

MX SITING INVESTIGATION GEDTECHNICAL SUMMARY

PRIME CHARACTERIZATION SITES RIO GRANDE/HIGHLANDS CANDIDATE SITING PROVINCE

PREPARED FOR SPACE AND MISSILE SYSTEMS ORGANIZATION (SAMSO) NORTON AIR FORCE BASE, CALIFORNIA

Ż

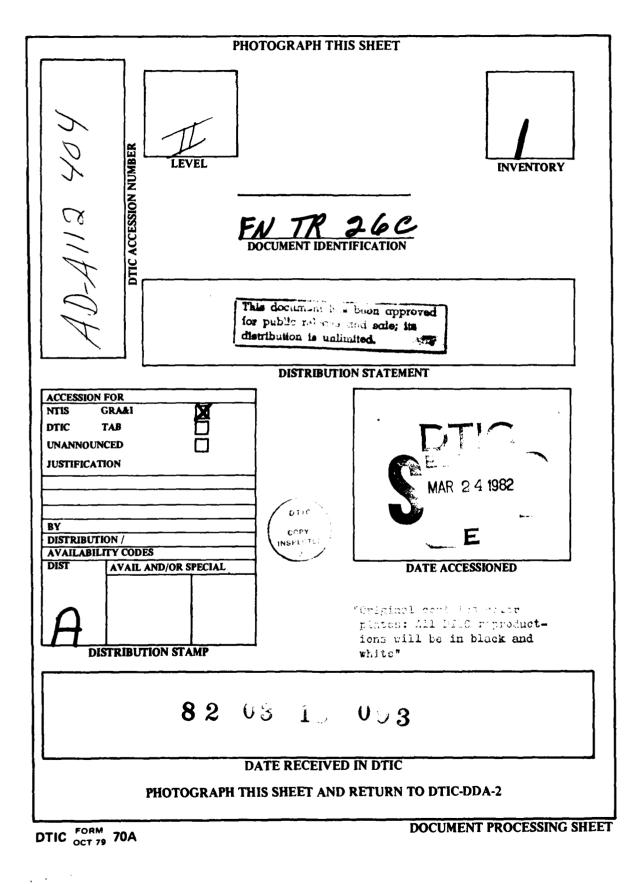
2404

-

A 1

B





.

: 1

SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered) READ INSTRUCTIONS BEFORE COMPLETING FORM REPORT DOCUMENTATION PAGE 1. REPORT NUMBER 3. RECIPIENT'S CATALOG NUMBER 2. GOVT ACCESSION NO. FN TR 26C 4. TITLE (and Subtitle) 5. TYPE OF REPORT & PERIOD COVERED Prime (Inprovterization sites Ricksrande) Highland's condiciate 6. PERFORMING ORG. REPORT NUMBER T. AUTHOR(A) SITING Prounces EN-TR-260 FUCTO Notional, Inc. FC4704-77-C COO 9. PERFORMING ORGANIZATION NAME AND ADDRESS 10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS Atten warden at Ton Chamer C. Forger yEnded PC 150X 7-55 0 24312 F Los mants Contral 2 DER CONTROLLING OFFICE NAME AND ADDRESS STATES STAT 1. CONTROLLING OFFICE NAME AND ADDRESS 12. REPORT DATE 15 Feb 79 (re.) The area of a second second 13. NUMBER O PAC Section Acres Cost Cost 5.1.515 14. MONITORING AGENCY HAME & ADDRESS(if different from Controlling Office) 15. SECURITY CLASS, (of this report) 15# DECLASSIFICATION DOWNGRADING SCHEDULE 16. DISTRIBUTION STATEMENT (of this Report) Orstalization Contrasted 17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from Report) Distribution Contrarited 18. SUPPLEMENTARY NOTES 19. KEY WORDS (Continue on reverse side if necessary and identify by block number) rain, site, sorticial HURFTO DED - DIADICECE BEANA LEAGINGOTION any the server here and should be Ina wek mj. 2) 1.8 ABSTRACT (Continue on reverse sitte Il necessary and identify by block number) rea real This report presents the more sum and identify by block number) This report presents the results of peotechnical field incestige-tions peoperned in the Jornial per moerto + san simon Characterization sites, lecated in central New Macico in the Ric scande (Bridiciate siting Province + in southwaterin Acizona in the Mill - In Arizona in the Highlands. DD 1 JAN 73 1473 EDITION OF 1 NOV 65 IS OBSOLETE

SELUPIT CLASSIFICATION OF THIS HAGE When have Science)

MX SITING INVESTIGATION GEOTECHNICAL SUMMARY PRIME CHARACTERIZATION SITES RIO GRANDE/HIGHLANDS CANDIDATE SITING PROVINCES

Prepared for:

U. S. Department of the Air Force Space and Missile Systems Organization (SAMSO) Norton Air Force Base, California 92409

Prepared by:

Fugro National, Inc. 3777 Long Beach Boulevard Long Beach, California 90807

29 September 1978 15 February 1979 (rev.)

TUBRO NATIONAL ING.

PRIME CHARACTERIZATION SITES RIO GRANDE/HIGHLANDS CSPs

ERRATA

Replace the following figures with revised ones which accompany this sheet: Figures 6 (p. 16), 7 (p. 18), 13 (p. 38), and 14 (p. 40).

Replace the following tables with revised ones which accompany this sheet: Tables 5 (p.19 and 20), 14 (p. 41), and 15 (p. 42).

The following corrections are to be made to the original text:

page 2, Figure 2: Generalized geologic map base revised, see Figure 6.

page 7, Section 2.2, line 3: Change "The..., intermediate..."
V to read "Fluvial deposits cover approximately six percent
of the area, younger alluvial fan deposits cover approximately 14 percent, intermediate..."

page 25, first 3 lines: Delete first three printed lines, duplication from page 23.

page 33, Figure 10: Generalized geologic map base revised, see Figure 13.

TURRO NATIONAL INC.

FOREWORD

This report was prepared for the Department of the Air Force, Space and Missile Systems Organization (SAMSO) in compliance with conditions of Contract No. F04704-77-C-0010, and is a geotechnical summary of the prime Characterization sites in the Rio Grande and Highlands Candidate Siting Provinces (CSPs). The prime site in the Rio Grande CSP is Jornada del Muerto, New Mexico and the prime site in the Highlands CSP is San Simon Valley, Arizona.

The report presents representative data obtained from geotechnical field investigations performed at both sites as part of the Characterization program. The information obtained from these studies, in combination with data obtained in the Screening studies, has been used for geotechnical ranking (FN-TR-25).

-TURNO NATIONAL INC

ļ

TABLE OF CONTENTS

•.

		Page
1.0	INTRODUCTION	1
2.0	JORNADA DEL MUERTO SITE	7
2.1	SCOPE OF INVESTIGATION	7
2.2	SURFICIAL GEOLOGY AND TERRAIN	7
2.3	SUBSURFACE CONDITIONS	12
2.3.1	Soil Profiles	12
2.3.2	Depth to Shallow Rock and Water	12
2.3.3	Basin Configuration	17
2.4	GEOPHYSICAL PROPERTIES	17
2.5	ENGINEERING PROPERTIES	23
3.0	SAN SIMON SITE	29
3.1	SCOPE OF INVESTIGATION	29
3.2	SURFICIAL GEOLOGY AND TERRAIN	29
3.3	SUBSURFACE CONDITIONS	34
3.3.1	Soil Profiles	34
3.3.2	Depth to Shallow Rock and Water	34
3.3.3	Basin Configuration	39
3.4	GEOPHYSICAL PROPERTIES	39
3.5	ENGINEERING PROPERTIES	43
4.0	DISCUSSION	48
4.1	JORNADA DEL MUERTO SITE	48
4.2	SAN SIMON SITE	48
5.0	CONSTRUCTION CONSIDERATIONS	49
6.0	CONCLUSIONS	52

-TURRO NATIONAL, INC.

- -

--

m

.

LIST OF FIGURES

Text Figures	Page
1	Characterization Sites and Field Activities, Rio Grande CSP
2	Characterization Sites and Field Activities, Highlands CSP
3	Generalized Geologic Map and Field Activities, Jornada del Muerto 11
4	Soil Profile AA', Jornada del Muerto 14
5	Soil Profile BB', Jornada del Muerto 15
6	Generalized Geologic Map and Selected Subsurface Features, Jornada del Muerto . 16
7	Generalized Geologic Cross Section, Jornada del Muerto
8,9	Range of Gradation of Geologic Units, Jornada del Muerto
10	Generalized Geologic Map and Field Activities, San Simon Valley
11	Soil Profile AA', San Simon Valley 36
12	Soil Profile BB', San Simon Valley 37
13	Generalized Geologic Map and Selected Subsurface Features, San Simon Valley 38
14	Generalized Geologic Cross Section, San Simon Valley 40
15⁄	Range of Gradation of Geologic Units, San Simon Valley

.....

LIST OF TABLES

.

Text Tables		Page
1	Scope of Field and Laboratory Activities, Jornada del Muerto	8
2	Engineering Field Activities - Borings, Jornada del Muerto	9
3	Engineering Field Activities - Trenches, Jornada del Muerto	10
4	Description of Surficial Geologic Units, Jornada del Muerto	13
5	Shallow Seismic Refraction Results, Jornada del Muerto	.9, 20
6	Deep Seismic Refraction Results, Jornada del Muerto	21
7	Downhole Velocity Survey Results, Jornada del Muerto	22
8	Range of Engineering and Geophysical Properties, Jornada del Muerto	24
9	Summary of Chemical Test Results, Jornada del Muerto	28
10	Scope of Field and Laboratory Activities, San Simon Valley	30
11	Engineering Field Activities - Borings, San Simon Valley	31
12	Engineering Field Activities - Trenches, San Simon Valley	32
13	Description of Surficial Geologic Units, San Simon Valley	35
14	Shallow Seismic Refraction Results, San Simon Valley	41
15	Conductivity Survey Results, San Simon Valley	42
16	Range of Engineering and Geophysical Properties, San Simon Valley	44
17	Summary of Chemical Test Results, San Simon Valley	46

TUORO RATIONAL INC.

- .

٠

Contract States Transfer Toront Proves and a series of the

-

LIST OF APPENDICES

•.

Appendix		Page
A	General Geotechnical Information	A-1
В	Geotechnical Data - Jornada del Muerto	B-1
с	Geotechnical Data - San Simon Valley	C-1

iv

÷.,

•

1.0 INTRODUCTION

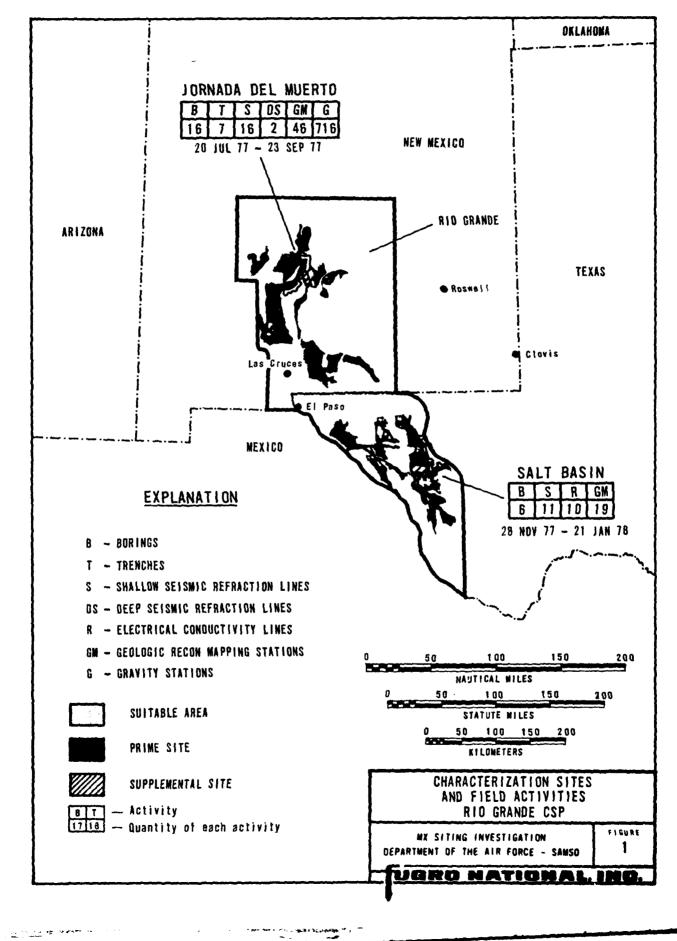
This report presents the results of geotechnical field investigations performed in the Jornada del Muerto and San Simon characterization sites, located in central New Mexico in the Rio Grande Candidate Siting Province (CSP) and in southeastern Arizona in the Highlands CSP, respectively. These provinces are two of six selected for geotechnical characterization studies. This report presents representative data collected and analyzed for these sites. Access to the remaining data can be arranged through SAMSO/MNND, Norton Air Force Base, California.

The Rio Grande CSP lies within the states of New Mexico and Texas (Figure 1). It is characterized by predominately north and northwest trending mountain ranges separated by fault controlled closed basins possessing a variety of geologic and engineering conditions.

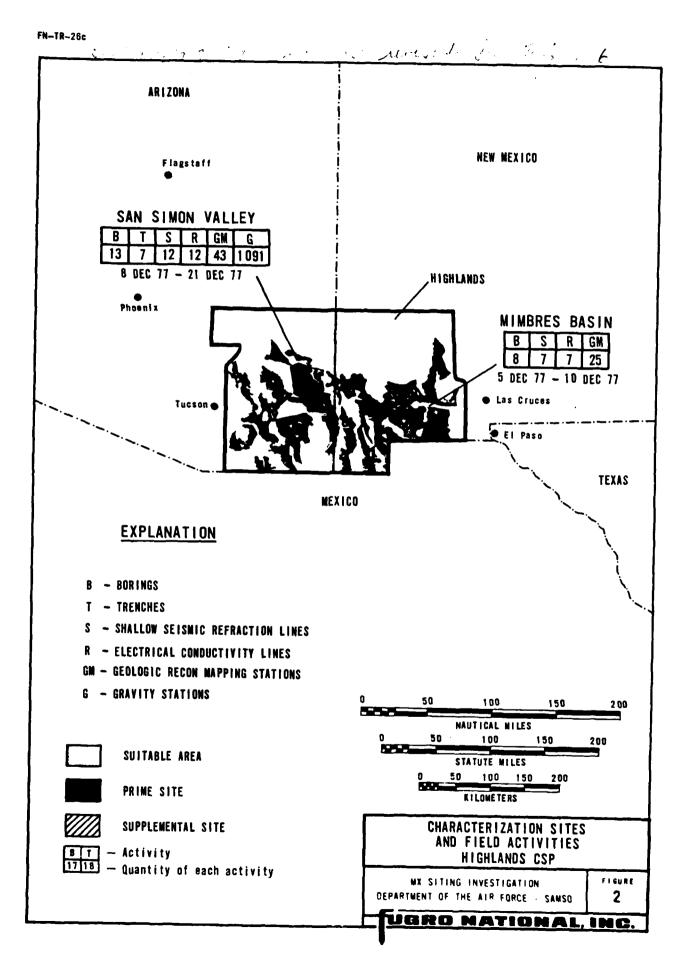
The Highlands CSP lies within the states of New Mexico and Arizona (Figure 2) and is characterized by variable geologic structures and topography. North to northwest trending mountain ranges comprise from 25 to 33 percent of its area. Drainage is moderately well developed, however, a few basins are closed and contain playas.

Suitable areas for deployment of MX missile system remaining after Intermediate Screening were divided into CSPs based on similar geotechnical characteristics. The results of Intermediate Screening (FN-TR-17) indicated that existing data

TUBRO NATIONAL INC



Marked and a second



Í

Ş

and the

were not adequate in type or level of detail for follow-on geotechnical and geo-environmental evaluations, screening, site selection, and ranking studies. Therefore, the Characterization studies were developed to provide a rapid, relatively inexpensive method of gathering geotechnical data in small areas (maximum about 700 nm²; 2400 km²) which are considered to be representative of a larger area within the CSP.

Emphasis was placed on the collection of information allowing characterization of geological units with respect to the construction aspects of MX missile basing options. Objectives of the Characterization studies were to obtain data that address the following geotechnical aspects:

- o Surficial geology and terrain
- o Subsurface conditions
- o Geophysical properties
- o Engineering properties

Although the program originally emphasized data collection for the trench and horizontal shelter basing modes, the data were utilized for evaluation of the vertical shelter basing mode as well. Characterization was, therefore, a refinement of the screening process whereby the necessary geotechnical information was developed to support the broader MX system design activities. These activities were taking place concurrently and provided a more firm basis from which to geotechnically rank the remaining suitable area considering different alternative basing modes.

UGRO NATIONAL INC

4

ł.

Two Characterization sites (one prime and one supplementary) were selected within each CSP (Figures 1 and 2). This report discusses only the prime site within each CSP. The characterization site selection process began with a delineation of geotechnically similar areas within each CSP having analogous depositional and geologic histories, rock and water depths, and tectonic settings. Once these areas had been identified, non-geotechnical factors were applied to delineate the actual Characterization site boundaries. These non-geotechnical selection factors included access, proximity to support facilities, environmental sensitivities, and local logistical requirements.

Geologic, geophysical, and soils engineering techniques were used to determine the surface and subsurface geotechnical conditions in Jornada del Muerto and San Simon. These include:

- o Analysis of available data
- Aerial photo interpretation of surficial geologic units utilizing black and white stereographic pairs at scales of approximately 1:30,000 (Jornada del Muerto) and 1:60,000 (San Simon)
- Geologic field check of aerial photo interpretation and determination of physical properties of the surficial units at selected field stations
- Shallow and deep seismic refraction, down hole seismic velocity, and electrical conductivity surveys to obtain subsurface profile information

TUGRO NATIONAL INC

- Gravity and ground magnetic surveys to aid in interpretation of basin configuration
- Drilling and trenching to determine subsurface soil
 characteristics and obtain soil samples
- Laboratory testing of soil samples to determine engineering properties

Prior to initiating any field work, an archeological and environmental inspection was conducted at each site to ensure minimal impact to the local environment and to avoid damage to archeologic and historic sites. To further minimize potential impacts, all field activities were performed adjacent to existing roads or other previously disturbed areas.

Site access to the Jornada del Muerto and Salt Basin characterization sites in the Rio Grande CSP was coordinated through the Base Engineers Office, White Sands Missile Range, and the U.S. Army Corps of Engineers, respectively. Access to characterization sites in the Highlands CSP was gained through BLM permits and the U.S. Army Corps of Engineers.

2.0 JORNADA DEL MUERTO SITE

The Jornada del Muerto characterization site covers an area of 330 nm² (1132 km²) in Socorro and Sierra counties, New Mexico. The site is bounded by mountain ranges on the east and south. The Rio Grande River lies just outside the site area to the west. Chupadera Mesa lies to the north. A network of paved and graded roads as well as four-wheel drive trails provide access within the site.

7

2.1 SCOPE OF INVESTIGATION

Scope of geologic, geophysical, and soils engineering field activities performed at the site and laboratory tests performed on soil samples from the site is presented in Table 1. Detailed information about the soils engineering field activities (17 borings and seven trenches) is summarized in Tables 2 and 3. Locations of all the field activities are shown in Figure 3.

2.2 SURFICIAL GEOLOGY AND TERRAIN

Alluvial fan deposits of younger and intermediate age and eolian sheet sand are the predominant surficial geologic units within the Characterization site (Figure 3). Fluvial deposits cover approximately 6 percent of the area, younger alluvial fan deposits cover approximately 14 percent, intermediate alluvial fan deposits cover 22 percent, and eolian sheet sand covers approximately 45 percent. Playa and older lacustrine deposits cover approximately three and nine percent of the surface area, respectively. These deposits do not represent a large percentage of the surface area, but they are generally of great thickness and interfinger with alluvial

TUBRO NATIONAL INC

TYPE OF ACTIVITY	NUMBER OF ACTIVITIES
Geological mapping stations	46
Shallow refraction	16
Deep refraction	2
Downhole velocity	3
Gravity survey	7 16

GEOLOGY AND GEOPHYSICS

ENGINEERING

NUMBER OF BORINGS	NOMINAL DEPTH FEET (NETERS)
13	100 (30)
3	300 (91)
NUMBER OF TRENCHES	NOMINAL DEPTH Feet (Neters)
1	16 (5)
6	18 (6)

ENGINEERING-LABORATORY TESTS

. . .

TYPE OF TEST	NUMBER OF TESTS
Noisture/density	237
Specific gravity	19
Sieve analysis	130
Hydrometer	53
Atterberg limits	90
Consolidation	10

TYPE OF TEST	NUMBER OF TESTS
Unconfined compression	29
Triaxial compression	15
Direct shear	30
Compaction	7
CBR	3
Chemical analysis	8

JORNADA DEL MUERTO, NEW MEXICO, RIO GRANDE CS MX SITING INVESTIGATION DEPARTMENT OF THE AIR EARCE - SAMSO 1	SCOPE OF FIELD AND LABORATOR Activities	Y
WX SITING INVESTIGATION	JORNADA DEL MUERTO, NEW MEXICO, RIO	GRANDE CSP
octonenter of the win fonce " Jamau	MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - SAMSO	TABLE 1

BORING NUMBER	TOTAL DEPTH Feet (Meters)	TYPE OF DRILL RIG USED	TYPE OF SAMPLES* Obtained
JM-8-1	102.0 (31.1)	Rotary Wash	B, P
JM-8-2	108.0 (32.9)	Rotary Wash	B, P, C
J M-B-3	105.0 (32.0)	Rotary Wash	B, P
JM-B-4	102.5 (31.2)	Rotary Wash	Р
J M -B-5	100.0 (30.5)	Rotary Wash	B, P
JM-8-6	101.0 (30.8)	Rotary Wash	B, P
JMB-7	102.5 (31.2)	Rotary Wash	P, D, C
JM-8-8	99.0 (30.2)	Rotary Wash	B, P
JM-8-9	102.5 (31.2)	Rotary Wash	8, P, SS
JM-8-10	301.5 (91.9)	Rotary Wash	B, P
JMB-11	304.0 (92.7)	Rotary Wash	B, P
JM-8-12	102.5 (31.2)	Rotary Wash	B, P
JM-B-13	101.0 (30.0)	Rotary Wash	8, P
J N- B-14	102.5 (31.2)	Rotary Wash	B, P
JM-B-15	302.5 (92.2)	Rotary Wash	B, P
JM-B-17	102.0 (31.1)	Rotary Wash	B, P, SS

***P** = Pitcher sample (undisturbed)

D = Fugro Drive sample (relatively undisturbed)

B = Bulk sample (disturbed, but representative)

SS = Split Spoon sample (disturbed, but representative)

C = Rock Core

ENGINEERING FIELD ACTIVITIES - BORINGS JORNADA DEL MUERTO, NEW MEXICO RIO GRANDE CSP WX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE SANSO TUGERO MATIONAL, INC.

-

FN-TR-28c

Ŋ

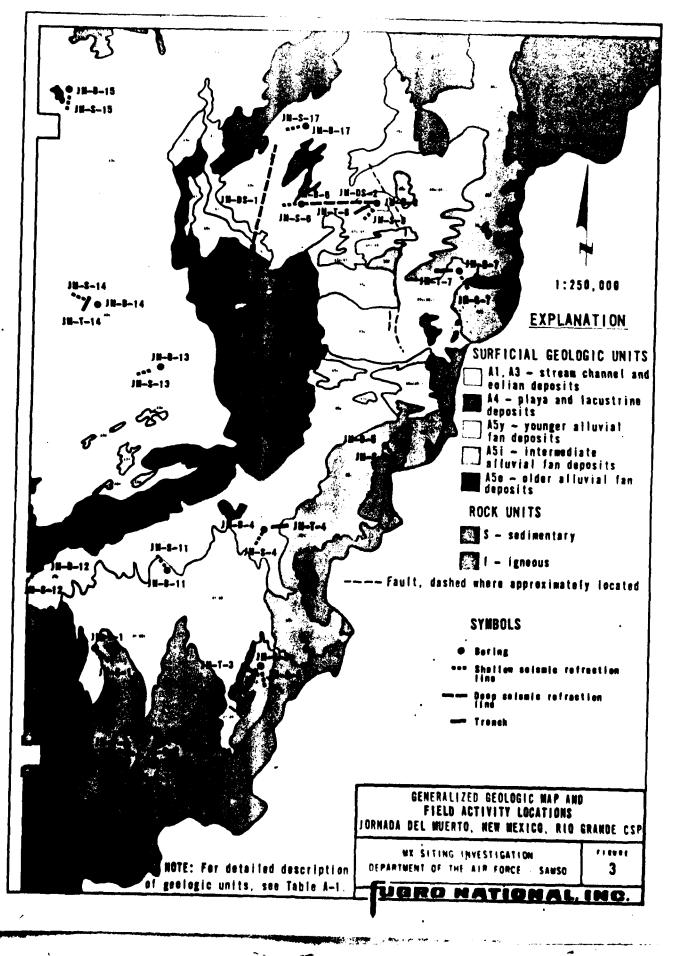
1

Ş.

TRENCH NUMBER	TOTAL DEPTH Feet(meters)	STABILITY OF VERTICAL EXCAVATION WALLS
JM-T-2	16.5 (5.0)	stable; 15.5-16.5" (4.7-5.0m), stage II Caliche
JM-T-3	18.0 (5.5)	stable; 16.5-18.0' (5.0-5.5m), stage II Caliche
JM-T-4	18.0 (5.5)	stable
JM-T-7	18.0 (5.5)	stable; 1-3' (0.3-0.9m), stage I Caliche
JM-T-8	18.0 (5.5)	stable; 0-7' (0-2m) some sloughing into trench: 7-17' (2.1-5.2m) stable; 17-18' (5.2-5.5m) stage I Caliche, 3-9' (0.9-2.7m)
JM-T-12	18.0 (5.5)	stable; 3-12' (0.9-3.7m), stage I Caliche
JM-T-14	18.5 (5.6)	unstable; heavy sloughing, 0-5' (0-1.5m) stable; stage I Caliche, 5-10' (1.5-3.0m) unstable; some sloughing into trench, 10-18.5' (3.0-5.6m)

FIELD ACTIVITIES - Del Muerto, New Mei Rio grande CSP	
NG INVESTIGATION THE AIR FORCE - SAMSO	TABL





deposits in the subsurface within the construction zone. The alluvial fan deposits are typically silty sands with gravel, ranging from sandy gravels near the mountain fronts to sandy silts near the playas. Playa and older lacustrine deposits are generally clayey silts. These units along with the remaining surficial units are described in Table 4.

Surface slopes and depths of drainage incision vary with geologic units, both generally increasing with proximity to the mountain fronts (Table 4). Maximum surface slope is five percent with typical slopes of less than one percent. Drainages are typically shallow (less than 10 feet; 3 m), with gently sloping sides except near mountainous areas. Locally they may be deep (greater than 20 ft; 6 m) and steep sided.

2.3 SUBSURFACE CONDITIONS

2.3.1 Soil Profiles

Varying thicknesses of eolian sheet sand and alluvial fan deposits typically overlie several hundred feet of older lacustrine deposits in the Jornada site. The subsurface conditions and the composition of the soils with depth are illustrated by the soil profiles shown in Figures 4 and 5. Eolian sheet sand and alluvial deposits predominantly consist of coarse-grained soils whereas lacustrine deposits consist of fine-grained soils.

2.3.2 Depth to Shallow (<150 ft;<46 m) Rock and Water Figure 6 shows the portions of the site in which rock and water are estimated to be encountered within a depth of 150 feet

TUBRO NATIONAL INC

SURFICIAL GEOLOGIC UNIT (a)		THICKNESS		USCS	AREAL EXTENT (SITE)			
	GEOLOGIC AGE	FEET (METERS)	DESCRIPTIVE NAME(S)	(b)	nm ² (km ²)	PERCENT		
Fluvial Deposits (A1)	Quaternary	Unknown	Silty Sand with Clay	SM	20 (69)	6		
Eolian Deposits Sheet and Dune Sand (A3s, A3d)	Quaternary	0-18 (0-6)	Sand, Silty Sand, Silty Sand with Clay	SP, SM	149 (511)	45		
Playa Deposits (A4)	Quaternary	Unknown	Silt and Clay	ML, CL	10 (34)	3		
Older Lacustrine and Playa Deposits (A4o)	Quaternary~ Tertiary	Unknown	Clay, Gypsiferous Silt. Silty Sand	CL. ML. SM	30 (103),	9		
Younger Alluvial Fan Deposits (A5y)	Quaternary	Unknown	Silty and Clayey Sand with Gravel, Silt, Clay	SM, SC	46 (158)	14		
Intermediate Alluvial Fan Deposits (A5i)	Quaternary	Unknown	Silty Sand with Gravel and Cobbles	SM	73 (250)	22		
Older Alluvial Fan Deposits (A5o)	Quaternary- Tertiary	Unknown	Sandy Gravel with Cobbles and Boulders	SP-GW	3 (10)	1		
						_		

terrander (* Sand all and steller in second and and second a

-

NOTES:

1

- (a) For generic description of geologic units, see Table A-1.
- (b) For description of USCS, see Table A-2.
- (c) For description of stage of caliche, see Figure A-1.
- (d) Mixed with A51 deposits in the southerm part of the site; designated A1 A5y on Figures 3 and 6
- (e) Dune sands comprise one percent of the site area.
- (f) This gypsiferous deposit occurs extensively in the subsurface.

- -

REAL EXTI	ENT (SITE)		PROPERTIE	SURFACE						
n ² (km ²) PERCENT GF		GRADATION	CEMENTATION	MAXIMUM Grain Size	PAVEMENT/ Patina	STAGE OF Caliche (c)	SLOPE (PERCENT)	DRAINAGE DEPTHS FEET(METERS)	NOTES	
?0 (69)	9) 6 Well None- Moderate		Sand	None/None	I	<	None	(d)		
9 (511)	45	Poor- Moderately well	None- Weak	Sand	None/None	I	<]	<1	(e)	
0 (34)	3	Poor	None- Weak	Silt	None/None	None	< 1	None		
0 (103),	9	Poor	Weak-Strong	Sand	None/None	None-11	<1	0-5 (0-2)	(†)	
6 (158)	14	Moderately well	None-Weak	Sand	None/None	I	1-4	<1-6 (<1-2)		
(250)	22	Moderately well-Well	Moderate- Strong	Cobble	Poor/Poor	п	2-9	5-25 (2-8)		
3 (10)	1	Moderately well	Moderate- Strong	Boulder	None /None	п-ш	9-12	50-100 (16-33)		
							 		<u> </u>	
									ļ	

•

DESCRIPTION OF SURFICIAL GEOLOGIC UNITS JORNADA DEL MUERTO, NEW MEXICO, RID GRANDE CSP NX SITING INVESTIGATION DEPARTMENT OF THE AIR FURCE SAMSO TUBBECO MATICOMAL, INC.

• • • •

- -

an san s

. -

.

2

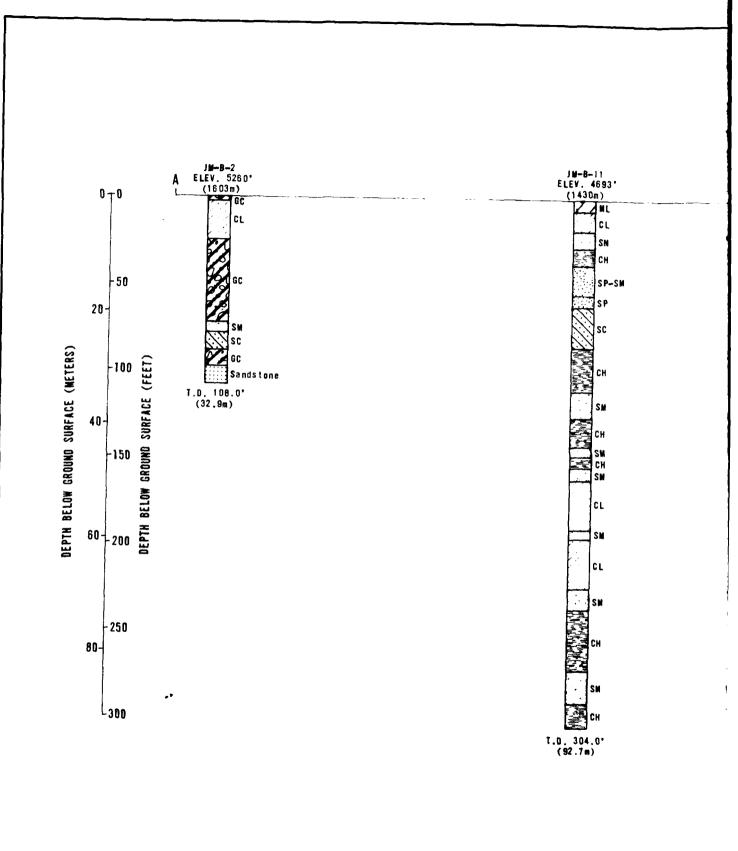
. . .

 \mathbb{D}^{r}

-

.

) In ۰. ۲۰۰۰



- ---

~--

.

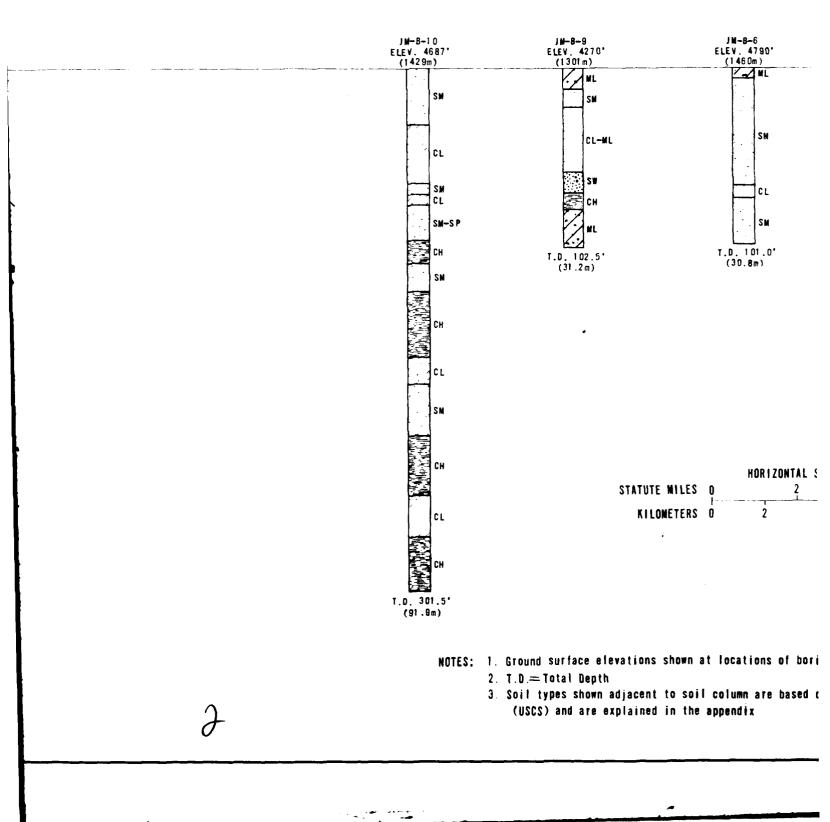
···· ··· ·

.

ł

,

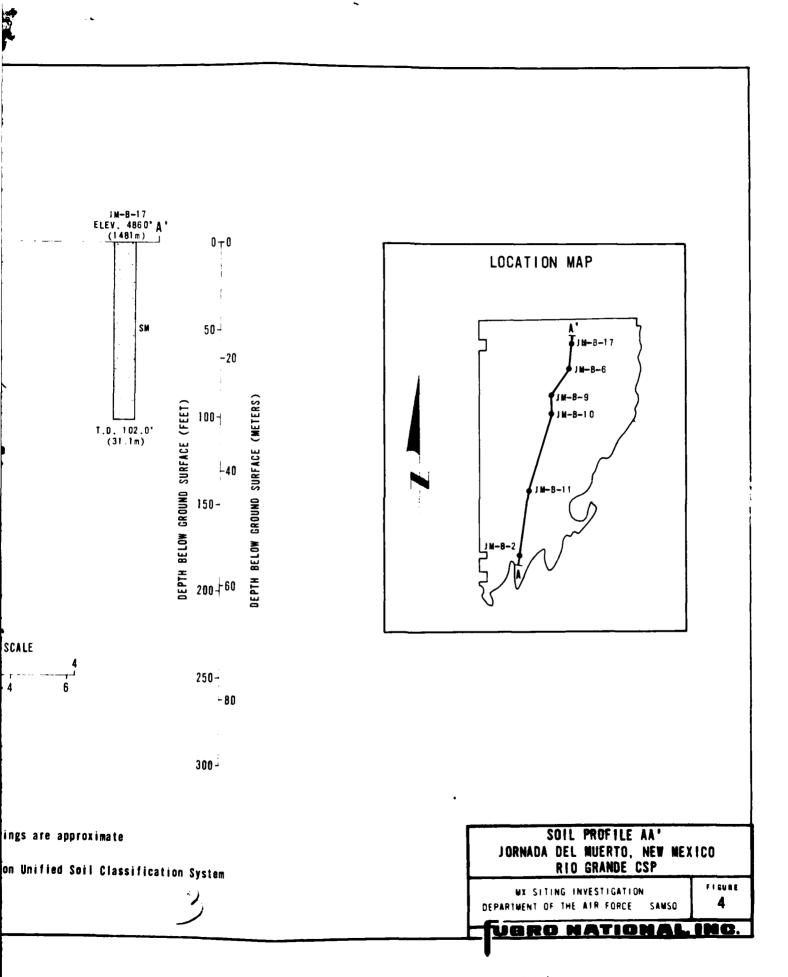
25

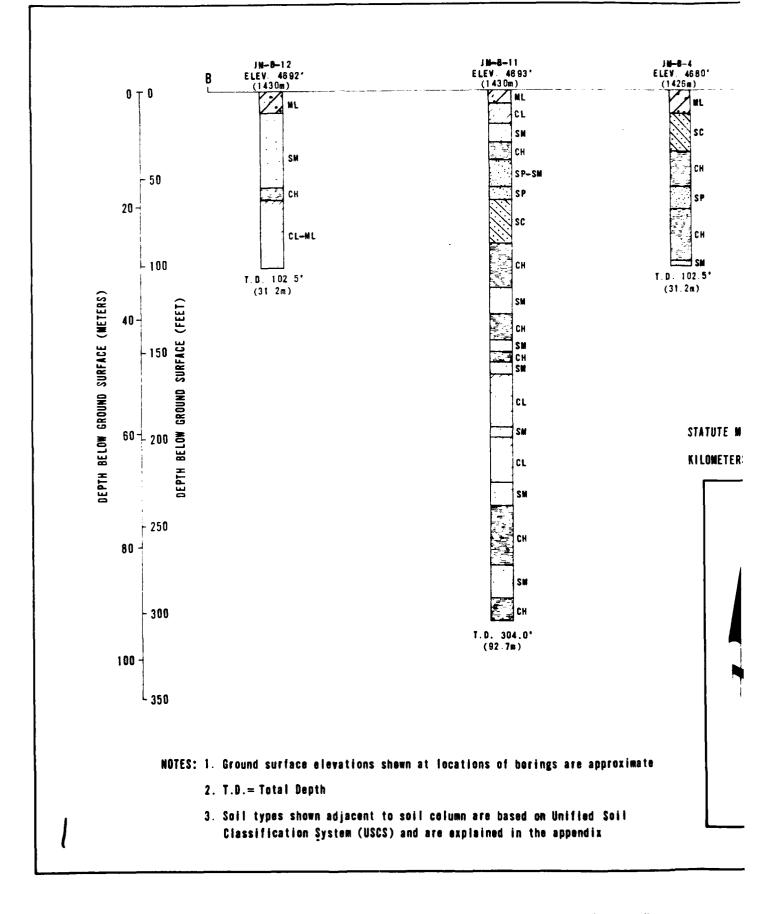


.....

.

Ì

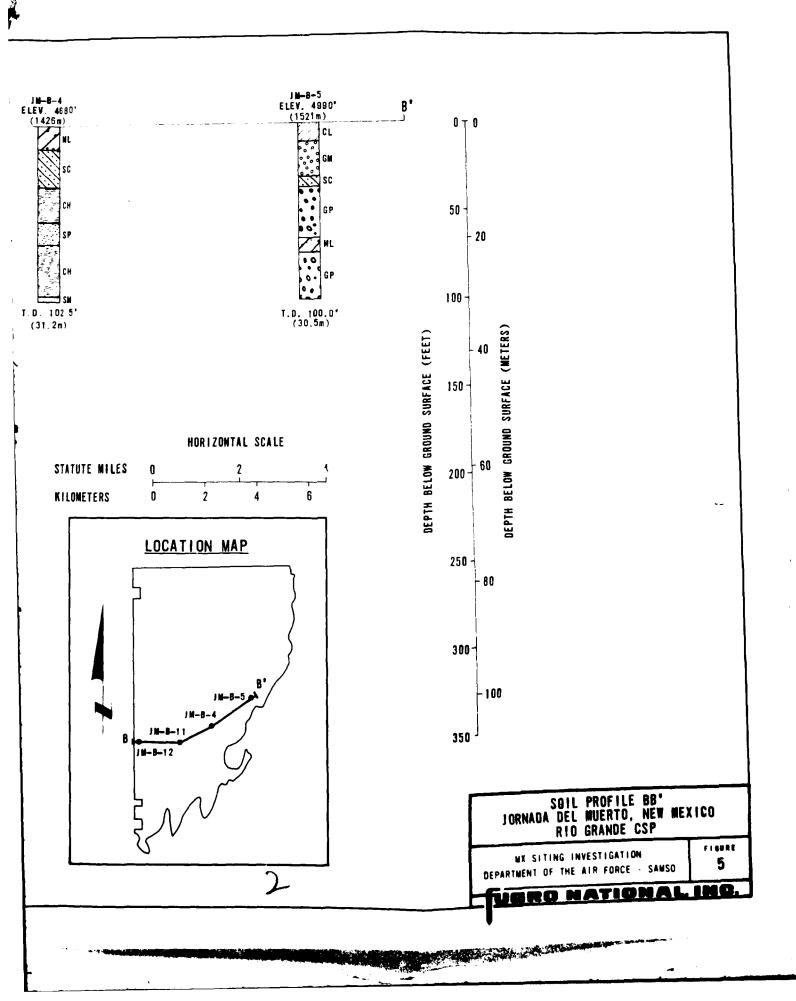


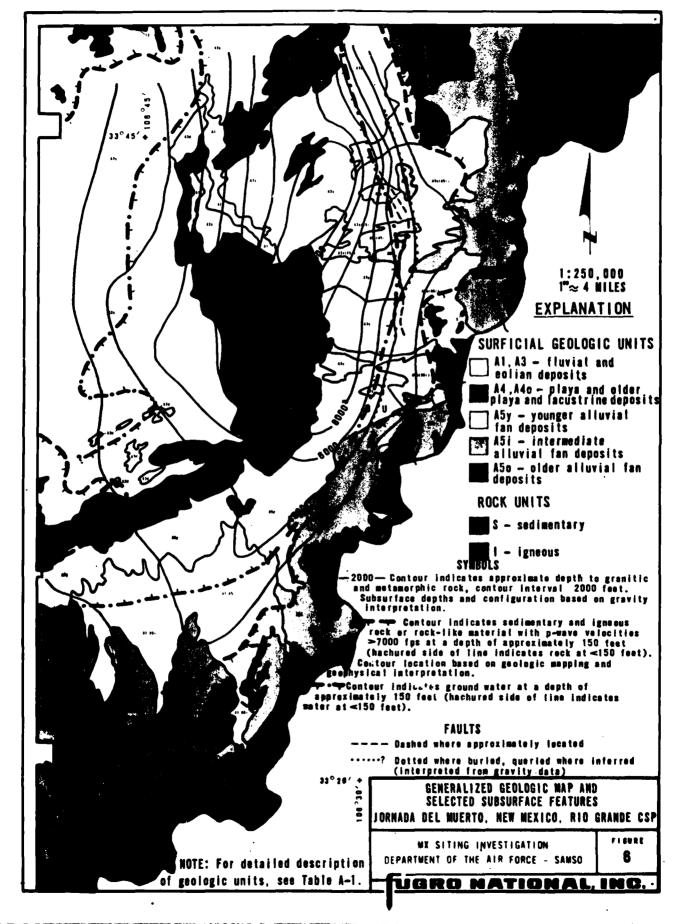


and a second and a second and a second and a second a s

~ .

Sec. 100





below ground surface. Shallow rock comprises approximately ten percent of the site based on boring, seismic, gravity, geologic, topographic and other available data. Ground water will be encountered at depths of less than 150 feet (46 m) over approximately 30 percent of the site area. Areas of shallow ground water will generally coincide with the surficial distribution of older lacustine and playa deposits. Elsewhere, local areas of perched water may also be encountered at depths of less than 150 feet (46 m).

2.3.3 Basin Configuration

Geophysical investigations indicate bedrock within 50 feet (15 m) of the surface near the mountains in the southern part of the site. Deep seismic lines near the middle of the valley encountered high velocity materials, probably well indurated older alluvium, at a depth of 450 feet (140 m).

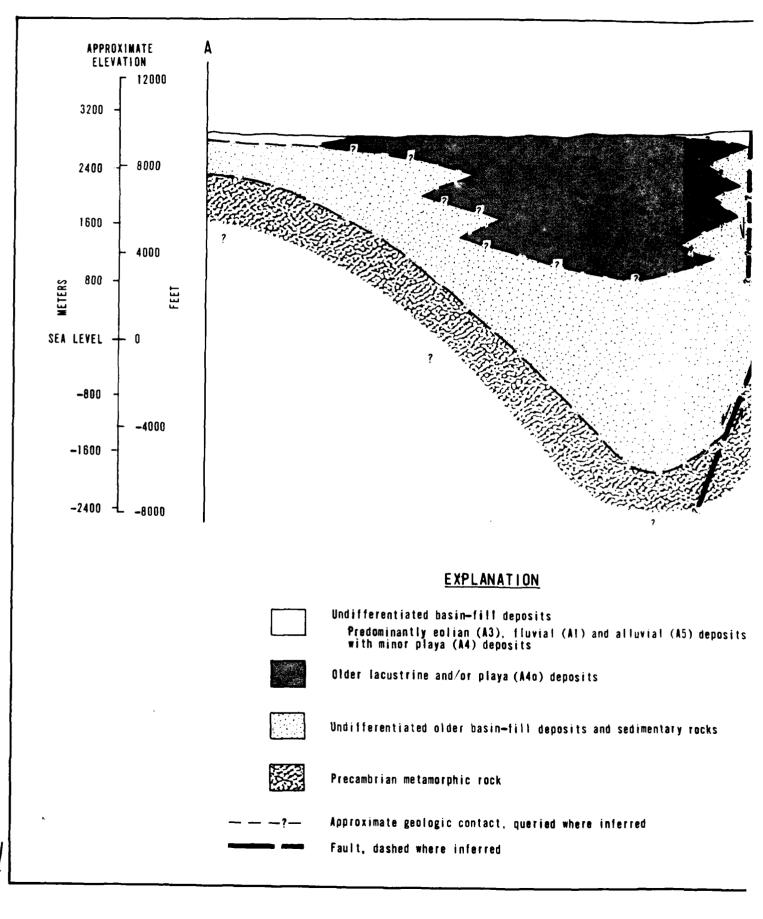
Gravity data indicate the basement topography of the site is dominated by a 16,000-foot (4900 m) deep basin that is bounded on the east by a steep fault and on the west by a gently sloping plane in the vicinity of the generalized geologic cross-section (Figure 7). Near the south end of the site, an east-west trending basement ridge separates the large basin from a smaller one. Buried basalt from the Jornada Malpais basalt field may overlie much of this ridge.

2.4 GEOPHYSICAL PROPERTIES

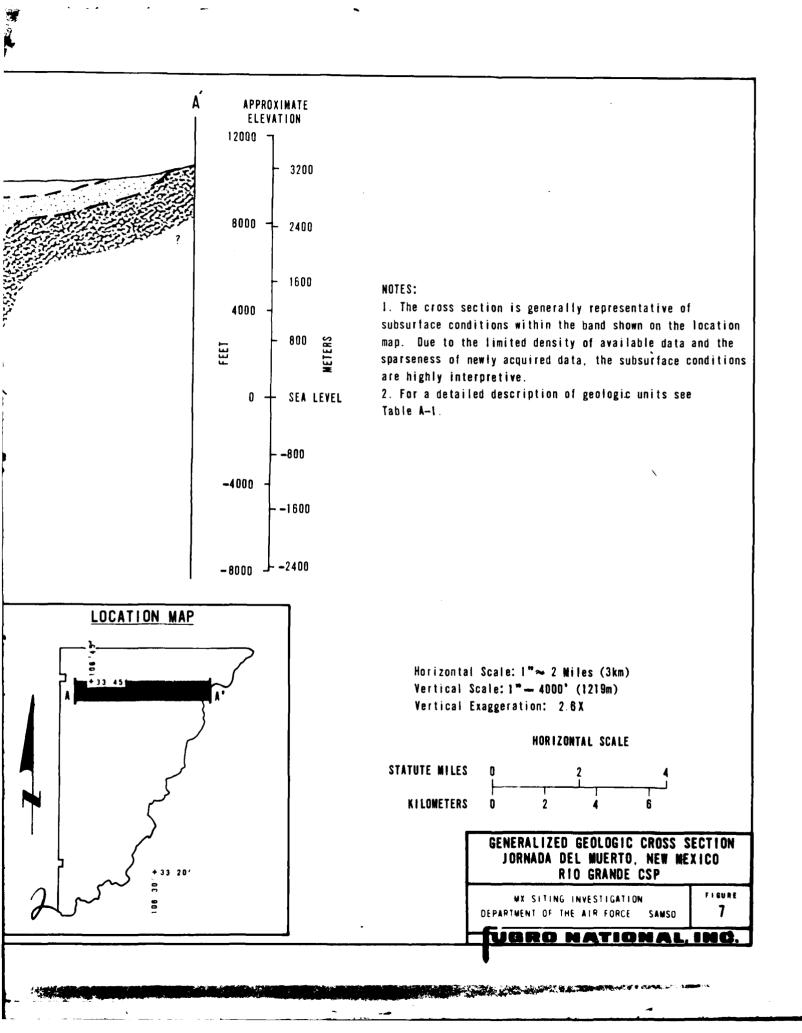
Results of shallow and deep seismic refraction surveys and downhole velocity surveys are presented in Tables, 5, 6 and 7.

1

11



A SHOW THE STORE



			133 (41)	272 (83)	264 (80)	246 (75)	261 (80)	243 (74)		215 (66)	246 (75)		313 (95)	FEET (M)		
340 (104) 1300 (2530)														FT (M) F		
•	< 10600 (3231) -	•	•			-		X	Å	Å	A	Å	A		40 130 140	
4800 (1463)	7500 (2286)	7200 (2195)	6050 (1844)	2600 (792)	3460 (1055)	3100 (945)	2800 (853)	3750 (1143)	7200 (2195)	6450 (1966)	4750 (1448)	7000 (2134)	3300 (1006)		20 25 30 35 1 1 1 1 1 2 30 35 3 60 70 80 90 100 110 0 6PTH INTERVAL	
1700 (518) ◄	▲ 1700 (518) ►	 ▲ 1850 (564) → ▲ 4450 (1356) → 	(518)	1700 (518) -	1740 (530) -	1500 (457) ◄	1500 ► (457) ►	¥	1560 (475) 4500 -	▲ 1700 (518) ► 3800 (1158) ►		200 (366)←	< 1500 (457) > <		5 10 20 30 40	
1-S-H	JM-S-2	5-S-#1	JM-S-4	J.M-S-5	J M- S-6	1 M- S-7	J H- S-8	JN-S-9) M-S-10	11-S-NI	JM-S-12	JM-S-13	JM-S-14			•
									•	J ORN	ADA 	DEL RIO NG IN	MUE GR/	RTO, ANDE (NEW MEX	
	1700 (518) - 4800 (1463) - 8340 (8300 (1700 (518) ▲ 4800 (1463) ■ 340 (▲ 1700 (518) ▲ 7500 (2286) ■ 10600 (3231)	1700 (518) ▲ 4800 (1463) > 340 (▲ 1700 (518) ★ 7500 (2286) > ▲ 1700 (518) ★ 4450 (1356) ★	1700 (518) ▲ 4800 (1463) > 340 (▲ 1700 (518) ★ 7500 (2286) > 8300 (▲ 1850 (564) ★ 4450 (1356) > > 1700 ★ 10500 (3231) > > (518) ★ 6050 (1844) > >	$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	$ \begin{array}{ $	$\begin{array}{ $	$\begin{array}{ $	$ \begin{array}{ $	$ \begin{array}{ $	$ \begin{array}{ c c c c c c c c c c c c c c c c c c $	$ \begin{array}{ c c c c c c c c c c c c c c c c c c $

*ROCK Exclusion Depth To 7000 FPS (2134 MPS)	251 (7B)	240 (73)											FEET (M)			
DEEPER Refractors Depth Velocity	i '	1											FT (M) FPS (MPS)			
	A	•		 			_						╎	45	140 150	
														40	130 1	
														35	110 120	
(SAW					-	Î								30	100	
ION FPS (()	89)												25 -	80 90 VAL	
VELOCITY DISTRIBUTION FPS (MPS)	3050 (930)	3900 (1189)												20	60 70 80 DEPTH INTERVAL	
VELOCIT														- 12	20	
														5 10	20 30 40	
	1600 - (488) -	V												V	5 10	
SEISMIC LINE NO.	JIM-S-15 (- 11-S-HI												METERS 0	FEET 0	
								SHA	LLO	W SE Ada	ISM DEL RIO	IC F Mue Gra	EFRA RTO, NDE	CTI NE CSP	ON RE W Mex	SUI 1 C
									MX	SITI	NGIN	IVEST	IGATIC)N	_ [Ţ

FN-TR-26c

ŧ.

			.
VELOCITY Layer	COMPRESSIONAL WAVE Velocity FPS (MPS)	AVERAGE THICKNESS FT (M)	COMMENTS
1	2000 (610)	20 (6)	
2	4000 (1219)	150 (46)	
3	6200 (1890)	300 (91)	Begin Saturated Sediments
4	7500 (2286)	700 (213)	
5	9000 (2743)	Undetermined	

.....

DEEP SEISMIC REFRACTION RES Jornada del muerto, new med Rio grande CSP	
MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - SAMSO	TABLE

UBRO NATIONAL, INC.

£2.

VELOCITY DISTRIBUTION FPS (MPS) (613) 5000 (1524) 2030 (619) 5000 (1524) 2030 (619) 2930 (693) 4150 (1264) 6600 (2012) (501) 2120 (646) 4500 (1372) (561) 2120 (646) 1450 (1372) (561) 1200 (365) 1460 (445) (561) 1200 (365) 1460 (445) 5 10 15 20 20 40 15 20 35 6 70 80 90 100 140 6 70 80 90 10 120 140	VELOCITY DISTRIBUTION FPS (MPS) 2010 (613) 2030 (619) 5000 (1524) 1120 (341) 2030 (619) 5000 (1524) 1650 (503) 4150 (1264) 2030 (693) 1000 (305) 4150 (1264) 5000 (1372) 1000 (305) 1650) 2120 (646) 4500 (1372) 1840 (561) 1200 (366) 1460 (445) 5 10 15 20 35 40 4 5 10 50 80 70 80 30 100 140 4 5 10 15 20 25 30 30 40 4 5 10 50 80 70 80 30 100 120 40 4 5 10 50 80 70 80 100 110 120 140	VELOCITY DISTRIBUTION FPS (MPS) Z010 (613) Z010 (1524) 5000 (1524) 2010 (301) 4150 (1264) 2030 (619) 2930 (693) 1120 (341) Z010 (1264) 2030 (1324) 2000 (1629) 1000 (305) 4150 (1264) 2120 (646) 4500 (1372) 5000 (1829) 1000 (305) 1650 2120 (646) 4500 (1372) 5000 (1829) 1000 (305) 160 (197) 1200 (306) 1460 (445) 40 2010 (201) 1200 (306) 1200 (306) 1460 (445) 40 5 10 15 20 23 30 140 40 5 10 20 30 10 1200 (306) 10 140 40 5 10 20 30 30 10 120 10 120 140 40 5 10 20 30 10 10 120 10 120 140 40 6 10 10 10 10 10 120 140 40	VELOCITY DISTRIBUTION FPS (MPS) Z010 (613) Z010 (1524) 5000 (1524) 2010 (301) 1150 (1264) 2030 (813) 2930 (893) 1120 (301) 4150 (1264) 2030 (137) 5000 (1629) 1000 (305) 1650) 2120 (646) 4500 (137) 5000 (1829) 1000 (305) 1650) 1200 (366) 1460 (445) 1460 (445) 100 (305) 10 1200 (366) 1460 (145) 1460 (445) 5 10 15 20 23 30 1460 (445) 5 10 20 30 10 1200 (366) 1460 (45) 40 40 5 10 20 30 10 120 10 120 10 140 40 5 10 20 30 10 10 120 10 120 10 140 40	$\frac{1120(31)}{14-0H-15} \frac{1120(31)}{1120(31)} \frac{1120(31)}{2000(1524)} \frac{1120(31)}{2000(1524)} \frac{1120(31)}{2000(1524)} \frac{1120(31)}{2000(1524)} \frac{1120(31)}{2000(1524)} \frac{1120(31)}{2000(1524)} \frac{1000(1323)}{2000(1524)} \frac{1120(31)}{2000(1322)} \frac{1000(1323)}{2000(1322)} \frac{1000(1322)}{2000(1322)} \frac{10000(1322)}{2000(1322)} 1000(1322$	WAVE TYPE	•	S	a .	s	a .	5	
VELOCITY DISTRIBUTION FPS (MPS) (613) 5000 (1524) 2030 (619) 2030 (693) 2030 (1264) 2930 (893) 4150 (1264) 6600 (2012) (503) 2725 (830) (613) 2725 (830) (561) 1200 (366) 5 10 15 20 30 4500 (1372) 5 10 15 20 20 30 90 90 5 10 15 20 20 50 60 70 660 70 80 90	VELOCITY DISTRIBUTION FPS (MFS) 2010 (613) 5000 (1524) 1120 (341) 2030 (619) 5000 (1524) 1120 (305) 4150 (1264) 6600 (2012) 1000 (305) 1500 (1264) 7125 (830) 1000 (305) 1650 2120 (646) 4500 (1372) 1840 (561) 1200 (365) 1200 (365) 300 5 10 15 20 25 30 5 10 25 60 70 80 90 10 5 10 25 60 70 80 90 10 5 10 25 60 70 80 90 10	VELOCITY DISTRIBUTION FPS (MPS) 2010 (613) 5000 (1524) 1120 (341) 2030 (619) 5000 (1524) 1650 (503) 4150 (1264) 8600 (2012) 1000 (305) 1550 2120 (646) 4500 (1372) 1840 (561) 1200 (365) 1200 (365) 300 5 10 15 20 25 30 6 0 10 15 20 25 30 5 10 15 20 25 30 90 100 5 10 15 20 26 10	Dommidle VELOCITY DISTRIBUTION FPS (MPS) SURVEY MO. VELOCITY DISTRIBUTION FPS (MPS) SURVEY MO. 2010 (613) 5000 (1524) IM-DH-15 1120 (341) 2030 (619) 2830 (883) IM-DH-15 1120 (341) 2030 (619) 2830 (883) IM-DH-15 1120 (31) 1650 (503) 4150 (1264) 2830 (893) IM-DH-10 1840 (561) 1650 2120 (646) 2125 (830) IM-DH-10 1840 (561) 1200 (365) 2125 (830) 2125 (830) Image: Image	DOMMOLE XELOCITY OISTRIBUTION FPS (MPS) UNHER NO. 2010 (613) XELOCITY OISTRIBUTION FPS (MPS) UNHER NO. 2010 (613) 2030 (613) 2030 (613) UNHULL 2010 (613) 2030 (613) 2030 (613) UNHULL 2030 (613) 2030 (613) 2030 (1324) UNHULL 2030 (613) 2120 (613) 2123 (630) UNHULL 3060 (303) 1200 (303) 2123 (630) UNHULL 3060 (303) 2120 (646) 2125 (830) UNHULL 1200 (305) 100 (305) 100 (305) METH 0 300 10 100 FEET 0 30 40 50 60 FEET 0 30 40 50 60 100						6000 (1829)	1460 (445)	35 40 120 130 140
(613) (613) 2030 (619) (1264) (503) (939) (939) (939) (939) 20 30 40	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	DOWNHOLE IM-DH-15 IM-DH-15 IM-DH-15 IM-DH-15 IM-DH-15 IM-DH-16 IM-DH-15 IM-DH-10 IM-10 IM-DH-10 IM-10 I	DOWNHOLE SURVEY NO SURVEY NO S	ON FPS (MPS)	5000 (1524)	2930 (893)	6600 (2012)	2725 (830)	4500 (1372)	(366)	30 100 100
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	DOWNHOLE VICE VICE VICE VICE VICE VICE VICE VIC	DOWNHOLE VELOC ITY SURVEY RESL 1000 (302) 1000 (302) 1000 (303) 4120 (13) 1150 (341) 5030 1000 (1302) 1020 (1303) 1000 (1303) 1000 (1303) 1000 (1303) 1000 (1303) 1000 (1303) 1000 (1303) 1000 (1303) 1000 NADA DEL MUERTO, NEW MEXT RIO GRANDE CSP WX SITING INVESTIGATION	VELOCITY DISTRIBUTI					2120 (646)	1200	
	5 5	5 5	DOWNHOLE VELOCITY SURVEY RESU	DOWNHOLE VELOCITY SURVEY RESU JORNADA DEL MUERTO, NEW MEXT RIO GRANDE CSP MX SITING INVESTIGATION		(613)	2030	4150 (1264)	1650 (503)			20 30

~.

. .

FN-TR-26c

Shallow refraction results (Table 5) indicate a surficial layer having a velocity range between 1200 and 1850 fps (366 and 564 mps) with an average thickness of eight feet (2.5 m). Velocities greater than 7000 fps (2134 mps) were encountered at the location of four lines. Five major velocity zones were observed on the deep seismic lines (Table 6). Velocities representative of crystalline basement were not observed.

The compressional wave velocities from downhole surveys (Table 7) do not correspond with those from shallow seismic refraction (Table 5) due to the anisotropy of the soils and method of measurement. However, shear wave velocities of the site soils were obtained from the downhole surveys.

2.5 ENGINEERING PROPERTIES

Engineering properties of the subsoils representing various geologic units were determined from laboratory tests. The tests consisted of the following: classification, consolidation, shear strength, compaction, CBR, and chemical. The range of engineering properties and compressional wave velocityes are presented in Table 8.

Eolian sheet sand consists of medium dense to dense poorly graded sands and silty and clayey sands with little gravel. Intermediate alluvial fan deposits consist of dense to very dense sands and gravels. Younger alluvial fan deposits are comprised of medium dense to dense silty and clayey sands with some stiff silts and clays. Older lacustrine deposits consist of stiff to very stiff silts and clays which are

TUGRO NATIONAL INC

23

ł

ļ

ENGINEERING AND GEOPHYSICAL PROPERTIES	Eolian sheet sand (A3s)	Intermediate alluviai fan
UNIFIED SOIL CLASSIFICATION SYMBOL(S)	SP. SM. SC.	SP. SM, SC, GP. G
GENERAL PROPERTIES		
DRY DENSITY pcf(kg m ³)	100-120 (1602-1922)	95-133 (1522-21)
MOISTURE CONTENT (%)	7-17	7-19
DEGREE OF SATURATION (7)	30-78	47-83
SPECIFIC GRAVITY	2.64-2.66	2.68-2.77
DEGREE OF CEMENTATION	None to moderate	Moderate to st
COMPRESSIONAL WAVE VELOCITIES fps(mps)	1420-3220 (433-981)	1000-7200 (305.
ELECTRICAL CONDUCTIVITY (mhos m)	DNA	DNA
GRAIN SIZE DISTRIBUTION (2)		
BOULDERS >12 inches(30cm)	0-5	0-3
COBBLES 3 to 12 inches(8to 30cm)	0-5	0-10
GRAVEL	0-12	0-90
SAND	50-97	5-95
SILT AND CLAY	3-50	5-48
PLASTICITY DATA		
LIQUID LIMIT	20±	24-26
PLASTICITY INDEX	NP-7	NP-12
COMPRESSIBILITY DATA		
COMPRESSION AT 4 ksf(192kN/m ²) (5)	DNA	DNA
SWELL OR COLLAPSE UPON SATURATION (*)	DNA	DNA
SHEAR STRENGTH DATA		
UNCONFINED COMPRESSION ksf(kn m ²)	DNA	2.0±(96±
CD TRIAXIAL COMPRESSION	$c = 0 - 1.0 \text{ ksf} (48 \text{ kN m}^2), \varphi = 34^{\circ} - 39^{\circ}$	DNA
DIRECT SHEAR ksj(kn m ²)	0.7-5.5 (34-263)	2 5-7.7 (120-
COMPACTION AND CBR DATA		
MAXIMUM DRY DENSITY pcf(kg m ³)	125-138 (2002-2211)	DNA
OPTIMUM MOISTURE CONTENT (%)	6.0-8.5	DNA
CBR AT 90% RELATIVE COMPACTION	16±	DNA

1

··- _

DNA - DATA NOT AVAILABLE (INSUFFICIENT DATA OR TESTS NOT PERFORMED)

ι,

r

GEOLOGIC	UNITS					
diate alluvial fan deposits (A5i)	Younger alluvial fan deposits (A5y)	Older lacustrine deposits (A4o)				
SP. SM, SC, GP. GM. GC	SM. SC. HL. CL	CL. ML. SC				
95-133 (152 2 -2130)	80-116(1281-1858)	84-120(1346-1922)				
7-19	4-20	3-43				
4,-83	19-49	10-89				
2.68-2.77	2 67-2 72	2.59-2.73				
Moderate to strong	None to weak	Weak to strong				
1000-7200 (305-2195)	1500-3100 (457-945)	1700-6750 (518-2057)				
DNA	DNA	DNA				
0-3		0				
0-10	0-10	0				
0-90	0-20	0-15				
5-95	7-75	5-95				
5-48	10-85	5-95				
24-26	24-27	21-60				
NP-12	NP-13	NP-31				
DNA	DNA	1 5-4 5				
DNA	DNA	0 1-3 0(Swell)				
2.0±(96±)	0.75±(36±)	0.5-8.2 (24-393)				
DNA	DNA	$c = 0 - 1.5 \text{ ksf} (72 \text{ kN m}^2), \varphi = 18^{\circ} - 34$				
2 5-7.7 (120-369)	DNA	1.7-6.7(81-321)				
DNA	136 ± (2179 ±)	121-126 (19 36- 20!8)				
DN A	6.0±	9.0-9.5				
DNA	18±	7±				

~

•

RANGE OF ENGINEERING AND Geophysical properties Jornada del Nuerto. New Mexico, Rio	GRANDE CSP
WX SITING INVESTIGATION DEPARTWENT OF THE AIR FURCE SAMSO	TABLE B
TUGRO NATIONAL	INC.

. -

· . -

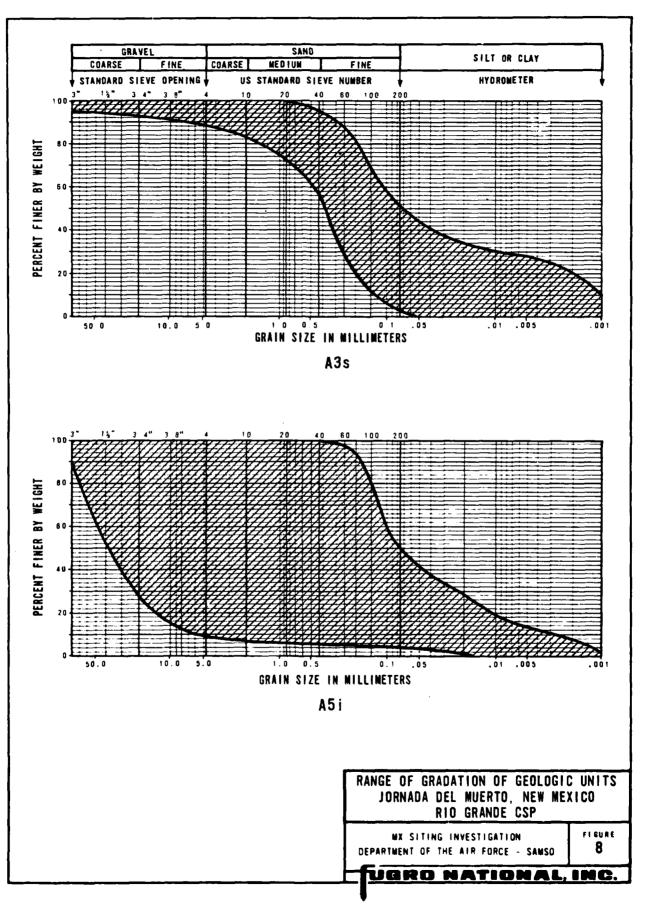
.

- ..

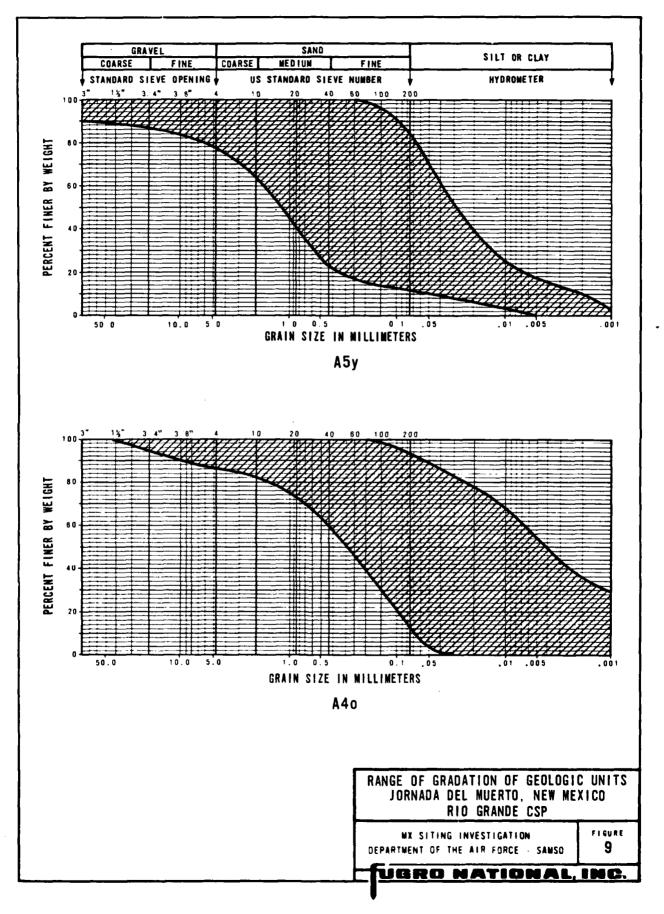
comprised of medium dense to dense silty and clayey sands with some stiff silts and clays. Older lacustrine deposits consist of stiff to very stiff silts and clays which are moderately compressible. Eolian sheet sand and intermediate alluvial fan deposits possess moderately high shear strengths; younger alluvial fan and older lacustrine deposits possess moderate shear strengths. The site soils are generally neither expansive nor collapsible. Range of the gradation of the four geologic units is shown in Figures 8 and 9.

Results of chemical tests on soil samples are shown in Table 9. The test results indicate that sulfate attack of soils on concrete will be "severe."

Representative logs of three borings and three trenches from the site are contained in Appendix B. Results of the shear strength and CBR tests performed on soil samples from the site and a summary of all the laboratory tests performed on soil samples obtained from boring JM-B-11 are also included in Appendix B.



n marten - mensen i 199 a. i 19 billen 1960 (n. 1966) (n. 1956) i



The second s

FN-TR-28c

Ŕ

E

1. N. 1

CALCIUM		e mg/kg	┠╌	7875	8950	7750	192	7300		8050	8250	 								-		
щ	CALCIUM	mg/kg	2900	3150	3580	3100	7	2920		3220	3300											
WATER SOLUBLE	SULPHATE	mg/kg	9600	7940	10500	11000	377	9600		8340	9260											
M	CHLORIDE	mg/kg	109	33	195	242	29	10		38	43											
	SODIUM	mg/kg	345	75	975	500	185	61		83	116											
	Æ		1.4	7.6	7.4	7.4	8.0	7.5	-	7.3	7.4											
	TYPE		כר	ЖL	ML	G	SM	SM		ML	%											
CAMPLE INTEDVAL		METERS	4.82-5.27	3.29-3.60	3.05-3.26	3.05-3.60	1.52-2.29	3.05-3.38		1.07-1.83	0.91-1.52											
CAMPLE		FEET	15.8-17.3	10.8-11.8	10.0-10.7	10.0-11.8	5.0-7.5	10.0-11.1		3.5-6.0	3.0-5.0											
	SAMPLE NO.		P-3	P-2	P-2	P-2	2	P-3		B-2												
BORING /		NO.	JM-8-4	JM-8-9	01-8-ML	JM-B-13	JM-B-14	JM-8-17		JM-7-4	JM-T-12											
													<u></u>	J OR	NAD	A DI R	EL 0 (MUEI Grai	RTO NDE	, N <u>CS</u> on	EW	

_

and a loop be the state of the

3.0 SAN SIMON SITE

The San Simon characteriztion site covers an area of 715 nm² (2452 km²) in Graham and Cochise counties, Arizona. The site is bounded by mountain ranges on the east and west. The valley extends beyond the bounds of the site to the north and south. Major streams within the site drain toward the Gila River to the north. The site is accessible via Interstate 10 along the southern edge and via state route 666 along the western edge. A network of graded roads and four-wheel drive trails provides access within the site.

3.1 SCOPE OF INVESTIGATION

Scope of geologic, geophysical, and soils engineering field activities performed at the site and laboratory tests performed on soil samples from the site are presented in Table 10. Detailed information about the soils engineering field activities (13 borings and seven trenches) is summarized in Tables 11 and 12. Locations of all the field activities are shown in Figure 10.

3.2 SURFICIAL GEOLOGY AND TERRAIN

Alluvial fan deposits of younger and intermediate age and older lacustrine deposits are the predominant surficial geologic units within the San Simon site (Figure 10). Younger alluvial fans cover approximately 35 percent of the area, intermediate alluvial fans cover 25 percent, and older lacustrine deposits cover 30 percent. The alluvial fan deposits are typically silty sands with gravel, ranging from sandy gravels near the mountain front to sandy silts near the valley interiors. The lacustrine

TUBRO NATIONAL INC

 \mathcal{D}

	GEOL	OGY	AND	GEOPHYSICS
--	------	-----	-----	------------

-

TYPE OF ACTIVITY	NUMBER OF ACTIVITIES
Geological mapping stations	43
Shallow refraction	12
Conductivity	12

NG
NOMINAL DEPTH Feet (Meters)
25 (8)
50 (15)
100 (30)
300 (91)
NOMINAL DEPTH FEET (METERS)
10 (3)
12 (4)

ENGINEERING-LABORATORY TESTS

TYPE OF TEST	NUMBER OF Tests
Moisture/density	135/99
Specific gravity	4
Sieve analysis	66
Hydrometer	50
Atterberg limits	37
Consolidation	4

-

÷.,

۰.

. .

TYPE OF TEST	NUMBER OF TESTS
Unconfined compression	9
Triaxial compression	4
Direct shear	24
Compaction	6
CBR	2
Chemical analysis	6

SCOPE OF FIELD AND LABORATORY Activities	1
SAN SIMON VALLEY, ARIZONA, HIGHLAN	IDS CSP
	TABLE
WX SITING INVESTIGATION	10
DEPARTMENT OF THE AIR FORCE SAMSO	
TUGRO NATIONAL,	INC.

ENGINEERING

BORING NUMBER	TOTAL DEPTH Feet (meters)	TYPE OF DRILL RIG USED	TYPE OF SAMPLES* Obtained
SS-B-1	302.5 (92.2)	Rotary wash	0, P
SS-B-2	72.5 (22.1)	Rotary wash	D, P
SS-B-3	50.9 (15.5)	Hollow Stem Auger	SS, D
SS-B-4	51.0 (15.5)	Hollow Stem Auger	D
SS8 5	55.5 (16.9)	Hollow Stem Auger	D
SS-B-6	51.0 (15.5)	Hollow Stem Auger	D
SS-8-7	45.0 (13.7)	Hollow Stem Auger	D
8-8-22	54.0 (18.5)	Rotary Wash	D
SS-8-9	48.0 (14.6)	Hollow Stem Auger	D
SS-8-10	20.0 (6.1)	Hollow Stem Auger	8, D
SS-B-11	101.5 (30.9)	Rotary Wash	D, P
SS-8-12	61.0 (18.6)	Rotary Wash	D
SS-8-13	50.0 (15.2)	Hollow Stem Auger	D
	<u></u>		<u> </u>

* P = Pitcher sample (undisturbed)

D = Fugro Drive sample (relatively undisturbed)

B = Bulk sample (disturbed, but representative)

SS = Split Spoon sample (disturbed, but representative)

ENGINEERING FIELD ACTIVITIES - San Simon Valley, Arizon Highlands CSP	
MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE SAMSC	TABLE
TUGRO NATIONAL,	INC.

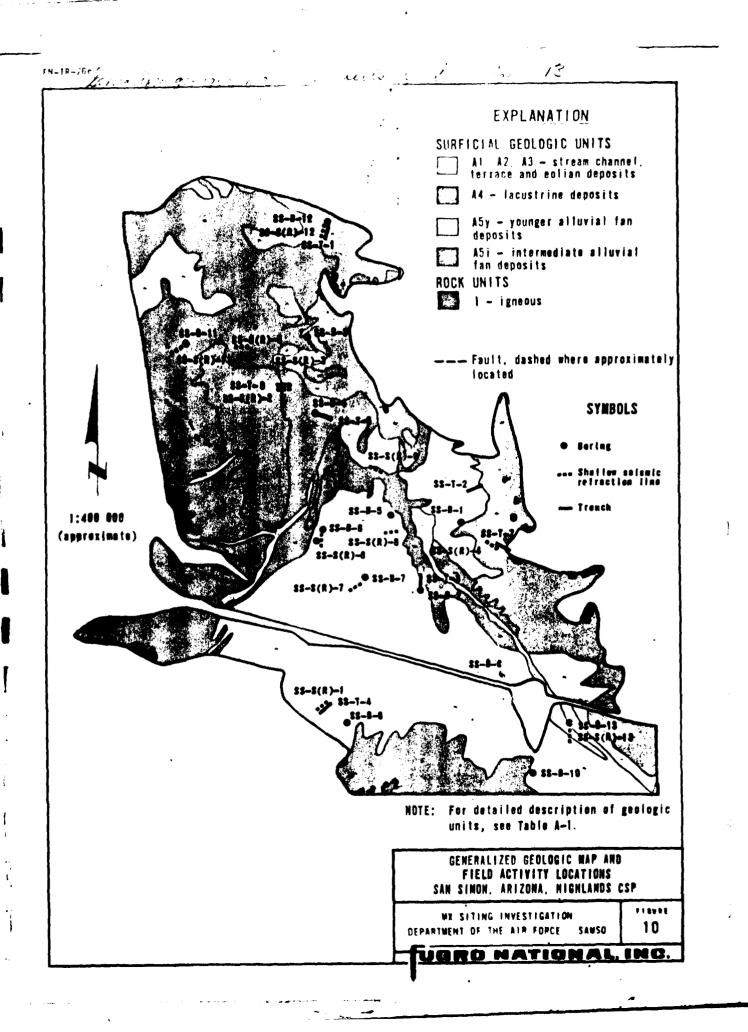
TRENCH NUMBER	TOTAL DEPTH Feet(Neters)	STABILITY OF VERTICAL EXCAVATION WALLS
SS-T-1	11.5 (3.5)	stable
SS-T-2	12.5 (3.8)	stable
E-T-22	12.0 (3.7)	stable
SS-T-4	10.2 (3.1)	stable , some sloughing at 3.5-9.5'(1.1-2.9m)
SS-T-5	12.5 (3.8)	stable
SS-T-6	12.8 (3.9)	stable
SS-T-7	10.2 (3.1)	stable

.........

ENGINEERII	NG FIELD ACTIVITIES -	- TRENC
	N SIMON VALLEY, ARIZ	
901	•	UNA
	HIGHLANDS CSP	
		TABL
12 YM	TING INVESTIGATION	1

...

<u>n</u>' _



deposits are very coarse along the periphery of the valley but are fine grained along the valley axis. These units along with the remaining surficial units are described in Table 13.

Surface slopes and depths of drainage incision vary with geologic units, both generally increasing with proximity to the mountain fronts (Table 13). Maximum observed surface slope was three percent with typical slopes of less than one percent. Drainages are typically shallow (less than 10 ft; 3 m) with gently sloping sides except near mountainous areas and in the older lacustrine material.

3.3 SUBSURFACE CONDITIONS

3.3.1 Soil Profiles

Silty and clayey sands and gravels are the predominant surficial soils which are typically underlain by several hundred feet of clay and silty clay deposits throughout much of the valley. The general subsurface conditions are illustrated by two representative soil profiles shown in Figures 11 and 12. The percentage of fines generally increases towards the valley basin. Cobbles and boulders are encountered only in the close proximity of mountain fronts.

3.3.2 Depth to Shallow (<150 ft; <46 m) Rock and Water Figure 13 shows portions of the site in which rock and water are estimated to be encountered within a depth of 150 feet (46m) below ground surface. Shallow rock exists in approximately five percent of the site based on boring, seismic, gravity, geologic, topographic, and other available data.

34

TURNO NATIONAL INC

Quaternary	Unknown	Sand and Silt	SM, ML	50 (172)	1
Quaternary- Tertiary	Unknown	Clay, Silt, Sand. Gravel with cobbles and boulders	CL, ML. SP-SW, GP-GW	215 (737)	30
Quaternary	Unknown	Silty sand, Sand with Gravel	SM, SP	243 (833)	35
Quaternary	Unknown	Silty Sand with Gravel	SM	179 (614)	25
				+	
	}				

AL EXTE	ENT (SITE)	_	PROPERTIE	S OF SURFACE	MATERIALS		SURFACE	ORPHOLOGY	
(km ²)	PERCENT	GRADATION	CEMENTATION	MAXIMUM GRAIN SIZE	PAVEMENT/ PATINA	STAGE OF Caliche (c)	SLOPE (PERCENT)	DRAINAGE DEPTHS FEET(METERS)	NOTES
(48)	2	Poor- Moderately well	Weak- Moderate	Gravel	None/None	I-11	<]	12-15 (4-5)	(d)
(24)	1	Poor	Strong	Boulder	None/None	п-ш	< 1	0-5 (0-1.5)	
(172)	1	Poor	None-Weak	Sand	None None	None	≤	0-5 (0-1.5)	
(737)	30	Poor	Weak-Strong	Cobble	None/None	I-III	1-2	10-15 (3-5)	(e)
(833)	35	Poor- Moderately well	None-Weak	Gravel	None/None	None-I	≤ 1	0-2 (0-0 6)	(†)
(614)	25	Well	Weak- Moderate	Boulder	1009/1009	None-III	1-3	06 (0-2)	(†)

•

•

DESCRIPTION OF SURFICIAL GEOLOGIC UNITS SAN SIMON VALLEY. ARIZONA. HIGHLANDS CSP TARLE MX SITING INVESTIGATION 13 DEPARTMENT OF THE AIR FORCE SAMSO UGRO NATIONAL, INC.

· · · · · ·

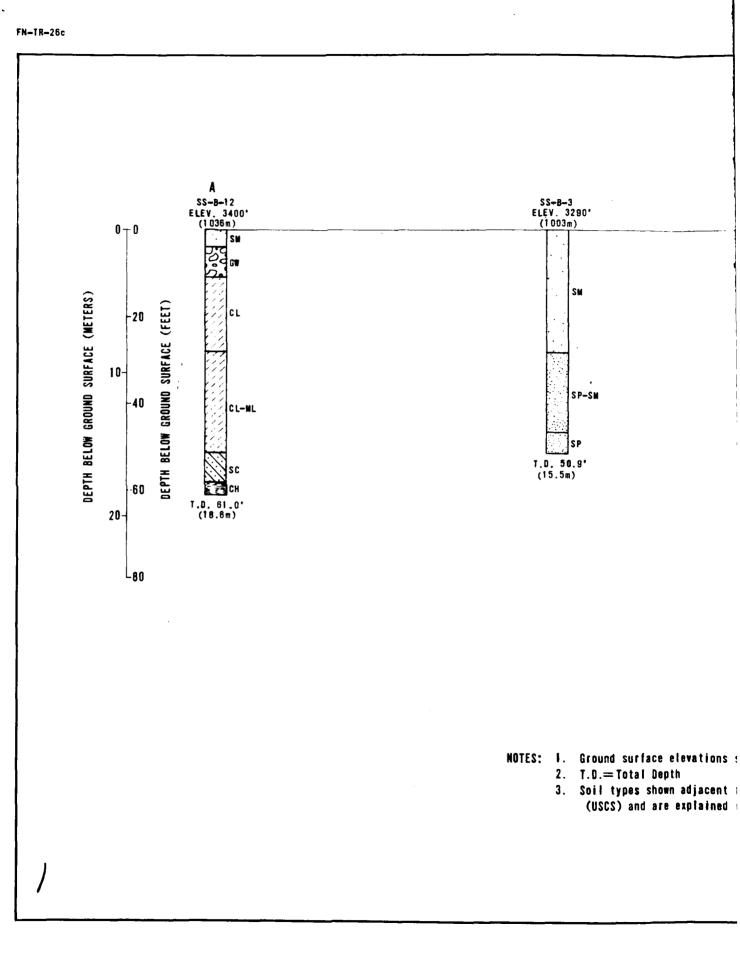
•••··· • •

Э

•

•

ř



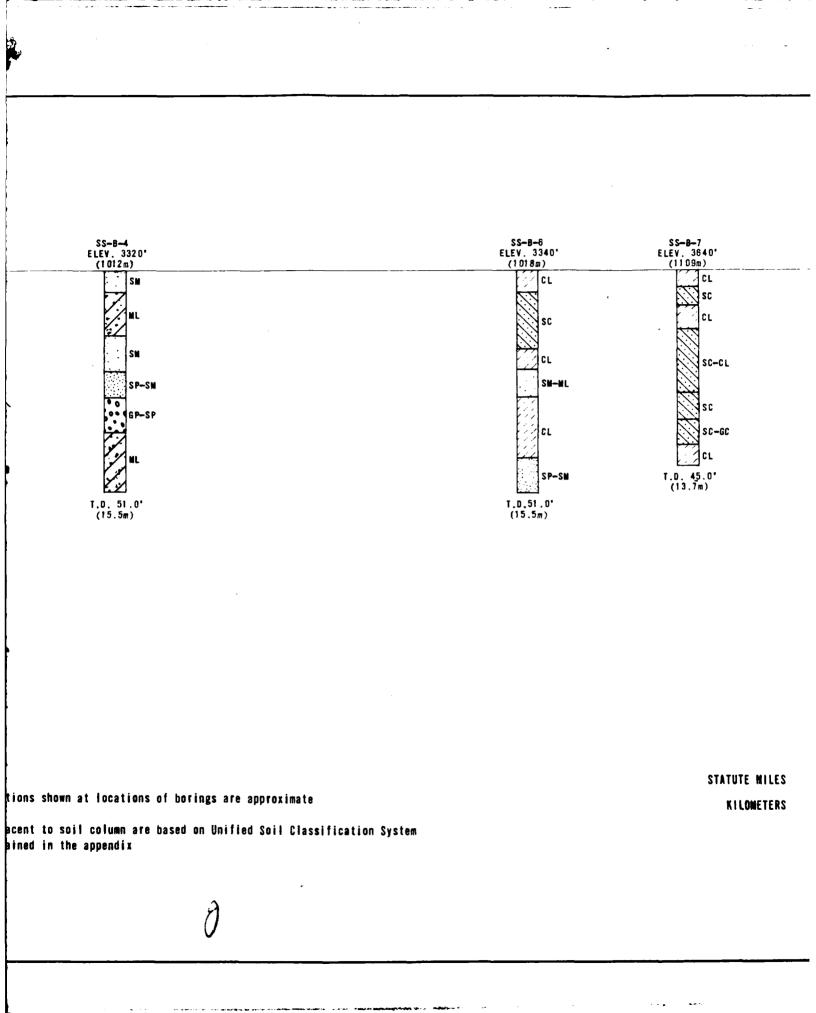
and a subality of the second second

· · · · · · ·

-

.

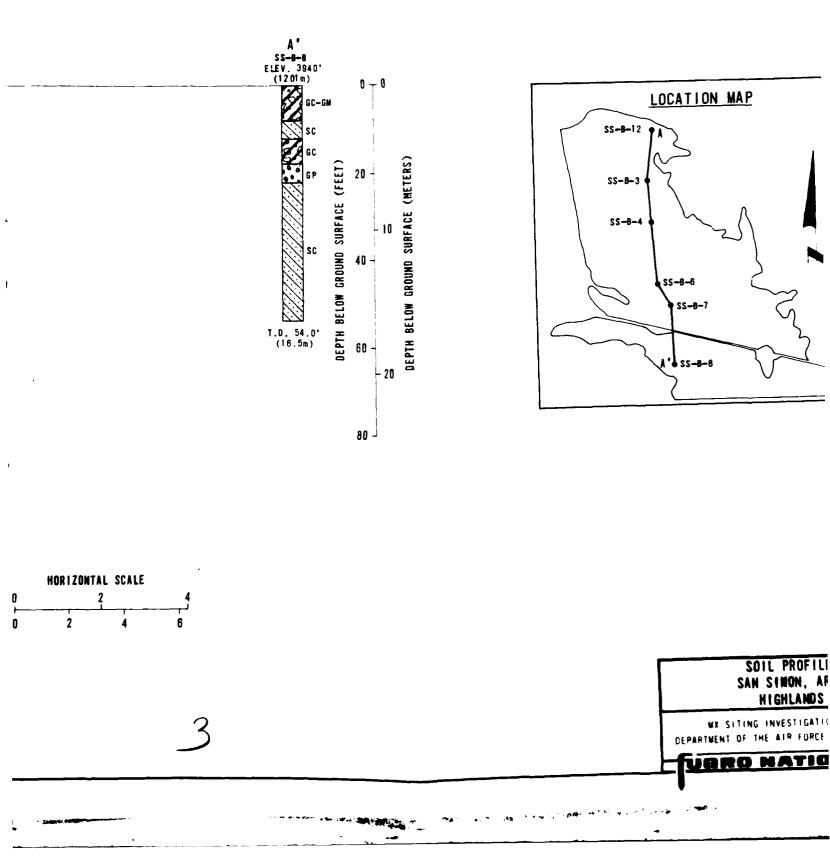
....



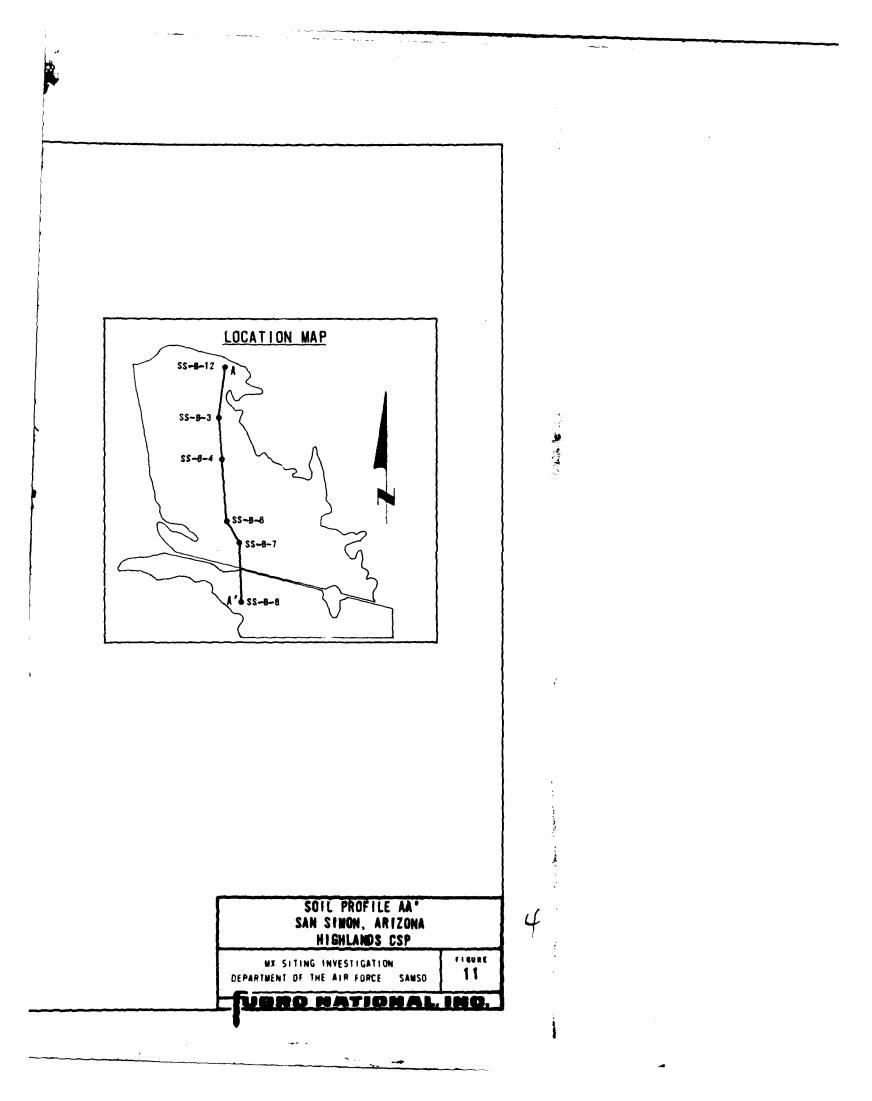
~

-

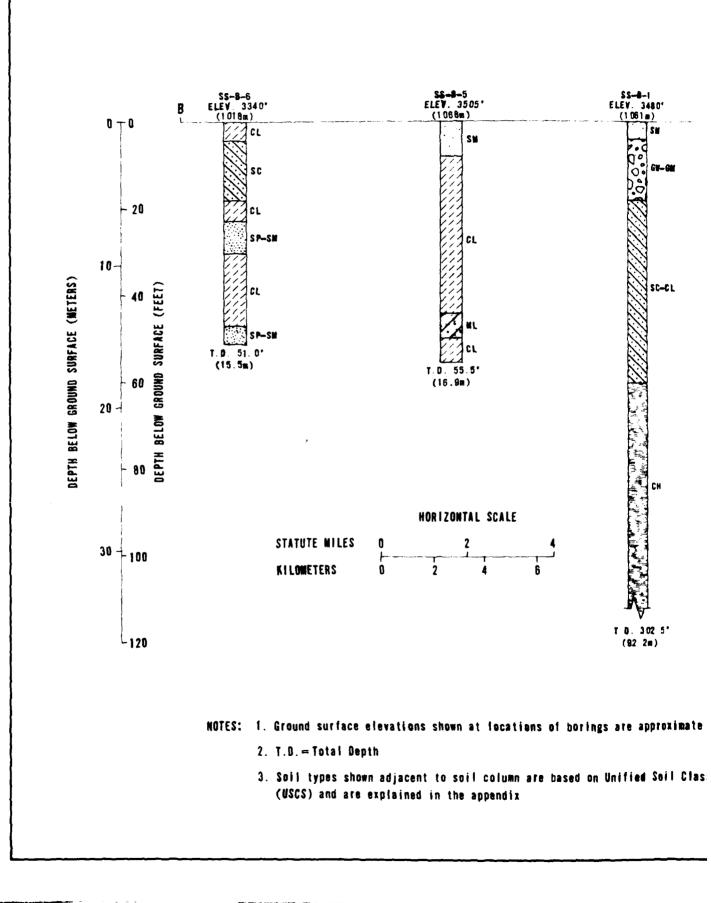
ange de la mandelle de la companya d

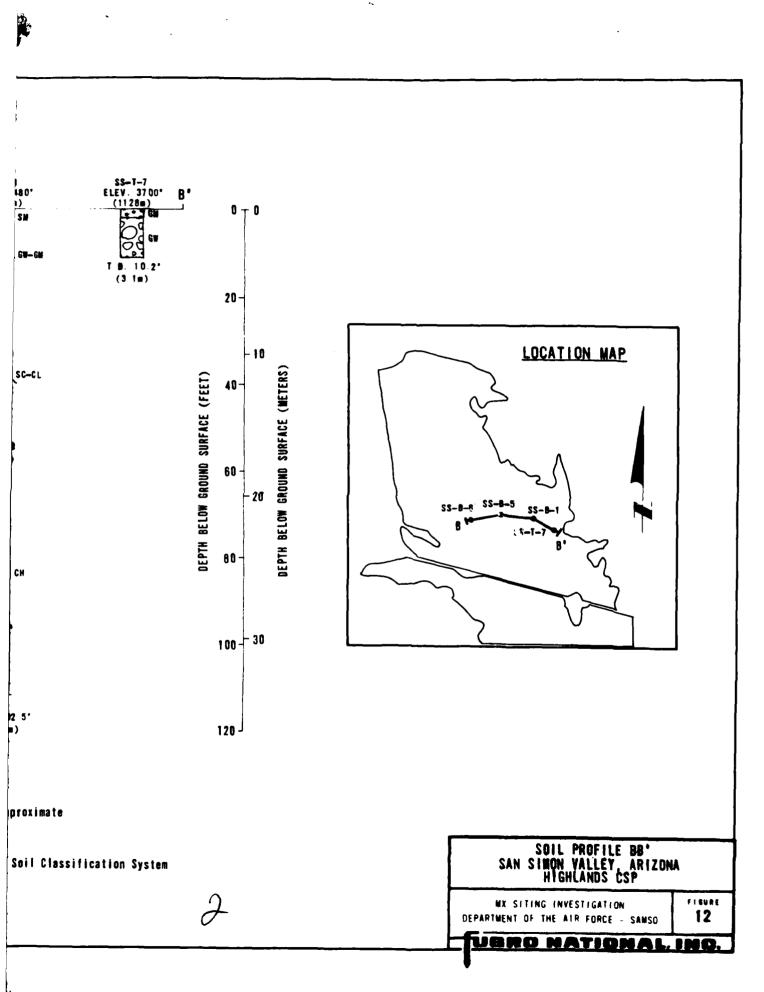


- - -- --



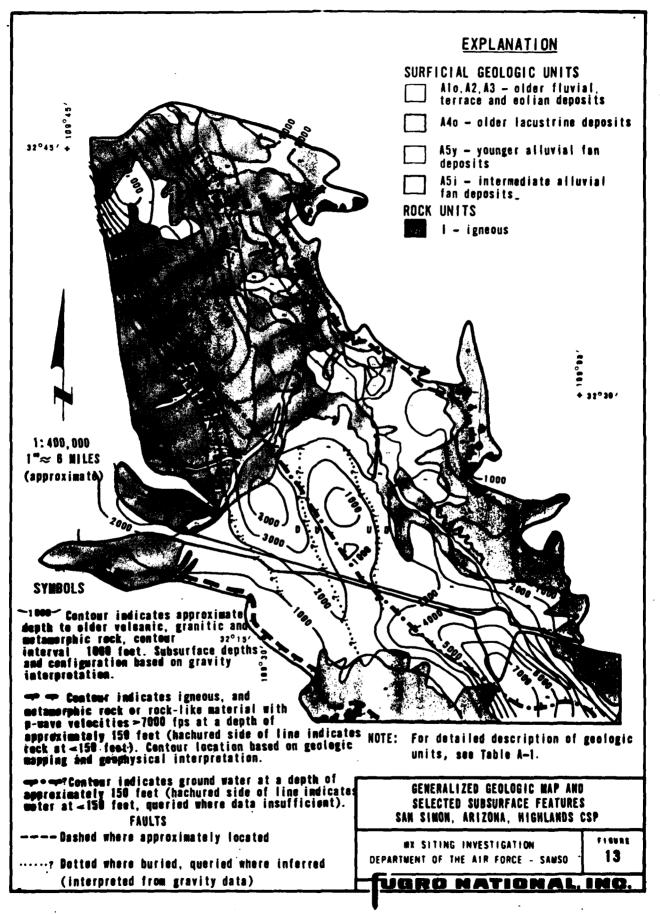






.....





······ .

Ground water occurs at depths of less than 150 feet (46 m) over approximately 45 percent of the San Simon site. However, extrapolation of known data indicates shallow ground water may be encountered over an additional 20 percent of the site.

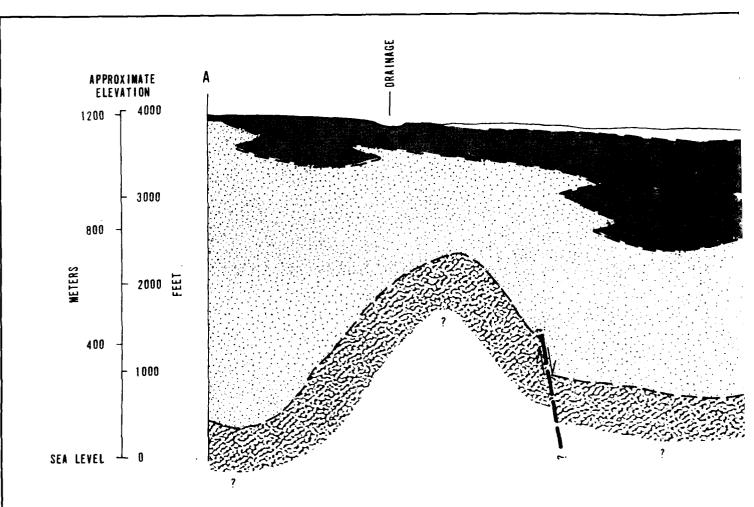
3.3.3 Basin Configuration

Results of the gravity survey were used to define the basin configuration (Figure 13). The basin appears to be a deep trough which has been uplifted and faulted in the central part of the valley (Figure 14). The trough is roproximately

feet deep (3050m) in the northern portion and 7000 feet deep (2130 m) in the southern portion. Steep gradients on both sides of the valley indicate that the basin is fault controlled. A section through the central portion of the valley perpendicular to the valley axis shows a horst-graben structure with a maximum depth of about 4500 feet (1377 m) in the graben. This structure is located in the uplifted area in the south-central portion of the trough. The subsurface basin configuration is illustrated in Figure 14.

3.4 GEOPHYSICAL PROPERTIES

Results of the shallow seismic refraction and electrical conductivity surveys are shown in Tables 14 and 15. Observed seismic velocities ranged from 1000 to 6330 fps (305 mps to 1930 mps). Surface layer velocities ranged from 1000 to 1440 fps (305 mps to 439 mps). This layer is 25 feet (8 m) thick, although it is typically about 5 feet (1.5 m) thick at other locations. Low velocity (<2000 fps; 610 mps) layers at SS-S-6 and 9 extend to



NOTES:

1 The cross section is generally representative of subsurface conditions within the band shown on the location map. Due to the limited density of available data and the sparseness of new'y acquired data, the subsurface conditions are highly interpretive. 2. For a detailed description of geologic units see Table A-1

EXPLANATION

Undifferentiated basin-fill deposits Predominantly alluvial (A5) deposits, with eolian () and stream terrace (A2) deposits



Older lacustrine (A4o) deposits



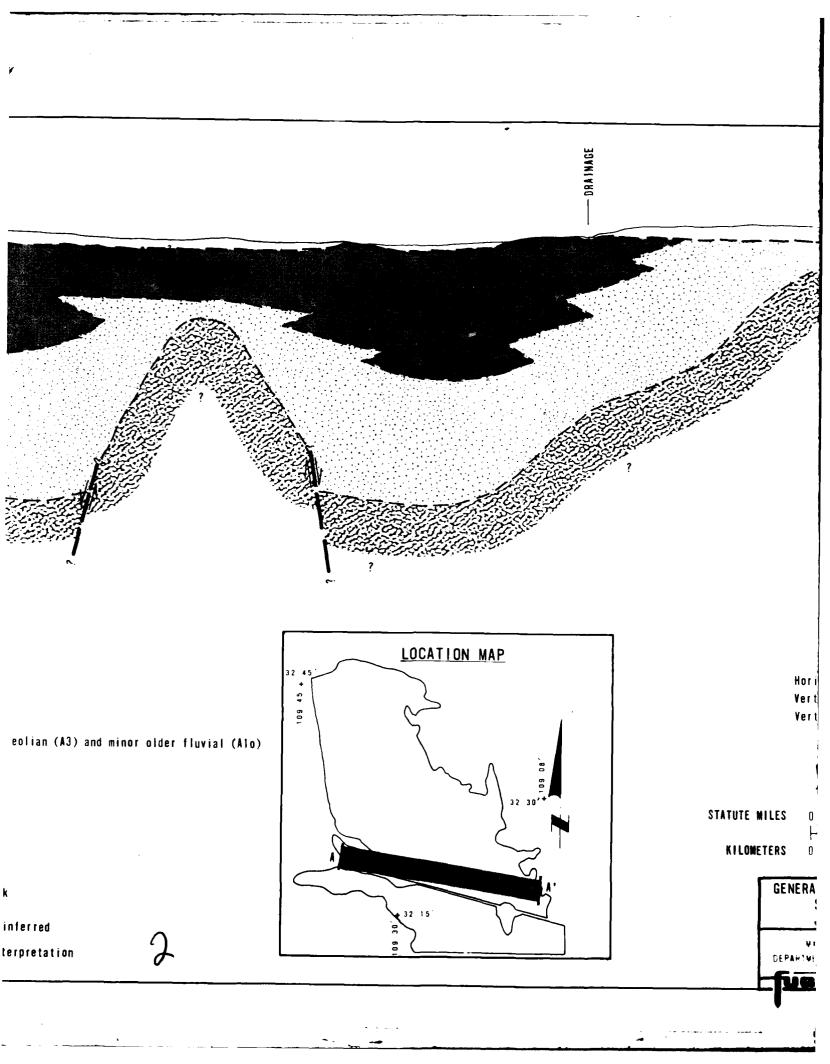
Undifferentiated older basin-fill deposits

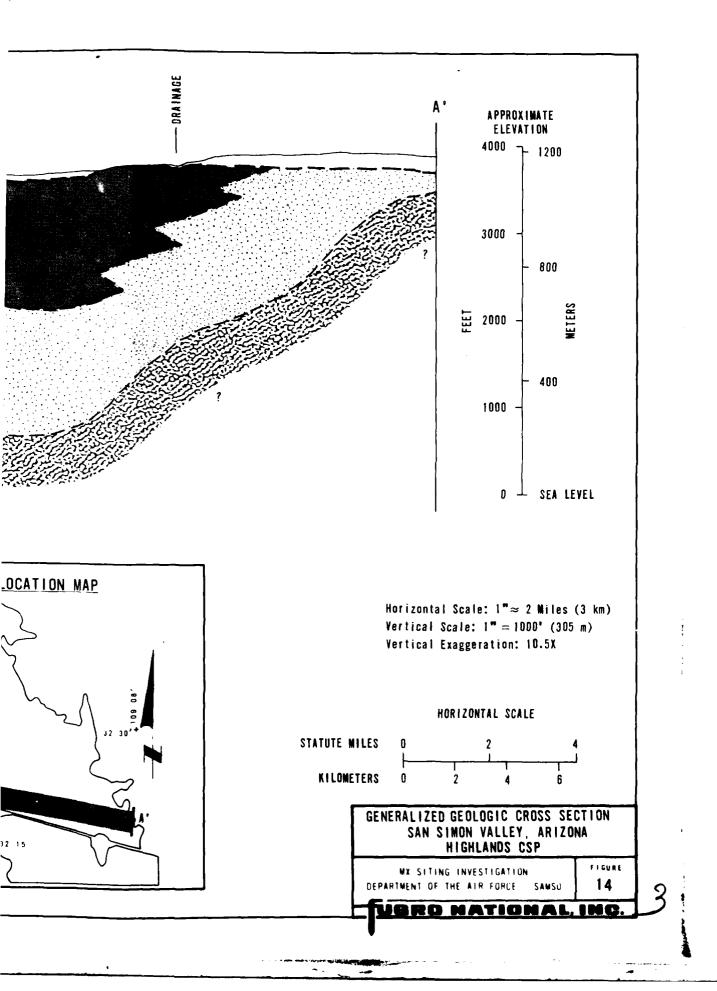


Undifferentiated igneous and metamorphic rock

Approximate geologic contact, queried where inferred

Fault, dashed where inferred from gravity interpretati

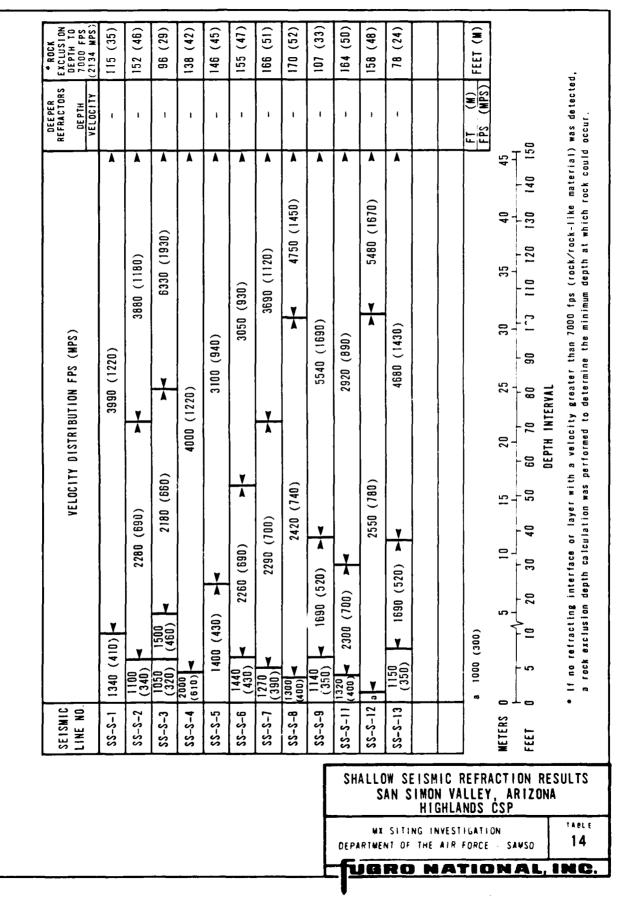




J

÷

i



nersynaat dal waarde die Staat als die Staat die St

.

\$

ACTIVITY LOCATION*	AVERAGE CONDUCTIVITY (mhos m)**
R-1	0.021
R-2	0.014
R-3	0.016
R-4	0.014
R-5	0.038
R-6	0.032
R-7	0.047
R~8	0.019
8~9	0.118
R-11	0.011
8-12	0.129
R-13	0.074
	+

~ .

*Resistivity was determined using a Schlumberger Array at each location where a seismic refraction survey was conducted.

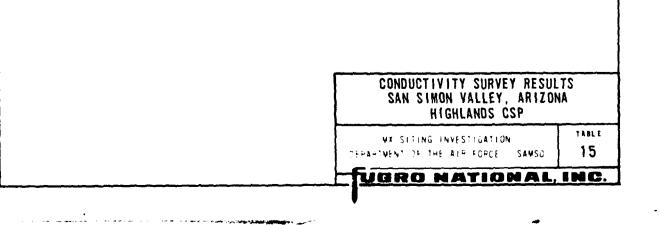
**Conductivity is the inverse of resistivity. Numbers presented are the average of values determined to a depth of 50 feet, computed as follows:

Average
Conductivity =
$$(C_1 t_1 + C_2 t_2 + \dots + C_n t_n)$$
 50 feet

Where

Average Conductivity = mhos/m C₁ through C_n = Conductivity (mhos/m) of layers 1 through n

t, through t_n=Thickness (feet) of layers 1 through n to 50 feet



depths of 40 feet (12 m). Average conductivities for the upper 50 feet (15 m) of soil are between 0.011 and 0.129 mhos/m.

3.5 ENGINEERING PROPERTIES

Laboratory tests were performed to determine the engineering properties of soil samples obtained from the various geologic units. The testing program consisted of classification, consolidation, shear strength, compaction, CBR and chemical. The range of engineering and geophysical properties of predominant geologic units is presented in Table 16.

Younger and intermediate alluvial fan deposits could not be differentiated at depth, and they were combined into one unit (A5) due to their similar grain size and engineering properties. These deposits consist predominantly of dense to very dense, silty and clayey sands, and gravels which are only slightly compressible and have high shear strengths.

Older lacustrine deposits were generally encountered at depths below alluvial fan deposits. These deposits are predominantly stiff to very stiff clays and silty clays near the valley interiors and coarse sandy gravels along the periphery. Clays and silty clays are expansive when saturated, only slighty compressible and have high shear strengths. Gravels are generally coarse to very coarse and mixed with cobbles and boulders which are relatively incompressible and have very high shear strengths.

The gradation ranges of the geologic units are shown in Figure 15. Table 17 shows the results of chemical tests on

IO NATIONAL ING

.

. .

•

.....

ENGINEERING AND GEOPHYSICAL PROPERTIES	Youngor and intermediate alluvial ran deposits (ASi and ASv)	Older lacustrine depos
UNIFIED SOIL CLASSIFICATION SYMBOL(S)	SM, SC. GM, GC. GP. CL	CL. CH. ML. SW
GENERAL PROPERTIES		
DRY DENSITY pcf(kg m ³)	90-122 (1442-1954)	85-117 (1362-18
MOISTURE CONTENT (%)	2-28	2-30
DEGREE OF SATURATION (")	25-85	14-90
SPECIFIC GRAVITY	2.63-2.70	2 63-2 68
DEGREE OF CEMENTATION	None to moderate	Weak to strong
COMPRESSIONAL WAVE VELOCITIES fps(mps)	1000-4680 (305-1426)	1050-2920 (320-8
ELECTRICAL CONDUCTIVITY (mhos m)	DNA	.005191
GRAIN SIZE DISTRIBUTION (%)		
BOULDERS >12 inches(30cm)	D-10	0
COBBLES 3 to 12 inches(8to 30cm)	0-25	0-5
GRAVEL	0-55	0-20
SAND	0-90	0-88
SILT AND CLAY	10-97	0-98
PLASTICITY DATA		
LIQUID LIMIT	19-72	26-84
PLASTICITY INDEX	NP-44	NP-60
COMPRESSIBILITY DATA	· · · · · · · · · · · · · · · · · · ·	
COMPRESSION AT 4 ksf(192kN/m ²) (2)	0.8-2.3	0.5-1.6
SWELL OR COLLAPSE UPON SATURATION (%)	0.2-1.4(Sweil)	0 3-1 2 (5
SHEAR STRENGTH DATA		
UNCONFINED COMPRESSION ksf(kn/m²)	2 0-6 9 (96-330)	3.6-34.4 (172-16
CD TRIAXIAL COMPRESSION	DNA	$c = 1-3 \text{ ksf}(48-144 \text{ kN m}^2)$
DIRECT SHEAR ksf(kn/m ²)	0.4-5.7 (19-273)	0.4-3.7 (19-17
COMPACTION AND CBR DATA		
MAXIMUM DRY DENSITY pcf(kg m ³)	118-135 (1890 - 2162)	115-126 (1842-20
OPTIMUM MOISTURE CONTENT (~)	9.8-12.5	10.5-15.8
CBR AT 90% RELATIVE COMPACTION	20-30	6-9

- - -----

DNA=DATA NOT AVAILABLE (INSUFFICIENT DAJA OR TESTS NOT PERFORMED)

_ ____

GEOLOGI	CUNITS
acustrine deposits (A4o)	
CL. CH. ML, SW, SP	
85-117 (1362-1874)	
2-30	
14-90	
2.63-2.68	
Weak to strong	
050-2920 (320-890)	
.005191	
0	
0-5	
0-20	
0-88	
0-98	
26-84	
NP-60	
0 5-1 6	
0 3-1 2 (Swell)	
5-34.4 (172-1647)	
8-144 kN m ²), $\phi = 12 - 25^{\circ}$	
0.4-3.7 (19-177)	
-126 (1842-2018)	
10.5-15.8	
6-9	

.

Ì

~

• • • •

.

RANGE OF ENGINEERING AND GEOPHYSICAL PROPERTIES SAN SIMON VALLEY, ARIZONA, HIGHLANDS CSP WX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE SANSO 16 FUGRO MATIONAL, INC.

 \mathcal{A}

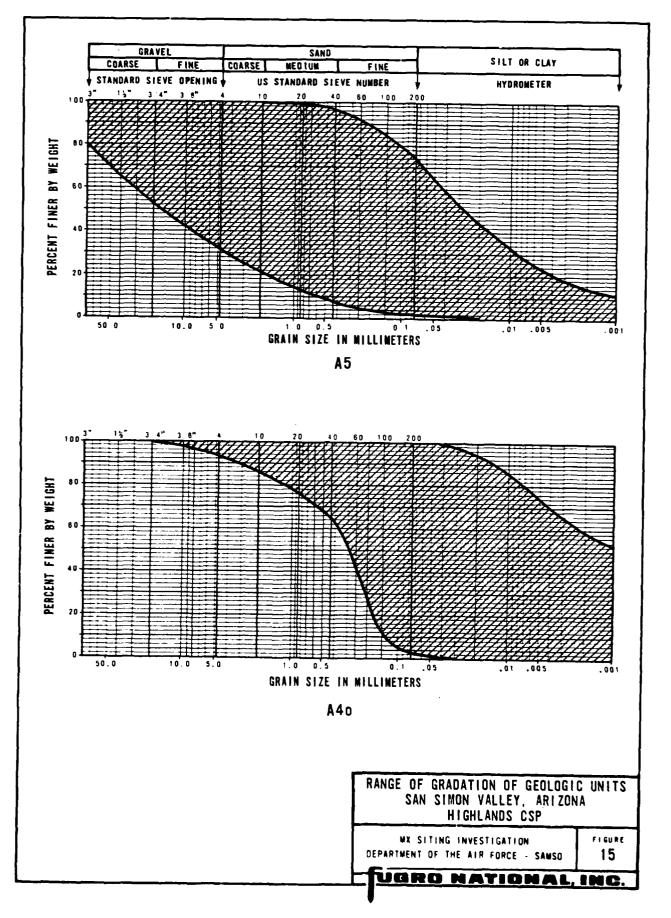
a and a second second

-

FN-TR-28c

··· ••

....



Ŷ

	BORING /		CAMPIE	CAMDIE INTEDVAI				*	WATER SOLUBLE	.Ε	CALCIUM
	TRENCH	SAMPLE	JAM TLC	INI CATAL	1 SULL	Ħ	SODIUM	CHLORIDE	SULPHATE	CALCIUM	CARBONATE
	NO.		FEET	METERS			mg/kg	mg/kg	mg/kg	mg/kg	mg/kg
	SS-B-1	P.4	20.0-20.7	6.10-6.31	CH-MH	9.7	1440	29	820	1740	1510
	SS-B-4	5	20.0-20.5	6.10-6.25	N.	8.4	069	670	1020	320	473
	8-8-8	0-5	20.0-20.5	6.10-8.25	ΞS	1.1	1420	2670	920	530	1950
	1-1-55	2	10.0-11.5	3.05-3.50	CL-HL	7.8	068	230	560	60	180
	1-1-SS	2	2.5-4.5	0.76-1.37	SM	6.3	470	81	370	119	478
	9-1-SS	8-2	10.0-12.5	3.05-3.81	SP	7.2	540	200	640	75	227
F											
IA D											
/ 07											
T											
T											
RES											
ULTS										-	

CONTRACTOR OF TAXA

FN-TR-28c

R

soil samples and they indicate that sulfate attack potential of soil on concrete is generally "negligible."

Logs of three representative borings and trenches are shown in Appendix C. Results of shear strength and CBR tests performed on soil samples from the site and a summary of all the laboratory tests performed on soil samples from boring SS-B-l are also included in Appendix C.

4.

4.0 DISCUSSION

4.1 JORNADA DEL MUERTO SITE

Based on regional geologic information, geotechnical conditions of the Jornada Del Muerto site are generally representative of approximately 95 percent of the Rio Grande CSP and are summarized below:

- o The eolian sheet sand, younger alluvial fan deposits, and intermediate alluvial fan deposits are the predominant surficial geologic units. (Eolian sheet sand is the most dominant).
- o Older lacustrine material is predominant within the construction zone (150 ft; 46 m).
- o The site area is generally a structural basin bounded by a potentially active fault on the east.
- The terrain slopes gently towards closed central basin areas.

4.2 SAN SIMON SITE

Based on regional geologic information, geotechnical conditions of the San Simon site are generally representative of approximately 40 percent of the Highlands CSP and are summarized below.

- o The younger alluvial fan deposits, intermediate alluvial fan deposits and older lacustrine deposits are the predominant surficial geologic units (the younger alluvial fan deposits are the most dominant).
- The site area is generally a down-dropped structural block (graben) bounded by potentially active faults.
- o The terrain slopes gently towards open central valleys.

TURRO NATIONAL INC

5.0 CONSTRUCTION CONSIDERATIONS

In this section geotechnical factors and conditions applicable to construction of the MX system are discussed. The three basing mode concepts presently considered are vertical shelter, in-line hybrid trench, and horizontal shelter. The important geotechnical factors for a vertical shelter are primary, secondary and interconnecting roads, excavation of shelters, and drainage crossings. For the in-line hybrid trench, important geotechnical factors are excavation and backfill, roads (primary, secondary, and temporary), drainage crossings, and aggregates for roads and concrete. For the horizontal shelter, roads and drainage crossings are the important geotechnical factors. A summary of the applicable geotechnical factors is presented in the following paragraphs:

- Terrain Surficial slopes are typically less than
 one percent, requiring little preconstruction grading for
 roads and trenches. Depths of drainage incision are
 generally less than ten feet (3 m) minimizing the need
 for major drainage structures for roads and trenches.
 However, in the northeastern portion of San Simon Site
 depths of drainage incision are ten feet (3 m) or greater
 requiring major drainage structures.
- o Roads A good network of paved and unpaved roads exists at the Jornada Site. However, no regular network of paved or unpaved roads exists at the San Simon site. Therefore, a network of new roads will be required.

49

TUBRO NATIONAL, INC.

Most of the surficial soils at both the sites have good to excellent subgrade characteristics when compacted, resulting in economical road sections. In approximately 25 percent of the Jornada site the surficial soils do not have good support characteristics for use as road subgrade, thus requiring thicker, more costly road sections.

o Excavation - Most of the subsurface soils are dense, weakly to moderately cemented, and possess moderately high shear strength. Except in areas close to mountain fronts, compressional wave velocities range from 1000 to 6000 fps (305 to 1830 mps) up to depths of 150 feet (46 m) below ground surface, indicating good excavatability. The soils are suitable for excavation of: vertical shelters by augers, continuous trenches (castin-place trench construction) by an MX trencher, and horizontal shelters using conventional equipment. In approximately 20 percent of the Jornada site, vertical walls of excavations for trenches and vertical shelters may be unstable, requiring additional expense for other excavation techniques. Approximately five percent of the area of the two sites has zones of concentrated cobbles and boulders where an MX trencher will not be able to excavate a trench suitable for cast-in-place construction.

Depth to rock is greater than 150 feet (46 m) over a major portion of the sites, therefore, additional

RO NATIONAL INC

50

expense for excavation of vertical shelters is minimal. Depth to ground water is less than 150 feet (46 m) in approximately 30 and 65 percent of Jornada and San Simon sites, respectively. In areas where the depth to ground water is less than 120 feet (37 m), additional costs for excavation of vertical shelters can be expected.

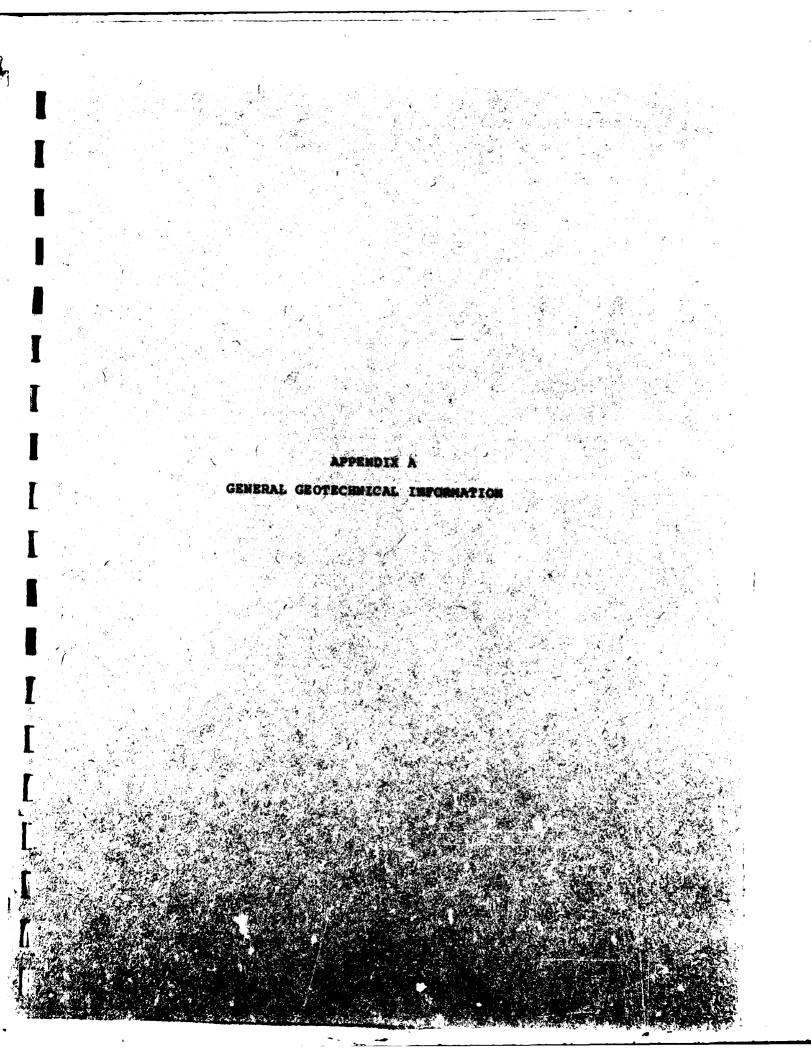
- Backfill Subsurface soils are generally suitable for backfill and compaction in trench excavations. Backfill will have to be imported from within the sites for areas of concentrated cobbles and boulders.
- Aggregates and Water Sufficient quantities of aggregates and water required for roads and concrete of all basing modes are available within and/or adjacent to the sites, thus minimizing haul costs.

51

6.0 CONCLUSIONS

In summary, Jornada del Muerto and San Simon sites present favorable geotechnical conditions for deployment of any of the three present MX basing mode concepts. For the vertical shelter basing mode, cost of excavation for vertical shelters will be high in many areas due to the presence of ground water within the construction zone. As an alternative, areas with ground water in the construction zone can be excluded for the vertical shelter mode. Geotechnical conditions from the Jornada and San Simon sites can be extrapolated to approximately 95 and 40 percent of the Rio Grande and Highlands CSPs, respecively.

"TUARO NATIONAL, INC.



FN-TR-26c

,

Ģ

TABLE OF CONTENTSAPPENDIX A

۰.

. . .

A-2

TEXT

	Page
GLOSSARY OF TERMS	A-1
LIST OF FIGURES	
	Figure
SUMMARY OF CALICHE DEVELOPMENT	A-1
LIST OF TABLES	
	Table
EXPLANATION OF GEOLOGIC UNITS	A-1

UNIFIED SOIL CLASSIFICATION SYSTEM

TUGRO NATIONAL, INC.

GLOSSARY OF TERMS

- ACTIVITY NUMBER A designation composed of the valley abbreviation followed by the activity type and a unique number; may also be used to designate a particular location in a valley.
- AEROMAGNETIC DATA Magnetometer observations made from an airplane.
- ALLUVIAL BASIN A lowland area, generally between uplifted mountain blocks, filled with alluvial deposits.
- ALLUVIAL FAN A low, outspread, relatively flat to gently sloping mass of alluvium, shaped like an open fan or a segment of a cone, deposited by a stream (especially in a semiarid region) at the place where it issues from a narrow mountain valley upon a plain or broad valley. It is steepest near the mouth of the valley where its apex points upstream, and it slopes gently and convexly outward with gradually decreasing gradient.
- ALLUVIAL FAN DEPOSITS Alluvium deposited by a stream or other body of running water as a sorted or semisorted sediment in the form of a cone or fan at the base of a mountain slope.
- ALLUVIAL PLAIN A level or gently sloping tract or a slightly undulating land surface produced by extensive deposition of alluvium, usually adjacent to a river that periodically overflows its banks; it may be situated on a flood plain, a delta, or an alluvial fan.
- ALLUVIUM A general term for unconsolidated clay, silt, sand, gravel, and boulders deposited during relatively recent geologic time by a stream or other body of running water as a sorted or semisorted sediment in the bed of a stream or on its flood plain or delta, or as a cone or fan at the base of a mountain slope.
- ANOMALY 1) A deviation from uniformity in physical properties; especially a deviation from uniformity in physical properties of exploration interest. 2) A portion of a geophysical survey which is different in appearance from the survey in general.
- AQUIFER A permeable saturated zone below the earth's surface capable of conducting and yielding water as to a well.

"TUBRO NATIONAL, INC.

- ARKOSIC SANDSTONE A sandstone with considerable feldspar, such as one containing minerals from coarse-grained quartzofeldspathic rocks (granites, granodiorites, medium or high-grade schists) or from older, highly feldspathic sedimentary rocks; specifically a sandstone containing more than 25% feldspar and less than 20% matrix material of clay, sericite, and chlorite.
- ARRIVAL An event; the appearance of seismic energy on a seismic record; a line-up of coherent energy signifying the arrival of a new wave train.
- ATTERBERG LIMITS A general term applied to the various tests used to determine the various states of consistency of fine grained soils. The four states of consistency are solid, semisolid, plastic, and liquid.

Liquid limit (LL) - The water content corresponding to the arbitrary limit between the liquid and plastic states of consistency of a soil (ASTM D423-66).

Plastic limit (PL) - The water content corresponding to an arbitrary limit between the plastic and the semisolid states of consistency of a soil (ASTM D424-59).

Plasticity index (PI) - Numerical difference between the liquid limit and the plastic limit.

- BASIN-FILL MATERIAL/BASIN-FILL DEPOSITS Heterogenous detrital material deposited in a sedimentary basin.
- BEDROCK Rock with a seismic p-wave velocity of 7000 ft (2333 m) per second or more.
- BOUGUER ANOMALY The residual value obtained after latitude, elevation and terrain corrections have been applied to gravity data.
- BOULDER FIELD Five or more rocks, each with diameters of 6 ft or more occurring within an acre.
- BULK SAMPLE A disturbed soil sample (bag sample) obtained from cuttings brought to the ground surface by a drill rig auger or obtained from the walls of a trench excavation.
- c Cohesion (Shear strength of a soil not related to interparticle friction).
- CALICHE Gravel, sand or other material cemented principally by calcium carbonate.

A-2

- CALIFORNIA BEARING RATIO (CBR) A test performed on a specifically prepared soil sample which is useful in the design of road pavement to be supported by the soil tested (ASTM D1833-73). The load is applied on the penetration piston which is penetrated into the soil sample at a constant penetration rate. The bearing ratio reported for the soil is normally the one at 0.1 inches (2.5 mm) penetration.
- CANDIDATE One of some group of regions, areas or sites being considered for MX deployment. Removal of candidate from a specifically named region, area or site term indicates selection by SAMSO/MNND.
- CANDIDATE DEPLOYMENT AREA (CDA) An area encompassing between 500 and 1000 square nautical miles of potentially suitable land with either naturally or artificially defined boundaries designated for convenience of study, discussion and data depiction. The candidate deployment area could be composed of two to four parcels and should have a specific place name description.
- CANDIDATE DEPLOYMENT PARCEL (CDP) An area of 150 to 500 square nautical miles potentially suitable for MX siting which, when aggregated with others, forms a Candidate Deployment Area. Each parcel should have a specific geographic description. (In the Basin and Range Physiographic province a parcel may correspond to a geographic valley and in Texas to some agri-economic unit.)
- CANDIDATE DEPLOYMENT SITE (CDS) A non-specific (i.e. not finally approved) site proposed for some element of the MX system within a chosen deployment area (i.e. trench or shelter site).
- CANDIDATE SITING PROVINCE (CSP) An area potentially suitable for deployment of the MX system generally encompassing more than 6000 square nautical miles which, in a broad sense, is homogeneous with respect to most of the important characteristics governing siting of a total MX system.
- CANDIDATE SITING REGION (CSR) Potentially suitable area between 4000 and 6000 square nautical miles within one, or encompassing portions of more than one, candidate siting province which allows for full MX deployment.

A-3

CAPABLE (fault) - Movement at or near the surface at least once in the past 35,000 years, and/or more than once in the past 500,000 years, (Nuclear Regulatory Commission).

- CAPROCK A resistant, moderately to strongly cemented caliche layer forming a "cap" over less resistant layers.
- CD TRIAXIAL SHEAR-A type of test to measure the shear strength of an undisturbed soil sample
- CLOSED BASIN A catchment area draining to some depression or lake within its area, from which water escapes only by evaporation.
- COARSE-GRAINED A term which applies to a soil of which more than one-half of the soil particles, by weight, are larger than 0.075 mm in diameter (passing the No. 200 U.S. size).
- COARSER-GRAINED A term applied to alluvial fan deposits which are predominantly composed of material larger than 3 inches (76 mm) in diameter.
- COLLUVIAL DEPOSITS A general term applied to any loose, heterogenous, and incoherent mass of soil material or rock fragments deposited chiefly by dislodgement and downslope transport of the material under the direct application of gravitational body stresses. Material is usually found at the base of a steep slope or cliff.
- COMPACTION TEST A type of test to determine the relationship between the moisture content and density of a soil sample which is prepared in compacted layers at various water contents (ASTM D1557-70).
- COMPRESSIBILITY-Property of a soil pertaining to its susceptibility to decrease in volume when subjected to load.
- COMPRESSIONAL WAVE -An elastic body wave in which particle motion is in the direction of propagation; the type of seismic wave assumed in conventional seismic exploration. Also called <u>P-wave</u>, <u>dilatational wave</u>, and <u>longitudinal</u> wave.
- CONSOLIDATION TEST A type of test to determine the compressibility of a soil sample. The sample is enclosed in the consolidometer which is then placed in the loading device. The load is applied in increments at certain time intervals and the change in thickness is recorded.

CONTERMINOUS UNITED STATES - The contiguous 48 states.

- CORE SAMPLE A cylindrical sample obtained with a rotating core barrel with a cutting bit at its lower end. Core samples are obtained from indurated deposits and in rock.
- DEBRIS FLOW A high-density flow of mud containing abundant coarse-grained materials (boulders, cobbles, gravel, sand) that frequently result from an unusually heavy rain.
- DEGREE OF SATURATION Ratio of volume of water in soil to total volume of voids.

DETECTOR - See GEOPHONE.

- DIRECT SHEAR TEST A type of test to measure the shear strength of a soil sample where the sample is forced to fail on a predetermined plane.
- DISSECTION/DISSECTED (alluvial fans) The cutting of stream channels into the surface of an alluvial fan by the movement (or flow) of water.
- DISTAL That portion of an alluvial deposit farthest from its point of origin.
- DRY UNIT WEIGHT/DRY DENSITY Weight per unit volume of the solid particles in a soil mass.
- ELECTRICAL CONDUCTIVITY Ability of a material to conduct electrical current
- ELECTRICAL RESISTIVITY Property of a material which resists flow of electrical current
- ENTRENCH The process whereby a stream erodes downward to form a trench.
- EOLIAN A term applied to materials which are deposited by wind.
- EPHEMERAL(stream) A stream in which water flow is discontinuous and of short duration.
- EXTERNAL DRAINAGE Stream drainage system whose downgradient flow is unrestricted by any topographic impediments.
- EXTRUSIVE (rock) Igneous rock that has been ejected onto the earth's surface (e.g., lava, basalt, rhyolite, andesite; detrital material, volcanic tuff, pumice).

A-5

UGRO NATIONAL ING

FAULT - A plane or zone of rock fracture along which there has been displacement.

- FAULT BLOCK MOUNTAINS Mountains that are formed by normal faulting in which the surface crust is divided into structural, partially to entirely fault-bounded blocks of different elevations.
- FINE-GRAINED A term which applies to a soil of which more than one-half of the soil particles, by weight, are smaller than 0.075 mm in diameter (passing the No. 200 U.S. size sieve).
- FINER-GRAINED A term applied to alluvial fan deposits, which are composed predominantly of material less than 3 inches (76 mm).
- FLOODING/LOW ENERGY FLOW Flood waters flowing on a slope
 of low gradient.
- F_CVIAL DEPOSITS Material produced by river action; generally loose, moderately well-graded sands and gravel.
- FORMATION A mappable assemblage of rocks characterized by some degree of homogeneity or distinctiveness
- FREE AIR ANOMALY Gravity data which have been corrected for latitude and elevation (free air correction) but not for the density of rock between the datum and the plane of measurement (Bouguer correction).
- FUGRO DRIVE SAMPLE A 2.50 inch (6.4 cm) diameter soil sample obtained from a drill hole with a Fugro Drive Sampler. The Fugro drive sampler is a ring-lined barrel sampler containing 12 one-inch (2.54 cm) long brass sample rings. The sampler is advanced into the soil using a drop-hammer.
- GAMMA A unit of magnetic-field intensity. A gamma is 10^{-5} oersteds; sometimes expressed (incorrectly) as 10^{-5} gauss with which it is numerically equal.
- GEOMORPHOLOGY The study, classification, description, nature, origin, and development of present landforms and their relationships to underlying structures, and of the history of geologic changes as recorded by these surface features.
- GEOPHONE The instrument used to transform seismic energy into electrical voltage; a <u>seismometer</u>, jug, or <u>pick-up</u>.

TUGRO NATIONAL INC

- GRAIN-SIZE ANALYSIS (GRADATION) A type of test to determine the distribution of soil particle sizes in a given soil sample. The distribution of particle sizes larger than 0.075mm (retained on the No. 200 sieve) is determined by sieving, while the distribution of the particle sizes smaller than 0.075 mm is determined by a sedimentation process, using a hydrometer.
- GRAVEL Particles of rock that pass a 3-in. (76.2 mm) sieve and retained on a No. 4(4.75 mm0 sieve
- GRAVITY The force of attraction between bodies because of their mass. Usually measured as the acceleration of gravity.
- GRAVITY GRADIENT The partial derivative of the acceleration of gravity with respect to distance in a particular direction, for which purpose the acceleration of gravity is considered as a scalar.
- INTERIOR DRAINAGE Stream drainage system that flows into a closed topographic low (basin).
- INTRUSIVE (rock) A rock formed by the process of emplacement of magma (liquid rock) in pre-existing rock. (e.g. granite, granodiorite, guartz monzonite).

LACUSTRINE DEPOSITS - Materials deposited in lake environment.

LINE - A linear array of observation points, such as a seismic line.

LIQUID LIMIT - See ATTERBERG LIMITS.

LOESS - A wind blown deposit predominantly silt or silty clay or clayey silt.

LOW ENERGY FLOW - See FLOODING.

- MAGNETIC INTENSITY A vector quantity measuring magnetic field strength. The unit of magnetic intensity commonly used in geophysical exploration is the gamma (see GAMMA).
- MANTLED PLAYA A playa surface or a portion of the surface that is covered with younger geologic material such as windblown sand, or alluvium.

MILLIGAL - A unit of acceleration used with gravity measurements; 1 milligal = 10^{-5} m/sec.². Abbreviated mgal.

A-7

TUBRO NATIONAL INC

MOISTURE CONTENT - The ratio, expressed as a percentage, of the weight of water contained in a soil sample to the oven-dry weight of the sample.

- N VALUE Penetration resistance, number of blows required to drive the standard split spoon sampler for the second and third six inches (0.15 m) with a 140 pound (63.5 kg) hammer falling 30 inches (0.76 m) (ASTM D1586-67).
- OPTIMUM MOISTURE CONTENT Moisture content at which a soil can be compacted to a maximum dry unit weight by a given compactive effort
- OVERBANK FLOODING A large flow of water that overflows the sides of A stream channel.
- 0 Angle of internal friction
- PATINA A dark coating or thin outer layer produced on the surface of a rock or other material by weathering after long exposure (e.g., desert varnish).
- PAVEMENT/DESERT PAVEMENT When loose material containing pebble-sized or larger rocks is exposed to rainfall and wind action the finer dust and sand are blown or washed away and the pebbles gradually accumulate on the surface, forming a mosaic which protects the underlying finer material from wind attack. Pavement can also develop in finer-grained materials. In this case the armored surface is formed by dissolution and cementation of the grains involved.
- PEGAMATITE DIKE A coarse grained igneous rock of granitic composition that forms as a tabular intrusion that cuts across the planar structures of the surrounding rock.

P-WAVE - See COMPRESSIONAL WAVE.

- PERIMETER SEISMIC REFRACTION SURVEY Shallow seismic refraction measurements made around the perimeter of a valley.
- **PERMEABLE** The ability of liquid to pass through soil and/or rock material.

PICK-UP - See GEOPHONE.

A-8

TUBRO NATIONAL INC

1.

PITCHER TUBE SAMPLE - An undisturbed, 2.87 inch (73 mm) diameter soil sample obtained from a drill hole with a Pitcher tube sampler. The primary components of this sampler are an outer rotating core barrel with a bit and an inner stationary, spring-loaded, thin-wall sampling tube which leads or trails the outer barrel drilling bit, depending upon the hardness of the material being penetrated.

PLASTIC LIMIT - See ATTERBERG LIMITS.

PLASTICITY INDEX - See ATTERBERG LIMITS.

- PLAYA/PLAYA DEPOSITS A term used in the southwest U.S. for a dried-up, flat-floored area composed of thin, evenly stratified sheets of fine clay, silt, or sand, and representing the lowest part of a shallow, completely closed or undrained, desert lake basin in which water accumulates and is quickly evaporated, usually leaving deposits of soluble salts.
- PONDING (of water) The accumulating of water in a topographic depression.
- PRIME Modifier used to indicate the highest ranking province, region, area, or site. If not an interdisciplinary ranking, then a qualifier should be used such as "prime" geotechnical candidate siting area".
- PROXIMAL That portion of an alluvial deposit nearest to its point of origin.
- REGIONAL The general attitude or configuration disregarding features smaller than a given size. The regional gravity is the gravity field produced by large-scale variations ignoring anomalies of smaller size. See residualize.
- RELATIVE AGE The relationship in age (oldest to youngest) between geologic units without specific regard to number of years.
- RESIDUAL What is left after a regional field has been removed, as in gravity or magnetic analysis. See RESIDUALIZE.

A-9

- RESIDUALIZE The process of separating a graphically depicted curve or a surface into its low-frequency parts (called the regional) and its high-frequency parts (called the residual). Residualizing is an attempt to sort out of the total field those anomalies which result from local structure; that is, to fine local anomalies by subtracting gross (regional) effects.
- ROCK UNITS Distinct rock masses with different characteristics (e.g., igneous, metamorphic, sedimentary).
- S-WAVE See SHEAR WAVE.
- SAND Soil passing through No. 4(4.75 mm) sieve and retained on No. 200 (0.075 mm) sieve
- SAND DUNE A low ridge or hill consisting of loose sand deposited by the wind, found in various desert and coastal regions and generally where there is abundant surface sand.
- SEISMIC Having to do with elastic waves. Energy may be transmitted through the body of an elastic solid as P-waves (compressional waves) or S-waves(shear waves).
- SEISMIC REFRACTION DATA: deep/shallow Data derived from a type of seismic shooting based on the measurement of seismic energy as a function of time after the shot and of distance from the shot, by determining the arrival times of seismic waves which have travelled nearly parallel to the bedding in high-velocity layers, in order to map the depth to such layers.

SEISMOGRAM - A seismic record.

SEISMOMETER - See GEOPHONE.

- SHEAR WAVE A body wave in which the particle motion is perpendicular to the direction of propagation. Also called S-Wave or transverse wave.
- SHEET FLOW A process in which storm-borne water spreads as a thin, continuous veneer (sheet) over a large area.
- SHEET SAND A blanket deposit of sand which accumulates in shallow depressions or against rock outcrops, but does not have characteristic dune form.

SHOT - Any source of seismic energy; e.g., the detonation of an explosive.

A-10

- SHOT POINT The location of any source of seismic energy; e.g., the location where an explosive charge is detonated in one hole or in a pattern of holes to generate seismic energy. Abbreviated SP.
- SILT AND CLAY Fine-grained soil passing through No. 200 (0.075 mm) sieve.
- SITE Location of some specific activity or reference point. The term should always be modified to a precise meaning or be clearly understood from the context of the discussion.
- SPECIFIC GRAVITY The ratio of the weight in air of a given volume of soil solids at a stated temperature to the weight in air of an equal volume of distilled water at a stated temperature.
- SPLIT SPOON SAMPLE A disturbed sample obtained with a
 split spoon sampler with an outside diameter of 2.0
 inches (5.1 cm). The sample consists of a split barrel
 which is driven into the soil using a drop-hammer.
- SPREAD The layout of geophone groups from which data from a single shot are recorded simultaneously. Spreads containing twenty-four geophones have been used in Fugro's seismic refraction surveys.
- STREAM CHANNEL DEPOSITS Materials (clay, silt, sand, gravel, cobbles, boulders) which have been deposited in a stream channel.
- STREAM TERRACE DEPOSITS Stream channel deposits no longer part of an active stream system, generally loose, moderately well graded sand and gravel.
- SURFICIAL DEPOSIT Unconsolidated residual and alluvial deposits occurring on or near the earth's surface.
- TRANSITORY A poorly defined, shallow ephemeral stream across an alluvial fan surface, the position of which is temporary and tends to shift frequently.
- UNCONFINED COMPRESSION A type of test to measure the compressive strength of an undisturbed soil sample.

UNIFIED SOIL CLASSIFICATION SYSTEM (USCS) - A system which determines soil classification on the basis of grain-size distribution and Atterberg Limits. (See page A-17).

A-11

TUBRO NATIONAL INC

- VALLEY SEISMIC REFRACTION SURVEY Deep seismic refraction measurements made near the middle of a valley to determine seismic wave propagation velocities and thickness of basin fill.
- VELOCITY Refers to the propagation rate of a seismic wave without implying any direction. Velocity is a property of the medium and not a vector quantity when used in this sense.
- VELOCITY LAYER A layer of rock or soil with a homogenous seismic velocity.
- VELOCITY PROFILE A cross-section showing the distribution of material seismic velocities as a function of depth and its configuration.
- WASH SAMPLE A sample obtained by screening the returned drilling fluid during rotary wash drilling to obtain lithologic information between samples.

Definitions were derived in part from Webster's New Collegiate Dictionary (1972 edition), Glossary of Geology (American Geological Institute, 1972), Encyclopedic Dictionary of Exploration Geophysics (Sheriff, 1973), and 1976 Annual Book Book of ASTM Standards. FN-TR-26c

~ ~

		DIAGNOSTIC	CARBONATE MOR	PHOLOGY		
STAGE	GRAV	ELLY SOILS		NO	NGRAVELLY SDILS	
I	Thin, disco	ntinuous pebble	coatings	Few filam	ents or faint coa	tings
п	Continuous interpebble	pebble coatings fillings	, Som e	Few to ab filaments	undant nodules, f	lakes,
ш	Many interp	ebble fillings		Many nodu fillings	les and internodu	lar
Ш	Laminar hor horizon	izon overlying (lugged	Laminar h horizon	orizon overlying (plugged
ST	AGE	I Weak Ca	II Strong Ca	Ш К	I¥ Indurated K	
GR	AVELLY SOILS				K21 K22 K3	
NO	NGRAVELLY SOILS	A A A A A A A A A A A A A A A A A A A		1958 4	K21 K2 K3	
	evelopment of a cal n, followed by cont plugged.		of carbonate	until, in St	•	is
The I	,L.H. Peterson,F.F., a K.horizon; A.master h mulation; Soil Scienci	orizon of carbona	te Official	MX SITING IN RTMENT OF THE		+ 1 G A-

_ ---

١.

-

.....

....

FN-TR-26c

١

,

(1) Arei Symbols	MX (2 Geologic Units	NON-ROCK UNITS	(1) AREI Symbols	MX (2 GEOLDGIC UNITS)
Au. Ast	Au	Non-rock Deposits (undifferentiated); fine- to coarse-grained materials deposited by alluvial, fluvial, eolian, lacustrine, gravity or glacial processes.			Showi rock type
Aal	A 1	Fluvial Deposits; predominantly composed of poorly- to well-graded sand and gravel with lesser amounts of silt- and boulder-sized material. The unit pre- dominantly consists of recent water-laid deposits occupying present drainages	gr	I	IGNE) part I1
	-	and flood plains. – Older Fluvial Deposits (Alo) are generally thicker, more extensive units deposited in ancestral fluvial systems.	¥u		I2
	-	Alluvial Outwash Deposits (Alw) consist of mixed, geomorphically nondescript alluvial and fluvial deposits covering large, relatively flat, river and playa basins.	۷b		13
At	A2	Terrace Deposits; predocent thy composed of moderately to well graded, clay- to gravel-sized material to compally elevated terraces bordering modern streams (A2s) and lakes playas (A21).	Su	S	I 4 Sedit
	A3	Eolian Deposits; predominantly composed of poorly graded sand-sized material deposited by wind action. Deposits may consist of mixed sand, silt, and clay (A3u), or be differentiated on the basis of predominant grain size and landform.	Qtz	5	some evapo S1
		 A3s d - Predominantly fine sand-sized material deposited in sheets (A3s) or dunes (A3d). A31 - Loess composed predominantly of silt-sized material with lesser amounts 	Psa, Pn Ph, Cau Ls, Py, Par	ı .	S2 S3 S4
		of clay and fine sand. A3f — Predominantly clay-sized material with lesser amounts of silt and fine sand.			S 5
	A4	Lacustrine, Estuarine, and Playa Deposits; predominantly composed of poorly graded clay, silt, and fine sand deposited in bodies of standing water. Older lacustrine, estuarine, and playa deposits (A4o) are thicker, more extensive units occupying ancestral lake basins.	Qtz. gn	M	METAI sedi weat
A af	A5	Alluvial Fan Deposits; predominantly composed of well graded sand and gravel with varying amounts of silt-, cobble-, and boulder-sized material. Deposited principally by distributary channels adjacent to mountain fronts. Relative ages are indicated by o - older, i - intermediate, or y - younger.		C	ROCK perc
	A 6	Pediment, Pediment Deposits, and Areas of Shallow Rock; planated bedrock shelf or near surface rock generally overlain by a thin mantle of sand- to boulder- sized residual or alluvial material.			Modi (cal A3s A5y(
	A7	Colluvial Deposits; predominantly composed of moderately- to well-graded sand and gravel with varying amounts of silt-, cobble-, and boulder-sized material. Deposited locally by gravity and water adjacent to steep gradients.			S5to
		ols were developed for use on the Aggregate Resources Evaluation Investigation on 5.1 and Drawings 5.1A chrough 5.1C)			
(2)	MX Geolog	ic units were used for Methodology, Screening, and Characterization studies.			

ļ

ł,

	MX (2) Ologic Inits	ROCK UNITS
YMBOLS L		Shown in regions where rock is expose; the areally predominant (greater.than 70 percent) rock type is indicated. In those are swere two rock types occur the predominant rock type is shown followed by the subord are rock type (e.g. S2MP 14T).
	•	IGNEOUS (UNDIFFERENTIATED). Rocks formed by solidification of a molten or partially molten mass.
gr		It Intrusive - Typically crystalline, formed by the solidification of molten material below the surface (e.g., granite, syenite, diorite).
Vu		I2 Extrusive (undifferentiated). Formed by solid=fication of molten material at or near the surface.
¥b		I3 Extrusive (flows). Extrusive rocks formed by solidification of lava (e.g. basalt, dacite) is denotes young basaltic flows which may be interbedded with basis of materials.
		I4 Extrusive (volcaniclas) Formed by accumulation, welding and or cementation of deposits of volcanic ejecta (e.g. tuff, agglomerate, lapilli).
Su		SEDIMENTARY (UNDIFFERENTIATED). Coarse- to fine-grained materials that exhibit some degree of cementation and were deposited by water, wind, gravity, or evaporation.
Otz sa. Pm. h. Cau. s. Py.		 Sandstone. Composed predominantly of sand-sized particles. Limestone and Dolomite. Composed predominantly of carbonate material. Shale. Composed predominantly of clay- and silt-sized particles
ar		 (e.g. shale, siltstone, mudstone). S4 Evaporites. Sediments deposited from solution as a result of evaporation (e.g. gypsum, anhydrite, halite).
		S5 Clastics. Undifferentiated deposits composed of silt- to boulder-sized material. May be angular to rounded.
Qtz. gn	M	METAMORPHIC (UNDIFFERENTIATED). Rocks formed through alteration of igneous or sedimentary rock material by pressure, heat, or chemical changes below the weathered zone (e.g. gneiss, schist, slate, marble, quartzite).
	C	ROCK COMPLEXES. Indicated where no areally predominant (greater than 70 percent) rock type is present.
		<u>USEAGE</u>
		 Modifying letter (r) indicater concentrations of resistant secondary carbonate (caliche), silicious, ferruginous and or gypsiferous material, e.g. A5ir. A3s A5y - Mixed non-rock units; cost areally extensive unit is listed first. A5y(A5i) - Parenthetic unit underlies thin veneer of overlying mapped unit. S5to - Established formations may have a supplemental letter added to distinguish formal designation (e.g. Tertiary Ogallala Fm.).
		EXPLANATION OF GEOLOGIC UNITS
		DEPARTMENT OF THE AIR FORCE SAMSO A-
		FUGRO NATIONAL IN

~

· _

UNIFIED SOIL CLASSIFICATION SYSTEM

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE SANSO

UGRO NATIONAL, INC.

-

A-2

TABLE

FN-TR-26c

1.

• • • • • • •

- .

--

ie M			slavi vels	ind .		en sús a	spu	5	t trailen	а з эц:	tuode	si əsie		Nº 50	ગ્યા			Highly
Major Divisions	~	urqı) A pe na ieve size ieve size	ai noit a to.V a to.V	ni- 2/	nshi ion, the	o fo fiane smaller ieve size fasificat for for	si nois: s è .oN o lausiv	ושמ			O bns Dí bin	-	8	γad⊃br , ,timil,b		:	Organic
		ines) Le or Dravels	Clean (Liti	s with secable unt	istevel Fir Appro Appro Appro Appro	uea) je or Sands	Clean (Lit	with with with	sbris nig Appre oms ni lo			nadt i			Ereater than 50		Soils	
Group Symbols	-	GW	GP	GM	CC	SW	ЧS	SM	sc			ML	τJ	10	MP	СН	но	Ρı
Typical Names	-	Well-graded gravels, gravel-sand mix- tures, little or no fines.	Poorly-graded gravels, gravel-sand mix- tures, little or no fines.	Silty gravels, gravel-sand-silt mixtures.	Clayey gravels, gravel-sand-clay mix- turca.	Well-graded sands, gravelly sands, little or no fines.	Poorly graded sands, gravelly sands, httle or no fines.	Silty sands, sand-silt mixtures.	Clayey sands, sand-clay mixtures.			Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.	Inorganic clays of low to medium plas- ticity. gravely clays, sandy clays, silty clays, lean clays.	Organic silts and organic silty clays of low plasticity.	Inorganic silts, micaceous or diatoma- ceous fine sandy or silty soils, elastic silts.	Inorganic clays of high plasticity, fat clays.	Organic clays of medium to high plas- ticity, organic silts.	Peat and other highly organic soils.
Excluding 1 and hasing fr		Wide range in amounts of 1	Predominantly with some int	Nonplastic fines or (for identification	Plastic fines (f. CL tielow).	Wide range in amounts of a	Predominantly with some int	Nonplastic fines (for identifica	Plastic fines (10 CL below).	Iden on Fraction S	Dry Strength (Crushing characteristics)	None to sl it	Medium to high	Slight to medium	Slight to medium	High to very high	Medium to high	Readily identifie and frequently
Even Jaconingation Procedures (Excluding particles larger than 3 inches and having fractions on estimated weights)	~	in grain sizes a all intermediate	Predominantly one size or a with some intermediate sizes	onplastic fines or fines with low (for identification procedures see	Plastic fines (for identification procedures see CL helow).	n grain sizes and substantial all intermediate particle sizes.	Predominantly one size of a range o with some intermediate sizes missing.	Nonplastic fints or fines with low plasticity. (for identification procedures see ML below)	Plastic fines (for identification procedures CL below).	Identification Procedures on Fraction Smaller than No. 40 Sieve Size	Dilatancy (Reaction to shaking)	Quick to slow	None to very slow	Slow	Slow to none	None	None to very slow	Readily identified by color, odor, spongy and frequently by fibrous texture.
cedures han 3 inches ated weights)		rrain sizes and substantial intermediate particle sizes.	range of sizes missing.	plasticity. ML below)	procedures see	und substantial particle sizes.	range of sizes mitsing.	ow plasticity. He ML below)	procedures see	lures 40 Sieve Size	Toughness (Consistency near PL.)	None	Medium	Slight	Slight to medium	High	Slight to medium	r, spongy feel ture.
Information Required for Describing Soils	0	For undisturbed soils add information	on autaincation, urgree to conditions ness, cementation, moisture conditions and drainage characteristics.		Give typical name; indicate approximate percentages of aand and gravel, max size; angularity, surface cordi- tion, and hardness of the coarse	grains; local or geologic name and other pertinent descriptive informa- tion; and symbol in parentheses.	Example: Silty sand, eravelly: about 20% hard.	angular gravel particles X-in maximum size; rounded and sub angular and grains corse to fine;	dry strength; well compacted and moist in place; alluvial sand; (SM).			Give typical name, indicate degree and character of plasticity, amount and maximum size of coarse grains, color	in wet condition, odor if any. local or geologe: name, and other periment descriptive information; and symbol in narratheses.	-	For undisturbed soits and intorna- tion on structure, stratification, consistency in undisturbed and re- molded states, moisture and drain-	age conditions.	E.:ample: Clayey silt, brown, slightly plastic,	small percentage of fine sand, numerous vertical root holes, firm and dry in place, loess, (ML).

APPENDIX B

GEOTECHNICAL DATA - JORNADA DEL MUERTO

I

]

I

I

FN-TR-26c

2

1

TABLE OF CONTENTS APPENDIX B

- ----

BORING AND TRENCH LOGS

LOG OF BORING JM-B-2	Figure B-l
LOG OF BORING JM-B-6	Figure B-2
LOG OF BORING JM-B-11	Figure B-3
LOG OF TRENCH JM-T-2	Figure B-4
LOG OF TRENCH JM-T-4	Figure B-5
LOG OF TRENCH JM-T-14	Figure B-6
SUMMARY OF LABORATORY TEST RESULTS BORING JM-B-11 SUMMARY OF SHEAR STRENGTH TESTS	Table B-1
UNCONFINED COMPRESSION TEST RESULTS	Table B-2
TRIAXIAL SHEAR TEST RESULTS	Table B-3
DIRECT SHEAR TEST RESULTS	Table B-4

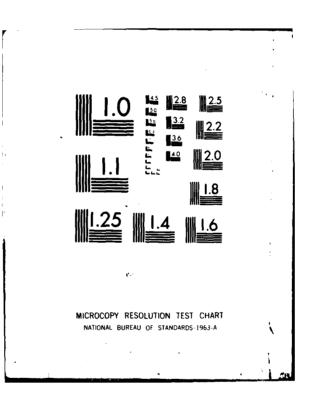
SUMMARY OF CALIFORNIA BEARING RATIO (CBR) TESTS

CALIFORNIA	BEARING	RATIO (CBR)	TEST	RESULTS	Table	B-5
CALIFORNIA	BEARING	RATIO (CBR)	CURVE	IS	Figure	B-7
GRAIN SIZE	CURVES,	CBR TES	TS			Figure	B-8

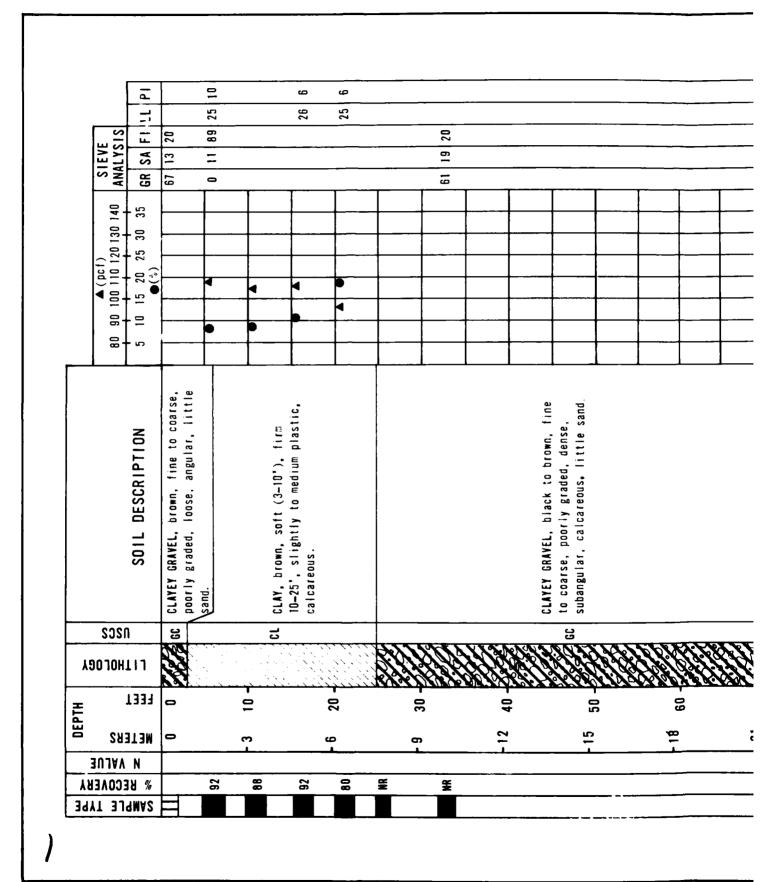
·· ·.

-

AD-A112 404	FUGRO NATIONAL INC LONG BEACH CA MX SITING INVESTIGATION. PRIME CHARACTERIZATION SITES PIO GRANDETC(U) FEB 79 FO4704-77-C-0010 FN-TR-26C NL
_	END
	Levit Mario 4 192 Dric
and the second sec	



FN-TR-26c



.

.~

CHEC

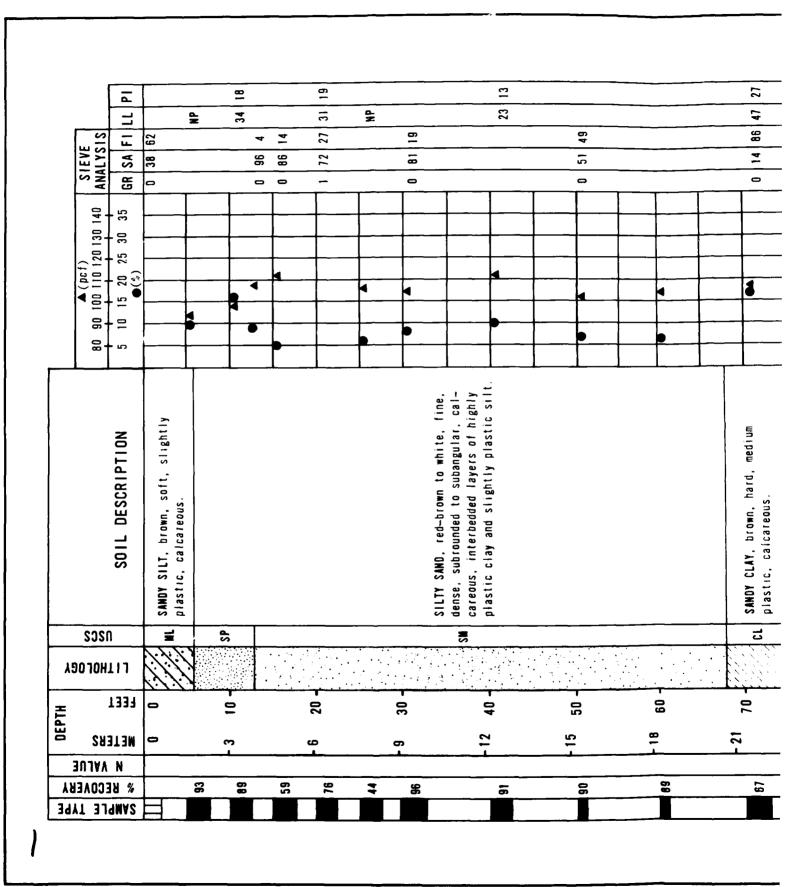
		- 21	07								;
	99	<u>,</u>	<u>.</u>		SiLTY SAND, gray, fine, poorly graded, very dense, calcareous, little fine gravel.	irly ous, little		1 21			
	85	- 24	©		SC poorly graded, very dense.	. En j		51 49	3 32		
ት 		- 27			SANDY GRAVEL, black to gray, fine, poorly graded, very dense, calcareous, some slightly plastic clay.	. fine. calcareous.	46	31 23 33 27 33	3 23	4	
	75		100		ROCK, fractured sandstone, light gray, stratified, 1/8" thick clay layers (104-108').	light ck clay					
		- 33	<u> </u>		Total Depth 108.0° (32.9m)						
		- 36 12	120 -								
		- 39 13	30								
			i			1400 1800 ▲(kg·m ³)	2200 3				
		SA	MPL	SAMPLE TYPES	ES						
				TANDARD	STANDARD PENETRATION TEST	BORING	NG DETAILS				
]		-	FUGRO DRIVE	RIVE	ELEV	ELEVATION	: 5260	Ē.	5260' (1603m)	
MX SI TMENT	ORNAC	Ħ	_	BULK		DATE	DATE DRILLED Drilling method	:10 A : Rota	ugus iry W	10 August 1977 Rotary Wash	-
OF TH	DA DE Ri		<u>م</u> ـ	PITCHER	t TUBE	HOLE	HOLE DIAMETER Casing installed	• • • •	4 7/8" (124m 108" (32.9m)	1 24mm) 9m)	
	O GR			CORE		WATE	WATER LEVEL		> 100°0'	(30.5m)	(m
FORC	ERT(ANDI		NGIN	ENGINEERING	VG PARAMETERS						

•

۰. ~

·

	•	
1400 1800 2200 ▲(kg·m ³)	BORING DETAILS ELEVATION : 5260°(1603m) DATE DRILLED : 10 August 1977 DRILLING METHOD : Rotary Wash HOLE DIAMETER : 4 7/8"(124mm) CASING INSTALLED : 108°(32.9m) WATER LEVEL : >100.0°(30.5m)	
39 130	SAMPLE TYPES SAMPLE TYPES SAMDARD PENETRATION TEST FUGRO DRIVE BULK BULK PITCHER TUBE PITCHER TUBE CORE Core Core Core Core Core Core Core Core	
	LOG OF BORING JM-B-2 JORNADA DEL MUERTO, NEW MEXICO RID GRANDE CSP MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - SAMSO FIGURE B-1 FURBED MATIONAL, EMC.	3



- .

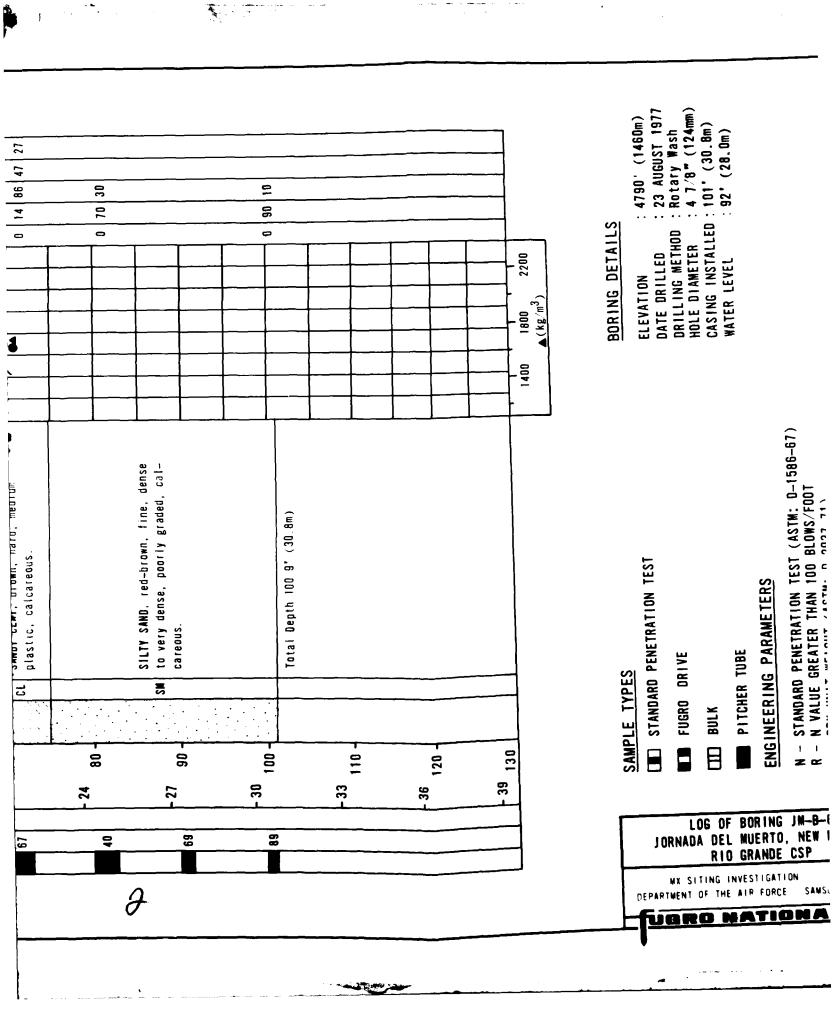
•

Ĩ

Î

۰.

FN-TR-26c



. .

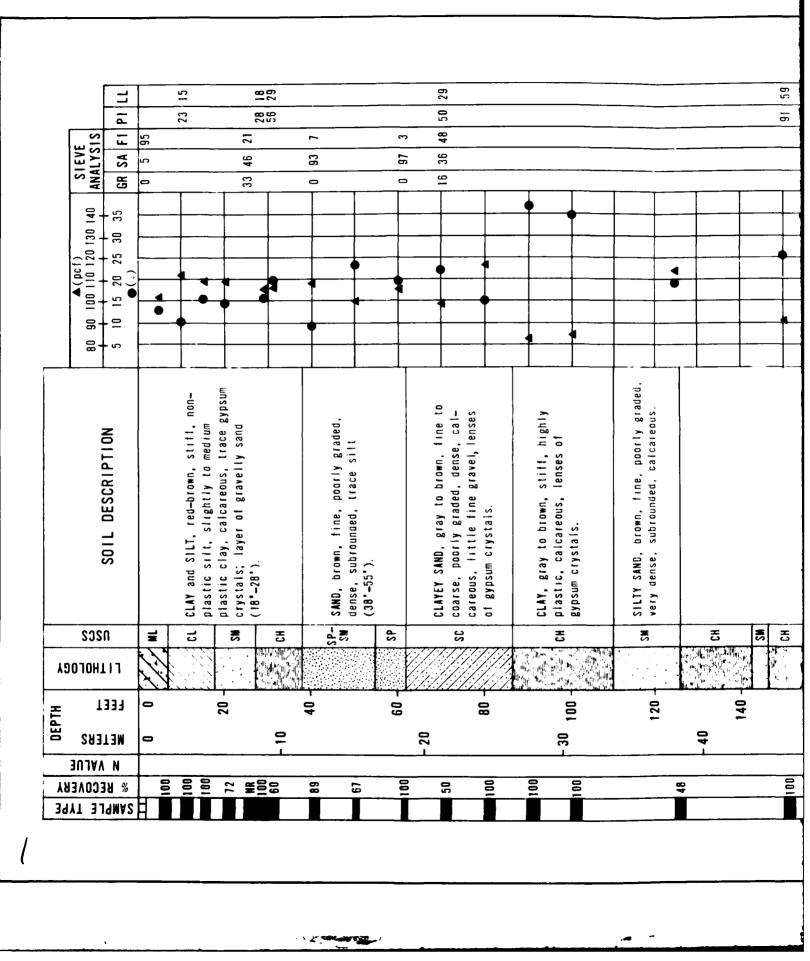
4

「「「「「「「「」」」」」 23 AUGUST 1977 Rotary Wash 4 7/8" (124mm) 4790' (1460m) :101' (30.8m) 92' (28.0m) BORING DETAILS CASING INSTALLED DRILLING METHOD HOLE DIAMETER 2200 DATE DRILLED WATER LEVEL **ELEVATION** ▲(kg/m³) 1800 1400 - STANDARD PENETRATION TEST (ASTM: D-1586-67) N VALUE GREATER THAN 100 BLOWS/FOOT DRY UNIT WEIGHT (ASTM: D-2937-71) MOISTURE CONTENT (ASTM: D-2216-71) STANDARD PENETRATION TEST ENGINEERING PARAMETERS FUGRO DRIVE PITCHER TUBE NO RECOVERY SAMPLE TYPES BULK 130 ł I I I E Æ z æ ◀ • 39 LOG OF BORING JM-B-6 Jornada del muerto, new mexico Rio grande CSP FIGURE B-2 INVESTIGATION SITING THE AIR FORCE SAMSO 0F INC 0

•.

Carl & Carl Carl Carl Carl Carl Carl Carl

FN-TR-26c



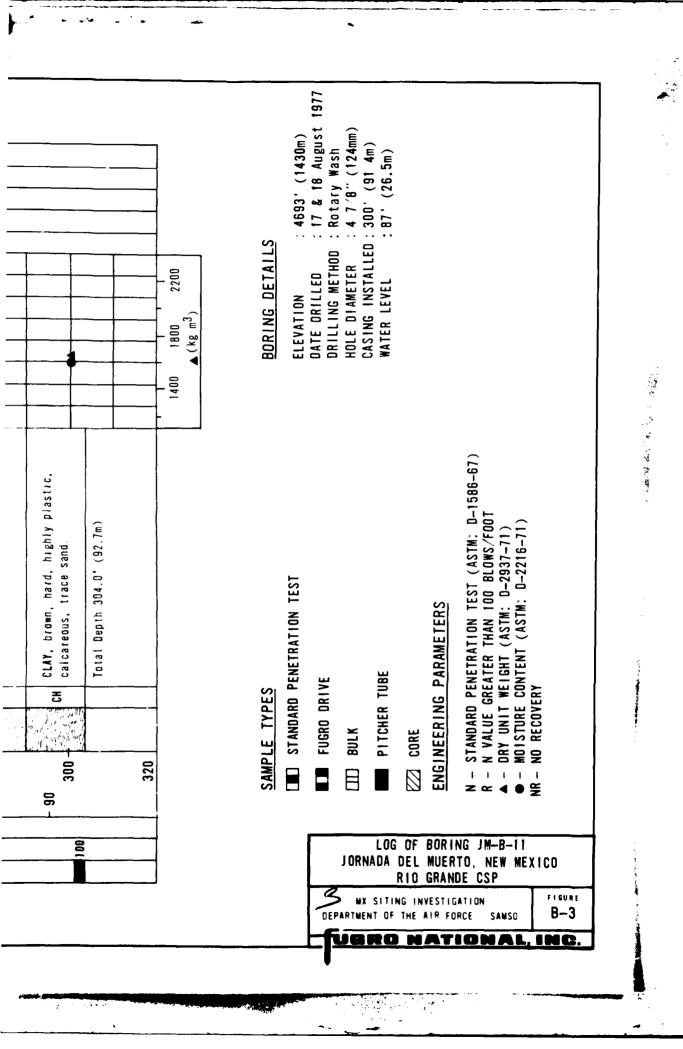
•.

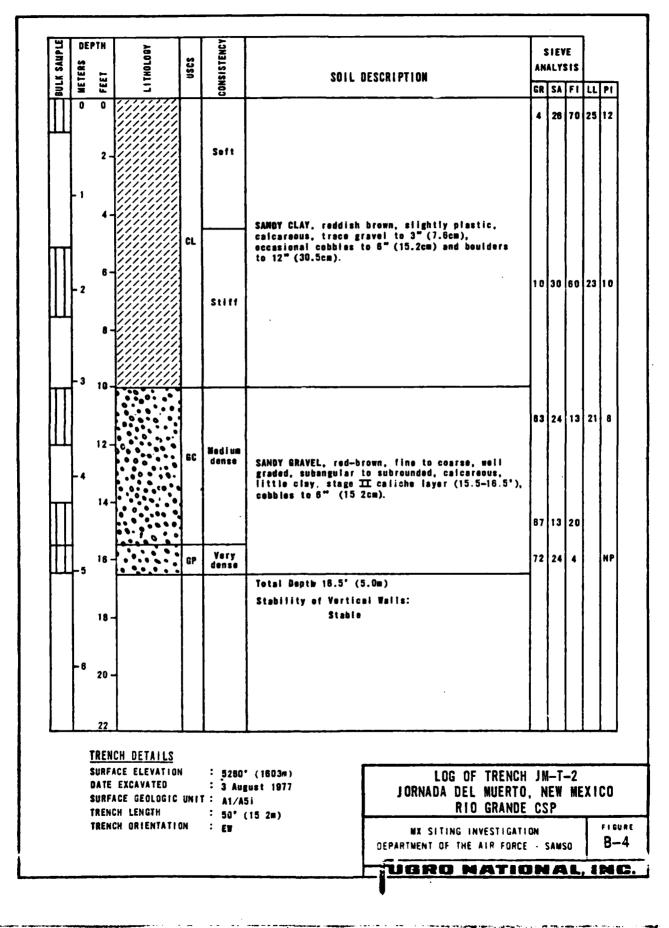
			••					•
1 p · · · ·	t Agiji Gora a sa Arag							a ' 4
		<u></u>				<u></u>	··	
ດ ເ					<u>. </u>			
.								
				·····				
						[
	•							2200
• • • • • • • • • • • • • • • • • • • •		•					_	. 0 . .
				•				1400
					·			.
•	<u> </u>		•	2				
			cal-	brown, hard, highly plastic, gypsum crystals, trace fine sand.	lense.			
CLAY, brown to red-brown, stiff to very stiff, slightly to highly plastic, calcareous, interbedded with layers of silty sand, trace gypsum crystals.			dense,	pias ace 1	very dense	plastic.	~	
rown, to h inte sand			fine.	s, tr		ighly nd.	92.7m	
CLAY, brown to red-br very stiff, slightly plastic, calcareous, with layers of silty gypsum crystals.			0#U, 1	ystal	brown, fine, calcareous	hard, highly trace sand	304.0° (92.7m)	
CLAY, brown to r very stiff, slig plastic, calcare with layers of s gypsum crystals.			۵	in, ha sum cr				
CLAY, brown to very stiff, si plastic, calca mith layers of wypsum crystal			SILTY SAND careous.		SILTY SAND, subrounded,	CLAY, brown, calcareous, i	Total Depth	
CLAY, very s plasti mith l gypsum			SILTY SA careous.	CLAY. trace	SILT	CLAY, calcar	Tota	
5 8 5 5	3	CC	NN S	5 	×.	B		
						n an		
160 -	200 -	220 -		240 - 260 -	280 -	- 00£	320	
20	09	F. -1	- 70	8		06		
	001	001	2	00	9	100		
2								

•.

...

-





<u>۱</u>

-

.

APPHOVED

CHELMED BY

5

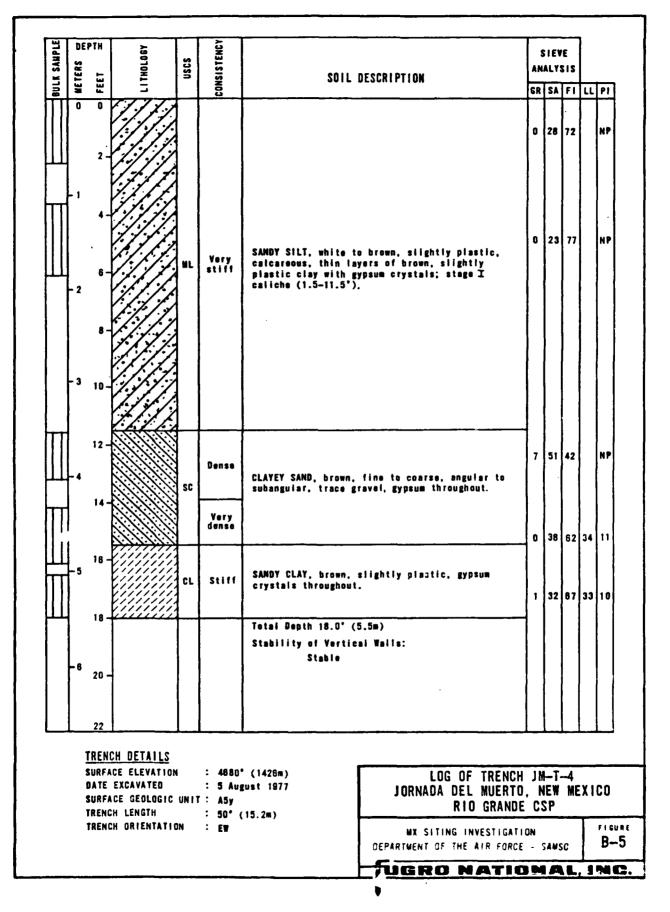
LUULED

FN-TR-28c

\$2 }

PRUVLU DI

÷ .

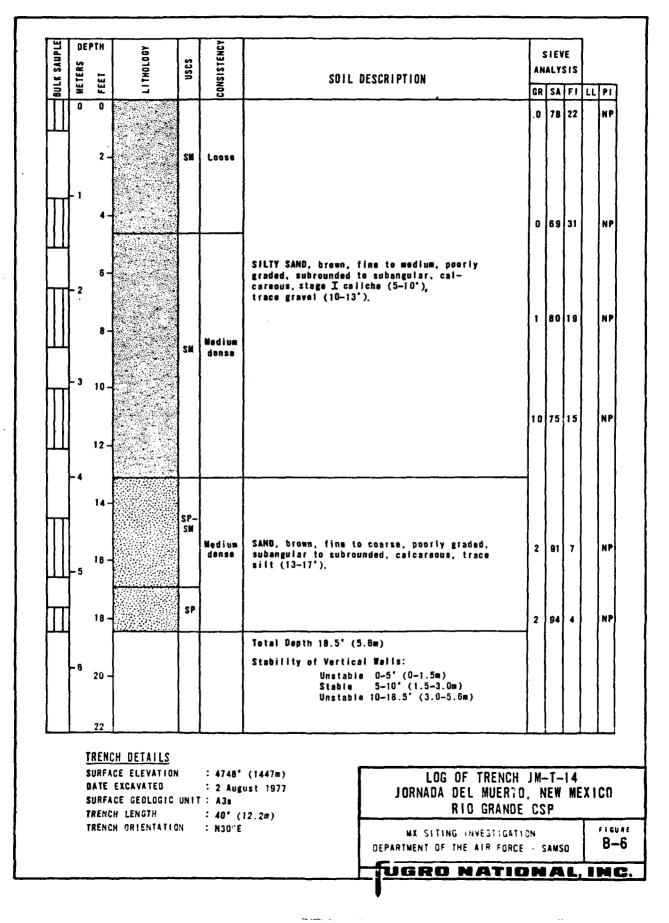


.

a a construction of the second s

....

FN-TR-26c



- .

و محمد الديم

ATTRUVED BI

CHECKES BY

1

7

•

t

	(a)									NT FIN		_				
N N N		SAMPLE I	NTERVAL	L,		TANDARI	SIEV				US	STAN	DARD	SIEVE I		5
BOR I NG Number	SAMPLE Number			BLDRS.	COBE	_		GRA				SA		-	<u>\$11</u>	
		FEET	METERS	24 "	12"	6"	3"	15"	3/4"	3/8"	4	10	40	100	200	Ŀ
M-B-11	B-1A	0.0-2.0	0.00-0.61					├ ───		_		100	99	97	95	-
	P1	5.5-6.2	1.68-1.89						L	· · ·				 	┟	-
	P-2	10.5-11.2	3.20-3.41	· · · ·		 									 	
	P3	15.5-16.2	4.72-4.94			┫────				 				 	 	┝
	P-4	20.7-21.4	6.31-6.52			-		100	85	76	67	(0)			21	┝
	B-5 P-6	25.5-26.5 28.0-28.7	7.77-8.08	i	<u>-</u>			100		-/0	67	60	46	27		┝
	P-7	31.0-31.7	9.45-9.66	<u> </u>				<u> </u>							} ── ──	┢
	P-8	40.2-40.9	12.25-12.47			<u> </u>				<u> </u>		100	97	43	7	╞
	P-9	50.5-51.5	15.39-15.70			<u> </u>				<u> </u>		100		+	<u> </u>	ŀ
	P-10	61.0-61.8	18.59-18.84			╂───		┠───		<u> </u>		100	80	3	3	┢
	P-11	71.1-71.8	21.67-21.88			<u> </u>		t	100	96	83	66	54	50	48	t
	P-12	80.5-81.2	24.54-24.75			1			1	<u> </u>				1	1	Γ
	P-13	90.5-91.2	27.58-27.80			1		[[1			<u> </u>	Γ
	P-14	100.5-101.2	30.63-30.85											1		Γ
	P-15	126.3-127.0	38.50-38.71						[[1		Γ
	P-1 6		45.87-46.09													
	F-17		53.49-53.71													L
	P-18	200.5-201.2	61.11-61.33			L				L			L	_		
	P-19					L			L	L				L		Ļ
	P-20		•		·	 	•		,	L		L	<u> </u>	ļ		ł
	P-21	275.4-276.0				L				<u> </u>					L	Ł
	P-22	302.0-302.7	92.05-92.26			_		I		·		ļ		_	ł	Ł
		· · · · · · · · · · · · · · · · · · ·		i		ļ				ļ			 _	 	↓	Ł
						<u> </u>		<u> </u>	<u> </u>				 	_	┥───	╀
						┣───		 	┢────	 	ļ			}		╀
				ł		┣───		 		<u> </u>				┨───	<u> </u>	╀
						<u> </u>		<u> </u>	├ ──	┨─────				╉───	╂───	ł
				┢──┤								┣───	<u> </u>	╉────	<u> </u>	┢
						┣───		<u> </u>		┣───			<u> </u>	<u> </u>	╋───	t
			······································			f		f	f	<u>{</u> −−		f	f	t	1	t
						1			[1		1		1	1	T
									[Γ
																ſ
																ſ
																I
																Ļ
											L	L		 	 	Ļ
													 			Ļ
		·		┠───┤										 	┣	Ļ
						L		L	L	L		L	I	┣		ł

·· ---

٠.

• •

1 .

B - Bulk

(b) NP - Not Plastic

l

	_		_	_	_		_									_	_				_
					r					11	N-SITU			C	OMPACTE	D		-			
ARD	SIEVE	10	PART SIZE	ICLE (mm)		TERBE Mits		USCS	ORY		MOISTURE Content (%)	SATURATION (\$)		MAX		OPTINUM Noisture (\$)	SPECIFIC Gravity Of Solids	TRIAXIAL	UNCONFINED Compression	5.4	
p		S11	T OR C	LAY	_		(- <i>)</i>	(c)	WEI	GHT	(%) (%)	TURA (\$)	V01D RATIO	DRY DE	INSITY		SPECIFIC Gravity Of Solid	N	ICON R	DIRECT Shear	
40	100	200	.005	.001	LL	PL	PI	1	(pcf)	(kg/m ³)	¥ 5	SA	N N	(pcf)	(kg/m ³)	53	9 8 P	Ĩ	58	22	
99	97	95					Ι	ML.	_												
								ML	101.3	1623	13.2	54	.66								
					23	8	15	CL	113.4	1817	11.0	61	.49						*		Γ
								CL	108.9	1745	16.4	81	.55								
· · ·	f							SM	108.1	1732	13.6	66	.56			[*	
-16	27	21				Ι		SM		[I		
	1				28	10	18	CL	105.1	1684	15.6	70	.60								
				Γ	56	27	29	СН	112.3	1799	13.5	73	.50						*	I	
97	43	7						SP-SM	108.0	1730	8.8	42	.56								
								SP-SM	100.3	1607	22.7	90	.68						T		L
50	3	3						SP	107.0	1714	18.8	88	. 57				Τ				
-4	50	48	3	3	50	21	29	SC	99.7	1597	19.7	77	.69						*		L
								SC	116.4	1865	15.0	91	. 45								L
								СН	81.8	1310	37.3	95	1.06								L
							I –	СН	84.9	1360	34.3	94	.99								L
								CH	103.5	1658	19.4	83	.63				I				L
					91	32	59	СН	98.6	1580	15.4	59	71						*	L	Ļ
								СН	92.8	1487	21.8	72	.82						_	1	t
								CH	85.5	1370	32.4	91	.97					L			Ļ
								SM	97.2	1557	25.7	95	.73					<u> </u>	1	↓	₽
								CH	94.7	1517	23.3	81	.78						- I		Ļ
L								SM	111.9	1793	15.8	84	.51		<u> </u>			_	1	┢	∔
								CH	102.8	1647	15.3	65	.64					L	+	_	Ļ
												Ι						_	_		∔
												_]	1			_	∔
									· · · · · ·	r		t	+			T			i	1	

.

_

~

. . .

SUMMARY OF LABORATI BORING JI JORNADA DEL MUERTO, NEW I WX SITING INVESTIG DEPARTMENT OF THE AIR FO TOGERO MATI

- ----

CONSOLIDATION

*

2

-

Î.

		1	N-SITU			C	ONPACTE	0					Z		
CS)	DRY		MOISTURE Content (%)	SATURATION (%)	V010 Ratio	MAX Dry de		OPTIMUM Moisture (%)	SPECIFIC Gravity Of Solids	TRIAXIAL	UNCONFINED Compression	DIRECT Shear	CONSOL IDATION	CHEMICAL	RELATIVE Dens: Ty
	(pc1)	(kg/m ³)	83	SAT	V0 RA	(pcf)	(kg/m ³)	L ON	SPE GRA	TRI	CONC	DIR	CONS	CHE	
											<u> </u>	_			
	101.3	1623	13.2	54	.66										
_	113.4	1817	11.0	61	.49						*				L
4	108.9 108.1	<u>1745</u> 1732	16.4 13.6	<u>81</u> 66	.55						 				
-	100.1	1/32	13.0	00			}	 			┟───	*			┨
	105.1	1684	15.6	70	.60		 -						*		<u> </u>
1	112.3	1799	13.5	73	.50						*				1
	108.0	1730	8.8	42	.56										
_	100.3	1607	22.7	90	.68									L	\downarrow
-	107.0 99.7	1714 1597	<u>18.8</u> 19.7	88 77	.57		┣────		┣───┤		+			 	┫
	$\frac{99.7}{116.4}$	1865	15.0	91	.45				┨────┤		*			 	t –
-	81.8	1310	37.3	95	1.06	·	t	<u> </u>	╂──┤		 			<u> </u>	t
	84.9	1360	34.3	94	.99		[1			1				
_	103.5	1658	19.4	83	.63										
-	98.6	1580	15.4	59	71		 	 	┟──┤		*	 	ļ	┨───	┣
-	92.8 85.5	<u>1487</u> 1370	21.8	72 91	.82 .97		}							 	↓
1	97.2	1557	25.7	95	.73		ł				╂───			<u>├</u>	+
1	94.7	1517	23.3	81	.78	··	<u></u>				<u>†</u>				
	111.9	1793	15.8	84	.51										
	102.8	1647	15.3	65	. 64										
				ļ				L			L	ļ		L	<u> </u>
							ļ							 	╂───
							ł	<u>↓</u>				 		<u> </u>	+
				<u>-</u>			┣───					<u> </u>	h		<u>† </u>
				<u> </u>			<u> </u>	t				t			
				_											
ļ								L			<u> </u>	ļ		ļ	_
							┞───	┟───				┨───	┣───	∤	╂──
							<u> </u>	<u> </u>	<u> </u>		+-		<u> </u>	<u>├</u> ──	+
l								+				<u>├</u> ──			
								1							
													[I
l							ļ	┢───		ļ	╂	┟	 	╂──	┨
ŀ							┫		 	┠───	+	 	<u>├</u> ──	 	+
						ļ	┣───	╂			+	<u>├</u> ──-	<u></u>	<u></u>	+
								<u> </u>	1		†			1	1
														+	

~

~

· -

- 「二」「「「」」」

| UTANCIER | 2.40 | 2.40 | 2.40 | 2.40 | 2.00 | 2.40 | 2.40

 | 2.09
 | 2.09 | 2.09
 | 2.09 | 2.09 | 2.09 | 1.74
 | 2.40 | 2.00 | 2.40 | 2.40 | 2.40 | 2.40 | 2.40 | 2.40 | 2.40 | 2.40
 | 2.40 | 2.40 | 2.40 | 2.40 | 2.40 |
|-------------------|--|---|---|---|--|---
--
--
---|---|--
--
---|---|--|---|---|---
---|--|--|---|--|---|---|--|---
---|---|---|--|---|
| (%) | 53.7 | 75.2 | 81.1 | 60.2 | 87.4 | 89.0 | 81.2

 | 64.0
 | 47.0 | 71.0
 | 92.8 | 89.1 | 49.3 | 41.8
 | 92.1 | 84.0 | 59.1 | 66.9 | 85.7 | 68.4 | 87.6 | 83.7 | 61.4 | 72.8
 | 11.1 | 58.6 | B1.8 | 85.6 | 50.9 |
| (%) | 16.0 | 13.8 | 15.6 | 12.8 | 24.3 | 14.5 | 19.3

 | 1.11
 | 10.7 | 14.1
 | 31.2 | 27.2 | 20.4 | 6,8
 | 25.2 | 15.1 | 9.4 | 14.3 | 22.3 | 16.0 | 24.0 | 19.1 | 11.0 | 13.5
 | 19.7 | 15.4 | 16.3 | 22.0 | 10.0 |
| kg∕m ³ | 1497 | 1809 | 1780 | 1715 | 1544 | 1878 | 1647

 | 1843
 | 1672 | 1762
 | 1417 | 1481 | 1276 | 1881
 | 1557 | 1818 | 1892 | 1714 | 1589 | 1656 | 1555 | 1672 | 1820 | 1802
 | 1600 | 1583 | 1757 | 1597 | 1769 |
| pcf | 93.3 | 112.7 | 110.9 | 106.9 | 96.2 | 117.0 | 102.6

 | 114.8
 | 104.2 | 109.8
 | 88.3 | 92.3 | 79.5 | 117.2
 | 97.0 | 113.3 | 117.9 | 106.8 | 99.0 | 103.2 | 6.96 | 104.2 | 113.4 | 112.3
 | 99.7 | <u>98.6</u> | 109.5 | 99.5 | 110.2 |
| kN ∕m2 | 105 | 116 | 97 | 111 | 20 | 115 | 51

 | 211
 | 235 | 138
 | 190 | 80 | 11 | 160
 | 385 | 166 | 528 | 108 | 350 | 155 | 787 | 517 | 132 | 167
 | 129 | 452 | 219 | 342 | 193 |
| kst | 2.2 | 2.4 | 2.0 | 2.3 | 0.4 | 2.4 |

 | 4.4
 | 4.9 | 2.9
 | 4.0 | 1.7 | 1.5 | 3.4
 | 0.0 | 16.0 | 11.0 | 2.3 | 1.3 | 3.2 | 16.4 | 10.8 | 2.8 | 3.5
 | 2.7 | 9.4 | 4.6 | 1.1 | 4.0 |
| 2 | ថ | 5 | HL. | 13 | CL | 10 | СГ

 | 5
 | C. | 5
 | Ð | CL | CL | SC
 | 10 | CL | 73 | 10 | ст
С | 10 | H | CL | CL | נד
(
 | £ | 5 | 5 | с, | Ŧ |
| METERS | 4.3-4.5 | 24.4-24.6 | 30.7~31.0 | 1.5-1.8 | 6.1-6.3 | 24.4-24.6 | 1.5-1.8

 | 1.6-7.9
 | 1.8-2.0 | 4.6-4.8
 | 15.2-15.5 | 24.4-24.6 | 1.5-1.8 | 6.4-6.7
 | 21.3-21.6 | 6.2-6.4 | 21.3-21.6 | 7.9-8.1 | 22.8-23.0 | 12.2-12.4 | 45.0-45.2 | 68.6-68.8 | 3.2-3.4 | 9.4-9.7
 | 21.7-21.9 | 45.9-46.1 | 21.3-21.5 | 15.2-15.5 | 45.7-48.0 |
| FEET | 14.0-14.8 | 80.0-80.8 | 100.8-101.6 | 5.0-5.8 | 20.0-20.8 | 80.0-80.8 | 5.0-6.0

 | 25.0-25.9
 | 5.8-6.4 | 15.0-15.8
 | 50.0-50.8 | 80.0-80.8 | 5.0-5.9 | 21.1-21.9
 | 70.0-70.8 | 20.5-21.0 | 70.0-70.8 | 25.8-26.5 | 74.8-75.5 | 40.0-40.7 | 147.5-148.2 | 225.0-225.7 | 10.5-11.2 | 31.0-31.7
 | 71.1-71.8 | 150.5-151.2 | 10.0-10.1 | 50.0-50.8 | 150.0-150.9 |
| лU. | P-3 | | P-13 | P-1 | P-4 | P-8 | 1-d

 | P5
 | 1-d | P-3
 | 14 | P-10 |
- | P-4
 | P-10 | D5 | P-10 | P-5 | P-10 | P-7 | P-15 | P-18 | P-2 | 1-4
 | 11-4 | P-16 | P-10 | P-8 | P-14 |
| M | 1-8-N | | | JMB2 | | | 14-8-3

 |
 | JM-8-4 |
 | | | 1M-B-5 | JM-8-6
 | | JM-B-7 | JM-8-8 | JM89 | | JM-8-10 | | | 11-8-11 |
 | | | JM-8-12 | 1M-8-14 | JM-8-15 |
| | | | | | | |

 |
 | |
 | | | |
 | | ORN | | DE | 1 | NEF | TE
RTD | ST
, N | RES
EW | ULT
Nex
 | | | | | |
| | NU. FEET METERS ITTE kst kN/m ² pct kg/m ³ (%) | NU. FEET METERS ITPL ksi kN/m2 pct kg/m3 (%) -1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 | NU. FEET METERS 11 PL ks1 kN/m2 pct kg/m3 (%) 1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 | NU. FEET METERS 17PL ks1 kN/m2 pcf kg/m3 (%) 1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 | NU. FEET METERS 17FL ks1 kN/m2 pc1 kg/m3 (%) 1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 2 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 15.6 | NU. FEET METERS 11 PL ks1 kN/m2 pcf kg/m3 (%) 1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 2 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 2 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 2 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 | NU. FEET METERS 11 Ve. ks1 kN/m2 pcf kg/m3 (%) 1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 1780 15.6 P-13 100.8 1.51.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 2.4 24.3 24.3 P-8 80.0-80.8 <t< td=""><td>NU. FEET METERS 11 Ve. ks1 kN/m2 pcf kg/m3 (%) P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 10 24.3 P-4 20.0-80.0 1.54 24.4 24.3 P-4 20.0-80.0 1.51 117.0 1876<</td><td>NU. FEET METERS 11 Ve ks1 kN/m2 pcf kg/m3 (%) 1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 20 96.2 1544 24.3 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1876 14.5 P-4 80.0-0-80.8 24.4-24.6 CL 2.4 115 117.0 1878 14.5 <</td><td>NU. FEET METERS 11 Pc ks1 km/m² pcf kg/m³ (%) P-13 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 180.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 12 14.5 14.5 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1878 14.5 P-4 20.0-20.8 6.1-6.3 7.4 2.4 24.3 24.3 P-1 5.0-6.0 1.5.1 115 117.0<!--</td--><td>NU. FEET METERS 11 Pc ks1 ku/m2 pcf kg/m3 (%) P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1878 14.5 P-8 80.0-80.8 2.4-24.6 CL 2.4 115 117.0 1878 14.5 P-9 80.0-80.8 2.4-24.6 CL 2.4 115 117.0 1878 14.5 P-9 5.0-</td><td>NU. FEET METERS 11 Pc ks1 ku/m2 pcf kg/m3 (%) P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 180.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1878 14.5 P-8 80.0-80.8 24.4-24.6 CL 2.4 115 117.0 1878 14.5 P-8 80.0-080.8 24.4-24.6 CL 10.4 20 96.2 14.5 24.3 P-9</td><td>MU. FEET METERS 17Pc ks1 kM/m² pcf kg/m³ (%) P-11 80.0-80.8 24.4-24.6 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.2 105 97.3 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9
 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 117.0 1909 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 117.0 190 15.6 P-13 100.8-101.6 20.7-31.0 ML 2.0 96.2 15.4 24.3 P-4 20.0-5.8 1.5-1.8 CL 2.4 115 117.0 1878 14.5 P-5 20.0-5.9 7.6-1.9 CL 2.4 115 110.2 16.7 19.3 P-5 25.0-25.9</td><td>MU. FEET METERS 11 Pc ks1 km/m2 pcf kg/m3 (%) P-13 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1818 14.5 P-4 20.0-20.8 5.1-6.0 1.5-1.8 CL 2.4 24 24.3 P-5 25.0-25.9 7.6 1.1 51 102.6 1677 10.7 P-5 25.0-25.9 7.6 1.4.4</td><td>NU. FEET METERS ITCL ks1 ku/m2 pcf kg/m3 (%) P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1781 14.5 P-4 20.0-20.8 6.1-6.3 CL 2.4 24 24.3 P-4 20.0-50.9 1.5-1.8 CL 2.4 115 112.0 1843 11.1 P-1 5.0-6.0 1.5-1.8 CL 2.4 24 24.3 24.3 P-1 5.0-6.0 1.5-1.1 114.6 14.5</td><td>NU. FEET METERS 17PL ks1 MM^{-2} pcf kg/m^3 (s) P-11 80.0-80.8 24.4-24.6 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1760 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 105.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.3 111 105.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1878 14.5 M-8-3 P-1 5.0-6.0 1.5-1.8 CL 2.4 24.3 24.3 M-8-4 P-1 5.0-5.9 1.6-7.1.8 CL 2.4 21.3 10.1 10.7 10.7 10.7 10.7</td><td>NU. FEET METERS 17PL ks1 $kN/m2$ peri $kg/m3$ (%) IM-B-1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 B0.0-B0.8 24.4-24.6 CL 2.4 116 112.7 1009 13.6 P-13 190.8-101.6 30.7-31.0 ML 2.0 97 110.9 1716 12.8 P-13 190.8-101.6 30.7-31.0 ML 2.0 97 113.9 180 15.6 P-13 190.8-101.6 30.7-31.0 ML 2.0 97 119.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 0.4 20 96.2 154 24.3 P-4 20.0-50.8 5.1-6.3 7.6 0.4 20 96.2 164 14.5 IM-B-4 P-1 5.0-50.8 1.6-7.9 CL 2.4 24.1 24.4 24.3 111.10</td><td>NU. FEET NETERS 17% ks1 MM.m2 perf kg/m3 (%) IM-B-1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 96.2 15.4 24.3 P-4 20-5.0 1.5-1.8 CL 2.4 117.0 1878 14.5 M-8-3 P-1 5.0-5.0 1.5-1.8 CL 2.4 24.4 24.3 24.3 M-8-4 P-1 5.0-5.0 1.5-1.8 CL 2.4 117.0 1878 10.1 M-8-4 P-1 5.</td><td>MU. FECT METERS 17% ksi km/m2 pcf kg/m3 (%) P-11 B0.0-B0.B $244-24.6$ CL 2.2 105 93.3 1497 16.0 P-13 100.B-101.6 $30.7-31.0$ ML 2.0 97 110.9 1780 13.8 P-13 100.B-101.6 $30.7-31.0$ ML 2.0 97 110.9 1780 13.8 P-13 100.B-101.6 $30.7-31.0$ ML 2.0 97 110.9 1780 13.8 P-13 100.B-101.6 $30.7-31.0$ ML 2.0 97 119.9 171.0 1970 12.9 P-13 P00.B-101.6 $30.7-31.0$ NL 2.0 $91.42.1$ $24.24.5$ CL 2.3 141.7 14.3 14.5 14.5 14.5 14.5 11.1 11.1 11.1 11.1 $11.2.7$ 180.9 12.4 24.3 $24.24.5$ CL 2.4 <</td><td>MU. FEET METERS 1172 kai km/m2 per kg/m3 (%) 1H-B-1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 B00.0-B0.8 24.4-24.6 CL 2.4 110 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 111.0 1810 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 111.0 1810 14.5 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 111.0 1810 12.8 P-4 20.0-20.8 5.1-6.1 CL 2.3 111.1</td><td>MOL. FEET NETERS 17PL Kai Kanizz pot kenizz (%) IM-B-1 P-3 14.0-14.6 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 110 112.7 1009 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 176 12.8 P-13 70.0-80.8 24.4-24.6 CL 2.0 11 106.9 13.8 P-4 9.0<0-80.8</td> 24.4-24.6 CL 2.3 111.1 117.0 1093 13.8 P-4 9.1 5.0-6.0 1.5-1.8 CL 2.3 103.7 1073 103 14.5 13.3 MM-B-3 P-1 5.0-6.0 1.5-1.8 CL 2.3 103 103 11.1 11.1 MM-B-3 P-1 5.0-5.0 1.5-1.8 CL 2.4 2.1 13.2 11</td><td>MU. RECT NETERS ITE ks1 km/m2 poil kg/m3 (4) IM-B-1 P-3 14.0-14.6 4.3-4.5 CL 2.2 105 39.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML
 2.0 97 110.9 1706 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1716 12.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 16.0 13.8 P-14 50.0-50.8 1.5-1.8 CL 2.3 101 188 12.3 P-14 50.0-50.8 1.5-1.8 CL 1.4 211 108.2 14.5 P-2 50.0-50.8 1.5-1.9 CL 1.4 211 104.2 16.0 11.1 ML-B-3 P-1 50.0-50.8 1.5-1.9 CL 1.4 211.2 161.7 161.7 161.7 161.7 171.7 161.7</td><td>MU: FEET WETERS 1172 Au Au FET WETERS 1172 Au Au</td><td>MU. MU. FEET WETERS 11% Kai Munic peri Kai Munic peri Kai Munic feet Kai Kai</td><td>MUL. MUL. FEET WETERS 11PL A.1 D. C. 2.0 19.0 10.0 13.</td><td>Mol. FEET WETRAS ITVE kei Mol.2 per 110.0 130.1 100.1 130.1 100.1 130.1</td><td>Mo. FECT WETRS IPPE Let Ref Ref Ref Let Ref Ref Let Ref Let Ref Let Ref Let Let Let Let Let Let Let Let</td><td>MUL MUL FEET WETRS IPE MURAC Def MURAC Def MURAC Def MURAC Def MURAC Def MULAC MULAC MULAC Def MULAC MULAC</td><td>M0. FEET WETERS 11% kei ww.az pol her kei ww.az pol her kei <th< td=""></th<></td></t<> | NU. FEET METERS 11 Ve. ks1 kN/m2 pcf kg/m3 (%) P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 10 24.3 P-4 20.0-80.0 1.54 24.4 24.3 P-4 20.0-80.0 1.51 117.0 1876< | NU. FEET METERS 11 Ve ks1 kN/m2 pcf kg/m3 (%) 1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 20 96.2 1544 24.3 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1876 14.5 P-4 80.0-0-80.8 24.4-24.6 CL 2.4 115 117.0 1878 14.5 < | NU. FEET METERS 11 Pc ks1 km/m² pcf kg/m³ (%) P-13 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 180.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 12 14.5 14.5 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1878 14.5 P-4 20.0-20.8 6.1-6.3 7.4 2.4 24.3 24.3 P-1 5.0-6.0 1.5.1 115 117.0 </td <td>NU. FEET METERS 11 Pc ks1 ku/m2 pcf kg/m3 (%) P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1878 14.5 P-8 80.0-80.8 2.4-24.6 CL 2.4 115 117.0 1878 14.5 P-9 80.0-80.8 2.4-24.6 CL 2.4 115 117.0 1878 14.5 P-9 5.0-</td> <td>NU. FEET METERS 11 Pc ks1 ku/m2 pcf kg/m3 (%) P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 180.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1878 14.5 P-8 80.0-80.8 24.4-24.6 CL 2.4 115 117.0 1878 14.5 P-8 80.0-080.8 24.4-24.6 CL 10.4 20
96.2 14.5 24.3 P-9</td> <td>MU. FEET METERS 17Pc ks1 kM/m² pcf kg/m³ (%) P-11 80.0-80.8 24.4-24.6 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.2 105 97.3 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 117.0 1909 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 117.0 190 15.6 P-13 100.8-101.6 20.7-31.0 ML 2.0 96.2 15.4 24.3 P-4 20.0-5.8 1.5-1.8 CL 2.4 115 117.0 1878 14.5 P-5 20.0-5.9 7.6-1.9 CL 2.4 115 110.2 16.7 19.3 P-5 25.0-25.9</td> <td>MU. FEET METERS 11 Pc ks1 km/m2 pcf kg/m3 (%) P-13 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1818 14.5 P-4 20.0-20.8 5.1-6.0 1.5-1.8 CL 2.4 24 24.3 P-5 25.0-25.9 7.6 1.1 51 102.6 1677 10.7 P-5 25.0-25.9 7.6 1.4.4</td> <td>NU. FEET METERS ITCL ks1 ku/m2 pcf kg/m3 (%) P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1781 14.5 P-4 20.0-20.8 6.1-6.3 CL 2.4 24 24.3 P-4 20.0-50.9 1.5-1.8 CL 2.4 115 112.0 1843 11.1 P-1 5.0-6.0 1.5-1.8 CL 2.4 24 24.3 24.3 P-1 5.0-6.0 1.5-1.1 114.6 14.5</td> <td>NU. FEET METERS 17PL ks1 MM^{-2} pcf kg/m^3 (s) P-11 80.0-80.8 24.4-24.6 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1760 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 105.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.3 111 105.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1878 14.5 M-8-3 P-1 5.0-6.0 1.5-1.8 CL 2.4 24.3 24.3 M-8-4 P-1 5.0-5.9 1.6-7.1.8 CL 2.4 21.3 10.1 10.7 10.7 10.7 10.7</td> <td>NU. FEET METERS 17PL ks1 $kN/m2$ peri $kg/m3$ (%) IM-B-1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 B0.0-B0.8 24.4-24.6 CL 2.4 116 112.7 1009 13.6 P-13 190.8-101.6 30.7-31.0 ML 2.0 97 110.9 1716 12.8 P-13 190.8-101.6 30.7-31.0 ML 2.0 97 113.9 180 15.6 P-13 190.8-101.6 30.7-31.0 ML 2.0 97 119.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 0.4 20 96.2 154 24.3 P-4 20.0-50.8 5.1-6.3 7.6 0.4 20 96.2 164 14.5 IM-B-4 P-1 5.0-50.8 1.6-7.9 CL 2.4 24.1 24.4 24.3 111.10</td> <td>NU. FEET NETERS 17% ks1 MM.m2 perf kg/m3 (%) IM-B-1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 96.2 15.4 24.3 P-4 20-5.0 1.5-1.8 CL 2.4 117.0 1878 14.5 M-8-3 P-1 5.0-5.0 1.5-1.8 CL 2.4 24.4 24.3 24.3 M-8-4 P-1 5.0-5.0 1.5-1.8 CL 2.4 117.0 1878 10.1 M-8-4 P-1 5.</td> <td>MU. FECT METERS 17% ksi km/m2 pcf kg/m3 (%) P-11 B0.0-B0.B $244-24.6$ CL 2.2 105 93.3 1497 16.0 P-13 100.B-101.6 $30.7-31.0$ ML 2.0 97 110.9 1780 13.8 P-13 100.B-101.6 $30.7-31.0$ ML 2.0 97 110.9 1780 13.8 P-13 100.B-101.6 $30.7-31.0$ ML 2.0 97 110.9 1780 13.8 P-13 100.B-101.6 $30.7-31.0$ ML 2.0 97 119.9 171.0 1970 12.9 P-13 P00.B-101.6 $30.7-31.0$ NL 2.0 $91.42.1$ $24.24.5$ CL 2.3 141.7 14.3 14.5 14.5 14.5 14.5 11.1 11.1 11.1 11.1 $11.2.7$ 180.9 12.4 24.3 $24.24.5$ CL 2.4 <</td> <td>MU. FEET METERS 1172 kai km/m2 per kg/m3 (%) 1H-B-1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 B00.0-B0.8 24.4-24.6 CL 2.4 110 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 111.0 1810 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 111.0 1810 14.5 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 111.0 1810 12.8 P-4 20.0-20.8 5.1-6.1 CL 2.3 111.1</td> <td>MOL. FEET NETERS 17PL Kai Kanizz pot kenizz (%) IM-B-1 P-3 14.0-14.6 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 110 112.7 1009 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 176 12.8 P-13 70.0-80.8 24.4-24.6 CL 2.0 11 106.9 13.8 P-4 9.0<0-80.8</td> 24.4-24.6 CL 2.3 111.1 117.0 1093 13.8 P-4 9.1 5.0-6.0 1.5-1.8 CL 2.3 103.7 1073 103 14.5 13.3 MM-B-3 P-1 5.0-6.0
1.5-1.8 CL 2.3 103 103 11.1 11.1 MM-B-3 P-1 5.0-5.0 1.5-1.8 CL 2.4 2.1 13.2 11 | NU. FEET METERS 11 Pc ks1 ku/m2 pcf kg/m3 (%) P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1878 14.5 P-8 80.0-80.8 2.4-24.6 CL 2.4 115 117.0 1878 14.5 P-9 80.0-80.8 2.4-24.6 CL 2.4 115 117.0 1878 14.5 P-9 5.0- | NU. FEET METERS 11 Pc ks1 ku/m2 pcf kg/m3 (%) P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 180.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1878 14.5 P-8 80.0-80.8 24.4-24.6 CL 2.4 115 117.0 1878 14.5 P-8 80.0-080.8 24.4-24.6 CL 10.4 20 96.2 14.5 24.3 P-9 | MU. FEET METERS 17Pc ks1 kM/m² pcf kg/m³ (%) P-11 80.0-80.8 24.4-24.6 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.2 105 97.3 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 117.0 1909 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 117.0 190 15.6 P-13 100.8-101.6 20.7-31.0 ML 2.0 96.2 15.4 24.3 P-4 20.0-5.8 1.5-1.8 CL 2.4 115 117.0 1878 14.5 P-5 20.0-5.9 7.6-1.9 CL 2.4 115 110.2 16.7 19.3 P-5 25.0-25.9 | MU. FEET METERS 11 Pc ks1 km/m2 pcf kg/m3 (%) P-13 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1818 14.5 P-4 20.0-20.8 5.1-6.0 1.5-1.8 CL 2.4 24 24.3 P-5 25.0-25.9 7.6 1.1 51 102.6 1677 10.7 P-5 25.0-25.9 7.6 1.4.4 | NU. FEET METERS ITCL ks1 ku/m2 pcf kg/m3 (%) P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 106.9 1781 14.5 P-4 20.0-20.8 6.1-6.3 CL 2.4 24 24.3 P-4 20.0-50.9 1.5-1.8 CL 2.4 115 112.0 1843 11.1 P-1 5.0-6.0 1.5-1.8 CL 2.4 24 24.3 24.3 P-1 5.0-6.0 1.5-1.1 114.6 14.5 | NU. FEET METERS 17PL ks1 MM^{-2} pcf kg/m^3 (s) P-11 80.0-80.8 24.4-24.6 CL 2.2 105 93.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1760 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-1 5.0-5.8 1.5-1.8 CL 2.3 111 105.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.3 111 105.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 2.4 115 117.0 1878 14.5 M-8-3 P-1 5.0-6.0 1.5-1.8 CL 2.4 24.3 24.3 M-8-4 P-1 5.0-5.9 1.6-7.1.8 CL 2.4 21.3 10.1 10.7 10.7 10.7 10.7 | NU. FEET METERS 17PL ks1 $kN/m2$ peri $kg/m3$ (%) IM-B-1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 B0.0-B0.8 24.4-24.6 CL 2.4 116 112.7 1009 13.6 P-13 190.8-101.6 30.7-31.0 ML 2.0 97 110.9 1716 12.8 P-13 190.8-101.6 30.7-31.0 ML 2.0 97 113.9 180 15.6 P-13 190.8-101.6 30.7-31.0 ML 2.0 97 119.9 1716 12.8 P-4 20.0-20.8 6.1-6.3 CL 0.4 20 96.2 154 24.3 P-4 20.0-50.8 5.1-6.3 7.6 0.4 20 96.2 164 14.5 IM-B-4 P-1 5.0-50.8 1.6-7.9 CL 2.4 24.1 24.4 24.3 111.10 | NU. FEET NETERS 17% ks1 MM.m2 perf kg/m3 (%) IM-B-1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 116 112.7 1809 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1780 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 96.2 15.4 24.3 P-4 20-5.0 1.5-1.8 CL 2.4 117.0 1878 14.5 M-8-3 P-1 5.0-5.0 1.5-1.8 CL 2.4 24.4 24.3 24.3 M-8-4 P-1 5.0-5.0 1.5-1.8 CL 2.4 117.0 1878 10.1 M-8-4 P-1 5. | MU. FECT METERS 17% ksi km/m2 pcf kg/m3 (%) P-11 B0.0-B0.B $244-24.6$ CL 2.2 105 93.3 1497 16.0 P-13 100.B-101.6 $30.7-31.0$ ML 2.0 97 110.9 1780 13.8 P-13 100.B-101.6 $30.7-31.0$ ML 2.0 97 110.9 1780 13.8 P-13 100.B-101.6 $30.7-31.0$ ML 2.0 97 110.9 1780 13.8 P-13 100.B-101.6 $30.7-31.0$ ML 2.0 97 119.9 171.0 1970 12.9 P-13 P00.B-101.6 $30.7-31.0$ NL 2.0 $91.42.1$ $24.24.5$ CL 2.3 141.7 14.3 14.5 14.5 14.5 14.5 11.1 11.1 11.1 11.1 $11.2.7$ 180.9 12.4 24.3 $24.24.5$ CL 2.4 < | MU. FEET METERS
 1172 kai km/m2 per kg/m3 (%) 1H-B-1 P-3 14.0-14.8 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 B00.0-B0.8 24.4-24.6 CL 2.4 110 110.9 1780 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 111.0 1810 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 111.0 1810 14.5 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 111.0 1810 12.8 P-4 20.0-20.8 5.1-6.1 CL 2.3 111.1 | MOL. FEET NETERS 17PL Kai Kanizz pot kenizz (%) IM-B-1 P-3 14.0-14.6 4.3-4.5 CL 2.2 105 93.3 1497 16.0 P-11 80.0-80.8 24.4-24.6 CL 2.4 110 112.7 1009 13.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 176 12.8 P-13 70.0-80.8 24.4-24.6 CL 2.0 11 106.9 13.8 P-4 9.0<0-80.8 | MU. RECT NETERS ITE ks1 km/m2 poil kg/m3 (4) IM-B-1 P-3 14.0-14.6 4.3-4.5 CL 2.2 105 39.3 1497 16.0 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1706 15.6 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 110.9 1716 12.8 P-13 100.8-101.6 30.7-31.0 ML 2.0 97 16.0 13.8 P-14 50.0-50.8 1.5-1.8 CL 2.3 101 188 12.3 P-14 50.0-50.8 1.5-1.8 CL 1.4 211 108.2 14.5 P-2 50.0-50.8 1.5-1.9 CL 1.4 211 104.2 16.0 11.1 ML-B-3 P-1 50.0-50.8 1.5-1.9 CL 1.4 211.2 161.7 161.7 161.7 161.7 171.7 161.7 | MU: FEET WETERS 1172 Au Au FET WETERS 1172 Au Au | MU. MU. FEET WETERS 11% Kai Munic peri Kai Munic peri Kai Munic feet Kai Kai | MUL. MUL. FEET WETERS 11PL A.1 D. C. 2.0 19.0 10.0 13. | Mol. FEET WETRAS ITVE kei Mol.2 per 110.0 130.1 100.1 130.1 100.1 130.1 | Mo. FECT WETRS IPPE Let Ref Ref Ref Let Ref Ref Let Ref Let Ref Let Ref Let Let Let Let Let Let Let Let | MUL MUL FEET WETRS IPE MURAC Def MURAC Def MURAC Def MURAC Def MURAC Def MULAC MULAC MULAC Def MULAC MULAC | M0. FEET WETERS 11% kei ww.az pol her kei ww.az pol her kei kei <th< td=""></th<> |

≁.

.....

۰.

Si F

. . .

BACK PRESSURE	f kN/m ²	0.0	.0 0.0	0.0	0 0.0	0.0	0 0.0	0 0.0	0.0	0 0.0	0 0.0	0 0.0	0 0.0	0 0.0	0 0.0	0.0				-			
) ksf	ē		0	0	ē	6	0	0.	<u>.</u>	0		ė	0	<u>.</u>	6							
STRAIN RATE	\mathbb{C}	0.03	0.03	0.03	0.9	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1							
NAX INUN DEVIATOR (CO-O))	kN/m ²	517	800	1447	1044	259	565	469	249	1001	738	891	666	2299	580	829			 				
5	ks f	10.8	16.7	30.2	21.8	5.4	11.8	9.8	5.2	20.9	15.4	18.6	13.9	34.1	12.1	17.3							
N I NG Re (0 ₃)	кN/m ²	240	378	584	378	48	240	240	48	288	144	378	115	412	378	723							
CONFINING PRESSURE (03)	ksí	5.0	7.9	12.2	7.9	1.0	5.0	5.0	1.0	B.0	3.0	1.9	2.4	8.6	7.9	15.1							
MO I STURE	(")	14.6	14.6	14.6	7.8	11.6	13.7	13.7	5.7	6.5	11.2	10.5	8.9	9.6	20.3	14.7							
DENSITY	k@/m ³	1568	1568	1568	1618	1735	1685	1599	1759	1721	1946	1970	1674	1934	1681	1812							
DRY DEI	pc f	97.7	97.7	97.7	100.8	108.1	105.0	99.6	109.6	107.2	121.2	122.7	104.3	120.5	104.7	112.9							
TYPE	TEST	CD	c0	CD	3	3	8	ß	9	60	3	ng	00	CD	CD CD	9							
=	TYPE	13	٦J	CL	SM	CL CL	СL	CL	SP	SP	СГ	CL	SM	SH	CL CL	CL							
NTERVAL	METERS	15.2-15.5	15.2-15.5	15.2-15.5	15.5-15.7	3.0-3.3	4.6-4.8	4.8-5.1	4.6-4.8	4.8-5.1	8.5-8.7	8.7-8.9	7.8-7.9	7.9-8.1	24.6-24.8	24.8-25.1							
SAMPLE	FEET	50.0-50.9	50.0-50.9	50.0-50.9	50.9-51.4	10.0-10.8	15.0-15.8	15.8-16.6	15.0-15.8	15.8-16.6	27.8-28.6	28.6-29.3	25.0-25.8	25.8-26.8	80.8-81.5	81.5-82.5							102
SAMPLE	NO.	P-8	9-8	P-8	8-4	P-2	P-3	P-3	6-4	P-3	P-6	P-6	P-5	P-5	P-12	P-12							2 1 a K a
NG	.0N	1 ₩ -8-1*				JM-8-3			JM-8-6		JM-8-7		JM-8-8		11-8-W1							* Multi-etace teet	
													SUMN	101	RN AI	DA (EL 10	MUI GR/	AND	0, 1 <u>e c</u> :	R TE New Sp		
													_	ARTM	ENT		HE A	IR F	ORCI	E .	SANS		[

۰.

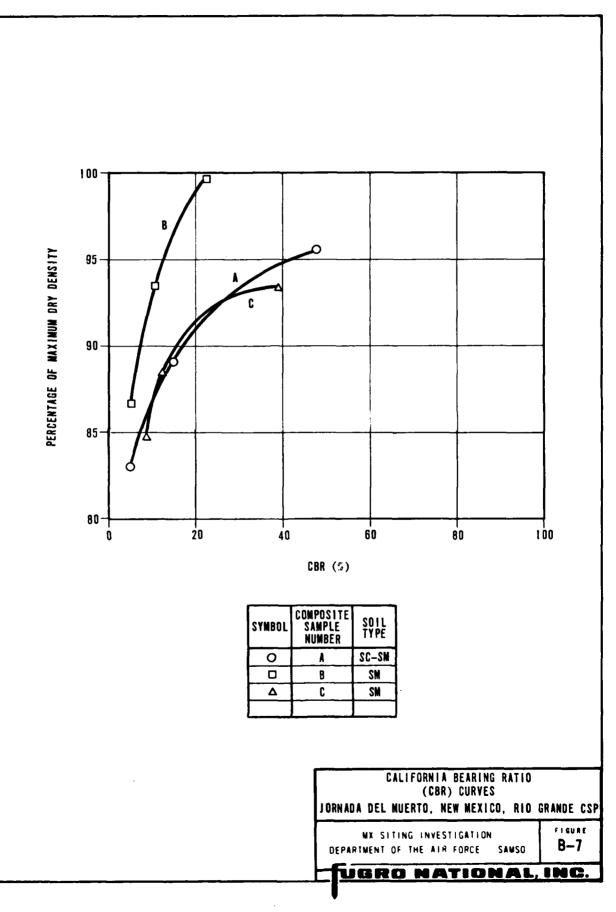
·• ··

BORING	SAMPLE	SAMPLE I	NTERVAL	SOIL	NORMAL	STRESS	MAX Shear	INUN Strength
NO.	NO.	FEET	METERS	ТҮРЕ	ksf	kN/m ²	ksf	kN/m 2
JM-B-2	P-3	15.0-15.8	4.57-4.82	CL	6.0	290	3.7	177
	P-3	15.0-15.8	4.57-4.82	CI	1.0	48	1.2	56
J M8 5	P-5	32.3-33.0	9.85-10.06	SC	8.0	386	5.1	246
	P-5	32.3-33.0	9.85–10. ਹਰੇ	SC	3.0	145	2.5	118
	P-9	73.0-73.7	22.25-22.46	ML	10.9	524	7.7	368
	P-9	73.0-73.7	22.25-22.46	ML	7.1	338	4.8	228
JM-8 -6	P-8	50.0-50.8	15.2-15.5	SM	10.0	479	6.5	312
	P-8	50.0-50.8	15.2-15.5	SM	5.0	241	4.0	191
J M- B-7	P-7	40.0-40.5	12.2-12.3	SM	12.2	586	11.4	548
	P-7	40.0-40.5	12.2-12.3	SM	4.0	193	7.1	339
JM-8-8	P-3	15.0-15.5	4.6-4.7	SM	7.9	379	5.4	261
	P-3	15.0-15.5	4.6-4.7	SM	1.0	48	0.7	34
JM-8-9	P-1	5.0-5.8	1.5-1.8	ML	6.5	310	5.0	241
	P-1	5.0-5.8	1.5-1.8	ML	1.0	48	1.3	61
JM-B-10	P-1	5.0-5.7	1.5-1.7	ML	6.5	310	5.4	261
	P-1	5.0-5.7	1.5-1.7	NL	i.0	48	2.2	105
	P-4	20.0-20.7	6.1-6.3	SM	7.2	345	4.8	228
	P-4	20.0-20.7	6.1-6.3	SM	2.2	103	1.8	872
J M- 8-11	P-4	20.7-21.4	6.3-6.5	ML	5.8	276	6.3	302
	P-4	20.7-21.4	6.3-6.5	ML	2.0	97	3.4	165
JM-B- 12	P-5	25.0-25.7	7.6-7.8	SP-SM	7.9	379	5.8	275
	P-5	25.0-25.7	7.6-7.8	SP-SM	2.2	103	1.7	81
JMB13	P-1	5.0-5.7	1.5-1.7	SC	5.8	276	4.5	217
	P-1	5.0-5.7	1.5-1.7	SC	1.0	48	1.3	632
JM-8-14	P-1	5.0-5.7	1.5-1.7	SM	5.8	276	4.0	193
	P-1	5.0-5.7	1.5-1.7	SM	1.0	48	1.0	48
IN-B-15	P-3	17.0-17.7	5.2-5.4	SP-SM	7.9	379	5.4	260
	P-3	17.0-17.7	5.2-5.4	SP-SM	2.2	103	1.6	335
JM-8-17	P-1	5.0-5.7	1.5-1.7	ML	5.8	276	4.6	221
	P-1	5.0-5.7	1.5-1.7	ML	1.0	48	2.2	104
				<u> </u>	JORNAI MX SI	DF DIREC DA DEL N RIO C TING INVE OF THE ALL	IUERTD, <u>GRANDE</u> C Stigation	NEW MEX SP
						O NA		
			<u> </u>	╾╼┺╼╼╏				

....

. CBR (\$)	43.7	17.5	4.9	22.3	11.0	5.3			39.1	12.7	8.6							
PERCENT OF Maximum Dry density	95.6	89.1	83.1	<u>99,6</u>		86.7			93.3	88.6	84.8							
COMPACTED NOISTURE (%)	5.0	4.7	4.7	8.2		1.4		- 1	7.2	6.5	7.6							
COMPACTED DRY DENSITY pcf kg/m ³	2074	1933	1804	1930	1812	1680			1873	1778	1701							
1 1	129.5	120.7	112.8	120.5	113.1	104.9			116.9	111.0	106.2	ļ						
OPTIMUM Moisture (%)			0.0			9.5	L			4	8.5					•		
MAXIMUM DRY DENSITY pcf kg/m ³			2171			1938					2007							
MAX DRY DI Pcf			135.5			121.0					125.3							
SPECIFIC GRAVITY			2.7			2.8					2.65						:	
ITERBERG Limits L Pi			œ			* d N					*dN							
ATTERBERG LIMITS LL PI			24															
PERCENT Passing #200			30			34					14							NSTIC
S01L TYPE			SC-SM			SM					SM							NOT PLASTIC
COMPOSITE Sample Number			•								ట							4 N *
									IOR	NAC		MU	(C Ert	BR) '0,	TE NE	EST W M	RES	IG RATIO SULTS CO. RIO GRANDE C
									DI	EPA							TIO Ce	- SANSO B-5

₿.

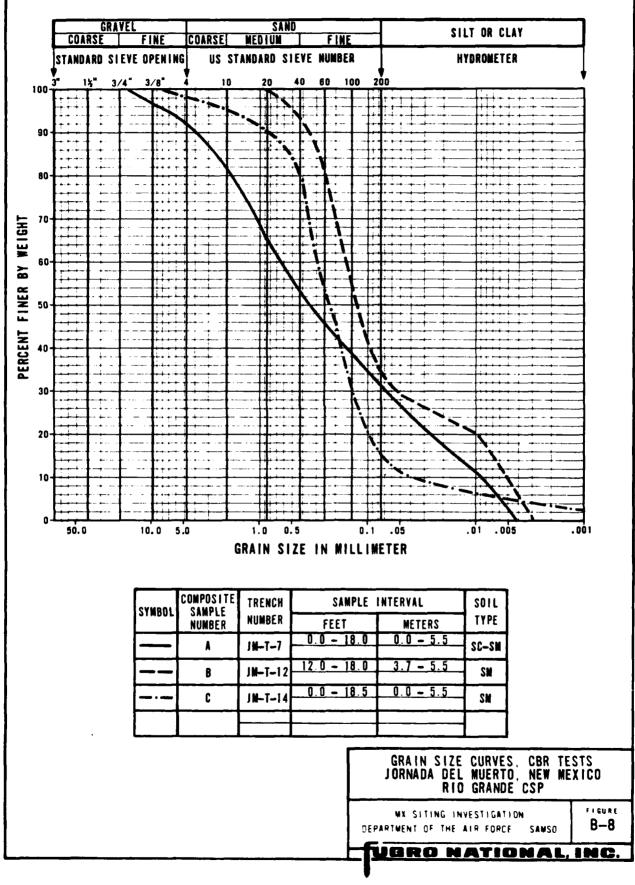


• •

大学の



-••



N-26C

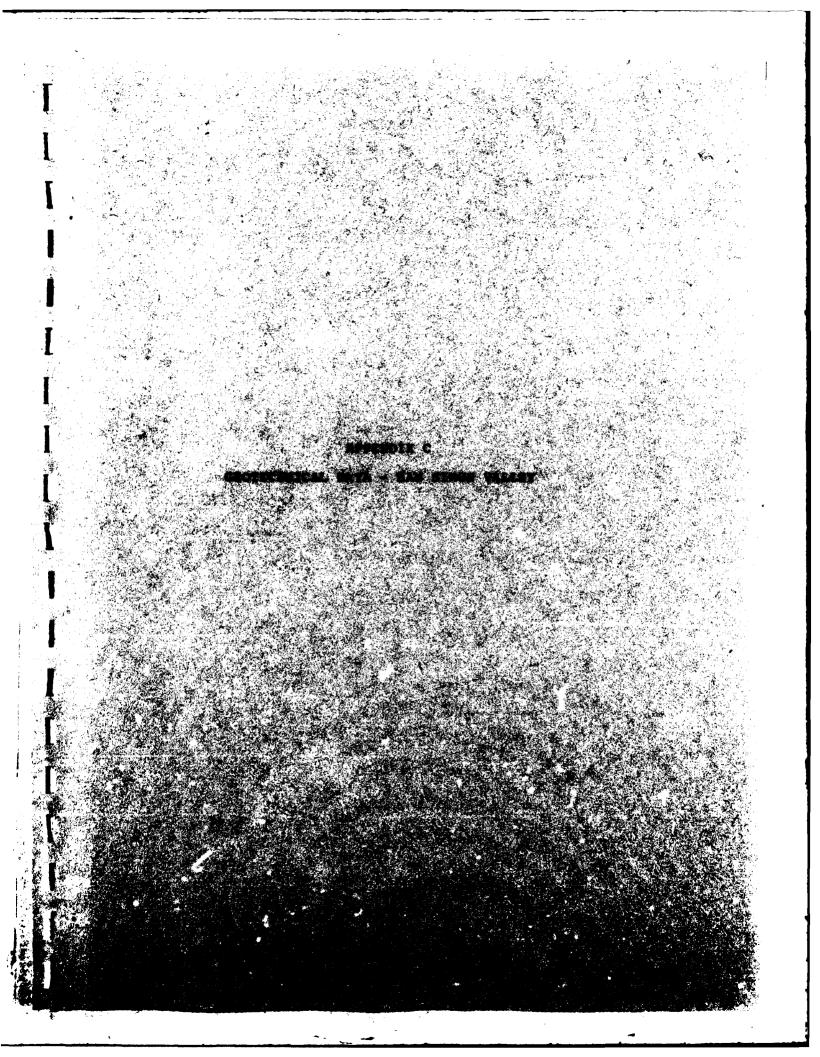


TABLE OF CONTENTS APPENDIX C

BORING AND TRENCH LOGS

LOG OF BORING SS-B-1	Figure C-l
LOG OF BORING SS-B-4	Figure C-2
LOG OF BORING SS-B-8	Figure C-3
LOG OF TRENCH SS-T-1	Figure C-4
LOG OF TRENCH SS-T-3	Figure C-5
LOG OF TRENCH SS-T-5	Figure C-6
SUMMARY OF LABORATORY TEST RESULTS	

BORING SS-B-1

SUMMARY OF SHEAR STRENGTH TESTS

UNCONFINED COMPRESSION TEST RESULTS	Table C-2
TRIAXIAL SHEAR TEST RESULTS	Table C-3
DIRECT SHEAR TEST RESULTS	Table C-4

Table C-1

SUMMARY OF CALIFORNIA BEARING RATIO (CBR) TESTS

CALIFORNIA BEARIN	G RATIO (CBR)	TEST RESULTS	Table C-5
CALIFORNIA BEARIN	G RATIO (CBR)	CURVES	Figure C-7
GRAIN SIZE CURVES	, CBR TESTS		Figure C-8

TUBRO NATIONAL ING

-

	[a		4			22		21										48				_						46			
_		L					5		43		_								17										72			
ĺ	SIS	Ē			_		95		66	<u> </u>	·								61										66			
	SI EVE ANALYSIS	SA					<u>ی</u>												<u>~</u>						_							
	A.	GR		1	<u> </u>	_	-					,		-					T		10		1						õ			
	● (pcf) 90 100 110 120 130 140	25 30 35)									43		46								46			
	0 100 110	15 20 ●(%)				•		1			-														+			_				
	6 T	5 10					Γ					•			(Ι			_	◀									
	<u> </u>		•											stift					•										A			
			I SILTY SAND, light brown, fine to coarse noorly oraded dense angular	\langle		4	2		Citt Constant and Constant and Constant and Constant	\leq	Medium, poorly graded, dense, angular		Vplasticity clay.	SILTY CLAY, green and red brown.		trace of fine sand.	CLAY, green, stift to very stiff.	Medium plasticity, calcareous, frace A of silt and sand (26.5-29° and 38-	41.5°), layers of fine to medium	Vilty sand, medium dense (24-26.5")		SiLTY CLAY, gray green, stift, medium plasticity, calcareous, trace of fine	sand.									
	SOSI	ן	3	30		20	53	9	$\overline{\mathbf{x}}$] נו		N		5		\downarrow						·····		1910 - 2 -11			5			- C.J.	,	
19	010HT				0.0	2	ار ب ر) ,,,					r		997 	94 9	" :1: :1: 1	an în Înți în	ن. برا	1	1 1	la an	มี จ มี มี มี					
DEPTH	13	337	0			ç	70				UV					00	0			0	20				100			120			140	
	LERS	NE1	0							2								- 20						- 30	5					- 40		
	VALUE	N																			_											
	ECOVE		70	80		77	88	92	å	3		96		9R	2		2		80		64		78		76				80			
34/	11 37	AMA 2	U.			I.																										

٠

•.

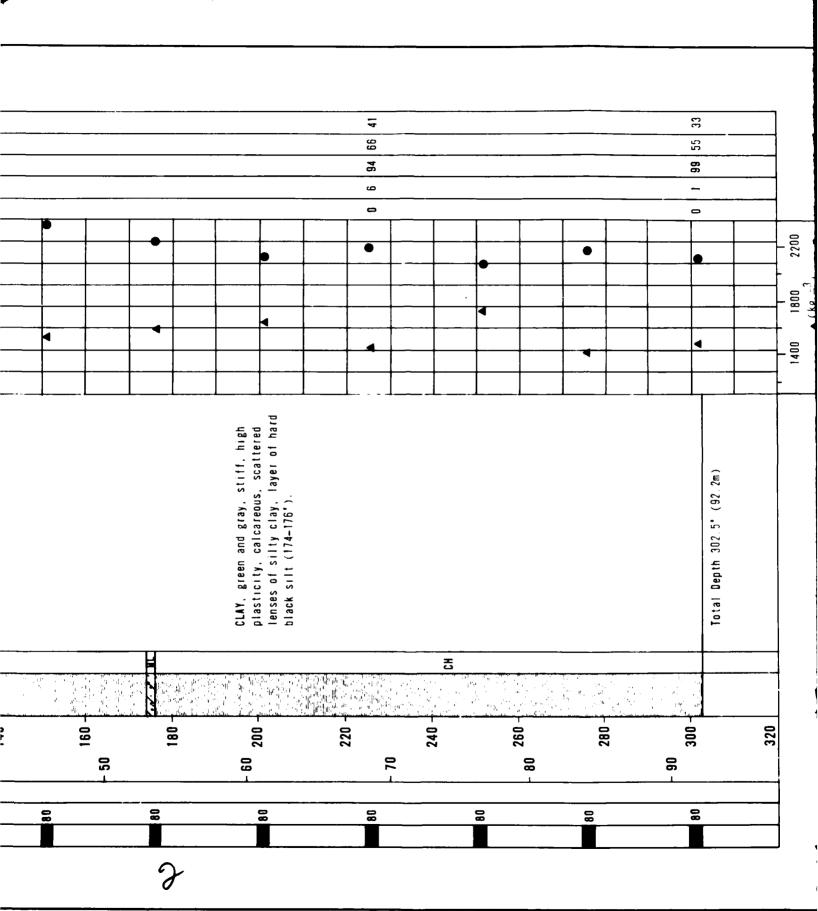
.

۱.

ļ

FN-TR-26c

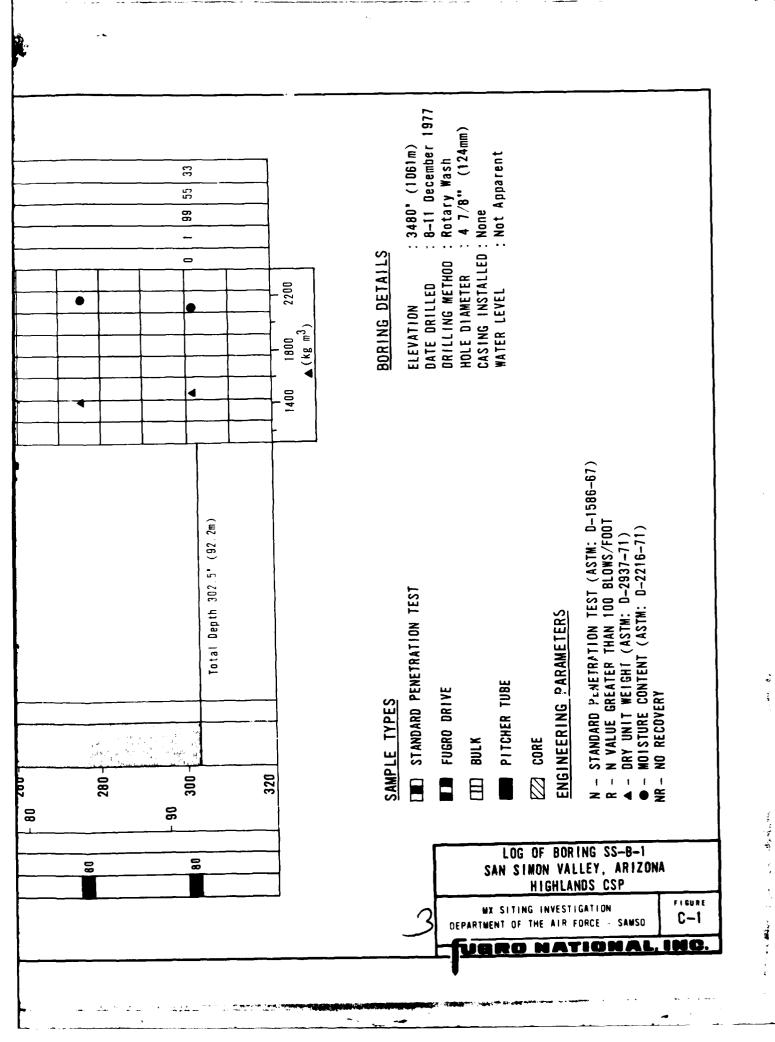
.

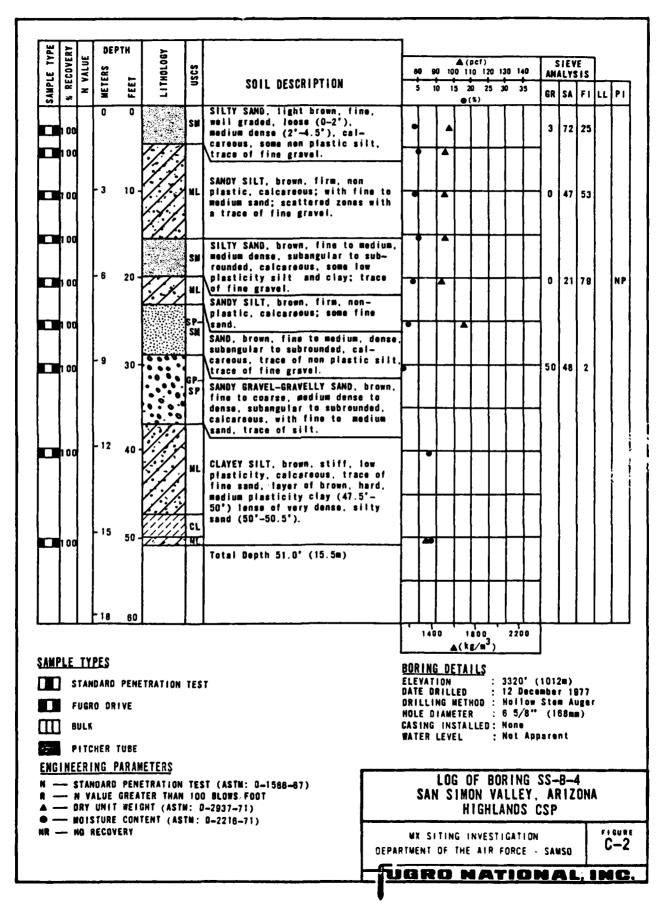


÷.,

. . .

.





۱

he LRED

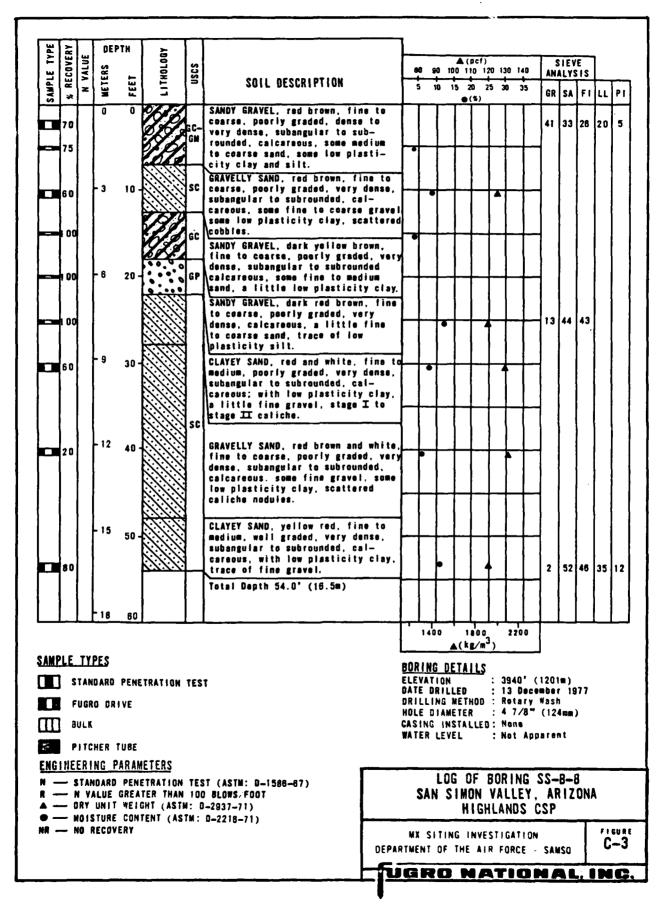
ā

ā

AFFROVED

ļ

the state of the state



••• ••• ••• •••

1 🖥

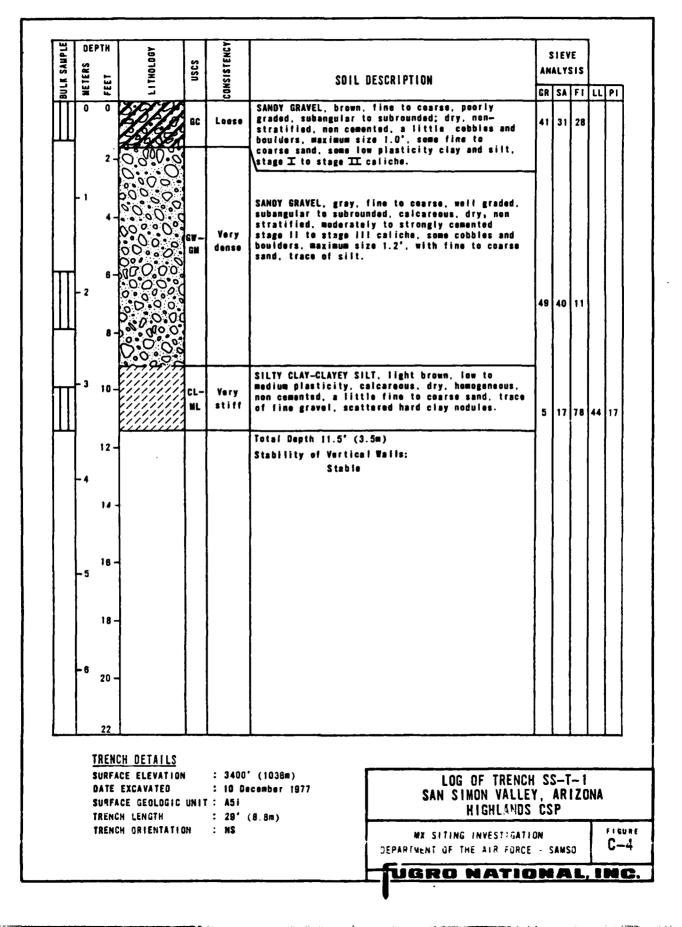
"NUYEU

1

l

2

CHERRIC



÷.,

• .

.....

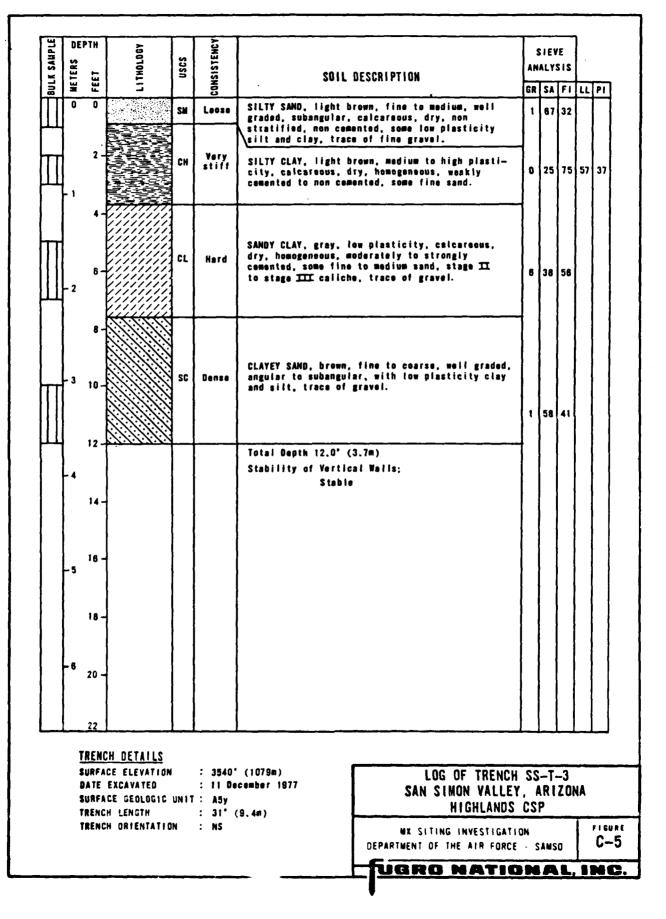
. .

.

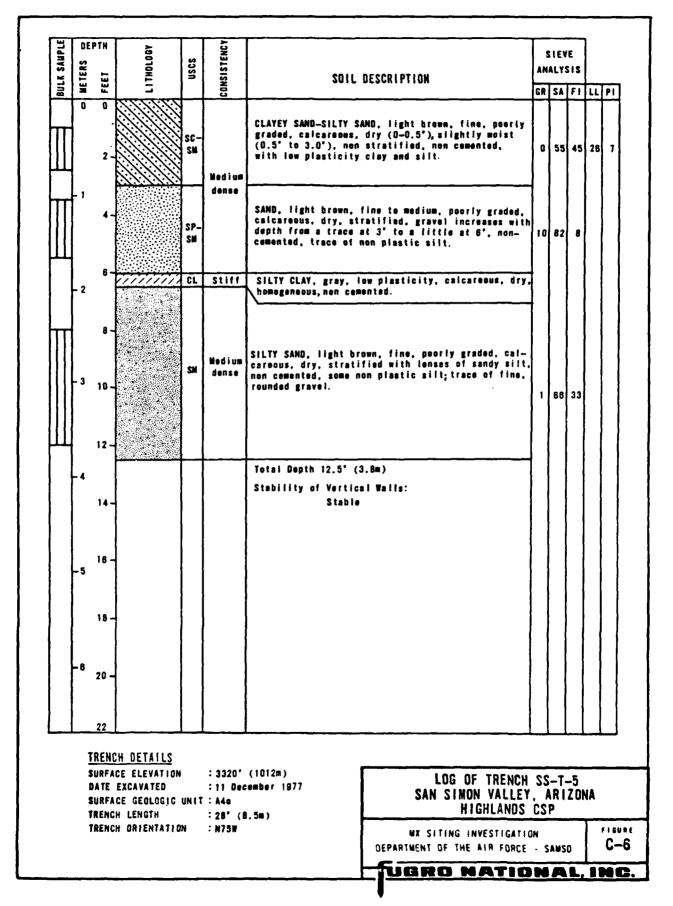


Ţ,

ti



ł



. . .

. . . .

.....

5

· · · · · 2

١

1

ArrKOVEN BI

Ì

CHECKED BY

9

ti,

...

Ŷ

I									PERCE	NT FIN	ER BY	WEIGHT				
ا _ہ ی	E (a)	SAMPLE I	NTERVAL		S	TANDAR	SIEV				· · · · ·	_		SIEVE	10	P/ SI
BOR I NG Number	SAMPLE NUMBER			BLDRS.	COBE	BLES		GRA	VEL		<u> </u>	SI	ND		SI.	LTO
8 2	S AI NU	FEET	METERS	24 "	12"	6"	3"	15"	3 . 4 "	3 8**	4	10	40	100	200	. 0
S-B-1	D-1		0.61-0.76													t-
	5-2	10.0-10.5	3.05-3.20					100	-84	69	50	34,	23	1.2	7	T
		10.0-10.5	3.05-3.20									Ι			Ī	Ι
		10.0-10.5	3.05-3.20		<u> </u>							L	I	L	L	I
	D-3	15.0-15.2	4.57-4.63		, .			ι -						 		ļ
	P-4	21.4,2.2	0.52-6.77						. .	4 .		100	49	<u> 98</u>	- 95	↓ -
	<u> </u>	25.4-25.7	7.62-7.83							 		 		_		┣
ł	<u> </u>	30.0-30.7	9.14-9.36							} i		┟╴╶╌╴	1		<u> </u>	+ -
		4-34	9.57-9.88							•		<u> </u>	100	99	<u></u>	+
		40.7-11.4	12.41-12.64									+	+	 		┢─
		50.7-51.4	15.45-15.67									ŧ	<u> </u>	<u>↓</u>	+	
	F= +		18.50-18.71		•	· · · · · · · · · · · · · · · · · · ·	·····	[1	<u>†</u>	<u>†</u>	†	t
	4-14-	71.4-72.	21.76-21.94	1					Γ			100	- 19	98	47	1 4
1	r-11	80.7-81.6	24.60-24.87									t		T	f	
	P-12	90.7-91.4	27.64-27.86									1	1	[T '	1
	₹-1}	100.7-101.4														I
	F-14	125.0-125.7	38.10-38.31	L						ļ		 	100	. 99	99	ļ.
		125.9-126.4						-				Ļ	 	Į	L	Ļ
		126.4-127.0	38.53-38.71										 		 	∔
	P-15	153.9-150.7	45,72-45,93					Į .	ł			ł	┢──	 	 	ł
	<u>P-1c</u>		53.55-53.77	 				 		.				╉ ───	 	+
	P-17 F-18	260.0-200.7	60.96-61.17					 		+		1 200	+	+	<u> </u>	+
	P-10 P-19	250.7-251.4	68.79-69.01 76.41-76.63	·						.		100	22	90	94	┢
	P-20		84.03-84.25					<u> </u>		1		+	╂	t	+	╉──
	P-21	300.7-301.4										+	100	99	99	+ 7
					· · · · ·			 				1		<u> </u>	<u> </u>	┢
		• ··· · · · · · · · · · · · · · · · · ·											<u> </u>	1	t	1
						· · · · · · · · · · · · · · · · · · ·		1				1	1	1	† -	t
													Ι			Γ
							L		ļ	ļ		_	 	 	ļ	┡
ł										 		.	┢	 	 	4
							ļ	· · · · ·				 	┫────	<u>↓</u>	 	+-
ł									l	↓		┫	ł	↓	 	╉
-							<u> </u>		ļ	├ ──┤		 	∤	 	 	∔
												╂	┫────	+	╉────	╉──
		·····					_					t	t	t	ŧ	+
				┣		i				┣───┤		┢───	 -	<u> </u>	 	╋──
		···								<u> </u>		╂───	t	t —	<u>+</u>	t
				t		· · · · ·	<u></u>			 		t	1	<u>† </u>	<u> </u>	\mathbf{t}

۰.

- ----

- - - -

i.

. .

. . . ._

						1	N-SITU			(OMPACTE			-÷	0 E		N		
ICLE (mm)		ITERBI Mits		11000	DRY	UNIT	MOISTURE Content (\$)	SATURATION (%)		MAX	INUN	0PT1MUM M01STURE (\$)	SPECIFIC Gravity Of Solids	TRIAXIAL (d)	UNCONFINED COMPRESSION		CONSOL I DATION	٩L	ΥE
LAY	L I	m 112	(0)	USCS (c)	WEI	GHT	*) EN	TURA1 (\$)	a=	DRY D	ENSITY	UNU STU	VIT SOL	AXI	P. E.	DIRECT SHEAR	0110	CHEMICAL	RELATIVE Density
.001	ш	PL	PI		(pcf)	(kg/m ³)	물흉~	SAT	V01D Ratio	(pcf)	(kg. m ³)	L ON	SPE GRA	TRI	38	DIR	SNO	CHE	REL Den
		Î –		SM		1					<u> </u>				<u> </u>				
I		[NF	GW-GM	110.3	1767	2.4	14.3	0.53	<u> </u>					 	*			
		ļ.		;WM		<u>.</u>	I									*			
				GW-GM												*			
1.	51	-29-	22	SC CH-MH	110.3	1767 1562	15.6 18.9	79.0							 				
- <u></u> -		<u> </u>	÷	SM	$\frac{97.5}{104.1}$	1668	14.8	70.2							*		*	*	
		t		CL		1.1.0.0		04.1	0.02					*	┢	┨─────			
-51	43	22	21	CL	87.7	1405	34.0	99.7	6.92				2.68	*	t	<u></u>			
				CL			I							*					
		L		CL	83.3	1334	31.1	31.9											
				CT. CH	87.3	1 398	34.0	98.5					[Į			↓
·		29	4.4	с <u>н</u>	81.2	1301	<u>39.2</u> 42.7	96.5	1.1		 		 		 	 	┣		
				ੰਸ	87.7	1405	39.0	100	0.92		 		 		ŧ	+		├ ───	
				CH	82.6	1323	46.1	100	1.0		†		t		<u>†</u>	1		<u> </u>	
				СН	30.4	1288	39.8	08.2							I				
- + · ·	72	16	41.	СН			45.9							*	_		ļ	 	
				<u>СН</u> СН			45.1				 		 	*		}		 	┥───
				CH		1543	38.2	100	1. 75		{		{		+-*	<u></u>	•	╡───	╂
ł	4			MI	99.4	1592	$\frac{36.2}{3(3)}$	100	0.75 0.70		ł	·		· ·	+	╉		<u> </u>	<u>↓</u>
				СН	102	1655	31.8		0.63		<u>+</u> -		t ——		+	<u>†</u>		1	
	r.t.	15	41	CH	90.8	1455	32.7		0.86				1						
				СН	102.9	1719	29.3	160	0.56						_	_	ļ	ļ	
- 11,	55	22		<u>्म</u> (11	85.9	1424	33.3	100	9 .89		ļ		┨────			╡───		<u> </u>	╂
	<u></u>		33	<u> </u>	92.6	1483	31.8	100	0.82				}		+ *	╉────	<u> </u>	┨───	ł
											<u> </u>	f	t	t —	<u>†</u>	f	†	1	† ·
														t	t				
															I	I		ļ	Ļ
												L	_		╂	┨────	╂───	 	
											_		 		+	+	<u></u> +	<u>+</u>	+
											┟	ł			+	+	†	╀───	1
											 	<u> </u>	<u>+</u>	t	+	1	1	1	1
											<u> </u>	t	1	1		1	Ι	L	I
																	ļ	\downarrow	
													 	 	+		╂──	┫────	
}]					ļ	 	╉────		-	+	+	+	+
											 	<u> </u>	╂	 	+	+	+	+	+
											┣	 	1					<u>t</u>	1
		0	}										DE	AN SI MX PARTNE	NON VI SITIN	BORIN LLEY, G INVES THE AIF	ARIZOI	<u>IA, HI</u>	INLAND

~

¥

· • •

- . .

														5	511	MON	YA	IF U TE LLE	ST Y,	RE: ARI	SUL Zon	TS IA,		S C TA	SP
BORING SAMPLE	NO.	SS-B-1			SS-B-6	SS-B-7	SS-B-9	SS-B-11	SS-B-12																
SAMPLE	. ON	1	P-18	P-21	<u>-</u> 3	ī	6-0		1-0	0-10															
SAMPLE	FEET	21.6-22.2	225.9-226.4	300.9-301.4	10.0-10.5	2.5-3.0	47.0-47.5	10.9-11.4	30.0-30.5	60.8-60.5															
SAMPLE INTERVAL	METERS	6.58-6.77	68.76-69.10	91.71-91.87	3.05-3.20	0.76-0.91	14.33-14.48	3.32-3.47	9.14-9.30	18.29-18.44															
SOIL	TYPE	CH-MH	СН	CH	SC	٦IJ	C.	CL	CL-ML	CH									1						
COMP. S	ksf	3.6	11.3	11.4	7.2	13.7	5.5	9.7	6.5	34.4															
UNCONFINED Comp. Strength	kN ∕@2	172	541	546	344	658	263	464	311	1647						 									
DRY DENSITY	pcf	90.3	91.1	93.3	107.4	109.0	110.8	93.4	95.4	94.8															
VT I SN:	kg∕m3	1447	1459	1495	1721	1746	1775	1496	1528	1519															
MO I STURE Content	(%)	25.9	31.8	30.4	10.2	10.0	13.3	13.5	10.1	22.7															
DEGREE OF Saturation	(<i>v</i>)	81.0	100	001	48.6	48.4	69.2	45.3	35.7	78.8															
HEIGHT	DIAMETER	2.4	2.4	2.4	2.4	2.4	2.4	2.4	2.4	2.4															

.

FN-TR-26c

1

......

SAMPLE SAMPLE INTERINAL SOIL DF DRY DEMSITY MOISTURE CONFINIT MAXIMUNAL MALLE STRAIN NO. FEET METERS TYPE TEST ACT XXI MAYE XXI XXII XXIII XXIII XXIIII XXIIIII XXIIIIII XXIIIIIIIIIIIIIIIIIIIIII	SAMPLE INTERVAL SOIL OF DEV DEN MOISTURE CONFENT RESSURE CONSTRESS OF CONSTRES OF CONSTRESS OF CONSTRESS OF CO
Soll TYPE DRY DENSITY MOISTURE CONFINING MAX MURATOR RS TYPE TEST peri hg/m3 () hsi hk/m2 hsi hk/m2 B. 57 CL CD 93.4 1496 28.7 3.3 158 14.2 680 B. 88 CL CD 94.7 1517 27.4 13.2 6.32 26.8 1283 B. 88 CL CD 15.17 27.4 13.2 6.32 26.8 1087 B. 88 CL CD 15.73 8C-CL CD 13.2 6.32 26.8 1283 15.73 SC-CL CD 126.4 2025 8.0 10.1 484 49.3 2361 15.73 SC-CL CD 126.4 2025 8.0 10.1 484 49.3 2361 15.73 SC-CL CD 126.4 2025 8.0 10.1 484 49.3	Solution Type DRX MOISTURE CONFINING RAVANDAA STRAIN PRES RS TYPE
TYPE DRY DENSITY MOISTURE CONFINING MAXIMUG 0F DEN 1496 28.7 3.3 158 14.2 680 CD 33.4 1496 28.7 3.3 158 14.2 680 CD 33.4 1517 27.4 13.2 532 26.8 1283 CD 115.6 1852 8.0 10.1 484 22.7 1087 CD 115.6 1852 8.0 10.1 484 49.3 2361 CD 126.4 2025 8.0 10.1 484 49.3 2361 CD 126.4 2026 10.1 484 49.3 24.4	TYPE DRY DENSITY MOISTURE CONTENT MOISTURE CONTENT RATE Cumin State RATE RATE <t< td=""></t<>
DRY DENT MOISTURE CONFINING MAXIMUM pc1 kg/m ³ (2) ks1 kn/m ² 93.4 1496 28.7 3.3 158 14.2 680 93.4 1517 27.4 13.2 6.3 26.8 1283 1517 27.4 13.2 6.32 26.8 1283 115.6 1852 8.0 10.1 484 22.7 1087 126.4 2025 8.0 10.1 484 49.3 2361 126.4 2025 8.0 10.1 484 49.3 2361 126.4 2025 8.0 10.1 484 49.3 2361 126.4 2025 8.0 10.1 484 49.3 2361 126.4 2025 8.0 10.1 484 49.3 2361 136 10.1 484 49.3 2361 10.1 10.1 148 13.2 10.1	DRY DENSITY MOISTURE CONTENT CONFINING RATE SOURE STRAIN RATE SOURCOL STRAIN RATE RATIONA RATE RATIONA RATE RATIONA RATE RATIONA RATE RATIONA 92.4 131 2.3 156 14.2 660 0.1 0 94.7 1517 27.4 13.2 632 26.8 1283 0.1 0 115.6 1852 8.0 10.1 484 49.3 2351 0.1 0 126.4 2025 8.0 10.1 484 49.3 2361 0.1 0 126.4 2025 8.0 10.1 484 49.3 2361 0.1 0 126.4 2025 8.0 10.1 484 49.3 2361 0.1 0 126.4 2025 8.0 10.1 484 49.3 2361 0.1 0 126.4 2025 8.0 10.1 484 49.3 2361 0.1 0 126.4 <t< td=""></t<>
RY DENSITY MOISTURE CONFINIT MAXIMUM pc(kg/m ³ (2) xst kn/m ² 30 1 14.2 680 14.2 680 14.7 1517 27.4 13.2 632 26.8 1283 15.6 1852 8.4 10.1 484 22.7 1007 26.4 2025 8.0 10.1 484 23.7 1007 26.4 2025 8.0 10.1 484 49.3 2361 26.4 2025 8.0 10.1 484 49.3 2361 26.4 2025 8.0 10.1 484 49.3 2361 26.4 2025 8.0 10.1 494 49.3 2361 26.4 2025 8.0 10.1 494 49.3 2361 26.4 2025 8.0 10.1 494 49.3 2361 27.4 10.1 494 4	IRY DENSITY MOISTURE CONFINING MATE Deci MATE RATE RATE RATE Doci ks/m3 (2) xsi ku/m2 (2) xsi 13.4 1496 28.7 3.3 158 14.2 680 0.1 0 14.7 1517 27.4 13.2 632 26.8 128.3 0.1 0 15.6 1852 8.4 10.1 484 22.7 1097 0.1 0 15.6 1852 8.0 10.1 484 49.3 2361 0.1 0 26.4 2025 8.0 10.1 484 49.3 2361 0.1 0 26.4 2025 8.0 10.1 484 49.3 2361 0.1 0 26.4 2025 8.0 10.1 484 49.3 2361 0.1 0 15.6 10.1 484 49.3 2361 0.1 0 16.1 10
MOISTURE CONFENT PRESSURE MAXIMUM (2) hst kn/m2 kst kn/m2 28.7 3.3 158 14.2 680 27.4 13.2 632 26.8 1283 8.0 10.1 484 22.7 1087 8.0 10.1 484 23.2 2361 9.4 10.1 484 49.3 2361 9.0 10.1 484 49.3 2361 9.0 10.1 484 49.3 2361	MOISTURE CONFINING CONTENT MAX MUNAL National Astional STRAIN Astional STRAIN Astional RATE Astional PRESS Astional PRESS Astional PRESS Astional PRESS Astional BA 28.7 3.3 158 14.2 680 0.1 0
CONFINING MAXIMUM HESSURE (0-) STRESS AT AUM her kn/m ² stress kn/m ² 3.3 158 14.2 680 13.2 632 26.8 1283 10.1 484 49.3 2361 10.1 484 49.3 2361 10.1 484 49.3 2361	CONFINING MAXIMUNA STRAIN PRESSURE (C) STRAIN PRESSURE (C) STRAIN RATE AN/M2 ks1 AN/M2 (C min) ks1 and a and and
CONFINING MAXIMUM HESSURE (0-) STRESS AT AUM her kn/m ² stress kn/m ² 3.3 158 14.2 680 13.2 632 26.8 1283 10.1 484 49.3 2361 10.1 484 49.3 2361 10.1 484 49.3 2361	CONFINING MAXIMUNA STRAIN PRESSURE (C) STRAIN PRESSURE (C) STRAIN RATE AN/M2 ks1 AN/M2 (C min) ks1 and a and and
MAX INUM ks f kn/m ² 14.2 680 26.8 1283 22.7 1087 49.3 2361 49.3 2361	BAXIMUNA SIRESS ACION NST STRAIN RATE KS1 MATE KS1 MATE KS1 MATE AS1 MATE 14.2 680 25.3 1087 26.8 1283 27.1 1087 28.3 2361 29.3 2361 29.3 2361 21.1 0 21.1 0 22.7 1087 21.1 0 22.7 1087 21.1 0 22.1 1087 2361 0.1 2361 0.1 21.1 0 22.7 1087 21.1 0 22.7 1087 2361 0.1 2361 0.1 2361 0.1 2361 0.1 237 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 23
MAX INUM ks f kn/m ² 14.2 680 26.8 1283 22.7 1087 49.3 2361 49.3 2361	BAXIMUNA SIRESS ACION NST STRAIN RATE KS1 MATE KS1 MATE KS1 MATE AS1 MATE 14.2 680 25.3 1087 26.8 1283 27.1 1087 28.3 2361 29.3 2361 29.3 2361 21.1 0 21.1 0 22.7 1087 21.1 0 22.7 1087 21.1 0 22.1 1087 2361 0.1 2361 0.1 21.1 0 22.7 1087 21.1 0 22.7 1087 2361 0.1 2361 0.1 2361 0.1 2361 0.1 237 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 2381 0.1 23
MUM STRAIN 108 RATE MV/m ² (° min) 680 0.1 1283 0.1 1283 0.1 2361 0.1 2361 0.1	STRAIN RATE 0.1 BA Nation 0.1 0 0.1 0 0.1 0 0.1 0 0.1 0 0.1 0 0.1 0 0.1 0
STRAIN RATE 0.1 0.1 0.1 0.1	Paga Againation of o o o o o o o o o o o o o o o o o

.....

BORING	SAMPLE	SAMPLE I	NTERVAL	SOIL	NORMAL	STRESS		IMUM Strength
NO.	NO.	FEET	METERS	Түре	ksf	kN/m ²	kst	kN/m 2
SS-B-1	D-2	10.0-10.5	3.05-3.20	GW-GM	1.0	48	1.3	62
	D-2	10.0-10.5	3.05-3.20	GW-GN	2.0	96	1.6	17
	D-2	10.0-10.5	3.05-3.20	GW-GM	4.0	192	2.8	134
SS-B-2	0-1	2.0-2.5	0.61-0.76	CH	0.2	10	1.6	11
	D-1	2.0-2.5	0.61-0.76	CH	0.5	24	2.0	96
	D-1	2.0-2.5	0.61-0.76	CH	1.0	48	4.8	230
	P-3	10.9-11.4	3.32-3.47	SC	1.0	48	1.7	81
	P-3	10.9-11.4	3.32-3.47	SC	2.0	96	3.4	163
	P-3	10.9-11.4	3.32-3.47	SC	4.0	192	5.7	273
SS-B-3	D-3	10.0-10.5	3.05-3.20	SM	1.0	48	0.8	38
	D-3	10.0-10.5	3.05-3.20	SM	2.0	96	1.5,	72
	B-3	10.0-10.5	3.05-3.20	SM	4.0	192	3.7	177
SS-B-4	D-1	2.5-3.0	0.76-0.91	SM	0.2	10	0.4	19
	D-1	2.5-3.0	0.76-0.91	SM	0.5	24	0.5	24
	D-1	2.5-3.0	0.76-0.91	SM	1.0	48	0.8	38
SS -B- 6	D-1	2.5-3.0	0.76-0.91	CL	0.2	10	0.6	29
	D-1	2.5-3.0	0.76-0.91	CL	0.5	24	0.9	43
	0-1	2.5-3.0	0.76-0.91	CL	1.0	48	3.7	177
S-B-11	D-1	2.0-2.5	0.61-0.76	SM	0.2	10	0.5	24
	D-1	2.0-2.5	0.61-0.76	SM	0.5	24	0.5	24
	D-1	2.0-2.5	0.61-0.76	SM	1.0	48	2.0	96
S-8-13	D2	5.0-5.5	1.52-1.68	SC	0.5	24	1.5	72
	D-2	5.0-5.5	1.52-1.68	SC	1.0	48	2.0	96
	D-2	5.0-5.5	1.52-1.68	SC	2.0	96	4.3	206

·

.

SUMMARY OF DIRECT SHEAR TEST RESULTS SAN SIMON VALLEY, ARIZONA HIGHLANDS CSP

WX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - SAMSO

UGRO NATIONAL, INC.

TABLE

C-4

S.

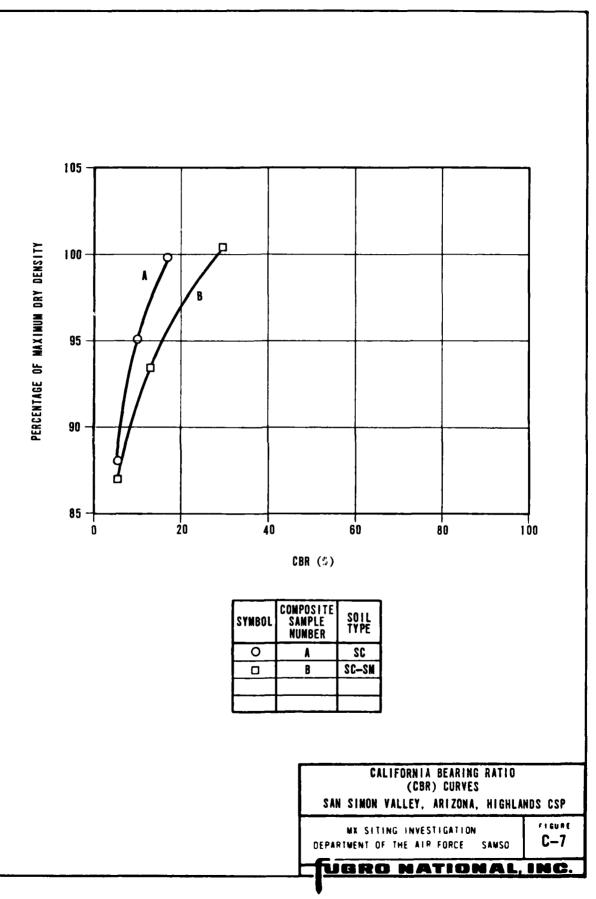
:

FN-TR-	-26	Ç
--------	-----	---

Я́

COMPOSITE Sample Number	E SOIL TYPE	PERCENT PASS ING #200	ATTERBERG LIMITS LL PI	BERG ITS PI	SPECIFIC GRAVITY	MAX DRY DE		OPTIMUM Moisture (%)		COMPACTED DRY DENSITY pcf kg/m ³	COMPACTED Molsture (%)	PERCENT OF Maximum Dry density	CBR (%)
									110.8	1775	10.5	88.1	60
				-					119.8	1916	10.2	95.1	10
-	SC					125.8	2015	10.5	125.5	1102	10.6	8.88	11
									106.3	1703	12.2	87.0	9
									114.3	1831	12.1	93.5	13
æ	SC-SM	45	26	~		122.2	1958	12	123.1	1972	12.0	100.7	30
	<u></u>												
								_					
				_									
	CALIFORNIA BEARING RATIO (CBR) TEST RESULTS			CALIFORNIA BEARING RATIO	11 97 11 97 80 97 97 98 97 98 97 98 97 98 98 97 98	Id I Id I Id I Id Id Id <	Id I 11 90 12 90 13 90 14 90 15 90 16 90 17 90 18 90 19 90 10 90 10 90 10 90 10 90 10 90 10 90 10	Id I Id <td>MINING 100000 110000 110000 110000 110000 111000 111000 111000 111000 111000 111000 111000 111000 111000 111000</td> <td>NUMBER I.I. #200 I.I. R 38 11 0 R 36 10 10 I 122.2 1958 12 I 122.2 1958 12</td> <td>NUMERA III MILL #2000 III PI PI N 20 11 100.8 100.8 100.8 100.8 N 20 20 11 100.8 100.8 100.8 N 20 20 10 123.2 130.8 10.5 110.8 N 20 28 2 28 1 123.2 130.8 10.1 N 20 28 1 123.2 130.8 13 108.3 13 N 20 28 1 123.2 130.8 10 11 N 20 28 1 123.2 130.8 10 11 N 20 28 1 123.2 130.8 13 108.3 N 20 28 1 123.2 130.8 13 137.1 N 20 28 28 28 123.2 130.8 13 N 20 28 38 38 38 38 N 28 28 38 38 38 N 28 28 38 38 38 N 28 28</td> <td>NUMBER III Pict LE/M3 (3) pict LE/M3 (3) NUMBER III 31</td> <td>NUMBERA IL PI Control # 200 LL PI Control # 201 HOLBS HOLBS</td>	MINING 100000 110000 110000 110000 110000 111000 111000 111000 111000 111000 111000 111000 111000 111000 111000	NUMBER I.I. #200 I.I. R 38 11 0 R 36 10 10 I 122.2 1958 12 I 122.2 1958 12	NUMERA III MILL #2000 III PI PI N 20 11 100.8 100.8 100.8 100.8 N 20 20 11 100.8 100.8 100.8 N 20 20 10 123.2 130.8 10.5 110.8 N 20 28 2 28 1 123.2 130.8 10.1 N 20 28 1 123.2 130.8 13 108.3 13 N 20 28 1 123.2 130.8 10 11 N 20 28 1 123.2 130.8 10 11 N 20 28 1 123.2 130.8 13 108.3 N 20 28 1 123.2 130.8 13 137.1 N 20 28 28 28 123.2 130.8 13 N 20 28 38 38 38 38 N 28 28 38 38 38 N 28 28 38 38 38 N 28 28	NUMBER III Pict LE/M3 (3) pict LE/M3 (3) NUMBER III 31	NUMBERA IL PI Control # 200 LL PI Control # 201 HOLBS HOLBS

.



and a l

÷.,

A Star.

ŝ.

