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#### PREFACE

This Guide describes procedures to be used by all US Army Corps of Engineers personnel using the structural analysis software RandMicas. Following the procedures in this guide should simplify the process of model generation, load application, and structural analysis.

The Guide establishes procedures for using the software by giving detailed steps for four typical structural models. Suggestions and recommendations are given that should reduce repetition of work and eliminate avoidable errors.

The following references were of great assistance in preparation of the Guide:

Intergraph-RandMicas (IRM) Analysis Grahics Interface (Structural), November 1, 1988. Intergraph Corporation, Huntsville, AL

Intergraph-RandMicas (IRM) Analysis User's Guide with Alphanumeric Interface, November 1, 1988, Intergraph Corporation, Huntsville, AL

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The Computer-Aided Design and Drafting Center (CADD-C) convened a group of structural engineers to create the examples in this guide. Members of the group included Mr. Dean Spenser, Savannah District, Mr. Paul Blackburn, Tulsa District, Mr. Elias Arredondo, Sacramento District, and Mr. Robert Grause, Intergraph Corporation. The Guide was compiled by Mr. Steven Hatton, structural engineer, CADD Center, Information Technology Laboratory. The time consuming efforts of all participants in the preparation of this Guide are gratefully acknowledged. The work was performed under the direction of Dr. N. Rahdakrishan, Chief, Information Technology Laboratory (ITL), Dr. Edward Middlteton and Mr. Carl S. Stephens, Chiefs of the Computer-Aided Engineering Division and the CADD-C respectively. The Commander and Director and Technical Director of WES during the preparation of this Guide were COL Larry B. Fulton and Mr. Robert W. Whalin, respectively.

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# 1. Introduction

# 1.1 Purpose and Scope

The purpose of this report is to present step by step procedures for the solution of some common structural models using Intergraph's RandMicas. It is an attempt to familiarize new users with the operation of IRM by providing hands on experience with checkable results. No attempt is made to reproduce the User's Guide for RandMicas. It is assumed that the user is familiar with the fundamentals of IGDS, and has some exposure to SMS, and IRM. Reference is made to the Intergraph manuals for these products for specific instructions.

1.2 Origin of Guide

Groundwork for this report began in a meeting at the Sacremento District in April 1989. At this meeting, the "SWAT" team, Messrs. Dean Spenser, Savannah District; Paul Blackburn, Tulsa District; Robert Grause, Intergraph Corporation; and Elias Arredondo, Sacremento District; created example problems 1-4 respectively. They each prepared an outline of the procedure required to generate and analyze their specific problem. The CADD Center assimilated these outlines into a common format and produced this guide.

1.3 Creation of the Model

a. When modeling a complex geometric structure with IRM Version 8.8.2, it is recommended that the user begin the process in the Structural Modeling System (SMS), formerly CSM. SMS facilitates the modeling of structures by allowing the user to select members directly from the AISC or user defined tables. The user will not be required to input mechanical and physical properties for the members since they are automatically stored. Version 8.8.3 of IRM provides a similar modeling approach and reduces the need for this interim step. However, the drawing extraction feature of SMS may still make it a desirable place to begin modeling.

b. If a structure has been created through SMS, it can be written to the Common Structural Database (CSD) and accessed by IRM through the use of a .BLU, or "blue" file. Loads and node constraints are then placed in IRM and the structure is ready for analysis.

c. Appendix A contains guidance for accessing IRM and creating a typical project. The example given is for a 3D model using the Intergraph default database as a seed file. Other model types or customized seed files may be used in a similar way.

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# 1.4 Coordinate System, Sign Convention, and Units

IRM uses a global coordinate system which is based upon a right-handed Cartesian coordinate system. All angles, rotations, and moments are based upon the right-hand rule. Output is given in the global system except for line elements and shell elements which are output in their local systems. Translations and forces are positive when acting in the positive direction of their axes. Units of output are consistent with those selected in the model database and those input into the load vector.

1.5 Generation of Output

a. The format for output for an IRM run must be setup prior to analyzing the model. The Analysis Setup Menu provides a great deal of flexibility to the user for determining what the output will contain and how the analysis should be performed. The analysis setup may be done in the alphanumeric environment or from the screen menu in the Graphics Interface.

b. Repetitive entering of the Analysis Setup may be avoided by utilizing a customized seed file when creating a project file. Commonly used options for the Analysis Setup include:

- 1. Turn on the Automatic Node Stabilization from "Processing Options-General Processing Options".
- 2. Select static analysis from "Processing Solution Modes".
- 3. From "Results Options Static and Response Spectrum", turn on Load Case Results, Load Combination Results, Node Displacements, and Line Element End Actions.
- 4. From "Report Selection-Model Definition", turn on Analysis Options and Model Summary, Units, Load Case/Load Combination Data, Line Element Data, and Line Element Loads.
- 5. From "Report Selection-Results", turn on Solution Statistics and Unstable Node Report, as well as desired reactions, displacements, and end actions.

c. Output generated upon completion of an IRM analysis will generate three print files of the format (Your Project Name)001.PTR. File 001 will contain project units, load case and combination data, material properties, node coordinates, and element properties. File 002 will be the node stabilization report. File 003 contains the specified output including the process warning report, displacements, reactions, and member forces. The 001 number of the files will increase each time the model is analyzed unless previous files have been deleted. These files may be output to a printer or viewed on screen with a text editor.

# 2. Specific Example Problem Instructions

2.1 Cantilevered Beam

a. Description: This is a simple demonstration problem consisting of one 10 foot long cantilevered steel beam, fixed at the left end, with a concentrated load of 1 kip located at the right end and directed downwards. The self-weight of the beam will also be included in this analysis. See Appendix B for typical results output and a model sketch.

b. Solution Approach: No special seed file is used for this problem, therefore, all parameters not adequately set by the product default seed file will be set in this example.

(1) Begin model by setting display characteristics:

c. Input Problem Data:

-Select ACTIVE PARAMS. -Select MODELING PARAMETERS - LABELS. -Toggle to LABEL FOR Nodes. -Set levels, colors, etc. as desired. Recommend size be 30 dits for 1/8" scale plotting. -SAVE and repeat for Elements, then SAVE and RETURN. -Select LOADING PARAMETERS - NODAL LOADS. -Select NODAL LOAD VECTORS. -Set display attributes of loads. Recommend: L1=2, L2=4, F1=1, and F2=5. -SAVE and Exit all tutorials. (2) Set material types and element properties: -Select MATERIAL. -Select ISOTROPIC and set material values. For this example use an A36 steel beam. -Set Density Flag to Weight. -Return OK and BYE -Select ELEMENT PROPS - BEAM ELEMENT STANDARD SECTION PROPERTIES. -Keyin W8X24 for Section Name. -Return OK and BYE. -Select PLACEMENT DATA PARAMS. -Toggle Design Type to BEAM. -Select Material as A36 and keyin W8X24 for element property. -Set the Cardinal Point at 8 (Top of steel). -Set no member end releases. -Toggle all prompt boxes to no. -Exit tutorial.

(3) Place member directly without first placing IGDS element: -Keyin VI=Front and select a view. -Select PLACE - MEMBER - BEAM and give a dp for the left end of the beam in the front view. -Keyin DX = 10,0,0.-Reset twice. -Select LABEL and COMPONENT NODE. -Place dp in the front view for UID. -Select GROUP ALL, Reset to backup, and place dp for permanent labels. -Select COMPONENT MEMBER. -Repeat labeling procedure for members. -Reset to Exit. (4) Generate loads and place onto the beam: -Select GENERAL LOADS - CASE/COMB. -Select LOAD CASE and keyin: LC1, static analysis Dead load, Active, etc. -SAVE -Kevin: LC2, static analysis, Dead load, Active, etc. -Save and select LOAD COMB. -Set Load Combination Name to: COMB1. -Set Load Combination to: LC1+0.75\*LC2 -Set the Global Multiplier to: 1.0 -Save and Exit tutorials. -Select PLACE - LOAD - NODE CONCENTRATED. -Keyin: LC2, 0 (for Force), RESET (to reorient vector), (0,0,-1), 1 (magnitude). -Snap to Node 2 and dp to accept. -Reset 3 times and Exit tutorial. (5) Set Boundary Conditions (Supports): -Select BC's command and CONSTRAINTS. -Toggle fixity to: T1 T2 T3 / R1 R2 R3 -dp to accept. -Select node 1 for single component and accept. -Reset twice and Exit tutorial. (6) Setup Analysis: -Select ANALYSIS SETUP. -Select PROCESSING - SOLUTION MODES. -Select STATIC ANALYSIS. -Select RESULTS OPTIONS - STATIC & RESPONSE SPECTRUM. -Toggle on items: 4, 5, 6, 9, & 10. -Select REPORT SELECTION - MODEL DEFIN. -Toggle on items: 2, 3, 6, 12, 16, & 18. -Select REPORT SELECTION - RESULTS.

4

-Toggle on items: 7, 8, & 16. -Exit tutorial. -File Design. -Stop IRM and exit design file.

d. Execute Analysis:

(1) Self-weight loads must be applied in the alphanumeric environment. To set self weight loads, after exiting the design file:

-Select option #6 to Terminate.
-Select option #2, Alphanumeric Interface.
-Select option #1, Analysis.
-Select option #7, Load Entry.
-Select option #3, Line Element Load Entry.
-Keyin LC1 for Load Case Number.
-Keyin 1 for Line Element.
-On one line at the LOAD prompt keyin: BODY Z -.49
-Keyin END at the Line Element prompt.
-Exit to the ANALYZE JOB Option #9 and select.
-Choose between Interactive or Batch.
-Exit IRM.

# 2.2 2D Rigid Frame

a. Description: This example is a three member moment resisting frame, comprised of two twenty-foot columns pinned at the base, and a twenty-foot beam connecting them at the top. The beam is loaded full length with a 1 kip/ft uniform dead load. One column is loaded at the top with a 2 kip concentrated lateral live load. See Appendix C for typical results output and a model sketch.

b. Solution Approach: IRM was used to create the model for this example. A customized seed file was used to create the project. The seed file presets all parameters required for this analysis which are not specifically set in this example.

c. Input Problem Data:

(1) The structure was drawn graphically using IGDS commands. The sequence of commands is as follows:

-Type LV=1 to place structure on level one.
-Type VI=FRONT and select a view by placing a data point(dp) in that view.
-Select the PLACE LINE command.
-Place a dp for the bottom of the left column.
-Type DX=0,20,0
-Type DX=20,0,0

-Type DX=0,-20,0 -Reset

(2) Set material properties and element type through the following commands:

-Select the MATERIAL command. -Select ISOTROPIC from the tutorial. -Verify that the material listed is A36 steel. -Change or "RETURN OK" and BYE. -Select the ELEMENT PROPS command. -Select BEAM ELEMENT STANDARD SECTION PROPERTIES. -Verify that the AISC table is listed as current. -Keyin W18X35 for Section Name. -Return OK and Bye.

(3) Define the load parameters to set levels, colors, and relative magnitudes of load cases with the following:

-Select the ACTIVE PARAMS command.
-Select LOADING PARAMETERS - ELEMENT LOADS.
-Select LINE ELEMENT LOAD PARAMS - L.E. DISTRIBUTED.
-Set the appropriate boxes to:
 L1=1.75 L2=3.75 F1=1 F2=5
-SAVE and Return
-Select LINE ELEMENT LOAD PARAMS - L.E. CONCENTRATED
-Set the appropriate boxes to:
 L1=2 L2=4 F1=1 F2=5
-Save and Exit the tutorial completely.

(4) To set the member placement parameters the following steps should be used:

-Select PLACEMENT DATA PARAMS
-Toggle design type field to BEAM Material Property = A36 Element Property = W18X35 No member end releases Cardinal Point(CP) = 15 (shear center) ingle = 0
-Toggle element type field to COLUMN
-Repeat the beam parameters but place a dp in the element property prompt box. This will cause you to be prompted for the section when placing the columns.
-Exit

(5) Set up the load cases and load case combinations in the following manner:

-Select the GENERAL LOADS - CASE/COMB command. -Choose LOAD CASE from the tutorial

Load Case Name	:	LC1		LC2
Analysis Type	:	Static	;	
Load Type	:	Dead		Live
Active	:	Yes		Yes
Convert Nodes	:	No		No
Level	:	20		21
Color	:	2		4
		SAVE		SAVE
-Choose LOAD COMB				
Load Case Comb:	ir	nation	Name:	COMB1
Load Combinatio	or	า	:	LC1+LC2
Multiplier			:	1
-SAVE and Exit Tutor	ri	al		

(6) Place the beam and columns through the following commands:

-Select PLACE MEMBER with active design type COLUMN. -Place a dp at the bottom of the left column and then another at the top. -Type in W10X22 for the element property. -Reset to the "start of member" prompt. -Place a dp at the top of the right column and then another at the bottom. -Keyin W12x53. -Reset to start of member prompt. -Select active design type BEAM. -Place a dp at the start and end of the beam. -Reset out of the command. (7) Establish Boundary Conditions with the following: -Select the BC's command and CONSTRAINT (The first box of the tutorial that appears.) -Hit dp or resets to modify the constraints until they appear as: T1 T2 T3 / R1 **R3** -Select Group Fence and place a fence around both column bases. -Accept group of two column bases. -Reset twice and exit the tutorial. (8) Apply the loads to the structure as follows: -Select PLACE and the component LOAD. -Select the uniformly distributed load symbol. -Answer the prompts: LC1, Global Direct, Z, -1 -Select SINGLE as the group and place a dp on the beam. -dp to accept then reset three times to Exit. -Select the NODE LOAD - CONCENTRATED -Answer the prompts: LC2, Force, Reset (to reorient the load vector), (1,0,0), 2

-Place a dp at the upper node on the left column and another to accept the node. -Reset three times and exit the tutorial. -File Design.

d. Execute Analysis: If the Analysis Setup was not set in the seed file it will have to be set at this point. In this example, the output format was specified in the seed file. The structure may now be analyzed by selecting the ANALYZE command at the top of the AEC Menu.

2.3 2D Hanger Truss

a. Description: The problem involves a 2D model of an aircraft hanger truss. The truss is comprised of wide flanges and WT sections. Concentrated loads are applied at various top and bottom chord panel points in five separate load cases. See Appendix D for typical results output and a model sketch.

b. Solution Approach: This problem was analyzed by inputting members directly into the TRM model without using SMS or IGDS graphics. The main objective of this example is to show how a 2D model can be analyzed in a 3D Thin Shell model. This will be desirable in many cases primarily because the vertical axis may be set to Z, allowing direct translations to and from SMS. By using the "Active/Inactive" capability of IRM, this method can also be used to analyze and design 2D portions of a full 3D model. Almost all Active, Element, Material, and Analysis Setup parameters can be set in the seed file and are done so in this example. Procedures for presetting these parameters is as described in the preceding examples.

c. Input Problem Data:

(1) Set physical member parameters:

-Select PLACEMENT DATA PARAMS. -Set design type to COLUMN. -Set element property to W12X50. -Set Orientation to VECTOR (1, 0, 0). -Set Cardinal Point prompt box to On. -Set design type to BEAM. -Set element property to WT7X49.5. -Set Orientation to VECTOR (0, 0, 1). -Set member end releases to: Start: RY RZ End: RY RZ -Turn on prompt boxes for Rotate Section Properties and Cardinal Point. -Set design type to BRACE. -Keyin WT4X9 as section name, Card. Point = 15. -Set Orientation to VECTOR (0, 0, 1). -Turn on prompt for Rotate Section Properties

and Orientation. -Exit the tutorial. (2) Set AEC locks: -Select LOCKS command. -Verify AEC locks set to: At Connection, On Split New Member, Unloaded Interior, Yes, Yes -Exit the tutorial. (3) Place physical members: -Select PLACE MEMBER COLUMN. -Keyin XY=0,0,0 for start of member. -Keyin DL=0,0,39.5 for end of member. -For Cardinal Point keyin 2 and dp to accept. -Reset to backup and tentative snap to the base of first member. -Keyin DL=6:11.5,0,0 for start and DL=0,0,39.5 for end. -Set Cardinal Point to 8, dp to accept, resets to exit. -Select PLACE MEMBER BEAM, snap to top of the first column and place a dp. -Keyin DL=46.5,0,0 for the end of the member. -Select 15 for Cardinal Point and dp to accept. -Reset twice to exit command. (4) Relocate top chord to correct position. -Select the MOVE END ASSOCIATIVE command. -Snap to right end of the top chord and accept. -Keyin DL=0,0,7.5. -dp to accept and reset to exit. (5) Continue placing web members: -Select PLACE MEMBER BRACE. -Snap and dp at bottom of second column. -Snap to bottom of first column and keyin DL=0,0,4. -Accept default Orientation and Vector. -At Orientation ? prompt give dp to accept. -At End of Member prompt, snap to bottom of second column and keyin DL=0,0,8. -Repeat procedure to place all web members. (Spacing varies see Appenaix D) -Reset twice to exit. (6) Relocate end of second column. -Set AEC associative move lock to Proportionality. -Select MOVE END ASSOCIATIVE command.

-Snap to base of second column and accept.

-Keyin XY = 2,0,0.-dp to accept and reset to exit. (7) Mirror copy placed members and nodes. -Zoom out in the ISO view. -Select MIRROR COPY command. -Verify Component is set to Member and set Group to View. -Accept highlighted group and keyin C for copy. -Snap to the centerline of the truss in the Front View. -Keyin DL=0,0,1 and dp to accept mirrored copy. -Reset to exit. (8) Modify Cardinal Points of copied columns. -Select the Edit Cardinal point command. -Follow prompts to modify the left column to CP=2 and the right column to CP=8. (9) Place the bottom chord of the truss. -Select PLACE MEMBER BEAM and snap to desired nodes on column one and two. -Keyin a CP of 10 and dp to accept. -Reset until section icon is oriented correctly, then place a dp. -Reset to Exit. (10) Place truss web members. -Select PLACE MEMBER BRACE and place similar to column web members. Member sizes are shown in Appendix D. (11) Set Boundary Conditions as done for the Rigid

Frame.

(12) Set up Load Entry by selecting the CASE/COMB command and setting up as follows:

NAME	LEVEL	COLOR
و هی هم خده هن حله خده خده ها الله خاه خاه		
Body	9	2
Dead	21	2
Live	22	4
Cranerail	23	1
WindX	24	7

(13) Set the load combinations similarly by selecting the CASE/COMB command and setting up as follows:

NAME	COMBINATION
	*****
COMB1	BODY+DEAD+LIVE
COMB2	BODY+DEAD+LIVE+CRANERAIL
COMB3	BODY+DEAD+LIVE+WINDX
COMB4	BODY+DEAD+LIVE+CRANERAIL+WINDX

-Set the Global Multiplier to 1.0 for combinations 1 and 2, and to .75 for combinations 3 and 4.

(14) Place loads on structure.

-Select PLACE LOADS CONCENTRATED NODAL. -Follow the prompts to place the following loads:

LOAD CASE	ORIENTATION	MAGNITUDE	GROUP
DEAD	0,0,1	-2.5	TOP CHORD
LIVE	0,0,1	-1.5	TOP CHORD
CRANERAIL	0,0,1	-5.0	3,9 BOTT CHORD
WINDX	1,0,0	1.8	TOP CHORD

(15) Exit place loads tutorial and Stop and Exit IRM.

d. Execute Analysis:

(1) Add dead weight of structure in alpha environment.

-From the Analysis Main Menu keyin LLD. -Follow prompts to place a BODY load of -0.49 in the Z direction for members 1:200.

(2) If the Analysis Setup is not complete at this point it should be entered now.

-From the alpha main menu keyin SCAN (model info scan). -From the alpha main menu keyin MREP (model def. rpt). -From the alpha main menu keyin RREP (results report). -Configure reports and analysis as in other examples.

(3) Execute analysis by selecting appropriate options from the alphanumeric environment menus.

2.4 3D Aircraft Hanger

a. Description: The problem involves a 3D model of an aircraft hanger 93' x 195'. Vertical and lateral support is provided by four braced steel frames, while longitudinal support is provided by spandrel trusses and a braced frame at the end of the hanger. Concentrated loads are applied at various top and bottom chord panel points. See Appendix E for typical results output and a model sketch. b. Solution Approach:

(1) To begin modeling the problem, first enter SMS to create a database:

-Select option #1, "Create Structural Model".
-Input a name for the model.
-Choose a model size.
-Choose model units.
-Select option #3, "Run SMS Graphics".
-Keyin project name and answer prompts to attach menus.
(2) Setup project parameters as follows:
-Select the ACTIVE PARAMS and PROJECT commands.

-Set UNITS to English -Select STEEL for material. -Select AISC for section table. -Select A36 for grade. -Reset to Retain and Exit "Active Parameters". -Select FILE command.

(3) Use IGDS commands to lay out complex geometry prior to placing members. Place structural members into the database using SMS commands. A brief summary of member placement is as follows:

(4) To place columns:

-Select PLACE ELEMENT - X-SECTION "COLUMN". -Keyin column section name. -Snap to start of column IGDS graphic element and dp. -Snap and dp at end of column.

(5) Place all beams and braces for one bent in a similar way. Copy all members of the created bent by:

-Fit Front View. -Select COPY MEMBER - GROUP FENCE. -Place fence around entire bent. -dp to accept group or modify. -Select the base of one column as start point. -Snap to base of same column and keyin "DX=0,0,-z" to copy the bent "z" feet. -Repeat procedure to copy all bents required.

(6) Place beams and braces for the spandrel trusses just as for the bent frames. Use the COPY GROUP command as well to fill in between all bents.

(7) When the structure is complete, use the CSD command to WRITE the model to a .BLU (blue) file. The .BLU file will be used by IRM for analysis.

-Select the CSD command from Utilities. -Select WRITE from the tutorial. -Select Active Group command ALL. -dp to accept group. -Keyin or accept default for .BLU file. -Reset for default origin. -Exit tutorial at completion. -Stop SMS and Exit.

c. Input Problem Data:

(1) To begin the IRM analysis of the model enter IRM:
-Select option #3, "Structural Products Interface".
-Enter a new jobname.
-Use default database.
-Toggle to desired model size with ENTER key.
-Use same length units used when creating the CSD.

(2) Import Common Structural Database:

-Select option #1,"Common Structural Database to IRM Conversion".
-Enter the name of the .BLU file used in SMS.
-Enter '0,0,0' for Node Tolerance.
-Enter '0' for Complete Model.
-Select option #1, Unrestrained Supports.
-Pin all connections when prompted.
-The IRM Main Menu will then return.

(3) Enter the Graphics Interface.

(4) Set Boundary Conditions:

-Select BC's command and CONSTRAINTS. -dp and Reset until constraints appear as: T1 T2 T3 / -Select GROUP FENCE command and place fence around all column bases.

(5) Place loads: Refer to IRM manual for Load Placement commands.

d. Execute Analysis: If the Analysis Setup is not complete at this point it should be entered now. The member placement parameters and element types were placed into the database in SMS and imported to IRM previously. The structure may now be analyzed by selecting the ANALYZE command at the top of the AEC Menu or by entering the Alphanumeric Interface and executing the commands there.

# 3. <u>Recommendations</u>

3.1 Modeling Recommendations

a. Use of the Structural Modeling System (SMS) may simplify the creation of many models, particularly if using Version 8.8.2 of IRM. IGDS should be used to create the geometry of a complex structure and actual elements placed on the sketch once it is complete. This will make it easier to keep track of member orientations and help to keep the database clean.

b. It is recommended that a thin shell model be used in order to promote economy in the structures analyzed in 3D and to eliminate problems associated with converting from 2D to 3D.

c. When modeling a large structure the design file quickly becomes congested with elements and loads. The best way to alleviate this problem is to put members and separate load cases on different levels. In very large structures each group of members should be placed on a different level. When the model becomes too complex, some of the levels can be turned off. This becomes especially important when labeling member end releases, marked groups, member orientation, nodes, or members, since the display depth does not stop members in other planes from being labeled along with those in the current display. The user should plan his modeling sequence such that levels, colors, etc., are set prior to getting into any design file.

d. The use of seed files to preset material properties, output format, output requirements, and other common file attributes will greatly simplify the use of IRM.

#### 3.2 Miscellaneous Recommendations

a. Make nodes large enough to be visible when using the FLASH command and the UID group. Select the FLASH, NODE, and UID commands to locate unstable nodes identified in the Unstable Node Report.

b. Never renumber nodes with the renumber UID command unless you are sending the model for third party processing and want to reduce the bandwidth. Renumbering will not relabel your model in a logical series or format. IRM will automatically renumber nodes internally to reduce the bandwidth but this will not affect your numbering scheme.

c. Utilize available warning reports and review them until you are confident in the accuracy of the model. Look for unstable nodes and watch for unusually large, or small, nodal translations or rotations, which may indicate a modeling problem. d. Beware of the ROTATE PROPERTIES command as it rotates the section properties but not the cross-section. In IRM, the local member axis and the start and end of a member determine the top and bottom of that member, such as may be required for lateral bracing. Therefore, if the outside face of a building is to be the top side of some columns, such that the girt spacings will determine the lateral bracing spacing for strong axis bending, the columns on one side of the building will have to be placed from the bottom up, with the opposite side columns being placed from the top down.

e. Recognize the power of the active/inactive element concept. The edit command can be used to make parts of the structure active or inactive for analysis, which can be used to quickly isolate a part of the structure to view its behavior in a 2D model.

f. Beware the database rebuild command. It rebuilds the IRM database but not IRMD. Anything done in steel or concrete design will be lost.

# APPENDIX A

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GETTING STARTED

### APPENDIX A

# GETTING STARTED

- Step #1 Load all example files onto the system.
  - #2 Login to the system.
  - #3 Copy the seed file COESEED.\* into files named:

# YOUR\_PROJECT\_NAME.\*

or accept the product default seed file when prompted in IRM.

- #4 -At the \$ prompt, keyin IRM. -Select Option #1, "Graphical Interface". -Select Option #2, "Structural Model Generation". -Keyin a Jobname (Design file w/o .DGN extension). -If COESEED was used go to CONTINUE, else..... -Accept default or keyin seed file database. -Toggle to correct model size. -Move cursor to "(CONTINUE)" then hit Enter key. -Keyin a job type description. -Toggle model type to Thin Shell. -Set desired type of Units. -CONTINUE -You will now be put into the design file. #5 - dp or Reset for pull-down menus.
- #6 Attach table menu by reset or screen menu by placing a data point in any view but the Tutorial View.
   #7 - Type AM=MENU, CM to attach IGDS menu.

The example problems are in the following files:

CANTILEVERED BEAM - SPENCER.\*

2D RIGID FRAME - BENT.\*

2D HANGER TRUSS - COETRUSS.\*

3D AIRCRAFT HANGER - HANGER.\* CSMHANG.\*

The seed file used for examples 2 and 4 was STEEL.\* and for examples 1 and 3, COESEED.\*. Output data for all example problems is in files of the form, PROJECT\_NAMEOOi.PTR.

ÀPPENDIX B

EXAMPLE NO. 1 - CANTILEVERED BEAM



		spencer				
	CANT	ILEVERED BEAM				
*********	****************	***********	<b></b>	********	*****	*****
IRM REV	8.8.3		OCT	10,1989		15:17
ARALYSIS N	D.9 T	HIN SHELL			PACE	1

# \*\*\* Units Definition \*\*\*

Unit Group	Unit
1 - Lengths	FRET
2 - Element Properties	INCHES
3 - Forces	KIPS
4 - Angles	DEGREES
5 - Displacements	INCHES
6 - Hasses	MASS
7 - Time	SECONDS
8 - Stress Forces	KLPS

Vertical Aris = Z

Gravitational Constant (g) =32.2 FT /SEC /SEC

#### spencer CANTILEVERED BEAH IRM REV 8.8.3 OCT 10,1989 15:17 THIN SHELL PAGE 2 ARALYSIS HO.9

\*\*\* Material Property Tables \*\*\*

Hame/Ho.	Table Data	
1 1	Haterial Type Modulus of Elasticity (E) Poisson's Ratio (V) Shear Modulus (G) Alpha Weight Density	= ISOTROFIC = 29000.0 ksi. = 0.3 = 11153.8457 ksi. = 0.0 = 0.49 k/ft^3.

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	spencer		
	CANTILEVERED BEAM		
**********	***********	****************	*******
IRM REV 8.8.3		OCT 10,1989	15:17
ANALYSIS NO.9	THIN SHELL	I	AGE 3
*** Load Cases	***		
Haze/No.	Table Data		
ICI	Analysis type	= Static	
3	Load case type	= Dead load	
-	Status	= Active	
	Level	= 9	
	Color	<b>=</b> 2	
	Convert nodal loads to masses	= No	
LC2	Analysis type	= Static	
2	Load case type	= Dead load	
	Status	= Active	
	Level	<b>=</b> 9	
	Color	= 2	
	Convert nodal loads to masses	= Ho	

	spencer		
	CANTILEVERED BEAM		
********************	*******	******************	******
IRH REV 8.8.3		OCT 10,1989	15:17
ANALYSIS NO.9	THIN SHELL	PA	GR 4

\*\*\* Load Combinations \*\*\*

Name/No.	Туре	Print Results	Global Multiplier	Load Case Name	Load Case Hultiplier
COHB1 2	Linear	Yes	1.0	LCI LC2	1.0 0.75

		spencer		
	C	CANTILEVERED BEAM		
*****	*************	*****	***************	***********
IRM REV 8.8.3			OCT 10,1989	15:17
ANALYSIS NO.9		THIN SHELL		PAGE 5
*** Nodal Loads	***			
Node Loc	Load			Load
Id/Label Type Glo	b Case	Load	Vector or Dof	Туре
6 VEC GLO	 B	LC2 1.000	(0,0,-1)	FOR

	spencer		
	CANTILEVERED BEAM		
*********************	**********************	****************	*******
IRH REV 8.8.3		OCT 10,1989	15:17
ANALYSIS RO.9	THIN SHELL	PA	ge 6

\*\*\* Line Element \*\*\*

Element Id/Label	Elemen Type	t A/I	Haterial Properti	<b>e</b> 8	Elem Prop	ent erties	Design Type	Shr/Fict Stiff	Rot Prop
1	BEAM	A Angle of	$\frac{A36}{E Roll = 0}$	000	WBX24		BEAH	ŅŅ	Ņ
		Nodes:	Start	<b>x</b> 5		End =	6		
		I-J Leng	zth	= 10	.000	Number	of VHD S	egments =	16
		Non-Rig	ld Length	= 10	.000	Cardi	nal Poin	t a	15
		Physical	l Hember Id	<b>=</b> ]		Refle	ction Ax	is =	None

		spe	encer				
		CANTILE	ered beal	H			
********	*******	****************	********	**********	******	*****	******
IRM REV	8.8.3			0CT	10,1989		15:17
ANALYSIS	W.9	THIN	SHELL		·	PAGE	7

\*\*\* Line Element Loads \*\*\*

į

Element Label/Id	Load Case	Iype	Frame	Dir	Load (Velocity)	Abs/ Rel	Location (Shape Factor)
1	ICI	BODY	GLOBAL	Z	-0.490		

			s CANTIL	pencer EVERED BEA	R		
IRH Ro Analys	************* ev 8.8.3 sis No. 7	*****	**************************************	************ n Shell	**************************************	6,1989	15:55:23 Page 1
±±± ]	Displacement	s <del>***</del>					
Node	Cas/Cub	TX IN	ti In	TZ IN	R <b>X</b> DEG	R <b>Y</b> Deg	RZ DEG
5	LCI 3 LC2 2	0.0000	0.0000 0.0000	0.0000	0.0000 0.0000	0.0000 0.9000	0.0000
6	LC1 3 LC2 2	0.0000	0.0000	0.0000	0.0000 1.439e-08	0.0000	0.0000
	COMB1 2	0.0000	0.0000	-0.2399	1./92e-07 1.488e-07	0.1718	0.0000

			1	spencer			
			CANTI	LEVERED BE	Ш		
******	*******	*** <del>*******</del> *	*******	*********	**********	*******	*********
IRM Rev	8.8.3				OCT	6,1989	15:55:24
Analysis	No. 7		Thin Shell			•	Page 2
*** Disp	lacement	ts ***					
Quantity	Limit	Value	Unit	Node	Ldcmb/Cs		
11	Kar	0.0000	IN	5	LCI 3	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
	Min	0.0000	IN	5	LCI 3		
π	Hax	0.0000	IN	5	LCI 3		
	Min	0.0000	IN	5	LCI 3		
TZ	Hax	0.0000	IN	5	LC1 3		
	Min	-0.2399	IN	6	LC2 2		
RX	Kax	1.792e-07	DEG	6	LC2 2		
	Hin	0.0000	DEG	5	LC1 3		
RY	Hax	0.1718	DEG	6	LC2 2		
	Min	0.000	DEG	5	LC1 3		
RZ	Hax	0.0000	DEG	5	LCI 3		
	Min	0.0000	DEG	5	LCI 3		

			S CANTIL	pencer EVERED REA	ж		
*****	*********	********	******	****	 :***********	******	*********
IRH Re Analys	ev 8.8.3 sis No. 7		Thi	n Shell	001	6,1989	15:55:24 Page 3
*** 5	Support Read	tions **	ŧ				
Node	Cas/Cmb	FX KPS	fy KPS	FZ KPS	HX FT-KPS	hy FT-RPS	HZ FT-KPS
5	LC! 3	0.0000	0.0000	0.2409	_1.2560_06	_1.2046	0.000
	LC2 2	0.0000	0.0000	1.0000	-1.043e-05	-10,0000	0.0000
	COMB1 2	0.0000	0.0000	0.9909	-9.080e-06	-8.7046	0.0000

#### 

		spencer			
		CANTILEVERED	BEAM		
*******	*******	**** <del>***</del> ************************	**** <del>**************</del> *	********	**********
IRH Rev	8.8.3		OCT	6,1989	15:55:24
Analysis	No. 7	Thin Shell	1		Page 4

# \*\*\* Support Reactions \*\*\*

Quantity	Limit	Value	Unit	Node	Ldcmb/Cs	
FX.	<u>Hax</u> Hin	0.0000 0.0000	KPS KPS	5 5	ICI 3 ICI 3	
m	Hax Hin	0.0000 0.0000	kps kps	5 5	LC1 3 LC1 3	
77	Kax Hin	1.0000 0.2409	kps kps	5 5	LC2 2 LC1 3	
нх	Hax Hin	-1.256e-06 -1.043e-05	PT-KPS FT-KPS	5 5	LC1 3 LC2 2	
หรื	Kax Kin	-1.2046 -10.0000	PT-RPS FT-RPS	5 5	LC1 3 LC2 2	
ЪZ	Yax Hin	0.0000 0.0000	FT-KPS FT-KPS	5 5	LCI 3 LCI 3	

	spencer CANTILEVERED BEAM								
******	********	**********	******	******	******	*****	*******	******	
IRM Re	v 8.8.3					0CT	6,1989	15:55:24	
Analys	is No. 7		Thi	n Shell				Page 5	
*** L	ine Elem	ent End Action	8 ***						
Llen	Cas/Cmb	FX.	M	PZ	HX		hi	ΗZ	
	Node	KPS	KPS	KPS	FT-KPS		FT-KPS	IT-KPS	
1								•••••	
•	LC1 3								
	5	0.0000	0.2409	0.0000	0.00	00	0.0000	1.2046	
	6	0.0000	0.0000	0.000	) 0.00	00	0.0000	0.0000	
	LC2 2								
	5	0.0000	1.0000	0.000	) 9.095e-	13	0.0000	10.0000	
	6	0.0000	-1.0000	0.000(	) 0.00	00	0.0000	0.0000	
	COMBI 2	0 0000	0 0000	0 000	. ( 001.	19		0 70/(	
	3	0.0000	0.7500	0.0000	J 0.0210-	90 13	0.0000	ð./040 0.0000	
	U	V.0000	-0.7500	0.000	J 0.00	00	0.0000	0.0000	
****	***************************************								
			CANTI	LEVERED B	eah				
*****	********	********	*****	*******	*******	*****	*******	**********	
IRM R	ev 8.8.3	}				0CT	6,1989	15:55:25	
Analy	sis No. 7	1	Th	nin Shell				Page 6	
***	Line Eler	ment End Actio	ms <b>***</b>						
							1-		
Quant	ity Lim	lt Value	Unit	Elem	Node	Ldcmb	Cs		
77	Var	۰۰۰۰۰ ۵ ۵۰۰۰	700	••••••••••••••••••••••••••••••••••••••	ς	101 3			
IA	na. Min	0.000	NFD VDC	1	5	101 3			
		0.000	NI 0	1	,	TOT 2			
FY	Har	1,0000	KPS	1	5	IC2 2			
••	Min	-1.0000	KPS	i	6	LC2 2			
				•	•				
12	Kax	0.0000	KPS	1	5	LC1 3			
	Hin	0.0000	KPS	1	5	LC1 3			
нх	Hax	9.095e-13	PT-KPS	1	5	LC2 2			
	Min	0.0000	FT-RPS	1	5	LCI 3			
1.000				1	r				
ni	Max	0.0000	PT-KPS	1	<u>ל</u>	LUI 3			
	nin	0.000	ft-kps	1	)	101.3			
1/77	¥	10 6620	<b>54 40</b> 0	1	5	102 2			
Esta	rua z Min	0.000	FT_RPS	L	6	102 2			
	*****	******			•				

APPENDIX C

EXAMPLE NO. 2 - 2D RIGID FRAME



# 2D RIGID FRAME

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		BENT						
	RIGID FRAME							
*******	******	***************************************	******	**********				
IRH REV	8.8.3	OCT	6,1989	09:2J				
ANALYSIS	NO.11	THIN SHELL		PAGE 1				

\*\*\* Units Definition \*\*\*

Unit Group	Unit	
1 - Lengths	FEET	
2 - Element Properties	INCHES	
3 - Forces	KIPS	
4 - Angles	DEGREES	
5 - Displacements	INCHES	
6 - Kasses	MASS	
7 - Time	SECONDS	
8 - Stress Forces	KIPS	

Vertical Axis = Z

Gravitational Constant (g) =32.2 FT /SEC /SEC

# BENT RIGID FRAME

WIOLD LINE							
***************************************							
IRM REV	8.8.3		OCT	6,1989	09:20		
ANALYSIS	NO.11	THIN SH	ELL	-	PAGE 2		

\*\*\* Material Property Tables \*\*\*

Hame/Ho.	Table Data		
A36 1	Material Type Modulus of Elasticity (E) Poisson's Ratio (V) Shear Modulus (G) Alpha	= ISOTROPIC = 29000.0 ksi. = 0.3 = 11153.8457 ksi. = 0.0	
***********	*************	*****************	*************************
-------------	---------------------------------------	---------------------------------------	---------------------------------------
	· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·

	BENT RIGID FRAME		
**************************************	**************************************	••••••••••••••••••••••••••••••••••••••	************ 09:20 PAGE 3
*** Load Cases Name/No.	*** Table Data		
	Analysis type Load case type Status Level Color Convert nodal loads to masses	= Static = Dead load = Active = 9 = 7 = No	
L/32 2	Analysis type Load case type Status Level Color Convert nodal loads to masses	= Static = Live load = Active = 9 = 4 = No	

	BENT			
	RIGID FRAME			
********************	*****	*********	*******	********
IRM REV 8.8.3		OCT	6,1989	09:20
ANALYSIS NO.11	THIN SHELL			PAGE 4

\*\*\* Load Combinations \*\*\*

Name/No.	Type	Print Results	Global Multiplier	Load Case Name	Load Case Multiplier
COMB1 1	Linear	Yes	1.0	LC1 LC2	1.0 1.0

	•
***************************************	
***************************************	

			BE RIGID	NT FRAME				
ANALYSIS KO.	******* 8.3 11	*****	******** THIN	********* SHELL	************** 0CT	******* 6,1989	******** ( PAGE !	***** )9:20 ;
*** Nodal L	oads *	<del>tt</del>						
Hode Id/Label Typ	Loc e Glob	Load Case		Load	Vector or	Dof		Load Type
2 VEC	GLOB	~~~~~~~~~	LC2	2.000	(1,	0,0)		For
*******	*******	********		********	***********	* * * * * * * * *	*****	*****
			RIGI	D FRAME				
IRM REV 8 ANALYSIS NO	******* .8.3 .11	*********	**************************************	**********	•************** 0CT	6,1989	PAGE	09:20 6
*** Line E	lement	***						
Element Id/Label	Eleme Type	nt A/I I	Haterial Properti	E les P	lement roperties	Design Type	Shr/Fi Stiff	et Rot Prop
1	BEAM	A Å	36 	W1	0822	Column	NO	0
		Nodes: I-J Length Non-Rigid 1 Physical M	Stari Stari Length ember Io	z = 1 = 20.000 = 20.000 1 = 1	End = Number o Cardin Reflec	2 f VMD Se al Point tion Ax:	egments : ls	= 2 = 15 = None
2	BEAM	A 4	36	W1	2153	Colurn	NO	0
		Angle of R Nodes: I-J Length Non-Rigid Physical M	oll = 0 Stari Length ember Io	.000 t = 3 = 20.000 = 20.000 d = 2	End = Number o Cardin Reflec	4 f VHD So al Point tion Ax	egments t is	= 2 = 15 = None
3	BEAM	Á Á Angle of R	36 oll = 0.	W1	8X35	BEAM	HO	0
		Nodes: I-J Length Non-Rigid Physical M	Star Length ember Io	t = 2 = 20.000 = 20.000 t = 3	End = Number o Cardin Reflec	3 f VHD So al Point tion Ax:	egments t is	= 16 = 15 = None

		B	ent				
		RIGU	d frame				
*******	*******	*******	******	************	******	*****	*****
IRM REV	8.8.3			OCT	6,1989		09:20
AHALYSIS	NO.11	THIN	SHELL			PAGE	7

Element Label/Id	Load Case	Туре	Frame	Dir	Load (Velocity)	Abs/ Rel	Location (Shape Factor)
3	LC1	DIST	GLOBAL	Z	-1.000		

			RI	GID FRAME			
IRM P Analy	lev 8.8.3 vsis No. 12		Tb	Thin Shell		6,1989	09:21:09 Page 1
***	Displacement	:8 ***					
Node	Cas/Cmb	TX IN	ty In	TZ IN	RX Jeg	RY DEG	RZ DEG
1	LCI I	0.0000	0.0000	0.0000	0.0000	-0.2967	0.0000
	COMBI 1	0.0000	0.0000	0.00 0	0.0000	1.2394	0.0000
2	LCI 1	0.0000	0.0000	0.0000	0.0000	0.9427	0.0000
	LC2 2	-0.5860	0.0000	-0.0128	0.0000	0.1737	0.0000
	COMB1 1	2.8125	0.0000	-0.0102	0.0000	0.1288	0.0000
3	LC1 1	-0.5860	0.000	-5.305e-03	0.0000	-0.1772	0.0000
	LCZ Z	3.3970	0.0000	-1.061e-03	0.0000	0.1209	0.0000
4	LCI 1	2.8110	0.0000	-6.366e-03	0.0000	-0.0563	0.0000
	LC2 2	0.0000	0.0000	0.0000	0.0000	-0.1212	0.0000
	COMB1 1	0.0000	0.0000	0.0000	0.0000	1.1560	0.0000

BENT

BEI	IT
PICIN	TDAVE

********						*******	**********
IRM Rev Analysis	8.8.3 No. 12		Th	in Shell	OCT	6,1989	09:21:10 Page 2
*** Disp	lacemen	ts ***					
Quantity	Limit	Yalue	Jnit	Node	Ldcmb/Cs		
TI	Hax Min	3.3984 -0.5860	IR IN	2 3	LC2 2 LC1 1		
TY	Max Min	0.0000	IN IN	1 1	LCI 1 LCI 1		
72	Max Kin	2.550e-03 -0.0128	IN IN	2 2	LC2 2 LC1 1		
FX	Kax Hin	0.0000 0.0000	DEG DEG	1 1	LCI I LCI I		
RY	Max Min	1.2394 -0.2967	DEG DEG	1 1	LC2 2 LC1 1		
rz	Hax Hin	0.0000 0.0000	DEG Deg	$\frac{1}{1}$	LCI I LCI I		

<u> </u>	******************	\*.`}***********************************
----------	--------------------	--

			l RIG	bent Ld Frame				
***** IRM R Analy	************* ev 8.8.3 sis No. 12	********	**************************************	**************************************	************* 0C1	OCT 6,1989		
***	Support Rea	ctions **	t					
Node	Cas/Cub	FX KPS	FY KPS	FZ KPS	hx FT-KPS	hy FT-KPS	HZ FT-KPS	
1	LCI 1	0.0942	0.0000	10.0000	0.0000	0.0000	0.0000	
	LC2 2	-0.2573	0.0000	-2.0000	0.0000	0.0000	0.0000	
2	LCI 1	-0.1631	0.0000	8.0000	0.0000	0.0000	0.0000	
	LC2 2	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
	COHB1 1	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
3	LCI I	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
	LCZ Z	0.0000	0.0000	0.0000	0.0000	0.000	0.0000	
4	LCI I	0.000	0.0000	0.0000	0.0000	0.0000	0.0000	
	LC2 2	-0.0942	0.0000	10.0000	0.0000	0.0000	0.0000	
	COHB1 1	-1.7427 -1.8369	0.0000	12.0000	0.0000	0.0000	0.0000	

		BENT				
RIGID FRAME						
******	***************	********	*****	ŧŔŔŔŔŔŔŔŔ	*********	
IRM Rev 8.	8.3		003	6,1989	09:21:11	
Analysis No	. 12	Thin Shel	1		Page 4	

\*\*\* Support Reactions \*\*\*

Quantity	Limit	Value	Unit	Node	Licmb/Cs	
M	Max Min	0.0942 -1.8369	KPS KPS	1 4	LC1 1 COMB1 1	
m	Hax Hin	0.0000 0.0000	kps kps	1 1	LC1 1 LC1 1	
ГŻ	Hax Hin	12.0000 -2.0000	KPS KPS	4 1	COHB1 1 1.52 2	
нх	Hax Hin	0.0000 0.0000	PT-KPS FT-KPS	1 1	LCI I LCI I	
нı	Hax Hin	0.0000	PT-KPS FT-KPS	1 1	LCI 1 LCI 1	
ЫZ	Hax Hin	0.0000	PT-KPS PT-KPS	1 1	ICI I ICI I	

			RTG	DENT TD FRAME					
*****	***************************************								
IRM Re	8.8.3				00	T 6,1989	09:21:11		
Analys	sis No. 12		Thi	n Shell			Page 5		
*** 1	line Eleme	nt End Acti	.ons ***						
Elen	Cas/Cmb	FX	FT	FZ	HX	ht	HZ		
	Node	KPS	KPS	KPS	FT-KPS	FT-KPS	FT-KPS		
1		*******			*********				
Ţ	101-1								
	1	10.0000	0.0000	0.0942	0.0000	0.0000	0.0000		
	2	-10.0000	0.0000	-0.0942	0.0000	-1.8846	0.0000		
	LC2 2								
	1	-2.0000	0.0000	-0.2573	0.0000	8.327e-17	0.0000		
		2.0000	0.0000	0.2573	0.0000	5.1457	0.0000		
		e 0000	0 0000	0 1621	0 0000	0 227- 17	0 0000		
	2	-8.0000	0.0000	-0,1031	0.0000	3 2612	0.0000		
2	2	-010000	0.0000	0.1071	0.0000	2.2016	0.0000		
-	LCI 1								
	3	10.0000	0.0000	0.0942	0.0000	-1.8846	0.0000		
	4	-10.0000	0.0000	-0.0942	0.0000	0.0000	0.0000		
	LC2 2								
	3	2.0000	0.0000	1.7427	0.0000	-34.8543	0.0000		
	4 COX0311	-2.0000	0.0000	-1.1441	0.0000	1.//08-15	0.000		
	3	12,0000	0.0000	1.8369	6.0000	-36 7388	0 0000		
	4	-12.0000	0.0000	-1.8369	0.0000	1.776e-15	0.0000		
3									
	LC1 1								
	2	0.0942	10.0000	0.0000	0.0000	0.0000	1.8846		
	3	-0.0942	10.0000	0.0000	0.0000	0.0000	-1.8846		
	LC2 2	1 7/07	2 0000	0 0000	0 0000	0 0000			
	2	1+/42/	-2.0000	0.0000	0.0000	0.0000	-2.142/		
	COMBIL	-1+1461	2.0000	0.0000	0.0000	0.0000	-24.0243		
	2	1.8369	8.0000	0.000	0.0000	0.0000	-3,2612		
	3	-1.8369	12.0000	0.0000	0.0000	0.000	-36.7388		

RVUT

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BENT					
RICID	TRANT				

KIGLD FRAME							
***************************************							
IRM Rev	8.8.3	•		OCT	6,1989	09:21:12	
Analysis	No.	2	Thin Shell			Page 6	

\*\*\* Line Element End Actions \*\*\*

Quantity	Livit	Value	Unit	Elen	Node	Ldcmb/Cs
FX	Max	12.0000	KPS	2	3	COHBI I
	Min	-12.0000	KPS	2	4	COHBI 1
FT	Max	12.0000	KPS	3	3	COHB1 1
	Kin	-2.0000	KPS	3	2	LC2 2
77	Kax	1.8369	kps	2	3	COHB1 1
	Min	-1.8369	Kps	2	4	COHB1 1
нī	Hax	0.0000	PT-KPS	1	1	LCI 1
	Hin	0.0000	PT-KPS	1	1	LCI 1
нı	Max	5.1457	FT-KPS	1	2	LC2 2
	Min	-36.7388	FT-KPS	2	3	Combi 1
НZ	Hax	1.8846	PT-KPS	3	2	LC1 1
	Hin	-36.7388	PT-KPS	3	3	Combi 1

.

APPENDIX D

EXAMPLE NO. 3. - 2D HANGER TRUSS



\*\*\* Units Definition \*\*\*

Unit Group	Unit
l - Lengths	FEET
2 - Element Properties	INCHES
3 - Forces	KIPS
4 - Angles	DEGREES
5 - Displacements	INCHES
6 - Hasses	HASS
7 - Time	SECONDS
8 - Stress Forces	KIPS

Vertical Axis = Z

2.2.2

Gravitational Constant (g) =32.2 FT /SEC /SEC

Name/No.	Table Data	
A36 1	Katerial Type Kodulus of Elasticity (E) Foisson's Ratio (v) Shear Modulus (G) Alpha	= ISOTROPIC = 29000.0 ksi. = 0.3 = 11153.8457 ksi. = 0.0

	COETRUSS	
**********	2-D Aircrait Hanger Ex	3mjle
TRH REV 8.8.3		OCT 11, 1989 13+38
ANALYSIS NO.8	THIN SHELL	PAGE 3
*** Load Cases	***	
Name/No.	Table Data	
BODY	Analysis type	= Static
1	Load case type	= Dead load
	Status	= Active
	Level	= 9
	Color	= 2
	Convert nodal loads to masses	= No
DEAD	Analysis type	= Static
2	Load case type	= Dead load
	Status	= Active
	Level	= 21
	Color	= 2
	Convert nodal loads to masses	≖ No
LIVE	Analysis type	= Static
3	Load case type	🔍 = Dead load
	Status	= Active
	Level	= 22
	Color	= 4
	Convert nodal loads to masses	= No
CRANERALL	Analysis type	= Static
4	Load case type	= Dead load
	Status	= Active
	Level	= 23
	Color	= ]
	Convert nodal loads to masses	= Ho
VINDX	Analysis type	= Static
5	Load case type	= Dead load
	Status	= Active /
	Level	<b>≖</b> 24
	Color	= 7
	Convert nodal loads to masses	= No

	COETRUSS		
	2-D Aircraft Hanger	Example	
*****************	*****	*****************	*********
IRM REV 8.8.3		OCT 11,1989	13:38
ANALYSIS NO.8	THIN SHELL		PAGE 4

\*\*\* Load Combinations \*\*\*

Name/No.	Туре	Print Resu	Global Hultiplier	Load Case Name	Load Case Multiplier
COMBI	Linear	Yes	1.0	BODY	1.0
1				DEAD	1.0
				LIVE	1.0
COMB2	Linear	Yes	1.0	BODY	1.0
2				DEAD	1.0
				LIVE	1.0
				CRANERALL	1.0
COHB3	Linear	Yes	0.75	BODY	1.0
3				DEAD	1.0
				LIVE	1.0
				WINDX	1.0
C0):34	Linear	Yes	0.75	BODY	1.0
4				DEAD	1.0
				LIVE	1.0
				CRANERALL	1.0
				WINDX	1.0

\*\*\* Nodal Loads \*\*\*

Node		Loc	Load			Load
Id/Label	Туре	Glob	Case	Load	Yector or Dof	Туре
						*********
2	VEC	GLOB	DEAD	-2.500	(0,0,1)	FOR
	VEC	GLOB	LIVE	-1.500	(0,0,1)	FOR
	VEC	GLOB	WINDX	1.800	(1,0,0)	FOR
4	VEC	GLOB	DEAD	-2.500	(0,0,1)	FOR
	VEC	GLOB	LIVE	-1.500	(0,0,1)	FOR
	VEC	GLOB	WINDX	1.800	(1,0,0)	FOR
5	VEC	CLOB	DEAD	-2.500	(0,0,1)	FOR
	VEC	CLOB	LIVE	-1.500	(0,0,1)	FOR
	YEC	GLOB	WINDX	1.800	(1,0,0)	FOR
15	VEC	GLOB	DEAD	-2.500	(0,0,1)	FOR
	VEC	CLOB	LIVE	-1.500	(0,0,1)	FOR
	VEC	CLOB	WINDX	1.800	(1,0,0)	FOR
17	VEC	CLOB	DEAD	-2.500	(0,0,1)	FOR
	VEC	CLOB	LIVE	-1.500	(0,0,1)	FOR
	VEC	GLOB	WINDX	1.800	(1,0,0)	FOR
34	VEC	GLOB	CRANERALL	-5.000	(0,0,1)	FOR
36	VEC	GLOB	DEAD	-2.500	(0,0,1)	for
	VEC	GLOB	LIVE	-1.500	(0,0,1)	FOR
	VEC	GLOB	WINDX	1.800	(1,0,0)	FOR
37	VEC	GLOB	DEAD	-2.500	(0,0,1)	FOR
	VEC	GLOB	LIVE	-1.500	(0,0,1)	FOR
	VEC	GLOB	HINDX	1.800	(1,0,0)	FOR
38	VEC	GLOB	DEAD	-2.500	(0,0,1)	FOR
	VEC	GLOB	LIVE	-1.500	(0,0,1)	FOR
	YEC	CLOB	WINDX	1.800	(1,0,0)	FOR
39	VEC	GLOB	DEAD	-2.500	(0,0,1)	FOR
	VEC	GLOB	LIVE	-1.500	(0,0,1)	FOR
	VEC	GLOB	WINDX	1.800	(1,0,0)	FOR
46	VEC	GLOB	CRANERAIL	-5.000	(0,0,1)	FOR
48	VEC	GLOB	DEAD	-2.500	(0,0,1)	FOR
	VEC	GLOB	LIVE	-1.500	(0,0,1)	FOR
	VEC	CLOB	WINDX	1.800	(1,0,0)	FOR
49	VEC	GLOB	DEAD	-2.500	(0,0,1)	FOR
	VEC	CLOB	LIVE	-1.500	(0,0,1)	FOR
	YEC	GLOB	WINDA	1.800	(1,0,0)	FUR
50	VEC	GLOB	DEAD	-2.500	(0,0,1)	FOR
	VEC	GLOB	LIVE	-1.500	(0,0,1)	FOR
	VEC	GLOB	WINDX	1.800	(1,0,0)	FOR
51	VEC	GLOB	DEAD	-2.500	(0,0,1)	FOR
	VEC	GLOB	LIVE	-1.500	(0,0,1)	FOR
	VEC	GLOB	WINDX	1.800	(1,0,0)	FOR

IRM Re Analys	ev 8.8.3 sis No. 10		Thi	in Shell	001	ſ 12,1989	07:57 Page 1
*** {	Support Rea	ctions **	*				
Node	Cas/Cmb	FX KPS	FY KPS	FZ KPS	MX FT-KPS	MY FT-KPS	HZ FT-KP
1	BODY 1 DEAD 2	0.7325	0.0000	5.7545	0.0000	0.0000	0.0
	LIVE 3	1.2678	0.0000	10.8800	0.0000	0.0000	0.0
	CD ANED A	0.7607	0.0000	6.5280	0.0000	0.0000	0.0
		0.3968	0.0000	3.7462	0.0000	0.0000	0.0
		-3.7230	0.0000	-87,2170	0.0000	0.0000	0.0
	CONBI 1	2.7610	0.0000	23.1625	0.0000	0.0000	0.0
	COHB2 2	3.1577	0.0000	26.9087	0.0000	0.0000	0.0
	COHB3 3	-0.7215	0.0000	-48.0408	0.0000	0.0000	0.0
	COHB4 4	-0.4239	0.0000	-45.2312	0.0000	0.0000	0.0
3	BODY 1	0.2962	0.0000	3.9778	0.0000	0.0000	0.0
	DEAD 2	1 0/72	0 0000	5 3700	0 0000	0.0000	0.0
	LIVE 3	1.0472	0.0000	2.0000	0.0000	0.0000	0.0
	CRANERA	U.0205 IL 4	0.0000	3.2220	0.0000	0.0000	0.0
	WINDX 5	0.5225	0.0000	1.2538	0.0000	0.0000	0.0
	COHB1 1	-7.9770	0.0000	79.8219	0.0000	0.0000	0.0
	CO1B2 2	1.9718	0.0000	12.5697	0.0000	0.0000	0.0
	COMB3 3	2.4943	0.0000	13.8235	0.0000	0.0000	0.0
	<u>с</u> (ла <i>). 1</i>	-4.5039	0.0000	69.2938	0.0000	0.0000	0.0
17	UV[104 4	-4.1121	0.0000	70.2341	0.0000	0.0000	0.0
14	BUDY I	-0.7325	0.0000	5.7545	0.0000	0.0000	9.0
	dead 2	-1.2678	0.0000	10.8800	0.0000	0,0000	0.0

			CO) 2-D Aircraft	ETRUSS t Hanger Ex	ample		
*****	********	*******	********	**********	********	*****	*******
IRH R Analys	ev 8.8.3 sis No. 10		Thi	n Shell	001	12,1989	07:57:28 Page 2
±±± (	Support Rea	ctions **	±				
Node	Cas/Cmb	FX KPS	fy KPS	PZ KPS	HI FT-KPS	ht FT-kps	HZ FT-KPS
14	LIVE 3 CRANERA WINDX 9 COHB1 1 COHB2 2 COHB3 2 COHB3 2 COHB4 4 BODY 1 DEAD 2 LIVE 3	KPS -0.7607 ML 4 -0.3958 -3.7229 -2.7609 -3.1577 -4.8629 -5.1605 -0.2962 -1.0472 -0.6283	KPS 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	KPS 6.5280 3.7462 87.2167 23.1625 26.9087 82.7844 85.5941 3.9778 5.3700 3.2220	FT-KPS 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	FT-KPS 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 7.0000 0.0000 0.0000	FT-KPS 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
	CRANER WINDX COMB 1 COMB 2 COMB 3 COMB 4	-0.6283 AIL 4 -0.5225 5 -7.9771 1 -1.9718 2 -2.4943 3 -7.4617 4 -7.9526	0.0000 0.0000 0.0000 0.0000 0.0000	3.2220 1.2538 -79.8217 12.5697 13.8235 -50.4390	0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	0.0000 0.0000 0.0000 0.0000 0.0000 0.0000

	COETRUSS			
	2-D Aircraft Hanger Exampl	e		
***************	**********************	******	********	*********
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Analysis No. 10	Thin Shell			Page 3

\*\*\* Support Reactions \*\*\*

Quantity	Limit	Value	Unit	Node	Ldcmb/Cs
FX	Kax	3.1577	kps	1	COMB2 2
	Hin	-7.9771	kps	16	WINDX 5
M	Kax	0.0000	kps	1	BODY 1
	Hin	0.0000	kps	1	BODY 1
rz.	Kax	87.2167	KPS :	14	WINDX 5
	Kin	-87.2170	KPS	1	WINDX 5
нх	Hax	0.0000	FT-KPS	1	BODY 1
	Hin	0.0000	FT-KPS	1	BODY 1
нı	Hax	0.0000	FT-KPS	1	BODY 1
	Hin	0.0000	FT-KPS	1	BODY 1
НZ	Kax	0.0000	FT-KPS	1	BODY 1
	Min	0.0000	FT-KPS	1	BODY 1

APPENDIX E

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EXAMPLE NO. 4 - 3D AIRCRAFT HANGER



#### \*

			HANCI	R				
	3D	ANALYSIS	OF AN	AIRCRAFT	HANCER			
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\*\*\* Units Definition \*\*\*

· · · //

C/ - 2

Unit Group	Unit
·····	
1 - Lengths	1661
2 - Element Properties	INCHES
3 - Forces	KIPS
4 - Angles	DEGREES
5 - Displacements	INCHES
6 - Masses	HASS
7 - Tize	SECONDS
8 - Stress Forces	KIPS

Vertical Axis = Y

Gravitational Constant (g) =32.2 FT /SEC /SEC

				HAN	JER				
		3D AL	NALYSIS	OF AI	AIRCRAFT	HANGER			
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\*\*\* Material Property Tables \*\*\*

Hame/Ho.	Table Data	
*************	***************************************	*************************************
SIDDL	Haterial Type	= isotropic
1	Modulus of Elasticity (E)	= 30000.002 ksi.
	Poisson's Ratio (v)	<b>=</b> 0.3
	Shear Hodulus (G)	= 11538.4619 ksi.
	Alpha	= 0.0

HANCER

E)

		3D	ANALYSIS	OF A	I AIRCRAFT	HANGER			
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## \*\*\* Load Cases \*\*\*

1 10 Sec. 1.

A REAL PROPERTY OF

Hame/Ho.	Table Data		
ICI	Analysis type	= Static	••
1	Load case type	= Dead load	
	Status	= Active	
	Level.	¥ 9	
	Color	<b>=</b> 2	
	Convert nodel loads to masses	= No	
LC2	Analysis type	= Static	
2	Load case type	= Dead load	
	Status	= Active	
	Level	<b>=</b> 9	
	Color	<b>≖</b> 2	
	Convert nodal loads to masses	= No	
LC4	Analysis type	= Static	
3	Load case type	= Dead load	
	Status	= Active	
	Level	= 9	
	Color	= 2	
	Convert nodal loads to masses	= No	
LC3	Analysis type	= Static	
4	Load case type	= Dead load	
	Status	= Active	
	Level	= 9	
	Color	= 2	
	Convert nodal loads to masses	= No	
LCS	Analysis type	= Static	
5	Load case type	= Dead load	
-	Status	= Active	
	Level	<b>z</b> 9	
	Color	= 2	
	Convert nodal loads to masses	= No	

HANGER					
3	D ANALYSIS OF AN AIRCRAFT	HANCER			
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\*\*\* Load Combinations \*\*\*

None

## HANGER 3D ANALYSIS OF AN AIRCRAFT HANGER

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\*\*\* Nodal Loads \*\*\*

Node		Loc	Load				Load
Id/Label	Туре	Glob	Case		Load	Vector or Dof	Туре
2	VIC	GLOB		1.03	1,200	(01.0)	FOR
ĩ	VEC	GLOB		IC3	1.200	(0, -1, 0)	FOR
6	VIC	GLOB		LC3	1.200	(0, -1, 0)	FOR
5	VIC	GLOB		1.04	-6.000	(-1.0.0)	FOR
6	VEC	GLOB		LC3	1.200	(0, -1, 0)	FOR
•	VIC	GLOB		IC3	6.000	(0, -1, 0)	FOR
7	VEC	GLOB		LC3	1.740	(0, -1, 0)	FOR
ġ	VEC	GLOB		LC3	1.200	(0, -1, 0)	FOR
10	VEC	GLOB		LC3	1.200	(0, -1, 0)	FOR
11	VEC	GLOB		LC3	1.200	(0, -1, 0)	FOR
12	VEC	GLOB		LC4	-6.000	(-1,0,0)	FOR
13	VEC	GLOB		LC3	1.200	(0, -1, 0)	FOR
	VEC	GLOB		LC3	6.000	(0, -1, 0)	FOR
14	VEC	GLOB		LC3	1.740	(0, -1, 0)	FOR
21	VEC	GLOB		lci	-18,050	(0,0,1)	TOR
	VEC	GLOB		LC3	1.740	(Ů,-1,Ű)	FOR
28	VEC	GLOB		IC1	-18.050	(0,0,1)	FOR
	VEC	GLOB		LC3	1.743	(0,-1,0)	FOR
34	VEC	CLOB		LC3	1.740	(0,-1,0)	FOR
40	VEC	CLOB		LC3	1.740	(0;-1,0)	FOR
41	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR
42	VEC	GLOB		LC1	-36.100	(0,0,1)	FOR
	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR
43	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR
44	VEC	CLOB		IC3	1.740	(0, -1, 0)	FOR
45	VEC	CLOB		LCI	-36.100	(0,0,1)	FOR
	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR
46	VEC	CLOB		LC3	1.740	(0,-1,0)	FOR
47	VEC	GLOB		LC3	1.740	(0, -1, 0)	FOR
48	VEC	GLOB		121	-36.100	(0,0,1)	FOR
	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR
49	VEC	GLOB		1C3	1.740	(0,-1,0)	FOR
57	VEC	GLOB		LC2	18.500	(0,0,-1)	FOR
64	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR
70	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR
71	YEC	GLOB		103	1.740	(0,-1,0)	FOR
12	VEC	GLOB			-36.100	(0,0,1)	FOR
-	YEC	GLOB		LC3	1.740	(0,-1,0)	FOR
73	VEC	GLOB		LC3	1.740	(9,-1,0)	FOR
74	VEC	GLOB		LC3	1.740	(0,-1,0)	for

	ELA	NGER			
	3D ANALYSIS OF	AN AIRCRAFT	HANCER		
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\*\*\* Nodal Loads \*\*\*

Node Id/Lahel 1	lvne	Lee Glob	Load Case		Load	Vector or Dof	Load Type
							•/1/•
75 1	TTC .	GLOB		LCI	-36.100	(0,0,1)	FOR
1	<b>TEC</b>	GLOB		LC3	1.740	(0,-1,0)	FOR
76 1	TEC .	GLOB		LC3	1.740	(0, -1, 0)	FOR
77 \	VEC	CLOB		LC3	1.740	(0,-1,0)	FOR
78 1	VEC	GLOB		IC!	-36.100	(0,0,1)	FOR
1	PEC	CLOB		TC3	1.740	(0,-1,0)	FOR
79	VEC	CLOB		LC3	1.740	(0, -1, 0)	FOR
80 \	VEC	CLOB		101	9.000	(0,0,-1)	FOR
81 \	VIC	CLOB		LC2	9.250	(0,0,-1)	FOR
87	VEC	GLOB		LC2	9.250	(0,0,-1)	FOR
88	VEC	GLOB			9.000	(0,0,-1)	FUR
90	YEC	GLUB		103	1.200	(0, -1, 0)	FOR
91 1	YLC .	GLUB		103	1.200	(0, -1, 0)	FUR
92	VLC	GLUB		LUS	1.200	(0, -1, 0)	FUR
93	120	GLUD		1.02	-0.000	(-1,0,0)	PUK
59	126 UPC	OLUD CT OB		102	1.200	(0, -1, 0)	Pak
05 1	120 UTC	CTOR		102	0.000	(0, -1, 0)	PUK
102	750 VEC			100	10 050	(0, -1, 0)	IVA
102	VIC	CTOR		101	-10.000	(0,0,1)	IVA
108	VEC	CTOR		103	1 760	(0, -1, 0)	IVA FOR
114	VEC	CLOB		LC3	1.740	(0,~1,0)	100
115	VEC	GLOB		LC3	1.740	(0, -1, 0)	TOR
116	VEC	GLOB		ICI	-36,100	(0, 0, 1)	FOR
	VEC	GLOB		LC3	1.740	(0, -1, 0)	FOR
117	VEC	CLOB		LC3	1.740	(0, -1, 0)	FOR
118	VEC	CLOB		LC3	1.740	(0, -1, 0)	FOR
119	VEC	CLOB		LC1	-36.100	(0.0,1)	FOR
	VEC	CLOB		LC3	1.740	(0, -1, 0)	FOR
120	VEC	CLOB		LC3	1.740	(0, -1, 0)	FOR
121	VEC	CLOB		LC3	1.740	(0, -1, 0)	FOR
122	VEC	CLOB		LC1	-36.100	(0,0,1)	FOR
	VEC	CLOB		LC3	1.740	(0,-1,0)	FOR
123	VEC	GLOB		<u>LC3</u>	1,740	(0,-1,0)	<u>tûr</u>
124	VEC	GLOB		121	9.000	(0,0,-1)	FOR
131	VEC	CLOB		102	18.500	(0,0,-1)	FOR
132	VEC	GLOB		101	9.000	(0,0,-1)	FOR
133	VEC	GLOB			-2.300	(0,0,1)	FOR
134	VEC	GLOB			-2.300	(0,0,1)	FOR
135	YEC	GLOB		ICI	-2.300	(0,0,1)	FOR

HANGER

		3D	ANALYSIS	07	AN .	AIRCRAFT	HANGER					
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AFALYSIS	NO.9			THIN	I SH	ELL				PAGE	1	

\*\*\* Nodal Loads \*\*\*

Node		Loc	Load				Load
Id/iabel	Type	Glod	Case		Load	Vector or Gof	Туре
136	VEC	GLOB		LCI	-2.300	(0,0,1)	FOR
137	VEC	GLOB		LC1	-2.300	(0, 0, 1)	FOR
138	VLC	GLOB		LC1	-2.300	(0, 0, 1)	FOR
139	VEC	CLOB		LC1	-2.300	(0, 0, 1)	FOR
144	VEC	CLOB		LC1:	-2.300	(0,0,1)	FOR
145	YEC	CLOB		LC1	-2.300	(0, 0, 1)	FOR
146	VEC	CLOB		LC1	-2.300	(0, 0, 1)	FOR
147	VEC	CLOB		101	-2.300	(0, 0, 1)	FOR
148	VEC	CLOB		LC1	-2.300	(0, 0, 1)	FOR
149	VEC	GLOB		LCI	-2.300	(0, 0, 1)	FOR
150	VEC	GLOB		101	-2.300	(0, 0, 1)	FOR
155	VEC	GLOB		LC1	-2.300	(0, 0, 1)	FOR
156	VEC	GLOB		LC1	-2.300	(0,0,1)	FOR
157	VEC	GLOB		101	-2.300	(0,0,1)	FOR
158	VEC	GLOB		LC1	-2.300	(0, 0, 1)	FOR
159	VEC	CLOB		LCI	-2.300	(0,0,1)	FOR
160	VEC	GLOB		LCI	-2.300	(0, 0, 1)	FOR
161	VEC	GLOB		LCI	-2.300	(0,0,1)	FOR
166	VEC	GLOB		LC3	1.740	(0, -1, 0)	FOR
167	VEĆ	GLOB		LC4	-6.000	(-1;0,0)	FOR
169	VEC	GLOB		LC3	1.200	(0, -1, 0)	FOR
170	VEC	CLOB		LC3	1.200	(0, -1, 0)	FOR
171	VEC	GLOB		LC3	1.200	(0,-1,0)	FOR
172	VEC	GLOB		LC3	1.200	(0,-1,0)	FOR
	VEC	GLOB		LC3	6.000	(0,-1,0)	FOR
179	VEC	CLOB		lci	-18.050	(0,0,1)	FOR
	YIC	CLOB		LC3	1.740	(0,-1,0)	FOR
185	VEC	CLOB		LC3	1.740	(0,-1,0)	FOR
191	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR
192	YEC	glob		LC3	1.740	(0,-1,0)	FOR
193	VEC	GLOB		ICI	-36.100	(0,0,1)	FOR
	VEC	CLOB		LC3	1.740	(0,-1,0)	FOR
194	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR
195	VEC	GLOB		LC3	1.740	(0,-1,0)	for
196	VEC	glob		LC1	-36.100	(0,0,1)	FOR
	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR
197	VEC	CLOB		LC3	1.740	(0,-1,0)	FOR
198	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR
199	VEC	GLOB		ICI	-36.100	(0,0,1)	for
	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR

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*****	3D AN/	HANG. LIYSIS OF AN	? \IRCRAFT	HANGER	******	*****
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\*\*\* Hodal Loads \*\*\*

Node Id/Label	Type	Loc Glod	Load Case		Load	Vector or Dof	Load Type
200	VEC	GLOB		LC3	1.740	(0,-1,0)	FOR
202	VEC	GLOB		LC2	9,250	(0, 0, -1)	FOR
208	VIC	GLOB		LC2	9.250	(0, 0, -1)	FOR
212	VEC	GLOB		LC1.	-2.300	(0,0,1)	FOR
213	VEC	GLOB		ICI İ	-2.300	(0,0,1)	FOR

				HANC	ER					
		3D	ANALYSIS	OF AN	AIRCRAFT	HANGER				
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Element	Load				Load	Abs/	Location
Label/Id	Case	Туре	Frame	Dir	(Velocity)	Rel	(Shape Factor)
Z44	LC4	DIST	GLOBAL	X	0.120		
	LC5	DIST	GLOBAL	Y	-0.120		
245	LC4	DIST	GLOBAL	X	0.120		
	LCS	DIST	GLOBAL.	ÿ	-0.120		
		0101		-	- ~ • • • • •		
246	1.04	DIST	CLOBAL	T	0.120		
<i>2</i> ,10	105	DICT	CT OPAL	v v	0.120		
		0101	OTVOUD	T	-0.170		
267	104	DICT	OT OD AT	•	0 120		
247	104	D191	GLADAL	Å	0.120		
	105	DIST	GLUBAL	I	-0.120		
		5	~ ~ ~ ~ ~ ~ ~	_			
248	LC4	DIST	GLOBAL	X	0.120		
	LC5	DIST	GLOBAL	Y	-0.120		
249	LC4	DIST	GLOBAL	X	0.120		
	LCS	DIST	GLOBAL	Y	-0.120		
				-			
250	1.04	DIST	GLOBAL	X	0.120		
	1.05	DIST	CLORAL	7	-0 120		
	607	0101	030010	•	-01170		
251	164	DICT	CLORAT	Y	0 120		
271	104	DICT		A V	0.120		
	μ.)	0151	GLUDAL	1	-0.120		
252	1.07	D748	01 00 11		A 100		
727	LL)	DIST	GLUBAL	I	-0.120		
253	LC5	DIST	GLOBAL	Y	-0.120		
254	LCS	DIST	GLOBAL	Y	-0.120		
255	LC5	DIST	<b>GLOBAL</b>	Y	-0.120		
256	LC5	DIST	GLOBAL	7	-0.120		
2.50		0101	000010	•	-01100		
260	10%	ntc7	CLOBAL	Y	0 120		
203	104	D101		• A •	0.120		
	1Y)	DT21	GLUDAL	1	-0.120		
			an	_			
270	LC4	DIST	GLOBAL	X	0.120		
	LC5	DIST	GLOBAL	Ĩ	-0.120	I	

HANGER 3D ANALYSIS OF AN AIRCRAFT HANGER

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ARALYSIS	NO.9		1	THIN	SHELL				PACE	10

Element	Load				Load	Abs/	Location
Label/Id	Case	Туре	Frame	Dir	(Velocity)	Rel	(Shape Factor)
********		•••••					
271	LC4	DIST	GLOBAL	X	0.120		
	LC5	DIST	GLOBAL	Y	-0.120		
272	LC4	DIST	GLOBAL	I	0.120		
	LC5	DIST	GLOBAL	Ť	-0.120		
	200			•			
273	1.64	DIST	GLOBAL	T	0,120		
215	105	דפות	CLORAL	7	-0 120		
	<b>L</b> WJ	VIOI	otopan	1	-0.120		
27%	106	nter	CLUBAT	¥	0 120		
214	104	DIGI	CT OP AT	- A - T	0 120		
	102	p151	GLUBAL	I	-0.120		
170	101		~ ~ ~ ~ ~ ~ ~ ~		0 100		
215	104	DIST	GLOBAL	Ă	0.120		
	LC5	DIST	GLOBAL	Υ	-0.120		
276	LC4	DIST	GLOBAL	X	0.120		
	LC5	DIST	GLOBAL	, Y	-0.120		
277	LC5	DIST	GLOBAL	Y	-0.120		
	-						
278	1.05	DIST	GLOBAT	. 7	-0.120		
				•••			
279	105	DIST	CLOBAL	. 7	-0.120		
217	505	0101	OLIVDAL		-01120		
280	1.05	DICT	(11 AB 11	4	6 120	•	
200	100	0191	GLUDAL	. 1	-0.120		
201	1.05	<b>N7.0</b> M	(11 0.5.11		0 100		
201	143	0121	GLUBAL	1	-0.120		
207	10/	5105	07 07 11		0.100		
299	11.4	DIST	GLUBAL	À	0.120		
	LCS	DIST	GLOBAL	ιĭ	-0.120	)	
				-	• • • •		
295	LC4	DIST	GLOBAI	ιX	0.120	)	
	LC5	DIST	CLOBAI	I I	-0.120	)	
296	LC4	DIST	GLOBAI	X	0.120	)	
	LC5	DIST	CLOBA	Y	-0.120	)	
				-			
297	LC4	DIST	GLOBAI	X	0.120	)	
	1.05	DIST	GLOBAI	. γ	.0.120	Ì	
		****	ARANTA	w 🔺	-~***61	,	

HANGER 3D ANALYSIS OF AN AIRCRAFT HANGER						
IRM REV 8.8.3 ANALYSIS NO.9	THIN SHELL	OCT	6,1989	09:32 PAGE 11		

Element Label/Id	Load Case	Туре	Frame	Dir	Load (Velocity)	Abs/ Rel	Location (Shape Factor)
298	LC4	DIST	GLOBAL	X	0.120		***********
	LCS	DIST	GLOBAL	Y	-0.120		
299	LC4	DIST	GLOBAL	X	0.120		
	1.05	DIST	GLOBAL	Y	-0.120		
300	LC4	DIST	GLOBAL	X	0.120		
	LCS	DIST	GLOBAL	I	-0.120		
301	LC5	DIST	GLOBAL	Y	-0.120		
302	LC5	DIST	GLOBAL	Y	-0.120		
303	LC5	DIST	CLOBAL	Y	-0.120		
304	LC5	DIST	GLOBAL	Y	-0.120		
316	LC1	CONCEN	GLOBAL	Z	-2.300	ABS	3.000
	LC4	DIST	CLOBAL	X	0.120		
	LC5	DIST	GLOBAL	Y	-0.120		
318	LC5	DIST	GLOBAL	Y	-0.120		
402	LCI	CONCEN	GLOBAL	Z	-2.300	ABS	8.000
	LC4	DIST	GLOBAL	X	0.120		
403	LC1	CONCEN	GLOBAL	Z	-2.300	ABS	8.000
	LC4	DIST	<b>GLOBAL</b>	X	0.120		

****		3[	ANALYSIS O	P AN AIRCRA	FT FANGER	********	
IRM R Analy	lev 8.8.3 1918 Ko. 10		Th	in Shell	00	T 6,1989	09:35:16 Page 1
***	Support Read	tions f	**				
Node	Cas/Cmb	FX KPS	PY KPS	FZ KPS	HI PT-KPS	hy FT-kps	HZ FT-KPS
1	LC1 1 -3.	.764e-03	0.1414	51.5898	0.0000	0.0000	0.000
	LC2 2 3.	.174e-05	0.0632	1.7657	0.0000	0.0000	0.0000
	LC4 3	-0.1724	-1.444e-04	-0.0654	0.0000	0.0000	0.0000
	LC3 4 2.	.469e-03	-0.3515	144.2092	0.0000	0.0000	0.0000
_	LC5 5	.273e-04	-0.3722	76.3472	0.0000	0.0000	0.0000
8	LC1 1 3.	237e-03	0.1618	57.4144	0.0000	0.0000	0.0000
		334e-06	0.0222	7.3836	6.0000	0.0000	0.0000
	109 J	-0.1475	-1.083e-03	-0.5122	0.0000	0.0000	v.0000
	105 4 3.	.066e-03	-0.3494	144.0016	0.0000	0.0000	0.0000
15	5.	.061e-04	-0.3566	73.1910	0.0000	0.0000	0.0000
.,	-9,	354e-07	-0.1412	51.4570	0.0000	0.0000	0.0000
	6. LC4 3	193e-06	0.0200	13.2128	0.0000	0.0000	0.0000
	2. LC3 4	759e-05	2.413e-05	9.909e-03	0.0000	0.0000	0.0000
	-9. LC5 5	794e-07	-0.3922	-138.0613	0.0000	0.0000	0.0000
21	-1. ICI 1	161e-03	-0.2009	-69.8883	0.0000	0.0000	0.0000
	4. LC2 2	583e-06	0.0000	0.0000	0.0000	0.0000	0.0000
	-3. 104 3	758e-05	0.0000	0.0000	0.0000	0.0000	0.0000
	-2. LC3 4	100e-04	0.0000	0.0000	0.0000	0.0000	0.0000
	6. LCS 5	553e-06	0.0000	0.0000	0.0000	0.0000	0.0000
	7.	034e-03	0.0000	0.0000	0.0000	0.0000	0.0000

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HANGER 3D ANALYSTS OF AN ATRCRAFT KAMGER							
titti IRM H Analy	k*********** Rev 8.8.3 7515 No. 10	)		in Shell	0CT	********** 6,1989	09:35:18 Page 2
***	Support Re	eactions *	ŧû				
Node	Cas/Cub	ıx KPS	FY KPS	FZ KPS	HI FT-KPS	hy FT-RPS	HZ FT-KPS
22	LC1 1	-9.635e-07	-0.1630	58.1963	0.0000	0.0000	0.0000
	LC2 Z	4.711e-06	-0.0215	7.5825	0.0000	0.0000	0.0000
	LC3 4	2.155e-05	1.952e-04	0.0795	0.0000	0.0000	0.0000
	LC5 5	-9.229e-07	-0.3905	-137.7014	0.0000	0.0000	0.0000
28	LCI 1	9.18/e-04	-0.1922	-00.9080	0.0000	0.0000	0.0000
	LC2 2	2 8560-05	0.0000	0.0000	0.0000	0.0000	0.0000
	LC4 3	-1.587e-04	0.0000	0.0000	0.0000	0.0000	0.0000
	LC3 4	6.225e-06	0.0000	0.0000	0.0000	0.0000	0.0000
	LC5 5	-5.556e-03	0.0000	0.0000	0.0000	0.0000	0.0000
29	LCI I	-5.190e-07	-11.2074	20.7408	0.0000	0.0000	0.0000
	LCZ Z	-8.131e-08	-2.0223	-9.1136	0.0000	0.0000	0.0000
	1C3 6	3.620e-05	-1 <b>.</b> 149e-03	-8.243e-03	0.0000	0.0000	0.0000
	ICS 5	-5.173e-07	15.5225	130.2448	0.0000	0.0000	0.0000
35	LC1 1	3.716e-05	7.8543	66.6324	0.0000	0.0000	0.0000
	LC2 2	5.190e-07	11.2074	20.8444	0.0000	0.000	0.0000
	LC4 3	8.131e-08	1.9604	12.6537	0.0000	0.000	0.0000
	LC3 4	-3.0200-03	1.20/0-03	-0.0852	0.0000	0.0000	0.0000
	LCS S	-3.716e-05	10.1058	-73.1204	0.0000	0.0000	0.0000

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	HANGER 3D ANALYSIS OF AN AIRGEAFT HANGER								
IRM Ro Analys	ev 8.8.3 sis No. 10	*******	Thi	in Shell	OCT	6,1989	09:35:19 Page 3		
*** (	Support Re	actions *	**						
Node	Cas/Cub	FX KPS	ri Kps	YZ KPS	HI FT-KPS	ht Ft-kps	MZ FT-KPS		
59		5 <b>.995e-</b> 07	-13.0001	23.0209	0.0000	0.0000	0.0000		
	LC4 3	7.615e-08	-2.0135	1.6720	0.0000	0.0000	0.0000		
	LC3 4	2.608e-05	-9.241e-03	-0.0667	0.0000	0.0000	0.0000		
	LCS 5	4.419e-07	15.5244	129.8627	0.0000	• 0.0000	0.0000		
65	ICI 1	5.995e-07	13.0009	22.2235	0.0000	0.0000	0.0000		
	LC2 2	7.615e-08	1.9911	1.8389	0.0000	0.0000	0.0000		
	LC4 3	-2.608e-05	9.895e-03	-0.6552	0.0000	0.0000	0.0000		
	LC5 5	4.419e-07	18.6360	-135.7733	0.0000	0.0000	0.0000		
89	LCI 1	3.489e-05	9.7073	-70.0318	0.0000	0.0000	0.0000		
	LC2 2	-0.0255	0.1572	56.0116	0.0000	0.0000	0.0000		
	LC4 3	-25.3971	-3.681e-03	- 46.7342	0.0000	0.0000	0.0000		
	LC3 4	-6.322e-03	-0.3507	143.3361	0.0000	0.0000	0.000		
96	103 5 101 1	-3.914e-03	-0.1325	27.1509	0.0000	0.0000	0.0000		
	LC2 2	<b>-4.408e</b> -06	-0.1630	58.2946	0.0000	0.0000	0.0000		
	LC4 3	-2.798e-05	0.0204	13.3637	0.0000	0.0000	0.0000		
	LC3 4	2.035e-05	1.9538-03	0.6998	0.0000	0.0000	Q.0000		
	LCS S	-3.078e-03	-0.0714	-24.8876	0.0000	0.0000	0.0000		

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HANGER 3D ANALYSTS OF AN ATRCRAFT HANGER							
inn R Analy	*********** ev 8.8.3 sis No. 10	******	**************************************	**************************************	**************************************	r 6,1989	09:35:21 Page 4
***	Support Re	actions f	**				
Node	Cas/Cub	n LPS	rt Kps	TZ KPS	HI FT-KPS	ht FT-kps	HZ FT-KPS
102	LC1 1	2.107e-05	0.0000	0.0000	0.0000	0.0000	0.0000
	LC4 3	1.696e-04	0.0000	0.0000	0.0000	0.0000	0.0000
	LC3 4	1.337e-04	0.0000	0.0000	0.0000	0.0000	0.0000
103	LCS 5	0.0186	0.0000	0.0000	0.0000	0.0000	0.0000
103	LC2 2	7 <b>.909e-</b> 07	-13.0090	22.9713	0.0000	0.0000	0.0000
	LC4 3	3.339e-07 1.848e-05	-2.0411 -0.0781	-9.2657 -0.6473	0.0000	0.0000	0.0000
	LC3 4 LC5 5	1.490e-07	15.5096	129.7886	0.0000	0.0000	0.0000
109	LCI 1	1.023e-04	2.8032	23.7223	0.0000	0.0000	0.0000
	LC2 2	3.339e-07	1.9642	12.7933	0.0000	0.0000	0.0000
	LC3 4	1.848e-05	0.0797	-3.0838	0.0000	0.0000	0.0000
	LC5 5 	1.490e-07	18.6511 3.6052	-136.6190 -26.0774	0.0000	0.0000	0.0000
168	LC1 1	0.0367	0.1321	47.1577	0.0000	0.0000	0.0000
	LC4 3	2.874e-04	0.0218	7.4493	0.0000	0.0000	0.0000
	LC3 4	-V.18/6	-3.2//e-05 -0.3515	-0.0172 144.2225	0.0000	0.0000	0.0000
	LCS 5	<b>.386e-</b> 03	-0,1389	28,5896	0,0000	0,0000	0,000

*****	HANGER 3D ANALYSIS OF AN AIRCRAFT HANGER							
IRM R Analy	ev 8.8.3 sis No. 10		Th	in Shell	007	6,1989	09:35:24 Page 5	
***	Support Re	actions *	**					
Node	Cas/Cub	FI KPS	FT KPS	77 KPS	MI FT-KPS	ht FT-kps	HZ FT-KPS	
173	LCI 1	1 780-05	.0 1305	57 1405	0 0000	0 0000	0 0000	
	LC2 2	1./0VE-VJ	-4.1323	J6+14JJ	0.000	V.VVVV	0.000	
	LC4 3	2.686e-05	-0.0216	7.5115	0.0000	0.0000	0.0000	
	102 4	2.695e-05	5.627e-06	2.171e-03	0.0000	0.0000	0.0000	
	-	4.285e-08	-0.3922	-138.0671	0.0000	0.0000	0.0000	
	105 5	3.494e-03	-0.0752	-26.1706	0.0000	0.0000	0.0000	
179	1 121	7.882e-05	0.0000	0.000	0.0000	0.0000	0.0000	
	LC2 2	1 620- 04	0.0000	0 0000	A AAAA	0.0000		
	LC4 3	1.0298-04	0.0000	0.0000	0.0000	0.0000	0.0000	
	LC3 4	2.128e-04	0.0000	0.0000	0.0000	0.0000	0.0000	
	1/5 E	1.071e-06	0.0000	0.0000	0.0000	0.0000	0.0000	
	5 6 6 6	-0.0212	0.0000	0.0000	0.0000	0.0000	0.0000	
120	TCI I	1.090e-07	-11.2873	20.1570	0.0000	0.0000	0.0000	
	LC2 2	3.1880-07	_1.0037	1.7300	0 0000	0 0000	0.0000	
	LC4 3	A A7A . AF	- 107			••••••	0.0000	
	- LC3 4	3.8/98-03	-2.43/e-04	-1.8806-03	0.0000	0.0000	0.0000	
	1.C5 5	5.425e-07	15.5232	130.2504	0.0000	0.0000	0.0000	
146		1.094e-04	2.9413	24.9505	0.0000	0.0000	0.0000	
198	Del I	1.090e-07	11.2948	15.2025	0.0000	0.0000	0.000	
	LCZ 2	3.1880-07	1:9866	1.7936	0.0000	0.000	0 0000	
	LC4 3-	9 876 - 04	E AA1 - A4	••••••	~ ~~~~	• • • • • •	•••••	
	LC3 4	·3.8/9e-05	) <b>.</b> 701e-04	-0.0217	0.0000	0.0000	0.000	
	105 5	5.425e-07	18.6407	-136.4043	0.0000	0.0000	0.0000	
	<i></i>	1.094e-04	3.7782	-27.3452	0.0000	0.0000	0.000	

		30	ANALYSIS (	HANGER )F AN AIRCRAI	T HANGER			
IRM Rev 8.8.3 Analysis No. 10			Ti	iin Shell	007	OCT 6,1989		
*** ;	Support Re	actions f	***					
Node	Cas/Cmb	FI LPS	ri Kps	FZ KPS	MI FT-KPS	NT FT-KPS	ML. FT-KPS	
214		-0.0106	-3.469e-18	3.8682	0.0000	0.0000	0.0000	
	LC4 3	4.800e-05	0.0000	-7.106e-03	0.0000	0.0000	0.0000	
	LC3 4	-25.3348	2.168e-19	51.1078	0.0000	0.0000	0.0000	
	LCS 5	-2.116e-04	-3.469e-18	0.0743	0.0000	0.0000	0.0000	
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HANGER										
		3D	ANALYSIS	OF AN	AIRCRAFT	HANGER				
*******	*******	********	********	*****	*******	*******	*******	*******		
IRM Rev	8.8.3					OCT	6,1989	09:35:26		
Analysis	No. 10			Thin S	hell		•	Page 7		

\*\*\* Support Reactions \*\*\*

Quantity	Linit	Value	Unit	Node	Ldcmb/Cs
π	Max	0.0367	<b>RPS</b>	168	ICI 1
	Min	-25.3971	KPS	89	LC4 3
п	Max	18.6511	KPS	109	LC3 4
	Min	-13.0090	<b>KPS</b>	103	LC1 1
172	Max	144.2225	KPS	168	LC3 4
	Hin	-138.0671	<b>LPS</b>	173	LC3 4
Ж	Max	0.0000	PT-KPS	1	ICI-1
	Min	0.0000	PT-LPS	1	LC1 1
Ы	Max	0.0000	FT-KPS	1	LCI 1
	Min	0.0000	FT-KPS	1	LCI 1
ЫZ	Hax	0.0000	FT-KPS	1	LCI I
	Min	0.0000	FT-LPS	1	ICI I

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