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TERRAIN CHARACTERISTICS AT GATOR MINE IMPACT AND PENETRATION TEST SITES ABERDEEN PROVING GROUND, MARYLAND

Ьу

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September 1977 Final Report

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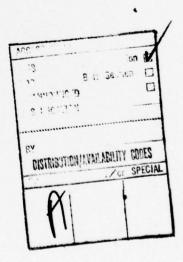
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to provide a baseline for theoretical extrapolation of the Gator Mine performance to other world environments. The results of this study are summarized as follows:

- a. The variation in soil consistency with depth and location within the site provided a wide range of soil conditions for evaluating mine performance but did not include locations exhibiting soil strengths that would result in marginal vehicle performance.
- b. The average 0- to 12-in. cone index (CI) ranged from 82- to 626+ with most of the readings occurring in the 200-400 range.
- c. The average 0- to 12-in. dynamic cone penetrometer index (DCPI) ranged from 5 to 57 with most of the readings in the 10-30 range.
- d. A wide range of values for horizontal displacement from impact point, penetration, impact angle, at-rest angle, and depth of overburden occurred during testing.
- e. Based on soil strength conditions, the I Field site is analagous to 83 percent of the Fulda Gap area in West Germany.
 - f. Standard vehicles could operate with ease in the test area.

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Preface

The U. S. Army Engineer Waterways Experiment Station (WES) participation in this phase of the Gator Mine System coordinated test program as requested by Picatinny Arsenal in a telephone conversation on 18 October 1976. Funds were provided by U. S. Army Armament Research and Development Command (ARRADCOM) on MIPR No. 7311-0031 dated 18 April 1977. The study was conducted during the period 11-15 April 1977.

The study was conducted under the general supervision of Mr. W. G. Shockley, Chief, Mobility and Environmental Systems Laboratory (MESL), and under the direct supervision of Mr. A. A. Rula, Chief, Mobility Systems Division (MSD), MESL. The U. S. Army Aberdeen Proving Ground, Maryland, supported WES in the collection of field data. The field program was conducted by Messrs. C. E. Green, L. M. Lewis, and D. E. Strong, MSD. This report was prepared by Mr. Green.

Commander and Director of WES during this study and the preparation of this report was COL J. L. Cannon, CE. Mr. F. R. Brown was Technical Director.

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Conversion Factors, Metric (SI) to U. S. Customary and U. S. Customary to Metric (SI) Units of Measurement

Units of measurement used in this report can be converted as follows:

Multiply	Ву	To Obtain
	Metric (SI) to U. S. Customary	
millimetres	0.03937007	inches
centimetres	0.3937007	inches
grams per cubic centimetre	0.0361273	pounds (mass) per cubic inch
	U. S. Customary to Metric (SI)	
inches	2.54	centimetres
feet	0.3048	metres
square inches	6.4516	square centimetres
pounds (mass)	0.4535924	kilograms
pounds (force) per square inch	6.894757	kilopascals
degrees (angle)	0.01745329	radians

TERRAIN CHARACTERISTICS AT GATOR MINE IMPACT AND PENETRATION TEST SITES, ABERDEEN PROVING GROUND, MARYLAND

Background

1. A series of impact and penetration tests were conducted to evaluate the performance of the Gator mine. The tests were conducted by Picatinny Arsenal at I Field, Aberdeen Proving Ground (APG), Maryland, during the period 11-15 April 1977 to provide data to compare the penetration performance of three types of Gator mines: Air Force, S-shaped, and 110 (Figure 1). The U. S. Army Engineer Waterways Experiment Station (WES) had collected field data in support of similar impact and penetration tests for another munitions system, the Remote Anti-Armor Mine Systems (RAAMS) mine, in July 1976 at APG. Data from these tests were useful in providing a general description of the test areas to relate penetration performance to soil strength. However, because soil strength changes as a function of moisture content, these data did not provide sufficient information to document conditions for the Gator mine tests. Therefore, Picatinny Arsenal requested that WES take additional terrain data in support of the Gator mine tests.

Purpose and Scope

- 2. The purpose of this study was to document terrain characteristics at the sites at APG used in the impact and penetration tests of the Gator mine. The terrain characteristics were needed to develop mine penetrability relations and to provide a baseline for theoretical extrapolation of the Gator mine performance to other environments.
- 3. The following types of data were collected to describe site terrain properties for these purposes, as described in Reference 2.

- a. Soil strength in terms of cone index (CI) and/or rating cone index (RCI) at selected locations for the 0- to 12-in.* layer.
- $\underline{\mathbf{b}}$. Soil strength in terms of dynamic cone penetration index (DCPI).
- c. Description of vegetation or ground cover.
- d. Soil moisture content at selected locations.
- e. Soil descriptions according to the Unified Soil Classification System (USCS) and U. S. Department of Agriculture (USDA) plus specific gravity and organic content.
- f. Soil density.

In addition, a general description was made of each site, and photographs were taken where appropriate. The depth of penetration and orientation of the mine were documented for approximately 80 mine tests. Soil trafficability for standard military vehicles within the test area was estimated. A comparison of I Field to the Fulda Gap area in West Germany was made.

Definitions

4. Special terms used in this report, taken from Reference 1, are defined below:

a. Soil terms

- (1) Fine-grained soil. A soil of which more than 50 percent of the grains, by weight, will pass a No. 200 sieve (smaller than 0.074 mm in diameter).
- (2) Fines. Grain sizes that will pass the No. 200 sieve (smaller than 0.074 mm in diameter).
- (3) Coarse-grained soil. A soil of which more than 50 percent of the grains, by weight, will be retained on a No. 200 sieve (larger than 0.074 mm in diameter).
- (4) <u>Sand.</u> A coarse-grained soil with the greater percentage of the coarse fraction (larger than 0.074 mm) passing the No. 4 sieve (4.76 mm).
- (5) Sand with fines, poorly drained. A sand that contains some fines and is slow-draining when wet. Such sands

^{*} A table of factors for converting metric (SI) units to U. S. customary units and U. S. customary units to metric (SI) units of measurement is presented on page 4.

behave similarly to wet, fine-grained soils under vehicular traffic.

- (6) Liquid limit. The liquid limit is generally conceded to represent the moisture content at which the characteristics of a mixture of soil and water change from plastic to liquid.
- (7) Plastic limit. The plastic limit is generally conceded to represent the moisture content at which a mixture of soil and water begins to take on plastic properties (i.e. undergoes appreciable deformation with little change in volume).
- (8) Plasticity index. The numerical difference between the liquid and plastic limits. The numerical value of the plasticity index is generally a good indication of the plasticity or clayeyness of a soil: highly plastic clays generally have high plasticity indexes; less plastic clays have lower plasticity indexes.
- (9) Moisture content. The ratio, expressed as a percentage, of the weight of water in the soil to the weight of the solid particles.
- (10) <u>Density.</u> The unit weight in pounds per cubic foot. Unless specifically stated otherwise, the density is the dry unit weight.

b. Strength terms

- (1) Cone index (CI). An index of the shearing resistance of soil obtained with the cone penetrometer. The CI is considered to be a dimensionless number representing the resistance of a medium to penetration of a 30-deg, right-circular cone of 0.05-sq-in. base area. The number, although considered dimensionless, is actually the number of pounds of force exerted on the handle divided by the area of the cone base in square inches.
- (2) Remolding index (RI). A ratio that expresses the change in strength of a fine-grained soil or a sand with fines, poorly drained, that may occur under traffic of a vehicle.
- (3) Rating cone index (RCI). The product of the measured CI and RI for the same layer of soil. This index is valid only for fine-grained soils and for sands with fines, poorly drained.
- (4) Dynamic cone penetration index (DCPI). An index of the shearing resistance of soil obtained with a drop cone penetrometer. The index is the number of blows of a 12.3-1b hammer dropped 8 in. required to drive a

30-deg, right-circular cone of 0.5-sq-in. base area a given distance into a medium.

Vehicle cone index (VCI). An index assigned to a given vehicle, based on certain vehicle characteristics, that indicates that minimum soil strength in terms of RCI (for fine-grained soils) or CI (for coarse-grained soils) required for a prescribed number of passes. VCI for coarse-grained soils is distinguished from VCI for fine-grained soils by the addition of an S for coarse-grained soils. Furthermore, numbers are used to distinguish the VCI's for one tire inflation pressure from another; e.g., VCIS-35 and VCIS-72.5 are the VCI's for 35- and 72.5-psi tire inflation pressures, respectively, on sand. VCIS applied to one pass only. VCI for fine-grained soils is identified for a given number of passes by a subscript, usually VCI₁ or VCI₅₀ for 1 or 50 passes, respectively.

Site Description

- 5. Picatinny Arsenal selected I Field at APG for characterization and mine tests. The location, topography, vegetation, and soil moisture conditions are described in the following paragraphs.
- 6. I Field is on Gunpowder Neck Peninsula. The field is bounded on the west by Ricketts Point Road, on the east by Chesapeake Bay, and on the north and south by woodlands. A rectangle approximately 1600 by 2200 ft, with the length oriented in an east-west direction, was chosen for characterization. The topography of I Field ranges from an upland flat near the western boundary to a bottomland depression beginning in the north-central section and extending in a south-southeast direction through the site near its eastern boundary. Site relief ranges up to 50 ft, and slopes range up to about 5 percent. At the time the site was sampled (11-15 April 1977), the area was bare and there was considerable evidence of erosion on the slopes throughout the test area (Figure 2). Prior to testing, no specific site preparation was required; however, during RAAMS tests in July 1976 at I Field, 1 various sections were disked or rototilled (up to a depth of 10 in. in places) to simulate agricultural lands.
- 7. The soil³ at this site, which was derived from Coastal Plain deposits, is deep and is medium to light in texture. In the upper

topographic layer, the soil is brownish yellow and well drained. In places, the subsoil is mottled gray, and large patches of gray subsoil were exposed along the eastern-facing slope. According to USDA, the upland soil is classified as Sassafras loam, and the bottomland as Keyport silt loam. The subsoil of the Keyport series is mottled gray, which is evidence of imperfect internal drainage. The soil in the 0- to 12-in. depth varied from loam to silty clay loam. Classification according to the USCS varied from sandy clay (CL) to sandy silt (ML).

Data Collected and Procedures Used

8. The data collected at I Field included the strength, density, moisture content, classification, specific gravity, and organic content of the surface soil for site characterization and additional postimpact data on the Gator mine (Tables 1-3). The data collected and the procedures used are described in the following paragraphs. The procedures used to characterize the sites for this study were generally in accordance with those established by WES for soil trafficability studies and those outlined in References 1 and 2.

Soils data

9. Prior to the actual data collection in the field, an inspection was made of the soil conditions on I Field. Obvious changes in soil moisture content were noted so that measurements could be made later at these sites. The initial inspection did not reveal any significant changes in soil type at I Field; however, to document this uniformity, several of the sites used for moisture content determination were also sampled for soil type data. Bulk soil samples were taken at selected sites from the 0- to 6- and 6- to 12-in. layers for laboratory determination of organic matter content, grain-size distribution, Atterberg limits, and specific gravity; organic matter content was determined by means of a modified Walkey rapid dichromate oxidation and the values expressed as percentages by weight. Soil was classified according to the USCS and USDA systems. A 2-in.-diam trafficability sampler was used to obtain moisture content-density samples in the 0- to 6- and 6- to

12-in. soil layers. When the soil was too firm to allow penetration in 3-in. vertical increments with the trafficability sampler, a disturbed soil sample was taken from the prescribed depths for determination of moisture content. Laboratory-determined data are listed in Table 1.

10. Soil strength as a function of depth was measured in terms of CI and DCPI. These data (Table 2) were taken near the intersections of the various grid lines shown in Figure 3. Figure 3 shows the distribution of soil strength throughout the test area. This map was developed by assuming that the average soil strength for the 0- to 12-in. layer varied linearly between grid intersection points. Although this assumption is often incorrect, the map does indicate average soil strengths to be expected throughout the test site. At each location sampled, three sets of CI measurements were made at the surface and at 1-in. vertical increments to a depth of 6 in., then at 3-in. increments to 12 in. A plus sign was used to indicate that the soil strength exceeded the capacity of the cone penetrometer. (If the average CI was determined using one or more readings that exceeded the capacity of the penetrometer, a plus sign was placed after the average value in Table 2.) At each sample location, one set of DCPI measurements was made at 6-in. vertical increments to a depth of 12 in.

Site data at mine impact locations

11. Because the site characterization indicated a variation in soil consistency with depth and location within the site, it was necessary to obtain soil strength data adjacent to mine impact locations. These data are listed in Table 3. At each impact location, three sets of CI measurements were made at each point of an equilateral triangle whose sides were about 3 ft long. One set of DCPI measurements was made near the center of the triangular sampling pattern.

Mine impact data

12. Measurements were made at each mine location to determine
(a) initial and final penetration depths, (b) horizontal displacement
from impact point, (c) impact angle and attitude, (d) final attitude,
(e) at-rest angle, and (f) overburden (Figure 4). Standard survey
techniques and instruments were used to take these measurements.

Discussion of Results

13. The data collected at APG were not analyzed rigorously; however, some observations were made. These are discussed in the following paragraphs.

Soils data

- 14. Site characterization. The site was free of surface vegetation and the soil was generally classified as a lean clay (CL). The average 0- to 12-in. CI (Table 2) ranged from 82- to 626+ with most of the readings occurring in the 200-400 range. CI was assumed to be equal to RCI because the soil gained strength with compaction. The average 0- to 12-in. DCPI (Table 2) ranged from 5 to 57, with most of the readings in the 10-30 range. Figure 3 shows that several high-strength areas (based on CI) occurred near the B-1, C-3, E-2, and E-5 intersections, and low-strength areas occurred near the B-6, B-10, B-11, and B-12 intersections.
- 15. Mine impact locations. The average 0- to 12-in. CI (Table 3) ranged from 58 to 672 with most of the readings occurring in the 200-400 range. The number of blows required to penetrate the surface foot ranged from 3 to 49; most of the readings were in the 10-30 range. Mine performance
- 16. The measured mine data for the twenty-seven Air Force, twenty-nine S-shaped, and twenty-four 110 mines are presented in Table 4 (8 of the 88 mines dropped were lost and could not be documented). As can be seen in Table 4, a majority of the mines displaced horizontally after impact. The distance displaced varied from 8 in. to a maximum of 324 in. (mine 38). The initial and final penetration of the mines varied from 0 in. to a maximum of 9.5 in. The impact angle and at-rest angle varied from 0 to 90 deg. Overburden (i.e. the amount of material atop the mine after impact) in most cases was 0 in.; however, in three tests overburden depths were 1.75, 2.5, and 5.0 in. for mines 33, 18, and 61, respectively.
- 17. Plots of initial depth of penetration versus average CI and versus DCPI for the 0- to 12-in. layer are shown in Figures 5 and 6, respectively, for the Air Force mine, in Figures 7 and 8, respectively,

for the S-shaped mine, and in Figures 9 and 10, respectively, for the 110 mine. The 0- to 12-in. layer was assumed to be the critical layer for this study. These figures indicate the scatter pattern for the various soil strengths encountered and show that the mines do not penetrate uniformly. This scatter pattern could be due to the effects of various glide patterns and impact velocities at which the mines hit the ground. A much better correlation could possibly be determined if the previously mentioned variables were considered.

Comparison of I Field to other environments

- 18. Terrain data similar to that for I Field has been developed in a previous study at WES (HIMO Study) for the Fulda Gap area in West Germany. Previous data taken at I Field (Reference 1) were extrapolated to that area. The procedure used for extrapolation is presented in detail in Reference 1. Generally, the procedure consists of (a) assigning soil strength in terms of CI and RCI to mapped terrain units (soil type-drainage situation) on the basis of predictions made with the WES soil moisture and soil strength predictions (SMSP) model; (b) using a cumulative areal occupancy of RCI curve to compute weighted averages that imply areal occupancy to be expected for selected RCI values; and (c) computing the expected areal occupancy for a data base year.
- 19. By using the procedure presented in Reference 1, it was determined that an RCI >100 applied to 83 percent of the Fulda Gap area in West Germany. The 0- to 12-in. average CI (CI assumed equal to RCI) for the 80 impact sites (Table 3) at I Field was greater than 100 for all the sites except No. 87. This indicates that the I Field site is similar in terms of soil strength to 83 percent of the Fulda Gap area in West Germany.

Estimation of vehicle performance

20. The Gator mine system includes both an antivehicle and an antipersonnel mine. Therefore, to evaluate the mine performances in a given environment such as I Field, an estimation of vehicle performance for the same environment should be made. If the vehicles in question could not operate in the environment, the mine would not be required.

- 21. Therefore, to provide a guide for the Gator mine performance documented during the APG tests in terms of vehicle performance, the WES VCI method of computing minimum soil strength requirements for ground-crawling vehicles to complete a prescribed number of passes in a straight-line path was used to estimate the soil trafficability of standard military vehicles. The minimum CI requirements for various standard military vehicles are shown in Table 5. It must be noted that the values shown in Table 5 are somewhat generalized and that each vehicle has a specific CI requirement for a given number of passes. For example, the minimum soil strengths in terms of RCI required for an M60Al tank to complete one pass (VCI₁) and 50 passes (VCI₅₀) are 20 and 48, respectively. If the soil strength (RCI) exceeds these numbers, the vehicle can accelerate, climb a slope, or tow a load.
- 22. An examination of Table 2 shows that the average soil strength data for the 6- to 12-in. layer within I Field (with the exception of one site) was greater than the soil strength requirements for the standard military vehicles shown in Table 5. This indicates that the standard military vehicles can make 50 passes at almost any location within the area.

Summary of Results

- 23. The results of this study are summarized as follows:
 - a. The variation in soil consistency with depth and location within the site provided a wide range of soil conditions for evaluating mine performance but did not include locations exhibiting soil strengths that would result in marginal vehicle performance.
 - b. The average 0- to 12-in. CI ranged from 82- to 626+ with most of the readings occurring in the 200-400 range.
 - <u>c</u>. The average 0- to 12-in. DCPI ranged from 5 to 57 with most of the readings in the 10-30 range.
 - d. A wide range of values for horizontal displacement from impact point, penetration, impact angle, at-rest angle, and depth of overburden occurred during testing.
 - e. Based on soil strength conditions, the I Field site is analogous to 83 percent of the Fulda Gap area in West Germany.
 - <u>f</u>. Standard military vehicles could operate with ease in the test area.

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Site Characterization, I Field, APG, Maryland Table 1

									Soil C	Classification	uc			
							USDA	A				SCS		
Site	Depth	Moisture Content	Density	Organic	Specific	Gré	Grain Size by Weight. %	Å.	Soil	Percent				Soil
Location*	in.	54	8/cm3	24	Gravity	Sand	Silt	Clay	Type	<0.074	킈	ᆲ	II.	Typesty
A-2	0-1	13.8	1	1	•	•	•		B	1	1.		ı	1
	9-0	13.9	1.76	1	•	1	1	1	1		1	,		1
	6-12	17.0		•		•				•	1	1		
A-12	0-1	19.3				1						ı		
	9-0	21.4	1.82	•		•	1	•					•	
	6-12	21.9	•	•		•	•	1		•			•	1
3	1-0	11.4									•			
	9-0	15.3	1.64	1.6	2.69	35	77	21	Loam	55	53	19	10	CL
	6-12	16.1	•	1.6	2.70	29	54	17	Silt,	55	53	19	10	t t
									loam					
6-7	1-0	7.5									,			
-	100							1			1		1	
	9-0	15.9	1.95				•	1	1		1	1		
	6-12	16.7		•										
D-5	0-1	8.2	,	•	•		•	•		,		,	,	
	9-0	10.2	1.97	1.2	2.68	36	51	13	Silt,	20	21	17	4	CL
									loam					
	6-12	11.1	•	•	•	20	51	29	Silty	09	54	19	2	CL-ML
									clay					
									loam					
01-0	1-0	16.6												
2-10	1.0	0.01												
	9-0	19.1	1.77	4.4	2.62	38	43	19	Loam	87	56	=	15	CL
	6-12	19.3		4.1		32	20	18	Silt,	55	53	23	9	C,
					(Continued)	(per			loam					
						,								

NOTE: Tests were run on 12 April 1977. LL = liquid limit; PL = plastic limit; PI = plasticity index. * Alphanumeric system used to identify grid squares is shown in Figure 3. ** CL = lean clay; ML = silt.

Table 1 (Concluded)

11	=	Type						.1											_	ں			,			
	S	A		•			75	占	•	•						•			5	占		•				
		PI	•		•	•	#	13	•	1	1	•	•	1	•	1	•	•	œ	10	•	1	1	•	1	•
USCS		띪	1		•	•	23	14	1	•	1	•	1	•	•	•	ı	•	14	21	•	1	•	•	1	•
uo		킈	1		•	•	34	27	1	•	1	•	1	•	•	•	•		22	31	•	•	•	•	•	•
Soil Classification	Percent	<0.074					99	89		•		•		ı	•		•	•	65	89		•		•	•	
Soil C	Sof1	Type	•	•			Silt,	Silt,	-		•		1		•				Silt,	Silt, loam		•	•			
A	, A	Clay	•			•	17	16	•	•	1	,			•	•	1		15	27		•	•	•		1
USDA	Grain Size by Weight. %	Silt		•			65	62		•		,							19	62					1	
	Gra	Sand		1			24	22		•			1						54	=	,					•
	Specific	Gravity					2.67	2.67		•					•			•	2.65	2.66				1		
	Organic	*	•	•	•	•	2.5	2.1		•						,		•	2.6	2.1						
	Density	g/cm ³	1	1.93			1.94		1	1.80	1		1.75			1.84	,		1.87	•	•	1.94			1.75	ı
	Moisture	84	11.2	13.3	13.1	14.9	14.4	14.5	15.8	17.2	17.5	24.6	20.8	22.1	13.2	14.2	14.7	13.3	15.8	15.1	10.8	18.3	18.9	16.5	17.4	17.7
	Depth	in.	0-1	9-0	6-12	0-1	9-0	6-12	0-1	9-0	6-12	0-1	9-0	6-12	0-1	9-0	6-12	0-1	9-0	6-12	0-1	9-0	6-12	0-1	9-0	6-12
	Site	Location	E-8			E-7			F-3			6-7			6-11			H-4			H-7			1-1		

Table 2 Soil Strength Data at Site Locations, I Field, APG, Maryland

-				-					-	1	- 1			-	-
										AT	Average (CI			
Site			Aver	Average CI	at Depth,		in.			at	Layer,	in.	DCPI	at Layer	r, in.
Location*	SFC	-1	2	3	4	2	9	6	12	9-0	6-12	0-12	9-0	6-12	0-12
A-1	80		140		250	250	260	300	220	183	260	222	9	6	15
A-2	09		130		240	300	310	300	310	184	307	246	2	8	13
A-3	120		200		240	270	250	300	300	196	293	240	6	6	18
A-4	09		100		220	250	400	200	510	183	487	335	7	13	20
A-5	30		20		80	06	100	250	360	99	237	152	2	2	7
A-6	70		70		20	20	80	100	210	74	130	102	3	4	7
A-7	09		09		100	120	110	400	650	80	387	234	2	17	19
A-8	180		220		170	190	150	150	170	190	157	174	9	7	13
6-A	40		10		30	09	80	110	170	43	120	82	1	4	2
A-10	09		80		80	06	80	150	230	92	153	114	4	S	6
A-11	20		40		70	100	09	120	140	67	107	87	23	9	6
A-12	45		20		20	80	06	110	170	62	123	92	3	4	7
B-1	70		350		750+	750+	750+	750+	750+	200	750+	625+	19	38	57
B-2	20		150		210	350	370	350	350	210	357	384	7	10	17
B-3	120		240		240	300	320	250	200	241	357	299	8	6	17
B-4	40		70		120	120	250	270	300	109	273	191	23	9	6
B-5	20		40		09	80	06	130	400	53	207	130	3	16	19
B-6	20		40		20	20	80	120	130	53	110	82	7	3	2
B-7	40		70		06	140	230	320	420	101	323	212	3	8	11
B-8	09		140		250	380	490	5.50	450	229	497	363	9	13	19
B-9	20		100		120	140	230	230	230	123	230	176	4	∞	12
B-10	70		70		110	140	150	200	350	26	233	165	23	7	10
B-11	09		100		120	130	130	210	280	96	207	152	2	7	6
B-12	20		06		130	110	140	200	290	100	210	155	4	2	6
							(Cont	(Continued)							

(Sheet 1 of 5) NOTE: Plus sign indicates that the capacity of the cone penetrometer was exceeded. * See Figure 3 for site locations. Average CI

(Sheet 3 of 5)

	or, 111.	71-0	15	52	17	23	42	15	23	11	13	12	10	6	10	20	2 2	17	2	10	11	23	13	12	12	13	
1	at Layer,	0-17	13	33	11	17	31	12	16	7	6	6	8	7	œ	14	= =	12	18	7	∞	18	6	10	8	6	
TODE	DCFI	0	2	19	9	9	11	3	7	4	4	3	2	7	~	ی ا	7	2	7	3	3	2	4	7	4	4	
_ ·	In.	0-17	260	626+	267	256	563+	322	388	145	193	246	156	160	192	308	336	270	338	178	189	292	157	242	202	202	
Average C		0-17	450	750+	337	423	750+	467	547	220	300	397	233	237	310	573	457	417	200	247	297	457	190	347	283	283	
Av	at	0-1	71	503	197	06	376	176	228	70	98	96	79	83	75	200	216	124	176	81	81	127	123	136	121	120	
	12	77	750	750+	400	570	750+	200	650	240	430	009	380	360	450	550	540	520	200	310	420	620	150	420	380	370	
	0	n	400	750+	350	470	750+	450	009	320	340	460	210	230	350	550	430	450	550	240	320	450	170	450	280	300	
i			200	750+	260	230	750+	450	390	100	130	130	110	120	130	620	400	280	450	190	150	300	250	170	190	180	
		0	70	750+	230	06	750+	350	350	06	100	120	06	96	115	350	310	140	310	110	150	115	130	180	170	160	
at Donth	at De	+	09	750+	260	100	470	120	380	80	110	110	100	110	70	140	260	130	230	80	20	130	130	170	150	160	
T) one	age of	6 1	55	750+	290	06	290	110	300	20	06	100	80	06	09	140	240	120	100	65	20	110	110	150	120	130	
Amor	Aver	1	20	400	190	70	250	06	130	09	70	06	80	80	65	120	150	100	70	20	09	110	06	125	100	80	
	-	-	40	80	100	40	80	82	40	09	20	80	20	09	45	120	100	80	20	40	20	75	100	110	80	96	
	CEC	or c	20	40	20	10	40	30	S	30	30	40	25	30	40	70	20	20	20	30	20	20	20	40	40	40	
	Site	rocarion	E-1	F-2	E-3	E-4	E-5	E-6	E-7	E-8	E-9	E-10	E-11	E-12	F-1	F-2	F-3	F-4	F-5	F-6	F-7	F-8	F-9	F-10	F-11	F-12	

(Sheet 4 of 5)

	r, in.	0-12	1	,	•	16	1	•	1	19			1	•	15	11	16	10	18	22	12	6	15	16	15	18
	at Layer,	6-12	,	1		11	1	1	1	13	•	1	•	•	∞	6	11	7	12	13	6	8	6	12	11	13
	DCPI	9-0	•	1	1	2	1	•	ı	9	'	1	1	1	7	2	2	3	9	6	ы	1	9	4	4	2
	CI in.	0-12	448	385	272	251	196	216	210	143	247	130	134	135	128	132	190	112	210	334	276	121	241	192	165	178
	erage Layer,	6-12	583	540	330	310	237	263	333	210	330	173	177	180	190	203	250	153	283	403	433	190	340	277	227	250
	Av	9-0	312	230	213	191	155	169	98	92	164	98	61	06	99	09	130	71	137	264	119	51	141	107	103	106
(per		12	750	720	300	280	260	250	550	300	270	170	200	210	260	300	260	220	350	380	750	360	430	300	340	360
Contin		6	530	200	330	300	250	300	300	230	320	220	170	180	210	200	220	130	320	300	300	150	430	370	190	210
Table 2 (Continued)	in.	9	470	400	360	350	200	240	150	100	400	130	160	150	100	110	270	110	180	530	250	09	160	160	150	180
Tab	at Depths,	2	420	330	400	350	200	220	125	105	150	115	130	120	80	20	250	100	170	200	200	09	170	130	140	120
	1	4	450	270	310	290	190	230	110	110	160	100	110	100	90	20	115	80	150	400	115	20	170	120	100	110
	Average CI	2	400	250	210	130	200	250	90	80	150	85	06	100	80	20	115	80	150	160	120	20	160	100	06	06
	Aver	2	250	180	120	80	160	120	60	70	130	20	80	80	20	20	80	09	150	130	80	09	140	100	120	110
		-1	130	140	09	80	06	85	45	20	105	09	40	30	40	30	20	40	110	06	20	09	130	80	20	06
		SFC	65	40	30	09	45	40	20	30	20	40	30	20	20	20	30	30	20	40	20	20	09	09	20	46
	Site	Location	G-1	6-2	6-3	6-4	6-5	9-9	6-7	8-9	6-9	G-10	6-11	G-12	H-1	H-2	H-3	H-4	H-5	9-H	H-7	H-8	6-H	H-10	H-11	H-12

Table 2 (Concluded)

	., in.	0-12		1	1	1	23	,			•	1	56	•	1		32	27	6	6	56	28	19	10	18	25	23
	at Layer, in	6-12	1		•		19				•		20		•	1	23	23	9	2	20	21	14	7	10	16	15
		9-0		•	,	1	4	1		,	•	1	9	,		1	6	4	23	4	9	7	S	3	8	6	∞
	in.	0-12	1	264	487	260	194	184		97	224	166	380	294	967	1	320	260	116	104	228	281	222	104	191	226	220
Average C	Layer,	6-12	ı	410	750+	403	310	277		130	333	240	209	443	453	•	433	440	157	130	283	393	330	140	240	307	297
Ave	ш	9-0		117	224	118	79	91		64	116	92	157	145	138	1	207	80	9/	77	174	169	114	29	142	146	144
		12		530	750+	009	480	440		100	380	350	750+	610	009		450	270	150	150	150	550	200	130	200	340	300
		6	,	510	750+	370	350	260		180	370	190	750+	440	510		470	570	180	120	320	330	300	150	300	320	340
	in.	9	,	190	750+	240	100	130		110	250	180	310	280	250		380	180	140	120	380	300	190	140	220	260	250
	at Depths,	2		140	280	150	06	120		70	150	130	200	210	190	,	350	06	100	110	220	290	180	80	220	210	210
	at De	4		130	200	130	06	100	,	09	140	85	200	190	170		250	70	980	06	180	240	135	20	210	200	190
	age CI	2 3	,	120	130	100	80	100		09	110	80	150	130	140	•	180	20	80	9	170	160	06	20	150	140	160
	Aver	2		110	06	85	80	80	;	09	09	70	110	100	110	•	150	20	09	20	150	80	80	20	100	110	110
		-1	•	06	70	20	70	20		20	09	09	80	70	70	•	100	20	40	20	80	70	75	50	2.0	09	20
		SFC	1	40	20	20	40	35		35	40	40	20	35	40		40	20	30	09	40	40	20	20	30	40	40
	Site	Location	1-1	1-2	I-3	I-4	1-5	9-I		1-7	I-8	6-I	1-10	1-11	1-12	J-1	J-2	J-3	J-4	J-5	9-6	J-7	J-8	J-9	J-10	J-11	J-12

Table 3 Soil Strength Data at Mine Impact Locations, I Field, APG, Maryland

		0-12	19	1	56	35	30	18	30	1	29	16	20	24	34	8	1	38	12	∞	29	30	16	36	22	1	22	
DCPI at	r, in.	2	12	1	16	25	17	15	21	1	24	12	18	14	25	9	1	31	6	9	20	24	10	18	13	1	19	
DC	Layer,	9 9-0	7	1	10	10	13	3	6	1	2	4					1						9	18	6	1	3	
at		0-12 0	284	,			465	210	290	1	347	272										318	291	528	372	1	309	
CI	r, in.	2	378	1	999	572	287		644													984						
Average	Layer,	9 9-0			270																	150					121	
		12	510	ı	677	630	572	433	717	1	550	400	750	373	049	337	ı	750	247	360	209	249	387	750	683	1	740	
		6	327	1	265	630	623	343	407	1	583	487	113	330	503	153	ı	557	187	207	750	627	453	750	260	1	260	(peni
	in.	9	297	1	423	457	267	197	223	1	413	327	62	297	740	77	1	105	167	107	407	183	367	750	417	1	190	(Continued)
	Depth,	5	260	1	410	377	460	118	193	1	217	243	32	277	433	67	1	86	92	113	357	167	337	533	320	1	160	
	CI at]	4	233	1	417	250	457	100	183	1	173	168	27	280	363	09	ı	103	47	107	270	173	150	303	187	1	127	
	Average	3	207	1	353	163	397	87	150	1	143	90	33	270	243	55	1	95	20	53	153	170	110	187	172	1	110	
		2	173				257						38					72					107			1		
		1	137	1	70	93	160	57	52	1	113	09	28	213	147	40	1	65	48	99	63	130	100	137	80	1	93	
		SFC																										
	ine	No. Type	AF	110*	S	AF	110	S	AF	110*	S	AF	110	S	AF	110	×S	AF	AF	AF	AF	AF	110	110	110	110*	110	
	M	No.	1	2	3	4	2	9	7	8	6	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	

* Mine lost.

* Mine lost.

(Sheet 2 of 4)

** Mine hit pavement behind shop.

1		2	~	6	7	7	0	\ +	m	7	0	\	6	10	7	6	1	7	1		8	2	10	8	10	0	6	
	in.	0-12	28	3	2	7	5	14	18	5	7	24		2	1	7	31	1	2	1	1	7	ī	5	2	5	ī	
1 10	Layer, i	1641	20	28	22	15	12	80	10	18	15	16	4	18	15	32	56	14	20	ı	15	80	11	19	12	12	10	
	Lê	9-0	80	11	2	12	00	9	80	6	2	∞	2	7	2	17	5	3	7	1	3	7	4	6	13	80	6	
at		0-12	357	408	336	313	265	172	255	767	288	292	109	295	275	378	468	328	388	1	183	185	227	273	797	315	248	
	Layer, in.	6-12	473	969	536	455	369	237	348	999	904	383	103	452	777	450	799	471	280	ı	263	222	302	416	335	301	325	
Ave	La	9-0	242	220	137	172	162	108	163	322	169	201	114	138	105	305	271	184	195	1	103	148	153	129	194	229	191	
		12	417	249	200	247	443	317	420	670	623	343	73	199	510	200	750	456	750	1	336	266	473	949	406	436	346	
		6	528	650	570	420	390	233	333	707	323	427	130	450	490	460	750	370	750	1	290	186	166	430	323	410	323	(panu
	in.	9	473	490	337	397	273	160	290	620	273	380	107	240	333	390	493	587	240	1	163	213	266	273	276	356	306	(Continued)
	at Depth,	2	393	280	127	300	233	180	237	557	253	257	113	150	113	337	367	173	223	1	130	200	183	233	220	300	236	
		4	313	213	93	300	213	163	157	370	213	203	133	150	90	370	257	155	235	1	111	186	173	160	196	283	223	
	Average CI	[3]	267	187	90	83	190	83	147	313	163	177	117	150	11	350	223	137	210	1	143	160	166	43	203	266	213	
	Av	2	157	157	127	63	130	77	133	223	123	150	130	133	20	340	223	113	210	1	93	133	130	94	193	213	166	
		-	73	137	120	47	67	53	140	123	113	137	113	103	40	253	220	80	170	1	26	100	103	96	193	133	120	
		SFC	20	73	63	15	27	37	37	47	43	100	87	40	30	97	117	43	9/	ı	23	94	20	53	80	20	73	
	Mine	Type	110	110	110	110	110	s	S	S	S	co	AF	AF	AF	AF	AF	110	110	110*	110	110	S	S	S	S	S	
	Mi	No.	51	52	53	54	22	99	57	28	59	09	61	62	63	49	65	99	19	89	69	70	71	72	73	14	75	

* Mine lost.

	0-12	18	19	38	47	39	13	18	20	59	19	35	3	1
OCPI at ayer, in	6-12	11	10	56	21	56	10	8	15	22	11	25	1	1
DCPI	9-0	7	6	12	56	13	3	10	2	7	8	10	2	1
at.	0-12	360	356	767	672	327	220	282	268	372	332	346	28	•
Average CI Layer, in	6-12	484	977	949	750	462	313	334	388	571	484	502	29	1
Aver	9-0	235	265	344	594	192	127	230	148	173	180	189	84	1
	12	580	530	750	750	713	909	276	516	750	650	650	131	1
	6	526	396	623	750	300	310	366	423	290	516	530	30	•
in.	9	346	413	260	750	373	123	360	226	375	286	326	40	1
Depth,	5	320	333	260	750	313	116	230	506	303	260	250	43	1
CI at D	4	310	313	450	750	273	130	230	190	213	240	210	28	1
Average (3	276	303	383	750	176	146	246	163	126	196	186	70	1
Ave	2	200	250	356	633	110	123	230	120	93	120	173	43	•
	1	133	176	56	376	99	110	506	90	09	90	133	40	1
	SFC	63	20	73	153	33	140	110	40	43	99	43	40	1
ne	Type	S	S	S	S	S	110	110	110	110	110	AF	AF	AF*
Mi	No.	9/	17	78	79	80	81	82	83	84	85	98	87	88

Table 4

Mine Data at I Field, APG, Maryland

Mi	Mine		Horizontal Displacement After Impact	Penetration in.	tion	Impact	Impact	Final	At-Rest Angle	Depth of Overburden	Location Relative to
No.	Туре	Site*	fu.	Initial	Final	Attitude**	deg	Attitude**	deg	in.	Site Position
_	AF	E-3	0	ы	1.5	A	43	A	27.5	0	100 ft N, 40 ft W
+2	110	E-3	•	•	1	•		•			
	s	D-5	36	1.5	0	В	98	B	06	0	90 ft E
_	AF	D-5	15	2.5	0	ш	46	A	MN	0	
	110	6-A	36	7	0	A	38	U	7.5	0	40 ft N, 700 ft W
	s	F-10	0	3.5	3.5	A	06	A	59	0	25 ft N
_	AF	E-7	36	2.5	0	Q	89	A	S	0	60 ft N, 60 ft W
+	110	E-7	•	1	1					•	•
•	s	H-4	16	2.5	0	o	UK	A	UK	0	70 ft N, 80 ft E
-	AF	9-Q	0	S	s	A	69	o	23	0	100 ft N, 100 ft W
	110	4-9	0	4	4	A	41.5	D	32	0	60 ft S, 480 ft W
2	S	A-1	34	2.5	0	В	52	A	1	0	300 ft S .
2	AF	9-I	36	3.25	0	v	48	A	7	0	80 ft N, 40 ft E
-	110	H-7	0	4	4	ш	20	υ	22.5	0	20 ft N, 20 ft E
+5	s	H-7	•	•	•	1	ı		•	1	•
,0		9-9	0	4.25	4.25	A	46.5	A	12	0	40 ft S, 100 ft W
1		H-5	0	4	4	v	44	A	8.5	0	40 ft N, 60 ft W
18		6-7	0	9.5	9.5	Y	20	Α.	37	2.5	40 ft E
•	AF	E-6	0	2.75	2	A	47	A	∞	0	80 ft N, 20 ft E
0		E-8	36	3.5	0	A	33	A	10	0	100 ft N, 100 ft E
							(Continued)	(pen			

...

(Sheet 1 of 5)

NOTE: UK = unknown, NM = not measured.

* Alphanumeric system used to identify grid intersections shown in Figure 3.

** See Figure 4 for attitude definition.

† Lost.

																							1
	I ocetion Deletine to	Site Position	100 ft N, 100 ft E	40 ft E	60 ft S, 40 ft E		80 ft N, 100 ft W	30 ft N, 90 ft E	30 ft S, 20 ft W	80 ft N, 80 ft W	40 ft N	80 ft N	60 ft N, 80 ft W	20 ft S, 60 ft E	40 ft N	80 ft N	100 ft N, 20 ft E	500 ft S		Hit asphalt	40 ft N, 40 ft E	100 ft N, 20 ft W	
	Depth of	in.	0	0	0		0	0	0	0	0	0	0	0	1.75	0	0	0		0	0	0	
	At-Rest	deg	06	0	4.5		0	25	12.4	WN	WN	MN	76	38	20	21	11.5	9.5		06	24	15	
	Figure	Attitude	æ	М	A		В	υ	o	В	υ	A	ш	U	В	A	A	A		æ	υ	A	(Pa
	Impact	deg	87	32	20	•	42	51	24	98	MN	NM	92	38	NA	64	51	0	•	UK	24	15	(Continued)
	Tabact	Attitude	A	A	NM		ш	υ	υ	8	υ	A	ш	υ	æ	A	A	A		A	U	A	
	ation	Final	0	0	0	•	0	5.5	2.75	0	ъ	0	7	3.75	6.75	6.75	3.5	3.75	•	0	2.25	1.75	
	Penetration	Init	3	2.5	2		3.5	5.5	2.75	3	3	0	7	3.75	6.75	6.75	3.5	3.75		0	2.25	1.75	
iorizontal	isplacement	in.	6	40	7	•	115	0	0	37	0	12	0	0	0	0	0	0		324	0	0	
-	ÖĀ	Site	D-10	6-11	F-3	F-3	6-11	K-5	E-2	I-3	A-2	6-3	C-3	A-2	B-4	9-Q	9-8	A-1	A-1	A-1	A-12	C-10	
	Mine	Type	110	110	110	110	110	S	s	s	s	s	AF	AF	AF	AF	AF	110	110	110	110	110	
	M	No.	21	22	23	244	25	56	27	28	53	30	31	32	33	34	35	36	37+	38	39	40	

(Sheet 2 of 5)

+ Lost.

	O W	Displacement After Impact	Penetration in.	ation	Impact	Impact	Final	At-Rest	Depth of Overburden	Location Relative to
Type	Site	fu.	Initial	Final	Attitude	deg	Attitude	deg	in.	Site Position
	1-1	43	ю	0	v	44	A	s	0	40 ft 8, 40 ft B
	H-3	0	2.75	2.25	v	34.5	v	34.5	0	80 ft W
	H-5	30	1.25	0	A	45	A	4	0	100 ft N, 40 ft W
	D-10	19	2.75	0	υ	59	A	21.5	0	40 ft N, 40 ft W
	E-3	23	2.25	0	A	36	A	1	0	60 ft N, 60 ft E
	C-11	0	4	4	ш	61	ш	61	0	20 ft S, 60 ft W
	D-4	18	3.25	0	æ	45	Y	. 00	0	40 ft S, 80 ft W
	F-4	13	4.5	0	A	06	A	6	0	60 ft S, 60 ft E
	E-3	0	2.5	2.5	¥	UK	A	S	0	20 ft N, 80 ft E
AF	B-3	6	2.25	0	A	0	A	2	0	60 ft E
110	B-7	78	1.5	0	æ	MN	A	S	0	20 ft S, 60 ft E
110	D-10	38	1.75	0	A	UK	A	21	0	20 ft N, 20 ft W
110	D-12	0	4	4	υ	46	υ	46	0	60 ft S, 60 ft E
110	9-0	0	2.25	2.25	A	32	A	6.5	0	100 ft N, 60 ft E
_	D-5	142	2.25	0	¥	37	æ	7	0	40 ft N, 20 ft W
	H-4	34	2.5	0	v	31	A	7	0	60 ft S, 40 ft W
	6-3	0	7	7	A	06	A	10	0	60 ft E
	E-1	183	4	0	U	34	æ	0	0	100 ft N, 20 ft E
	F-1	0	3	3	U	38	A	18	0	200 ft S, 20 ft E
-	F-4	80	2.5	0	A	51	A	6	0	100 ft S, 100 ft E
						(Continued)	d)			

		Iloi	rizontal								
, i			placement	Penetration	ation	Twocot	Impact	Pfnnl	At-Rest	Depth of	
No. T	Type Si	15	After Impact in.	Initial	Final	Attitude	deg	Attitude	deg	overburden in.	Site Position
19	AF A	80	0	7.75	7.75	v	43	o	43	5.0	40 ft N, 40 ft E
62 ,	AF F	S	0	5.5	5.5	Q	49.5	Q	49.5	9	20 ft S, 100 ft E
63	AF B	9	80	s	0	æ	52.5	A	23	0	20 ft N, 80 ft E
64	AF F	80	Old Rut	2	0	¥	0	A	13	0	100 ft N, 40 ft E
65	AF E	7	0	3.75	3.75	ш	42	ш	42	0	20 ft N, 20 ft W
66 1	10 A	1	0	2.5	2.5	¥	79	¥	=	0	200 ft W
67 1	10 J	9	25	1.75	1.75	A	38.5	8	83	0	40 ft S
68† 1	10 J	9-	•	•	1	•				,	
69	10 G	-10	4	2	3	æ	47	В	75	0	80 ft N, 20 ft E
70 1	10 G	1-7	80	7	1	Э	34	υ	28	0	40 ft S, 40 ft E
11	s G	5-5	0	3.5	3.5	v	40	v	74	0	40 ft N
72	S H		9	2	0	A	56.5	A	0	0	120 ft S, 80 ft E
73	S	-5	34	2.25	0	o	25.5	A	13.5	0	40 ft S, 20 ft E
74	SE	1-3	4	1.25	0	A	28	A	15	0	20 ft N, 40 ft E
75	s G	1-1	0	2.75	2.75	v	19	υ	25	0	100 ft N, 20 ft E
92	s G	3-2	12	2.25	0	U	35	υ	12	0	40 ft S, 20 ft W
11	s D	4-0	6	8	0	J	34	A	6	0	60 ft S, 20 ft W
78	SB	3-4	73	7	0	Y	NM	A	00	0	40 ft S, 60 ft W
62	SB	1-1	140	1.75	0	v	NM	¥	06	0	100 ft N, 40 ft E
80	S	9-:	0	2.5	2.5	o	45	v	45	0	40 ft N, 80 ft W
							(Continued)	(pa			

(Sheet 4 of 5)

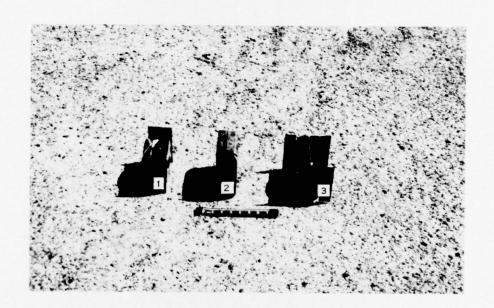
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34			Displacement	Penetra	tion		Impact		At-Rest		
2	THE			TH.	-	Impact	Angre	Fluar	Angre	Overburden	Location Relative to
No.	Type	Site		Initial	Final	Attitude	deg	Attitude	deg	in.	Site Position
81	110	I-10	81	2.25	0	A	35	A	14	0	60 ft W
82	110	G-10	180	7	0	¥	MM	A	NM	0	40 ft N, 80 ft E
83	110	B-7	0	2.5	2.5	A	59	A	31	0	40 ft S, 80 ft W
84	110	1-1	6	2.75	0	A	41	o	37	0	20 ft S, 20 ft W
85	110	D-7	44	3.5	0	ပ	31	A	0	0	80 ft E
98	AF	9-Y	0	7.5	7.5	æ	67	Q	29	0	40 ft S, 40 ft W
87	AF	8-A	0	6	6	æ	65	В	25	Ä	40 ft E
+88	AF	A-8	•	•		•		•		•	•

Table 5 CI Trafficability Data (from Reference 4)

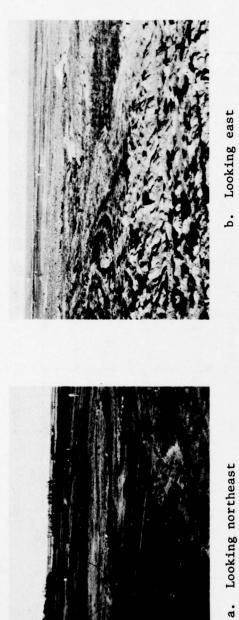
for for ses - to Standard Military Vehicles	M29 Weasel, M76 Otter, and Canadian Snowmobile are the only known military vehicles in this category	Engineer and high-speed tractors with comparatively wide tracks and low contact pressures	Tractors with average contact pressures, tanks with comparatively low contact pressures, and some trailed vehicles with very low contact pressures	Most medium tanks, tractors with high contact pressures, and all-wheel-drive trucks and trailed vehicles with low contact pressures	Most all-wheel-drive trucks, a great number of trailed vehicles, and heavy tanks	A great number of all-wheel-drive and rear-wheel-drive trucks, and trailed vehicles intended primarily for highway use	ater Rear-wheel-drive vehicles and others that generally are not expected to operate
Minimum CI Required for 50 Passes of the 6- to 12-in. Layer	20-29	30-49	50-59	69-09	70-79	8099	100 or greater



LEGEND

- 1 S-SHAPED
- 2 AIR FORCE
- 3 110

Figure 1. Mines tested



b. Looking east



d. Looking west

c. Looking southeast

Figure 2. Overviews of I Field (17 April 1977)

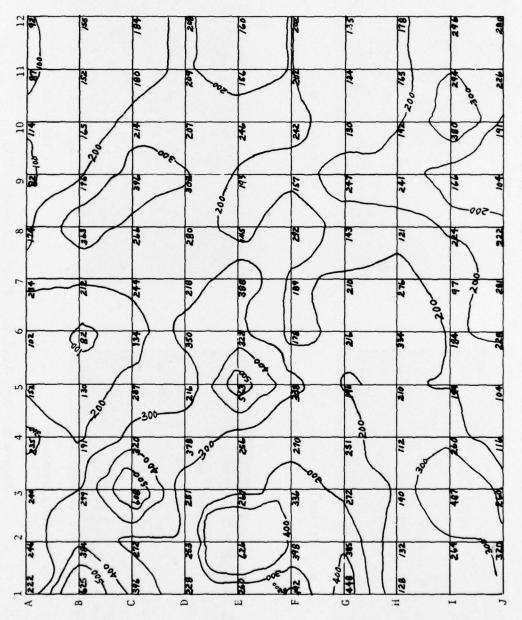
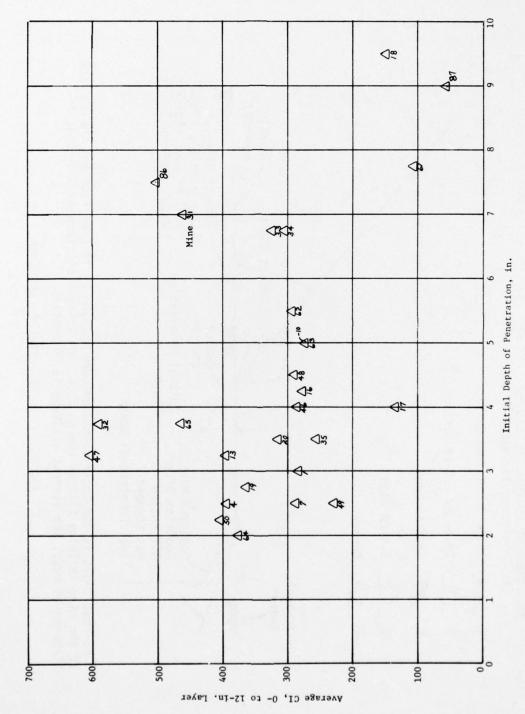


Figure 3. Distribution of average 0- to 12-in. CI

Any Material, Either Soil or Rock, Against Which the Mine Impacts Terrain Material: Terrain Surface Impact Angle Trajectory-- Initial Penetration Depth Crater -Impact 14.6- by 14.8-cm Surface 14.6- by 5.8-cm Surface LEGEND FOR IMPACT ATTITUDE - Final Penetration Depth (As Considered in Study) Position After All Motion Has Stopped Deployed Mine: 14.6-cm Edge 5.8-cm Edge Corner 9 At-Rest Angle* V B ပ

* At-rest angle is the angle formed between a 14.6- by 14.6-cm face and the surface. If the mine is lying flat on the surface, the at-rest attitude is 0 degree; if standing on edge, the at-rest attitude is 90 degrees.

Figure 4. Mine impact definitions



Initial depth of penetration versus average CI for the Air Force mine Figure 5.

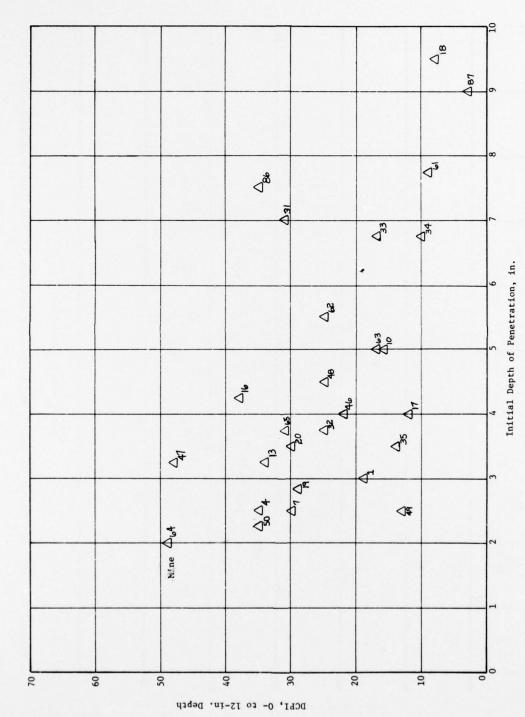


Figure 6. Initial depth of penetration versus DCPI for the Air Force mine

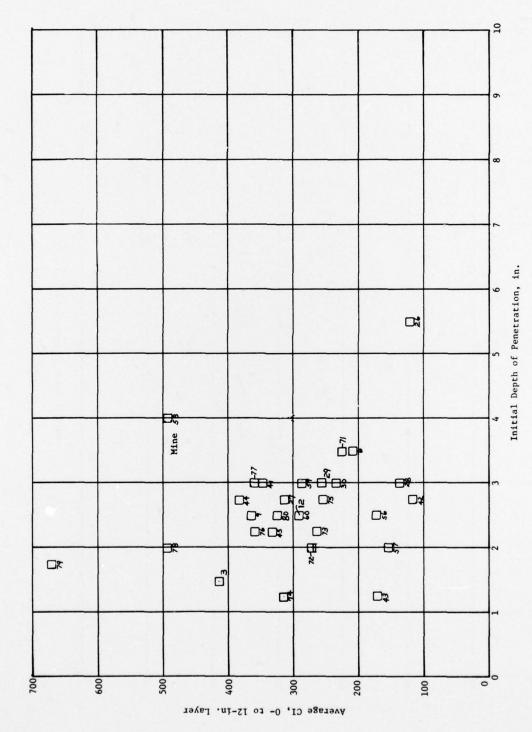


Figure 7. Initial depth of penetration versus average CI for the S-shaped mine

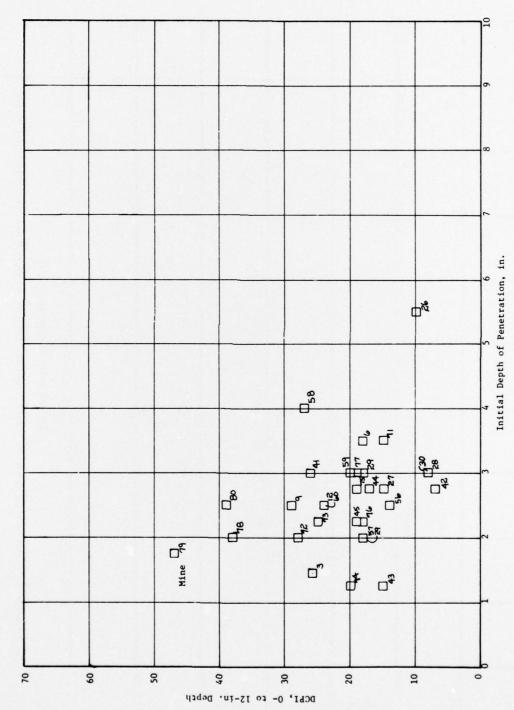


Figure 8. Initial depth of penetration versus DCPI for the S-shaped mine

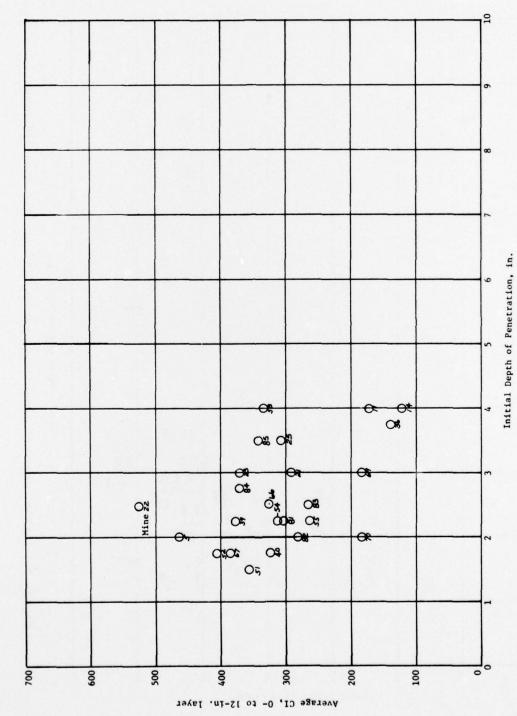


Figure 9. Initial depth of penetration versus average CI for the 110 mine

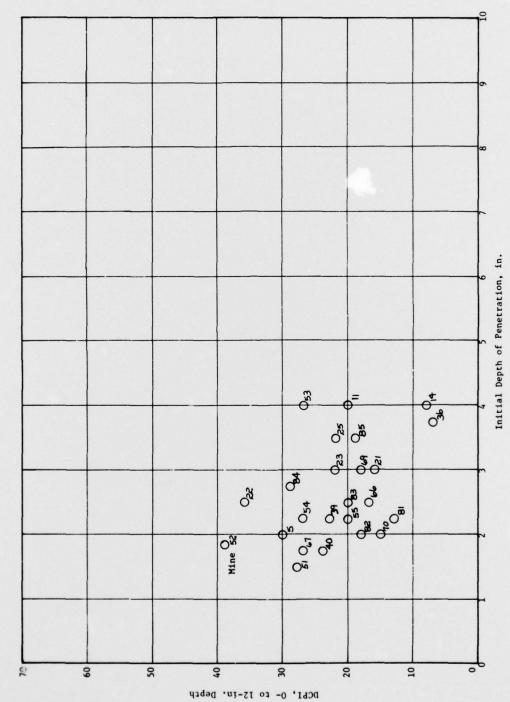


Figure 10. Initial depth of penetration versus DCPI for the 110 mine

In accordance with letter from DAEN-RDC, DAEN-ASI dated 22 July 1977, Subject: Facsimile Catalog Cards for Laboratory Technical Publications, a facsimile catalog card in Library of Congress MARC format is reproduced below.

Green, Charles E

Terrain characteristics at Gator Mine impact and penetration test sites, Aberdeen Proving Ground, Maryland / by Charles E. Green. Vicksburg, Miss.: U. S. Waterways Experiment Station; Springfield, Va.: available from National Technical Information Service, 1977.

14, 271 p.; 27 cm. (Miscellaneous paper - U. S. Army Engineer Waterways Experiment Station; M-77-13) Prepared for Picatinny Arsenal, Dover, New Jersey. References: p. 14.

1. Impact tests. 2. Mines (Ordnance). 3. Off-road mobility. 4. Penetration tests. 5. Soil penetration tests. 6. Soil strength. 7. Terrain. I. United States. Picatinny Arsenal, Dover, N. J. II. Series: United States. Waterways Experiment Station, Vicksburg, Miss. Miscellaneous paper; M-77-13. TA7.W34m no.M-77-13