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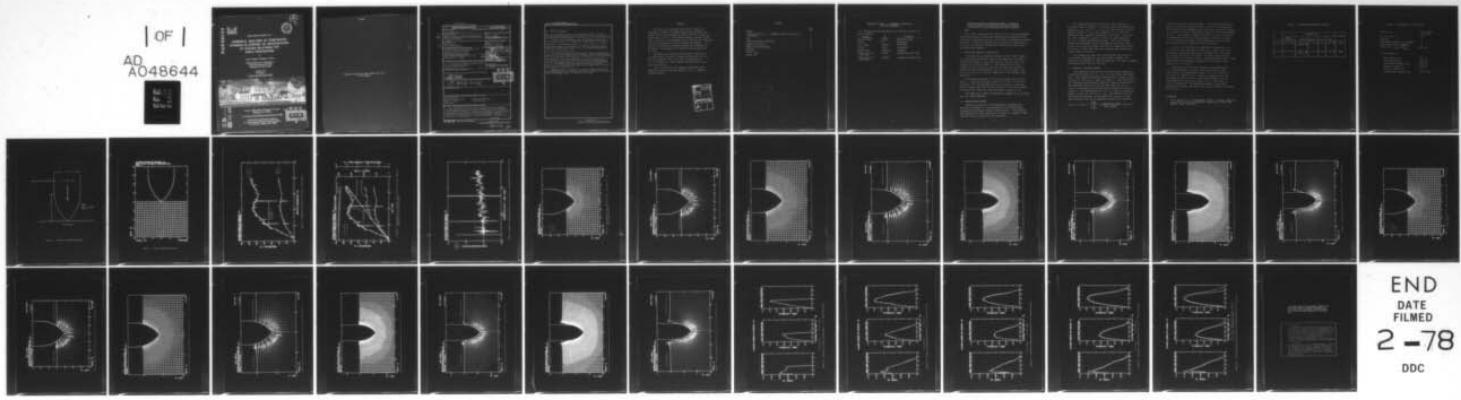
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NUMERICAL ANALYSES OF PENETRATION DYNAMICS IN SUPPORT OF INVEST--ETC(U)
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NUMERICAL ANALYSES OF PENETRATION DYNAMICS IN SUPPORT OF INVESTIGATIONS OF SCALING RELATIONS FOR EARTH PENETRATORS

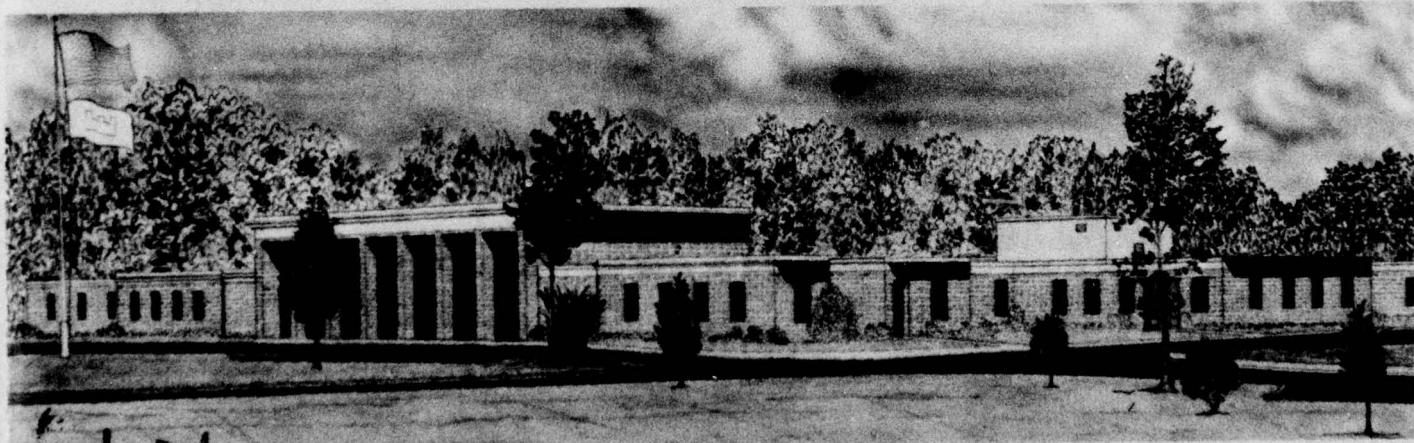
by

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November 1977
Final Report

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Under Project 4A161102AT22, Task A2, Work Unit 006

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20. ABSTRACT (Continued).

were 6 in. in diameter with a 6-in. long, ogive nose ($CRH = 1.25$). Two penetrator weights were selected, 141.4 lb (Case 1) and 424.1 lb (Case 2), to give projectile sectional pressures of 5 psi for Case 1 and 15 psi for Case 2.

At impact, the force loadings on the rigid penetrators for the two cases are the same, $F_1 = F_2$, so the ratio of decelerations will be inversely proportional to the penetrator weights, i.e., $a_1/a_2 = W_2/W_1 = 3$. As the penetration proceeds, the velocities will differ, due to the different decelerations, and the ratio of forces, F_2/F_1 , or scaled deceleration ratio, a_2W_2/a_1W_1 , will depart from unity.

Each penetrator problem was run from initial impact to a penetration depth of 8 in., or 2 in. below full nose embedment. From the solution results, the scaled deceleration ratio, a_2W_2/a_1W_1 , was computed as a function of depth of penetration. These results showed that the scaled deceleration ratio oscillated around a value of 1 for about the first half of nose embedment; the ratio then steadily increased to a value of 1.06 at a penetration depth of 8 in. The gradually larger force applied to the heavier projectile (Case 2) is due to its penetration velocity staying higher (due to its lesser deceleration), relative to Case 1.

Detailed results of the numerical solutions, including time histories of the penetration dynamics variables, force loading distributions, and field plots of the target response, are presented in the report.

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PREFACE

This report was prepared by Messrs. Mark H. Wagner and Christopher C. Fulton of California Research and Technology, Inc., Woodland Hills, California, under PO DACA 39-77-M-0136 as a part of ongoing work at the U. S. Army Engineer Waterways Experiment Station (WES), DA Project No. 4A161102AT22, Task A2, Work Unit 006, "Effectiveness of Earth Penetrators in Various Geologic Environments." The principal investigator at California Research and Technology was Mr. Wagner. The programming and computer runs were conducted by Mr. Fulton.

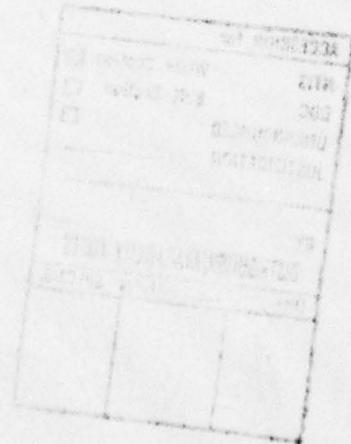
The problems were specified and the work was monitored by Dr. Behzad Rohani of the Soil Dynamics Division (SDD), Soils and Pavements Laboratory (S&PL), WES, under the supervision of Dr. J. G. Jackson, Jr., and the general direction of Mr. James P. Sale, Chief of S&PL.

COL J. L. Cannon, CE, was Commander and Director of WES during the period this work was performed. Mr. F. R. Brown was Technical Director.

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CONVERSION FACTORS, U. S. CUSTOMARY TO METRIC (SI)
UNITS OF MEASUREMENT

U. S. customary units of measurement can be converted to metric (SI) units as follows:

Multiply	By	To Obtain
feet per second	0.3048	metres per second
inches	25.4	millimetres
kips (force)	4.448222	kiloneutons
microns	0.001	millimetres
pounds (mass)	0.4535924	kilograms
pounds (mass) per cubic foot	16.01846	kilograms per cubic metre
pounds (mass) per square foot	4.882428	kilograms per square metre

NUMERICAL ANALYSES OF PENETRATION DYNAMICS IN SUPPORT OF
INVESTIGATIONS OF SCALING RELATIONS FOR EARTH PENETRATORS

1. SCOPE

This report describes the results of two finite-difference code calculations of projectile penetration conducted by CRT to provide information of use in the study of penetrator scaling relations. These two calculations were designed specifically to assess the effect of projectile sectional pressure (W/A) on penetration dynamics.

2. PENETRATOR PROBLEM--DESCRIPTION

The penetrator problem considered was the normal impact of a rigid body projectile into a sand target at 200 ft/sec, as shown in Figure 1. The conditions of the two cases considered are listed in Table 1. The projectile for Case 2 weighed 3 times as much as the Case 1 projectile, giving a projectile sectional pressure, W/A, of 5 psi for Case 1 and 15 psi for Case 2; all other conditions of the problems were identical.

The nominal properties of the sand target are listed in Table 2. The material model used was the same as that used for Layer 2 of the Watchung Hill Site at DRES in a previous study¹, except that the low-pressure end of the yield surface was modified so that the cohesion would be equal to zero.

The penetrator/target interface was assumed to be frictionless for these problems.

3. COMPUTATIONAL METHOD

Numerical solutions of the penetrator problems were obtained with WAVE-L, a two-dimensional finite-difference Lagrangian hydrodynamic-elastic-plastic code. This code has been employed to solve penetrator problems on a number of DNA and WES projects during the past few years.

The computational grid set up for these problems is partially shown in Figure 2. The basic cell size used in the target was $\Delta r_0 = \Delta z_0 = 0.5$ in., or 1/6 the projectile radius. Beyond a radius of 6 in. and a depth of 12 in., the cell dimensions were gradually increased, in 2% steps. The overall grid extended to a radius of 30.5 in. and a depth of 36.5 in., comprising 2800 grid points. This grid extends beyond the boundaries of wave propagation that took place during these problems (which were run to a penetration depth of 8 in.), so that they could be restarted and continued to longer times if desired.

The integration time step varies during the course of the problem, depending on the sound speed, minimum cell dimension, and stability criteria selected. For these problems, the average time step was 1.5 μ sec.

4. NUMERICAL SOLUTION RESULTS

Each penetrator problem was run from initial impact to a penetration depth of 8 in., or 2 in. below full nose embedment. The computed penetrator deceleration vs depth of penetration for each case is shown in Figure 3. Time histories of the penetrator deceleration, velocity, depth of penetration, and attached length (vertical distance from nose tip to point of separation of projectile from target) are shown in Figure 4. The deceleration for Case 1 is seen to be approximately 3 times that of Case 2, i.e., the decelerations are inversely proportional to the penetrator weights (or W/A). This scaling ratio can be examined in detail from Figure 5,

which is a plot of $\frac{a_2(\frac{W}{A})_2}{a_1(\frac{W}{A})_1}$ vs penetration depth, where the nose length

subscripts refer to the case number. The scaled deceleration ratio is seen to oscillate around a value of 1 for about the first half of nose embedment; the ratio then steadily increases to a value of 1.06 at a penetration depth of 1-1/3 nose lengths (8 in.). The gradually larger force seen in Case 2 is due to its penetration velocity staying higher, relative to Case 1. At 8 in. penetration depth, the penetration velocities were 191 ft/sec for Case 1 and 197 ft/sec for Case 2.

A series of field plots at penetration depths of 2, 4, 6, and 8 in. were obtained for each case. These plots show the overall projectile and target configuration, the extent of plastic deformation in terms of the generalized plastic strain ($\bar{\epsilon}^P$), the velocity vectors, and the principal stress field. These plots are presented in Figures 6-13 for Case 1 and Figures 14-21 for Case 2.

Comparative plots of the loadings on the penetrator, i.e., distributions of normal stress, axial force, and radial force, for penetration depths of 2, 4, 6, and 8 in. are shown in Figures 22-25. Finally, a comparison of the loadings for Case 2 at 8 in. depth and for Case 1 at the same penetration velocity as Case 2, i.e., 197 ft/sec, is shown in Figure 26. The penetration depth at which the penetration velocity for Case 1 was at this value (197 ft/sec) was 3.85 in.

REFERENCE

1. M. H. Wagner, K. N. Kreyenhagen, and W. S. Goerke, *Numerical Analysis of DNA Earth Penetrator Experiment at DRES*, DNA Report 3537F, June 1975.

TABLE 1. PENETRATOR PROBLEM CONDITIONS

	Projectile					Target
	Impact Velocity	Weight	Diameter	W/A	Nose	
Case 1	200 ft/sec	64125 gm (141.4 lb)	6 in.	5 psi	1.25 CRH	Sand
Case 2	200 ft/sec	192375 gm (424.1 lb)	6 in.	15 psi	1.25 CRH	Sand

TABLE 2. PROPERTIES OF SAND TARGET

Density, ρ_0	1.4256 gm/cm ³ 89 lb/ft ³
Air fraction, V_a	40%
Cohesion	0
Unconfined compressive strength	0
Unconfined tensile strength	0
Mises limit, $Y_{max} = \sqrt{3J_2'}$	2906 psi

Initial value of:

Bulk modulus, K_0	3292 psi
Shear modulus, G_0	2469 psi
Constrained modulus, M_0	6585 psi
Young's modulus, E_0	5926 psi
Poisson's ratio, ν_0	0.2
Dilatational velocity, c_0	585 ft/sec

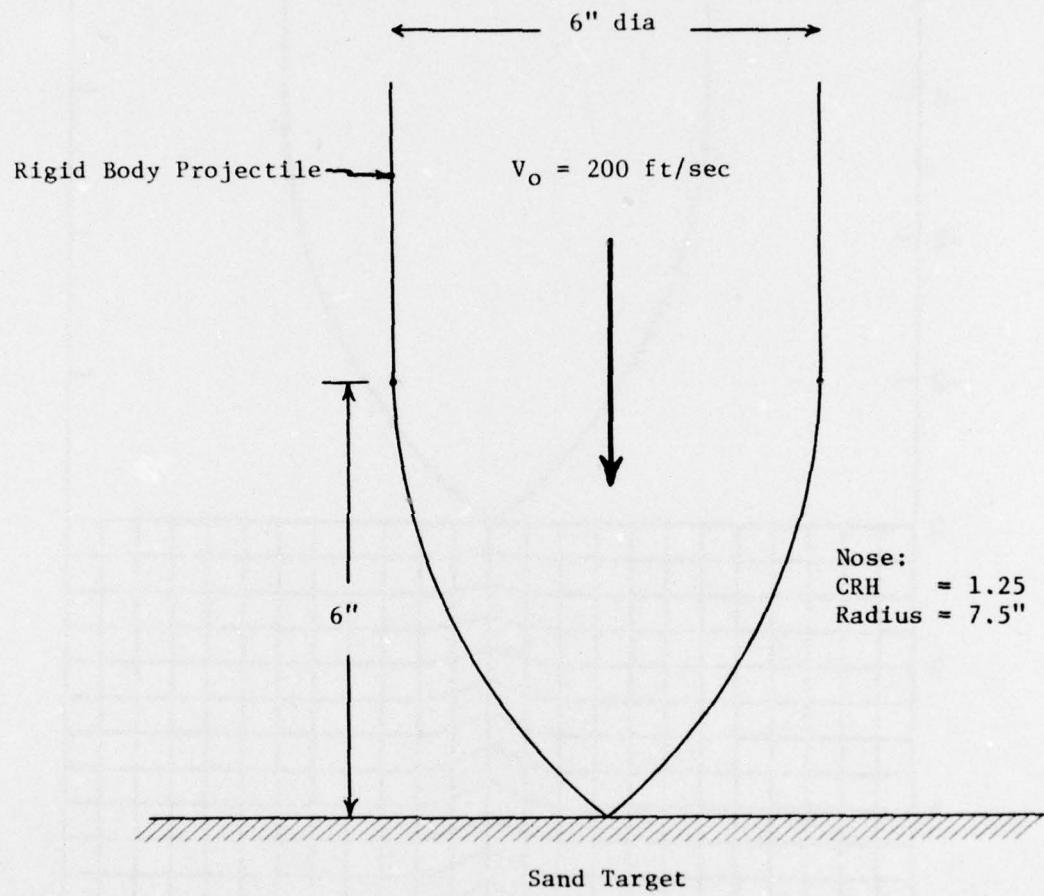


Figure 1. Penetrator Problem Conditions

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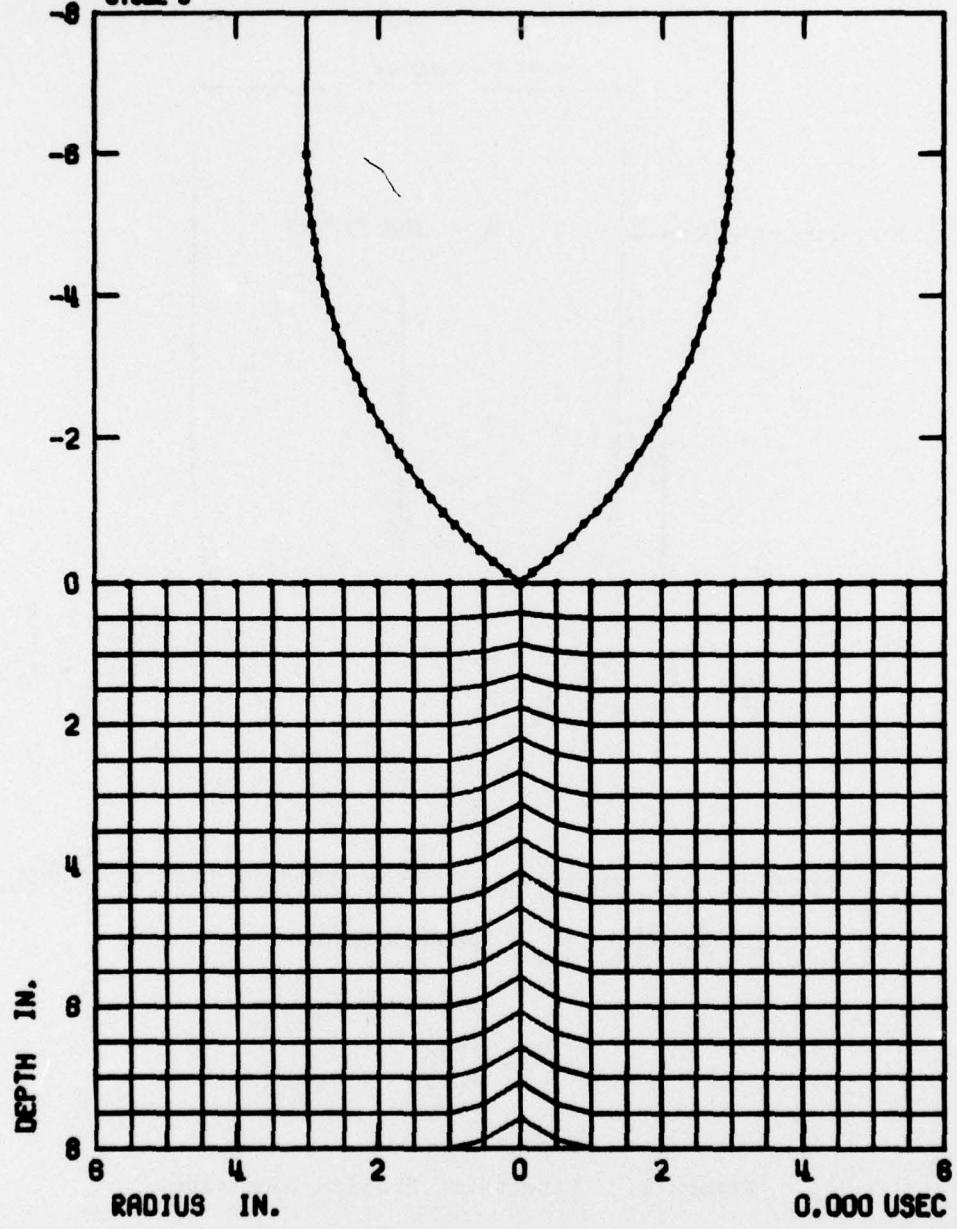


Figure 2. Initial Computational Grid.

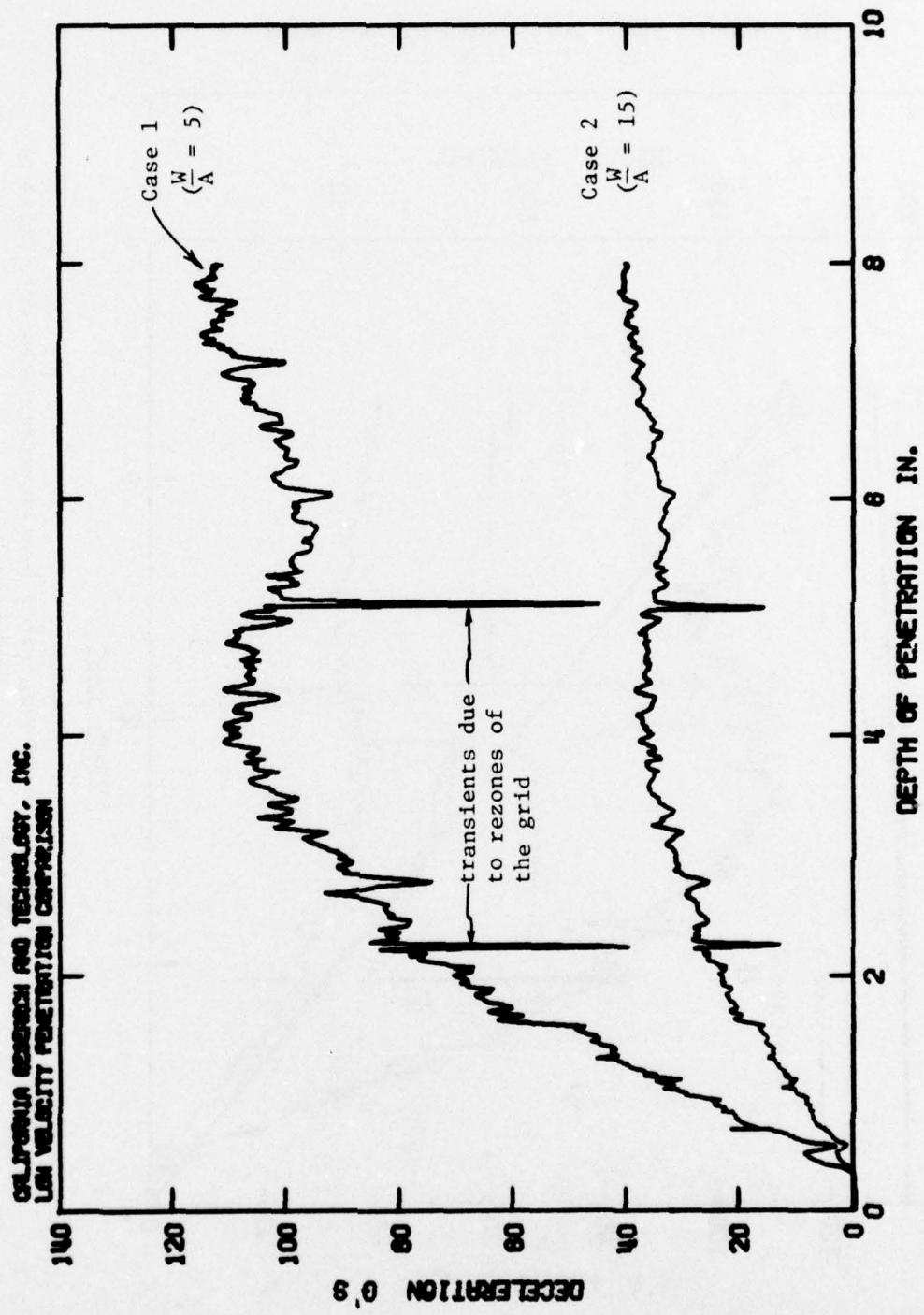


Figure 3. Penetrator Deceleration vs Depth of Penetration.

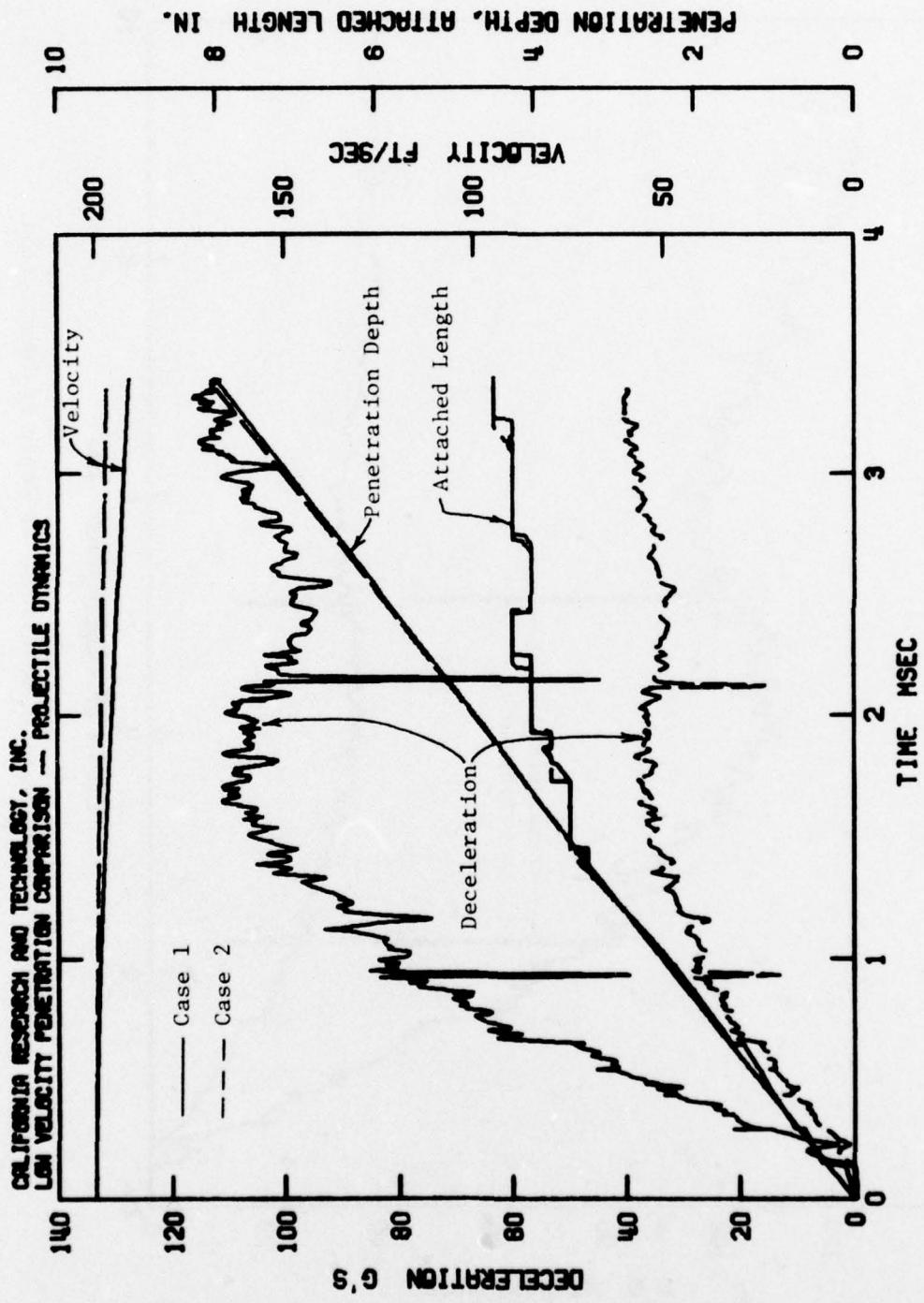


Figure 4. Time Histories of Penetrator Deceleration, Velocity, Depth of Penetration, and Attached Length.

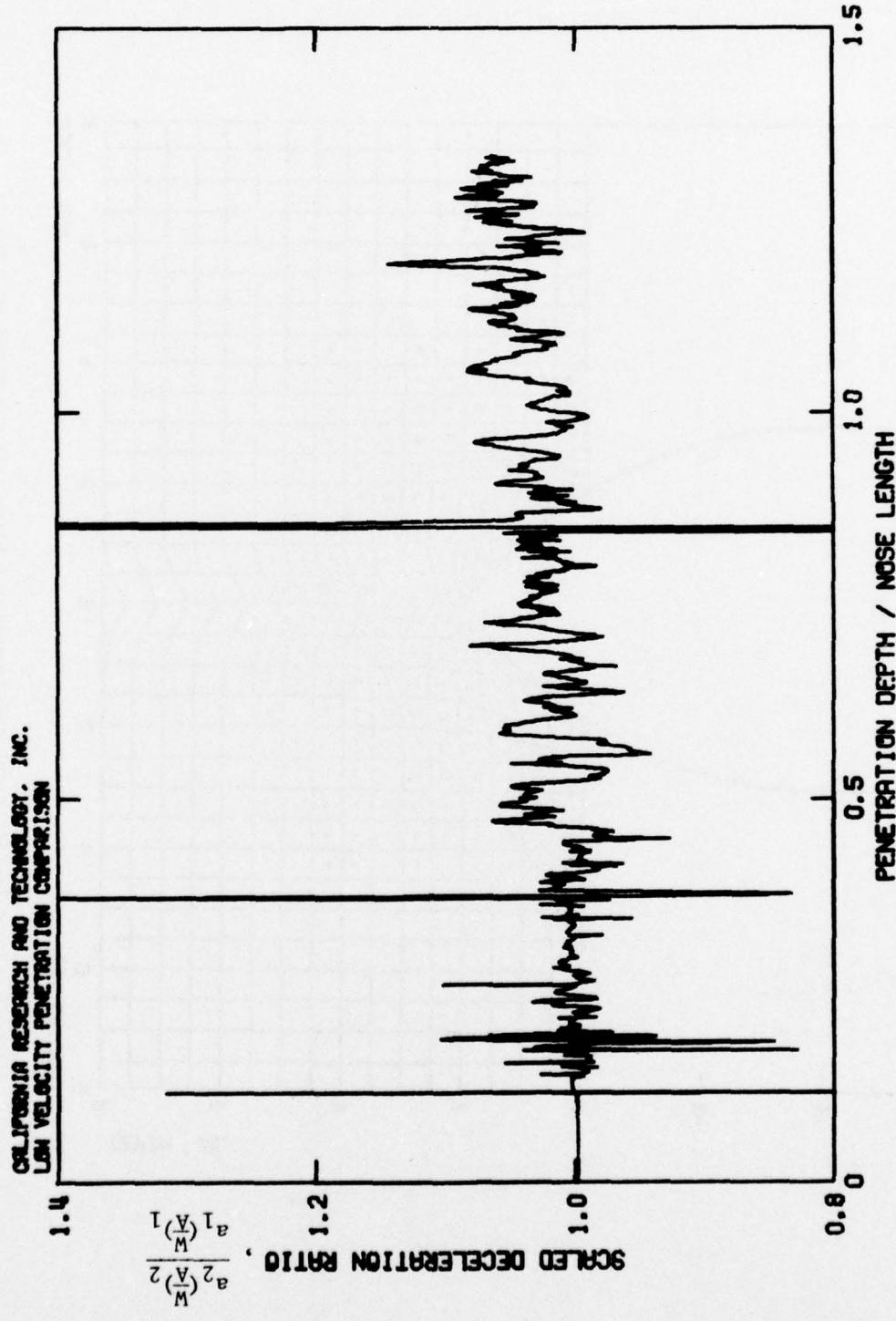


Figure 5. Ratio of Scaled Deceleration (Case 2:Case 1) vs Penetration Depth/Nose Length.

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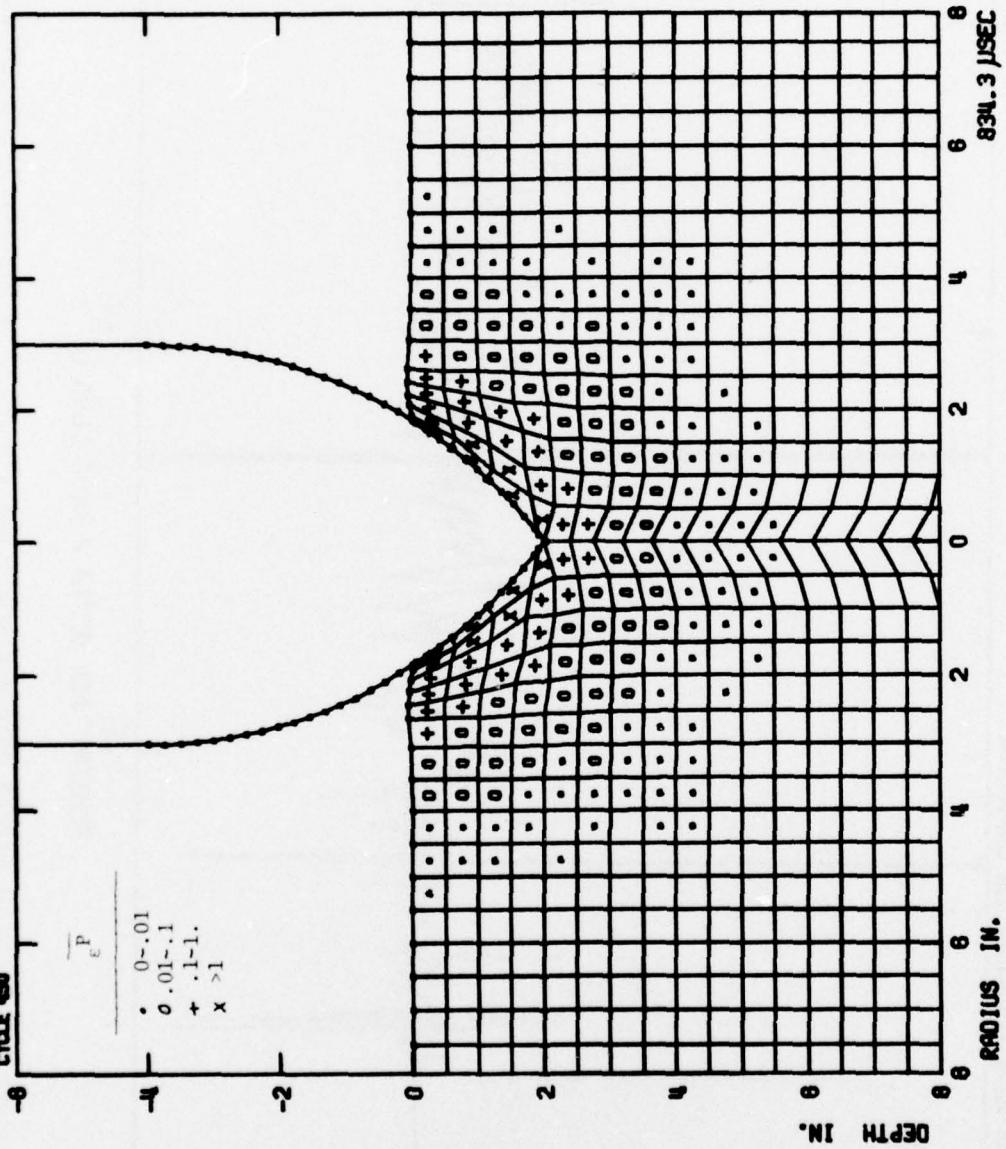


Figure 6. Grid Configuration and Generalized Plastic Strain Field at 2 in.
Penetration Depth, Case 1.

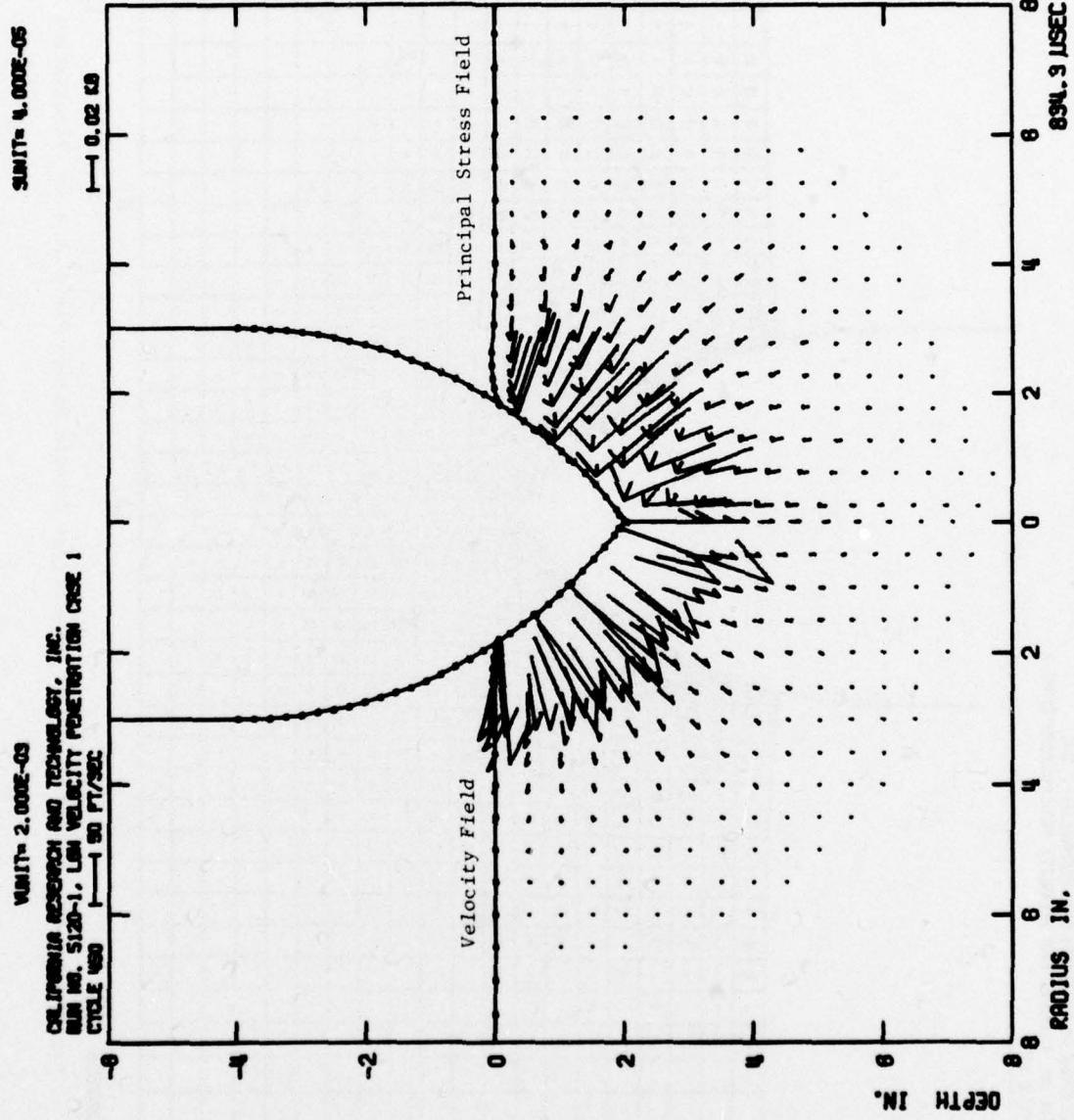


Figure 7. Velocity Field and Principal Stress Field at 2 in. Penetration Depth, Case 1.

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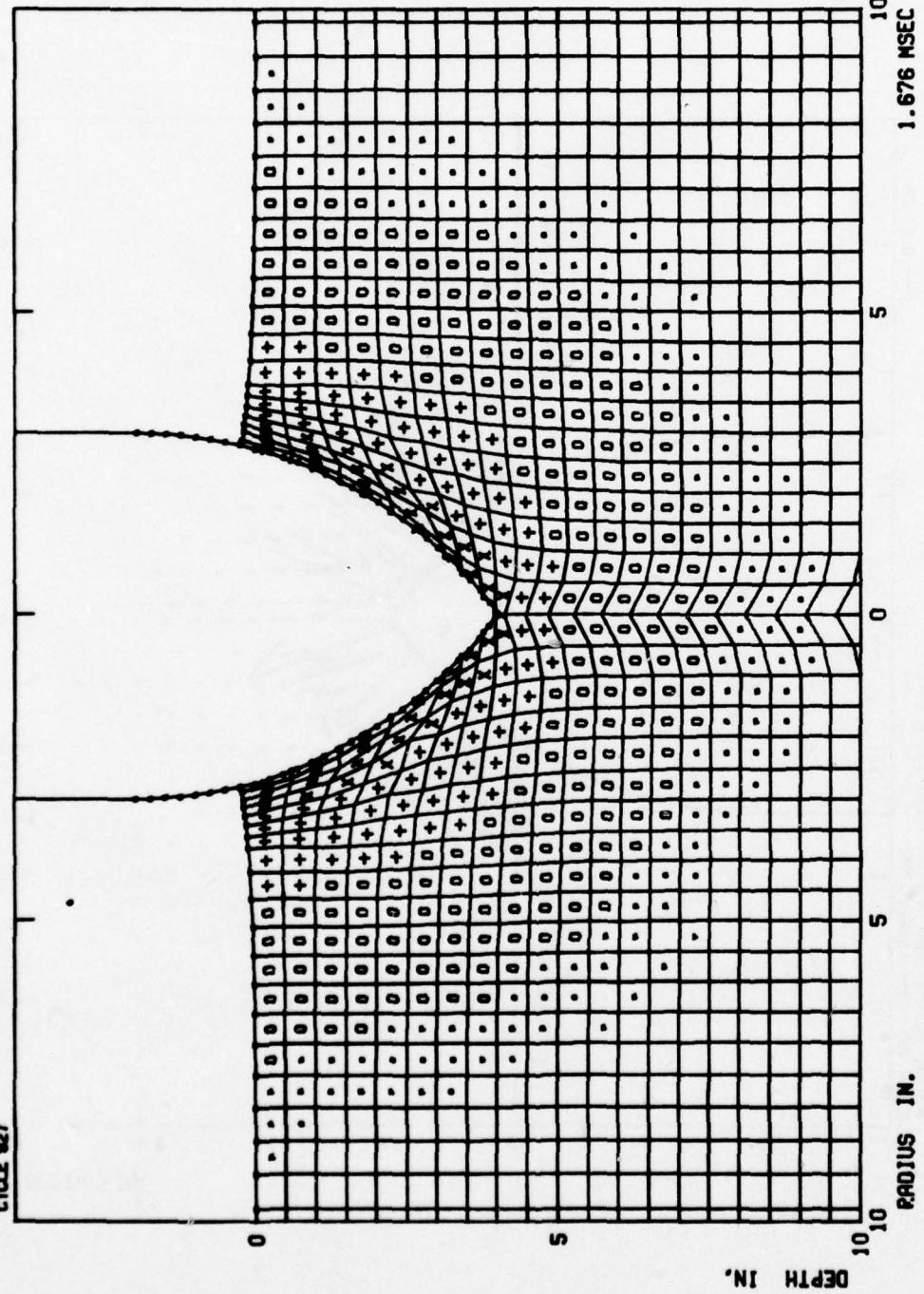


Figure 8. Grid Configuration and Generalized Plastic Strain Field at 4 in. Penetration Depth, Case 1.

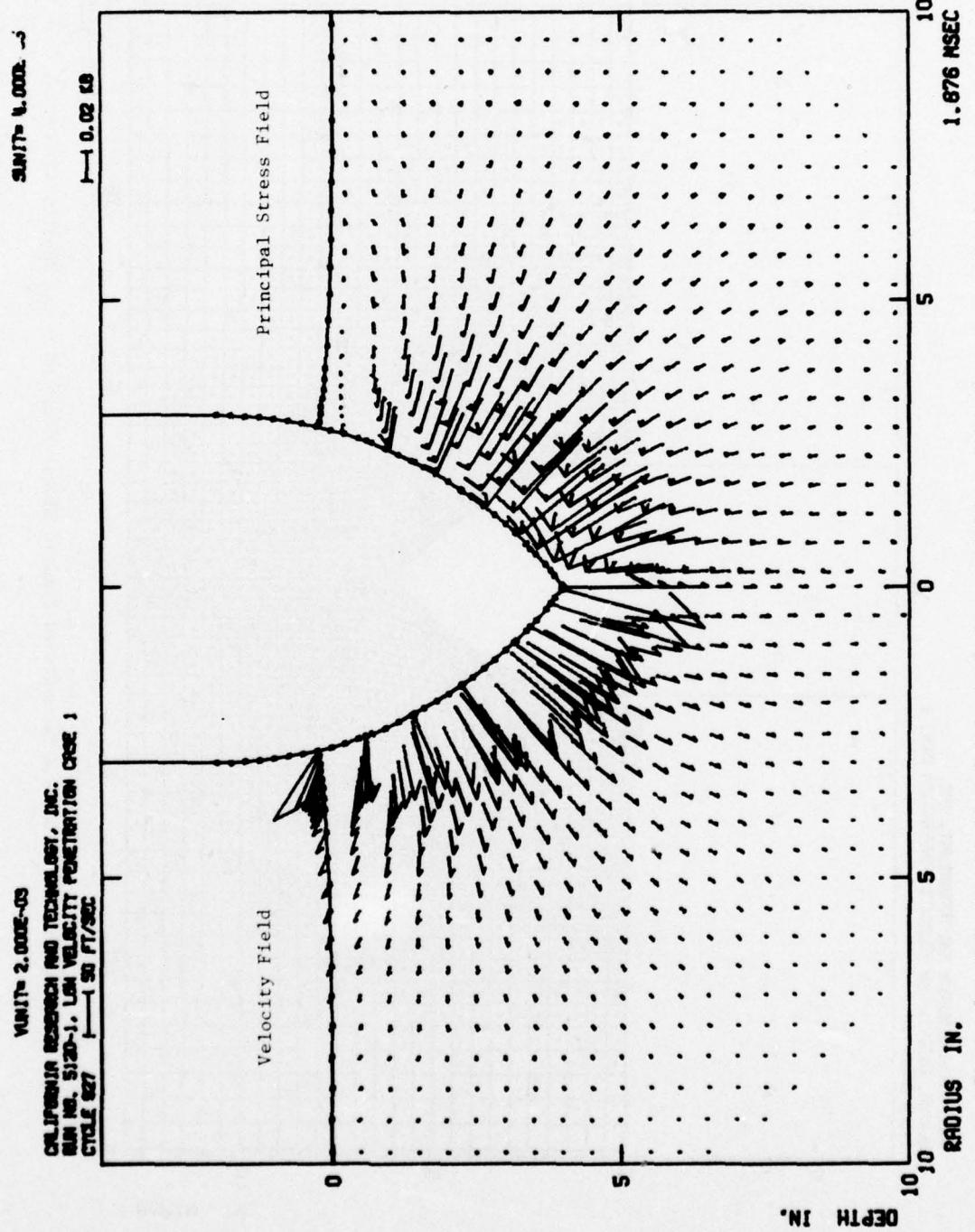


Figure 9. Velocity Field and Principal Stress Field at 4 in. Penetration Depth, Case 1.

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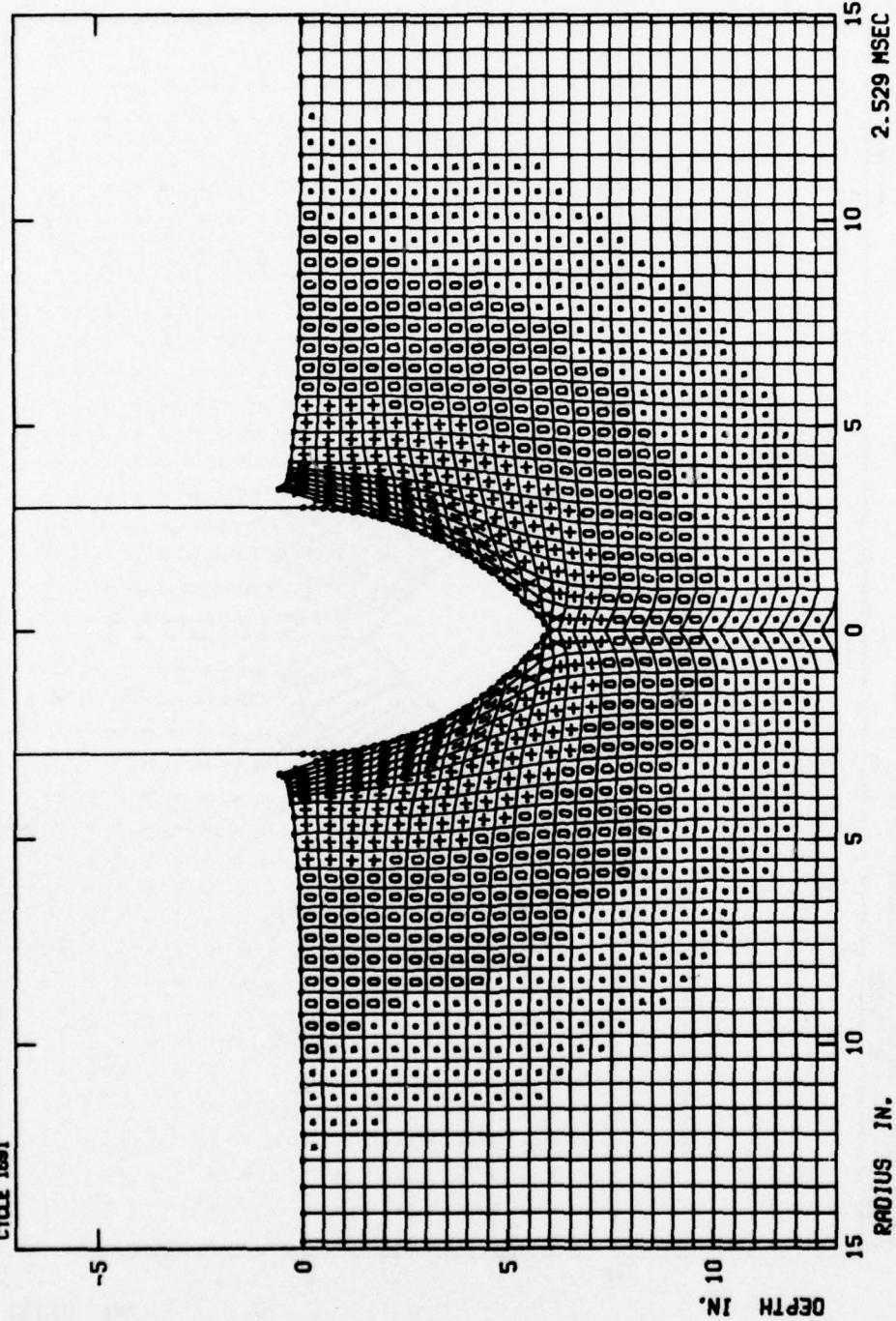


Figure 10. Grid Configuration and Generalized Plastic Strain Field at 6 in. Penetration Depth, Case 1.

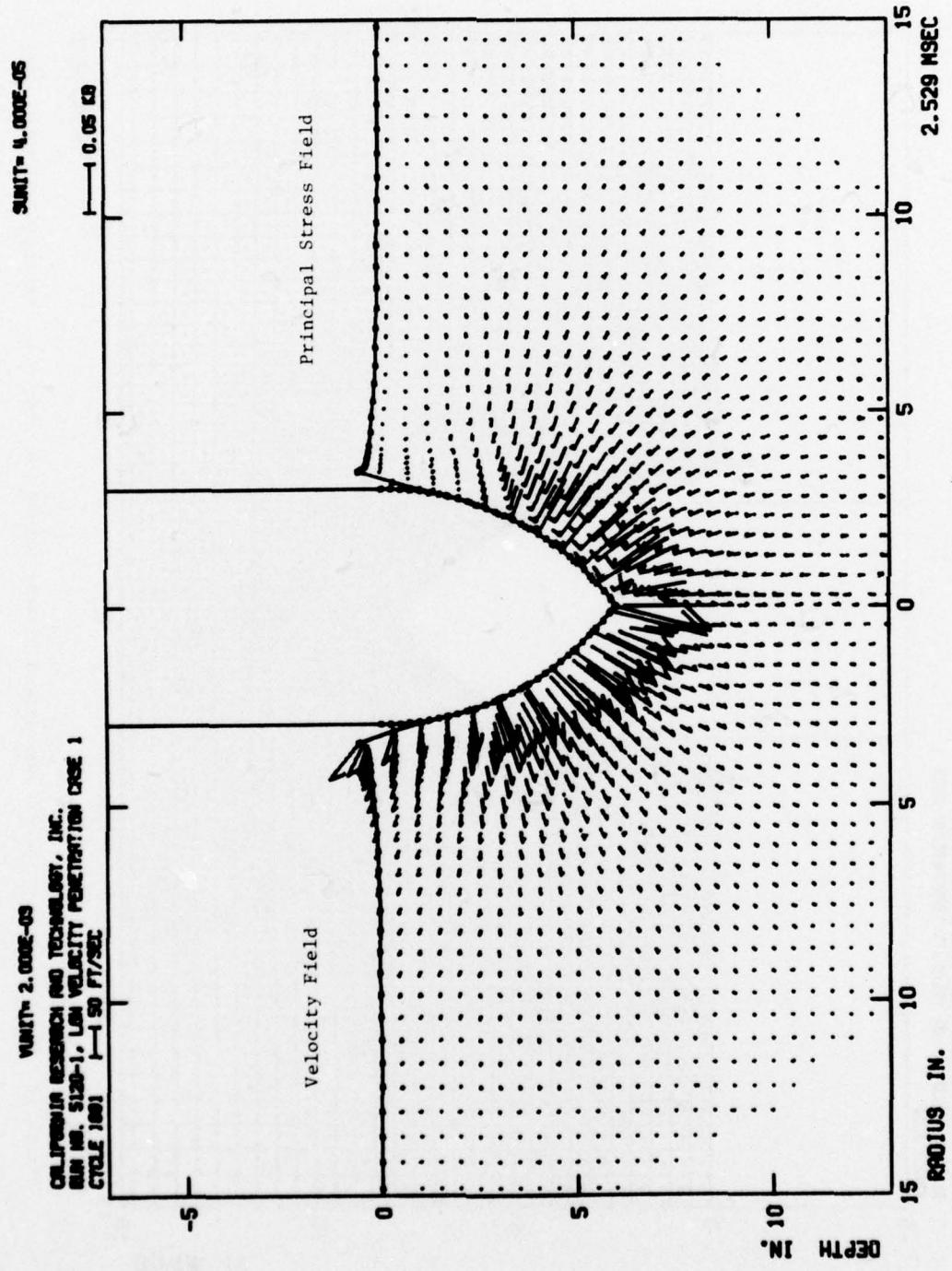


Figure 11. Velocity Field and Principal Stress Field at 6 in. Penetration Depth, Case 1.

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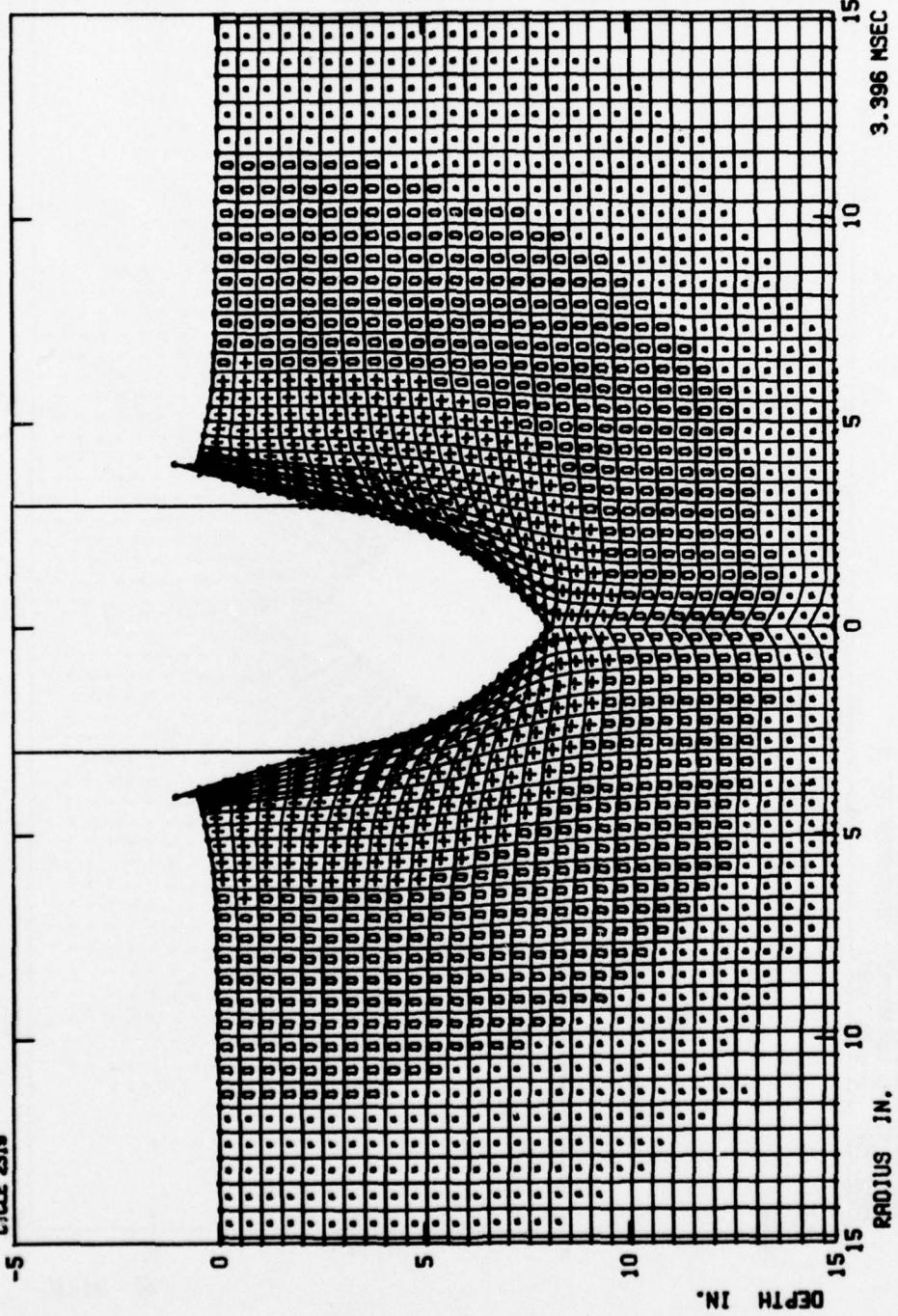


Figure 12. Grid Configuration and Generalized Plastic Strain Field at 8 in. Penetration Depth, Case 1.

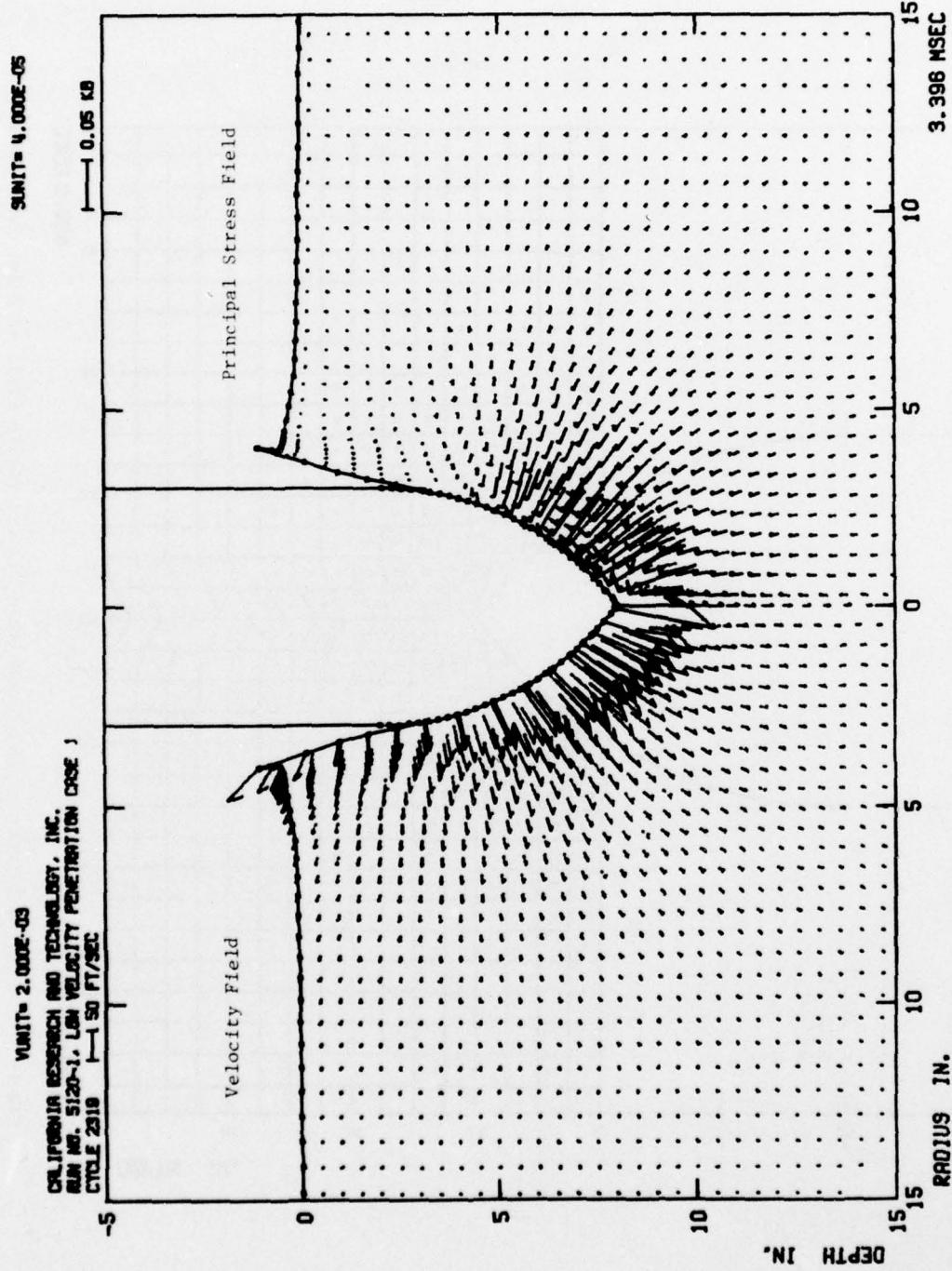


Figure 13. Velocity Field and Principal Stress Field at 8 in. Penetration Depth, Case 1.

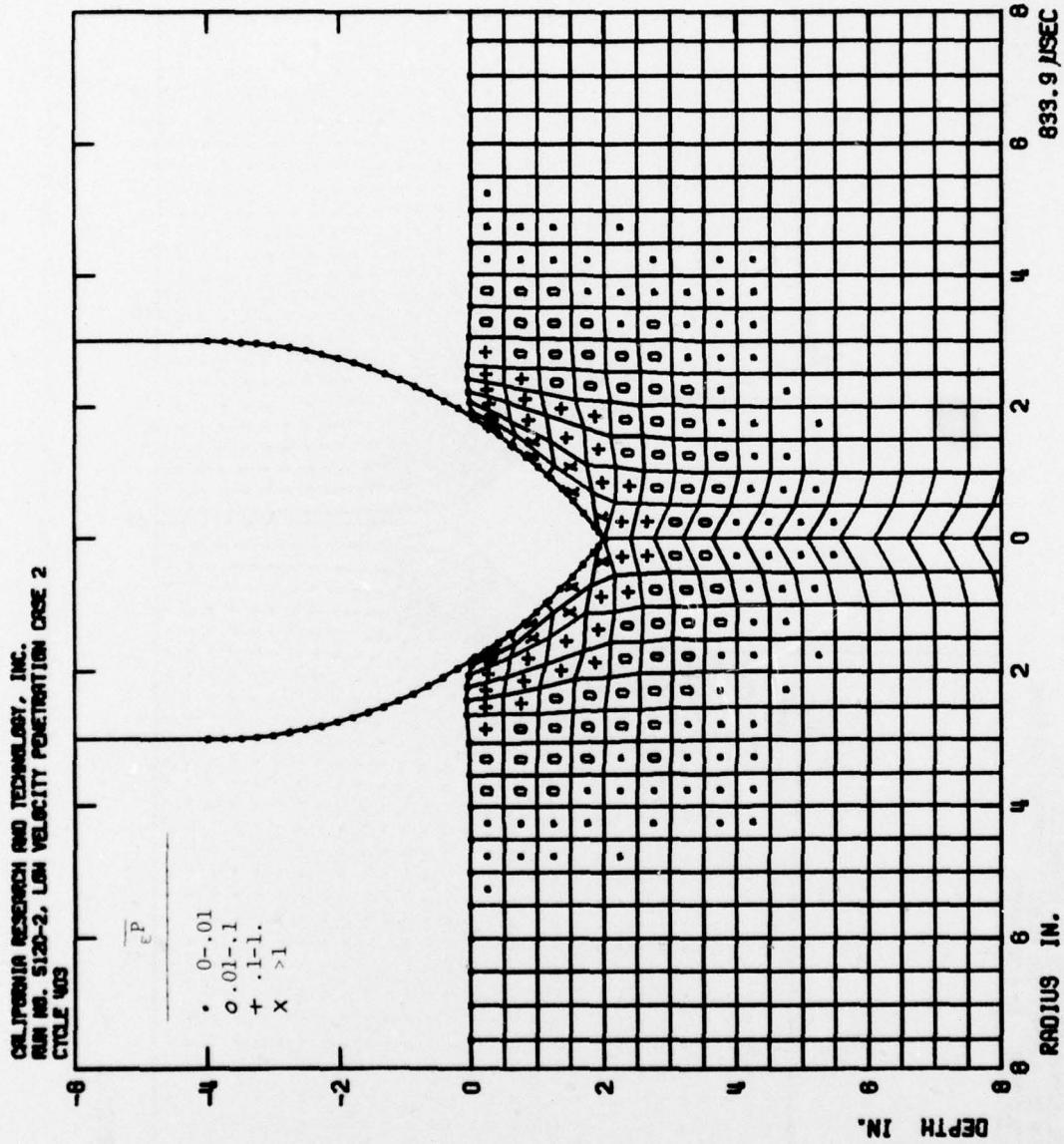


Figure 14. Grid Configuration and Generalized Plastic Strain Field at 2 in. Penetration Depth, Case 2.

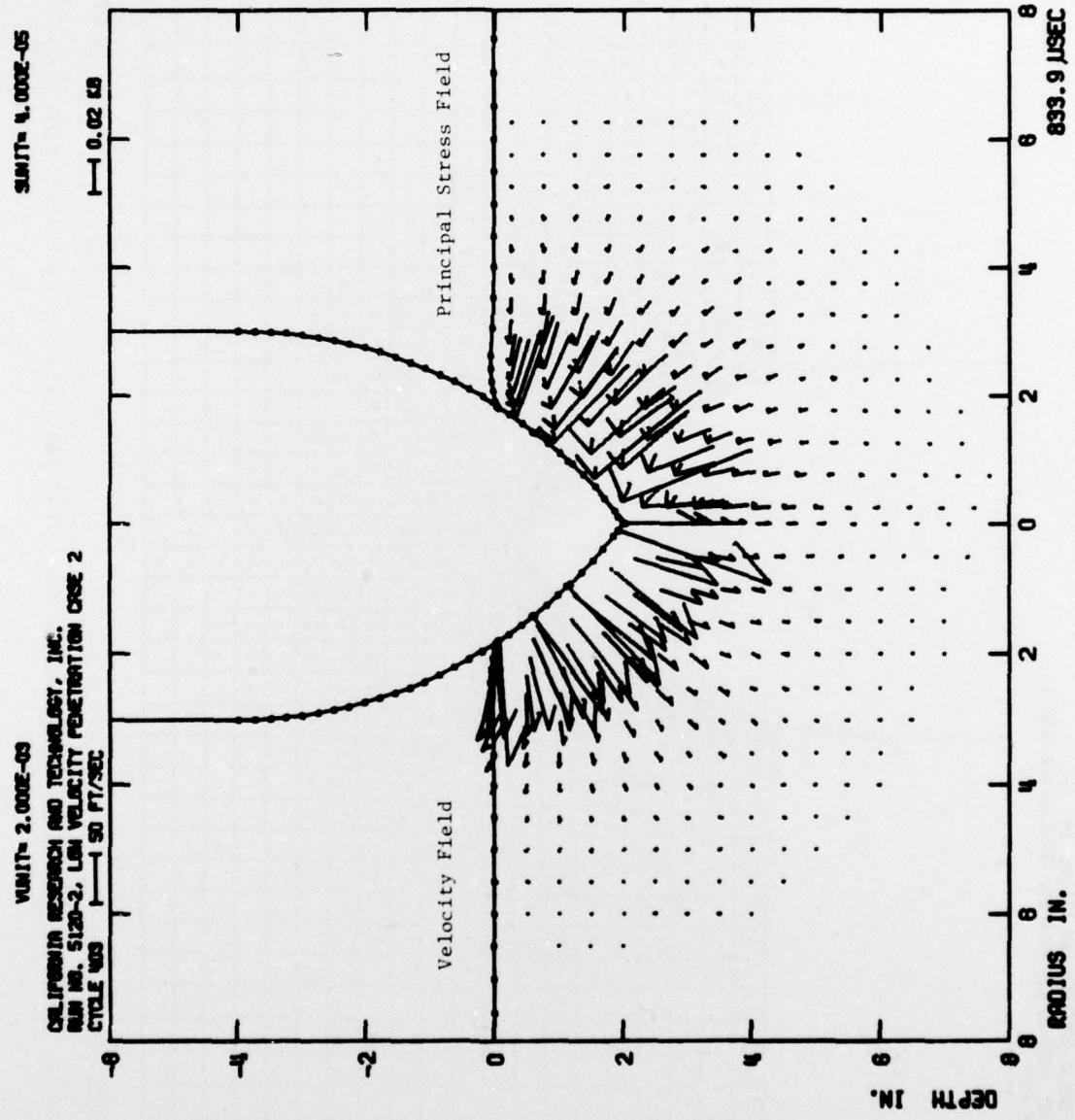


Figure 15. Velocity Field and Principal Stress Field at 2 in. Penetration Depth, Case 2.

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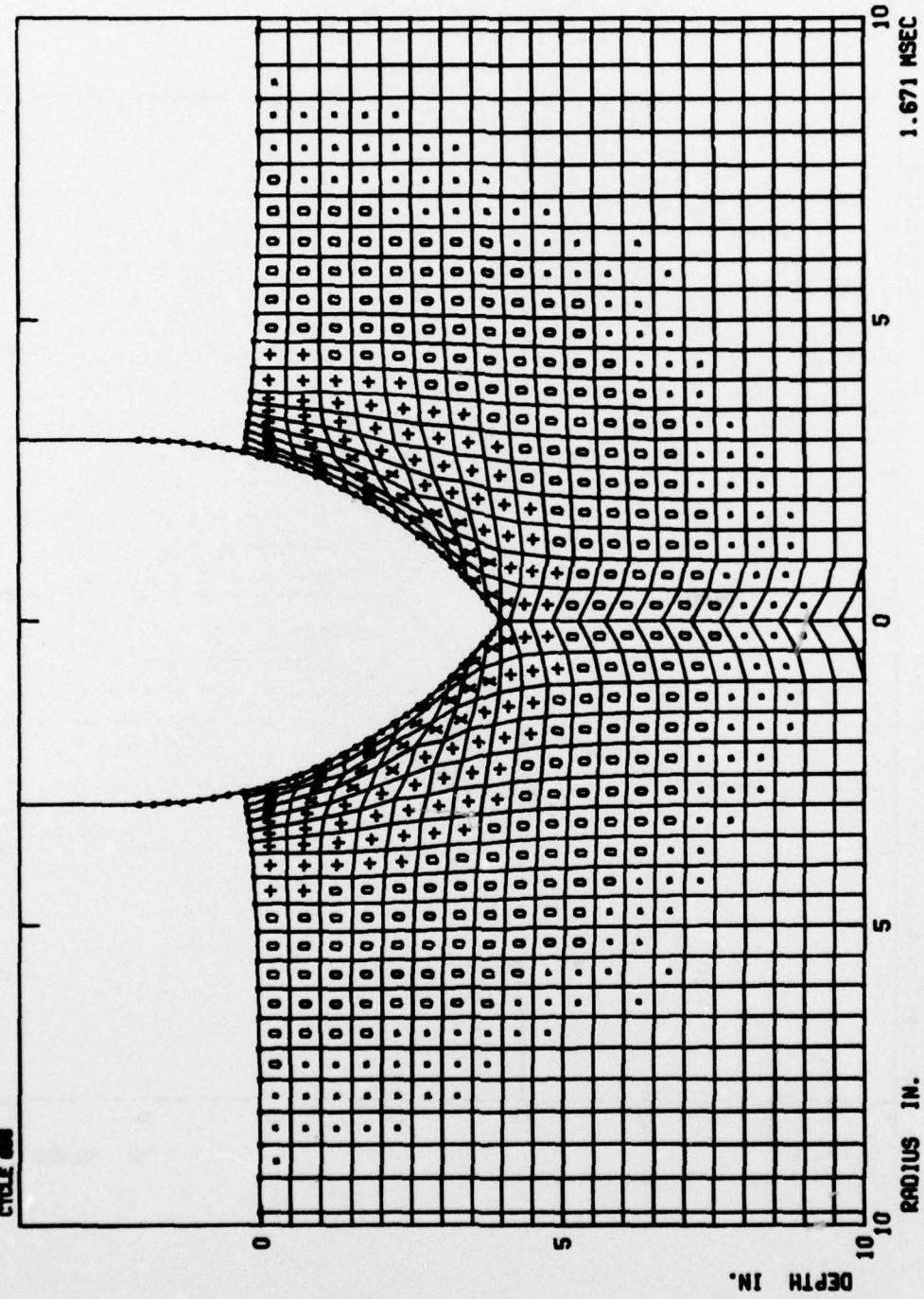


Figure 16. Grid Configuration and Generalized Plastic Strain Field at 4 in. Penetration Depth, Case 2.

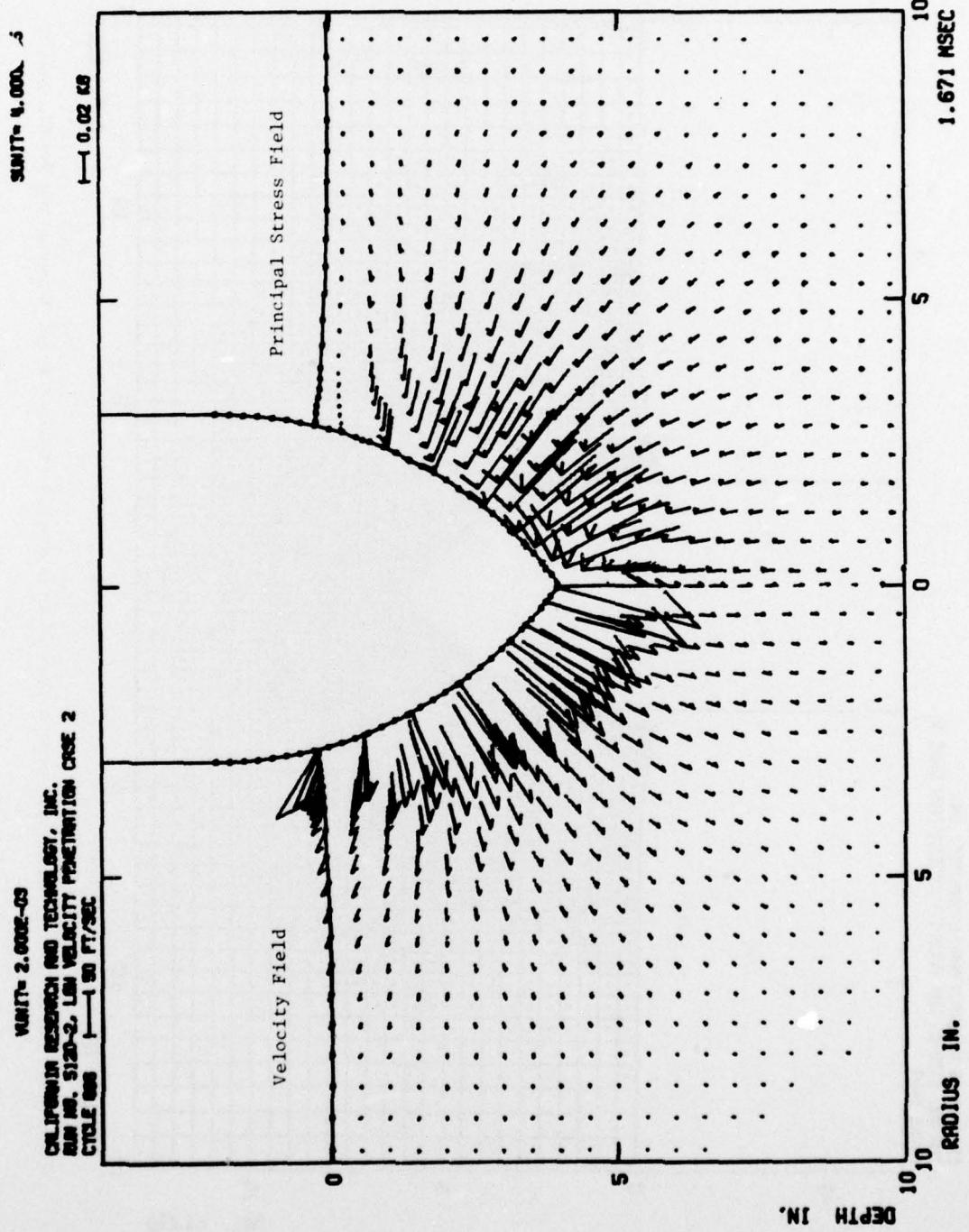


Figure 17. Velocity Field and Principal Stress Field at 4 in. Penetration Depth, Case 2.

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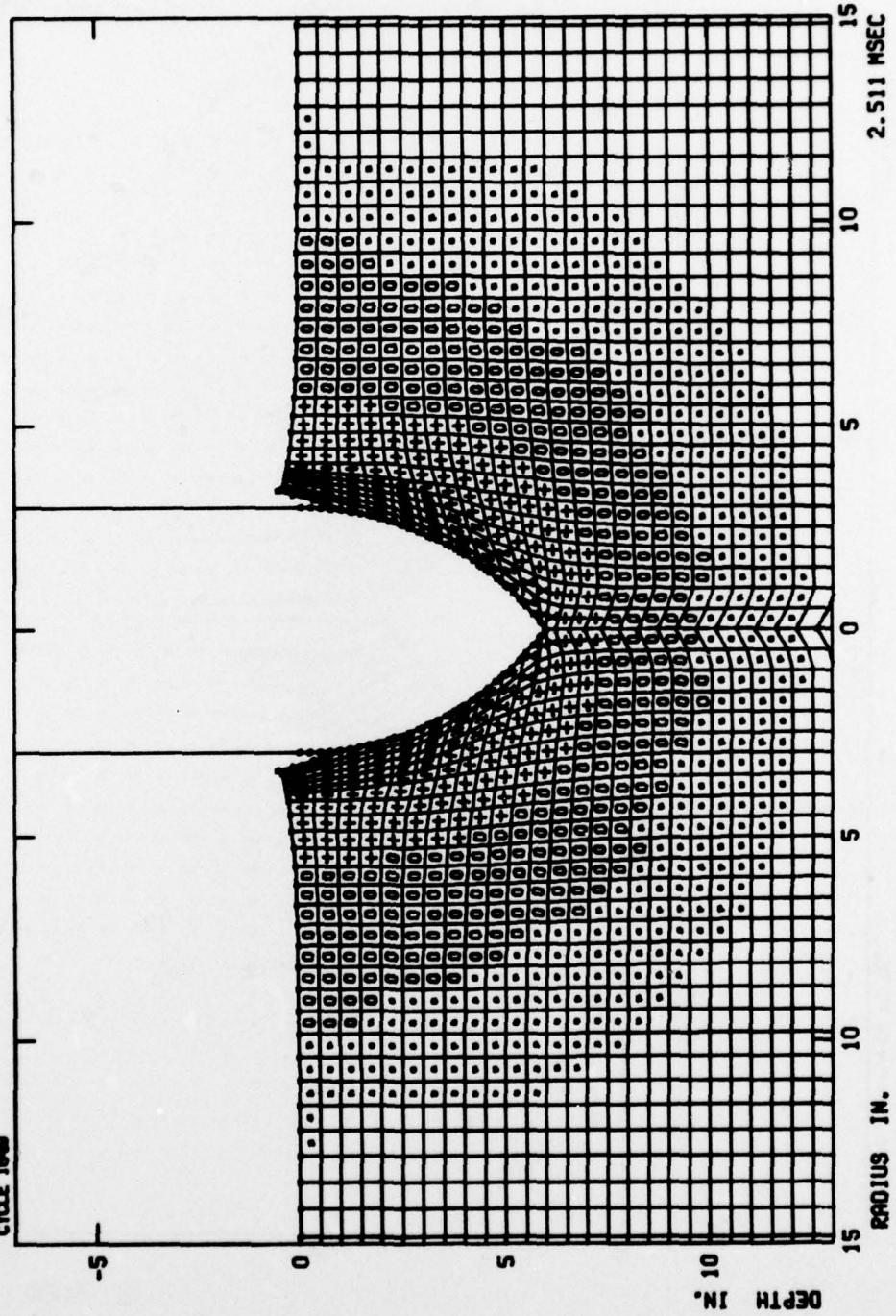


Figure 18. Grid Configuration and Generalized Plastic Strain Field at 6 in. Penetration Depth, Case 2.

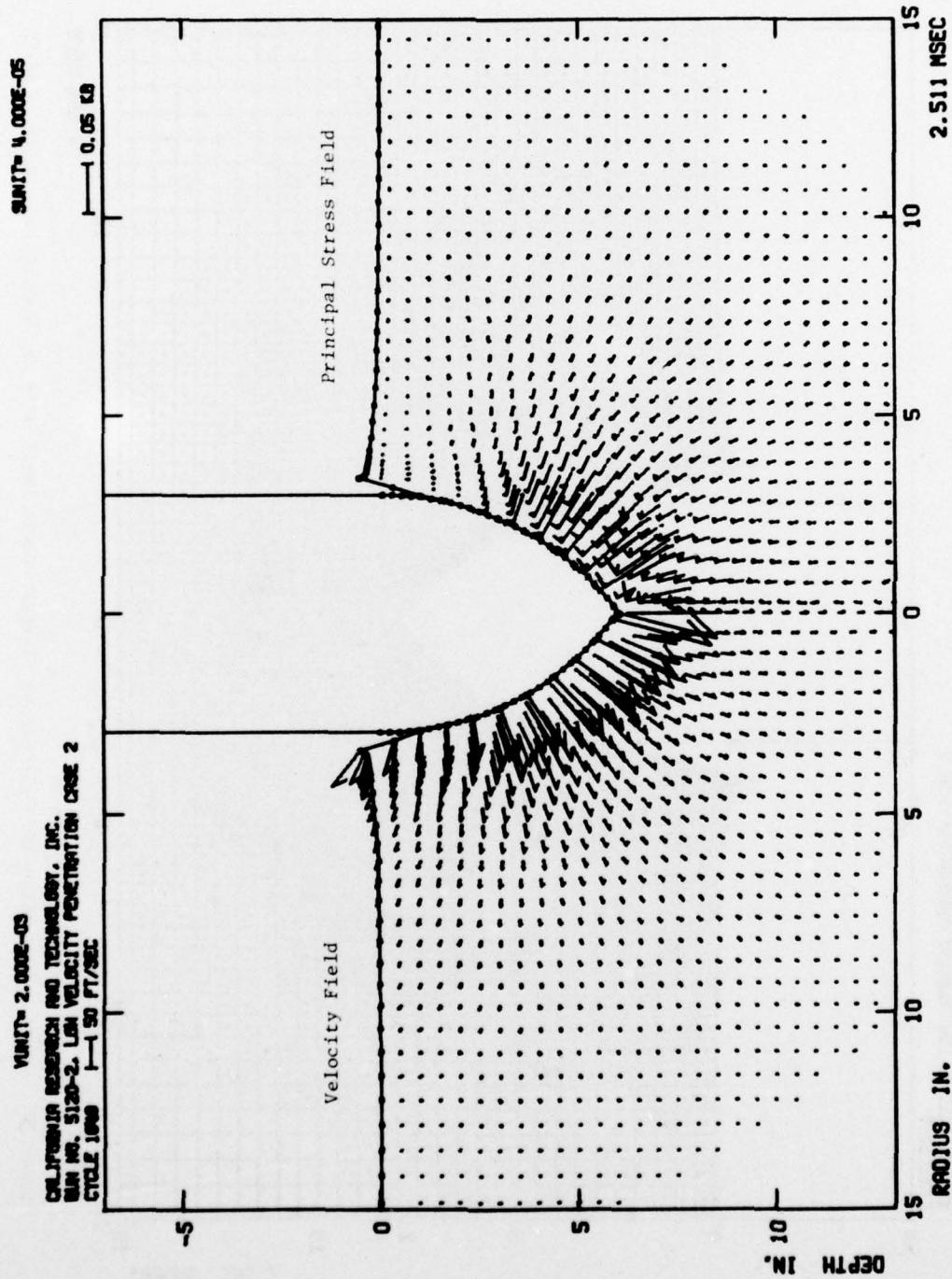


Figure 19. Velocity Field and Principal Stress Field at 6 in. Penetration Depth, Case 2.

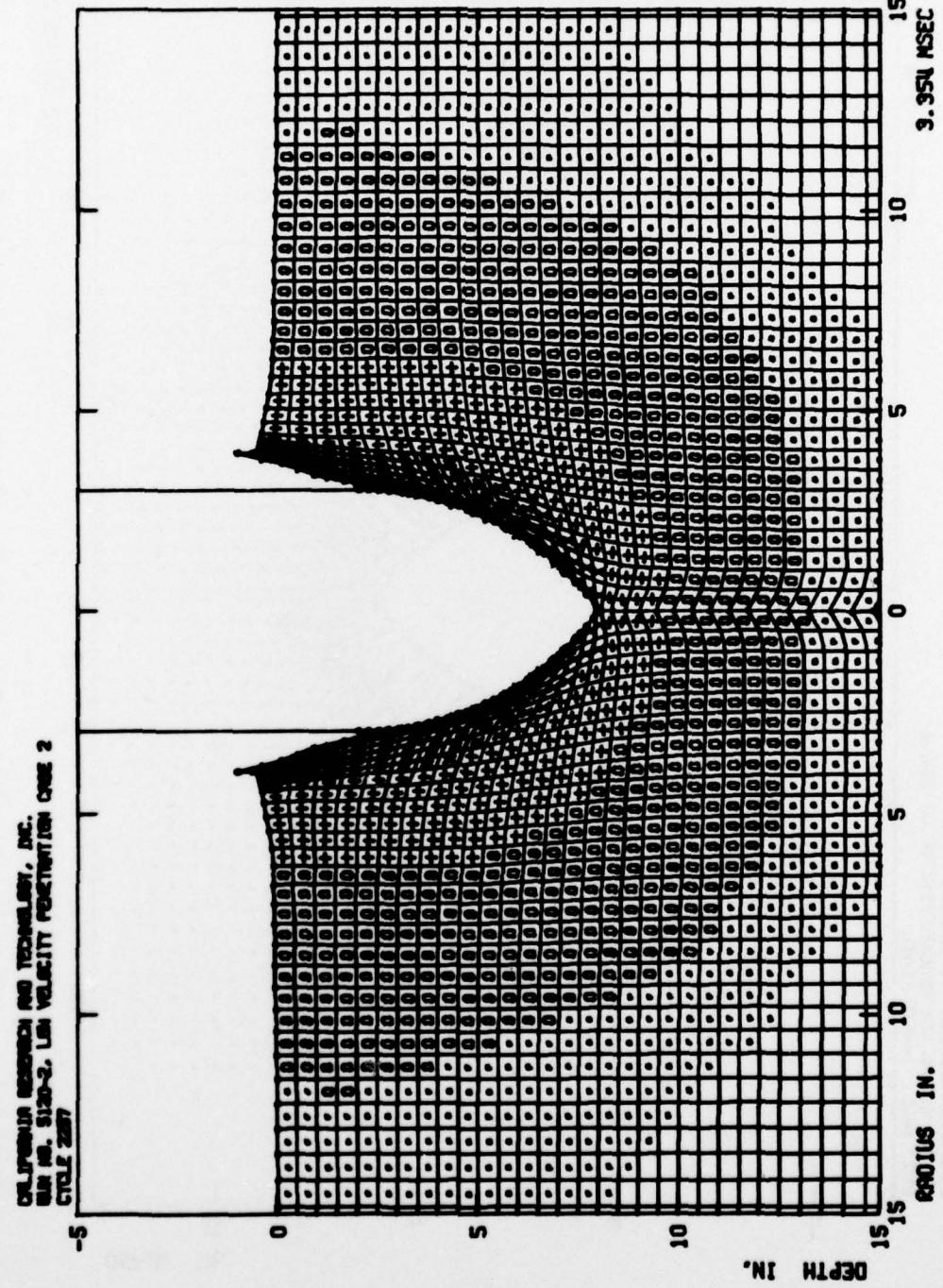


Figure 20. Grid Configuration and Generalized Plastic Strain Field at 8 in. Penetration Depth, Case 2.

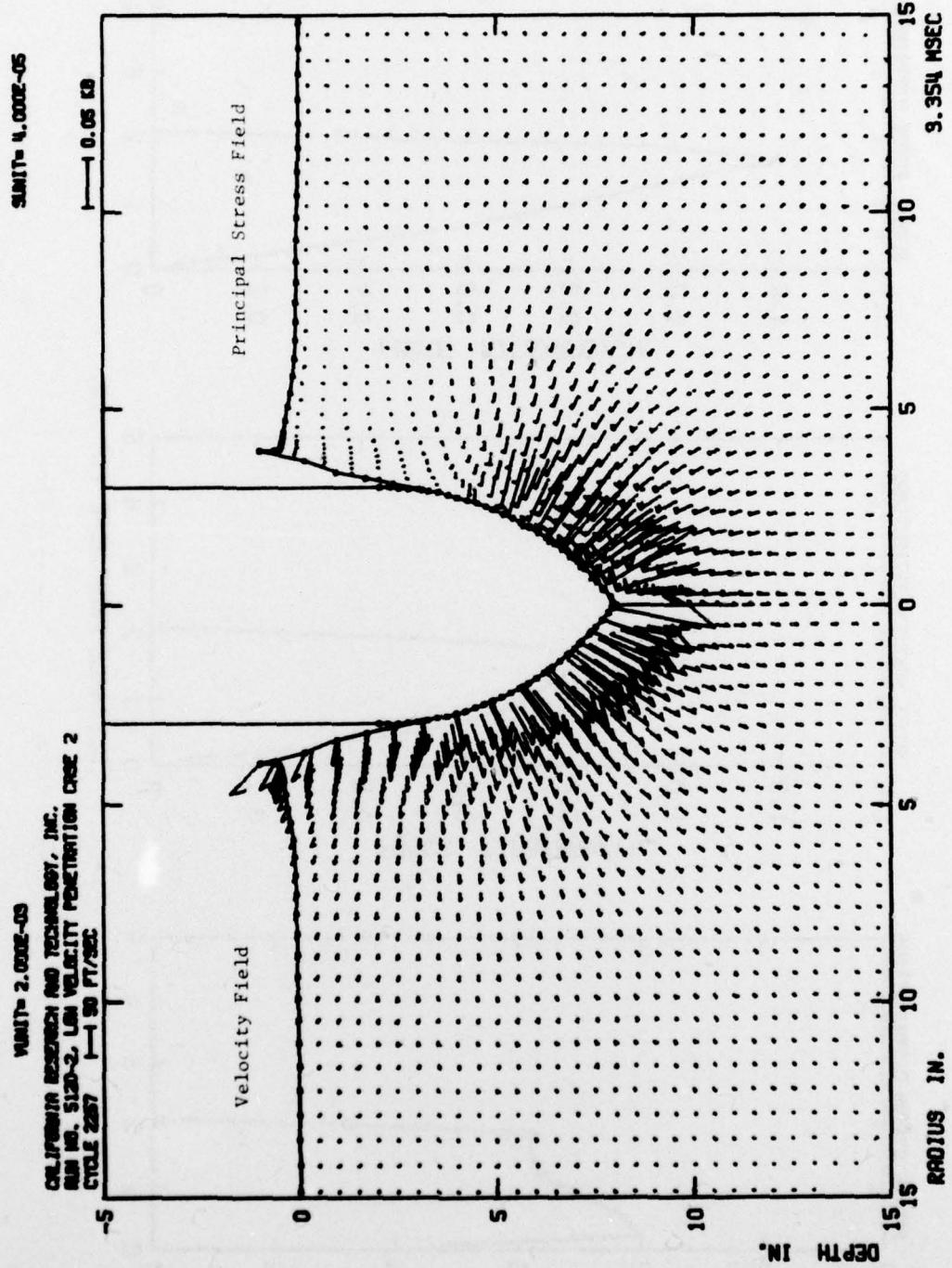


Figure 21. Velocity Field and Principal Stress Field at 8 in. Penetration Depth, Case 2.

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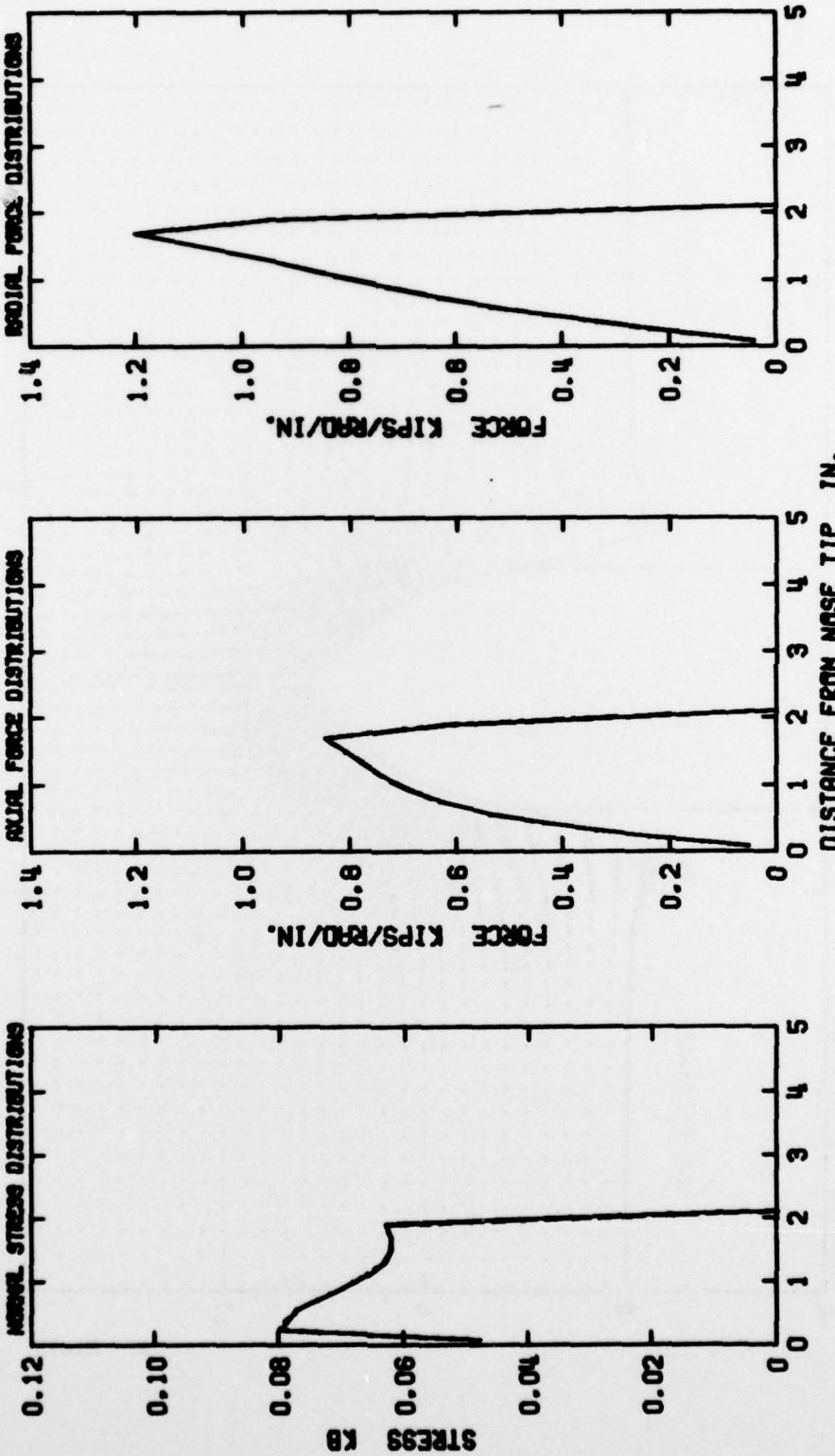


Figure 22. Penetrator Loading Distributions at 2 in. Penetration Depth.

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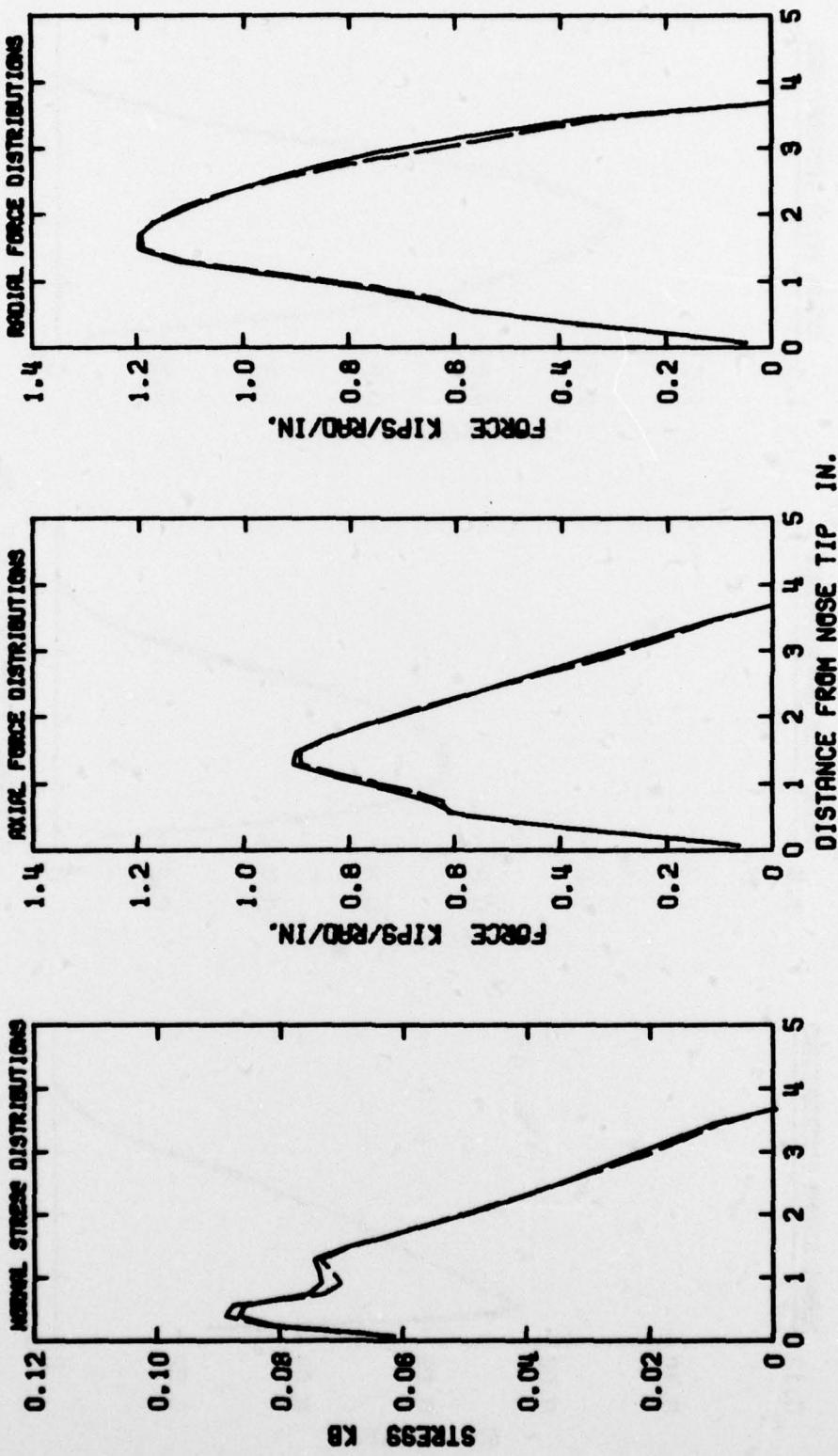


Figure 23. Penetrator Loading Distributions at 4 in. Penetration Depth.

— Case 1 - - - Case 2

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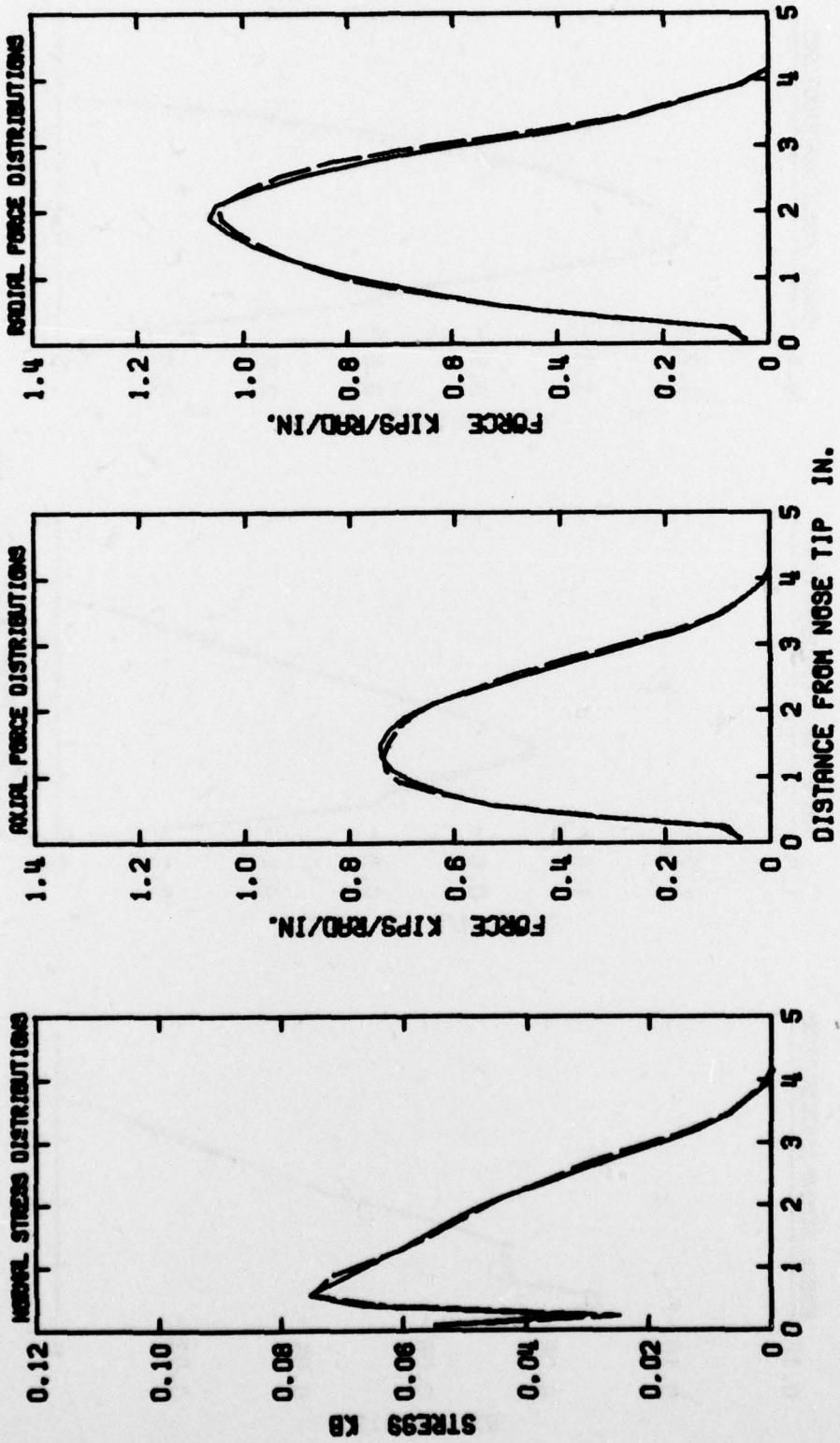


Figure 24. Penetrator Loading Distributions at 6 in. Penetration Depth.

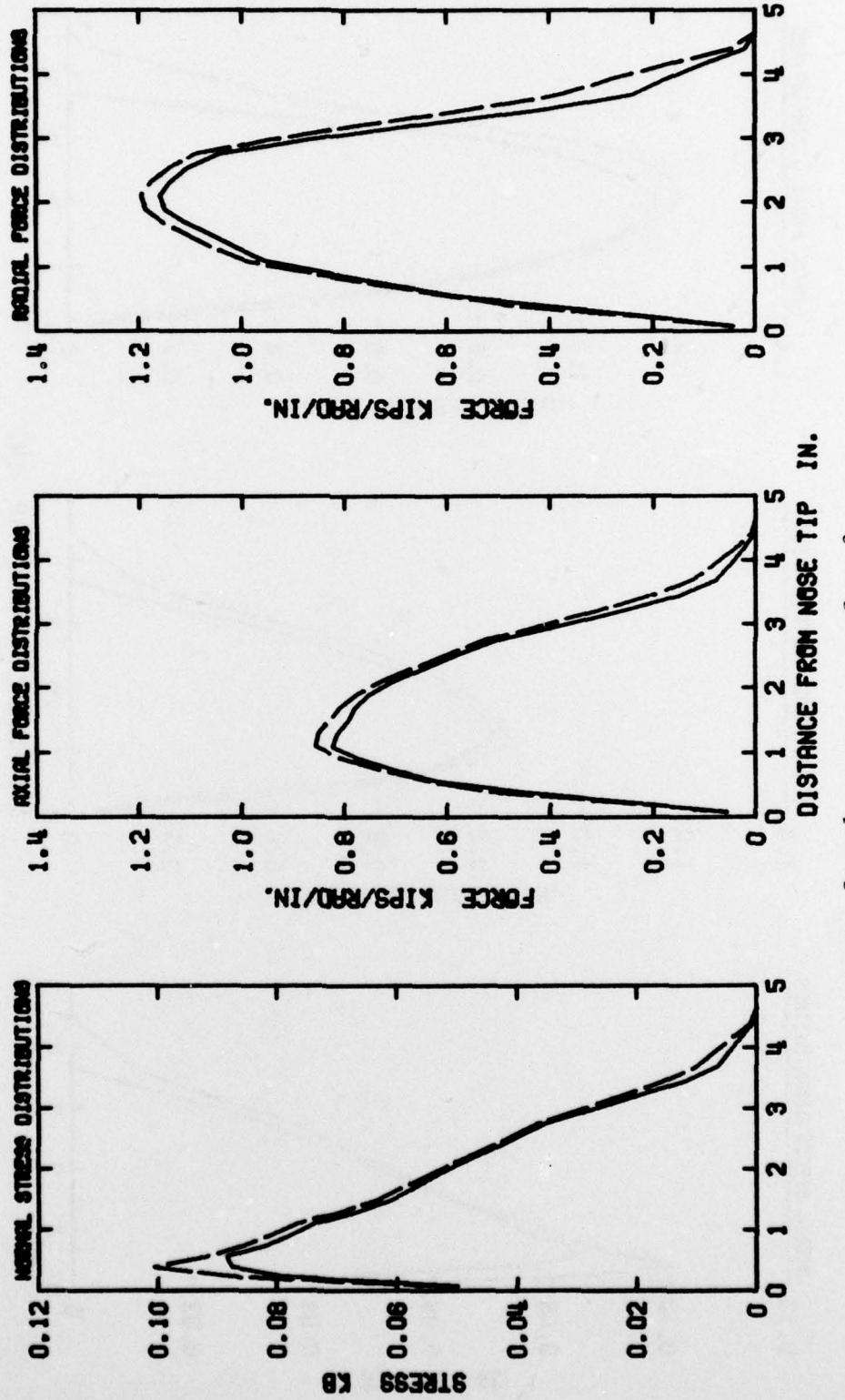
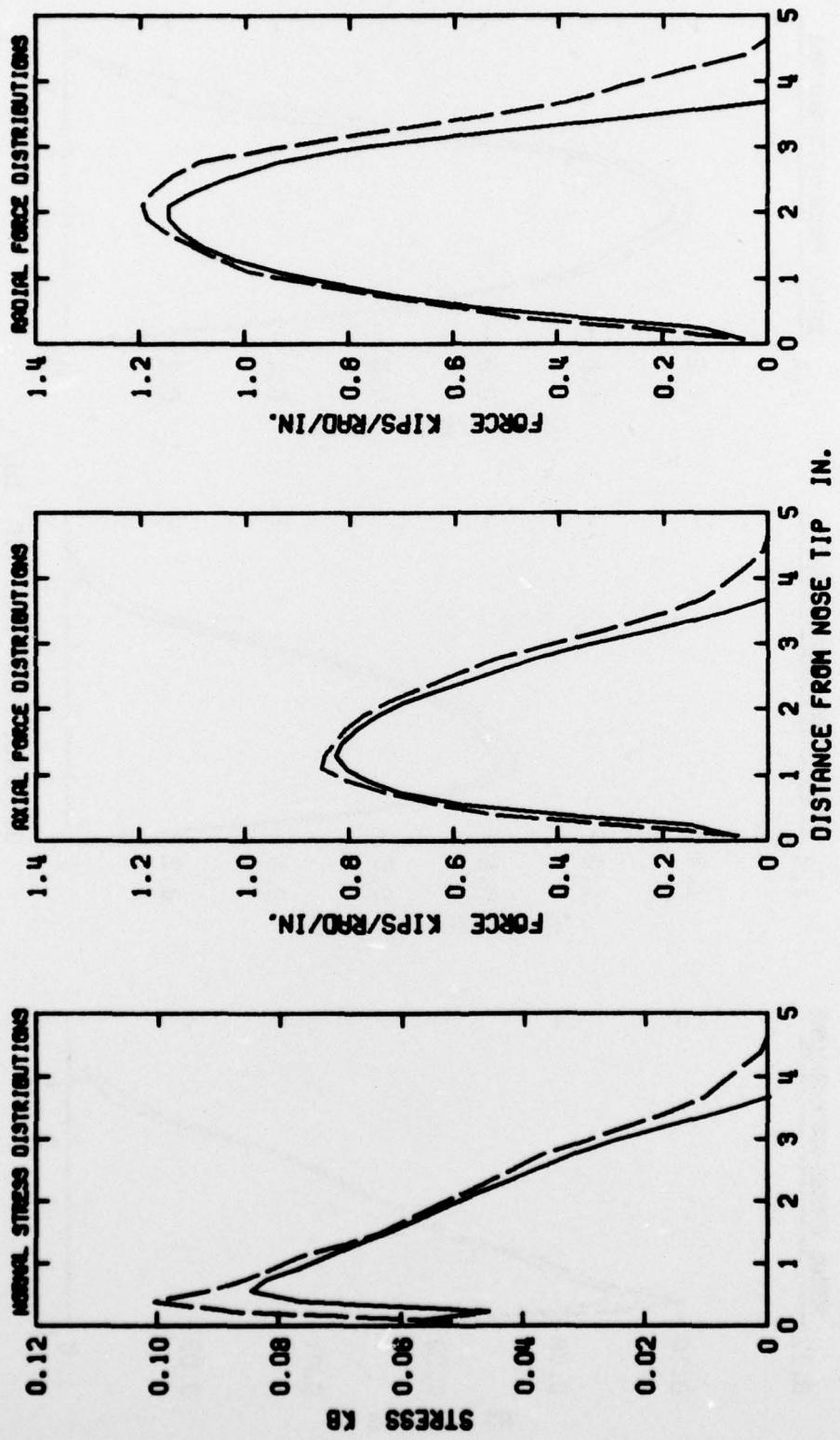


Figure 25. Penetrator Loading Distributions at 8 in. Penetration Depth.
 — Case 1 - - - Case 2



— Case 1 (penetration depth = 3.85 in.)
 - - Case 2 (penetration depth = 8 in.)

Figure 26. Penetrator Loading Distributions for Penetrator Velocity of 197 ft/sec.

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1. Earth penetrators. 2. Finite difference method. 3. Numerical analysis. 4. Penetration. 5. Projectile penetration.
6. Soil penetration. I. Fulton, Christopher C., joint author. II. California Research and Technology, Inc. III. United States. Army. Corps of Engineers. IV. Series: United States. Waterways Experiment Station, Vicksburg, Miss. Miscellaneous paper ; S-77-23.

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