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Program Engineering and Maintenance Service Washington, D.C. 20591 **Resilient Modulus of Freeze-Thaw Affected Granular Soils for Pavement Design and Evaluation**

Part 3. Laboratory Tests on Soils from Albany County Airport

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U.S. Army Cold Regions Research and Engineering Laboratory Hanover, New Hampshire 03755-1290

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PREFACE

This report was prepared by David M. Cole, Research Civil Engineer, Applied Research Branch, Experimental Engineering Division; Diane L. Bentley, Research Civil Engineer, Civil Engineering Research Branch, Experimental Engineering Division; Glenn D. Durell, Mechanical Engineering Technician, Engineering and Measurement Services Branch, Technical Services Division, and Thaddeus C. Johnson, Civil Engineer and Chief of the Civil Engineering Research Branch, Experimental Engineering Division, U.S. Army Cold Regions Research and Engineering Laboratory.

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The work was done at CRREL and a number of people contributed to the successful conclusion of this area of the project. The authors acknowledge in particular E. Chamberlain who was closely involved in equipment development, D. Carbee for his help in specimen preparation, D. Keller who assisted in field coring and sample preparation, L. Irwin for helpful discussions of the test results, J. Ingersoll who was responsible for generating the moisture characteristic curves and who assisted in the development of the tensiometer systems, and A. Tice who generated the unfrozen water content data for the test soils.

This report was technically reviewed by E.J. Chamberlain and F. Sayles of CRREL.

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Resilient Modulus of Freeze-Thaw Affected Granular Soils for Pavement Design and Evaluation Part 3. Laboratory Tests on Soils from Albany County Airport

D.M. COLE, D.L. BENTLEY, G.D. DURELL AND T.C. JOHNSON

INTRODUCTION

This is one of four reports that document the laboratory and field test results of an extensive research effort jointly funded by the U.S. Army Corps of Engineers, the Federal Highway Administration and the Federal Aviation Administration. The project, entitled Full-Scale Field Tests to Evaluate Frost Action Predictive Techniques. called for laboratory testing and field verification of the resilient properties of a number of test soils located at Winchendon, Massachusetts (the subject of Parts 1 and 2 [Cole et al. 1986, Johnson et al. 1986a] of this series of reports) and Albany, New York (the subject of this report and Part 4 [Johnson et al. 1986b]). Part 1 includes detailed descriptions of the laboratory testing procedures and methods of data analysis and interpretation. Consequently, this report does not dwell on such matters, but instead concentrates on the presentation and analysis of results from the two taxiways that we investigated at the Albany County Airport.

The objectives of the work call for characterizing the test soils under a variety of seasonal conditions: frozen, thawed and recovered. The first two conditions are self-explanatory; "recovered" refers to soil that has drained and possibly consolidated after thawing and has consequently regained (or recovered) the same degree of stiffness it possessed prior to the freezing and thawing cycle. The testing sequence used in the laboratory work is designed to simulate the progression of events that the soils experience in the field. This process relies heavily on the use of soil moisture tension and temperature as links between laboratory and field results.

Part 4 (Johnson et al. 1986b), the companion to this report, presents the results of the surface deflection measurements on the two taxiways and verifies the laboratory-determined resilient modulus expressions developed in the present work. The verification is accomplished through the use of a computer code (called NELAPAV) that carries out a layered elastic analysis of the pavement system. The program and the verification procedure have been covered in detail elsewhere (Irwin and Johnson 1981).

Field data on the temperature and moisture tension history of the test sections provided the appropriate range of these variables in the laboratory testing. Specimens were first tested in repeated-load triaxial compression in the frozen state, beginning with the lowest temperature, at several values of axial stress and a single value (69.0 kPa) of confining stress. Next, the specimens were completely thawed on specially designed triaxial cell bases (see Cole et al. 1986) and tested at up to five levels of soil moisture tension. The increases in moisture tension were achieved by drawing water from the specimen via the triaxial cell's drainage system. This procedure simulates the gradual recovery of stiffness experienced by thawweakened soils.

The repeated-load triaxial testing yields the resilient modulus, M_r (defined as cyclic stress divided by recoverable axial strain), as a function of applied stresses, temperature (for soils in the frozen state), moisture tension ψ (for the unfrozen state), and dry unit weight γ_d where applicable. A simple nonlinear relationship of the form

$$M_{\rm r} = k_1 [f(\sigma)]^{k_1} \tag{1}$$

is used to represent the test data— k_1 is generally a function of ψ , and in some cases γ_d , and k_2 is a constant. A linear regression technique is used to find constants that give the best fit to the test data.

The stress function $f(\sigma)$ is taken as either the commonly used first stress invariant J_1 (sum of the principal stresses) or the ratio J_2/τ_{oct} (ratio of the second stress invariant to the octahedral shear stress). The latter function has been examined at length by Cole et al. (1981, 1986). Its usefulness stems from its ability to adequately reflect the tendency of many granular soils to exhibit an increasing modulus with both increasing confining stress (σ_3) and decreasing principal stress ratio (σ_1/σ_3). All analyses are carried out in terms of both stress functions for comparison.

The reader is referred to Cole et al. (1986) for extensive background information on the project in general as well as for details of the laboratory testing methodology. The Albany County Airport work closely follows the Winchendon, Massachusetts, activity with one exception: we tested no field cores from the Albany site. All specimens were remolded in the laboratory using material that had been remixed according to the original gradation specifications for the taxiway sections.

TEST SECTIONS AND MATERIALS

Figure 1 gives cross sections of each taxiway. Field instrumentation yielded temperature and moisture tension profiles for each section, which are presented by Johnson et al. (1986b). Gradation curves for the test soils appear in Figure 2, and Table 1 gives some physical characteristics and classifications for the soils.

The water table fluctuated seasonally between 1.5 and 2.0 m at both sites. Frost penetration depths for the periods of observation are given by Johnson et al. (1986b).

Taxiway A consists of a layer of asphalt concrete, a crushed stone base, a gravelly sand subbase and a silty fine sand subgrade. Taxiway B consists of a badly broken layer of asphalt concrete, an asphalt penetration macadam stone base, a silty sandy gravel subbase and a silty fine sand subgrade.

Since the moisture retention characteristics of these materials are of interest, the moisture tension versus water content curves were determined for several of the soils in the laboratory. Curves for the Taxiway A base and subbase and the Taxiway B subgrade appear in Appendix A. The subgrades for both taxiways were nearly identical, so the Taxiway B subgrade curve is assumed valid for Taxiway A as well. We were not able to obtain such data for the Taxiway B subbase since it was too coarse to test in our cell.



Figure 1. Albany Airport taxiway profiles.



a. Taxiway A.

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b. Taxiway B. Figure 2. Gradation curves for Albany soils.

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		Maximum				
	Unified Soil size		Coeff	Specifi		
Soil	Classification	(mm)	C _u	_C,	gravity	
Taxiway A subbase	SM	19.1	95.8	2.2	2.73	
Taxiway A subgrade	SM	0.42	4.0	1.6	2.67	
Taxiway B subbase	GM	19.1	16.3	0.22	2.68	
Taxiway B subgrade	SM	0.42	2.7	1.2	2.69	

Table 1. Physical characteristics and classification of the Albany Airport soils.

SPECIMEN PREPARATION

Test soils

We obtained shovel samples of all the layers of material. Since it was impossible to distinguish between the base and subbase materials of Taxiway B because of the deterioration and insufficient thickness of the base, both layers were sampled and tested as a single material.

The various soils were sieved and remixed in the laboratory according to original specifications. The coarse-grained materials were compacted in a 152-mm-diameter, 305-mm-high mold and were frozen at a rate of 25 mm/day under open system conditions. These specimens were capped in the manner described by Cole et al. (1986). They did not heave appreciably (i.e., less than 10% of specimen height). The fine-grained subgrade material was compacted in a tapered 152-mm-diameter, 152-mm-high mold and subjected to the same freezing conditions. Once the material was frozen, several 51-mm-diameter, 127-mm-long specimens were machined from the samples and carefully trimmed prior to testing.

Since the frost did not penetrate to the depth of interest insofar as the layered elastic analysis was concerned, it was necessary to characterize the subgrade in the unfrozen (as well as frozen and thawed) state. For this purpose, specimens (51 mm in diameter by 127 mm long) were merely compacted to design specifications and tested.

Additional details of the preparation procedures are given by Cole et al. (1986).

Asphalt concrete

We were able to obtain usable cores of the asphalt concrete layers for both taxiways. Taxiway A was sufficiently thick to yield 102-mm-diameter, 250-mm-long cores, which were easily trimmed and tested. The thin asphalt layer of Taxiway B, however, made it necessary for us to form a specimen of adequate height by stacking three of the short cores and binding them together with a thin layer of asphalt emulsion. The asphalt concrete was tested in the dry state, although moisture content is expected to affect the resilient behavior (Johnson et al. 1978).

LABORATORY TESTING

All testing of the soils was carried out in one of two triaxial cells, depending upon specimen size. The asphalt concrete was tested only in uniaxial compression. For all laboratory tests, we used an electro-hydraulic, closed-loop testing machine operated in LOAD control.

To achieve a steady-state response, 200 loading cycles were applied at each combination of axial and radial stress. The M_r values were calculated from a representative cycle near the end of each run.

The test equipment and procedures are fully described by Cole et al. (1986).

Soil testing

Two triaxial cells, with several unique features, were designed and built for this testing program. The cells differed primarily in size: one accommodated the 51-mm-diameter specimens while the other accommodated the 152-mm-diameter specimens. The cells featured removable bases, which facilitated the sequential testing of each specimen, and built-in tensiometer systems to continuously monitor soil moisture tension.

Since handling of the frozen specimens presented no serious problems given adequate coldroom facilities, a single cell base equipped with a thermocouple was used for the frozen state tests. However, since many specimens were often extremely weak and deformable upon thawing, the removable cell base concept was developed. This approach called for designing triaxial cells that could be completely assembled about a specimen that was mounted on the cell base. We used up to six bases for the small cell and four for the large cell. In this manner, a number of specimens could be tested sequentially without removing them from their respective cell bases. The major cell components and the deformation and load measuring devices were easily transferred from one base to another. Cole et al. (1986) give details of this procedure and of the equipment design.

The sequential testing approach was used to allow the maximum amount of testing on each specimen and to allow use of the major cell components while tested specimens were equilibrating at new moisture tension levels. Simulation of the recovery period after thawing was achieved by alternately testing and drying each specimen until the moisture tension reached the level observed in the field. At each level of moisture tension, a specimen was subjected to the sequence of confining and nominal deviator stresses given in Cole et al. (1986). The actual deviator stresses at each data point, with slight corrections for the changes in specimen area, are given in Appendix B. All of the triaxial tests were carried out with a vacuum applied to the specimen through the drainage system. The vacuum level coincided with the desired soil moisture tension level for the test. This was done to ensure a constant moisture tension level throughout load cycling.

Axial deformation was measured on the specimen with a system of four Linear Variable Differential Transformers (LVDTs), which measured the relative displacement of two circumferentially mounted rings. Radial displacement was measured at three points, equally spaced about the circumference, at midheight on the specimen. The load was monitored by a miniature load cell, mounted in the triaxial cell, in direct contact with the top cap of the specimen. This load cell also served as a feedback, controlling the load applied by the testing machine.

These measurements allowed the calculation of both resilient and permanent strains in the axial and radial directions, which in turn allowed the calculation of resilient modulus and resilient Poisson's ratio (μ_{τ}).

Waveforms of applied stress

The soils were subjected to two loading waveforms that correspond to the loading characteristics of the two devices used in the surface deflection tests done in the field. The waveform simulating the Repeated-load Plate-Bearing apparatus (designated RPB) was a 1-s-on, 2-s-off pulse. A 28-ms haversine, repeated every 2 s was used to simulate the load pulse produced by the Falling-Weight Deflectometer (designated FWD) (Fig. 3).

Throughout the course of this study, we made a gradual shift in the field verification work from the use of the RPB device to the FWD device. In the Albany County Airport work, we used the FWD device exclusively, but continued to apply the RPB loading waveform in the laboratory testing for the sake of continuity with earlier work.

Initial tests indicated that there was no significant difference in the modulus determined with these two waveforms, so we decided to apply the FWD pulse as a rule and spot-check the modulus



Figure 3. Load pulse waveforms used in the repeated load triaxial tests (repeated load plate-bearing apparatus [RPB] waveform and falling-weight deflectometer [FWD] waveform).

periodically with the RPB pulse. Consequently, in contrast to Cole et al. (1986) where modulus equations were presented for each waveform separately, the equations presented in this work are applicable to both waveforms for all granular materials.

Asphalt concrete

The asphalt concrete cores were tested at temperatures of -10° , 5° , 25° and 40° C in uniaxial compression. Maximum axial cyclic stresses of approximately 68.0, 103.0, 136.0, 174.0 and 228.0 kPa were applied under three waveforms.

Axial deformation was measured using LVDTs mounted on circumferential clamps. Load was measured by a load cell mounted on the actuator of the testing machine. The machine was operated in LOAD control, as in the soil tests.

The asphalt concrete tests employed three waveforms: the RPB and FWD pulses described earlier and a continuous haversine at frequencies of 1, 4 and 16 Hz. The latter loading condition was included for completeness and is according to ASTM D3497-79T (ASTM 1981).

DATA REDUCTION AND ANALYSIS

Soil

The frozen state test on a given soil yields an M_r value for a certain stress level and temperature. Testing in the thawed or unfrozen states yields an $M_{\rm r}$ value for a given applied stress state and moisture tension level. Not all of the stress levels given in Cole et al. (1986) could be applied to each specimen at all values of moisture tension, ψ . Since each specimen was tested a number of times, it was important to avoid excessive permanent deformation in the early stages of testing. Consequently, the testing of thawed material at low ψ values was often terminated before the higher stress levels were applied. Appendix B gives the actual stress levels applied for each test. In general, newly thawed specimens ($\psi = 2$ kPa) were tested to deviator and confining stress levels of approximately 28 kPa; the associated resilient axial strains were approximately 3×10^{-4} to 4×10^{-4} . Stiffer specimens were tested to stress levels of approximately 70 kPa and corresponding strain levels near 8×10^{-4} .

As a result of the testing sequence, each specimen generated from 50 to 70 data points. Each of these data points represents a nominally steady state material response after 200 load cycles. The resilient behavior generally stabilized within 10-20 cycles for the lower stress levels and within about 50 cycles for the higher stress levels.

The test data were subjected to multiple linear regression analysis, the details of which are given in Cole et al. (1986). We employed the simple non-linear expression given by eq 1 to represent the material in the thawed state. The coefficient k_1 was treated as a function of ψ and γ_d , where applicable. The exponent k_2 was considered constant for a given material with a given freeze-thaw history. Earlier work indicated that k_2 does not vary systematically with ψ (Cole et al. 1981).

The analyses employ one of two stress functions to model the stress dependency of the thawed soils: J_1 the first stress invariant, and J_2/τ_{oct} , the second stress invariant divided by the octahedral shear stress. The former stress function is traditional and reflects the tendency of the modulus to increase with increasing bulk stress. However, J_1 is insensitive to the effect of the principal stress ratio σ_1/σ_3 . It is frequently observed for granular soils that modulus decreases as the principal stress ratio increases. The latter stress function, J_2/τ_{oct} , addresses the effect of principal stress ratio and thus proves useful in the present analysis.

In a common repeated-load triaxial test, where $\sigma_2 = \sigma_3$ and $\sigma_1 = \sigma_3 + \sigma_d$, the two stress functions are given as:

$$J_1 = \sigma_d + 3\sigma_3 \tag{2}$$

$$J_2/\tau_{\rm oct} = \frac{9\sigma_i^2 + 6\sigma_i\sigma_d}{\sqrt{2}\sigma_d} \quad . \tag{3}$$

where $J_1 = \sigma_1 + \sigma_2 + \sigma_3$

$$J_{2} = \sigma_{1}\sigma_{2} + \sigma_{2}\sigma_{3} + \sigma_{1}\sigma_{3}$$

$$\tau_{\text{oct}} = \frac{1}{2} [(\sigma_{1} - \sigma_{2})^{2} + (\sigma_{2} - \sigma_{3})^{2} + (\sigma_{1} - \sigma_{3})^{2}]^{\frac{1}{2}}.$$

See Cole et al. (1981) for details regarding eq 3.

Moisture tension is incorporated in the modulus expression (eq 1) through the term $[(101.36-\psi)/\psi_0]^{A_1}$ where ψ is in kilopascals, ψ_0 is a reference stress of 1 kPa, and 101.36 is atmospheric pressure in kilopascals. For soils in which dry unit weight varied significantly, the term γ_d/γ_0 entered into the analysis. γ_0 is a reference density of 1 Mg/m³.

As in Part 1 of this series (Cole et al. 1986), the frozen state test data were analyzed in terms of the unfrozen water content, W_u , normalized to the total gravimetric water content, W_T . The expressions for W_u are of the form

$$W_{\rm u} = a(-\theta/\theta_{\rm o})^{-b} \tag{4}$$

where W_{ij} is gravimetric water content expressed as a decimal, a and b are regression constants, θ is the temperature in degrees celsius, and θ_0 is a reference temperature of 1°C. The expressions for $W_{\rm u}$ were obtained using the pulsed Nuclear Magnetic Resonance (NMR) method* (for additional details, see Cole 1984). The Taxiway A base and subbase materials were too coarse for testing in the NMR device, so it was necessary to estimate the constants needed in eq 4. The exponent b is the more important of the two. A value of 0.25, approximately in the middle of the range of typical values, proved suitable, producing values of R^2 = 0.92 in the resilient modulus regression analyses for each soil in the frozen state. No attempt was made to account for the physical characteristics of the soils in the determination.

The range of validity of the frozen state tests is from -5.0° or -8.8°C, depending on soil type, to the completely thawed state. The analysis was accomplished by including a number of data points representing the condition of the material upon thaw. Clearly, problems are encountered with eq 4 if the soil temperature, θ , is set equal to zero. However, this problem vanishes upon the following consideration. As the temperature of the frozen soil increases, it eventually reaches a point below 0°C at which all the soil water is unfrozen. The temperature at which the soil is completely thawed may be very close to 0°C. This is true for fine-grained soils in general. As a consequence of the mathematical formulation, the unfrozen water content term $W_{\rm u}/W_{\rm T}$ goes to 1 before the temperature term goes to 0 and the singularity in eq 2 is thus avoided. Temperatures greater than that required to completely thaw the soil are not meaningful in the frozen soil model. Thus, once the soil is completely thawed, the equations given for the thawed state are used. The equations for the frozen state give sensible values for modulus when the temperature term goes to unity. However, the expressions are generally stress-independent, and should be used only for cases where at least some pore ice is present.

Asphalt concrete

The results of the cyclic uniaxial testing of the asphalt concrete were analyzed, for each type of waveform, in terms of temperature, stress and frequency (for the continuous haversine loading). A second-order expression proved adequate to model the temperature dependency of the resilient modulus.

RESULTS AND DISCUSSION

General

Appendix B gives a tabulation of all the laboratory test results on the frozen, thawed and unfrozen soil specimens. The tabulation gives confining and deviator stress levels, resilient axial and radial strains, μ_r , M_r , γ_d , gravimetric moisture content and ψ . Temperature is given for all frozen-state tests.

Table 2 gives the results of the regression analyses for all soils under all test conditions. The asphalt concrete analysis results are also given in Table 2, and the results of the analysis are plotted in Figure 4. These equations produce M_r values in megapascals, provided the units of all variables are appropriate (see notes, Table 2). Two or more equations appear for a given soil and state in Table 2. This was done to demonstrate the influence of either different stress functions or additional terms (i.e., a density term) on the empirical result. Subsequent work on the verification of these results using a layered elastic analysis (Johnson et al. 1986b) employs the simplest of these equations with the highest R^2 values to represent a given layer.

A change in the procedure used to analyze the frozen-state test data resulted in somewhat different constants in the regression equations for the frozen soils. The frozen state equations given in Table 2 are based solely on data points obtained from frozen specimens. The highest temperatures were in the range of -0.2° to -0.5° C, and strictly speaking these temperatures define the limit of applicability of the equations. The frozen state equations in Table 2 were used in the layered elastic analysis of the test sections.

A subsequent analysis provided a means to extend the range of applicability of the frozen state equations. This analysis incorporated data points from tests performed upon thawing, and thus resulted in regression equations that are valid at temperatures between the limits of the equations in Table 2 and the melting point. These equations are given in Table 3.

The equations in Table 2 appear somewhat different from the form given in eq 1. The aggregation of all terms other than the stress function raised to a power is to be considered as the term k_1 in eq 1. For the thawed soils, then, k_1 is a function

Personal communication with A. Tice, CRREL 1984.

Table 2. Results of regression analyses—asphalt concrete and test soils from Albany Airport (the standard error is referenced to the natural log of M_r value).

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Material	Load pulse	Regression equation	<u></u>	R ¹	Sid. error	Eq.
Taxiway A						
Asphalt/concrete	FWD	${}^{\dagger}M_{\rm r}({\rm MPa}) = 1.84 \times 10^4 \exp[-3.80 \times 10^{-2} T - 9.14 \times 10^{-4} T^2]$	88	0.97	0.19	1
	RPB	$M_{\rm r}({\rm MPa}) = 1.01 \times 10^4 \exp[-6.50 \times 10^{-2} T - 6.50 \times 10^{-4} T^2]$	93	0.98	0.24	2
	Haversine	$M_{\rm r}({\rm MPa}) = 1.09 \times 10^4 \exp[-4.75 \times 10^{-2} T - 7.81 \times 10^{-4} T^2] f_{\rm Hz}^{0.20}$	280	0.97	0.22	3
Thawed base	FWD/RPB	${}^{\dagger}M_{\rm r}({\rm MPa}) = 1.10 \times 10^{6} [f(\psi)]^{-2.40} f_{1}(\sigma)^{0.30}$	222	0.82	0.16	4
		$M_{\rm r}({\rm MPa}) = 4.44 \times 10^3 [f(\psi)]^{-2.20} f_1(\sigma)^{0.37}$	222	0.82	0.16	5
		$M_{\rm r}({\rm MPa}) = 3.68 \times 10^4 [f(\psi)]^{-2.15} f_1(\sigma)^{0.30} f(\gamma_{\rm d})^{3.44}$	222	0.84	0.16	6
		$M_{\rm r}({\rm MPa}) = 2.56 \times 10^4 [f(\psi)]^{-1.99} f_1(\sigma)^{0.37} f(\gamma_{\rm d})^{2.90}$	222	0.82	0.16	7
Frozen base		$^{\dagger}M_{\rm r}({\rm MPa}) = 1.89 \times 10^{\rm s} (w_{\rm u}/w_{\rm f})^{-4.82}, w_{\rm u} = 3 \times 10^{-2} (-7)^{-0.25}, w_{\rm f} = 0.075$	78	0.78	0.66	8
Thawed subbase	FWD/RPB	${}^{\dagger}M_{\rm r}({\rm MPa}) = 2.07 \times 10^{2} [f(\psi)]^{-3.05} f_{2}(\sigma)^{0.29}$	149	0.80	0.20	9
		$M_{\rm r}({\rm MPa}) = 4.35 \times 10^{4} [f(\psi)]^{-2.72} f_1(\sigma)^{0.37}$	149	0.80	0.20	10
		$M_{\rm r}({\rm MPa}) = 1.39 \times 10^{10} [f(\psi)]^{-3.38} f_{\rm r}(\sigma)^{0.29} f(\gamma_{\rm d})^{-7.00}$	149	0.82	0.20	11
		$M_r(MPa) = 8.00 \times 10^4 [f(\psi)]^{-2.99} f_1(\sigma)^{0.37} f(\gamma_d)^{-5.55}$	149	0.82	0.19	12
Frozen subbase		$^{\dagger}M_{r}(MPa) = 8.18 \times 10^{4} (w_{u}/w_{t})^{-4.02}, w_{u} = 3 \times 10^{-2} (-T)^{-0.25}, w_{t} = 0.055$	53	0.70	0.84	13
Non-frozen	FWD/RPB	${}^{\dagger}M_{\rm r}({\rm MPa}) = 1.34 \times 10^{4} [f(\psi)]^{-1.50} f_2(\sigma)^{0.33}$	262	0.80	0.80	14
subgrade		$M_{\rm r}({\rm MPa}) = 7.73 \times 10^3 [f(\psi)]^{-1.34} f_1(\sigma)^{0.35}$	262	0.78	0.17	15
Taxiway B						
Thawed base/	FWD/RPB	${}^{\dagger}M_{r}(MPa) = 5.55 \times 10^{10} [f(\psi)]^{-4.72} f_{1}(\sigma)^{0.27}$	173	0.69	0.26	16
subbase		$M_{\rm r}({\rm MPa}) = 9.67 \times 10^{9} [f(\psi)]^{-4.36} f_1(\sigma)^{0.36}$	173	0.73	0.24	17
		$M_{\rm r}({\rm MPa}) = 4.28 \times 10^{6} [f(\psi)]^{-3.99} f_{\rm 2}(\sigma)^{0.27} f(\gamma_{\rm d})^{8.35}$	173	0.71	0.25	18
		$M_r(MPa) = 1.56 \times 10^6 [f(\psi)]^{-3.69} f_1(\sigma)^{0.36} f(\gamma_d)^{7.72}$	173	0.74	0.23	19
Frozen base/subt	ase	${}^{\dagger}M_{r}(MPa) = 1.00 \times 10^{3} (w_{u}/w_{t})^{-2.63}, w_{u} = 3 \times 10^{-2} (-T)^{-0.22}, w_{t} = 0.05$	92	0.96	0.42	20
Thawed subgrade	FWD/RPB	${}^{\dagger}M_{\rm r}({\rm MPa}) = 8.76 \times 10^{3} [f(\psi)]^{-2.38} f_2(\sigma)^{0.30}$	293	0.72	0.20	21
		$M_{\rm r}({\rm MPa}) = 3.36 \times 10^{5} [f(\psi)]^{-2.15} f_1(\sigma)^{0.34}$	293	0.68	0.21	22
		$M_r(MPa) = 3.80 \times 10^4 [f(\psi)]^{-2.36} f_2(\sigma)^{-3.25} f(\gamma_d)^{-3.06}$	293	0.74	0.19	23
		$M_{\rm r}({\rm MPa}) = 1.35 \times 10^{4} [f(\psi)]^{-2.13} f_{\rm i}(\sigma)^{0.34} f(\gamma_{\rm d})^{-3.06}$	293	0.70	0.20	24
Frozen subgrade		$M_{\rm r}({\rm MPa}) = 2.66(w_{\rm u}/w_{\rm t})^{-1.02}f_2(\sigma)^{0.78}, w_{\rm u} = 3.14 \times 10^{-2}(-7)^{-0.29}, w_{\rm t} = 0.29$	152	0.82	0.92	25
		$M_{\rm r}({\rm MPa}) = 2.59(w_{\rm u}/w_{\rm f})^{-0.85}f_{\rm i}(\sigma)^{0.93}, \ w_{\rm u} = 3.14 \times 10^{-2}(-7)^{-0.29}, w_{\rm v} = 0.29$	152	0.84	0.85	26
		$M_{\rm r}({\rm MPa}) = 3.31 \times 10^{1} (w_{\rm u}/w_{\rm l})^{-0.87} f_{\rm s}(\sigma)^{0.68}, w_{\rm u} = 3.14 \times 10^{-2} (-T)^{-0.29}, w_{\rm s} = 0.29$	152	0.82	0.92	27
Nonfrozen subgra	ade	$M_{\rm r}({\rm MPa}) = 5.16 \times 10^6 [f(\psi)]^{-2.71} f_2(\sigma)^{0.26}$	278	0.81	0.15	28
-		$M_{\rm r}({\rm MPa}) = 5.48 \times 10^{6} [f(\psi)]^{-2.71} f_2(\sigma)^{0.26}$	278	0.72	0.18	29
		$M_{\rm r}({\rm MPa}) = 2.49 \times 10^{6} [f(\psi)]^{-2.73} f_{1}(\sigma)^{0.26} f(\gamma_{\rm d})^{2.07}$	278	0.82	0.14	30

NOTES:

RPB = repeated-load plate-bearing apparatus waveform

FWD =	falling-weight	deflectometer	waveform
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	•		
t = equations used in analysis	$f(\gamma) = (101.36 - \omega)/\omega_0$	J , =	first stress invariant (kPa)
π = number of points	ω = moisture tension (kPa)	$J_i =$	second stress invariant (kPa)
$R^{1} = \text{coefficient of determination}$	$\psi_0 = 1 \text{ kPa}$	τ _{oct} =	octahedral shear stress (kPa)
$M_r = resilient modulus$	$f_1(\sigma) = (J_1/\sigma_0)$	$f(\gamma_d) =$	7d/70
$T = \theta/\theta_0$	$f_1(\sigma) = (J_1/\tau_{\rm oct})/\sigma_0$	γ d =	dry unit weight (Mg/m³)
θ = temperature (°C)	$f_i(\sigma) = \tau_{oct})/\sigma_0$	γ ₀ =	1 Mg/m³
$\theta_o = 1^{\circ}C$	$\sigma = \text{stress} (kPa)$	w _u =	unfrozen water content (decimal)
$f_{u_{r}} = load$ waveform frequency (Hz)	σ _α ≃ IkPa	w _t =	total water content (decimal)



Figure 4. Regression analysis results showing resilient modulus versus temperature for various waveforms for asphalt concrete specimens in repeated load, unconfined compression.

Table 3.	Additional reg	ression equations	for some frozen soils.	The data bases for these	equations include points
represent	ative of the soils	upon thawing.			•

Material	Load pulse	Regression equation*	n	R ²	Std. error
Taxiway A					
Base, frozen	FWD/RPB	$M_r(MPa) = 5.80 \times 10^{\circ} (W_u/WT)^{-3.88} W_u = 3 \times 10^{\circ} (-7)^{-0.25}, WT = 0.075$	104	0.92	0.63
Subbase, frozen	FWD/RPB	$M_r(MPa) = 6.66 \times 10^{\circ} (W_u/WT)^{4.68} W_u = 3 \times 10^{\circ} (-T)^{-0.25}, WT = 0.055$	76	0.92	0.74
Taxiway B					
Subgrade, frozen	FWD/RPB	$M_r(MPa) = 1.36 \times 10^2 (W_u/WT)^{-5.26} W_u = 3 \times 10^4 (-T)^{-0.22}, WT = 0.05$	92	0.83	0.89
Subgrade, frozen	FWD/RPB	$M_{f}(MPa) = 1.36 \times 10^{2} (W_{u}/WT)^{-5.26} W_{u} = 3 \times 10^{-4} (-T)^{-0.22}, WT = 0.05$	92	0.83	

* See notes of Table 2 for definitions of terms.

<u> </u>	μ _τ		<u> </u>
Taxiway A		Taxiway B	
Base	0.33	Base-subbase	0.30
Subbase	0.39	Subgrade	0.35
Subgrade	0.26		

of the term $f(\psi)$, and occasionally a function of dry density through the term $f(\gamma_d)$. The exponent on the $f(\sigma)$ term is, of course, k_2 .

As found in Part 1 of this series, the resilient Poisson's ratio, $\mu_{\rm T}$, was not found to be a systematic function of any of the test variables. Regression analyses similar to those performed for the resilient moduli yielded unacceptably low values of R^2 , indicating no clear dependency of $\mu_{\rm T}$ on any of the test variables. Table 4 gives the average values of Poisson's ratio calculated from all the thawed-state test results for each soil.

The regression equations generated the curves given in this section with certain exceptions, noted below.

Resilient modulus

Frozen soil

Figure 5 shows plots of the regression equations for the frozen soils. These equations represent the data rather well: the R^2 values range from 0.83 to 0.92. As can be seen by the form of the equations for the frozen state, the curvature of these relationships is a strong function of the unfrozen water content versus temperature relationship for a particular soil. The modulus of frozen soil can be between two and three orders of magnitude higher than that of the same soil in the thawed state. Some representative data points are also shown in Figure 5. The Taxiway B subgrade was the only soil to exhibit a significant stress dependency. The plotted curve is based on representative values of J_1 for each temperature.

The relatively fine-grained subgrade layers have noticeably lower moduli than the coarse-grained base and subbase layers. The greater unfrozen water content of the fine-grained material undoubtedly contributes substantially to the lower stiffness. Additionally, the Taxiway B subgrade was the only soil to exhibit a systematic stress dependency in the frozen state. The reason for this is unclear. Generally, the stress level effects are so completely overshadowed by the temperature effects that temperature (via the unfrozen water content term) is the only significant variable in the analysis. Inspection of the R^2 values associated with these equations indicates that the inclusion of a stress term only marginally improves the correlation.

As with the soils tested in the earlier phase of this work (Cole et al. 1986), the resilient deformation was not sufficiently large to produce consistently measurable radial deformation in the frozen soil. As a result, we were not able to calculate reliable values for the resilient Poisson's ratio for any of the soils in the frozen state.

Thawed soils

Upon thaw, virtually all test soils developed a moisture tension level of 2.0 kPa, indicating a state of less than complete saturation. As noted above, these soils were tested at several levels of moisture tension up to 24 kPa, which was the highest value recorded in the field test sections.

The dependency of M_r on moisture tension was addressed analytically through the term

$$\left(\frac{101.36-\psi}{\psi_0}\right)^A$$

The values of A_1 ranged from -1.34 to -4.72 for the Taxiway A subgrade and Taxiway B base-subbase materials respectively. Most values, however, were in the range of -2.2 to -4.0.

The influence of the moisture tension term governs the response of the mathematical model to the thaw recovery phase of the soil. All soils experienced an increase in stiffness with increasing ψ level and the absolute value of the exponent A_1 gives a relative indication of how rapidly the stiffness increases with ψ .

Figure 6 shows the effect of moisture tension level on the term k_1 , in eq 1, over the range of 0 to 24 kPa. The curves in Figure 6 were generated from the regression equations and are shown for the k_1 values determined for both stress functions.

As mentioned earlier, and in other work (Cole et al. 1986), the stress function J_2/τ_{oct} proved very effective in representing the stress sensitivity of a number of the test soils. We do not yet have sufficient data to ascertain why certain soils are more favorably represented by this function than by the bulk stress model. Consequently, the stress function that best represents a particular data set is employed in the present work.



Figure 5. Regression analysis results showing resilient modulus versus temperature.

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b. Taxiway B.

Figure 6. Dependence on moisture tension of k_1 , the coefficient of either of two stress functions, J_1 or J_2/τ_{oct} , that characterizes the resilient moduli of thawed and recovering soils.

Figure 7 shows M_r versus the two stress functions for actual test data from the Taxiway A base layer, $\psi = 13.00$ kPa. The stress ratio for all test points is indicated. Each grouping of points in Figure 7a corresponds to tests conducted under a constant confining pressure and increasing deviator stress levels. The bulk stress, of course, increases as the deviator stress increases, but the resulting increase in stress ratio brings about a decrease in resilient modulus. This systematic variation of modulus with stress is reduced to virtually random scatter when the data are plotted using the J_2/τ_{oct} stress function as seen in Figure 7b.

in using the J_2/τ_{oct} stress function is that it has a singularity when $\tau_{oct} = 0$, i.e., in the case of hydrostatic compression. Under most loading circumstances this would present no problem. However, in the case where the lateral stresses are greater than the vertical stress in the unloaded state, there exists a certain level of applied vertical stress that can, in theory, bring the soil to a hydrostatic stress state and thus cause the denominator in the stress function to go to 0. We are continuing work on this aspect of the analysis with the goal of developing a similar stress function without the singularity problem.

The only drawback that we have found to date



a. J₁.



Figure 7. Resilient modulus versus stress functions for several principal stress ratios; actual test data on thawed subgrade from Taxiway B.

Figure 8 shows modulus versus stress function for various levels of moisture tension. The curves were generated by eq 9 and 21, respectively, of Table 2. Note that while the stress function exponents are similar for these two soils, the exponents of the moisture tension level terms differ significantly (3.05 versus -1.5). The fact that the thawed Taxiway A subbase is more sensitive to changes in moisture tension level than the Taxiway A subgrade is evidenced by the wider spacing of the constant moisture tension level curves.

The magnitude of the increase in M_r as a result of natural increases in ψ during thaw recovery varied from a factor of 1.5 to a factor of 3.5 for the Taxiway A subgrade and the Taxiway B base-subbase materials respectively. The dry unit weight, γ_d , varied little through the course of testing. Consequently, a clear dependency of M_r on γ_d does not emerge from these data. Occasionally, as in the case of the thawed Taxiway A base, and the thawed Taxiway B base-subbase, inclusion of a dry unit weight term in the regression analysis improved the correlation coefficient very slightly. The Taxiway B unfrozen subgrade, however, showed a significant improvement in the R^2 value (0.72 to 0.82) by inclusion of the dry unit weight term. Care must be taken in applying the regression equations that contain a γ_d density term. Be-



Figure 8. Resilient modulus versus stress function for various levels of moisture tension in the Taxiway A subbase and subgrade (curves on left from eq 9 of Table 2; curves on right from eq 21 of Table 2).

cause the dry unit weights in the SI system of units are close to unity, they can bring about rather large exponents on this term, and substitution of values outside of the range of the data set may result in unrealistic modulus values.

SUMMARY

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Frozen- and thawed-soil testing methods and analytical techniques developed in other work (Cole et al. 1986) were applied in a study of frost effects on pavement materials from the Albany County Airport. We developed empirical models of the response of the test soils to cyclic loading in the frozen, thawed and recovered states. The models give resilient modulus as a function of temperature (for soils in the frozen state), stress state, soil moisture tension (for unfrozen soils), and in some cases dry unit weight.

The results of this study are in general agreement with our previous work regarding the effects of temperature, stress level and soil moisture tension level on the resilient modulus. Although we measured Poisson's ratio in all tests, it did not appear to vary systematically with the quantities affecting the resilient modulus, and was thus taken as a constant.

One area of this study indicates that the variations in soil stiffness over a freeze-thaw-recovery cycle can be determined using laboratory test techniques. Another area of this study, reported by Johnson et al. (1986b), verifies the present results using a layered elastic analysis to predict the surface deflections of the Albany County Airport test sections.

CONCLUSIONS

From the foregoing test results and analyses, the following conclusions may be drawn.

1. For the test conditions of this study, the resilient modulus, M_r , of the granular soils tested in the thawed state is well represented by a simple nonlinear model of the form

$$M_r = k_1 f(\sigma)^{k_1}$$

where $f(\sigma) = J_1$ or J_2/τ_{oct} $k_1 = f(\psi)$, a function of moisture tension $k_2 = \text{constant.}$

2. The stress function J_2/τ_{oct} was found to adequately reflect the tendency of the granular soils' moduli to increase with increasing confining stress and decrease with increasing principal stress ratio.

3. The increase in stiffness observed subsequent to a freeze-thaw cycle can be expressed through the term k_1 , which increases as the soil desaturates.

4. The temperature dependence of the resilient modulus can be expressed through the unfrozen water content:

$$M_{\rm r} = A_{\rm l} \left(\frac{W_{\rm u}}{W_{\rm ave}} \right)^{A}$$

where A_1, A_2 = constants

 $W_{\rm u}$ = unfrozen water content

 W_{ave} = total gravimetric water content.

5. Poisson's ratio did not vary systematically with stress or moisture tension level and may consequently be taken as a constant.

6. The variations in soil stiffness throughout a freeze-thaw-recovery cycle can be simulated in the laboratory through the use of open system freezing and proper testing methodology.

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APPENDIX A: SOIL MOISTURE TENSION VERSUS WATER CONTENT FOR SEVERAL TEST SOILS



a. Taxiway A base material.



b. Taxiway A subbase material.

c. Taxiway B subgrade.



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Taxiway A

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Base layer, frozen

Confining pressure	Deviator stress	Axial strain	Resilient modulus	Dry density	Moisture content	Temperature
(kpa)	(kra)	XIU	(GPE)	(88/8)	(4)	(0)
69.]	66.9 136.9 206.4 273.7	5.29 7.564 7.143 1.229	23.67 21.39 14.46 11.95	1.893	7.50	-5+0
	486.9 136.9 205.0	0.543 0.155 0.525 0.595	2.96 4.21 2.95	1.895	7.51	- ^ • 2
27.5	273.7 66.9 136.7 40.1 53.5	1+169 1+569 3+755 1+170 1+532	2.54 2.45 2.56 2.36 2.35	1.891	7.5:	-0
69.3	66.9 67.5 138.0 208.0	1.950 0.107 0.257 0.429	2.34 6.31 2.37 4.85	1.932	7.5?	-5. î
	344.J 67.5 138.0 208.5	J.686 C.207 J.529 C.386	5.11 3.26 2.61 2.35	1.932	7.50	-1.0
27.6	54 0 67.5	0.129 0.214	4.18 3.15			
69.3	67.5 138.J 208.5	0.139 C.472	4.85	1.932	7.50	-2.2
27.6	40.5	0.056	7.23			
69.0	67.5 67.5 138.0 208.5 276.2	0.139 0.028 0.139 0.216	4.65 24.10 21.90 15.00 12.78	1.932	7.50	-7.8
	343.5 490.7 68.3 139.8 211.2 279.5	0.272 0.445 0.059 0.158 0.290	11.76 11.03 11.58 8.85 7.28	1.887	7.50	-5.0
27.6	347.9 68.3 27.3 41.0 54.7	0.579 0.056 0.027 0.068 0.108	6.01 12.20 10.12 6.03 5.06	1.887	7.50	-1.0
69.0	68.3 68.3 139.8 211.2 68.3	0.162 0.162 0.676 1.218 0.244	4.22 4.22 2.07 1.73 2.80	1.887	7.50	-2.2
27.6	137.8 27.3 41.0 54.7	0.625 0.041 0.067 0.122 0.190	2.24 6.67 6.12 4.48			
69.0	68.4 1339.8 211.2 279.5 347.9	0.019 0.050 0.125 0.213 0.287	36.03 267.96 16.90 13.12 12.12	1.887	7.50	-7.8
	396.9 67.3 137.8 208.2 275.5	0.500 0.042 0.139 0.292 0.375	9.94 16.04 9.91 10.01 9.43	1.940	7.50	-5.0
	489.6 67.3 137.8 208.2	0.583 0.167 0.501 0.891	Ú • 4 Ŭ 4 • 03 2 • 75 2 • 3 4	1.940	7.50	-2 • 2
27.6	26.9	2.507		1.940	7.50	-0.2
27.6 69.0	40.4 67.3 137.8	3.681 3.969 7.993	0.11 J.17 J.17	1.943	7.50	-0.2

Base layer, thawed

1.11

Confining pressure (kPa)	Deviator stress (kPa)	Radial strain x10	Axial strain x10	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Mg/m)	Moisture content (%)	Moisture tension (kPa)
6.9	5 4 E 3	721	C.7:5 1:5:2	Ú.475 .453	44.4	1.932	7.35	2.0
13.8	10.5 6.8 14.5	1	1.111	415	64.1 59.2			
27.6	26	1.442	3.541		59 3 197 6			
6.7	27.9	1.54	3.249 2.411		23.9	1.575	5.43	6.0
	1.5	0.502	1.512	117	46.1 (=			
13.8	17.2	1.223	2.546	C 367	66.4			
	14.2	.762	1.646	3.317	86. 84.0			
	28.3	1.340	3.443	5.421	81.3 c1.2			
27.6	26.3	0+009 0+694	2.412	1.490	124.2			
69.1	56.6	2.122	5.72	0.071 0.415 0.315				
6.9	73.9	1.229	3.857	0.323 C.445	194.J 136.6	1.988	4.50	13.0
	6.9 13.5	224 2.391	6.570 0.981	0.193	121.2			
18.0	17.3	0.559	1.772	6.402 6.410	97.5			
17+0	14.2	G.391	1.013	0.386	140.1			
	28.4	0.895 1.174	2.426	0.372 0.395	118.0			
27.6	14.2	0.168	0.855	0.196 0.252	166.0			
49 A	42.0 56.8		2.723	0.288	154.1			
6.9	74.0	J.951 0.112	3.167	0.300	233.8	1.993	3-80	24.0
•••	6.9 10.5	C-224 2-336	0.647	C.346 D.336	106.9			
	14.2	0.503	1.353	0.172	105.0			
13+8	14.2	0.392	1.206	0.266	117.6			
	28.4	0.783	2.353	0.335	120.7			
27.6	14.2	0.280	0.732	0.399 C.327	202.3			
	42.0 56.8	J.895 1.175	2.648	G.338 2.322	158.6			
6.9	74.1	1.306	3.237		228 -8	1.007	7 89	2 4
,	6.8 10.3	3.499 9.944	1.112	0.449 0.485	61.3		1.55	
13.8	14.0 .6.8	1.332	2.615	0.509 0.374	53.5			
27 4	20.7	1.221	2.896	6.422				
6.9	28.0	0.944	2.837	3.326	96.6	1.929	5.40	6.0
•••	6.9 10.5	0.392 C.616	C.947 1.527	3.414 C.403	73.1			
	14.2	5.896 1.126	2.1.6	J.425 J.453	67.5			
13.8	14.2	V-560	1.520	2.154	90.ŭ 90.ŭ			
27.0	24.4	1.343	1.1.5	0.411 0.104				
27.5	24.4	C . r. 15 1 . 4 . u	2.171	1+254 1+296	12	1.92-	5+43	6.0
69.1	56.9 34.6	271	4.742		119.H 193.J			
6.9	64.9 1.5	1.12	3.272		198.5	1.425	4.50	13.**
	1.5	5.52	1.372	3.266	76.6			
13.6	17.3		2.217	354	78.1			
	14.2	3 72 5	1.363	0.000	89+8 99+9			
27. ¢	21.4 34.6	1.58	2.537	0 • 34 / 0 • 36 8 7 • 33 6	97.5			
2100	28.4	1.115	2 . 2 7 3	0.196	125.3			
69.0	56.9	1.568	4.214		134.7			
	74.2	1.120	3.434	3.326	216.1			

Confining pressure (kPa)	Deviator stress (kPa)	Radial strain x10	Axial strain x10	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Mg/m ³)	Moisture content (%)	Moisture tension (kPa)
6.9	3.5 7.0 10.6 14.3	U.U14 C.253 0.336	0.219 0.500 0.813 1.156	0 - 28 5 - 311 5 - 291	159.2 139.3 133.0 123.7	1.946	3.00	24.5
13.6	17.4 7.0 14.3 21.1 23.6	0.112 0.337 0.449 0.730	1.530 0.469 1.030 1.530 2.125	0.299 0.239 0.337 0.299 0.344	116.1 148.5 143.J 141.0 134.6			
27.6	34.8 14.3 28.6 42.3 57.2	2.898 3.281 3.505 0.786 1.291	2.688 0.813 1.750 2.563	0.334 0.346 0.289 0.307 0.362	129.6 175.9 163.5 165.0			
69.0	34 .8 7 <u>1</u> .5	0.393	1.438	0.273	242.2	1 847	3 10	2.0
0.7	6.7 10.2	0.550 0.681	1.91	0.504	61.4 55.9	1.940	1.30	2.00
13.8	13.8 6.7 13.8	1.156 C.330 0.771	2.427	0.476 0.418 0.454	56./ 84.9 80.9			
27.6	20.3 13.8 27.5	1.321 0.495 0.990	2.731 1.153 2.733	0.484 0.429 0.362	74.4 119.3 100.6			
6.9	3.3		0.485	0.227	69.0 69.0	1.959	5.40	6.0
	13.8	C.771 0.990	2.243	0.344 0.371	61.3			
27.6	27.5 43.6	1.211	2.422 3.457 3.337	0.350 0.350 0.350	79.5 121.8			
69.0	55.0 33.5	1.706	4.675	0.365	117.6 196.9			
6.9	3.4	C.055 C.165	C.333 Q.694	0.165	101.0	1.974	4.50	13.0
13.8	13.8 16.6 6.7 13.8 20.4	C.441 C.552 S.110 C.496	1.356 1.556 1.834 0.583 1.333 1.889	0 • 294 0 • 294 0 • 301 0 • 189 0 • 248 0 • 263	92.1 91.7 115.4 103.7			
27.6	27.6 33.6 13.8 27.6 40.8	C.772 D.992 J.221 D.496 C.772	2.501 3.058 1.001 1.946 2.891) • 309 0 • 324) • 221 0 • 255 0 • 267	110.5 110.0 138.1 142.0 141.2			
69.0	33.6	0.331 0.772	3.899 1.592 3.339	0.220 0.231	223.8 215.8			
6.9	5.4	C.355 C.110	0.250	0.220	134.6 134.6	1.974	3.60	24.0
5.°	12 13.0 16.5		C.77E 1.56 1.53		131.2 133.9 126.2	1.974	3.03	24.0
13.5	6+7 13+8 23+4	11. 276 346	C • • • • • • • • • • • • • • • • • • •		151.5 155.5 155.2 146.5			
27.n	23+6 137+6 27+6 55+3	C • 221 • 4 46 • 772 1 • 159	2.534 J.778 1.611 2.534 2.54	4 0 0 1 4 0 2 1 6 0 2 1 6 0 2 5 6 0 1 6 0 1 6 0 1	177.6 171.5 175.1 141.7			
69.J	75	27	1+112	1 . 147 	352.5	1.847	7.30	2 . 0
18.	7.5	• c 76	1.154		51.7			
13.6	14.1 21.3	1.450 1.901	2.15	0.218	65.4			
27.6	14.4 25.8	225	1.477 2.457	0.152 0.228 0.271	97.4 97.3 84.1	1.91	5.40	6.7
	7.0	0.225	C.893	2.252	78.5	••••		
13.8	17.5 7.0 14.4 21.3	0.169 0.169 0.394 1.676	2.501 0.774 1.608 2.501	0.315 0.218 0.245 0.275	70.1 90.6 89.6 85.1			
27.6	35.1 14.4 28.8 42.6	1.634	4.293 1.312 2.564 3.580 5.075	0.381 0.171 0.242 0.283	81.7 109.8 112.4 119.0 113.5			
69.0	35.1 75.1	0.394	1.792	0.220 0.275	195.7 193.4			
6.9	3.5 7.6 12.7	0.113 0.226 0.395	U • 363 C • 754 1 • 1 73	0.311 0.300 0.337	97.0 93.4 91.1	1.930	4.70	13.0
13.8	14.5 17.6 7.0 14.5 21.4	C.734 C.734 C.198 C.395 C.565	1.564 2.011 3.671 1.341 1.956 2.571	0+361 0+365 0+295 0+295 0+289	92.5 87.6 104.9 107.8 107.8			
	35.2	1.242	3.186	0.390	110.5			

STATUS.

<u>ben</u>

Confining pressure	Deviator stress	Radial strain	Axial strain	Resilient Poisson's	Resilient modulus	Dry density	Moisture	Moisture tension
(KP&)	(KPA)	X10	XIU	TACIO	(mra)	(mg/m)	(4)	(KFE)
27.6	14.5 28.9 42.8 57.8	C.895 C.846 1.354 1.912	1.118 2.124 3.186 4.195	0.353 0.398 0.425 3.456	129.3 136.2 134.2 137.9			
	72:3	1.354	3.470	0.390	208.4			
69.3	35.2	0.790	1.734	0.456	203.1			
6.9	3.5 7.0 10.7 14.5	L.169 0.282 C.395	C.3C8 C.672 1.007 1.399	0.251 0.282 0.282 0.282	114.3 104.8 106.2 103.4	1.93^	3.80	24.0
13.8	7+0 14+5 21+4 28+9	5.169 5.339 5.528 0.677 5.303	0.56C 1.175 1.791 2.406 2.91C	0.362 0.289 0.284 0.281	125.8 123.1 119.4 120.2			
27.6	14.5 28.9 42.8 57.8	0.282	1.007 2.015 2.798 3.750	0.280 0.260 0.323 0.323	143.6 143.5 152.8 154.3			
69.0	35.2	5.452	1.567	0.288	224.7			

Subbase layer, frozen

Confining pressure (kPa)	Deviator stress (kPa)	Azial strain zlo	Resilient modulus (GPs)	Dry density (Mg/m ²)	Moisture content (1)	Temperature ([°] C)
69.)	136.3 236.0 272.7	0.042 0.083 0.139	32.38 24.82 19.62	2.020	7.50	-5.3
	484 •8 136 •3 272 •7	2.486 0.139 0.361 0.639	9.95 9.81 5.71 4.27	2.023	7.50	-2.2
27.5	339.3 13.3 26.7 43.3	1.113 2.646 4.184	3.49 3.12 3.13 3.13	2.023	7.50	-0.2
69.J	66.6 136.3 275.4 271.d	6.294 11.230 0.250 0.417	1.11 1.12 1.22 5.52	5 • 2 8 8	7.50	-1.5
	537.7 69.5 135.9 255.4	0.584	5.7d 12.40 14.01 14.73	2.094	7.50	-5.i
	271.5 338.2 483.2 65.9 235.4 271.8	0.220 0.250 0.019 0.051 0.0577 0.109	12 • 2 • 13 • 53 13 • 39 34 • 97 26 • 65 26 • 68 24 • 94	2.394	7.50	-8.8
	536-3 493-2 66-4 135-7 235-4 271-8	1.180 1.295 1.108 7.257 1.405	18.79 16.38 6.15 5.26 5.07 4.79	2.398	7.50	~0.5
	338.3 136.1 205.6 272.2	0.811 C.J42 D.J83 0.125	4.17 32.40 24.77 21.78	2.044	7.50	-5.2
	338.7 483.8 69.5 136.1 205.6 272.2	0.181 0.361 0.153 0.333 0.567	16.71 13.40 13.08 8.90 6.17	2.044	7.50	-1.4
	338.7 136.1 205.6 272.2	1.301 0.053 0.105 0.158	3.38 25.68 19.58 17.23	2.044	7.50	-8.8
	483.8 136.1 295.6 272.2 338.7	0.329 0.184 0.316 0.474 0.684	10.13 14.71 7.40 6.51 5.74 4.95	2.044	7.50	-0.5

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Confining	Deviator	Radial	Axial	Resilient	Resilient	Dry	Moisture	Moisture
pressure (kPa)	stress (kPa)	strain x10	strain x10	Poisson's ratio	modulus (MPa)	density (Mg/m ³)	content (Z)	tension (kPa)
6.9	3.3	46	0.215	0.534	36.1	2.094	7.53	2.0
	6 • 6 7 • 9	1+314 2+190	1.961	2.E/ 3.744	33.7			
13.0	5.6	1.766	1.3-5	5.555	46.0			
27.6	13.3	0.996	1.7.1	0.583	17.3	2 454	6.51	6.3
8.7	6.5			0.048	73.6	2 • - 2 •		0.5
	13.2	2+359	1.412	0.636	58.5			
17.6	17.3	1.926	2.649	2.EA9	65.3			
1	13.7	ŭ 752	1 589	2 473	56.1			
	27.3	1.826	3.179	2.574	85.9			
27.6	13.7	1.128	1.119	3.485 3.491	122.1			
6.9	3.4	1127	0.393	0.3.3	96.7	2.024	6.09	13.0
	15.2	6.376	1.59	1.355	96.7			
	13.7	0.805	1.530	0.386 0.402	85.3			
13.8	17.1	0.805	1.941	J.415 J.456	87.9			
1300	13.7	0.430	1.177	0.365	116.1			
	27.3	1.181	2.648	0.446	103.2			
27.6	13.7 27.3	C.322 0.752	0.765 1.883	J+421 0,399	178.6			
	41.0	1.449	2.945	J 492	139-1			
69.0	34.1	c.591	1.472	0.401	231.9			
6.9	68.3	0.161	3.514		133.2	2.035	5.50	24.0
	10.3	0.322	0.857	0.376	119.8 114.1			
	17.i	0.645	1.543	0.418	110.9			
13+8	13.7	0.484	1.200	0.403	114.1			
	23.5	0.645	1.600	0.403	128.3			
27 6	34.2	1.344	2.972	0.452	115-1			
27.0	27.4	0.207	1.772	0.364	154.5			
	41.1 54.8	1.612	2.743	0.392	197.4			
69.0	34.2	0.484	1.258	0.385	272.0			
6.9	3.3	0.327	0.667	Ç.490	42.1	2.044	7.50	2.0
	9.8	1.307	2.243	0.583	43.8			
13.8		0.436	1.273	0.342	51.5			
27.6	19.7	1.960	3.458	3.567	56.9			
2110	26.2	1.525	3.338	2.457	78.6			
6.7	5.5	2.242	1.306	3.185	50.2	2.044	E.JV	0 • V
	9-8 11-1	C.485 0.647	2.1776	0.238	48.2			
13.8	6.6	0.202	0.980	0.206	66.9			
	19.7	0.687	3.186	3.216	61.7			
21.6	26.2	0.566	2.860	0.198	91.7			
6.9	3.5	0.219	0.353	0.339	130.1	2.080	6+03	13.0
	10.6	(.437	1.236	0.354	85.8			
	17.7	0.874	2.119	2.412	83.3			
13.8	21.2	3.819	2.115	3.287	168.6			
27.6	28.2	1.201	3.303	0.400 5.327	94.C 141.U			
	28.2	1.764	2.179	3.351	129.6			
27.0	42+3 56+5	1.474	4.716	0.313	119.7	2.98.	6.10	1201
69.5	32.9	2.546	1.651	J.231	109.5			
6.7	3.5		0.236		149 5	2.075	5.50	24.0
	10.6	C . 3 3 2		5.432	119.7			
	14.1	7.546	1.1592	2+416	119.6			
13.8	6.8	5+109	6.531	1.175	128.5			
	21.2	6.601	1.552	2.364	123.1			
	35.5	-982	2.714		130.0			
27.6	14.1 28.2	1.546	J.944 1.775	C.289 J.J.J	149.5			
	42.3	0.819	2.555	0	159.5			
69.0	32.3	0.437	1.298	2.37	271.9			
6.9	3.3	0.220	0.572	0.385	58.3	2.320	7.50	2.0
	13.1	C.989	2.132	J.458 J.464	55.6 47.5			

Confining pressure (kPa)	Deviator stress (kPa)	Radial strain x10	Axiel strein x10	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Mg/m ³)	Moisture content (%)	Moisture tension (kPa)
1 3 .8	6.7 13.7	C.330 D.879	0.948	0.348 0.424	70.4			
27.6	13.7	0.494	1.482	0.333	92.4			
6.9	10.1 13.7	2.033 0.604 0.879	1.143	0.528	88.6 80.C	2.020	6.50	6.0
13.8	16.7 13.7 20.3 27.4	1.259 0.659 C.989 1.539	2.286 1.315 1.829 2.801	0.529 0.501 0.541 0.549	104.3 110.8 97.9			
27.6	33.4 13.7 27.4	2.133 0.385 0.879 1.539	3.716 0.915 2.173 3.431	0.547 0.421 0.405 0.449	89.8 149.8 126.1 121.6			
69.0	54.8 33.4	2.198 0.550	4.577	0.480 0.343	119.8 208.3 190.9			
6.9	3.4 6.8	0.983	0.342	0.243	99.2 92.1	2.065	6.00	13.0
13.8	10.3 13.9 17.0 6.8 13.9 20.6 27.9	0.388 0.498 0.665 0.111 0.388 0.609 0.887	1.158 1.579 2.108 0.695 1.263 1.895 2.632	0.335 0.315 0.355 0.355 0.355 0.357 0.322 0.337	88.2 88.2 112.2 110.3 108.7 105.9			
27.6	33.9 13.9 27.9 41.2	1.219 0.277 C.664 1.253	3.422 1.000 2.000 3.001	G • 356 0 • 277 0 • 332 0 • 351	99.2 139.3 139.3 137.3			
69.0	55.7 33.9	1.385	4.55	0.342 0.429	137.5			
6.9	72•7 3•4 ۥ8	0.055	0.211	0.251	166.8 143.2	2.069	5.50	24.0
13.8	13.9 13.9 17.0	0.332 C.443 C.111	1.105 1.421 0.447	C • 332 G • 312 J • 248	126.1 119.4 151.8			
	23.6	C.498 C.720	1.526	0.326	135.0 135.8 137.1			
27.6	13.9	0.222	0.342	0.264 0.383	165.4 160.4 156.5			
69.0	55.7 33.9 72.7	1.441 3.498 1.219	3.422 1.369 3.012	0.421 0.364 0.406	162.9 247.8 242.2			

ENTER I

Subgrade layer, thawed

Confining pressure (kPa)	Devistor stress (kPa)	Radial strain x10	Axial str <u>si</u> n x10	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Mg/m ³)	Moisture content (%)	Moisture tension (kPa)
13.6	7.0 14.0 20.8 27.5	0.333 2.499 2.999 1.165	1.556 1.961 3.018 4.150	0.315 0.254 0.331 0.261	66.2 71.3 68.6	1.657	24.60	2.0
27.0	14.0	2.333	1.434 3.018	C • 232	57.6 91.6			
69.3	40.4	(•999 :•333	4.529	0.221	P9.3 149.6			
	72.0	1.831	4.958 7.933	0.251	132.2			
13.8	14.0	1.499	1.360	3.367	102.9	1.650	22.90	6.3
	28.4	1.165	3.401	0.343 0.386	83.6			
27.6	26.4	0.832	2.419	0.344 C.368	117.5			
69.J	56.8 35.0	1.997	6.55C 1.966	6.33C C.25	177.9			
	104.9	2.163	7.945	G.272	132.5			
13.8	14.0	0.499 0.532	1.440	Ú.347 U.343	97.1 85.6	1.650	20.70	11.0
27.6	34.9 28.4	1.497	4.395	0 • 3 • 1 0 • 27 •	79.5 117.1			
	41.5 56.8	1.331	4.168		99.6 93.6			
69.0	69.9 34.9	2.661	1.895	0.263	92.2 184.4 153.7			
	164.8	1.996	6.976	0.286	150.2			
13.8	14.0	0.333	1.062		131.6	1.650	15.90	21.0
	28 • 4 34 • 9	0.832	2.276	0.366 0.329	124.7 115.1			

Confining pressure (kPa)	Deviator stress (kPa)	Radial strain x10	Azial strain x10	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Mg/m ³)	Moisture content (I)	Moisture tension (kPa)
27.6	28.4 41.5 56.8	0.665 0.998 1.331	1.821 3.035 4.552	0 • 365 0 • 329 0 • 292	155.9 136.7 124.7			
69.0	34.9 69.9 104.8	C.333 0.832 1.663	1.821 3.794 6.451	0.183 0.219 0.258	191.9 184.2 162.5			
13.8	139.8 14.0 21.8 21.8 27.3	2.162 C.333 Q.499 C.499 Q.665	8.349 1.062 1.821 2.049 2.429	0.259 0.314 G.274 0.244 3.274	167.4 131.6 119.9 136.6 112.4	1.651	14.83	26.0
27.6	34.9 28.4 41.5 54.6 49.9	0.998 0.499 0.832 1.331	3.036 1.670 3.036 4.327	0.299 0.299 0.274 0.274	115.1 173.0 136.7 126.2			
69.0	69.9 34.9 69.9 124.8	1.497 2.333 J.832 1.331	6.J73 1.67J 3.568 6.J73	0 • 2 • 7 3 • 199 C • 233 0 • 219	115+1 209-2 195-9 172-6			
6.9	139.8	2.162	8.354 0.449 1.446	C.259	167.3 78.0 66.9	1.650	24.23	3.0
13+8	10.5 14.J 21.9 21.9	0.333 2.253 C.333 0.333	1.868 2.617 1.869 2.991 3.765	0.178 C.C96 0.111 2.C99	56.2 53.5 74.9 73.2			
69.0	26.3 35.0 70.0 105.0	C-333 1-333	4.062 2.769 5.612 8.234	0.168 6.059 6.162	54.0 126.4 124.8 127.5			
6.9	10.5	0.167	1.348	0.279 0.247 3.297	123+J 56+4 51+9 46-7	1.650	22.13	9•0
6.7	14.2	0.716	1095		46.7	1.65	22.10	8.0
13.8	7.J 1.J 21.3 2P.4	2-599 2-599 1-232	1.198	1+215 1+215 0+294 0+395	5 8 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5			
27.6	10.3 28.4 41.6 56.9	1.532 1.532	1.472 3.523 4.560 6.741	174 1.244 1.74 0.474 0.496	74+n 74+n 43+4 45+4			
69.3	70.3		6+244 5+244 7+169	0 • 222 • 226	85.J 133.5 133.4			
6.9	3.5 7.0 12.5	2.167	1.450	2.159	77.8 56.7 63.6	1.65	17.50	17.0
13.8	14.0 17.5 7.0 14.0 20.8	0.500 0.666 0.167 0.506 0.833	2.133 2.624 1.350 1.950 2.774	0 • 238 0 • 254 0 • 159 0 • 156 0 • 300	66.7 66.7 66.7 71.8 74.9			
27.6	28.4 35.0 14.0 28.4	1.166 1.332 C.333 G.833	3.749 4.649 1.530 3.149 4.499	0 • 31 1 0 • 28 7 0 • 22 2 0 • 26 5 0 • 25 9	75+9 75+3 93+3 90+3 92+4			
69.0	56.9 72.0 35.0 70.0 165.0	1.832 2.331 0.500 0.999 1.832	5.999 7.274 2.4(0 4.499 6.751	0 - 320 0 - 320 0 - 220 0 - 271	94.8 96.3 145.9 155.6 155.5			
6.9	140.0 140.0 3.5 7.0 10.5		9.373 9.373 0.300 0.825 1.351	0.302 0.284 0.292 0.292 0.296	149.4 149.4 116.7 84.9 77.7	1.657	14.80	26.0
13.8	14.0 17.5 7.0 14.0 20.8	0.500 0.666 0.167 0.333 0.666	1.501 2.401 0.825 1.651 2.401	0 • 278 0 • 277 0 • 202 0 • 202 0 • 277	77+7 72+9 84+9 84+8 84+8			
27.6	28.4 35.0 14.0 28.4 41.6	0.833 1.166 C.333 G.666 G.999	3.376 4.128 1.351 2.052 3.903	0 • 24 7 0 • 282 0 • 246 0 • 256	84.3 84.8 163.6 99.8 106.5			
4 9 0	56.9 70.0 70.0	1.998	5.404 6.755 6.755	0.277 0.296 0.196	105.3 103.7 133.7			
070U	143.0	2.331	8.634 6.588	0.270	162.1	1.657	24.48	2.9
¥ • 7	6.9 10.3 13.7	0.495 0.825 1.154	1.469 2.2.4 2.943	0.337 0.374 0.392	46.8 46.7 46.7		. 7 0₩	683
13.8	6.9 13.7 20.4	0.330 0.659 1.319	1.)30 2.354 3.460	0.320 0.260 J.381	66.7 58.3 58.9			
27.6	27.9 13.7 27.9 41.8 55.7	1.813 J.495 1.153 1.153 1.648 2.765	5.015 1.769 3.686 5.531 7.388	v • 362 0 • 280 0 • 313 0 • 313 0 • 298 0 • 401	55.7 77.6 75.6 75.6 75.4			

Confising pressure (kPs)	Devistor strees (kPa)	Radial str <u>ai</u> n x10	Axial str <u>ai</u> n x10	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Hg/m ³)	Hoisture content (%)	Moisture tension (kPs)
69.0	34.3 68.5 102.8	0.659 1.318 2.470	2.586 5.690 8.527	0 • 255 3 • 232 5 • 295	132.5 120.4 120.8			
6.9	3.4 6.9 10.3 13.7	C.165 D.329 D.824 D.988	5.444 1.257 1.957 2.590	0.262 0.413 0.381	/7+2 54+5 51+5 52+9	1.650	22.90	B. U
13.8	17:7	1.318	3.1.7	0.316	55.3			
13-8	25.3	3.989	2.959	6.334	68.7	1.650	22.90	6.0
	27.9	1.565	4.175	1.155	63.4			
27.6	27.8		3.1.8	265	89.6			
	21-8	1.482	4.212	6.314	2 1 . 7			
	68.5	2.96	8+148	1.364	, 8 2 • 1			
69.0	34.3	1.412	2.019		126.1			
6.9	3.4	165	1.297	6.556	115.4	1+650	18.57	15.0
	1:.3	0.494	1.464	0.333	69.3			
	13.3	2.576	2.677	2.277	63.9			
13.8	6.7	0.165	0.895	0.185	77. 5			
	13.3	Q.412	1.432	3.252	81.4			
	25.7	0.9e8	3.339	3.296	77.2			
27.6	34.3	1.482	1.187	277	115.4			
2	27.8	C.124	2.527	9 • 31 7	107.2			
	55.7	1.647	6.308	3.261	88.3			
	68 • 5	C.988	4.622	0.215	148.9			
67.V	68.5	5.988	4.692	215	148.9			
	102.8	1.647	7.052	0.284	147.6	_		
6.9	6.9	. 100	8.594	0.316	115.4	1.650	15.70	22.9
	16.3	0.412	1.337	0.308	102.5			
	17.1	0.659	1.931	0.341	88.7			
13.8	13.7	0.329	1.337	3.246	102.5			
	20.3 27.8	0.637 0.824	2.971	0.277	33.7			
	34.3	1.153	3.862	0.299	131.7			
21.0	27.8	0.659	2.154	0.326	129.2			
	41+8	0.988	3.862	0.256	108.1			
	68.5	1.976	6.462	0.306	106.0			
69.0	34.3	0.329	4.308	0.229	159.1			
	102.0	1.482	6.092	0.243	168.7			
	137.0	2.470	8.546	0.289	160.3		25.20	1.0
6.9	3.5	0.333	0.635	0.245	58.2 51.7	1.020	23.20	
	10.5	0.503	2.263	0.221	46.5			
	14.0	1.001	3.774	0.265	46.5			
13.8	7.0	0.167	1.132	0.148	62.0 62.1			
	20.8	0.834	3.472	0.240	60.1			
	28.5	1.334	4.908	0.272	58.1 61.9			
27.6	14.6	2.333	1.813	0.184	77.4			
	41.7	1.167	5.062	0.231	82.3			
	57.0	2.000	6+854	3.307	83.8			
69.0	35-1	0.500	2.800	0.179	125.4			
	105.3	2.000	8.326	0.240	126.5			
	140.4	3.000	10.980	0.273	127.9	1.650	23.30	5.0
5 . 7	7.0	0.333	1.363	0.244	51.5	•••••		
	19.5	6.667	3.161	0.21 C	41.4			
	17-5	1.229	3.938	0.254	44.6 84.2			
21.6	28.5	0.667	3.788	Ģ.176	75.3			
	41.7	1.167	5+114	0.285	88.6			
27.j	72.2	2.353	7.367	9.317	95.3	1.65:	23.33	5.0
69.5	35.1	1.167	2.652	6.225	132.4			
	1 3 5 . 5	1.835	1.959	§ 23 ;	132.3			
6.3	143.4	2.667	10.620	3.4231	56.1	1.651	20.70	:1.0
	. Ť. Š	2.167	1.213	0.134	57.9			
	13.2	6.50C	2.730	C.183	18.2			
£ 8. 4	17.5	2.667	3.564	0.167 0.157	49.2			
1300	14.0	Ž 333	2.275	2.146	61.7			
	20.8	1.000	4.550	3.42.	62.7			
27.6	14.Ĵ 28.K	0.333	1.517	0.220	92.6 83.6			
	<u>11.7</u>	ĩ. 20 ý	4.929	0.203	84.6			
	57.3	1.500	8.44/	v • < 3 3	9999			

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Confining pressure (kPa)	Deviator stress (kPa)	Radial strain x10	Axial str <u>ei</u> u x10	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Mg/m ³)	Moisture content (%)	Moisture tension (kPa)
6.9	3.5 7.0 10.5	C.250	0.455 0.910 1.441	C.173	77.1 77.1 73.1 71.2	1.650	15.13	25.0
13.8	17.5 7.0 14.0 20.8 28.5	0.417 C.333 D.500 D.750	2.579 C.759 1.744 2.656 3.634	0.162 0.191 0.188 0.208	68.0 92.6 80.5 78.5 79.1			
27.6	35.1 14.0 28.5 41.7 57.0	1.000 0.250 0.833 1.333	4.552 1.366 3.355 4.173 5.691	0.220 0.183 0.165 0.200 0.234	77.1 102.8 94.0 99.9 100.2			
69.0	35.1 70.2 105.3 140.4	1.000 1.667 2.167	2.504 4.932 6.831 9.490	0.167 0.203 0.244 0.228	140.2 142.3 154.2 147.9			

Taxiway B

Subbase layer, frozen

Confining pressure	Deviator stress	Axial strain	Resilient modulus	Dry density	Moisture content	Temperature
(Era)	(KPR)	XIU	(GFE)	(ng/m)	(*)	(0)
69.3	71.5 139.8 211.3 279.7 348.0	C.551 0.128 0.192 0.256 0.321	14.J1 10.92 11.C1 1J.93 10.84	1.976	5.50	-5.2
	497.2 71.5 139.8 211.3 283.0 348.0	0.346 2.058 0.195 0.186 0.279 0.349	14.37 12.32 13.31 11.36 10.04 9.97	1.976	5.50	-2.0
	497.2 71.5 139.8 211.3 279.7	0.535 0.282 0.693 1.155	9.29 2.53 2.02 1.83	1.976	5.50	-0.3
27.6	-14.3 28.0 41.0 55.9	0.039 0.116 0.193 0.295	3.66 2.41 2.13 1.90			
69.0	68.4 70.3 137.8 208.0 275.0 342.5	0.372 0.048 0.107 0.167 0.202 0.238	1.84 14.65 12.88 12.46 13.61 14.39	2.004	5.50	-5.2
	48.9 70.3 137.6 208.0 275.0	0.321 0.107 0.238 0.333 0.429	1.52 6.57 5.78 6.25 6.41	2.004	5.50	-2.0
	489.4 70.5 137.6 208.0 275.3	0.691 0.163 0.550 1.126 1.627	7.08 4.32 2.50 1.85 1.69	2.004	5.50	-0.3
27.6	27.5 41.6 55.0 67.3	0.081 0.106 0.175 0.036	5.14 5.19 3.85	1.965	5.50	-5.0
•7••	135.7 205.1 271.4 337.8	0.119 0.226 0.286 0.333	11.40 9.08 9.49 10.14	•••••		
	482.5 69.4 135.7 205.1 271.4	0.119 0.214 0.274 0.321	11.57 5.83 6.34 7.49 8.45	1.965	5.50	-2.0
	337.8 482.5 69.4 135.7 205.1	0.381 0.476 0.286 0.548 0.881	8.87 10.14 2.43 2.48 2.33	1.965	5.50	-0.3
27.6	271.4 13.3 27.1 41.0 54.3 69.4	1.311 0.048 0.107 0.215 0.262	2.76 2.54 2.56 2.55 2.55			

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Subbase layer, thawed

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Confining pressure (kPa)	Deviator stress (kPa)	Radial strain x10	Axiel strain x10 ⁻⁴	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Mg/m ³)	Moisture content (%)	Moisture tension (kPa)
0 + ⁹	3.5 7.3	0.393 0.843 1.405	1.571 1.238	2 • 68 3 2 • 68 1 4 • 73 8	61+1 56+4 55+6	2.C7t	5.50	2.0
13.5	14.3 7.1 14.3	2.121 2.786 1.797	2.621 1.259 2.385	2.771	54.6 56.3 65.1			
27.6	21 • 2 14 • 3 28 • 6	2+594 3+574 1+085	3.576 1.574 3.121	0.425 0.543	07.4 02.9 02.3			
6.9	42.3 3.5 12.5	2.357 5.112 5.672	4.297 5.390 1.220	0.049 0.287 0.551 0.576	98+5 88+9 46+3	2.091	5.10	6.0
13.8	17.3 6.9 14.3 21.1 28.5	1.344 C.224 C.728 1.269 1.905	2 • 1 76 0 • 7 - 8 1 • 4 15 2 • 2 • 4 3 • 1 72	0.612 0.316 0.514 0.562 0.601	79+0 98+0 100+7 91+8 89+6			
27.6	34.7 14.3 28.5 42.1	2.688 0.504 1.121 1.680	4.247 1.172 2.344 3.516	0.633 L.430 0.478 C.478	81.7 121.6 121.6 119.8			
69.0	57.0 34.7	2.352 0.448	4.385	6.451 0.237	116.7 221.9			
6.9	74.3	1.233	3.666	0.336	202.8	2.101	4.83	12.0
	10.5 14.3 17.4	0.393 6.617 0.841	1.463	C • 394 C • 422 C • 466	103.0 97.5 96.2			
13.8	6.9 14.3 21.1 28.5	0.140 0.449 0.729 1.065	0.537 1.171 1.805 2.449	0.261 0.383 0.404 0.436	129.4 121.9 116.8 117.0			
27.6	37.7 14.3 28.5 42.2	1.514 0.280 0.673 1.365	3.171 0.878 1.903 2.684	0.319 0.354 0.397	119.0 162.5 150.0 157.2			
69.0	57.1 34.7	1.570	1.318	0.255	263.6			
6.9	3.5	1.009	0.225	0.328	154.7	2.107	4.70	17.0
13.8	10.6 14.3 17.4 7.0 14.3 21.1 28.6	0+168 0-281 0-337 0-168 0-337 0-449	0.750 1.300 0.400 0.850 1.350 1.350	0 • 224 0 • 268 0 • 259 0 • 140 0 • 1498 0 • 250 0 • 249	1340.8 136.1 133.8 174.0 168.1 158.8			
27.6	34.8 14.3 28.6 42.3	0.112 0.281 0.505	2.250 0.700 1.400 2.201	0.160	104.1 204.1 192.0			
69.0	34.8	0.168	0.950	0.177	366.3			
6.9	3.4 6.8 10.3	0.222	0.439 0.927 1.464	0.506 0.537 0.530	77.3 73.2 70.4	2.080	5.50	2.0
13.8	13.9 6.8 13.9 2.6	1.108 0.332 0.610 1.274	2.050 8.732 1.563 2.442	C • 34 U C • 35 C C • 35 D C • 52 2	68.0 92.7 89.2 10.6			
27.6	13.9 27.9 41.2	0.443 1.053 1.995	1.172 2.541 4.157	0.378 0.414 0.480	118.9 109.7 99.1	2 481	E 10	6.0
8.7	6.8 10.3 13.9 16.9	0.498 C.720 C.941	0.619 1.600 1.429 1.762	0.447 0.498 0.504 0.534	109.4 102.8 97.3 96.1	26071	3013	
13.8	6.8 13.9	0.166	0.500	0.332 0.455 0.471	135.5			
13.8	27.8	1.162	2.382	G 488	116.8	2.091	5.10	6.0
27.6	13.9 27.8 41.1	C.532 C.886 1.635	0.056 1.906 3.098	0.287 0.465 0.518	162.1 145.9 132.7			
69.0	33.0	0.609	1.526	0.359	221.6			
6.9	2.J 3.4 6.8 1.J.3 13.9	0.555 0.222 0.554 0.720	C • 275 V • 600 0 • 95V 1 • 350 1 • 700	0.20 0.379 0.349 0.349	123.1 112.8 138.1 103.0	2.597	4-80	12.0
13.8	6.8 13.9 20.5 27.8	0.166 0.388 0.609 0.996	0.50C 1.15C 1.700 2.401	0 • 532 0 • 337 0 • 358 0 • 415	135.4 120.9 120.9			
27.6	13.9 27.8 41.1	C.222 0.554 J.996	0.750	0.296 0.316 0.383	185.3 158.8 157.9			
69.0	33.8	0.388	1.301 3.103	0.296	260.1 233.7			

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Confining pressure (kPa)	Deviator stress (kPa)	Radial strain x10	Axial strain x10	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Mg/m ³)	Moisture content (%)	Moisture tension (kPa)
6.9	3.4 6.8 10.3	0.355 0.166 0.249	0.200 0.500 0.850	0.275 0.332 0.293	169.4 135.5 120.9	2.098	4.70	17.8
13.8	13.9 16.9 6.8 13.9 20.6	G.388 0.554 0.111 C.277 0.471	1.150 1.400 0.425 0.900 1.400	0 • 35 / 0 • 396 0 • 26 1 0 • 368 0 • 336	121.0 121.0 159.4 154.6 146.9			
27.6	27.8 33.9 13.9 27.8	0.720 0.941 0.166 0.443	1.900 2.350 J.700 1.400	0.379 0.400 0.237 0.316 0.378	146.5 144.2 198.7 198.8 200.6			
49.0	55.6	1.107	2.751	0.402	202.3			
6.9	72.6	0.775	2.251	0.344	322.5	2.092	5.59	2.0
8+7		3.960	1.334	0.720	5 Ú .1			
13.8	13.7 6.7 13.7	1.815 J.385 J.824	2.824 0.822 1.797	0.643 0.468 0.459	48.6 81.3 76.4			
27.6	13.7	0.495	1.131	0.438	121.4			
<i>.</i>	40.6	1.870	3.859	0.485	105.2	2.125	5.10	6.0
6+7	6.7	0.276	0.675	3.409	22.2	20125	J. 1 4	
	13.8	0.773	1.500	0.515	92.3			
13.8	16.9	0.221	0.550	0.432	122.6			
	13.8	0.552 0.884	1.751	C.505	116.9			
	27.7	1.215	2.351	6.552	112.3			
27.6	13.8		0.826	0.463	167.7			
	40.9 55.4	1.215	2.613	0.467	157.3			
69.0	33.7 72.3	J.552 1.325	1.302	0.424 G.441	259+9 243-3			
6.9	3.4	0.138	0.244	0.225 0.257	158.7 126.1	2.137	4.80	12.0
	10.3	0.277	3.478	(+315)-335	117.C 118.7			
13.8	16.9	0.553 0.138	1.561	3.254	158.4			
1340	13.9	3.277	1.512	2.271	135.7			
	27.8	1.715	2.49	3.351	135.7	2.137	4-80	12-0
27.6	13.9	2.221	3.732	2.202	189.9			
	41.1	ŭ.775	2.294	5.38	179.1			
69.3	55+6	1.162	1.171	0.350	183./			
6.9	72.5	C.996 C.055	2.686	0.292	174.2	2.146	4.70	17.0
	10.3	J.111 J.166	0.439 0.737	C+253 C+235	154+7 145+8			
	13.5	0.222	0.927	C • 239 0 • 237	150.5 145.0			
13.8	16.8	0.355	0.390	0.141	174.2			
	20.6	6.277	1.317	6.210	156.6			
27 6	34.0	8.554	2.147	2.258	158-2			
21.0	27.9	č:277	1.366	0.203	234.2			
	41.2 55.8	0.776	2.098	0.238	200.6			
69.0	34.0 72.8	0.277	1.171 2.538	0.237	286.8			

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Subgrade layer, frozen

Confining pressure (kPa)	Deviator stress (kPa)	Axial stra <u>in</u> x10	Resilient modulus (GPa)	Dry density (Mg/m ³)	Moisture content (I)	Temperature (⁰ C)
13.8	14.0 20.6 27.4	0.188 0.313 0.563	0.75 C.66 C.51	1.339	29.20	-0.2
27.6	14.0 27.4 41.6	0.250 0.530 0.613	0.55 0.55 0.51 0.49			
69.0	102.6 139.7 174.6	0.170 C.273 0.375	6.04 5.12 4.66	1.339	29.20	-1.P
	205.1 102.6 139.7	0.034	4.30 30.18 16.44	1.339	29.20	-5.1

Confining	onfining Deviator		Resilient	Dry	Noisture	Temperature	
(kPa)	(kPa)	x10	(GPa)	(Mg/m)	(%)	(°c)	
69.0	174.6	0.136	12.84				
	205.2		12:07			• •	
6.9	3.6	0.063	0.33	1.331	29.20	-0.2	
	10.7	0.688	0.21				
13.8	7.2	0.217	0.33				
	21.0	1.032	0.20				
27.6	35.4	1.940	8.18 0.29				
2/00	27.9	1.064	0.26				
69.8	55.9 35.8	2.568	0.22				
••••	71.6	3.451	2.21 7.10	1.331	29.28	-1.8	
	71.0	0.132	5.38				
		8.377 9.627	3.57				
	2 17:2	1.127	3.35			-5.0	
	104.2		17.3	1.331	27.28	-3.4	
	177-3	1.212	į.;;				
4 . 9	277.2	0.357	7.76	1.330	29.28	-8.2	
•••	,13	0.245				•••	
	14.3	0.676	1:31				
13.8	14.3	1.249	0.29				
	20.2	1.996	0.21				
27.6	35.6	1:398	0.25				
	27.8	1.140	0.23				
67.8	35. 6	7:63	1:22				
	11.2 69.9	3.110	6.46	1.330	29.20	-1.8	
	137.2	8.287	1.86				
	205.1	Ø.502	4.09				
	70.2	0.01	39.81 19.45	1.338	29.20	-5.0	
	140.4		13.12				
	206.2		9.68				
69.0	34.7	0.037	9.38 9.37	1.318	29.20	-1.8 -1.3	
•	101.9	0.148	6.89			•••	
	173.4	0.704	3.91 2.89				
	271.0	1.111 0.037	2.44 18.71	1.318	29.20	-4.2	
	101.7 138.5	0.055 0.092	18.49				
	173.1 203.9	0.129	13.42				
	207.4 32.1		1.72	1.318	29.28	-0.2	
	105.0	1.515	0.69				
	176.7	3.029	0.57				
	70.8	0.056	12.65	1.333	29.20	-1.8	
	144:1	111	9.57				
	208.1	0.370	5.62				
	113:3	1:11	32.31	1.333	27.20	-4.2	
	141.5	0.126	15.72				
	it.:	§:112	11:33				
	35.4		2.46	1.333	27.20	~#.2	

Confining pressure (kPa)	Deviator stress (kPa)	Radial strain x10	Aziel strain zlo	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Hg/m ⁻)	Moisture content (%)	Moisture tension (kPa)
6.9	1.6	1.252 1.515	1.760 1.750	0.332	47.1 4(.9	1.638	23.80	1.0
13.8	1 .7	C+326	2.746	2.267	39-1 56-8			
27.6	21.0 14.5	1.514	1.230	0.353 0.224	49.L 76.2			
69.1	26.3	1.09	3.833	0.263	70.0			
6.9	71.4	1.434	6.175 0.714	2.237	115.7 50.8	1.639	16.20	5.0
	7.3 12.9 14.5	C.593	1.588	0.213 0.249 0.290	45.6			
13.8	17.2	1.185	1.192	0.267 1.142	1.6 60.7			
	20.8	1.015	2.345	J.290 J.290 J.299	59.5 53.3			
27.6	36.2	2.537	6.776	0.374	53.4 75.6			
	42.9 56.4	1.777	5.903 7.528	0.301	72.7			
69.0	36.1	0.676	2.875	0.235	125.6			
6.9	3.6 7.2	0.253	G.536 1.112	0.157	67.3	1.638	9.30	12.0
	10.8	C.422 D.675	1.840	0.229 0.275	58.7 58.7			
13.8	7.2	0.168 0.506	0.844	C.199 0.260	85.2 74.0			
	20.7 28.1	0.675	3.068	0.220	67.4 67.8			
27.6	14.4 28.1	Č.253 0.844	1.535	0.165	93.7 87.1			
69.0	42.6 56.1 35.9		4.604 6.450 2.611	0.293 0.313 0.194	92.6 87.8 137.5			
	71.8 103.2	1.349 2.360	5.376	0.251	133.6			
6.9	3.4 6.8 10.9	C.169	0.364 0.889 1.617	0.196	93.6 76.6 67.4	1.638	6.10	17.0
	13.6 17.2	0.424	2.182	0.194	62.4 68.8			
13.8	13.6		1.778	0.191	76.5			
37 6	27.2	0.932	3.558	0.262	76.4			
2100	27.2	6.593 1.316	2.749	0.242	98.9 97.0			
69.0	55.5 34.0	1.524	56662	0.269 0.142 0.224	98.0 142.4			
	106.4	2.031	7.448 9.731	0.273	142.9			
6.9	3.6 7.3 10.9	0.340	0.642	0.530 0.490 0.529	56.8 46.8 42.6	1.637	16.20	5.0
	14.6 17.3	2.038	3.490	0.584	41.8 42.8			
13.8	14.6	0.340 1.319 1.699	2.297	0.308 0.444 0.514	63.5 63.4			
37 4	27.3	2.463	4.596	0.536	59.5 60.0			
27.6	27.3	1.109	3.128	0.380	87.3	1.637	16.23	5.0
69.0	55.7	3.225	6.44A 2.395		85. 151.9			
6.9	104.6	2.036	5.160 7.380 0.469	0.437	141.0 141.7 73.5	1.637	9.30	12.0
	6.9 10.6	2.341 9.597	0.901	1.359 J.359	76.3			
13.8	13.8	1.022	2.731	0.374 0.301	63.9 80.6			
-	13.8	0.597	1.792	0.333	76.3			
27.6	34.4	i.789 Q.426	4.614	0.388	74.5 100.6			
	27.5	G.937 1.704 2.144	2.734	0.343 0.383	100.6			
6.9	3.5	C.086 9.171	0.363		95.6 73.4	1.637	6.10	17.0
	10.6 13.8 17.5	0.428 2.986 2.990	1.591 2.228 2.547	0.269 1.340 1.174	66.8 62.2 68.8			

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Confining pressure (kPa)	Deviator stress (kPa)	Radial strain x10	Axial strain x10	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Mg/m ³)	Moisture content (%)	Noisture tension (kPa)
13.8	6.9 13.8 21.2	0.171 6.598 6.940	0.819 1.729 2.729	0.209 0.346 0.344	84.5 80.0 77.8			
27.6	27.6 34.6 13.8 27.6 41.5	1.367 1.537 0.342 0.854 1.367	3.457 4.367 1.410 2.911 4.186	0 • 395 0 • 352 0 • 243 0 • 293 0 • 327	80.0 79.1 98.0 95.0 99.1			
69.0	55.3 34.6 69.1 106.0	2.049 0.598 1.452 2.734	5.645 2.367 4.736 7.294	0.363 0.253 0.307 0.375	98.0 146.0 145.9 145.3			
6.9		3.928	9.496	0.414	145.4	1.634	23.80	1.0
13.8	14.4 7.2 14.4	1.350 0.337 0.843	2.831 0.832 1.915	0.477 0.405 0.440	50.8 86.4 75.1			
27.6	28.0	2.192	4.508	0.486 D.357	62.2 101.2			
6.9		1.011	2.923 4.853 0.332		96.0 85.5 110.3	1.634	9.30	12.0
	7.3	8.178 9.511	0.830	0.205	88.2 73.5	10004		
13.8	14.6 17.4 7.3 14.6	0.851 0.170 0.510	1.991 2.572 0.788 1.659	0.331 0.216 0.307	/5.5 67.6 92.9 88.2			
27.6	27.5 36.6 14.6	1.192	3.320	0 • 359 0 • 366 0 • 293	82.7 78.7 126.0			
	41.2	1.277	3.988	0.320 0.336	103.3			
69.0	36.6 73.1		2.162	0.236	169.1 151.6			
6.9	3.4 6.8 10.9	0.089	0.490 0.899 1.552	0.182 0.188 0.219	69.7 75.9 70.4	1.634	6.10	17.0
13.8	17.3	0.509	2.533		68.2 83.5			
	13.6 20.9	0.339 0.509	1.634	0.201	83.4 82.5 82.7			
27.6	34.1 13.6	1.017	1.318	239	AJ.2 164.2			
27.6	27.3	C.539 1.17	2.615	0.195 3.259	154.2	1.634	6.10	17.0
69.0	56.6 34.1 68.1	1.272	5.396	0.129	105.2			
_	104.5	1.357	6.547	5.257 5.057	159.6			
6.9	3.8	0.348 1.j42	0.666 1.733	0.521	57.2	1.625	23.80	1-0
13.8	7.6	2.428 6[7 1.648	1.106	J.549 0.613	68.5			
27.6	22.3	3.115	4.762	0.364	46.7 79.5			
69.0	27.6 44.9 37.8	3.305	3.814 6.378 2.711	0.597	70.4			
6.9	75.6	2.249	5.593	0 402 0 256	135.1	1.625	13.50	6.0
	11.4		1.353	0.576 0.585 0.613	56.1 48.0			
13.8	7.6	0.346	1.186	0.292	63.9 63.8			
27.6	21.8 28.3 15.1 28.3	1.991 3.285 C.692 1.555	3.561 5.108 1.700 3.400	0 • 55 9 0 • 64 4 0 • 40 7 0 • 45 7	61.1 55.5 88.8 83.3			
69.0	44.8 58.8 37.6	2.766 4.833 0.863	5.616 7.682 2.561	8.493 0.629 0.337	79.8 76.5 146.9			
	75.3 110.4	2.070	5.637	0.367	133.5			
6.7	11.5	0.522	1.337	0.467 0.390	102.6	1.625	9.30	12.0
13.8	18.2	1.044 0.261	2.362		77.0 121.6			
	15.3	0.610	1.496	0.408 0.442 0.421	102.4 95.2			
27.6	38.2 15.3	2.087	4.419	0.472 0.294	86.5 129.1			
	28.7 45.4	0.870	2.525	0.345 0.424	113.5			
69.0	38.2	0.783	2.055	0.381 0.366	185.8			
6.9	109.8	2.956	7.127	0.415	154.1 153.5	1.625	6.10	17.0
	10.9	0.547	U.968 1.394	U • 358 D • 374				

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<mark>በእንደርብር በርድርብር እንደር</mark>ብር እንደርጉር እንደርጉር እንዲያስ እንደርጉር እንደረግ እንደረግ እንደረግ እንደረግ እንደረግ እንደ እንደ እንደ እንደ እንደ እንደ እንደ እንደ እ

Confining pressure (kPa)	Deviator stress (kPa)	Radial strain x10	Axial strain x10 ^{°°}	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Mg/m ³)	Moisture content (%)	Moisture tension (kPa)
13.8	14.3	0.434	1.240	0.350 0.403	115.1			
27.6	28.5 35.7 14.3 28.5	1.041 1.476 0.347 0.781	2.790 3.565 1.007 2.170	0.373 0.414 0.345 0.360	102.3 100.1 141.7			
69.0	42 •8 54 •7 35 •6	1.215	3.565 4.651 1.860	0.341 0.413 0.280	120.0 117.5 191.7			
6.9	109.2	2.515	7.138	0.352	153.0	1.530	9.30	12.0
	7.7 11.0	C.348 C.522	1.341	0.334 C.317	73.5			
13.8	17.7 7.2 14.3 21.5 28.6	1.J44 C.261 J.609 J.957 1.391	2.776 C.824 1.735 2.689 3.645	0.376 0.317 0.351 3.356 0.382	63.8 87.0 82.6 80.0 78.6			
27.6	55.8 14.3	1.913	4.514	0.424 C.251	79.3			
27.6	28.6 44.1 54.9	2.956	2.778	0.344 0.375	103.1	1.530	9+30	12.0
69.0	35.8	5.509 1.391	2.384	0.292	171.9			
6.9	109.7 3.8 7.6 11.4	2.626 0.173 0.433 0.693	6.954 0.497 1.525 1.988	C.175 D.348 D.327 D.349	157.8 76.3 57.2 57.2	1.607	23.23	1.0
13.8	15.1 7.6 15.1 22.3	1.126 C.173 C.519 1.038	2.936 0.995 2.157 3.322	3.377 3.174 3.241 2.312	50.7 76.1 70.2 67.0			
27.6	29•5 15•1 29•5	1.385 0.346 0.865	4.496 1.495 3.331	0.338 0.231 0.263	65.7 105.9 88.7			
69.0	44.9 37.8	1.817	5.339	0.340 0.276	84.1 151.1			
6.9	7.4	0.343	0.767	0.231 0.447	129.3	1.607	13.50	6.0
_	14.8	0.857 1.028	1.994	0.430 0.432	74.5			
13.8	14.8	J.630 1.28	1.535	0.391 2.419	96.7 88.8			
27.6	37.0	2.054	3+226 4+459 1-230	0.425 0.461 0.279	86.2 83.1			
	27.8	0.856	2.460	0.348	113.0			
69.0	55.5 37.0	2.226 0.513	5.388	0.413 0.256	103.0			
6.9	106.3	2.224	4.314 6.487 0.368	0.318 0.343 0.239	171.6	1 607		
	7.7 11.6	0.263	0.818	0.322 0.401	94.7	1.60/	7.30	12.0
11 0	15.5 18.4	0.700	1.759 2.128	0.398	87.9 86.4			
13+0	15.5	0.525	0.655 1.432 2.291	0.367				
	29.0 38.7	1.312	3.027 4.090	0.433	95.8 94.5			
27.6	15.5 29.9	C.350 9.709	1.146 2.456	0.305	135.0 118.1			
69.0	58.0	2.099	3.079 2.089	0.413	118.7 114.2			
	72.5	1.400	4.261	0.329	170.1			
6.9	3.5	0.171	0.337	0.253	102.9	1.607	6.10	17.0
	13.8	0.342 0.684	1.518	0.225	89.9 91.2			
13.8	6.9 13.8	0.170 0.342	0.675	0.252	102.5			
	21.2	C.684 0.940	2.025	0.348	104.8			
27.6	13.8 26.5	1.281 0.342 0.512		0.290 0.233	97.5 117.0			
	42.6	1.025	3.375	0.304	126.3			
69.3	34.5	0.427	1.941 3.884	0.220	177.8			
	102+2	1.964	8.250	U • 314	169.4			

KKA - KNZZ - XX

No. Crowner

Subgrade layer, unfrozen

Confining pressure (kPa)	Deviator stress (kPs)	Radial strain x10	Axial strain x10	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Mg/m ⁻)	Moisture content (%)	Moisture tension (kPa)
•								
6.9	7.1 19.7 14.3	2.336 3.420 3.673	0.714 1.371 1.643	0.471 5.392 0.410		1.533	16.13	12.0
13.8	17.9	2.641	2.143	0.392	83.3 100.0 87.3			
	27.9 35.7	1.345	3.144	0.428 0.462	88.8			
27.6	14.3 27.9	C.504 1.008	1.572	0.470 0.392				
	55.7 71.3	1.848	5.304	J.369 0.137	111.3			
69.0	35.6	0.672	1.717 3.865	0.391	207.5	1.497	6.80	17.0
0.7	7.0	C.167 G.250	C.739 1.109	0.225	95.2 95.2			
13.8		0.333	1.552	3.251	86.0 118.9			
1000	14 1 20.9	6.334 6.501	1.331 2.219	0.251	105.8			
27.6	28.6 14.1 27.5	0.835		0.269	126.8 119.8			
69.0	41.8 35.2	1.168 0.584	3.111 1.852	0.375 0.315	134.3 190.0			
6.9				0.385 0.408	81.8	1.521	14.80	6.0
	14.3 17.8		2.035		70.1			
13.8	14.3	0.336 D.504 1.008	1.599	0.315 0.385	89.2 80.9			
	27.8	1.512	3.781	0.400	73.7 73.1			
27.6	33.6 14.3 27.8		1.164	0.361	122.5			
(0.0	42.3		4.075	0.371 0.444	103.9 98.1 195.7			
6.9	71.2	1.679	4.225	0.397 0.319	16 5.6 67.7	1.523	26.10	1.0
	7.1	C.504 0.756	1.448	0.348 0.302 0.319	49.2			
13.8	17.8	1.678 0.671	4.083	0.411 0.291	43.5 61.7			
27.6	21.1 27.7		5.689 5.140 1.581	0.364 0.391 0.318	54.0 89.8			
••••	27.7	1.006	3.295	0.305	84.2 80.0			
69.0	35.5		2.309	0.363	153.6			
27.6 13.8	14.0	0.333	1.289	0.258 0.168	108.5	1.469 1.496	12.30 23.80	8.0 3.0
	21.1	1.006	3.858 4.860	0.261	54.6 57.1			
27.6	35.5 14.2 27.7		6+293 1+645 3+504	0.204	56.4 86.3 79.1			
	42.1	1.508	5.293	0.285	79.6 77.4			
69.0 6.9	35.5		2.5//	0.253	133.9	1.496	10.40	12.0
	10.6	C.335 C.502	1.642	0.321	101.6			
13.8	21.6	S+670	2.359	3.283	80+5 88+1	1.496	10.43	12.9
27.6	35.4	1.339	4.183	0.320	84.5 126.3			
	27.6	C.669 1.334 1.673	3.735	0.269	112.4			
69.0	7C.7 35.4	2.509	6.728	0.373	165.1			
6.9	5.6 7.1	1.200	C.3C3 0.6C6	0.277	117.4	1.506	9.30	18.0
13.8	7.1	0.168 0.336	0.682	0.246	104.3 98.8 92.9			
	27.8 35.5	0.923	3.103	0.297	89.5			
27.6	14.2 27.8		1.061 2.426 3.487	0.317 0.277 0.289	114.5			
	55.5		4.859	0.311 0.342	114.3			
69.0 6.9	35.5	J.503 1.090	4.250	0.256	167.1	1.456	10.70	12.0

· ELLIS RELEAS PRESS PRESS BERRE

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	Confining pressure (kPa)	Deviator stress (kPa)	Radial strain x10	Axial strain x10	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Mg/m ³)	Moisture content (I)	Moisture tension (kPa)
									()
	13.8	7.0 13.9 20.6	0.332	0.714 1.715 2.715	0.194	88.0 97.4 81.1 76.0			
	27.6	13.9 27.2 41.3	0.332	1.215 2.788	0.273	114.5 97.4 93.1			
	6.9	59.5 5.5 7.0	0.334	4.873 0.357 1.000	0.334	161.9 142.7 98.6 70.4	1.474	21.30	4.0
	13.8	14.1 17.6 7.0 14.1	C.835 1.168 C.250 O.668	2.572 3.431 0.715 1.787	0.325 0.340 0.350 0.374	54.7 51.3 98.4 78.8			
	27.6	20.9 27.5 35.2 27.5	1.502 2.169 1.501	3.003 4.005 5.295 2.647 4.008	0.333 0.375 0.410 0.378	69.6 68.6 66.4 103.9			
	6.9	54.9 14.2	2.669	5.732	0.466	95.9 53.6	1.489	23.90	3.0
	13.8	7.1		0.736 1.913	0.455 0.438	96.5 74.2			
5 1		21.1 27.7 35.5	1.508 2.010 3.315	3.691 3.927 5.007	0 • 488 0 • 525 0 • 602	68.2 72.4 73.8			
4) 1	27.6	27.7	1.005	2.504	0.431 0.447	110.7			
2	6.9	7.0	0.500	1.172	0.427	60.0 56.8	1.477	26.70	2.0
	13.8	14.1		1.788	0.466	78.6			
5	27.6	27.5 35.2 16.6	1.168 1.501 0.335	2.578 3.870 0.811	0.453 0.388 0.413	156.6 90.9 131.3	1.505	9.33	18-9
	13.8	14.2 17.3 14.2	0.503 0.671 0.335	1.032	0.487 0.414 0.325	137.6 106.8			
	•	21.1 27.7	0.671	1.843	0.364 0.379	114.4			
	27.6	27.7	C.671 1.174	1.919 2.949	0.398	144.6			
	27.6 69.0	73.9 35.5 72.9	2.346	5.618 1.476 3.395	ú.41P C.395 D.370	126.5	1.505	5 . 30	18.3
	6.9	3.5 7.0 12.5	3.166	0-290 0-52 1-047	0.153	122.5	1.476	7.19	19.0
	13.8	14.J 20.8		1.377 2.174	0.242	101.5			
	27.6	34.9 14.0	J.432 Q.166	3.768	9.221 (.127	92.7 107.2			
		28.4 41.5 54.6	C.832 1.165	2.536 4.259 5.675	C.263 C.265 J.233	111.9 102.2 107.6			
	6.9	3.5 7.1 10.6	0.167	0.468 1.č14 1.561	0.165	75.3 69.5 67.8	1.496	15.10	6.0
	13.8	14.1 17.6 7.1	0.501 C.668	2.185 2.731 0.936	0.229	64+5 64+6 75-3			
	••	14.1 20.9 26.1	0.334 9.668	1.795	0.186 0.252	78.6 78.9			
	27.6	35.3	1.337 C.334	4.528	0.295	77.9 95.1			
		28.6	0.501	2.967	0.169	96.6 95.7			
	69.0	70.4 35.2	2.505	7.431 2.190	0.285 0.337 0.153	93.9 94.8 160.8			
	6.9	70.4 3.5 7.1	1.000 0.167 0.335	4.382 0.714 1.393	0.228 0.234 0.240	169.8 49.5 50.8	1.504	26.40	1.8
Ŕ		10.6	0.502	2.143	0.234 0.293	49.5			
Č.	13.8	7.1	0.167	1.108	0.151 0.302	63.8 63.8			
		27.6	1.505	4.575	0.329 0.351	60.3 61.8			
	< 7 • ₩	14.1 27.6	0.334	1.645	0.203	83.9 85.9 85.8			
	6.9	55.1 3.5	1.338	4.721 6.153 0.294	0.283	88+9 89+6 120+5	1.509	10.30	12.0
2		7.1 10.6 14.2	0.168 C.335 0.502	0.662 1.103 1.618	0.254 0.304 0.310	107.1 96.4 87.6			
		17.7	0.670	2.059	0,325	86.1			
					35				
9									
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	DATE OF A	all		A Callada	XaXadada	Sale Carlo	to interio	a fairt	

Confining pressure (kPa)	Deviator stress (kPa)	Radial strain x10 ⁻⁴	Axial str <u>ai</u> n x10	Resilient Poisson's ratio	Resilient modulus (MPa)	Dry density (Ng/m ³)	Moisture content (I)	Moisture tension (kPa)
13.8	7.1 14.2	C.168 0.335	0.588	0.286 0.240	120.5 101.5			
27.6	21.0 27.6 35.4 14.2 27.6 42.0	0.670 0.837 1.172 0.168 0.502 1.094	2.206 2.795 3.678 1.103 2.207 3.384	0.299 0.319 0.152 0.227 0.227 0.297	95+3 98+9 96+2 128+4 125+3 124+2			
	55.3 70.8	1.506	4.857		113.9			
69.0	30.7	1.004	2.208	0.152	171.6		6 0 0	
6.9	14.1	0.417	1.146	0.364	122.9	1.503	0.00	11.0
13.8	14.1 20.9 28.6	C.334 0.501 0.835	1.648	0.311 3.304 0.364	131.2 126.9 124.7			
27.6	35.2 14.1 27.5 41.8	1.002 C.250 C.667 1.302	3.297 0.788 1.792 3.298	0.317 5.372 0.304	178.7 153.4 126.7			
67.0	35.2	1.166	1.5PC 3.c53		222.7 181.4	1.503	£.80	17.0
6.9	7 • 1 1 0 • 6 1 4 • 2 1 7 • 7	0.168 0.135 0.587 0.938	1.571 2.155 2.858	5.164 D.13 5.165 5.292	69•3 67•7 64•5 61•9	1.52	14.89	6.0
13.8	17.7 7.1 14.2 21.1 27.7	C.078 C.078 C.078 L.05	2.868 3.658 1.844 2.731 3.689	0.292 0.189 0.227 0.245 0.272	61.9 79.6 77.5 77.1 75.1			
27.6	35.5 14.2 27.7 42.1 98.8	1.508 0.251 0.670 1.173	4.646 1.296 2.665 4.101 5.607	3.325 3.193 C.251 3.286 3.329	76.3 109.3 104.0 102.7 176.3			
69.0	35.5		2.872	C.175 C.252	123.5			
6.9	7.1 10.6 14.1	0.334 3.669 1.003	1.143 1.714 2.501 3.145	0.292 0.390 0.401 0.425	61.7 61.8 56.5 56.1	1.517	26.20	1.0
13.8	7.1 14.1 20.9 27.6	0.167 0.501 1.003 1.671	0.929 1.930 2.860	0 • 18 0 0 • 26 C 0 • 35 1 0 • 41 7	76.0 73.2 73.3 68.8			
27.6	55.3 14.1 27.5 41.9	2.537 0.417 1.002 1.671	1.288	0.324 0.369 0.389	109.5 101.3 97.5			
6.9	3.5 7.1 10.6	0.335	0.606 1.212 1.895 2.425	0.276	58.3 58.3 55.9 58.3			
	17.6 3.6 7.1 10.7	1.003 0.168 0.336	3.184 0.379 0.909 1.364	0.315 0.185 0.246	55.4 94.2 78.5 78.5	1.542	10.10	12.0
	14.3 17.8	0.504	1.742	0.289	82.0 84.1			
13.8	7.1 14.3 21.2 27.9	0.168 0.336 0.588 0.841	0.758 1.516 2.273 3.031	0 • 22 2 0 • 22 2 0 • 25 9 0 • 27 7	94.2 94.2 93.2 9 <u>2</u> .0			
27.6	35.7 14.3 27.9 42.4	1.177 0.336 0.672 1.008	4.093 1.137 2.426 3.866	0 • 288 0 • 296 0 • 277 0 • 261 0 • 261	87.2 125.6 114.9 109.6			
69.0	71.4 35.7	2.016	6.829	0.295 0.229	104.5			

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