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DEPARTMENT OF THE ARMY NEW ENGLAND DIVISION, CORPS OF ENGINEERS 424 TRAPELO ROAD

WALTHAM, MASSACHUSETTS 02154

REPLY TO ATTENTION OF: NEDED

DEC 0 6 1979

Honorable Hugh J. Gallen Governor of the State of New Hampshire State House Concord, New Hampshire 03301

Dear Governor Gallen:

Inclosed is a copy of the Baker Floodwater Reservoir Site 11A Phase I Inspection Report, which was prepared under the National Program for Inspection of Non-Federal Dams. This report is presented for your use and is based upon a visual inspection, a review of the past performance and a brief hydrological study of the dam. A brief assessment is included at the beginning of the report. I have approved the report and support the findings and recommendations described in Section 7 and ask that you keep me informed of the actions taken to implement them. This follow-up action is a vitally important part of this program.

A copy of this report has been forwarded to the Water Resources Board, the cooperating agency for the State of New Hampshire and the owner of the dam.

Copies of this report will be made available to the public, upon request, by this office under the Freedom of Information Act. In the case of this report the release date will be thirty days from the date of this letter.

I wish to take this opportunity to thank you and the Water Resources Board for your cooperation in carrying out this program.

Sincerely,

Incl As stated MAX B. SCHEIDER Colonel, Corps of Engineers Division Engineer



BAKER FLOODWATER RESERVOIR SITE 11A

NH 00247

NHWRB 249.14

MERRIMACK RIVER BASIN WENTWORTH, NEW HAMPSHIRE

PHASE I INSPECTION REPORT NATIONAL DAM INSPECTION PROGRAM



NATIONAL DAM INSPECTION PROGRAM PHASE I - INSPECTION REPORT BRIEF ASSESSMENT

Identification No.: NH00247

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Name of Dam: Baker Floodwater Reservoir Site 11A

Town: Wentworth

County and State: Grafton, New Hampshire

Stream: Tributary to Baker River

Date of Inspection: May 16, 1979

Baker Floodwater Reservoir Site 11A is a 640 foot long 24 foot high earthen structure. There is one fill zone in the dam which includes a cutoff wall. Top width of the dam is 12 feet. The upstream and downstream embankments are on a 3 horizontal to 1 vertical slope. Appurtenant structures consist of a principal spillway, plunge pool stilling basin and emergency spillway. The principal spillway has two inlets, a low stage orifice and a high stage covered top spillway. The inlets discharge through the riser to a 2.5 foot diameter concrete pipe. The reservoir can be drained by a 12 inch diameter gated pipe. The dam construction was completed in November of 1971. Plans, design calculations and construction data were prepared by the Soil Conservation Service and are available for inspection.

The visual inspection revealed that the dam is in good condition. The visual inspection revealed random surface cracks and scaling of the concrete on the riser structure and a fallen tree in the channel downstream of the dam.

Based on the small size of the dam and its high hazard classification and in accordance with Corps of Engineers guidelines, the test flood inflow should be between 1/2 the Probable Maximum Flood (PMF) and the full PMF. A test flood inflow equal to 1/2 the PMF, which is equal to 1,580 cfs, was used. The routed test flood outflow of 670 cfs does not overtop the dam. With the water level at the top of the dam, the spillways will pass the routed test flood outflow. The hydraulic design calculations indicate that the principal spillway was designed for a 100 year frequency flood. The crest elevation of the dam was designed using a watershed runoff depth of 5.81 inches.

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There are no recommendations resulting from the Phase I Inspection. Remedial measures include the establishment of a downstream warning system in the event of emergency, the removal of a fallen tree in the downstream channel, and repair of the cracks and scaling of concrete on the riser structure.

The remedial measures are described in Section 7 and should be completed within two (2) years of the receipt of this report by the owner.



Fordon H. Slaney, Jr., P.E. Project Engineer

Howard, Needles, Tammen & Bergendoff Boston, Massachusetts



This Phase I Inspection Report on Baker Floodwater Reservoir Site 11A has been reviewed by the undersigned Review Board members. In our opinion, the reported findings, conclusions, and recommendations are consistent with the <u>Recommended Guidelines for Safety Inspection of Dams</u>, and with good engineering judgment and practice, and is hereby submitted for approval.

OOSEPH W. FINEGAN, JR., MEMBER Wayer Control Branch

Water Control Branch Engineering Division

CARNEY M. TERZIAN, MEMBER Design Branch Engineering Division

traph Q. Mr. Elroy

JOSEPH A. MCELROY, CHAIRMAN Chief, NED Materials Testing Lab. Foundations & Materials Branch Engineering Division

APPROVAL RECOMMENDED:

UE E. FRYAR

Chief, Engineering Division

PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation and analyses involving topographic mapping, subsurface investigations, testing and detailed computational evaluations are beyond the scope of a Phase I Investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued care and inspection can there by any chance that unsafe conditions be detected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test Flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aide in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential. TABLE OF CONTENTS

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NATIONAL DAM INSPECTION PROGRAM PHASE I INSPECTION REPORT BAKER FLOODWATER RESERVOIR SITE 11A

SECTION 1 PROJECT INFORMATION

1.1 General

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a. <u>Authority</u>. Public Law 92-367, August 8, 1972, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a National Program of Dam Inspection throughout the United States. The New England Division of the Corps of Engineers has been assigned the responsibility of supervising the inspection of dams within the New England Region. Howard, Needles, Tammen & Bergendoff has been retained by the New England Division to inspect and report on selected dams in the State of New Hampshire, Authorization and notice to proceed were issued to Howard, Needles, Tammen & Bergendoff under a letter of March 30, 1979 from John P. Chandler, Colonel, Corps of Engineers. Contract No. DACW33-79-C-0060 has been assigned by the Corps of Engineers for this work.

b. Purpose.

(1) To perform technical inspection and evaluation of non-Federal dams to identify conditions which threaten the public safety and thus permit correction in a timely manner by non-Federal interests.

(2) To encourage and prepare the states to initiate quickly effective dam safety programs for non-Federal dams.

(3) To update, verify and complete the National Inventory of dams.

1.2 Description of Project

a. Location. Baker Floodwater Reservoir Site 11A (Baker Dam Site 11A) is located on a tributary to the Baker River approximately 0.5 miles upstream of Buffalo Road in the Town of Wentworth, New Hampshire. The Dam is shown on U.S.G.S. Quadrangle Wentworth, New Hampshire, with approximate coordinates N43 -50'-54", W71 -53'-30", Grafton County, New Hampshire. The location of Baker Dam Site 11A is shown on the preceding page. b. Description of Dam and Appurtenances. Baker Dam Site 11A is an earthen embankment structure. Total length of the dam, according to existing drawings, is 640 feet. Maximum structural height is 35 feet with a 24 foot height from top of dam to the stream bed. According to the plans, there is one fill zone in the structure, which includes a cutoff wall. Top width of the dam is 12 feet and the embankment is on a 3 horizontal to 1 vertical slope both up and downstream.

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Appurtenant structures consist of a concrete riser and pipe principal spillway with a covered top inlet. There are two stages to the inlet structure, a low stage orifice and a high stage covered inlet. The riser discharges through a 2.5 foot diameter concrete pipe and a plunge pool type stilling basin. The emergency spillway is located on the right side of the dam and has a width of 140 feet. It is an excavated earthen structure with a vegetative cover. A 12 inch diameter pond drain pipe can be opened from the riser structure to lower the water level. There is a 12 inch diameter gate valve at the riser. A "WYE" fitting on the pond drain pipe, which is ungated, discharges to the low stage trash rack to serve as a secondary approach channel to the low stage intake.

Figures 1 and 2, located in Appendix B, show a plan of the dam and appurtenant structures. Photographs of each structure are shown in Appendix C.

c. <u>Size Classification</u>. Small (hydraulic height - 24 feet, storage - 355 acre-feet) classification based on height being less than 40 feet and storage being less than 1,000 acrefeet as given in Recommended Guidelines for Safety Inspection of Dams.

d. <u>Hazard Classification</u>. The potential for hazard posed by this dam is classified as high. Failure of this dam at maximum pool elevation (top of dam) would result in an average flood wave about 17 feet high through the reach studied, for 1.1 miles downstream of the dam. Four dwellings in the reach would be flooded, and the bridge at Buffalo Road would be overtopped.

e. <u>Ownership</u>. This dam is owned by the New Hampshire Water Resources Board, 37 Pleasant Street, Concord, New Hampshire.

f. Operator. This dam is maintained and operated by the New Hampshire Water Resources Board. Chairman of the Water Resources Board is Mr. George McGee, Sr.; Mr. Vernon Knowlton is Chief Engineer, Telephone No. 603/271-1110.

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g. <u>Purpose of Dam</u>. This dam is used for floodwater control. The normal pool is maintained by the low stage intake in the riser. The storage between the low stage outlet and the emergency spillway crest is used for floodwater control.

h. Design and Construction History. The construction of this dam was completed in November of 1971. Design and construction inspection of this dam were done by the Soil Conservation Service, Durham, New Hampshire. The construction contractor was Robie Construction Company, Inc.

i. <u>Normal Operating Procedures</u>. The normal pool is maintained by the low stage inlet on the riser. Under flood conditions, when the capacity of the low stage orifice is exceeded, the storage is utilized. The high stage outlet will reach maximum design discharge before the reservoir reaches the crest of the emergency spillway The dam does not require any manual operation in order to function.

1.3 Pertinent Data

a. Drainage Area. The area tributary to Baker Site 11A consists of 1.05 square miles of mountainous terrain. There is no development in the watershed except for a road and several dwellings. Maximum elevation is about 2,060 feet, MSL, and the crest of the dam is at elevation 681.5.

The area around the reservoir is mostly wooded. There are no cottages or dwellings along the shorline. A roadway passes to the right of the reservoir area. The pool area is swamp with many stumps and dead trees.

b. Discharge of Dam Site.

Outlet works for Baker Dam Site 11A consist of an (1)emergency spillway, a riser with a low stage orifice and a high stage covered top spillway, and a 12 inch pond drain pipe controlled by a gate. Invert of the pond drain is at 659.18 feet, MSL. Maximum discharge of the pipe when the reservoir is at the normal pool level of 663.5 feet is about The low stage orifice has two openings 6 inches by 7 9 cfs. inches in size set at invert 663.5. Capacity of the low stage inlet when the reservoir is at the crest of the high stage inlet (676.76) is 11 cfs. The high stage covered inlet crest set at elevation 676.76 has a capacity of 97 cfs when the water level is at the emergency spillway crest of The 140 foot wide emergency spillway has a crest of 678.5. elevation 678.5 when the water surface is at the top of dam (elevation 681.5) the spillway will have a capacity of 1,169 cfs.

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(2) There are no records available of maximum discharge at the site.

(3) The spillway and riser capacity with the water surface at the top of the dam is approximately 1,275 cfs at elevation 681.5.

(4) Spillway and riser capacity with the water surface elevation at the test flood elevation of 680.5 feet is approximately 670 cfs.

(5) The total project discharge at the test flood elevation of 680.5 feet is 570 cfs_{+}

c. Elevation (feet above MSL)

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- (1) Streambed at centerline of dam 657.5
- (2) Maximum tailwater unknown
- (3) Upstream portal invert pond drain 659.18
- (4) Normal pool 663.5
- (5) Full flood control pool 678.4
- (6) Spillway crest (riser crest) 676.76 (emergency

spillway) - 678.5.

- (7) Design surcharge 678.4
- (8) Top Dam 681.5
- (9) Test Flood Surcharge 680.5.
- d. Reservoir (miles)
- (1) Length of Maximum Pool .77
- (2) Length of Normal Pool .14
- (3) Length of Flood Control Pool .74
- e. Storage (gross acre-foot)
- (1) Normal Pool 9.3
- (2) Flood Control Pool 232
- (3) Emergency Spillway Crest Pool 238
- (4) Top of Dam 355

f. Reservoir Surface (acres) (1) Normal Pool - 4 (2) Flood Control Pool - 31 (3) Spillway Crest - 33 (4) Test Flood Pool - 37 (5) Top Dam - 37 q. Dam (1)Type - earth (2) Length - 640 feet (3) Height - 24 feet hydraulic 35 feet structural (4) Top Width - 12 feet (5) Side Slopes - upstream and downstream 3 horizontal to 1 vertical (6) Zoning - one fill zone (7) Impervious core - none (8) Cutoff - zone 1 fill Grout Curtain - none (9) (10)Other - none Diversion and Regulating Tunnel h. See Section j i. Principal Spillway (1) Type - concrete riser, covered top Length of Weir - total 15 feet (2) (3) Crest Elevation - 676.76 (4) Gates - outlet pipe 2.5 feet diameter (5) U/S Channel - none

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Emergency Spillway

(1) Type - earth

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- (2) Length of Weir 140 feet wide
- (3) Crest Elevation 678.5
- (4) Gates none
- U/S Channel Approach channel from reservoir is
 140 feet wide with 2½ to 1 side slopes
- (6) Downstream Channel Below the outlet structure for a distance of 150 feet the channel has grass lined banks, and a rip-rapped channel. Downstream of this section the channel enters a wooded area. Within the wooded area there are many large fallen trees in the channel.

j. <u>Regulating Outlets</u>. The normal level of the reservoir is controlled by two 7 inch by 6 inch orifice inlets set in the riser at invert elevation 663.5. There is a trash rack for each opening but no control gates. The 12 inch pond drain pipe set at invert 659.18 extends 17 feet into the reservoir from the riser, and has a trash rack at the intake. The pipe is controlled at the riser by a 12 inch diameter gate valve. A "wye" connection on the pond drain pipe discharges to the low stage trash rack and functions as a secondary low stage inlet.

SECTION 2 ENGINEERING DATA

2.1 Design

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A complete set of design data including layout, hydraulic design, foundation and embankment design, geology and soils reports, structural design, quantities and specifications are available for Baker Dam Site 11A. In addition, there are construction drawings available. Design of the dam was done by the Soil Conservation Service, Durham, New Hampshire.

2.2 Construction

The dam construction was completed in November of 1971. A complete record of construction documents were made available. These documents include: as-built plans, job diaries surveying records, test drilling logs, compaction test results, concrete tests and certificate of completion. Construction was by Robie Construction Co., Inc. and was inspected by the Soil Conservation Service, Durham, New Hampshire.

2.3 Operation

Normally, the pond drain line gate is closed. The normal level of 663.5 feet is maintained by the low stage orifice openings. The principal spillway riser and reservoir storage is designed to retard runoff from up to a 100 year frequency storm without discharge occurring in the emergency spillway (Crest 673.5).

2.4 Evaluation

a. <u>Availability</u>. Engineering data available for Baker Dam Site 11A consists of the information outlined in Sections 2.1 and 2.2. The plans, design data, and construction records are available at the offices of the Soil Conservation Service, Federal Building, Durham, New Hampshire 03824.

b. <u>Adequacy</u>. A complete set of design and construction data did allow for a definitive review within the confines of this Phase I - Inspection Report. Therefore, the adequacy of this dam is based on the design and construction data reviewed, visual inspection, past performance history and sound engineering judgement.

c. <u>Validity</u>. The field inspection indicated that the external features of Baker Dam Site 11A substantially agree with those shown on the available plans.

SECTION 3 VISUAL INSPECTION

3.1 Findings

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a. <u>General</u>. The field inspection of Baker Dam Site 11A was made on May 16, 1979. The inspection team consisted of personnel from Howard, Needles, Tammen & Bergendoff and Geotechnical Engineers, Inc. Inspection checklists, completed during the inspection, are included in Appendix A. At the time of inspection, the water level was approximately 1.0 foot above the invert of the low stage inlet. The upstream face of the dam could only be inspected above this water level.

b. Dam. Visual inspection of the dam indicated that it is in good condition.

The dam consists of an earth embankment about 640 feet long and 24 feet high. The embankment is a homogeneous fill of silty fine to medium sand with a cutoff trench extending to rock or an impervious silty till. There is a trench drain along the downstream toe.

There is an unpaved emergency spillway cut into the right abutment passing around the embankment.

A principal spillway consisting of an intake structure, concrete conduit and riprap stilling basin is located near the left abutment.

Upstream Slope. The upstream slope is 3 horizontal to 1 vertical and has a 10-foot-wide berm at approximately 16 feet below the crest. At the time of inspection, the pool was slightly below the level of the berm.

The upstream slope has a good grass cover, as shown in Photo No. 2.

Crest. The crest of dam is 12 feet wide and is grass covered, as shown in Photo No. 4. No significant misalignment of the crest was observed.

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Downstream Slope. The downstream slope is 3 horizontal to 1 vertical. The slope, shown in Photo No. 3, is grass covered and in good condition.

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There is a riprap-lined gutter at the juncture of the downstream slope and the left abutment, which discharges into the outlet works stilling basin.

The trench drain beneath the downstream toe discharges into a riprap-lined pool at the outlet end of the principal spillway. The 12-inch diameter drainpipes were clear and unobstructed.

c. Apurtenant Structures. Visual inspection of the concrete riser principal spillway structure, auxiliary earth spillway and outlet works structure did not reveal any evidence of stability problems with respect to sliding and overturning. The concrete riser structure generally appeared to be in good condition except for some concrete staining, minor random cracks and surface scaling. The principal spillway trash racks are in good condition.

The concrete riser structure consists of three functional elements; a principal spillway with low and high stage inlets, a vertical transition and a closed discharge conduit. The riser structure is located in the embankment.

Field inspection revealed that the riser structure appeared to be in good condition except for rust and water staining, surface scaling, and random surface cracks, see Photos No. 7, 8, 9. The trash racks for the low and high stage intakes consist of standard shape angles and grating. Both trash racks assembles are in good condition.

The principal spillway structure has a riprap-lined approach channel which parallels the upstream toe. The portion of the riprap which could be observed is shown in Photo No. 6 and is in good condition.

The pond drain inlet structure, pipe and gate were under water at the time of inspection. The gate control mechanism located on the top of the riser appeared to be in good operational order.

The 2.5 foot diameter principal spillway discharge pipe and concrete support bedding shown in Photos. No. 13 and 14 appeared to be in good condition. The portion of the riprap above the level of the plunge pool shown in Photo No. 12 appears to be in good condition.

The emergency spillway is about 140 feet wide and clear of obstructions with the exception of some low brush. Photos No. 15 and 16 show the emergency spillway upstream and downstream from the dam axis.

d. <u>Reservoir Area</u>. The area around the reservoir is mostly wooded. There are no cottages or dwellings along the shoreline. A roadway (Buffalo Road) passes along the right shore of the reservoir area. The pool area is swamp with many stumps and dead trees.

e. <u>Downstream Channel</u>. Below the outlet structure and plunge pool the channel is riprapped with grassed banks for a distance of about 150 feet. Downstream of this section the channel enters a wooded area. Just into this portion, there is a large fallen tree in the channel. Photo No. 11 shows a portion of the channel. With the exception of minor cattail growth, the channel is in good condition.

3.2 Evaluation

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Visual examination indicates the dam is in good condition. The inspection of the dam revealed the following:

(a) The riser structure has some surface sealing, and random surface cracks.

(b) A large fallen tree in the downstream channel.

SECTION 4 OPERATIONAL PROCEDURES

4.1 Procedure

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Baker Dam Site 11A is used for floodwater control. Under normal operating procedures the normal pool level is maintained by the low stage orifice opening in the riser. Flood events up to a 100 year frequency are retarded by the reservoir storage between the normal pool and the emergency spillway crest. The emergency spillway is utilized only with events greater than a 100 year frequency.

4.2 Maintenance of Dam

The dam is inspected on an annual basis by the New Hampshire Water Resources Board and the Soil Conservation Service. Maintenance is undertaken as a result of the inspection on an as needed basis. The dam is visited on a regular basis (approximately monthly) by representatives of the owner to perform regular maintenance

4.3 Maintenance of Operating Facilities

Maintenance of the outlet works is performed as in Section 4.2.

4.4 Description of Warning Systems

There are no warning systems in effect for this facility.

4.5 Evaluation

The current operation and maintenance procedure for this facility appear to be adequate to insure that any problems encountered can be remedied within a reasonable period of time. However, the owner should establish a warning system to follow in the event of flood flow conditions or imminent dam failure.

SECTION 5 HYDROLOGY AND HYDRAULIC ANALYSIS

5.1 Evaluation of Features

a. <u>General</u>. Baker Dam Site 11A is an earthen embankment dam 640 feet long with a hydraulic height of 24 feet. The dam is constructed with one fill zone and a earth fill core. Appurtenant works consist of a two stage riser and a 2.5 foot diameter concrete pipe which discharges to a plunge pool type stilling basin. An emergency spillway 140 feet wide is located on the left side of the dam. There is a 12 inch diameter gated pond drain pipe which discharges to the riser structure.

The dam is used for floodwater control. The dam is classified as small in size having a height of 24 feet and maximum storage of 355 acre-feet.

b. Design Data. According to the Soil Conservation Service design data, this dam is constructed to retard flood flows of up to a 100 year frequency storm without utilizing the emergency spillway. The design flood control elevation is 678.4 feet or 0.1 feet below the emergency spillway crest. Total runoff for this condition is 3.14 inches during a six hour Type IIB storm. The crest elevation of the dam was designed using a watershed runoff depth of 5.81 inches. The structure is classified as having a "B" hazard which is defined as "being located in a predominately rural and agricutural area, where failure may cause damage to isolated homes, main highways or major railroads or cause interruption of use or service of relatively important public utilities."

c. Experience Data. There are no records available of maximum discharge at the dam site.

d. <u>Visual Observations</u>. No evidence of damage to any portion of the project from overtopping was visible at the time of inspection.

e. Test Flood Analysis. Detailed design data is available for this dam, and the basic conditions are noted above in Paragraph b. The hydrologic evaluation was preformed using information gathered by field investigation, watershed characteristics and Probable Maximum Flood (PMF) guide curves prepared by the Corps of Engineers. In accordance with Corps of Engineers guidelines, the high hazard classification and small size of the dam warrent a test flood magnitude ranging from 1/2 the Probable Maximum Flood to the full PMF. A test flood of 1/2 the PMF was used as the hazard classification is on the lower end of the range with four buildings affected. A test flood inflow of 1,580 cfs is based on a drainage area of 1.05 square miles in mountainous terrain.

The routed test flood outflow was determined in accordance with Corps of Engineers guidance for Estimating Effect of Surcharge Storage on Maximum Probable Discharge and the hydraulic characteristics of the dam. Discharge through both the primary spillway and emergency spillway was considered. The routing was started with the water surface at the normal pool elevation. The routed test flood outflow was determined to be approximately 670 cfs. As the maximum capacity of the spillways is 1,274 cfs there will be a freeboard of 1.0 feet.

f. Dam Failure Analysis. The impact of failure of the dam was assessed using the "Rule of Thumb" Guidance for Estimating Downstream Dam Failure Hydrographs prepared by the Corps of Engineers. The breach discharge was estimated with the water surface at the crest of the dam and a breach width equal to 40 percent of the total length of the dam. The downstream hydrograph is a sum of the breach discharge and the maximum spillway capacity. Prior to the breach of dam, the downstream river stage would be about 4.5 feet the spillways at a full capacity of 1,274 cfs. Breach of dam would result in an additional 50,600 cfs for a total of about 51,900 cfs. The downstream flood stage was estimated through three reaches for a total distance of 1.3 miles from the dam to the Baker River. The flood wave would be about 20.8 feet high at the dam and 14.1 feet at the Baker River. Four dwellings along this reach would be affected. Two dwellings would be flooded by about 6 feet and the other two would be flooded by about 3 feet. In addition, a shack will be totally inundated and a bridge at Buffalo Road will be overtopped by about 11 feet.

SECTION 6 STRUCTURAL STABILITY

6.1 Evaluation of Structural Stability

a. <u>Visual Observations</u>. The visual inspection of Baker Dam Site 11A did not disclose any immediate stability problems.

b. <u>Design and Construction Data</u>. Design drawings and construction specifications exist and indicate the dam is a homogeneous embankment composed of silty fine to medium sand. The dam has a cutoff trench extending to bedrock on an impervious silty till. There is a trench drain along the downstream toe which extends up both abutments.

An emergency spillway cut into the right abutment passes around the embankment.

A review of the construction data available indicates that the dam and appurtenant structures were constructed according to the plans and specifications.

c. <u>Operating Records</u>. There are no operating records available for this facility.

d. <u>Post-Construction Changes</u>. There is no record of post-construction changes.

e. <u>Seismic Stability</u>. The dam is located in Seismic Zone 2 and in accordance with the recommended Phase I guidelines does not warrant seismic analysis.

SECTION 7 ASSESSMENT, RECOMMENDATIONS AND REMEDIAL MEASURES

7.1 Dam Assessment

Ν

a. Condition. The visual inspection of Baker Floodwater Reservoir Site 11A indicated the dam is in good condition.

(1) Random surface cracks and scaling of concrete on the riser structure.

(2) A fallen tree across the channel downstream of the dam.

The hydraulic analysis reveals that the spillways can pass the routed test flood without overtopping the dam.

b. Adequacy of Information. A complete set of design and construction data did allow for a definitive review with the confines of this Phase I - Inspection Report. Therefore, the adequacy of this dam is based on the design and construction data review, visual inspection, past performance history and sound engineering judgement.

c. <u>Urgency</u>. This dam is in generally good condition. The remedial measures described in Section 7.3 should be accomplished within 2 years of the receipt of this Phase I-Inspection Report by the owner.

d. <u>Necessity of Additional Investigation</u>. No additional investigation is needed to complete the Phase I Inspection.

7.2 Recommendations

There are no recommendations resulting from the Phase I Inspection.

7.3 Remedial Measures

(a) Devise a warning system to follow in the event of emergency conditions.

(b) Remove the fallen tree from the downstream channel.

(c) Repair surface cracks and scaling on the riser structure.

(d) Establish a system such that the reservoir level can be monitored during periods of intense rainfall.

(e) The periodic inspection should be continued on no less than a biennial frequency.

7.4 Alternatives

D

There are no practical alternatives to the remedial measures described in Section 7.3.

APPENDIX A

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INSPECTION CHECKLIST

VISUAL INSPEC PARTY ORG	TION CHECK LIST A - 1 ANIZATION
QJECT Site 11A, Baker Dam	
	TIME2:30 PM
	WEATHER Fair
	W.S. ELEV. 664.5U.S DN.S
RTY:	,
G. Slaney HNTB	~~~ 0
S. Mazur HNTR	· · · · · · · · · · · · · · · · · · ·
D. LaGatta GEL	Q.
<u> </u>	10.
	10
PROJECT FEATURE	INSPECTED BY REMARKS
Dam	D. LaGatta, C. Osgood
Spillway, Outlet Works	S. Mazur
and Downstream Channel	G, Slaney
	······································

ROJECT BAKER SITE NO. 11A DAM	DATE May 16, 1979	
ROJECT FEATURE Earth Embankment	NAME D. P. LaGatta	
OISCIPLINE Geotechnical Engineer	NAME C. E. Osgood	
AREA EVALUATED	CONDITION	
AM EMBANKMENT		_
Crest Elevation	681.5	•
Current Pool Elevation	664.5	
Maximum Impoundment to Date	Unknown.	-
Surface Cracks	None observed.	. • ·
Pavement Condition	No pavement.	• • •
Movement or Settlement of Crest	None observed.	-
Lateral Movement	None observed.	<u>.</u>
Vertical Alignment	No misalignment observed.	
Horizontal Alignment	No misalignment observed.	1
Condition at Abutment and at Concrete Structures	Good.	-
Indications of Movement of Structural Items on Slopes	No structural items.	
Trespassing on Slopes	No evidence of trespassing.	
Sloughing or Erosion of Slopes or Abutments	None observed.	
Rock Slope Protection - Riprap Failures	No failure observed in gutter or	1
Unusual Movement or Cracking at or near Toes	None observed.	
Unusual Embankment or Downstream Seepage	None. Water level low.	<u>l</u>
Piping or Boils	None observed.	
Foundation Drainage Features	Left and right 12" drainpipe exits are unobstructed.	
Toe Drains	None.	
Instrumentation System Vegetation	None.	

PROJECT Site 11A. Baker Dam	DATE May 16, 1979	•
PROJECT FEATURE Intake Channel/Struc	ture NAME D. LaGatta, C. Osgood	.
DISCIPLINE <u>Geotechnical/Structural</u>	NAME S. Mazur	
AREA EVALUATED	CONDITION	
OUTLET WORKS - INTAKE CHANNEL AND INTAKE STRUCTURE		_
a. Approach Channel	Riprap is in good condition as	•
Slope Conditions	Observable.	_
Bottom Conditions	™ot observable.	- - 21.
Rock Slides or Falls	None.	
Log Boom	None.	
Debris	Some debris at low trash rack.	
Condition of Concrete Lining		
Drains or Weep Holes		
b. Intake Structure	Galvanized trash rack and concrete	•
Condition of Concrete	and low stages of inlets are in good condition. Bottom water	
Stop Logs and Slots	release structure was under water.	
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PROJECT Site 11A, Baker Dam	DATE May 16, 1979	
PROJECT FEATURE Control Tower	NAME G. Slaney	
DISCIPLINE Structural/Hydraulic/Eng.	NAME S. Mazur	
AREA EVALUATED	CONDITION	
OUTLET WORKS - CONTROL TOWER		
a. Concrete and Structural	Bottom water release structure	
General Condition	(pond drain) consists of inlet structure and 12" ID cast iron	
Condition of Joints	Pond drain structure and control	
Spalling	gate were under water.	
Visible Reinforcing		
Rusting or Staining of Concrete		
Any Seepage or Efflorescence		
Joint Alignment		
Unusual Seepage or Leaks in Gate Chamber	-	
Cracks		
Rusting or Corrosion of Steel		
b. Mechanical and Electrical	Mechanically operated gate and	
Air Vents	control mechanism are housed in riser tower structure. Gate is	
Float Wells	structure. Gate and control	
Crane Hoist	operational condition.	
Elevator		
Hydraulic System		
Service Gates		
Emergency Gates		
Lightning Protection System		
Emergency Power System		
Wiring and Lighting System		
PROJECT Site 11A, Baker Dam	DATEMay 16, 1979	
--	---	
PROJECT FEATURE Spillway/Outlet Works	Conduit NAME G. Slaney	
DISCIPLINE <u>Structural/Hydraulic</u>	NAME S. Mazur	
AREA EVALUATED	CONDITION	
OUTLET WORKS - TRANSITION AND CONDUIT		
General Condition of Concrete	At the time of inspection, outlet	
Rust or Staining on Concrete	Riser discharge channel consists of 30" reinforced concrete pipe	
Spalling	which is placed on concrete bedding. Discharge conduit	
Erosion or Cavitation	appears to be in good condition.	
Cracking		
Alignment of Monoliths		
Alignment of Joints		
Numbering of Monoliths		

PROJECT FEATURE Outlet Structure (Cha	NAME D. LaGatta, C. Osgood
DISCIPLINE Structural/Hydraulic/Ge	otechnical NAMES. Mazur, G. Slaney
AREA EVALUATED	CONDITION
OUTLET WORKS - OUTLET STRUCTURE AND OUTLET CHANNEL	
General Condition of Concrete	Concrete outlet pipe and concrete support bedding are in good con-
Rust or Staining	dition. Some water staining.
Spalling	None
Erosion or Cavitation Visible Reinforcing	Some erosion at end of supporting bedding
Any Seepage or Efflorescence	None.
Condition at Joints	Good.
Drain Holes	None.
Channel	Riprap for 30 feet from outlet.
Loose Rock or Trees Overhanging Channel	None.
Condition of Discharge Channel	Clear to edge of woods 150 feet from outlet.

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FERIODIC INSI 201	
PROJECT Site 11A, Baker Dam	DATE <u>May 16, 1979</u>
PROJECT FEATURE Outlet Works/Spillwa	ay NAME D. LaGatta, C. Osggod
DISCIPLINE_Structural/Hydraulic/Geo	technical NAME S. Mazur, G. Slaney
AREA EVALUATED	CONDITION
OUTLET WORKS - SPILLWAY WEIR, APPROACH AND DISCHARGE CHANNELS	
a. Approach Channel	
General Condition	Grass covered.
Loose Rock Overhanding Channel	None.
Trees Overhanging Channel	None.
Floor of Approach Channel	Grass covered.
b. Weir and Training Walls	This facility has two spillway
General Condition of Concrete	two inlets and auxiliary earth
Rust or Staining	ment. Both spillways are in
Spalling	Water stain. Some surface scaling.
Any Visible Reinforcing	None.
Any Seepage or Efflorescence	None.
Drain Holes	None.
c. Discharge Channel	
General Condition	Good, grass covered.
Loose Rock Overhanging Channel	None.
Trees Overhanging Channel	None.
Floor of Channel	Grass coverel.
Other Obstructions	None.

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PROJECTSite 11A, Baker Dam	DATE May 16, 1979	
PROJECT FEATURE Service Bridge	NAME	
DISCIPLINE	NAME	
AREA EVALUATED	CONDITION	
OUTLET WORKS - SERVICE BRIDGE		
a. Super Structure	This facility has no service	
Bearings	bridge.	
Anchor Bolts		-
Bridge Seat		
Longitudinal Members		
Under Side of Deck		
Secondary Bracing		
Deck		
Drainage System		
Railings		
Expansion Joints		
Paint		
b. Abutment & Piers		
General Condition of Concrete		
Alignment of Abutment		٣
Approach to Bridge		
Condition of Seat & Backwall		

APPENDIX B

ENGINEERING DATA

- 1. LIST OF DESIGN, CONSTRUCTION AND MAINTENANCE RECORDS
- 2. PAST INSPECTION REPORTS
- 3. PLAN AND DETAILS

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AVAILABLE ENGINEERING DATA

1. A set of drawings (25 sheets), dated November 1970 showing plans and details of the dam and appurtenant structures.

2. Design Data: including layout, hydraulic design, geology and soils reports, structural design, quantities and specifications.

3. Construction Data: including as-built plans, job diarys, surveying records, test drilling logs, compaction test results, concrete tests and certificate of completion.

All of the above are on file with the U.S.D.A. Soil Conservation Service, Federal Building, Durham, New Hampshire 03824.

PAST INSPECTION REPORTS

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State of New Hampshire

WATER RESOURCES BOARD

37 Pleasant Street Concord, N.H. 05304

Turn one not allo

September 18, 1978

Mr. Keith MacPherson Soil Conservation Service Federal Building Durham, New Hampshire 03824

Dear Mr. MacPherson:

This letter is to inform you of the prevailing conditions at two of the Baker River System Flood Control Sites.

Site No. 6

- 1- Trash racks have been cleared of debris.
- 2- All bushes and tree sprouts on the dam have been pulled, cut or sprayed.
- 3- The concrete is still spalled in several areas of the channel wall and has broken away from the railing posts. The Board feels that it is your agency's responsibility for this repair.
- 4- To date we have not received your agency's recommendation of corrective action regarding the ponding against the right bank channel wall for our review. During this year's inspection this item was of some concern to Ray Winninger.

5- The traffic signs and riprap have been removed from the outlet channel.

Site No. 11-A 249.14

- 1- The bushes and tree sprouts on the dam and in the emergency spillway have been pulled, cut or sprayed.
- 2- The roadway guardrail repair is to be completed by the Town and not by us.

A more complete report will follow indicating all the work accomplished this year with respect to this year's O & M maintenance field inspection reports.

Very truly yours,

George Mc Lee Sr. George Mc Lee Sr. Chairman

GMM:GLK:paf

MAINTENANCE CHECKLIST FOR PL 566 FLOOD CONTROL STRUCTURES

This maintenance checklist is a guide for determining the maintenance required for Public Law 566 flood control structures in New Hampshire. It doesn't take the place of experience and judgment and is not inclusive. Items of a difficult nature to check, such as principal spillway conduit condition, are not included. Intensive checks of these items are necessary at proper intervals. Review of As Built drawings, the design folder, structure history, and previous maintenance reports should be part of the inspection. Prompt maintenance is a vital part of safe and effective operation.

 \therefore Except where otherwise indicated, completion of this form may be facilitated \therefore by ranking maintenance items on a 1 to 4 basis where

- 1 = satisfactory
- 2 = satisfactory, but check carefully at next inspection
- 3 = requires maintenance this season
- 4 = requires immediate attention.

SHED Ba	ker	SI1	CE114	DA'	re <u>6-</u>	13-78	
ECTED BY Gary Kerr,	Lyall Milliga	n (WRB); M	ike Danne	hy, Nicl	k Luhtala.	Rav Wennin	ger
GENERAL ITEMS						(SGS	,
Access Road.	• •	• •	•	• •	•	. N/A	
Site Fencing.	• •	• •	•	• •	•	. N/A	
Traffic Conditi	.ons	• •	•	• •	•	• _1	
Trash Control.		•••	•	• •	•	$\frac{1}{1}$	
						فسينفسنكم	
COMMENTS	·····		~				
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Timber stand at Debris and slas Sediment level COMMENTS	: reservoir. sh in relation to	low stage	e inlet	• •	• •	$\begin{array}{c} 1 \\ 1 \\ 1 \\ 1 \\ \end{array}$	
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7				SOIL	CONSERVALL	IN SERVICE	
7					EPARTMENT	OF AGRICULTU	RE

Report riprap and vegetation an erosion condition under Items 4 and 5.)	nd Emergency Spillways ₁ Other Dam Dika left right () ()	
Sliding or sloughing Holes (rodent and other) (check especially at embankmen Excessive settlement (embankmen Cracks	$\frac{1}{1} - \frac{1}{1} - \frac{1}$	
Traverse Longitudinal Seepage <u>2</u> /	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	
Piping <u>/</u>		
COMMENTS		
·····		
IPRAP ·		•
IPRAP	Displ. Loss Loss Erosion Break- of of of of down Rock Spalls Bedding Found. of Rock	
Dam Upstream berm Principal Spillway Outlet	Displ. Loss Loss Erosion Break- of of of of down Rock Spalls Bedding Found. of Rock	
Dam Upstream berm Principal Spillway Outlet Embankment Gutters	Displ. Loss Loss Erosion Break- of of of of down Rock Spalls Bedding Found. of Rock $\frac{1}{3} \frac{1}{1} \frac{1}{1} \frac{1}{1} \frac{1}{1}$	
Dam Upstream berm Principal Spillway Outlet Embankment Gutters left right	Displ. Loss Loss Erosion Break- of of of of down Rock Spalls Bedding Found. of Rock $\frac{1}{3} \frac{1}{1} \frac{1}{1} \frac{1}{1} \frac{1}{1}$ $\frac{1}{1} \frac{1}{1} \frac{1}{1}$	
Dam Upstream berm Principal Spillway Outlet Embankment Gutters left right Emergency Spillway	Displ.LossLossErosionBreak- downofofofofdownRockSpallsBeddingFound.of Rock $\frac{1}{3}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$	
Dam Upstream berm Principal Spillway Outlet Embankment Gutters left right Emergency Spillway location location	Displ.LossLossErosionBreak- downofofofofdownRockSpallsBeddingFound.of Rock $\frac{1}{3}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$	
Dam Upstream berm Principal Spillway Outlet Embankment Gutters left right Emergency Spillway location location Waterways	Displ.LossLossErosionBreak- downofofofofdownRockSpallsBeddingFound.of Rock $\frac{1}{3}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{3}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{2}$	
Dam Upstream berm Principal Spillway Outlet Embankment Gutters left right Emergency Spillway location location Waterways location	Displ. Loss Loss Erosion Break- down of of of of down Rock Spalls Bedding Found. of Rock $\frac{1}{3}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{3}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{2}$	
Dam Upstream berm Principal Spillway Outlet Embankment Gutters left right Emergency Spillway location location Waterways location location Outlet Channel	Displ. Loss Loss Erosion Break-of of of of of of down Rock Spalls Bedding Found. of Rock $\frac{1}{3}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{3}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{2}$ $\frac{1}{2$	
Dam Upstream berm Principal Spillway Outlet Embankment Gutters left right Emergency Spillway location location Waterways location location Outlet Channel Other	Displ. Loss Loss Erosion Break- down of of of of down Rock Spalls Bedding Found. of Rock $\frac{1}{3}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{3}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{2}$ <	
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Dam Upstream berm Principal Spillway Outlet Embankment Gutters left right Emergency Spillway location location Naterways location location Outlet Channel Other COMMENTS A few stones at PS out	Displ. Loss Loss Erosion Break- of of of of down Rock Spalls Bedding Found. of Rock	
Dam Upstream berm Principal Spillway Outlet Embankment Gutters left right Emergency Spillway location location Vaterways location location Outlet Channel Other COMMENTS A few stones at PS out back to the side of the plunge	Displ. Loss Loss Erosion Break- of of of of down Rock Spalls Bedding Found. of Rock	

1/Looking downstream. Z/Check especially at downstream face of embankments.

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	Dam	Emerg Spill left	gency lways <u>right</u> /	<u>Dike</u>	Outlet Channel	Water _way_	Other ()
Condition of stand (including need for lime and fertilizer)	_1		1				
Undesirable vegetation		<u> </u>					
Frosion 2/		<u>-</u>					
Sedimentation							
Condition of planting		<u> </u>					
Pest control							
Fire control							
File concioi					-		
COMMENTS Emergency spillwa	v - wet	and	aquatic	veget	ation com	ming in	1.

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Vegetation looks good.

STRUCTURAL, & OTHER DRAINS EMBANKMENT.

(in inches above invert)	Without any obstruction	none none		
Turbidity of Discharge (yes, no)	With any obstruction Without any obstruction			
Condition of Protective Coating	Outside Inside	$\frac{1}{1} \frac{1}{1}$		*******
Obstruction in Flow (yes, no)	•	no no		•
Animal Guard Condition Outlet Condition		$\frac{1}{1}$ $\frac{1}{1}$		
Retarding Pool Elevation (ft. msl) or0	<u>.1</u> (ft.) sbo	ve top of	_orifi
Other	-			
COMMENTS				
·				
				<u></u>

____RISER

Ladders: inside and out

Concrete:

Trashracks: low and high stage

Manhole:

Gate: including lifting device, stem, guides, disc

Safety Items:

Condition of protective coating___; Corrosion__; Damaged parts___; Loose__; Other___.

Caution Be extremely careful when using ladders. Check condition before using.

Ladders are sometimes broken, loose, corroded,

Cracking 1; Spalling 1; Other deterioration 1; Excessive movement (check joint at riser and conduit)__; Other__.

Condition of protective coatings 1; Corrosion 1; Damaged parts 1; Condition of fastenings ; Need of gratings due to beaver__; Safety condition (protruding fastenings, sharp edges, etc.)__; Other__.

Condition of protective coatings__; Corrosion __; Damage__; Lock operable__; Other__.

Condition of protective coating__; Corrosion __; Damaged parts__; Condition of fastenings__; Stem alignment__; Lubrication__; Operation__; Other___.

Condition of warning signs__; Condition of safety equipment__; Other__.

COMMENTS Did not go down riser. Should check interior of riser, gate

and or slippery. Use safety harness.

operation and conduit at suitable intervals.

(specify)		
N/A		
oncrete:	Cracking : Spalling : Other deterioration	
inside and out	; Excessive movement (check joints);	
	Waterstops; Joint sealant; Other	
Tashracks:	Condition of protective coatings; Corrosion	
low and high stage	; Damaged parts ; Condition of fasten-	
·	Safety condition (protruding fastenings, sharp	
	edges, etc.); Other	u ••• ••
Sates:	Condition of protective coating; Corrosion	
including lifting	; Damaged parts ; Condition of fasten-	
disc, flap	Lubrication; Wood decay; Other	••••
Structure Drainage:	Report under "Embankment and Other Drains"	
Structure, Railing, States. Barriers.	Condition of protective coating_; Corrosion : Damaged parts : Condition of Fasten-	
etc.	ings; Wood decay; Safety condition	
	(protruding fastenings, sharp edges, etc.)	la l
	; ······	
Safety Items:	Condition of warning signs ; Condition of	
	Jarecy equipment, Jener	
COMMENTS		1
·		
	· · · · · · · · · · · · · · · · · · ·	
MANNEL	. •	
Stream obstructions	• • • • • • • • • <u>2</u>	
Sediment bars controlled.		· · ·
Plunge pool stability		
Pish habitat appurtenances Moran Report under "Ri	S	
COMMENTS LITTLE Vegetat	ion in outlet channel.	
· · · · · · · · · · · · · · · · · · ·		
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MAINTENANCE CHECKLIST FOR PL 566 FLCOD CONTROL STRUCTURES

This maintenance checklist is a guide for determining the maintenance required * ivr Public Law 366 flood control structures in New Hampshire. It doesn't take . the place of experience and judgment and is not inclusive. Items of a difficult * nature to check, such as principal spillway conduit condition, are not included. Etensive checks of these items are necessary at proper intervals. Review of Built drawings, the design folder, structure history, and previous maintenance reports should be part of the inspection. Prompt maintenance is a vital part of safe and effective operation.

Nicept where otherwise indicated, completion of this form may be facilitated by ranking maintenance items on a 1 to 4 basis where

l = satisfactory

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- 2 = satisfactory, but check carefully at next inspection
- 3 = requires maintenance this season
- manuface immediate attention

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CENERAL	TTEVS		<u>zocy</u>						<u></u>
GENERAL	<u> </u>								
Acces	ss Road.	• •	•	•	•	•	•	•	/_
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RESERVO Timbe Debri Sedin COMME	PIR er stand at r is and slash. ment level in ENTS	reservoin relatio	r. on to lov		e inlet			NSERVATI PARTMENT	ON SERVICE

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MBANKMENT AND EXCAVATED SL	OPES					
Report riprap and vegetation an erosion condition under Items 4 and 5.)	d Dam Di	Emer Spil ke left	gency lways right 1/	Other		
Sliding or sloughing Holes (rodent and other) (check especially at embankmen Excessive settlement (embankmen	<u></u> <u></u> ts) ts)_/				^ 	
Cracks Traverse Longitudinal			 			•
Seepage <u>2</u> / Piping <u>2</u> /						
IPRAP		_	_			
	Displ. of <u>Rock</u>	Loss of Spalls	Loss of <u>Bedding</u>	Erosion of Found.	Break- down of Rock	
Upstream berm Principal Spillway Outlet Embankment Gutters	/			` `		
left right Emergency Spillway	<u>+</u>					
location location Waterways						
location location Dutlet Channel	<u> </u>					
COMMENTS <u>REMOUS</u> ROLKS	 	 	 	IDER_	 <u>D:uT:65</u> 1	
OF P.A. SPILLINA-17	120					
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VEGETATION

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D		Dam	Emerg Spill <u>left</u>	ency ways right 1/	Dike	Outlet Channel	Water way	Other ()
•	Condition of stand (including need for lime	1				ЦA		
	and fertilizer)							
<u>.</u>	Undesirable vegetation	3		2		1		
	Drainage (surface)	NA				NA		
-	Erosion 2/	T		1				
	Sedimentation			-				
	Condition of planting	A10					فسيقصيده	
	Condicion of plancing	CE		CLE		ALA		
	Pest control							·
· .	Fire control							

COMMENTS POPLARS GROUDING ON DAM

EMBANKMENT, STRUCTURAL, & OTHER DRAINS

Depth of Flow	With any obstruction		
(in inches above invert)	Without any obstruction	0	
Turbidity of Discharge (yes, no)	With any obstruction Without any obstruction	NO NO	
Condition of Protective Coating	Outside Inside		
Obstruction in Flow (yes, no)		NO NO	
Animal Guard Condition Outlet Condition			
Retarding Pool Elevation ((ft. msl) or	(ft.) abc	ve
Other			
COMMENTS			
<u></u>			

RISER

Ladders: inside and out

Condition of protective coating___; Corrosion___; Damaged parts___; Loose___; Other___.

and conduit) ; Other .

etc.)__; Other__.

ladders. Check condition before using.

Be extremely careful when using

Ladders are sometimes broken, loose, corroded,

Cracking___; Spalling___; Other deterioration

Condition of protective coatings ; Corrosion

condition (protruding fastenings, sharp edges,

Condition of protective coatings___; Corrosion ___; Damage___; Lock operable___; Other__.

Condition of protective coating___; Corrosion

; Damaged parts ; Condition of fastenings ; Stem alignment ; Lubrication ;

___; Damaged parts___; Condition of fastenings ___; Need of gratings due to beaver__; Safety

; Excessive movement (check joint at riser

Concrete: inside and out

Trashracks: low and high stage

Manhole:

Cate: including lifting device, stem, guides, disc

Safety Items:

Condition of warning signs__; Condition of safety equipment__; Other__.

COMMENTS WAR WILL CHECK RISER & AFRIASEMANLES AT LATER DATE

Operation ; Other .

Caution

and or slippery. Use safety harness. IMPACT BASIN, SAF, BOX INLET, & MISCELLANEOUS CONCRETE STRUCTURES

(specify) CRADLE

Concrete: inside and out Cracking /; Spalling /; Other deterioration _/; Excessive movement (check joints)__; Waterstops__; Joint sealant__; Other__.

5

Trashracks: low and high stage Condition of protective coatings_; Corrosion _; Damaged parts_; Condition of fastenings_; Need of gratings due to beaver_; Safety condition (protruding fastenings, sharp edges, etc.)_; Other__.

Condition of protective coating__; Corrosion
; Damaged parts; Condition of fastenings_; Stem alignment_; Operation_;

Gates: including lifting device, stem, guides, disc, flap

Report under "Embankment and Other Drains"

Lubrication ; Wood decay ; Other .

Structure, Railing, Grates, Barriers, etc.

Structure Drainage:

Safety Items:

Condition of protective coating__; Corrosion __; Damaged parts__; Condition of Fastenings__; Wood decay_; Safety condition (protruding fastenings, sharp edges, etc.) __; Other__.

Condition of warning signs__; Condition of safety equipment__; Other__.

COMMENTS

CHANNEL

Stream obstructions.	•	•	•	•	•	•	•	•	•	. 3_
Debris in stream	•	•	•	•	•	•	•	•	•	•
Sediment bars controlle	ed.	•	•	•	•	•	•	•	•	• 1
Plunge pool stability.	•	•	•	•	•	•	•	•	•	• _/_
Fish habitat appurtenar	ices	•	•	•	•	•	•	•	•	•
Riorap Report under	"Rip	rap"	(item	4)						

COMMENTS REMOUL CATO-NINE FALLES" FROM CHANNEL







APPENDIX C

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PHOTOGRAPHS

FOR LOCATION OF PHOTOS, SEE FIGURE 1 LOCATED IN APPENDIX B



PHOTO NO. 1 - View of reservoir area from dam.

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PHOTO NO. 2 - View of upstream face of dam from right abutment.



PHOTO NO. 3 - View of downstream face of dam from right abutment.



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PHOTO NO. 4 View of dam crest from right abutment.



PHOTO NO. 5 - View of dam crest from left abutment.



PHOTO NO. 6 - View of principal spillway and riser.



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PHOTO NO. 7 - View of right side of principal spillway.

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PHOTO NO. 8 - View of left side of principal spillway.



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PHOTO NO. 9 - View of low stage intake trash rack.



PHOTO NO. 10 - View of high stage trash rack.



PHOTO NO. 11 - View of outlet works and discharge channel from dam.



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PHOTO NO. 12 - View of outlet works and foundation drain pipes.





from axis of dam.

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PHOTO NO. 16 - Downstream end of emergency spillway as seen from axis of dam.

APPENDIX D

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HYDROLOGIC AND HYDRAULIC COMPUTATIONS

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•	HNTB	Made by	RY	Date 6/18/79	JOD NO 595=	5-11-28	
		Checked by	НМ	Date 7/ 3 74	Sheet No.	/	
•	Baker River Dam	<u> # //,</u>	A				
D	Hydraul	(CS 2	e Hydro	legy			•
	Baker River Dam Sit to the Baker	<u>e No.</u> River	11A: Local in the T	ated on.	$= t_{nb_{n}}$	tary h, N.H.	
	<u>Classification</u> : Si. Ha	ze zard.	Small High				
	<u> 3asie Data</u> : Dra Upst	maye Feam l	Area: 1.0 Casin: M	25 Jani	7745	Ini	
	Reservoir	: N	formal Bol	: elev-6 Stos9	264.0 2012-ft 715		
K			TJI: of Dam	elev a Stor 3	5×10 5×10 555 2000	<i>1</i> 2	
	D=m : E. Lei He	arth ngth íght	6401t 74 ft				•
	Spillway	is F E,	Riser Crast Length of Cr mer. Spillway	676.75 est 15 t Crest 67	(* 18:5		
•			Width 14	0-ft.			
•	See Appendix	Bfo	Plan of	Dam			

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Date 6/18/73 JOONO 5965-11-08 Made by КY Date _____ Sheet No. Checked by 411 HOWARD NEEDLES TAMMEN & SERGENDOF Baker #11A Step 1 _alculation of Test Flood Inflow Π Classification Size Small Hazard High Hydrologic Audeline Recommends 1/2 PMF to PMF use EPMF as Height 24 feet in mid-range of classification range of 12 = 40 but ind Wasard is on lower nange & range - 4 duellings affected. With Mountainous Curve FMF 3000 csm (max reammended value) Test Flood = 3000LSM × 1.05 1/2= 1580 Inglow As that is a flood control reserver the postion of the stringe above the normal pool can be used to store a portion of the PMF Storage @ 9 2022 - ft normal for 238 acce- et @ mestof emerancy Apillaray 229 aneft wailable for storage of PMF. Notume of 1/2 PMF = 19/12 mg × 640 mie × 1.05 mie × 1/2 = 532 20 att

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Ţ	HNTB	Made by K	Date	6/18/79 JOD NO 5	765-11-08	
		Checked by +	Date Date	e	3	
	Baker #11A	<u> </u>				
_	Star 2 Colouin	L	<	1		
	Step Z Calculat	ion of Te	st F_CUr	charge		
	Stage-Dischard	Re Curv	'e			
_		0	- D a	G .		
•	ft. above	Riser	D Emerge	ncy C Crest	- +- /	
	Elev. Cmerg. Spillway	Pipe Flow	Spille	ay of Dam	10721	
-	6785 0	98			92	
	679.33 . 83	97	100	>	197	
	679.95 1.45	101	300	>	401	
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	C. Computed as flow	over a k	proadcrests	ed weir		•
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Qpz=Q	$P_{i} \times \left(1 - \frac{57}{9.5}\right)$			
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642 34 623 44	78	5.55 7.30	490 365	
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From	figure 2	Juty	low 67	0 40
		Stage	2.0 40	Lose Spilles=x
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MARO NEEDLES TAMMEN & SERGENDOFF	Checked by	!	Date 7/19/70	Sheet No.	
Baker #11A		·········			
		_			
Estimate of D	ownstream	1 Da	mage		P .
Tep Keservoir ST	orage				
at top of dam	$\eta - Fley.$	6815	· 14		
	Storage.	355	Pare-It.		
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step 2 Breach Outfle	ω				
	- ,3/2				
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For Roles #	MMEN & BERGENOOFF Checked by HL! Date Sheet No. 6
LDAKEF I	\ <i>H</i>
Step 4	Downstream Stage 5=355 son (2
	Qp. = 51,900 eps Jength 6000 PE
ALU	Stage, 20.8 ft 2020;= 3600/22
1900 Liet of	VI = <u>36007t × 1900 ft</u> = 157 inne-14 5 <u>355</u> 43560 te jane
60007	Reach length OK
	EP2 mind = 51,900(1-157) = 28,950=1
	Stagiz = 16.7 pt = 2226 pt
	$V_2 = \frac{2226 \times 1900}{43550} = 97 \text{ love}(t)$
	Vare = 127 acre-Pt
	Rpz=51,900(1-127)=33,33024
B	Stage, = 17.6 it area, = 2500 fo
i cride 1000 xe	V, = 2500 × 2000 = 115 met
st 6000 ($Q_{P_{-}} = 33320 / 1 - \frac{115}{15} = 775500 \cdot 10$
	$\left(1_{2}\right) = \left(3_{35}\right) = \left(3_{5}\right)^{2} = \left(3$
	1805 < 2000 (2) F
	Vz= 43560 = 83 sou (t
	Vave = 99. acre (t
	Qpz= 33,330 (1- 355.) 24,030 efs
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HNTB	Made by 7	2 Date	5/24/79 JOONO 59	5-11-08
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Baker#11A				
QP. = 24,100	ef.s			
le Stage	= 15.6 ft	area,=1.	913 H	
- 02000# 1 = 1	913 × 2100	- 91 0	<i>Q</i> ,	
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U. D. DLIMMAN U. MUNUULIUNE SCS - 319 SOIL CONSERVATION SERVICE 11-22-6 6.58 1 1-1-1-NH-6E8 12-11-HYDROGRAPH COMPUTATION EMERGENCY SPILLUNG HYDROGRAPH WATERSHED OR PROJECT _____ BATER RIVER STATE _____ STATE ______ STRUCTURE SITE OR SUBAREA SITE 11.5 DR. APEA 1.05 SQ. MI. T 1.8 HR. RUNOFF CONDITION NO. 22 RUNOFF CURVE NO. (38). STORM DISTRIB. CURVE B. HYDROGRAPH FAMILY NO. 31x7.0 = 0.92x7.0 =RAINFALL: POINT 7.0 IN. AREAL 6.66 IN. STORM DURATION 6 HR. TH: 0.7 TC Q 3.14 IN. COMPUTED T 1.26 HR. To 4.45 HR .. $(T_{a} + T_{b}):$ COMPUTED 3.53; USED 4.0. REVISED T_ 1.11. $q_p = \frac{484}{REV.T_p} = \frac{457.74}{CFS}$ CFS. Qqp = <u>10.37.62</u> CFS. $t(COLUMN) = (t/T_p) REV. T_p$, $q(COLUMN) = (qc/q_p) Qq_p$ t . LINE ۵ LINE t G. LINE t ٥ t CFS CFS ND. HOURS E0. HOURS NO. HOURS CFS 8.63 0.00 7.99 6.00 21 41 1 0.40 2.31 8.39 5.75 22 2 42 2.00 0.20 63.25 8.7% 3____ 23 43 1.20 201.24-9.19 1.22 4 24 44 1.60 575.05 9.59 25 0.00 5 45 2.00 6 627.18 26 46 2.40 646.73 7 27 47 2.80 570.74 8 28 48 3.20 491.67 9 29 49 3.60 425.54 Eg= 5,398.5% 10 30 50 0= 4= (= ?) 4.00 31.9.47 31 11 51 64577 6.40 336.40 12 32 52 4.80 301.90 0.40 4 5.39.27 13 33 53 645×1.05 5.19 142.76 14 34 54 12 - 2159.31 5.59 157.56 15 35 55 6.7.25 5.99 76.32 16 36 56 Q = 3.19 " 6.39 53.19 17 37 57 6.79 31.63 18 38 58 100x 0.05 1.59 % 7.19 20.13 .39

U. S. DEPARTMENT OF AGRICULTURE 5CS-319 6.58 SOIL CONSERVATION SERVICE NH-685 PPP 12-11-6 HYDROGRAPH COMPUTATION FREEDED & ITTOMS - 154 WATERSHED OR PROJECT ______ RIVER ______ STATE ______ N. 14. F STRUCTURE SITE OR SUBARLA STE 117 DR. APEA 1.05 SQ. MIL T 1.8 HR. RUNOFF CONDITION NO. IT RUNDEF CURVE NO. 43. STORM DISTRIB. CURVE 3. HYDROGRAPH FAMILY NO. 3 $7 \times 7.5 = 0.952 \times 10.5 =$ STORM DURATION 4 HE. RAINFALL: POINT 10.5 IN. AREAL 10.0 IN. $T_{D} = 0.7 T_{C}$ 0.5.76 III. COMPUTED T 1.26 HR T. 4.87 HR. $(T_{a} + T_{p})$: COMPUTED 3.27 : USED 4.0. REVISED T_ 1.22. $q_p = \frac{100}{100000} = \frac{2100000}{24000000} CFS. Qq_p = -2.400000 CFS.$. $t(COLUMN) = (t/T_p) REV. T_p,$ $q(COLUMN) = (qc/q_p) Qq_p,$ ł LDVE 11135 a t a: LINE t a t HOURS CES NO. ROURS CFS NO. HOULS CES NO. 14.90 0.00 21 8.78 2.50 41 9.44 7.45 9.22 9.93 22 2 42 127.35 23 2.92 9.63 4.97 з 43 1.32 502.99 10.10 2.43 4 24 44 1.75 - 972, 29 || 10.54 5 25 0.00 45 1 . 2 . 72 6 25 46 - 64 1,117. 2.2 7 . 27 47 5-5-63 2.07 8 28 48 2.51 249.09 9 29 49 3.95 734.21 EQ = 9.322.55 10 30 50 11= (57) 2.39 432.05 11 31 51 645A 59.1.95 4.53 12 32 52 2 0.44 - 7.3.22.55 5.57 524.37 13 33 53 64: ×1.05 017.58 14 34 54 4,101.92 6.15 575.53 15 35 55 311.25 4.59 1. 16 36 5**6** 7.03 21.24 4. = 6.015 " 17 37 57 7 47 يتي بر که جن 18 38 58 5.26 = 1.68% 1007 7.91 34.76 30 59

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•• CURVE NO. 49., RAINFALL 10.40, Q 3.78 1.05 DPATTASC AREA APRIL 1964 HANNING'S 'N' VALUE 0.012 BASE FLOW IS TOOL CSM (1.00 CFS). ľ 0.00 0.00 0.00 0.00 0.00 .00 6.0J 143.03 .00 **c**o. 5 00° 40 0.10 COMDULT SIZE IS 30. INCHES. DISCHARGE TALLE GIVEN. PRINCIPAL SPILLWAY RUNTING 510%555 5.00 5.00 63.60 177.00 177.00 154.00 01.0 0.9 * 2 0 272.00 12.05 5.78. 2.45 00*997 95*509 601107313 677-50 670-01 67.4.70 572.00 676.76 677.00 67.1.45 67% 01 013.30 674.34 SIFE 5 BAKER RIVER WALERSHED 11A LEVGTH OF PIPE 110. 68., RAIVFALL 2 CURVE ND. TC 1-20 1 8 .

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APPENDIX E

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INFORMATION AS CONTAINED IN THE NATIONAL INVENTORY OF DAMS



