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BLACK AND VEATCH KANSAS CITY MO
NATIONAL DAM SAFETY PROGRAM. LAKE CYRENE DAM (MO 11596), MISSOURI-ETC(U)
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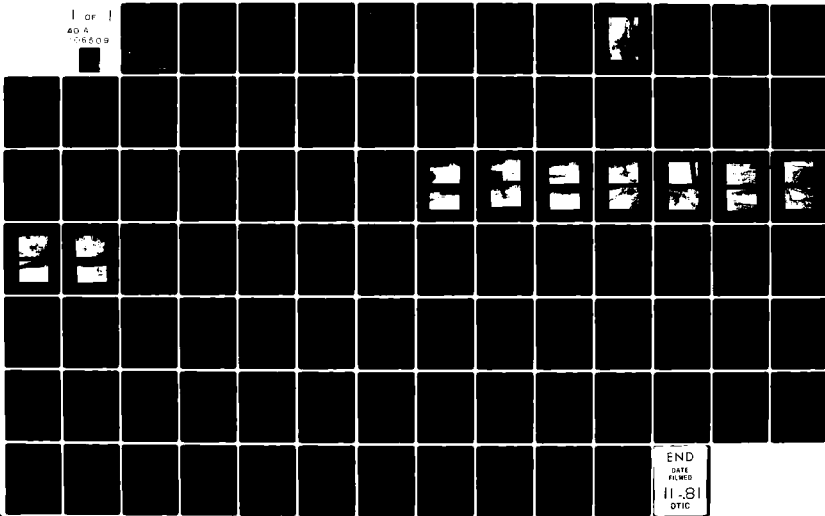
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**LAKE CYRENE DAM
BOONE COUNTY, MISSOURI
MO 11596**

**PHASE 1 INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM**

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PREPARED BY: U.S. ARMY ENGINEER DISTRICT. ST. LOUIS

FOR: STATE OF MISSOURI

MARCH 1981

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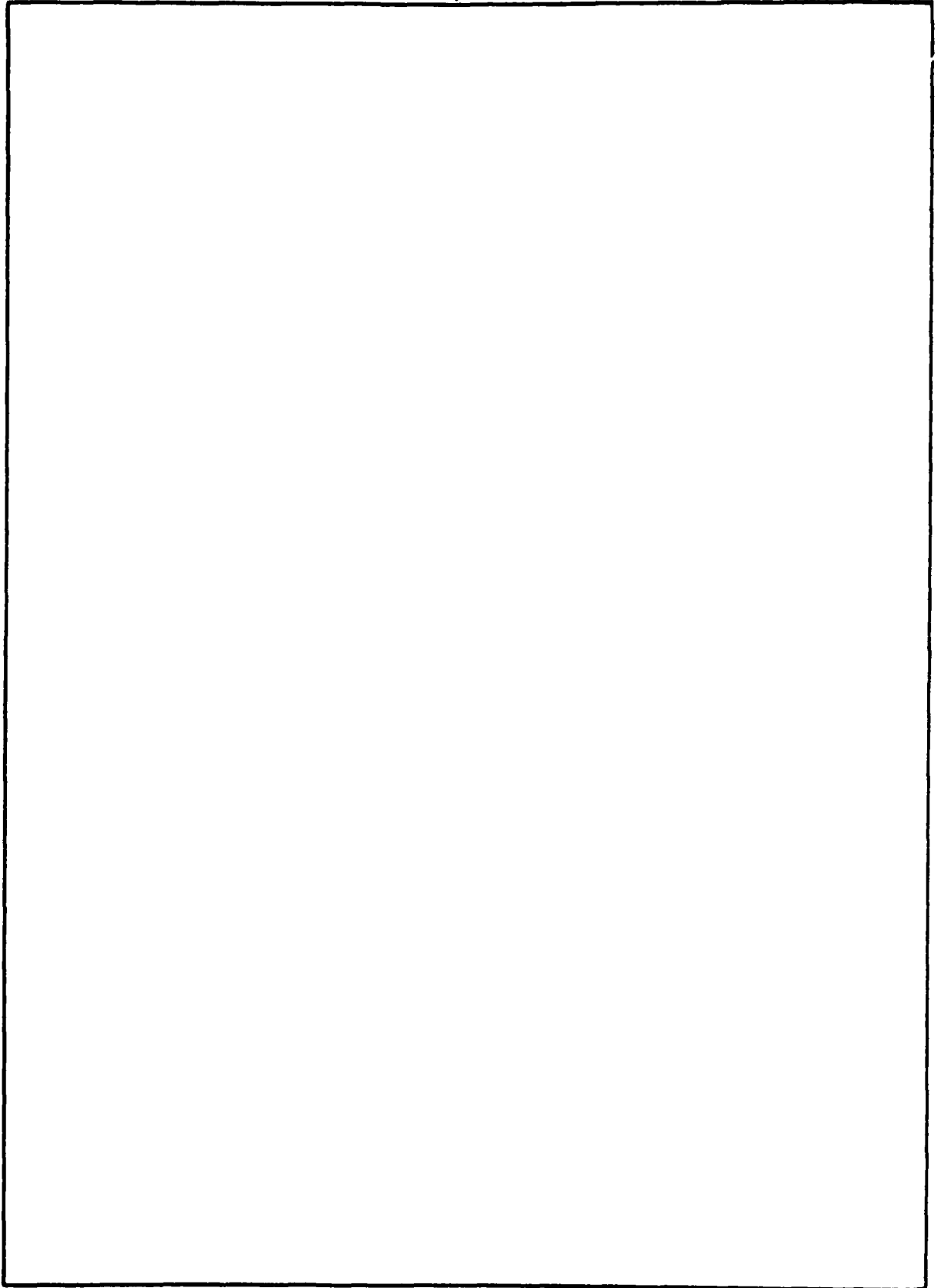
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MISSOURI-KANSAS CITY BASIN

LAKE CYRENE DAM
BOONE COUNTY, MISSOURI
MO 11596

PHASE 1 INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM



United States Army
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St. Louis District

PREPARED BY: U.S. ARMY ENGINEER DISTRICT. ST. LOUIS

FOR: STATE OF MISSOURI

MARCH 1981



DEPARTMENT OF THE ARMY
ST. LOUIS DISTRICT CORPS OF ENGINEERS
 210 TUCKER BOULEVARD NORTH
 ST. LOUIS, MISSOURI 63101

REPLY TO
 ATTENTION OF

SUBJECT: Lake Cyrene Dam, (MO 11596)
 Phase I Inspection Report

This report presents the results of field inspection and evaluation of the Lake Cyrene Dam (MO 11596).

It was prepared under the National Program of Inspection of Non-Federal Dams.

This dam has been classified as unsafe, non-emergency by the St. Louis District as a result of the application of the following criteria:

- a. Spillway will not pass 50 percent of the Probable Maximum Flood without overtopping the dam.
- b. Overtopping of the dam could result in failure of the dam.
- c. Dam failure significantly increases the hazard to loss of life downstream.

SUBMITTED BY: SIGNED 22 JUL 1981
 Chief, Engineering Division Date

APPROVED BY: SIGNED 23 JUL 1981
 Colonel, CE, Commanding Date

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LAKE CYRENE DAM
BOONE COUNTY, MISSOURI
MISSOURI INVENTORY NO. 11596

PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM

PREPARED BY:
BLACK & VEATCH
CONSULTING ENGINEERS
KANSAS CITY, MISSOURI

UNDER DIRECTION OF
ST. LOUIS DISTRICT CORPS OF ENGINEERS
FOR
GOVERNOR OF MISSOURI

MARCH 1981

PHASE I REPORT
NATIONAL DAM SAFETY PROGRAM

Name of Dam	Lake Cyrene Dam
State Located	Missouri
County Located	Boone County
Stream	Tributary to Hinkson Creek
Date of Inspection	11 March 1981

Lake Cyrene Dam was inspected by a team of engineers from Black & Veatch, Consulting Engineers for the St. Louis District, Corps of Engineers. The purpose of the inspection was to make an assessment of the general condition of the dam with respect to safety, based upon available data and visual inspection, in order to determine if the dam poses hazards to human life or property.

The guidelines used in the assessment were furnished by the Department of the Army, Office of the Chief of Engineers and developed with the help of several Federal and state agencies, professional engineering organizations, and private engineers. Based on these guidelines, this dam is classified as a small size dam with a high downstream hazard potential. According to the St. Louis District, Corps of Engineers, failure would threaten lives and property. The estimated damage zone extends approximately two miles downstream of the dam. Within the estimated damage zone are four dwellings, one building, three apartment buildings, a gasoline station, a trailer court consisting of approximately 15 trailers, and the junction of Highways 740 and 63. Contents of the estimated downstream damage zone were verified by the inspection team.

Our inspection and evaluation indicates the spillways do not meet the criteria set forth in the guidelines for a dam having the above size and hazard potential. The spillways will not pass the probable maximum flood without overtopping but will pass 10 percent of the probable maximum flood. The spillways will not pass the flood which has a one percent chance of occurrence in any given year (100-year flood), but will pass the flood with a 10 percent chance of occurrence (10-year flood). The spillway design flood recommended by the guidelines is 50 to 100 percent of the probable maximum flood. Considering the hazard zone and the reservoir storage volume, the spillway design flood should be 100 percent of the probable maximum flood. The probable maximum flood is defined as the flood discharge which may be expected from the most severe combination of critical meteorologic and hydrologic conditions which are reasonably possible in the region.

Based on visual observations, this dam appears to be in less than satisfactory condition. Deficiencies visually observed by the inspection team were erosion and sloughing of the upstream face, seepage on the downstream face, trees growing on the embankment and animal burrows on the embankment. Seepage and stability analyses required by the guidelines were not available.

There were no observed deficiencies or conditions existing at the time of the inspection which indicated an immediate safety hazard. Future corrective action and regular maintenance will be required to correct or control the described deficiencies. In addition, detailed seepage and stability analyses of the existing dam, as required by the guidelines, should be performed. A detailed report discussing each of these deficiencies is attached.

Edwin R. Burton

Edwin R. Burton, PE
Missouri E-10137

Harry L. Callahan

Harry L. Callahan, Partner
Black & Veatch



OVERVIEW OF DAM

PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM
LAKE CYRENE DAM

TABLE OF CONTENTS

<u>Paragraph No.</u>	<u>Title</u>	<u>Page No.</u>
SECTION 1 - PROJECT INFORMATION		
1.1	General	1
1.2	Description of Project	1
1.3	Pertinent Data	2
SECTION 2 - ENGINEERING DATA		
2.1	Design	6
2.2	Construction	6
2.3	Operation	6
2.4	Geology	6
2.5	Evaluation	6
SECTION 3 - VISUAL INSPECTION		
3.1	Findings	7
3.2	Evaluation	9
SECTION 4 - OPERATIONAL PROCEDURES		
4.1	Procedures	10
4.2	Maintenance of Dam	10
4.3	Maintenance of Operating Facilities	10
4.4	Description of Any Warning System in Effect	10
4.5	Evaluation	10
SECTION 5 - HYDRAULIC/HYDROLOGIC		
5.1	Evaluation of Features	11
SECTION 6 - STRUCTURAL STABILITY		
6.1	Evaluation of Structural Stability	13
SECTION 7 - ASSESSMENT/REMEDIAL MEASURES		
7.1	Dam Assessment	14
7.2	Remedial Measures	14

TABLE OF CONTENTS (Cont'd)

LIST OF PLATES

<u>Plate No.</u>	<u>Title</u>
1	Location Map
2	Vicinity Topography
3	Dam Plan
4	Dam Crest Profile, Dam Cross Section
5	Emergency Spillway Profile, Emergency Spillway Cross Section
6	Photo Index

LIST OF PHOTOGRAPHS

<u>Photo No.</u>	<u>Title</u>
1	Upstream Face of Dam West End
2	Upstream Face of Dam at Mid-Dam
3	Upstream Face of Dam East End
4	Crest of Dam Looking East
5	Crest of Dam Looking West
6	Downstream Face of Dam Looking East
7	Downstream Face of Dam At Mid-Dam
8	Drop Inlet to Principal Spillway
9	Outlet from Drop Inlet
10	Downstream End of Principal Spillway Pipe
11	Principal Spillway Outlet and Downstream Channel
12	Overview of Emergency Spillway

TABLE OF CONTENTS (Cont'd)

LIST OF PHOTOGRAPHS

<u>Photo No.</u>	<u>Title</u>
13	Emergency Spillway Channel Looking Downstream
14	Animal Burrow on Downstream Face of Dam
15	Erosion of Upstream Face of Dam
16	Upstream Watershed Area
17	Abandoned Pump House Downstream of Dam
18	Area Downstream of Dam

APPENDIX

Appendix A - Hydrologic and Hydraulic Analyses

SECTION 1 - PROJECT INFORMATION

1.1 GENERAL

a. Authority. The National Dam Inspection Act, Public Law 92-367, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a program of safety inspection of dams throughout the United States. Pursuant to the above, the District Engineer of the St. Louis District, Corps of Engineers, directed that a safety inspection of the Lake Cyrene Dam be made.

b. Purpose of Inspection. The purpose of the inspection was to make an assessment of the general condition of the dam with respect to safety, based upon available data and visual inspection, in order to determine if the dam poses hazards to human life or property.

c. Evaluation Criteria. Criteria used to evaluate the dam were furnished by the Department of the Army, Office of the Chief of Engineers, in "Recommended Guidelines for Safety Inspection of Dams." These guidelines were developed with the help of several Federal agencies and many state agencies, professional engineering organizations, and private engineers.

1.2 DESCRIPTION OF PROJECT

a. Description of Dam and Appurtenances.

(1) The dam is an earth structure located in the valley of a tributary to Hinkson Creek, (See Plate 1). The watershed is an area of low hills with fairly steep slopes which is totally developed as a residential area. The dam is approximately 670 feet long along its crest. Its alignment forms a "Z". The embankment height is 25 feet from its crest to the toe of slope at its highest point. The crest width is about 10 feet. The downstream face of the dam has a nonuniform slope from the crest to the valley floor below.

(2) The principal spillway is a 36-inch corrugated metal pipe drop inlet connected to an 18-inch corrugated metal pipe through the dam. The pipe outlet is downstream of the dam and discharges into a small pond then to the natural stream below.

(3) The emergency spillway is a grass lined, open channel cut through the natural abutment at the left end of the dam. (Left or right as used herein is referenced while looking in a downstream direction.) The spillway channel is trapezoidal with a bottom width of about 7 feet. There is a low berm constructed along the right bank to prevent flow across the downstream slope of the embankment. The emergency spillway discharges onto a wooded hillside downstream of the dam.

(4) Pertinent physical data are given in paragraph 1.3.

b. Location. The dam is located in the eastern part of the City of Columbia in central Boone County, Missouri, as indicated on Plate 1. The lake formed by the dam is shown on the United States Geological Survey 7.5 minute series quadrangle map for Columbia, Missouri in Section 19 of T48N, R12W.

c. Size Classification. Criteria for determining the size classification of dams and impoundments are presented in the guidelines referenced in paragraph 1.1c above. Based on these criteria, the dam and impoundment are in the small size category. A small size dam is classified as having a height less than 40 feet, but greater than or equal to 25 feet and/or a storage capacity less than 1,000 acre-feet, but greater than or equal to 50 acre-feet. Lake Cyrene Dam is 25-feet high with a normal storage volume of 50 acre-feet.

d. Hazard Classification. The hazard classification assigned by the Corps of Engineers for this dam is as follows: The Lake Cyrene Dam has a high hazard potential, meaning that the dam is located where failure may cause loss of life, and serious damage to homes, agricultural, industrial and commercial facilities, and to important public utilities, main highways, or railroads. For the Lake Cyrene Dam the estimated flood damage zone extends approximately two miles downstream of the dam. Within the estimated damage zone are four dwellings, one building, three apartment buildings, a gasoline station, a trailer court with approximately 15 trailers, and the junction of Highways 740 and 63. Contents of the estimated downstream damage zone were verified by the inspection team.

e. Ownership. The dam is owned by the Lake Cyrene Corporation, 1132 Business Highway 63 South, Columbia, Mo. 65201, c/o William Platt, President.

f. Purpose of Dam. The dam forms a 6.3-acre lake used for recreation.

g. Design and Construction History. Data relating to the design and construction were not available. According to Mr. William Platt, president of the Lake Cyrene Corporation, the dam was built in the 1930's and provided water for dairy farming operations.

h. Normal Operating Procedure. Normal rainfall, runoff, transpiration, evaporation, and overflow through the uncontrolled spillway all combine to maintain a relatively stable water surface elevation.

1.3 PERTINENT DATA

a. Drainage Area - 64 acres

b. Discharge at Damsite.

(1) Normal discharge at the damsite overflows to a 36-inch diameter corrugated metal pipe drop inlet to an 18-inch diameter corrugated metal pipe.

(2) Estimated experienced maximum flood at damsite - Unknown.

(3) Estimated ungated spillway capacity at maximum pool elevation 110 cfs (Probable Maximum Flood Pool El. 686.5).

c. Elevation (Feet above m.s.l. Approximate Tie to USGS Map).

(1) Top of dam - 685.5 (see Plates 3 & 4)

(2) Principal spillway crest - 684.0

(3) Streambed at toe of dam - 660.3

(4) Maximum tailwater - Unknown.

d. Reservoir.

(1) Length of maximum pool - 700 feet \pm (Probable maximum flood pool level)

(2) Length of normal pool - 625 feet \pm (Principal spillway crest)

e. Storage (Acre-feet).

(1) Top of dam - 60

(2) Principal spillway crest - 50

(3) Design surcharge - Not available.

f. Reservoir Surface (Acres).

(1) Top of dam - 6.8

(2) Principal spillway crest - 6.3

g. Dam.

(1) Type - Earth embankment

- (2) Length - 670 feet
- (3) Height - 25 feet \pm
- (4) Top width - 10 feet
- (5) Side slopes - upstream face 1.0 V on 2.3 H, downstream face between 1.0 V on 2.5 H and 1.0 V on 6.7 H (see Plate 4)
- (6) Zoning - Unknown.
- (7) Impervious core - Unknown.
- (8) Cutoff - Unknown.
- (9) Grout curtain - Unknown.

h. Diversion and Regulating Tunnel - None.

i. Principal Spillway.

- (1) Type - 36-inch diameter CMP drop inlet with 18-inch diameter CMP through the dam.
- (2) Inlet crest elevation - 684.0 feet m.s.l.
- (3) Inlet invert elevation - 677.7.
- (4) Outlet invert elevation - 669.8 feet m.s.l.
- (5) Gates - None.
- (6) Upstream channel - None.
- (7) Downstream channel - Spillway discharges to a small pond downstream of the dam then to the natural stream below.

j. Emergency Spillway.

- (1) Type - Grass lined open channel.
- (2) Crest Elevation - 684.6 feet m.s.l.
- (3) Gates - None.
- (4) Upstream Channel - Grass-lined approach channel.

(5) Downstream Channel - Spillway, discharges to wooded hillside then to a small pond downstream of the dam then to stream below.

k. Regulating Outlets - None.

SECTION 2 - ENGINEERING DATA

2.1 DESIGN

Design data were not available.

2.2 CONSTRUCTION

Construction records were unavailable.

2.3 OPERATION

Operational records and documentation of past floods were unavailable.

2.4 GEOLOGY

The site of the dam and reservoir is located across a broad, moderately steep sided valley. The dam impounds the drainage from an intermittent tributary of Hinkson Creek.

The soil in the dam and reservoir area consists of Union silt loam and silty clay loam. The Union series consists of moderately well drained soils formed in loess over cherty residuum weathered from limestone. The upper 8 inches of this series is classified for engineering purposes as clayey silt to silty clay (Unified Classification CL-ML to CL). The remaining part of the soil profile is classified as silty clay (CL) with clayey sand (SC), clayey gravel (GC) and highly plastic clay (CH) present below a depth of 40 inches. Bedrock of the area consists of Mississippian age cherty, crinoidal limestones of the Osagean Series. Depth to bedrock could not be determined visually at the site but is assumed to be greater than 5 feet based on Soil Conservation Service information.

2.5 EVALUATION

a. Availability. No engineering data were available.

b. Adequacy. No engineering data were available. Thus, an assessment of the design, construction, and operation could not be made. Seepage and stability analyses comparable to the requirements of the "Recommended Guidelines for Safety Inspection of Dams" were not available, which is considered a deficiency. These seepage and stability analyses should be performed for appropriate loading conditions (including earthquake loads) and made a matter of record.

c. Validity. The validity of the design, construction, and operation could not be determined due to the lack of engineering data.

SECTION 3 - VISUAL INSPECTION

3.1 FINDINGS

a. General. A visual inspection of Lake Cyrene Dam was made on 11 March 1981. The inspection team consisted of Edwin Burton, team leader; Shannon Casey, geologist; Gary Van Riessen, geotechnical engineer; and John Ruhl, hydraulic/hydrologic engineer. The dam appeared to be sound but is in less than satisfactory condition due to erosion of the upstream face and trees and animal burrows on the embankment. Specific observations are discussed below. No observations were made of the condition of the upstream face of the dam below the pool elevation at the time of the inspection.

b. Dam. The inspection team observed the following conditions at the dam. There was no cracking or evidence of sliding of the embankment slopes. The dam crest elevation was very irregular but there was no visible sign that the dam had ever been overtopped. Mr. Platt, who has lived in the residence at the right end of the dam for over ten years, stated that he has no knowledge of the dam ever being overtopped.

The water line along the upstream face and the slope of the upstream face are very irregular due to wave action erosion and sloughing, a condition that is aggravated by burrowing animals, (muskrat). The upstream face has riprap slope protection; which is located at an elevation lower than the principal spillway inlet and the erosion and sloughing is taking place above the riprap, (Photo 15). Several large animal burrows (Photo 14), probably groundhog, were observed on the downstream face of the dam. An area of nonflowing seepage was observed on the downstream slope (Photo 7) and below the downstream toe near and to the right of an abandoned pumphouse. The seepage was evidenced by the abundant growth of cattails and aquatic plants. Another possible seepage area exists on the right abutment downstream of the dam. Mr. Platt indicated that this area stays wet most of the time. This area did not appear to be unusually wet to the inspection team; however, the lake level was low on the day of inspection. There are many trees growing on both faces of the embankment, (Photos 6, 7 and 15) some with trunks as large as 12 to 14 inches. A row of poplar trees 2 to 4 inches have been planted along the downstream edge of the crest. Erosion protection on the crest and downstream face consist of a good grass cover with some weeds.

c. Appurtenant Structures. Appurtenant structures observed by the inspection team include the principal spillway, the emergency spillway, and the old pump house.

The principal spillway is a 36-inch corrugated metal pipe drop inlet to a 18-inch corrugated metal pipe through the embankment. The

drop inlet has a welded steel bar trash screen at the inlet crest, (Photo 8). Only the inlet and about two feet of the pipe at the outlet end were observable. The pipe alignment was observed by looking through the pipe from the outlet end. The pipe appeared to turn up slightly at about 30 feet from the outlet. There appeared to be a slight crimp at the crown of the pipe at the point where it turned up. The pipe and drop inlet were of sound metal with some surface rust. There was no flow through the pipe at the time of the inspection.

The emergency spillway is an open channel cut through natural material of the left abutment. No signs of erosion of the emergency spillway were observed. The channel floor and banks were protected by a good dense cover of grass. The channel is non-uniform in slope and cross section. The emergency spillway channel ends at a brush and tree covered slope about 75 feet downstream from the dam, (Photo 13).

There is no development in the spillway areas which would suffer damage due to flow through the spillways. High flows through the emergency spillway may overflow the protective berm onto the downstream slope of the dam.

An old pumphouse (Photo 17) was located downstream of the dam just beyond the toe of slope, (Plate 3). The pumphouse contained a pump and valves which appeared to have not been used for many years. The interior of the pumphouse was dry. Connecting piping to the pump and valves was not observable. There appeared to be no electrical connections to the pumphouse.

d. Geology. The soils surrounding the dam and reservoir consist of clayey silt to silty clay (CL-ML, to CL) to a depth of 8 inches, overlying silty clay (CL), with clayey sand (SC), clayey gravel (GC) and highly plastic clay (CH) present below a depth of 40 inches. The foundation of the dam is silty clay (CL) to clayey silt (CL-ML) as are both abutments. The emergency spillway was cut through the same material.

Based on visual examination of auger samples taken near the crest of the dam, the embankment material consists of silty clay (CL).

e. Reservoir Area. No slumping or slides of the reservoir banks were observed. The lake was noted to be clean with no siltation.

f. Downstream Channel. The principal spillway discharges to a small pond, then to the natural stream. The emergency spillway discharges to a brush and tree covered slope, then to a small pond and to the natural stream below. The natural stream is tree lined and leads to a culvert under Highways 740 and 63.

3.2 EVALUATION

The various deficiencies observed at the time of the inspection are not believed to represent an immediate safety hazard. They do, however, warrant monitoring and control or repair.

It is believed by the inspection team that the erosion and sloughing of the upstream face is due to wave action when the lake level is higher than the riprap. This condition will continue to worsen as long as the water level is allowed to rise above the slope protection.

The growth of trees and brush and the uncut grass, if allowed to go unchecked, could cause deterioration of the embankment. The roots of trees can loosen the embankment material and also can leave voids through which water can pass. Brush on the dam prevents inspection of the embankment and kills the smaller grasses whose roots are more effective in protecting the surface soil of the slope from erosion. The brush and tall uncut grass provides habitat for burrowing animals which can damage the embankment.

The area of seepage on the downstream slope which was observed should be monitored regularly for quality and quantity. Seepage can cause internal erosion creating cavities and underground channels, thereby weakening the embankment and/or abutments. It is believed that the wet area on the right abutment is seepage through an old spillway channel that has been filled. If so, it probably does not constitute a safety deficiency but is a nuisance.

Burrowing animals will continue to damage the embankment if no program is undertaken to eliminate them. Piping failure of the embankment has resulted in similar small earth dams due to burrowing animal damage.

SECTION 4 - OPERATIONAL PROCEDURES

4.1 PROCEDURES

The pool is primarily controlled by rainfall, runoff, evaporation, transpiration, and capacity of the uncontrolled spillway.

4.2 MAINTENANCE OF DAM

The only maintenance evident was that the grass on the crest of the dam had been mowed and trees had been removed from under the powerline that crosses a part of the downstream slope. Mr. Platt indicated that the residents of the lake area mow the crest and that the power company has removed trees along its right of way. He also indicated that silt had been removed from the lake area by dredging.

4.3 MAINTENANCE OF OPERATING FACILITIES

No operating facilities exist.

4.4 DESCRIPTION OF ANY WARNING SYSTEM IN EFFECT

There is no existing warning system or preplanned scheme for alerting downstream residents for this dam.

4.5 EVALUATION

The existing maintenance effort has not been adequate in preserving the dam in a satisfactory condition.

SECTION 5 - HYDRAULIC/HYDROLOGIC

5.1 EVALUATION OF FEATURES

- a. Design Data. No design data were available.
- b. Experience Data. The drainage area and lake surface area are developed from USGS Columbia (1967, Photo revised 1974), Mo. Quadrangle Map. The dam layout is from a survey made during the inspection.
- c. Visual Observations.
 - (1) The principal spillway appears to be in good condition. The lake level at the time of the inspection (El. 681.1) was below the principal spillway pipe inlet. There were no obstructions to flow in the downstream channel. The existence of the steel grate at the top of the inlet has no appreciable effect on discharge through the principal spillway.
 - (2) The emergency spillway for this dam consists of a grass-lined open channel cut through the left abutment.
 - (3) Excessive discharges through the emergency spillway could endanger the integrity of the dam, due to potential for overflow across downstream embankment slope.
- d. Overtopping Potential. The spillways will not pass the probable maximum flood without overtopping the dam. The probable maximum flood is defined as the flood discharge that may be expected from the most severe combination of critical meteorologic and hydrologic conditions that are reasonably possible in the region. The spillways will pass 10 percent of the probable maximum flood without overtopping the dam. The spillways will not pass the one percent chance flood (100-year flood) developed from a 24-hour, one percent chance rainfall, but will pass the 10 percent chance flood (10-year flood) developed from a 24-hour, 10 percent chance rainfall. According to the recommended guidelines from the Department of the Army, Office of the Chief of Engineers, a high hazard dam of small size should pass 50 to 100 percent of the probable maximum flood. Considering the downstream hazard, the appropriate spillway design flood should be 100 percent of the probable maximum flood. The portion of the estimated peak discharge of 50 percent of the probable maximum flood overtopping the dam would be 506 cfs of the total discharge from the reservoir of 592 cfs. The estimated duration of overtopping is 5.2 hours with a maximum height of 0.7 feet. The portion of the estimated peak discharge of the probable maximum flood overtopping the dam would be 1,105 cfs of the total discharge from the reservoir of 1,215 cfs. The estimated duration of overtopping is 6.4 hours with a maximum height of 1.0 feet. The embankment could be jeopardized should overtopping occur for these periods of time.

According to the St. Louis District, Corps of Engineers, the effect from rupture of the dam could extend approximately two miles downstream of the dam. Four dwellings, one building, three apartment buildings, a gasoline station, a trailer court consisting of approximately 15 trailers, and the junction of Highways 740 and 63 are located within the estimated damage zone, and lives could be lost should failure of the dam occur. Contents of the estimated downstream damage zone were verified by the inspection team. Lake Cyrene Dam and the downstream damage zone lie within the City of Columbia. Floodplain development below the dam is regulated by the city in accordance with the requirements of the Flood Insurance Program.

SECTION 6 - STRUCTURAL STABILITY

6.1 EVALUATION OF STRUCTURAL STABILITY

a. Visual Observations. Visual observations of conditions which affect the structural stability of this dam are discussed in Section 3, paragraph 3.1b.

b. Design and Construction Data. No design data relating to the structural stability of the dam were found. Seepage and stability analyses comparable to the requirements of the "Recommended Guidelines for Safety Inspection of Dams" were not available, which is considered a deficiency.

c. Operating Records. No operational records exist.

d. Postconstruction Changes. According to Mr. William Platt, in 1970 a rock lined spillway channel through the right abutment was filled in with clay and a drop inlet and 18-inch pipe was installed through the dam. At a later date, the inlet crest of the drop inlet was raised by adding a section of corrugated metal pipe to the top of the drop inlet.

e. Seismic Stability. The dam is located in Seismic Zone 1 which is a zone of minor seismic risk. A properly designed and constructed earth dam using sound engineering principles and conservatism should pose no serious stability problems during earthquakes in this zone. The seismic stability of an earth dam is dependent upon a number of factors: embankment and foundation material classifications and shear strengths; abutment materials, conditions, and strengths; embankment zoning; and embankment geometry. Adequate descriptions of embankment design parameters, foundation and abutment conditions, or static stability analyses to assess the seismic stability of this embankment were not available and therefore no inferences will be made regarding the seismic stability. An assessment of the seismic stability should be included as part of the stability analysis required by the guidelines.

SECTION 7 - ASSESSMENT/REMEDIAL MEASURES

7.1 DAM ASSESSMENT

a. Safety. Several conditions observed during the visual inspection by the inspection team should be corrected or monitored and controlled. These are erosion and sloughing on the upstream face, seepage on the downstream slope, the growth of and trees on both the upstream and downstream faces of the dam, and animal burrows in the embankment. Seepage and stability analyses comparable to the requirements of the "Recommended Guidelines for Safety Inspection of Dams" were not available, which is considered a deficiency.

b. Adequacy of Information. Due to the absence of engineering design data, the conclusions in this report were based only on performance history and visual conditions. The inspection team considers that these data are sufficient to support the conclusions herein. Seepage and stability analyses comparable to the requirements of the "Recommended Guidelines for Safety Inspection of Dams" were not available, which is considered a deficiency.

c. Urgency. It is the opinion of the inspection team that a program should be developed as soon as possible to implement remedial measures recommended in paragraph 7.2b. If the safety deficiencies listed in paragraph 7.1a are not corrected, they will continue to deteriorate and lead to a serious potential of failure. The item recommended in paragraph 7.2a should be pursued on a high priority basis.

d. Necessity for Phase II. The Phase I investigation does not raise any serious questions relating to the safety of the dam nor does it identify any serious dangers which would require a Phase II investigation. However, the additional analyses noted in paragraph 2.5b are necessary for compliance with the guidelines.

e. Seismic Stability. This dam is located in Seismic Zone 1. Adequate description of embankment design parameters, foundation and abutment conditions, or static stability analyses to assess the seismic stability of this embankment were not available and therefore no inferences will be made regarding the seismic stability. An assessment of the seismic stability should be included as part of the recommended stability analysis.

7.2 REMEDIAL MEASURES

a. Alternatives. Spillway capacity and/or the reservoir storage volume would need to be increased or the lake level would need to be permanently lowered to increase available flood storage in order to

effectively pass the spillway design flood. Spillway capacity could be increased by modifying the existing grass-lined emergency spillway or by increasing the principal spillway pipe size. The storage volume could be increased by raising the crest of the dam or by lowering the lake level by lowering the principal spillway inlet.

b. Operation and Maintenance Procedures. The following operation and maintenance procedures are recommended and should be carried out under the direction of a professional engineer experienced in the design, construction, and maintenance of earth dams.

(1) The erosion and sloughing on the upstream face should be repaired and the slope redressed. Riprap should be placed on the upstream face to an elevation above normal lake level to prevent erosion of the embankment material.

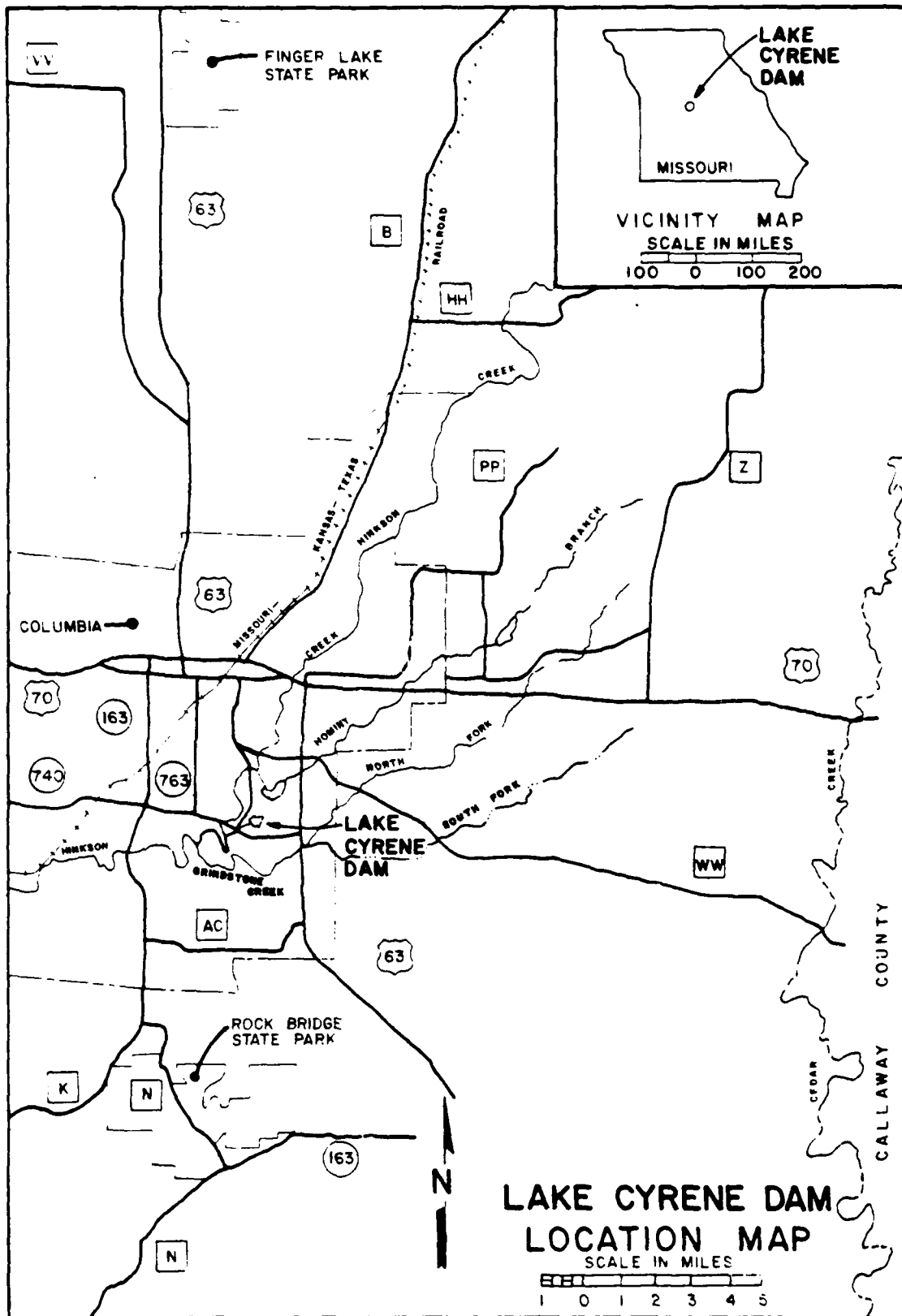
(2) The seepage areas noted during the visual inspection should be closely monitored and documented as to quantity and quality of flow. Any significant changes should be evaluated.

(3) A maintenance program should be formulated and implemented to remove and control the growth of trees on the embankments. Grass/weed cover on the embankments should be cut periodically.

(4) The animal burrows in the embankment should be repaired since they can contribute to the occurrence of piping. Control measures should be implemented to discourage animal activity in the area. The embankment slope should be monitored by a qualified engineer during repair of the embankment.

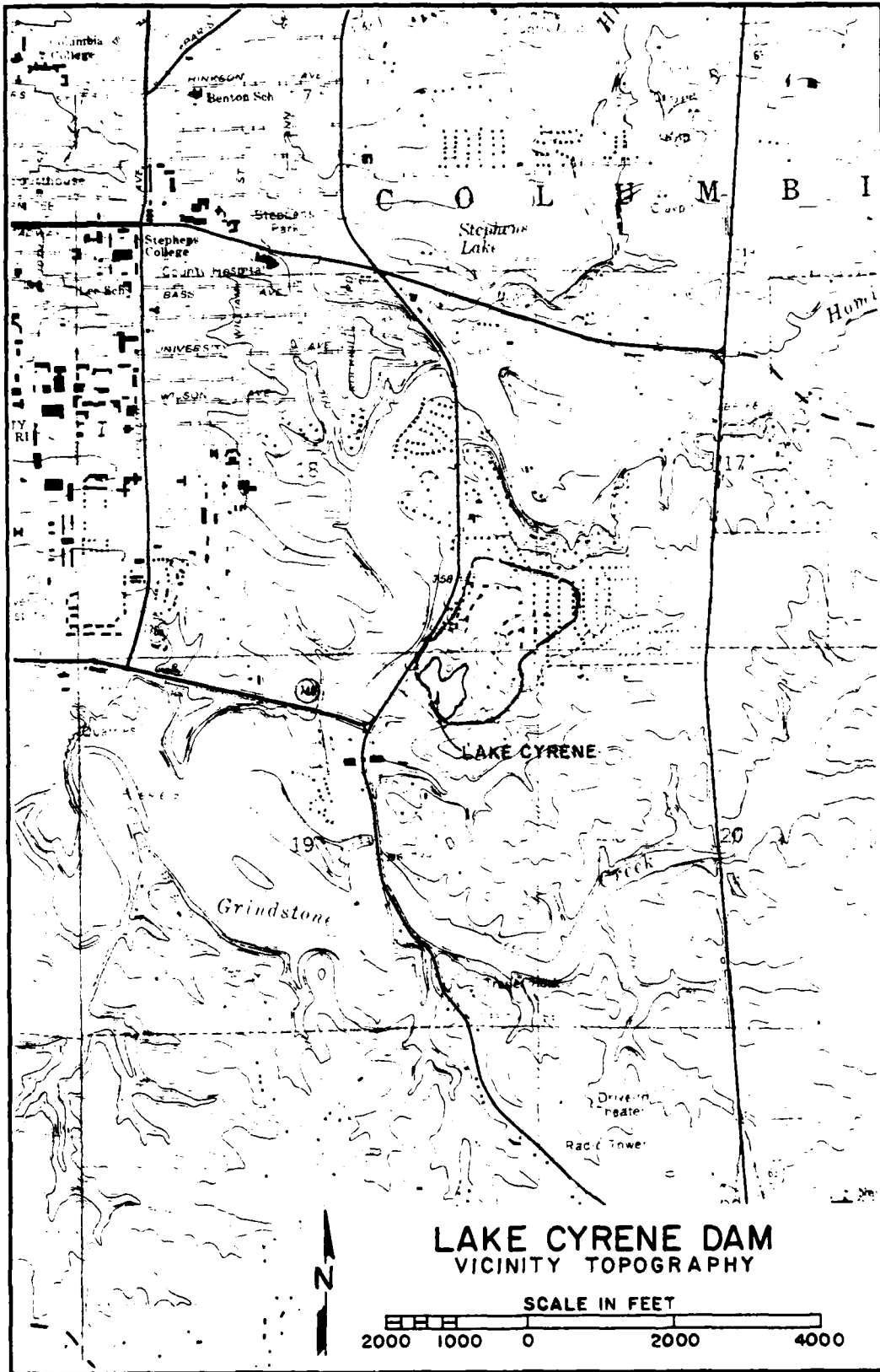
(5) Seepage and stability analyses should be performed.

(6) A detailed inspection of the dam should be made periodically and the findings of the inspections should be documented and made a matter of record. More frequent inspections may be required if additional deficiencies are observed or the severity of the reported deficiencies increase.



**LAKE CYRENE DAM
LOCATION MAP**
SCALE IN MILES
0 1 2 3 4 5

PLATE 1



**LAKE CYRENE DAM
VICINITY TOPOGRAPHY**

SCALE IN FEET



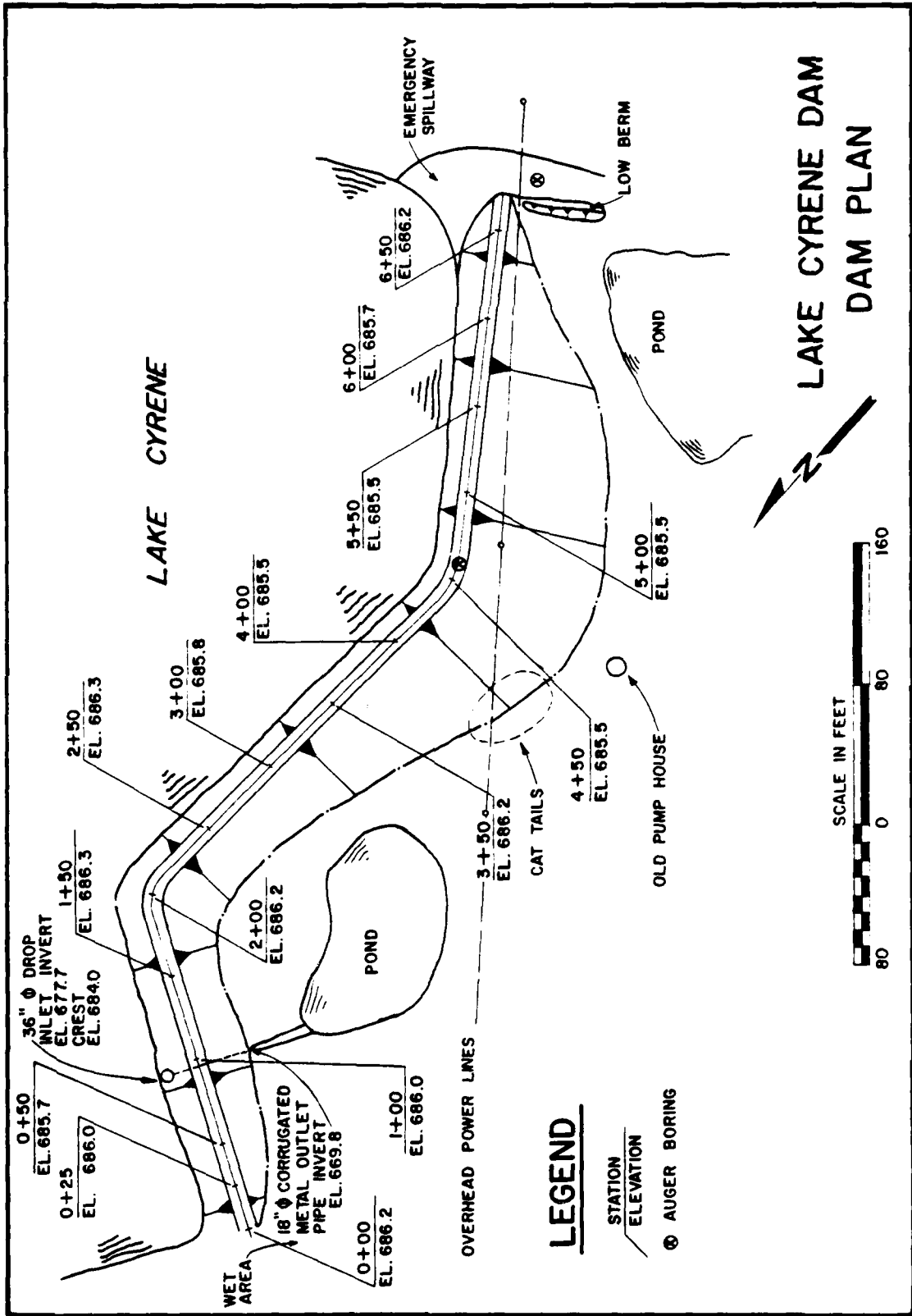
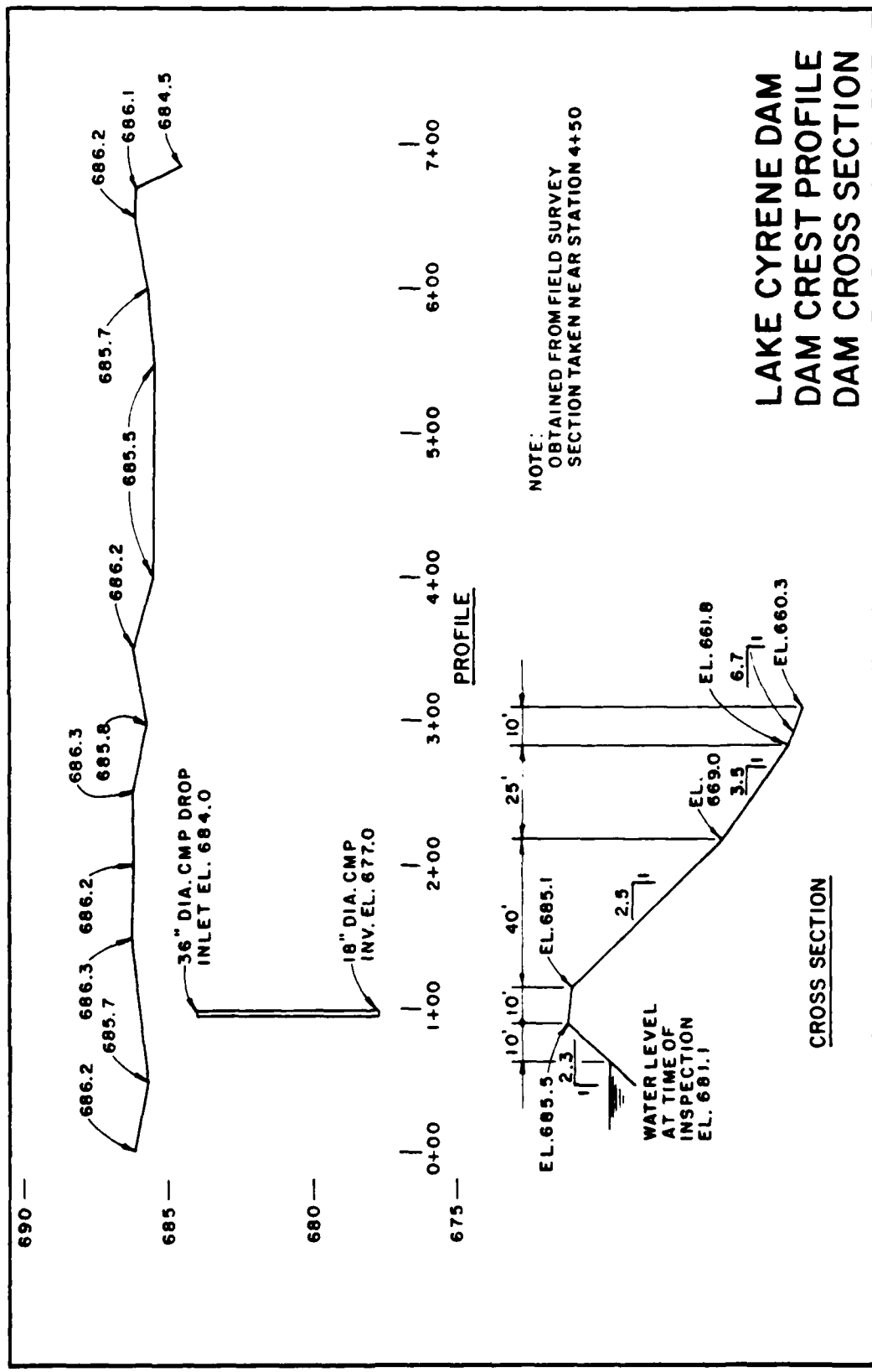
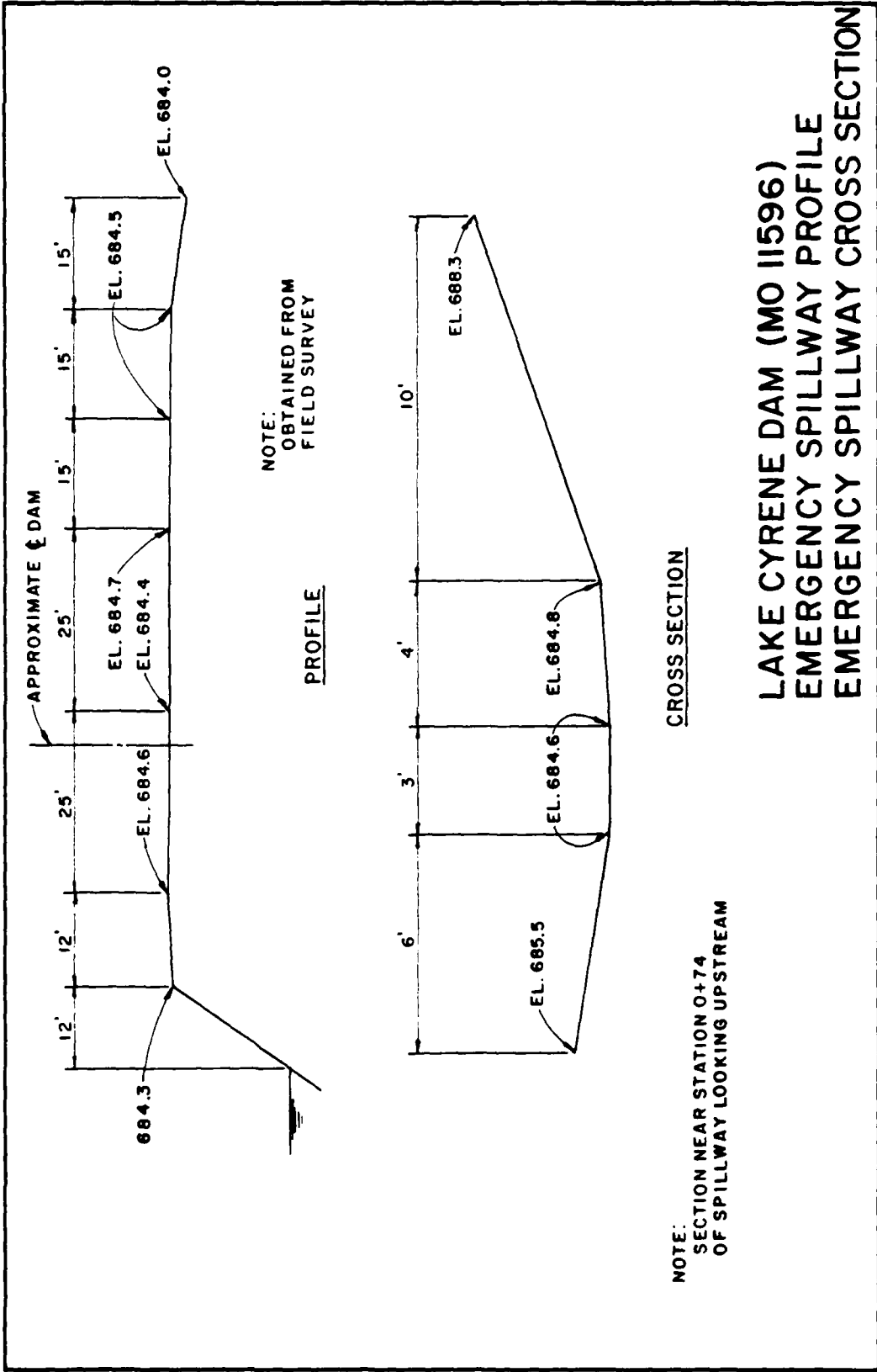


PLATE 3



**LAKE CYRENE DAM
DAM CREST PROFILE
DAM CROSS SECTION**



**LAKE CYRENE DAM (MO 11596)
EMERGENCY SPILLWAY PROFILE
EMERGENCY SPILLWAY CROSS SECTION**

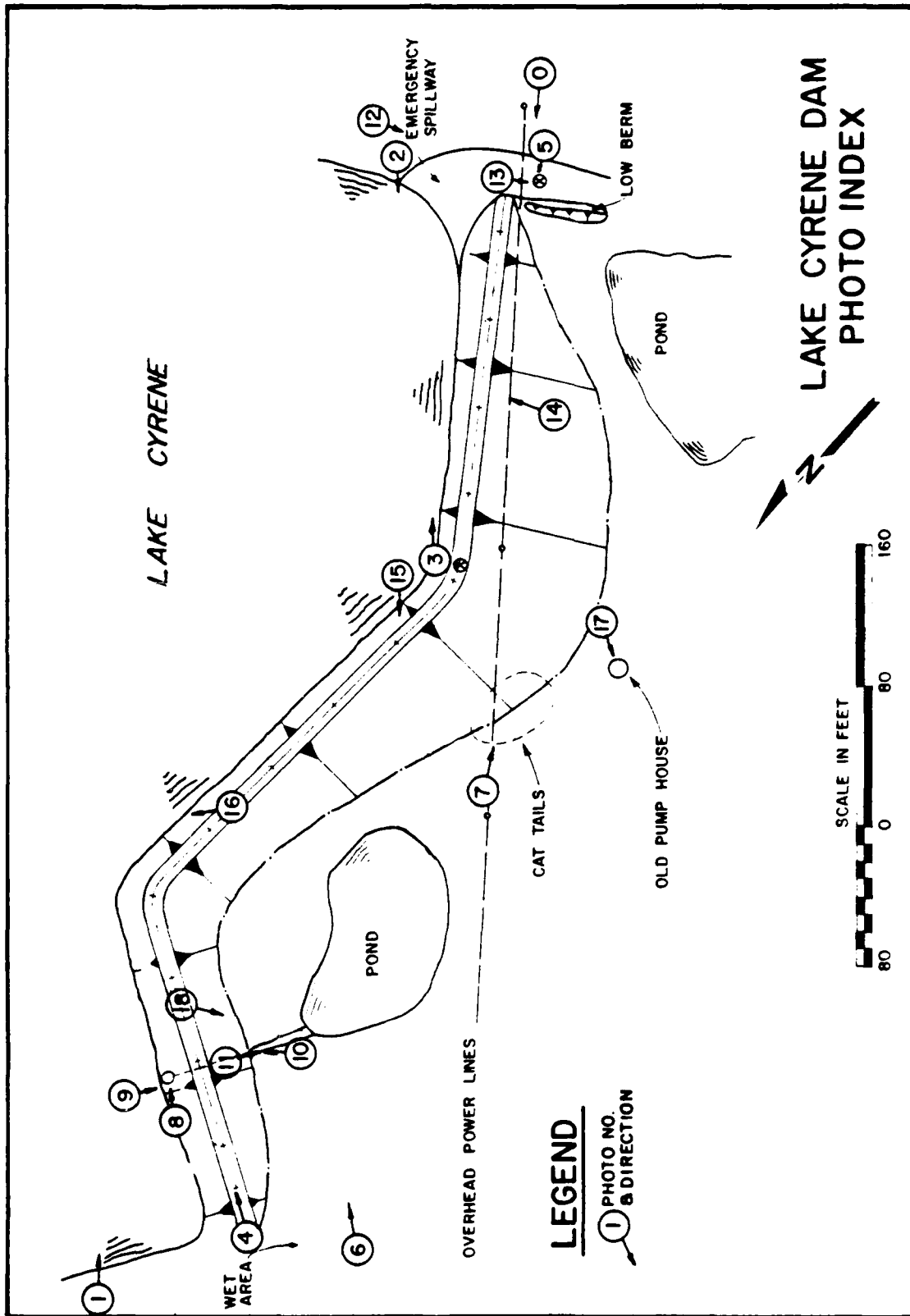


PLATE 6



PHOTO 1: UPSTREAM FACE OF DAM WEST END



PHOTO 2: UPSTREAM FACE OF DAM AT MID-DAM

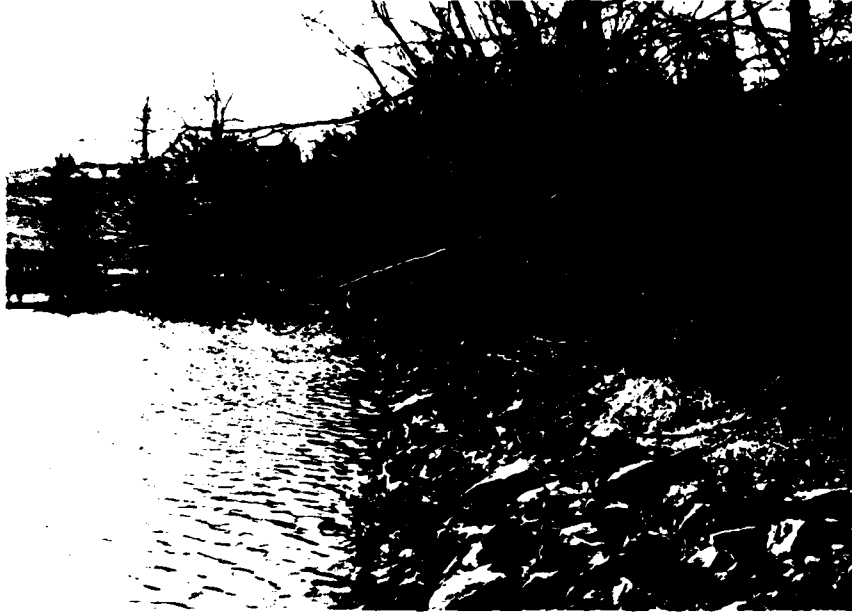


PHOTO 3: UPSTREAM FACE OF DAM EAST END



PHOTO 4: CREST OF DAM LOOKING EAST



PHOTO 5: CREST OF DAM LOOKING WEST



PHOTO 6: DOWNSTREAM FACE OF DAM LOOKING EAST



PHOTO 7: DOWNSTREAM FACE OF DAM AT MEADON



PHOTO 8: DROP INLET TO PRINCIPAL SPILLWAY

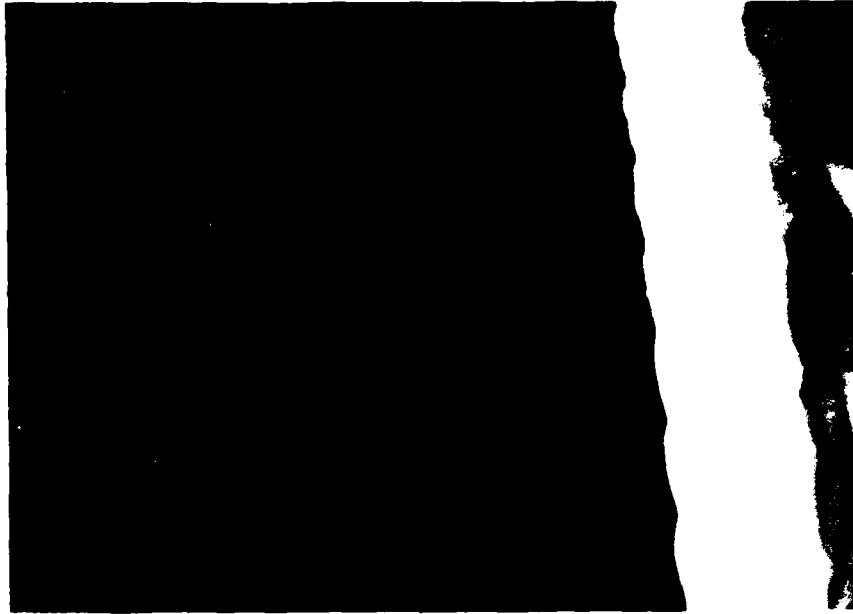


PHOTO 9: OUTLET FROM DROP INLET



PHOTO 10: DOWNSTREAM END OF PRINCIPAL SPILLWAY PIPE



PHOTO 11: PRINCIPAL SPILLWAY OUTLET AND DOWNSTREAM CHANNEL



PHOTO 12: OVERVIEW OF EMERGENCY SPILLWAY



PHOTO 13: EMERGENCY SPILLWAY CHANNEL LOOKING DOWN



PHOTO 14: ANIMAL BURROW ON DOWNSTREAM FACE OF DAM



PHOTO 15: EROSION OF UPSTREAM FACE OF DAM



PHOTO 16: UPSTREAM WATERSHED AREA



PHOTO 17: ABANDONED PUMP HOUSE DOWNSTREAM OF DAM



PHOTO 18: AREA DOWNSTREAM OF DAM

APPENDIX A
HYDROLOGIC AND HYDRAULIC ANALYSES

HYDROLOGIC AND HYDRAULIC ANALYSES

To determine the overtopping potential, flood routings were performed by applying the Probable Maximum Precipitation (PMP) to a synthetic unit hydrograph to develop the inflow hydrograph. The inflow hydrograph was then routed through the reservoir and spillway. The overtopping analysis was determined using the computer program HEC-1 (Dam Safety Version) (1).

The PMP was determined from regional charts prepared by the National Weather Service in "Hydrometeorological Report No. 33" (HMR-33) (2). Reduction factors were not applied. The rainfall distribution for the 24-hour PMP storm was determined according to the procedures outlined in HMR-33 and EM 1110-2-1411 (3). The Jefferson City, Missouri rainfall distribution (5 min. interval - 24 hours duration), as provided by the St. Louis District, Corps of Engineers, was used when the one percent and ten percent chance probability floods were routed through the reservoir and spillways.

The synthetic unit hydrograph for the watershed was developed by the computer program using the Soil Conservation Service (SCS) method (1,5). The parameters for the unit hydrograph are shown in Table 1. The formula from which the lag time was derived is noted in Table 1. The lag time was verified by the SCS curve number method (7).

The SCS curve number (CN) method was used in computing the infiltration losses for the rainfall-runoff relationship. The CN values used, and the result from the computer output, are shown in Table 2.

The reservoir routing was performed using the modified Puls Method. The initial reservoir pool elevation for the routing of each storm was assumed to be equivalent to the crest elevation of the principal spillway at elevation 684.0 feet m.s.l. in accordance with antecedent storm conditions AMC II, and AMC III preceding the one percent probability and probable maximum storms respectively, outlined by the U.S. Army Corps of Engineers, St. Louis District (4). The hydraulic capacity of the spillway and the storage capacity of the reservoir were defined by the elevation, surface area, storage, and discharge relationships shown in Table 3.

The rating curve for the spillway is shown in Table 4. The flow over the crest of the dam was determined using the non-level dam crest option (\$L and \$V cards) of the HEC-1 program. The program assumes critical flow over a broad-crested weir.

The result of the routing analysis indicates that a flood equivalent to a maximum of 10 percent of the PMF will not overtop the dam.

A summary of the routing analysis for different ratios of the PMF is shown in Table 5.

The computer input data and a summary of the output data are presented at the back of this appendix.

TABLE 1
SYNTHETIC UNIT HYDROGRAPH

Parameters:

Drainage Area (A)	64 acres
Length of Watercourse (L)	0.32 miles
Difference in Elevation (H)	91 feet
Time of concentration (T_c)	0.20 hours
Lag Time (L_g)	0.13 hours
Duration (D)	1.0 min. (use 5 min.)

<u>Time (Min.) *</u>	<u>Discharge (cfs) *</u>
0	0
5	126
10	282
15	205
20	88
25	40
30	18
35	8
40	4
45	2

* From HEC-1 computer output

FORMULAS USED:

$$T_c = (11.9 \times L^3/H)^{0.385} \quad (5)$$

$$D = 0.133 T_c$$

$$L_g = 0.6 T_c$$

TABLE 2
RAINFALL-RUNOFF VALUES

<u>Selected Storm Event</u>	<u>Storm Duration (Hours)</u>	<u>Rainfall (Inches)</u>	<u>Runoff (Inches)</u>	<u>Loss (Inches)</u>
PMP	24	32.37	31.35	1.02
1% Probability	24	7.44	5.33	2.11
10% Probability	24	5.21	3.27	1.94

Additional Data:

- 1) The soil associations in this watershed are Union, Mandeville, and Weldon (6).
 15 percent of drainage area is hydrologic soil Group B
 60 percent of drainage area is hydrologic soil Group C
 25 percent of drainage area is hydrologic soil Group D
 100 percent of the land use was residential
- 2) SCS Runoff Curve CN = 92 (AMC III) for the PMF.
- 3) SCS Runoff Curve CN = 82 (AMC II) for the one percent and ten percent probability floods (7).

TABLE 3
ELEVATION, SURFACE AREA, STORAGE, AND DISCHARGE RELATIONSHIPS

<u>Elevation (feet-MSL)</u>	<u>Lake Surface Area (acres)</u>	<u>Lake Storage (acre-ft)</u>	<u>Spillway Discharge (cfs)</u>
*684.0	6.3	50	0
**684.6	6.6	53	11
***685.5	6.8	60	50
685.9	6.9	63	70
686.2	7.0	65	86
686.5	7.2	67	110

- *Principal Spillway Inlet Crest Elevation
- **Emergency Spillway Crest Elevation
- ***Top of Dam Elevation

The relationships in Table 3 were developed from the Columbia, Missouri 7.5 minute quadrangle map and field measurements.

TABLE 4
SPILLWAY RATING CURVE

<u>Reservoir Elevation (ft)</u>	<u>Principal Spillway Discharge (cfs)</u>	<u>Emergency Spillway Discharge (cfs)</u>	<u>Total Spillway Discharge (cfs)</u>
*684.0	0	-	0
**684.6	11	0	11
685.0	21	5	26
***685.5	22	28	50
685.9	22	43	65
686.2	22	64	86
686.5	23	87	110

*Principal Spillway Inlet Crest Elevation

**Emergency Spillway Crest Elevation

***Top of Dam Elevation

METHOD USED:

Principal spillway release rates are based on the orifice equations of flow through a pipe culvert (8)

Emergency spillway release rates are based on the weir flow equation:

$$Q = CL \times (H^{3/2})$$

C = Coefficient of Discharge = 3.2 (9)

L = Length of Weir Crest = 9.4 (feet)

H = Head over Crest (feet)

TABLE 5
RESULTS OF FLOOD ROUTINGS

Ratio of PMF	Peak Inflow (cfs)	Peak Lake Elevation (ft.-msl)	Total Storage (ac.-ft.)	Peak Outflow (cfs)	Depth (ft.) Over Top of Dam	Duration of Over- topping (hrs)
-	0	*684.0	50	0	-	-
0.10	131	685.1	57	30	-	-
0.15	197	685.6	61	57	0.05	0.05
0.50	656	686.2	65	592	0.71	5.2
1.00	1,312	686.5	67	1,215	1.01	6.4

* Principal spillway inlet crest elevation

- (1) U.S. Army Corps of Engineers, Hydrologic Engineering Center, Flood Hydrograph Package (HEC-1), Dam Safety Version, July 1978, Modification April 1980, Davis, California.
- (2) HMR 33, Seasonal Variations of Probable Maximum Precipitation, East of the 105th Meridian for Areas 10 to 1000 Square Miles and Durations from 6 to 48 Hours, U.S. Department of Commerce, NOAA, National Weather Service, 1956.
- (3) EM-1110-2-1411, Standard Project Flood Determinations, U.S. Army Corps of Engineers, 26 March 1952.
- (4) U.S. Army Corps of Engineers, St. Louis District, Hydrologic/Hydraulic Standards, Phase I Safety Inspection of Non-Federal Dams, 22 August 1980.
- (5) U.S. Department of the Interior, Bureau of Reclamation, Design of Small Dams, 1974, Washington, D.C.
- (6) U.S. Department of Agriculture, Soil Conservation Service, Soil Survey of Boone County Missouri, 1962.
- (7) U.S. Department of Agriculture, Soil Conservation Service, National Engineering Handbook, Section 4, Hydrology, August 1972.
- (8) Ernest F. Brater and Horace Williams King, Handbook of Hydraulics, New York: McGraw-Hill Book Co., Inc., 6th Edition, 1976, pp. 4-22-4-24.
- (9) U.S. Department of the Interior, Geological Survey, Techniques of Water Resources Investigations of the United States Geological Survey, Chapter A5, 1967.

LISTING OF INPUT DATA

1 0 M21 02-1 0457
 2 A1 MISSOURI DAM INSPECTIONS
 3 A2 LAKE CHARLES DAM
 4 A3 PVI AND RATIOS
 5 U 26F
 6 B1 5
 7 J 1
 8 J10.1 0.15 5 0.2 0.25 0.3 0.35 0.4 0.5 1.0
 9 K HEAD
 10 R1 COMPUTE INFLOW HYDROGRAPH
 11 A 1 20.1 1.0
 12 P 24.0 1.1 12.0 17.0
 13 T
 14 B2 0.17 1.
 15 A
 16 R1 ROUTE HYDROGRAPH THROUGH DAM 1
 17 V 1
 18 V1 1
 19 V2 64.5 65. 96. 97. 44. 45. 46. 47. 48. 49. 50.
 20 V3 11. 12. 13. 14. 15. 16. 17. 18. 19. 20. 21. 22. 23. 24. 25. 26. 27. 28. 29. 30. 31. 32. 33. 34. 35. 36. 37. 38. 39. 40. 41. 42. 43. 44. 45. 46. 47. 48. 49. 50.
 21 V4 6.3 6.1 11.7
 22 V5 6.3 6.1 11.7
 23 V6 6.3 6.1 11.7
 24 V7 6.3 6.1 11.7
 25 V8 6.3 6.1 11.7
 26 V9 6.3 6.1 11.7
 27 V10 6.3 6.1 11.7
 28 V11 6.3 6.1 11.7
 29 V12 6.3 6.1 11.7
 30 V13 6.3 6.1 11.7
 31 V14 6.3 6.1 11.7
 32 V15 6.3 6.1 11.7
 33 V16 6.3 6.1 11.7
 34 V17 6.3 6.1 11.7
 35 V18 6.3 6.1 11.7
 36 V19 6.3 6.1 11.7
 37 V20 6.3 6.1 11.7
 38 V21 6.3 6.1 11.7
 39 V22 6.3 6.1 11.7
 40 V23 6.3 6.1 11.7
 41 V24 6.3 6.1 11.7
 42 V25 6.3 6.1 11.7
 43 V26 6.3 6.1 11.7
 44 V27 6.3 6.1 11.7
 45 V28 6.3 6.1 11.7
 46 V29 6.3 6.1 11.7
 47 V30 6.3 6.1 11.7
 48 V31 6.3 6.1 11.7
 49 V32 6.3 6.1 11.7
 50 V33 6.3 6.1 11.7

BASG.A SYSLEP=007. 0420010400
 FAC WARNING
 BASG.A SYSLEP=007. 0420010400
 FAC WARNING
 BASG.A SYSLEP=021. 0402000400
 FAC WARNING
 BASG.T 2.
 BASG.T 3.
 BASG.T 4.
 BASG.T 7.
 BASG.T 10.
 BASG.T 11.
 BASG.T 13.
 BASG.T 14.

MISSOURI DAM INSPECTIONS
 LAKE CROWN DAM
 FPM AND RATIOS

NO VNR NMIN IDAY IMR IMIN IETRC IFLT IPRT NSTAN
 200 3 2 2 0 0 3 0 3 0

JOPER MAT LROPT TRACE
 5 2 0 0

MULTI-PLAN ANALYSES TO BE PERFORMED
 NPLAN= 1 NPTIO= 9 LRTO= 1
 RTICS= .10 .15 .20 .25 .30 .35 .40 .45 .50 1.00

***** SUB-AREA RUN-OFF COMPUTATION *****

COMPUTE INFLOW HYDROGRAPH

ISTRN IICPP IFCOM ITRPE IJPLT JPRT IMAPE IJSTAGE IAUTO
 HEAD 0 0 0 0 0 0 0 0

HYDROGRAPH DATA
 IMT06 IIMG TAFEA SWAP TRSDP TASCPC RATIO ISNOH ISAME LOCAL
 1 2 .10 .00 .10 1.00 .000 2 1 0

PRECIP DATA
 SPTS PPS R6 R1 R24 R48 R72 R96
 .00 24.96 101.00 120.00 130.00 .00 .00 .00

LOSS DATA
 LCOFT STRFN DLTRF RTIOL ERATN STRKS RTIOW STRYL CNSTL ALSPK RTIMP
 0 .00 .00 1.00 .00 .00 1.00 -1.00 -92.00 .00

CURVE NO = -02.00 WEIKNSS = -1.00 EFFLCT CN = 02.00

UNIT HYDROGRAPH DATA
 TC= .00 LAG= .13

RECCESSION DATA
 SRTCN= .00 GRCSM= .00 RTIOW= 1.00

TIME INCREMENT TOO LARGE--(ANG IS GT LAG/2)

UNIT HYDROGRAPH IC END OF PERIOD ORDINATES, TC= 1A. .00 HOURS, LAG= 4. .13 VOL= 1.00
 124. 204. 8A. 40. 8. 2.

126. UNIT HYDROGRAPH 10 END OF PERIOD ORDINATES, TC= 18. .50 HOURS, LAG= 4. .75 VOL= 1.00
 282. 80. 2. 0.

B L A C K R E V E A T C H PROJECT 94572 DATE 23 MAR 81 PAGE 4
 FLOOD HYDROGRAPH PACKAGE - MEC-1 PROGPAP M21/A2-1V TIME 14:19:17 CASE

TIME	HR.	MIN.	PERIOD	RAINF	EXCS	LOSS	COMP	END-OF-PERIOD FLOW			PERIOD	RAIN	EXCS	LOSS	COMP
								PO.0A	HR.	PM.					
1:01	05		1	.01	.20	.01	0.	1:01	12:05	145	.21	.21	.00	68.	
1:01	10		2	.01	.20	.01	0.	1:01	12:10	146	.21	.21	.00	108.	
1:01	15		3	.01	.20	.01	0.	1:01	12:15	147	.21	.21	.00	177.	
1:01	20		4	.01	.20	.01	0.	1:01	12:20	148	.21	.21	.00	149.	
1:01	25		5	.01	.20	.01	0.	1:01	12:25	149	.21	.21	.00	155.	
1:01	30		6	.01	.20	.01	0.	1:01	12:30	150	.21	.21	.00	156.	
1:01	35		7	.01	.20	.01	0.	1:01	12:35	151	.21	.21	.00	159.	
1:01	40		8	.01	.20	.01	0.	1:01	12:40	152	.21	.21	.00	160.	
1:01	45		9	.01	.20	.01	0.	1:01	12:45	153	.21	.21	.00	163.	
1:01	50		10	.01	.20	.01	0.	1:01	12:50	154	.21	.21	.00	160.	
1:01	55		11	.01	.20	.01	0.	1:01	12:55	155	.21	.21	.00	163.	
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1:01	05		13	.01	.20	.01	0.	1:01	13:05	157	.25	.25	.00	176.	
1:01	10		14	.01	.20	.01	0.	1:01	13:10	158	.25	.25	.00	176.	
1:01	15		15	.01	.20	.01	0.	1:01	13:15	159	.25	.25	.00	176.	
1:01	20		16	.01	.20	.01	0.	1:01	13:20	160	.25	.25	.00	176.	
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1:01	35		19	.01	.20	.01	0.	1:01	13:35	163	.25	.25	.00	193.	
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1:01	45		21	.01	.20	.01	0.	1:01	13:45	165	.25	.25	.00	193.	
1:01	50		22	.01	.20	.01	0.	1:01	13:50	166	.25	.25	.00	193.	
1:01	55		23	.01	.20	.01	0.	1:01	13:55	167	.25	.25	.00	193.	
1:01	00		24	.01	.20	.01	0.	1:01	14:00	168	.25	.25	.00	194.	
1:01	05		25	.01	.20	.01	0.	1:01	14:05	169	.31	.31	.00	201.	
1:01	10		26	.01	.20	.01	0.	1:01	14:10	170	.31	.31	.00	219.	
1:01	15		27	.01	.20	.01	0.	1:01	14:15	171	.31	.31	.00	232.	
1:01	20		28	.01	.20	.01	0.	1:01	14:20	172	.31	.31	.00	278.	
1:01	25		29	.01	.20	.01	0.	1:01	14:25	173	.31	.31	.00	240.	
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1:01	35		31	.01	.20	.01	0.	1:01	14:35	175	.31	.31	.00	242.	
1:01	40		32	.01	.20	.01	0.	1:01	14:40	176	.31	.31	.00	242.	
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1:01	20		40	.01	.20	.01	0.	1:01	15:20	184	.57	.57	.00	258.	
1:01	25		41	.01	.20	.01	0.	1:01	15:25	185	.67	.67	.00	375.	
1:01	30		42	.01	.20	.01	0.	1:01	15:30	186	1.62	1.62	.00	567.	
1:01	35		43	.01	.20	.01	0.	1:01	15:35	187	2.68	2.68	.00	1008.	
1:01	40		44	.01	.20	.01	0.	1:01	15:40	188	1.05	1.05	.00	1312.	
1:01	45		45	.01	.20	.01	0.	1:01	15:45	189	.67	.67	.00	1113.	
1:01	50		46	.01	.20	.01	0.	1:01	15:50	190	.57	.57	.00	794.	
1:01	55		47	.01	.20	.01	0.	1:01	15:55	191	.38	.38	.00	584.	
1:01	00		48	.01	.20	.01	0.	1:01	16:00	192	.38	.38	.00	440.	
1:01	05		49	.01	.20	.01	0.	1:01	16:05	193	.29	.29	.00	349.	
1:01	10		50	.01	.20	.01	0.	1:01	16:10	194	.20	.20	.00	289.	
1:01	15		51	.01	.20	.01	0.	1:01	16:15	195	.20	.20	.00	254.	
1:01	20		52	.01	.20	.01	0.	1:01	16:20	196	.20	.20	.00	276.	
1:01	25		53	.01	.20	.01	0.	1:01	16:25	197	.20	.20	.00	251.	

1.01	4.30	14	.01	.01	.01	7.	1.01	16.30	158	.29	.00	229.
1.01	4.35	55	.01	.01	.01	7.	1.01	16.35	199	.29	.00	227.
1.01	4.40	56	.01	.01	.01	7.	1.01	16.40	200	.29	.00	227.
1.01	4.45	57	.01	.01	.01	7.	1.01	16.45	201	.29	.00	227.
1.01	4.50	58	.01	.01	.01	7.	1.01	16.50	202	.29	.00	227.
1.01	4.55	59	.01	.01	.01	7.	1.01	16.55	203	.29	.00	227.
1.01	4.60	60	.01	.01	.01	7.	1.01	17.00	204	.29	.00	227.
1.01	4.65	61	.01	.01	.01	7.	1.01	17.05	205	.29	.00	227.
1.01	4.70	62	.01	.01	.01	7.	1.01	17.10	206	.29	.00	227.
1.01	4.75	63	.01	.01	.01	7.	1.01	17.15	207	.29	.00	227.
1.01	4.80	64	.01	.01	.01	7.	1.01	17.20	208	.29	.00	227.
1.01	4.85	65	.01	.01	.01	7.	1.01	17.25	209	.29	.00	227.
1.01	4.90	66	.01	.01	.01	7.	1.01	17.30	210	.29	.00	227.
1.01	4.95	67	.01	.01	.01	6.	1.01	17.35	211	.29	.00	227.
1.01	5.00	68	.01	.01	.01	6.	1.01	17.40	212	.29	.00	227.
1.01	5.05	69	.01	.01	.01	6.	1.01	17.45	213	.29	.00	227.
1.01	5.10	70	.01	.01	.01	6.	1.01	17.50	214	.29	.00	227.
1.01	5.15	71	.01	.01	.01	6.	1.01	17.55	215	.29	.00	227.
1.01	5.20	72	.01	.01	.01	6.	1.01	18.00	216	.29	.00	227.
1.01	5.25	73	.01	.01	.01	13.	1.01	18.05	217	.02	.00	152.
1.01	5.30	74	.01	.01	.01	24.	1.01	18.10	218	.02	.00	17.
1.01	5.35	75	.01	.01	.01	32.	1.01	18.15	219	.02	.00	59.
1.01	5.40	76	.01	.01	.01	36.	1.01	18.20	220	.02	.00	31.
1.01	5.45	77	.01	.01	.01	35.	1.01	18.25	221	.02	.00	223.
1.01	5.50	78	.01	.01	.01	41.	1.01	18.30	222	.02	.00	19.
1.01	5.55	79	.01	.01	.01	41.	1.01	18.35	223	.02	.00	17.
1.01	5.60	80	.01	.01	.01	42.	1.01	18.40	224	.02	.00	17.
1.01	5.65	81	.01	.01	.01	42.	1.01	18.45	225	.02	.00	16.
1.01	5.70	82	.01	.01	.01	43.	1.01	18.50	226	.02	.00	16.
1.01	5.75	83	.01	.01	.01	44.	1.01	18.55	227	.02	.00	16.
1.01	5.80	84	.01	.01	.01	44.	1.01	19.00	228	.02	.00	16.
1.01	5.85	85	.01	.01	.01	44.	1.01	19.05	229	.02	.00	16.
1.01	5.90	86	.01	.01	.01	45.	1.01	19.10	230	.02	.00	16.
1.01	5.95	87	.01	.01	.01	45.	1.01	19.15	231	.02	.00	16.
1.01	6.00	88	.01	.01	.01	45.	1.01	19.20	232	.02	.00	16.
1.01	6.05	89	.01	.01	.01	45.	1.01	19.25	233	.02	.00	16.
1.01	6.10	90	.01	.01	.01	46.	1.01	19.30	234	.02	.00	16.
1.01	6.15	91	.01	.01	.01	46.	1.01	19.35	235	.02	.00	16.
1.01	6.20	92	.01	.01	.01	46.	1.01	19.40	236	.02	.00	16.
1.01	6.25	93	.01	.01	.01	46.	1.01	19.45	237	.02	.00	16.
1.01	6.30	94	.01	.01	.01	47.	1.01	19.50	238	.02	.00	16.
1.01	6.35	95	.01	.01	.01	47.	1.01	19.55	239	.02	.00	16.
1.01	6.40	96	.01	.01	.01	47.	1.01	20.00	240	.02	.00	16.
1.01	6.45	97	.01	.01	.01	47.	1.01	20.05	241	.02	.00	16.
1.01	6.50	98	.01	.01	.01	47.	1.01	20.10	242	.02	.00	16.
1.01	6.55	99	.01	.01	.01	47.	1.01	20.15	243	.02	.00	16.
1.01	6.60	100	.01	.01	.01	48.	1.01	20.20	244	.02	.00	16.
1.01	6.65	101	.01	.01	.01	48.	1.01	20.25	245	.02	.00	16.
1.01	6.70	102	.01	.01	.01	48.	1.01	20.30	246	.02	.00	16.
1.01	6.75	103	.01	.01	.01	48.	1.01	20.35	247	.02	.00	16.
1.01	6.80	104	.01	.01	.01	48.	1.01	20.40	248	.02	.00	16.
1.01	6.85	105	.01	.01	.01	48.	1.01	20.45	249	.02	.00	16.
1.01	6.90	106	.01	.01	.01	48.	1.01	20.50	250	.02	.00	16.
1.01	6.95	107	.01	.01	.01	48.	1.01	20.55	251	.02	.00	16.
1.01	7.00	108	.01	.01	.01	48.	1.01	21.00	252	.02	.00	16.
1.01	7.05	109	.01	.01	.01	48.	1.01	21.05	253	.02	.00	16.

TIME	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
1.00 8.50	106	42.	1.01 20.50	254	16.
1.01 9.05	107	42.	1.01 20.55	255	16.
1.02 9.50	108	48.	1.01 21.20	256	16.
1.03 10.05	107	48.	1.01 21.25	257	16.
1.04 10.20	107	49.	1.01 21.30	258	16.
1.05 10.35	107	49.	1.01 21.35	259	16.
1.06 10.50	107	49.	1.01 21.40	260	16.
1.07 11.05	107	49.	1.01 21.45	261	16.
1.08 11.20	107	49.	1.01 21.50	262	16.
1.09 11.35	107	49.	1.01 21.55	263	16.
1.10 11.50	107	49.	1.01 22.00	264	16.
1.11 12.05	107	49.	1.01 22.05	265	16.
1.12 12.20	107	49.	1.01 22.10	266	16.
1.13 12.35	107	49.	1.01 22.15	267	16.
1.14 12.50	107	49.	1.01 22.20	268	16.
1.15 13.05	107	49.	1.01 22.25	269	16.
1.16 13.20	107	49.	1.01 22.30	270	16.
1.17 13.35	107	49.	1.01 22.35	271	16.
1.18 13.50	107	49.	1.01 22.40	272	16.
1.19 14.05	107	49.	1.01 22.45	273	16.
1.20 14.20	107	49.	1.01 22.50	274	16.
1.21 14.35	107	49.	1.01 22.55	275	16.
1.22 14.50	107	49.	1.01 23.00	276	16.
1.23 15.05	107	49.	1.01 23.05	277	16.
1.24 15.20	107	49.	1.01 23.10	278	16.
1.25 15.35	107	49.	1.01 23.15	279	16.
1.26 15.50	107	49.	1.01 23.20	280	16.
1.27 16.05	107	49.	1.01 23.25	281	16.
1.28 16.20	107	49.	1.01 23.30	282	16.
1.29 16.35	107	49.	1.01 23.35	283	16.
1.30 16.50	107	49.	1.01 23.40	284	16.
1.31 17.05	107	49.	1.01 23.45	285	16.
1.32 17.20	107	49.	1.01 23.50	286	16.
1.33 17.35	107	49.	1.01 23.55	287	16.
1.34 17.50	107	49.	1.02	288	16.

SUP 32.37 31.75 1.02 24233.
 (822.3)(796.3)(26.3)(6P6.20)

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
1312.	267.	84.	84.	24229.
17.	8.	2.	2.	484.
	24.34	31.30	31.30	31.10
	639.83	793.13	793.13	793.13
	173.	167.	167.	167.
	164.	276.	276.	206.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 1

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
171.	27.	8.	8.	2423.
4.	1.	0.	0.	69.
	2.49	3.17	3.17	3.13
	63.17	79.51	79.51	79.51

CFS
 CMS
 INCHES
 AC-FT
 TMOUS CUP

AC-FT 15. 17. 17.
 THOUS CU M 16. 21. 21.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 2

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
197.	4.4	13.	13.	3734.
6.	1.	6.	0.	10.
	3.77	4.70	4.70	4.70
	94.77	119.27	119.27	119.27
AC-FT	26.	25.	25.	25.
THOUS CU M	25.	71.	71.	51.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 3

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
262.	53.	17.	17.	4944.
7.	2.	0.	0.	137.
	6.08	6.08	6.26	6.26
	120.37	159.03	159.03	159.03
AC-FT	27.	13.	13.	33.
THOUS CU M	13.	41.	41.	41.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 4

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
328.	42.	21.	21.	6582.
9.	2.	1.	1.	172.
	6.22	7.83	7.83	7.83
	157.56	198.79	198.79	198.79
AC-FT	43.	42.	42.	42.
THOUS CU M	41.	51.	51.	51.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 5

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
390.	40.	25.	25.	7269.
11.	2.	1.	1.	266.
	7.66	9.30	9.30	9.30
	189.55	228.54	228.54	228.54
AC-FT	42.	46.	46.	50.
THOUS CU M	49.	62.	62.	62.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 6

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
459.	54.	29.	29.	8480.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 6

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
657.	54.	79.	79.	8480.
13.	2.	1.	1.	240.
	8.71	10.96	10.96	10.96
	221.14	278.26	278.26	278.26
	46.	58.	58.	58.
	57.	72.	72.	72.

BLACK & VEATCH PROJECT 94571 DATE 23 MAR 81 PAGE 8
 FLOOD HYDROGRAPH PACKAGE - REC-1 PROGRAM 421/CZ-1V TIME 14:19:37 CASE

CFS	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
525.	107.	74.	74.	9492.
15.	5.	1.	1.	274.
	9.97	12.52	12.52	12.52
	272.77	318.65	318.65	318.65
	43.	57.	57.	57.
	65.	82.	82.	82.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 7

CFS	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
525.	107.	74.	74.	9492.
15.	5.	1.	1.	274.
	9.97	12.52	12.52	12.52
	272.77	318.65	318.65	318.65
	43.	57.	57.	57.
	65.	82.	82.	82.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 8

CFS	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
525.	107.	74.	74.	9492.
15.	5.	1.	1.	274.
	9.97	12.52	12.52	12.52
	272.77	318.65	318.65	318.65
	43.	57.	57.	57.
	65.	82.	82.	82.

HYDROGRAPH AT STA HEAD FOR PLAN 1, RTIO 9

CFS	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
525.	107.	74.	74.	9492.
15.	5.	1.	1.	274.
	9.97	12.52	12.52	12.52
	272.77	318.65	318.65	318.65
	43.	57.	57.	57.
	65.	82.	82.	82.

HYDROGRAPH ROUTING

ROUTE HYDROGRAPH THROUGH DAM

ISDA	ICUP	IFCN	ISDAE	JPLT	JPRY	ISDAE	IFAUTO
04K	1	0	0	0	0	0	0
		ROUTING DATA					

BLK BVEATCM
 FLOOD HYDROGRAPH PACKAGE - HEC-1

-LUGS CLUS AVG TRIS ISAPE IOFT IRPP LSTR
 .00 .000 .00 1 1 C W 0
 NSTPS NSTEL LAG APSEK R TSK STOKA ISPRAY
 1 0 .00V .000 .000 -PPK. -1

STAGE 684.00 685.00 686.00 687.00 688.00 689.00 690.00 692.00
 FLOW .00 11.00 26.00 75.00 148.00 278.00 434.00 625.00

SURFACE AREA 70 60 120
 CAPACITY 0 50 97 192
 ELEVATION 600 620 630

CRST LENGTH 150 215 300 425 550 650
 AT OR BELOW
 ELEVATION 685.5 685.7 686.0 686.4 686.8 690.0

DAM DATA
 TOPPEL EOOD TRPD DAMWID
 695.5 .00 .00 0.0

STATION DAM, PLAN 1, PATIC 1
 END-OF-PERIOD HYDROGRAPH ORBITATES

STATION	DAM, PLAN 1, PATIC 1	END-OF-PERIOD HYDROGRAPH ORBITATES
10	0	0
11	0	0
12	0	0
13	0	0
14	0	0
15	0	0
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17	0	0
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BLACK R V E A T C H
 PROJECT 0457: DATE 23 MAR 81 PAGE 10
 FLOOD HYDROGRAPH PACKAGE MEC-1 PROGRAM HET/22-IV TIME 14:19:17 CASE

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STAGE

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85	85	85	85	85	85
86	86	86	86	86	86
87	87	87	87	87	87
88	88	88	88	88	88
89	89	89	89	89	89
90	90	90	90	90	90
91	91	91	91	91	91
92	92	92	92	92	92
93	93	93	93	93	93
94	94	94	94	94	94
95	95	95	95	95	95
96	96	96	96	96	96
97	97	97	97	97	97
98	98	98	98	98	98
99	99	99	99	99	99
100	100	100	100	100	100

STATION	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
084.0	084.0	084.0	084.7	084.2
085.1	085.1	085.1	085.1	085.1
085.0	085.0	085.0	085.0	085.0
085.0	085.0	085.0	084.9	084.9
084.9	084.9	084.8	084.7	084.7
084.7	084.7	084.6	084.6	084.6
084.5	084.5	084.5	084.5	084.5
084.4	084.4	084.4	084.4	084.4
084.4	084.4	084.3	084.3	084.3
084.3	084.3	084.3	084.3	084.3
084.3	084.2	084.2	084.2	084.2

PEAK OUTFLOW IS 30. AT TIME 16.17 HOURS

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
30.	21.	6.	6.	2212.
1.	1.	0.	0.	63.
	1.91	2.66	2.64	2.90
	48.03	72.02	72.02	72.60
	1.	15.	15.	15.
	13.	19.	19.	19.

THOUS CU M
 AC-FT
 HP
 INCHES
 FPS
 CFS

STATION BAR, PLAN 1, RATIO 2
 END-OF-PELLOD HYDROGRAPH ORDINATES

STATION	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
084.0	084.0	084.0	084.7	084.2
085.1	085.1	085.1	085.1	085.1
085.0	085.0	085.0	085.0	085.0
085.0	085.0	085.0	084.9	084.9
084.9	084.9	084.8	084.7	084.7
084.7	084.7	084.6	084.6	084.6
084.5	084.5	084.5	084.5	084.5
084.4	084.4	084.4	084.4	084.4
084.4	084.4	084.3	084.3	084.3
084.3	084.3	084.3	084.3	084.3
084.3	084.2	084.2	084.2	084.2

PEAK OUTFLOW IS 457. AT TIME 15.75 HOURS

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
457.	98.	32.	32.	9240.
13.	3.	1.	1.	202.
	4.16	11.67	11.65	11.65
	272.61	303.52	303.52	103.53
	49.	60.	54.	66.
	60.	79.	79.	79.

STATION DAM, PLAN 1, RATIO P
 END-OF-PEPICE HYDROGRAPH ORDINATES

STATION	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
1.	12.	17.	17.	12.
2.	15.	16.	17.	15.
3.	16.	16.	17.	15.
4.	16.	16.	17.	15.
5.	16.	16.	17.	15.
6.	16.	16.	17.	15.
7.	16.	16.	17.	15.
8.	16.	16.	17.	15.
9.	16.	16.	17.	15.
10.	16.	16.	17.	15.
11.	16.	16.	17.	15.
12.	16.	16.	17.	15.
13.	16.	16.	17.	15.
14.	16.	16.	17.	15.
15.	16.	16.	17.	15.
16.	16.	16.	17.	15.
17.	16.	16.	17.	15.
18.	16.	16.	17.	15.
19.	16.	16.	17.	15.
20.	16.	16.	17.	15.
21.	16.	16.	17.	15.
22.	16.	16.	17.	15.
23.	16.	16.	17.	15.
24.	16.	16.	17.	15.
25.	16.	16.	17.	15.
26.	16.	16.	17.	15.
27.	16.	16.	17.	15.
28.	16.	16.	17.	15.
29.	16.	16.	17.	15.
30.	16.	16.	17.	15.
31.	16.	16.	17.	15.
32.	16.	16.	17.	15.
33.	16.	16.	17.	15.
34.	16.	16.	17.	15.
35.	16.	16.	17.	15.
36.	16.	16.	17.	15.
37.	16.	16.	17.	15.
38.	16.	16.	17.	15.
39.	16.	16.	17.	15.
40.	16.	16.	17.	15.
41.	16.	16.	17.	15.
42.	16.	16.	17.	15.
43.	16.	16.	17.	15.
44.	16.	16.	17.	15.
45.	16.	16.	17.	15.
46.	16.	16.	17.	15.
47.	16.	16.	17.	15.
48.	16.	16.	17.	15.
49.	16.	16.	17.	15.
50.	16.	16.	17.	15.
51.	16.	16.	17.	15.
52.	16.	16.	17.	15.
53.	16.	16.	17.	15.
54.	16.	16.	17.	15.
55.	16.	16.	17.	15.
56.	16.	16.	17.	15.
57.	16.	16.	17.	15.
58.	16.	16.	17.	15.
59.	16.	16.	17.	15.
60.	16.	16.	17.	15.
61.	16.	16.	17.	15.
62.	16.	16.	17.	15.
63.	16.	16.	17.	15.
64.	16.	16.	17.	15.
65.	16.	16.	17.	15.
66.	16.	16.	17.	15.
67.	16.	16.	17.	15.
68.	16.	16.	17.	15.
69.	16.	16.	17.	15.
70.	16.	16.	17.	15.
71.	16.	16.	17.	15.
72.	16.	16.	17.	15.
73.	16.	16.	17.	15.
74.	16.	16.	17.	15.
75.	16.	16.	17.	15.
76.	16.	16.	17.	15.
77.	16.	16.	17.	15.
78.	16.	16.	17.	15.
79.	16.	16.	17.	15.
80.	16.	16.	17.	15.
81.	16.	16.	17.	15.
82.	16.	16.	17.	15.
83.	16.	16.	17.	15.
84.	16.	16.	17.	15.
85.	16.	16.	17.	15.
86.	16.	16.	17.	15.
87.	16.	16.	17.	15.
88.	16.	16.	17.	15.
89.	16.	16.	17.	15.
90.	16.	16.	17.	15.
91.	16.	16.	17.	15.
92.	16.	16.	17.	15.
93.	16.	16.	17.	15.
94.	16.	16.	17.	15.
95.	16.	16.	17.	15.
96.	16.	16.	17.	15.
97.	16.	16.	17.	15.
98.	16.	16.	17.	15.
99.	16.	16.	17.	15.
100.	16.	16.	17.	15.

51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.
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51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.	51.
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STAGE																									
6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0	6F4.0
6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1	6F4.1
6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2	6F4.2
6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3	6F4.3

CFC 12.15 475. 92. 94. 23459.
 CFS 34. 8. 2. 4. 46.
 ENCLMS 24.66 20.36 20.36 20.36
 NP 625.99 771.16 771.16 771.16
 AC-17 178. 162. 162.
 INHLS (U M) 172. 200. 200.

PEAK FLOW AND STORAGE (END OF PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)
 AREA IN SQUARE FEET (SQARE KILOMETERS)

OPERATION	STATION	AREA	PLAN RATIO	1	2	3	4	5	6	7	8	9
HYDROGRAPH AT	HEAD	.10 (.28)	1	131. (3.72)	197. (5.57)	262. (7.47)	318. (9.03)	394. (11.15)	450. (13.00)	525. (14.86)	656. (18.58)	1312. (37.16)
	DAM	.10 (.28)	1	35. (.86)	57. (1.61)	147. (4.17)	239. (6.74)	325. (9.21)	395. (11.14)	462. (13.06)	592. (16.78)	1215. (34.47)

SUPPLY OF DAM SAFETY ANALYSIS

SUPPLY OF DAM SAFETY ANALYSIS

PLAN 1	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM	ELEVATION STORAGE OUTFLOW	MAXIMUM STORAGE AC-FT	MAXIMUM DEPTH AVER DAM	MAXIMUM OUTFLOW CFS	DURATION CYCL TCP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
010	694.00	694.00	695.50	50.	57.	.60	30.	.50	16.17	.90
015	695.55	695.55	697.05	50.	61.	.65	57.	.50	16.08	.90
020	697.10	697.10	698.60	50.	64.	.68	147.	1.58	15.87	.90
025	698.65	698.65	700.15	50.	67.	.71	230.	2.42	15.75	.90
030	700.20	700.20	701.70	50.	69.	.73	325.	3.33	15.77	.90
035	701.75	701.75	703.25	50.	71.	.75	402.	4.02	15.75	.90
040	703.30	703.30	704.80	50.	72.	.76	462.	4.50	15.75	.90
045	704.85	704.85	706.35	50.	73.	.77	502.	5.17	15.75	.90
050	706.40	706.40	707.90	50.	74.	.78	525.	6.42	15.75	.90

TIME	STAGE	OUTLET	RTIOL	ERAIN	STAKS	RTIOL	ENSTL	ALSTL	RTIMP
1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
3.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
4.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
5.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
6.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
7.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
8.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
9.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
10.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
11.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
12.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
13.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
14.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
15.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
16.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
17.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
18.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
19.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
20.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
21.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
22.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
23.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
24.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

LOSS DATA
 CURVE NO = 12.000 FITNESS = -1.000 EFFECT CN = P2.000

UNIT HYDROGRAPH DATA
 TC = .20 LAG = .20

REFLECTION DATA
 STRATF = 1.00 REFLC = .00 RTIOL = 1.00

UNIT HYDROGRAPH IN END OF PERIOD ORDINATES, TC = 1.00 HOURS, LAG = 17.00 VOLT 1.00 5.00

TIME	STAGE	OUTLET	RTIOL	ERAIN	STAKS	RTIOL	ENSTL	ALSTL	RTIMP	LOSS	COMP B
1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
3.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
4.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
5.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
6.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
7.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
8.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
9.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
10.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
11.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
12.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
13.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
14.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
15.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
16.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
17.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
18.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
19.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
20.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
21.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
22.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
23.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
24.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

1.01 17.22 176 5.0 1.01 23.30 282 .01 .01 .00 5.
 .01 17.35 170 4.1 1.01 23.75 283 .01 .01 .00 5.
 1.01 17.40 160 3.2 1.01 23.60 284 .01 .01 .00 5.
 1.01 17.45 151 2.3 1.01 23.45 285 .01 .01 .00 5.

B L A C K R V E A T C M PROJECT 9457: DATE 25 MAR 81 PAGE 7
 FLOOD HYDROGRAPH PACKAGE - HEC-1 PROGRAM H21/C2-1V TYPE 13:17:44 CASE

TIME	INCHES	FEET	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
1.01 17.50	142	1.18	65	14	14	4112
1.01 17.55	143	1.18	83	0	0	116
1.01 17.60	144	1.16	116	0	0	5.31
						134.03
						28
						35
SUM	7.46	5.31	2.11	40%		
						(149.31 134.03 54.31 115.99)

PEAK 376 9. 14. 14. 14.
 CFS 1. 1. 1. 1.
 CFS 4.33 5.31 5.31 5.31
 INCHES 109.97 134.03 134.03 134.03
 AC-FT 23. 28. 28. 28.
 THOUS CU W 28. 35. 35. 35.

 HYDROGRAPH ROUTING

 ROUTE HYDROGRAPH THROUGH DAM

STAGE	ICOMP	IECON	ITAPE	JPLT	JPRY	INAME	ISTAGL	IPUTO
44.00	1	0	0	0	0	1	0	0
45.00	1	0	0	0	0	1	0	0
46.00	1	0	0	0	0	1	0	0
47.00	1	0	0	0	0	1	0	0
48.00	1	0	0	0	0	1	0	0
49.00	1	0	0	0	0	1	0	0
50.00	1	0	0	0	0	1	0	0
51.00	1	0	0	0	0	1	0	0
52.00	1	0	0	0	0	1	0	0
53.00	1	0	0	0	0	1	0	0
54.00	1	0	0	0	0	1	0	0
55.00	1	0	0	0	0	1	0	0
56.00	1	0	0	0	0	1	0	0
57.00	1	0	0	0	0	1	0	0
58.00	1	0	0	0	0	1	0	0
59.00	1	0	0	0	0	1	0	0
60.00	1	0	0	0	0	1	0	0
61.00	1	0	0	0	0	1	0	0
62.00	1	0	0	0	0	1	0	0
63.00	1	0	0	0	0	1	0	0
64.00	1	0	0	0	0	1	0	0
65.00	1	0	0	0	0	1	0	0
66.00	1	0	0	0	0	1	0	0
67.00	1	0	0	0	0	1	0	0
68.00	1	0	0	0	0	1	0	0
69.00	1	0	0	0	0	1	0	0
70.00	1	0	0	0	0	1	0	0
71.00	1	0	0	0	0	1	0	0
72.00	1	0	0	0	0	1	0	0
73.00	1	0	0	0	0	1	0	0
74.00	1	0	0	0	0	1	0	0
75.00	1	0	0	0	0	1	0	0
76.00	1	0	0	0	0	1	0	0
77.00	1	0	0	0	0	1	0	0
78.00	1	0	0	0	0	1	0	0
79.00	1	0	0	0	0	1	0	0
80.00	1	0	0	0	0	1	0	0
81.00	1	0	0	0	0	1	0	0
82.00	1	0	0	0	0	1	0	0
83.00	1	0	0	0	0	1	0	0
84.00	1	0	0	0	0	1	0	0
85.00	1	0	0	0	0	1	0	0
86.00	1	0	0	0	0	1	0	0
87.00	1	0	0	0	0	1	0	0
88.00	1	0	0	0	0	1	0	0
89.00	1	0	0	0	0	1	0	0
90.00	1	0	0	0	0	1	0	0
91.00	1	0	0	0	0	1	0	0
92.00	1	0	0	0	0	1	0	0
93.00	1	0	0	0	0	1	0	0
94.00	1	0	0	0	0	1	0	0
95.00	1	0	0	0	0	1	0	0
96.00	1	0	0	0	0	1	0	0
97.00	1	0	0	0	0	1	0	0
98.00	1	0	0	0	0	1	0	0
99.00	1	0	0	0	0	1	0	0
100.00	1	0	0	0	0	1	0	0

SURFACE AREA 0. 0. 12.
 CAPACITY 0. 50. 102.
 ELEVATION 44. 46. 48.

DAM DATA
 TOTAL COULD EXPD DAMBIC
 855.5 0.0 0.0 0.0

CROSS ELEVATION 150. 210. 310. 420. 530. 650.
 AT 25 HOURS 100.5 100.7 100.9 101.2 101.5 101.8

END-OF-FIELD HYDROGRAPH COORDINATES

NO	DATA	MAX	PERIOD	HOURS	INFLOW	OUTFLOW	STORAGE	START
1	1.01	4.5	1	1.01	0.	0.	50.	064.0
2	1.01	4.5	2	1.01	0.	0.	50.	064.0
3	1.01	4.5	3	1.01	0.	0.	50.	064.0
4	1.01	4.5	4	1.01	0.	0.	50.	064.0
5	1.01	4.5	5	1.01	0.	0.	50.	064.0
6	1.01	4.5	6	1.01	0.	0.	50.	064.0
7	1.01	4.5	7	1.01	0.	0.	50.	064.0
8	1.01	4.5	8	1.01	0.	0.	50.	064.0
9	1.01	4.5	9	1.01	0.	0.	50.	064.0
10	1.01	4.5	10	1.01	0.	0.	50.	064.0
11	1.01	4.5	11	1.01	0.	0.	50.	064.0
12	1.01	4.5	12	1.01	0.	0.	50.	064.0
13	1.01	4.5	13	1.01	0.	0.	50.	064.0
14	1.01	4.5	14	1.01	0.	0.	50.	064.0
15	1.01	4.5	15	1.01	0.	0.	50.	064.0
16	1.01	4.5	16	1.01	0.	0.	50.	064.0
17	1.01	4.5	17	1.01	0.	0.	50.	064.0
18	1.01	4.5	18	1.01	0.	0.	50.	064.0
19	1.01	4.5	19	1.01	0.	0.	50.	064.0
20	1.01	4.5	20	1.01	0.	0.	50.	064.0
21	1.01	4.5	21	1.01	0.	0.	50.	064.0
22	1.01	4.5	22	1.01	0.	0.	50.	064.0
23	1.01	4.5	23	1.01	0.	0.	50.	064.0
24	1.01	4.5	24	1.01	0.	0.	50.	064.0
25	1.01	4.5	25	1.01	0.	0.	50.	064.0
26	1.01	4.5	26	1.01	0.	0.	50.	064.0
27	1.01	4.5	27	1.01	0.	0.	50.	064.0
28	1.01	4.5	28	1.01	0.	0.	50.	064.0
29	1.01	4.5	29	1.01	0.	0.	50.	064.0
30	1.01	4.5	30	1.01	0.	0.	50.	064.0
31	1.01	4.5	31	1.01	0.	0.	50.	064.0
32	1.01	4.5	32	1.01	0.	0.	50.	064.0
33	1.01	4.5	33	1.01	0.	0.	50.	064.0
34	1.01	4.5	34	1.01	0.	0.	50.	064.0
35	1.01	4.5	35	1.01	0.	0.	50.	064.0
36	1.01	4.5	36	1.01	0.	0.	50.	064.0
37	1.01	4.5	37	1.01	0.	0.	50.	064.0
38	1.01	4.5	38	1.01	0.	0.	50.	064.0
39	1.01	4.5	39	1.01	0.	0.	50.	064.0
40	1.01	4.5	40	1.01	0.	0.	50.	064.0
41	1.01	4.5	41	1.01	0.	0.	50.	064.0
42	1.01	4.5	42	1.01	0.	0.	50.	064.0
43	1.01	4.5	43	1.01	0.	0.	50.	064.0
44	1.01	4.5	44	1.01	0.	0.	50.	064.0
45	1.01	4.5	45	1.01	0.	0.	50.	064.0
46	1.01	4.5	46	1.01	0.	0.	50.	064.0
47	1.01	4.5	47	1.01	0.	0.	50.	064.0
48	1.01	4.5	48	1.01	0.	0.	50.	064.0
49	1.01	4.5	49	1.01	0.	0.	50.	064.0
50	1.01	4.5	50	1.01	0.	0.	50.	064.0
51	1.01	4.5	51	1.01	0.	0.	50.	064.0
52	1.01	4.5	52	1.01	0.	0.	50.	064.0
53	1.01	4.5	53	1.01	0.	0.	50.	064.0
54	1.01	4.5	54	1.01	0.	0.	50.	064.0

1.01	6.38	51	4.25	0.	0.	50.	684.0
1.01	6.44	54	4.67	0.	0.	50.	684.0
1.01	6.45	57	4.75	0.	0.	50.	684.0
1.01	6.50	58	4.83	0.	0.	50.	684.0
1.01	6.55	50	4.82	0.	0.	50.	684.0
1.01	6.59	61	5.09	0.	0.	50.	684.0
1.01	6.65	62	5.17	0.	0.	50.	684.0
1.01	6.75	63	5.25	0.	0.	50.	684.0
1.01	6.82	64	5.33	0.	0.	50.	684.0
1.01	6.85	65	5.42	0.	0.	50.	684.0
1.01	6.88	66	5.52	0.	0.	50.	684.0
1.01	6.92	67	5.58	0.	0.	50.	684.0
1.01	6.95	69	5.75	0.	0.	50.	684.0
1.01	6.98	70	5.81	0.	0.	50.	684.0
1.01	7.01	71	5.91	0.	0.	50.	684.0
1.01	7.04	72	6.00	0.	0.	50.	684.0
1.01	7.07	73	6.09	0.	0.	50.	684.0
1.01	7.10	74	6.17	0.	0.	50.	684.0
1.01	7.13	75	6.25	0.	0.	50.	684.0
1.01	7.16	76	6.33	0.	0.	50.	684.0
1.01	7.19	77	6.42	0.	0.	50.	684.0
1.01	7.22	78	6.50	0.	0.	50.	684.0
1.01	7.25	79	6.58	0.	0.	50.	684.0
1.01	7.28	80	6.67	0.	0.	50.	684.0
1.01	7.31	81	6.75	0.	0.	50.	684.0
1.01	7.34	82	6.83	0.	0.	50.	684.0
1.01	7.37	83	6.92	0.	0.	50.	684.0
1.01	7.40	84	7.00	0.	0.	50.	684.0
1.01	7.43	85	7.09	0.	0.	50.	684.0
1.01	7.46	86	7.17	0.	0.	50.	684.0
1.01	7.49	87	7.25	0.	0.	50.	684.0
1.01	7.52	88	7.33	0.	0.	50.	684.0
1.01	7.55	89	7.42	0.	0.	50.	684.0
1.01	7.58	90	7.50	0.	0.	50.	684.0
1.01	7.61	91	7.58	0.	0.	50.	684.0
1.01	7.64	92	7.67	0.	0.	50.	684.0
1.01	7.67	93	7.75	0.	0.	50.	684.0
1.01	7.70	94	7.83	0.	0.	50.	684.0
1.01	7.73	95	7.92	0.	0.	50.	684.0
1.01	7.76	96	8.00	0.	0.	50.	684.0
1.01	7.79	97	8.08	0.	0.	50.	684.0
1.01	7.82	98	8.17	0.	0.	50.	684.0
1.01	7.85	99	8.25	0.	0.	50.	684.0
1.01	7.88	100	8.33	0.	0.	50.	684.0
1.01	7.91	101	8.42	0.	0.	50.	684.0
1.01	7.94	102	8.50	0.	0.	50.	684.0
1.01	7.97	103	8.58	0.	0.	50.	684.0
1.01	8.00	104	8.67	0.	0.	50.	684.0
1.01	8.03	105	8.75	0.	0.	50.	684.0
1.01	8.06	106	8.83	0.	0.	50.	684.0
1.01	8.09	107	8.92	0.	0.	50.	684.0
1.01	8.12	108	9.00	0.	0.	50.	684.0
1.01	8.15	109	9.08	0.	0.	50.	684.0
1.01	8.18	110	9.17	0.	0.	50.	684.0
1.01	8.21	111	9.25	0.	0.	50.	684.0
1.01	8.24	112	9.33	0.	0.	50.	684.0
1.01	8.27	113	9.42	0.	0.	50.	684.0
1.01	8.30	114	9.50	0.	0.	50.	684.0
1.01	8.33	115	9.58	0.	0.	50.	684.0
1.01	8.36	116	9.67	0.	0.	50.	684.0
1.01	8.39	117	9.75	0.	0.	50.	684.0
1.01	8.42	118	9.83	0.	0.	50.	684.0
1.01	8.45	119	9.92	0.	0.	50.	684.0
1.01	8.48	120	10.00	0.	0.	50.	684.0

1.01	9.15	119	9.23	6.	1.	51.	6F4.1
1.01	9.20	119	9.28	6.	2.	51.	6F4.1
1.01	9.25	119	9.33	7.	3.	51.	6F4.1
1.01	9.30	119	9.38	8.	4.	51.	6F4.1
1.01	9.35	119	9.43	9.	5.	51.	6F4.1
1.01	9.40	119	9.48	10.	6.	51.	6F4.1
1.01	9.45	119	9.53	11.	7.	51.	6F4.1
1.01	9.50	119	9.58	12.	8.	51.	6F4.1
1.01	9.55	119	10.03	13.	9.	51.	6F4.1
1.01	10.00	119	10.08	14.	10.	51.	6F4.1
1.01	10.05	119	10.13	15.	11.	51.	6F4.1
1.01	10.10	119	10.18	16.	12.	51.	6F4.1
1.01	10.15	119	10.23	17.	13.	51.	6F4.1
1.01	10.20	119	10.28	18.	14.	51.	6F4.1
1.01	10.25	119	10.33	19.	15.	51.	6F4.1
1.01	10.30	119	10.38	20.	16.	51.	6F4.1
1.01	10.35	119	10.43	21.	17.	51.	6F4.1
1.01	10.40	119	10.48	22.	18.	51.	6F4.1
1.01	10.45	119	10.53	23.	19.	51.	6F4.1
1.01	10.50	119	10.58	24.	20.	51.	6F4.1
1.01	10.55	119	11.03	25.	21.	51.	6F4.1
1.01	11.00	119	11.08	26.	22.	51.	6F4.1
1.01	11.05	119	11.13	27.	23.	51.	6F4.1
1.01	11.10	119	11.18	28.	24.	51.	6F4.1
1.01	11.15	119	11.23	29.	25.	51.	6F4.1
1.01	11.20	119	11.28	30.	26.	51.	6F4.1
1.01	11.25	119	11.33	31.	27.	51.	6F4.1
1.01	11.30	119	11.38	32.	28.	51.	6F4.1
1.01	11.35	119	11.43	33.	29.	51.	6F4.1
1.01	11.40	119	11.48	34.	30.	51.	6F4.1
1.01	11.45	119	11.53	35.	31.	51.	6F4.1
1.01	11.50	119	11.58	36.	32.	51.	6F4.1
1.01	11.55	119	12.03	37.	33.	51.	6F4.1
1.01	12.00	119	12.08	38.	34.	51.	6F4.1
1.01	12.05	119	12.13	39.	35.	51.	6F4.1
1.01	12.10	119	12.18	40.	36.	51.	6F4.1
1.01	12.15	119	12.23	41.	37.	51.	6F4.1
1.01	12.20	119	12.28	42.	38.	51.	6F4.1
1.01	12.25	119	12.33	43.	39.	51.	6F4.1
1.01	12.30	119	12.38	44.	40.	51.	6F4.1
1.01	12.35	119	12.43	45.	41.	51.	6F4.1
1.01	12.40	119	12.48	46.	42.	51.	6F4.1
1.01	12.45	119	12.53	47.	43.	51.	6F4.1
1.01	12.50	119	12.58	48.	44.	51.	6F4.1
1.01	12.55	119	13.03	49.	45.	51.	6F4.1
1.01	13.00	119	13.08	50.	46.	51.	6F4.1
1.01	13.05	119	13.13	51.	47.	51.	6F4.1
1.01	13.10	119	13.18	52.	48.	51.	6F4.1
1.01	13.15	119	13.23	53.	49.	51.	6F4.1
1.01	13.20	119	13.28	54.	50.	51.	6F4.1
1.01	13.25	119	13.33	55.	51.	51.	6F4.1
1.01	13.30	119	13.38	56.	52.	51.	6F4.1
1.01	13.35	119	13.43	57.	53.	51.	6F4.1
1.01	13.40	119	13.48	58.	54.	51.	6F4.1
1.01	13.45	119	13.53	59.	55.	51.	6F4.1
1.01	13.50	119	13.58	60.	56.	51.	6F4.1

1.01	13.55	167	13.62	19.	43.	59.	685.4
1.01	14.00	168	14.00	19.	42.	59.	685.3
1.01	14.05	169	14.08	19.	41.	59.	685.3
1.01	14.10	170	14.17	19.	40.	59.	685.3
1.01	14.15	171	14.25	19.	39.	59.	685.3
1.01	14.20	172	14.33	19.	38.	59.	685.3
1.01	14.25	173	14.42	19.	37.	59.	685.2
1.01	14.30	174	14.50	19.	36.	59.	685.2
1.01	14.35	175	14.58	19.	35.	59.	685.2
1.01	14.40	176	14.67	19.	35.	59.	685.2
1.01	14.45	177	14.75	19.	34.	59.	685.2
1.01	14.50	178	14.83	19.	33.	59.	685.2
1.01	14.55	179	14.92	19.	32.	59.	685.1
1.01	15.00	180	15.00	19.	31.	59.	685.1
1.01	15.05	181	15.08	19.	31.	59.	685.1
1.01	15.10	182	15.17	19.	30.	59.	685.1
1.01	15.15	183	15.25	19.	29.	59.	685.1
1.01	15.20	184	15.33	19.	28.	59.	685.1
1.01	15.25	185	15.42	19.	27.	59.	685.0
1.01	15.30	186	15.50	19.	27.	59.	685.0
1.01	15.35	187	15.58	19.	26.	59.	685.0
1.01	15.40	188	15.67	19.	25.	59.	685.0
1.01	15.45	189	15.75	19.	25.	59.	685.0
1.01	15.50	190	15.83	9.	25.	59.	685.0
1.01	15.55	191	15.92	9.	24.	59.	684.9
1.01	16.00	192	16.00	9.	24.	59.	684.9
1.01	16.05	193	16.08	9.	24.	59.	684.9
1.01	16.10	194	16.17	9.	23.	59.	684.9
1.01	16.15	195	16.25	9.	23.	59.	684.9
1.01	16.20	196	16.33	9.	22.	59.	684.9
1.01	16.25	197	16.42	9.	22.	59.	684.9
1.01	16.30	198	16.50	9.	22.	59.	684.9
1.01	16.35	199	16.58	9.	21.	59.	684.9
1.01	16.40	200	16.67	9.	21.	59.	684.8
1.01	16.45	201	16.75	9.	20.	59.	684.8
1.01	16.50	202	16.83	9.	20.	59.	684.8
1.01	16.55	203	16.92	9.	20.	59.	684.8
1.01	17.00	204	17.00	9.	19.	59.	684.8
1.01	17.05	205	17.08	9.	19.	59.	684.8
1.01	17.10	206	17.17	9.	19.	59.	684.8
1.01	17.15	207	17.25	9.	19.	59.	684.8
1.01	17.20	208	17.33	9.	18.	59.	684.7
1.01	17.25	209	17.42	9.	18.	59.	684.7
1.01	17.30	210	17.50	9.	18.	59.	684.7
1.01	17.35	211	17.58	9.	18.	59.	684.7
1.01	17.40	212	17.67	9.	17.	59.	684.7
1.01	17.45	213	17.75	9.	17.	59.	684.7
1.01	17.50	214	17.83	9.	17.	59.	684.7
1.01	17.55	215	17.92	9.	17.	59.	684.7
1.01	18.00	216	18.00	9.	16.	59.	684.7
1.01	18.05	217	18.08	9.	16.	59.	684.7
1.01	18.10	218	18.17	9.	16.	59.	684.7
1.01	18.15	219	18.25	9.	15.	59.	684.6
1.01	18.20	220	18.33	9.	15.	59.	684.6
1.01	18.25	221	18.42	9.	15.	59.	684.6
1.01	18.30	222	18.50	9.	14.	59.	684.6
1.01	18.35	223	18.59	9.	14.	59.	684.6

1.01	15.55	267	15.55	54	084.6
1.01	15.57	264	15.57	54	084.6
1.01	15.75	264	15.75	54	084.6
1.01	15.81	264	15.81	54	084.6
1.01	15.85	267	15.85	54	084.6
1.01	15.97	267	15.97	54	084.6
1.01	16.07	267	16.07	54	084.6
1.01	16.17	267	16.17	54	084.6
1.01	16.25	267	16.25	54	084.6
1.01	16.33	267	16.33	54	084.6
1.01	16.42	267	16.42	54	084.6
1.01	16.50	267	16.50	54	084.6
1.01	16.55	267	16.55	54	084.6
1.01	16.67	267	16.67	54	084.6
1.01	16.75	267	16.75	54	084.6
1.01	16.83	267	16.83	54	084.6
1.01	16.92	267	16.92	54	084.6
1.01	17.00	267	17.00	54	084.6
1.01	17.07	267	17.07	54	084.6
1.01	17.17	267	17.17	54	084.6
1.01	17.25	267	17.25	54	084.6
1.01	17.33	267	17.33	54	084.6
1.01	17.42	267	17.42	54	084.6
1.01	17.50	267	17.50	54	084.6
1.01	17.55	267	17.55	54	084.6
1.01	17.67	267	17.67	54	084.6
1.01	17.75	267	17.75	54	084.6
1.01	17.83	267	17.83	54	084.6
1.01	17.92	267	17.92	54	084.6
1.01	18.00	267	18.00	54	084.6
1.01	18.07	267	18.07	54	084.6
1.01	18.17	267	18.17	54	084.6
1.01	18.25	267	18.25	54	084.6
1.01	18.33	267	18.33	54	084.6
1.01	18.42	267	18.42	54	084.6
1.01	18.50	267	18.50	54	084.6
1.01	18.55	267	18.55	54	084.6
1.01	18.67	267	18.67	54	084.6
1.01	18.75	267	18.75	54	084.6
1.01	18.83	267	18.83	54	084.6
1.01	18.92	267	18.92	54	084.6
1.01	19.00	267	19.00	54	084.6
1.01	19.07	267	19.07	54	084.6
1.01	19.17	267	19.17	54	084.6
1.01	19.25	267	19.25	54	084.6
1.01	19.33	267	19.33	54	084.6
1.01	19.42	267	19.42	54	084.6
1.01	19.50	267	19.50	54	084.6
1.01	19.55	267	19.55	54	084.6
1.01	19.67	267	19.67	54	084.6
1.01	19.75	267	19.75	54	084.6
1.01	19.83	267	19.83	54	084.6
1.01	19.92	267	19.92	54	084.6
1.01	20.00	267	20.00	54	084.6
1.01	20.07	267	20.07	54	084.6
1.01	20.17	267	20.17	54	084.6
1.01	20.25	267	20.25	54	084.6
1.01	20.33	267	20.33	54	084.6
1.01	20.42	267	20.42	54	084.6
1.01	20.50	267	20.50	54	084.6
1.01	20.55	267	20.55	54	084.6
1.01	20.67	267	20.67	54	084.6
1.01	20.75	267	20.75	54	084.6
1.01	20.83	267	20.83	54	084.6
1.01	20.92	267	20.92	54	084.6
1.01	21.00	267	21.00	54	084.6
1.01	21.07	267	21.07	54	084.6
1.01	21.17	267	21.17	54	084.6
1.01	21.25	267	21.25	54	084.6
1.01	21.33	267	21.33	54	084.6
1.01	21.42	267	21.42	54	084.6
1.01	21.50	267	21.50	54	084.6
1.01	21.55	267	21.55	54	084.6
1.01	21.67	267	21.67	54	084.6
1.01	21.75	267	21.75	54	084.6
1.01	21.83	267	21.83	54	084.6
1.01	21.92	267	21.92	54	084.6
1.01	22.00	267	22.00	54	084.6
1.01	22.07	267	22.07	54	084.6
1.01	22.17	267	22.17	54	084.6
1.01	22.25	267	22.25	54	084.6
1.01	22.33	267	22.33	54	084.6
1.01	22.42	267	22.42	54	084.6
1.01	22.50	267	22.50	54	084.6
1.01	22.55	267	22.55	54	084.6
1.01	22.67	267	22.67	54	084.6
1.01	22.75	267	22.75	54	084.6
1.01	22.83	267	22.83	54	084.6
1.01	22.92	267	22.92	54	084.6
1.01	23.00	267	23.00	54	084.6
1.01	23.07	267	23.07	54	084.6
1.01	23.17	267	23.17	54	084.6
1.01	23.25	267	23.25	54	084.6
1.01	23.33	267	23.33	54	084.6
1.01	23.42	267	23.42	54	084.6
1.01	23.50	267	23.50	54	084.6
1.01	23.55	267	23.55	54	084.6
1.01	23.67	267	23.67	54	084.6
1.01	23.75	267	23.75	54	084.6
1.01	23.83	267	23.83	54	084.6
1.01	23.92	267	23.92	54	084.6
1.01	24.00	267	24.00	54	084.6

1.01	23.15	279	23.25	5.	7.	51.	684.3
1.01	23.20	260	23.33	5.	7.	53.	684.3
1.01	23.25	281	23.42	5.	7.	52.	684.3
1.01	23.30	292	23.50	5.	7.	52.	684.3
1.01	23.35	287	23.57	5.	7.	52.	684.3
1.01	23.40	284	23.67	5.	7.	52.	684.3
1.01	23.45	266	23.75	5.	7.	52.	684.3
1.01	23.50	286	23.83	5.	7.	51.	684.3
1.01	23.55	257	23.92	5.	7.	52.	684.3
1.02	23.55	264	24.03	5.	7.	52.	684.3

PEAK OUTFLOW IS 344. AT TIME 12.5P HOURS

PEAK	144.	41.	13.	0.	0.	10P.
CFS	4.	1.	0.	0.	0.	10P.
INCHES	3.78	4.95	4.95	4.95	4.95	4.95
AC-FT	95.07	125.68	125.68	125.68	125.68	125.68
THOUS CU 4	20.	26.	26.	26.	26.	26.
	25.	33.	33.	33.	33.	33.

RUNOFF SUMMARY, AVERAGE FLOW IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)
 AREA IN SQUARE KILOMETERS (KILOMETERS)

HYDROGRAPH AT	HEAD	PEAK	6-HOUR	24-HOUR	72-HOUR	72-HOUR	AREA
	(3.33	47.	16.	16.	16.	.10
	(9.33	1.72	.80	.43	.43	.26
ROUTED TO	PAV	144.	41.	13.	13.	13.	.10
	(4.00	1.15	.30	.30	.30	.26

SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1	ELEVATION STORAGE OUTFLOW	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TYPE OF FAILURE HOURS
	085.77	084.00	084.00	085.50	1.17	12.58	0.00
	45.5-FLV	50.	50.	60.			
	AC-FT	3.	4.	49.			

100-year

FIN

LOSS DATA
 CURVE NO = -P.00 FITNESS = -1.00 EFFECT CM = P2.00
 UNIT HYDROGRAPH DATA
 TC = .90 LAG = .20
 RECESSION DATA
 WFLSN = .30 RTION = 1.00
 STRT = .00
 UNIT HYDROGRAPH 14 END OF PERIOD ORIGINATES, TC = .00 HOURS, LAG = .20 VOL = 1.00 S.
 1. 199. 159. 89. 51.
 2. 1. 1.
 3. 1. 1.

LUPT	SIPP	BLTR	RTIOL	ERAIN	STRES	RTIOL	STRTL	CNSTL	ALSMR	RTIMP
1	.00	.00	1.00	.00	1.00	1.00	-1.00	-82.00	.00	.00
2	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
3	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
4	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
5	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
6	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
7	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
8	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
9	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
10	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
11	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
12	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
13	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
14	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
15	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
16	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
17	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
18	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
19	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
20	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
21	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
22	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
23	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
24	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
25	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
26	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
27	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
28	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
29	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

PO.DA	HR.MN	PERIOD	RAIN	EXCS	LOSS	END-OF-PERIOD FLOD	PO.DA	HR.MN	PERIOD	RAIN	EXCS	LOSS	CONF
1.01	05	1	.00	.00	.00	0.	1.01	12.05	165	.59	.65	.14	107.
1.01	10	2	.00	.00	.00	0.	1.01	12.10	146	.24	.21	.05	162.
1.01	15	3	.00	.00	.00	0.	1.01	12.15	147	.15	.12	.03	185.
1.01	20	4	.00	.00	.00	0.	1.01	12.20	144	.00	.06	.01	147.
1.01	25	5	.00	.00	.00	0.	1.01	12.25	149	.00	.00	.01	129.
1.01	30	6	.00	.00	.00	0.	1.01	12.30	150	.00	.00	.01	57.
1.01	35	7	.00	.00	.00	0.	1.01	12.35	151	.00	.05	.01	76.
1.01	40	8	.00	.00	.00	0.	1.01	12.40	152	.00	.05	.01	62.
1.01	45	9	.00	.00	.00	0.	1.01	12.45	153	.04	.05	.01	53.
1.01	50	10	.00	.00	.00	0.	1.01	12.50	154	.00	.05	.01	47.
1.01	55	11	.00	.00	.00	0.	1.01	12.55	155	.06	.05	.01	43.
1.01	00	12	.00	.00	.00	0.	1.01	13.00	156	.00	.05	.01	41.
1.01	05	13	.00	.00	.00	0.	1.01	13.05	157	.00	.02	.00	39.
1.01	10	14	.00	.00	.00	0.	1.01	13.10	158	.00	.02	.00	34.
1.01	15	15	.00	.00	.00	0.	1.01	13.15	159	.00	.02	.00	29.
1.01	20	16	.00	.00	.00	0.	1.01	13.20	146	.03	.02	.00	24.
1.01	25	17	.00	.00	.00	0.	1.01	13.25	141	.00	.02	.00	22.
1.01	30	18	.00	.00	.00	0.	1.01	13.30	142	.00	.02	.00	21.
1.01	35	19	.00	.00	.00	0.	1.01	13.35	163	.01	.00	.00	20.
1.01	40	20	.00	.00	.00	0.	1.01	13.40	174	.01	.00	.00	17.
1.01	45	21	.00	.00	.00	0.	1.01	13.45	165	.01	.01	.00	15.
1.01	50	22	.00	.00	.00	0.	1.01	13.50	176	.01	.01	.00	13.
1.01	55	23	.00	.00	.00	0.	1.01	13.55	167	.01	.00	.00	11.
1.01	00	24	.00	.00	.00	0.	1.01	14.00	168	.01	.01	.00	11.
1.01	05	25	.00	.00	.00	0.	1.01	14.05	169	.01	.01	.00	10.
1.01	10	26	.00	.00	.00	0.	1.01	14.10	170	.01	.01	.00	10.
1.01	15	27	.00	.00	.00	0.	1.01	14.15	171	.01	.01	.00	10.
1.01	20	28	.00	.00	.00	0.	1.01	14.20	172	.01	.01	.00	10.
1.01	25	29	.00	.00	.00	0.	1.01	14.25	173	.01	.01	.00	10.

D L A C K R V E A T I C H

F L O O D H Y D R O G R A P H P A C K A G E - H E C - 1

1.01	2.372	30	0.00	0.00	0.00	0.00	1.01	14.30	174	.01	.01	.00	10.
1.01	2.375	31	.00	.00	.00	.00	1.01	14.35	175	.01	.01	.00	10.
1.01	2.400	34	.00	.00	.00	.00	1.01	14.40	176	.01	.01	.00	10.
1.01	2.405	35	.00	.00	.00	.00	1.01	14.45	177	.01	.01	.00	10.
1.01	2.550	34	.00	.00	.00	.00	1.01	14.50	178	.01	.01	.00	10.
1.01	2.575	35	.00	.00	.00	.00	1.01	14.55	179	.01	.01	.00	10.
1.01	2.600	36	.00	.00	.00	.00	1.01	15.00	180	.01	.01	.00	10.
1.01	2.705	37	.00	.00	.00	.00	1.01	15.05	181	.01	.01	.00	10.
1.01	2.710	38	.00	.00	.00	.00	1.01	15.10	182	.01	.01	.00	10.
1.01	2.715	39	.00	.00	.00	.00	1.01	15.15	183	.01	.01	.00	10.
1.01	2.720	40	.00	.00	.00	.00	1.01	15.20	184	.01	.01	.00	10.
1.01	2.725	41	.00	.00	.00	.00	1.01	15.25	185	.01	.01	.00	10.
1.01	2.730	42	.00	.00	.00	.00	1.01	15.30	186	.01	.01	.00	10.
1.01	2.735	43	.00	.00	.00	.00	1.01	15.35	187	.01	.01	.00	10.
1.01	2.740	44	.00	.00	.00	.00	1.01	15.40	188	.01	.01	.00	10.
1.01	2.745	45	.00	.00	.00	.00	1.01	15.45	189	.01	.01	.00	10.
1.01	2.750	46	.00	.00	.00	.00	1.01	15.50	190	.01	.01	.00	10.
1.01	2.755	47	.00	.00	.00	.00	1.01	15.55	191	.01	.01	.00	10.
1.01	2.760	48	.00	.00	.00	.00	1.01	15.60	192	.01	.01	.00	10.
1.01	2.765	49	.00	.00	.00	.00	1.01	16.05	193	.01	.01	.00	10.
1.01	2.770	50	.00	.00	.00	.00	1.01	16.10	194	.01	.01	.00	10.
1.01	2.775	51	.00	.00	.00	.00	1.01	16.15	195	.01	.01	.00	10.
1.01	2.780	52	.00	.00	.00	.00	1.01	16.20	196	.01	.01	.00	10.
1.01	2.785	53	.00	.00	.00	.00	1.01	16.25	197	.01	.01	.00	10.
1.01	2.790	54	.00	.00	.00	.00	1.01	16.30	198	.01	.01	.00	10.
1.01	2.795	55	.00	.00	.00	.00	1.01	16.35	199	.01	.01	.00	10.
1.01	2.800	56	.00	.00	.00	.00	1.01	16.40	200	.01	.01	.00	10.
1.01	2.805	57	.00	.00	.00	.00	1.01	16.45	201	.01	.01	.00	10.
1.01	2.810	58	.00	.00	.00	.00	1.01	16.50	202	.01	.01	.00	10.
1.01	2.815	59	.00	.00	.00	.00	1.01	16.55	203	.01	.01	.00	10.
1.01	2.820	60	.00	.00	.00	.00	1.01	16.60	204	.01	.01	.00	10.
1.01	2.825	61	.00	.00	.00	.00	1.01	17.05	205	.01	.01	.00	10.
1.01	2.830	62	.00	.00	.00	.00	1.01	17.10	206	.01	.01	.00	10.
1.01	2.835	63	.00	.00	.00	.00	1.01	17.15	207	.01	.01	.00	10.
1.01	2.840	64	.00	.00	.00	.00	1.01	17.20	208	.01	.01	.00	10.
1.01	2.845	65	.00	.00	.00	.00	1.01	17.25	209	.01	.01	.00	10.
1.01	2.850	66	.00	.00	.00	.00	1.01	17.30	210	.01	.01	.00	10.
1.01	2.855	67	.00	.00	.00	.00	1.01	17.35	211	.01	.01	.00	10.
1.01	2.860	68	.00	.00	.00	.00	1.01	17.40	212	.01	.01	.00	10.
1.01	2.865	69	.00	.00	.00	.00	1.01	17.45	213	.01	.01	.00	10.
1.01	2.870	70	.00	.00	.00	.00	1.01	17.50	214	.01	.01	.00	10.
1.01	2.875	71	.00	.00	.00	.00	1.01	17.55	215	.01	.01	.00	10.
1.01	2.880	72	.00	.00	.00	.00	1.01	18.00	216	.01	.01	.00	10.
1.01	2.885	73	.00	.00	.00	.00	1.01	18.05	217	.01	.01	.00	10.
1.01	2.890	74	.00	.00	.00	.00	1.01	18.10	218	.01	.01	.00	10.
1.01	2.895	75	.00	.00	.00	.00	1.01	18.15	219	.01	.01	.00	10.
1.01	2.900	76	.00	.00	.00	.00	1.01	18.20	220	.01	.01	.00	10.
1.01	2.905	77	.00	.00	.00	.00	1.01	18.25	221	.01	.01	.00	10.
1.01	2.910	78	.00	.00	.00	.00	1.01	18.30	222	.01	.01	.00	10.
1.01	2.915	79	.00	.00	.00	.00	1.01	18.35	223	.01	.01	.00	10.
1.01	2.920	80	.00	.00	.00	.00	1.01	18.40	224	.01	.01	.00	10.
1.01	2.925	81	.00	.00	.00	.00	1.01	18.45	225	.01	.01	.00	10.
1.01	2.930	82	.00	.00	.00	.00	1.01	18.50	226	.01	.01	.00	10.
1.01	2.935	83	.00	.00	.00	.00	1.01	18.55	227	.01	.01	.00	10.
1.01	2.940	84	.00	.00	.00	.00	1.01	19.00	228	.01	.01	.00	10.
1.01	2.945	85	.00	.00	.00	.00	1.01	19.05	229	.01	.01	.00	10.

TIME	PC	CU	UI	U	1-01	17.10	270	UC	.00	3.
1501 7.10	Pc	.01	.00	.01	1-01	17.10	270	.00	.00	3.
1501 7.15	Pb	.01	.00	.01	1-01	17.15	271	.00	.00	3.
1501 7.20	Pc	.01	.00	.01	1-01	17.20	272	.00	.00	3.
1501 7.25	Pd	.01	.00	.01	1-01	17.25	273	.00	.00	3.
1501 7.30	CU	.01	.00	.01	1-01	17.30	274	.00	.00	3.
1501 7.35	CU	.01	.00	.01	1-01	17.35	275	.00	.00	3.
1501 7.40	CU	.01	.00	.01	1-01	17.40	276	.00	.00	3.
1501 7.45	CU	.01	.00	.01	1-01	17.45	277	.00	.00	3.
1501 7.50	CU	.01	.00	.01	1-01	17.50	278	.00	.00	3.
1501 7.55	CU	.01	.00	.01	1-01	17.55	279	.00	.00	3.
1501 P.00	CU	.01	.00	.01	1-01	20.00	280	.00	.00	3.
1501 P.05	CU	.01	.00	.01	1-01	20.05	281	.00	.00	3.
1501 P.10	CU	.01	.00	.01	1-01	20.10	282	.00	.00	3.
1501 P.15	CU	.01	.00	.01	1-01	20.15	283	.00	.00	3.
1501 P.20	CU	.01	.00	.01	1-01	20.20	284	.00	.00	3.
1501 P.25	CU	.01	.00	.01	1-01	20.25	285	.00	.00	3.
1501 P.30	CU	.01	.00	.01	1-01	20.30	286	.00	.00	3.
1501 P.35	CU	.01	.00	.01	1-01	20.35	287	.00	.00	3.
1501 P.40	CU	.01	.00	.01	1-01	20.40	288	.00	.00	3.
1501 P.45	CU	.01	.00	.01	1-01	20.45	289	.00	.00	3.
1501 P.50	CU	.01	.00	.01	1-01	20.50	290	.00	.00	3.
1501 P.55	CU	.01	.00	.01	1-01	20.55	291	.00	.00	3.
1501 P.00	CU	.01	.00	.01	1-01	21.00	292	.00	.00	3.
1501 P.05	CU	.01	.00	.01	1-01	21.05	293	.00	.00	3.
1501 P.10	CU	.01	.00	.01	1-01	21.10	294	.00	.00	3.
1501 P.15	CU	.01	.00	.01	1-01	21.15	295	.00	.00	3.
1501 P.20	CU	.01	.00	.01	1-01	21.20	296	.00	.00	3.
1501 P.25	CU	.01	.00	.01	1-01	21.25	297	.00	.00	3.
1501 P.30	CU	.01	.00	.01	1-01	21.30	298	.00	.00	3.
1501 P.35	CU	.01	.00	.01	1-01	21.35	299	.00	.00	3.
1501 P.40	CU	.01	.00	.01	1-01	21.40	300	.00	.00	3.
1501 P.45	CU	.01	.00	.01	1-01	21.45	301	.00	.00	3.
1501 P.50	CU	.01	.00	.01	1-01	21.50	302	.00	.00	3.
1501 P.55	CU	.01	.00	.01	1-01	21.55	303	.00	.00	3.
1501 10.00	CU	.01	.00	.01	1-01	22.00	304	.00	.00	3.
1501 10.05	CU	.01	.00	.01	1-01	22.05	305	.00	.00	3.
1501 10.10	CU	.01	.00	.01	1-01	22.10	306	.00	.00	3.
1501 10.15	CU	.01	.00	.01	1-01	22.15	307	.00	.00	3.
1501 10.20	CU	.01	.00	.01	1-01	22.20	308	.00	.00	3.
1501 10.25	CU	.01	.00	.01	1-01	22.25	309	.00	.00	3.
1501 10.30	CU	.01	.00	.01	1-01	22.30	310	.00	.00	3.
1501 10.35	CU	.01	.00	.01	1-01	22.35	311	.00	.00	3.
1501 10.40	CU	.01	.00	.01	1-01	22.40	312	.00	.00	3.
1501 10.45	CU	.01	.00	.01	1-01	22.45	313	.00	.00	3.
1501 10.50	CU	.01	.00	.01	1-01	22.50	314	.00	.00	3.
1501 10.55	CU	.01	.00	.01	1-01	22.55	315	.00	.00	3.
1501 11.00	CU	.01	.00	.01	1-01	23.00	316	.00	.00	3.
1501 11.05	CU	.01	.00	.01	1-01	23.05	317	.00	.00	3.
1501 11.10	CU	.01	.00	.01	1-01	23.10	318	.00	.00	3.
1501 11.15	CU	.01	.00	.01	1-01	23.15	319	.00	.00	3.
1501 11.20	CU	.01	.00	.01	1-01	23.20	320	.00	.00	3.
1501 11.25	CU	.01	.00	.01	1-01	23.25	321	.00	.00	3.
1501 11.30	CU	.01	.00	.01	1-01	23.30	322	.00	.00	3.
1501 11.35	CU	.01	.00	.01	1-01	23.35	323	.00	.00	3.
1501 11.40	CU	.01	.00	.01	1-01	23.40	324	.00	.00	3.
1501 11.45	CU	.01	.00	.01	1-01	23.45	325	.00	.00	3.
1501 11.50	CU	.01	.00	.01	1-01	23.50	326	.00	.00	3.
1501 11.55	CU	.01	.00	.01	1-01	23.55	327	.00	.00	3.
1501 12.00	CU	.01	.00	.01	1-01	23.60	328	.00	.00	3.
1501 12.05	CU	.01	.00	.01	1-01	23.65	329	.00	.00	3.
1501 12.10	CU	.01	.00	.01	1-01	23.70	330	.00	.00	3.
1501 12.15	CU	.01	.00	.01	1-01	23.75	331	.00	.00	3.
1501 12.20	CU	.01	.00	.01	1-01	23.80	332	.00	.00	3.
1501 12.25	CU	.01	.00	.01	1-01	23.85	333	.00	.00	3.
1501 12.30	CU	.01	.00	.01	1-01	23.90	334	.00	.00	3.
1501 12.35	CU	.01	.00	.01	1-01	23.95	335	.00	.00	3.
1501 12.40	CU	.01	.00	.01	1-01	24.00	336	.00	.00	3.
1501 12.45	CU	.01	.00	.01	1-01	24.05	337	.00	.00	3.
1501 12.50	CU	.01	.00	.01	1-01	24.10	338	.00	.00	3.
1501 12.55	CU	.01	.00	.01	1-01	24.15	339	.00	.00	3.
1501 13.00	CU	.01	.00	.01	1-01	24.20	340	.00	.00	3.
1501 13.05	CU	.01	.00	.01	1-01	24.25	341	.00	.00	3.
1501 13.10	CU	.01	.00	.01	1-01	24.30	342	.00	.00	3.
1501 13.15	CU	.01	.00	.01	1-01	24.35	343	.00	.00	3.
1501 13.20	CU	.01	.00	.01	1-01	24.40	344	.00	.00	3.
1501 13.25	CU	.01	.00	.01	1-01	24.45	345	.00	.00	3.
1501 13.30	CU	.01	.00	.01	1-01	24.50	346	.00	.00	3.
1501 13.35	CU	.01	.00	.01	1-01	24.55	347	.00	.00	3.
1501 13.40	CU	.01	.00	.01	1-01	24.60	348	.00	.00	3.
1501 13.45	CU	.01	.00	.01	1-01	24.65	349	.00	.00	3.

U L A C K R V E A T C M PROJECT 9457: DATE 26 MAR PAGE 8
 FLOOD HYDROGRAPH PACKAGE - HEC-1 PROGRAM M21/21-1V TIME 09:46:00 CASE

MC	DA	HR	MIN	PERIOD	HOURS	INFLO	OUTFL	STORAGE	STAGE
1.01		05		1	0.06	0.	0.	50.	674.0
1.01		10		2	0.17	0.	0.	50.	674.0
1.01		15		3	0.25	0.	0.	50.	674.0
1.01		20		4	0.33	0.	0.	50.	674.0
1.01		25		5	0.42	0.	0.	50.	674.0
1.01		30		6	0.50	0.	0.	50.	674.0
1.01		35		7	0.58	0.	0.	50.	674.0
1.01		40		8	0.67	0.	0.	50.	674.0
1.01		45		9	0.75	0.	0.	50.	674.0
1.01		50		10	0.83	0.	0.	50.	674.0
1.01		55		11	0.92	0.	0.	50.	674.0
1.01		00		12	1.00	0.	0.	50.	674.0
1.01		05		13	1.08	0.	0.	50.	674.0
1.01		10		14	1.17	0.	0.	50.	674.0
1.01		15		15	1.25	0.	0.	50.	674.0
1.01		20		16	1.33	0.	0.	50.	674.0
1.01		25		17	1.42	0.	0.	50.	674.0
1.01		30		18	1.51	0.	0.	50.	674.0
1.01		35		19	1.58	0.	0.	50.	674.0
1.01		40		20	1.67	0.	0.	50.	674.0
1.01		45		21	1.75	0.	0.	50.	674.0
1.01		50		22	1.83	0.	0.	50.	674.0
1.01		55		23	1.92	0.	0.	50.	674.0
1.01		00		24	2.00	0.	0.	50.	674.0
1.01		05		25	2.08	0.	0.	50.	674.0
1.01		10		26	2.17	0.	0.	50.	674.0
1.01		15		27	2.25	0.	0.	50.	674.0
1.01		20		28	2.33	0.	0.	50.	674.0
1.01		25		29	2.42	0.	0.	50.	674.0
1.01		30		30	2.50	0.	0.	50.	674.0
1.01		35		31	2.58	0.	0.	50.	674.0
1.01		40		32	2.67	0.	0.	50.	674.0
1.01		45		33	2.75	0.	0.	50.	674.0
1.01		50		34	2.83	0.	0.	50.	674.0
1.01		55		35	2.92	0.	0.	50.	674.0
1.01		00		36	3.00	0.	0.	50.	674.0
1.01		05		37	3.08	0.	0.	50.	674.0
1.01		10		38	3.17	0.	0.	50.	674.0
1.01		15		39	3.25	0.	0.	50.	674.0
1.01		20		40	3.33	0.	0.	50.	674.0
1.01		25		41	3.42	0.	0.	50.	674.0
1.01		30		42	3.50	0.	0.	50.	674.0
1.01		35		43	3.58	0.	0.	50.	674.0
1.01		40		44	3.67	0.	0.	50.	674.0
1.01		45		45	3.75	0.	0.	50.	674.0
1.01		50		46	3.83	0.	0.	50.	674.0
1.01		55		47	3.92	0.	0.	50.	674.0
1.01		00		48	4.00	0.	0.	50.	674.0
1.01		05		49	4.08	0.	0.	50.	674.0
1.01		10		50	4.17	0.	0.	50.	674.0
1.01		15		51	4.25	0.	0.	50.	674.0
1.01		20		52	4.33	0.	0.	50.	674.0
1.01		25		53	4.42	0.	0.	50.	674.0
1.01		30		54	4.50	0.	0.	50.	674.0

U L A C K R V E A T C M PROJECT 9457: DATE 26 MAR 81 PAGE 9
 FLOOD HYDROGRAPH PACKAGE - HEC-1 PROGRAM M21/21-1V TIME 09:46:00 CASE

1.01	4.35	55	4.58	0.	0.	50.	674.0
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 1.01 4.35 29 4.25 5.0 6P4.0
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1.01	10.20	124	10.33	1.	64.0
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1.01	10.30	126	10.50	1.	64.0
1.01	10.35	127	10.58	1.	64.0
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1.01	10.55	131	10.92	2.	64.0
1.01	11.00	132	11.00	2.	64.0
1.01	11.05	133	11.08	2.	64.0
1.01	11.10	134	11.17	2.	64.0
1.01	11.15	135	11.25	3.	64.0
1.01	11.20	136	11.33	3.	64.0
1.01	11.25	137	11.42	3.	64.0
1.01	11.30	138	11.50	4.	64.0
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1.01	12.20	148	12.33	9.	64.0
1.01	12.25	149	12.42	9.	64.0
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1.01	12.35	151	12.58	10.	64.0
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1.01	13.30	162	13.50	16.	64.0
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1.01	13.50	166	13.83	18.	64.0

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1.01	13.55	167	13.92	18.	64.0
1.01	13.55	167	13.92	18.	64.0

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1.01	13.45	163	13.75	16.	33.	6P5.2
1.01	13.50	166	13.83	15.	32.	6P5.1
1.01	13.55	167	13.92	14.	31.	6P5.1
1.01	14.00	169	14.08	10.	29.	6P5.1
1.01	14.10	170	14.17	10.	28.	6P5.1C
1.01	14.15	171	14.25	10.	27.	6P5.1C
1.01	14.20	172	14.33	10.	26.	6P5.1C
1.01	14.25	173	14.42	10.	25.	6P5.1C
1.01	14.30	174	14.50	10.	24.	6P4.9
1.01	14.35	175	14.58	10.	23.	6P4.9
1.01	14.40	176	14.67	10.	22.	6P4.9
1.01	14.45	177	14.75	10.	21.	6P4.8
1.01	14.50	178	14.83	10.	20.	6P4.8
1.01	14.55	179	14.92	10.	20.	6P4.8
1.01	15.00	180	15.00	10.	19.	6P4.8
1.01	15.05	181	15.08	10.	18.	6P4.7
1.01	15.10	182	15.17	9.	18.	6P4.7
1.01	15.15	183	15.25	9.	17.	6P4.7
1.01	15.20	184	15.33	8.	16.	6P4.7
1.01	15.25	185	15.42	8.	15.	6P4.7
1.01	15.30	186	15.50	8.	14.	6P4.7
1.01	15.35	187	15.58	8.	14.	6P4.7
1.01	15.40	188	15.67	7.	13.	6P4.7
1.01	15.45	189	15.75	7.	13.	6P4.7
1.01	15.50	190	15.83	7.	12.	6P4.7
1.01	15.55	191	15.92	7.	11.	6P4.7
1.01	16.00	192	16.00	7.	11.	6P4.7
1.01	16.05	193	16.08	7.	10.	6P4.7
1.01	16.10	194	16.17	7.	10.	6P4.7
1.01	16.15	195	16.25	7.	9.	6P4.7
1.01	16.20	196	16.33	7.	9.	6P4.7
1.01	16.25	197	16.42	7.	8.	6P4.7
1.01	16.30	198	16.50	7.	8.	6P4.7
1.01	16.35	199	16.58	7.	7.	6P4.7
1.01	16.40	200	16.67	7.	7.	6P4.7
1.01	16.45	201	16.75	7.	6.	6P4.7
1.01	16.50	202	16.83	7.	6.	6P4.7
1.01	16.55	203	16.92	7.	5.	6P4.6
1.01	17.00	204	17.00	7.	5.	6P4.6
1.01	17.05	205	17.08	7.	5.	6P4.6
1.01	17.10	206	17.17	7.	4.	6P4.6
1.01	17.15	207	17.25	7.	4.	6P4.6
1.01	17.20	208	17.33	7.	4.	6P4.6
1.01	17.25	209	17.42	7.	3.	6P4.6
1.01	17.30	210	17.50	7.	3.	6P4.6
1.01	17.35	211	17.58	7.	3.	6P4.6
1.01	17.40	212	17.67	7.	2.	6P4.6
1.01	17.45	213	17.75	7.	2.	6P4.6
1.01	17.50	214	17.83	7.	2.	6P4.6
1.01	17.55	215	17.92	7.	1.	6P4.6
1.01	18.00	216	18.00	7.	1.	6P4.6
1.01	18.05	217	18.08	7.	1.	6P4.6
1.01	18.10	218	18.17	6.	1.	6P4.5
1.01	18.15	219	18.25	6.	1.	6P4.5
1.01	18.20	220	18.33	6.	1.	6P4.5
1.01	18.25	221	18.42	6.	1.	6P4.5
1.01	18.30	222	18.50	4.	1.	6P4.5
1.01	18.35	223	18.58	4.	1.	6P4.5
1.01	18.40	224	19.06	4.	1.	6P4.5

1.01	16.58	227	17.50	4.	11.	54.	6P4.5
1.01	16.46	226	18.67	4.	11.	54.	6P4.5
1.01	16.56	229	18.75	3.	11.	54.	6P4.5
1.01	16.50	228	18.83	3.	11.	54.	6P4.5
1.01	16.55	227	18.92	3.	10.	51.	6P4.5
1.01	16.50	228	19.00	3.	10.	51.	6P4.5
1.01	16.45	229	19.08	3.	10.	51.	6P4.5
1.01	16.10	230	19.17	2.	10.	51.	6P4.5
1.01	16.15	231	19.25	3.	10.	51.	6P4.4
1.01	16.20	232	19.33	3.	10.	51.	6P4.4
1.01	16.22	233	19.42	3.	9.	51.	6P4.4
1.01	16.30	234	19.50	3.	9.	51.	6P4.4
1.01	16.35	235	19.58	2.	9.	51.	6P4.4
1.01	16.40	236	19.67	3.	9.	51.	6P4.4
1.01	16.46	237	19.75	3.	9.	51.	6P4.4
1.01	16.52	238	19.83	3.	9.	51.	6P4.4
1.01	16.55	239	19.92	2.	9.	51.	6P4.4
1.01	20.00	240	20.00	3.	9.	51.	6P4.4
1.01	20.05	241	20.08	3.	8.	51.	6P4.4
1.01	20.10	242	20.17	3.	8.	51.	6P4.4
1.01	20.15	243	20.25	3.	8.	51.	6P4.4
1.01	20.20	244	20.33	3.	8.	51.	6P4.4
1.01	20.25	245	20.42	3.	8.	51.	6P4.4
1.01	20.30	246	20.50	3.	8.	51.	6P4.4
1.01	20.35	247	20.58	3.	8.	51.	6P4.4
1.01	20.40	248	20.67	3.	8.	51.	6P4.4
1.01	20.45	249	20.75	3.	8.	51.	6P4.3
1.01	20.50	250	20.83	3.	8.	51.	6P4.3
1.01	20.55	251	20.92	3.	7.	51.	6P4.3
1.01	21.00	252	21.00	3.	7.	51.	6P4.3
1.01	21.05	253	21.08	3.	7.	51.	6P4.3
1.01	21.10	254	21.17	3.	7.	51.	6P4.3
1.01	21.15	255	21.25	3.	7.	51.	6P4.3
1.01	21.20	256	21.33	3.	7.	51.	6P4.3
1.01	21.25	257	21.42	3.	7.	51.	6P4.3
1.01	21.30	258	21.50	3.	7.	51.	6P4.3
1.01	21.35	259	21.58	3.	7.	51.	6P4.3
1.01	21.40	260	21.67	3.	7.	51.	6P4.3
1.01	21.45	261	21.75	3.	7.	51.	6P4.3
1.01	21.50	262	21.83	3.	7.	51.	6P4.3
1.01	21.55	263	21.92	3.	7.	51.	6P4.3
1.01	22.00	264	22.00	3.	6.	51.	6P4.3
1.01	22.05	265	22.08	3.	6.	51.	6P4.3
1.01	22.10	266	22.17	3.	6.	51.	6P4.3
1.01	22.15	267	22.25	3.	6.	51.	6P4.3
1.01	22.20	268	22.33	3.	6.	51.	6P4.3
1.01	22.25	269	22.42	3.	6.	51.	6P4.3
1.01	22.30	270	22.51	3.	6.	51.	6P4.3
1.01	22.35	271	22.59	3.	6.	51.	6P4.3
1.01	22.40	272	22.67	3.	6.	51.	6P4.3
1.01	22.45	273	22.75	3.	6.	51.	6P4.3
1.01	22.50	274	22.83	3.	6.	51.	6P4.3
1.01	22.55	275	22.92	3.	6.	51.	6P4.3
1.01	23.00	276	23.00	3.	6.	51.	6P4.3
1.01	23.05	277	23.08	3.	6.	51.	6P4.3
1.01	23.10	278	23.17	3.	6.	51.	6P4.3

1.01	22.55	275	22.02	3.	6.	52.	694.3
1.01	23.07	276	23.00	3.	6.	52.	694.3
1.01	23.25	277	23.19	3.	6.	52.	694.3
1.01	23.10	277	23.17	3.	6.	52.	694.3

1.01	23.15	279	23.25	3.	6.	52.	694.3
1.01	23.20	280	23.35	3.	5.	52.	694.2
1.01	23.25	281	23.42	3.	5.	52.	694.2
1.01	23.30	282	23.50	3.	5.	52.	694.2
1.01	23.35	283	23.58	3.	5.	52.	694.2
1.01	23.40	284	23.67	3.	5.	52.	694.2
1.01	23.45	285	23.75	3.	5.	52.	694.2
1.01	23.50	286	23.83	3.	5.	52.	694.2
1.01	23.55	287	23.92	3.	5.	52.	694.2
1.01	24.00	288	24.00	3.	5.	52.	694.2

PEAK OUTFLOW IS 39. AT TIME 13.49 HOURS

PEAK	0-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
38.	73.	6.	8.	2314.
1.	1.	4.	4.	66.
140.00	2.17	1.90	2.09	2.09
140.00	55.04	75.97	75.97	75.97
140.00	14.	16.	16.	16.
140.00	14.	20.	20.	20.

HUNTOFF SUMMARY, AVERAGE FLOW IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)
 AREA IN SQUARE KILOMETERS (KILOMETERS)

HYDROGRAPH AT	HEAD	6-HOUR	24-HOUR	72-HOUR	AREA
DA	185.	20.	9.	9.	.10
(5.25)(.93)(.25)(.25)(.26)
DA	35.	25.	8.	8.	.10
(1.07)(.66)(.23)(.23)(.26)

SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1

ELEVATION STOPALL OUTFLOW	INITIAL VALUE 676.00 50. 0.	SPILLWAY CREST 676.00 50. 0.	TOP OF DAM 675.50 50. 49.	DURATION		TIME OF FAILURE HOURS
				OVER TOP HOURS	MAX OUTFLOW HOURS	
				0.00	13.00	0.00

RATIO
OF
PFC
10-Year
~~4.00~~

MAXIMUM
RESERVOIR
DEPTH
OUTFLOW
CFS
675.25

MAXIMUM
STORAGE
AC-FT
59.

MAXIMUM
OUTFLOW
CFS
38.

SFIN

METTLER FIN

ATE
MED
-8