-1.422870E+02	1.422870E+02	-1.414805E+02	1.422870E+02	1.422870E+02
4	1	2	3	4	1.774877E+02	-1.422870E+02	1.413442E+02	1.413442E+02	-1.413442E+02	1.413442E+02	1.413442E+02
5	1	2	3	4	3.718623E+01	-1.621621E+02	1.843854E+02	1.843854E+02	-1.621621E+02	1.843854E+02	1.843854E+02
6	1	2	3	4	6.524404E+01	7.875435E+01	1.843854E+02	1.843854E+02	-1.843854E+02	1.843854E+02	1.843854E+02
7	1	2	3	4	2.272035E+01	-7.882162E+01	1.843854E+02	1.843854E+02	-1.843854E+02	1.843854E+02	1.843854E+02
8	1	2	3	4	2.673664E+01	-1.871744E+01	1.843854E+02	1.843854E+02	-1.843854E+02	1.843854E+02	1.843854E+02
9	1	2	3	4	1.351365E+01	1.395241E+01	-9.82851E+01	9.82851E+01	-9.82851E+01	9.82851E+01	9.82851E+01
10	1	2	3	4	2.744323E+01	-1.654865E+01	1.843854E+02	1.843854E+02	-1.843854E+02	1.843854E+02	1.843854E+02
11	1	2	3	4	4.128792E+01	4.178163E+01	-1.843854E+02	-1.843854E+02	1.843854E+02	1.843854E+02	1.843854E+02
12	1	2	3	4	8.116206E+01	-1.843854E+02	1.843854E+02	1.843854E+02	-1.843854E+02	1.843854E+02	1.843854E+02
13	1	2	3	4	1.485481E+02	9.285811E+01	-9.82851E+01	9.82851E+01	-9.82851E+01	9.82851E+01	9.82851E+01
14	1	2	3	4	4.125481E+02	-9.82851E+01	9.82851E+01	9.82851E+01	-9.82851E+01	9.82851E+01	9.82851E+01
15	1	2	3	4	-4.327589E+02	9.82851E+01	-9.82851E+01	-9.82851E+01	9.82851E+01	-9.82851E+01	-9.82851E+01
16	1	2	3	4	8.128212E+02	-9.82851E+01	9.82851E+01	9.82851E+01	-9.82851E+01	9.82851E+01	9.82851E+01
17	1	2	3	4	9.096221E+02	9.82851E+01	-9.82851E+01	-9.82851E+01	9.82851E+01	-9.82851E+01	-9.82851E+01
18	1	2	3	4	6.344911E+02	-9.82851E+01	9.82851E+01	9.82851E+01	-9.82851E+01	9.82851E+01	9.82851E+01
19	1	2	3	4	-2.194911E+02	9.82851E+01	-9.82851E+01	-9.82851E+01	9.82851E+01	-9.82851E+01	-9.82851E+01
20	1	2	3	4	1.127285E+02	-9.82851E+01	9.82851E+01	9.82851E+01	-9.82851E+01	9.82851E+01	9.82851E+01

SUBCASE 1

STRESSES IN GENERAL QUADRILATERAL ELEMENTS (COUAD4)

ELEMENT ID.	FIBRE DISTANCE	STRESSES IN STRESS COORD. SYSTEM			PRINCIPAL STRESSES (ZERO SHEAR)			MAX SHEAR
		NORMAL-X	NORMAL-Y	SHEAR-XY	MAJOR	MINOR	ANGLE	
46	-2.00000E-02	4.451425E+02	-4.766131E+02	-9.51430E+02	1.041447E+03	-1.072017E+03	-38.0771	1.067183E+03
46	-2.00000E-02	-4.562843E+02	-4.803330E+02	9.546343E+02	1.077021E+03	-1.047878E+03	57.9810	1.042897E+03
46	-2.00000E-02	3.823891E+02	-4.183081E+02	-8.421855E+02	9.145178E+02	-9.606149E+02	-38.2944	9.385163E+02
47	-2.00000E-02	3.867291E+02	4.272970E+02	8.458401E+02	9.588918E+02	-8.191829E+02	57.8922	9.385163E+02
47	-2.00000E-02	3.700993E+02	-4.600643E+02	-7.429104E+02	8.300907E+02	-8.000549E+02	-31.4318	8.450883E+02
48	-2.00000E-02	3.801053E+02	3.890944E+02	7.477198E+02	8.483047E+02	-8.384037E+02	58.0080	8.400852E+02
48	-2.00000E-02	1.974444E+02	-4.830641E+02	-6.895409E+02	5.376524E+02	-8.241618E+02	-99.0881	6.800972E+02
49	-2.00000E-02	1.097107E+02	4.935733E+02	6.111145E+02	8.510044E+02	-5.409400E+02	59.0415	7.003730E+02
49	-2.00000E-02	1.097623E+03	-1.685951E+03	-1.988865E+03	2.684401E+03	-2.078723E+03	-87.0938	2.370662E+03
50	-2.00000E-02	1.106593E+03	1.753674E+03	1.897814E+03	2.690917E+03	-2.063334E+03	63.4949	2.370662E+03
50	-2.00000E-02	1.547234E+03	-1.468749E+03	-1.353611E+03	2.045044E+03	-1.087158E+03	-80.9560	2.026401E+03
51	-2.00000E-02	1.552481E+03	1.348701E+03	1.381995E+03	1.991797E+03	-2.106407E+03	68.1940	2.026401E+03
51	-2.00000E-02	1.059356E+03	-1.204200E+03	-1.028817E+03	1.457002E+03	-1.601930E+03	-81.1368	1.589507E+03
52	-2.00000E-02	9.084793E+02	1.866765E+03	1.020660E+03	1.658750E+03	-1.461785E+03	69.2554	1.554267E+03
52	-2.00000E-02	9.080865E+02	-9.938111E+02	-8.43990E+02	1.287162E+03	-1.385177E+03	-70.6988	1.278170E+03
53	-2.00000E-02	7.600919E+02	-8.444304E+02	-7.185389E+02	1.315302E+03	-1.231300E+03	69.1877	1.273301E+03
53	-2.00000E-02	-7.723681E+02	8.533844E+02	7.192034E+02	1.042605E+03	-1.118033E+03	-20.8457	1.089319E+03
54	-2.00000E-02	6.759612E+02	-7.345895E+02	-6.360739E+02	1.185875E+03	-1.044859E+03	69.2484	1.085367E+03
54	-2.00000E-02	-6.813014E+02	7.463837E+02	6.380007E+02	9.212958E+02	-9.789242E+02	-81.0132	9.501100E+02
55	-2.00000E-02	6.470916E+02	-6.995323E+02	-5.758900E+02	8.604937E+02	-9.253928E+02	69.0877	8.863091E+02
55	-2.00000E-02	-6.600839E+02	6.821069E+02	5.717683E+02	8.926444E+02	-8.700213E+02	-20.2818	8.863091E+02

SUBCASE 1

STRESSES IN GENERAL QUADRILATERAL ELEMENTS (COUARD4)

ELEMENT ID.	FIBRE DISTANCE	STRESSES IN STRESS COORD. SYSTEM			ANGLE	PRINCIPAL STRESSES (ZERO SHEAR)		MAX SHEAR
		NORMAL-X	NORMAL-Y	SHEAR-XY		MAJOR	MINOR	
56	-2.000000E-02	3.753140E+08	-8.025401E+08	-4.610000E+08	-19.0408	5.34292E+08	-9.615149E+08	7.470010E+08
57	2.000000E-02	-3.681681E+08	8.198762E+08	4.700044E+08	70.6411	9.869400E+08	-5.364303E+08	7.611434E+08
58	-2.000000E-02	1.329560E+08	-3.725230E+08	-1.042377E+08	-11.2230	1.527300E+08	-3.933077E+08	8.729730E+08
59	2.000000E-02	-1.334960E+08	3.899010E+08	1.031884E+08	79.0679	4.00327E+08	-1.533360E+08	8.770843E+08
60	-2.000000E-02	2.184900E+08	-2.120430E+08	-6.896447E+08	-3.3732	2.281843E+08	-2.500373E+08	2.205500E+08
61	2.000000E-02	-2.190830E+08	2.044300E+08	6.890113E+08	3.3717	2.144364E+08	-2.200636E+08	2.221501E+08
62	-2.000000E-02	1.404372E+08	-1.818581E+08	-4.700030E+08	-8.5119	1.470650E+08	-1.606847E+08	1.608753E+08
63	2.000000E-02	-1.414832E+08	1.894553E+08	4.689783E+08	81.3808	1.748743E+08	-1.480020E+08	1.628782E+08
64	-2.000000E-02	1.126104E+08	-1.896664E+08	-4.012553E+08	-8.9278	1.258190E+08	-1.350080E+08	1.300844E+08
65	2.000000E-02	-1.203920E+08	1.276870E+08	4.078502E+08	80.8953	1.342940E+08	-1.280889E+08	1.305184E+08
66	-2.000000E-02	9.893725E+08	-1.081381E+08	-3.820210E+08	-9.4112	1.047860E+08	-1.139870E+08	1.093874E+08
67	2.000000E-02	-9.966730E+08	1.094490E+08	3.806900E+08	89.8840	1.15327E+08	-1.065810E+08	1.104710E+08
68	-2.000000E-02	8.74513E+08	-9.391750E+08	-3.230974E+08	-9.8269	9.307540E+08	-9.068703E+08	9.630170E+08
69	2.000000E-02	-8.814023E+08	9.519237E+08	3.276530E+08	89.1555	1.008720E+08	-9.342090E+08	9.734616E+08
70	-2.000000E-02	8.265627E+08	-8.933854E+08	-3.091230E+08	-9.8565	8.000977E+08	-9.468603E+08	9.134700E+08
71	2.000000E-02	-8.406327E+08	8.733512E+08	3.065300E+08	89.1592	9.265244E+08	-8.938060E+08	9.101653E+08
72	-2.000000E-02	4.976617E+08	-9.893533E+08	-2.614152E+08	-9.6204	5.414140E+08	-1.043706E+08	7.925466E+08
73	2.000000E-02	-4.908603E+08	1.021147E+08	2.679753E+08	89.2413	1.067236E+08	-5.369560E+08	8.020963E+08

PROBLEM #10

PROGRAM EXECUTIVE CONTROL DECK ECHO

1. AREA INFORMATION PAGE 1 OF 2
2. AREA INFORMATION PAGE 2 OF 2
3. AREA INFORMATION PAGE 3 OF 2
4. AREA INFORMATION PAGE 4 OF 2
5. AREA INFORMATION PAGE 5 OF 2
6. AREA INFORMATION PAGE 6 OF 2
7. AREA INFORMATION PAGE 7 OF 2
8. AREA INFORMATION PAGE 8 OF 2
9. AREA INFORMATION PAGE 9 OF 2
10. AREA INFORMATION PAGE 10 OF 2
11. AREA INFORMATION PAGE 11 OF 2
12. AREA INFORMATION PAGE 12 OF 2
13. AREA INFORMATION PAGE 13 OF 2
14. AREA INFORMATION PAGE 14 OF 2
15. AREA INFORMATION PAGE 15 OF 2
16. AREA INFORMATION PAGE 16 OF 2
17. AREA INFORMATION PAGE 17 OF 2
18. AREA INFORMATION PAGE 18 OF 2
19. AREA INFORMATION PAGE 19 OF 2
20. AREA INFORMATION PAGE 20 OF 2

0404 COMPOSITE TUBE
 SYMMETRIC LAYUP [45/-45/0/90]S
 TUBE UNDER CONSTANT PRESSURE P

CASE CONTROL DECK ECHO

TITLE : 0404 COMPOSITE TUBE
 SUBTITLE : SYMMETRIC LAYUP [45/-45/0/90]S
 LABEL : TUBE UNDER CONSTANT PRESSURE P

MODEL : SECTION OF A OPEN TUBE RADIUS R, UNDER PRESSURE P.
 SYMMETRIC LAYUP [45/-45/0/90/90/-45/45]

1 : COMPARISON OF HOOP LOADING FY FOR ELEMENT ID 8 & 1

HOOP LOADING FY * P * R * 10.5 * 50 * 5.25E5

UAI/MASTRAN	MSC/MASTRAN	THEORETICAL
5.15E2	5.15E2	5.25E2

2 : COMPARISON OF LAYER STRESSES FOR ELEMENT ID 8 & 1

UAI/MASTRAN		MSC/MASTRAN	
LAYER	SIG1	SIG2	TAUIZ
LAYER 1	2.524E2	1.741E1	2.277E1
LAYER 2	2.494E2	1.751E1	-2.276E1
LAYER 3	-2.253E2	3.231E1	1.944E-2
LAYER 4	7.271E2	2.652E0	1.556E-2
LAYER 5	7.270E2	2.660E0	5.854E-2
LAYER 6	-2.252E2	3.230E1	-8.551E-2
LAYER 7	2.534E2	1.741E1	-2.273E1
LAYER 8	2.477E2	1.759E1	2.275E1

MSC/MASTRAN

0404 COMPOSITE TUBE
 SYMMETRIC LAYUP [45/-45/0/90]S
 TUBE UNDER CONSTANT PRESSURE P

CASE CONTROL DECK ECHO

UAI/MASTRAN		MSC/MASTRAN	
LAYER	SIG1	SIG2	TAUIZ
LAYER 1	2.519E2	1.746E1	2.273E1
LAYER 2	2.494E2	1.746E1	-2.273E1
LAYER 3	-2.252E2	3.228E1	1.198E-2
LAYER 4	7.268E2	2.651E0	2.221E-2
LAYER 5	7.268E2	2.657E0	5.551E-2
LAYER 6	-2.252E2	3.231E1	-8.881E-2
LAYER 7	2.525E2	1.741E1	-2.274E1
LAYER 8	2.478E2	1.768E1	2.274E1

SP1 : 1 29,45,61,77
 SP1 : 2 8,24
 DISP(PRINT) : 1

02 STRESS(LAYER) = 2
 03 SUBELEMENT = 2
 04 SUBCASE = 1
 05 LOAD = 1
 06 BEGIN BULK
 07
 08 BULK COMPOSITE TUBE
 09 METRIC GROUP L45-45-45/90JS
 10 TUBE UNDER CONSTANT PRESSURE P

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111 BULK INFORMATION MESSAGE 207, BULK DATA NOT SORTED, XSORT WILL RE-ORDER DECK.
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SORTED BULK DATA ECHO

---	++2+++	---3---	++4+++	---5---	++6+++	---7---	++8+++	---9---	++10+++
CORDC	1	0.0	0.0	0.0	0.0	0.0	0.0	1.0	1.0
MPOR101	1.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
ORDER	2	50.0	5.0	5.0	40.0	40.0	0.0	0.0	0.0
ABC	3	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
COUAD	4	17	35	35	19	19	2	2	2
COUAD	5	17	37	37	21	21	2	2	2
COUAD	6	17	34	34	33	33	2	2	2
COUAD	7	18	36	36	40	40	2	2	2
COUAD	8	24	26	26	42	42	2	2	2
COUAD	9	26	44	44	28	28	2	2	2
COUAD	10	24	38	38	22	22	2	2	2
COUAD	11	22	40	40	38	38	2	2	2
COUAD	12	25	39	39	41	41	2	2	2
COUAD	13	25	21	21	29	29	2	2	2
COUAD	14	25	41	41	43	43	2	2	2
COUAD	15	32	43	43	29	29	2	2	2
COUAD	16	32	46	46	30	30	2	2	2
COUAD	17	32	46	46	44	44	2	2	2
COUAD	18	32	31	31	47	47	2	2	2
COUAD	19	33	29	29	45	45	2	2	2
COUAD	20	35	49	49	51	51	2	2	2
COUAD	21	35	51	51	35	35	2	2	2
COUAD	22	33	53	53	37	37	2	2	2
COUAD	23	33	50	50	49	49	2	2	2
COUAD	24	34	36	36	52	52	2	2	2
COUAD	25	40	42	42	55	55	2	2	2
COUAD	26	42	44	44	50	50	2	2	2
COUAD	27	48	54	54	38	38	2	2	2
COUAD	28	48	54	54	57	57	2	2	2
COUAD	29	41	37	37	55	55	2	2	2
COUAD	30	39	41	41	43	43	2	2	2
COUAD	31	41	57	57	59	59	2	2	2
COUAD	32	48	64	64	61	61	2	2	2
COUAD	33	48	64	64	62	62	2	2	2
COUAD	34	46	62	62	60	60	2	2	2

201 BULK COMPOSITE TUBE
 202 METRIC GROUP L45-45-45/90JS
 203 TUBE UNDER CONSTANT PRESSURE P

SORTED BULK DATA ECHO

---	++2+++	---3---	++4+++	---5---	++6+++	---7---	++8+++	---9---	++10+++
COUAD	31	40	47	47	63	63	2	2	2
COUAD	32	47	45	45	61	61	2	2	2
COUAD	33	48	67	67	51	51	2	2	2
COUAD	34	51	67	67	59	59	2	2	2
COUAD	35	48	59	59	66	66	2	2	2
COUAD	36	50	52	52	68	68	2	2	2

50.000	156.000	10.000
50.000	202.500	10.000
50.000	157.500	10.000
50.000	225.000	10.000
50.000	175.000	10.000
50.000	247.500	10.000
50.000	112.500	10.000
50.000	210.000	10.000
50.000	240.000	10.000
50.000	203.500	10.000
50.000	17.500	10.000
50.000	315.000	10.000
50.000	45.000	10.000
50.000	317.500	10.000
50.000	22.500	10.000
50.000	0.000	10.000
50.000	130.000	20.000
50.000	202.500	20.000
50.000	17.500	20.000
50.000	225.000	20.000
50.000	175.000	20.000
50.000	247.500	20.000
50.000	112.500	20.000
50.000	210.000	20.000
50.000	240.000	20.000
50.000	203.500	20.000
50.000	17.500	20.000
50.000	315.000	20.000
50.000	45.000	20.000
50.000	317.500	20.000
50.000	22.500	20.000
50.000	0.000	20.000
50.000	130.000	30.000
50.000	202.500	30.000
50.000	17.500	30.000
50.000	225.000	30.000
50.000	175.000	30.000
50.000	247.500	30.000
50.000	112.500	30.000
50.000	210.000	30.000
50.000	240.000	30.000
50.000	203.500	30.000
50.000	17.500	30.000
50.000	315.000	30.000
50.000	45.000	30.000
50.000	317.500	30.000
50.000	22.500	30.000
50.000	0.000	30.000
50.000	130.000	40.000
50.000	202.500	40.000
50.000	17.500	40.000
50.000	225.000	40.000
50.000	175.000	40.000
50.000	247.500	40.000
50.000	112.500	40.000
50.000	210.000	40.000
50.000	240.000	40.000
50.000	203.500	40.000
50.000	17.500	40.000
50.000	315.000	40.000
50.000	45.000	40.000
50.000	317.500	40.000
50.000	22.500	40.000
50.000	0.000	40.000
50.000	130.000	50.000
50.000	202.500	50.000
50.000	17.500	50.000
50.000	225.000	50.000
50.000	175.000	50.000
50.000	247.500	50.000
50.000	112.500	50.000
50.000	210.000	50.000
50.000	240.000	50.000
50.000	203.500	50.000
50.000	17.500	50.000
50.000	315.000	50.000
50.000	45.000	50.000
50.000	317.500	50.000
50.000	22.500	50.000
50.000	0.000	50.000

SORTED BULK DATA ECHO

50.000	112.500	20.000	++G++	---9---	+++10+++
50.000	270.000	20.000			
50.000	30.000	20.000			
50.000	22.500	20.000			
50.000	15.000	20.000			
50.000	45.000	20.000			
50.000	37.500	20.000			
50.000	0.000	20.000			
50.000	130.000	30.000			
50.000	202.500	30.000			
50.000	17.500	30.000			
50.000	225.000	30.000			
50.000	175.000	30.000			
50.000	247.500	30.000			
50.000	112.500	30.000			
50.000	210.000	30.000			
50.000	240.000	30.000			
50.000	203.500	30.000			
50.000	17.500	30.000			
50.000	315.000	30.000			
50.000	45.000	30.000			
50.000	317.500	30.000			
50.000	22.500	30.000			
50.000	0.000	30.000			
50.000	130.000	40.000			
50.000	202.500	40.000			
50.000	17.500	40.000			
50.000	225.000	40.000			
50.000	175.000	40.000			
50.000	247.500	40.000			
50.000	112.500	40.000			
50.000	210.000	40.000			
50.000	240.000	40.000			
50.000	203.500	40.000			
50.000	17.500	40.000			
50.000	315.000	40.000			
50.000	45.000	40.000			
50.000	317.500	40.000			
50.000	22.500	40.000			
50.000	0.000	40.000			
50.000	130.000	50.000			
50.000	202.500	50.000			
50.000	17.500	50.000			
50.000	225.000	50.000			
50.000	175.000	50.000			
50.000	247.500	50.000			
50.000	112.500	50.000			
50.000	210.000	50.000			
50.000	240.000	50.000			
50.000	203.500	50.000			
50.000	17.500	50.000			
50.000	315.000	50.000			
50.000	45.000	50.000			
50.000	317.500	50.000			
50.000	22.500	50.000			
50.000	0.000	50.000			

257- GRID 50.000 45.000 70.000
 258- GRID 50.000 337.500 70.000
 259- GRID 50.000 22.500 70.000
 260- GRID 50.000 0.000 70.000
 261- GRID 50.000 180.000 80.000
 262- GRID 50.000 202.500 80.000
 263- GRID 50.000 157.500 80.000
 264- GRID 50.000 225.000 80.000
 265- GRID 50.000 135.000 80.000
 266- GRID 50.000 247.500 80.000
 267- GRID 50.000 112.500 80.000
 268- GRID 50.000 270.000 80.000
 269- GRID 50.000 90.000 80.000
 270- GRID 50.000 292.500 80.000
 271- GRID 50.000 67.500 80.000
 272- GRID 50.000 315.000 80.000

1 QUADA COMPOSITE TUBE
 0 SYMMETRIC LAYOUT [45,-45/0/90]S
 0 TUBE UNDER CONSTANT PRESSURE P

CARD	COUNT	---	1---	++2++	---3---	+++4++	---	5---	+++6++	---	7---	+++8++	---	9---	+++10++
273- GRID	141	---	1	1	1	50.000	45.000	70.000	80.000	---	---	---	---	---	---
274- GRID	142	---	1	1	1	50.000	337.500	80.000	80.000	---	---	---	---	---	---
275- GRID	143	---	1	1	1	50.000	22.500	80.000	80.000	---	---	---	---	---	---
276- GRID	144	---	1	1	1	50.000	0.000	80.000	80.000	---	---	---	---	---	---
277- MAT8	1	---	73.8	E+33.75	E+30.4	1.74	E+3	---	---	---	---	---	---	---	---
278- +M01	2	---	-0.96	---	---	1680.	-229.0	20.9	---	---	---	---	---	---	---
279- PCOMP	1	---	.24	---	---	10000.e	HILL	---	---	---	---	---	---	---	---
280- +PC1	1	---	---	---	---	45.0	YES	---	---	---	---	---	---	---	---
281- +PC2	1	---	---	---	---	0.0	YES	---	---	---	---	---	---	---	---
282- PLOAD4	1	---	1	3	1	10.5	---	---	---	---	---	---	---	---	---
283- SPC1	1	---	6	1	1	---	THRU	---	---	---	---	---	---	---	---
284- SPC1	1	---	145	9	24	---	---	---	---	---	---	---	---	---	---
285- SPC1	1	---	245	1	16	---	---	---	---	---	---	---	---	---	---
286- ENDDATA	1	---	---	---	---	---	---	---	---	---	---	---	---	---	---

1 QUADA COMPOSITE TUBE
 0 SYMMETRIC LAYUP L45/-45/0/90J5
 TUBE UNDER CONSTANT PRESSURE P

SUBCASE 1

DISPLACEMENT VECTOR

POINT ID.	TYPE	T1	T2	T3	R1	R2	R3
29	G	3.467347E-01	3.438213E-01	-3.005464E-02	-4.671168E-04	2.562122E-04	.0
45	G	3.470449E-01	3.449179E-01	-6.224011E-02	-7.687557E-04	2.196043E-04	.0
61	G	3.470868E-01	3.455627E-01	-9.453406E-02	3.007555E-04	5.882263E-04	.0
77	G	3.485623E-01	3.478297E-01	-1.268109E-01	-2.582375E-04	-6.487657E-04	.0

9404 Composite Tube
 Composite Tube
 Tube

SUBCASE 1

FORCES IN LOWERED COMPOSITE ELEMENTS (QUADR)

ELEMENT NO	MEMBRANE FORCES		BENDING MOMENTS		TRANSVERSE SHEAR FORCES		
	FX	FY	MX	MY	VX	VY	
1	-3.2585E+01	5.1778E+02	-2.7869E+00	-3.5564E-01	-3.4703E-01	2.5866E+01	7.9845E+02
2	-1.7631E+04	5.1541E+02	-2.1714E+00	1.6764E-01	1.1237E-01	7.1069E+01	2.4473E+01

CONCRETE TEST
STRESS IN FIBER AND MATRIX
DIRECTIONAL MATRIX STRESS

TEST NO.	CONCRETE TYPE	STRESS IN FIBER AND MATRIX DIRECTIONAL MATRIX STRESS	CONCRETE TYPE	STRESS IN FIBER AND MATRIX DIRECTIONAL MATRIX STRESS
1	1	1.82E+01	1	1.82E+01
2	1	1.82E+01	2	1.82E+01
3	1	1.82E+01	3	1.82E+01
4	1	1.82E+01	4	1.82E+01
5	1	1.82E+01	5	1.82E+01
6	1	1.82E+01	6	1.82E+01
7	1	1.82E+01	7	1.82E+01
8	1	1.82E+01	8	1.82E+01
9	1	1.82E+01	9	1.82E+01
10	1	1.82E+01	10	1.82E+01
11	1	1.82E+01	11	1.82E+01
12	1	1.82E+01	12	1.82E+01
13	1	1.82E+01	13	1.82E+01
14	1	1.82E+01	14	1.82E+01
15	1	1.82E+01	15	1.82E+01
16	1	1.82E+01	16	1.82E+01
17	1	1.82E+01	17	1.82E+01
18	1	1.82E+01	18	1.82E+01
19	1	1.82E+01	19	1.82E+01
20	1	1.82E+01	20	1.82E+01
21	1	1.82E+01	21	1.82E+01
22	1	1.82E+01	22	1.82E+01
23	1	1.82E+01	23	1.82E+01
24	1	1.82E+01	24	1.82E+01
25	1	1.82E+01	25	1.82E+01
26	1	1.82E+01	26	1.82E+01
27	1	1.82E+01	27	1.82E+01
28	1	1.82E+01	28	1.82E+01
29	1	1.82E+01	29	1.82E+01
30	1	1.82E+01	30	1.82E+01
31	1	1.82E+01	31	1.82E+01
32	1	1.82E+01	32	1.82E+01
33	1	1.82E+01	33	1.82E+01
34	1	1.82E+01	34	1.82E+01
35	1	1.82E+01	35	1.82E+01
36	1	1.82E+01	36	1.82E+01
37	1	1.82E+01	37	1.82E+01
38	1	1.82E+01	38	1.82E+01
39	1	1.82E+01	39	1.82E+01
40	1	1.82E+01	40	1.82E+01
41	1	1.82E+01	41	1.82E+01
42	1	1.82E+01	42	1.82E+01
43	1	1.82E+01	43	1.82E+01
44	1	1.82E+01	44	1.82E+01
45	1	1.82E+01	45	1.82E+01
46	1	1.82E+01	46	1.82E+01
47	1	1.82E+01	47	1.82E+01
48	1	1.82E+01	48	1.82E+01
49	1	1.82E+01	49	1.82E+01
50	1	1.82E+01	50	1.82E+01

THEORY WAS USED FOR THIS ELEMENT

THEORY WAS USED FOR THIS ELEMENT

SORTED BULK DATA ECHO

CARD	1	2	3	4	5	6	7	8	9	10
00001	CRD	62	1	88	69	72	77	5	5	5
00002	CRD	63	1	88	78	79	78	5	5	5
00003	CRD	64	1	88	79	86	79	5	5	5
00004	CRD	65	1	88	71	81	88	5	5	5
00005	CRD	66	1	88	72	81	88	5	5	5
00006	CRD	67	1	88	72	81	88	5	5	5
00007	CRD	68	1	88	72	81	88	5	5	5
00008	CRD	69	1	88	72	81	88	5	5	5
00009	CRD	70	1	88	72	81	88	5	5	5
00010	CRD	71	1	88	72	81	88	5	5	5
00011	CRD	72	1	88	72	81	88	5	5	5
00012	CRD	73	1	88	72	81	88	5	5	5
00013	CRD	74	1	88	72	81	88	5	5	5
00014	CRD	75	1	88	72	81	88	5	5	5
00015	CRD	76	1	88	72	81	88	5	5	5
00016	CRD	77	1	88	72	81	88	5	5	5
00017	CRD	78	1	88	72	81	88	5	5	5
00018	CRD	79	1	88	72	81	88	5	5	5
00019	CRD	80	1	88	72	81	88	5	5	5
00020	CRD	81	1	88	72	81	88	5	5	5
00021	CRD	82	1	88	72	81	88	5	5	5
00022	CRD	83	1	88	72	81	88	5	5	5
00023	CRD	84	1	88	72	81	88	5	5	5
00024	CRD	85	1	88	72	81	88	5	5	5
00025	CRD	86	1	88	72	81	88	5	5	5
00026	CRD	87	1	88	72	81	88	5	5	5
00027	CRD	88	1	88	72	81	88	5	5	5
00028	CRD	89	1	88	72	81	88	5	5	5
00029	CRD	90	1	88	72	81	88	5	5	5
00030	CRD	91	1	88	72	81	88	5	5	5
00031	CRD	92	1	88	72	81	88	5	5	5
00032	CRD	93	1	88	72	81	88	5	5	5
00033	CRD	94	1	88	72	81	88	5	5	5
00034	CRD	95	1	88	72	81	88	5	5	5
00035	CRD	96	1	88	72	81	88	5	5	5
00036	CRD	97	1	88	72	81	88	5	5	5
00037	CRD	98	1	88	72	81	88	5	5	5
00038	CRD	99	1	88	72	81	88	5	5	5
00039	CRD	100	1	88	72	81	88	5	5	5
00040	CRD	101	1	88	72	81	88	5	5	5
00041	CRD	102	1	88	72	81	88	5	5	5
00042	CRD	103	1	88	72	81	88	5	5	5
00043	CRD	104	1	88	72	81	88	5	5	5
00044	CRD	105	1	88	72	81	88	5	5	5
00045	CRD	106	1	88	72	81	88	5	5	5
00046	CRD	107	1	88	72	81	88	5	5	5
00047	CRD	108	1	88	72	81	88	5	5	5
00048	CRD	109	1	88	72	81	88	5	5	5
00049	CRD	110	1	88	72	81	88	5	5	5
00050	CRD	111	1	88	72	81	88	5	5	5
00051	CRD	112	1	88	72	81	88	5	5	5
00052	CRD	113	1	88	72	81	88	5	5	5
00053	CRD	114	1	88	72	81	88	5	5	5
00054	CRD	115	1	88	72	81	88	5	5	5
00055	CRD	116	1	88	72	81	88	5	5	5
00056	CRD	117	1	88	72	81	88	5	5	5
00057	CRD	118	1	88	72	81	88	5	5	5
00058	CRD	119	1	88	72	81	88	5	5	5
00059	CRD	120	1	88	72	81	88	5	5	5
00060	CRD	121	1	88	72	81	88	5	5	5

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CRD SORTED BULK DATA ECHO

CARD	1	2	3	4	5	6	7	8	9	10
00001	CRD	30	1	25.000	10.000	9.375	1	10.000	10.000	10.000
00002	CRD	31	1	25.000	15.000	9.375	1	15.000	15.000	15.000
00003	CRD	32	1	25.000	20.000	9.375	1	20.000	20.000	20.000
00004	CRD	33	1	25.000	25.000	9.375	1	25.000	25.000	25.000
00005	CRD	34	1	25.000	30.000	9.375	1	30.000	30.000	30.000
00006	CRD	35	1	25.000	35.000	9.375	1	35.000	35.000	35.000
00007	CRD	36	1	25.000	40.000	9.375	1	40.000	40.000	40.000
00008	CRD	37	1	25.000	45.000	9.375	1	45.000	45.000	45.000
00009	CRD	38	1	25.000	50.000	9.375	1	50.000	50.000	50.000
00010	CRD	39	1	25.000	55.000	9.375	1	55.000	55.000	55.000
00011	CRD	40	1	25.000	60.000	9.375	1	60.000	60.000	60.000
00012	CRD	41	1	25.000	65.000	9.375	1	65.000	65.000	65.000
00013	CRD	42	1	25.000	70.000	9.375	1	70.000	70.000	70.000
00014	CRD	43	1	25.000	75.000	9.375	1	75.000	75.000	75.000
00015	CRD	44	1	25.000	80.000	9.375	1	80.000	80.000	80.000
00016	CRD	45	1	25.000	85.000	9.375	1	85.000	85.000	85.000
00017	CRD	46	1	25.000	90.000	9.375	1	90.000	90.000	90.000
00018	CRD	47	1	25.000	95.000	9.375	1	95.000	95.000	95.000
00019	CRD	48	1	25.000	100.000	9.375	1	100.000	100.000	100.000

CRD SORTED BULK DATA ECHO

COMPOS 277 UNIT 288 QUAD4 4-MODE SHELL ROOF TEST
 UNLINKED COMPOSITE SHELL

121	GRD	25.000	15.000	15.625	1
122	GRD	25.000	20.000	15.625	1
123	GRD	25.000	25.000	15.625	1
124	GRD	25.000	30.000	15.625	1
125	GRD	25.000	35.000	15.625	1
126	GRD	25.000	40.000	15.625	1
127	GRD	25.000	45.000	18.750	1
128	GRD	25.000	5.000	18.750	1
129	GRD	25.000	10.000	18.750	1
130	GRD	25.000	15.000	18.750	1
131	GRD	25.000	20.000	18.750	1
132	GRD	25.000	25.000	18.750	1
133	GRD	25.000	30.000	18.750	1
134	GRD	25.000	35.000	18.750	1
135	GRD	25.000	40.000	18.750	1
136	GRD	25.000	45.000	18.750	1

COMPOS 277 UNIT 288 QUAD4 4-MODE SHELL ROOF TEST
 UNLINKED COMPOSITE SHELL

137	GRD	25.000	0.000	21.875	1				
138	GRD	25.000	5.000	21.875	1				
139	GRD	25.000	10.000	21.875	1				
140	GRD	25.000	15.000	21.875	1				
141	GRD	25.000	20.000	21.875	1				
142	GRD	25.000	25.000	21.875	1				
143	GRD	25.000	30.000	21.875	1				
144	GRD	25.000	35.000	21.875	1				
145	GRD	25.000	40.000	21.875	1				
146	GRD	25.000	45.000	21.875	1				
147	GRD	25.000	0.000	25.000	1				
148	GRD	25.000	5.000	25.000	1				
149	GRD	25.000	10.000	25.000	1				
150	GRD	25.000	15.000	25.000	1				
151	GRD	25.000	20.000	25.000	1				
152	GRD	25.000	25.000	25.000	1				
153	GRD	25.000	30.000	25.000	1				
154	GRD	25.000	35.000	25.000	1				
155	GRD	25.000	40.000	25.000	1				
156	GRD	25.000	45.000	25.000	1				
157	PCO	20.0 E+70.5 E+70.25	0.25 E+70.25 E+70.25 E+7	0.25 E+70.25 E+7	3				
158	+PC1	.03125	45.0	YES	-45.0	YES	+PC1		
159	+PC2	.03125	15.0	YES	-15.0	YES	+PC2		
160	+PC3	.03125	-15.0	YES	15.0	YES	+PC3		
161	+PC4	.03125	-45.0	YES	45.0	YES	+PC4		
162	PLOD4	2	30.0	YES	TRU	65	+PL1		
163	SP1	4	0.0	-1.0	19	19	+MIN		
164	+MIN	28	37	45	46	54	+MIN		
165	SP2	1	1	2	3	4	5	6	+SP10001
166	+SP10005	54	73	10	19	26	37	46	+SP10005

COMPOS 277 UNIT 288 QUAD4 4-MODE SHELL ROOF TEST
 UNLINKED COMPOSITE SHELL

171	SP1	25	72	74	75	76	77	78	78	+SP10003
172	ENDATA	80	81							

DISPLACEMENT VECTOR

PCOMP ID	TYPE	T1	T2	T3	R1	R2	R3
1	0	3.325734E-02	2.872322E-02	2.891236E-02	5.887511E-02	6.817595E-03	1.755109E-03
2	0	1.589425E-02	2.378459E-02	3.002984E-02	1.468109E-02	1.468109E-02	2.568997E-03
3	0	3.112370E-02	1.760152E-03	3.261879E-02	1.100732E-01	1.595813E-02	3.448476E-02
4	0	1.616653E-01	3.890004E-02	3.381676E-02	1.481686E-01	3.852371E-02	3.961835E-03
5	0	1.890841E-01	1.642084E-02	2.946811E-02	1.687844E-01	-6.872395E-02	4.421340E-03
6	0	3.912721E-01	3.725333E-02	1.419331E-02	1.652240E-01	1.891839E-01	4.192573E-03
7	0	5.046315E-01	6.708049E-02	1.834959E-02	1.581470E-01	1.367115E-01	4.160266E-03
8	0	6.063562E-01	1.062398E-01	-7.528201E-02	1.244091E-01	-1.703734E-01	2.809102E-03
9	0	8.301502E-02	1.528468E-01	-1.644548E-01	.0	-1.968604E-01	.0
10	0	1.589425E-02	2.378459E-02	2.872322E-02	.0	6.782861E-03	.0
11	0	3.112370E-02	1.760152E-03	3.261879E-02	6.732956E-02	3.837667E-03	1.570899E-02
12	0	1.616653E-01	3.890004E-02	3.224522E-02	1.033408E-01	-9.959379E-03	2.724039E-02
13	0	1.890841E-01	1.642084E-02	3.226229E-02	1.355858E-01	-3.889346E-02	3.683246E-02
14	0	3.912721E-01	3.725333E-02	2.383363E-02	1.549632E-01	-5.761424E-02	4.454504E-02
15	0	5.046315E-01	6.708049E-02	1.371384E-02	1.638041E-01	-8.219250E-02	4.971170E-02
16	0	6.063562E-01	1.062398E-01	-1.321144E-02	1.573638E-01	-1.204452E-01	5.184007E-02
17	0	8.301502E-02	1.528468E-01	-7.362947E-02	1.519317E-01	-1.525352E-01	5.050477E-02
18	0	1.589425E-02	2.378459E-02	2.891236E-02	.0	-1.770848E-01	4.658273E-02
19	0	3.112370E-02	1.760152E-03	3.261879E-02	.0	4.977118E-03	.0
20	0	1.616653E-01	3.890004E-02	2.389788E-02	7.203986E-02	1.316321E-02	2.855850E-02
21	0	1.890841E-01	1.642084E-02	3.078166E-02	1.166358E-01	-1.038321E-02	5.176908E-02
22	0	3.912721E-01	3.725333E-02	3.128853E-02	1.454863E-01	-2.932057E-02	7.075454E-02
23	0	5.046315E-01	6.708049E-02	2.565387E-02	1.608323E-01	-5.323394E-02	8.531525E-02
24	0	6.063562E-01	1.062398E-01	1.210007E-02	1.609140E-01	-8.615975E-02	9.463396E-02
25	0	8.301502E-02	1.528468E-01	-1.778191E-02	1.516218E-01	-1.082232E-01	9.805495E-02
26	0	1.589425E-02	2.378459E-02	-6.868840E-02	.0	-1.360885E-01	9.828933E-02
27	0	3.112370E-02	1.760152E-03	-1.460617E-01	1.333956E-01	-1.586747E-01	9.264753E-02
28	0	1.616653E-01	3.890004E-02	2.812041E-02	.0	1.261244E-05	.0
29	0	1.890841E-01	1.642084E-02	2.676895E-02	8.743578E-02	5.296311E-04	9.931383E-02
30	0	3.912721E-01	3.725333E-02	2.798181E-02	9.962346E-02	-1.002300E-02	7.389390E-02
31	0	5.046315E-01	6.708049E-02	2.780328E-02	1.154709E-01	-2.578396E-02	1.004083E-01
32	0	6.063562E-01	1.062398E-01	2.307334E-02	1.218567E-01	-4.596176E-02	1.207275E-01
33	0	8.301502E-02	1.528468E-01	9.747822E-03	1.205370E-01	-6.877158E-02	1.332812E-01
34	0	1.589425E-02	2.378459E-02	-1.569204E-02	1.143434E-01	-9.128175E-02	1.379633E-01

POINT	TYPE	T1	T2	T3	U	V	W	R1	R2	R3
1	0	1.26432E+00	2.92005E-01	-6.97532E-02	1.15367E-01	-1.17692E-01	1.37189E-01			
2	0	1.62120E+00	4.13149E-01	-1.28231E-01	.0	-1.37542E-01	1.35458E-01			
3	0	1.16916E-02	-5.50915E-03	2.34331E-02	.0	6.03336E-04	4.80910E-02			
4	0	1.86559E-01	1.37742E-03	2.39362E-02	5.53408E-02	-2.22696E-03	9.00267E-02			
5	0	1.86559E-01	1.86578E-03	2.34638E-02	1.95623E-01	1.21189E-01	1.24533E-01			
6	0	1.42120E-02	6.24079E-02	1.86561E-02	1.26426E-01	-3.69541E-02	1.49733E-01			
7	0	1.32190E-01	1.24079E-01	7.34956E-02	1.21977E-01	-5.52184E-02	1.52288E-01			
8	0	1.86578E+00	3.82190E-01	-1.46501E-02	1.11931E-01	-7.49007E-02	1.71818E-01			
9	0	1.86578E+00	3.82190E-01	-5.11455E-02	9.45356E-02	-9.52295E-02	1.72678E-01			
10	0	1.86578E+00	3.82190E-01	-1.86153E-01	.0	-1.13119E-01	1.73460E-01			
11	0	1.86578E+00	3.82190E-01	1.87399E-02	.0	-6.33995E-03	.0			
12	0	1.86578E+00	3.82190E-01	1.86578E-02	9.85430E-02	-5.22897E-04	5.28342E-02			
13	0	1.86578E+00	3.82190E-01	1.86578E-02	1.86578E-02	-7.01659E-03	1.82630E-01			
14	0	1.86578E+00	3.82190E-01	1.73499E-02	7.51685E-02	-1.55965E-02	1.42629E-01			
15	0	1.86578E+00	3.82190E-01	1.46245E-02	7.08596E-02	-2.72795E-03	1.22287E-01			
16	0	1.86578E+00	3.82190E-01	5.02105E-03	6.63188E-02	-4.11145E-02	1.90534E-01			
17	0	1.86578E+00	3.82190E-01	-1.86245E-02	5.33758E-02	-5.15291E-02	1.98991E-01			
18	0	1.86578E+00	3.82190E-01	-3.94675E-02	7.65989E-02	-7.36849E-02	2.01551E-01			
19	0	1.86578E+00	3.82190E-01	-8.14930E-02	.0	-0.55217E-02	2.04399E-01			
20	0	1.86578E+00	3.82190E-01	1.30284E-02	.0	1.19269E-03	.0			
21	0	1.86578E+00	3.82190E-01	1.32816E-02	3.80582E-02	-4.52463E-03	5.78799E-02			
22	0	1.86578E+00	3.82190E-01	1.36775E-02	5.10407E-02	-5.31245E-03	1.10459E-01			
23	0	1.86578E+00	3.82190E-01	1.21182E-02	1.00372E-01	-1.05203E-02	1.55204E-01			
24	0	1.86578E+00	3.82190E-01	9.39502E-03	1.00372E-01	-1.78356E-02	1.88292E-01			
25	0	1.86578E+00	3.82190E-01	5.11882E-03	9.02432E-02	-6.95794E-02	2.08914E-01			
26	0	1.86578E+00	3.82190E-01	3.25582E-03	7.48657E-02	-3.72875E-02	2.18956E-01			
27	0	1.86578E+00	3.82190E-01	-2.70841E-02	4.52184E-02	-4.79928E-02	2.23919E-01			
28	0	1.86578E+00	3.82190E-01	-5.56219E-02	.0	-5.91737E-02	2.26526E-01			
29	0	1.86578E+00	3.82190E-01	6.92502E-03	.0	-1.64395E-02	.0			
30	0	1.86578E+00	3.82190E-01	6.92502E-03	1.04837E-01	9.26781E-04	5.79143E-02			
31	0	1.86578E+00	3.82190E-01	6.92502E-03	4.18160E-02	-1.22023E-03	1.14399E-01			
32	0	1.86578E+00	3.82190E-01	6.92502E-03	1.72559E-02	-3.82204E-03	1.51928E-01			
33	0	1.86578E+00	3.82190E-01	4.61464E-02	1.77546E-03	-7.42340E-03	1.97627E-01			

DISPLACEMENT VECTOR

POINT	ID	TYPE	T1	T2	R1	R2	R3
1	1	0	1.825181+00	1.421482-03	5.664192E-03	-1.232594E-02	2.201441E-01
2	2	0	-1.765583+00	-4.267287E-02	3.084882E-04	-1.822488E-02	2.316262E-01
3	3	0	-2.294031+00	-1.738434E-02	2.905895E-02	-2.613048E-02	2.367322E-01
4	4	0	-2.743022+00	-2.792335E-02	0	-2.787695E-02	2.423215E-01
5	5	0	-2.326621+00	0	-5.779191E-01	0	0
6	6	0	-1.834432E-02	0	-5.321521E-02	0	5.897225E-02
7	7	0	-1.651349E-01	0	8.793802E-02	0	1.133112E-01
8	8	0	-1.784362E-01	0	1.668075E-01	0	1.631115E-01
9	9	0	-8.663051E-01	0	1.012105E-01	0	2.002222E-01
10	10	0	-1.301752+00	0	8.663345E-02	0	2.244932E-01
11	11	0	-1.866912+00	0	4.666501E-02	0	2.361339E-01
12	12	0	-2.200112+00	0	1.252555E-02	0	2.432935E-01
13	13	0	-2.782621+00	0	-1.352325E-01	0	2.580792E-01
14	14	0	-1.123472+00	-2.152376E-00	-2.560358E-01	-1.251811E-01	-1.495545E-01

CONFIDENTIAL - SECURITY INFORMATION - SOURCE: SAC L. B. ROSS TEST

LOAD VECTOR

	R1	R2	R3
0	0	0	0
1	0	0	0
2	0	0	0
3	0	0	0
4	0	0	0
5	0	0	0
6	0	0	0
7	0	0	0
8	0	0	0
9	0	0	0
10	0	0	0
11	0	0	0
12	0	0	0
13	0	0	0
14	0	0	0
15	0	0	0
16	0	0	0
17	0	0	0
18	0	0	0
19	0	0	0
20	0	0	0
21	0	0	0
22	0	0	0
23	0	0	0
24	0	0	0
25	0	0	0
26	0	0	0
27	0	0	0
28	0	0	0
29	0	0	0
30	0	0	0
31	0	0	0
32	0	0	0
33	0	0	0
34	0	0	0
35	0	0	0
36	0	0	0
37	0	0	0
38	0	0	0
39	0	0	0
40	0	0	0
41	0	0	0
42	0	0	0
43	0	0	0
44	0	0	0
45	0	0	0
46	0	0	0
47	0	0	0
48	0	0	0
49	0	0	0
50	0	0	0
51	0	0	0
52	0	0	0
53	0	0	0
54	0	0	0
55	0	0	0
56	0	0	0
57	0	0	0
58	0	0	0
59	0	0	0
60	0	0	0
61	0	0	0
62	0	0	0
63	0	0	0
64	0	0	0
65	0	0	0
66	0	0	0
67	0	0	0
68	0	0	0
69	0	0	0
70	0	0	0
71	0	0	0
72	0	0	0
73	0	0	0
74	0	0	0
75	0	0	0
76	0	0	0
77	0	0	0
78	0	0	0
79	0	0	0
80	0	0	0
81	0	0	0
82	0	0	0
83	0	0	0
84	0	0	0
85	0	0	0
86	0	0	0
87	0	0	0
88	0	0	0
89	0	0	0
90	0	0	0
91	0	0	0
92	0	0	0
93	0	0	0
94	0	0	0
95	0	0	0
96	0	0	0
97	0	0	0
98	0	0	0
99	0	0	0
100	0	0	0

CONFIDENTIAL - SECURITY INFORMATION - SOURCE: SAC L. B. ROSS TEST

LOAD VECTOR

LOAD VECTOR	R1	R2	R3
1	0	0	0
2	0	0	0
3	0	0	0
4	0	0	0
5	0	0	0
6	0	0	0
7	0	0	0
8	0	0	0
9	0	0	0
10	0	0	0
11	0	0	0
12	0	0	0
13	0	0	0
14	0	0	0
15	0	0	0
16	0	0	0
17	0	0	0
18	0	0	0
19	0	0	0
20	0	0	0
21	0	0	0
22	0	0	0
23	0	0	0
24	0	0	0
25	0	0	0
26	0	0	0
27	0	0	0
28	0	0	0
29	0	0	0
30	0	0	0
31	0	0	0
32	0	0	0
33	0	0	0
34	0	0	0
35	0	0	0
36	0	0	0
37	0	0	0
38	0	0	0
39	0	0	0
40	0	0	0
41	0	0	0
42	0	0	0
43	0	0	0
44	0	0	0
45	0	0	0
46	0	0	0
47	0	0	0
48	0	0	0
49	0	0	0
50	0	0	0
51	0	0	0
52	0	0	0
53	0	0	0
54	0	0	0
55	0	0	0
56	0	0	0
57	0	0	0
58	0	0	0
59	0	0	0
60	0	0	0
61	0	0	0
62	0	0	0
63	0	0	0
64	0	0	0
65	0	0	0
66	0	0	0
67	0	0	0
68	0	0	0
69	0	0	0
70	0	0	0
71	0	0	0
72	0	0	0
73	0	0	0
74	0	0	0
75	0	0	0
76	0	0	0
77	0	0	0
78	0	0	0
79	0	0	0
80	0	0	0
81	0	0	0
82	0	0	0
83	0	0	0
84	0	0	0
85	0	0	0
86	0	0	0
87	0	0	0
88	0	0	0
89	0	0	0
90	0	0	0
91	0	0	0
92	0	0	0
93	0	0	0
94	0	0	0
95	0	0	0
96	0	0	0
97	0	0	0
98	0	0	0
99	0	0	0
100	0	0	0

COMPARISON OF 1981 AND 1982 4-HOLE SHELL POOF TEST
 COMPARISON OF 1981 AND 1982 SHELL

POOF	1981	1982	R1	R2	R3
1	15.59271E+02	2.59271E+02	.0	.0	.0
2	13.31271E+02	1.466988E+02	.0	.0	.0
3	13.42485E+02	1.518964E+02	.0	.0	.0
4	12.34949E+02	1.371422E+02	.0	.0	.0
5	11.53349E+02	.0	.0	.0	.0
6	11.85517E+02	1.673956E+01	.0	.0	.0
7	13.44815E+02	5.32529E+01	.0	.0	.0
8	12.26624E+02	7.077954E+01	.0	.0	.0
9	12.83242E+02	1.448972E+02	.0	.0	.0
10	12.77942E+02	1.295155E+02	.0	.0	.0
11	12.65482E+02	1.573451E+02	.0	.0	.0
12	12.31294E+02	1.753152E+02	.0	.0	.0
13	11.14725E+02	1.857119E+01	.0	.0	.0

NASTRAN EXECUTIVE CONTROL DECK ECHO

10 COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1980
 APP DISPLACEMENT
 SOL 1.0
 TIME 10
 CEND

CASE CONTROL DECK ECHO

```

CARD
COUNT
1  TITLE = PROBLEM 13 CASE 1 INPLANE LOADING
2  LOAD = 1
3  SPC = 1
4  OUTPUT
5  QLOAD=ALL
6  STRESS=ALL
7  DISPLACEMENTS=ALL
8  STRAIN=ALL
9  BEGIN BULK
10 CORDP 1
11 +XYP 1.0
12 QUAD4 1
13 QUAD4 2
14 QUAD4 3
15 QUAD4 4
16 QUAD4 5
17 QUAD4 6
18 QUAD4 7
19 QUAD4 8
20 QUAD4 9
21 QUAD4 10
22 QUAD4 11
23 QUAD4 12
24 QUAD4 13
25 QUAD4 14
26 QUAD4 15
27 QUAD4 16
28 QUAD4 17
29 QUAD4 18
30 QUAD4 19
31 QUAD4 20
32 QUAD4 21
33 QUAD4 22
34 QUAD4 23
35 QUAD4 24
36 QUAD4 25
37 FORCE 1
38 FORCE 1
39 FORCE 1
40

```

Case	Control	Deck	Echo	0.0	0.0	0.0	0.0	1.0	+XYP
7	8	0.0	0.0	0.0	0.0	0.0	0.0	1.0	7
8	9	0.0	0.0	0.0	0.0	0.0	0.0	1.0	8
9	10	0.0	0.0	0.0	0.0	0.0	0.0	1.0	9
10	11	0.0	0.0	0.0	0.0	0.0	0.0	1.0	10
11	12	0.0	0.0	0.0	0.0	0.0	0.0	1.0	11
12	13	0.0	0.0	0.0	0.0	0.0	0.0	1.0	12
13	14	0.0	0.0	0.0	0.0	0.0	0.0	1.0	13
14	15	0.0	0.0	0.0	0.0	0.0	0.0	1.0	14
15	16	0.0	0.0	0.0	0.0	0.0	0.0	1.0	15
16	17	0.0	0.0	0.0	0.0	0.0	0.0	1.0	16
17	18	0.0	0.0	0.0	0.0	0.0	0.0	1.0	17
18	19	0.0	0.0	0.0	0.0	0.0	0.0	1.0	18
19	20	0.0	0.0	0.0	0.0	0.0	0.0	1.0	19
20	21	0.0	0.0	0.0	0.0	0.0	0.0	1.0	20
21	22	0.0	0.0	0.0	0.0	0.0	0.0	1.0	21
22	23	0.0	0.0	0.0	0.0	0.0	0.0	1.0	22
23	24	0.0	0.0	0.0	0.0	0.0	0.0	1.0	23
24	25	0.0	0.0	0.0	0.0	0.0	0.0	1.0	24
25	26	0.0	0.0	0.0	0.0	0.0	0.0	1.0	25
26	27	0.0	0.0	0.0	0.0	0.0	0.0	1.0	26
27	28	0.0	0.0	0.0	0.0	0.0	0.0	1.0	27
28	29	0.0	0.0	0.0	0.0	0.0	0.0	1.0	28
29	30	0.0	0.0	0.0	0.0	0.0	0.0	1.0	29
30	31	0.0	0.0	0.0	0.0	0.0	0.0	1.0	30
31	32	0.0	0.0	0.0	0.0	0.0	0.0	1.0	31
32	33	0.0	0.0	0.0	0.0	0.0	0.0	1.0	32
33	34	0.0	0.0	0.0	0.0	0.0	0.0	1.0	33
34	35	0.0	0.0	0.0	0.0	0.0	0.0	1.0	34
35	36	0.0	0.0	0.0	0.0	0.0	0.0	1.0	35
36	37	0.0	0.0	0.0	0.0	0.0	0.0	1.0	36
37	38	0.0	0.0	0.0	0.0	0.0	0.0	1.0	37
38	39	0.0	0.0	0.0	0.0	0.0	0.0	1.0	38
39	40	0.0	0.0	0.0	0.0	0.0	0.0	1.0	39

D I S P L A C E M E N T V E C T O R

POINT ID.	TYPE	T1	T2	T3	R1	R2	R3
1	G	0.0	-4.586931E-06	0.0	-3.739354E-22	1.350191E-22	0.0
2	G	-2.200378E-06	-4.586931E-06	0.0	-2.377961E-21	-1.630386E-22	0.0
3	G	-4.400757E-06	-4.586931E-06	0.0	-3.200120E-21	-2.832612E-22	0.0
4	G	-6.601135E-06	-4.586931E-06	0.0	-2.924989E-21	-4.927746E-22	0.0
5	G	-8.801513E-06	-4.586931E-06	0.0	-1.749883E-21	-6.912656E-22	0.0
6	G	-1.100189E-05	-4.586931E-06	0.0	-2.375740E-22	-6.298611E-22	0.0
7	G	0.0	-3.669544E-06	0.0	-1.472087E-22	2.774235E-21	0.0
8	G	-2.200378E-06	-3.669544E-06	-7.287472E-21	-1.450458E-21	1.688386E-21	0.0
9	G	-4.400757E-06	-3.669544E-06	-1.095683E-20	-2.326285E-21	1.925028E-21	0.0
10	G	-6.601135E-06	-3.669544E-06	-1.044381E-20	-2.380581E-21	-1.288238E-21	0.0
11	G	-8.801513E-06	-3.669544E-06	-6.208032E-21	-1.622087E-21	-2.497188E-21	0.0
12	G	-1.100189E-05	-3.669544E-06	0.0	-3.016316E-22	-2.690360E-21	0.0
13	G	0.0	-2.752158E-06	0.0	-1.581260E-22	3.783715E-21	0.0
14	G	-2.200378E-06	-2.752158E-06	1.067713E-20	-4.566404E-22	2.795794E-21	0.0
15	G	-4.400757E-06	-2.752158E-06	-1.683721E-20	-8.041368E-22	6.983753E-22	0.0
16	G	-6.601135E-06	-2.752158E-06	-1.660366E-20	-9.408494E-22	-1.586746E-21	0.0
17	G	-8.801513E-06	-2.752158E-06	-1.021450E-20	-7.443478E-22	-3.527765E-21	0.0
18	G	-1.100189E-05	-2.752158E-06	0.0	-2.417337E-22	-4.189631E-21	0.0
19	G	0.0	-1.834772E-06	0.0	-3.906152E-24	3.473266E-21	0.0
20	G	-2.200378E-06	-1.834772E-06	-1.021954E-20	4.793626E-22	2.806174E-21	0.0
21	G	-4.400757E-06	-1.834772E-06	-1.660582E-20	6.688025E-22	8.647538E-22	0.0
22	G	-6.601135E-06	-1.834772E-06	-1.683507E-20	5.326254E-22	-1.420836E-21	0.0
23	G	-8.801513E-06	-1.834772E-06	-1.067168E-20	1.922288E-22	-3.517113E-21	0.0
24	G	-1.100189E-05	-1.834772E-06	0.0	-8.275889E-23	-4.500022E-21	0.0
25	G	0.0	-9.173861E-07	0.0	5.363293E-23	1.973869E-21	0.0
26	G	-2.200378E-06	-9.173861E-07	-6.212500E-21	1.355181E-21	1.77284E-21	0.0
27	G	-4.400757E-06	-9.173861E-07	-1.044628E-20	2.108294E-21	5.667328E-22	0.0
28	G	-6.601135E-06	-9.173861E-07	-1.095559E-20	2.054565E-21	-9.129689E-22	0.0
29	G	-8.801513E-06	-9.173861E-07	-7.282610E-21	1.187933E-21	-2.409765E-21	0.0
30	G	-1.100189E-05	-9.173861E-07	0.0	-9.025533E-23	-3.488668E-21	0.0
31	G	0.0	0.0	0.0	-1.634195E-23	-8.226114E-23	0.0
32	G	-2.200378E-06	0.0	0.0	1.483997E-21	-2.799380E-23	0.0
33	G	-4.400757E-06	0.0	0.0	2.653633E-21	-2.217385E-22	0.0
34	G	-6.601135E-06	0.0	0.0	2.928972E-21	-4.339853E-22	0.0
35	G	-8.801513E-06	0.0	0.0	2.114021E-21	-5.510358E-22	0.0
36	G	-1.100189E-05	0.0	0.0	1.404663E-22	-8.513567E-22	0.0

* * * END OF JOB * * *

N A S T R A N E X E C U T I V E C O N T R O L D E C K E C H O

ID COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1990
 APP DISPLACEMENT
 SOL 1
 TIME 10
 CEND

C A S E C O N T R O L D E C K E C H O

1 TITLE = PROBLEM 13 CASE 1 INPLANE LOADING
 2 LOAD = 1
 3 SPC = 1
 4 OUTPUT
 5 QLOAD=ALL
 6 STRESS=ALL
 7 **DISPLACEMENTS=ALL
 8 STRAIN=ALL
 9 BEGIN BULK

COUNT	1	2	3	4	5	6	7	8	9	10
1-	CCRD2R	1	0.0	0.0	0.0	0.0	0.0	0.0	1.0	1.0
2-	+XYP	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3-	QUAD4	1	1	2	3	4	5	6	7	8
4-	QUAD4	2	1	3	4	5	6	7	8	9
5-	QUAD4	3	1	4	5	6	7	8	9	10
6-	QUAD4	4	1	5	6	7	8	9	10	11
7-	QUAD4	5	1	6	7	8	9	10	11	12
8-	QUAD4	6	1	7	8	9	10	11	12	13
9-	QUAD4	7	1	8	9	10	11	12	13	14
10-	QUAD4	8	1	9	10	11	12	13	14	15
11-	QUAD4	9	1	10	11	12	13	14	15	16
12-	QUAD4	10	1	11	12	13	14	15	16	17
13-	QUAD4	11	1	12	13	14	15	16	17	18
14-	QUAD4	12	1	13	14	15	16	17	18	19
15-	QUAD4	13	1	14	15	16	17	18	19	20
16-	QUAD4	14	1	15	16	17	18	19	20	21
17-	QUAD4	15	1	16	17	18	19	20	21	22
18-	QUAD4	16	1	17	18	19	20	21	22	23
19-	QUAD4	17	1	18	19	20	21	22	23	24
20-	QUAD4	18	1	19	20	21	22	23	24	25
21-	QUAD4	19	1	20	21	22	23	24	25	26
22-	QUAD4	20	1	21	22	23	24	25	26	27
23-	QUAD4	21	1	22	23	24	25	26	27	28
24-	QUAD4	22	1	23	24	25	26	27	28	29
25-	QUAD4	23	1	24	25	26	27	28	29	30
26-	QUAD4	24	1	25	26	27	28	29	30	31
27-	QUAD4	25	1	26	27	28	29	30	31	32
28-	FORCE	1	6	30	31	32	33	34	35	36
29-	FORCE	1	12	-2.0	1.0	0.0	0.0	0.0	0.0	0.0
30-	FORCE	1	18	-4.0	1.0	0.0	0.0	0.0	0.0	0.0
31-	FORCE	1	24	-4.0	1.0	0.0	0.0	0.0	0.0	0.0

32-	FORCE	1	30	-4.0	1.0	0.0	1.0	0.0	0.0	0.0	1.0	0.0	0.0	
33-	FORCE	1	36	-2.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
34-	GRID	1		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
35-	GRID	2		3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
36-	GRID	3		6.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
37-	GRID	4		9.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
38-	GRID	5		12.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
39-	GRID	6		15.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
40-	GRID	7		0.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
41-	GRID	8		3.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
42-	GRID	9		6.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
43-	GRID	10		9.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
44-	GRID	11		12.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
45-	GRID	12		15.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
46-	GRID	13		0.0	8.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
47-	GRID	14		3.0	8.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
48-	GRID	15		6.0	8.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
49-	GRID	16		9.0	8.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
50-	GRID	17		12.0	8.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
51-	GRID	18		15.0	8.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
52-	GRID	19		0.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
53-	GRID	20		3.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
54-	GRID	21		6.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
55-	GRID	22		9.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
56-	GRID	23		12.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
57-	GRID	24		15.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
58-	GRID	25		0.0	16.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
59-	GRID	26		3.0	16.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
60-	GRID	27		6.0	16.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
61-	GRID	28		9.0	16.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
62-	GRID	29		12.0	16.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
63-	GRID	30		15.0	16.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
64-	GRID	31		0.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
65-	GRID	32		3.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
66-	GRID	33		6.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
67-	GRID	34		9.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
68-	GRID	35		12.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
69-	GRID	36		15.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
70-	MAT8	22		18.5E+06 1.6E+06 .25	.65E+06 4.0E+05 4.0E+05 .005									
71-	PARAM	GROUPNT	0											
72-	PCOMP	1												
73-	+KKQ	22		.0236 45.0	YES	.0236	22	.0236	-45.0	YES	22	.0236	-45.0	+KKP
74-	+KKQ	22		.0236 90.0	YES	.0236	22	.0236	0.0	YES	22	.0236	0.0	+KKQ
75-	+KKS	22		.0236 0.0	YES	.0236	22	.0236	90.0	YES	22	.0236	90.0	+KKS
76-	+KKR	22		.0236 -45.0	YES	.0236	22	.0236	45.0	YES	22	.0236	45.0	+KKR
77-	SPC1	1		6 1	THRU	6	36				6			
78-	SPC1	1		36 1	THRU	36	36				6			
79-	SPC1	1		36 6	12	36	36				18	24	30	36
80-	SPC1	1		136 1	7	136	36				13	19	25	31
81-	SPC1	1		236 31	THRU	236	36				36			
ENDDATA														

D I S P L A C E M E N T V E C T O R

POINT ID.	TYPE	T1	T2	T3	R1	R2	R3
1	G	0.0	-4.586931E-06	0.0	0.0	0.0	0.0
2	G	-2.200378E-06	-4.586931E-06	0.0	0.0	0.0	0.0
3	G	-4.400757E-06	-4.586931E-06	0.0	0.0	0.0	0.0
4	G	-6.601135E-06	-4.586931E-06	0.0	0.0	0.0	0.0
5	G	-8.801513E-06	-4.586931E-06	0.0	0.0	0.0	0.0
6	G	-1.100189E-05	-4.586931E-06	0.0	0.0	0.0	0.0
7	G	0.0	-3.669545E-06	0.0	0.0	0.0	0.0
8	G	-2.290378E-06	-3.669545E-06	0.0	0.0	0.0	0.0
9	G	-4.400757E-06	-3.669545E-06	0.0	0.0	0.0	0.0
10	G	-6.601135E-06	-3.669545E-06	0.0	0.0	0.0	0.0
11	G	-8.801513E-06	-3.669545E-06	0.0	0.0	0.0	0.0
12	G	-1.100189E-05	-3.669545E-06	0.0	0.0	0.0	0.0
13	G	0.0	-2.752158E-06	0.0	0.0	0.0	0.0
14	G	-2.200378E-06	-2.752158E-06	0.0	0.0	0.0	0.0
15	G	-4.400757E-06	-2.752158E-06	0.0	0.0	0.0	0.0
16	G	-6.601135E-06	-2.752158E-06	0.0	0.0	0.0	0.0
17	G	-8.801513E-06	-2.752158E-06	0.0	0.0	0.0	0.0
18	G	-1.100189E-05	-2.752158E-06	0.0	0.0	0.0	0.0
19	G	0.0	-1.834772E-06	0.0	0.0	0.0	0.0
20	G	-2.200378E-06	-1.834772E-06	0.0	0.0	0.0	0.0
21	G	-4.400757E-06	-1.834772E-06	0.0	0.0	0.0	0.0
22	G	-6.601135E-06	-1.834772E-06	0.0	0.0	0.0	0.0
23	G	-8.801513E-06	-1.834772E-06	0.0	0.0	0.0	0.0
24	G	-1.100189E-05	-1.834772E-06	0.0	0.0	0.0	0.0
25	G	0.0	-9.173861E-07	0.0	0.0	0.0	0.0
26	G	-2.200378E-06	-9.173861E-07	0.0	0.0	0.0	0.0
27	G	-4.400757E-06	-9.173861E-07	0.0	0.0	0.0	0.0
28	G	-6.601135E-06	-9.173861E-07	0.0	0.0	0.0	0.0
29	G	-8.801513E-06	-9.173861E-07	0.0	0.0	0.0	0.0
30	G	-1.100189E-05	-9.173861E-07	0.0	0.0	0.0	0.0
31	G	0.0	0.0	0.0	0.0	0.0	0.0
32	G	-2.200378E-06	0.0	0.0	0.0	0.0	0.0
33	G	-4.400757E-06	0.0	0.0	0.0	0.0	0.0
34	G	-6.601135E-06	0.0	0.0	0.0	0.0	0.0
35	G	-8.801513E-06	0.0	0.0	0.0	0.0	0.0
36	G	-1.100189E-05	0.0	0.0	0.0	0.0	0.0

* * * END OF JOB * * *

NASTRAN EXECUTIVE CONTROL DECK ECHO

ID COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1990
 APP DISPLACEMENT
 SOL 1,0
 TIME 10
 CEND

CASE CONTROL DECK ECHO

CARD COUNT	TITLE = PROBLEM 13	CASE 2	PRESSURE LOAD	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.0	+XYP
1	COR2R	1	1.0									
2	+XYP	1	1.0									
3	CQUAD4	1	1	2	8	7	1					
4	CQUAD4	2	1	3	9	6	1					
5	CQUAD4	3	1	4	10	9	1					
6	CQUAD4	4	1	5	11	10	1					
7	CQUAD4	5	1	6	12	11	1					
8	CQUAD4	6	1	7	13	12	1					
9	CQUAD4	7	1	8	14	13	1					
10	CQUAD4	8	1	9	15	14	1					
11	CQUAD4	9	1	10	16	15	1					
12	CQUAD4	10	1	11	17	16	1					
13	CQUAD4	11	1	12	18	17	1					
14	CQUAD4	12	1	13	19	18	1					
15	CQUAD4	13	1	14	20	19	1					
16	CQUAD4	14	1	15	21	20	1					
17	CQUAD4	15	1	16	22	21	1					
18	CQUAD4	16	1	17	23	22	1					
19	CQUAD4	17	1	18	24	23	1					
20	CQUAD4	18	1	19	25	24	1					
21	CQUAD4	19	1	20	26	25	1					
22	CQUAD4	20	1	21	27	26	1					
23	CQUAD4	21	1	22	28	27	1					
24	CQUAD4	22	1	23	29	28	1					
25	CQUAD4	23	1	24	30	29	1					
26	CQUAD4	24	1	25	31	30	1					
27	CQUAD4	25	1	26	32	31	1					
28	GRID	1	1	27	33	32	1					
29	GRID	2	1	28	34	33	1					
30	GRID	3	1	29	35	34	1					

D I S P L A C E M E N T V E C T O R

POINT ID.	TYPE	T1	T2	T3	R1	R2	R3
1	G	0.0	-1.778707E-19	0.0	-1.210504E-03	1.323176E-03	0.0
2	G	-6.450161E-20	-2.176825E-19	0.0	-1.668269E-02	-2.098859E-04	0.0
3	G	-1.034476E-19	-2.616967E-19	0.0	-2.286059E-02	1.153546E-04	0.0
4	G	-1.195788E-19	-2.633216E-19	0.0	-2.066865E-02	-5.393717E-04	0.0
5	G	-1.1218607E-19	-2.115229E-19	0.0	-1.161163E-02	-8.255243E-04	0.0
6	G	-1.204124E-19	-1.236314E-19	0.0	1.190341E-04	4.164568E-04	0.0
7	G	0.0	-1.481314E-19	0.0	6.716475E-05	2.187735E-02	0.0
8	G	-2.842967E-20	-1.810125E-19	-5.659059E-02	-9.983173E-03	1.411448E-02	0.0
9	G	-8.923749E-20	-2.230359E-19	-8.471517E-02	-1.663885E-02	3.682787E-03	0.0
10	G	-1.576236E-19	-2.311706E-19	-8.078058E-02	-1.696210E-02	-6.537446E-03	0.0
11	G	-2.094548E-19	-1.946830E-19	-4.827733E-02	-1.106920E-02	-1.479205E-02	0.0
12	G	-2.278775E-19	-1.268147E-19	0.0	-9.165117E-04	-1.563870E-02	0.0
13	G	0.0	-1.115810E-19	0.0	-1.869885E-04	2.978237E-02	0.0
14	G	-1.794663E-20	-1.250619E-19	-8.308844E-02	-2.497156E-03	2.273052E-02	0.0
15	G	-8.58504E-20	-1.460906E-19	-1.305235E-01	-5.125305E-03	7.580590E-03	0.0
16	G	-1.763567E-19	-1.563161E-19	-1.287307E-01	-6.098102E-03	-8.856641E-03	0.0
17	G	-2.526314E-19	-1.483047E-19	-7.95323E-02	-4.516714E-03	-2.280502E-02	0.0
18	G	-2.825843E-19	-1.291955E-19	0.0	-6.436033E-04	-2.735314E-02	0.0
19	G	0.0	-7.106225E-20	0.0	6.436033E-04	2.735314E-02	0.0
20	G	-1.210820E-20	-6.589079E-20	-7.95323E-02	4.516714E-03	2.280602E-02	0.0
21	G	-7.136112E-20	-6.327438E-20	-1.287307E-01	6.098102E-03	8.856641E-03	0.0
22	G	-1.546190E-19	-7.015307E-20	-1.305235E-01	5.125305E-03	-7.580590E-03	0.0
23	G	-2.290863E-19	-8.759032E-20	-8.308844E-02	2.497156E-03	-2.273052E-02	0.0
24	G	-2.619817E-19	-1.198988E-19	0.0	1.869885E-04	-2.978237E-02	0.0
25	G	0.0	-3.792629E-20	0.0	9.165117E-04	1.563870E-02	0.0
26	G	-1.775468E-20	-2.082724E-20	-4.827733E-02	1.106920E-02	1.479205E-02	0.0
27	G	-5.709451E-20	-6.595195E-21	-8.078058E-02	1.696210E-02	6.537446E-03	0.0
28	G	-1.057224E-19	-8.386640E-21	-8.471517E-02	1.663885E-02	-3.682787E-03	0.0
29	G	-1.524044E-19	-2.939486E-20	-5.659059E-02	9.983173E-03	-1.411448E-02	0.0
30	G	-1.795011E-19	-8.353070E-20	0.0	-6.716475E-05	-2.187735E-02	0.0
31	G	0.0	0.0	0.0	-1.190341E-04	-4.164568E-04	0.0
32	G	-3.228108E-20	0.0	0.0	1.161163E-02	8.255243E-04	0.0
33	G	-5.761368E-20	0.0	0.0	2.066865E-02	5.393717E-04	0.0
34	G	-7.971998E-20	0.0	0.0	2.286059E-02	-1.153546E-04	0.0
35	G	-9.792119E-20	0.0	0.0	1.668269E-02	2.098859E-04	0.0
36	G	-1.171910E-19	0.0	0.0	1.210504E-03	-1.323176E-03	0.0

* * * END OF JOB * * *

NASTRAN EXECUTIVE CONTROL DECK ECHO

ID COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1990
 APP DISPLACEMENT
 SOL 1
 TIME 10
 CEND

CASE CONTROL DECK ECHO

1 TITLE = PROBLEM 13 CASE 2 PRESSURE LOAD
 2 LOAD = 1
 3 SPC = 1
 4 OUTPUT
 5 OLLOAD=ALL
 6 STRESS=ALL
 7 DISPLACEMENTS=ALL
 8 STRAIN=ALL
 9 BEGIN BULK

SORTED BULK DATA ECHO

CARD COUNT	1	2	3	4	5	6	7	8	9	10
1-	CORD2R	1.0	0.0	0.0	0.0	0.0	0.0	0.0	1.0	+XYP
2-	+XYP	1.0	1.0	2	8	7	1	1		
3-	CQUADA	1	1	3	9	8	1	1		
4-	CQUADA	2	1	4	10	9	1	1		
5-	CQUADA	3	1	5	11	10	1	1		
6-	CQUADA	4	1	6	12	11	1	1		
7-	CQUADA	5	1	7	13	12	1	1		
8-	CQUADA	6	1	8	14	13	1	1		
9-	CQUADA	7	1	9	15	14	1	1		
10-	CQUADA	8	1	10	16	15	1	1		
11-	CQUADA	9	1	11	17	16	1	1		
12-	CQUADA	10	1	12	18	17	1	1		
13-	CQUADA	11	1	13	19	18	1	1		
14-	CQUADA	12	1	14	20	19	1	1		
15-	CQUADA	13	1	15	21	20	1	1		
16-	CQUADA	14	1	16	22	21	1	1		
17-	CQUADA	15	1	17	23	22	1	1		
18-	CQUADA	16	1	18	24	23	1	1		
19-	CQUADA	17	1	19	25	24	1	1		
20-	CQUADA	18	1	20	26	25	1	1		
21-	CQUADA	19	1	21	27	26	1	1		
22-	CQUADA	20	1	22	28	27	1	1		
23-	CQUADA	21	1	23	29	28	1	1		
24-	CQUADA	22	1	24	30	29	1	1		
25-	CQUADA	23	1	25	31	30	1	1		
26-	CQUADA	24	1	26	32	31	1	1		
27-	CQUADA	25	1	27	33	32	1	1		
28-	CQUADA	24	1	28	34	33	1	1		
29-	CQUADA	25	1	29	35	34	1	1		
30-	GRID	1	1	30	36	35	1	1		
31-	GRID	2	0.0	0.0	0.0	0.0	0.0	0.0		
32-	GRID	3	3.0	0.0	0.0	0.0	0.0	0.0		
33-	GRID	4	6.0	0.0	0.0	0.0	0.0	0.0		
34-	GRID	5	9.0	0.0	0.0	0.0	0.0	0.0		
35-	GRID	5	12.0	0.0	0.0	0.0	0.0	0.0		

33-	GRID	6	15.0	0.0	0.0	0.0	.65E+06	4.0E+05	.005	+KKP
34-	GRID	7	0.0	4.0	0.0	0.0				+KKQ
35-	GRID	8	3.0	4.0	0.0	0.0				+KKS
36-	GRID	9	6.0	4.0	0.0	0.0				+KKR
37-	GRID	10	9.0	4.0	0.0	0.0				
38-	GRID	11	12.0	4.0	0.0	0.0				
39-	GRID	12	15.0	4.0	0.0	0.0				
40-	GRID	13	0.0	8.0	0.0	0.0				
41-	GRID	14	3.0	8.0	0.0	0.0				
42-	GRID	15	6.0	8.0	0.0	0.0				
43-	GRID	16	9.0	8.0	0.0	0.0				
44-	GRID	17	12.0	8.0	0.0	0.0				
45-	GRID	18	15.0	8.0	0.0	0.0				
46-	GRID	19	0.0	12.0	0.0	0.0				
47-	GRID	20	3.0	12.0	0.0	0.0				
48-	GRID	21	6.0	12.0	0.0	0.0				
49-	GRID	22	9.0	12.0	0.0	0.0				
50-	GRID	23	12.0	12.0	0.0	0.0				
51-	GRID	24	15.0	12.0	0.0	0.0				
52-	GRID	25	0.0	16.0	0.0	0.0				
53-	GRID	26	3.0	16.0	0.0	0.0				
54-	GRID	27	6.0	16.0	0.0	0.0				
55-	GRID	28	9.0	16.0	0.0	0.0				
56-	GRID	29	12.0	16.0	0.0	0.0				
57-	GRID	30	15.0	16.0	0.0	0.0				
58-	GRID	31	0.0	20.0	0.0	0.0				
59-	GRID	32	3.0	20.0	0.0	0.0				
60-	GRID	33	6.0	20.0	0.0	0.0				
61-	GRID	34	9.0	20.0	0.0	0.0				
62-	GRID	35	12.0	20.0	0.0	0.0				
63-	GRID	36	15.0	20.0	0.0	0.0				
64-	MAT8	22	18.5E+06	1.6E+06	.25					
65-	PCGRP	1								
66-	+KKP	22	.0236	45.0	YES	22	.0236	-45.0	YES	+KKQ
67-	+KKQ	22	.0236	90.0	YES	22	.0236	0.0	YES	+KKS
68-	+KKS	22	.0236	0.0	YES	22	.0236	90.0	YES	+KKR
69-	+KKR	22	.0236	-45.0	YES	22	.0236	45.0	YES	
70-	PLAD2	1	-2.0	1	THRU	25				
71-	SPI	1	6	1	THRU	36				
72-	SPI	1	36	1	THRU	6				
73-	SPI	1	36	6	12	18	24	30	36	
74-	SPI	1	136	1	7	13	16	25	31	
75-	SPI	1	236	31	THRU	36				
	ENDDATA									
	TOTAL COUNT=	75								

D I S P L A C E M E N T V E C T O R

POINT ID.	TYPE	T1	T2	T3	R1	R2	R3
1	G	0.0	0.0	0.0	-1.659534E-03	1.630020E-03	0.0
2	G	0.0	0.0	0.0	-1.568235E-02	-2.24436E-04	0.0
3	G	0.0	0.0	0.0	-2.187067E-02	3.227942E-05	0.0
4	G	0.0	0.0	0.0	-1.995040E-02	-5.664070E-04	0.0
5	G	0.0	0.0	0.0	-1.137580E-02	-9.744150E-04	0.0
6	G	0.0	0.0	0.0	3.201080E-04	5.167378E-04	0.0
7	G	0.0	0.0	0.0	1.627950E-04	2.09241E-02	0.0
8	G	0.0	0.0	-5.535023E-02	-9.825311E-03	1.395945E-02	0.0
9	G	0.0	0.0	-8.360141E-02	-1.605320E-02	3.755062E-03	0.0
10	G	0.0	0.0	-8.008110E-02	-1.632954E-02	-6.32517E-03	0.0
11	G	0.0	0.0	-4.817439E-02	-1.057774E-02	-1.449008E-02	0.0
12	G	0.0	0.0	0.0	-1.202052E-03	-1.558065E-02	0.0
13	G	0.0	0.0	0.0	-4.916388E-05	2.914780E-02	0.0
14	G	0.0	0.0	-8.209246E-02	-2.443614E-03	2.221869E-02	0.0
15	G	0.0	0.0	-1.268863E-01	-5.031536E-03	7.453628E-03	0.0
16	G	0.0	0.0	-1.272077E-01	-5.982229E-03	-8.563215E-03	0.0
17	G	0.0	0.0	-7.872557E-02	-4.491568E-03	-2.233347E-02	0.0
18	G	0.0	0.0	0.0	-6.876041E-04	-2.682007E-02	0.0
19	G	0.0	0.0	0.0	6.876041E-04	2.682007E-02	0.0
20	G	0.0	0.0	-7.872557E-02	4.491668E-03	2.233347E-02	0.0
21	G	0.0	0.0	-1.272077E-01	5.982229E-03	8.563215E-03	0.0
22	G	0.0	0.0	-1.268863E-01	5.031536E-03	-7.453628E-03	0.0
23	G	0.0	0.0	-8.209246E-02	2.443614E-03	-2.221869E-02	0.0
24	G	0.0	0.0	0.0	4.916388E-05	-2.914780E-02	0.0
25	G	0.0	0.0	0.0	1.202066E-03	1.558065E-02	0.0
26	G	0.0	0.0	-4.817439E-02	1.057774E-02	-1.449008E-02	0.0
27	G	0.0	0.0	-8.008110E-02	1.632954E-02	6.32517E-03	0.0
28	G	0.0	0.0	-8.360141E-02	1.605320E-02	-3.755062E-03	0.0
29	G	0.0	0.0	-5.535023E-02	9.825311E-03	-1.395945E-02	0.0
30	G	0.0	0.0	0.0	-1.627950E-04	2.09241E-02	0.0
31	G	0.0	0.0	0.0	-3.201080E-04	5.167378E-04	0.0
32	G	0.0	0.0	0.0	1.627950E-04	2.09241E-02	0.0
33	G	0.0	0.0	0.0	1.137580E-02	9.744150E-04	0.0
34	G	0.0	0.0	0.0	1.995040E-02	5.664070E-04	0.0
35	G	0.0	0.0	0.0	2.187067E-02	-3.227942E-05	0.0
36	G	0.0	0.0	0.0	1.568235E-02	2.294436E-04	0.0
36	G	0.0	0.0	0.0	1.659534E-03	-1.630020E-03	0.0

* * * END OF JOB * * *

NASTRAN EXECUTIVE CONTROL DECK ECHO

ID COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1990
APP DISPLACEMENT
SOL 3,0
TIME 10
CEND

CASE CONTROL DECK ECHO

CARD
COUNT

METHOD = 1
SPC = 1
OUTPUT
DISPLACEMENTS(PRINT, PUNCH)=ALL
BEGIN BULK

CARD	COUNT	---1---	+++2+++	---3---	+++4+++	---5---	+++6+++	---7---	+++8+++	---9---	+++10+++
1	1	COOR2R	1.0	0.0	0.0	0.0	0.0	0.0	0.0	1.0	1.0
2	1	+ABC	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3	1	QUADA	1	1	2	3	4	5	6	7	8
4	1	QUADA	2	1	3	4	5	6	7	8	9
5	1	QUADA	3	1	4	5	6	7	8	9	10
6	1	QUADA	4	1	5	6	7	8	9	10	11
7	1	QUADA	5	1	6	7	8	9	10	11	12
8	1	QUADA	6	1	7	8	9	10	11	12	13
9	1	QUADA	7	1	8	9	10	11	12	13	14
10	1	QUADA	8	1	9	10	11	12	13	14	15
11	1	QUADA	9	1	10	11	12	13	14	15	16
12	1	QUADA	10	1	11	12	13	14	15	16	17
13	1	QUADA	11	1	12	13	14	15	16	17	18
14	1	QUADA	12	1	13	14	15	16	17	18	19
15	1	QUADA	13	1	14	15	16	17	18	19	20
16	1	QUADA	14	1	15	16	17	18	19	20	21
17	1	QUADA	15	1	16	17	18	19	20	21	22
18	1	QUADA	16	1	17	18	19	20	21	22	23
19	1	QUADA	17	1	18	19	20	21	22	23	24
20	1	QUADA	18	1	19	20	21	22	23	24	25
21	1	QUADA	19	1	20	21	22	23	24	25	26
22	1	QUADA	20	1	21	22	23	24	25	26	27
23	1	QUADA	21	1	22	23	24	25	26	27	28
24	1	QUADA	22	1	23	24	25	26	27	28	29
25	1	QUADA	23	1	24	25	26	27	28	29	30
26	1	QUADA	24	1	25	26	27	28	29	30	31
27	1	QUADA	25	1	26	27	28	29	30	31	32
28	1	EIGP	1	INV	29	30	31	32	33	34	35
29	1	+WXY	MAX		100.						

+WXY

Line	Label	Value 1	Value 2	Value 3	Value 4	Value 5	Value 6	Value 7	Value 8
30-	GRID	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
31-	GRID	3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
32-	GRID	6.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
33-	GRID	9.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
34-	GRID	12.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
35-	GRID	15.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
36-	GRID	0.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0
37-	GRID	3.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0
38-	GRID	6.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0
39-	GRID	9.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0
40-	GRID	12.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0
41-	GRID	15.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0
42-	GRID	0.0	8.0	0.0	0.0	0.0	0.0	0.0	0.0
43-	GRID	3.0	8.0	0.0	0.0	0.0	0.0	0.0	0.0
44-	GRID	6.0	8.0	0.0	0.0	0.0	0.0	0.0	0.0
45-	GRID	9.0	8.0	0.0	0.0	0.0	0.0	0.0	0.0
46-	GRID	12.0	8.0	0.0	0.0	0.0	0.0	0.0	0.0
47-	GRID	15.0	8.0	0.0	0.0	0.0	0.0	0.0	0.0
48-	GRID	0.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0
49-	GRID	3.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0
50-	GRID	6.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0
51-	GRID	9.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0
52-	GRID	12.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0
53-	GRID	15.0	12.0	0.0	0.0	0.0	0.0	0.0	0.0
54-	GRID	0.0	16.0	0.0	0.0	0.0	0.0	0.0	0.0
55-	GRID	3.0	16.0	0.0	0.0	0.0	0.0	0.0	0.0
56-	GRID	6.0	16.0	0.0	0.0	0.0	0.0	0.0	0.0
57-	GRID	9.0	16.0	0.0	0.0	0.0	0.0	0.0	0.0
58-	GRID	12.0	16.0	0.0	0.0	0.0	0.0	0.0	0.0
59-	GRID	15.0	16.0	0.0	0.0	0.0	0.0	0.0	0.0
60-	GRID	0.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0
61-	GRID	3.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0
62-	GRID	6.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0
63-	GRID	9.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0
64-	GRID	12.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0
65-	GRID	15.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0
66-	MATB	18.5E+06	18.5E+06	18.5E+06	18.5E+06	18.5E+06	18.5E+06	18.5E+06	18.5E+06
67-	PAPAM	0	0	0	0	0	0	0	0
68-	PCOMP1	45.0	-45.0	90.0	0.0	0.0	0.0	0.0	0.0
69-	+BC	1	6	1	THRU	THRU	THRU	THRU	THRU
70-	SPC1	1	36	1	6	6	6	6	6
71-	SPC1	1	36	6	12	12	12	12	12
72-	SPC1	1	36	6	7	7	7	7	7
73-	SPC1	1	236	236	7	7	7	7	7
74-	ENDDATA								

R E A L E I G E N V A L U E S

MODE NO.	EXTRACTION ORDER	EIGENVALUE	RADIAN FREQUENCY	CYCLIC FREQUENCY	GENERALIZED MASS	GENERALIZED STIFFNESS
1	1	2.206153E+04	1.485312E+02	2.363948E+01	8.482909E-02	1.871459E+03
2	2	9.134000E+04	3.022251E+02	4.810061E+01	8.233906E-02	7.520849E+03
3	3	1.469552E+05	3.833473E+02	6.101162E+01	7.591807E-02	1.130351E+04
4	4	2.678455E+05	5.175388E+02	8.236885E+01	6.357978E-02	1.697605E+04
5	5	3.130239E+05	5.595389E+02	8.905338E+01	6.320361E-02	1.978803E+04
6	6	5.373900E+05	7.330688E+02	1.166715E+02	5.950838E-02	3.197921E+04

* * * * * END OF JOB * * *

NASTRAN EXECUTIVE CONTROL DECK ECHO

ID COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1990
 APP DISPLACEMENT
 SOL 3
 TIME 10
 CEND

CASE CONTROL DECK ECHO

CARD
 COUNT
 1
 2
 3
 4
 5
 METHOD = 1
 SPC = 1
 OUTPUT
 DISPLACEMENTS(PRINT,PUNCH)=ALL
 BEGIN BULK

CARD COUNT	1	2	3	4	5	6	7	8	9	10
1-	CORD2R	1	0.0	0.0	0.0	0.0	0.0	0.0	1.0	10
2-	+ABC	1.0	0.0	1.0	0.0	0.0	0.0	0.0	1.0	+ABC
3-	QUADA	1	1	2	3	4	5	6	7	8
4-	QUADA	2	1	3	4	5	6	7	8	9
5-	QUADA	3	1	4	5	6	7	8	9	10
6-	QUADA	4	1	5	6	7	8	9	10	11
7-	QUADA	5	1	6	7	8	9	10	11	12
8-	QUADA	6	1	7	8	9	10	11	12	13
9-	QUADA	7	1	8	9	10	11	12	13	14
10-	QUADA	8	1	9	10	11	12	13	14	15
11-	QUADA	9	1	10	11	12	13	14	15	16
12-	QUADA	10	1	11	12	13	14	15	16	17
13-	QUADA	11	1	12	13	14	15	16	17	18
14-	QUADA	12	1	13	14	15	16	17	18	19
15-	QUADA	13	1	14	15	16	17	18	19	20
16-	QUADA	14	1	15	16	17	18	19	20	21
17-	QUADA	15	1	16	17	18	19	20	21	22
18-	QUADA	16	1	17	18	19	20	21	22	23
19-	QUADA	17	1	18	19	20	21	22	23	24
20-	QUADA	18	1	19	20	21	22	23	24	25
21-	QUADA	19	1	20	21	22	23	24	25	26
22-	QUADA	20	1	21	22	23	24	25	26	27
23-	QUADA	21	1	22	23	24	25	26	27	28
24-	QUADA	22	1	23	24	25	26	27	28	29
25-	QUADA	23	1	24	25	26	27	28	29	30
26-	QUADA	24	1	25	26	27	28	29	30	31
27-	QUADA	25	1	26	27	28	29	30	31	32
28-	ETGR	1	INV	0.0	100.	20	6			
29-	+WXY	MAX								+WXY
30-	GRID	1		0.0	0.0	0.0				0.0
31-	GRID	2		3.0	0.0	0.0				0.0
32-	GRID	3		6.0	0.0	0.0				0.0
33-	GRID	4		9.0	0.0	0.0				0.0

MODE NO.	EXTRACTION ORDER	EIGENVALUE	R E A L RADIANS	E I G E N V A L U E S CYCLES	GENERALIZED MASS	GENERALIZED STIFFNESS
1	1	2.232679E+04	1.494215E+02	2.378117E+01	8.489105E-02	1.895345E+03
2	2	9.354261E+04	3.058474E+02	4.867712E+01	8.149919E-02	7.523547E+03
3	3	1.493206E+05	3.864202E+02	6.150068E+01	7.644512E-02	1.141483E+04
4	4	2.815449E+05	5.306080E+02	8.444889E+01	6.521134E-02	1.835992E+04
5	5	3.217625E+05	5.672411E+02	9.027923E+01	6.557091E-02	2.109826E+04
6	6	5.805212E+05	7.619194E+02	1.212632E+02	7.699291E-02	4.469601E+04

* * * END OF JOB * * *

NASTRAN EXECUTIVE CONTROL DECK FCHO

ID COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1990
 APP DISPLACEMENT
 SOL 1,0
 TIME 10
 CEND

CASE CONTROL DECK ECHO

1 TITLE = PROBLEM 14 SANDWICH PLATE COMPOSITE FACE SHEETS
 2 LOAD = 1
 3 SPC = 1
 4 OUTPUT
 5 QLOAD=ALL
 6 STRESS=ALL
 7 DISPLACEMENTS=ALL
 8 STRAIN=ALL
 9 BEGIN BULK

CARD COUNT	---	1----	++2+++	---3---	+++4+++	---5---	+++6+++	---7---	+++8+++	---9---	+++10+++
1-	CORD2R	1	1.0	0.0	0.0	0.0	0.0	0.0	0.0	1.0	+XYP
2-	+XYP	1	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
3-	QUAD4	1	72	72	2	8	9	7	1		
4-	QUAD4	2	72	72	3	9	10	8	1		
5-	QUAD4	3	72	72	4	10	11	9	1		
6-	QUAD4	4	72	72	5	11	12	10	1		
7-	QUAD4	5	72	72	6	12	13	11	1		
8-	QUAD4	6	72	72	7	13	14	12	1		
9-	QUAD4	7	72	72	8	14	15	13	1		
10-	QUAD4	8	72	72	9	15	16	14	1		
11-	QUAD4	9	72	72	10	16	17	15	1		
12-	QUAD4	10	72	72	11	17	18	16	1		
13-	QUAD4	11	72	72	12	18	19	17	1		
14-	QUAD4	12	72	72	13	19	20	18	1		
15-	QUAD4	13	72	72	14	20	21	19	1		
16-	QUAD4	14	72	72	15	21	22	20	1		
17-	QUAD4	15	72	72	16	22	23	21	1		
18-	QUAD4	16	72	72	17	23	24	22	1		
19-	QUAD4	17	72	72	18	24	25	23	1		
20-	QUAD4	18	72	72	19	25	26	24	1		
21-	QUAD4	19	72	72	20	26	27	25	1		
22-	QUAD4	20	72	72	21	27	28	26	1		
23-	QUAD4	21	72	72	22	28	29	27	1		
24-	QUAD4	22	72	72	23	29	30	28	1		
25-	QUAD4	23	72	72	24	30	31	29	1		
26-	QUAD4	24	72	72	25	31	32	30	1		
27-	QUAD4	25	72	72	26	32	33	31	1		
28-	GRID	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
29-	GRID	2	3.0	0.0	0.0	0.0	0.0	0.0	0.0		
30-	GRID	3	6.0	0.0	0.0	0.0	0.0	0.0	0.0		

31-	GRID	4	9.0	0.0	0.0	.55E+06	4.0E+05	4.0E+05	.005		+ABC
32-	GRID	5	12.0	0.0	0.0						+DEF
33-	GRID	6	15.0	0.0	0.0						+EFG
34-	GRID	7	0.0	4.0	0.0						+FGH
35-	GRID	8	3.0	4.0	0.0						+GHI
36-	GRID	9	6.0	4.0	0.0						
37-	GRID	10	9.0	4.0	0.0						
38-	GRID	11	12.0	4.0	0.0						
39-	GRID	12	15.0	4.0	0.0						
40-	GRID	13	0.0	8.0	0.0						
41-	GRID	14	3.0	8.0	0.0						
42-	GRID	15	6.0	8.0	0.0						
43-	GRID	16	9.0	8.0	0.0						
44-	GRID	17	12.0	8.0	0.0						
45-	GRID	18	15.0	8.0	0.0						
46-	GRID	19	0.0	12.0	0.0						
47-	GRID	20	3.0	12.0	0.0						
48-	GRID	21	6.0	12.0	0.0						
49-	GRID	22	9.0	12.0	0.0						
50-	GRID	23	12.0	12.0	0.0						
51-	GRID	24	15.0	12.0	0.0						
52-	GRID	25	0.0	16.0	0.0						
53-	GRID	26	3.0	16.0	0.0						
54-	GRID	27	6.0	16.0	0.0						
55-	GRID	28	9.0	16.0	0.0						
56-	GRID	29	12.0	16.0	0.0						
57-	GRID	30	15.0	16.0	0.0						
58-	GRID	31	0.0	20.0	0.0						
59-	GRID	32	3.0	20.0	0.0						
60-	GRID	33	6.0	20.0	0.0						
61-	GRID	34	9.0	20.0	0.0						
62-	GRID	35	12.0	20.0	0.0						
63-	GRID	36	15.0	20.0	0.0						
64-	MAT1	10	3.0E+04	.25							
65-	MAT8	22	18.5E+06	1.6E+06	.25						
66-	PCOMP	72	.0236	45.	YES	22	.0236	-45.	YES		
67-	+ABC	22	.0236	90.	YES	22	.0236	0.0	YES		
68-	+DEF	22	1.0	0.0	YES	22	.0236	0.0	YES		
69-	+EFG	10	.0236	90.0	YES	22	.0236	-45.0	YES		
70-	+FGH	22	.0236	45.0	YES	22					
71-	+GHI	22	-2.0	1	THRU	25					
72-	PLOAD2	1	6	1	THRU	36					
73-	SPC1	1	36	1	THRU	6					
74-	SPC1	1	36	6	12	18	24	30	36		
75-	SPC1	1	136	1	7	13	19	25	31		
76-	SPC1	1	236	31	THRU	36					
77-	ENDATA										

D I S P L A C E M E N T V E C T O R

POINT ID.	TYPE	T1	T2	T3	R1	R2	R3
1	G	0.0	-1.262361E-21	0.0	-1.143164E-04	7.499255E-05	0.0
2	G	-4.790143E-22	-3.355112E-21	0.0	-1.790529E-04	7.432398E-05	0.0
3	G	-2.955672E-22	-4.562165E-21	0.0	-2.559781E-04	4.713882E-05	0.0
4	G	-1.264729E-22	-3.982578E-21	0.0	-2.455365E-04	-1.063998E-05	0.0
5	G	-3.145644E-22	-1.929802E-21	0.0	-1.535654E-04	-5.459381E-05	0.0
6	G	-6.515719E-22	-4.187489E-21	0.0	-9.322596E-05	-6.597897E-05	0.0
7	G	0.0	-1.765623E-21	0.0	-1.063590E-04	2.292916E-04	0.0
8	G	1.32215E-21	-3.720921E-21	-9.309109E-04	-1.386407E-04	1.791623E-04	0.0
9	G	1.130583E-21	-4.489881E-21	-1.361827E-03	-1.963340E-04	6.680239E-05	0.0
10	G	6.925907E-23	-3.550335E-21	-1.314377E-03	-1.916281E-04	-5.423765E-05	0.0
11	G	-1.060839E-21	-1.285317E-21	-8.131079E-04	-1.148891E-04	-1.635653E-04	0.0
12	G	-1.756958E-21	8.511679E-22	0.0	-7.354622E-05	-2.062164E-04	0.0
13	G	0.0	-3.076516E-21	0.0	-6.418965E-05	3.436314E-04	0.0
14	G	9.844685E-22	-4.408565E-21	-1.309181E-03	-5.804963E-05	2.753212E-04	0.0
15	G	2.347015E-22	-4.176753E-21	-2.026409E-03	-6.767749E-05	9.872849E-05	0.0
16	G	-1.484854E-21	-2.611802E-21	-2.008277E-03	-5.978833E-05	-1.001152E-04	0.0
17	G	-3.134186E-21	-2.276627E-23	-1.272991E-03	-2.516000E-05	-2.728444E-04	0.0
18	G	-4.084607E-21	3.144208E-21	0.0	-1.208915E-05	-3.363470E-04	0.0
19	G	0.0	-3.969974E-21	0.0	1.208915E-05	3.363470E-04	0.0
20	G	-3.581725E-22	-4.373653E-21	-1.272991E-03	2.516000E-05	2.728444E-04	0.0
21	G	-2.106933E-21	-3.416818E-21	-2.008277E-03	5.978833E-05	1.001152E-04	0.0
22	G	-4.344522E-21	-1.542182E-21	-2.026409E-03	6.767749E-05	-9.872849E-05	0.0
23	G	-5.108684E-21	1.052242E-21	-1.309181E-03	5.804963E-05	-2.753212E-04	0.0
24	G	-6.847433E-21	4.573085E-21	0.0	6.418965E-05	-3.436314E-04	0.0
25	G	0.0	-3.377656E-21	0.0	7.354622E-05	2.062164E-04	0.0
26	G	-2.382802E-21	-2.936029E-21	-8.131079E-04	1.148891E-04	1.635653E-04	0.0
27	G	-5.172902E-21	-1.979701E-21	-1.314377E-03	1.916281E-04	5.423765E-05	0.0
28	G	-7.664944E-21	-5.526538E-22	-1.361827E-03	1.983930E-04	-6.680239E-05	0.0
29	G	-9.268735E-21	1.075791E-21	-9.309109E-04	1.386407E-04	-1.791623E-04	0.0
30	G	-9.495322E-21	3.923865E-21	0.0	1.063590E-04	-2.292916E-04	0.0
31	G	0.0	0.0	0.0	9.322596E-05	6.597897E-05	0.0
32	G	-4.600099E-21	0.0	0.0	1.535654E-04	5.459381E-05	0.0
33	G	-8.168351E-21	0.0	0.0	2.455365E-04	1.063998E-05	0.0
34	G	-1.056813E-20	0.0	0.0	2.559781E-04	-4.713882E-05	0.0
35	G	-1.165361E-20	0.0	0.0	1.790529E-04	-7.432398E-05	0.0
36	G	-1.132367E-20	0.0	0.0	1.143164E-04	-7.499255E-05	0.0

* * * END OF JOB * * *

NASTRAN EXECUTIVE CONTROL DECK ECHO

IG COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1990
 APP DISPLACEMENT
 SOL 1
 TIME 10
 CEND

CASE CONTROL DECK ECHO

TITLE = PROBLEM 14 SANDWICH PLATE COMPOSITE FACE SHEETS
 LOAD = 1
 SPC = 1
 OUTPUT
 OLOAD=ALL
 STRESS=ALL
 DISPLACEMENTS=ALL
 STRAIN=ALL
 BEGIN BULK
 INPUT BULK DATA CARD COUNT = 78

CARD COUNT	1	2	3	4	5	6	7	8	9	10
1	CARD2R	1	0.0	0.0	0.0	0.0	0.0	0.0	1.0	10
2	+XYP	1.0	1.0							
3	QUADA 1	0.0	1	2	8	7	1	1		
4	QUADA 2	72	2	3	9	8	1	1		
5	QUADA 3	72	3	4	10	9	1	1		
6	QUADA 4	72	4	5	11	10	1	1		
7	QUADA 5	72	5	6	12	11	1	1		
8	QUADA 6	72	7	8	14	13	1	1		
9	QUADA 7	72	8	9	15	14	1	1		
10	QUADA 8	72	9	10	16	15	1	1		
11	QUADA 9	72	10	11	17	16	1	1		
12	QUADA 10	72	11	12	18	17	1	1		
13	QUADA 11	72	13	14	20	19	1	1		
14	QUADA 12	72	14	15	21	20	1	1		
15	QUADA 13	72	15	16	22	21	1	1		
16	QUADA 14	72	16	17	23	22	1	1		
17	QUADA 15	72	17	18	24	23	1	1		
18	QUADA 16	72	19	20	26	25	1	1		
19	QUADA 17	72	20	21	27	26	1	1		
20	QUADA 18	72	21	22	28	27	1	1		
21	QUADA 19	72	22	23	29	28	1	1		
22	QUADA 20	72	23	24	30	29	1	1		
23	QUADA 21	72	25	26	32	31	1	1		
24	QUADA 22	72	26	27	33	32	1	1		
25	QUADA 23	72	27	28	34	33	1	1		
26	QUADA 24	72	28	29	35	34	1	1		
27	QUADA 25	72	29	30	36	35	1	1		
28	GRID	1	0.0	0.0	0.0	0.0	0.0	0.0		

29-	GRID	2	3.0	0.0	0.0	0.0	.65E+06	4.0E+05	4.0E+05	.005	+ABC
30-	GRID	3	6.0	0.0	0.0	0.0					+DEF
31-	GRID	4	9.0	0.0	0.0	0.0					+EFG
32-	GRID	5	12.0	0.0	0.0	0.0					+FGH
33-	GRID	6	15.0	0.0	0.0	0.0					+GHI
34-	GRID	7	0.0	4.0	0.0	0.0					
35-	GRID	8	3.0	4.0	0.0	0.0					
36-	GRID	9	6.0	4.0	0.0	0.0					
37-	GRID	10	9.0	4.0	0.0	0.0					
38-	GRID	11	12.0	4.0	0.0	0.0					
39-	GRID	12	15.0	4.0	0.0	0.0					
40-	GRID	13	0.0	8.0	0.0	0.0					
41-	GRID	14	3.0	8.0	0.0	0.0					
42-	GRID	15	6.0	8.0	0.0	0.0					
43-	GRID	16	9.0	8.0	0.0	0.0					
44-	GRID	17	12.0	8.0	0.0	0.0					
45-	GRID	18	15.0	8.0	0.0	0.0					
46-	GRID	19	0.0	12.0	0.0	0.0					
47-	GRID	20	3.0	12.0	0.0	0.0					
48-	GRID	21	6.0	12.0	0.0	0.0					
49-	GRID	22	9.0	12.0	0.0	0.0					
50-	GRID	23	12.0	12.0	0.0	0.0					
51-	GRID	24	15.0	12.0	0.0	0.0					
52-	GRID	25	0.0	16.0	0.0	0.0					
53-	GRID	26	3.0	16.0	0.0	0.0					
54-	GRID	27	6.0	16.0	0.0	0.0					
55-	GRID	28	9.0	16.0	0.0	0.0					
56-	GRID	29	12.0	16.0	0.0	0.0					
57-	GRID	30	15.0	16.0	0.0	0.0					
58-	GRID	31	0.0	20.0	0.0	0.0					
59-	GRID	32	3.0	20.0	0.0	0.0					
60-	GRID	33	6.0	20.0	0.0	0.0					
61-	GRID	34	9.0	20.0	0.0	0.0					
62-	GRID	35	12.0	20.0	0.0	0.0					
63-	GRID	36	15.0	20.0	0.0	0.0					
64-	MAT1	10	3.0E+04	.25							
65-	MAT8	22	18.5E+06	1.6E+06	.25						
66-	PCOMP	72									
67-	+ABC	22	.0236	45.	YES		.0236	-45.	YES		
68-	+DEF	22	.0236	90.	YES		.0236	0.0	YES		
69-	+EFG	10	1.0	0.0			.0236	0.0	YES		
70-	+FGH	22	.0236	90.0	YES		.0236	-45.0	YES		
71-	+GHI	22	.0236	45.0	YES						
72-	PLOAD2	1	-2.0	1	THRU						
73-	SPC1	1	6	1	THRU						
74-	SPC1	1	36	1	THRU						
75-	SPC1	1	36	6	12			24	30	36	
76-	SPC1	1	136	1	7			19	25	31	
77-	SPC1	1	236	31	THRU						
	ENDDATA										

D I S P L A C E M E N T V E C T O R

POINT ID.	TYPE	T1	T2	T3	R1	R2	R3
1	G	0.0	0.0	0.0	-1.571077E-04	1.142508E-04	0.0
2	G	0.0	0.0	0.0	-1.933766E-04	9.752589E-05	0.0
3	G	0.0	0.0	0.0	-2.608906E-04	3.743675E-05	0.0
4	G	0.0	0.0	0.0	-2.607035E-04	-4.376641E-05	0.0
5	G	0.0	0.0	0.0	-1.934224E-04	-1.008716E-04	0.0
6	G	0.0	0.0	0.0	-1.578756E-04	-1.151408E-04	0.0
7	G	0.0	0.0	0.0	-1.300261E-04	2.268750E-04	0.0
8	G	0.0	0.0	0.0	-1.522363E-04	1.820725E-04	0.0
9	G	0.0	0.0	-1.290094E-03	-2.075847E-04	6.413873E-05	0.0
10	G	0.0	0.0	-1.923577E-03	-2.083385E-04	-6.804503E-05	0.0
11	G	0.0	0.0	-1.920106E-03	-1.548241E-04	-1.836399E-04	0.0
12	G	0.0	0.0	0.0	-1.342848E-04	-2.265799E-04	0.0
13	G	0.0	0.0	0.0	-4.759642E-05	3.379894E-04	0.0
14	G	0.0	0.0	-1.845502E-03	-5.010014E-05	2.752919E-04	0.0
15	G	0.0	0.0	-2.81518E-03	-6.906350E-05	1.000706E-04	0.0
16	G	0.0	0.0	-2.813420E-03	-7.083838E-05	-1.013518E-04	0.0
17	G	0.0	0.0	-1.842411E-03	-5.519034E-05	-2.755732E-04	0.0
18	G	0.0	0.0	0.0	-5.532292E-05	-3.366605E-04	0.0
19	G	0.0	0.0	0.0	5.532292E-05	3.366605E-04	0.0
20	G	0.0	0.0	-1.842411E-03	5.519034E-05	2.755732E-04	0.0
21	G	0.0	0.0	-2.813420E-03	7.083838E-05	1.013518E-04	0.0
22	G	0.0	0.0	-2.81518E-03	6.906350E-05	-1.000706E-04	0.0
23	G	0.0	0.0	-1.845502E-03	5.010014E-05	-2.752919E-04	0.0
24	G	0.0	0.0	0.0	4.759642E-05	-3.370894E-04	0.0
25	G	0.0	0.0	0.0	1.342848E-04	2.265799E-04	0.0
26	G	0.0	0.0	-1.283585E-03	1.548241E-04	1.836399E-04	0.0
27	G	0.0	0.0	-1.920106E-03	2.083385E-04	6.804503E-05	0.0
28	G	0.0	0.0	-1.923577E-03	2.075847E-04	-6.413873E-05	0.0
29	G	0.0	0.0	-1.290094E-03	1.522363E-04	-1.820725E-04	0.0
30	G	0.0	0.0	0.0	1.300261E-04	-2.268750E-04	0.0
31	G	0.0	0.0	0.0	1.578756E-04	1.151408E-04	0.0
32	G	0.0	0.0	0.0	1.934224E-04	1.008716E-04	0.0
33	G	0.0	0.0	0.0	2.607035E-04	4.376641E-05	0.0
34	G	0.0	0.0	0.0	2.608906E-04	-3.743675E-05	0.0
35	G	0.0	0.0	0.0	1.933766E-04	-9.752589E-05	0.0
36	G	0.0	0.0	0.0	1.571077E-04	-1.142508E-04	0.0

* * * END OF JOB * * *

PROBLEM #9

REGULAR SYMMETRIC CROSS-PLY LAMINATE

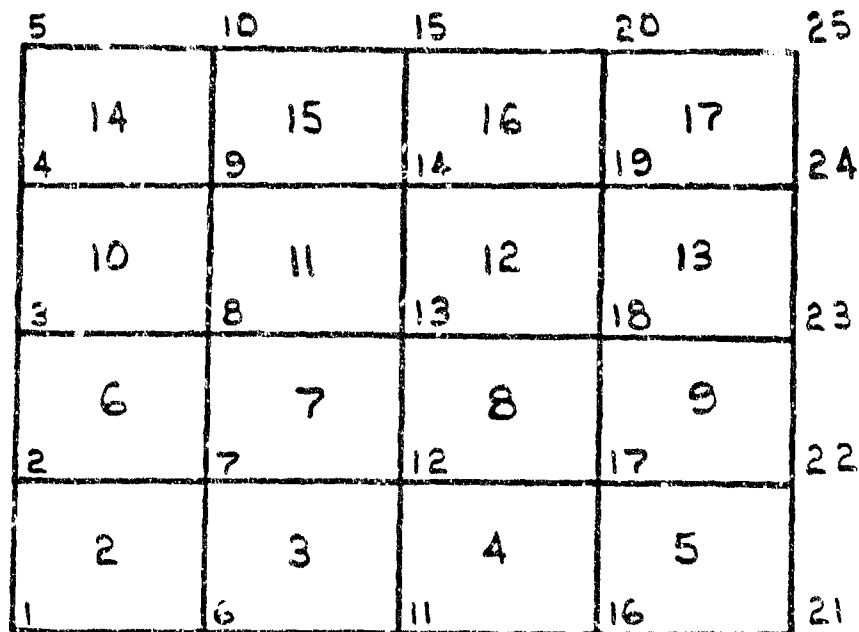


FIGURE 9

A quarter model of a composite square plate modeled as symmetric cross-ply laminate is shown in Fig. 9. The length of each side is 1.0, and the thickness of each ply is .000666. The material properties are given as $E_1 = 2.0 \times 10^7$, $E_2 = 5.0 \times 10^5$, $\nu_{12} = 0.25$ and $G_{12} = 2.5 \times 10^5$ and are the same for each ply. The full plate is simply supported and is subjected to a uniform pressure load of -1.0×10^{-4} . The finite element model contains 25 nodes and 16 elements. The theoretical solution for the Z deflection at Grid 25 (center of the plate) is -1.836×10^{-3} . Theoretical results are also given for the stresses in layers 1 and 3 in element 17 in Table 4.

σ_1	σ_2	τ_{12}
58.6	1.8	-.06

TABLE 4

PROBLEM 9 REGULAR SYMMETRIC CROSS-PLY LAMINATE MODELED WITH

QUAD4 ELEMENTS

COSMIC INPUT

CARD COUNT	---	1---	++2+++	---3---	+++4+++	---5---	+++6+++	---7---	+++8+++	---9---	+++10+++
1-	QUAD4	2	1	1	6	11	12	7			
2-	QUAD4	3	1	1	11	16	17	12			
3-	QUAD4	4	1	1	16	21	22	17			
4-	QUAD4	5	1	1	2	7	8	3			
5-	QUAD4	6	1	1	7	12	13	8			
6-	QUAD4	7	1	1	12	17	18	13			
7-	QUAD4	8	1	1	17	22	23	18			
8-	QUAD4	9	1	1	3	8	9	4			
9-	QUAD4	10	1	1	8	13	14	9			
10-	QUAD4	11	1	1	13	18	19	14			
11-	QUAD4	12	1	1	18	23	24	19			
12-	QUAD4	13	1	1	4	9	10	5			
13-	QUAD4	14	1	1	9	14	15	10			
14-	QUAD4	15	1	1	14	19	20	15			
15-	QUAD4	16	1	1	19	24	25	20			
16-	QUAD4	17	1	1	0.000	0.000	0.000	25			
17-	GRID	1			0.000	.250	0.000				
18-	GRID	2			0.000	.500	0.000				
19-	GRID	3			0.000	.750	0.000				
20-	GRID	4			0.000	1.000	0.000				
21-	GRID	5			.250	0.000	0.000				
22-	GRID	6			.250	.250	0.000				
23-	GRID	7			.250	.500	0.000				
24-	GRID	8			.250	.750	0.000				
25-	GRID	9			.500	1.000	0.000				
26-	GRID	10			.500	0.000	0.000				
27-	GRID	11			.500	.250	0.000				
28-	GRID	12			.500	.500	0.000				
29-	GRID	13			.500	.750	0.000				
30-	GRID	14			.500	1.000	0.000				

PROBLEM 9 REGULAR SYMMETRIC CROSS-PLY LAMINATE MODELED WITH QUAD4 ELEMENTS

COSMIC OUTPUT

POINT ID.	TYPE	DISPLACEMENT VECTOR					
		T1	T2	T3	R1	R2	R3
1	G	0.0	0.0	0.0	0.0	0.0	0.0
2	G	0.0	0.0	0.0	0.0	1.259793E-03	0.0
3	G	0.0	0.0	0.0	0.0	2.153710E-03	0.0
4	G	0.0	0.0	0.0	0.0	2.628845E-03	0.0
5	G	0.0	0.0	0.0	0.0	2.772239E-03	0.0
6	G	0.0	0.0	0.0	-1.423199E-03	0.0	0.0
7	C	0.0	0.0	-3.245146E-04	-1.171945E-03	1.144795E-03	0.0
8	C	0.0	0.0	-5.582579E-04	-7.010531E-04	1.969839E-03	0.0
9	G	0.0	0.0	-6.843077E-04	-3.124475E-04	2.410641E-03	0.0
10	G	0.0	0.0	-7.226401E-04	0.0	2.543014E-02	0.0
11	G	0.0	0.0	0.0	-2.575165E-03	0.0	0.0
12	G	0.0	0.0	-5.899490E-04	-2.141997E-03	8.477152E-04	0.0
13	G	0.0	0.0	-1.019073E-03	-1.294632E-03	1.476568E-03	0.0
14	G	0.0	0.0	-1.251502E-03	-5.713850E-04	1.817122E-03	0.0
15	G	0.0	0.0	-1.322001E-03	0.0	1.918498E-03	0.0
16	G	0.0	0.0	0.0	-3.312100E-03	0.0	0.0
17	G	0.0	0.0	-7.614346E-04	-2.775751E-03	4.475691E-04	0.0
18	G	0.0	0.0	-1.319674E-03	-1.693370E-03	7.857787E-04	0.0
19	G	0.0	0.0	-1.623366E-03	-7.425974E-04	9.717189E-04	0.0
20	G	0.0	0.0	-1.715300E-03	0.0	1.026845E-03	0.0
21	G	0.0	0.0	0.0	-3.565219E-03	0.0	0.0
22	G	0.0	0.0	-8.206014E-04	-2.995503E-03	0.0	0.0
23	G	0.0	0.0	-1.423963E-03	-1.834283E-03	0.0	0.0
24	G	0.0	0.0	-1.752818E-03	-8.027037E-04	0.0	0.0
25	G	0.0	0.0	-1.852310E-03	0.0	0.0	0.0

PROBLEM 9 REGULAR SYMMETRIC CROSS-PLY LAMINATE MODELED WITH TRIA3 ELEMENTS

COSMIC INPUT (CONTD)

36-	GRID	4	5.000	.750	0.000						
37-	GRID	5	0.000	1.000	0.000						
38-	GRID	6	.250	0.000	0.000						
39-	GRID	7	.250	.250	0.000						
40-	GRID	8	.250	.500	0.000						
41-	GRID	9	.250	.750	0.000						
42-	GRID	10	.250	1.000	0.000						
43-	GRID	11	.500	0.000	0.000						
44-	GRID	12	.500	.250	0.000						
45-	GRID	13	.500	.500	0.000						
46-	GRID	14	.500	.750	0.000						
47-	GRID	15	.500	1.000	0.000						
48-	GRID	16	.750	0.000	0.000						
49-	GRID	17	.750	.250	0.000						
50-	GRID	18	.750	.500	0.000						
51-	GRID	19	.750	.750	0.000						
52-	GRID	20	.750	1.000	0.000						
53-	GRID	21	1.000	0.000	0.000						
54-	GRID	22	1.000	.250	0.000						
55-	GRID	23	1.000	.500	0.000						
56-	GRID	24	1.000	.750	0.000						
57-	GRID	25	1.000	1.000	0.000						
58-	MAT8	1	20.0E+06	.50E+6	.25						
59-	FCOMP	1	.001								
60-	+PC1	1	.000666	0.0	YES	1					+PC1
61-	+PC2	1	.000666	0.0	YES	1					+PC2
62-	LOAD	1	-1.0E-0417	21		22					
63-	LOAD	1	-1.0E-0419	24		20					
64-	LOAD	1	-1.0E-043	7		9					
65-	LOAD	1	-1.0E-042	7		5					
66-	LOAD	1	-1.0E-0420	14		15					
67-	LOAD	1	-1.0E-0414	19		15					
68-	LOAD	1	-1.0E-0415	19		20					
69-	LOAD	1	-1.0E-0420	24		25					
70-	LOAD	1	-1.0E-0413	18		14					
71-	LOAD	1	-1.0E-0414	18		19					
72-	LOAD	1	-1.0E-0418	23		19					
73-	LOAD	1	-1.0E-049	13		14					
74-	LOAD	1	-1.0E-0419	23		24					

PROBLEM 9 REGULAR SYMMETRIC CROSS-PLY LAMINATE MODELED WITH TRIA3 ELEMENTS

COSMIC INPUT (CONTD)

75-	PLOAD	1	-1.0E-044	9	9
76-	PLOAD	1	-1.0E-045	9	10
77-	PLOAD	1	-1.0E-049	14	10
78-	PLOAD	1	-1.0E-0411	16	12
79-	PLOAD	1	-1.0E-0413	17	18
80-	PLOAD	1	-1.0E-046	11	7
81-	PLOAD	1	-1.0E-047	11	12
82-	PLOAD	1	-1.0E-0417	22	18
83-	PLOAD	1	-1.0E-048	13	9
84-	PLOAD	1	-1.0E-0418	22	23
85-	PLOAD	1	-1.0E-047	12	8
86-	PLOAD	1	-1.0E-0416	21	7
87-	PLOAD	1	-1.0E-048	12	13
88-	PLOAD	1	-1.0E-0412	16	17
89-	PLOAD	1	-1.0E-0412	17	13
90-	PLOAD	1	-1.0E-044	9	9
91-	PLOAD	1	-1.0E-042	6	7
92-	PLOAD	1	-1.0E-043	8	4
93-	PLOAD	1	-1.0E-041	6	2
94-	SPC1	1	6	THRU	25
95-	SPC1	1	15	23	24
96-	SPC1	1	24	15	20
97-	SPC1	1	1234	3	4
98-	SPC1	1	1235	6	11
99-	SPC1	1	1245	25	16
100-	SPC1	1	12345	1	21
	ENDATA				

PROBLEM 9 REGULAR SYMMETRIC CROSS-PLY LAMINATE MODELED WITH TRIA3 ELEMENTS

COSMIC OUTPUT

POINT NO.	TYPE	DISPLACEMENT VECTOR					
		T1	T2	T3	R1	R2	R3
1	G	0.0	0.0	0.0	0.0	0.0	0.0
2	G	0.0	0.0	0.0	0.0	1.174108E-03	0.0
3	G	0.0	0.0	0.0	0.0	2.080508E-03	0.0
4	G	0.0	0.0	0.0	0.0	2.531759E-03	0.0
5	G	0.0	0.0	0.0	0.0	2.112162E-03	0.0
6	G	0.0	0.0	0.0	-1.205491E-03	0.0	0.0
7	G	0.0	0.0	0.0	-1.039763E-03	1.355696E-03	0.0
8	G	0.0	0.0	-4.88340E-04	-6.172197E-04	1.355620E-03	0.0
9	G	0.0	0.0	-6.108230E-04	-2.823236E-04	2.317570E-03	0.0
10	C	0.0	0.0	-6.505204E-04	0.0	2.33258E-03	0.0
11	G	0.0	0.0	0.0	-2.270027E-03	0.0	0.0
12	G	0.0	0.0	-5.257187E-04	-1.91159E-03	9.229751E-04	0.0
13	G	0.0	0.0	-9.090877E-04	-1.163206E-03	1.81817E-03	0.0
14	G	0.0	0.0	-1.113404E-03	-5.163338E-04	1.780501E-03	0.0
15	G	0.0	0.0	-1.197048E-03	0.0	1.899247E-03	0.0
16	G	0.0	0.0	0.0	-3.017362E-02	0.0	0.0
17	G	0.0	0.0	-6.807542E-04	-2.467892E-03	4.625815E-04	0.0
18	G	0.0	0.0	-1.183225E-03	-1.520105E-03	7.172556E-04	0.0
19	G	0.0	0.0	-1.453424E-03	-6.559617E-04	1.274826E-04	0.0
20	G	0.0	0.0	-1.556301E-03	0.0	1.334608E-04	0.0
21	G	0.0	0.0	0.0	-3.37213E-03	0.0	0.0
22	G	0.0	0.0	-7.507631E-04	-2.64250E-03	0.0	0.0
23	G	0.0	0.0	-1.26122E-03	-1.62860E-03	0.0	0.0
24	G	0.0	0.0	-1.57061E-03	-6.878410E-04	0.0	0.0
25	G	0.0	0.0	-1.653666E-03	0.0	0.0	0.0

PROBLEM #12

COMPOSITE SHELL ROOF

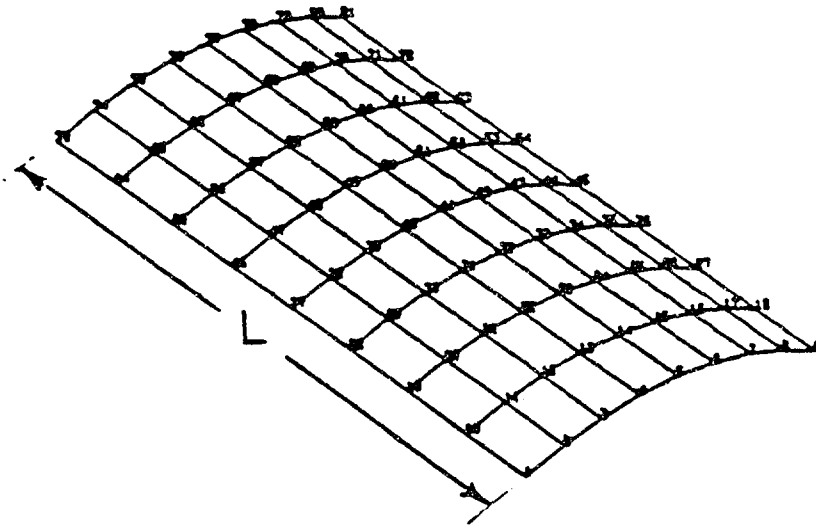


FIGURE 12

A finite element model of a composite shell roof modeled with the symmetric lay-up $[45, -45, 15, -15, -15, 15, -45, 45]$ is shown in Fig. 12. The length and radius of the shell are 25, and the thickness of each layer is .03125. The material properties are given as $E_1=2.0 \times 10^6$, $E_2=0.5 \times 10^6$, $\nu_{12}=0.25$, $G_{12}=2.5 \times 10^6$, and $G_{17}=G_{27}=2.5 \times 10^6$. The shell is subjected to a uniform pressure of 90.0. Results from MSC/NASTRAN are given in Table 8 for the radial deflection at selected nodes.

GRID	T1
34	-1.0662
35	-1.3441
36	-1.6074
43	-1.3267
44	-1.6739
45	-2.0079

TABLE 8

PROBLEM 12 COMPOSITE SHELL ROOF - CTR1A3 ELEMENTS

COSMIC OUTPUT

DISPLACEMENT VECTOR

POINT ID.	TYPE	T1	T2	T3	R1	R2	R3
1	G	0.0	0.0	1.476633E-02	0.0	1.361570E-02	0.0
2	G	0.0	0.0	1.612430E-02	3.239866E-02	7.457397E-03	1.813537E-03
3	G	0.0	0.0	1.942165E-02	1.313583E-01	-7.714272E-03	-5.444880E-01
4	G	0.0	0.0	2.215707E-02	1.976693E-01	-2.931339E-02	-1.551425E-03
5	G	0.0	0.0	2.054040E-02	2.512435E-01	-5.650782E-02	-2.533445E-03
6	G	0.0	0.0	9.672505E-03	2.894153E-01	-6.757029E-02	-3.402307E-03
7	G	0.0	0.0	-1.624594E-02	3.007707E-01	-1.200357E-01	-4.131101E-03
8	G	0.0	0.0	-6.365565E-02	2.776152E-01	-1.504282E-01	-4.490627E-03
9	G	0.0	0.0	-1.393812E-01	0.0	-1.643894E-01	-5.448329E-03
10	G	0.0	0.0	1.470382E-02	0.0	1.151773E-02	0.0
11	G	2.326459E-02	-3.264060E-03	1.605109E-02	8.395085E-02	8.155208E-03	1.493468E-02
12	G	-1.482768E-02	-3.972457E-03	1.922503E-02	1.259123E-01	-5.091122E-03	2.591116E-02
13	G	-1.955509E-02	4.956841E-05	2.184063E-02	1.694297E-01	-2.528226E-02	3.501230E-02
14	G	-1.617897E-01	1.051910E-02	2.020436E-02	1.832168E-01	-5.096502E-02	4.241037E-02
15	G	-2.545453E-01	2.873136E-02	9.543329E-03	1.904393E-01	-9.039026E-02	4.734479E-02
16	G	-3.567530E-01	5.541377E-02	-1.533482E-02	1.797236E-01	-1.134484E-01	4.872809E-02
17	G	-4.531212E-01	9.051105E-02	-6.169981E-02	1.563861E-01	-1.420324E-01	4.565815E-02
18	G	-5.363321E-01	1.328572E-01	-1.344102E-01	0.0	-1.656616E-01	3.715051E-02
19	G	8.354098E-02	0.0	1.460465E-02	0.0	9.452272E-03	0.0
20	G	5.485842E-02	-6.133685E-03	1.576311E-02	8.652643E-02	6.857764E-03	2.687922E-02
21	G	-2.71054E-02	-7.595237E-03	1.850582E-02	1.249557E-01	-4.624485E-03	4.874576E-02
22	G	-1.524426E-01	4.739278E-04	2.065561E-02	1.543775E-01	-2.283172E-02	6.676597E-02
23	G	-3.120429E-01	2.086395E-02	1.880291E-02	1.795106E-01	-4.615085E-02	8.086339E-02
24	G	-4.952009E-01	5.623564E-02	8.573793E-03	1.721470E-01	-7.276709E-02	9.305839E-02
25	G	-6.686350E-01	1.078973E-01	-1.504325E-02	1.632250E-01	-1.009693E-01	9.333315E-02
26	G	-8.776217E-01	1.757794E-01	-5.733753E-02	1.534643E-01	-1.296031E-01	9.074838E-02
27	G	-1.042164E+00	2.584161E-01	-1.236248E-01	0.0	-1.518993E-01	8.408472E-02
28	G	1.165276E-01	0.0	1.401622E-02	0.0	5.516557E-03	0.0
29	G	7.638276E-02	-8.429410E-03	1.489281E-02	9.887142E-02	5.185639E-03	3.709026E-02
30	G	-3.927651E-02	-9.976236E-03	1.701572E-02	1.290388E-01	-4.640827E-03	6.896443E-02
31	G	-2.181162E-01	1.456814E-03	1.851162E-02	1.488879E-01	-2.028534E-02	9.518006E-02
32	G	-4.443317E-01	3.074098E-02	1.643791E-02	1.590755E-01	-4.040500E-02	1.153411E-01
33	G	-7.027198E-01	8.141620E-02	7.045495E-03	1.537462E-01	-6.342431E-02	1.283439E-01
34	G	-9.638973E-01	1.552550E-01	-1.391042E-02	1.444555E-01	-3.793313E-02	1.236524E-01
35	G	-1.256662E+00	2.523485E-01	-5.089922E-02	1.320263E-01	-1.130087E-01	1.322963E-01
36	G	-1.512646E+00	3.712141E-01	-1.084606E-01	0.0	-1.287000E-01	1.278532E-01
37	G	1.416514E-01	0.0	1.214414E-02	0.0	2.540082E-03	0.0
38	G	8.209161E-02	-1.039582E-02	1.328789E-02	1.049389E-01	3.458103E-03	4.545156E-02
39	G	-5.137531E-02	-1.167172E-02	1.472694E-02	1.227285E-01	-4.346098E-03	8.551660E-02
40	G	-2.747474E-01	2.692822E-03	1.56273E-02	1.562093E-01	-1.685280E-02	1.189154E-01
41	G	-5.604917E-01	3.978029E-02	1.343068E-02	1.344321E-01	-3.318006E-02	1.443206E-01

PROBLEM 12 COMPOSITE SHELL ROOF - CTRIA3 ELEMENTS

COSMIC OUTPUT (CONTD)

42	G	-8.878149E-01	1.033431E-01	5.308660E-03	1.329748E-01	-5.205030E-02	1.606955E-01
43	G	-1.234267E+00	1.959281E-01	-1.222983E-02	1.214679E-01	-7.233550E-02	1.680550E-01
44	G	-1.579237E+00	3.176312E-01	-4.279044E-02	1.141284E-01	-9.345143E-02	1.683716E-01
45	G	-1.909306E+00	4.676751E-01	-9.003448E-02	0.0	-1.103693E-01	1.663368E-01
46	G	1.585377E-01	0.0	1.970312E-02	0.0	-3.328194E-04	0.0
47	G	1.020915E-01	-1.115413E-02	1.092877E-02	9.868991E-02	1.837580E-03	5.151105E-02
48	G	-6.221560E-02	1.256683E-02	1.171817E-02	1.181452E-01	-3.483323E-03	9.817950E-02
49	G	-3.200940E-01	4.571131E-03	1.201176E-02	1.233944E-01	-1.296991E-02	1.375043E-01
50	G	-6.512789E-01	4.746799E-02	1.006675E-02	1.199054E-01	-2.532743E-02	1.673537E-01
51	G	-1.031257E+00	2.212685E-01	3.596504E-03	1.095177E-01	-3.989144E-02	1.866436E-01
52	G	-1.434333E+00	2.287819E-01	-9.991137E-03	9.726910E-02	-5.572497E-02	1.959045E-01
53	G	-1.838202E+00	3.705621E-01	-3.339332E-02	8.642013E-02	-7.228899E-02	1.979190E-01
54	G	-2.229472E+00	5.453926E-01	-6.932452E-02	0.0	-8.547089E-02	1.961847E-01
55	G	1.668758E-01	0.0	7.928710E-03	0.0	-1.660479E-03	0.0
56	G	1.080008E-01	-1.169142E-02	7.812442E-03	1.191570E-01	1.410222E-03	5.540280E-02
57	G	-7.047822E-02	1.286581E-02	8.058028E-03	1.048704E-01	-2.586256E-03	1.070393E-01
58	G	-3.529197E-01	6.135389E-03	8.046384E-03	1.046178E-01	-8.673640E-03	1.509624E-01
59	G	-7.173160E-01	3.337095E-02	6.576133E-03	9.723432E-02	-1.716859E-02	1.842807E-01
60	G	-1.136189E+00	1.346256E-01	2.061663E-03	8.429071E-02	-2.724381E-02	2.058199E-01
61	G	-1.551390E+00	2.530920E-01	-7.243007E-03	7.151520E-02	-3.817935E-02	2.166686E-01
62	G	-2.029629E+00	4.095345E-01	-2.309458E-02	7.429327E-02	-5.004312E-02	2.203675E-01
63	G	-2.467970E+00	6.029455E-01	-4.722725E-02	0.0	-6.007985E-02	2.227727E-01
64	G	1.701813E-01	0.0	4.341663E-03	0.0	-6.951285E-03	0.0
65	G	1.092746E-01	-1.175825E-02	4.012782E-03	5.640501E-02	-1.511942E-03	5.643111E-02
66	G	-7.531671E-02	1.281390E-02	4.029745E-03	1.019095E-01	-9.911236E-04	1.113292E-01
67	G	-3.725972E-01	7.206568E-03	3.976561E-03	9.881937E-02	-4.479322E-03	1.592970E-01
68	G	-7.591181E-01	5.706112E-02	3.171894E-03	8.436854E-02	-1.003476E-02	1.954991E-01
69	G	-1.204372E+00	1.429999E-01	7.929730E-04	6.339596E-02	-1.680591E-02	2.186134E-01
70	G	-1.677765E+00	2.684475E-01	-4.026215E-03	4.313026E-02	-2.403195E-02	2.302631E-01
71	G	-2.155000E+00	4.342831E-01	-1.209223E-03	1.261471E-02	-3.079130E-02	2.346718E-01
72	G	-2.624267E+00	6.396778E-01	-2.407040E-02	0.0	-3.432402E-02	2.395364E-01
73	G	1.669936E-01	0.0	0.0	-6.855840E-02	0.0	0.0
74	G	1.072161E-01	-1.189289E-02	0.0	1.943815E-01	0.0	5.693102E-02
75	G	-7.771776E-02	1.293488E-02	0.0	1.544966E-01	0.0	1.154741E-01
76	G	-3.814610E-01	7.504007E-03	0.0	1.241305E-01	0.0	1.650839E-01
77	G	-7.779930E-01	5.849575E-02	0.0	6.962283E-02	0.0	2.022918E-01
78	G	-1.235319E+00	1.464317E-01	0.0	5.829034E-02	0.0	2.257639E-01
79	G	-1.721251E+00	2.747352E-01	0.0	4.354954E-02	0.0	2.374775E-01
80	G	-2.211148E+00	4.441634E-01	0.0	9.365131E-02	0.0	2.441724E-01
81	G	-2.697874E+00	6.388919E-01	0.0	-3.394089E-01	0.0	2.429941E-01
5001	G	0.0	0.0	0.0	0.0	0.0	0.0

PROBLEM 12 COMPOSITE SHELL ROOF - CTRIA3 ELEMENTS

MSC OUTPUT

D I S P L A C E M E N T V E C T O R

POINT ID.	TYPE	T1	T2	T3	R1	R2	R3
1	G	0.0	0.0	1.488159E-02	0.0	1.297565E-02	0.0
2	G	0.0	0.0	1.623873E-02	3.260004E-02	6.302241E-03	1.105459E-03
3	G	0.0	0.0	1.949219E-02	1.259453E-01	-8.333838E-03	-7.068854E-04
4	G	0.0	0.0	2.214893E-02	1.881285E-01	-2.981238E-02	-1.720419E-03
5	G	0.0	0.0	2.044334E-02	2.376534E-01	-5.672706E-02	-2.624311E-03
6	G	0.0	0.0	5.512377E-03	2.705136E-01	-8.735808E-02	-3.386547E-03
7	G	0.0	0.0	-1.639316E-02	2.789256E-01	-1.191964E-01	-3.942670E-03
8	G	0.0	0.0	-6.354493E-02	2.570521E-01	-1.489490E-01	-4.086468E-03
9	G	0.0	0.0	-1.389790E-01	0.0	-1.634667E-01	-5.355548E-03
10	G	4.370032E-02	0.0	1.432224E-02	0.0	1.071695E-02	0.0
11	G	4.370032E-02	0.0	1.615973E-02	7.232695E-02	7.423943E-03	1.472248E-02
12	G	2.754780E-02	-3.231469E-03	1.615973E-02	1.160193E-01	-5.489751E-03	2.558949E-02
13	G	-1.543843E-02	-3.879277E-03	1.929014E-02	1.517092E-01	-2.534725E-02	3.470048E-02
14	G	-7.970110E-02	1.803154E-04	2.183047E-02	1.739005E-01	-5.067214E-02	4.208433E-02
15	G	-1.612722E-01	1.063959E-02	2.010849E-02	1.803120E-01	-7.968822E-02	4.636886E-02
16	G	-2.551457E-01	2.877427E-02	9.386890E-03	1.691137E-01	-1.102678E-01	4.829599E-02
17	G	-3.542117E-01	5.529228E-02	-1.587797E-02	1.497871E-01	-1.404172E-01	4.528507E-02
18	G	-4.492348E-01	9.010967E-02	-6.169318E-02	0.0	-1.630579E-01	3.724567E-02
19	G	-5.294844E-01	1.320778E-01	-1.340410E-01	0.0	7.897929E-03	0.0
20	G	8.300618E-02	0.0	1.472343E-02	0.0	6.358043E-03	2.677065E-02
21	G	5.366356E-02	-6.061701E-03	1.586366E-02	7.629420E-02	-4.757439E-03	6.542776E-02
22	G	-2.822683E-02	-7.221833E-03	1.855773E-02	1.131810E-01	-2.264605E-02	8.939063E-02
23	G	-1.528755E-01	7.142373E-04	2.063585E-02	1.418970E-01	-4.568077E-02	8.939063E-02
24	G	-3.112799E-01	2.108605E-02	1.870670E-02	1.575670E-01	-7.203154E-02	9.246780E-02
25	G	-4.927777E-01	5.631252E-02	8.427706E-03	1.589888E-01	-9.991183E-02	8.974052E-02
26	G	-6.840820E-01	1.076638E-01	-1.516993E-02	1.486903E-01	-1.280042E-01	8.310466E-02
27	G	-8.704751E-01	1.750302E-01	-5.732376E-02	1.383166E-01	-1.501012E-01	8.310466E-02
28	G	-1.039088E+00	2.569164E-01	-1.232866E-01	0.0	5.148641E-03	0.0
29	G	1.152777E-01	0.0	1.413831E-02	0.0	4.714099E-03	3.693191E-02
30	G	7.472901E-02	-8.300558E-03	1.489932E-02	8.993725E-02	-4.714099E-03	6.547274E-02
31	G	-4.067949E-02	-9.711580E-03	1.706400E-02	1.168659E-01	-4.884850E-03	9.453129E-02
32	G	-2.185333E-01	1.802567E-03	1.848873E-02	1.389650E-01	-2.031904E-02	1.441566E-01
33	G	-4.450763E-01	3.107015E-02	1.634832E-02	1.457215E-01	-4.025970E-02	1.271481E-01
34	G	-7.041574E-01	8.151313E-02	6.915911E-03	1.424343E-01	-6.306400E-02	1.322146E-01
35	G	-9.774102E-01	1.549098E-01	-1.401832E-02	1.303859E-01	-8.721155E-02	1.306890E-01
36	G	-1.246591E+00	2.512781E-01	-5.088077E-02	1.166845E-01	-1.116734E-01	1.261592E-01
37	G	-1.498247E+00	3.650781E-01	-1.081577E-01	0.0	-1.313339E-01	1.261592E-01
38	G	1.395788E-01	0.0	1.285421E-02	0.0	2.296834E-03	0.0
39	G	8.94594E-02	-9.909747E-03	1.337338E-02	9.763631E-02	3.088094E-03	4.519776E-02
40	G	-5.298783E-02	-1.131881E-02	1.476189E-02	1.140346E-01	-4.611195E-03	8.497571E-02
41	G	-2.751050E-01	3.339947E-03	1.553642E-02	1.257372E-01	-1.706274E-02	1.180168E-01
42	G	-5.587978E-01	4.016229E-02	1.334898E-02	1.281200E-01	-3.329806E-02	1.430467E-01
43	G	-8.832884E-01	1.034509E-01	5.196754E-03	1.209811E-01	-5.198572E-02	1.590799E-01
44	G	-1.226153E+00	1.934803E-01	-1.231892E-02	1.083166E-01	-7.191026E-02	1.661673E-01

PROBLEM 12 COMPOSITE SHELL ROOF - CTRIA3 ELEMENTS

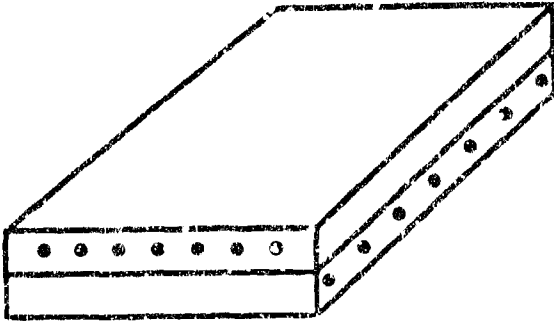
MSC OUTPUT (CONTD)

44	G	-1.566740E+00	3.164809E-01	-4.276921E-02	9.351392E-02	-9.236200E-02	1.662851E-01
45	G	-1.891454E+00	4.650046E-01	-8.977510E-02	0.0	-1.090747E-01	1.641039E-01
46	G	1.557078E-01	0.0	1.079951E-02	0.0	-3.798279E-04	0.0
47	G	9.953328E-02	-1.031489E-02	1.099804E-02	9.509884E-02	1.645601E-03	5.121743E-02
48	G	-6.400058E-02	-1.213516E-02	1.174218E-02	1.101552E-01	-3.827984E-03	9.746330E-02
49	G	-3.203450E-01	5.092703E-03	1.198581E-02	1.145105E-01	-1.322398E-02	1.363789E-01
50	G	-6.491557E-01	4.790815E-02	9.996753E-03	1.098994E-01	-2.567515E-02	1.657968E-01
51	G	-1.025912E+00	1.213806E-01	3.505278E-03	9.835787E-02	-4.009001E-02	1.847004E-01
52	G	-1.424911E+00	2.282475E-01	-1.006918E-02	8.471708E-02	-5.555328E-02	1.936597E-01
53	G	-1.823768E+00	3.689791E-01	-3.337160E-02	7.237545E-02	-7.145791E-02	1.954772E-01
54	G	-2.208994E+00	5.422893E-01	-6.911366E-02	0.0	-8.449339E-02	1.955358E-01
55	G	1.651378E-01	0.0	8.004886E-03	0.0	-1.534491E-03	0.0
56	G	1.050687E-01	-1.140391E-02	7.865760E-03	1.161919E-01	-1.292911E-03	5.495435E-02
57	G	-7.231621E-02	-1.237000E-02	8.075285E-03	1.011229E-01	-2.874816E-03	1.062388E-01
58	G	-3.530848E-01	6.19551E-03	8.021737E-03	9.764549E-02	-9.239995E-03	1.496353E-01
59	G	-7.149160E-01	5.385436E-02	6.520945E-03	8.794370E-02	-1.782336E-02	1.824604E-01
60	G	-1.130271E+00	1.347380E-01	1.996592E-03	7.388631E-02	-2.777240E-02	2.035848E-01
61	G	-1.571003E+00	2.24819E-01	-7.286583E-03	5.962552E-02	-3.835132E-02	2.141181E-01
62	G	-2.013773E+00	4.077701E-01	-2.306932E-02	5.969225E-02	-4.963985E-02	2.175228E-01
63	G	-2.445431E+00	5.995078E-01	-4.706422E-02	0.0	-5.927353E-02	2.197395E-01
64	G	1.658712E-01	0.0	4.412975E-03	0.0	-5.981099E-03	0.0
65	G	1.067296E-01	-1.143024E-02	4.039316E-03	7.359419E-02	-2.180564E-04	5.660340E-02
66	G	-7.722478E-02	-1.227560E-02	4.021035E-03	1.047540E-01	-1.248734E-03	1.105871E-01
67	G	-3.726837E-01	7.833989E-03	3.947961E-03	9.417622E-02	-5.160896E-03	1.580029E-01
68	G	-7.564764E-01	5.757254E-02	3.136728E-03	7.860163E-02	-1.075205E-02	1.906642E-01
69	G	-1.197937E+00	1.431109E-01	7.645905E-04	5.680722E-02	-1.726504E-02	2.163555E-01
70	G	-1.666436E+00	2.677848E-01	-4.036724E-03	3.941693E-02	-2.394976E-02	2.277070E-01
71	G	-2.137921E+00	4.323893E-01	-1.207806E-02	1.250840E-02	-2.978523E-02	2.323784E-01
72	G	-2.600875E+00	6.360321E-01	-2.400697E-02	0.0	-3.367934E-02	2.367507E-01
73	G	1.609050E-01	0.0	0.0	-1.104910E-01	0.0	0.0
74	G	1.037114E-01	-1.149914E-02	0.0	2.032132E-01	0.0	5.624982E-02
75	G	-7.952528E-02	-1.232689E-03	0.0	1.294580E-01	0.0	1.141604E-01
76	G	-3.810907E-01	8.167893E-03	0.0	1.086516E-01	0.0	1.633879E-01
77	G	-7.748047E-01	5.900292E-02	0.0	7.536310E-02	0.0	2.004330E-01
78	G	-1.228686E+00	1.465074E-01	0.0	4.785991E-02	0.0	2.238440E-01
79	G	-1.710450E+00	2.740414E-01	0.0	2.020216E-02	0.0	2.351301E-01
80	G	-2.194278E+00	4.422544E-01	0.0	8.441722E-02	0.0	2.411608E-01
81	G	-2.673986E+00	6.502062E-01	0.0	-2.788148E-01	0.0	2.413830E-01
5001	G	0.0	0.0	0.0	0.0	0.0	n.o

Appendix F: NASTRAN Input and Output for Problem #15

PROBLEM #15

COMPOSITE RECTANGULAR PLATE - UNSYMMETRIC CROSS-PLY LAMINATE



3	6	9	12	15
2	5	8	11	14
1	4	7	10	13

UNSYMMETRIC
CROSS-PLY LAMINATE

FIGURE A1

One half of a composite rectangular plate modeled with the unsymmetric cross-ply layup (0° , 90°) is shown in Fig. A1. The plate is modeled with 15 nodes and 8 elements. The length of the plate is 5", the width is 1", and the thickness of each layer is .005". The material properties matrix on the MAT2 card is given as $G_{11} = 1.94 \times 10^7$, $G_{12} = 4.66 \times 10^5$, $G_{22} = 1.33 \times 10^5$ and $G_{33} = 8.3 \times 10^5$. The thermal expansion coefficient vector is defined by $A_1 = -1 \times 10^{-6}$, $A_2 = 15 \times 10^{-6}$ and $A_{12} = 0$. A temperature field was defined for the plate by an average temperature of 230° over the cross-section of each element on the TEMPP1 card. The input data, the calculated thermal loading, and the displacements due to the thermal loading are also provided.

TEST PLATE SANDHU
 APP DISP
 SOL
 TIME 5
 END

PROBLEM 15

TEST PLATE FOR THERMAL LOAD FOR SANDHU

APRIL 6, 1992 CSA/NASTRAN 3/31/91

CASE CONTROL DECK ECHO

CARD
 COUNT

TEST PLATE FOR THERMAL LOAD FOR SANDHU
 1
 2
 3
 4
 5
 6
 7
 8
 9
 10
 11
 12
 13
 14
 15
 16
 17
 18
 19
 20
 21
 22
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 83
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 85
 86
 87
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 95
 96
 97
 98
 99
 100

TOTAL INPUT BULK DATA CARD COUNT BETWEEN BEGIN BULK AND ENDDATA = 31
 TOTAL NUMBER OF BULK DATA CARDS PROCESSED = 32
 APRIL 6, 1992 CSA/NASTRAN 3/31/91

200

CARD
 COUNT

CARD	COUNT	1	2	3	4	5	6	7	8	9	10
1	1	10001									
2	2	10001	1		4	5	2				0.0
3	3	10001	2		5	6	3				0.0
4	4	10001	4		7	8	5				0.0
5	5	10001	5		9	9	6				0.0
6	6	10001	7		10	11	8				0.0
7	7	10001	8		11	12	9				0.0
8	8	10001	10		13	14	11				0.0
9	9	10001	11		14	15	12				0.0
10	10	GRID	0.0		0.0	0.0					
11	11	GRID	0.0		0.50	0.0					
12	12	GRID	0.0		1.0	0.0					
13	13	GRID	1.25		0.0	0.0					
14	14	GRID	1.25		0.50	0.0					
15	15	GRID	1.25		1.0	0.0					
16	16	GRID	2.50		0.0	0.0					
17	17	GRID	2.50		0.50	0.0					
18	18	GRID	2.50		1.0	0.0					
19	19	GRID	3.75		0.0	0.0					
20	20	GRID	3.75		1.0	0.0					

21-	GRID	13	5.0	0.0	0.0		
22-	GRID	14	5.0	.50	0.0		
23-	GRID	15	5.0	1.0	0.0		
24-	MAT2	30001	1.94+7	4.66+5	1.33+6	8.3+5	+DEF
25-	+DEF	-1.-6	15.-6	0.			
26-	PCOMPI	10001		60000.	STRAIN	30001	.01
27-	+ABC	0.0	90.				
28-	SPC1	16	6	4	THRU	15	
29-	SPC1	16	123456	1	THRU	3	
30-	TEMPPI	100	1	230.			+TPA
31-	+TPA	2	THRU	8			

ENDDATA

L O A D V E C T O R

POINT NO.	TYPE	T1	T2	T3	R1	R2	R3
1	C	-4.067550E+00	-1.016887E+01	0.0	-2.292381E-01	-9.169525E-02	0.0
2	C	-8.135100E+00	1.705303E-13	0.0	0.0	-1.833905E-01	0.0
3	C	-4.067550E+00	1.016887E+01	0.0	2.292381E-01	-9.169525E-02	0.0
4	C	4.547474E-13	-2.033775E+01	0.0	-4.584762E-01	1.776357E-15	0.0
5	C	0.0	-5.684342E-14	0.0	0.0	4.440892E-16	0.0
6	C	-4.547474E-13	2.033775E+01	0.0	4.584762E-01	-1.776357E-15	0.0
7	C	4.547474E-13	-2.033775E+01	0.0	-4.584762E-01	1.776357E-15	0.0
8	C	0.0	5.684342E-14	0.0	0.0	4.440892E-16	0.0
9	C	-4.547474E-13	2.033775E+01	0.0	4.584762E-01	-1.776357E-15	0.0
10	C	4.547474E-13	-2.033775E+01	0.0	-4.584762E-01	1.776357E-15	0.0
11	C	0.0	-5.684342E-14	0.0	0.0	4.440892E-16	0.0
12	C	-4.547474E-13	2.033775E+01	0.0	4.584762E-01	-1.776357E-15	0.0
13	C	4.067550E+00	-1.016887E+01	0.0	-2.292381E-01	9.169525E-02	0.0
14	C	8.135100E+00	1.705303E-13	0.0	0.0	-1.833905E-01	0.0
15	C	4.067550E+00	1.016887E+01	0.0	2.292381E-01	-9.169525E-02	0.0

D I S P L A C E M E N T V E C T O R

POINT NO.	FILE	T1	T2	T3	R1	R2	R3
1	0	0.0	0.0	0.0	0.0	0.0	0.0
2	0	0.0	0.0	0.0	0.0	0.0	0.0
3	0	0.0	0.0	0.0	0.0	0.0	0.0
4	0	8.262617E-04	-3.794536E-04	-1.075627E-01	-8.041327E-02	1.728277E-01	0.0
5	0	9.649792E-04	-1.712008E-12	-1.275678E-01	2.652152E-11	2.033811E-01	0.0
6	0	6.282617E-04	3.794536E-04	-1.075627E-01	8.041327E-02	1.728277E-01	0.0
7	0	1.853320E-03	-3.837750E-04	-4.625432E-01	-8.305494E-02	3.947354E-01	0.0
8	0	1.227469E-03	-7.303463E-12	-4.633479E-01	1.825510E-11	3.662719E-01	0.0
9	0	1.853320E-03	3.837749E-04	-4.625432E-01	8.305494E-02	3.947354E-01	0.0
10	0	2.635011E-03	-3.380588E-04	-1.059582E+00	-7.217816E-02	5.607316E-01	0.0
11	0	2.736763E-03	-1.580492E-11	-1.077608E+00	1.814007E-10	5.843386E-01	0.0
12	0	2.635011E-03	3.380588E-04	-1.059582E+00	7.217816E-02	5.607316E-01	0.0
13	0	3.624009E-03	-3.779266E-04	-1.694176E+00	-7.962263E-02	7.745631E-01	0.0
14	0	3.53427E-03	-3.137500E-11	-1.914137E+00	-4.224974E-10	7.541634E-01	0.0
15	0	3.624009E-03	3.779266E-04	-1.694176E+00	7.962263E-02	7.745631E-01	0.0

ORIGINAL & DEFORMED PLATE

