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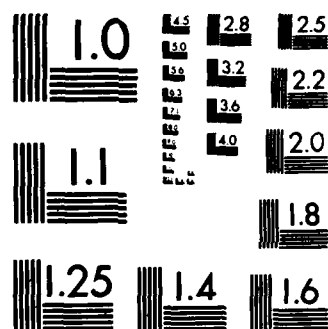
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AD-A142 710

CONNECTICUT WESTERN COASTAL AREA
STAMFORD - NEW CANAAN, CONNECTICUT

LAUREL RESERVOIR DAM
CT 00049

PHASE I INSPECTION REPORT
NATIONAL DAM INSPECTION PROGRAM



DEPARTMENT OF THE ARMY
NEW ENGLAND DIVISION, CORPS OF ENGINEERS
WALTHAM, MASS. 02154

AUGUST 1978

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20. ABSTRACT (Continue on reverse side if necessary and identify by block number) The Laurel Reservoir Dam is a linear concrete gravity structure, 1,950 ft. long with a 100 ft. spillway located on the west end of the dam. An earthen embankment on the downstream portion of the dam begins 10 ft. wide top width and a 2:1 slope. The maximum spillway capacity at top of dam is 15 percent of the peak inflow rate of the test flood. Therefore, the test flood cannot be passed by the spillway without overtopping the dam. The overflow will be 2.6 ft. above the top of the dam.		



DEPARTMENT OF THE ARMY
NEW ENGLAND DIVISION, CORPS OF ENGINEERS
424 TRAPELO ROAD
WALTHAM, MASSACHUSETTS 02154

REPLY TO
ATTENTION OF:
NEDED

AUG 29 1979

Honorable Ella T. Grasso
Governor of the State of Connecticut
State Capitol
Hartford, Connecticut 06115

Dear Governor Grasso:

I am forwarding to you a copy of the Laurel Reservoir Dam Phase I Inspection Report, which was prepared under the National Program for Inspection of Non-Federal Dams. This report is presented for your use and is based upon a visual inspection, a review of the past performance and a brief hydrological study of the dam. A brief assessment is included at the beginning of the report. I have approved the report and support the findings and recommendations described in Section 7 and ask that you keep me informed of the actions taken to implement them. This follow-up action is a vitally important part of this program.

A copy of this report has been forwarded to the Department of Environmental Protection, the cooperating agency for the State of Connecticut. In addition, a copy of the report has also been furnished the owner, Stamford Water Company, 103 Summer Street, Stamford, Connecticut 06901.

Copies of this report will be made available to the public, upon request, by this office under the Freedom of Information Act. In the case of this report the release date will be thirty days from the date of this letter.

I wish to take this opportunity to thank you and the Department of Environmental Protection for your cooperation in carrying out this program.

Sincerely yours,

Max B. Scheider

MAX B. SCHEIDER
Colonel, Corps of Engineers
Division Engineer

Incl
As stated

LAUREL RESERVOIR DAM

CT 00049

CONNECTICUT WESTERN COASTAL AREA

GREENWICH, CONNECTICUT

PHASE I INSPECTION REPORT

NATIONAL DAM INSPECTION PROGRAM

[illegible]

PHASE I REPORT
NATIONAL DAM SAFETY PROGRAM

Name of Dam: LAUREL RESERVOIR DAM

State Located: Connecticut

County Located: Fairfield County

Stream: Rippowam River

Date of Inspection: 15 JUNE 1978

BRIEF ASSESSMENT

The Laurel Reservoir Dam is a linear concrete gravity structure, 1,950 feet long with a 100 foot spillway located on the west end of the dam. An earthen embankment on the downstream portion of the dam begins 10 feet below the top of the dam, having a 10 foot wide top width and a 2:1 (horizontal to vertical) slope.

Based on a visual inspection of the site, review of available information and past performance of the dam, the dam is judged to be in good condition.

The maximum spillway capacity at top of dam is 15 per cent of the peak inflow rate of the test flood. Therefore, the test flood cannot be passed by the spillway without overtopping the dam. The overflow will be 2.6 feet above the top of the dam.

It is recommended that detailed engineering investigations be undertaken by the owner to determine methods for obtaining additional spillway capacity. Concrete surfaces affected by spalling should be repaired.

Due to the potential for overtopping and the high hazard classification, it is recommended that a definite plan for around the clock surveillance be implemented during periods of heavy rains and a formal warning system be developed by the owner.

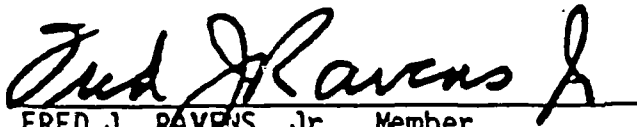

S. Giavara, P.E.
Principal

Registered, CT 7634

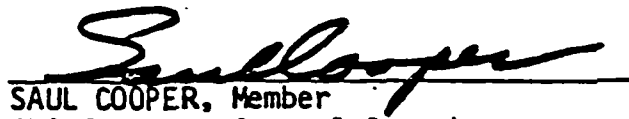
This Phase I Inspection Report on Laurel Reservoir Dam has been reviewed by the undersigned Review Board members. In our opinion; the reported findings, conclusions, and recommendations are consistent with the Recommended Guidelines for Safety Inspection of Dams, and with good engineering judgment and practice, and is hereby submitted for approval.



CHARLES G. TIERSCH, Chairman
Chief, Foundation and Materials Branch
Engineering Division

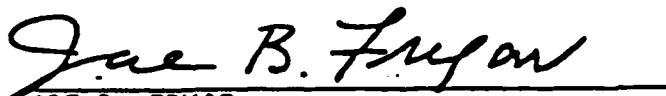


FRED J. RAVENS, Jr., Member
Chief, Design Branch
Engineering Division



SAUL COOPER, Member
Chief, Water Control Branch
Engineering Division

APPROVAL RECOMMENDED:



JOE B. FRYAR
Chief, Engineering Division

PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation, and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued care and inspection can there be any chance that unsafe conditions be detected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aide in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

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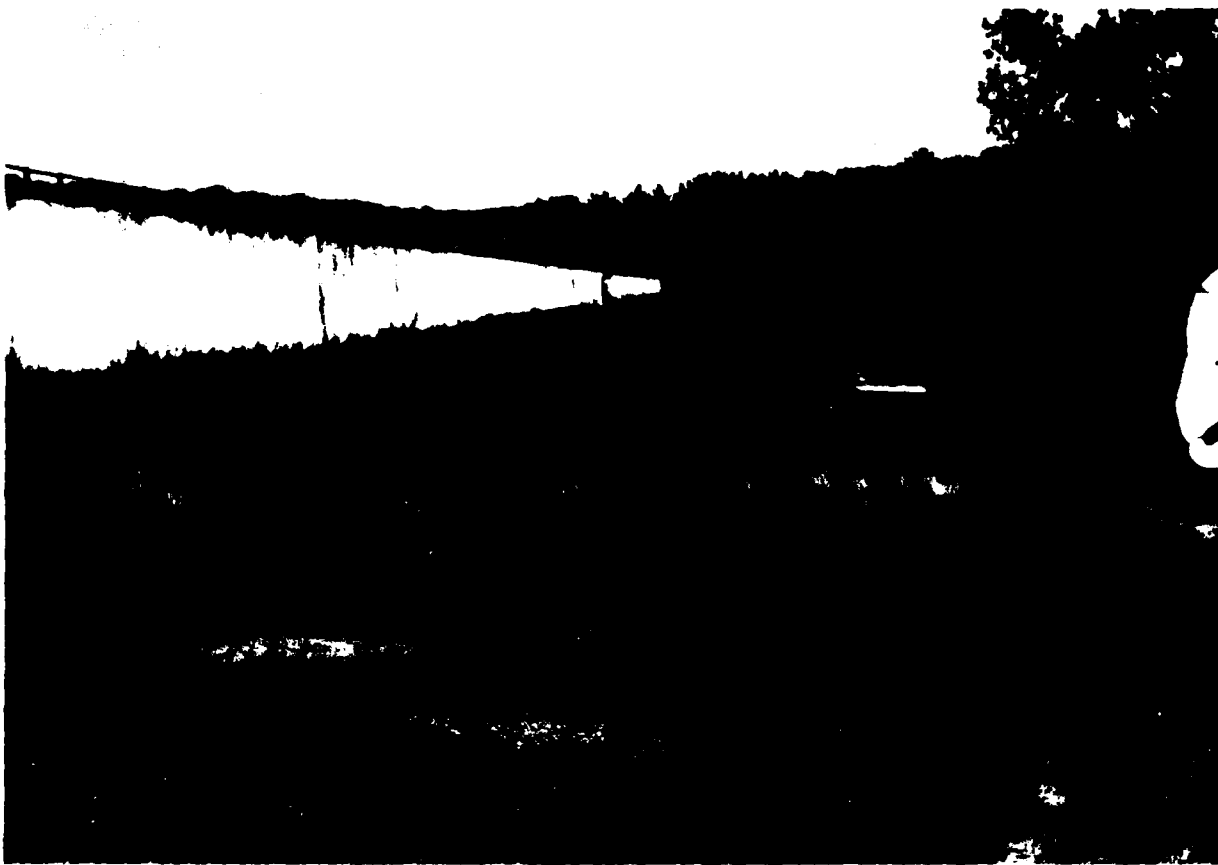
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7. ASSESSMENT, RECOMMENDATIONS AND REMEDIAL MEASURES

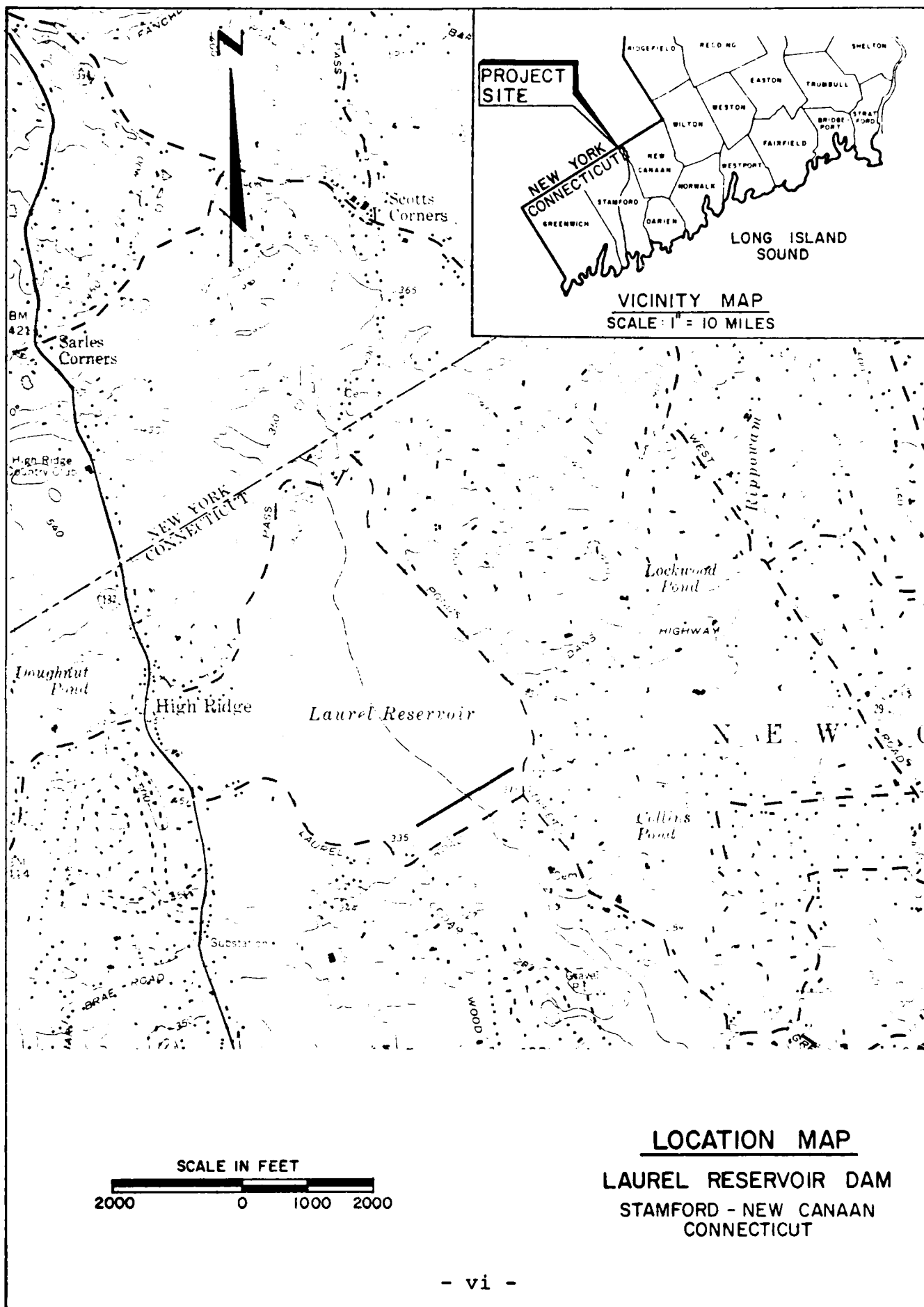
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IV	Engineering Data - Check List
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LAUREL RESERVOIR DAM



PHASE I INSPECTION REPORT
LAUREL RESERVOIR DAM CT 00049

SECTION 1 - PROJECT INFORMATION

1.1 GENERAL:

a. Authority. Public Law 92-367, August 8, 1972, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a national program of dam inspection through the United States. The New England Division of the Corps of Engineers has been assigned the responsibility of supervising the inspection of dams within the New England Region. Flaherty Giavara Associates, P.C. has been retained by the New England Division to inspect and report on selected dams in the State of Connecticut. Authorization and notice to proceed was issued to Flaherty Giavara Associates, P.C. under a letter of 25 April 1978 from Ralph T. Garver, Colonel, Corps of Engineers. Contract No. DACW33-78-C-0309 has been assigned by the Corps of Engineers for this work.

b. Purpose:

1) Perform technical inspection and evaluation of non-federal dams to identify conditions which threaten the public safety and thus permit correction in a timely manner by non-federal interests.

2) Encourage and assist the States to initiate quickly effective dam safety programs for non-federal dams.

3) To update, verify and complete the National Inventory of Dams.

1.2 DESCRIPTION OF PROJECT:

a. Description of Dam and Appurtenances. The Laurel Reservoir Dam is a linear concrete gravity structure, 1,950 feet long, with a 100-foot spillway located at the west end of the dam. An earthen embankment on the downstream portion of the dam begins 10 feet below the top of the dam, having a 10-foot wide top width and having a 2:1 (horizontal to vertical) slope. The top of the dam is about eleven feet wide with a railing on the upstream side. The dam is approximately 42 feet above the streambed. An intake structure is located in the center of the dam, providing 6 sluice gates for varying take off points from the reservoir. A 30-inch blow off and 8-inch drain discharge to the Rippowam River.

b. Location. Laurel Reservoir Dam is located on the Rippowam River, within the Connecticut western coastal area. The Town lines of Stamford and New Canaan bisect the reservoir. The dam is approximately 2 miles north of North Stamford.

c. Size Classification. The size classification may be determined by either storage or height, whichever gives the larger size category. Based on both the storage capacity and height of the dam, the size classification is intermediate. The applicable guidelines indicate that for an intermediate category the storage in acre-feet for the impoundment must be greater than or equal to 1,000 and less than 50,000. The height must be greater than or equal to 40 feet and less than 100 feet. The top of dam storage is 7,150 acre-feet and the height of dam is 45 feet.

d. Hazard Classification. The dam is designated as having a high hazard potential. About 100 houses are located in the floodplain. The dam is located in an area where failure may cause serious damage to homes, and to industrial and commercial facilities. Additionally, excessive damage to the Merritt Parkway and the Connecticut Turnpike could also be expected.

e. Ownership. The dam is owned by the Stamford Water Company, of Stamford, Connecticut.

f. Purpose of Dam. The dam was constructed to impound water for the Laurel Reservoir. The Laurel Reservoir forms part of the Stamford Water Company's water distribution system and supplies the people of Stamford.

g. Design and Construction History. The dam was designed in 1922 by Albert B. Hill, Consulting Engineer. Construction history is unknown.

h. Normal Operational Procedures. The dam is operated to supply water to the system. Water taken off at the Laurel Reservoir is chlorinated and piped to the North Stamford pump station 2 miles south of the dam. A 36-inch blow off is operated periodically, and an 8-inch drain is open to maintain flow of the Rippowam River.

1.3 PERTINENT DATA:

- a. Drainage Area - 13.4 sq. miles
- b. Discharge at Dam Site -
Maximum Known Flood Unknown
Warm Water Outlet Unknown
Div. Tunnel Low Pool Outlet Not Applicable
Diversion Tunnel Outlet Not Applicable
Gated Spillway Not Applicable
Ungated Spillway at Max. Pool 4,000 CFS
Total Spillway Cap. at Max. Pool 4,000 CFS
- c. Elevation (above M.S.L.) -
Top of Dam 315
Max. Design Pool Unknown
Full Flood Control Pool Not Applicable
Recreation Pool Not Applicable
Spillway Crest Ungated 310
Upstream Portal Invert. Div. Tunnel Not Applicable
Downstream Portal Invert. Div. Tunnel Not Applicable
Streambed at Centerline of Dam 265
Maximum Tailwater 270+
- d. Reservoir -
Length of Max. Pool 3,700 Ft.
Length of Recreation Pool Not Applicable
Length of Flood Control Pool Not Applicable
- e. Storage -
Recreation Pool Not Applicable
Flood Control Pool Not Applicable
Design Surcharge Not Available
Top of Dam 7,150 Acre-Feet
- f. Reservoir Surface (acres) -
Top of Dam Not Available
Max. Pool Not Available
Flood Control Pool Not Applicable
Recreation Pool Not Applicable
Spillway Crest 265
- g. Dam -
Type: Concrete Gravity with Earth Embankment
Length: 1,850 feet
Height: 50 feet
Top Width: 9 feet
Side Slopes: Upstream: Vertical
Downstream: 7 Horizontal/10 Vertical
Earth Embankment: Top Width: 10 feet
Side Slopes: 2 Horizontal/1 Vertical

h. Diversion and Regulating Tunnel -

Type: No Tunnel
Length: Not Applicable
Diameter: Not Applicable
Access: Not Applicable
Regulation: Not Applicable

i. Spillway -

Type: Ogee
Length of Weir: 100 feet
Crest Elevation: 310
Gates: Ungated
Upstream Channel: Reservoir
Downstream Channel: Riprap bottom, retaining walls
on sides
Spillway is founded on rock (plans)

j. Regulating Outlets -

6 - 2-foot by 3-foot sluice gates
2 - 30-inch supply mains
1 - 36-inch blow off
1 - 8-inch drain

SECTION 2 - ENGINEERING DATA

2.1 DESIGN:

The design of the dam and related structure was made by Arthur Hill, Consulting Engineer of New Haven, Connecticut. Pertinent parts of the following have been utilized in this report.

- a. Laurel Road Reservoir Plans (3 sheets).
- b. Hydrologic Study of Laurel Reservoir Watershed.
- c. Report on Raising Laurel Reservoir Dam.
- d. Files - Department of Environmental Protection, Supervisor of Dam Maintenance.

While no as-built drawings exist for this project, contract drawings were utilized for analysis purposes. No engineering values, assumptions, test results or calculations are available.

2.2 CONSTRUCTION:

No construction records are available.

2.3 OPERATION:

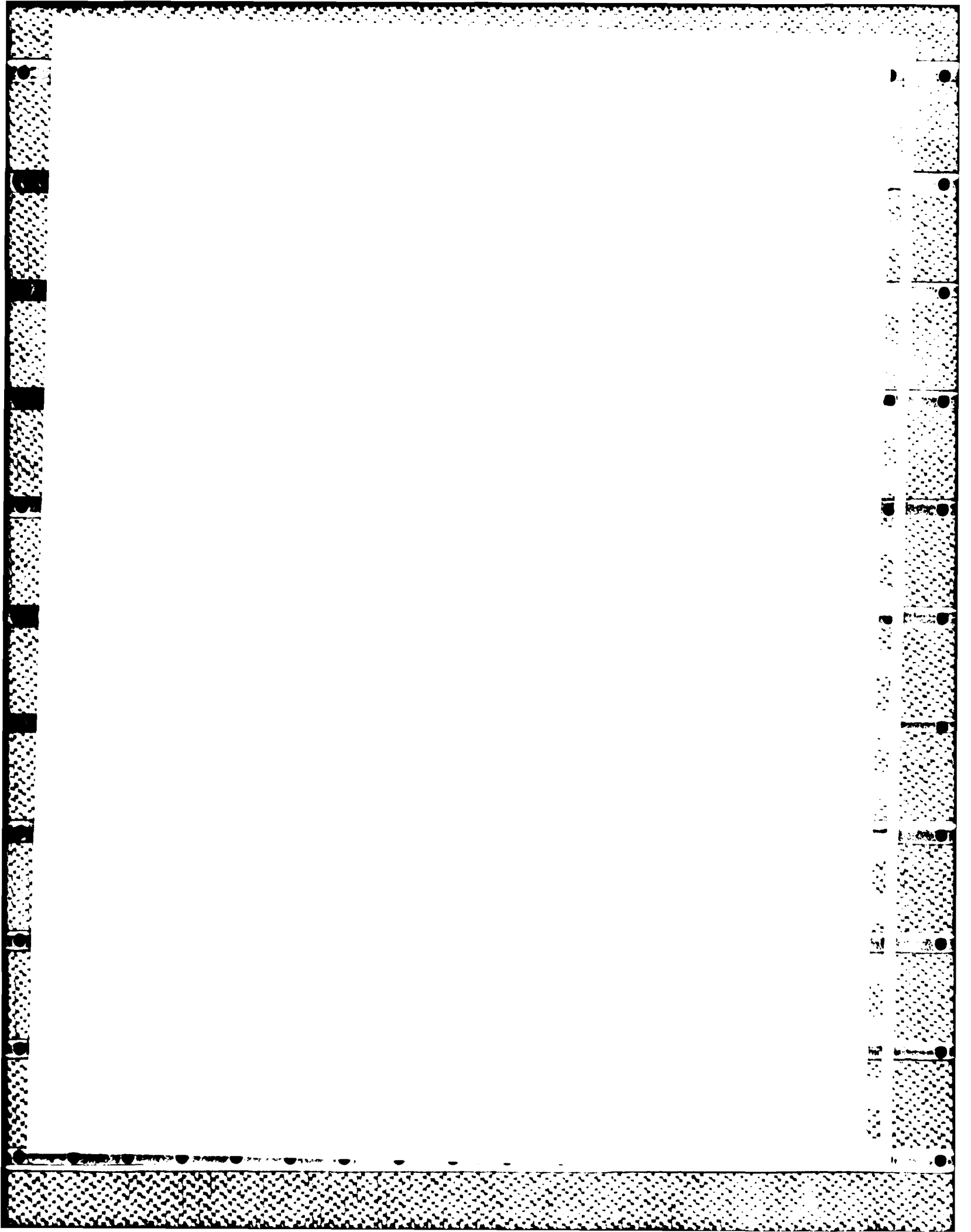
Operation records are available at the Stamford Water Company in Stamford, Connecticut.

2.4 EVALUATION:

a. Availability. Only plans showing dimensional features are available. Specifications indicating the properties of the materials used and construction procedures are not available.

b. Adequacy. Information available is adequate for Phase I investigation purposes.

c. Validity. There is no reason to question the validity of the documents reviewed.



4) Earth Embankment - The earth embankment is in good condition showing no indications of deformation, sloughing or erosion with the exception of one location near the downstream chlorination facility where a cut had been made into the side of the embankment to allow construction equipment access to the roof of the building. The downstream slope of the embankment was covered with extensive vegetation which has not been mowed this year. Brush heights in excess of 3 feet in some places were observed at some locations. No seeps were observed through either the embankment slope, the toe or downstream of the dam. The construction drawings indicate a stone drain placed against the downstream face of the dam with no apparent connection to any horizontal drain under the downstream embankment. The top of the drain, against the downstream face of the concrete dam, is shown on the drawings as 2 feet down from the surface of the downstream embankment crest. Thus the presence of the drain could not be visually verified.

Several animal holes (6 to 8 inch diameters) were noted at various locations in the high grass on the downstream slope of the earth embankment. Other animal holes may exist which were not visually observed.

c. Appurtenant Structures.

1) Gate House - The concrete and brick is in good condition. All the gates and valves are manually operated and appear in good condition and easy to turn. The 8-inch blow-off was discharging to the river at the time of inspection to maintain downstream flow. The 36-inch blow-off valve was not operated during the inspection, but was cracked slightly for a short period of time. All visible wiring in the gate house (used only for lighting) was enclosed in conduits and free of dirt and corrosion.

2) Access Road Bridge - Access to the dam is via a road that passes over the spillway discharge channel. The bridge has two spans of approximately 20 feet each, with a concrete center pier. The bridge is in excellent overall condition.

3) Bridge Over Spillway - This steel truss foot bridge is in good condition.

d. Reservoir Area. The reservoir perimeter has well vegetated banks at moderate to steep slopes. There was no evidence of sloughing. No noticeable debris or obstructions were seen in the vicinity of the intake tower. The depth of sediment, and rate of accumulation in the reservoir, is unknown.

SECTION 4 - OPERATIONAL PROCEDURES

4.1 PROCEDURES:

The Stamford Water Company by use of the 8-inch drain maintains flow of the Rippowam River. A small distribution facility at Laurel Reservoir supplies local customers with approximately 120,000 gallons per day. The Rippowam River continues about 2 miles south, emptying into the North Stamford Reservoir. A major distribution plant is located there to provide 16.5 million gallons per day to customers in Stamford.

4.2 MAINTENANCE OF DAM:

The dam and associated structures are well maintained with a regular program of grass mowing and general maintenance in effect.

4.3 MAINTENANCE OF OPERATING FACILITIES:

The regulating gates and valves were tested and appear to be in mechanically good operating condition, and are completely functional.

4.4 DESCRIPTION OF ANY WARNING SYSTEM IN EFFECT:

There was no warning system of any kind in effect at the time of the inspection.

4.5 EVALUATION:

The Laurel Reservoir Dam, which is approximately 55 years old, is well operated and maintained. Although not designed for rapid drawdown, it should be noted that, if the need should arise, drawdown could be effected by the following procedures:

- a. Allowing for maximum discharge through the 36-inch diameter blow-off.

- b. Allowing for maximum discharge through the 30-inch diameter supply main, which can blow-off at the discharge channel.

SECTION 5 - HYDRAULIC/HYDROLOGIC

5.1 EVALUATION OF FEATURES:

a. Design Data. The Laurel Reservoir Dam's original hydraulic design is not available. Under established criteria (OCE Guidelines), the recommended spillway test flood for the size (intermediate) and hazard potential (high) classification is the probable maximum flood (PMF). The PMF is the flood that may be expected from the most severe combination of critical meteorologic and hydrologic conditions that are reasonably possible in the region. The PMF has recently been calculated by a consultant to the Stamford Water Company (Roald Haestad, Inc.).

The hydrograph was developed using rainfall data and computational techniques as described in "Design of Small Dams" - Bureau of Reclamation.

The Laurel Reservoir inflow hydrograph was actually a composite, which included the inflow hydrographs of two upstream reservoirs (Trinity and Mill), and the runoff contributed by the land downstream of the two reservoirs. The flood hydrograph was then routed through Laurel Reservoir, with the following results:

<u>Storm Frequency</u>	<u>Inflow Peak Runoff</u>	<u>Outflow Peak Runoff</u>
100 years	5,300 CFS	2,700 CFS
1,000 years	8,060 CFS	4,900 CFS
PMF	29,000 CFS	29,000 CFS

The stage verse outflow rates from the Laurel Reservoir were computed and plotted by the consultant. The spillway capacity was identified as being approxiamtely 3,900 cubic feet per second.

As part of the current investigation of the Laurel Reservoir Dam, a rough check of the PMF was made. The computed peak flow was 20,800 CFS, noticeably lower than the consultant's value of 29,000 CFS. As a conservative approach to the investigation, the higher design PMF hydrograph was used.

b. Experience Data. The Laurel Reservoir Dam has been operational since the mid-1920's. During this time it has safely discharged the floods which have hit the Westchester-Connecticut area. During the storm of October 15-17, 1955, the discharge over the Laurel spillway was estimated to be 2,780 CFS.

c. Visual Observations. The on-site inspection of the dam revealed that the spillway has a crest length of 100 feet, while the original plans show a proposed length of 120 feet. Although flashboards are in place, they are designed to collapse under a head of a little over 6 inches.

d. Overtopping Potential. The existing Laurel Reservoir spillway is capable of discharging the 100-year storm, but the dam will be overtopped by the 1,000-year storm and the test flood. The test flood will result in a stage in the reservoir of elevation 317.6 (2.6 feet above the top of the dam).

SECTION 6 - STRUCTURAL STABILITY

6.1 EVALUATION OF STRUCTURAL STABILITY:

a. Visual Observations. Visual observations did not indicate any existing structural problems.

b. Design and Construction Data. The design and construction reflected in the three existing drawings indicate the concrete dam is founded in the underlying bedrock formation which is not exposed downstream of the dam. The stability of the concrete wall and the downstream earth embankment cannot be formally evaluated with the available information.

c. Operating Records. The dam was built before 1926 and to our knowledge, there has been no indications of any instability since construction. As the Laurel Reservoir Dam was constructed for water supply purposes and has been subjected to a full head of water since its construction, its stability is considered to be adequate based on performance.

d. Post-Construction Changes. The 36-inch blow off pipe, and 8-inch drain were extended approximately 30 feet south and a new endwall constructed. Additionally, at the same time (1974) a 30-inch blow off from the water supply main was constructed. It was reported that the excavation for this extension did not uncover any seepage problems.

e. Seismic Stability. This dam is located in Seismic Zone 1 and hence does not have to be evaluated for seismic stability.

SECTION 7 - ASSESSMENT, RECOMMENDATIONS AND REMEDIAL MEASURES

7.1 DAM ASSESSMENT:

a. Condition. Based on the visual inspection, records available and past operational performance, the dam is judged to be in good condition.

The project will not pass the test flood without overtopping the dam, and therefore, the spillway capacity is inadequate. The spillway will pass only 15 per cent of the test flood, and is considered seriously inadequate.

b. Adequacy of Information. The evaluation of the dam is mainly based on the visual inspection assisted by the general physical dimensions provided in the three available drawings, and the past operational performance of the structure.

c. Urgency. The measures recommended below should be implemented in the near term.

d. Need for Additional Investigation. Further investigation and engineering analysis of methods of increasing spillway capacity are warranted.

7.2 RECOMMENDATIONS:

It is recommended that the following measures be undertaken by the owner:

- 1) Detailed investigations should be initiated to determine methods for obtaining additional spillway capacity.
- 2) Concrete surfaces affected by spalling should be repaired.
- 3) Trees and brush obstructing the spillway discharge channel should be removed.

7.3 REMEDIAL MEASURES:

Although the dam is generally maintained in a good condition, it is considered important that the following items be accomplished:

- a. Alternatives. Not applicable.

b. Operation and Maintenance and Procedures.

1) Operation and maintenance manual for the project should be prepared.

2) A program of periodic inspections of the project features should be established.

3) Due to the potential for overtopping, it is recommended that a definite plan for around the clock surveillance be implemented during periods of unusually heavy rains and a formal warning system be developed for use in the event of an emergency.

APPENDIX I

PHOTOGRAPHS



PHOTO #1: Upstream Face of Dam, looking West.

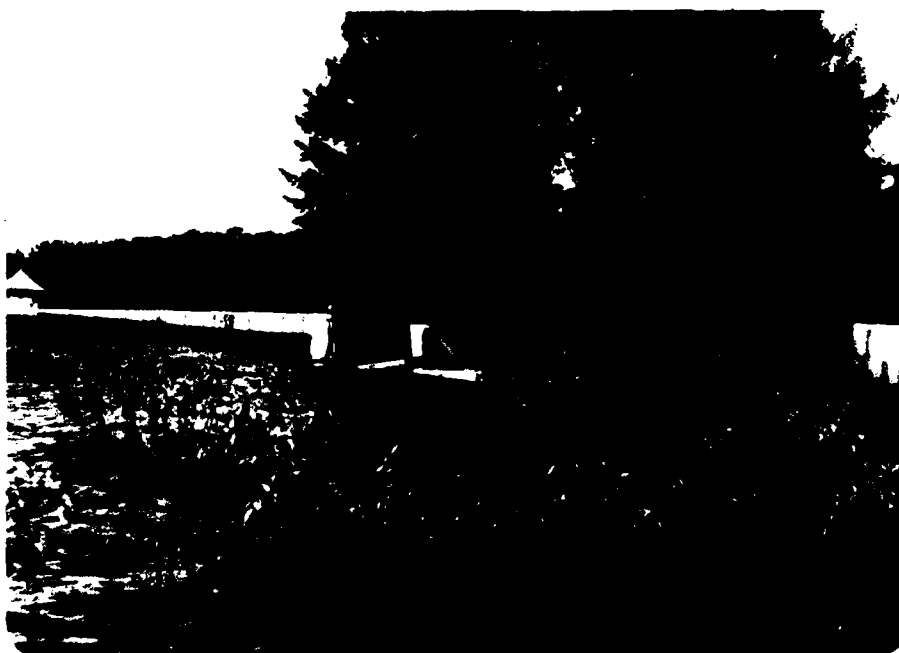


PHOTO #2: Upstream Face of Dam and Spillway Bridge, looking East.

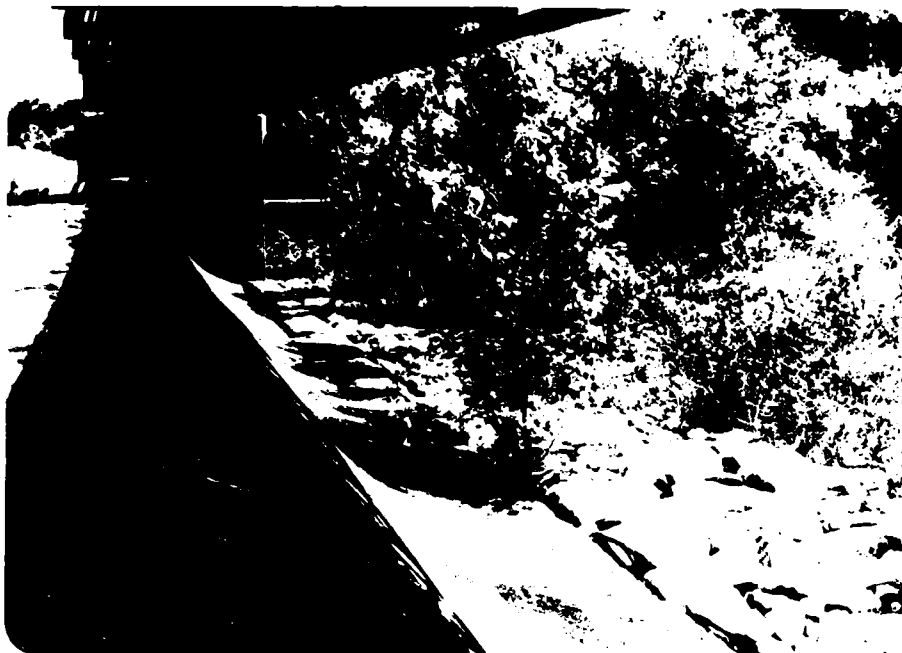


PHOTO #3: Spillway, looking East.

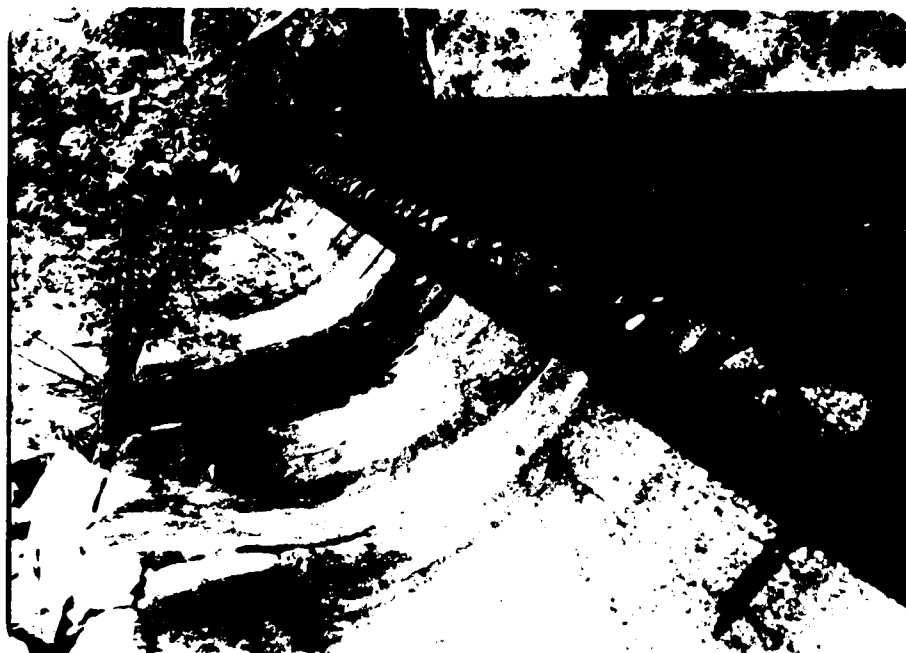


PHOTO #4: Spillway, looking West. Note Flashboards.



PHOTO #5: Outlet Works and Discharge Point and Channel.



PHOTO #6: Downstream Face of Dam and Embankment looking West.



PHOTO #7: Downstream Face of Dam and Embankment looking West.



PHOTO #8: Central Portion of Embankment, looking East.



PHOTO #9: Control Tower.



PHOTO #10: Typical Spalling at a Construction Joint on the Upstream Face of the Dam.

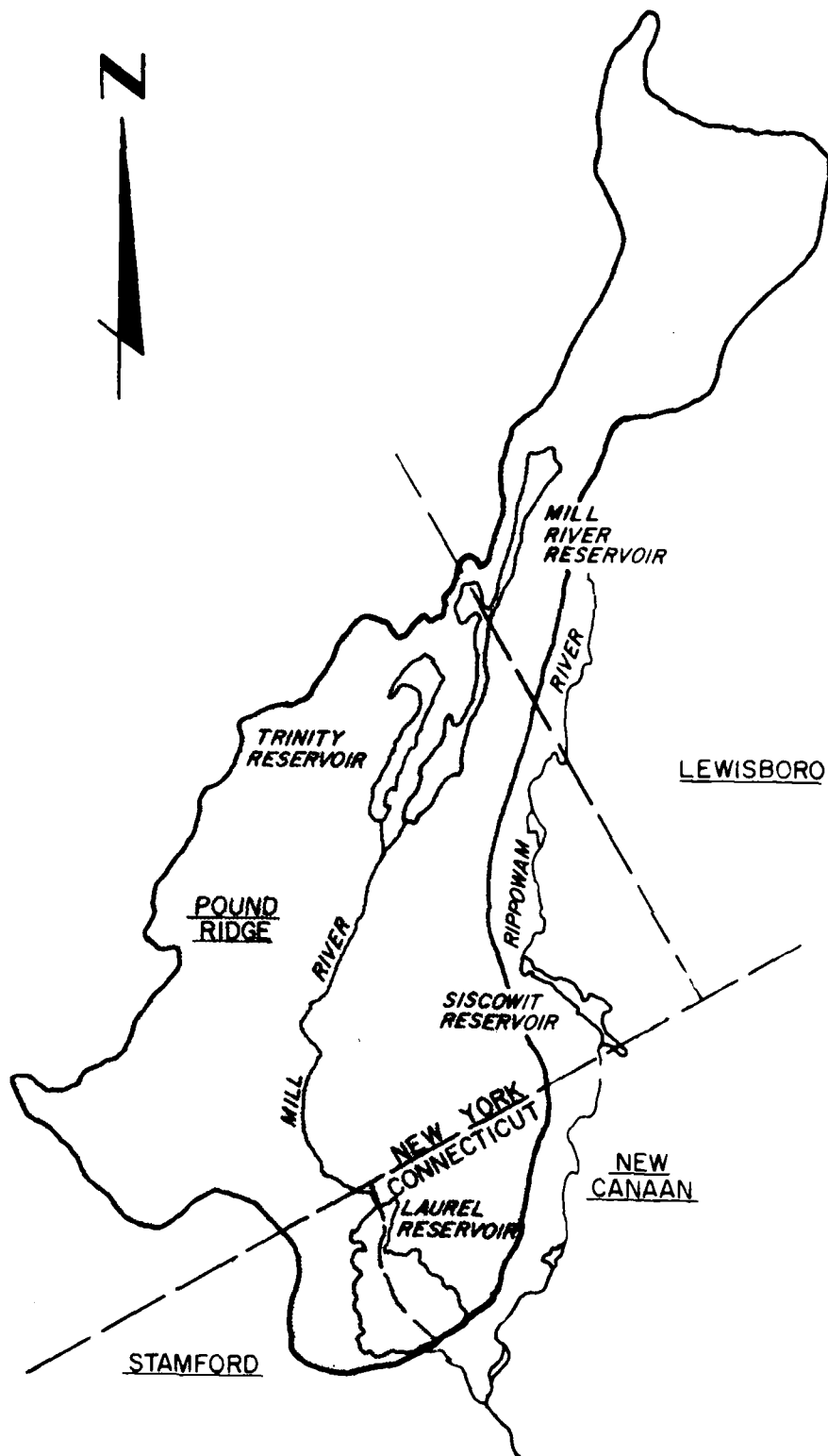


PHOTO #11: Typical Spalling at Joint,
Downstream Face.



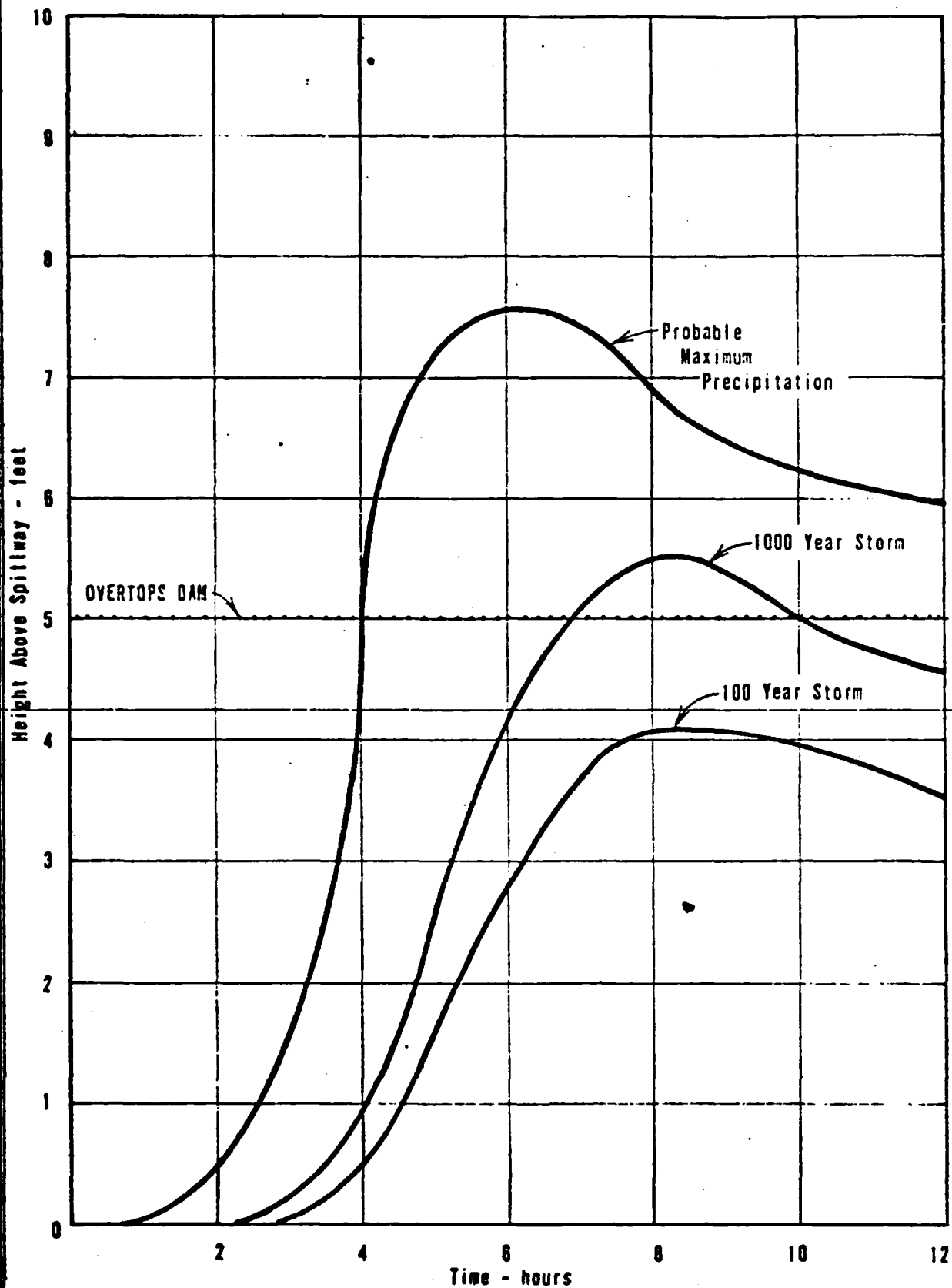
PHOTO #12: Typical Spalling, Downstream
Face, at Top of Earth Embankment.

APPENDIX II
HYDROLOGIC COMPUTATIONS



SCALE IN FEET
6000 0 3000 6000

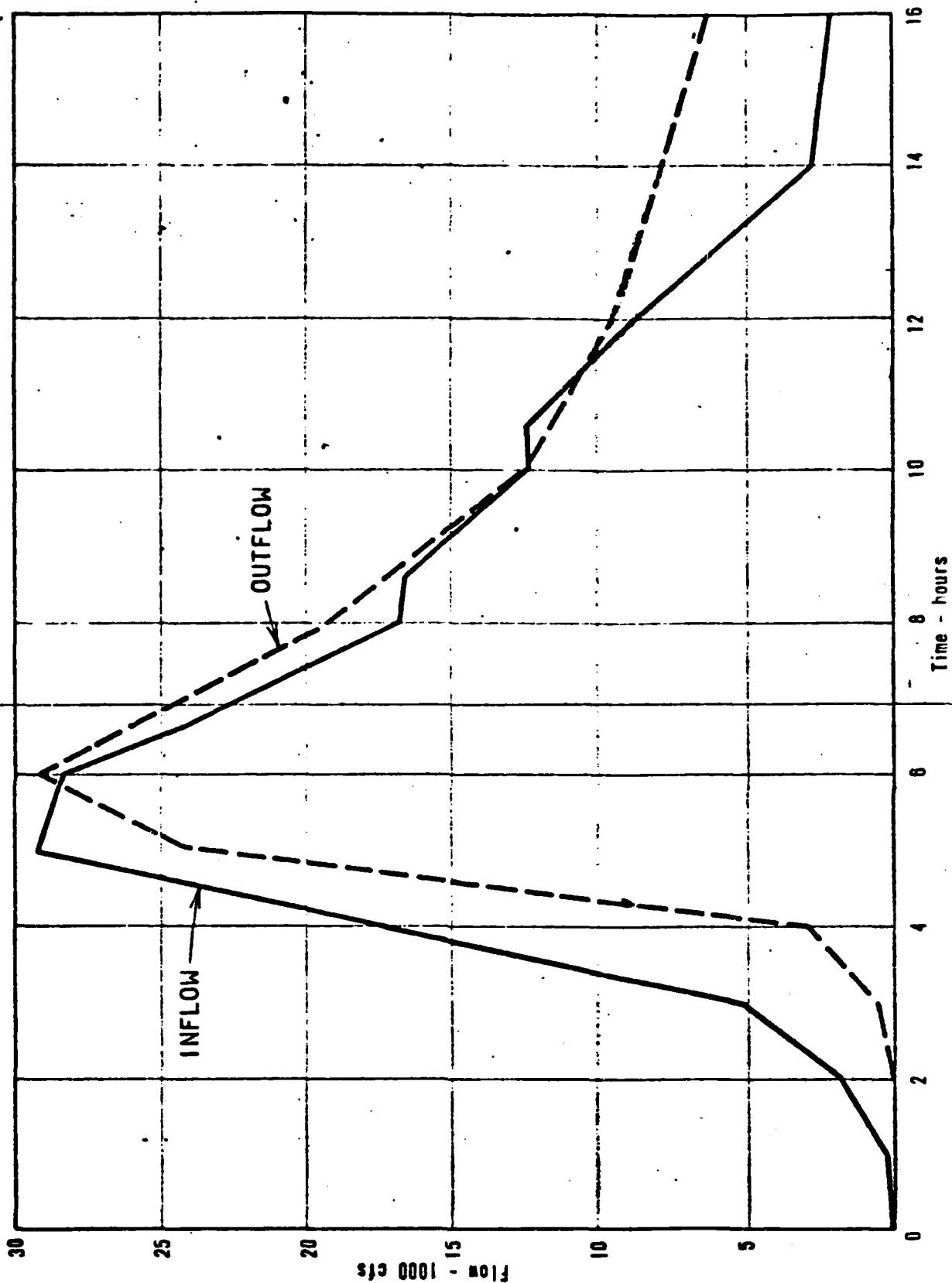
DRAINAGE AREA MAP
LAUREL RESERVOIR DAM
STAMFORD-NEW CANAAN, CONNECTICUT



STAMFORD WATER COMPANY
SUMMARY DISCHARGE CURVES
EXISTING SPILLWAY
LAUREL RESERVOIR

ROALD HAESTAD, INC.
Consulting Engineers

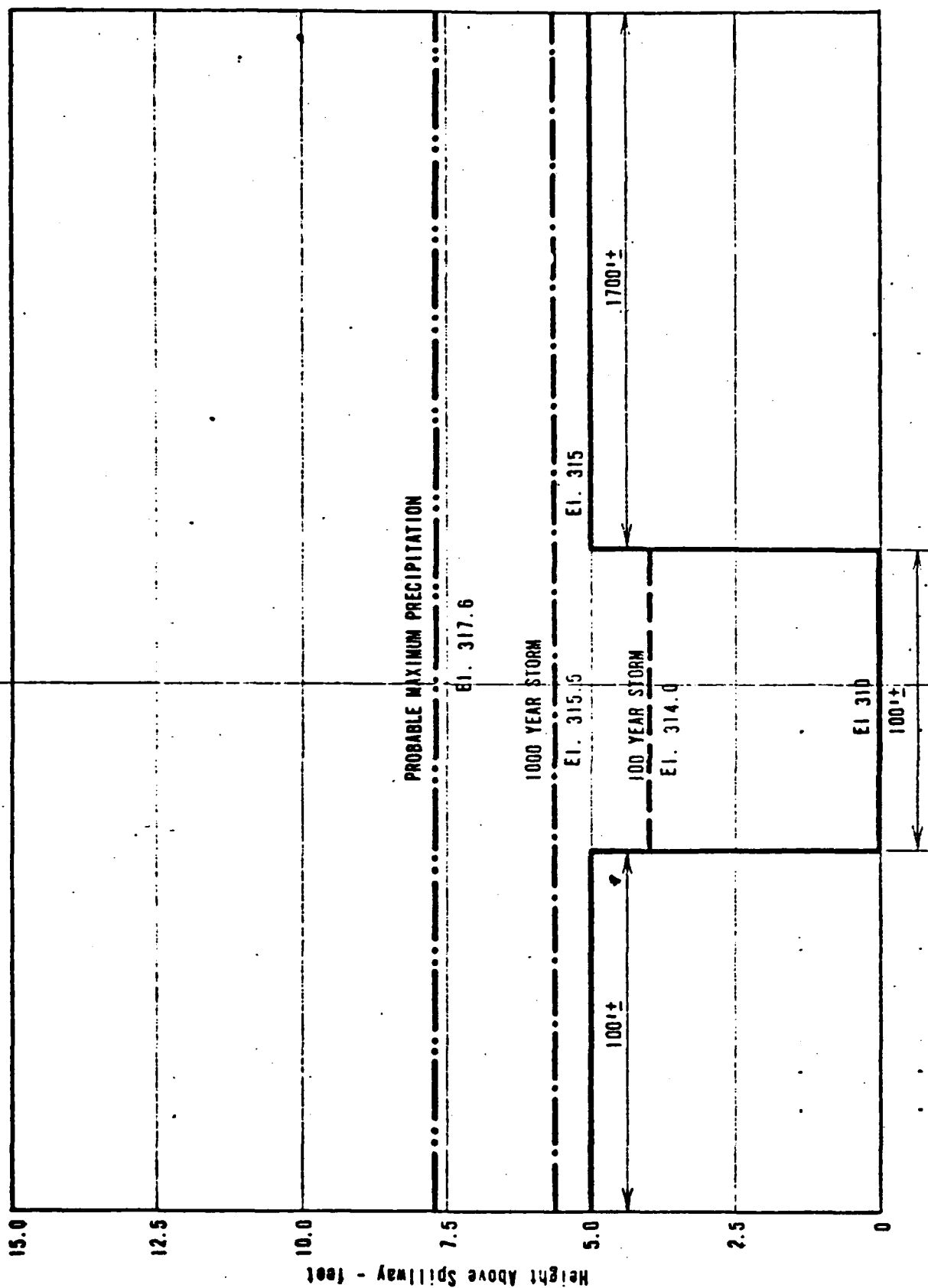
August 1972



STAMFORD WATER COMPANY
 INFLOW-OUTFLOW HYDROGRAPH - PMP STORM
 EXISTING SPILLWAY
 LAUREL RESERVOIR

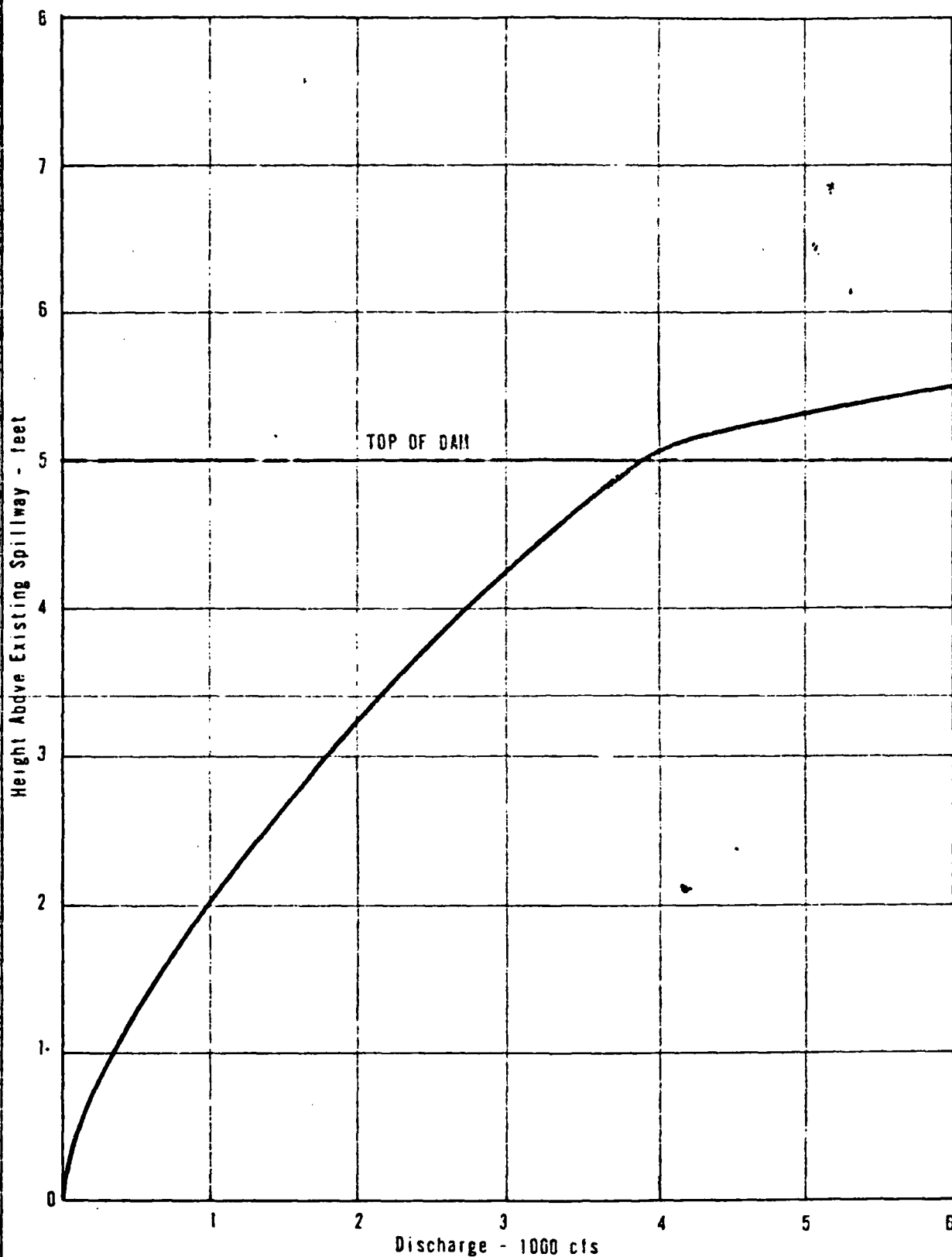
ROALD NAESTAD, INC.
 Consulting Engineers

August 1979



STAMFORD WATER COMPANY
EXISTING SPILLWAY PROFILE
LAUREL RESERVOIR

ROALD HAESTAD, INC.



STAMFORD WATER COMPANY
SPILLWAY RATING CURVE
LAUREL RESERVOIR
EXISTING SPILLWAY

ROALD HAESTAD, INC.
CONSULTING ENGINEERS

August 1973

APPENDIX III

VISUAL INSPECTION

CHECK LIST

PERIODIC INSPECTION CHECK LIST

PROJECT Laurel Reservoir Dam

DATE June 15, 1978

INSPECTOR Anthony Rummo

DISCIPLINE Structural

INSPECTOR _____

DISCIPLINE _____

AREA EVALUATED	CONDITION
<u>CONCRETE DAM STRUCTURE</u>	
General Condition Concrete Surfaces	
Movement or Settlement of Crest	None observed
Vertical Alignment	
Horizontal Alignment	
Condition at Abutment and Other Structures	
Structural Cracking	None observed
Spalling	Spalling noted on U/S and D/S faces
Visible Reinforcing	No visible reinforcing
Rusting or Staining of Concrete	None observed
Condition of Monolith/ Construction Joints	
Drains - Foundation, Joint, Faces	
Any Seepage or Efflorescence	
Foundation Damage, Undermining	
Water Passages	
Abutments	

PERIODIC INSPECTION CHECK LIST

PROJECT Laurel Reservoir Dam DATE June 15, 1978
 INSPECTOR Anthony Rummo DISCIPLINE Structural
 INSPECTOR _____ DISCIPLINE _____

AREA EVALUATED	CONDITION
<u>DAM EMBANKMENT</u>	
Crest Elevation	315
Current Pool Elevation	310(+)
Maximum Impoundment to Date	
Surface Cracks	
Pavement Condition	
Movement or Settlement of Crest	
Lateral Movement	
Vertical Alignment	
Horizontal Alignment	
Condition at Abutment and at Concrete Structures	
Indications of Movement of Structural Items on Slopes	
Trespassing on Slopes	Good, a few animal holes were encountered.
Sloughing or Erosion of Slopes or Abutments	Slight erosion near chlorination facility
Rock Slope Protection - Riprap Failures	None observed
Unusual Movement or Cracking at or near Toes	None observed
Unusual Embankment or Down-stream Seepage	None observed

PERIODIC INSPECTION CHECK LIST

PROJECT Laurel Reservoir Dam

DATE June 15, 1978

INSPECTOR Anthony Rummo

DISCIPLINE Structural

INSPECTOR _____

DISCIPLINE _____

AREA EVALUATED	CONDITION
<u>DAM EMBANKMENT</u> - (continued)	
Piping or Boils	None
Foundation Drainage Features	None observed
Toe Drains	None
Instrumentation System	

PERIODIC INSPECTION CHECK LIST

PROJECT Laurel Reservoir Dam

DATE June 15, 1978

INSPECTOR Anthony Rummo

DISCIPLINE Structural

INSPECTOR James MacBroom

DISCIPLINE Hydraulics/
Hydrology

AREA EVALUATED	CONDITION
<u>OUTLET WORKS - CONTROL TOWER</u>	
a. Concrete and Structural	
General Condition	Concrete (gunite) in good condition, some spalling at water line
Condition of Joints	
Spalling	
Visible Reinforcing	None observed
Rusting or Staining of Concrete	None
Any Seepage or Efflorescence	None
Joint Alignment	
Unusual Seepage or Leaks in Gate Chamber	
Cracks	Gunite separating from original concrete at bottom of application (2'-3' below water line)
Rusting or Corrosion of Steel	None
b. Mechanical and Electrical	
Air Vents	
Float Wells	
Crane Hoist	
Elevator	
Hydraulic System	

PERIODIC INSPECTION CHECK LIST

PROJECT Laurel Reservoir Dam

DATE June 15, 1978

INSPECTOR Anthony Rummo

DISCIPLINE Structural

INSPECTOR James MacBroom

DISCIPLINE Hydraulics/
Hydrology

AREA EVALUATED	CONDITION
<p><u>OUTLET WORKS - CONTROL TOWER</u> (continued)</p> <p>Service Gates</p> <p>Emergency Gates</p> <p>Lightning Protection System</p> <p>Emergency Power System</p> <p>Wiring and Lighting System In Gate Chamber</p>	<p>All gates/valves are manually operated, appear to be in good condition and easy to turn</p> <p>Wiring in gate house, enclosed in conduits, free of dirt and corrosion</p>

PERIODIC INSPECTION CHECK LIST

PROJECT Laurel Reservoir Dam

DATE June 15, 1978

INSPECTOR Richard Murdoch

DISCIPLINE Geotechnical

INSPECTOR James MacBroom

DISCIPLINE Hydraulics/
Hydrology

AREA EVALUATED	CONDITION
<p><u>OUTLET WORKS - INTAKE CHANNEL AND INTAKE STRUCTURE</u></p> <p>a. Approach Channel</p> <p>Slope Conditions</p> <p>Bottom Conditions</p> <p>Rock Slides or Falls</p> <p>Log Boom</p> <p>Debris</p> <p>Condition of Concrete Lining</p> <p>Drains or Weep Holes</p> <p>b. Intake Structure</p> <p>Condition of Concrete</p> <p>Stop Logs and Slots</p>	<p>Upstream face of dam</p> <p>8"-diameter log overhanging flashboards</p>

PERIODIC INSPECTION CHECK LIST

PROJECT Laurel Reservoir Dam

DATE June 15, 1978

INSPECTOR Anthony Rummo

DISCIPLINE Structural

INSPECTOR James MacBroom

DISCIPLINE Hydraulics/
Hydrology

AREA EVALUATED	CONDITION
<u>OUTLET WORKS - SPILLWAY WEIR,</u> <u>APPROACH AND DISCHARGE CHANNELS</u>	
a. Approach Channel	
General Condition	Reservoir, could not observe
Loose Rock Overhanging Channel	
Trees Overhanging Channel	
Floor of Approach Channel	
b. Weir and Training Walls	
General Condition of Concrete	
Rust or Staining	
Spalling	Spalling/crack in central portion of spillway
Any Visible Reinforcing	None
Any Seepage or Efflorescence	
Drain Holes	
c. Discharge Channel	
General Condition	Channel heavily overgrown, trees up to 2" in diameter
Loose Rock Overhanging Channel	None
Trees Overhanging Channel	Yes
Floor of Channel	Riprap at upper end, middle cut in bedrock, lower earth; no significant erosion, but overgrown with heavy brush
Other Obstructions	

PERIODIC INSPECTION CHECK LIST

PROJECT Laurel Reservoir Dam

DATE June 15, 1978

INSPECTOR Anthony Rummo

DISCIPLINE Structural

INSPECTOR James M. cBroom

DISCIPLINE Hydraulics/
Hydrology

AREA EVALUATED	CONDITION
<u>OUTLET WORKS - SERVICE BRIDGE</u>	(Access)
a. Super Structure	
Bearings	Bridge in excellent condition
Anchor Bolts	
Bridge Seat	
Longitudinal Members	3 double "tee" prestressed concrete, in good condition
Under Side of Deck	
Secondary Bracing	
Deck	Concrete in excellent shape
Drainage System	
Railings	
Expansion Joints	
Paint	
b. Abutments & Piers	
General Condition of Concrete	Center pier in excellent condition
Alignment of Abutment	
Approach to Bridge	
Condition of Seat & Backwall	

PERIODIC INSPECTION CHECK LIST

PROJECT Laurel Reservoir Dam

DATE June 15, 1978

INSPECTOR Anthony Rummo

DISCIPLINE Structural

INSPECTOR James MacBroom

DISCIPLINE Hydraulics/
Hydrology

AREA EVALUATED	CONDITION
<u>OUTLET WORKS - SERVICE BRIDGE</u>	(Over Spillway)
a. Super Structure	
Bearings	Bridge in excellent condition
Anchor Bolts	minor rusting noted
Bridge Seat	
Longitudinal Members	
Under Side of Deck	
Secondary Bracing	
Deck	
Drainage System	
Railings	
Expansion Joints	
Paint	
b. Abutments & Piers	
General Condition of Concrete	
Alignment of Abutment	
Approach to Bridge	
Condition of Seat & Backwall	

PERIODIC INSPECTION CHECK LIST

PROJECT _____

DATE _____

INSPECTOR _____

DISCIPLINE _____

INSPECTOR _____

DISCIPLINE _____

AREA EVALUATED

CONDITION

OUTLET WORKS - TRANSITION AND
CONDUIT

General Condition of Concrete

Rust or Staining on Concrete

Spalling

Erosion or Cavitation

Cracking

Alignment of Monoliths

Alignment of Joints

Numbering of Monoliths

PERIODIC INSPECTION CHECK LIST

PROJECT Laurel Reservoir Dam

DATE June 15, 1978

INSPECTOR James MacBroom

DISCIPLINE Hydraulics/
Hydrology

INSPECTOR Richard Murdock

DISCIPLINE Geotechnical

AREA EVALUATED	CONDITION
<p><u>OUTLET WORKS - OUTLET STRUCTURE AND OUTLET CHANNEL</u></p> <p>General Condition of Concrete</p> <p>Rust or Staining</p> <p>Spalling</p> <p>Erosion or Cavitation</p> <p>Visible Reinforcing</p> <p>Any Seepage or Efflorescence</p> <p>Condition at Joints</p> <p>Drain Holes</p> <p>Channel</p> <p>Loose Rock or Trees Over- hanging Channel</p> <p>Condition of Discharge Channel</p>	<p>Concrete endwall in excellent condition</p> <p>None</p> <p>The discharge channel is in good condition, no evidence of degradation or unstable banks</p>

APPENDIX IV
ENGINEERING DATA
CHECK LIST

**CHECK LIST
ENGINEERING DATA
DESIGN, CONSTRUCTION, OPERATION
PHASE I**

Lauf

NAME OF DAM Reservoir Dam

I.D. NO. 49

ITEM

REMARKS

POST-CONSTRUCTION SURVEYS OF DAM

None available

BORROW SOURCES

Unknown

MONITORING SYSTEMS

None

MODIFICATIONS

From Stamford Water Company

HIGH POOL RECORDS

None

**POST-CONSTRUCTION ENGINEERING
STUDIES AND REPORTS**

None

**PRIOR ACCIDENTS OR FAILURE OF DAM
DESCRIPTION
REPORTS**

None

MAINTENANCE OPERATION RECORDS

None

SPILLWAY PLAN

SECTIONS

From plans

DETAILS

From plans

**OPERATING EQUIPMENT
PLANS & DETAILS**

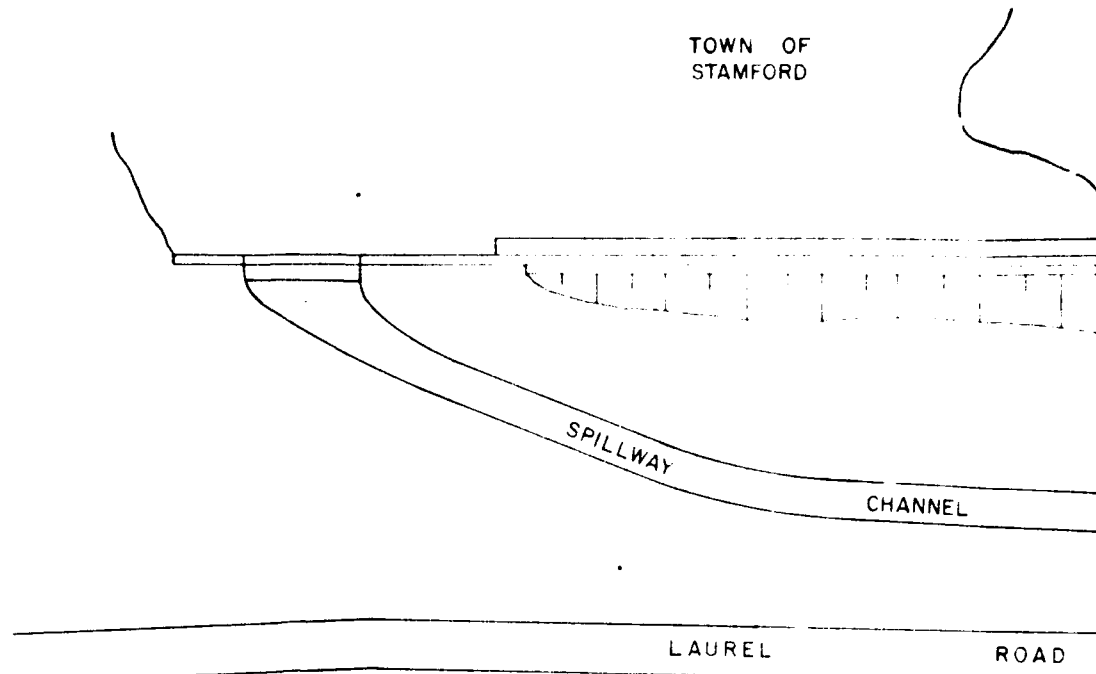
From plans

DRAWINGS

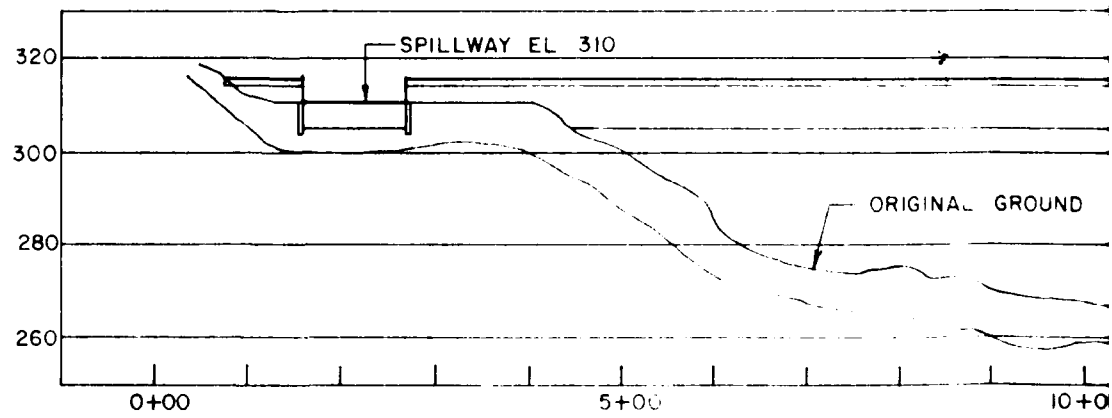
APPENDIX V

DRAWINGS

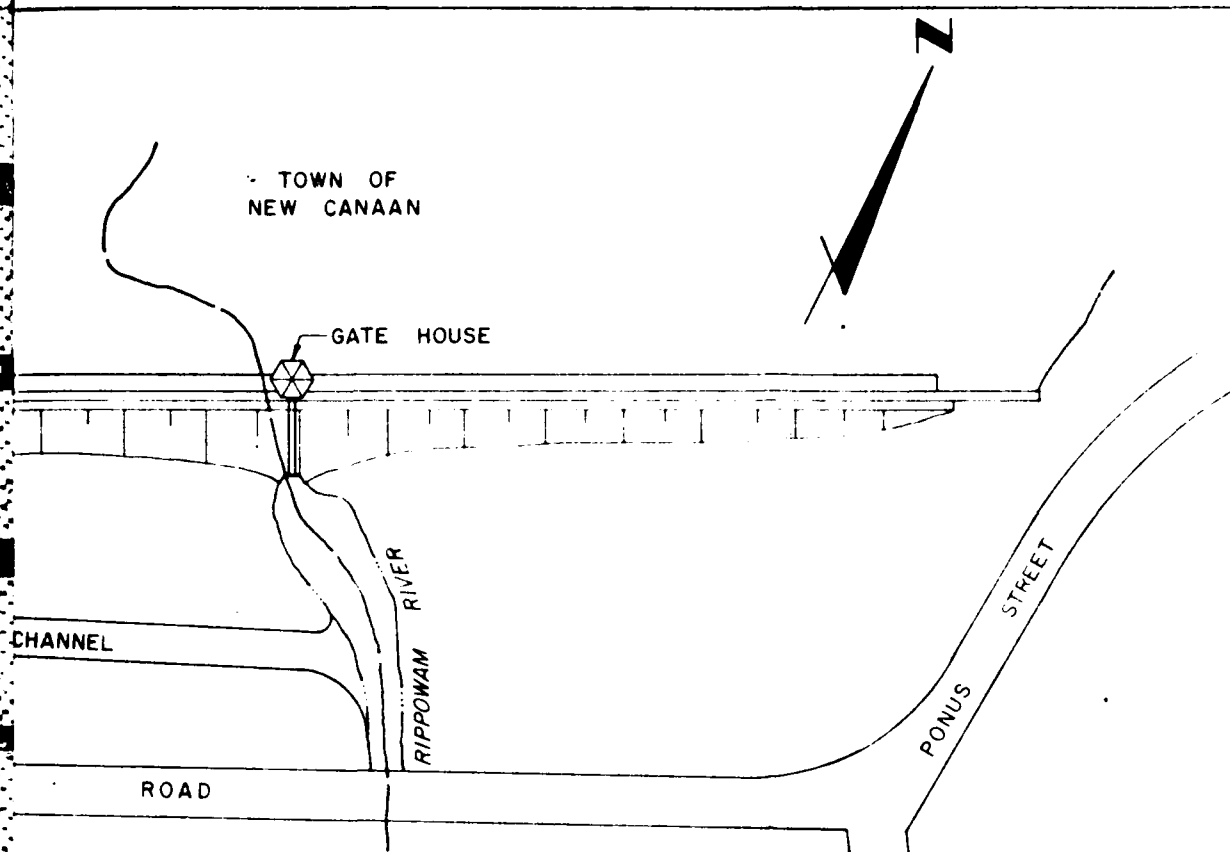
TOWN OF
STAMFORD



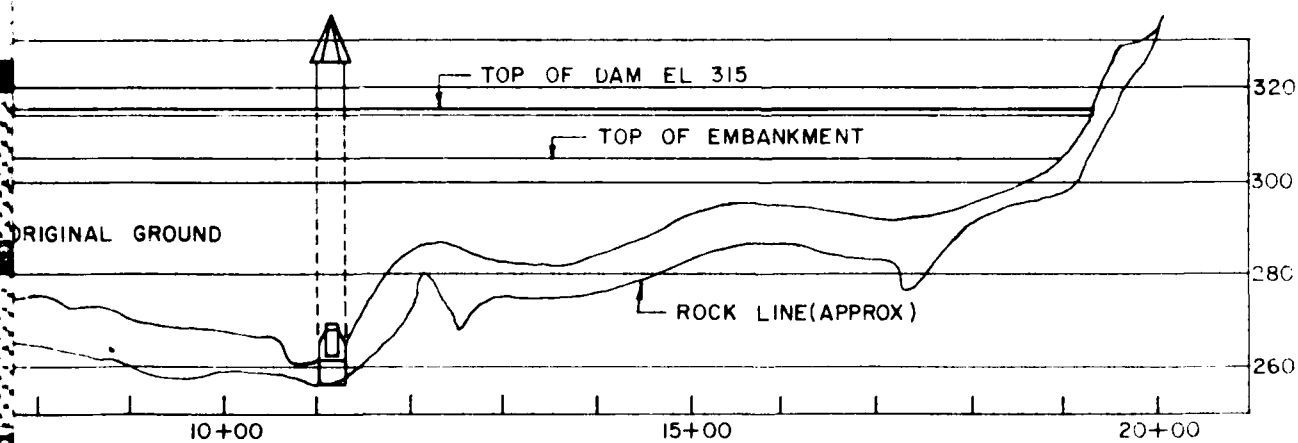
PLAN
NTS



DOWNSTREAM ELEV
NTS

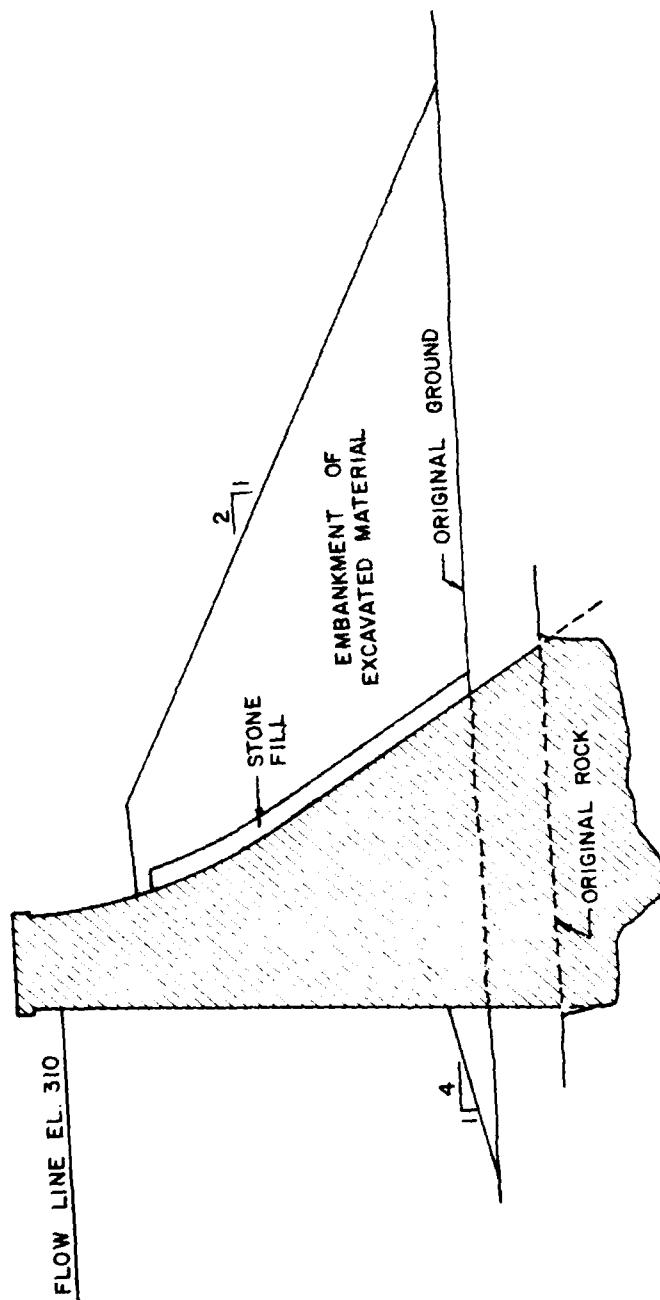


PLAN
NTS



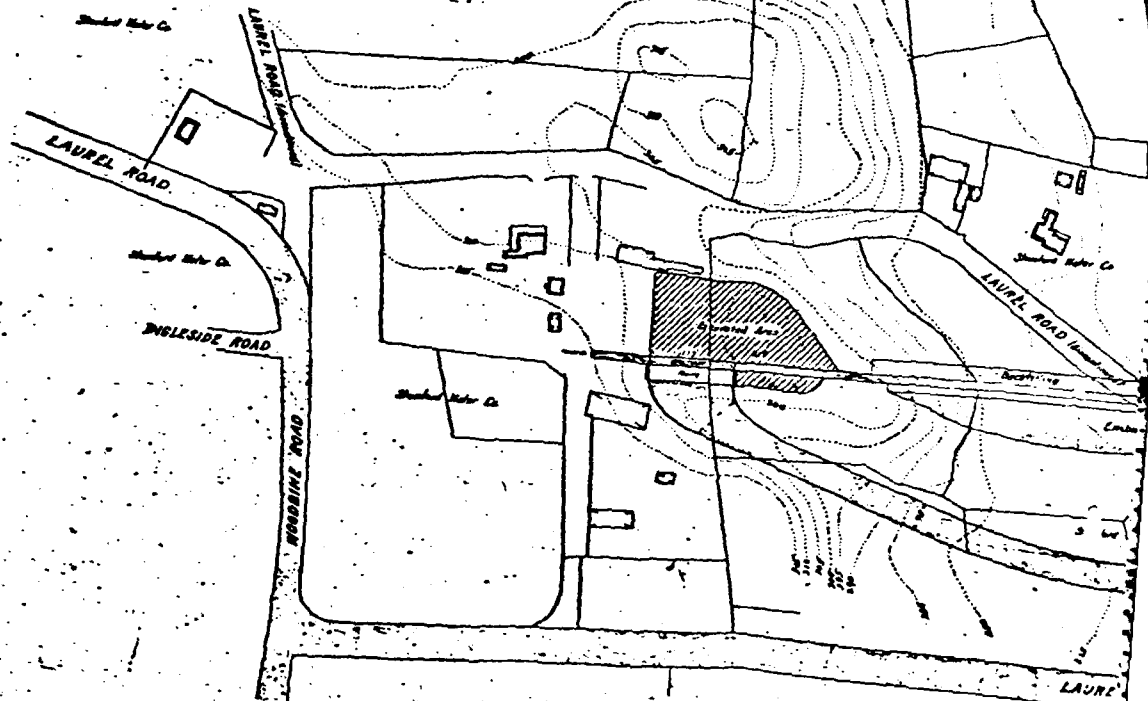
STREAM ELEVATION OF DAM
NTS

LAUREL RESERVOIR DAM
RIPPOWAM RIVER

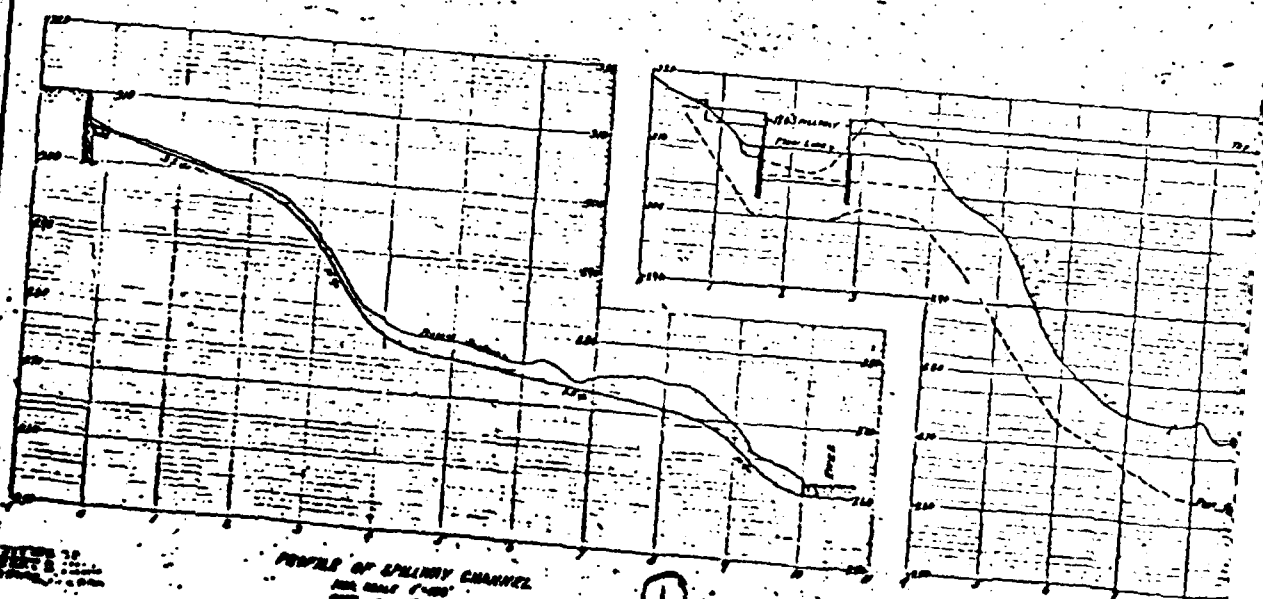


CROSS SECTION OF DAM
NTS

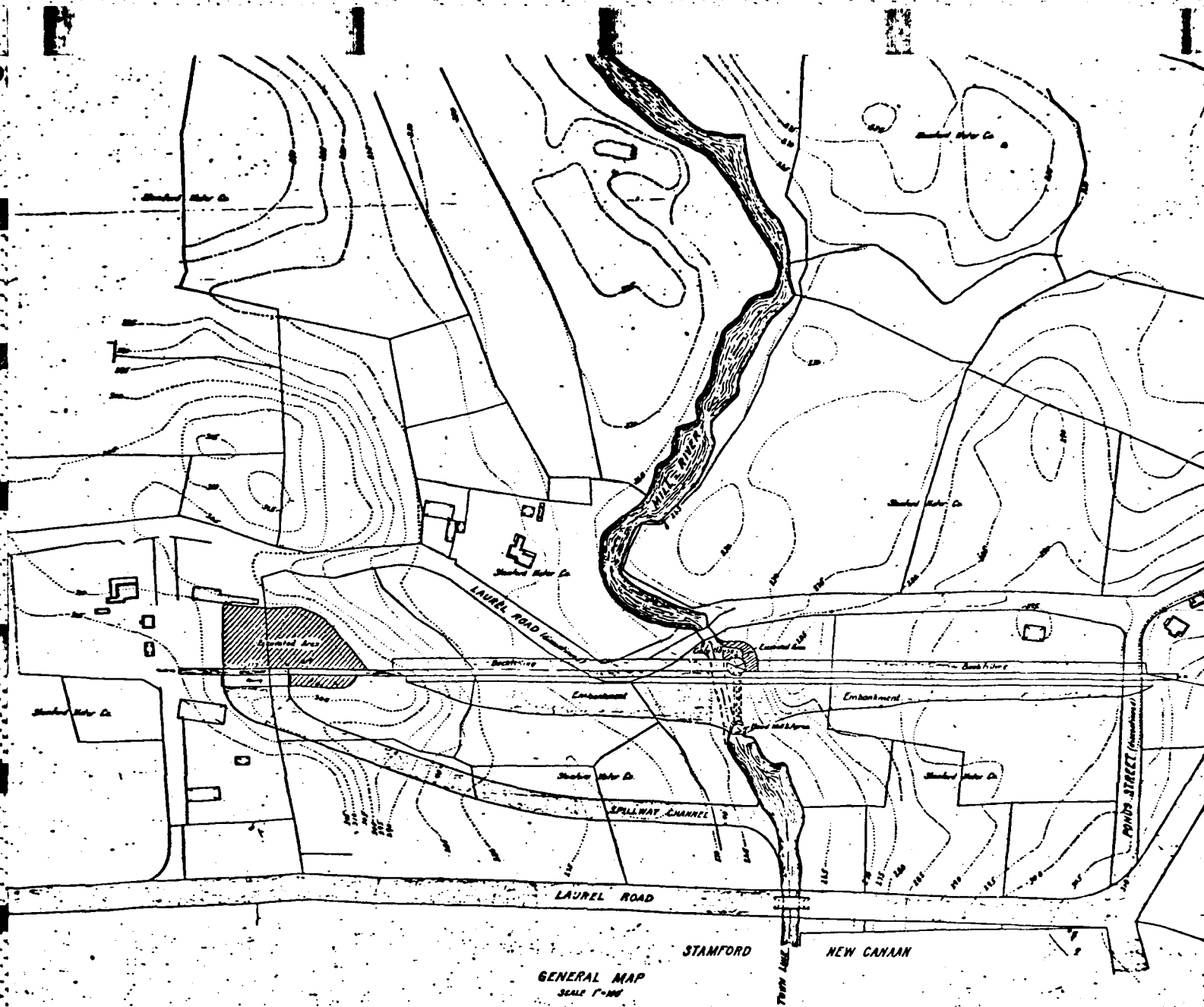
LAUREL RESERVOIR DAM
RIPPOWAM RIVER



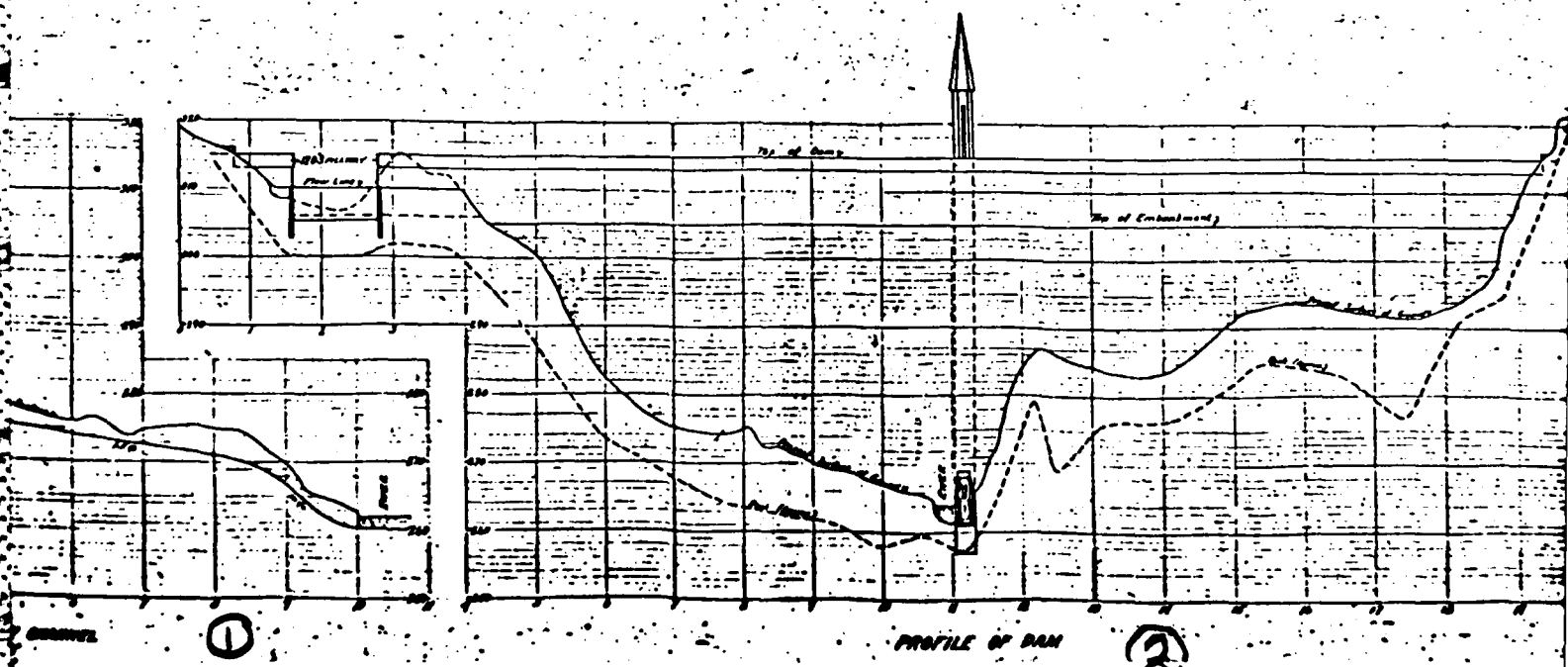
GENERAL
SCALE 1"



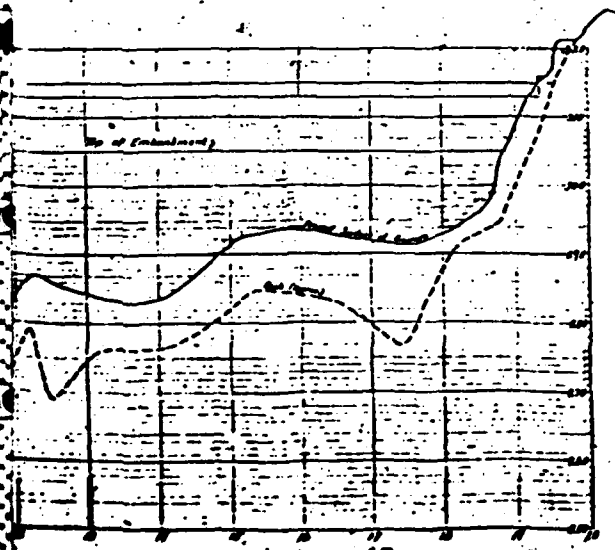
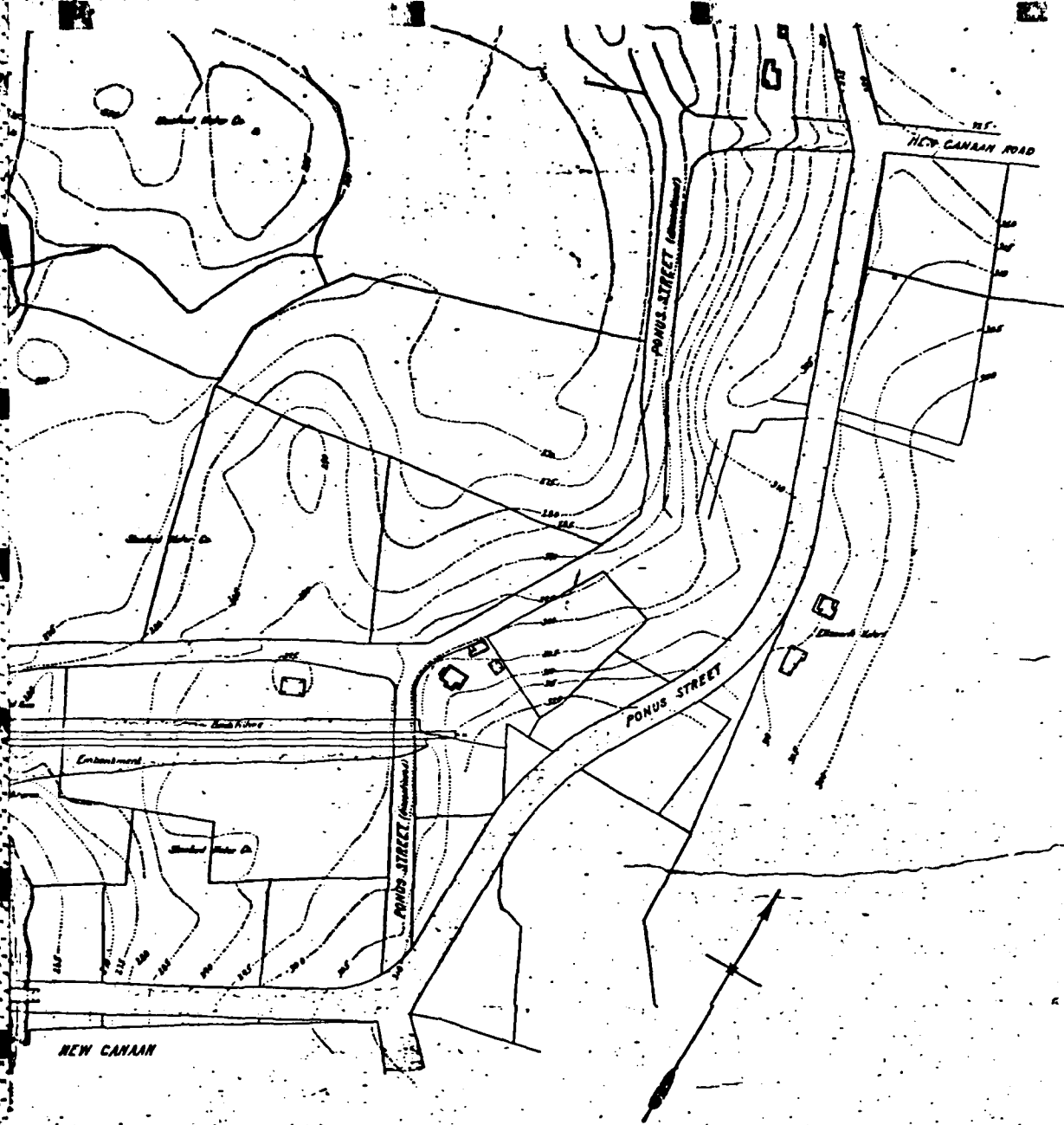
PROFILE OF SPERRY CHANNEL
SEE MAP P. 10



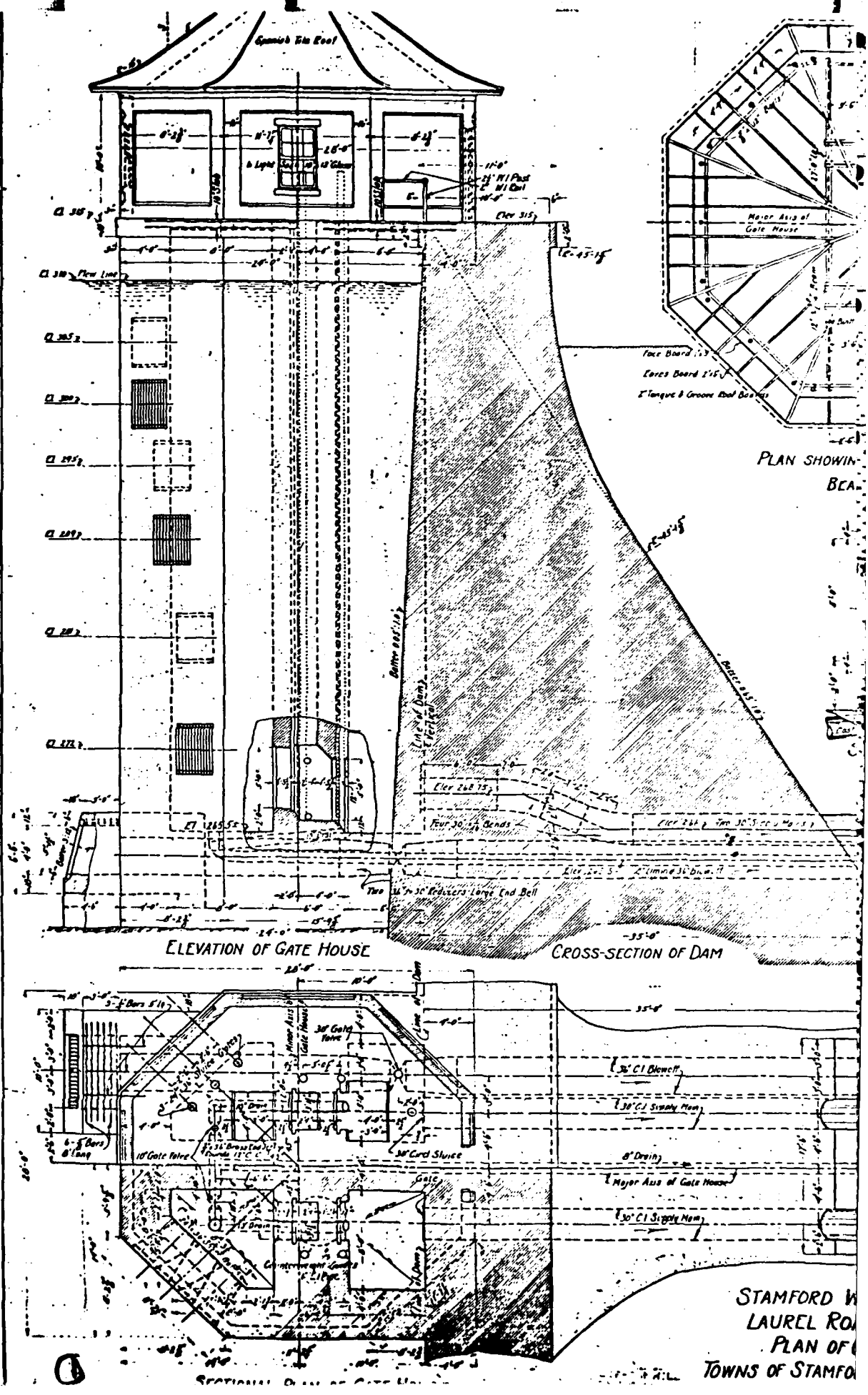
GENERAL MAP
SCALE 1"=100'

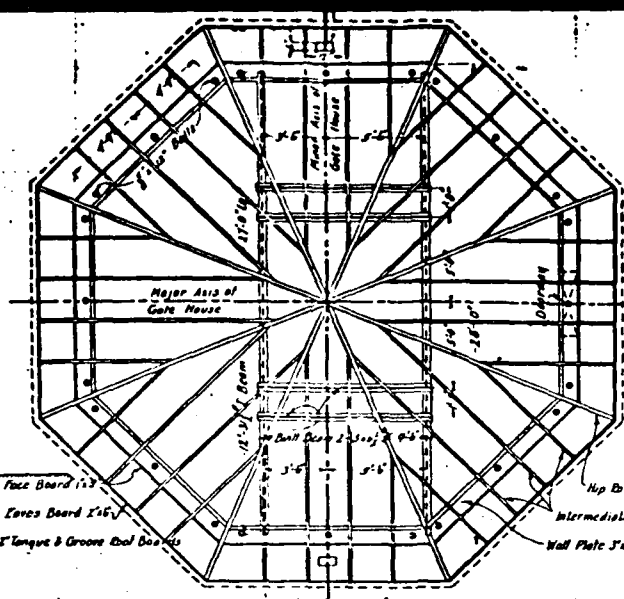


PROFILE OF DAM

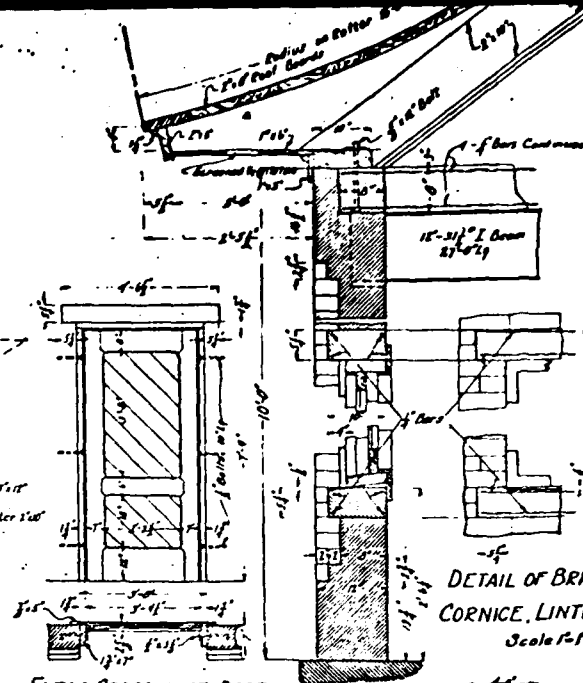


STAMFORD WATER COMPANY
 LAUREL ROAD RESERVOIR
 MAP AND PROFILE
 TOWNS OF STAMFORD & NEW CANAAN, CONN.
 SCALE (AS NOTED) APR. 16, 1952

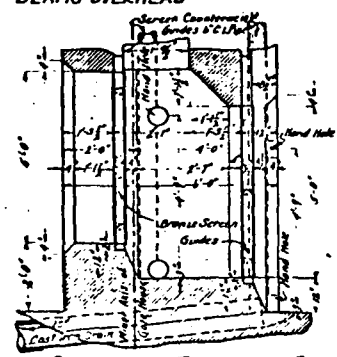




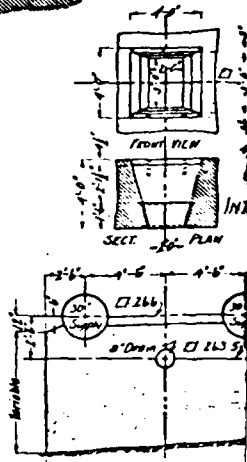
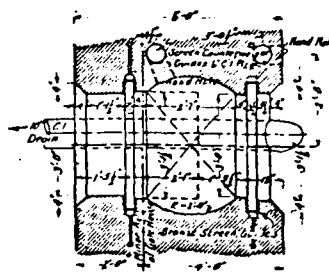
PLAN SHOWING ROOF FRAMING AND BEAMS OVERHEAD



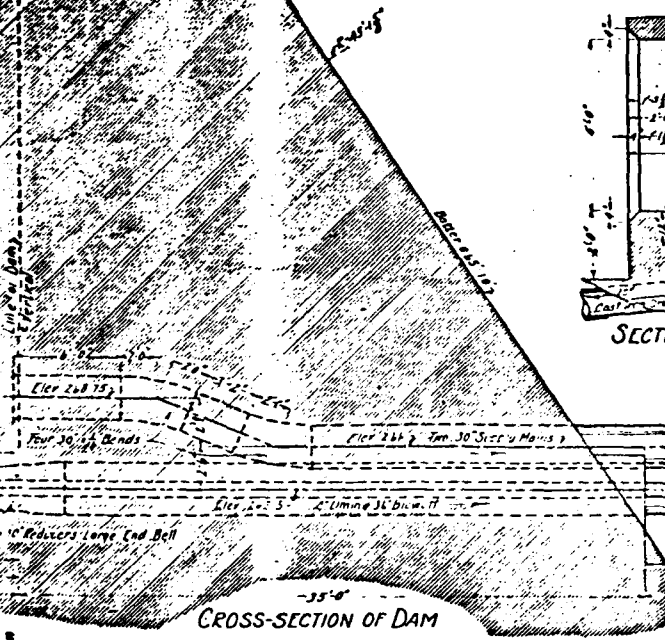
ELEV & CROSS-SECT. DOOR



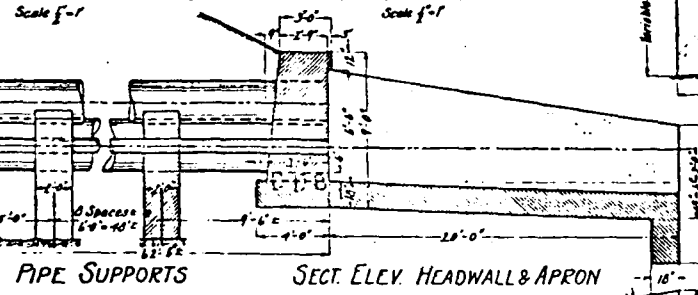
SECTIONAL ELEVATION - SCREEN OPENING - SECTIONAL PLAN



ELEV OF PIPE

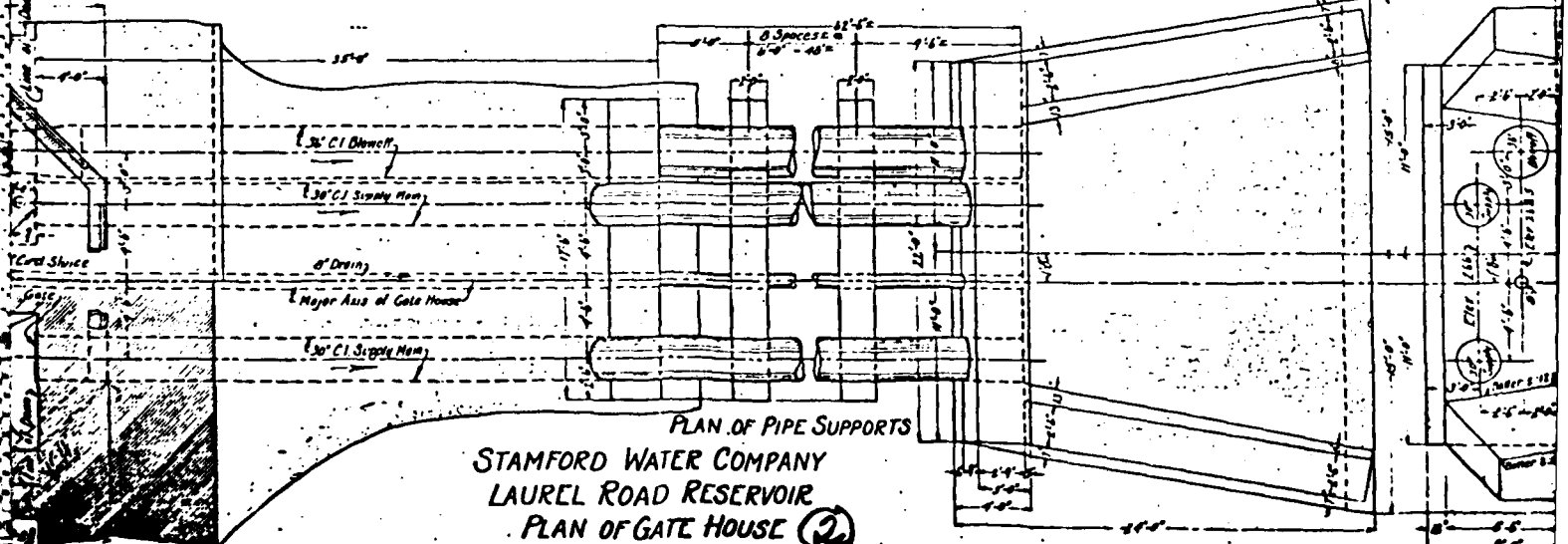


CROSS-SECTION OF DAM



PIPE SUPPORTS

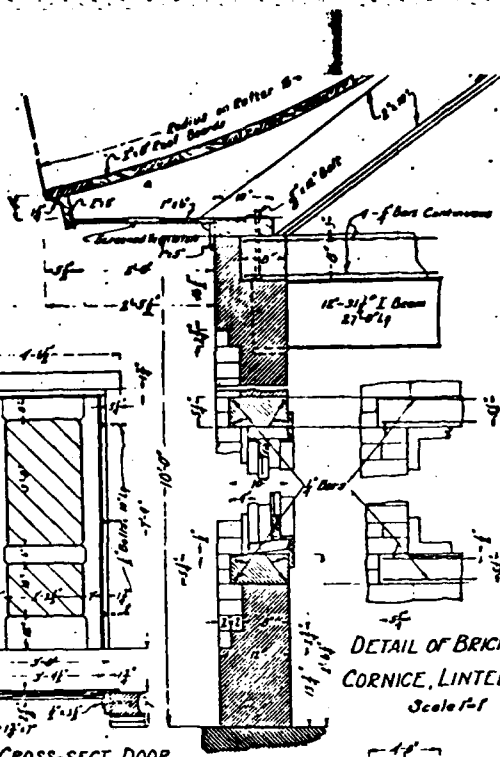
SECT. ELEV. HEADWALL & APRON



PLAN OF PIPE SUPPORTS

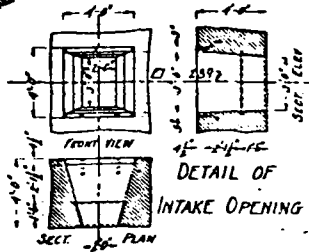
STAMFORD WATER COMPANY
LAUREL ROAD RESERVOIR
PLAN OF GATE HOUSE (2)

TOWNS OF STAMFORD & NEW CANAAN, CONN.

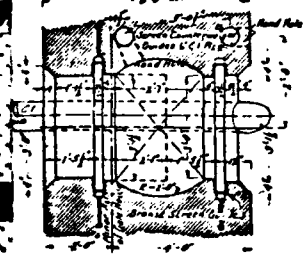


DETAIL OF BRICK WALL
CORNICE, LINTELS & SILLS
Scale 1/2"=1'-0"

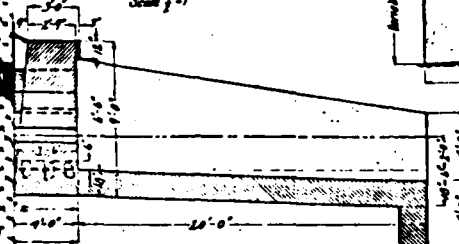
ELEV & CROSS-SECT. DOOR
Scale 1/2"=1'-0"



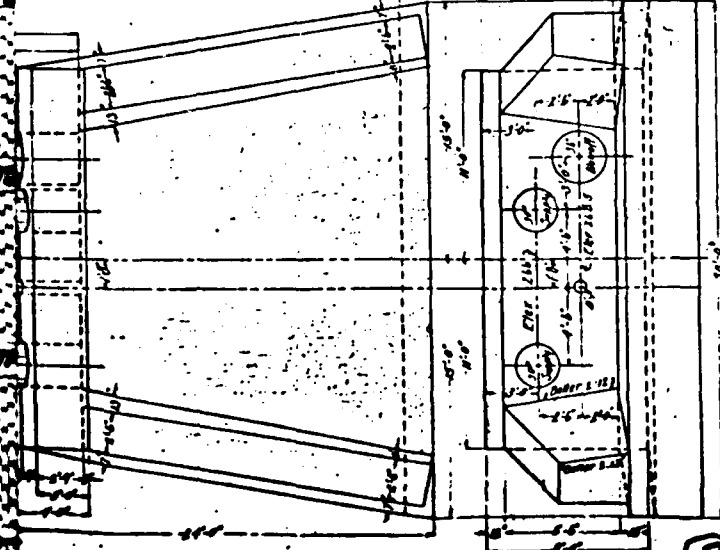
DETAIL OF
INTAKE OPENING
Scale 1/2"=1'-0"



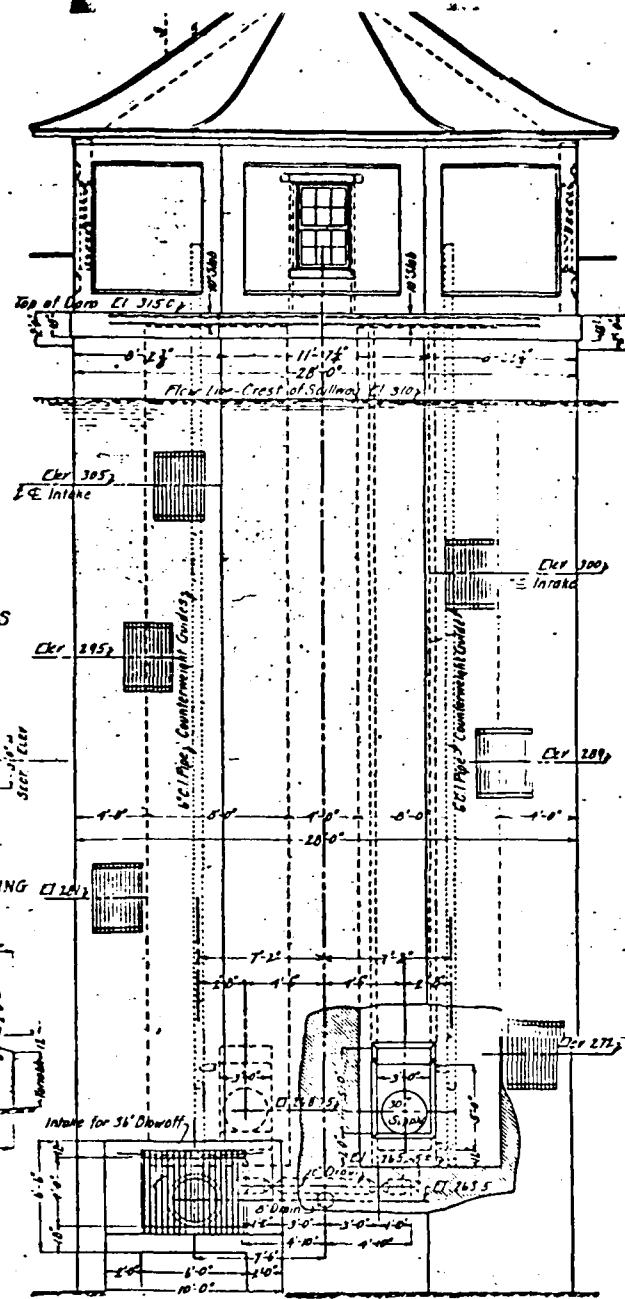
OPENING SECTIONAL PLAN
Scale 1/2"=1'-0"



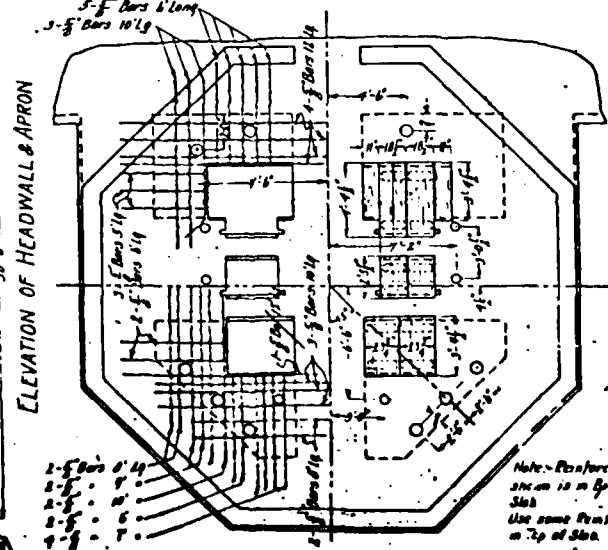
SECT. ELEV. HEADWALL & APRON
Scale 1/2"=1'-0"



PLAN OF HEADWALL & APRON
Scale 1/2"=1'-0"

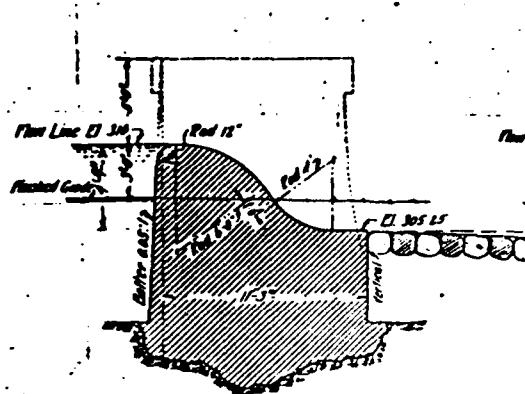


ELEVATION OF GATE HOUSE LOOKING DOWNSTREAM
Scale 1/2"=1'-0"

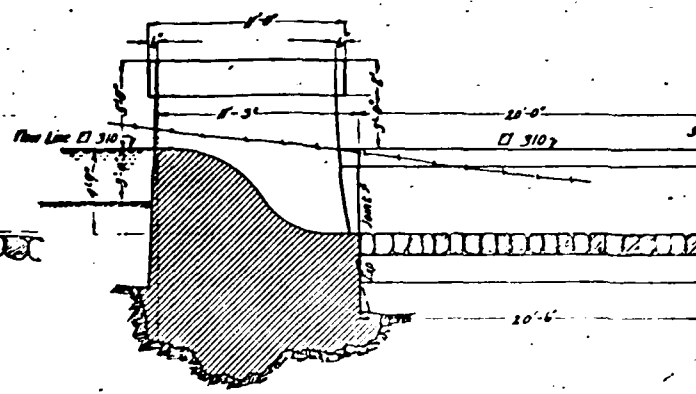


ELEVATION OF HEADWALL & APRON
Scale 1/2"=1'-0"

Note: Reinforcement
shown is in bottom
slab.
Use same Reinforcement
on top of slab.

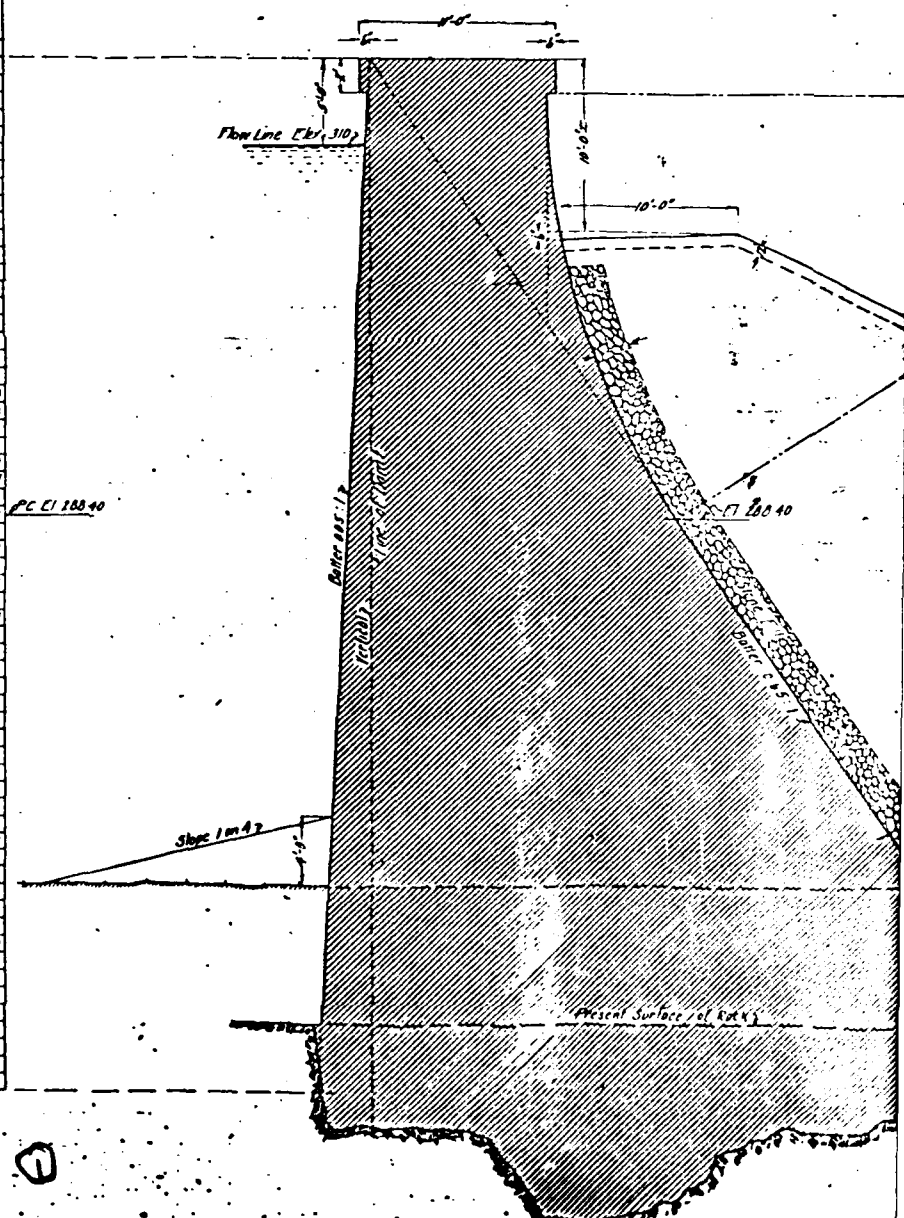


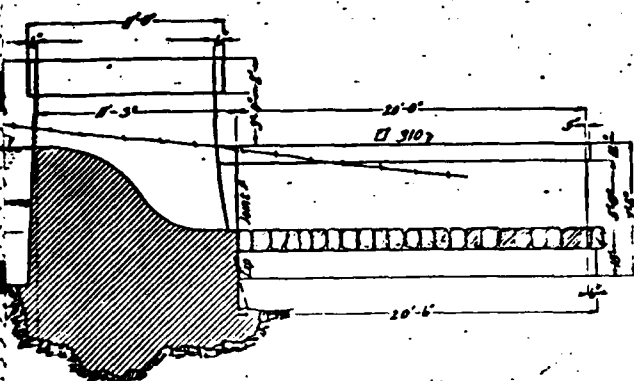
CROSS-SECTION OF SPILLWAY



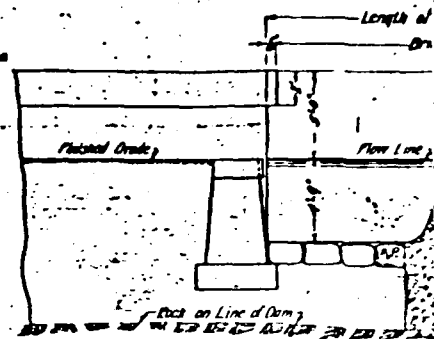
ELEVATION OF SPILLWAY ABUTMENT & RETAINING WALL

Dist. below Top	Dist.	Offset to Back	Offset to Face	Bottom Width of Section	Volume in cu yds. per lin. ft.
0	305	0.50	10.50	11.0	
1	304	0.50	10.50	11.0	0.807
2	313	0.50	10.50	11.0	0.815
3	312	0.15	10.85	10.17	1.190
4	311	0.20	10.80	10.30	1.510
5	310	0.25	10.75	10.35	1.840
6	309	0.30	10.70	10.40	2.335
7	308	0.35	10.65	10.45	2.726
8	307	0.40	10.60	10.50	3.123
9	306	0.45	10.55	10.55	3.516
10	305	0.50	10.50	11.22	3.910
11	304	0.55	10.45	11.46	4.350
12	303	0.60	10.40	11.72	4.787
13	302	0.65	10.35	12.01	5.226
14	301	0.70	10.30	12.32	5.672
15	300	0.75	10.25	12.67	6.140
16	299	0.80	10.20	13.05	6.616
17	298	0.85	10.15	13.45	7.106
18	297	0.90	10.10	13.83	7.610
19	296	0.95	10.05	14.27	8.131
20	295	1.00	10.00	14.75	8.660
21	294	1.05	9.95	15.25	9.213
22	293	1.10	9.90	15.70	9.790
23	292	1.15	9.85	16.20	10.393
24	291	1.20	9.80	16.72	11.009
25	290	1.25	9.75	17.25	11.647
26	289	1.30	9.70	17.83	12.310
27	288	1.35	9.65	18.40	12.997
28	287	1.40	9.60	19.00	13.710
29	286	1.45	9.55	19.65	14.440
30	285	1.50	9.50	20.30	15.195
31	284	1.55	9.45	21.00	15.974
32	283	1.60	9.40	21.70	16.787
33	282	1.65	9.35	22.40	17.624
34	281	1.70	9.30	23.10	18.485
35	280	1.75	9.25	23.80	19.370
36	279	1.80	9.20	24.50	20.279
37	278	1.85	9.15	25.20	21.212
38	277	1.90	9.10	25.90	22.169
39	276	1.95	9.05	26.60	23.150
40	275	2.00	9.00	27.30	24.155
41	274	2.05	8.95	28.00	25.185
42	273	2.10	8.90	28.70	26.239
43	272	2.15	8.85	29.40	27.317
44	271	2.20	8.80	30.10	28.419
45	270	2.25	8.75	30.80	29.544
46	269	2.30	8.70	31.50	30.692
47	268	2.35	8.65	32.20	31.862
48	267	2.40	8.60	32.90	33.054
49	266	2.45	8.55	33.60	34.267
50	265	2.50	8.50	34.30	35.499
51	264	2.55	8.45	35.00	36.750
52	263	2.60	8.40	35.70	38.019
53	262	2.65	8.35	36.40	39.306
54	261	2.70	8.30	37.10	40.610
55	260	2.75	8.25	37.80	41.930
56	259	2.80	8.20	38.50	43.265
57	258	2.85	8.15	39.20	44.615
58	257	2.90	8.10	39.90	45.980
59	256	2.95	8.05	40.60	47.359
60	255	3.00	8.00	41.30	48.752

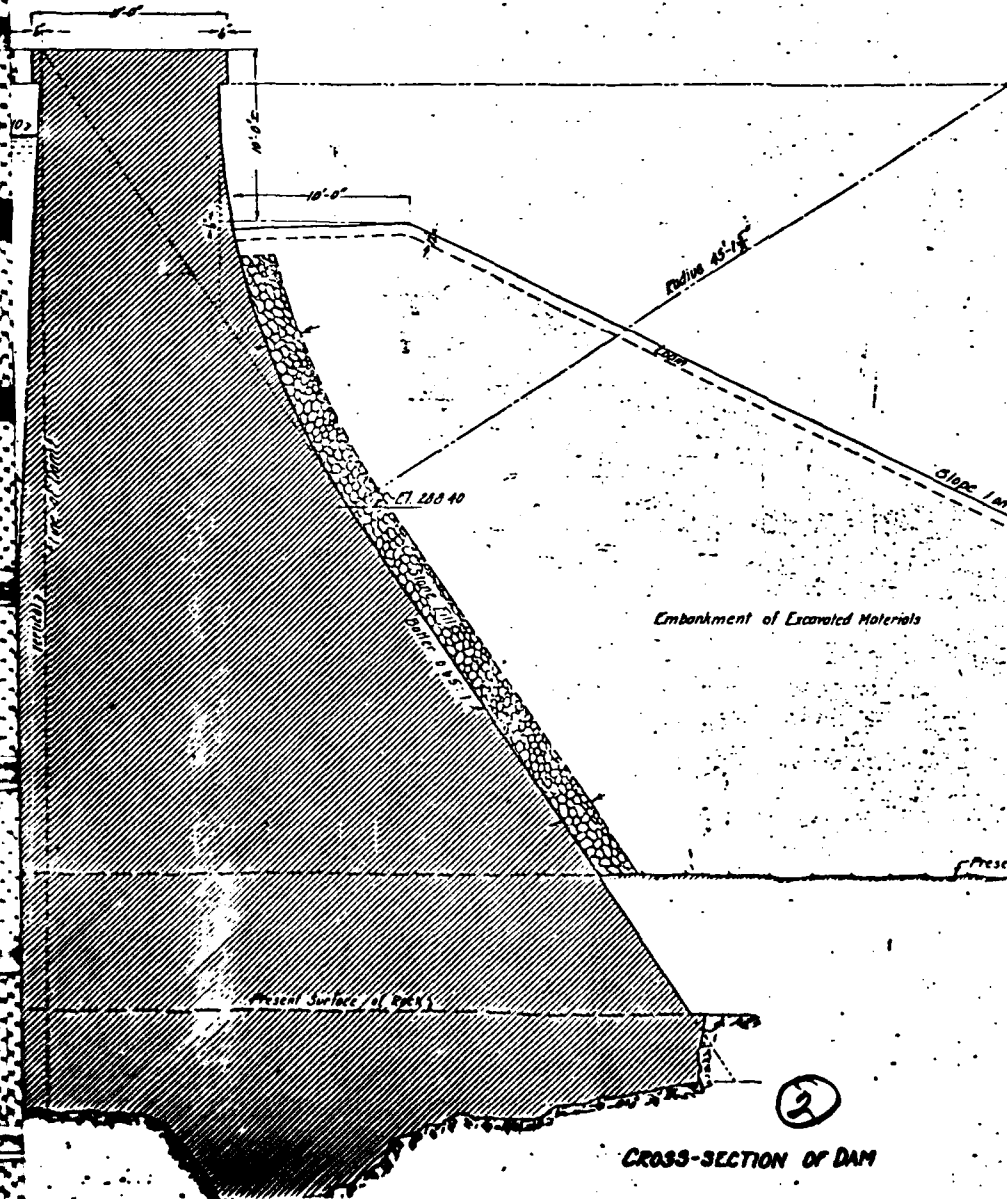




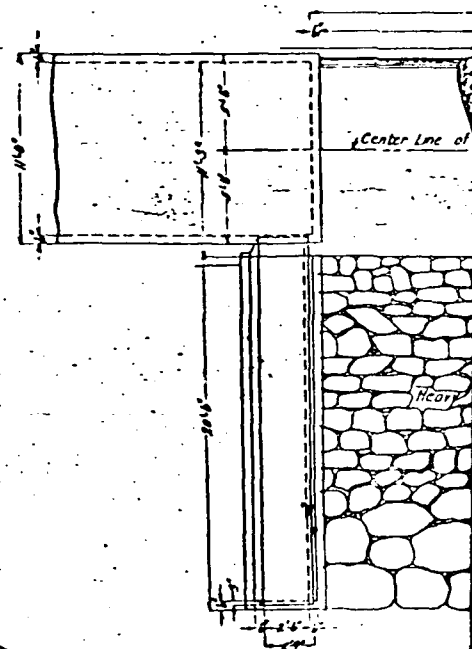
ELEVATION OF SPILLWAY ABUTMENT & RETAINING WALL



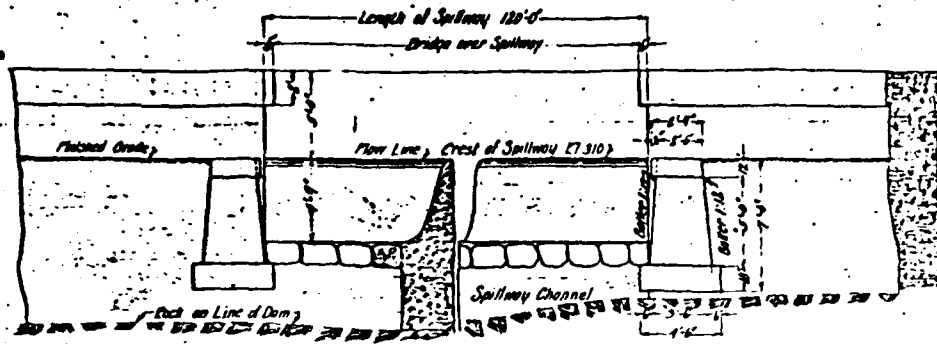
ELEVATION OF SPILLWAY



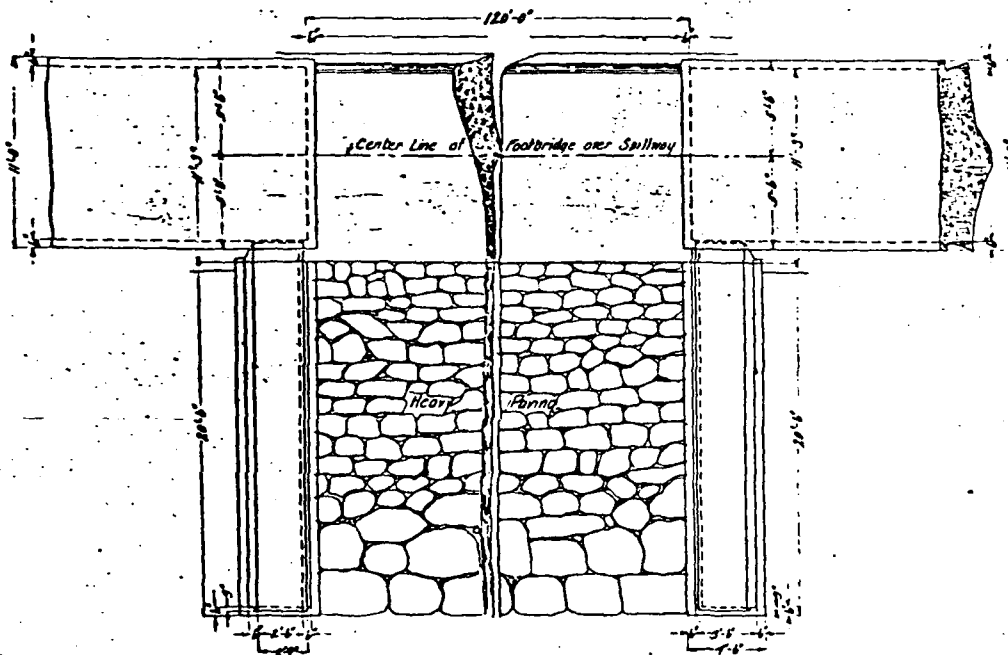
CROSS-SECTION OF DAM



PLAN OF SPILLWAY



ELEVATION OF SPILLWAY SHOWING RETAINING WALLS



PLAN OF SPILLWAY SHOWING RETAINING WALLS

STAMFORD WATER COMPANY
LAUREL ROAD RESERVOIR
PLAN OF DAM & SPILLWAY
TOWNS OF STAMFORD & NEW CANAAN, CONN.
Scale 1" = 10'
APRIL 24, 1922

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APPENDIX E

INFORMATION AS
CONTAINED IN THE NATIONAL
INVENTORY OF DAMS



INVENTORY OF DAMS IN THE UNITED STATES

IDENTITY NUMBER	DIVISION	CONCRETE	STATE	COUNTY	DIST.	NAME	LATITUDE (NORTH)	LONGITUDE (WEST)	REPORT DATE
CT	49	NED	CT	001	04	LAUREL RESERVOIR DAM	4109.9	7352.7	18 AUG 78

POPULAR NAME	NAME OF IMPOUNDMENT
	LAUREL RESERVOIR

REGION/DAM	RIVER OR STREAM	NEAREST DOWNSTREAM CITY - TOWN - VILLAGE	DIST FROM DAM (MI.)	POPULATION
01 07	HIPPOWAN RIVER	NORTH STAMFORD	0	1500

TYPE OF DAM	YEAR COMPLETED	PURPOSES	STRUCTURAL HEIGHT (FT.)	HYDRAULIC HEIGHT (FT.)	IMPOUNDING CAPACITIES (ACRE-FT.)
HECTPG	1925	S	50	45	MAXIMUM 1375 NORMAL 1000

DIST UWN FED R PRV/FED SCS A VER/DATE
N N N N N 16 AUG 78

REMARKS

D/S HAS	SPILLWAY	MAXIMUM DISCHARGE (CFS)	VOLUME OF DAM (CY)	POWER CAPACITY (MW)	INSTALLED	PHOTOGRAPHED	LENGTH (FT.)	WIDTH (FT.)	DEPTH (FT.)	NOTCH WIDTH (FT.)
1	1550 U	5900	155000							

OWNER	ENGINEERING BY	CONSTRUCTION BY
STAMFORD WATER CO	ALBERT B HILL	

DESIGN	CONSTRUCTION	OPERATION	MAINTENANCE
NONE	NONE	NONE	NONE

INSPECTION BY	INSPECTION DATE	AUTHORITY FOR INSPECTION
FLAMERTY-GIAVANA, ASSOC, P.C	15 JUN 78	P.L. 92 367

REMARKS

END

FILMED

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