

AD-A073 610

BAKER (MICHAEL) JR INC BEAVER PA
NATIONAL DAM SAFETY PROGRAM. DRY RUN NUMBER 101 (ID NUMBER VA-1--ETC(U)
FEB 79 M BAKER

F/G 13/2

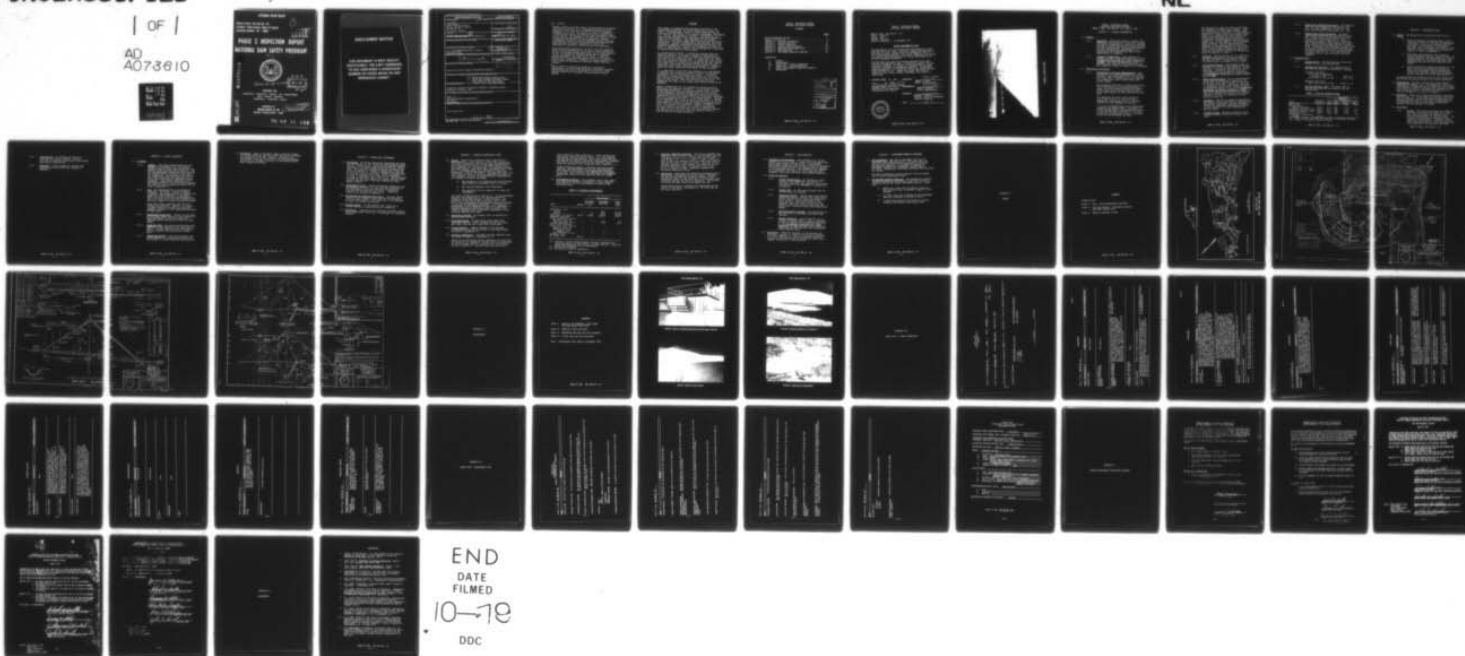
DACW65-78-D-0016

NL

UNCLASSIFIED

1 OF 1

AD
A073610





MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS-1963-A

POTOMAC RIVER BASIN

Name of Dam: Dry Run No. 101

Location: Page County, State of Virginia

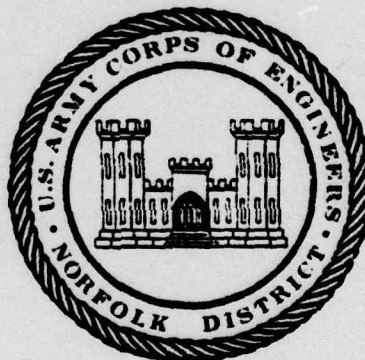
Inventory Number: VA 13902

① SC

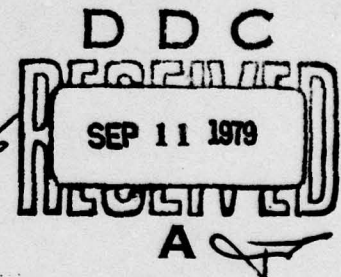
LEVEL

PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM

See 1473 in books



DACW 65-78-D-0016



AD A 073610

PREPARED FOR

NORFOLK DISTRICT CORPS OF ENGINEERS
803 FRONT STREET
NORFOLK, VIRGINIA 23510

PREPARED BY

MICHAEL BAKER, JR., INC.
BEAVER, PENNSYLVANIA 15009

410 775
DISTRIBUTION STATEMENT A
Approved for public release
Distribution Unlimited

DDC FILE COPY

79 09 10 124

DISCLAIMER NOTICE

**THIS DOCUMENT IS BEST QUALITY
PRACTICABLE. THE COPY FURNISHED
TO DDC CONTAINED A SIGNIFICANT
NUMBER OF PAGES WHICH DO NOT
REPRODUCE LEGIBLY.**

REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
1. REPORT NUMBER VA 13902	2. GOVT ACCESSION NO.	3. RECIPIENT'S CATALOG NUMBER
4. TITLE (and Subtitle) Phase I Inspection Report National Dam Safety Program Dry Run No. 101 Page County, Virginia		5. TYPE OF REPORT & PERIOD COVERED Final
7. AUTHOR(s) Michael Baker, III (10) Michael Baker, III		6. PERFORMING ORG. REPORT NUMBER
9. PERFORMING ORGANIZATION NAME AND ADDRESS		8. CONTRACT OR GRANT NUMBER(s) (15) DACW 65-78-D-0016
11. CONTROLLING OFFICE NAME AND ADDRESS U. S. Army Engineering District, Norfolk 803 Front Street Norfolk, VA 23510		10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS
12. MONITORING AGENCY NAME & ADDRESS (if different from Controlling Office)		12. REPORT DATE (11) February 1979
(9) Final Repts.		13. NUMBER OF PAGES
16. DISTRIBUTION STATEMENT (of this Report)		15. SECURITY CLASS. (of this report) Unclassified
Approved for public release; distribution unlimited.		15a. DECLASSIFICATION/DOWNGRADING SCHEDULE
17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from Report)		
(6) National Dam Safety Program, Dry Run Number 101 (ID Number VA-13902), Potomac River Basin, Dry Run, Page County, Virginia, Phase I Inspection Report.		
18. SUPPLEMENTARY NOTES Copies are obtainable from National Technical Information Service, Springfield, Virginia 22151		
19. KEY WORDS (Continue on reverse side if necessary and identify by block number) Dams - VA National Dam Safety Program Phase I Dam Safety Dam Inspection		
20. ABSTRACT (Continue on reverse side if necessary and identify by block number) (See reverse side) 470 795		

20. Abstract

Pursuant to Public Law 92-367, Phase I Inspection Reports are prepared under guidance contained in the recommended guidelines for safety inspection of dams, published by the Office of Chief of Engineers, Washington, D. C. 20314. The purpose of a Phase I investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general conditions of the dam is based upon available data and visual inspections. Detailed investigation and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify any need for such studies.

Based upon the field conditions at the time of the field inspection and all available engineering data, the Phase I report addresses the hydraulic, hydrologic, geologic, geotechnic, and structural aspects of the dam. The engineering techniques employed give a reasonably accurate assessment of the conditions of the dam. It should be realized that certain engineering aspects cannot be fully analyzed during a Phase I inspection. Assessment and remedial measures in the report include the requirements of additional indepth study when necessary.

Phase I reports include project information of the dam and appurtenances, all existing engineering data, operational procedures, hydraulic/hydrologic data of the watershed, dam stability, visual inspection report and an assessment including required remedial measures.

PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of the Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation and analyses involving topographic mapping, subsurface investigations testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued care and inspection can there be any chance that unsafe conditions be detected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established guidelines, the spillway design flood is based on the estimated "Probable Maximum Flood" for the region (flood discharges that may be expected from the most severe combination of critical meteorologic and hydrologic conditions that are reasonably possible), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the design flood should not be interpreted as necessarily posing a highly inadequate condition. The design flood provides a measure of relative spillway capacity and serves as an aide in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM

CONTENTS

	<u>Page</u>
Brief Assessment of Dam	1
Overall View of Dam	3
Section 1: Project Information	5
Section 2: Engineering Data	9
Section 3: Visual Inspection	11
Section 4: Operational Procedures	13
Section 5: Hydraulic/Hydrologic Data	15
Section 6: Dam Stability	19
Section 7: Assessment/Remedial Measures	21

Appendices

- I. Plates
- II. Photographs
- III. Check List - Visual Inspection
- IV. Check List - Engineering Data
- V. Annual Maintenance Inspection Reports
- VI. References

Accession For	
NTIS GAMA	<input checked="" type="checkbox"/>
DDC TAB	<input type="checkbox"/>
Unannounced	<input type="checkbox"/>
Justification	
By	
Distribution/	
Availability Codes	
Dist.	Availand/or special
A	23

NAME OF DAM: DRY RUN NO. 101

PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM

Name of Dam: Dry Run No. 101
State: Virginia
County: Page
Stream: Dry Run
Date of Inspection: 14 November 1978

BRIEF ASSESSMENT OF DAM

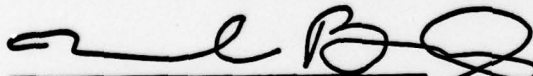
Dry Run Dam No. 101 is an earth dam approximately 67 feet high and 2100 feet long. The dam is owned and operated by the Town of Luray, Virginia and was designed by the U.S. Soil Conservation Service. The visual inspection and review of as-built drawings indicate no serious deficiencies requiring emergency attention.

The spillway will pass the Probable Maximum Flood (PMF) without overtopping the dam. A stability analysis was not available; however, no evidence of distress due to slope stability problems or seepage was observed. Recommended remedial measures to be scheduled during the annual maintenance program are to: remove any large logs from the reservoir area, repair several footpaths, and install a staff gage.

MICHAEL BAKER, JR., INC.

SUBMITTED:

Original signed by
JAMES A. WALSH



James A. Walsh
Chief, Design Branch
Original signed by
ZANE M. GOODWIN

Michael Baker, III, P.E.
Chairman of the Board and
Chief Executive Officer

RECOMMENDED:

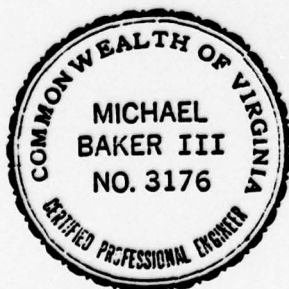
Zane M. Goodwin
Chief, Engineering by:

APPROVED:

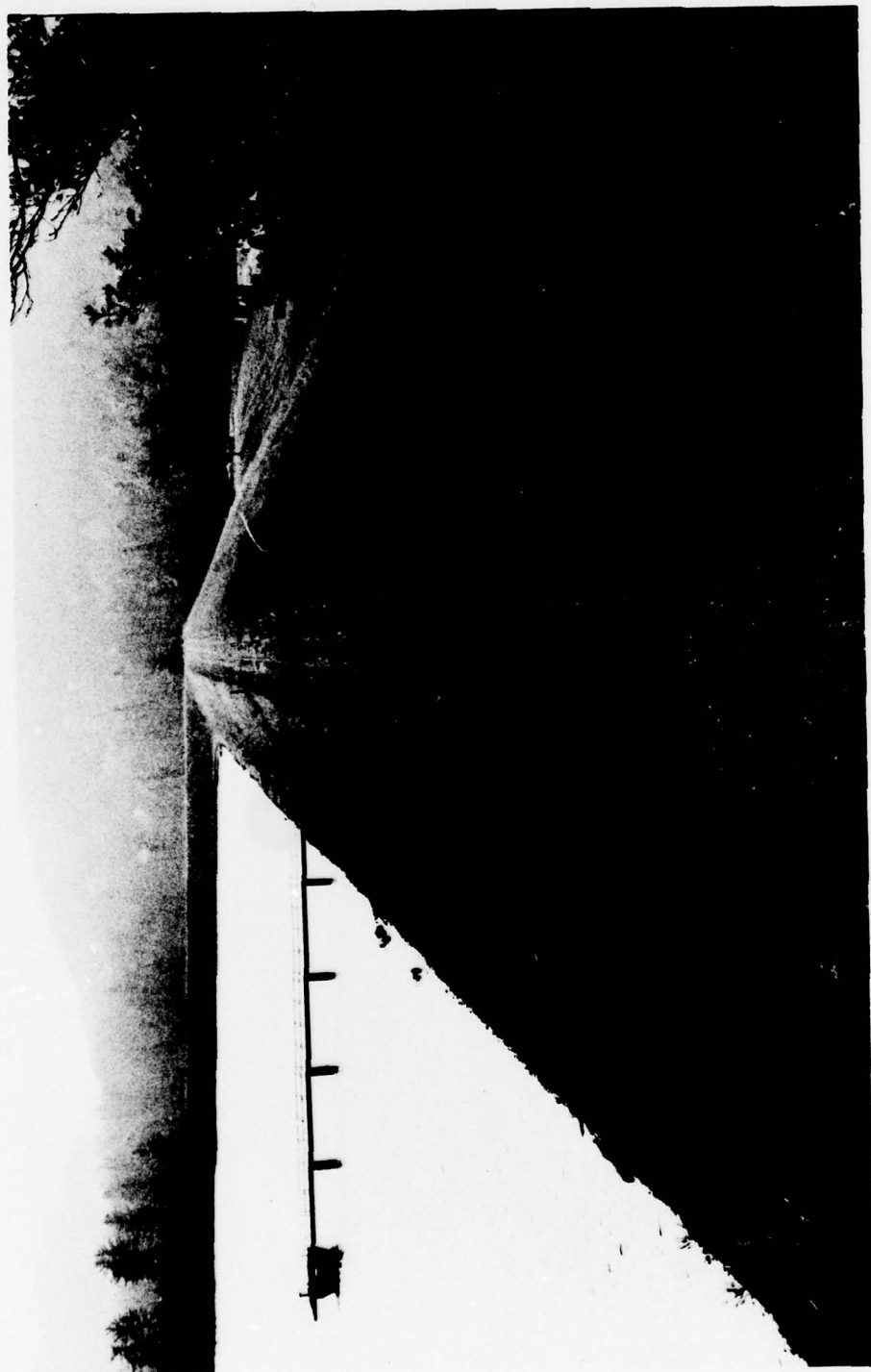
Douglas L. Haller
Douglas L. Haller
Colonel, Corps of Engineers
District Engineer

Date:

FEB 14 1979



NAME OF DAM: 1 DRY RUN NO. 101



OVERALL VIEW OF DAM

PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM
NAME OF DAM: DRY RUN NO. 101 ID# VA 13902

SECTION 1 - PROJECT INFORMATION

1.1 General

- 1.1.1 Authority: Public Law 92-367, 8 August 1972, authorized the Secretary of the Army, through the Corps of Engineers to initiate a national program of safety inspections of dams throughout the United States. The Norfolk District has been assigned the responsibility of supervising the inspection of dams in the Commonwealth of Virginia.
- 1.1.2 Purpose of Inspection: The purpose is to conduct a Phase I inspection according to the Recommended Guidelines for Safety Inspection of Dams. The main responsibility is to expeditiously identify those dams which may be a potential hazard to human life or property.

1.2 Description of Project

- 1.2.1 Description of Dam and Appurtenances: Dry Run Dam No. 101 (Lake Arrowhead) is a zoned earthfill dam approximately 67 feet high and 2100 feet long. Seepage control is provided by an impervious core, grout curtain and cutoff trench.

The principal spillway is a drop-inlet structure consisting of a reinforced concrete riser, 30 inch diameter reinforced concrete outlet pipe, and a riprap lined stilling basin approximately 60 feet long and 25 feet wide.

The emergency spillway (crest elevation 1151.6 feet M.S.L.), a 375 foot wide, vegetated, side-channel type, is located outside the left abutment of the dam.

A 10 inch corrugated metal pipe (perforated) comprises the seepage drain. The pipe is located downstream of the centerline profile of the dam and exits adjacent to a 30 inch reinforced concrete pipe, which is the main outlet of the principal spillway.

NAME OF DAM: DRY RUN NO. 101

The reservoir is used for flood control and was designed for future water supply. There is a side-inlet on the riser with an invert elevation of 1142.6 feet M.S.L. which controls normal pool elevation and a high-stage riser crest at an elevation of 1146.3 feet M.S.L. Six manually operated gate valves allow intake to a 16 inch diameter cast-iron water supply pipe, which flares to 18 inch diameter where it exits. The 30 inch slide gate anchored on the upstream face of the riser, which has an invert elevation of 1101.6 feet M.S.L., can be used to drain the reservoir. The plan and typical sections of the dam are given on Plates 1, 2 and 3.

- 1.2.2 Location: Dry Run Dam No. 101 is located on Dry Run approximately 3 miles southeast of the Town of Luray in Page County, Virginia. A Location Plan is included in this report.
- 1.2.3 Size Classification: The maximum height of the dam is 67 feet and the reservoir storage capacity to the top of dam elevation is 1232 acre-feet. Therefore, the dam is in the "intermediate" size category as defined by the Recommended Guidelines for Safety Inspection of Dams.
- 1.2.4 Hazard Classification: Due to the proximity of the Town of Luray with a population of 3800, many lives could be lost in the event of failure of the dam. Therefore, this dam is considered in the "high" hazard category as defined by Section 2.1.2 of the Recommended Guidelines for Safety Inspection of Dams. The hazard classification used to categorize dams is a function of location only and has nothing to do with its stability or probability of failure.
- 1.2.5 Ownership: The dam is owned and operated by the Town of Luray, Page County, Virginia with maintenance assistance from the Shenandoah Valley Soil and Water Conservation District and the regional U.S. Soil Conservation Service (SCS).
- 1.2.6 Purpose of Dam: The dam is used for flood control purposes within the Potomac River Basin.

NAME OF DAM: DRY RUN NO. 101

- 1.2.7 Design and Construction History: The existing facility was designed for the owner by the SCS. The dam, completed in June 1971, was built by the English Construction Co., Inc.
- 1.2.8 Normal Operational Procedures: No formal operational procedures are followed for this dam. Normal pool is maintained by an inlet on one side of the riser with an invert elevation of 1142.60 feet M.S.L. Since the dam is multipurpose, flood control and future water supply, the principal spillway (riser) crest is located at an elevation of 1146.3 feet M.S.L. with excess flow diverted through the emergency spillway which has a crest elevation of 1151.60 feet M.S.L. It is not known how frequently the 30 inch slide gate has been operated.

1.3 Pertinent Data

- 1.3.1 Drainage Area: The drainage area of Dry Run Dam No. 101 is 2.62 square miles.

- 1.3.2 Discharge at Dam Site: The maximum flow at the dam site through the spillway is unknown.

Principal Spillway:

Pool level at emergency
spillway crest 134 c.f.s.
Pool level at top of dam . . . 141 c.f.s.

Emergency Spillway:

Pool level at top of dam . . 15,540 c.f.s.

- 1.3.3 Dam and Reservoir Data: Pertinent data on the dam and reservoir are shown in the following table:

TABLE 1.1 DAM AND RESERVOIR DATA

Item	Elevation feet M.S.L.	Area acres	Reservoir Capacity		Length feet
			Acre- feet(a)	Watershed inches	
Top of dam	1157.6	56.4	1232	8.82	1300
Maximum pool, design surcharge	1154.0	51.0	1032	7.39	-
Emergency spillway crest	1151.6	47.0	922	7.10	1100
Principal spillway crest	1146.3	39.0	692	4.95	-
Normal pool (b)	1142.6	33.8	552	3.95	800
Streambed at centerline of dam	1090.4	-	-	-	-
(a) Total storage, includes 102 acre-feet of sediment storage.					
(b) Invert of low-level intake to riser.					

NAME OF DAM: DRY RUN NO. 101

SECTION 2 - ENGINEERING DATA

2.1 Design: The design data reviewed included the following:

- 1) As-built drawings indicating plans, elevations and sections of the dam and appurtenant structures. Logs of test borings and test pits were also included in the as-built drawings. Plan and typical sections as taken from the as-built drawings are included as Plates 1, 2 and 3 in Appendix I.
- 2) Hydrologic and hydraulic data including outlet discharge capacity, reservoir area and storage capacity, and hydrograph and routing determinations for principal spillway, the emergency spillway and the freeboard hydrographs.
- 3) Annual Maintenance Inspection Reports for the past five years conducted by the Town of Luray with assistance from the Shenandoah Valley Soil and Water Conservation District and the regional SCS office (see Appendix V).

All design data was obtained from the SCS and has been filed with the Norfolk District for future reference.

2.2 Construction: The dam, constructed by the English Construction Company, Inc., was completed in June 1971. Construction records were not available for this inspection; however, as-built drawings were reviewed and were verified in the field. Construction records are on file in Washington, District of Columbia.

2.3 Operation: There are no formal operational procedures for this dam. It is not known how regularly the slide gate is operated. Annual maintenance inspections are conducted by the Town of Luray with the assistance of the Shenandoah Valley Soil and Water Conservation District and the regional SCS office.

2.4 Evaluation

2.4.1 Design: The as-built drawings were adequate to assess the structural stability of the dam. The hydrologic and hydraulic data provided was adequate for design review. The as-built drawings show a relatively thin, impervious core section. Foundation conditions were determined by using the subsurface data presented in the as-built drawings.

NAME OF DAM: DRY RUN NO. 101

- 2.4.2 Construction: No construction logs were available for review; however, as-built drawings do indicate modifications and changes made during construction.
- 2.4.3 Operation: Annual inspection reports were available for review and are included in Appendix V.

SECTION 3 - VISUAL INSPECTION

3.1 Findings

- 3.1.1 General: The field investigation was conducted on 14 November 1978. No unusual weather conditions were experienced and the reservoir was at normal pool elevation. The dam and appurtenant structures were found to be in good overall condition at the time of the inspection. The problems noted during the visual inspection are considered to be minor and do not require immediate remedial treatment. Significant deficiencies are described briefly in the following paragraphs. The complete visual inspection check list is given in Appendix III.
- 3.1.2 Dam: The embankment was in good physical condition. No cracks, unusual movement, sloughing, appreciable erosion or seepage was observed on the embankment or abutments. There are several bare and eroded pathways and a cut slope above the right abutment that should be seeded to prevent increased erosion.
- Minor bare areas resulting from vehicular traffic were observed. However, no erosion in these tracks was evident and therefore remedial treatment is not considered necessary at this time.
- 3.1.3 Appurtenant Structures: Several of the stems to the water supply gates are disconnected or bent (see Photo 1) and would need to be repaired before they could be used for water supply.
- 3.1.4 Reservoir Area: No serious deficiencies were observed in the reservoir area (see Photos 2 and 3). A staff gage should be installed to monitor reservoir elevations above normal pool.
- 3.1.5 Downstream Channel: The stilling basin and outlet channel are functioning properly and the riprap is in good condition (see Photo 4).

NAME OF DAM: DRY RUN NO. 101

3.2 Evaluation: None of the above items is serious enough to warrant immediate attention since they do not threaten the integrity of the dam. However, the repair and replacement items are considered to be good maintenance and should be attended to as part of the annual maintenance inspection program.

NAME OF DAM: DRY RUN NO. 101

SECTION 4 - OPERATIONAL PROCEDURES

- 4.1 Procedures: No formal operational procedures are used for Dry Run Dam No. 101, since the dam presently serves only as a flood control structure and does not require operation of its water supply gates and valves at this time. The reservoir remains at normal pool elevation of 1142.6 feet M.S.L., which is maintained by the low-level inlet crest on the riser. There is an additional 9.0 feet of storage to the crest of the emergency spillway at elevation 1151.6 feet M.S.L. The difference in stage is less than typical for single purpose flood control dams because of the dam's design for water supply storage.
- 4.2 Maintenance of Dam: Annual maintenance inspections are performed by the Town of Luray with the assistance of the Shenandoah Valley Soil and Water Conservation District and the regional SCS office. Inspection reports are included in Appendix V.
- 4.3 Maintenance of Operating Facilities: The gate valves on the 16 inch diameter riser pipe are not presently used for water supply. The slide gate is not operated to check for proper functioning.
- 4.4 Warning System: At the present time, there is no warning system or evacuation plan in operation.
- 4.5 Evaluation: Considering the functions presently served by the operational facilities, maintenance is considered adequate.

NAME OF DAM: DRY RUN NO. 101

SECTION 5 - HYDRAULIC/HYDROLOGIC DATA

5.1 Design: Normal pool (elevation 1142.6 feet M.S.L.) maintained by a 1 foot high by 4 foot wide orifice on the right (northeast) side of the riser was established at an elevation to store 0.73 watershed inches (100-year sediment) plus an additional 3.23 watershed inches for water supply. The riser crest (elevation 1146.3 feet M.S.L.) was established at an elevation to store an additional 1 inch of floodwater. The capacity (134 c.f.s. with reservoir level at crest of emergency spillway) of the principal spillway was established by consideration of a number of factors including:

- 1) The capability of evacuating the flood storage space within a reasonable time (\pm 10 days).
- 2) Not passing damaging flows downstream.
- 3) The capability of the reservoir to store the floodwaters.

The crest (elevation 1151.6 feet M.S.L.) of the emergency spillway was established at the maximum elevation needed to store the 100-year, 10-day rainfall. The elevation of the top of dam (elevation 1157.6 feet M.S.L.) was established by the maximum elevation reached in passing the freeboard hydrograph. The freeboard hydrograph is that computed from rainfall comparable to Probable Maximum Precipitation (PMP) as used by the Corps of Engineers and is therefore comparable to the Probable Maximum Flood (PMF).

5.2 Hydrologic Records: No rainfall data of stream flow records were available.

5.3 Flood Experience: No exact high water marks were available; however, local citizens stated that water has risen over the top of the riser in the past.

5.4 Flood Potential: Design features of the dam were established by the SCS by routing various hydrographs as noted in Paragraph 5.1.

5.5 Reservoir Regulation: Pertinent dam and reservoir data are shown in Table 1.1, paragraph 1.3.3.

Except for future water supply, regulation of flow from the reservoir is automatic. Normal flows are maintained by the low stage orifice in the riser at an elevation of 1142.6 feet M.S.L. and the high stage drop-inlet

with a crest of 1146.3 feet M.S.L. Water flowing into the inlets flows through the dam in a 30 inch diameter reinforced concrete conduit. Water also flows past the dam through an ungated, vegetated, side-channel, emergency spillway in the event water in the reservoir rises above the spillway crest (elevation 1151.6 feet M.S.L.).

Outlet discharge capacity, reservoir area and storage capacity, and hydrograph and routing determinations were obtained from reports and computations furnished by the SCS. The routing of the emergency and freeboard hydrographs began with the reservoir level at normal pool.

- 5.6 Overtopping Potential: The probable rise in the reservoir and other pertinent information on the reservoir performance in various hydrographs is shown in the following table:

TABLE 5.1 RESERVOIR PERFORMANCE

Item	Normal	Hydrographs		
		Principal Spillway (a)	Emergency Spillway	Free-board (b)
Peak flow, c.f.s.				
Inflow	-	-	6833	18,153
Outflow	-	141	3360	15,810
Peak elev., ft. M.S.L.	1142.6	1156.6	1154.0	1157.6
Emergency spillway (elev. 1151.6 ft. M.S.L.)				
Depth of flow, ft.	-	-	2.4	6.0
Avg. velocity, f.p.s.	-	-	6.5	11.0
Non-overflow section (elev. 1157.6 ft. M.S.L.)				
Depth of flow, ft.	-	-	-	-
Duration of overtopping, hrs.	-	-	-	-
Avg. velocity, f.p.s.	-	-	-	-
Tailwater elev., ft. M.S.L. (c)	1191.0	-	-	-

- (a) 100-year, 10-day volume produces the most conservatively large indication of flood control storage required. Detailed discharge hydrograph was not determined.
(b) PMF by COE standards.
(c) Tailwater at time of inspection.

NAME OF DAM: DRY RUN NO. 101

5.7 Reservoir Emptying Potential: The time to drawdown the reservoir level from the crest of the emergency spillway (discharge of 134 c.f.s.) to the crest of the riser (discharge of 37 c.f.s.) is about 1.1 days. While the time to draw the reservoir down from the riser crest to the orifice crest is about 8.2 days. With the 30 inch diameter reservoir drain opened, the reservoir can be drained from normal pool to elevation 1101.6 feet M.S.L. in about 2.7 days. Reservoir drawdown was estimated neglecting inflow.

5.8 Evaluation: Hydrologic and hydraulic determinations of the project as prepared by the SCS appear reasonable. The dam and spillway are designed to pass a flood essentially equal to PMF which would be developed under standards used by the Corps of Engineers. The project will pass the PMF without overtopping the dam.

Conclusions pertain to present day conditions and the effect of the future development on the hydrology has not been considered.

SECTION 6 - DAM STABILITY

6.1 Foundation and Abutments: The foundation of the dam consists of 7 to 28 feet of clayey and silty sands with gravel and boulders overlying hard, cross-bedded, argillaceous sandstone with traces of shale and quartzite. The cutoff trench provided for seepage control extends to the weathered bedrock. Both abutments are composed of a relatively thin mantle of silty sand and rock fragments above gray and brown cross bedded sandstone.

6.2 Stability Analysis

6.2.1 Visual Observations: No evidence of instability in the embankment, cut slopes or concrete structures was observed. No seepage was observed in the embankment, abutments or foundation.

6.2.2 Design Data: No data was available that included stability analyses.

6.2.3 Operating Records: Recent annual maintenance inspection reports indicate that there are no serious deteriorative conditions except for minor erosion. High water marks on the upstream slope show that the level of the reservoir has risen several feet above the normal pool elevation without any significant damage.

6.2.4 Post-Construction Changes: No alterations of the dam since it has been constructed were apparent.

6.2.5 Seismic Stability: Dry Run Dam No. 101 is located in Seismic Zone 2, and is considered to have no hazard from earthquakes according to the Recommended Guidelines for Safety Inspection of Dams provided static stability conditions are satisfactory and conventional safety margins exist.

6.3 Evaluation: Stability analyses of the dam were not available. However, a study of the as-built drawings, the field observations, and field studies indicates no serious distressed external conditions which would suggest instability of the structure.

NAME OF DAM: DRY RUN NO. 101

SECTION 7 - ASSESSMENT/REMEDIAL MEASURES

- 7.1 Dam Assessment: The dam, as designed, will not be overtopped under PMF conditions. The hydrologic and hydraulic data available were sufficient to evaluate the spillway capacity. No evidence of seepage or embankment distress were observed. Design reports for soils, foundations, and stabilities were not available for review.

The dam is generally in good condition with the exception of minor maintenance items.

- 7.2 Recommended Remedial Measures: The inspection revealed certain preventative maintenance items which should be scheduled during the annual maintenance period. These are:

- 1) Remove all large logs and debris located in the reservoir area to prevent clogging of the intakes.
- 2) All bare areas and footpaths on the embankment should be reseeded to prevent erosion.
- 3) A staff gage should be installed to monitor reservoir elevations above normal pool.

NAME OF DAM: DRY RUN NO. 101

APPENDIX I

PLATES

CONTENTS

Location Plan

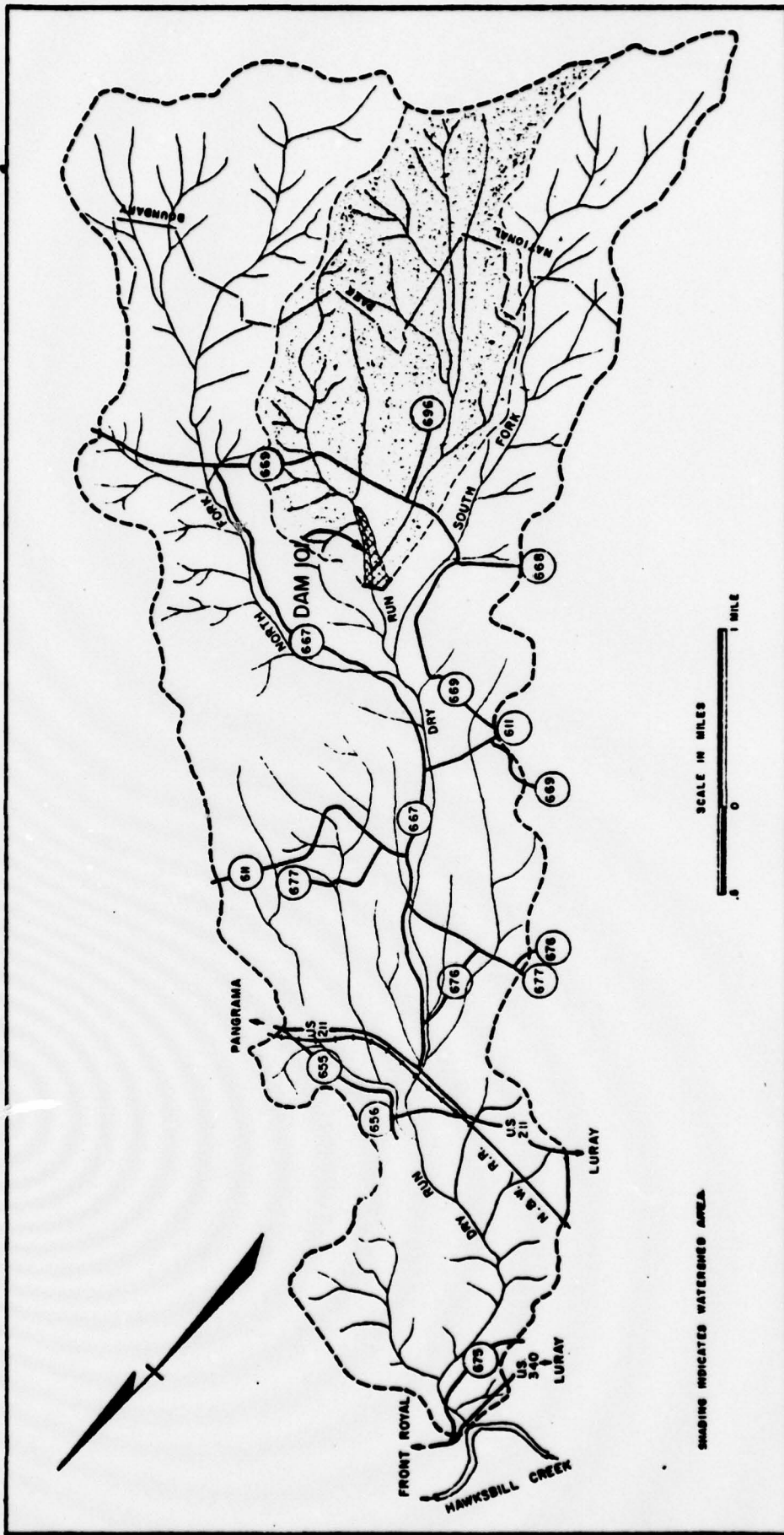
Plate 1: Plan - Dam and Emergency Spillway

Plate 2: Plan and Section - Principal Spillway
and Water Supply Line

Plate 3: Typical Sections of Dam

NAME OF DAM: DRY RUN NO. 101

Dry Run Dam No. 101



LOCATION PLAN

DRY RUN DAM NO. 101

Station	10+00	11+00	12+00
Elev.	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00

Station	10+00	11+00	12+00
Elev.	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00

Station	10+00	11+00	12+00
Elev.	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00
Ch. 20	1142.00	1142.00	1142.00

Drain Pipe and
Filter Material
See Contract Mod.
No. 15



Types of Emergency Spillway
Covered Drain

NOTE
1. 400' of 24" dia. covered drain
2. 400' of 24" dia. covered drain
3. 400' of 24" dia. covered drain



CONTRACTOR SHALL MAKE TRANSITION
AT EXISTING STREAM AS REQUIRED.

APPROX LOCATION
EXISTING STREAM

LIMITS OF CLEARING & GRUBBING

LEGEND

- FENCE
- POWER LINE
- EXISTING CONTOURS
- PROPOSED CONTOURS

BORROW AREA 'A'

AS BUILT

JUN 2 8 1971

PLATE 1

R. STUART ROYER & ASSOCIATES
CONSULTING ENGINEERS
RICHMOND, VIRGINIA

PROJECT NO. 6600



DAM NO 101, DRY RUN
DRY RUN WATERSHED.
PAGE COUNTY, VA.

PLAN — DAM & EMERGENCY SPILLWAY

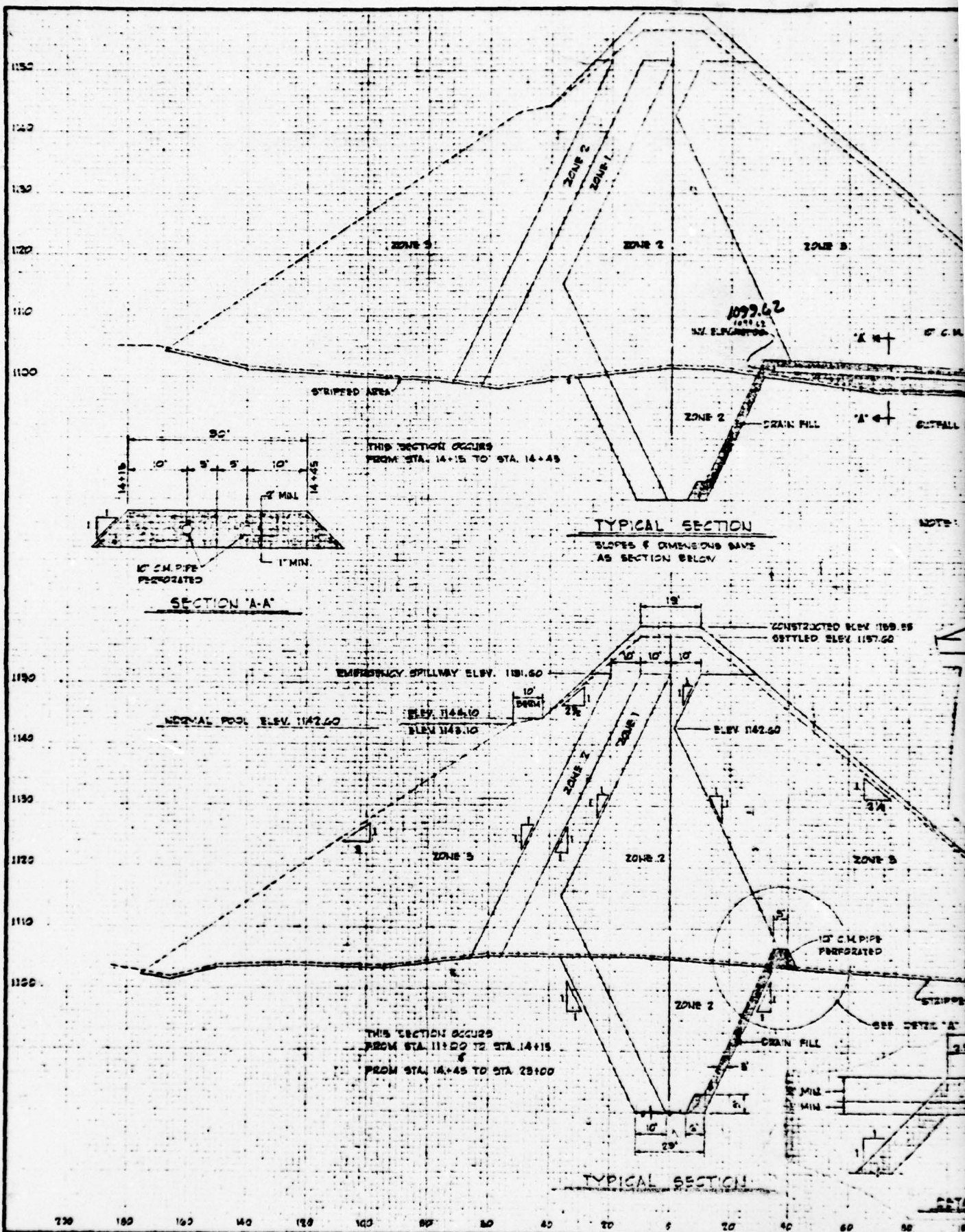
U. S. DEPARTMENT OF AGRICULTURE
SOIL CONSERVATION SERVICE

Designed by A. L. SUTHER	Date 9/68	Reviewed by T. H. ...
Drawn by K. J. ...	Date 5/68	Checked by T. H. ...
Traced by L. B. ...	Date 2/69	Clearing No. 3
		VA-516-P

2

SEE JOINTS

JOINT NO.	STATION	ELEVATION	REMARKS
1	100+00	100.00	
2	100+10	100.10	
3	100+20	100.20	
4	100+30	100.30	
5	100+40	100.40	
6	100+50	100.50	
7	100+60	100.60	
8	100+70	100.70	
9	100+80	100.80	
10	100+90	100.90	
11	101+00	101.00	
12	101+10	101.10	
13	101+20	101.20	
14	101+30	101.30	
15	101+40	101.40	
16	101+50	101.50	
17	101+60	101.60	
18	101+70	101.70	
19	101+80	101.80	
20	101+90	101.90	
21	102+00	102.00	
22	102+10	102.10	
23	102+20	102.20	
24	102+30	102.30	
25	102+40	102.40	
26	102+50	102.50	
27	102+60	102.60	
28	102+70	102.70	
29	102+80	102.80	
30	102+90	102.90	
31	103+00	103.00	
32	103+10	103.10	
33	103+20	103.20	
34	103+30	103.30	
35	103+40	103.40	
36	103+50	103.50	
37	103+60	103.60	
38	103+70	103.70	
39	103+80	103.80	
40	103+90	103.90	
41	104+00	104.00	
42	104+10	104.10	
43	104+20	104.20	
44	104+30	104.30	
45	104+40	104.40	
46	104+50	104.50	
47	104+60	104.60	
48	104+70	104.70	
49	104+80	104.80	
50	104+90	104.90	
51	105+00	105.00	
52	105+10	105.10	
53	105+20	105.20	
54	105+30	105.30	
55	105+40	105.40	
56	105+50	105.50	
57	105+60	105.60	
58	105+70	105.70	
59	105+80	105.80	
60	105+90	105.90	
61	106+00	106.00	
62	106+10	106.10	
63	106+20	106.20	
64	106+30	106.30	
65	106+40	106.40	
66	106+50	106.50	
67	106+60	106.60	
68	106+70	106.70	
69	106+80	106.80	
70	106+90	106.90	
71	107+00	107.00	
72	107+10	107.10	
73	107+20	107.20	
74	107+30	107.30	
75	107+40	107.40	
76	107+50	107.50	
77	107+60	107.60	
78	107+70	107.70	
79	107+80	107.80	
80	107+90	107.90	
81	108+00	108.00	
82	108+10	108.10	
83	108+20	108.20	
84	108+30	108.30	
85	108+40	108.40	
86	108+50	108.50	
87	108+60	108.60	
88	108+70	108.70	
89	108+80	108.80	
90	108+90	108.90	
91	109+00	109.00	
92	109+10	109.10	
93	109+20	109.20	
94	109+30	109.30	
95	109+40	109.40	
96	109+50	109.50	
97	109+60	109.60	
98	109+70	109.70	
99	109+80	109.80	
100	109+90	109.90	
101	110+00	110.00	
102	110+10	110.10	
103	110+20	110.20	
104	110+30	110.30	
105	110+40	110.40	
106	110+50	110.50	
107	110+60	110.60	
108	110+70	110.70	
109	110+80	110.80	
110	110+90	110.90	
111	111+00	111.00	
112	111+10	111.10	
113	111+20	111.20	
114	111+30	111.30	
115	111+40	111.40	
116	111+50	111.50	
117	111+60	111.60	
118	111+70	111.70	
119	111+80	111.80	
120	111+90	111.90	
121	112+00	112.00	
122	112+10	112.10	
123	112+20	112.20	
124	112+30	112.30	
125	112+40	112.40	
126	112+50	112.50	
127	112+60	112.60	
128	112+70	112.70	
129	112+80	112.80	
130	112+90	112.90	
131	113+00	113.00	
132	113+10	113.10	
133	113+20	113.20	
134	113+30	113.30	
135	113+40	113.40	
136	113+50	113.50	
137	113+60	113.60	
138	113+70	113.70	
139	113+80	113.80	
140	113+90	113.90	
141	114+00	114.00	
142	114+10	114.10	
143	114+20	114.20	
144	114+30	114.30	
145	114+40	114.40	
146	114+50	114.50	
147	114+60	114.60	
148	114+70	114.70	
149	114+80	114.80	
150	114+90	114.90	
151	115+00	115.00	
152	115+10	115.10	
153	115+20	115.20	
154	115+30	115.30	
155	115+40	115.40	
156	115+50	115.50	
157	115+60	115.60	
158	115+70	115.70	
159	115+80	115.80	
160	115+90	115.90	
161	116+00	116.00	
162	116+10	116.10	
163	116+20	116.20	
164	116+30	116.30	
165	116+40	116.40	
166	116+50	116.50	
167	116+60	116.60	
168	116+70	116.70	
169	116+80	116.80	
170	116+90	116.90	
171	117+00	117.00	
172	117+10	117.10	
173	117+20	117.20	
174	117+30	117.30	
175	117+40	117.40	
176	117+50	117.50	
177	117+60	117.60	
178	117+70	117.70	
179	117+80	117.80	
180	117+90	117.90	
181	118+00	118.00	
182	118+10	118.10	
183	118+20	118.20	
184	118+30	118.30	
185	118+40	118.40	
186	118+50	118.50	
187	118+60	118.60	
188	118+70	118.70	
189	118+80	118.80	
190	118+90	118.90	
191	119+00	119.00	
192	119+10	119.10	
193	119+20	119.20	
194	119+30	119.30	
195	119+40	119.40	
196	119+50	119.50	
197	119+60	119.60	
198	119+70	119.70	
199	119+80	119.80	
200	119+90	119.90	
201	120+00	120.00	
202	120+10	120.10	
203	120+20	120.20	
204	120+30	120.30	
205	120+40	120.40	
206	120+50	120.50	
207	120+60	120.60	
208	120+70	120.70	
209	120+80	120.80	
210	120+90	120.90	
211	121+00	121.00	
212	121+10	121.10	
213	121+20	121.20	
214	121+30	121.30	
215	121+40	121.40	
216	121+50	121.50	
217	121+60	121.60	
218	121+70	121.70	
219	121+80	121.80	
220	121+90	121.90	
221	122+00	122.00	
222	122+10	122.10	
223	122+20	122.20	
224	122+30	122.30	
225	122+40	122.40	
226	122+50	122.50	
227	122+60	122.60	
228	122+70	122.70	
229	122+80	122.80	
230	122+90	122.90	
231	123+00	123.00	
232	123+10	123.10	
233	123+20	123.20	
234	123+30	123.30	
235	123+40	123.40	
236	123+50	123.50	
237	123+60	123.60	
238	123+70	123.70	
239	123+80	123.80	
240	123+90	123.90	
241	124+00	124.00	
242	124+10	124.10	
243	124+20	124.20	
244	124+30	124.30	
245	124+40	124.40	
246	124+50	124.50	
247	124+60	124.60	
248	124+70	124.70	
249	124+80	124.80	
250	124+90	124.90	
251	125+00	125.00	
252	125+10	125.10	
253	125+20	125.20	
254	125+30	125.30	
255	125+40	125.40	
256	125+50	125.50	
257	125+60	125.60	
258	125+70	125.70	
259	125+80	125.80	
260	125+90	125.90	
261	126+00	126.00	
262	126+10	126.10	
263	126+20	126.20	
264	126+30	126.30	
265	126+40	126.40	
266	126+50	126.50	
267	126+60	126.60	
268	126+70	126.70	
269	126+80	126.80	
270	126+90	126.90	
271	127+00	127.00	
272	127+10	127.10	
273	127+20	127.20	
274	127+30	127.30	
275	127+40	127.40	
276	127+50	127.50	
277	127+60	127.60	
278	127+70	127.70	
279	127+80	127.80	
280	127+90	127.90	
281	128+00	128.00	
282	128+10	128.10	
283	128+20	128.20	
284	128+30	128.30	
285	128+40	128.40	
286	128+50	128.50	
287	128+60	128.60	
288	128+70	128.70	
289	128+80	128.80	
290	128+90	128.90	
291	129+00	129.00	
292	129+10	129.10	
293	129+20	129.20	
294	129+30	129.30	
295	129+40	129.40	
296	129+50	129.50	
297	129+60	129.60	
298	129+70	129.70	
299	129+80	129.80	
300	129+90	129.90	
301	130+00	130.00	
302	130+10	130.10	
303	130+20	130.20	
304	130+30	130.30	
305	130+40	130.40	
306	130+50	130.50	
307	130+60	130.60	
308	130+70	130.70	
309	130+80	130.80	
310	130+90	130.90	
311	131+00	131.00	
312	131+10	131.10	
313	131+20	131.20	
314	131+30	131.30	
315	131+40	131.40	
316	131+50	131.50	
317	131+60	131.60	
318	131+70	131.70	

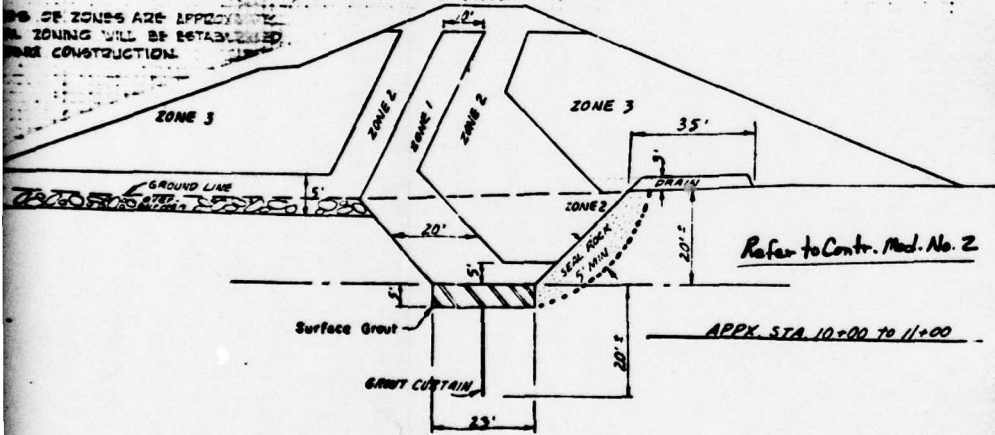


Design	Filter Material
Sieve No. 5	Passing
3"	100
2"	80-100
1 1/2"	60-80
1"	50-60
3/4"	40-50
1/2"	30-40
3/8"	20-30
2"	10-20
1"	5-10
3/4"	0-5
200	0-5

Zone	Material	Max. Rock Size (Thickness)	Max. Lift (ft.)	Required Rate (cu yd/ft)	Compaction (%)	Class	Definition
1	CL-ME material from borrow area B as shown on sheet 4	30	3	-15 to +35 of optimum	A	100% of max. moist by A.T.M.D-69 Method A	
2	Material from borrow area A, emergency spillway, and excavation common represented by T.F. 2-1 from 0' to 0.5'	30	12	See Spec. 5A	V	See Spec. 5A	
3	Material from borrow area A, emergency spillway, excavation common, and overlie material from zone 2.	30	30	See Spec. 5A	V	See Spec. 5A	

For fill adjacent to structures, max. rock size = 30"
 20 times of placement
 For typical compaction curves see sheet 16

THESE ZONES ARE APPROXIMATE
 ZONING WILL BE ESTABLISHED
 AFTER CONSTRUCTION



- 1.) Drill and grout (3) angle holes in a section at the intersection of the right abutment and the floodplain (STA 11+00) on 20' centers. Angles to be 30° to 45° from vertical directed along the dam axis.
- 2.) The cutoff trench is to be backfilled as shown and the downstream face of the slope sealed by grouting from the surface of the trench backfill. The blanket drain will then be installed as shown.
- 3.) ~~Remove~~ ^{over-} on upstream side from STA 10+00 to 11+00 to be removed and replaced with zone 2 material as shown.

PLATE 3 "AS BUILT" JUN 22 1971

R. STUART ROYER & ASSOCIATES CONSULTING ENGINEERS RICHMOND, VIRGINIA PROJECT NO. 6600		DAM NO. 101, DRY RUN DRY RUN WATERSHED PAGE COUNTY, VA. TYPICAL SECTIONS OF DAM	
U. S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE		VA-516-P	

APPENDIX II

PHOTOGRAPHS

CONTENTS

**Photo 1: Riser's Lift Pedestal, Gate Stems
and High Stage Trash Rack**

Photo 2: Reservoir Area and Riser

Photo 3: Emergency Spillway and Left Abutment

Photo 4: Outlet Pipe and Stilling Basin

Note: Photographs were taken 14 November 1978.

NAME OF DAM: DRY RUN NO. 101

DRY RUN DAM No. 101

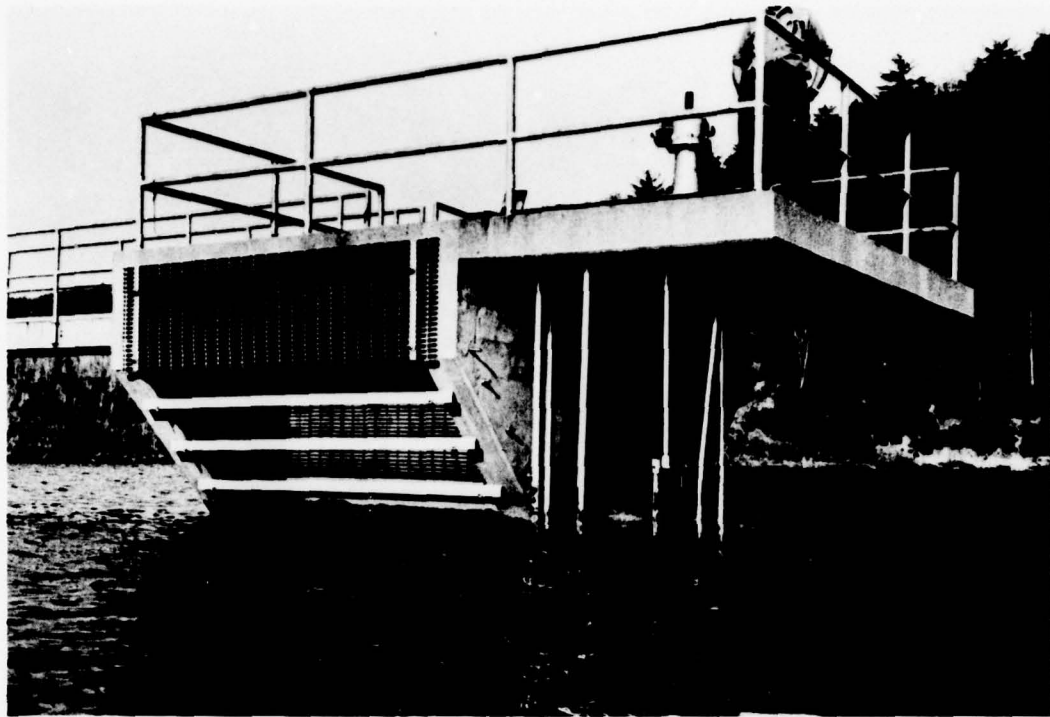


PHOTO 1. Riser's Lift Pedestal, Gate Stems and High Stage Trash Rack



PHOTO 2. Reservoir Area and Riser

DRY RUN DAM No. 101



PHOTO 3. Emergency Spillway in Left Abutment



PHOTO 4. Outlet Pipe and Stilling Basin

APPENDIX III

CHECK LIST - VISUAL INSPECTION

Check List
Visual Inspection
Phase 1

Name of Dam Dry Run No. 101 County _____ State Virginia Coordinates Lat. 3838.5
Long. 7823.5

Date Inspection 14 November 1978 Weather Sunny, Warm Temperature 70°F.

Pool Elevation at Time of Inspection 1142.5 ft. M.S.L. Tailwater at Time of Inspection 1191.0 ft. M.S.L.

HH-1

Inspection Personnel:

Michael Baker, Jr., Inc.:

T. W. Smith
T. J. Dougan
W. L. Shearer

Soil Conservation Service:

Sam Linkenhoker

T. W. Smith Recorder

EMBANKMENT

Name of Dam: DRY RUN NO. 101

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
SURFACE CRACKS	None observed.	
UNUSUAL MOVEMENT OR CRACKING AT OR BEYOND THE TOE	None observed.	
SLOUGHING OR EROSION OF EMBANKMENT AND ABUTMENT SLOPES	No sloughing was observed on embankment and abutment slopes. Minor erosion occurs in a path on upstream slope at the riser walkway and on a path at the end of the dam adjacent to the inlet of emergency spillway. The slope in a cut at the right abutment has sparse vegetation. The soil is exposed in tire tracks on the crest. There is no appreciable erosion. The dam has a thick vegetative cover.	Grass seeding is recommended for the bare areas.
VERTICAL AND HORIZONTAL ALIGNMENT OF THE CREST	Good	
RIPRAP FAILURES	There is no riprap on the embankment.	
SLOPES	The downstream and upper part of the upstream slope above the 10 ft. wide berm were constructed at a 2.5:1 ratio. The upstream slope below the normal pool elevation at the level of a 10 ft. wide berm was formed at a 3:1 ratio. There is minor scattered wood debris on the lower upstream slope.	It is recommended that the debris be removed periodically.

EMBANKMENT

Name of Dam: DRY RUN NO. 101

VISUAL EXAMINATION OF		OBSERVATIONS	REMARKS OR RECOMMENDATIONS
SLOPE AND FILL MATERIAL		According to the as-built drawings, the embankment was constructed in 3 zones including CL-M clay-silt core and a cut-off trench. The soils observed on the surface of the dam are generally comprised of brown-damp-dense silty sand, gravel, cobbles and rock fragments. According to the as-built drawings and boring logs, the soils in Zones 2 and 3 consist of clayey and silty sand with gravel, cobbles and boulders. Zone 3 also has oversize material. Traces of wood debris have been left by high water on the upstream slope.	
JUNCTION OF EMBANKMENT AND ABUTMENT, SPILLWAY AND DAM		The right abutment consists of a 1.5:1 slope of silty sand and rock fragments with two 4 x 4 ft. benches cut in the hill above the crest of the dam. Hard arkosic sandstone of the Erwin (Antietam) Formation in the Cambrian System is partially exposed with boulders on the downstream slope of the hill on the right side in a wooded area. The left side abuts against a small quartzite ridge mostly covered with a thin mantle of silty sand and rock fragments. The embankment is extended on the southeast side of the ridge adjacent to the emergency spillway.	
ANY NOTICEABLE SEEPAGE	No seepage was observed on the downstream slope or in the vicinity of the downstream toe.		
STAFF GAGE AND RECORDER		None observed.	
DRAINS	Two 10 in. C.M.P. are located on either side of outlet conduit. No flow was observed from either drain at the time of inspection.		

EMBANKMENT

Name of Dam: DRY RUN NO. 101

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
FOUNDATION	<p>According to the as-built drawings, the embankment is constructed on a river floodplain 10 to 28 ft. deep. The deposits consist of silty sand and gravel with boulders over hard cross-bedded argillaceous sandstone with traces of shale (based on the boring logs). A ridge of white very hard quartzite is partially exposed near the downstream toe where the dam curves. The cutoff trench to bedrock was grouted as indicated in the as-built drawings.</p>	

OUTLET WORKS

Name of Dam: DRY RUN NO. 101

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
CRACKING AND SPALLING OF CONCRETE SURFACES IN OUTLET CONDUIT	No cracking or spalling of the outlet conduit was observed.	
INTAKE STRUCTURE	Concrete riser with a 1 x 4 ft. orifice controlling normal pool. Also has a high stage orifice approxi- mately 3.7 ft. above orifice invert. Top of riser has one lift and pedestal for the 30 in. slide gate Also has 6 gates that can be operated from riser for future water supply.	Several of the stems to the water supply gates are disconnected or bent and would need repair before they could be used for water supply.
OUTLET STRUCTURE	Consists of a 30 inch C.M.P. encased in concrete extending through the embankment and exiting in the stilling pool. Flow was measured at 0.7 g.p.m.	
OUTLET CHANNEL	The outlet channel consists of a stilling pool approximately 25 ft. wide and 60 ft. long with limestone riprap bank protection.	
EMERGENCY GATE	Located on riser and can be used to drain reservoir (30 in. slide gate). Not operated periodically as far as can be determined.	Should be operated periodically to check for proper functioning.

UNGATED SPILLWAY

Name of Dam: DRY RUN NO. 101

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
CONCRETE WEIR	None	
APPROACH CHANNEL	Emergency spillway has an adverse slope to a control section approximately 460 ft. wide. There are 3 small watercourses in the channel which collect clear seepage from the cut and outlet into the reservoir. The channel is cut into silt, sand, gravel, cobbles and rock fragments. There is a good grass cover.	
DISCHARGE CHANNEL	Emergency spillway discharges downstream with a slope of approximately 3:1 across a stone road into a swale in the woods adjacent to a farm. The channel consists of silt, sand, gravel, cobbles and rock fragments.	
BRIDGE AND PIERS	None	
CONFINING SLOPES	The slope on the left is cut a maximum of 25 ft. deep at a ratio of 3:1 with grass cover in brown damp to moist silty sand, gravel, cobbles and rock fragments. There is some scattered clear seepage at the base which is collected in a ditch. The right slope consists of an extension of the dam (damp silty sand, gravel and rock fragments) and a small quartzite ridge with a cover of silty sand and rock fragments.	

INSTRUMENTATION

Name of Dam: DRY RUN NO. 101

<u>VISUAL EXAMINATION</u>	<u>OBSERVATIONS</u>	<u>REMARKS OR RECOMMENDATIONS</u>
MONUMENTATION/SURVEYS	None observed.	
OBSERVATION WELLS	None observed.	
WEIRS	None	
PIEZOMETERS	None	
OTHER		

RESERVOIR

Name of Dam: DRY RUN NO. 101

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
-----------------------	--------------	----------------------------

SLOPES

The forest covered slope is moderately steep.
The soil consists of damp, light-brown, silty
sand and rock fragments. There are very few
exposures of bedrock.

SEDIMENTATION

No unusual sedimentation was noted.

DOWNSTREAM CHANNEL

Name of Dam: DRY RUN NO. 101

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
CONDITION (OBSTRUCTIONS, DEBRIS, ETC.)	The stream channel is comprised of sand, gravel, cobbles and small boulders with some vegetation growing beside it. There are no significant obstructions or debris.	
SLOPES	The shallow slopes consist of silty sand, gravel, cobbles and boulders.	
APPROXIMATE NO. OF HOMES AND POPULATION	Approximately 500 ft. downstream of the dam is a farm house. Another 40 to 50 homes are located in the stream valley primarily along Rt. 667 for the next 3 mi. downstream. Population is estimated to be around 200 to 300.	

APPENDIX IV

CHECK LIST - ENGINEERING DATA

**CHECK LIST
ENGINEERING DATA
DESIGN, CONSTRUCTION, OPERATION**

Name of Dam: DRY RUN NO. 101

ITEM

REMARKS

PLAN OF DAM The Plan of Dam is shown on the as-built drawings.

REGIONAL VICINITY MAP The vicinity map is presented in this report as the Location Plan.

CONSTRUCTION HISTORY The contractor and completion date were obtained from the COE.
The dam was constructed by the English Construction Company in 1971.

IV-1

TYPICAL SECTIONS OF DAM Typical sections are included in the as-built drawings and are presented in this report as Plates 2 and 3.

HYDROLOGIC/HYDRAULIC DATA Hydrologic and hydraulic calculations were available.

OUTLETS - PLAN and DETAILS Shown on as-built drawings.

- CONSTRAINTS and DISCHARGE RATINGS Contained in the hydrologic/hydraulic calculations.

RAINFALL/RESERVOIR RECORDS No rainfall or reservoir records are available at the dam.

Name of Dam: DRY RUN NO. 101

ITEM	REMARKS
------	---------

DESIGN REPORTS	A Design Report was not available for this inspection report.
----------------	---

GEOLOGY REPORTS	A Geologic Report was not available for this inspection.
-----------------	--

DESIGN COMPUTATIONS	Hydrology and hydraulic calculations were available for this inspection report.
HYDROLOGY & HYDRAULICS	Stability analyses were not available.
DAM STABILITY	
SEEPAGE STUDIES	

IV-2

MATERIALS INVESTIGATIONS	Boring and test pit records and dry density compaction curves for soils in the 3
BORING RECORDS	Zones of the dam are presented in the as-built drawings.
LABORATORY	
FIELD	

POST-CONSTRUCTION SURVEYS OF DAM	No known post-construction surveys were found.
----------------------------------	--

BORROW SOURCES	Borrow areas in the reservoir area and south of the emergency spillway are shown in the as-builts.
----------------	--

Name of Dam: DRY RUN NO. 101

ITEM	REMARKS
------	---------

MONITORING SYSTEMS	No monitoring systems have been provided.
--------------------	---

MODIFICATIONS	Data obtained during inspection agrees very closely with as-built drawings indicating that no major modifications were made.
---------------	--

HIGH POOL RECORDS	None are available. Local citizens indicated that water has risen over the top of the riser in the past.
-------------------	--

3-1-21

POST-CONSTRUCTION ENGINEERING STUDIES AND REPORTS	None are available.
---	---------------------

PRIOR ACCIDENTS OR FAILURE OF DAM DESCRIPTION REPORTS	No prior accidents or failure of the dam have been noted.
---	---

MAINTENANCE OPERATION RECORDS	Annual inspections are conducted by the Town of Luray with the assistance of the Shenandoah Valley Soil and Water Conservation District and the SCS to make recommendations for maintenance and upgrading of the dam if needed.
-------------------------------	---

Name of Dam: DRY RUN NO. 101

<u>ITEM</u>	<u>REMARKS</u>
-------------	----------------

SPILLWAY PLAN

**SECTIONS
and
DETAILS**

Information contained in the as-built drawings.

**OPERATING EQUIPMENT
PLANS & DETAILS**

Information contained in the as-built drawings.

CHECK LIST
HYDROLOGIC AND HYDRAULIC DATA
ENGINEERING DATA

DRAINAGE AREA CHARACTERISTICS: 2.62 sq.mi.

ELEVATION TOP NORMAL POOL (STORAGE CAPACITY): 1142.6 ft. M.S.L.
(552 ac.-ft.)

ELEVATION TOP EMERGENCY SPILLWAY CREST
(STORAGE CAPACITY): 1151.6 ft. M.S.L. (922 ac.-ft.)

ELEVATION MAXIMUM DESIGN POOL: 1154.0 ft. M.S.L.

ELEVATION TOP DAM: 1157.6 ft. M.S.L. (settled)

CREST: Emergency Spillway

- a. Elevation 1151.6 ft. M.S.L.
- b. Type Earth side-channel with vegetative cover
- c. Width 375 ft.
- d. Length 830 ft. total length (500 ft. approach, 30 ft. level section, 300 ft. exit)
- e. Location Outside left abutment
- f. Number and Type of Gates None

OUTLET WORKS: _____

- a. Type Drop-inlet concrete riser
- b. Location Riser in reservoir with 30 in. diameter reinforced concrete outlet pipe
- c. Entrance inverts 1142.6 ft. M.S.L. (normal pool)
- d. Exit inverts 1093.0 ft. M.S.L. (30 in. diameter outlet pipe)
- e. Emergency draindown facilities reservoir drain (30 in. diameter slide gate)

HYDROMETEOROLOGICAL GAGES: None available

- a. Type _____
- b. Location _____
- c. Records _____

MAXIMUM NON-DAMAGING DISCHARGE Unknown

Name of Dam: DRY RUN NO. 101

APPENDIX V

ANNUAL MAINTENANCE INSPECTION REPORTS

**REPORT ON ANNUAL MAINTENANCE INSPECTION OF
DRY RUN WATERSHED FLOOD CONTROL STRUCTURES**

The Shenandoah Valley Soil and Water Conservation District performed their annual inspection of the completed flood water retarding structures on the Dry Run Watershed, in Page County, Virginia May 23, 1978. The structures inspected were dams 101 and 102. District Director Arlis Frymyer performed the inspection with the assistance of Don Smith, Luray Town Manager and Sam Linkenhoker, District Conservationist.

The results of the inspections are listed below as agreed to maintenance items.

Dam 101 (Lake Arrowhead)

1. Remove small cedars on downslope of dam.
2. Remove brush from knoll on eastern end of dam and spray undergrowth with brush killer and oil.
3. Spray and remove small trees on drainage ditches in the spillway.
4. Replace bolts on walkway structure.

Dam 102 (Lake Morning Star)

1. Paint a large warning (DANGER KEEP OUT) on the exit end of the principal spillway pipe.

Damaged areas on Dam 102, as a result of motor vehicles, had been repaired and appeared in excellent condition with seeding and planking.

Arlis Frymyer
Arlis Frymyer, Director

Donald Smith
Donald Smith, Town Manager

Samuel W. Linkenhoker
Samuel W. Linkenhoker,
District Conservationist

REPORT OF ANNUAL MAINTENANCE INSPECTION OF
DRY RUN WATERSHED FLOOD CONTROL STRUCTURES

The Shenandoah Valley Soil and Water Conservation District performed their annual inspection of the completed flood water retarding structures on the Dry Run Watershed, in Page County Virginia April 21, 1977. The structures inspected were dams 101 and 102. District directors Joyce Burner and Clark Spitler performed the inspection with the assistance of Don Smith, Luray Town Manager and Sam Linkenhoker, District Conservationist.

The results of the inspections are listed below as agreed to maintenance items.

Dam 101 (Lake Arrowhead)

1. Remove brush and trees from the upstream slope of dam and paint the stumps with Brush Killer and oil.
2. Remove the common ruellein (*Verbascum thapsus*) from the slopes of the dam and spillway by either grubbing or spraying with 2,4-D mixed in common detergent.
3. Spray the brush on the slopes of the spillway with Brush-Killer.
4. Install a post in the existing lane which is located around the southern edge of the lake to prohibit vehicular travel on said lane.
5. Remove all downed trees that are located within the flood pool area.

Dam 102 (Lake Loring Star)

1. Paint a large warning (DANGER KEEP OUT) on the exit end of the principal spillway pipe.
2. Remove all locust sprouts from the back side of the dam and paint the stumps with Brush-Killer and fuel oil.

Clark W. Spitler
Clark Spitler, Director

Joyce P. Burner
Joyce Burner, Director

Donald A. Smith
Donald Smith, Town Manager

Samuel W. Linkenhoker
Samuel W. Linkenhoker, District Conservationist

SHENANDOAH VALLEY SOIL AND WATER CONSERVATION DISTRICT
REPORT OF ANNUAL MAINTENANCE INSPECTION OF WATERSHED DAMS IN

DRY RUN WATERSHED PROGRAM

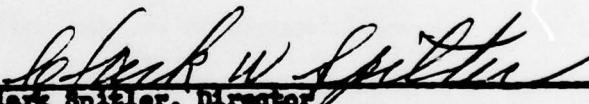
May 27, 1976

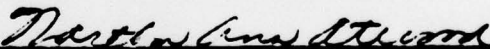
Inspection of the dams on Dry Run, Page County, Virginia was made May 27, 1976 by Clark Spittler, Martha Ann Atwood, and Joyce Burner, Shenandoah Valley Soil and Water Conservation District Directors. They were assisted by Donald Smith, Town Manager, and Jimmie Griffith, Water Superintendent, Town of Luray. The group was accompanied by John D. Crist, Soil Conservation Service.

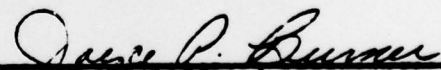
The following corrections were agreed upon by all parties involved:

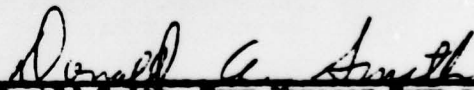
- Dam No. 102 - 1. Remove locust and willow trees from dam and spillway and paint stumps with 245T and fuel oil.
2. Remove log from floodpool area.
3. Paint sign on drain pipe at stilling pool to keep people out of pipe. Danger, Keep Out, Town of Luray.
- Dam No. 101 - 1. Remove locust and willow trees from dam and spillway and paint stumps with 245T and fuel oil.
2. Remove large logs from dam and waterline.

This report is concurred by:

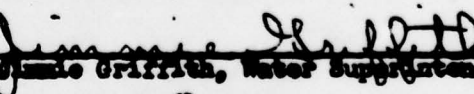

Clark Spittler, Director
Shenandoah Valley Soil and Water Conservation District

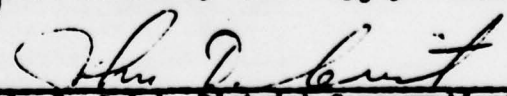

Martha Ann Atwood, Director
Shenandoah Valley Soil and Water Conservation District


Joyce Burner, Director
Shenandoah Valley Soil and Water Conservation District


Donald Smith, Town Manager Town of Luray

DISTR: State Office 2 cys.
Area Office
Luray Field Office
Town of Luray
Shenandoah Valley SAWCD


Jimmie Griffith, Water Superintendent Town of Luray


John D. Crist, District Conservationist
Soil Conservation Service

SHENANDOAH VALLEY SOIL AND WATER CONSERVATION DISTRICT
REPORT OF ANNUAL MAINTENANCE INSPECTION OF WATERSHED DAMS IN

DRY RUN WATERSHED PROGRAM

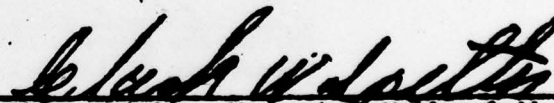
May 8, 1975

Inspection of the dams on Dry Run, Page County, Virginia was made May 8, 1975 by Clark Spitler, Shenandoah Valley Soil and Water Conservation District Director. He was assisted by Dempsey Stokes and Haywood Nichols from the Town of Luray and John D. Crist, Soil Conservation Service.

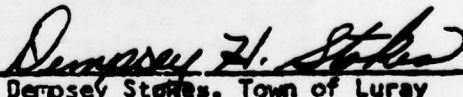
The following corrections were agreed upon by all parties involved:

- Dam No. 101 - 1. All small locust on dam and spillway will be cut and sprayed with 245T and fuel oil.
2. The larger logs will be removed from the dam to prevent clogging of spillway.
3. Two small galled areas will be seeded to Ky. 31 fescue and mulched with straw.
- Dam No. 102 - 1. All small locusts on the dam and spillway will be cut and sprayed with 245T and fuel oil.
2. Two small galled areas will be seeded to Ky. 31 fescue and mulched with straw. The overhang on the top galled area will be knocked off and the area planted to locust to hold the sod in place.

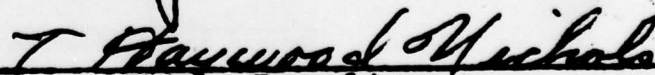
This report is concurred by:



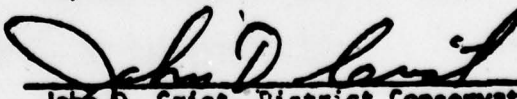
Clark Spitler, Shenandoah Valley Soil and Water
Conservation District Director



Dempsey Stokes, Town of Luray



Haywood Nichols, Town of Luray



John D. Crist, District Conservationist, Soil
Conservation Service

DISTR: State Office 2 cys.
Area Office
Luray Field Office
Town of Luray
Shenandoah Valley S&WCO

SHENANDOAH VALLEY SOIL AND WATER CONSERVATION DISTRICT
REPORT OF ANNUAL MAINTENANCE INSPECTION OF WATERSHED DAMS IN

DRY RUN WATERSHED PROGRAM

April 4, 1974

Inspection of the dams was made April 4, 1974 by James Aleshire and Clark Spitler along with other members of the Shenandoah Valley Soil and Water Conservation District Board. In addition, Dempsey Stokes and Tom Speake from the Town of Luray and William L. Blair, Jr. and John Crist from SCS accompanied the inspection team.

The following conditions were reported:

Dam No. 101 - Good condition, no extra maintenance is needed.

Dam No. 102 - Good condition, no maintenance needed.

The report is concurred by:

James E. Aleshire
James Aleshire, Director, Shenandoah Valley Soil
and Water Conservation District

Clark W. Spitler
Clark Spitler, Director, Shenandoah Valley Soil
and Water Conservation District

Dempsey H. Stokes
Dempsey Stokes, Town Manager of Luray, Virginia

Thomas E. Speake
Tom Speake, Town Mayor of Luray, Virginia

William L. Blair, Jr.
William L. Blair, Jr., Area Conservationist, SCS

John D. Crist
John D. Crist, District Conservationist, SCS

DEST: State Office 2 cys.
Area Office
Luray Field Office
Town of Luray
Shenandoah Valley SWCD

APPENDIX VI

REFERENCES

REFERENCES

1. Bureau of Reclamation, U.S. Department of the Interior, Design of Small Dams, A Water Resources Technical Publication, Revised Reprint, 1977.
2. Chow, Ven Te, Handbook of Applied Hydrology, McGraw - Hill Book Company, New York, 1964.
3. Chow, Ven Te, Open Channel Hydraulics, McGraw - Hill Book Company, New York, First Edition, 1959.
4. Commonwealth of Virginia, "Geologic Map of Virginia," Department of Construction and Economic Development, and Division of Mineral Resources, 1963.
5. Soil Conservation Service, "National Engineering Handbook - Section 5, Hydraulics," U.S. Department of Agriculture.
6. U.S. Army, "Inventory of United States Dams," Corps of Engineers, 9 September 1978.
7. U.S. Army, Office of the Chief of Engineers, "Appendix D, Recommended Guidelines for Safety Inspection of Dams," National Program of Inspection of Dams, Volume 1, Corps of Engineers, Washington, D.C., May 1975.
8. U.S. Army, Office of the Chief of Engineers, Engineering Circular EC-1110-2-163 (Draft Engineering Manual), "Spillway and Freeboard Requirements for Dams, Appendix C, Hydrometeorological Criteria and Hyetograph Estimates," (August 1975).
9. U.S. Army, Office of the Chief of Engineers, Engineering Circular EC-1110-2-188, "Engineering and Design, National Program of Inspection of Non-Federal Dams," Corps of Engineers, Washington, D.C., 30 December 1977.
10. U.S. Army, Office of the Chief of Engineers, Engineer Technical Letter No. ETL 1110-2-234, "Engineering and Design, National Program of Inspection of Non-Federal Dams, Review of Spillway Adequacy," Corps of Engineers, Washington, D.C., 10 May 1978.
11. U.S. Department of Commerce, "Technical Paper No. 40, Rainfall Frequency Atlas of the United States for Durations from 30 Minutes to 24 Hours and Return Periods from 1 to 100 Years," Weather Bureau, Washington, D.C., May 1961.

NAME OF DAM: DRY RUN NO. 101

VI-1