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FRACTURE MECHANICS: APPLICABILITY TO CRACKING AND FRACTURE OF CONCRETE

ABSTRACT This report contains a state-of-the-art summary of past and current research activities in the application of fracture mechanics methodologies to cracking and fracture of concrete as well as contains recommendations of a fracture model to determine the fracture process in concrete. Despite known problems, it is recommended that linear elastic fracture mechanics provides a suitable model. A detailed testing procedure and data evaluation are given using a three-point-loaded cracked beam specimen.

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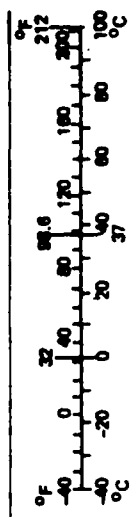
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METRIC CONVERSION FACTORS

When You Know		Multiply by	To Find	Symbol
LENGTH				
inches	feet	0.25	centimeters	cm
feet	yards	30	centimeters	cm
yards	miles	0.9	meters	m
miles		1.6	kilometers	km
AREA				
square inches	square feet	6.5	square centimeters	cm ²
square feet	square yards	0.09	square meters	m ²
square yards	square miles	0.8	square meters	m ²
square miles	acres	2.6	square kilometers	km ²
acres		0.4	hectares	ha
MASS (weight)				
ounces	pounds	28	grams	g
pounds	short tons (2,000 lb)	0.45	kilograms	kg
short tons		0.9	tonnes	t
VOLUME				
teaspoons	fluid ounces	5	milliliters	ml
tablespoons	fluid ounces	15	milliliters	ml
fluid ounces	quarts	30	milliliters	ml
quarts	gallons	0.24	liters	l
pints	gallons	0.47	liters	l
quarts	gallons	0.95	liters	l
gallons	cubic feet	3.8	liters	l
cubic feet	cubic yards	0.03	cubic meters	m ³
cubic yards		0.76	cubic meters	m ³
TEMPERATURE (exact)				
Fahrenheit temperature	Celsius temperature	5/9 (after subtracting 32)	temperature	°C

Approximate Conversions from Metric Measures

When You Know	Multiply by	To Find	Symbol
LENGTH			
millimeters	0.04	inches	in
centimeters	0.4	inches	in
meters	3.3	feet	ft
meters	1.1	yards	yd
kilometers	0.6	miles	mi
AREA			
square centimeters	0.16	square inches	in ²
square meters	1.2	square yards	yd ²
square kilometers	0.4	square miles	mi ²
hectares (10,000 m ²)	2.5	acres	
MASS (weight)			
grams	0.035	ounces	oz
kilograms	2.2	pounds	lb
tonnes (1,000 kg)	1.1	short tons	
VOLUME			
milliliters	0.03	fluid ounces	fl oz
liters	2.1	pints	pt
liters	1.06	quarts	qt
liters	0.26	gallons	gal
cubic meters	35	cubic feet	ft ³
cubic meters	1.3	cubic yards	yd ³
TEMPERATURE (exact)			
Celsius temperature	9/5 (then add 32)	Fahrenheit temperature	°F



*1 in = 2.54 exactly. For other exact conversions and more detailed tables, see NBS Misc. Publ. 286, Units of Weights and Measures, Price \$2.25, SO Catalog No. C13.10.286.

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INTRODUCTION

This report was generated within work unit ZR023.03.03.01.009, Fracture and Fatigue of Concrete, in the Structural Modeling Project, which is part of NAVFAC's 6.1 Basic Research Program Subelement 23, Mechanics, of Program Element 61153N. Application of fracture mechanics to concrete will impact the design of jet engine test cells, pier and pile structures, and other reinforced concrete structures in corrosive environments where current concrete design criteria based on ultimate compressive strength is not adequate. This work unit continues with the development of criteria for crack control in plate elements based on experimental and analytical results gathered in FY86 and FY87. This work unit is planned to be transitioned to 6.2 Exploratory Development in FY88.

This report contains a state-of-the-art summary of past and current activities by researchers in the application of fracture mechanics methodologies to cracking and fracture of plain and reinforced concrete.

The report consists of two major tasks. Task 1 is concerned with a review of available literature. While this is essentially never-ending because of continuing research activity, citations up to June 1985 are included here and updates can be performed readily since the entire file is formatted for use on a microcomputer. The report contains this file with some 780 entries. This is given by a printout and the appropriate data sets on diskettes along with non-proprietary software. Additionally, the available abstracts are printed out and may be made available on magnetic tape.

Task 2 contains recommendations of a fracture mechanics model to determine the fracture process in concrete. Also contained in this section of the report is a brief summary of the developments in fracture mechanics and the necessary adaptations, and problems, associated with applications to concrete. It is recommended that linear elastic fracture mechanics provides a suitable model - despite the known problems - but other models are also described and recommended. A detailed testing procedure and data evaluation are also given. This task has focused primarily on the aspects of testing to obtain valid fracture data because numerous methods of application to analysis and design already exist but are essentially nonfunctional due to lack of a reliable testing method.

Notation

A	One half the cracked area of a plate subjected to tension.
A_1-A_5	Coefficients defining K_I in Eq.5.
A_b	Cross-sectional area of a test beam.
a, a_o, a_e	Actual crack length, initial crack length and effective crack length, resp.
a_1, a_3, a_5	Coefficients describing normal stress σ_{π} in Eq. 2.
B, W, S, L	Width, depth, span, overall length resp. of a beam test specimen.
C	Compliance of test specimen (Fig. 10).
C_A	Roughness coefficient of cracked surface for concrete beams.
CLWL-DCB	Crack-line, wedge-loaded, double cantilever beam specimen (Fig. 14).
CMOD	Crack-mouth-opening displacement (Figs. 5, 9).
COD	Crack-opening-displacement for CLWL-DCB specimen (Fig. 14).
CTOD, $CTOD_c$	Crack-tip-opening-displacement and critical crack-tip-opening displacement, resp. (Figs. 5, 8).
c	Crack extension length from a starter notch.
d_a	Maximum aggregate size.
E, E_c	Modulus of elasticity--general, concrete, resp.
G_I, G_{IC}	Mode I energy release rate and critical value of this resp.
G_f or G_c	Fracture energy per unit area.
J, J_{IC}	J-integral (Eq. 11) and critical value for mode I fracture, resp.
K_I, K_{II}, K_{III}	Opening mode (I), sliding mode, in-plane (II) and tearing mode, out-of-plane (III) stress intensity factors.
K_{IC}, K_{IC}^S	Critical values of opening mode stress intensity factors, also called fracture toughness.

LEFM	Linear elastic fracture mechanics.
LPD = v	Load point displacement.
l_p	Length of process zone (Figs. 2, 4).
M	Applied beam bending moment.
mg	Beam weight.
P	Applied beam load.
δQ	Change in energy required to grow a crack.
R	Crack resistance energy.
r, θ	General coordinates (Figs. 1, 2).
r_p	Size of plastic zone preceding crack tip (Fig. 1).
SEN	Single edge notch.
T_i, u_i	Tension traction and associated displacement on any contour Γ surrounding the crack tip.
$U, \delta U$	Strain energy or fracture energy and change in strain energy associated with an initial crack and its extension, resp.
u	Elongation of infinitely-wide plate subjected to uniaxial tension (Fig. 6)
u_θ	Displacement in θ direction.
v_1, v_2	Displacements measured normal to crack in CLWL-DCB specimen (Fig. 14).
x_1, x_2	Local coordinate system associated with a contour Γ around the crack tip.
$z = 1 - \frac{a}{W}$	Parameter used to determine K_I (Eq. 5).
δ, δ_R	Displacement across crack zone in uniaxial tension specimen and value of this at maximum stress, σ_R (Figs. 3, 4).
δ_o	Final beam displacement of failure.
σ, σ_R	Uniaxial tensile stress and peak value obtained in test, resp.
$\sigma_{ij}, \epsilon_{ij}$	Stress and strain tensor respectively within region of crack tip bounded by contour Γ .

$\sigma_r, \sigma_\theta, \tau_{r\theta}$

Radial and tangential normal stresses and associated shear stress, resp. (Fig. 1).

σ_π

Stress normal to axis $\theta = \pi$ (Fig. 1).

ν

Poisson's Ratio.

Task 1.0 Literature Review

This task involved searching and compiling pertinent references. A description of the sources searched is contained herein, as well as a description of the compiled lists of references and citations and a computerized method to implement these compilations.

1.1.1 Description of Sources

References were obtained using the library's computer search facilities; an already publicized, annotated bibliography on the subject; and various collections of literature including material from the Lund Institute of Technology and the Swedish Detonic Research Foundation.

Use of the library's computer search facilities enabled a general selection of references from 7 different databases: COMPENDEX, TRIS, NTIS, FRIP, ISMEC, EI Meetings and Dissertations Abstracts.

The COMPENDEX database is the machine - readable version of the Engineering Index. This database provides abstracted information from approximately 3500 journals, publications of engineering societies and organizations, papers from conference proceedings, selected government reports and books in 26 different languages for the years 1970 to present.

TRIS is the Transportation Research Information Service database providing all types of transportation research information from the United States Department of Transportation and the Transportation Research Board for the years 1970 to present.

NTIS is the National Technical Information Service database which consists of government sponsored research, development and engineering over the years 1964 to present.

FRIP is the Federal Research in Progress database. Only 3 citations were retrieved - however, this file contains data only for two years after a project is completed.

ISMEC is the Information Service in Mechanical Engineering database covering all aspects of mechanical engineering in 250 journals published throughout the world for the years from 1973 to present.

The EI Meetings database provides coverage of over 2000 published proceedings of conferences, meetings and symposia each year. It has been available since 1982 and is updated with approximately 8000 records each month.

The Dissertation's Abstracts database contains more than 800,000 dissertations dating back to 1861. More than 30,000 dissertation citations and 2500 masters thesis citations are added each year covering most U.S. Universities.

The computer search used several select descriptors to determine a subset of each database which pertains to concrete cracking and fracture.

The publicized, annotated bibliography was the one by S. Mindess entitled "The Cracking and Fracture of Concrete: An Annotated Bibliography 1928-1981" appearing in the volume Fracture Mechanics of Concrete, edited by Folker H. Wittmann, Elsevier Science Publishers B. V., 1983. It contained 405 citations with abstracts for the years prior to 1982.

1.1.2 Description of Literature File Implementation

Having a great quantity of bibliographic information and a need to scan and categorize this information quickly, a decision was made to utilize a database manipulation software available for a microcomputer.

The data manipulation software package selected for this was DBASE III by Ashton-Tate. A number of different packages were investigated.

DBASE III was selected primarily because of the availability in the College of Engineering of a data-manipulation program oriented toward use with a bibliography written with DBASE II. Since DBASE III is an advanced - and faster - version of this it was purchased using funds available from a related project.

This software is a relational, data base management system for 16-bit micro-computers. It is being implemented on the Civil Engineering Department's Zenith Z-150 computer. It has full relational and programming features for interactive use and programmed applications development. Its capabilities include 128 fields, 4000 bytes per record and up to one billion records per file with fast sorting, indexing and searching.

The bibliography program written with DBASE II has been converted and debugged for present use. This program provides help menus for the creation, maintenance and searching of a bibliography data base.

Following is a description of the installation and use of the program.

To install the bibliography program on a hard disk system, copy BIBLI.PRG and PROC.PRG from the diskette to the hard disk; on a two drive system, make sure the diskette with BIBLI.PRG and PROC.PRG along with DBASE.COM is in drive A.

To use the bibliography program - get into DBASE III by typing DBASE while logged on the same drive as DBASE.COM. The prompt symbol for DBASE III is a period. At the prompt, type DO BIBLI. This activates the program contained in files BIBLI.PRG and PROC.PRG. The screen will display the program title, ask for confirmation that the printer is on and request the file name and index files (see p. 70-71 in Appendix I). Type BIBL1 or BIBL2 in response as these are the concrete fracture mechanics bibliography databases on Diskette 1 and Diskette 2. The index files keyed on year and

author are BIBL1.NDX or BIBL2.NDX depending which database file is in use; type BIBL1 or BIBL2. Type AUTH1 or AUTH2 for the author keyed index file and TITLE1 or TITLE2 for the title index. These index files must be on Drive A of a two drive system. The first record of the database will be displayed, along with the main menu of commands as given on p. 72 .

The display contains the Author(s), Title and Reference for each article. If there exists an abstract in the abstract list then there will be a list of keywords taken from the abstract and included on the screen display. The FLAG entry on the screen can contain any 5 character code to facilitate printing the reference or creating a text file of those citations particularly of interest.

At the lower right of the screen is displayed the record number, the status and an entry location labeled OPTION. The STATUS is OK if not deleted. STATUS displays DELETED if the record is marked for deletion.

OPTION is the prompt for a command.

To move within the database type f for forward or b for backward. This enables a one record at a time movement either in ascending or descending order of record numbers.

To print a record type p.

To delete a record type d. This actually marks the record for deletion. To purge the record from the memory two more steps are required: type m for the maintenance menu p. 73 and type the number 1 to verify that the records marked for deletion are not mistaken. As you verify the deletes type d to recall a record from being deleted if so desired. After verification type 2 on the maintenance menu to remove the records marked for deletion from the file permanently. A warning will be displayed which suggests a safer alternative than removing the records as described, p. 74 .

The report option is enabled by typing r. A Report Options menu is displayed which offers 6 more options dealing with the FLAG field (p. 75).

The Maintenance Menu is enabled by typing m on the main menu. It offers 7 options dealing with deletions, new entries or duplicate entries. The "Flag Duplicate Entries" option can be used only if there exists a title index file. The name of the title index file should be typed when prompted.

To quit the bibliography session type q on the main menu. This will return control to DBASE. To return to MSDOS type QUIT at the dot prompt (p. 76).

The locate option on the main menu is perhaps the most useful of the options. Type the letter l on the main menu to enable locating. The screen will display two additional options: locate entry by principal author and search for entry by a field as on p. 77 . The first option enables a quick search for a particular name listed first in the authors field of any record. Type the letter l to use this option and then type in the first five characters of the name when prompted. An index file indexed on the first 5 letters of the Author field must be available on the working drive. The second option, S, provides a means to look for a particular word within any field. When S is chosen the screen displays a choice of fields to search: authors field, keyword field, title field or reference field. After choosing the field to search the screen will prompt for a character string to search for. If the keyword field is chosen the screen will display a message prompting, for a logical operand and three character strings (p. 78-79). If all three strings are not needed hit return when prompted for the second or third string. After a record is located that

contains the desired string a locate menu prompts for continuance of the search, "return to main menu, print the record or set" flag. See pg. 80.

The add option enabled by typing a on the main menu, facilitates a means to add records to the database. See p. 81-82.

The edit option, enabled by typing e, facilitates a means to edit the record displayed on the screen. See p. 83-84.

It should be noted that in both the edit and add modes the complete record including every field is shown along with a help menu. This is not true when the record is viewed with the main menu. One field, CODE, displayed by depressing ^PgDn on the keyboard, is included to facilitate insertion of decimal numbers as record numbers, thereby enabling later additions to the file sequence in the correct order of year and author name. For this reason, the DBASE III record number as shown on the top of the add and edit menus is different from the record number with the Citation and Abstract Listings, and as viewed with the main menu. When adding records care should be taken to determine the correct record number for entry in the field CODE and insertion into the Citation and Abstract Listings sequence.

A complete program listing is included starting on p. 53 of Appendix I

1.1.3 Description of Diskettes and Listings

Three diskettes contain the complete bibliography database, related index files and bibliography programs.

Diskette #1 contains, BIBL1.DBF, the first half of the bibliography database. (Records 1 to 390)

Diskette #2 contains, BIBL2.DBF, the second half of the database.
(Records 391 to 783)

Diskette #3 contains the author, title and citation index files, AUTH1.NDX, etc.; the keywords database, KWDS.DBF; and the bibliography programs BIBLI.PRG and PROC.PRG.

The diskettes have been formatted for 9 sectors per track.

The authors, titles and sources with a related reference number are contained on one printout listing in Appendix II.

The citations are listed in order of the year the paper was published or given with authors listed alphabetically for each year. The reference number is a 4 digit decimal number allowing future additions in the present sequence.

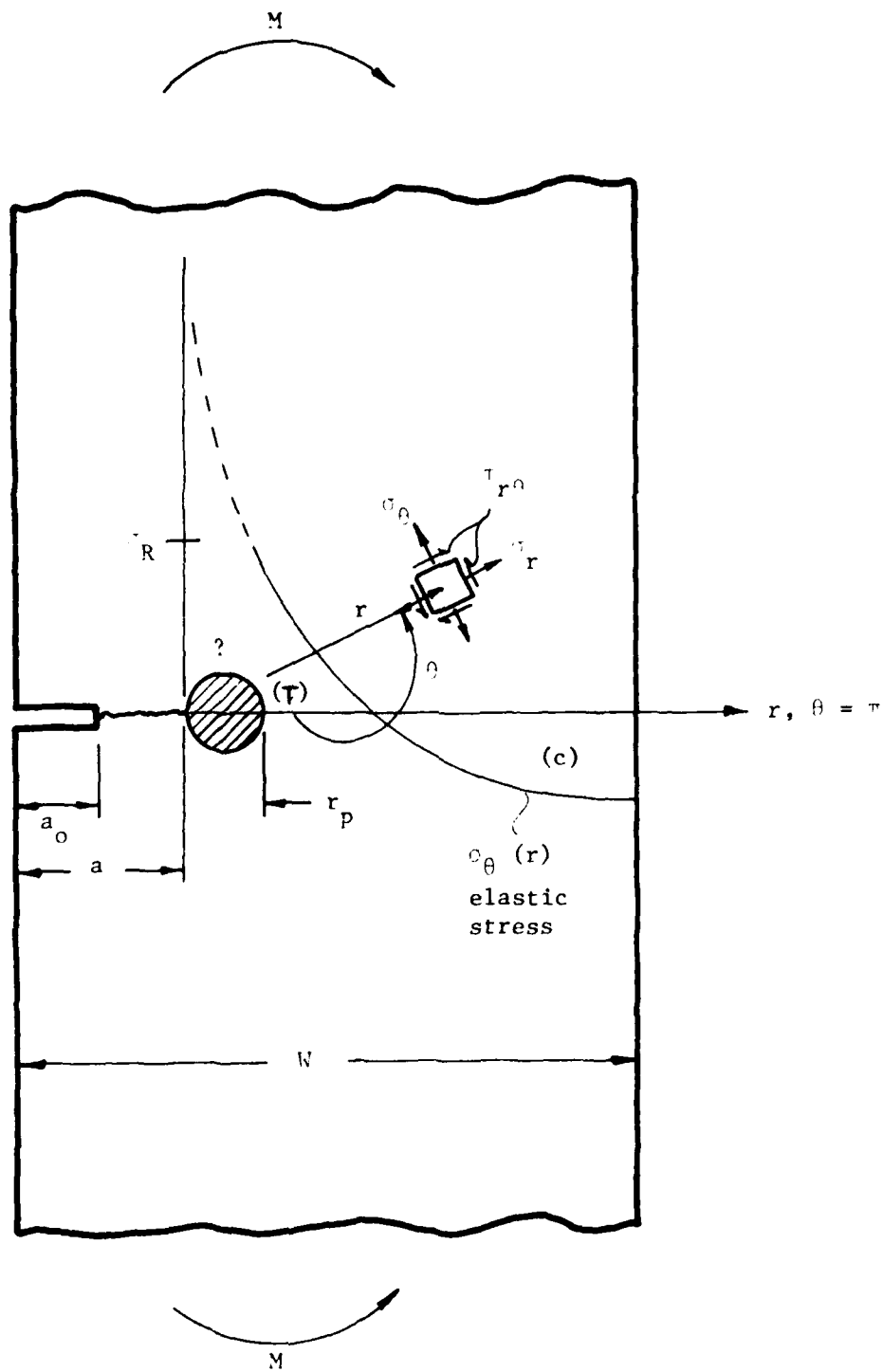


Fig. 1 Elastic Stress Distribution

$$\sigma_{\pi} = -a_1 r^{-1/2} + 3a_3 r^{1/2} - 5a_5 r^{3/2} + \dots \quad (2)$$

where a_1 , a_3 , a_5 are coefficients which may be evaluated by finite element (587) or collocation (670) techniques.

From the definition of the opening-mode, stress intensity factor (124)

$$K_I = \lim_{r \rightarrow 0} \sigma_{\theta} (r, \theta = \pi) \cdot \sqrt{2\pi r} \quad (3)$$

it is seen that the first coefficient in the series, a_1 , is related to K_I as

$$a_1 = - \frac{K_I}{\sqrt{2\pi}} \quad (4)$$

Thus, near the region of the crack tip, as $r \rightarrow 0$, the stress distribution is proportional to K_I , while away from the crack tip the stress distribution is still given by Eq. 1 or Eq. 2 after evaluation of the remaining constants.

Obviously, $\sigma_{\pi} \rightarrow \infty$ as $r \rightarrow 0$ and the elastic stress distribution is no longer correct. For metals, a zone of plasticity, denoted by r_p , is considered to precede the actual crack tip (655).

For concrete, the current thinking is that the crack tip is composed of an aggregate interlock region and a zone of microcracking which together are frequently called the process zone, ℓ_p (21, 86, 198, 359, 407, 408,

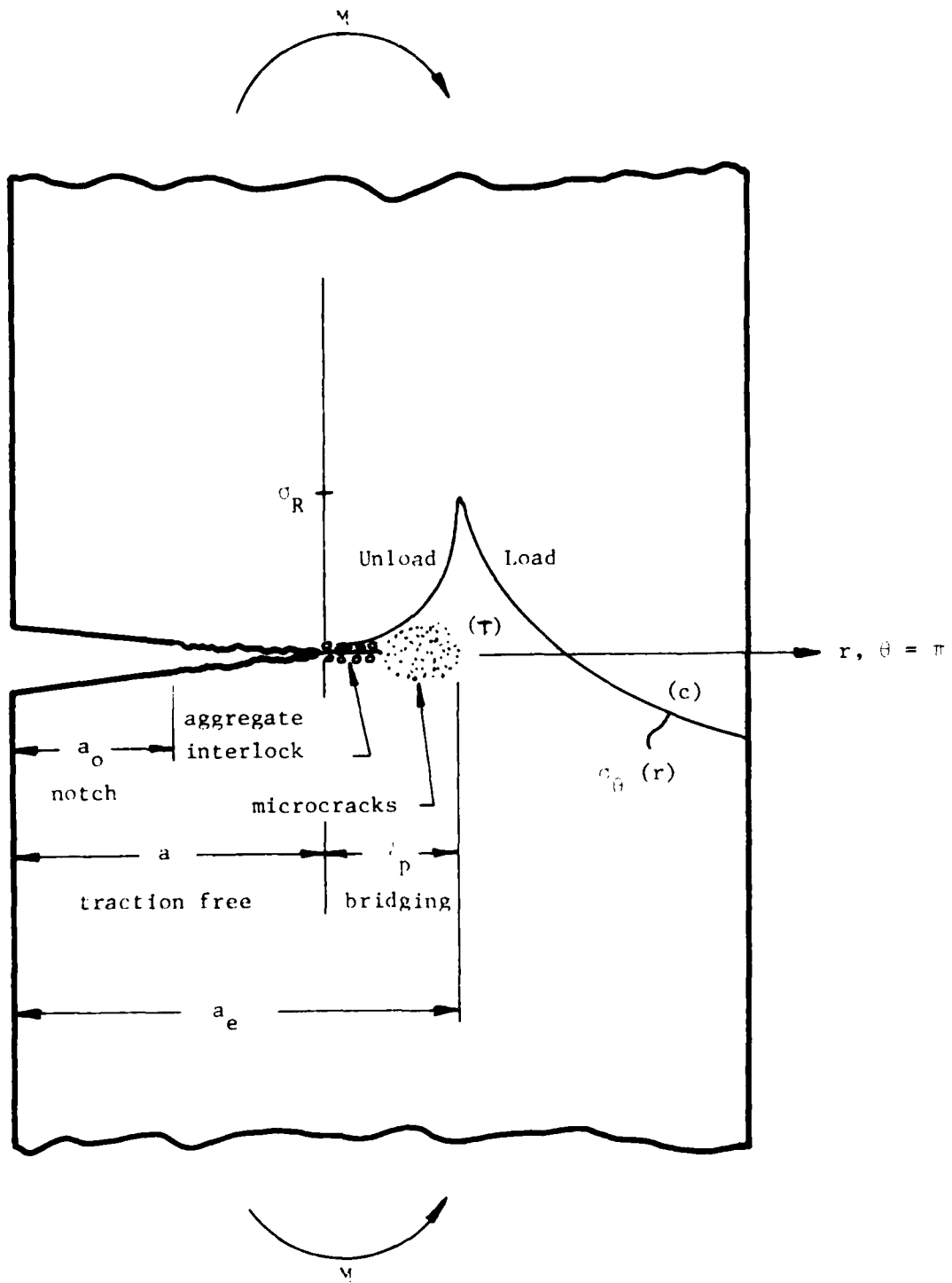


Fig. 2 Influence of Process Zone on Stress Distribution, Cementitious Material

467, 470, 550, 627, 629, 630, 631, 657, 659, 664, 667, 691, 694, 697, 705, 724, 736, 737, 747, 756, 757, 761, 762, 766, 771).

As described in Fig. 2, the elastic stress distribution is valid up to some stress level σ_R which corresponds to the beginning of the zone of microcracking.

As shown in Fig. 3, which corresponds generically to test results reported (470, 672, 761, 761.1) for direct tension of plain concrete, the stress σ_R is attained at a deformation denoted by δ_R and then the material unloads as σ decreases with increasing δ . The unloading response, which is called "strain-softening", is related to the type of test control - either load or deformation - and in the former case and sometimes the latter case is also related to machine stiffness vis-a-vis specimen stiffness.

It is seen then that the shape of the unload diagram given in Fig. 2 which occurs over the bridging length l_p , or process zone, is related to the strain-softening response of the material.

As shown in Fig. 4, the length of the process zone is related not only to the strain-softening response of the material but also to the rate of change of deformation, $\dot{\delta}$, within this zone. Thus a short process zone implies a more rapid change in δ as compared to a long process zone.

Furthermore, the length of the process zone is directly related to the fundamental question - is LEFM a suitable method?

2.1.0.2 Fundamental LEFM Relations

For linear elastic fracture mechanics the development usually presented starts with the work of Griffith (655) which describes an infinitely-wide plate with a central flaw (crack, notch) subjected to a nominal state of stress of uniaxial tension.

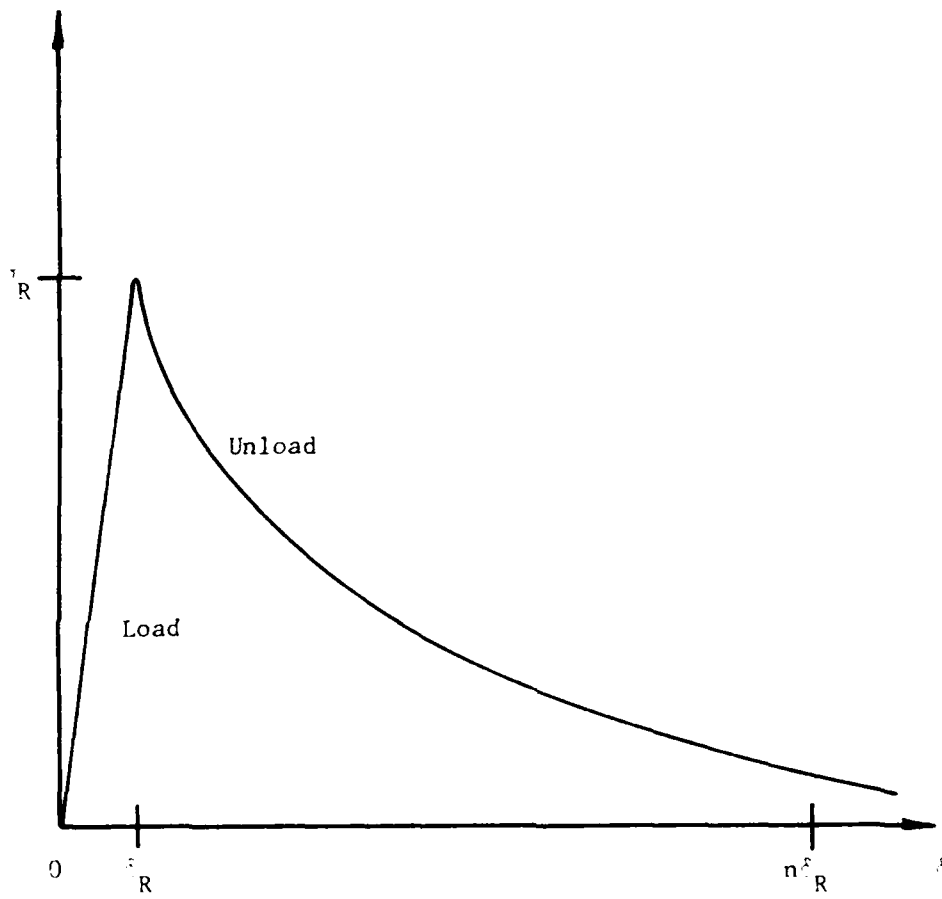
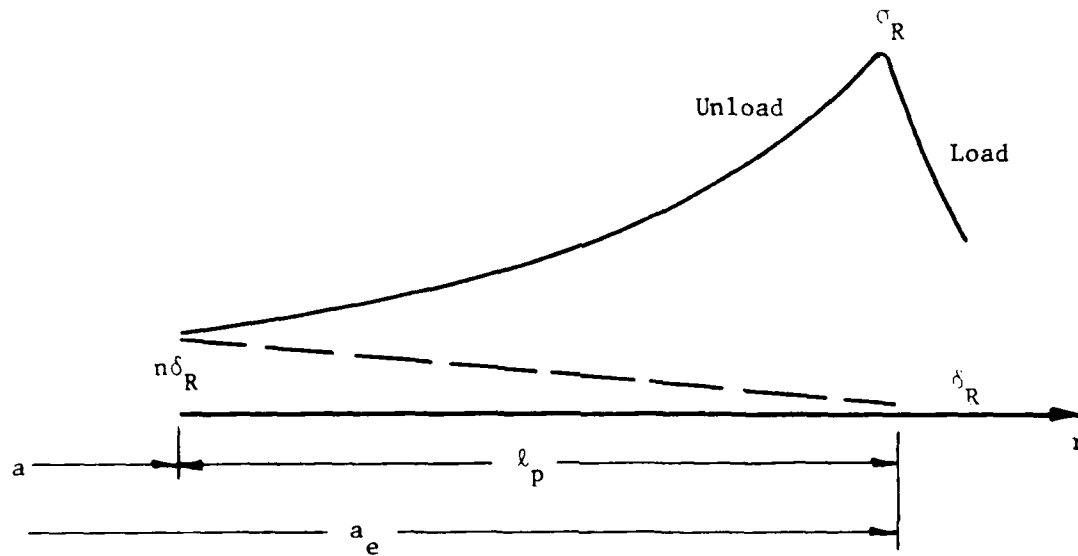
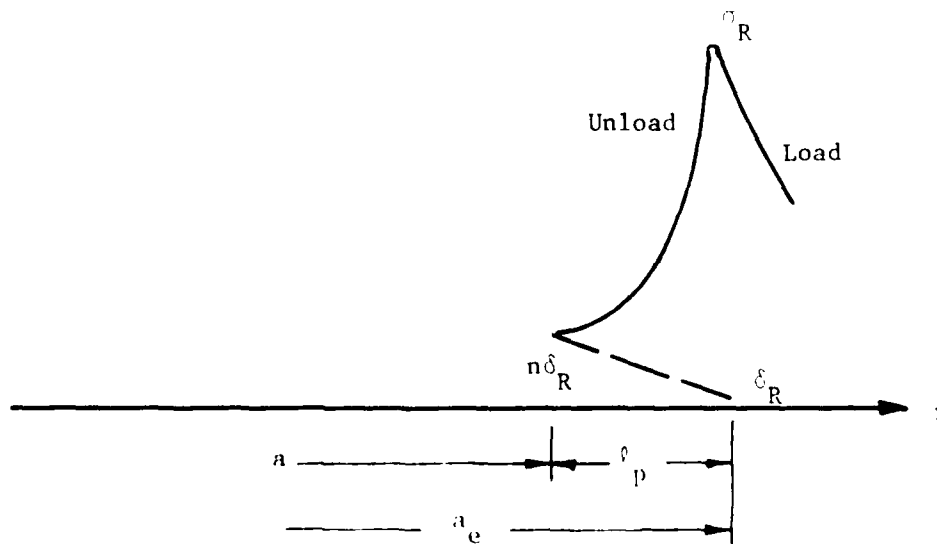


Fig. 3 Cementitious Material Tensile Stress-Strain Response



a. Long Process Zone



b. Short Process Zone

Fig. 4 Possible Unloading Cases For Different Length Process Zones for a Cementitious Material

However, here it is convenient to present the fundamental relationships for three-point, beam bending specimens with a single-edge-notch (SEN). Historically, this type of specimen was found to be attractive to those working in metals because of their familiarity with the Charpy Impact specimen. In concrete testing, the short span, modulus of rupture specimen is popular although this is a beam subjected to four-point bending.

Throughout this report, specific relationships for SEN beam specimens in three-point bending as shown in Fig. 5 will be given. Reference to equivalent expressions for other types of test specimens will be made via the Bibliography.

2.1.0.2.1 Stress Intensity Factors

The opening mode stress intensity factor K_I as defined in Eq. 3 is associated with a displacement which is parallel to the beam axis and near the crack tip. Specifically, with reference to Fig. 1 it is denoted u_0 ($r, \theta = \pi$). Displacements which are measured are the crack mouth opening displacement (CMOD) at the edge of the beam and across the notch (Fig. 5) and crack tip opening displacement (CTOD) at the crack tip.

It is noted that other stress intensity factors exist for shearing mode deformations. These are denoted K_{II} (sliding mode-in plane) and K_{III} (tearing mode out of plane) (cf. References 124, 655).

Expressions are available to evaluate K_I for the specimen geometry of Fig. 5 and with central load P . These are of the form

$$K_I = \frac{M}{BW^{1.5}} (A_1 Z^2 + A_2 Z + A_3 + A_4 Z^{-1} + A_5 Z^{-2}) \quad (5)$$

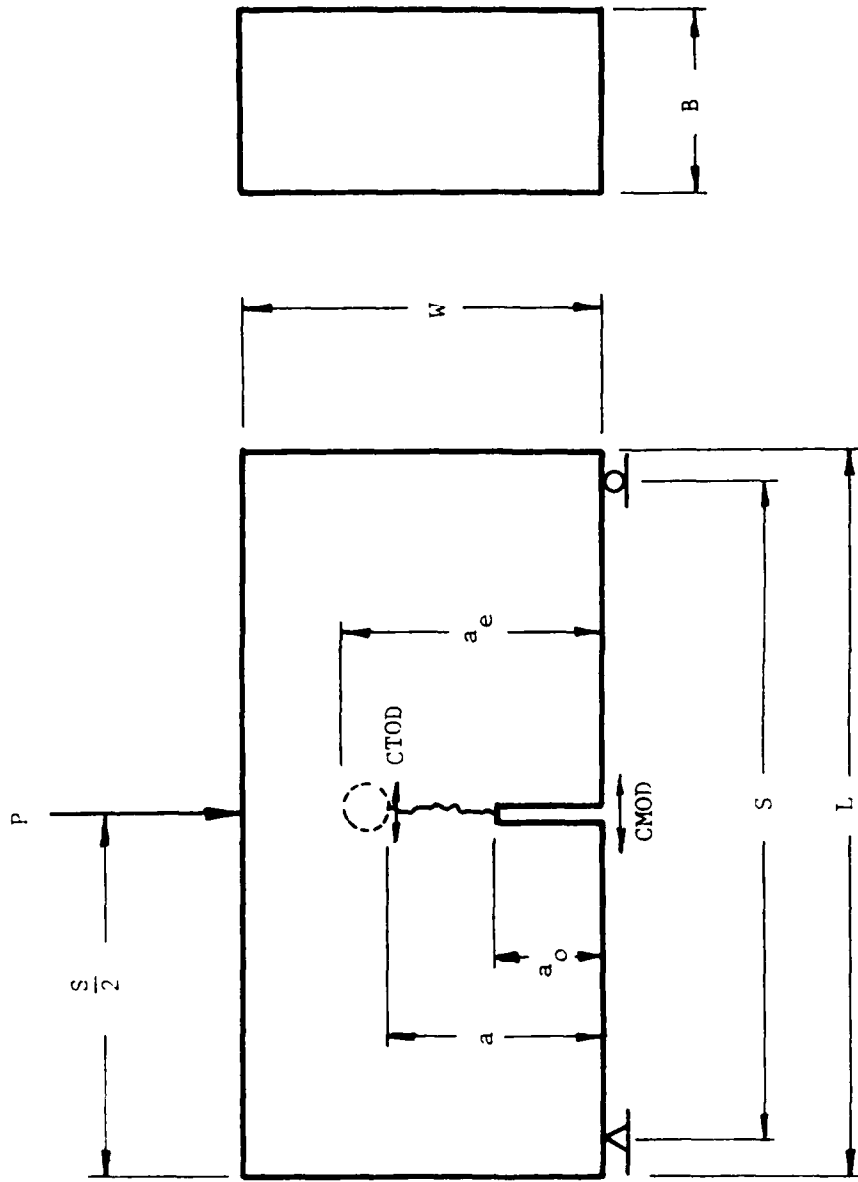


Fig. 5 SEN Beam Geometry

where $Z = 1 - \frac{a}{W}$, $A_1 - A_5$ depend on specimen proportions, and $M = 1/4 PS$. Values of these coefficients for various ratios of $\frac{S}{W}$ are given in Reference 670 and were obtained by solving Eq. 1 using a collocation technique.

A different form of Eq. 5, given by Srawley (655) for $\frac{S}{W} = 4$ and used in ASTM E 399 (558) is

$$K_I = \frac{PS}{BW^{3/2}} [2.9 \left(\frac{a}{W}\right)^{1/2} - 4.6 \left(\frac{a}{W}\right)^{3/2} + 21.8 \left(\frac{a}{W}\right)^{5/2} - 37.6 \left(\frac{a}{W}\right)^{7/2} + 38.7 \left(\frac{a}{W}\right)^{9/2}] \quad (6)$$

In application, the stress intensity factor defines the stress field outside of the zone of plasticity and is a measure of the stresses and strains. Crack extension will occur when these stresses and strains at the crack tip reach a critical value. In other words, fracture will be expected to occur when K_I reaches some critical value, called K_{IC} . This quantity is called the material fracture toughness and is considered to be a material parameter.

If this is the case, the implication is that for crack lengths greater than the critical length frequently considered to be the length associated with unstable crack growth - the value of $K_I = K_{IC}$ should be invariant with length. Furthermore, the value of K_{IC} obtained from a certain size of test specimen should agree with that obtained from a different size if the relative proportions of dimensions are equal.

For LEFM the procedure to find K_{IC} is to determine K_I for different values of (P, a) until invariance is established. In practice, ASTM E399 (558) describes a testing procedure using a plot of P versus displacement

(usually CMOD) from which an estimate of P at critical crack length can be made.

This procedure has been used by a number of investigators with concrete specimens with essentially no success (35, 54, 78, 81, 86, 99, 111, 214, 230, 278). Because of this, the general conclusion has been reached (not yet shared by this writer) that LEFM is not a valid method to be used to characterize cracking and fracture of concrete. However, the vast majority of these investigators have failed to satisfy one of the requirements necessary for valid results using this approach, to wit., the generation of a true, sharp crack prior to obtaining fracture data. Some other errors include use of specimen geometry which does not satisfy that imposed by Eq. (6), i.e., $\frac{S}{W} = 4$; failure to measure the crack length properly - or at all; failure to consider the difference between slow and rapid crack growth.

As will be reported later herein, in the last few years all of these effects are being considered, with consequently an improvement in consistency of results and some revival in interest in LEFM (by those who had abandoned it).

Other types of test specimens include: double-cantilever (325, 548, 614, 627) direct tension (21, 35, 78, 81, 86, 672, 761, 761.1); crack line, wedge-loaded double-cantilever beam (CLWL) (697, 756) and double-torsion (143.1, 300.1, 407, 550). Expressions for K_I for all these geometries, similar to Eqs. 5 and 6 are given.

2.1.0.2.2 Energy-Release-Rate

The Griffith criterion as described in Reference 655 may be established by considering Fig. 6 in which an infinitely-wide plate

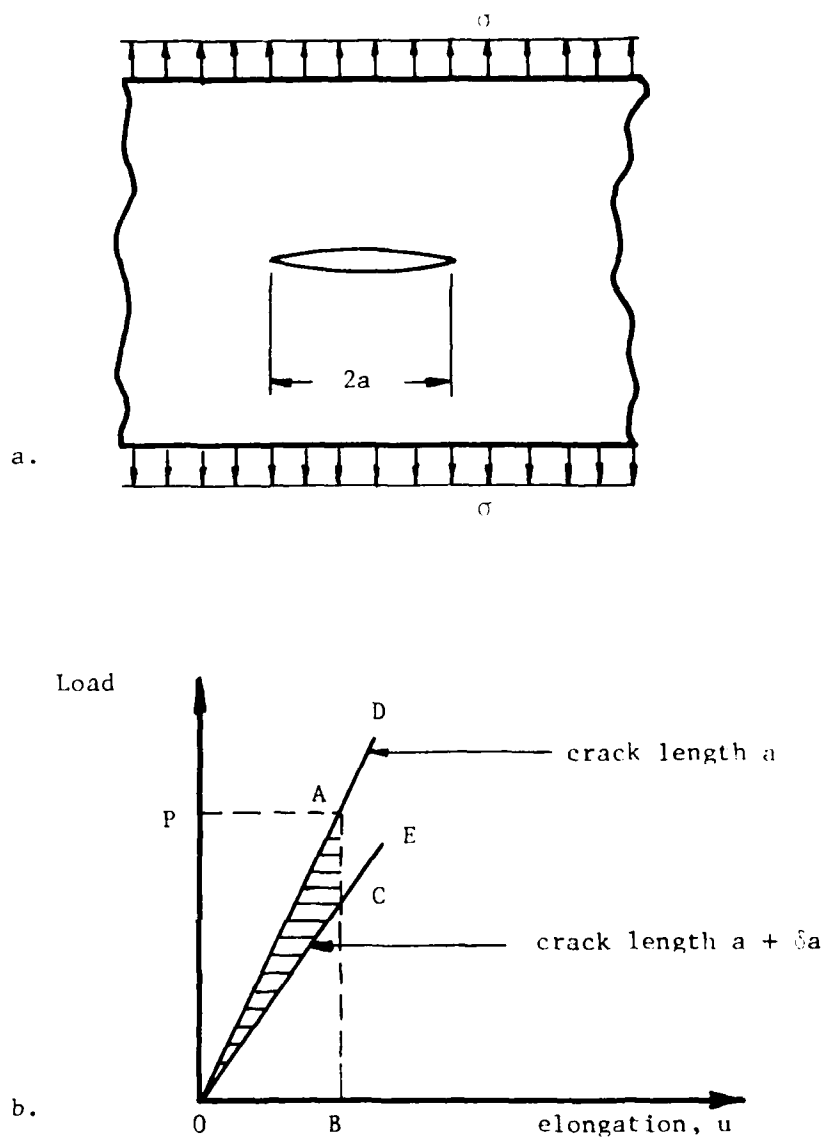


Fig. 6 Griffith Criterion for Fixed Edges
 a. Plate with flaw or notch $2a$
 b. Elastic energy

containing a flaw of length $2a$ is shown along with its elastic load-deformation response.

The elastic energy, U_A = area O A B up to a given load level for the body with cracked area = a B. The elastic energy up to the same level of deformation for a crack length = $a + \delta a$ is $U_{A+\delta A}$ = area O C B.

Then the change in strain energy corresponding to the change in crack length, δa , is

$$\delta U = U_A - U_{A+\delta A}$$

and is represented by the shaded area in the figure. Since the boundaries are fixed δU represents a release in elastic energy corresponding to the increase in crack length, δa .

If δU exceeds the energy, δQ , required to grow the crack, then the crack will propagate. The fracture criterion is

$$\frac{dU}{dA} > \frac{dQ}{dA}$$

or, since

$$A = aW \tag{7}$$

$$\frac{dU}{da} > \frac{dQ}{da}$$

(8)

Define

$$G_I = \frac{dU}{da}$$

called the mode I energy release rate (per unit area of crack extension) and define

$$R = \frac{dQ}{da} \tag{9}$$

the energy consumed per unit area during crack propagation and called the crack resistance. The critical value of G_I is $G_{IC} = R$.

For an elliptic crack of length $2a$, it is found that

$$G_I = \frac{1-\nu^2}{E} K_I^2 \quad (10)$$

for plane strain.

This relationship is considered to be valid for other specimen geometries and loads (655). Thus if K_{IC} is known, G_{IC} is also known and vice versa.

This is of great practical importance from an experimental viewpoint as it permits an independent method for determining the material fracture parameter.

In practice, G_I can be determined directly from load-deformation diagrams provided the crack length - deformation relationship is known.

2.1.0.2.3 J-Integral Method

The methods described above are based entirely on the assumption of linear material behavior. For concrete, this implies two requirements:

1. linear material response, 2. complete absence of a process zone.

Neither of these assumptions is satisfied completely. In the case of the material assumption, the non-linearities are, in fact, very small for the stress levels considered. For uniaxial tension, the material may be considered linear up to the point of maximum stress. For uniaxial compression, the material is also almost linear up to about 1/2 its compressive strength, f'_c , which is a stress level much higher than that associated with crack propagation.

The second consideration - the process zone - is currently the focus of much research activity. If the process zone is small, the following method should be applicable (655).

The J-Integral concept was developed to try to overcome the problem of plasticity present at the crack tip in metal specimens. As presented by Rice (45, 655), the following integral taken around any contour Γ which includes the crack tip is an invariant:

$$J = \int_{\Gamma} \left\{ \left[\int_0^{\epsilon} \sigma_{ij} d\epsilon_{ij} \right] dx_1 - T_i \frac{\partial u_i}{\partial x_2} ds \right\} \quad (11)$$

in which (x_1, x_2) is a coordinate system with x_1 in the direction of crack propagation, σ_{ij} is the stress tensor, ϵ_{ij} is the strain tensor, T_i is the tension traction on Γ and u_i is the displacement in the T_i direction.

It is shown by Rice that for a linear-elastic material $J = G_I$. Thus, it has been postulated that crack growth, or fracture occurs if J exceeds a critical value J_{IC} which is analogous to G_{IC} . If the process zone is small, then even for the elastic-plastic case $J_{IC} \approx G_{IC}$.

Thus, another measure of the fracture energy release rate can be obtained independently and the J-Integral approach also will allow consideration of the influence of plasticity.

The method of implementation of this approach may be considered first by examining Fig. 7. In this it is seen that J may be essentially the same value if load control is used or displacement control is used, even though the response is non-linear. This is the basis for a test method (418) in which one uses load-displacement curves for different crack lengths. The area between the curves for two cracks of slightly different size is determined. This is considered to be approximately $J B da$. Values of J obtained this way can be plotted versus a to determine J_{IC} .

An alternative procedure for concrete was suggested by Go (578) in which the fracture energy U is measured for specimens with different crack lengths and plotted versus a . The slope of the best straight-line

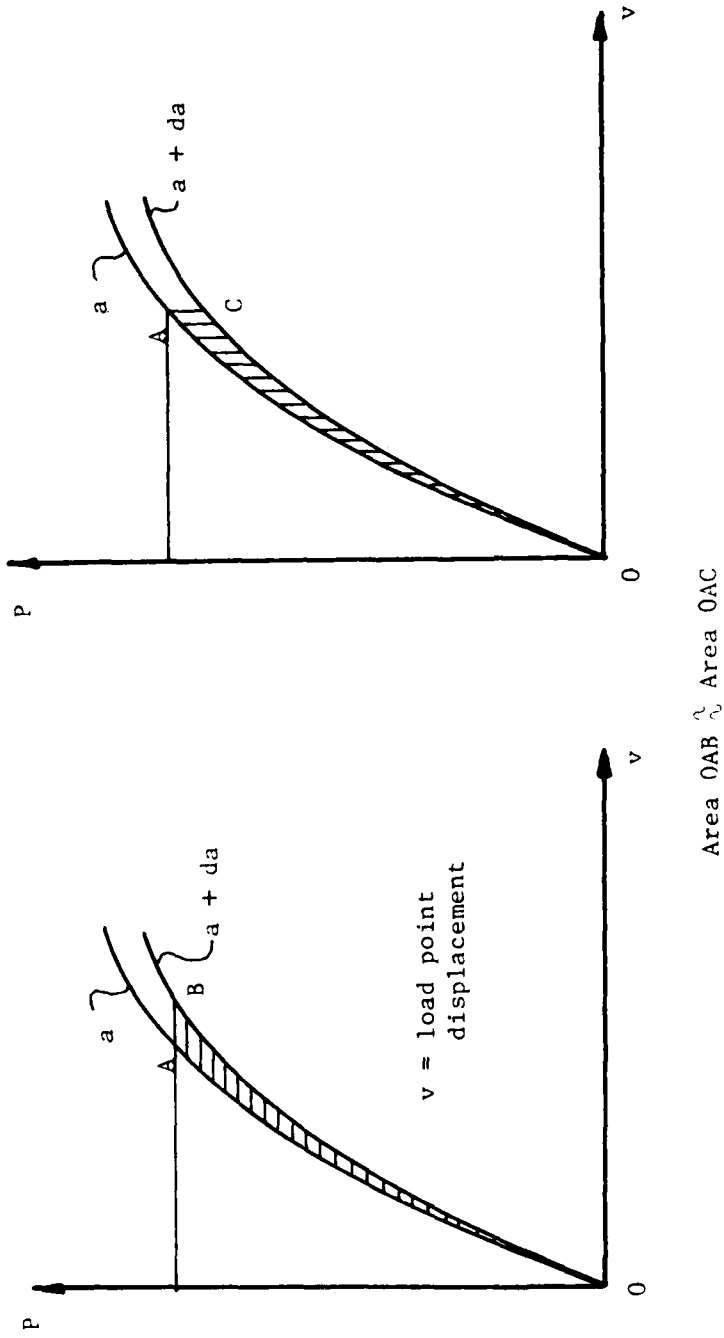


Fig. 7 Energy Change With da Using Load Control or Displacement Control

fit of these points is

$$J_{IC} = - \frac{dU}{da} \frac{1}{B} \quad (12)$$

and this value is compared to G_{IC} .

2.1.0.2.4 Crack Tip Opening Displacement

As shown in Fig. 8, a parameter considered to be of possible usefulness is the crack tip opening displacement (CTOD) associated with the end of the traction-free crack (length a). It is felt that this is a constant for a propagating crack and as such is a material constant. For LEFM it can be related to K_{IC} and G_{IC} but its primary value would be in materials with high plasticity or a large process zone. In this case it still would be possible to obtain a fracture parameter which is representative of the material and which can be used in design.

Proposed expressions to obtain the CTOD for concrete are given in References 671, 765 for beams and double-cantilever specimens and in Reference 417 for CLWL specimens.

2.1.0.3 Summary

By way of a brief summary of the concepts described above, the following considerations are presented as necessary for the validity of LEFM to concrete.

1. A "process zone", if it exists, must be relatively small compared to specimen dimensions and crack length. It may be a function of the size of aggregate - perhaps up to three times this size.
2. A critical value of K_I called K_{IC} or fracture toughness exists. This is associated with the onset or continuation of unstable crack growth.

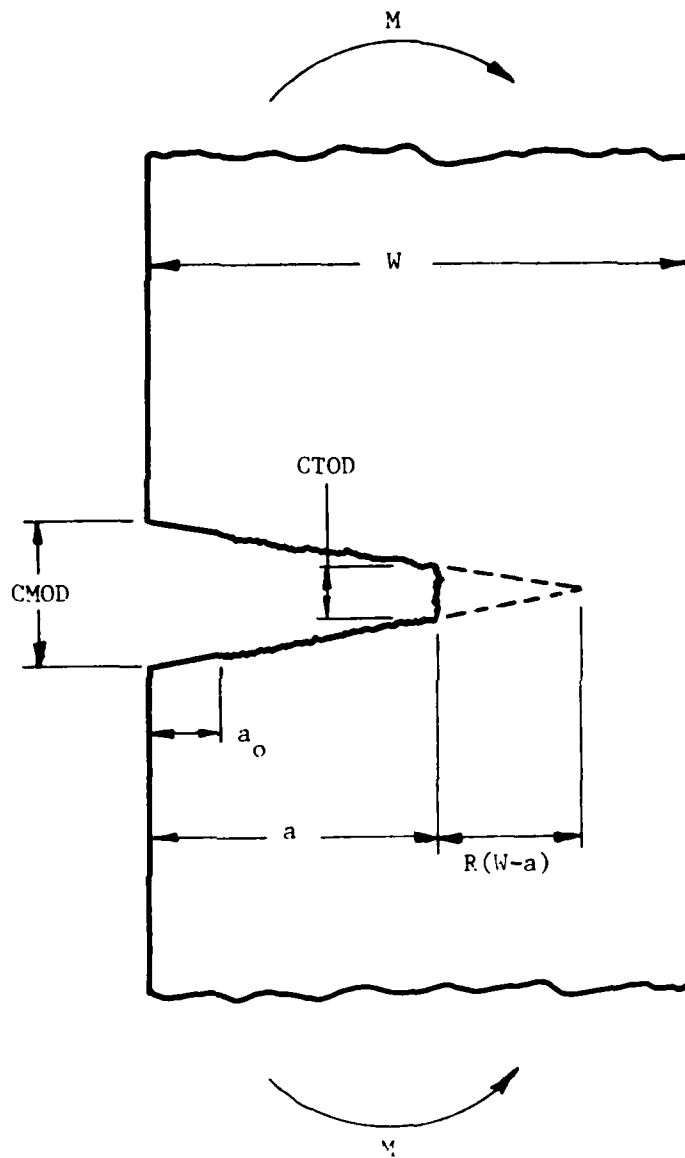


Fig. 8 Crack-Mouth-Opening-Displacement (CMOD) and Crack-Tip-Opening-Displacement (CTOD)

3. K_{IC} is independent of crack length a , or a_e , but not notch length a_o , beyond some length.
4. K_{IC} is independent of specimen size - beyond some size (which is probably related to the maximum size of aggregate).
5. K_{IC} is equivalent to G_{IC} , the energy release rate associated with unstable crack growth, e.g.

$$G_{IC} = \frac{1-\nu^2}{E} K_{IC}^2$$

6. The J-Integral approach should give results which are identical to G_{IC} .

If LEFM is not a valid approach, the general concept is still used in which non-linear effects are considered. Two such approaches in which a single parameter is sought are the J-Integral Method and Determination of the CTOD.

2.1.1 Experimental Methods Proposed to Determine Fracture Parameters for Concrete

2.1.1.0 Introduction

A number of investigations directed toward measuring the fracture toughness of cement pastes, mortars and concretes are reported (17, 21, 26, 35, 42, 54, 61, 78, 81, 86, 87, 90, 99, 111, 112, 117, 131, 133, 140, 159, 189, 194, 196, 197, 202, 204, 214, 220, 230, 276, 278, 303, 304, 325, 327, 383, 384, 391, 400, 402, 407, 408, 426, 441, 449, 450, 452, 470, 474, 528, 531, 544, 548, 578, 584, 624, 637, 669, 685, 686, 697, 716, 743, 751, 756, 761, 763, 777), as well as for rock (122, 181, 207, 231, 263, 380, 381, 454, 527, 535, 591, 597, 605, 701, 702, 709, 730, 741, 745). These investigations used beam bending specimens (54, 86, 99, 111, 158, 189, 197, 214, 230, 245.1, 278, 361, 383, 384, 385, 400, 441, 452, 531, 544, 578,

584, 651, 669, 685, 687, 694, 713, 743, 751, 777), double cantilever beam specimens (276, 325, 548, 614, 627), plate tension specimens (21, 35, 78, 81, 86, 672, 761), torsion specimens (143.1, 300.1, 407, 550), and CLWL specimens (697, 756). These were all concerned with Type I deformation. A limited number of investigations have been concerned with Type II deformation (418.1, 559, 560, 561).

The investigations can be conveniently categorized as those which used notched specimens with no consideration of precracking or influence of prior crack growth (35, 54, 78, 81, 111, 189, 197, 214, 230, 383, 384, 385, 531, 584, 651, 685, 687, 713, 751); and those which did consider precracking on estimates of fracture toughness or other fracture parameters (78, 99, 158, 194, 197, 245.1, 278, 361, 400, 441, 452, 544, 578, 669, 694, 743, 777). It was observed by Schmidt (207) in tests on limestone beams (1976) subjected to repeated loads that crack closure occurred upon load removal. Upon subsequent re-application of the load the material compliance was reduced until the crack re-opened. Thus, it was concluded that "...a crack probably cannot be adequately simulated by a sharp saw cut for the purpose of measuring fracture toughness. Even if the cut could be made sufficiently sharp so as to provide a stress singularity similar to that of a natural crack, the closure stresses would still be non-existent." The writer, in performing numerous beam tests has observed the same phenomenon (245.1, 278, 361, 400, 402, 544, 579, 669, 743, 777).

Furthermore, it has been shown for beams in three-point and four-point bending (441, 544) that failure loads and fracture toughness values are considerably higher for pre-cracked beams which are loaded to failure as compared with notched beams.

In fairness to those early investigators who used notched beams it should be mentioned that a major difficulty in the test is to estimate the crack length at the onset of unstable crack growth. The easiest approach is to simply notch the beam - or cast in a notch - and assume that this length (denoted a_0 in Figs. 1 and 2) is essentially the same as the critical crack length. Furthermore, the availability of displacement-controlled testing equipment of sufficient sensitivity, which is necessary to precrack (but not fail) plain-concrete beams did not exist, until the early 1970's.

The first work known to the writer in which commercial equipment was used to control the cracking process in plain concrete was his work performed with colleagues at Kansas State University in 1976, presented in 1977 (245.1), and published in 1978 (278). Since that time the compliance - calibration method for estimating crack length has been modified on the basis of actual crack measurements using dye-penetrant (578, 743). Thus, a major impediment to the determination of valid fracture data has been overcome. At the same time, it was found that surface measurements are not necessarily suitable as the crack front is not uniform except in the interior (743).

The writer feels strongly that many of the contradictory results obtained by earlier investigators which led to their pessimistic conclusion that fracture mechanics concepts (linear or non-linear) would not apply to concrete were caused by

- a. lack of appreciation of the behavior of a true crack, and
- b. inability to estimate accurately the true crack length at various stages of crack growth.

The tremendous increase in research activities in the 1980's supports

this viewpoint although most investigators still argue against the suitability of LEFM.

Another major consideration - first elucidated by Shah and McGarry (86) - is what has come to be called the influence of the process zone. As shown in Fig. 2 and described earlier, this zone consists of a combination of aggregate interlock behind the crack tip and microcracking proceeding around and through the aggregate in front of the crack tip. This zone could play a role in fracture mechanics calculations similar to that of the zone of plasticity which occurs in ductile metals.

This, in fact, is what most investigators attribute to the lack of consistency in test results for fracture toughness and the energy release rate (86, 503, 631, 649, 658, 691, 705, 747). As a result, there is some work being done by the writer and others to determine the size of this zone.

Based on dye measurements (578, 743, 777) the writer believes this zone to be fairly small - limited in depth to about the size of the largest aggregate. Few other direct measurements of the interior have been made but indirect measurements using acoustic emission with large, double-cantilever specimens have been made (325). These investigators state the process zone to be at least one meter (39 in.) long.

Recently, surface measurements obtained from high sensitivity Moiré interferometry have been reported by Cedolin, et.al., using fairly small tension specimens with single or double edge notches (571). The process zone appears to be about 15mm for specimens with a maximum aggregate size of 10mm.

The most recent results known to the writer are those presented by Barker, et.al. (756), on CLWL specimens in which they used a surface

inspection technique and also cut sections through the interior. They report "There was no evidence of the small microcracks or multiple branches normally postulated as existing in the process zone ahead of the main crack".

These results tend to reaffirm the writer's opinion stated previously that the source of error in applying LEFM to concrete lies in the area of crack length measurement and interpretation of the test record of load versus displacement.

2.1.1.1 SEN Bending Specimens and Test Methods

The bending specimen with a single edge notch subjected to either one or two concentrated loads has been by far the most popular as evidenced by the large number of investigations and test data which appear in the literature for over fifteen years (54, 86, 111, 158, 189, 197, 214, 230, 245.1, 278, 361, 383, 384, 385, 400, 441, 452, 531, 544, 578, 584, 651, 669, 685, 687, 694, 713, 743, 751, 777).

The writer has tested specimens in 3-point bending, 4-point bending, statically-precracked and pre-cracked in fatigue. His beam specimens were all 3 in. (76mm) wide and most of them had a depth of 4 in. (102mm), although some had a depth of 8 in. (203mm). It is conjectured by a number of investigators (214, 470, 503, 631, 658, 691) that valid test data cannot be obtained unless beams with depths 12 in. (305mm) or greater are used. The RILEM Committee on Fracture Mechanics of Concrete has recommended in a proposed testing specification (531) beam depths of 100mm, 200mm and 300mm. The writer is currently investigating further the influence of size with 8 in. and 12 in. deep beams to supplement data taken on 4 in. deep beams (669, 777).

The beam proportions generally specified (531, 655) are $\frac{S}{W} = 4.0$. This is primarily because formulas such as Eq. 6 by Srawley (558, 655) require this ratio. The writer is using $\frac{S}{W} = 3.75$. The only requirement here is that a ratio be used for which a stress intensity factor fracture toughness formula exists. Some investigators feel $\frac{S}{W} \geq 8$ is more suitable because of reduced shear, or "deep beam" effects.

Following the ideas expressed in Reference 579 the following specimen and test procedure is recommended. This recommendation does not preclude others which will be described later. However, since considerable research activity by the writer and others is currently underway, all the proposals reported herein are tentative and subject to further confirmation.

2.1.1.1.1 Recommendation. KSU Specimens, Test Setup, and Method

a. Referring to Fig. 5, the following proportions are recommended:

$$\frac{S}{W} \geq 4, W \geq 8 X \text{ (Maximum aggregate size)}$$

$$B \geq 4 X \text{ (Maximum aggregate size)}$$

For 3/4 in. aggregate a beam with $S = 30$ in. (760mm), $W = 8$ in. (203mm) and $B = 3$ in. (76mm) will satisfy the stated requirements and also will not be too heavy to handle conveniently (wt. ≈ 60 lb = 267 N).

The depth requirement will allow a fairly large amount of crack propagation before the process zone - presumed to be about 1 in. long - will interact with the compression face.

a.1 Alternatively, a beam with $W = 12$ in. (305mm), $S = 45$ in. (1.14m), $B = 3$ in. (76mm) may be used but to date there is very little experimental evidence to justify this size of beam.

b. Testing should be conducted using an electro-hydrodynamic machine. If load control is used a fairly stiff frame is needed (470). If strain control is used this requirement can be relaxed (761).

c. Test Setup

A conventional test setup in which the beam is configured as shown in Fig. 5 has been used extensively (245.1, 278, 400, 544, 579, 777). However, in order to use dye to reveal the crack front it is necessary to precrack the beam, remove it and turn it upside down, insert the dye, again turn the beam over, place in the setup and load to failure.

Therefore, it is recommended that the testing fixture be designed to allow the tension face of the beam with the exposed crack to be the top surface. Such a fixture is shown in Fig. 9 and is presently being used at KSU.

It will be necessary to monitor continuously load P versus crack-mouth-opening-displacement, CMOD and load P versus load-point-displacement, LPD. Thus, two displacement gages and two plotters are required.

A commercially - available displacement gage of sufficient sensitivity is MTS 632.05 which has a sensitivity of 2×10^{-4} in. / v output. When used with the plotter a direct, least reading of 1×10^{-6} in. (25×10^{-9} m) is obtained.

The gage used to monitor CMOD may be easily attached to the beam using a yoke or frame arrangement described in Reference 402, while the gage to monitor LPD can be attached in a variety of ways, one of which is described in Reference 668.

d. Specimen Preparation

After casting, the beams should be wet-cured using a 100% humidity chamber or lime water bath. It has been argued by some that at the time of

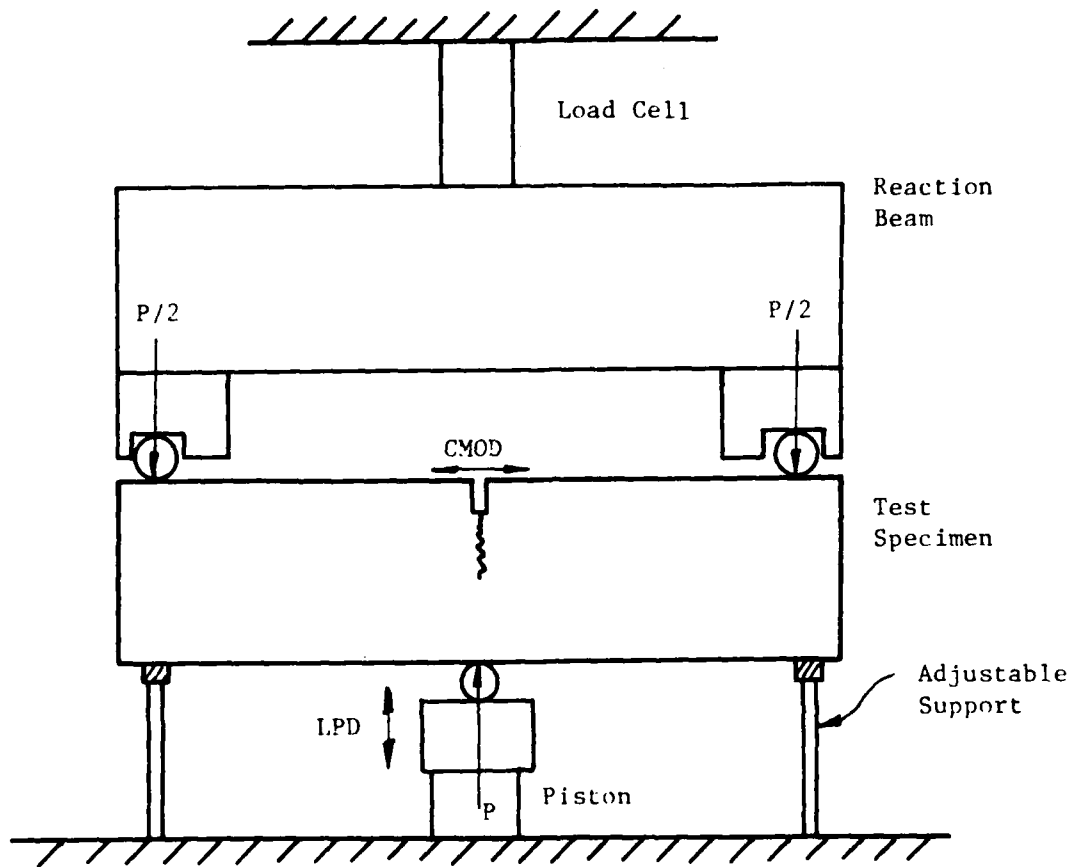


Fig. 9 Schematic of KSU Testing Arrangement
For 8 in. and 12 in. Beam Specimens

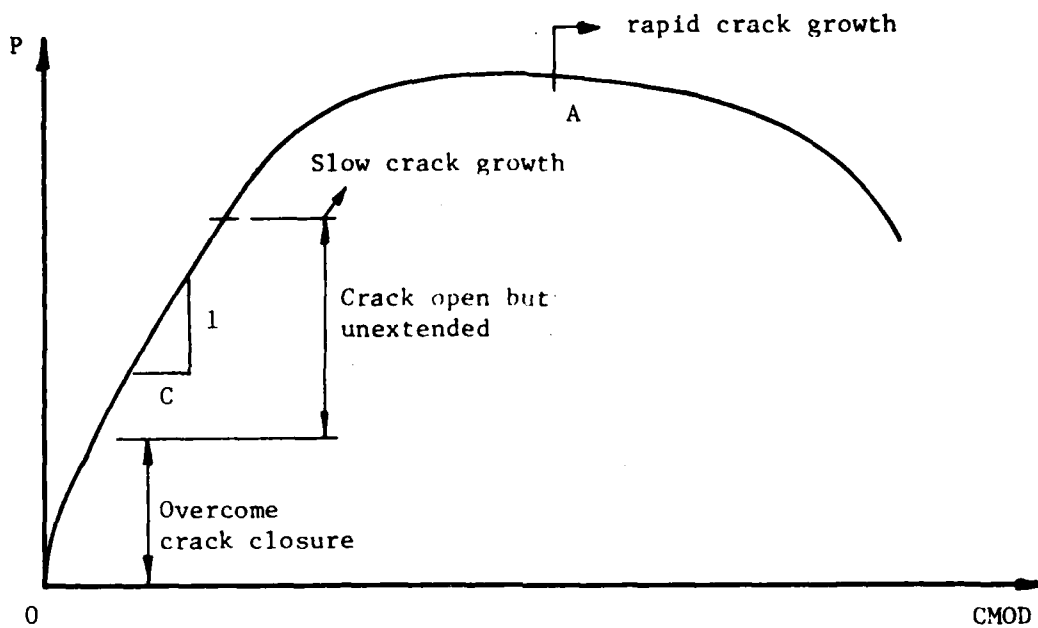
testing the beams should be wet, i.e. every precaution should be made to prevent any drying before or during the test (470, 531). Obviously, rather elaborate preparations are needed to assure this. The reason for doing this is to prevent shrinkage cracking due to drying.

The writer feels that while that is true, the resulting data is not representative of the behavior of most concrete structures. Perhaps for marine structures this would be an important consideration. However, in general, it is recommended that all specimens be air-dried after at least a 28 day wet cure and for one week prior to testing. The air-dry environment need not be high humidity but should be humidity controlled.

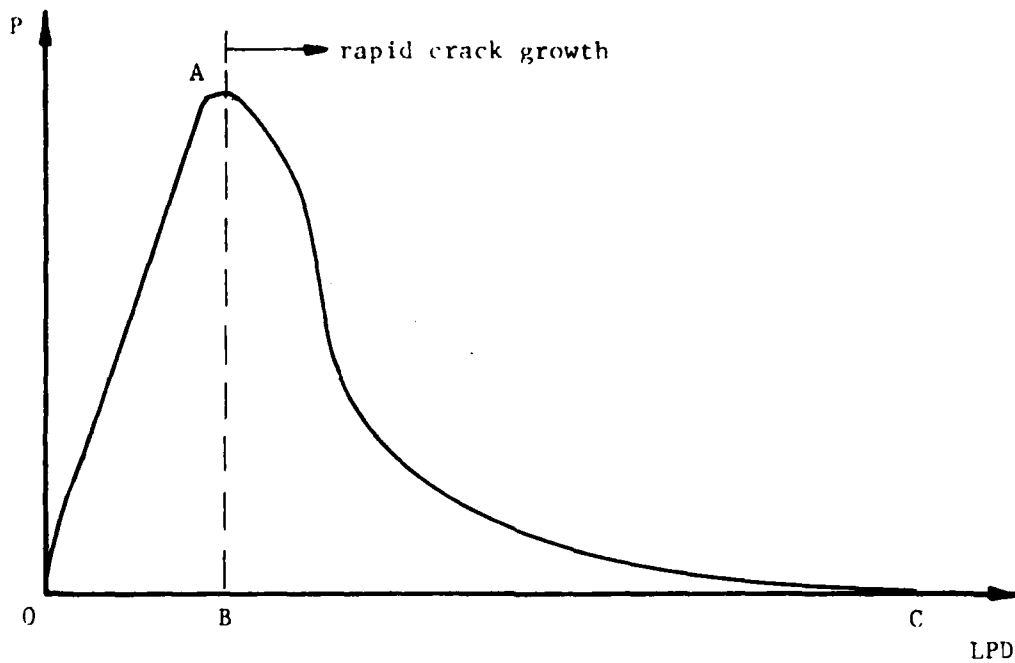
The other preparation needed for the specimens is to create a starter notch at the desired crack location - usually at midspan. At the present time, while research is still going on with regard to notch preparation, it appears to be reasonable to use a concrete saw to cut the notch (278, 531). The writer uses a saw with a cutting width of 0.13 in. (3.4mm) and cuts to a depth of $\frac{a_o}{W} = 0.1$.

e. Testing Procedure

1. Modified compliance calibration. The technique used previously by the writer (278) and others (207) in which compliance measurements were taken from notched beams is not recommended because of the influence of surface roughness (743). Instead, it is recommended that specimens be precracked, dye inserted, then loaded to failure. The inverse slope of the $P \sqrt{V}$ CMOD trace taken immediately after crack closure is overcome will yield the compliance value C - see Fig. 10. The average crack length is obtained from inspection of the dyed surface, and compliance C and $\frac{a}{W}$ obtained in this way are plotted. This gives a single point on the plot. It



a. Load Versus Crack-Mouth-Opening-Displacement



b. Load Versus Load-Point-Displacement

Fig. 10 Typical Load-Deflection Traces

is recommended that this process be conducted for approximate values of $\frac{a}{W} = 0.2$ thru 0.8 in 0.1 increments. Thus seven specimens are needed to obtain the compliance calibration curve - Fig. 11.

2. With the compliance information available, additional specimens can be precracked using strain control to pre-determined crack lengths. In practice this is done easily by drawing a straight line with the desired slope on the graph of the plotter to be used with the CMOD gage. Using the RAMP function and SPAN control the load is increased into the region of rapid crack growth but because of strain control the operator can stop and unload the beam at any stage. Upon reloading, and after overcoming crack closure, the slope of the reload line is compared to that drawn on the paper. When the slopes are equal, the beam has been precracked to the desired amount.

After the beam has been precracked, dye is inserted and allowed to stabilize.

3. Following this operation the beam is then loaded to failure. This can be done using either strain control or load control with virtually no difference in shape of the descending (strain softening) portion of the load-deformation diagram provided a very low rate of loading is used. Typically this should be set so that the peak load is reached in 30 sec. or more.

Typical diagrams of load versus CMOD and load versus LPD are given in Fig. 10.

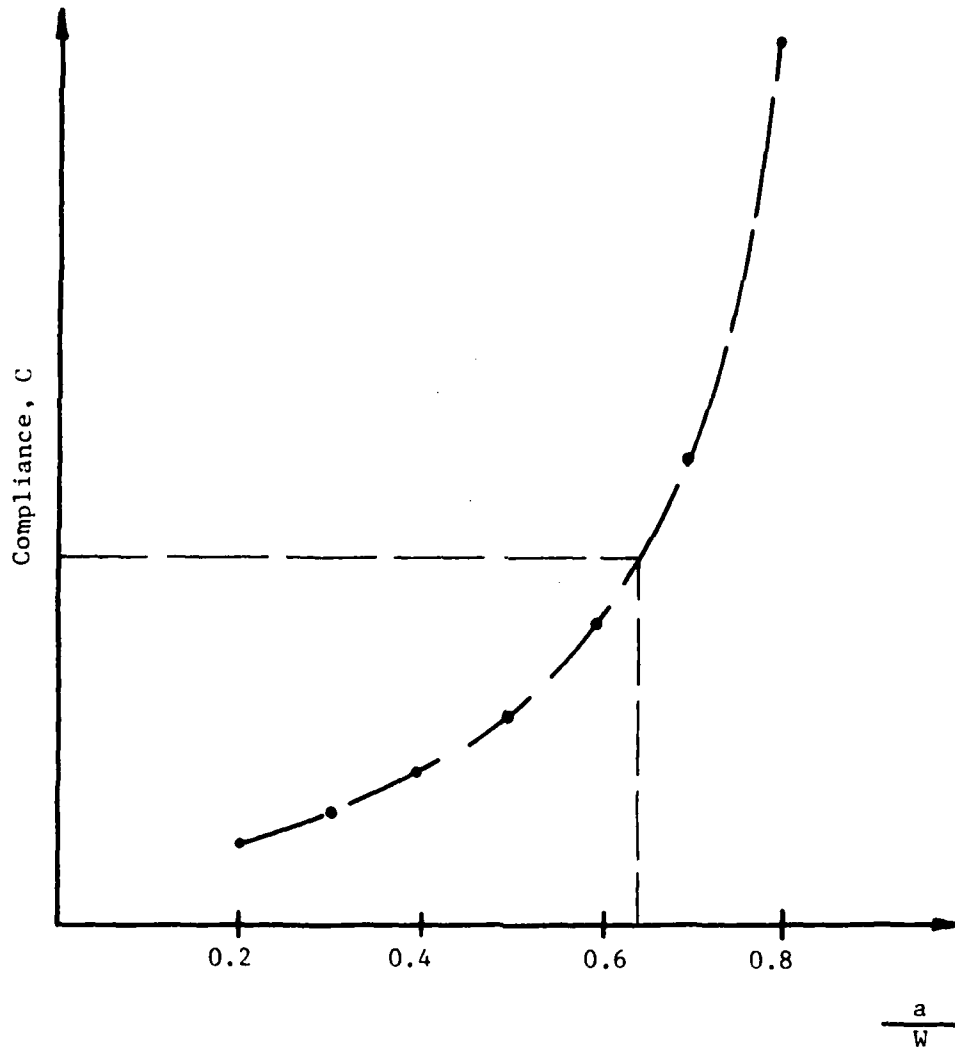


Fig. 11 Typical Compliance Calibration Curve

f. Data Interpretation

Steps e.2 and e.3 should be done with $\frac{a}{W} = 0.5$ and 0.7 , thus at least two beams need to be tested. We prefer to test at least five beams at each ratio to obtain average values with some confidence in a statistical sense.

As a lower bound on the energy absorption measurement and in keeping with the principles of LEFM (669), the energy absorbed up to the onset of rapid crack growth, that is $U = \text{Area OAB}$ in Fig. 10.b, is determined using a planimeter.

With this data the energy release rate G_{IC} can be determined two ways.

$$1. G_{IC} = \frac{\text{Area OAB}}{C_A \frac{BW}{W} (1-\frac{a}{W})}$$

in which C_A = a surface roughness coefficient of about 1.15 (578)

and, $\frac{a}{W}$ = crack length ratio at point of onset of rapid crack growth.

The crack length a at the onset of rapid crack growth is obtained from the compliance slope of a line drawn from the origin to the point of instability of the $P - \text{CMOD}$ curve - or a line from O to A in Fig. 10 a. - and then estimated from the compliance calibration curve. This approach is admittedly approximate but does yield fairly consistent G_{IC} values which are independent of $\frac{a}{W}$ (669, 777).

2. The recommended method is to plot U versus $\frac{a}{W}$ based on the initial crack length. Such a plot is shown in Fig. 12. In this and from Eq. (12) it is seen that

$$J_{IC} = G_{IC} = - \frac{\frac{dU}{d(\frac{a}{W})}}{C_A \frac{BW}{W}} = - \frac{\text{slope}}{C_A A_b}$$

where A_b = beam area.

Also shown in Fig. 12 is a plot based on using the extended crack length. In theory, the slopes should be the same since they represent the energy change per unit crack extension.

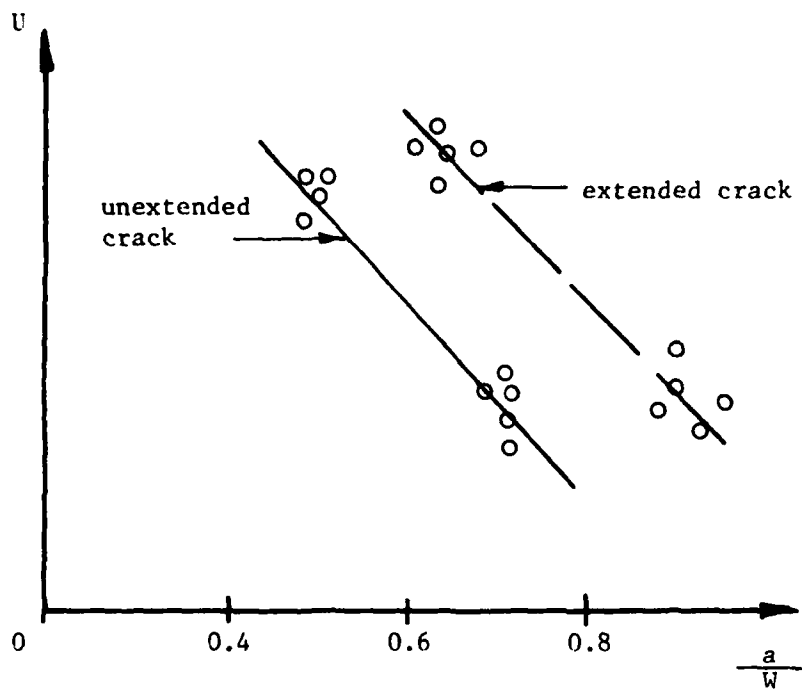


Fig. 12 $U \propto \left(\frac{a}{W}\right)$ Using Unextended Crack
(Initial Crack) and Extended Crack
(Estimated at Onset of Rapid Crack
Growth)

As described in References 669, 777 the results of tests on small beams ($W = 4$ in.) indicate very good agreement in results for G_{IC} obtained by either of these methods of data interpretation.

Note also that if the second method is used, it should be possible to avoid using the compliance calibration technique if only the initial crack length is considered since that is measured directly from the dyed surface.

2.1.1.1.2 Other Methods Using SEN Beams

The other proposed methods for beams typically use the three-point bending configuration of Fig. 5 although some use the "upside-down" setup of Fig. 9. These methods all consider the fracture relationship to be of a non-linear nature.

2.1.1.1.3 Proposed RILEM Recommendation. (531)

This recommendation is made by RILEM TC 50 Fracture Mechanics of Concrete. The dimensions of the bending specimen to be used are proposed to be

$$B = 100 \pm 5\text{mm (ca 4 in.)}$$

$$W = 100 \pm 5\text{mm (ca 4 in.)}$$

$$L = 840 \pm 5\text{mm (ca 33 in.)}$$

$$S = 800 \text{ mm (ca 31 in.)}$$

$$a_o = 50 \pm 5\text{mm (} \frac{a}{W} = 0.5)$$

$$\text{Notch width} < 10\text{mm (0.4 in.)}$$

This size is to be used with maximum aggregate size not more than 32mm (1.26 in.).

When larger aggregate is to be used the dimensions shall be increased uniformly. Also, the number of replicate tests varies with aggregate size

from six for maximum size less than 10mm to twenty-four for maximum size greater than or equal to thirty-two.

The specimens are to be cured in a 100% humidity environment and stored in lime-water. They are not to be removed until 15 min. before testing.

The test should be performed in a stiff machine using displacement control such that the peak load is reached in 30-60 sec. from the start of the test. The test record is a continuous trace of load versus load-point-displacement - eg. $P \text{ v } LPD$ in Fig. 10b.

The energy represented by the entire area under the curve - area O A C in Fig. 10b. is measured and the fracture energy per unit area is

$$G_f = \frac{U + mg \delta_o}{BW \left(1 - \frac{a_o}{W}\right)}$$

in which U = area under the curve,

δ_o = final displacement at failure,

mg = weight of the specimen,

a_o = initial notch length.

The term $mg \delta_o$ represents a correction factor due to energy associated with the beam's self weight. The writer and others (713) have found this term to be negligible.

The primary criticism the writer has with this method is that the initial conditions imposed are wrong - namely that the beams are not cracked initially.

This method has been used extensively in a series of round-robin tests using beams of different sizes. The results of these tests indicate that the energy G_f is not independent of a_o nor independent of beam size (584, 685, 686, 713, 763).

2.1.1.1.4 Recent method proposed by Jenq and Shah (766).

In this method the specimens and testing procedure are virtually identical to those given in the recommended method stated in 2.1.1.1.1 above. However, the compliance - calibration portion is not needed as all data is obtained from a single test of a notched, but non-precracked, beam. The method attempts to account for the non-linearity by using two parameters: the critical stress intensity factor, K_{IC}^s and the elastic critical crack tip opening displacement $CTOD_c$. Both of these are evaluated using a plot of load versus CMOD and LEFM.

Using this plot - again that shown in Fig. 10 is typical - the value of the effective, elastic crack is determined such that the calculated CMOD is equal to the measured one. The calculated CMOD is determined from

$$CMOD = \frac{24 Pa}{BW E} \left\{ 0.76 - 2.28 \left(\frac{a}{W}\right) + 3.87 \left(\frac{a}{W}\right)^2 - 2.04 \left(\frac{a}{W}\right)^3 + \frac{0.66}{\left[1 - \left(\frac{a}{W}\right)^2\right]} \right\} \quad (16)$$

This equation is used to determine a and then K_{IC}^s is determined from

$$K_{IC}^s = \frac{6 P\sqrt{a}}{BW} \frac{1.99 - \left(\frac{a}{W}\right)\left(1 - \frac{a}{W}\right)\left[2.15 - 3.93 \left(\frac{a}{W}\right) + 2.7 \left(\frac{a}{W}\right)^2\right]}{\left[1 + 2\left(\frac{a}{W}\right)\right]\left[1 - \frac{a}{W}\right]^{3/2}} \quad (17)$$

Finally, $CTOD_c$ is determined from a similar equation which requires an estimate of the location of the crack tip opening displacement relative to the end of the crack - and is highly sensitive to this. For instance, if this location is at the crack tip itself, the value of $CTOD_c$ is zero.

The writer believes the proposed method for determining K_{IC}^s is reasonable but to date very little experimental data is available to verify it.

2.1.1.1.5 Method Proposed Recently by Bazant (642).

This method is entitled "Fracture Energy of Concrete from Maximum Loads of Specimens of Various Sizes" and is based on the use of dimensional

theory (573, 759). Three groups of beams of different depths and spans but the same width are to be tested in three-point bending.

All beams are notched at midspan, using a diamond saw, to a depth a_o . The recommended ranges of parameters are (cf Fig. 5):

$$\frac{S}{W} \geq 4.0$$

$$0.3 \leq \frac{a_o}{W} \leq 0.5$$

$$B, W \geq 3 \text{ times the maximum aggregate size } d_a.$$

Of the three beam sizes the minimum depth should be $\leq 5d_a$ and the largest depth $\geq 15d_a$. Also, the ratio of maximum depth to minimum depth must be at least four.

As an example, consider a concrete with d_a of 3/4 in. Then

$$B, W \geq 2.25 \text{ in.}$$

$$W_{\min} \leq 3.75 \text{ in.}$$

$$W_{\max} \geq 11.25 \text{ in.}$$

$$\text{and } \frac{W_{\max}}{W_{\min}} \geq 4.$$

Beam specimen dimensions that satisfy these requirements would be:

Smallest beam $B = 3 \text{ in.}, W = 3 \text{ in.}, S = 12 \text{ in.}, a_o = 1.5 \text{ in.}$

Largest beam $B = 3 \text{ in.}, W = 12 \text{ in.}, S = 48 \text{ in.}, a_o = 6 \text{ in.}$

The specimens are loaded in three-point bending and only the failure load is recorded - no trace of load versus displacement is required. Furthermore, there are no restrictions imposed upon testing machine stiffness nor use of deformation control. However, it is recommended that load rates be such that the peak load is reached in 1 to 10 min.

Duplicate tests are conducted and average values of P and W obtained for each group of beams. The value of P is to be adjusted to account for the beam self-weight.

These results are then plotted with $\left(\frac{BW}{P}\right)^2$ as the ordinate and W as the abscissa. The measured slope of this line is denoted A .

Then, the fracture energy G_f is given as

$$G_f = \frac{g(\alpha_o)}{E_c A}$$

in which E_c = modulus of elasticity of concrete, $\alpha_o = \frac{a}{W}$ and $g(\alpha_o)$ is called the nondimensional energy release rate.

This is given by

$$g(\alpha_o) = \left(\frac{S}{W}\right)^2 \pi \alpha_o [1.5 F(\alpha_o)]^2 \quad (20)$$

and

$$F(\alpha_o) = 1.090 - 1.735 \alpha_o + 8.20 \alpha_o^2 - 14.18 \alpha_o^3 + 14.57 \alpha_o^4 \quad (21)$$

for $\frac{S}{W} = 4$.

Other functions of $F(\alpha_o)$ are available for other span/depth ratios.

This method appears also to be potentially useful, but again to date there is very little confirmation with test data.

As stated previously, the writer, at present, is conducting research on size effects in beams with the following dimensions:

Small: $B = 3$ in., $W = 4$ in., $S = 15$ in.

Intermediate: $B = 3$ in., $W = 8$ in., $S = 30$ in.

Large: $B = 3$ in., $W = 12$ in., $S = 45$ in.

The values of $\frac{a}{W}$ range as 0.3, 0.5, 0.7. Thus the current testing program will yield data which will be useful in evaluating the validity of this proposed method.

2.1.1.1.6 Summary of SEN Beam Methods

The first - and recommended - method has been proposed here for the first time. It is based on an extensive amount of data obtained by the writer and was first suggested by Go (578). Work is continuing to review

other experimental data in light of this method to obtain further confirmation of its validity. At the same time, it is recognized that the testing method is cumbersome. Therefore, the other proposed methods may become more popular.

The writer cannot recommend the proposed RILEM method for the reasons stated.

The methods proposed by Shah and Bazant both appear reasonable and have limited experimental justification. The writer currently is directing research to evaluate these methods using earlier data obtained at KSU as well as new data on different sizes of beams. Both of these methods are easier to implement than the writer's method - especially that of Bazant.

Although there has been an extensive amount of research carried out using beam specimens, the test specimens and methods given by the writer, RILEM and Bazant are the only one actually proposed for a standard.

Before leaving the subject of SEN beams, one other method should be mentioned even though it has not been proposed as a standard. This method is the R-Curve analysis approach which has been used by many investigators (354, 629, 631, 649). In this method it is assumed the fracture energy, G_f , is not independent of crack length until a sufficiently long crack is developed. If G_f is plotted as an ordinate against crack length a as the abscissa then the resulting curve can be considered an envelope of curves satisfying the requirements that (649)

$$G_f(a) = F(c) \quad (22)$$

and $\frac{dR}{dc} = \frac{dG}{da}$

where R = the energy required for crack growth (not necessarily a constant)

and c = the crack extension length from a starter notch.

Such an envelope can be constructed from test data by evaluating G_f from LEFM using the failure load P and crack length a . Results of this approach are given by Bazant, et.al. (651) based on test data from Jeng and Shah (694).

This approach also shows considerable promise and is currently being implemented by the writer using KSU data.

2.1.1.2 Other Types of Specimens

Over the years the following specimen geometries have been tried: plate or bar in tension, double cantilever beams in bending, double torsion beams. In no case has any of these actually been proposed to be a standard test specimen. All the proposed test standards, to the writer's knowledge, have been for SEN beams as described in the previous section.

Additionally, while some of the test results for these other geometries look promising, there is not a great deal of experimental data reported for any of them. Therefore, none of these methods are recommended at present.

2.1.1.2.1 Plate or Bar in Tension

Historically, the plate in which a central flaw is placed and loaded in direct tension has been one of the earliest specimens tried (21). It is still being evaluated today (761.1). It is potentially a useful specimen but does have the disadvantage of needing a fairly elaborate and carefully aligned testing fixture.

2.1.1.2.2 Double Cantilever Beam

This type of specimen - shown in Fig. 13 - has been used extensively over the years (276, 325, 548, 614, 627, 631). The geometry is such that

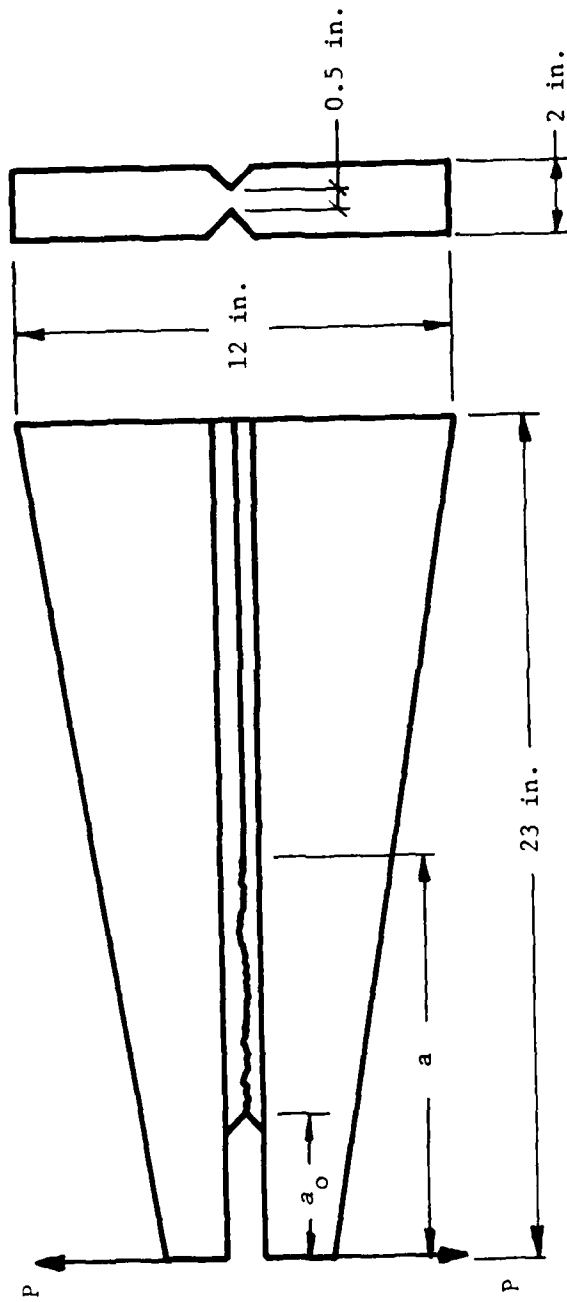


Fig. 13 Double Cantilever Beam Specimen
Dimensions From Reference 631

crack propagation is contained and can easily be measured. Further, it is fairly easy to control the cracking process.

However, this type of specimen has one serious limitation. In order to test concrete with a typical size of aggregate - say one inch - and if a minimum dimension of say three times the aggregate size is needed, then the specimen overall size becomes very large. For instance, if the specimen geometry of reference 325 is used, then the overall size would change from 23 in. to 192 in. Some investigators have claimed that indeed this size of specimen is needed (691). Unfortunately, this size does not lend itself well to a routine testing situation.

Within this category may also be considered the crack line, wedge-loaded, double cantilever beam specimen (CLWL-DCB) as defined in ASTM E 561-81 (417) and proposed for use in concrete by Barker, et.al. (756). The geometry for the larger specimens they tested is shown in Fig. 14.

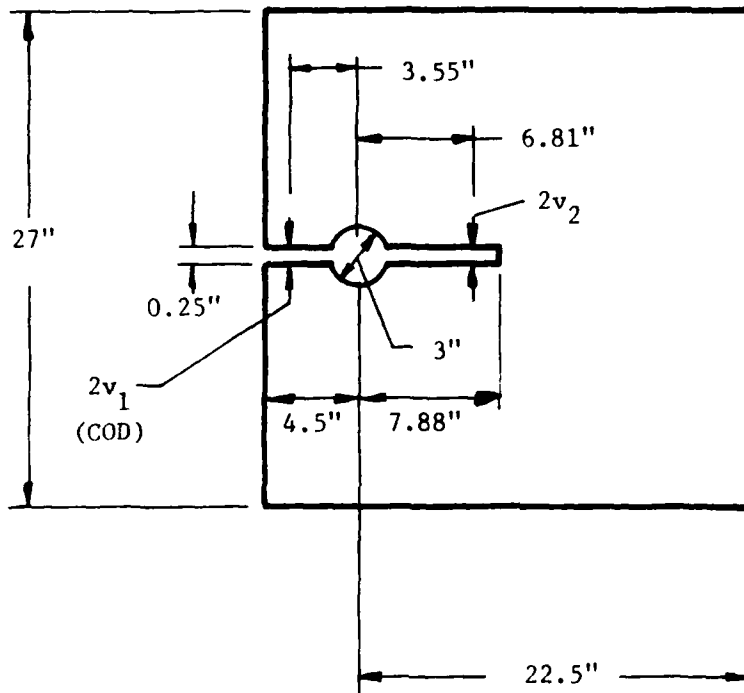
This specimen is loaded through the circular hole by a wedge which creates the relative movements indicated by v_1 and v_2 . The results they present indicate this may also be a viable candidate for a standard test specimen. Note that the crack is allowed to propagate naturally and the specimen thickness is suitable for concrete.

2.1.1.2.3 Double Torsion Beam (407)

These specimens have a geometry similar to the double cantilever beam but the loading is such that each "leg" is placed in torsion while the crack propagates along a central groove. This type of specimen suffers from the same limitation as the double cantilever beam specimen of Fig. 13.

2.1.2 Models for Crack Propagation and Fracture of Concrete

Many of the test methods presented above are based upon, or were used



Thickness = 3"

Fig. 14 CLWL-DCB Specimen
Used in Reference 756

to verify, analytical models for crack propagation in concrete. To date, most of these models are directed to unreinforced concrete.

Almost all of these models are based on the finite-element discretization of the cracked concrete with a combination of "super" or crack-tip element along with regular elements.

As may be expected, the early, analytical, models were based entirely on LEFM concepts in which crack propagation would occur if the state of stress at the crack tip corresponded to a critical value of K_I (or a combination of K_I , K_{II} , and K_{III}). Bazant (342, 343) indicates that in fact the crack propagation is dominated by the K_I mode but this has been disputed by others. The writer's co-workers and others have developed finite element models of this type (361, 587).

However, as it became considered by many that LEFM might not be valid, a number of models have appeared, again using the finite element approach, in which attempts to account for crack closure and the presence of a process zone are made. These include models by Ingraffea (691, 765), Hillerborg (198, 305, 359), Petersson (205, 386, 470), Wecharatana and Shah (631), Bazant (342, 343.1, 564, 569, 750), others (657, 710, 725) in which fictitious stresses are employed, the critical crack tip opening displacement is used ($CTOD_c$), or the crack driving process is controlled by the fracture energy G_c .

These models are lacking only valid experimental fracture data to become useful in the analysis and design of concrete structures.

At the present time, research activity is directed toward practical application of these models to reinforced concrete structures. In particular, work has appeared with regard to bond problems and crack growth in reinforced concrete beams (475, 658, 692). Ingraffea has successfully

applied LEFM concepts to the cracking of a concrete dam (434). Hawkins has indicated numerous areas within the ACI Building Code in which fracture mechanics concepts might be applied (684), see also Gustafsson and Hillerborg (676). Earlier he also attempted to apply LEFM to shear cracking of concrete beams (229) (with limited success).

The writer has no specific recommendations to make with regard to analytical models except to note that the ones by Ingraffea (765), Hillerborg (305), Bazant (564), Wecharatana and Shah (631) among others all appear to be valid. Extensive work still needs to be done to verify any of these with test results from both plain and reinforced concrete specimens.

2.2 Summary and Recommendations

The current state-of-the-art of fracture mechanics applications to concrete has been reviewed very briefly with the major emphasis being on testing methods to determine the appropriate fracture parameters. Historically, these methods evolved from previous experience obtained in metals which relied on the suitability of linear elastic fracture mechanics. The present thinking by most investigators is that non-linear effects - primarily associated with a process zone presumed to exist at the front of the crack - dominate the crack propagation mechanism. Nevertheless, the methods, equations and appropriate parameters from LEFM are still used by these investigators in one form or another to predict these non-linear effects.

As stated earlier, the writer still believes LEFM to be valid on the basis of test results obtained at KSU using the testing method described in section 2.1.1.1.1. This method of testing is recommended. A related test method described in section 2.1.1.1.4 by Shah at Northwestern University may also be appropriate.

The method proposed by Bazant which uses different beam sizes which is described in section 2.1.1.1.5 may also be valid.

All of these methods use beam bending specimens with edge notches and subjected to three point bending.

The parameters which are obtained and which characterize the crack growth and failure are the fracture toughness K_{IC} , critical energy release rate G_{IC} , the J-Integral J_{IC} . For LEFM these are all equivalent. If, in fact, non-linear effects are severe, it is expected that G_{IC} and J_{IC} will still be valid fracture parameters. In addition, for this case, the critical crack tip opening displacement, $CTOD_c$ is expected to be a pertinent parameter.

Of the other test methods and specimens described in this report the writer feels that the one most likely to be useful and accepted as a standard method is the crackline, wedge-loaded, double cantilever specimen, CLWL-DCB and associated test procedure described by Barker, Hawkins, Jeang, Cho and Kobayashi.

Appendix I

Bibliography Database Program

Pages 54-69

Source Listing of Program

Pages 70-84

Example of Program Implementation

```

*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
F>
SET HEADING OFF
SET SAFETY OFF
* main.prg
SET PROCEDURE TO PROC
public del,dn,fn,sno,command,reference,authors,atitle,flag,keywords,mode
do init
restore from add
set intensity off
*!! Logical constant converted.
do while .t.
  do delcheck
  set format to saydata
  store "a)dd      b)ackward  d)delete/recall  e)dit      " to prompt1
  store "f)orward h)elp      l)ocate      m)aintenance" to prompt2
  store "p)rint   q)uit      r)eport      s)et flag   " to prompt3
  store "**** MAIN MENU ****" to mode
  store " " to command
*!! 'set screen on' is no longer valid.
set device to screen
*!! set screen on
  do delcheck
read
do case
  case upper(command) = "S"
    do flag
  case upper(command) = "A"
    do add
  case upper(command) = "B"
    skip -1
  case upper(command) = "D"
    do delete
  case upper(command) = "E"
    do eddit
    do delcheck
  case upper(command) = "F"
    skip
  case upper(command) = "H"
    do help
  case upper(command) = "M"
    do maintain
  case upper(command) = "R"
    do reports
  case upper(command) = "L"
    do locate
  case upper(command) = "Q"
    CLOSE PROCEDURE
    cancel

```

```
case upper(commend) = "P"  
  do print  
endcase  
enddo  
CLOSE PROCEDURE
```

```

procedure verifyne
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* verifynew.prg
CLEAR
go top
locate for new
*!! Logical constant converted.
store .t. to more
do while more .and. (.not. EOF())
  set format to saydate
  store "VERIFY NEW RECORDS" TO mode
  store "a)cccept, c)ontinue, d)elete/recall" TO prompt1
  store "e)edit, p)rint, q)uit" TO prompt2
  store " " to prompt3
  store " " to command
  do delcheck
  read
  store UPPER(command) to command
  do case
    case command = "A"
*!! Logical constant converted.
      REPLACE new WITH .f.
      CONTINUE
    case command = "E"
      do eddit
    case command = "Q"
*!! Logical constant converted.
      store .f. to more
      case command = "C"
        continue
      case command = "P"
        do print
      case command = "D"
        do delete
  endcase
enddo
*!! Logical constant converted.
store .t. to more
store "4" to command
return
procedure eddit
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* eddit.prg
set format to
set menu on
set intensity on
edit
do delcheck
return
procedure add
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84

```

```
*
SET HEADING OFF
SET SAFETY OFF
* add.prg
close format
store " " to nn
*!! Logical constant converted.
store .t. to more
CLEAR
@ 4,6 say "DISPLAY KEYWORDS (y,n)"
set console off
wait to nn
set console on
if UPPER(nn) = "Y"
sels 3
use kwds
disp off wd while .not. eof()
use
sels 1
endif
set intensity on
set menu on
append
release
return
procedure maintain
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* maintain.prg
*!! Logical constant converted.
store .t. to more
do while more
    set format to mtain
    store " " to command
    read
    do case
        case command = "1"
            do verifyde
        case command = "2"
            do purge
        case command = "3"
            do dupchk
        case command = "5"
            CLEAR
            set DEVICE to screen
            clear ALL
            cancel
        case command = "6"
            set console off
            quit
        case command = "7"
    *!! Logical constant converted.
        store .f. to more
        case command = "4"
            do verifyne
    endcase
enddo
```

```

release more
store " " to command
return
procedure newtext
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  vl.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* newtext.prg
CLEAR
?
?
*!! There will be no automatic colon following this prompt string.
accept "OUTPUT FILE NAME (without extension) " to ofn
*!! There will be no automatic colon following this prompt string.
accept "OUTPUT DRIVE (a, b, c, etc.) " to ofd
CLEAR
store ofd+" ":"+trim(ofn) to ofn
set alternate to &ofn
set alternate on
set talk off
go top
do while .not. EOF()
if new
    ? trim(authors)
    ? trim(title)
    ? trim(reference)
    ? trim(keywords)
endif
skip
enddo
set alternate off
release ofn
release ofd
store " " to command
return
procedure locate
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  vl.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* locate.prg
private prompt1,prompt2,prompt3
CLEAR
go top
store " " to sno
store " " to command
store " " " to pra
?
?
?
?
? "      L. LOCATE ENTRY BY PRINCIPAL AUTHOR"
? "      S. SEARCH FOR ENTRY BY ANY FIELD"
?
? "      Select 1 or s"
set console off
wait to sno

```

```

set console on
store UPPER(sno) to sno
? "                                     "+sno
if sno = "L"
?
?
? " Enter up to the first five characters of the last name"
? " of the principal author."
accept to pra
*!! EOF() will be true if NO FIND, and RECNO() will equal BOTTOM, not 0.
* find &pra
locate for upper(authors)=upper(pra)
  if (EOF() .OR. BOF())
    ? "NO FIND"
    ?
    ? "STRIKE ANY KEY TO CONTINUE"
    set console off
    wait
    set console on
    go top
  endif
else
?
?
? "      a. author search"
? "      k. keyword search"
? "      t. title search"
? "      r. reference search"
?
? "      Choose a, k, t, or r"
set console off
set index to bibl,title,auth
wait to sno
set console on
store UPPER(SNO) to sno
? "                                     "+sno
?
if sno="K"
?"DISPLAY KEYWORDS(Y/N)"
SET CONSOLE OFF
wait to dkw
set console on
if upper(dkw)="Y"
  if .not. file("kwds.dbf")
    clear
    @ 2,10 say "SORRY, KEYWORD FILE NOT IN USE"
    @ 4,15 SAY "STRIKE ANY KEY TO CONTINUE"
    SET CONSOLE OFF
    wait
    set console on
  else
sele 3
use kwds
disp off wd while .not. eof()
use
sele 1
clear

```

```

endif
endif
clear
store " " to opnd
store " " to sg1
store " " to sg2
store " " to sg3
?
?"This routine will search through keywords for up to three
strings, as"
?
?"      string1 .AND. (string2 .AND. or .OR. string3)"
?
?"You will be asked for three strings and the logic .AND. or
.OR."
?
?"Null characters may be entered for strings, as they will return
TRUE"
?
?
?"Which logic for second operand: (a/o) for (AND/OR)?"
set console off
wait to opnd
set console on
clear
?
accept "Enter string1 " to sg1
?
accept "Enter string2 " to sg2
?
accept "Enter string3 " to sg3
clear
@ 4,20 say "SEARCHING"
if upper(opnd)="A"
  locate for sg1@(keywords) .and. (sg2@(keywords) .and.
sg3@(keywords))
endif
if upper(opnd)="O"
  locate for sg1@(keywords) .and. (sg2@(keywords) .or.
sg3@(keywords))
endif
?chr(7)
endif for sno
if sno="A"
accept " Enter exact search string <return>" to pra
store upper(pra) to pra
  locate for pra@(upper(authors))
?chr(7)
endif
if sno="T"
accept " Enter exact search string <return>" to pra
store upper(pra) to pra
  locate for pra@(upper(title))
?chr(7)
endif
if sno="R"
accept " Enter exact search string <return>" to pra
store upper(pra) to pra
  locate for pra@(upper(reference))
?chr(7)
endif

```



```

endif
do while .t.
  store code to rnum
do delcheck
  clear
  set format to saydata
store 'LOCATE MENU' to mode
store "          c. continue      r. return      p. print" to
prompt2
store " " to prompt1
store "          s. set flag" to prompt3
store " " to command
read
if upper(command)="C"
  continue
?chr(7)
if eof()
clear
?'          **** NO FIND ****'
?
?'          STRIKE ANY KEY TO CONTINUE'
SET CONSOLE OFF
wait
set console on
  goto rnum
endif
loop
endif
if upper(command)="S"
  do flag
endif
if upper(command)="R"
  release pra,sno,opnd,sg1,sg2,sg3
set index to bibl,title,auth
  return
endif
if upper(command)="P"
  do print
endif
enddo
set index to bibl,title,auth
RETURN
PROCEDURE INIT
*!!*          dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* init.prg
set talk off
set DELIMITER off
set bell off
set margin to 5
set console off
set DEVICE to print
set print on

```

```

? chr(15)
set print off
set DEVICE to screen
set console on
store " " to sno
store " " to dn
store " " to fn
CLEAR
?
?
?!! Unrecognized command.
?" -----"
?!! Unrecognized command.
?" |"
?!! Unrecognized command.
?" | BIBLIOGRAPHY |"
?!! Unrecognized command.
?" | DATABASE |"
?!! Unrecognized command.
?" |-----"
?
?
?
? " * * * CONFIRM THAT PRINTER IS ON * * * "
?
?
?
? " Enter Filename without Drive"
? " Designation or File Extension"
?
?
?
accept to fn
?
?" Have data and index files been"
?
? set console off
? wait to zz
? set console on
? if !(zz) = "Y"
? store "d" to dn
store 'd:'+fn to dn
use &dn
?" Name of index file keyed on year and author "
?" without extension....."
accept to bibl
?" Name of index file keyed on authors....."
accept to auth
?" Name of index file keyed on title....."
accept to title
set index to bibl,title,auth
if .not. file ('d:add.mem')
store ' ' to c
store 'ok' to del
store 1 to index

```

```

do while index <=79
  store c+' ' to c
  store index+1 to index
enddo
store c to mkeywords
store c+c to mreference
store c+c to mauthors
store c+c to mtitle
store ' ' to mflag
store '@' to flg
release c, index
save to add
*!! Unrecognized command.
endif
RETURN
PROCEDURE FLAG
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* flag.cmd
* sets flag symbol
if flag = " "
  CLEAR
  @ 10,1 say "Enter characters for flag "
  ?
  accept to flg
  replace flag with flg
else
  replace flag with " "
endif
RETURN
PROCEDURE PURGE
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* purge.prg
CLEAR
?
? "***** WARNING *****"
?
?
? "This will PERMANENTLY remove any deleted entries."
?
? "It is recommended that this command be aborted and that"
? "the COPY command be used under native dBASE [see manual]."
?
? "Type 'y' to continue. Type any other key to abort."
set console off
wait to next
set console on
if UPPER(next) = "Y"
  CLEAR
  ?
  ?
  ? "Records are now being removed from file."
  pack

```

```
endif
release next
RETURN
PROCEDURE DELCHECK
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* delcheck.prg
if DELETED()
    store "Deleted" to del
else
    store "Ok" to del
endif
RETURN
PROCEDURE FLAGCH
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* flagch.prg
* changes flag symbol
CLEAR
*!! There will be no automatic colon following this prompt string.
accept "ENTER NEW FLAG SYMBOL " TO flg
store " " to command
CLEAR
RETURN
PROCEDURE RENFLG
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF

* renflg.prg

CLEAR
@ 4,10 say "FLAGS ARE BEING REMOVED"
go top
do while .not. EOF()
if flag <> " "
```

```

    replace flag with " "
endif
skip
enddo
RETURN
PROCEDURE DELETE
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* delete.prg
if DELETE()
    recall
else
    delete
endif
RETURN
PROCEDURE FLAGDB
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* flagdb.prg
CLEAR
?
?
*!! There will be no automatic colon following this prompt string.
accept "OUTPUT FILE NAME (without extension) " to ofn
*!! There will be no automatic colon following this prompt string.
accept "OUTPUT FILE DRIVE (a, b, c, etc.) " to ofd
CLEAR
store ofd+" ":""+trim(ofn) to ofn
set talk off
copy to &ofn for flag <> "      "
release ofn
release ofd
store " " to command
RETURN
PROCEDURE REPORTS
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* reports.prg
*!! Logical constant converted.
store .t. to more
do while more
set format to rptn
store " " to command
read
do case
    case command = "1"
        do renflg
    case command = "2"
        do newtext
    case command = "3"
        do flagdb
    case command = "4"

```

```

        do flagtext
        case command = "5"
            do flagch
            case command = "6"
                *!! Logical constant converted.
                    store .f. to more
        endcase
    enddo
release more
store " " to command
RETURN
PROCEDURE DUPCHK
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
*dupchk.prg
*D.J.Roufa-8/16/84
save to dup
?"      Enter name of title index....."
accept to dex
set index to &dex
go top
*!! Logical constant converted.
store .t. to more
do while more .and. (.not. EOF())
CLEAR
?
?
?
*!! Unrecognized command.
?" SEARCHING FOR DUPLICATED RECORDS...."
store " * * S I M I L A R   F I L E S * * " to mode
store "C(ontinue) D(elte) P(rint) E(dit) Q(uit)" to prompt1
store substr(authors,1,5) to authors1
store SUBSTR(title,1,20) to title1
skip
if upper(SUBSTR(title,1,20)) = upper(title1) .and. (.not. EOF()) .and. upper(substr(
authors,1,5))=upper(authors1)
    do dupren
endif
enddo
set index to
rest from dup
store " " to command
RETURN
PROCEDURE DUPREN
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* dupren.prg
* d.j. roufa - 16-8-84
store authors to authors2
store title to title2
store reference to reference2
store CODE to recnum2

```

```

do delcheck
store del to del2
skip-1
store authors to authors1
store reference to referencel
store title to title1
store CODE to recnual
do delcheck
store del to dell
*!! Logical constant converted.
store .t. to more
do while more
    set format to shotwo
    store "C" to command
    store "A" to aorb
    read
    store UPPER(command) to command
    store UPPER(aorb) to aorb
    if aorb = "B"
        skip
    endif
    do case
        case command = "Q" .or. command="C"
*!! Logical constant converted.
            store .f. to more
            case command = "E"
                do eddit
            case command = "D"
                do delete
                if aorb = "A"
                    store "Deleted" to dell
                else
                    store "Deleted" to del2
                endif
            case command = "P"
                do print
        endcase
    if aorb = "A"
        skip
    endif
enddo
if command = "C"
*!! Logical constant converted.
    store .t. to more
endif
RETURN
PROCEDURE PRINT
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* print.prg
set console off
set DEVICE to print
set print on
if len(trim(authors)) > 120

```

```

? SUBSTR(authors,1,120)
? SUBSTR(trim(authors),121)
else
? trim(authors)
endif
if len(trim(title)) > 120
? SUBSTR(title,1,120)
? SUBSTR(trim(title),121)
else
? trim(title)
endif
if len(trim(reference)) > 120
? SUBSTR(reference,1,120)
? SUBSTR(trim(reference),121)
else
? trim(reference)
endif
? trim(keywords)
?
set print off
set DEVICE to screen
set console on
RETURN
PROCEDURE HELP
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* help.prg
CLEAR
store " " to sno
@ 5,10 say "?????????????????????????????????????????????????????????????"
@ 10,10 say "SORRY, YOU'LL JUST HAVE TO WORK IT OUT FOR YOURSELF!!!"
@ 15,20 say "Hit any key to continue..."
@ 20,10 say "?????????????????????????????????????????????????????????????"
set console off
wait to sno
set console on
release sno
RETURN
PROCEDURE FLAGTEXT
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* flagtext.prg
CLEAR
?
?
*!! There will be no automatic colon following this prompt string.
accept "OUTPUT FILE NAME (without extension) " to ofn
*!! There will be no automatic colon following this prompt string.
accept "OUTPUT FILE DRIVE (a, b, c, etc.) " to ofd
CLEAR
store ofd+"":'+trim(ofn) to ofn

```



```

set alternate to &ofn
set alternate on
set talk off
go top
do while .not. EOF()
if flag <> " "
    ? trim(authors)
    ? trim(title)
    ? trim(reference)
    ?
endif
skip
enddo
release ofn
release ofd
set alternate off
RETURN
PROCEDURE VERIFYDE
*!!*      dBASE CONVERT - dBASE III File Conversion Aid  v1.09 12/17/84
*
SET HEADING OFF
SET SAFETY OFF
* verifydel.prg
CLEAR
?
?
?
? " SEARCHING FOR DELETED RECORDS...."
locate for DELETE()
*!! Logical constant converted.
store .t. to more
do while more .and. (.not. EOF())
    set format to saydata
    store "VERIFY DELETES" to mode
    store "c)ontinue, e)dit, d)elc/recall, q)uit" to prompt2
    store " " to prompt1
    store " " to prompt3
    store " " to command
    do delcheck
    read
    store UPPER(command) to command
    do case
        case command = "D"
            do delete
        case command = "E"
            do edit
        case command = "Q"
*!! Logical constant converted.
            store .f. to more
            case command = "C"
                continue
            endcase
    enddo
*!! Logical constant converted.
store .t. to more
store "2" to command
RETURN

```

B I B L I O G R A P H Y
D A T A B A S E

*** CONFIRM THAT PRINTER IS ON ***

Enter Filename without Drive
Designation or File Extension

bibl

```
| BIBLIOGRAPHY |  
| DATABASE     |  
|-----|
```

*** CONFIRM THAT PRINTER IS ON ***

Enter Filename without Drive
Designation or File Extension

bibl

Name of index file keyed on year and author
without extension.....

bibl

Name of index file keyed on authors.....

auth

Name of index file keyed on title.....

title

AUTHORS: Richart, F.E., A. Brandtzaeg and R.L. Brown

TITLE: A Study of the Failure of Concrete Under Combined Compressive Stresses

REFERENCE: Bulletin No. 185, Engineering Experiment Station, University of Illinois, 1928

KEYWORDS:

FLAG:

```
=====
a)dd      b)ackward  d)elete/recall  e)dit          RECORD NO.      1
f)orward  h)elp      l)ocate         m)maintenance  STATUS:   Ok
p)rint    q(uit)     r)eport        s)et flag     OPTION
=====
```

**** MAIN MENU ****

FILE MAINTENANCE MENU

1. Verify Deleted Entries
2. Remove Entries Marked For Deletion
3. Flag Duplicate Entries
4. Verify New Entries
5. Quit To dBASE II
6. Quit To DOS
7. Return To Main Menu

PLEASE CHOOSE AN OPTION

..... WARNING

This will PERMANENTLY remove any deleted entries.

It is recommended that this command be aborted and that the COPY command be used under native DBASE (see manual).

Type 'y' to continue. Type any other key to abort.

R E P O R T O P T I O N S

1. Remove all flags.
2. Copy new entries to text file (with keywords).
3. Copy flagged entries to dBASE file.
4. Copy flagged entries to text file (without keywords).
5. Change flag character.
6. Return to main menu.

SELECT AN OPTION:

AUTHORS: Richart, F.E., A. Brandtzaeg and R.L. Brown

TITLE: A Study of the Failure of Concrete Under Combined Compressive Stresses

REFERENCE: Bulletin No. 185, Engineering Experiment Station, University of Illinois, 1928

KEYWORDS:

FLAG:

```
=====
a)dd      b)ackward  d)delete/recall  e)dit          RECORD NO.      1
f)orward  h)elp      l)ocate          m)maintenance  STATUS:   Ok
p)rint    q(uit)    r)eport         s)et flag      OPTION q
=====
```

**** MAIN MENU ****

Do cancelled
. quit

- L. LOCATE ENTRY BY PRINCIPAL AUTHOR
- S. SEARCH FOR ENTRY BY ANY FIELD

Select 1 or s

- L. LOCATE ENTRY BY PRINCIPAL AUTHOR
- S. SEARCH FOR ENTRY BY ANY FIELD

Select 1 or s

S

- a. author search
- k. keyword search
- t. title search
- r. reference search

Choose a, k, t, or r

- L. LOCATE ENTRY BY PRINCIPAL AUTHOR
- S. SEARCH FOR ENTRY BY ANY FIELD

Select 1 or s

S

- a. author search
- k. keyword search
- t. title search
- r. reference search

Choose a, k, t, or r

A

Enter exact search string <return>Swartz

This routine will search through keywords for up to three strings, as
string1 .AND. (string2 .AND. or .OR. string3)

You will be asked for three strings and the logic .AND. or .OR.

Null characters may be entered for strings, as they will return TRUE

Which logic for second operand: (a/o) for (AND/OR)?

Enter string1 beams

Enter string2 cantilever

Enter string3 double cantilever

AUTHORS: Swartz, S.E., K-K. Hu and G.L. Jones

TITLE: Compliance Monitoring of Crack Growth in Concrete

REFERENCE: Journal of the Engineering Mechanics Division, ASCE, Vol. 104, 1978,
pp. 789-800

KEYWORDS:

FLAG:

=====

c. continue	r. return	p. print	RECORD NO.	278
s. set flag			STATUS:	Ok
			OPTION	

LOCATE MENU

Appendix II

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APPLICABILITY OF FRACTURE MECHANICS METHODOLOGY TO
CRACKING AND FRACTURE O. (U) KANSAS STATE UNIV
MANHATTAN DEPT OF CIVIL ENGINEERING S E SWARTZ FEB 86

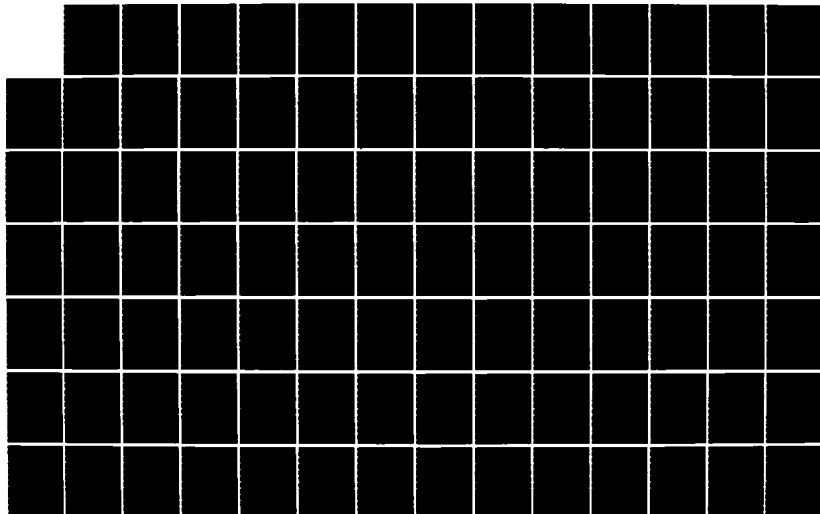
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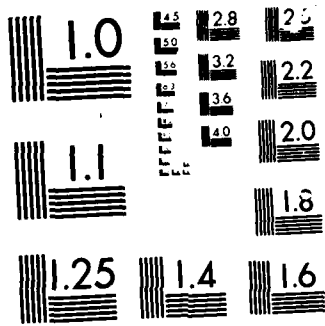
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MICROCOPY RESOLUTION TEST CHART

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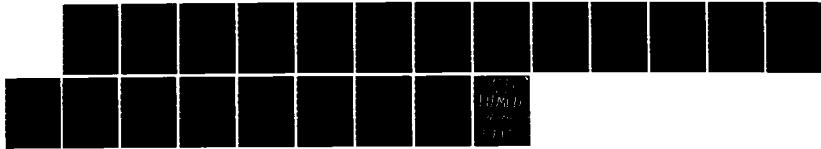
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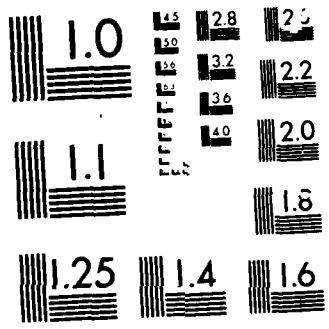
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