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SHOALING PROBLEMS ON MISSISSIPPI RIVER-GULF OUTLET LOUISIANA

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SHOALING PROBLEMS ON MISSISSIPPI RIVER-GULF OUTLET LOUISIANA

1. <u>Purpose.</u> The purpose of this report is to update written report "Construction Procedures and Shoaling Problems on the Mississippi River-Gulf Outlet," which was presented by Messrs. B. M. Johnson and G. A. Price, U. S. Army Engineer District, New Orleans, to the Committee on Tidal Hydraulics, Corps of Engineers, U. S. Army, at their 51st meeting in New Orleans, La., on 20-22 October 1964.

2. <u>Project Authorization</u>. The Mississippi River-Gulf Outlet, La., a modification of the existing project, "Mississippi River, Baton Rouge to the Gulf of Mexico," was authorized by the River and Harbor Act of 29 March 1956 (Public Law 455, 84th Congress, 2d Session), substantially in accordance with the report of the Chief of Engineers dated 5 May 1948 printed in House Document No. 245, 82d Congress, 1st Session.

3. Project Status.

a. Dredging of the project channel (-36 feet by 500 feet and -38 feet by 600 feet), as shown on plate 1 and table 4, was initiated 17 March 1958 and, except for the narrow restriction at Paris Road, was completed 22 July 1965, including a turning basin at the Inner Harbor Navigation Canal. An "interim channel" (-36 feet by 250 feet and -38 feet by 300 feet) usable for shipping was completed 5 July 1963 and dedicated 25 July 1963, at which time the first ship traveled from New Orleans to the Gulf of Mexico via the Mississippi River-Gulf Outlet. The narrow restriction at Paris Road will be removed upon completion of the high-level bridge which is a feature of the project scheduled for completion in December 1966.

b. Maintenance dredging during construction was initiated 4 September 1963 and completed 23 January 1966 (see table 5). Regular maintenance dredging, using operation and maintenance funds, was initiated 1 July 1965 and is currently in progress (see table 6). All spc.3 from construction and maintenance dredging, beyond the outer end of the existing dikes in Breton Sound, has been discharged not closer than 2,000 and 1,500 feet, respectively, to the project channel centerline. The greatest requirement for maintenance dredging has occurred in the Breton Sound area (see plates 2 and 5).

4. Shoaling History.

a. Predicted shoaling rates, as shown in the general design memorandum, were based on current measurements, suspended

TABLE 1

DERIVATION OF SHOALING RATE FROM NATURAL SOURCES - MILE 20.2 TO MILE 14.9(a)

Project channel mileage	20.2 14.9
Equivalent route "B" mileage	25.2 19.9
BEP predicted minimum shoaling rate, Cu. Yds/Yr/Mi	300,000 220,000
Avg. minimum shoaling rate, Cu.Yds/Yr/Mi	260,000
Avg. maximum shoaling rate, Cu.Yds/Yr/Mi	520,000
Shoaling for the 5.3-mile reach, CuYds/Yr	2,800,000

(a)
 From Mississippi River-Gulf Outlet, Louisiana, Design Memorandum No. 2, General Design, Appendix I, Plate 13, Route "B" and Par. 14, p. 81. Beach Erosion Board (BEB) is now known as Coastal Engineering Research Center.

TABLE 2

TABULATION OF CURRENT DIRECTION AT TOWER # 2(a)

Record from 17 Jan 64 to 15 Jun 65

Direction	Percent of Time Prevailing
in Degrees	in Given Direction
18-62	9.48
63-107	11.29
108-152	10.58
153-197	11.37
198-241	5.42
242-287	12.03
288-332	15.09
333-17	12.18
Intermediate or no velocity	12.56

(a) Tower No. 2 located at Mile 16.4 on the natural sound bottom 700 feet southwest of the channel centerline.

sediment samples, and bottom samples collected in Breton Sound and the Gulf of Mexico, together with shoaling data obtained from a series of five test pits dredged to approximate depths of -30feet. The test pit located in an original depth of less than -6feet indicated the maximum shoaling rate and dictated the decision to construct dikes from the shoreline at mile 23.0 to the -6-footcontour at mile 20.2. However, surveys have indicated that the shallow area extends beyond mile 20.2 to mile 14.9. A profile run in December 1965 along the projection of the southwest dike indicated ground elevation of -6.5 at mile 15.3, a maximum elevation of -5.7 at mile 19.6, and a minimum elevation of -6.8 at mile 18.0.

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b. Based on maintenance dredging performed to date, the highest shoaling rate has occurred in Breton Sound, extending several miles beyond the outer end of the existing dikes at mile 20.2 (see plate 2). The shoaling rates for the first and second maintenance dredging averaged 755,000 and 982,000 cubic yards per year per mile, respectively, for the reach from mile 20.2 to mile 14.9. The latter rate is 3.8 times higher than the average minimum rate of 260,000 cubic yards per year per mile estimated in the General Design Memorandum.

The shoaling rates are shown on plate 2, in three curves, representing the rates between completion of:

- (1) construction and first maintenance dredging;
- (2) first and second maintenance dredging; and
- (3) second and third maintenance dredging.

These computed rates are based on actual end areas used for payments to the contractors tabulated at 2,000-foot intervals.

The shoaling rates stated above for the first and с. second maintenance dredging, converted to total cubic yards per year for the reach from mile 20.2 to mile 14.9, are 4.0 and 5.2 million cubic yards per year, respectively. Based on shoaling estimates made by the Beach Erosion Board in connection with the preparation of "Design Memorandum No. 2, General Design," the average maximum shoaling rate from natural sources; i.e., sources other than recirculation of spoil, amounts to 520,000 cubic yards per year per mile for the reach from mile 20.2 to mile 14.9. Determination of this value is shown on table 1. This rate, converted to total shoaling per year in the reach, is 2,800,000 :ubic yards. Based on this premise, recirculation accounted for at least 1,200,000 cubic yards of shoaling in the reach between project construction and first maintenance dredging, and 2,400,000 cubic yards between the first and second maintenance dredging.

TABLE 3

وتراهده كمعدها ومعادمات فالمرجع ومتابعتها فالمحمط كمعارضاتهم المكملية للمراجع ومعادلات مامير والانتخاصة

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MISSISSIPPI RIVER-GULF OUTLET - BED MATERIAL Fercent of material retained on U.S. 230 sieve^(a)

			19	64 Observa	tions				
Channel									
mileage	4/13	4/27	5/11	5/26	6/8	6/22	7/21	8/18	Average
		1,00	0 feet nor	theast of	channe]. c	enterline			
19.1	7.2	9.4	18.4	16.4	9.2	4.2	0°†	3.4	0.0
16.8	29.7	34.7	36.0	26.5	30.2	2.0	30.0	24.1	26.7
14.2	35.0	I	1	30.7	38.6	36.4	42.4	11.8	32.5
11.5	41.1	46.3	40.8	43.3	49.1	53.6	35.6	32.4	42.8
7.0	22.0	26.6	13.6	46.1	26.4	28.0	23.5	53.1	29.9
Average							1		28.2
	I		125 feet	northeast	of center	rline			
19.1	2.8	3°0	1.3	3.3	1.0	2.7	2.2	3.4	2.5
16.8	4.6	2.3	1.7	18.6	2.6	32.0	1.7	5.0	8.6
14.2	6.7	1	I	4.7	н. С.	3.0	1.7	1.7	3.2
11.5	6.0	3.7	4.4	18.0	1.7	0.7	0.7	29.3	8.1
7.0	6.7	3.0	0.7	3.0	26.9	1.7	1.3	4.6	6.0
Average									5.7
			125 feet	southwest	of cente	rline			
19.1	2.0	1.0	0.7	1.7	1.7	1.7	3.0	3.2	1.9
16. 8	4.8	2.0	2.7	19.3	2.0	2.0	1.3	6.6	5.1
14.2	7.6	ł	ı	3.7	1.0	2.0	4.0	1.3	3 . 3
11.5	1.4	4.7	4.4	20.8	1.0	2.0	1.0	30.1	
7.0	5.3	14.6	1.7	4.0	27.7	2.3	1.3	4.8	7.7
Average			,						
		1,000) feet sou	thwest of (channel ce	enterline			
19.1	2.8	2.0	2.0	4.2	3.4	2.0	6.6	7.8	3.9
16.8	2.7	13.4	9.2	9.2	9.6	8.4	4.4	8.3	8.1
14.2	6.0	1	I	2.0	1.8	2.0	2.0	1.3	2.5
11.5	18.4	33.3	4.6	25.3	1.0	2.1	0.7	1. 3	10.9
0-1	1.6	4.0	3.2	2.7	1.0	2.8	1.3	3.4	2.5
Average)	5.6
(a) _{The} sand	fraction is	the mater	rial retai	aed on the	No. 230 s	sieve (0.06	2 mm). T	he silt fro	action
include	s all of th	te fine mat	cerial pas	sing the Nc	o. 230 si€	eve.			

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5. Significant Observation Data.

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a. Current observations made adjacent to the channel for the past 1-1/2 years indicate that there is no predominant direction of flow across the channel, but that they are about equally divided in time from the northeast and southwest side as shown in table 2.

b. Supply for shoaling from natural sources comes partly from in situ material in the shallow bed and bays of the sound, and partly from material contributed by the Mississippi River. This material is stirred from the bottom by wave action and is transported to the channel by tidal and wind-generated currents. This supply may be considered inexhaustible and if the dikes were not extended, shoaling rates for this material would remain constant within the reach in question for the life of the project.

c. Analyses of sediment samples in the spoil runoff area 1000 feet to the southwest of the channel centerline, and the natural sound bottom 1000 feet to the northeast indicate that the shoal material is being generated predominantly in the former area (see table 3). This is demonstrated by the fact that shoal material samples show a marked dissimilarity to samples from the northeast, insofar as concentration of sand is concerned. It will be noted that the average percent of material retained on U. S. 230 sieve for bed material samples taken 1000 feet southwest of channel centerline, in the channel (shoaled material), and 1000 feet northeast of channel centerline was 5.6, 5.5, and 28.2, respectively. Accordingly, it may be concluded that shoaled material in the channel is largely composed of recirculated spoil material.

6. Plan of Improvement for Reduction of Excessive Shoaling.

a. The plan for the reduction of excessive shoaling presented herein and shown on plate 3 consists of the extension of the southwest dike from mile 20.2 to mile 14.9, including a 1,000-fcot flanking dike at the outer end, behind which all future adjacent dredged spoil from maintenance dredging would be deposited. This plan will largely overcome the problem of recirculation in this reach of the channel.

b. <u>Settlement.</u> It is estimated that settlement due to displacement and consolidation of the foundation will be approximately 1 foot during construction of the first stage, 1/2 foot between the first stage and the beginning of the second-stage construction, 1/2 foot during construction of the second stage, and 1 foot after the second stage is completed. c. <u>Method of Construction</u>. The dike will be constructed, as shown on plate 4, of clamshells, riprap, and derrick stone. Because of the low shear strength of the foundation, the dike will be constructed in two stages. In the initial construction stage, the shell will be placed to elevation -1.0 foot m.l.g. and riprap will be placed to elevation 1.0 foot m.l.g. To prevent loss of shell during construction, the placement of the riprap will follow closely behind placement of the shell. To add to the stability of the dike and help retain the shell during construction, excavated material will be placed in the form of berms on both sides of the dike. In the second construction stage, derrick stone will be placed to elevation +3.0 feet m.l.g. with a crown 12 feet wide. The existing dikes were constructed to elevation +5.0 feet m.l.g.

d. <u>Cost Estimate</u>. The estimated cost for extending the southwest dike is \$5,010,000, consisting of \$4,674,000 for construction, \$56,000 for engineering and design, and \$280,000 for supervision and administration.

e. <u>Schedule</u>. The first stage work will be completed under two contracts over a period of 18 months with the first contract to be awarded in March 1966. The second stage work will be completed under two contracts 18 months after completion of the first stage. This schedule is contingent upon the availability of funds.

f. <u>Reduction in Maintenance Dredging.</u> With the proposed extension of the southwest dike and flanking dike in place, forming an effective spoil retention dike, it is anticipated that the annual shoaling rate will be reduced by at least 2,400,000 cubic yards. Other benefits would be the reduction in length of the floating discharge lines of dredges by at least 1,300 feet in the reach between mile 20.2 and mile 14.9, plus a semisheltered area for the dredges to operate.

g. <u>Other Plans Considered.</u> Consideration was given to reducing recirculation by increasing the distance between the channel centerline and the point of deposit of dredged spoil; by the use of hopper barges and rehandling the spoil therefrom to spoil areas on the shore; and the use of a self-unloading hopper dredge with discharge line behind existing dikes. However, because of the excessive costs involved, these plans are considered impractical. The selfunloading hopper dredge with discharge lines behind the extended rock retention dike, inshore from mile 14.9, will be further evaluated in determining the feasibility of further dike extension.

7. Current and Future Observation Program.

a. The analyses of the shoaling problems and its sources in the Breton Sound reach of the Mississippi River-Gulf Outlet project between mile 20.2 and mile 6.2 are being continued for the purpose of providing additional information required for an economic study to determine the feasibility of further dike extension. A dike across Breton Sound is authorized as a feature of the project if experience indicates that maintenance by dredging alone is more costly, impracticable or unduly obstructive to navigation. Therefore, investigations are warranted to determine whether further dike extension will reduce shoaling and save maintenance dredging cost in excess of the annual cost of the dike extension.

b. Future investigations are warranted to further define potential sources of shoal material and provide information as to the rate of shoaling, quantities of sediment in transport and probable sources of sediment. Investigations are to be conducted in the following three areas: the channel, the spoil deposit area southwest of the navigation channel, and the area adjacent to the northeast of the navigation channel.

(1) <u>Channel Area.</u> The reach where high shoaling occurs will be investigated to determine characteristics of the materials deposited, rate of deposit, characteristics of the currents transporting the material, salinity of the water and effects of salinity on the distribution of the currents and shoaling materials.

Immediately after dredging, periodic surveys will be taken to determine rate of shoaling deposits as influenced by maintenance dredging, and these surveys will be continued to determine seasonal changes, storm surges and other local factors on rate of shoaling.

Shoaling rates shown on plate 2 and rates to be computed after subsequent maintenance dredging are being standardized on the basis of computing end areas for a section having a bottom width of 900 feet at elevation -50.0 m.l.g. and side slopes of 50 feet on the vertical to 0.1 foot on the horizontal.

(2) <u>Southwest Area Adjacent to Channel (Spoil Area)</u>. Past surveys have indicated erosion of the spoil deposit area southwest of the navigation channel. An investigation of the spoil deposit area to determine the density of the in situ material by the radioactive density probe accompanied by undisturbed borings is now in progress. Transport of the eroded material by currents and the velocity of the currents doing the transporting will be studied to determine its contribution to channel shoaling.

(3) Northeast Area Adjacent to Channel. The northeast area adjacent to the channel will be investigated to determine its potential contribution of channel shoaling. Investigations of the erosion of sediments from the bed and banks of the area and transportation of the material to the channel by currents will be conducted.

8. <u>Discussion</u>. Maintenance of the Breton Sound reach of the Mississippi River-Gulf Outlet has proven to be a most difficult task to meet the demands of shipping. The second maintenance dredging contract, extending from mile 23.3 to mile 2.0, was initiated and completed 9 December 1964 and 23 January 1966, respectively. The quantity of material to be dredged was revised by addendum during advertisement from 11,450,000 to 14,124,000 cubic yards, to reflect approximately 2 months of shoaling. The quantity was further revised to 23,338,000 cubic yards by modification of the contract to reflect additional shoaling. The extension of the southwest dike from mile 20.2 to mile 14.9 and flanking dike at mile 14.9 will alleviate this maintenance dredging problem.

9. <u>Concluding Remark.</u> In addition to the estimated reduction of shoaling, the extension of the southwest rock dike from mile 20.2 to mile 14.9 will serve as a prototype in the collection of data from future maintenance dredging experience to determine the justification for additional dike extension or alternates to reduce shoaling in the waterway.

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TABLE 4

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MISSISSIPPI RIVER-GULF OUTLET Tabulation of Dredging Contracts

Contract		LOCATI	N O N				Quantity of	
Sequence No.	C/L Sti From	ationing To	A EIOT	lle To	Date Initiated	Date Completed(f)	Material Dredged Cu. Yds.	Contract Cost (e)
				Phase No. 1 - Dredging 5.1 36 by 500 fee	Miles of Project t (including 1/2	Channel - mile transition	a to 200 feet at	
5	0+00 130+00	130+00 268+00	66.0 63.5	63.5 60.9 (Paris Rd.)	17 Mar 58 6 Jun 58	29 Dec 58 7 May 59	9,518,999 10,182,600	\$ 1,627,800 1,619,000
Tot	al Phase No.	l - Project	Channel :	Dredging			19,701,599	\$ 3,246,800
				Phase No. 2 - Dredging 37.2 Gulf Intracos 18 by 140 fee	Miles of Access stal Waterway to	Channel - Breton Sound -		
3 (a) 4	301+20 683+78	683+78 1267+00	60.3 53.1	53.1 (Bayou Dupre) 41.6 (Bayou Yscloskey)	8 May 59 5 Aug 59	4 F eb 60 28 Sep 60	4,322,000 9,241,000	\$ 1,041,200 1,601,000
5 0 7 (b)	1293+00 1561+70 1950+00	1561+70 1950+00 2270+00	41.5 36.4 29.1	36.4 (Beyou LaLoutre) 29.1 (Bayou Pointe en Pointe) 23.0 (Breton Sound)	8 Feb 60 18 Apr 60 24 May 60	22 Sep 60 6 Feb 61 27 Feb 61	3,945,000 5,208,000 4,646,000	598,900 7 89,000 702,600
Tot	al Phase No.	2 - Access C	bannel D	redging			27,362,000	\$ 4,732,700
				Phase No. 3 - Dredging (9.9 Paris Road to 35 by 250 fee	Miles of Interi 38-foot Contour t and 38 by 300	<pre>Channel - in Gulf of Mexi feet</pre>	-	
8 (c)	50+622	683+78	60.5	53.1 (Bayou Dupre)	28 Feb 50	27 Mar 61	14,312,900	\$ 1,723,100
٥	c83+78	918+00	53.1	48.6	10 Aug 60	15 Jun 61	7,878,436	1,062,800
11 (d)	910400	1561+70	40.0 10.0	42.0 (Pipeline Canal) 36.4 (Bavou Ialoutre)	17 Oct 80 % Ther 50	28 Sep 61	10,197,562	1,341,000
13	1561+70	1809-80	36.4	31.7	7 Feb 61	3 Jul 61	8.375.024	779.100
77	1809+80	504-6402	31.7	21.2	li Feb 61	5 Feb 62	1,699,664	639,300
5 5	0446402	2210+08	21.2	23.0 (Breton Sound)	20 Mar 61	29 Mar 62	6, 383, 352 5 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	452,900
16	2420400	2800+00	2.2	20.2 (ULLIVEIN CIA OI FOCK 1".	19 TIN 19 19	To Sink Co	5,902,055	001,100 715 200
17	2800+00	3160+00	13.0	6.1	22 Jun 61	22 Apr 62	9.613.928	793.400
18	3160+00	3380+00	6.1	2.0	15 Jul 61	21 Sep 62	5,697,826	723,600
19	3380+00	3830+00	2.0	-6.5	Il Nov 61	5 Jul 63	10,547,766	2,079,100
Hired Labor &	3830+00		-6-5	-9.4	(24 Nov 62	25 Dec 62)	1,331,000	199,700
U.S.Hopper Dre	1ge				(6 Jun 63	20 Jun 63)		
Toti	al Phase No.	- Interim	Channel]	Dredging			111,204,529	\$12.257,600

(Continued on Table 5) Prepared: Oct. 64

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TABLE 5

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	Contract	Cost (e)				* 25,000	76, 300	1 218 500	1.230.100	1,496,500	1. 393, 500	1,027,100	404,300	780, 100	332, 100 671 200		1.407.800	1,192,200	817, 500	1, 500, 500	165,700		14,705,300		<u>291,000</u> 35,233,400			101,200	23,400	64,000	77,000	36,100	1,600	232,400	1, 044, 500	96,000	2, 203, 300	4,104,100	39, 337, 500			0 10 1
	Quantity of Material Dredged	Cu. Yds.				- 120,000	341,034	11, 241, 80B	8, 862, 760	12,676,000	12,017,075	9,459,194	4, 347, 713	4, 421, 169	3, 2(1, 5/5 3, 803 002	4.651.574	10, 268, 711	8, 303, 700	4°, 946, 705	8,988,420	1, 346, 000	0-0-010	050,166,211	:	1, 768, 201 272, 594, 167		1 187 610	520, 571	229,088	585, 456	74,085		74, 190	2,111,251	13,006,958	200,000	23, 330, 241	<u>+3,850,655</u>	316, 444, 822			100 COSUS.
cont'd)	Date	Completed(f)	ct Channel -	ot contour -		21 Mar 61	10 TON 17	14 Mar 65	15 Dec 63	28 May 64	28 Dec 63	22 Oct 64	2 Mar 05	- 23 UCT 03	23 Oct 63	1 Apr 62	18 Apr 63	20 Dec 62	10 Mov 62	23 Oct 63	1.5 AUG OF		fon Canal		20 mm 12		14 Mar 65	2 Mar 65	2 Mar 65		20 204 65	2 Mar 65	2 Mar 65	27 Dec 63		22 Tar 27	00 1120 (3				direct construct	
VER-CULF OUTLET (o	Date	Initiated	0.3 Miles of Proje	feet and 38 by 600	Dec 66 (Rat	2 Mar 61	3 Aug 61	22 Nov 63	13 Nov 62				12 Teb 62	4 Apr 64	12 Feb 63	likes) 19 Dec 61	3 1 2 2 2 2 3 2 3 3 3 3 3 3 3 3 3 3 3 3						ner Rarbor Mavigat	20 11- 6	5	ng Construction	21 Peb 64	17 Apr 64			5 10 6 10 7 10	6 Jun 64				9 Dec 61				Station 591+24 2238+00 2231+00	" 12084-91 ction and other in	
DA INTERINSING			use No. 4 - Dredging 7 Vicinity P	<u>36 by 500</u>			(Beyou Bienvenue)	(Bayou Dupre)	(Pite)ine (and)	(Barou Ialoutre)					(Breton Sound)	Intrinsition of the state of							ming Basin Vicinity In			intenance Dredging Duri	Bayou Dupre)			Beyou LaLoutre)	•						(e)	tion Dradaine (z)		rom Station 588+24 to 2233+50 to 279+05 to	<pre>" 1207+31 to llization, dike constru</pre>	
	Mi le To		£.		60.0	6.7 8 9	0.95	1.55.1	42.6	36.4	31.7	30.4	21.2	3.5			6.1	2.0	-6.5	4.6-		Dredging		65.4) 7.62 23.1 (2-22 2-2	1.04	36.4 (4.46		20.5	6.1	2.0	2.0	a Dredging	Construct		Channel : : :	ion, Demobi	
					9 9 9	8.4	33	53.1	48.6	42.6	36-4	31.7	†.62	21.2	08	2.02	13.0	6.1	2.0	-6.5	I	(Thanne)		66.0			0.65	15.6	41.6	1-04	4. 8. 8.	23.6	 ເ	20.2	6.1 22 2	53.3	struction	ice Durine		Project	bbilizati	
T 0 C V	ationing			`	516+912		683475	918+00	1235+50	1561+70	1809+80	1930+00			5420400	2800+00	3160+00	3380+00	3630+00			- Project			bredging		20+502	1287+00	1365+00	1561+70	00+0/01	2255400	2420400	3160+00	on the contract		During Con	Maint ener	500 S	mot-mc	includes)	ble ú.
	C/L St				270475	201+00	370+60	683+78	918+00	1235+50	1561+70	1809+80	00100106	2160+00	2270+00	2420400	2800+00	3160+00	5300+00	redare		ALL L'INNE NO. 4			al New Work - I		504-00 683478	1235+50	1287+00	1561+70	201-01-0102	2238+00	2255400	2420+00			il Maintenance]	il Nev Vork and	800 feet - 36-h	50 feet - 195 feet - 195 feet - 195 feet - 195 feet - 196 feet - 1	trest \$100 and 1 date of contwo	(c) and (d) Tai
Contract	Sequence No.			Ч.	ନକ୍ଷ	5	53	ос С	21	ର ନ	58	*≈	5	۲¥	8	đ 5	88	8 =	Hired Jahor a	U.S.Hopper D			33		Tot	50	J₩	R 8	£7 8	48	۲ ۳	25	ត	Ey Tessed Durden	34		. Tota	Tota	(a) Includes 3	(b) Includes 4 (c) Includes 1 (d) Includes 1	(e) To the new (f) Completion	(g) See notes

Prepared: Oct 64 Revised: Feb 66

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TABLE 6

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3		
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0+25 70+70 441+50 194+50 271+20 679+50 299+80 376+22 689+00 299+80 376+22 689+00 271+20 679+50
679-60 697-60 ^(a) 1327-55 697-60 710-50 1327-55 710-50 715-70(1) 1487-50 710-55 721-70(1) 1487-50 721-70 748-25(11) 1527-45 781-60 818-25 1671-75 781-60 912-40(a) 1741-70 1191-25 1122-45(a) 1924-30 930-40 938-90(a) 1777-70 1191-25 1122-45(a) 1924-30 2241-35 1222-45(a) 1924-30 204-50 1302-50(b) 1234-80 204-50 1302-50(b) 1242-50 204-50 1242-50 2
3385+00 3840+00
523+50 525+15 ^(b) 1059-15 525+15 680-40 ^(b) 1125-80 680-40 682+90 ^(b) 1125-80 709+90 711-445 (b) 12284-90 711-445 747+50 ^(b) 1322-80 7477-50 767+50 ^(b) 1313-50 817+65 825+60 ^(b) 1313-50 817-45 825-60 ^(b) 1411-47 ¹

(a) NE¹₂ of channel
(b) SW¹₂ of channel
(c) Construction general funds
(d) Construction general funds
(d) Construction general funds
(e) To the nearest \$100 and includes mobilization, demobilization, dike construction, and other indirect construction costs
(f) Hauled yardage
(g) OaM funds
(h) 340 ft. cut on south side of channel
(h) 1/0 ft. cut on south side of channel
(i) 1/0 ft. cut on south side of channel
(c) Tot. cut on south side of channel
(d) 1/0 ft. cut on south side of channel
(e) 1/0 ft. cut on south side of channel

Prepared: 1 Feb 66

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