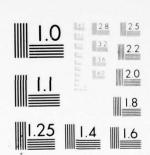


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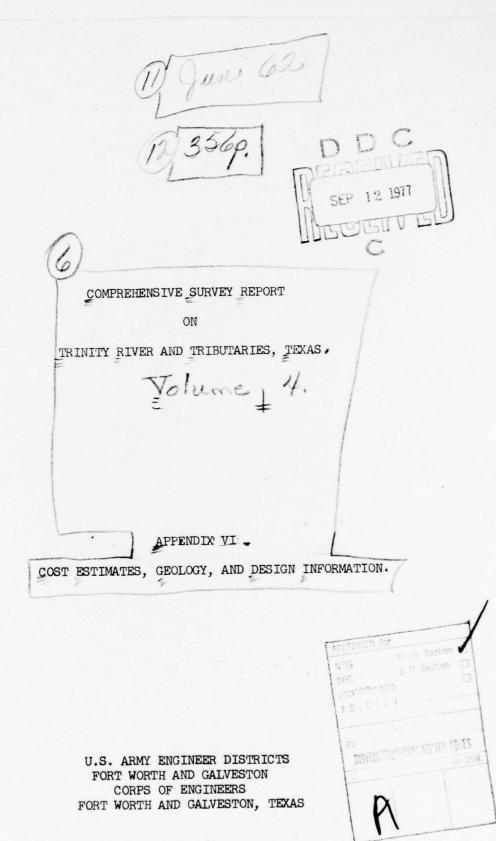
VOLUME 4

APPENDIX VI-COST ESTIMATES, GEOLOGY, AND DESIGN INFORMATION

U.S. ARMY/ENGINEER DISTRICTS FORT WORTH AND GALVESTON, TEXAS JUNE 1962

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CONTENTS

COMPREHENSIVE SURVEY REPORT ON TRINITY RIVER AND TRIBUTARIES, TEXAS

This volume comprises Appendix VI - Cost Estimates, Geology, and Design Information. The complete report consists of the following volumes:

Volume 1 - Main Report

Volume 2 - Appendix I - Project Formulation

Attachment - Information required by Senate

Resolution No. 148

Appendix III - Navigation and Navigation Economics

Appendix IV - Flood Control Economics

Volume 3 - Appendix II - Hydrology, Hydraulic Design, and

Water Resources

Volume 4 - Appendix VI - COST ESTIMATES, GEOLOGY, AND

DESIGN INFORMATION

Volume 5 - Appendix V - Recreation and Fish and Wildlife

Appendix VII - Economic Base Study

Appendix VIII - Comments of Other Agencies

Appendix IX - Resolutions, Public Hearings,

Prior Reports

ON

TRINITY RIVER AND TRIBUTARIES, TEXAS

APPENDIX VI

COST ESTIMATES, GEOLOGY, AND DESIGN INFORMATION

TABLE OF CONTENTS

| | Page Number |
|--|----------------|
| GENERAL | |
| INTRODUCTION | VI-1 |
| RELATIONSHIP OF THIS APPENDIX TO OTHER PARTS OF THE REPORT | VI- 1 |
| BASIS OF COST ESTIMATES AND SUMMARY | VI-2 |
| AVAILABILITY OF CONSTRUCTION MATERIALS | VI-3 |
| MULTIPLE PURPOSE CHANNEL | |
| OVERALL SUMMARY | VI-5 |
| REQUIRED LAND | VI-9 |
| CHANNEL AND CANALS | VI-16 |
| LOCKS | VI-36 |
| DAMS | VI-52 |
| HIGHWAY RELOCATIONS | VI-59 |
| RAILROAD RELOCATIONS | VI-68 |
| PIPELINE RELOCATIONS | VI-76 |
| POWER TRANSMISSION LINE RELOCATIONS | VI-83 |
| COMMUNICATION LINES RELOCATIONS | VI-85 |
| WATER AND SEWER LINES RELOCATIONS | VI-88 |
| ACCESS ROADS | VI-88 |

APPENDIX VI COST ESTIMATES, GEOLOGY, AND DESIGN INFORMATION

TABLE OF CONTENTS (CONT'D)

| <u>Title</u> | Number Number |
|---|---------------|
| MULTIPLE PURPOSE CHANNEL (CONT'D) | |
| PERMANENT OPERATING EQUIPMENT | VI-92 |
| BUILDINGS, GROUNDS & UTILITIES | VI-96 |
| AIDS TO NAVIGATION | VI-97 |
| ANNUAL CHARGES FOR MAINTENANCE, OPERATION & MAJOR REPLACEMENT | VI-100 |
| RESERVOIRS | |
| LAND CRITERIA | VI-113 |
| LAKEVIEW RESERVOIR PROJECT | VI-113 |
| AUBREY RESERVOIR | VI-129 |
| ROANOKE RESERVOIR | VI-138 |
| TENNESSEE COLONY RESERVOIR PROJECT | VI-147 |
| LOCAL PROTECTION UNITS | |
| GENERAL | VI-172 |
| WEST FORK | VI-172 |
| ELM FORK | VI-183 |
| EXTENSION OF DALLAS FLOODWAY | VI-194 |
| DUCK CREEK | VI-204 |
| LIBERTY | VI-210 |
| TRANSMISSION AND PUMPING FACILITIES | |
| PIPELINE | VI-215 |
| PUMPING PLANTS | VI-215 |

APPENDIX VI COST ESTIMATES, GEOLOGY, AND DESIGN INFORMATION

LIST OF PLATES

| Plate Number | <u>Title</u> | Page Number |
|-----------------|---|------------------|
| 1 | LOCATION OF PROJECTS RECOMMENDED IN THIS REPORT | . AI-5 |
| 2 | AREAL GEOLOGY | VI-7 |
| 3 | TOPOGRAPHIC MAP | vI-8 |
| 4 thru 11 | GENERAL PLANS | VI-18 thru VI-25 |
| 12 | PROFILE - MULTIPLE PURPOSE CHANNEL | VI-56 |
| 13 | PROFILE - MULTIPLE PURPOSE CHANNEL | VI-27 |
| 14 | PROFILE - MULTIPLE PURPOSE CHANNEL | VI- 28 |
| 15 | PROFILE - MULTIPLE PURPOSE CHANNEL | VI- 29 |
| 16 | SITE PLAN, LOCK 5A & 5B & DAM 19 | VI- 32 |
| 17 | SITE PLAN, LOCK LOA & LOB | VI- 34 |
| 18 | SITE PLAN, LOCK & DAM 1 & 2 | VI- 35 |
| 19 | TYPICAL LOCK (GRAVITY TYPE) & DAM | VI-40 |
| 20 | TYPICAL LOCK (GRAVITY TYPE) & DAM | VI-41 |
| 21 | TYPICAL LOCK (U-FRAME - LOCK 10B) | VI-42 |
| 22 | TYPICAL LOCK (U-FRAME - LOCK 10B) | VI-43 |
| 23 | PROFILE & LOGS OF BORINGS | VI-44 |
| 24 | PROFILE & LOGS OF BORINGS | VI-45 |
| 25 | SITE PLAN, LOCK & DAM 6 & 11 | VI-46 |
| 26 | TYPICAL HICHWAY & RAILROAD BRIDGE | VI-67 |

APPENDIX VI COST ESTIMATES, GEOLOGY, AND DESIGN INFORMATION

LIST OF PLATES (CONT'D)

| Plate Number | <u>Title</u> | Page Number |
|-----------------|---|----------------|
| 27 | RESERVOIR MAP, LAKEVIEW RESERVOIR | VI-114 |
| 28 | PLAN, PROFILE & SECTIONS, LAKEVIEW RESERVOIR | VI-115 |
| 29 | GENERAL HYDRAULICS DATA - LAKEVIEW RESERVOIR | VI-116 |
| 30 | SUBSURFACE INVESTIGATION - LAKEVIEW RESERVOIR | VI-120 |
| 31 | RESERVOIR MAP - AUBREY RESERVOIR | VI-132 |
| 32 | PLAN, PROFILE & SECTION - AUBREY RESERVOIR | VI-133 |
| 33 | GENERAL HYDRAULICS - AUBREY RESERVOIR | VI-134 |
| 34 | SUBSURFACE INVESTIGATION - AUBREY RESERVOIR | VI-137 |
| 35 | RESERVOIR MAP - ROANOKE RESERVOIR | VI-141 |
| 36 | PLAN, PROFILE & SECTIONS - ROANOKE RESERVOIR | VI-142 |
| 37 | GENERAL HYDRAULIC DATA - ROANOKE RESERVOIR | VI-143 |
| 38 | SUBSURFACE INVESTIGATION - ROANOKE RESERVOIR | VI-146 |
| 39 | RESERVOIR MAP - TENNESSEE COLONY RESERVOIR | VI-151 |
| 40 | PIAN, PROFILE & SECTION - TENNESSEE COLONY RESERVOIR | VI-152 |
| 41 | GENERAL HYDRAULIC DATA - TENNESSEE COLONY RESERVOIR | VI-153 |
| 42 | SUBSURFACE INVESTIGATION - TENNESSEE COLONY RESERVOIR | VI-155 |
| 43 | PLAN OF IMPROVEMENT - WEST FORK & TRIBUTARIES | VI-177 |
| 44 | PLAN OF IMPROVEMENT - WEST FORK & TRIBUTARIES | VI-178 |
| 45 | DETAILED PROFILES, WEST FORK & TRIBUTARIES | VI-179 |

A PPENDIX VI COST ESTIMATES, GEOLOGY, AND DESIGN INFORMATION

LIST OF PLATES (CONT'D)

| Plate Number | Title | Page Number |
|-----------------|---|----------------|
| 46 | DETAILED PROFILES, WEST FORK & TRIBUTARIES | VI-180 |
| 47 | DETAILED PROFILES, WEST FORK & TRIBUTARIES | VJ-181 |
| 48 | DETAILED PROFILES, WEST FORK & TRIBUTARIES | VI-182 |
| 49 | PLAN OF IMPROVEMENT - ELM FORK & TRIBUTARIES | VI- 184 |
| 50 | PLAN OF IMPROVEMENT - ELM FORK & TRIBUTARIES | VI-185 |
| 51 | DETAILED PROFILE - ELM FORK & TRIBUTARIES | VI-186 |
| 52 | DETAILED PROFILE - ELM FORK & TRIBUTARIES | VI-187 |
| 53 | DETAILED PROFILE - ELM FORK & TRIBUTARIES | VI-188 |
| 54 | PLAN OF IMPROVEMENT - DALLAS FLOODWAY | VI- 195 |
| 55 | DETAILED PROFILE - DALLAS FLOODWAY | VI-196 |
| 56 | PLAN OF IMPROVEMENT - DALLAS FLOODWAY EXTENSION | VI- 197 |
| 57 | DETAILED PROFILE - DALLAS FLOODWAY EXTENSION | VI-198 |
| 58 | PLAN OF IMPROVEMENT - DUCK CREEK | VI-208 |
| 59 | PROFILE - DUCK CREEK | VI-209 |
| 60 | PLAN OF IMPROVEMENT - LIBERTY LEVEE | VI-213 |
| 61 | PROFILE - LIBERTY LEVEE | VI-214 |
| 62 | PLAN OF IMPROVEMENT - TRANSMISSION AND PUMPING FACILITIES | VI-216 |

ON

TRINITY RIVER AND TRIBUTARIES, TEXAS

APPENDIX VI COST ESTIMATES, GEOLOGY, AND DESIGN INFORMATION

LIST OF TABLES

| | DIOI OF INDIES | |
|-----------------|--|----------------|
| Table Number | | Page Number |
| 1 | SUMMARY ESTIMATE OF FIRST COST OF PROPOSED MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-5 |
| 2 | LAND REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL FROM HOUSTON SHIP CHANNEL TO FORT WORTH, TEXAS | VI-10 |
| 3 | SUMMARY OF ESTIMATED FIRST COST FOR LANDS AND DAMAGES REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-13 |
| 4 | SUMMARY OF FIRST COST FOR LAND AND DAMAGES REQUIRED FOR PUBLIC-USE AREAS FOR MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS | VI-14 |
| 5 | SUMMARY OF ESTIMATED FIRST COST OF MULTIPLE PURPOSE CHANNEL WORK REQUIRED FOR TRINITY RIVER CHANNEL TO FORT WORTH | VI-17 |
| 6 | SUMMARY OF FIRST COST OF NAVIGATION LOCKS REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-36 |
| 7 | PERTINENT DATA-NAVIGATION LOCKS MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-39 |
| 8 | ESTIMATED FIRST COST OF NAVIGATION LOCKS NOS. 4, 6 & 19 REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH | VI-51 |
| 9 | SUMMARY OF FIRST COSTS OF NAVIGATION DAMS REQUIRED FOR THE MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS | VI-52 |
| 10 | PERTINENT DATA-NAVIGATION DAMS MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-54 |

VI-viii

TRINITY RIVER AND TRIBUTARIES, TEXAS

APPENDIX VI COST ESTIMATES, GEOLOGY, AND DESIGN INFORMATION

| | LIST OF TABLES (CONT'D) | |
|-----------------|---|----------------|
| Table Number | | Page Number |
| 11 | ESTIMATED FIRST COST OF NAVIGATION DAMS NOS. 4, 6 & 19 REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-58 |
| 12 | SUMMARY OF ESTIMATED FIRST COST AND COMPONENT LENGTH OF HIGHWAY BRIDGES REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-60 |
| 13 | PERTINENT DESIGN DATA OF PROPOSED HIGHWAY BRIDGES REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-63 |
| 14 | DATA RELATIVE TO INCREASED HEIGHT OF HIGHWAY BRIDGES REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL | VI-64 |
| 15 | DETAILED ESTIMATES OF FIRST COST FOR U.S. HIGHWAY BRIDGE NO. 90 (WESTBOUND) AT CHANNEL MILE 47.88 OVER MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH | VI-65 |
| 16 | SUMMARY OF ESTIMATED FIRST COSTS AND PERTINENT DATA CONCERNING RAILROAD BRIDGES REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-69 |
| 17 | PERTINENT DESIGN DATA OF PROPOSED RAILROAD BRIDGES REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-71 |
| 18 | DETAILED ESTIMATES OF FIRST COSTS FOR THE T & N.O. RR (SP) BRIDGE AT CHANNEL MILE 91.93 OVER MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH | VI-72 |
| 19 | DETAILED ESTIMATES OF FIRST COSTS FOR THE G.C. & S.F. RR BRIDGE AT CHANNEL MILE 331.09 OVER MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH | VI-74 |

ON

TRINITY RIVER AND TRIBUTARIES, TEXAS

APPENDIX VI COST ESTIMATES, GEOLOGY, AND DESIGN INFORMATION

LIST OF TABLES (CONT'D)

| T | able | LIDI OF INDIED (COMI D) | Page |
|---|-------|--|--------|
| N | umber | | Number |
| | 20 | SUMMARY OF COST OF PIPELINE RELOCATIONS REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-78 |
| | 21 | SUMMARY OF FIRST COSTS OF MODIFICATIONS TO ELECTRIC-POWER TRANSMISSION LINES TO PROVIDE FOR THE MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-83 |
| | 22 | ESTIMATED FIRST COST FOR RELOCATION OF BELL TELEPHONE COMPANY'S SIX CABLE CROSSING AT MODIFIED CADIZ STREET BRIDGE AT CHANNEL MILE 332.2 AND NINE CABLE CROSSING AT MODIFIED HOUSTON STREET BRIDGE, MILE 332.6 | vi-86 |
| | 23 | SUMMARY OF ESTIMATED FIRST COST TO RELOCATE COMMUNICATION LINES REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-87 |
| | 24 | SUMMARY OF FIRST COSTS OF WATER AND SEWER LINE MODIFICATIONS REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-89 |
| | 25 | PERTINENT DATA CONCERNING PROPOSED ACCESS ROADS TO LOCK SITES | VI-90 |
| | 26 | SUMMARY OF ESTIMATED FIRST COST OF ACCESS ROADS TO LOCKS AND DAMS REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH | VI-91 |
| | 27 | ESTIMATED FIRST COST OF ONE SET OF LOCK AND DAM PERMANENT OPERATING EQUIPMENT | VI-92 |
| | 28 | ESTIMATED FIRST COST OF ONE SET OF HEAVY DUTY EQUIPMENT | VI-93 |

ON TRINITY RIVER AND TRIBUTARIES, TEXAS

APPENDIX VI COST ESTIMATES, GEOLOGY AND DESIGN INFORMATION

LIST OF TABLES (CONTID)

| | LIDI OF IADIES (CONT D) | _ |
|-----------------|---|----------------|
| Table Number | | Page Number |
| 29 | SUMMARY OF ESTIMATED FIRST COST OF PERMANENT OPERATING EQUIPMENT REQUIRED FOR MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS | VI-94 |
| 30 | ESTIMATED FIRST COST OF BUILDINGS, GROUNDS, AND UTILITIES REQUIRED FOR MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS | VI-96 |
| 31 | SUMMARY OF ESTIMATED FIRST COST OF AIDS TO NAVIGATION REQUIRED FOR THE MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-97 |
| 32 | SUMMARY OF ESTIMATED FIRST COST OF RECREATIONAL FACILITIES FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-99 |
| 33 | ESTIMATED ANNUAL COST OF MAINTENANCE, OPERATION AND REPLACEMENT FOR DESIGNATED REACHES OF MULTIPLE PURPOSE CHANNEL FROM HOUSTON SHIP CHANNEL TO FORT WORTH, TEXAS | VI-101 |
| 34 | ESTIMATED ANNUAL COST OF MAJOR REPLACEMENTS FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL | VI-103 |
| 35 | ESTIMATE OF ANNUAL MAINTENANCE DESIGNG COSTS FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS | VI-109 |
| 36 | ESTIMATED ANNUAL COST OF MAINTENANCE AND OPERATION OF RECREATIONAL FACILITIES IN DESIGNATED REACHES OF THE MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS | VI-111 |
| 37 | PERTINENT DATA PROPOSED LAKEVIEW RESERVOIR TRINITY RIVER | VI-118 |
| 38 | SUMMARY OF FIRST COST AND ANNUAL CHARGES PROPOSED LAKEVIEW RESERVOIR, MOUNTAIN CREEK | VI-122 |

ON

TRINITY RIVER AND TRIBUTARIES, TEXAS

APPENDIX VI COST ESTIMATES, GEOLOGY AND DESIGN INFORMATION

LIST OF TABLES (CONT'D)

| Table Number | LIST OF TABLES (CONT.D) | Page Number |
|-----------------|--|-----------------|
| 39 | DETAILED ESTIMATE OF FIRST COST LAKEVIEW RESERVOIR-MULTIPLE PURPOSE MOUNTAIN CREEK | VI-123 |
| 40 | PERTINENT DATA PROPOSED AUBREY RESERVOIR ELM FORK OF TRINITY RIVER | VI-129 |
| 41 | SUMMARY OF FIRST COST AND ANNUAL CHARGES PROPOSED AUBREY RESERVOIR, ELM FORK | VI-136 |
| 42 | PERTINENT DATA PROPOSED ROANOKE RESERVOIR DENTON CREEK | VI-138 |
| 43 | SUMMARY OF FIRST COST AND ANNUAL CHARGES PROPOSED ROANOKE RESERVOIR, DENTON CREEK | VI-144 |
| 44 | PERTINENT DATA PROPOSED TENNESSEE COLONY RESERVOIR, TRINITY RIVER | VI-148 |
| 45 | SUMMARY OF FIRST COST AND ANNUAL CHARGES PROPOSED TENNESSEE COLONY RESERVOIR TRINITY RIVER | VI-156 |
| 46 | DETAILED ESTIMATE OF FIRST COST TENNESSEE COLONY RESERVOIR-MULTIPLE PURPOSE TRINITY RIVER | VI-157 |
| 47 | SUMMARY OF FIRST COST AND ANNUAL CHARGES WEST FORK FLOOD PROTECTION PROJECT, WEST FORK, TRINITY RIVER | VI-173 |
| 48 | DETAILED ESTIMATE OF FIRST COST WEST FORK FLOOD PROTECTION PROJECT WEST FORK, TRINITY RIVER | VI-1 7 5 |
| 49 | SUMMARY OF FIRST COST AND ANNUAL CHARGES ELM FORK FLOOD PROTECTION PROJECT ELM FORK, TRINITY RIVER | VI-189 |

VI-xii

ON

TRINITY RIVER AND TRIBUTARIES, TEXAS

APPENDIX VI COST ESTIMATES, GEOLOGY AND DESIGN INFORMATION

LIST OF TABLES (CONT'D)

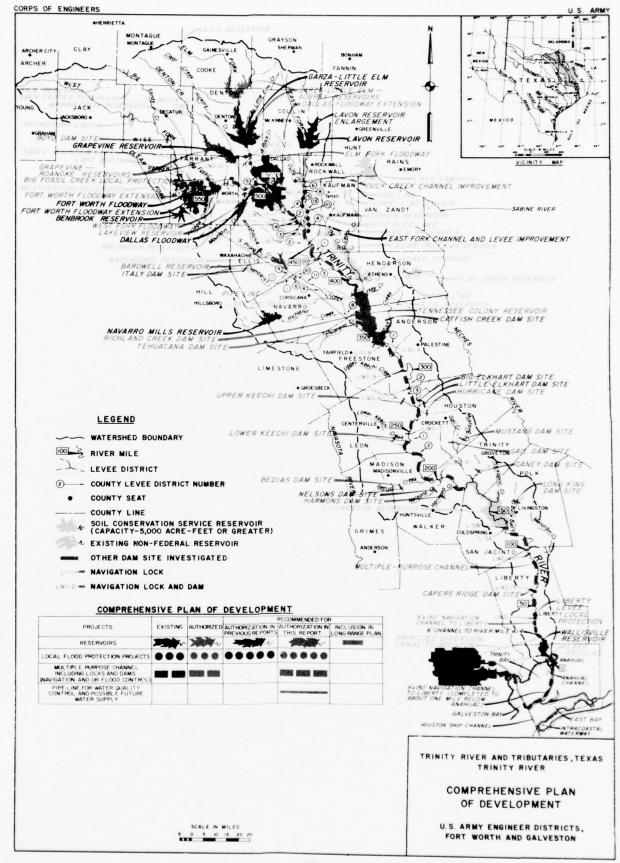
| Table Number | | Page Number |
|-----------------|--|----------------|
| 50 | DETAILED ESTIMATE OF FIRST COST ELM FORK FLOOD PROTECTION PROJECT ELM FORK, TRINITY RIVER | VI-191 |
| 51 | SUMMARY OF FIRST COSTS AND ANNUAL CHARGES DALLAS FLOODWAY EXTENSION, FLOOD PROTECTION PROJECT, TRINITY RIVER | VI-199 |
| 52 | DETAILED ESTIMATE OF FIRST COST, DALIAS FLOODWAY EXTENSION, FLOOD PROTECTION PROJECT, TRINITY RIVER | VI-201 |
| 53 | SUMMARY OF FIRST COSTS AND ANNUAL CHARGES LOCAL FLOOD PROTECTION PROJECT, GARLAND, TEXAS, DUCK CREEK | VI-204 |
| 54 | DETAILED ESTIMATE OF FIRST COST, LOCAL FLOOD PROTECTION PROJECT, GARLAND, TEXAS DUCK CREEK | VI-206 |
| 55 | SUMMARY OF FIRST COSTS AND ANNUAL CHARGES LOCAL FLOOD PROTECTION PROJECT, LIBERTY, TEXAS | VI-211 |

APPENDIX VI

COST ESTIMATES, GEOLOGY, AND DESIGN INFORMATION

GENERAL

- 1. INTRODUCTION. This appendix presents the general geologic features of the basin, foundation conditions, and the availability of construction materials with respect to the projects recommended in this report for authorization. Also presented herein are the pertinent design information, detailed cost estimates, and annual charges for appropriate elements of the recommended projects for the Trinity River Basin.
- 2. RELATIONSHIP OF THIS APPENDIX TO OTHER PARTS OF THE REPORT.—
 This appendix is related primarily to Appendix I Project formulation,
 Appendix II Hydrology, hydraulic design, and water resources, and
 Appendix V Recreation and fish and wildlife. In general the cost
 estimates given in this appendix have been adapted and adjusted for use
 in determining the cost allocation analysis of projects considered in the
 project formulation processes presented in appendix I. Hydraulic design
 information given herein is based on the detailed information contained
 in appendix II. Recreation and fish and wildlife cost estimates given
 herein have been used or have been adapted for use for projects covered
 in appendix V.
- 3. BASIS OF COST ESTIMATES AND SUMMARY. Unit prices used in this appendix are based on January 1962 price levels for comparable construction work in this area. The cost of lands, including improvements and damages, is based on field results of inspection and appraisal of individual areas. Estimates of annual charges for the multiple purpose channel are included in project formulation studies given in appendix I. Estimates of annual charges for all other recommended projects are contained in this appendix. The annual charges were computed using an interest rate of 2-7/8% for Federal work and 3% for non-Federal work. The first cost of the recommended projects were amortized over a 100-year period. The total first cost of the projects recommended in this report for authorization is estimated at \$900,747,000 of which \$164,797,000 is for contingency allowances. The locations of the individual projects are shown on plate 1, page 2.



4. The following tabulation is a summary of first costs including contingencies, engineering and design costs, and supervision and inspection costs for projects recommended in this report.

| Project | First cost |
|--|-------------------|
| Multiple purpose channel, including locks and dams | \$568,738,000 (1) |
| Lakeview Reservoir | 31,180,000 |
| Aubrey Reservoir | 34,073,000 |
| Roanoke Reservoir | 16,900,000 |
| Tennessee Colony Reservoir | 137,138,000 (2) |
| West Fork Floodway | 17,809,000 |
| Elm Fork Floodway | 16,823,000 |
| Dallas Floodway Extension | 14,327,000 |
| Duck Creek Channel | 5,024,000 |
| Liberty Levee | 2,091,000 |
| Transmission and pumping facilities | 56,643,000 |
| Total | \$900,746,000 |

- (1) Excludes cost of locks, dams, channel and appurtenant facilities required for navigation through the Tennessee Colony Reservoir, estimated at \$42,884,420.
- (2) Includes an estimated cost of \$42,884,420 for navigation facilities in reservoir.
- 5. AVAILABILITY OF CONSTRUCTION MATERIALS. Sources considered herewith are relevant to the availability of concrete aggregates, riprap, and bedding material. The upper region of the Trinity River Basin, from the northern extremities of the watershed, through Dallas and Tarrant Counties, to the intersection of the Trinity River with State Highway 34, has an abundance of construction materials. Crushed stone is available from a number of commercial sources in Wise County near Bridgeport at the northwest edge of the basin, and from quarries in Kaufman County near Terrell to the east. That portion of the Trinity River traversing Tarrant and Dallas Counties is flanked at various points by sand and gravel companies producing from the Trinity River or its tributaries. The upper

central region of the Trinity River from State Highway 34 on the north, along the Navarro-Henderson and Freestone-Anderson County boundary lines, to Long Lake, Texas, has limestone formations in Navarro and Freestone Counties that could supply ample quantities of crushed limestone. Current production is limited, being utilized primarily for highway construction; however, the potential exists in these areas for production on a much larger scale. In addition there are potential sources of sand and gravel along the Trinity River in these counties which could be developed for construction of the proposed locks, dams, roads, etc. The lower central region from Long Lake, Texas, to the junction of the Trinity River with the northern Polk County line lacks any sizable commercial materials presently being worked; however, a potential source of sandstone exists in northern Walker County which could be used as a source for road and riprap material. Hauling of materials would be required from the existing resources now being mined near Hearne, Temple, Waco, or from the previously mentioned sources in Navarro, Freestone and Walker Counties. depending upon the specific portion of the region in which construction is being accomplished. Haul distances would range from 50 to 70 miles for sand and grawel, and from 25 to 70 miles for crushed stone. Projects to be constructed in the lower region from the northern Polk County line to Galveston Bay would utilize potential sand and gravel sources located adjacent to the Trinity River in Liberty and San Jacinto Counties, involving maximum hauls of about 50 miles. Sand and gravel materials could also be obtained from resources now being mined about 50 miles west of Houston, or from near Lake Charles, Louisiana. Either resource would entail a haul of about 100 miles.

MULTIPLE PURPOSE CHANNEL

6. OVERALL SUMMARY .- The total cost of the multiple purpose channel extending from the Houston Ship Channel to the Riverside Drive bridges in Fort Worth, Texas, in accordance with the plan of improvement described in appendix III, is estimated at \$568,737,710, as summarized in table 1, which excludes the sost of channel, locks, dams and appurtenant facilities required for mavigation through the Tennessee Colony Reservoir. The cost of the channel and navigation features in the Tennessee Colony Reservoir is estimated at \$42,884,420, as shown in table 1. The cost of the navigation features in the Tennessee Colony Reservoir are included in the estimated cost of the Tennessee Colony Reservoirs for the purpose of determining the allocation of cost to the reservoir project purposes, which is considered in appendix I. The estimated cost of the multiple purpose channel in designated reaches between the Houston Ship Channel and Fort Worth, as given in table 1, is presented for use in determining the allocation of cost of the multiple purpose channel, which is also considered in appendix I.

TABLE 1
SUMMARY ESTIMATE OF FIRST COST FOR PROPOSED
MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH

| Cost : classi - : fication: number : Item of cost | : Houston Ship : Channel to : upper reach : of Wallisvil : Reservoir : (0.0 to 35.5 | : reservoir to : :head of author-: le: ized channel : : to Liberty : | authorized : channel to : Liberty to : Tennessee : Colony dam | Tennessee Colony dam to Lock & Dam No. 12 (233.5 to 274.4) | Lock & Dam No. 12 to Five-mil Creek (274.4 to 322.0) |
|--|--|--|--|--|--|
| (01.0) Lands & damages Contingenceis 25% Acquisition Costs Total cost | \$ 51,300 12,830 7,300 71,430 | \$ 218,670 54,670 40,510 313,850 | \$ 3,666,710 916,680 483,730 5,067,120 | \$ 34,900 8,800 3,700 47,400 | \$ 1,970,35 492,59 119,4 2,582,4 |
| (02.0) Relocations Railroads Highways Pipelines Powerlines Communication lines Water & sewer lines Total relocations | None 1,926,000 419,960 None None 2,345,960 | None None 54,240 69,800 5,870 None | 12,127,000 10,915,000 1,799,790 332,200 68,200 None 25,242,190 | (2) (2) (2) (2) (2) (2) | None 4,392,00 55,00 32,00 None None |
| (04.0) Navigation dams (05.0) Navigation locks (08.0) Roads, railroads & bridges (09.0) Channels and canals (14.0) Recreation facilities (19.0) Buildings, grounds & utilities (20.0) Permanent operating equipment Subtotal construction cost | None None None 2,254,490 287,300 None 2,950 4,890,700 | None None None 5,176,940 66,600 None 5,000 | 19,774,350 90,702,570 2,878,170 72,234,380 1,177,400 1,276,000 488,840 213,773,900 | 2,112,930 19,946,060 163,330 8,287,290 168,200 255,200 120,380 31,053,390 | 12,941,0 45,098,0 1,313,9 18,317,3 777,8 765,6 266,1 83,958,9 |
| Contingencies 25% Total construction cost (30.0) Engineering & design (31.0) Supervision & administration Total including lands & damages U.S. Coast Guard navigation aids Total first cost including cost of navigation facilities in | 1,222,670 6,113,370 232,310 354,580 6,771,690 None 6,771,690 | 1,344,610 6,723,060 255,480 389,940 7,682,330 None 7,682,330 | 53,443,480 267,217,380 10,154,260 15,498,610 297,937,370 418,500 298,355,870 | 7,763,350 38,816,740 1,475,040 2,251,240 42,590,420 294,000 42,884,420 | 20,989,7 104,948,7 3,988,0 6,087,0 117,666,2 47,00 117,653,2 |
| Tennessee Colony reservoir Total first cost excluding cost of navigation facilities in Tennessee Colony reservoir | 6,771,690 | 7,682,330 | 298,355,870 | (excluded) | 117,653,2 |

All costs for authorized channel to Liberty are excluded.
 Costs for lands and relocations within the Tennessee Colony reservoir are excluded.

NOTE: Costs of \$435,000 were expended on the multiple purpose channel studies.

TABLE 1 ATE OF FIRST COST FOR PROPOSED LTY RIVER CHANNEL TO FORT WORTH, TEXAS

| 3.5): | Tennessee : Colony dam : to Lock & : Dan No. 12 : (233.5 to : 274.4) : | Lock & Dam No. 12 to Five-mile Creek (274.4 to | Dallas : | terminus to Dallas floodway (326.7 to | : Dallas : floodway : (331.1 : to : 338.8) | Dallas floodway to & including Fort Worth terminus (338.8 to 362.8) | : Fort Worth : terminus to : & including : Riverside : Drive : bridges):(362.8 to 369.8) | : : : : : Total : (0.0 to 369.8) |
|--|--|---|---|--|---|---|---|---|
| 710 580 730 120 | \$ 34,900 8,800 3,700 47,400 | \$ 1,970,350 492,590 119,470 2,582,410 | \$ 292,460 73,110 9,870 375,440 | \$ 281,150 70,290 9,490 360,930 | \$ 402,590 100,600 8,140 511,330 | \$ 1,356,760 339,190 38,550 1,734,500 | \$ 167,000 41,750 6,320 215,070 | \$ 8,441,890 2,110,510 727,080 11,279,480 |
| 200 200 200 200 200 200 200 200 200 200 | (2) (2) (2) (2) (2) (2) (2) (2) (2) (2) | None 4,392,000 55,000 32,000 None None 14,479,000 12,941,090 45,098,070 1,313,940 18,317,370 777,800 765,600 266,160 83,958,980 20,989,740 104,948,720 3,988,050 6,087,030 117,606,210 47,000 117,653,210 | None 2,079,000 72,630 None 59,250 146,820 2,357,700 None None 1,343,220 None None 1,530 3,702,450 925,610 4,628,060 175,870 268,430 5,447,800 4,500 5,452,300 | 4,119,000 2,764,000 147,130 52,200 None 169,360 7,251,690 None None 1,232,000 None None 1,430 8,485,120 2,121,250 10,606,370 403,040 615,170 11,985,510 4,500 11,990,010 | 5,578,000 12,855,000 52,880 24,200 770,140 217,650 19,497,870 2,356,080 107,050 1,836,390 101,600 127,600 30,056,400 7,514,100 37,570,500 1,427,680 2,179,090 41,688,600 41,696,100 | 556,500 9,631,980 287,300 | None 662,000 24,690 None None None 86,690 None None None 3,815,100 None None 2,280 4,504,070 1,126,020 5,630,090 213,940 326,550 6,385,650 None 6,385,650 | 25,398,000 44,024,000 2,828,770 710,800 941,560 588,290 74,491,420 45,491,000 181,705,560 5,018,990 124,129,110 2,866,200 2,807,200 1,113,000 437,622,480 109,405,580 547,028,060 20,787,070 31,727,520 610,822,130 800,000 611,622,130 |
| 370 | (excluded) | 117,653,210 | 5,452,300 | 11,990,010 | 41,696,100 | 72,750,550 | 6,385,650 | 568,737,710 |

7. The total estimated first cost including 25% contingencies engineering, design, supervision and administration costs, and annual charges of the multiple purpose channel are as follows:

ESTIMATED FIRST COST MULTIPLE PURPOSE CHANNEL (Including Recreation)

| | :Houston Ship | Dallas t | : 0 |
|--------------------------------|---------------|-----------------|---------------------|
| Item | :Channel to I | Dallas:Ft.Worth | 1 : Total |
| | 40 120 050 | A 0 002 000 | + 33 000 000 |
| Lands & damages | \$8,410,250 | \$ 2,821,830 | \$ 11,232,080 |
| Channel & canals | 136,077,100 | 22,626,190 | 158,703,290 |
| Navigation locks | 186,046,900 | 35,563,600 | 221,610,500 |
| Navigation dams | 44,820,150 | 14,607,800 | 59,427,950 |
| Highways and bridges | 26,457,480 | 33,855,400 | 60,312,880 |
| Railways and bridges | 16,614,000 | 18,181,260 | 34,795,260 |
| Pipelines | 3,290,230 | 585,180 | 3,875,410 |
| Powerlines | 594,600 | 379,200 | 973,800 |
| Communication lines | 182,540 | 1,107,400 | 1,289,940 |
| Water and sewer lines | 201,160 | 604,800 | 805,960 |
| Access roads to locks and dams | 5,743,150 | 909,100 | 6,652,250 |
| Permanent operating equipment | 1,047,340 | 312,550 | 1,359,890 |
| Buildings, grounds and utiliti | es 2,797,040 | 699,200 | 3,496,240 |
| Aids to navigation | 470,000 | 36,000 | 506,000 |
| Recreation | 3,163,460 | 532,800 | 3,696,260 |
| Total first cost | \$435,915,400 | \$132,822,310 | \$568,737,710 |

ESTIMATED ANNUAL CHARGES MULTIPLE PURPOSE CHANNEL (Including Recreation)

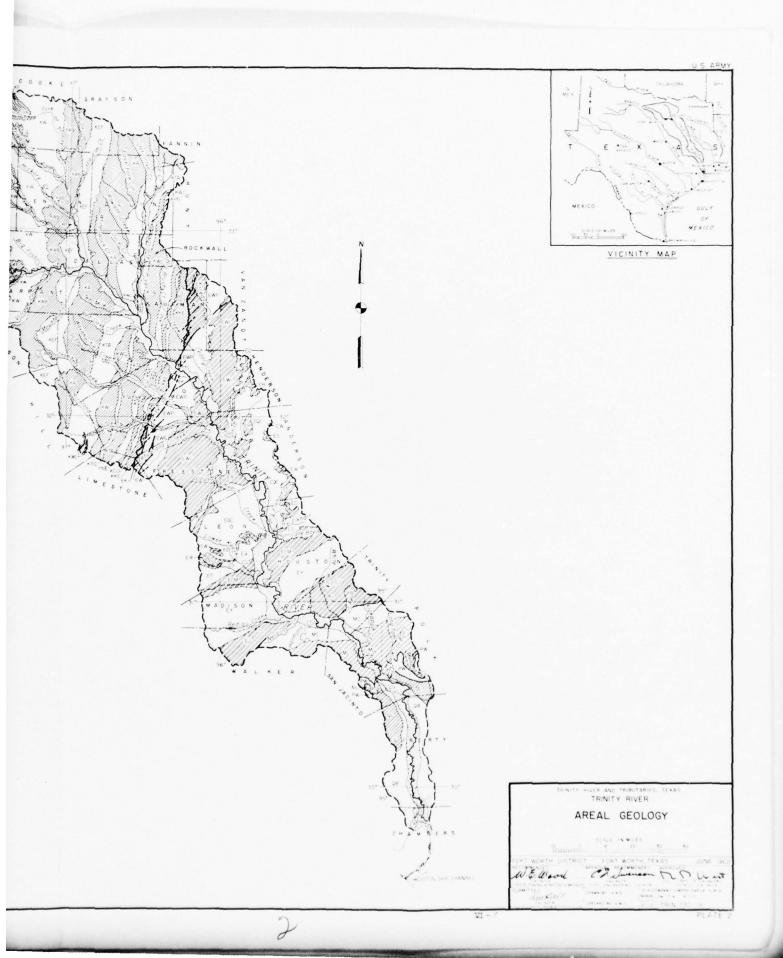
| • | | | |
|------------------------------|-------------------|-----------------------------|---------------|
| | :Houston Ship | :Dallas to: | |
| Item | :Channel to Dal | las:Ft.Worth : | Total |
| | (10 yr.constr.per | iod)(4 yr.constr period) | |
| Federal Investment | 41 1 - 0 | 4.15 (01 000 | A=05 .00 100 |
| First cost | \$419,428,100 | \$117,601,000 | \$537,029,100 |
| Interest during construction | 60,292,800 | 6,762,100 | |
| Total investment | 479,720,900 | 124,363,100 | 604,084,000 |
| Non Federal Investment | | | |
| First cost | 16,487,300 | 15,221,300 | 31,708,600 |
| Interest during construction | 2,473,100 | 913,300 | 3,386,400 |
| Total investment | 18,960,400 | 16,134,600 | 35,095,000 |
| | ,, , | , - , | |

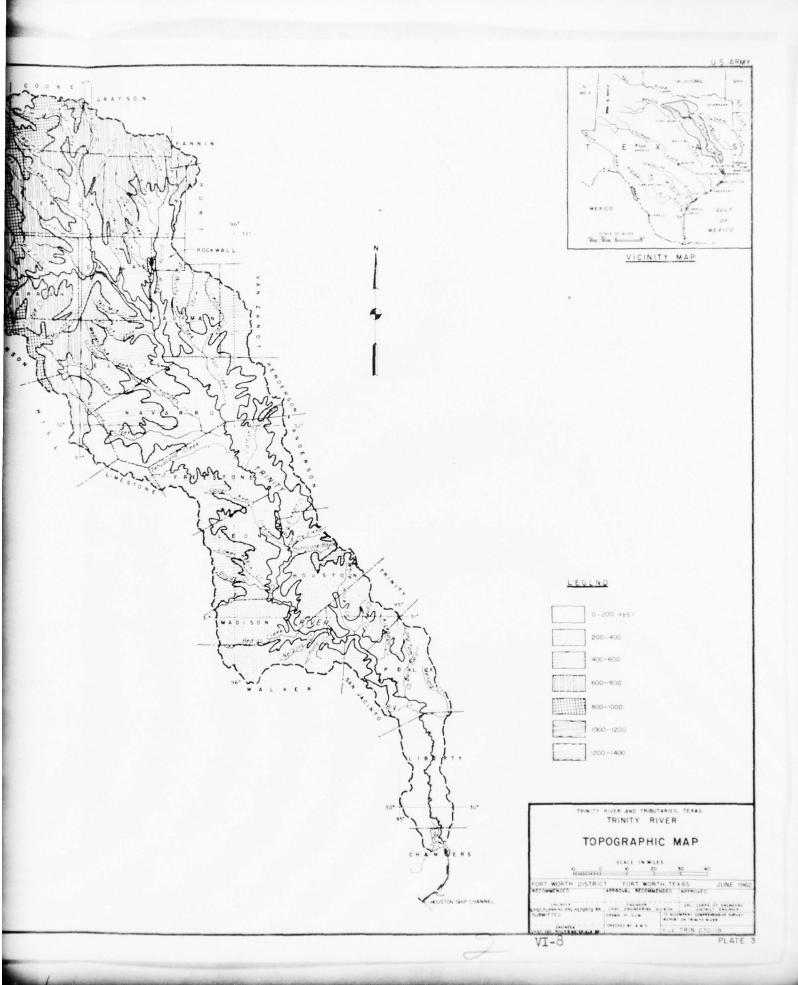
ESTIMATED ANNUAL CHARGES (CONT'D) MULTIPLE PURPOSE CHANNEL (Including Recreation)

| | :Houston Ship | | |
|---|------------------------------------|-----------------------------------|--------------------------------------|
| Item | :Channel to Da. | llas:Ft.Worth: | Total |
| | (10 yr.constr.pe | | str iod) |
| Federal annual charges | | | |
| Interest on investment Amortization (100 yr life) | \$13,792,000 858,700 | \$3,575,400 222,600 | \$17,367,400 1,081,300 |
| Operation, maintenance and major replacement Total investment | 3,121,700 17,772,400 | 741,500 4,539,500 | 3,863,200 22,311,900 |
| Non-Federal annual charges Interest on investment Amortization (100 yr life) | 568,800 31,300 | 484,000 26,600 | 1,052,800 57,900 |
| Operation, maintenance and major replacement Total Federal annual charges Total annual charges | 145,000 745,000 \$18,517,500 | 144,900 655,500 \$5,195,000 | 289,900 1,400,600 \$23,712,500 |

Note: Interest rates are 2-7/8% for Federal and 3% for non-Federal.

8. AREAL GEOLOGY AND TOPOGRAPHY. The Trinity River Basin includes the outcrops of formations belonging to the following geologic periods: Pennsylvanian, Lower and Upper Cretaceous, Eocene, Miocene, Pliocene, and Pleistocene. The larger stream valleys contain deposits of alluvium of Quaternary age, which includes Pleistocene and recent deposits. The Pennsylvanian formations outcrop in the northwest corner of the basin and dip stratographically to the northwest. The remaining formations, which lie unconformably on the Pennsylvanian, dip easterly and southeasterly following roughly the general direction of the Trinity River from Fort Worth to the mouth. The southeasterly dip of the strata is greater than the general slope of the land and consequently, the outcrops of successively younger formations are encountered progressing downstream. These formations outcrop in zones of varying width which traverse the basin in a northeasterly direction as shown on plate 2. The topography of the basin is depicted on plate 3, which shows that the general land elevation rises gradually from a few feet above sea level near the mouth of the Trinity River to about 550 feet on the interstream divides in the vicinity of Dallas, thence the slope of the land increases to elevation 1250 on the divide in the northwest corner of the basin. Unless otherwise stated, elevations given hereinafter refer to U. S. Coast and Geodetic Survey mean sea level datum, which is established 1.36 feet above U. S. Corps of Engineers mean low tide datum near Anahuac, Texas.





- 9. IANDS AND DAMAGES.— The lands required for construction and maintenance of the multiple purpose channel from the Houston Ship Channel to the Riverside Drive bridges in Fort Worth, Texas, includes lands for channel rights-of-way, construction and maintenance spoil areas, access road rights-of-way, lock and dam sites and public-use areas. The multiple purpose channel involves all types of land from heavily timbered areas used for limited grazing to highly improved farming and grazing lands as well as areas classified as potential industrial lands in the Dallas-Fort Worth area. The land values for the multiple purpose channel project are based on the findings of a gross real estate appraisal made in 1961. The channel was divided into 88 segments and each segment was investigated by a survey team to determine the present fair market value of all required lands.
- 10. INTEREST TO BE ACQUIRED. Real estate interests for rights-of-way, spoil areas, and access roads would be acquired on a perpetual easement basis with mineral exploration subordinated to the Government's right to regulate such development in a manner that will not interfere with the primary purpose of the project. A fee less mineral interest with mineral exploration prohibited thereon should be acquired for the lock and dam sites and public use areas. Since there are several oil and gas producing areas within this project area, it is considered to be in the best interest of the Government to place a moratorium on exploration rather than acquire ownership of the minerals.
- ll. CRITERIA RELATED TO LAND REQUIREMENTS. Channel rights-of-way were computed on the basis of providing right-of-way limits 50 feet beyond the top of cut of the multiple purpose channel. Where the channel is partly or wholly within a section of the river it was assumed that a minimum of 50 feet of right-of-way would be acquired on each bank of the river. Channel excavation is based on side slopes of 2 horizontal to 1 vertical and the additional 50 feet of right-of-way on both sides of the channel should be sufficient to take care of flatter slopes where needed and also provide for probable future erosion of the channel banks. The proposed channel alignment would sever many bends of the river leaving areas of severed lands. These severed lands would be used as spoil areas whenever practicable. Severance damages would be paid to owners of severed lands.
- 12. Rights-of-way for lock and dam sites includes lands for the locks, dams, esplanades, reservation buildings and service roads. Access roads would be provided from existing all weather roads to lock and dam sites. Land requirements for the access roads were based on providing rights-of-way 100 to 120 feet wide where required for new road construction. Where existing roads are to be improved, additional rights-of-way for the improved roads were based on the consideration that existing rights-of-way would be made available at no additional cost.
- 13. A land area of two acres is required for the light attendant stations to be located in the Livingston and Tennessee Colony Reservoirs, as requested by Commander, Eighth Coast Guard District, New Orleans, Louisiana. The cost of providing these land areas is negligible, and is not explicitly included as a separate item in table 2 on page 10.

TABLE 2

LAND REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL FROM HOUSTON SHIP CHANNEL TO FORT WORTH, TEXAS

| : | I | Rights-o | | res) | : : | | |
|----------|------------|----------|----------|-----------|--------------|-----------|--------------|
| : | | : Lock | | : | : Spoil : | Severed | |
| Pool : | | : dam | : Access | :Public | | lands | Total |
| Number: | Channel | : sites | : roads | : use | :(acres)(1): | (acres) | (acres) |
| Tidal | 18 | _ | _ | 205 | 200 | _ | 423 |
| 1 | 288 | _ | | 125 | 1,715 | 442 | 2,570 |
| 2 | 408 | 48 | 6 | 134 | 1,467 | 578 | 2,641 |
| 3 | 596 | 48 | 83 | 50 | 1,221 | 1,410 | 3,1408 |
| 3 | 585 | 50 | 15 | 112 | 1,284 | 1,014 | 3,060 |
| 5A | 170 | 30 | 21 | 137 | 80 | - | 438 |
| 5B 6 | 1,116 | 30 | - | 134 | 3,346 | | 4,626 |
| 6 | 965 | 43 | 67 | 171 | 2,377 | 79 | 3,702 |
| 7 8 | 747 | 58 | 79 | 184 | 2,495 | 836 | 4,399 |
| 8 | 483 | 44 | 27 | 50 | 1,236 | 2,427 | 4,267 |
| 9 | 518 | 40 | 72 | 50 | 1,344 | 1,252 | 3,276 |
| lOA | 58 | 30 | 122 | 137 | 40 | - | 387 |
| lob | - | - | | - | - | - | - |
| 11 | 1.75 | - | 44 | 187 | - | - | 231 |
| 12 | 529 | 49 | 61 | 112 | 1,000 | 142 | 1,893 |
| 13 | 431 | 48 | 23 | 137 | 843 | 66 | 1,548 |
| 14 | 303 | 45 | 2 | - | 586 | 668 | 1,604 |
| 15 | 149 | 43 | 46 | 62 | 311 444 | 85 | 696 |
| 16 | 217 | 44 | 24 6 | 124 | | 107 | 960 |
| 17 18 | 565 | 51 | | 124 | 375 | 845 | 1,966 871 |
| 19 | 291 308 | 38 | 2 7 | 137 | 330 261 | 73 | 864 |
| 20 | 267 | 37 42 | 22 | 59 | 238 | 192 71 | 699 |
| 21 | 166 | 41 | 2 | 59 112 | 138 | 23 | 482 |
| (2) | 151 | -17 | 2 | 1.1.2 | 130 | 244 | 395 |
| (2) | 1)1 | | | | | 24.4 | 377 |
| Total | 9,329 | 859 | 731 | 2,602 | 21,331 | 10,554 | 45,406 |

Provides for both construction and channel maintenance requirements.
 Requirements for flood control purpose upstream of the spur channel to the Fort Worth terminus.

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- 14. The plan of improvement for the canalized multiple purpose channel to Fort Worth, Texas, proposes that public-use areas with access roads thereto, including necessary recreational facilities be provided along the route of the channel. Preliminary investigation indicates that a total of 2,350 acres of land would be required for the public-use areas and about 250 acres would be required for access roads exclusively related thereto.
- 15. The lands required in addition to the lands for the authorized channel to Liberty as modified by the recommended Wallisville Reservoir project have been included in the multiple purpose channel estimate. Lands now under perpetual easement to the Federal Government for the completed portion of the 9 X 150-foot channel to Liberty project, from the Houston Ship Channel to its upstream ending at channel mile 23.2 are sufficient for the proposed deepening of the channel to 12 feet. From channel mile 23.2 to Lock No. 1 located in the Wallisville Dam at mile 28.3, approximately 18 acres of additional rights-of-way would be required to provide for easing of a bend in the river. The bend easing was not provided for in the authorized channel project because the river channel was then considered adequate for navigation to Liberty. An additional 34 acres of rights-of-way would be required for the multiple purpose channel river cut-off alignment at Wallisville proposed in this report. Excluding the proposed bend easing and cut-off channel alignment at Wallisville, additional lands would not be required for the proposed deepening to 12 feet of the uncompleted channel from channel mile 23.2 to 35.5. Between mile 35.5 and 47.4, the authorized head of the channel to Liberty, lands would be required for the multiple purpose channel in addition to the lands to be provided under the authorized channel to Liberty project.
- 16. For the purpose of this report, it was considered that channel rights-of-way through the non-Federal Livingston Reservoir would be required for the multiple purpose channel. The areal extent of the required lands through the reservoir were determined on the same basis as other sections of the channel. The values of lands in the reservoir required for the multiple purpose channel were estimated as though the reservoir were not to be constructed by local interests.
- 17. Rights-of-way for the Dallas spur channel and turning basin are included in the rights-of-way estimate for navigation pool No. 17. Rights-of-way for the Fort Worth spur channel and turning basin are included in the estimate for navigation pool No. 21.
- 18. The channel rights-of-way and spoil areas within the Tennessee Colony Reservoir are included in the lands required for the Tennessee Colony Reservoir.
- 19. Construction spoil area requirements vary with the method of construction. Studies show that hydraulic dredging would be the most economic and practicable means of excavating the multiple purpose channel in the tidal section and pools No. 1, 2 and 5. Hydraulic spoil area requirements were computed on the consideration that hydraulic spoil can be economically placed to an average depth of 5 feet over the spoil area. This would amount to about 8,066 cubic yards per acre of spoil area.

DEST AVAILABLE COPY

- 20. The existing spoil disposal areas for the completed portion of the channel to Liberty would be used for disposition of spoil dredged in deepening the existing channel between the Houston Ship Channel and channel mile 23.2. No spoil would be placed on live oyster beds and 1500-foot openings between spoil areas would be provided at intervals of 3500 feet for the channel reach between the Houston Ship Channel and Smith Point. Spoil dredged from the channel in Wallisville Reservoir would be deposited in adjacent low areas in the reservoir. Spoil from the cut-off below the Wallisville dam and the river cut-off channel at the town of Wallisville would be deposited on 124 acres of spoil area adjoining the Wallisville Reservoir adjacent to the proposed cut-off channels.
- 21. Spoil dredged from the channel through the Livingston reservoir would be deposited in the reservoir at separated areas located a minimum distance of 500 feet from the channel. Land requirements for spoil disposal areas in the Livingston reservoir were based on the assumption that 8066 cubic yards of spoil can be placed on each acre of spoil area.
- 22. Excavation of the multiple purpose channel in pools No. 3, 4 and 6 thru 21 would be accomplished by land based dragline equipment. Below channel mile 268 the spoil would be cast into spoil areas adjacent to the channel, with the exception that some of the spoil would be used to construct river diversion dams across the upstream end of river bends severed by the proposed channel. Tand requirements for dragline spoil areas were computed on the basis that the spoil would be deposited to an average depth of 15 feet in the spoil areas or approximately 24,200 cubic yards of spoil would be placed on each acre of spoil area.
- 23. Between channel miles 286 and 322, the channel is located within existing agricultural floodway levees. In this reach the spoil would be used to fill severed river bends and low lying areas, or placed inside and adjacent to the levees to a height of about 15 feet.
- 24. Above channel mile 322, the channel would be located within existing and proposed leveed floodways. Spoil in this reach would be used for construction of new levees, filling low areas of the floodplain or used to fill severed river bends in the floodway. No construction spoil areas would be required above channel mile 322.
- 25. SUMMARY OF REQUIRED LANDS. The interest in land required for various purposes to provide for the multiple purpose channel from the Houston Ship Channel to Fort Worth, Texas, would involve about 45,406 acres summarized for the various navigation pools in table 2. A summary of the estimated first costs for the lands and damages excluding costs of public-use areas is given in table 3. Table 4 gives a summary of land requirements for public-use areas that would be provided in connection with the multiple purpose channel. The costs of these lands and damages given in table 3 and 4 totals \$11,279,480, including a contingency cost item of 25 percent and an estimated \$727,080 for acquisition costs.

| | : | | 1 | | : Access | Roads | : | | : | | | | | |
|---|---|--|--|--|---|---|---|---|--|---|--|---|---|---|
| Pool No. | : Channe : Acres | L R. O. W. | :Lock & | Dam Sites : Cost | : R. c | Cost | Construct Acres | ion Spoil : Cost | : Mainter : Acres | ance Spoil : Cost | : Severance : Acres : | Damages: Cost : | | Acquisition : Cost |
| Houston Ship Tidal 1 Total C | Channel 18 32 | \$1,800 3,200 le 0.0 to 3 | ach of | \$0(1) 0(1) | Reservoi | r (Mile 0. \$0(1) 0(1) | 0 to 35.5) | \$15,000 0(1) | : | \$0(1) 0(1) | : | \$0(1) 0(1) | \$16,800 3,200 20,000 | \$3,490 510 4,000 |
| Upper Reach | of Wallis 256 | ville Rese | rvoir to | | thorized - | Channel to | 1,715 | Mile 35.5 128,620 | to 47.4) | 0(1) | 442 | 33,150 | 187,370 | 38,610 |
| Head of Auth 2 3 3 4 5A 5B 66 7 8 9 10A Total C | 408 596 585 170 1,116 965 747 463 518 58 | 3,240 71,520 78,970 34,000 172,930 139,920 39,640 82,110 75,110 8,410 | 48 48 50 30 30 43 58 44 40 30 | 10,300 6,000 7,500 6,000 6,000 6,000 6,000 6,000 4,400 6,000 4,500 | Colony 1 6 83 15 21 - 67 79 27 72 122 | Dam (Mile 4 1,350 10,370 2,250 4,200 (3) 13,400 7,900 4,050 10,300 18,300 | 17.4 to 233 1,467 762 667 None 3,346 1,344 1,818 940 919 None | 176,040 57,150 63,360 None 434,980 154,560 172,710 126,900 None | 1,033 677 296 425 40 | (2) 34,430 58,620 7,600 (2) 118,800 63,370 39,960 46,750 4,400 | 578 1,410 1,014 None None 79 836 2,427 1,252 None | 75,140 112,500 86,190 None 13,040 83,600 351,920 112,680 None | 326,570 292,270 296,590 51,800 613,960 448,320 423,020 609,340 352,430 35,610 3,450,210 | 40,110 53,730 47,170 4,820 71,870 56,500 67,440 67,470 51,620 460 464,730 |
| Tennessee Co 10B 11 Total C | - 1 | to Lock & 1 0(4) 0(4) le 233.5 to | - | 0(4) | 3.5 to 27 | 6,600 | : | 0(4) | : | 0(4) 0(4) | | 0(4) | 0(4) 6,600 6,600 | 0(4) 700 700 |
| Lock & Dam N 12 13 14 15 16 17 Total C | 529 431 303 149 217 182 | Five-mile (124,320 13,270 45,450 52,150 103,500 106,470 le 274.4 to | 49 48 45 43 44 51 | 8,570 7,290 6,750 15,050 22,000 38,250 | 322.0) 61 23 2 46 24 6 | 9,150 5,750 600 23,000 12,000 4,500 | 648 515 356 167 257 | 106,920 107,100 74,760 62,620 96,370 (5) | 352 333 230 144 187 121 | 58,080 69,930 48,300 54,000 70,130 53,240 | 142 66 668 65 107 272 | 22,720 9,240 76,320 22,530 40,130 112,330 | 329,760 272,490 252,630 229,350 349,130 315,340 1,743,750 | 26,500 22,580 25,660 10,140 13,380 10,110 110,370 |
| Five-mile Cr | eek to an | i including | Dallas | Terminus (| Mile 322. | 0 to 326.7 | 12 - | 0(5) | 130 | 57,200 | 292 | 121,180 | 292,460 | 9,870 |
| Dallas Termi: 17 | nus to Da. | llas Floody 109,980 | my (Mil | e 326.7 to | 331.1) | | | 0(5) | 124 | 54,560 | 281 | 116,610 | 281,150 | 9,490 |
| Dellas Flood 18 | way (Mile | 331.1 to 3 | 38.8) 38 | 32,300 | 2 | 1,700 | | 0(5) | 118 | 75,400 | 26 | 15,640 | 284,990 | 5,940 |
| Dallas Flood 18 19 20 21 Total C | 104 308 267 166 | 1 including 88,400 215,600 133,500 83,000 Le 338.8 to | 37 42 41 | 25,900 21,000 20,500 | 7 22 2 | 4,900 11,000 1,000 | 52.8) | 0(5) 0(5) 0(5) 0(5) | 212 261 238 138 | 135,800 137,030 89,250 51,750 | 47 192 71 23 | 30,080 100,800 26,630 8,620 | 254,280 484,230 281,380 164,870 1,184,760 | 5,610 12,880 10,240 5,920 34,850 |
| Fort Worth Te Flood Contro. Channel | | and inclu | ding Ri | verside Dri | ve Bridge | s (Mile 36 | 6.8 to 369 | .8) | | | 244 | 91,500 | 167,000 | 6,320 |
| | Mile 0.0 | | | | | | 100 | | | | | 22,300 | 7,623,290 | 684,380 |

TABLE 3 SUMMARY OF ESTIMATED FIRST COST FOR LANDS & DAMAGES REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

Note: Prices are as of January 1962

(1) Costs of Lands for authorized channel to Liberty ad modified by the Walliaville Reservoir have been excluded.

(2) Maintenance spoil will be placed in Hydraulic spoil areas acquired for construction of channel

(3) Access roads to Loca 58 will be constructed on Lock & das rights-of-way.

(4) Lands required are within the Penneasee Colony reservoir.

(5) Spoil will be used in construction of proposed levees or placed in low areas of floodway.

TABLE 4
SUMMARY OF FIRST COST FOR LANDS AND DAMAGES REQUIRED FOR
PUBLIC-USE AREAS FOR MULTIPLE PURPOSE CHANNEL TO
FORT WORTH, TEXAS

| A SAME PROPERTY AND THE PROPERTY AND ADDRESS OF THE PARTY | | | | |
|--|----------------|------------|----------|-----------------|
| | : | • | : Unit | |
| Item | :Quantit; | y: Unit | : cost | cost |
| Houston Ship Channel to head of | Walliewille 1 | r iourasas | (Mile C | 0.0 to 35.5) |
| Public-use area | 175 | Acres | \$150 | \$26,300 |
| Access roads | 30 | Acres | 150 | 4,500 |
| Severance area | | | 150 | 500 |
| | 3 | Acres | 150 | |
| Total | | | | 31,300 3,300 |
| Acquisition costs | | | | |
| Total | | | | 34,600 |
| Head of Wallisville Reservoir to | o head of auti | norized ch | nannel t | o Liberty |
| (Mile 35.5 to 47.4) | | | | |
| Public-use area | 125 | Acres | 250 | 31,300 |
| Acquisition cost | | | | 1,900 |
| Total | | | | 33,200 |
| 20002 | | | | 55,200 |
| Head of authorized channel to L | iberty to Ten | nessee Col | lony Dan | (Mile 47.4 |
| to 233.5) | | | | |
| Public Use area | 125 | Acres | 300 | 37,500 |
| 17 | 50 | 17 | 225 | 11,300 |
| 10 | 350 | 11 | 200 | 70,000 |
| 19 | 450 | 11 | 150 | 67,500 |
| 18 | 50 | 19 | 135 | 6,800 |
| ** | 50 | 19 | 125 | 6,300 |
| Access road | 9 | 48 | 300 | 2,700 |
| it. | 21 | 17 | 200 | 4,200 |
| | 9 | 88 | 225 | 2,000 |
| 19 | 33 | ** | 150 | 5,000 |
| , | | 9• | | |
| 0 | 12 | 19 | 135 | 1,600 1.600 |
| Severance area | 0 | | 200 | |
| Total | | | | 216,500 |
| Acquisition costs | | | | 19,000 |
| Total | | | | 235,500 |
| Tennessee Colony to Lock & Dam 1 | % 12 (Mile 3 | 23.5 to 2 | 74.41 | |
| Public use area | 175 | Acres | 150 | 26,300 |
| Access road | 12 | Acres | 150 | 1,800 |
| Severance area | ī | Acres | 150 | 200 |
| Total | _ | VOTED | 1)0 | 28,300 |
| Acquisition cost | | | | 3,000 |
| Total | | | | |
| 10080 | | | | 31,300 |

TABLE 4(CONT'D) SUMMARY OF FIRST COST FOR LANDS AND DAMAGES REQUIRED FOR PUBLIC-USE AREAS FOR MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS

| | 0 0 | • | Unit | : Total |
|--|----------------|-----------|------------|------------------|
| Item | :Quantity: | | cost | cost |
| | | | | |
| Lock & Dam No. 12 to Five-mile Cre | | | | \$7 5 000 |
| Public-use area | 100 50 | Acres | \$750 | \$75,000 |
| n | 50 | ** | 500 350 | 25,000 17,500 |
| n | 50 | 11 | 300 | 15,000 |
| " | 125 | 11 | 250 | 31,300 |
| 11 | 50 | ** | 175 | 8,800 |
| n | 50 | 11 | 150 | 7,500 |
| Access roads | 24 | 17 | 750 | 18,000 |
| U | 12 | 11 | 500 | 6,000 |
| ti . | 12 | tr | 350 | 4,200 |
| 11 | 24 | 19 | 300 | 7,200 |
| 11 | 12 | " | 175 | 2,100 |
| Severance area | 10 | ** | 400 | 4,000 |
| Total | | | | 221,600 |
| Acquisition cost | | | | 9,100 |
| Total | | | | 230,700 |
| to 326.7) - Dallas Terminus to existing Dallas | | le 326.7 | to 331 | None |
| Dallas Floodway (Mile 331.1 to 338 | | | 0 | |
| Public use area | 125 | Acres | 850 | 106,500 |
| Access road | 12 | Acres | 850 | 10,200 |
| Severance area | 1 | Acres | 850 | 900 |
| Total | | | | 117,600 |
| Acquisition cost Total | | | | 2,200 |
| Dallas Floodway to & including For | + Worth Morris | inua (Mil | 228 8 | |
| 362.8) | c worder term | musimin | 330.0 | - 00 |
| Public use area | 50 | Acres | 850 | 42,500 |
| rublic use area | 150 | W | 700 | 105,000 |
| Access road | 9 | 19 | 850 | 7,700 |
| " | 21 | 11 | 700 | 14,700 |
| Severance area | 3 | 11 | 700 | 2,100 |
| Total | | | | 172,000 |
| Acquisition cost | | | | 3,700 |
| Total | | | | 175,700 |
| Fort Worth Terminus to Riverside b | ridges (Mile | 362.8 to | 369.8 |) None |
| Total (Mile 0.0 to 369.8) | | | | 860,800 |
| | | | | |

3

26. SUMMARY OF CHANNEL AND CANALS (INCLUDING COST). This section includes information regarding the design and cost of the multiple purpose channel to Fort Worth, Texas, considered in determining the cost for clearing and grubbing of the channel rights-of-way, clearing of spoil areas, snagging in river sections, removal of abandoned structures, construction of tributary inflow drop structures, channel excavation, navigation pools Nos. 5A and 10A, river diversion dams, channel bank revetment works. The cost of the multiple purpose channel work totals \$124,129,110, as shown in table 5. Plates 4 through 15 show the plan and profile of subject channel and location of locks and dams.

27. CANAL DESIGN INFORMATION. The design of the multiple purpose channel is based primarily on the requirements for navigation and flood control. Channel-size formulation studies for navigation show that a channel having dimensions of 12 feet deep and 150 feet bottom width would be the most economical for modern barge navigation required to transport the prospective commerce on the channel. Detailed channel design information is given in table 3 of appendix II, Hydrology, hydraulic design, and water resources. The channel capacities for various reaches of the multiple purpose channel from the head of Wallisville reservoir to the lower end of the Fort Worth floodway are as follows:

| Reach | :Length :in miles | : Design discharge : capacity (C.F.S.) |
|--|----------------------|---|
| Head of Wallisville reservoir to Tennessee Colony reservoir | 199.1 | 45,000 |
| Head of Tennessee Colony reserve to East Fork junction | oir 29.5 | 32,000 |
| East Fork junction to White Roci Creek junction | 22.0 | 27,000 |
| White Rock Creek junction to Elm Fork junction | 11.3 | 23,000 |
| Elm Fork junction to Fort Worth floodway | 32.5 | 15,000 |

TABLE 5
SUMMARY OF ESTIMATED FIRST COST OF MULTIPLE PURPOSE CHANNEL WORK REQUIRED FOR TRINITY SIVES CRAMMEL TO FORT WORTH

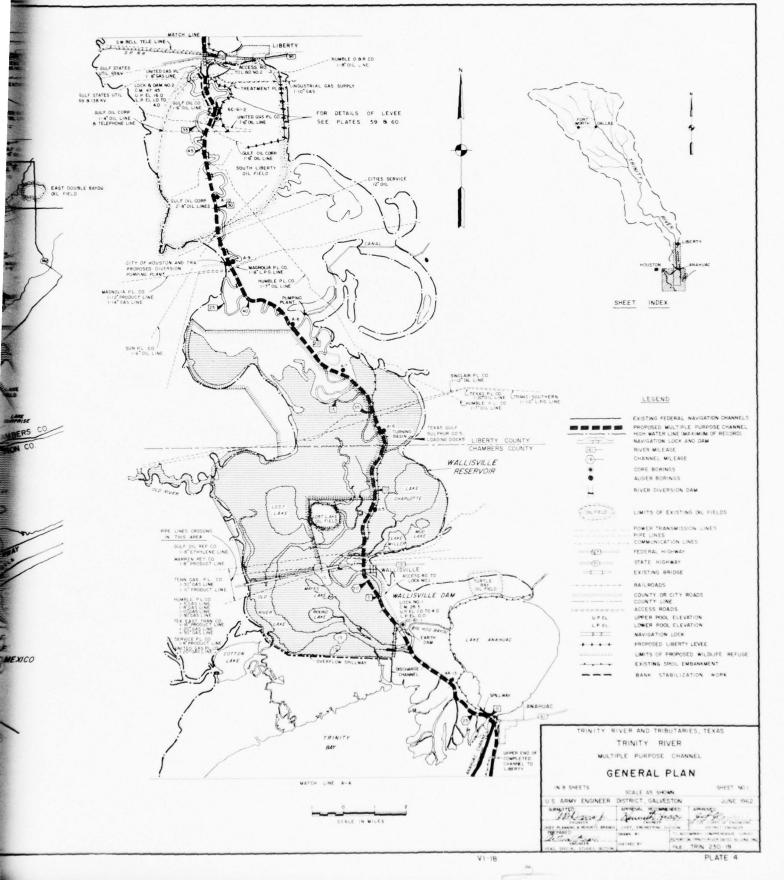
| lo1 | : 0f R. | O. W | : areas | ng of Spoil : | of | : Common Exca | VB LI On | Воек Елез | vation : | River Div | ersion : | |
|--|--|--|--|---|---|---|--|-------------------|----------------------|-------------|------------------|---|
| | | Cost : | : Acres | : Cost : | | : C.Y. : | Cout : | C. Y. : | Cost : | Quantity: | Cust : | Total rost |
| | | | | | | | | | | | | |
| uston | Ship Chan | nel to upper | | of Wallisvill | | (Mile 0.0 to 3 | | | | | | |
| GHL | - | 0(1) | - | | | 2,312,000 | | | - | - | | \$ 731,170(1 |
| 191 | | 0(1) | - | (1) | Hydraulic | 9,077,900 | 1,5=3,370() | L) - | - | - | . / | 1,3-3,370() |
| Total | I - Mile O | .0 to 35.5 | | | | | | | | | | 2,254,490(1 |
| DO 10 100 | Call Society | Tironitie D | is a response to the | to book at a | of head and as | month on them | and the same | | , | | | |
| per re | coll of wa | STH TEO | eservol. | 0(1) | uthorized ch | muncl to 1, ber 15,528,300 | P) (W) TO 321 | to 4/.4 | 2 | | | |
| | stabilizat | | | 5(1) | n, armane | C. 7.0C. 7.000 | 4,00,040 | - | | | 103,200 | 4, 240, 30(1 |
| | | .5 to 47.4 | | | | | | | | | | 330,350 |
| Tour | - Mile 37 | . > 00 -1 | | | | | | | | | | 5,176,940 |
| d of | authorize | d channel to | o Libert | v to Tennesse | e Colony days | (Mile 47.4 to | 133 47 | | | | | |
| | 351 | 122,850 | - | 0(2) | Dragline | 11,732,100 | 3,540,330 | | | 8 | 103,200 | 3,775,385 |
| | 536 | 107,200 | 762 | 76,200 | Dragline | 18,431,300 | 4,607,030 | | | 3 | 103,200 | 4,894,430 |
| | 400 | 60,000 | 667 | 50,030 | Dragline | 15,732,480 | 4,719,740 | - | | 7 | 90,300 | 4,920,070 |
| | 50 | 7,500 | | (3) | Dragline | 399,420 | 119,830 | | | | ,0,500 | 127,330 |
| | - | 0(3) | - | 0(2) | Eversulie | | 16,561,930 | | | | 51,600 | 16,613,530 |
| | 772 | 54,400 | 1,344 | 134,400 | Dragline | | 9,753,480 | | \$2,252,500 | 11 | 141,900 | 14,336,000 |
| | 642 | 160,500 | 1,010 | 74,700 | Dragline | 25,599,600 | 5.577.500 | 407,300 | 584,130 | 8 | | 9,700,410 |
| | 302 | 72,400 | 940 | 04,000 | Dragline | 22,749,860 | 7,064,080 | 1.872,500 | 2,340,630 | - 2 | 25,300 | 10,494,910 |
| | 415 | 89,000 | 919 | 91,900 | Drugline | 82,047,300 | | | | 4 | 51,600 | 7,287,640 |
| | | 10,000 | | 0(3) | Dragline | 192,500 | | | | | - | 71,600 |
| Remov | of aber | ndoned lock | at mile | 129.2 | | | | | | | | 20,000 |
| Bank | stabilizat | tion | | | | | | | | | | 1,933,400 |
| Pribu | story inch | ow drop stru | nctures | | | | | | | | | 60,000 |
| lotal | - Mile 47 | 7.4 to 233.5 | | | | | | | | | | 72,234,380 |
| | | | | | | | | | | | | |
| nesse | e Colony I | him to Lock | à Dan No | o. 12 (Mile 8) | 33.5 60 =74. | 7) - | | | | | | |
| | 887 | 310,450 | 2,234 | 446,800 | Dragline | 15,017,300 | 4,364,570 | - | - | - | - | 5,621,920 |
| | 330 | 66,000 | 344 | 34,400 | Dragline | 6,334,900 | 2,500,470 | - | | . 5 | 64,500 | 2,665,370 |
| Total | - Mile 23 | 33.5 to 274. | .4 | | | | | | | | | 8,237,290 |
| | | | | | | | | | | | | |
| | H | | | freez and t | V | | | | | | | |
| is de L | um No. 12 | | | (Mile 274.4 | | | | | 1 | | | |
| K & D | 487 | 97,400 | 546 | 64,800 | Drugline | 15,674,900 | | | 1,266,830 | 7 | 90,300 | 7,005,590 |
| K & D | 487 418 | 97,400 83,500 | 548 510 | 64,800 51,100 | Dragline Dragline | 12,337,000 | 3,701,340 | 1,013,500 | 1,266,000 | | 54,500 | 3,900,540 |
| c de D | 487 418 288 | 97,400 83,500 57,600 | 648 510 356 | 64,800 51,100 35,600 | Dragline Dragline Dragline | 12,337,000 6,607,300 | 3,701,340 | | | 7 5 4 | | 2,726,990 |
| s à l | 487 418 283 143 | 97,400 83,500 57,500 28,600 | 510 356 167 | 64,800 51,100 35,600 15,700 | Drugline Drugline Drugline Drugline | 12,337,000 6,607,300 4,042,500 | 3,701,340 2,58,190 1,212,750 | | | | 54,500 | 3,900,540 2,726,990 1,253,050 |
| c & D | 487 418 288 143 215 | 97,400 63,500 57,500 28,600 43,000 | 510 356 167 257 | 64,800 51,100 35,600 15,700 25,700 | Dragline Dragline Dragline Dragline Dragline | 12,337,000 8,607,300 4,042,900 6,25,200 | 3,701,340 2,58,190 1,212,750 2,178,820 | | | | 54,500 | 3,900,540 2,726,990 1,253,050 2,247,520 |
| | 487 418 280 143 215 164 | 97,460 63,500 57,500 28,600 43,000 52,400 | 648 510 356 167 257 | 64,800 51,100 35,600 15,700 25,700 (4) | Drugline Drugline Drugline Drugline | 12,337,000 6,607,300 4,042,500 | 3,701,340 2,58,190 1,212,750 | | | | 54,500 | 3,900,540 2,726,990 1,258,050 2,247,520 1,158,630 |
| Remov | 487 418 283 143 215 164 val of aban | 97,400 63,500 57,500 28,600 43,000 32,400 Monest lock | 546 510 356 167 257 | 64,800 51,100 35,600 15,700 25,700 (4) | Dragline Dragline Dragline Dragline Dragline | 12,337,000 8,607,300 4,042,900 6,25,200 | 3,701,340 2,58,190 1,212,750 2,178,820 | | | | 54,500 | 3,900,540 2,726,990 1,298,050 2,247,520 1,158,630 20,000 |
| Remov | 487 418 283 143 215 164 val of aban | 97,460 63,500 57,500 28,600 43,000 52,400 | 546 510 356 167 257 | 64,800 51,100 35,600 15,700 25,700 (4) | Dragline Dragline Dragline Dragline Dragline | 12,337,000 8,607,300 4,042,900 6,25,200 | 3,701,340 2,58,190 1,212,750 2,178,820 | | | | 54,500 | 3,900,540 2,726,990 1,298,050 2,247,520 1,158,630 20,000 |
| Remov Potul | 487 418 280 143 215 16 ral of aban | 97,400 63,500 57,500 28,600 43,000 32,400 idones lock | 646 510 356 167 257 61 mile | 64,000 51,100 33,600 15,700 25,700 (4) 311.0 | Drigline Dragline Dragline Dregline Drigline Dragline | 12,337,00 6,607,300 4,042,500 6,25,200 3,217,500 | 3,701,340 2,58,190 1,212,750 2,178,820 | | | | 54,500 | 3,900,540 2,726,990 1,258,050 2,247,520 1,158,630 |
| Remove Potul | 487 418 283 143 215 16 ral of aban - Mule 27 | 97,400 63,500 57,500 28,600 43,000 32,400 idoned lock r4.h to 322. | 548 510 356 167 257 61 mile | 64,000 51,100 35,600 15,700 25,700 (4) 311.0 | Dragline Dragline Dragline Dragline Dragline Dragline | 12,337,00 8,607,300 4,042,300 6,25,200 3,217,500 | 3,701,340 2,58,190 1,212,750 2,178,520 1,120,230 | | | | 54,500 | 3,900,540 2,726,990 1,258,950 2,247,580 1,138,530 20,000 13,317,320 |
| temov Potal | 487 418 283 143 215 16. ral of aban - Mule 27 s Creek to | 97,400 83,500 97,500 26,600 43,000 32,400 idones lock th.h to 322. | 648 510 356 167 257 at mile 0 | 64,000 51,100 35,500 15,700 25,700 (4) 311.0 | Drigline Dragline Dragline Dregline Drigline Dragline | 12,337,00 6,607,300 4,042,500 6,25,200 3,217,500 | 3,701,340 2,58,190 1,212,750 2,178,820 | | | | 54,500 | 3,500,540 2,726,990 1,536,950 2,247,520 1,136,630 20,000 13,317,320 |
| Removed to the second s | 187 418 280 143 215 160 ral of aban - Mule 27 2 Creek to 177 al of aban | 97,400 63,500 57,500 28,600 43,000 43,000 idoned lock th.h to 322. 6 includin 35,400 idoned lock | 648 510 356 167 257 at mile 0 | 64,000 51,100 35,500 15,700 25,700 (4) 311.0 | Dragline Dragline Dragline Dragline Dragline Dragline | 12,337,00 8,607,300 4,042,300 6,25,200 3,217,500 | 3,701,340 2,58,190 1,212,750 2,178,520 1,120,230 | | | | 54,500 | 3,900,540 2,726,990 1,98,000 2,247,520 1,158,530 20,000 13,317,30 1,323,400 20,000 |
| Remove Potul | 187 418 280 143 215 160 ral of aban - Mule 27 2 Creek to 177 al of aban | 97,400 83,500 97,500 26,600 43,000 32,400 idones lock th.h to 322. | 648 510 356 167 257 at mile 0 | 64,000 51,100 35,500 15,700 25,700 (4) 311.0 | Dragline Dragline Dragline Dragline Dragline Dragline | 12,337,00 8,607,300 4,042,300 6,25,200 3,217,500 | 3,701,340 2,58,190 1,212,750 2,178,520 1,120,230 | | | | 54,500 | 3,500,540 2,726,990 1,536,950 2,247,520 1,136,630 20,000 13,317,320 |
| Remov Potul Potul Remov Potul | 487 418 280 143 219 162 val of aban - Mule 27 E Creek to 177 ul of sban - Mile 3 | 97,400 53,500 57,500 28,600 43,000 43,000 43,000 44.4 to 322. 5 & Including 100 35,400 Moned Lock 2.0 to 325. | 648 510 356 167 257 et mile 0 nt mile | 64,300 51,100 35,500 15,700 (4) 311.0 5 Terminus (M (5) 323.0 | Drigline | 12,337,00 8,607,300 4,042,300 6,25,200 3,217,500 | 3,701,340 2,58,190 1,212,750 2,178,520 1,120,230 | | | | 54,500 | 3,900,540 2,726,990 1,98,000 2,247,520 1,158,530 20,000 13,317,30 1,323,400 20,000 |
| Remov Potul Potul Remov Potul | 487 418 280 183 219 160 18 Creek to 177 21 of sban - Mile 3 | 97,400 53,500 57,500 28,600 43,000 32,400 40 to 322. 5 preludin 35,400 40 to 325. | 548 510 356 167 257 at mile 0 at mile 7 | 64,300 51,100 35,500 15,700 (4) 311.0 5 Terminus (M (4) 323.0 | Drigline Drigline Drigline Drigline Drigline Drigline Drigline Drigline Drigline | 12,337,000 5,607,300 6,007,300 6,25,200 3,217,500 3,217,500 | 3,701,340 2,520,190 1,212,790 2,178,520 1,120,30 | | | | 54,500 | 2,900,540 2,726,990 1,538,900 2,247,520 1,535,530 20,000 13,317,320 1,533,530 20,000 1,343,220 |
| Remov Total c-mil Remov Total | 487 418 280 183 219 160 18 Creek to 177 21 of sban - Mile 3 | 97,400 53,500 57,500 28,600 43,000 43,000 43,000 44.4 to 322. 5 & Including 100 35,400 Moned Lock 2.0 to 325. | 548 510 356 167 257 at mile 0 at mile 7 | 64,300 51,100 35,500 15,700 (4) 311.0 5 Terminus (M (4) 323.0 | Drigline | 12,337,00 8,607,300 4,042,300 6,25,200 3,217,500 | 3,701,340 2,58,190 1,212,750 2,178,520 1,120,230 | | | | 54,500 | 3,900,540 2,726,990 1,98,090 2,247,520 1,158,530 20,000 13,317,50 |
| Remov Total c-mil Remov Total | 487 418 200 103 210 164 val of aban - Mile 27 s Creek to 177 al of aban - Mile 30 erminus to 130 | 97,400 53,500 57,500 28,600 43,000 32,400 43,000 32,400 43,000 50,400 6 including 35,400 40,000 20,000 | 548 510 356 167 257 at mile mg Dellan at mile .7 | 04,300 91,100 35,500 13,700 25,700 (4) 311.0 Forminus (M (5) 323.0 (4) | Drigline Drigline Drigline Drigline Drigline Drigline Drigline Drigline Drigline | 12,337,000 5,607,300 6,007,300 6,25,200 3,217,500 3,217,500 | 3,701,340 2,520,190 1,212,790 2,178,520 1,120,30 | | | | 54,500 | 2,900,540 2,726,990 1,258,900 2,247,520 1,365,530 20,000 13,347,530 1,323,530 20,000 1,343,220 |
| Remov Potul Pemov Potul Las T | 487 418 283 143 215 162 al of aban - Mile 27 8 Creek to 17 11 of sban - Mile 30 terminus to 130 loodwey (Mile 27) | 97,400 53,500 57,500 26,600 43,000 52,600 44,8 to 322. 56 including 35,400 80 including 10ck 20 to 325. 50 Dellas Flo 20,000 411 331.1 t | 548 510 356 167 257 at mile mg Dellan at mile .7 | 04,300 91,100 35,500 13,700 25,700 (4) 311.0 Forminus (M (5) 323.0 (4) | Desgline Dragline Dragline Dragline Dragline Dragline Dragline Dragline Dragline | 12,337,500 8,607,500 4,042,500 6,25,200 3,417,500 3,26.7) 5,070,450 | 3,701,340 2,58,190 1,812,750 2,178,820 1,125,830 1,287,820 | | | | 54,500 | 3,900,540 2,726,990 2,297,520 2,247,520 1,136,530 20,000 13,317,300 1,923,920 20,000 1,343,220 |
| Remov Potul Potul Remov Potul Las T | 487 418 200 103 210 164 val of aban - Mile 27 s Creek to 177 al of aban - Mile 30 erminus to 130 | 97,400 53,500 57,500 26,600 43,000 52,600 44,8 to 322. 56 including 35,400 80 including 10ck 20 to 325. 50 Dellas Flo 20,000 411 331.1 t | 548 510 356 167 257 at mile mg Dellan at mile .7 | 04,300 51,100 35,500 13,700 25,700 (4) 311.0 s Terminus (M (4) 320.0 M Le 366.7 to | Drigline Drigline Drigline Drigline Drigline Drigline Drigline Drigline Drigline | 12,337,000 5,607,300 6,007,300 6,25,200 3,217,500 3,217,500 | 3,701,340 2,520,190 1,212,790 2,178,520 1,120,30 | | | | 54,500 | 2,900,540 2,726,990 1,258,900 2,247,520 1,365,530 20,000 13,347,530 1,323,530 20,000 1,343,220 |
| Semov Potul c-mil Remov Potul Las T | hB7 h18 283 163 215 t6c ral or aban 177 er or bean 177 170 1 of aban 150 150 100 150 150 100 150 150 150 150 | 97,400 53,500 57,500 28,600 28,600 38,400 38,400 39,400 39,400 39,400 30,400 31,400 31,400 31,400 31,400 31,400 31,400 31,400 31,400 31,400 31,400 31,400 31,400 31,400 | 548 510 356 167 217 at mile on Dellar at mile on Tile 3 333.3 | 64,300 51,100 35,500 15,700 25,700 (4) 311.0 6 7 8 Terminus (M (5) 323.0 (4) (5) (6) (7) (7) (8) | Desgline | 12,337,500 6,607,300 6,042,500 6,25,200 3,217,500 3,217,500 3,60.7) 3,073,400 3,404,700 3,230,840 | 3,701,340 2,58,190 1,812,750 2,178,820 1,125,830 1,287,820 | | | | 54,500 | 3,900,540 2,726,990 2,297,520 2,247,520 1,136,530 20,000 13,317,300 1,923,920 20,000 1,343,220 |
| Semov Potul c-mil Remov Potul Las T | 487 418 286 143 215 t6 21 fte al of abar - Mile 3 27 21 of sban - Mile 3 21 of sban - Mile 3 21 of sban 136 1000te/(N | 97,400 53,500 57,500 57,500 28,600 43,000 38,400 38,400 38,400 38,400 38,400 38,400 38,400 38,400 38,400 38,500 48,100 48,000 48,000 48,000 | 548 510 356 167 217 at mile on Dellar at mile on Tile 3 333.3 | 04,300 51,100 35,500 13,700 25,700 (4) 311.0 s Terminus (M (4) 320.0 M Le 366.7 to | Desgline Dragline Dragline Dragline Dragline Dragline Dragline Dragline 331.1) Dragline Dragline Dragline | 11,337,500 5,607,300 6,042,500 6,25,600 3,417,500 3,66.7) 5,070,450 3,445,750 5,230,840 8 to 26-3) | 3,701,340 2,520,190 1,512,750 2,178,520 1,185,530 1,287,820 1,205,000 | | | | 54,500 | 1,966,540 2,746,990 1,958,690 2,247,560 2,247,560 20,000 13,317,300 1,933,500 1,933,500 1,332,000 1,332,000 |
| Remov Potal Remov Potal Las T | 487 h18 285 195 1215 t6. 215 t6. 215 t6. 21 of aban Mile 27 t1 of aban 201e 3. 21 to 150 derminus to 130 10 odiver/ (Miles 200 10 odiver) (Miles 200 200 200 200 200 200 200 200 200 20 | 97,400 83,500 87,500 82,500 84,000 32,400 44,600 44,600 33,400 400 400 400 400 400 400 400 | 548 510 356 167 217 at mile on Dellar at mile on Tile 3 333.3 | 64,300 51,100 35,500 15,700 25,700 (4) 311.0 5 Terminus (M (*) 325.0 4 Le 326.7 to (h) (h) (orth Terminus (h) | Dengline | 12,337,500 6,607,300 6,042,500 6,25,200 3,217,500 3,217,500 3,60.7) 3,073,400 3,404,700 3,230,840 | 3,701,340 2,58,190 1,812,750 2,178,820 1,125,830 1,287,820 | | | | 54,500 | 3,900,540 2,726,990 2,297,520 2,247,520 1,136,530 20,000 13,317,300 1,923,920 20,000 1,343,220 |
| Semov Potul c-mil Remov Potul Las T | h87 h18 283 163 215 t6a - Mile 27 & Creex to 177 at 1 of *bana - Mile 30 terminus to 136 to 1000se/(Miles) | 97,000 53,500 57,500 26,600 34,000 34,000 34,000 34,000 34,000 35,000 35,000 35,000 35,000 35,000 35,000 36,000 | 548 510 356 167 217 at mile on Dellar at mile on Tile 3 333.3 | 64,300 51,100 35,500 15,700 25,700 (4) 311.0 5 Terminus (M (*) 325.0 4 Le 326.7 to (h) (h) (orth Terminus (h) | Dengline Drogline Drogline Drogline Drogline Drogline Drogline Drogline 331.1) Drogline (Nile 338. | 10,337,500 8,607,500 8,002,500 6,25,200 3,417,500 3,47,500 3,445,776 3,436,840 3,503,840 3,503,840 3,603,350 | 3,701,340 2,56,190 1,21,790 2,176,620 1,185,030 1,287,820 1,205,000 1,832,690 941,280 | | | | 54,500 | 1,900,940 2,746,990 1,958,050 2,247,560 2,247,560 1,136,590 20,000 13,317,300 20,000 1,343,250 1,232,000 1,343,250 1,232,000 1,343,250 1,232,000 |
| Semove Cotal Committee of the Cotal | 467 418 285 143 215 162 217 163 177 1 of sban - Mile 37 177 1 of sban - Mile 3 6 6 150 100 100 100 100 100 100 100 | 97,400 53,500 57,500 58,500 58,500 43,000 32,400 32,400 32,400 33,400 33,400 33,400 33,400 4,100 33,400 4,10 | oth side of the si | 64,300 51,100 35,500 15,700 25,700 (4) 311.0 5 Terminus (M (5) 323.0 (4) (4) (5) (4) (6) (6) (6) (6) | Dengline | 3,445,750 3,445,750 3,445,750 3,445,750 3,455,750 3,455,750 3,455,750 3,455,750 3,455,750 3,657,350 | 3,701,340 2,56,190 1,212,750 2,178,520 1,125,30 1,205,000 1,205,000 1,832,690 941,200 2,723,000 | 34,100 | | | 54,500 | 1,900,540 2,746,990 2,947,590 2,947,590 20,000 13,347,300 1,933,280 20,000 1,343,280 1,238,000 1,836,390 943,280 2,744,600 943,280 2,744,600 |
| Semovotal Cotal Co | 467 418 285 143 215 162 217 163 177 1 of sban - Mile 37 177 1 of sban - Mile 3 6 6 150 100 100 100 100 100 100 100 | 97,000 53,500 57,500 26,600 34,000 34,000 34,000 34,000 34,000 35,000 35,000 35,000 35,000 35,000 35,000 36,000 | oth side of the si | 64,300 51,100 35,500 15,700 25,700 (4) 311.0 5 Terminus (M (*) 325.0 4 Le 326.7 to (h) (h) (orth Terminus (h) | Dengline Drogline Drogline Drogline Drogline Drogline Drogline Drogline 331.1) Drogline (Nile 338. | 3,445,730 3,647,300 6,27,300 6,27,300 3,17,500 3,47,500 3,445,730 3,435,440 3,637,300 7,78,000 3,475,000 | 3,701,390 2,56,190 2,56,190 2,176,520 1,185,530 1,287,880 1,285,600 941,280 6,763,000 6,763,000 | 34,100 | 170,000 | | 54,500 | 3,900,940 2,746,950 2,247,550 2,247,550 20,000 13,317,300 1,333,350 20,000 1,343,250 1,336,350 1,336,350 20,000 1,343,250 1,326,000 |
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| Remove Cotal Communication of the Cotal Co | has also has a second of the s | 97,400 83,500 97,500 28,600 43,000 30,400 44,6 to 322 0 & including lock 4,6 to 352, 0 & including lock 20,000 10,000 11,500 12,600 13,000 16,0 | 540 510 306 167 257 at mile 0 at mile 0 333.8, | (4) 300 51,100 35,500 13,700 25,700 (4) 311.0 321.0 (5) 322.0 (6) 322.0 (7) (6) (7) (7) (8) (8) (8) (8) (8) (8) (8) (8) (8) (8 | Dengline Dragline Dragline Dragline Dragline Dragline Drasline Drasline 331.1) Dragline Dragline Jugline Jugline Jugline "" | 3,445,730 3,647,300 6,27,300 6,27,300 3,17,500 3,47,500 3,445,730 3,435,440 3,637,300 7,78,000 3,475,000 | 3,701,340 1,50,190 1,70,200 1,73,540 1,125,500 1,267,340 1,265,000 1,832,500 1,832,500 1,905,100 1,907,390 | 34,100 | 170,000 | | 54,500 | 1,900,540 2,76,90 2,76,90 1,193,60 2,247,50 20,000 13,317,30 20,000 1,343,20 20,000 1,343,20 1,232,000 1,343,20 1,232,000 1,343,20 1,232,000 1,343,20 1,241,00 3,177,710 1,714,00 3,177,710 1,714,00 |
| Gemove and a control of the control | her has been also been als | 97,400 83,500 97,500 98,400 43,000 38,400 44,4 to 332 6 recluded lock 2.0 to 345, 9 Dellas Fle 76,000 8 holustines 1,000 4,500 34,500 34,500 34,500 34,500 6,310 6,310 6,50 | 540 510 306 167 257 at mile 0 at mile 0 333.8, | (4) 300 51,100 31,500 10,700 25,700 (4) 311.0 (5) 323.0 (4) 323.0 (5) (5) (6) (6) (7) (7) (7) (7) (7) (7) (8) (8) (8) (8) (8) (8) (8) (8) (8) (8 | Dengline Dragline Dragline Dragline Dragline Dragline Drasline Drasline 331.1) Dragline Dragline Jugline Jugline Jugline "" | 3,445,750 3,445,950 3,445,950 3,417,500 3,417,500 3,445,750 3,45,750 3,45,750 3,45,750 3,477,500 3,477,500 3,477,500 3,477,500 3,477,500 | 3,701,340 1,50,190 1,70,270 1,71,540 1,125,500 1,267,340 1,265,000 1,832,500 1,832,500 1,905,100 1,907,390 | 34,100 | 170,000 | | 64,500 51,800 | 3,966,940 2,766,990 1,195,090 2,247,590 2,247,590 20,000 13,317,380 20,000 1,343,280 20,000 1,343,280 1,832,000 1,83 |
| Remove of the February Formation of Countries of Countrie | 487 h18 283 215 16 al of uban Mile 27 177 al of sban- 136 locate/ (N 210 210 210 210 210 210 211 211 211 211 | 97,400 83,500 97,500 98,400 43,000 38,400 44,4 to 332 6 recluded lock 2.0 to 345, 9 Dellas Fle 76,000 8 holustines 1,000 4,500 34,500 34,500 34,500 34,500 6,310 6,310 6,50 | 540 510 306 167 257 at mile 0 at mile 0 333.8, | (4) 300 51,100 35,500 13,700 25,700 (4) 311.0 321.0 (5) 322.0 (6) 322.0 (7) (6) (7) (7) (8) (8) (8) (8) (8) (8) (8) (8) (8) (8 | Dengline Dragline Dragline Dragline Dragline Dragline Dragline Dragline 331.1) Bragline Dragline Jungline Jungline Jungline Jungline Jungline Jungline | 3,445,750 3,445,950 3,445,950 3,417,500 3,417,500 3,445,750 3,45,750 3,45,750 3,45,750 3,477,500 3,477,500 3,477,500 3,477,500 3,477,500 | 3,701,340 -5,56,190 -5,56,190 -5,56,190 -1,76,500 1,267,340 1,267,340 1,265,000 941,280 -5,50 | 3y,100 4e1,-30 | 175, 800 She, 800 | | 64,500 51,800 | 3,966,546 2,766,990 1,956,090 2,247,530 20,000 13,317,380 1,933,280 20,000 1,343,280 20,000 1,343,280 1,836,390 04,3,280 2,744,600 3,170,710 2,724,280 |

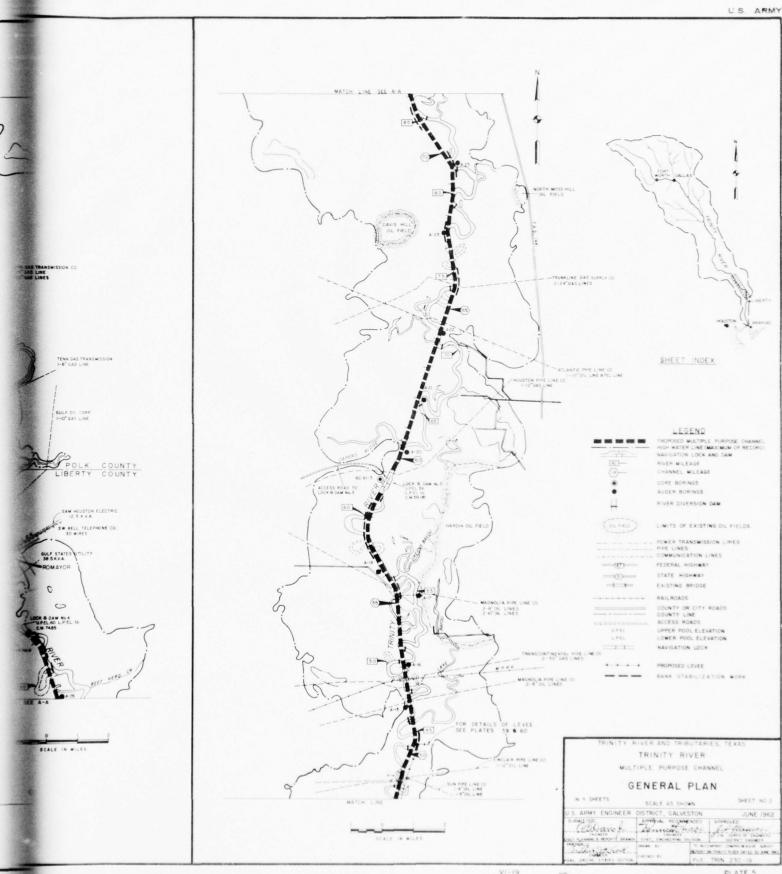
(1) Limin for uncompletes portion of authorized channel to Liberty are excluded.

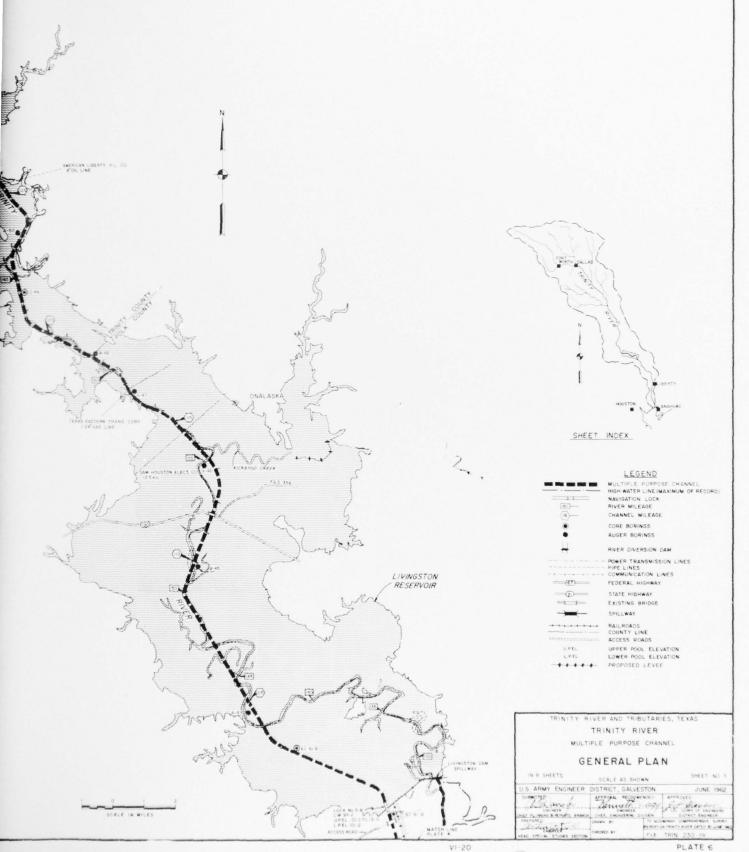
(4) We clearing necessary due to Mylmodic dredging.

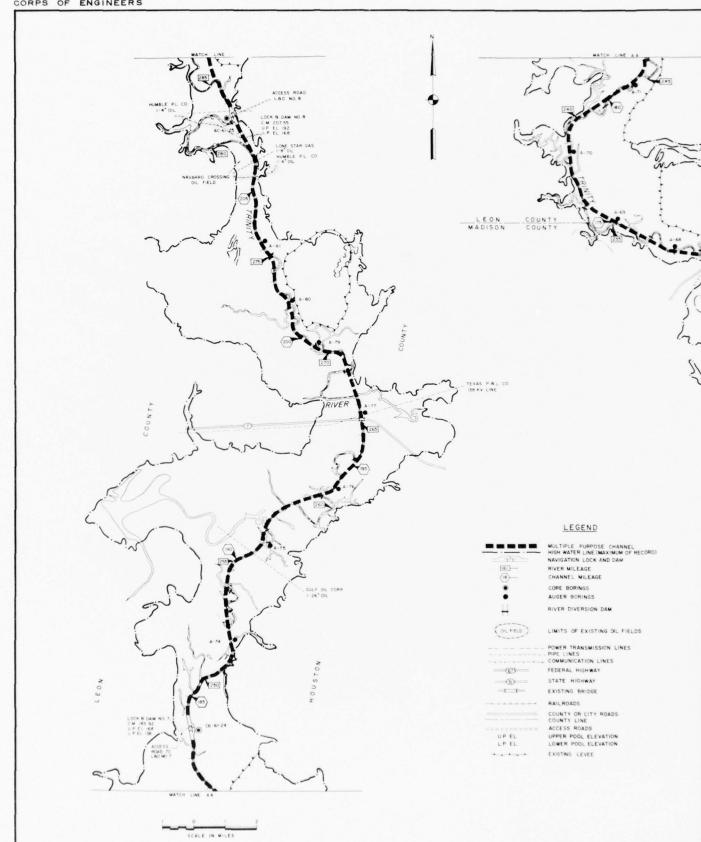
(3) Clearing and grabbing are included in cost of dredging.

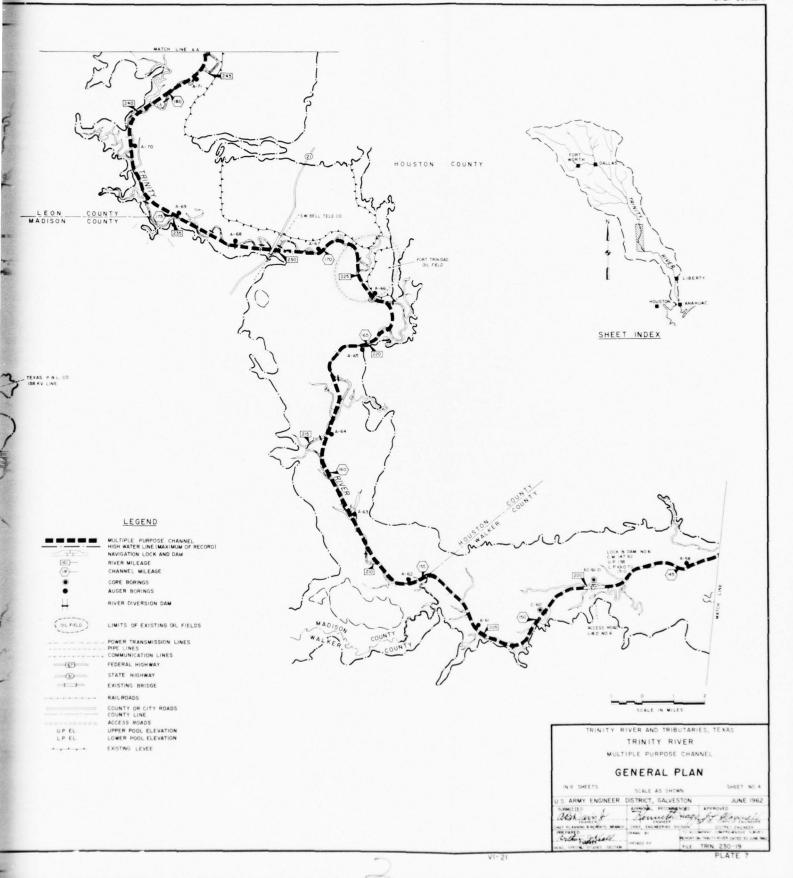
(4) Based on using spoil for cost raction of new levees and/or for spoil is river bend out-offs.

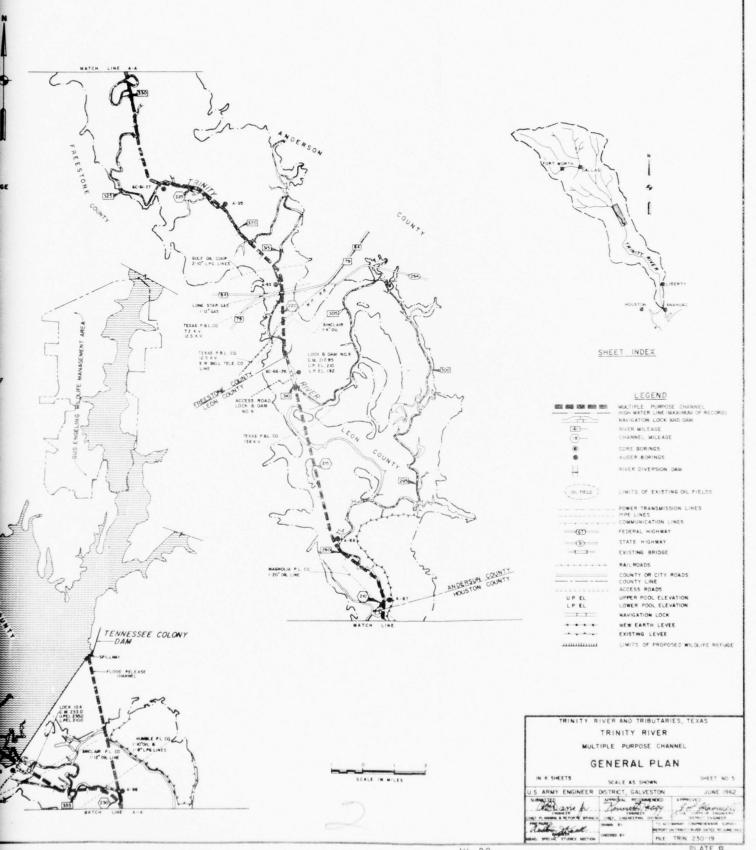


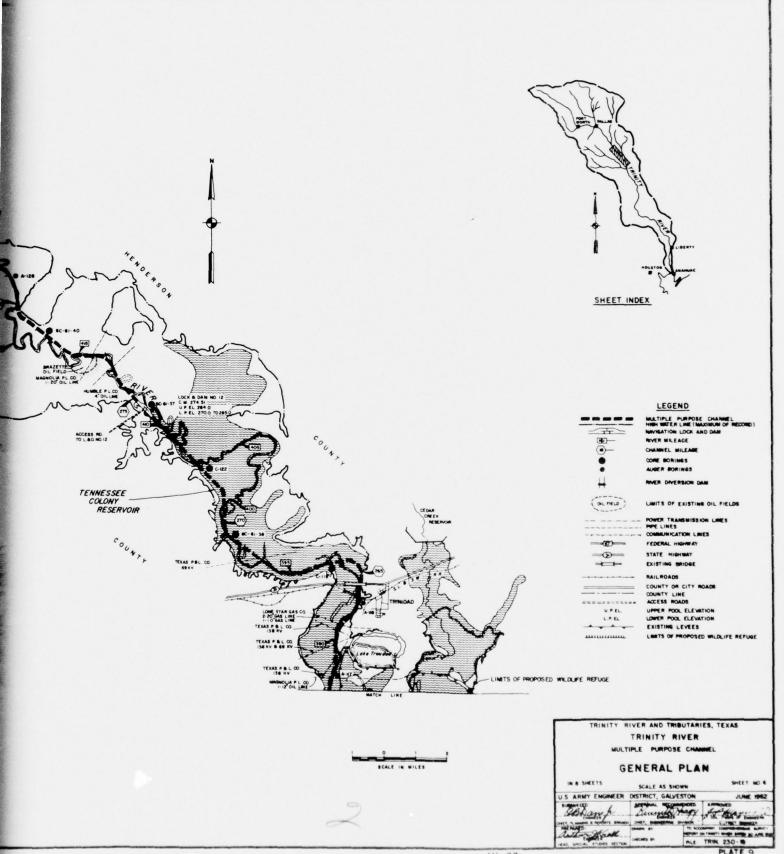


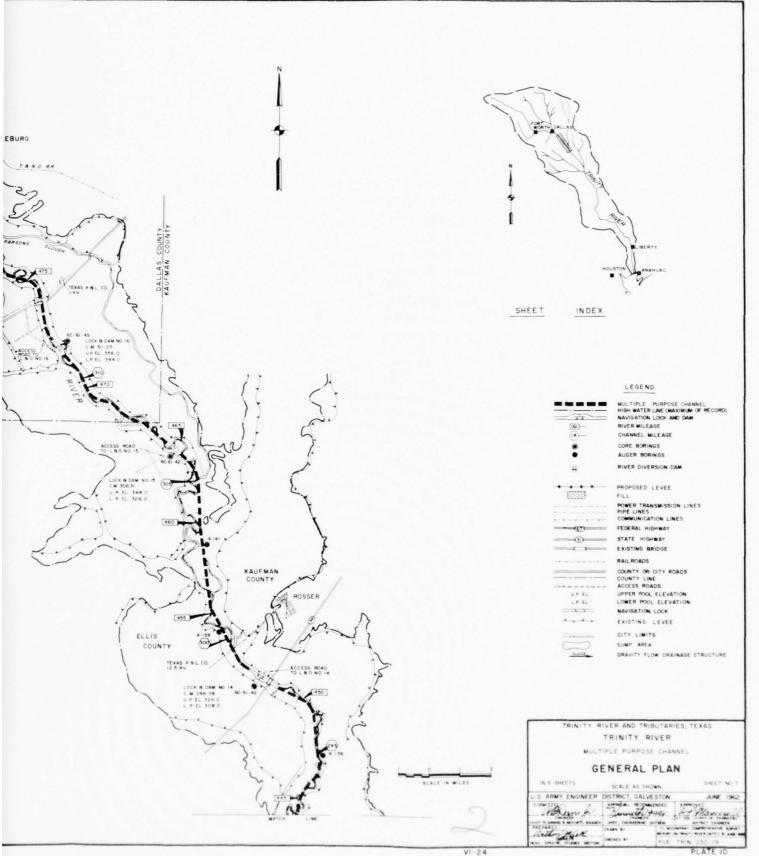


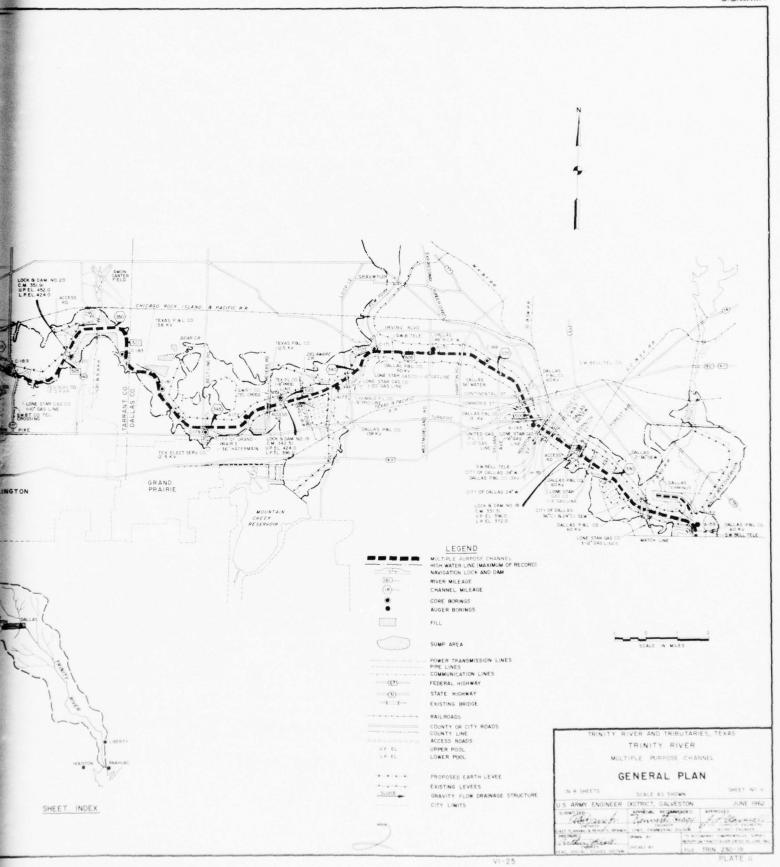


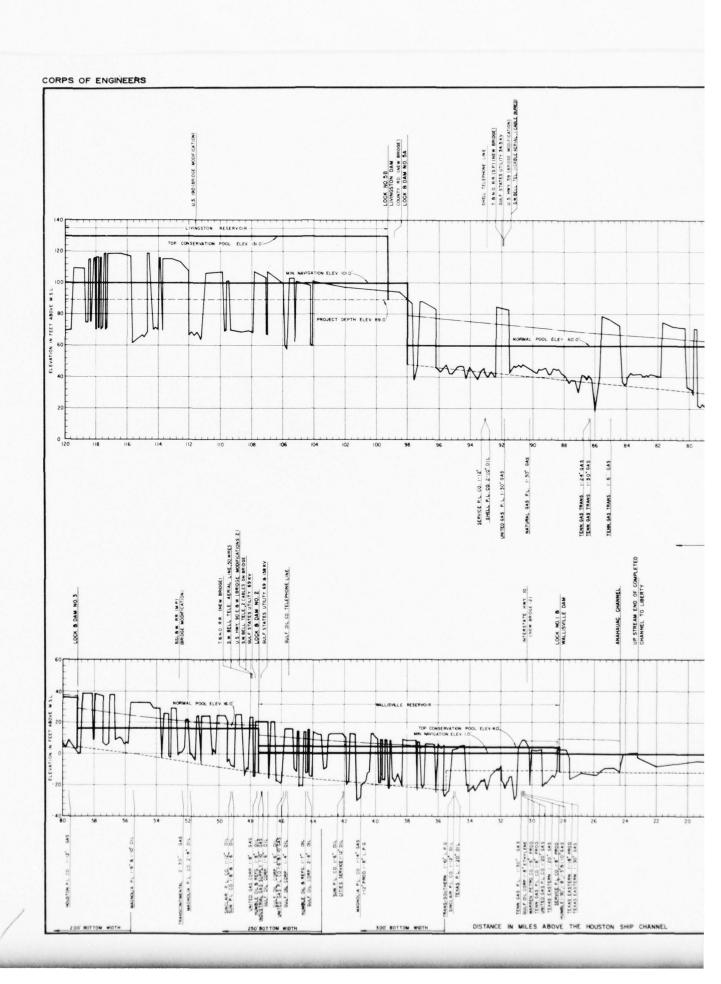


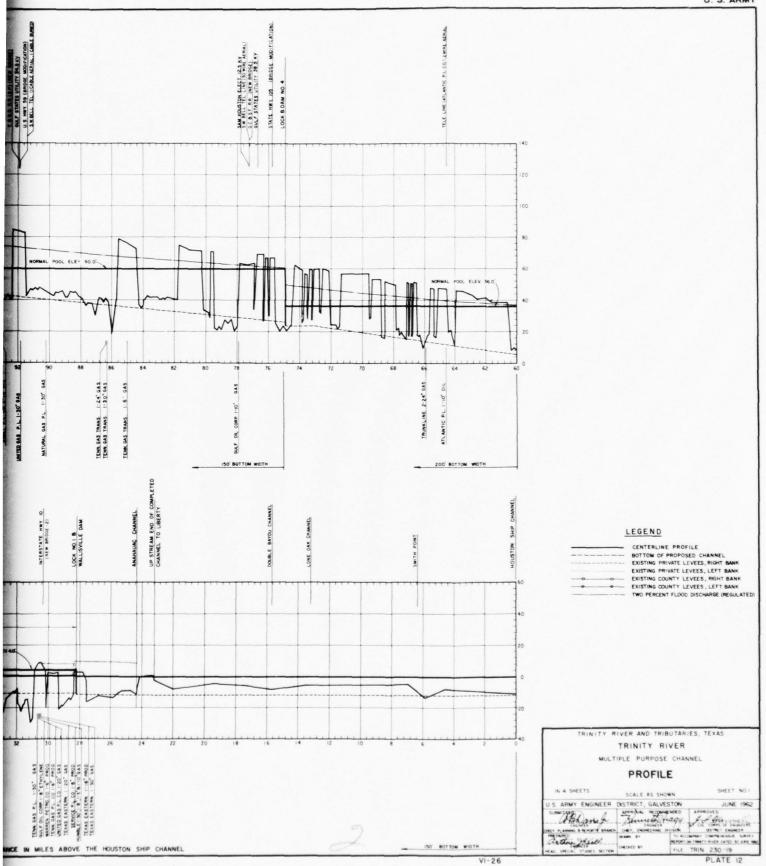


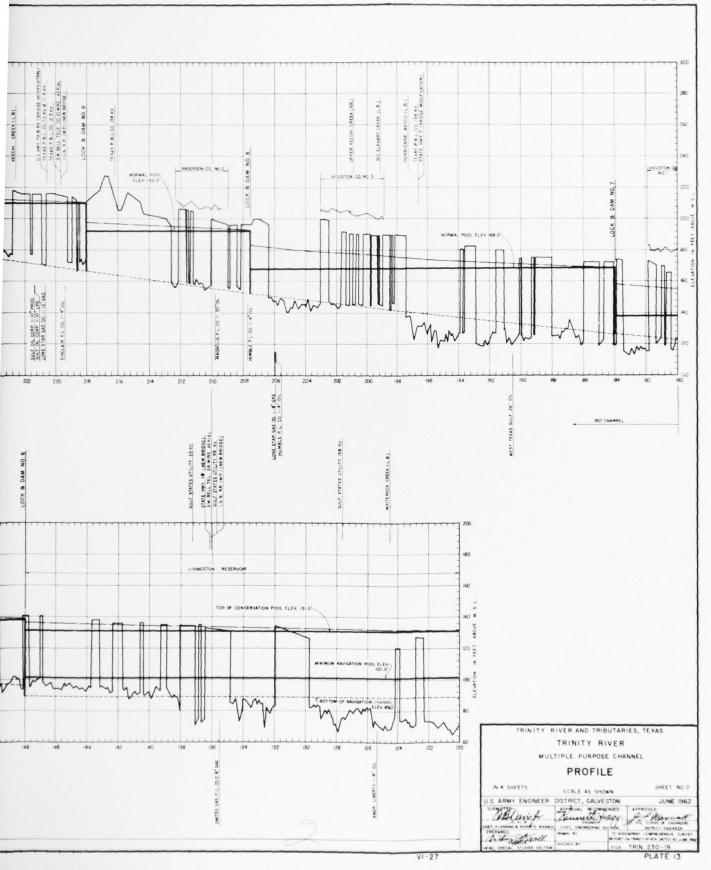


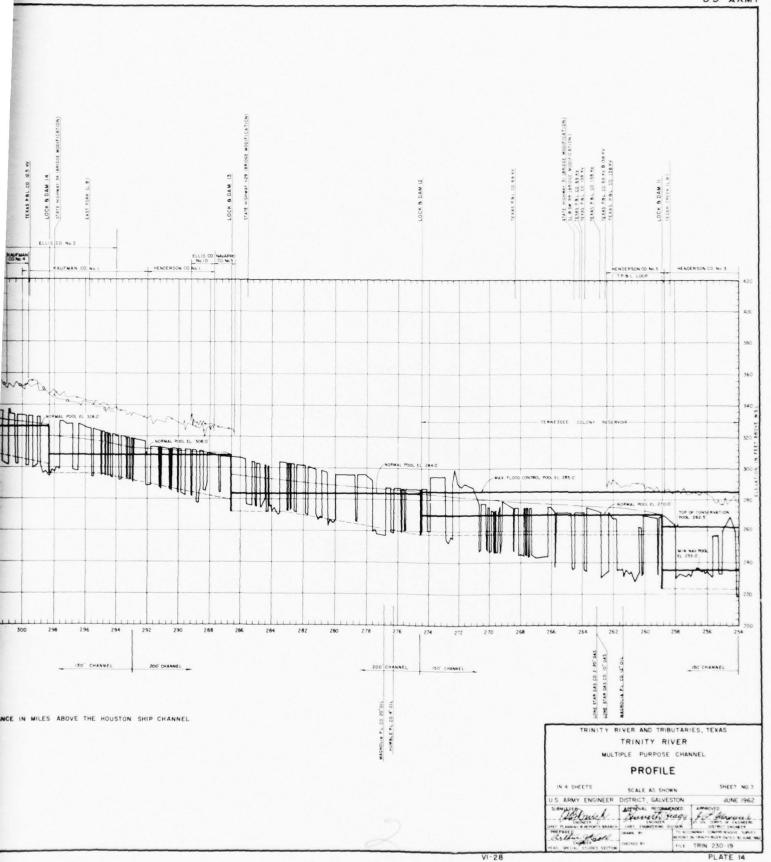


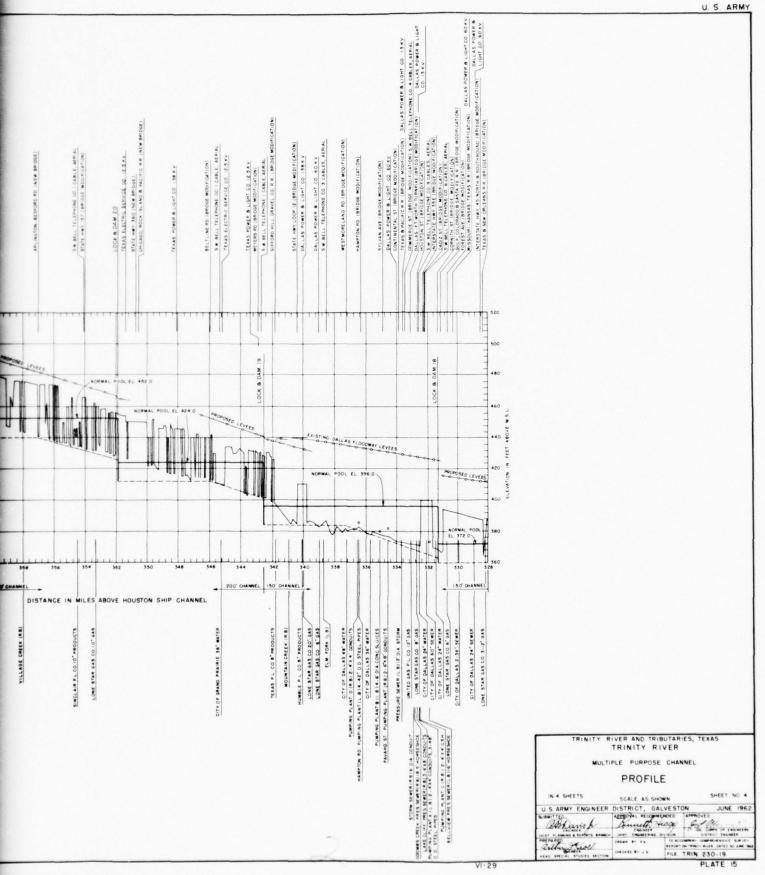








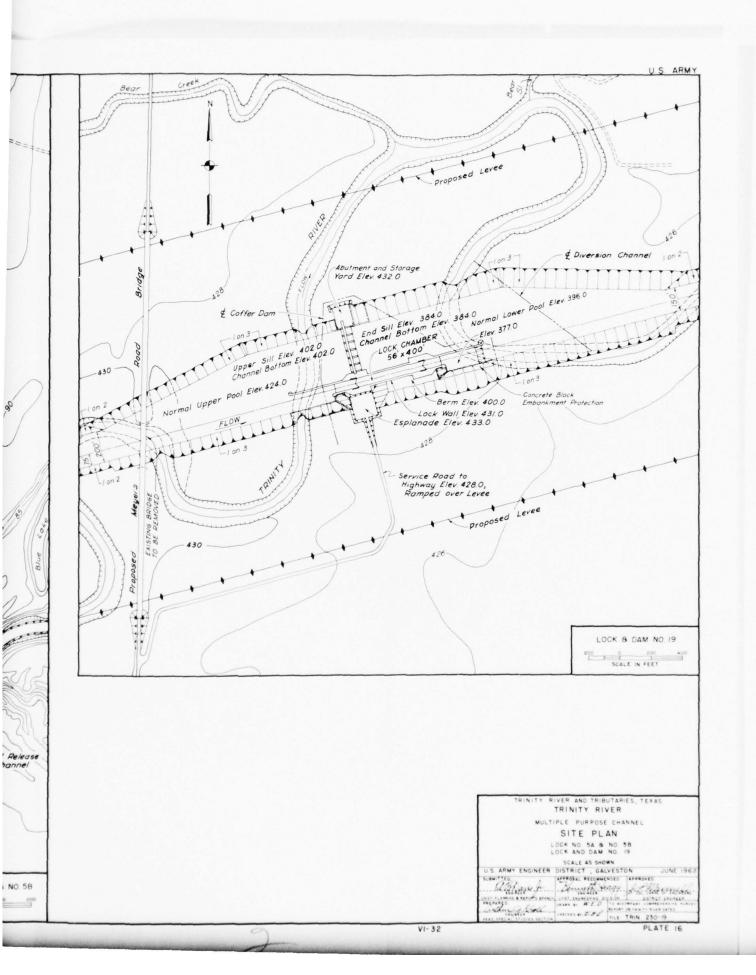




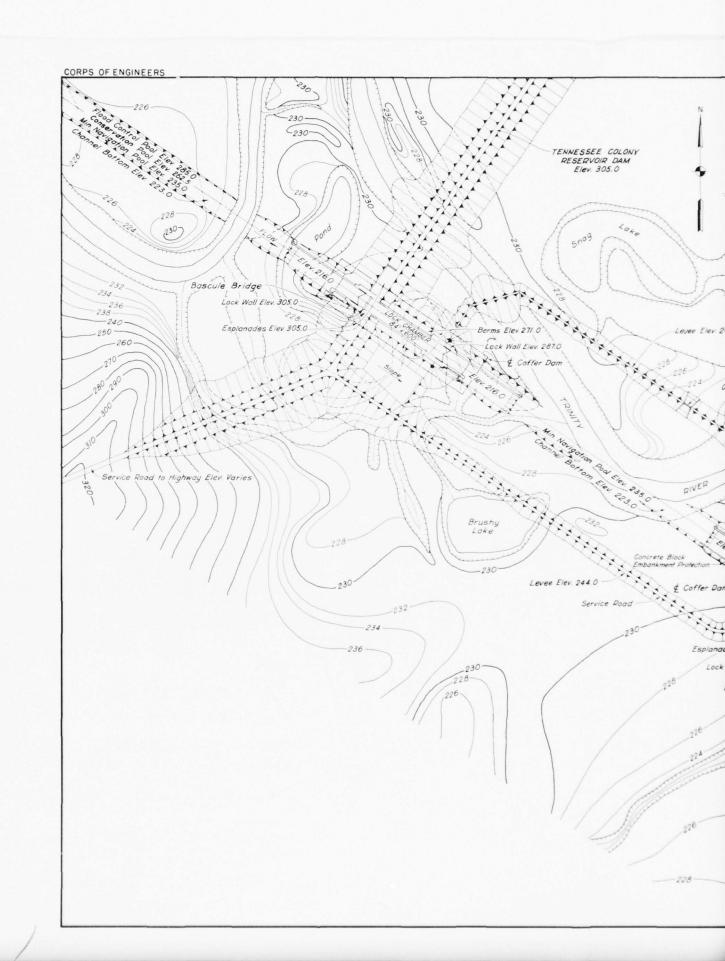
- 28. The plan of improvement provides for modification of the authorized channel to Liberty project by deepening to 12 feet the existing and uncompleted portion of the 9 × 150-foot project channel from the Houston Ship Channel to channel mile 35.5 at the upper limits of the recommended Wallisville Reservoir. In this reach the plan provided for realignment of the channel to Liberty project in the vicinity of channel mile 25.8 to provide a channel 12 feet deep and 150 feet wide across a bend in the river to meet the minimum channel alignment criteria of 2500-foot radius at adjoining tangent reaches. Curve easing of the river channel for navigation to Liberty was not provided for in connection with the channel to Liberty project. The plan also proposes a major river bend cut-off at Wallisville, Texas, extending between channel miles 29.3 and 31.2 as shown on plate 4, and provides for a channel 12 feet deep below elevation 1.0 and 150 feet wide on the proposed river cut-off alignment. The Wallisville Reservoir project provides for conservation storage between elevation 1.0 to 4.0 above mean sea level, which constitutes a modification of the authorized sea-level channel to Liberty by establishing the minimum elevation of 1.0 for the project channel from the Wallisville dam at channel mile 28.3 to the head of authorized project at channel mile 47.4.
- 29. At channel mile 35.5, the proposed 12 × 150-foot uncompleted channel to Liberty project joins with the proposed multiple purpose channel which would have a width of 300 feet at bottom elevation minus 23. Below mile 35.5, the proposed 12 × 150-foot channel to Liberty is located generally in the Trinity River channel and no enlargement of the Trinity River is proposed. Flows from the multiple purpose channel at mile 35.5 would discharge into and flow in the Trinity River channel through the Wallisville Reservoir to a gate controlled river diversion channel leading to Trinity Bay, as shown on plate 4.
- 30. The multiple purpose channel upstream of channel mile 35.5 to the Tennessee Colony Dam would have a capacity to pass a maximum discharge of 45,000 cubic feet per second. The bottom width of the channel would be decreased to a minimum width of 150 feet at the Livingston Dam, and upstream thereof excepting several reaches where the channel would have a bottom width of 200 feet. The plan of improvement proposes that the multiple purpose channel be formed at channel mile 96.94 below the Livingston Reservoir Dam by the joining of the flood release channel extending from the spillway near the east end of the Livingston Dam and the navigation channel extending from the west end of the dam as shown on plates 5 and 6. The plan provides for the navigation channel, 12 feet deep and 150-foot bottom width at elevation 89 to extend throughout the Livingston Reservoir to lock and dam No. 6 at channel mile 147.92, where it would join with the multiple purpose channel.
- 31. A similar divergence of channels would be provided below the Tennessee Colony Dam at channel mile 229.70 as shown on plate 8. The navigation channel, 12 feet deep and 150-foot bottom at elevation 223 would extend throughout the conservation pool of the Tennessee Colony Reservoir, thence, 12 feet deep and 150-foot bottom at elevation 258 throughout the navigation pool No. 11 in the flood storage pool of the Tennessee Colony Reservoir to look and dam No. 12 at channel mile 274.5 where it would join with the multiple purpose channel.

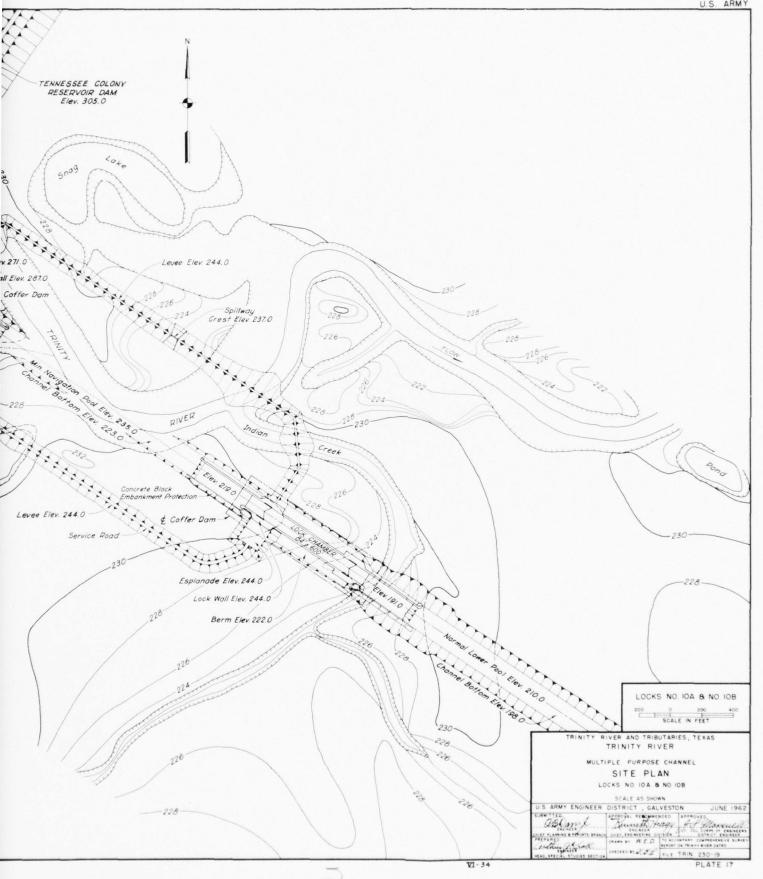
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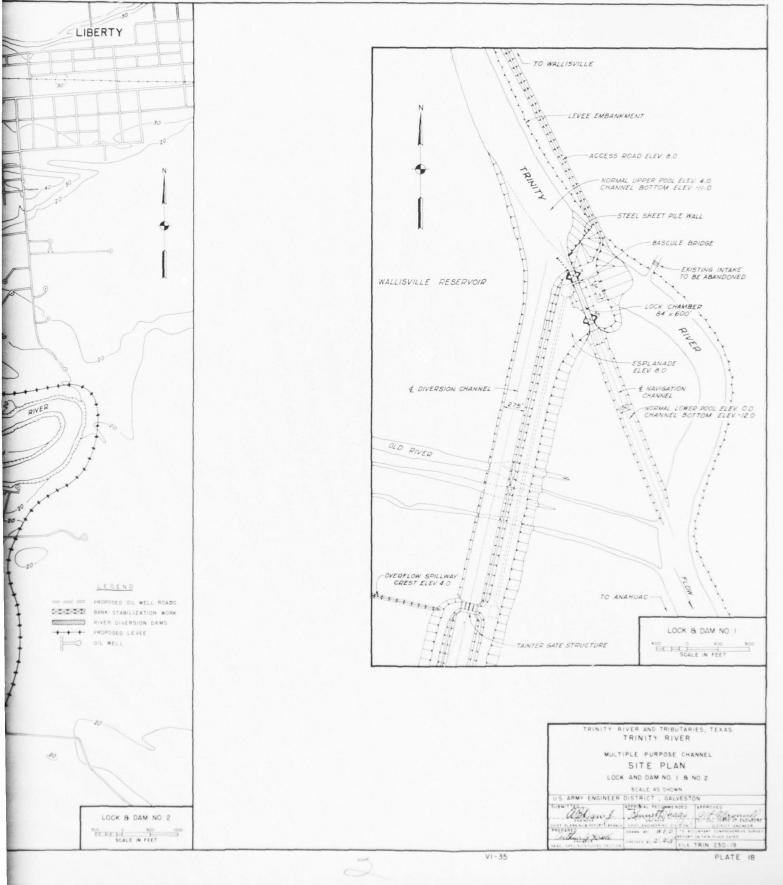
- 32. The plan provides for spur channels, 12 feet deep by 150 feet wide, extending to and including turning basin, 400 feet square, at Dallas and at Fort Worth. A floodway channel with no provisions for navigation would extend from channel mile 362.8 upstream to the lower end of the Fort Worth floodway extension at Riverside Drive bridges over the West Fork.
- 33. NAVIGATION POOLS NOS. 5A & 10A. The plan of improvement for the multiple purpose channel proposes two locks to overcome a total lift of 71 feet at the Livingston Reservoir dam to provide for 100 percent navigation through the reservoir when conservation storage is full to elevation 131.0 or depleted to elevation 101.0. Preliminary subsurface investigation indicates that foundation conditions are inadequate to support adjoining tandem locks and furthermore the tandem locks would cause delays in transits of barge tows through the locks. The plan provides for the upper lock No. 5B to be located in the Livingston Dam and the lower lock No. 5A to be located about 6300 feet downstream of the upper lock, with an earth levee extending between the locks to form navigation pool No. 5A, generally as shown on plate 16 on page 32.
- 34. The levee would have a top elevation of 110 and be constructed of channel spoil material. A service road would extend on the levee to both locks. In conjunction with the side hills on the opposite side of the channel the levee would create a small reservoir providing a minimum navigation elevation of 101.0, and a maximum elevation of 103.0. Storage between these elvations would provide a water supply for adverse lock operation that may occur at lock No. 5A. When conservation storage in Livingston Reservoir is depleted to elevation 103, the gates of lock No. 5B would remain open and barge tows would be able to traverse pools No. 5A and 5B without delay to navigation. During the drought period when the gates of lock No. 5B are open, water supply for operation of lock No. 5A would be drawn from the Livingston Reservoir.
- 35. The reservoir would average about 1200 feet in width and have a length of about 6000 feet. The navigation channel would be 12 feet deep below elevation 101 and have a bottom width of 150 feet. It is considered that barge tows would not encounter any unusual delays in navigating locks Nos. 5A and 5B and the intervening navigation pool No. 5A.
- 36. The reservoir would serve to dampen any adverse surges resulting from the emptying of lock No. 5B. It would also provide temporary storage for run-off from the side hill drainage area of about 630 acres until such storage could be released through lock No. 5A. The cost to shape the spoil materials into graded levee form is provided for in the cost of channel excavation.



- 37. The plan provides for navigation pool No. 10A, to be contained within a leveed reservoir area as shown on plate 17. The reservoir would be about 1100 feet wide and 2600 feet long providing storage to a minimum navigation elevation of 235 and a maximum elevation of 237. Storage between these elevations would provide a water supply for adverse lock operation that may occur at lock No. 10A. When conservation storage in Tennessee Colony Reservoir is depleted to elevation 237, the gates in lock No. 10B would remain open and barge tows would be able to traverse pools 10A and 10B without delay. During the drought period when the gates of lock No. 10B are open, water supply for operation of lock No. 10A would be drawn from the Tennessee Colony Reservoir.
- 38. The proposed reservoir would be constructed of channel spoil materials to a top elevation of 244 and of sufficient width to dispose of the spoil materials. The navigation channel would be 12 feet deep below elevation 235 and have a bottom width of 150 feet. The reservoir would be of variable depth, averaging about 10 feet deep below elevation 235, and would serve to dampen any adverse surges resulting from the emptying of lock 10B under all conditions of storage in the Tennessee Colony Reservoir.
- 39. DIVERSION DAMS. Eighty-one dams would be required for the multiple purpose channel. Construction of each dam would require approximately 86,000 cubic yards of channel spoil material. River diversion dam costs include the cost of hauling and placing spoil in the dam. The estimate is based on a unit price of \$0.15 per cubic yard. Top of dams would be constructed to the elevation of the flood of record. Dams will have a length of two hundred feet greater than the distance between tops of bank of the river measured along the alignment of the dam. Plates 4 through 10 show the location of the proposed river diversion dams.
- 40. CHANNEL BANK REVETMENT WORKS.- Based on the analysis of channel bank revetment works considered for the multiple purpose channel as discussed in appendix III, it is proposed that the outside banks of the sharper curves and adjoining short length of tangents on each end of the curves be protected to prevent erosion of the channel banks. The protection would consist of 24-inch thickness of quarry-run stone ranging from one-half inch to 200 pound stone extending from two feet above normal pool elevation to bottom gradient of proposed channel. The channel from mile 45 to lock and dam No. 2 traverses a reach of the river that is now actively eroding the river banks. Extensive bank revetment works in this reach are proposed extending for a distance of about a mile below lock & dam No. 2, as shown on plate 18. Plates 4 and 5 show the locations of all proposed bank revetment works.







41. SUMMARY OF NAVIGATION LOCKS (INCLUDING COST).- The plan of improvement for the multiple purpose channel provides for construction of 22 navigation locks in addition to the lock provided for in the recommended Wallisville Reservoir project to serve in conjunction with the proposed navigation dams and the Wallisville, Livingston and Tennessee Colony Reservoirs to provide for navigation to Fort Worth, Texas. The locations of the proposed locks are shown on general plans, plates 4 thru 11, and in profile on plates 12 thru 15, page 18 to 29. The first cost of the proposed 22 locks is estimated at \$181,705,560, as shown in table 6.

TABLE 6
SUMMARY OF FIRST COST OF NAVIGATION LOCKS
REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL
TO FORT WORTH, TEXAS

| Lock: Size of : Normal: number : lock (ft.) : lift (ft.) : Cost Houston Ship Channel to upper reach of Wallisville Reservoir (Mile 0.0 to 35.5) 1 84 × 600 4 (1) |
|--|
| Houston Ship Channel to upper reach of Wallisville Reservoir (Mile 0.0 to 35.5) |
| to 35.5) |
| |
| 1 84 × 600 4 (1) |
| |
| Upper reach of Wallisville Reservoir to head of authorized channel to |
| Liberty (Mile 35.5 to 47.4) None |
| Emiliare dum Continues and Continues and provide in the Continues and American |
| Head of authorized channel to Liberty to Tennessee Colony Dam (Mile 47.4 |
| to 233.5) |
| 2 84 × 600 12 \$ 8,240,500 |
| 3 84 × 600 20 8,885,720 4 84 × 600 24 9,575,610 |
| ****** |
| 5A 84 × 600 41 9,673,700 |
| 5B 84 × 600 30 11,328,120 |
| 6 84 × 600 7 8,757,490 |
| 7 84 × 600 30 9,338,030 8 84 × 600 24 8,580,720 |
| 8 84 × 600 24 8,580,720 |
| 9 84 × 600 18 7,504,280 |
| 10A 84 × 600 25 8,818,400 |
| Total 90,702,570 |
| Tennessee Colony Dam to lock & dam No. 12 (Mile 233.5 to 274.4) |
| 10B 84 × 600 27.5 10,424,090 |
| 11 84 × 600 7.5 9,521,970 |
| Total 19,946,060 |
| Lock & dam No. 12 to Five-mile Creek (Mile 274.4 to 322.0) |
| 12 84 × 600 14 7,170,470 |
| 13 84 × 600 24 8,801,889 |
| 14 84 × 600 18 7,973,280 |
| 15 84 × 600 18 7,563,650 |
| 16 84 × 600 12 6,546,180 |
| 17 84 × 600 16 7,042,610 |
| Total 45,098,070 |

TABLE 6 (CONT'D)

SUMMARY OF FIRST COST OF NAVIGATION LOCKS REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

| Lock | : | Size of | : | Normal | : | |
|-----------|------------|---------------------|---------|------------|-------------|------------|
| number | : | lock (ft.) | : | lift (ft. |): | Cost |
| Five-mile | Creek to | Dallas terminus (Mi | le 322. | .0 to 326. | 7) - | None |
| Dallas te | rminus to | Dallas floodway (Mi | le 326. | 7 to 331. | <u>L)</u> - | None |
| | oodway (mi | le 331.1 to 338.8) | | | | |
| 18 | | 56 x 400 | | 24 | \$5 | ,973,800 |
| Dallas fl | oodway to | Fort Worth terminus | (Mile | 338.8 to | 362.8) | |
| 19 | | 56 X 400 | | 28 | 6 | ,353,500 |
| 20 | | 56 x 400 | | 28 | 6 | ,859,340 |
| 21 | | 56 x 400 | | 28 | 6 | ,772,220 |
| | Total | | | | | ,985,060 |
| Fort Wort | h terminus | to and including F | iversid | de bridges | | |
| 369.8). | - | | | | | None |
| | | Mile 0.0 to 369.8 | | | 1 - 0 | 31,705,560 |

Note: Estimated first costs are based on prices as of January 1962.

(1) Costs for a lock 84 feet wide by 600 feet long are included in Wallisville reservoir project recommended in separate report.

- 42. LOCK DESIGN INFORMATION.- The lock sizes selected for the proposed multiple purpose channel are based on a project formulation lock-size study, given in appendix III. The study considered the size of barges and probable make-up of the barge tows that would be used for movement of the prospective commerce on the channel, the water demand of the locks on the available water supply for lockages and the lockage time required for transits of the standard barge tows through the locks. The study shows that locks with clear basin dimensions of 84 feet wide by 600 feet long would be the most feasible lock size for transits of tows moving on the channel below Dallas, and that locks with clear basin dimensions of 56 feet wide by 400 feet long would be most feasible for the channel upstream of Dallas.
- 43. A lock 56 feet wide by 400 feet long is recommended for Wallisville Dam in the Chief of Engineers report contained in House Document No. 215, 87th Congress, 1st Session, to serve the needs of navigation to Liberty, Texas, with such modification thereof as in the discretion of the Chief of Engineers may be advisable, including a large lock if found justified. The project formulation lock-size study shows that the prior recommended 56- x 400-foot lock would not serve the needs of prospective commerce on the multiple purpose channel at the Wallisville Reservoir Dam, and that a 84- x 600-foot lock would be most feasible at the Wallisville Dam. Accordingly, the plan of improvement for the multiple purpose channel provides for an 84- x 600 foot

multiple purpose channel provides for an 84- x 600-foot lock at the Wallisville Dam. The cost of the recommended Wallisville Reservoir project is estimated at \$9,162,000, which includes the cost of providing a lock, 84 feet wide by 600 feet long, in the Wallisville Dam, as set forth in the report of the District Engineer, accompanying House Document No. 215, 87th Congress, 1st Session.

- 44. The plan of improvement proposes that 18 locks be of the Arkansas River gravity wall type design, and that four locks, Nos. 5A, 6, 10B and 11, be of the U-frame type design. Pertinent data regarding the proposed navigation locks are given in table 7. Plates 19 and 20 show a plan layout and sections of a typical gravity type lock, having clear basin dimensions of 84 feet wide by 600 feet long. Locks providing clear dimensions of 56 feet wide by 400 feet long are of similar design as shown on plates 19 and 20. Plates 21 and 22 show a plan layout and sections of the U-frame type lock 10B, which illustrates the typical features of the other U-frame type locks Nos.5A, 6 and 11. Plates 23 and 24 show the log of borings made at the lock sites which were considered in determining the foundation requirements of the respective locks. The plan proposes that the Arkansas River type of locks be founded on steel bearing piles, battered in two directions to withstand lateral loads. With respect to the "U"-frame type of lock, the design does not preclude the use of piling, should detail foundation explorations show the need for such piling. Available data, however, does not indicate the need for such piling and they are not provided in the design. To reduce seepage losses under and around the lock structures, adequate steel sheet-pile walls, 20 feet maximum in length have been provided.
- 45. The requirements for diversion and care of water during construction of the several locks and dams varies at the several sites because of topographic differences, character of subsurface materials, and frequency of high river flows. Based on preliminary investigation, it is considered that earth levee coffer dams providing protection against a flood having a minimum frequency of occurrence of once in 10 years, would be adequate to serve the needs of estimating the cost of coffer dam construction for this report.
- 46. Typical illustrations of the extent of coffer dams considered in determining the estimated first cost of the proposed Trinity River locks and dams are shown on site maps as follows:

Lock and Dams No. 1 and No. 2, plate 18, page 35. Lock No. 5B and Lock and Dam No. 19, plate 16, page 32. Lock No. 10A and 10B, plate 17, page 34. Lock and Dam Nos. 6 and 11, plate 25, page 46.

PERFORMENT DATA - NAVIGATION LOCKS MULTIPLE PURPOSE THOUTH RIVER CHANNEL TO FORT WORTH, TEXAS

1

| Lock | | Channel | : Normal (MSL) : Pool elevation | (MSL) | Lift | : navigation star | esign n stage | : top of | f sabla- | board : | of wall | Type | | Thomas of adliba | : Height of | f Lock : | Top elevati | ation | : Guidewall | 100 | 38 | 111 me(10) |
|----------|---------|---------|------------------------------------|---------|--------|-------------------|------------------|----------|----------|---------|---------|-----------|---------|------------------|-------------|----------|-------------|-------|-------------|-------|------|---------------|
| | Size | Mile | : Lower | . Upper | : feet | : pool(1) : | : pool(2) | | nade : | (3) : | feet(4) | oundation | Lower : | Upper | LOWER | | Lower : | Upper | . Lower | Upper | É | odion : |
| (5) 8 | 4x600 | 28,30 | 0.0 | 0.4 | 4.0 | 5.0 | | 8.0 | 8.0 | 0.4 | 24.0 | Filing | -16.0 | -16.0 | 24.0 | 24.0 | 8.0 | 8.0 | 8.0 | 4.0 | e, | 3.0 |
| (5) | 4x600 | 47.45 | 0.4 | 16.0 | 12.0 | 17.0 | | 25.0 | 59.0 | 0.6 | 42.0 | 11 | -14.0 | 1.0 | 33.0 | 18.0 | 55.0 | 85.0 | 21.0 | 0.6 | 13. | 8.0 |
| 3 | 4X600 | 59.08 | 16.0 | 36.0 | 20.0 | 37.0 | | 43.0 | 0.94 | 7.0 | 45.0 | | 1.0 | 21.0 | 38.0 | 18.0 | 43.0 | 13.0 | 27.0 | 7.0 | 15. | 0.0 |
| ro | 34X60C | 74.85 | 36.0 | 0.09 | 24.0 | 61.0 | 51.0 | 0.79 | 75.0 | 7.0 | 0.64 | : | 21.0 | 45.0 | 42.0 | 18.0 | 67.0 | 0.19 | 31.0 | 7.0 | 16.0 | 0.0 |
| | 009X+ | 98.00 | 0.09 | 101.0 | 41.0 | 103.0 | | 110.0 | 110.0 | 0.6 | 68.0 | U-France | 45.0 | 86.0 | 61.0 | 20.0 | 96.0 | 110.0 | 26.0 | 0.6 | 7.1 | 0.0 |
| 58(6) 84 | 00000 | 99.20 | 101.0 | 131.0 | 30.0 | 131.0 | | 240.0 | 140.0 | 7.0 | 57-0(11 | Piling | 96.0 | 86.0 | 48.0 | 54.0 | 110.0 | 138.0 | 0.6 | 7.0 | 7.0 | 0. |
| (4,) | 4X600 | 147.92 | 131.0 | 138.0 | 7.0 | 139.0 | | 145.0 | 145.0 | 7.0 | 62.0 | U-France | 36.0 | 123.0 | 55.0 | 10.0 | 138.0 | 145.0 | 7.0 | 7.0 | 5.0 | 04 |
| (D) | 009X+ | 183.92 | 138.0 | 168.0 | 30.0 | 169.0 | | 175.0 | 182.0 | 7.0 | 55.0 | Piling | 123.0 | 153.0 | 48.0 | 18.0 | 175.0 | 175.0 | 37.0 | 7.0 | 10.0 | 07 |
| 10.7 | 4x600 | 207.55 | 168.0 | 136.0 | 24.0 | 193.0 | | 199.0 | 208.0 | 7.0 | 0.64 | | 153.0 | 177.0 | 42.0 | 18,0 | 199.0 | 199.0 | 36.0 | 7.0 | 16.0 | 0.0 |
| 14.7 | 00971 | 217.95 | 192.0 | 210.0 | 18.0 | 211.0 | | 217.0 | 225.0 | 7.0 | 43.0 | - 0 | 177.0 | 195.0 | 36.0 | 13.0 | 217.0 | 217.0 | 35.0 | 7.0 | 18,0 | |
| 147 | 009X4 | 233.0 | 210.0 | 235.0 | 25.0 | 237.0 | | 244.0 | 244.0 | 7.0 | 55.0 | ** | 195.0 | 220.0 | 45.0 | 20.0 | 226.0 | 244.0 | 16.0 | 0.0 | 7.5 | 0.4 |
| | 009X4 | 233.61 | 235.0 | 262.5 | 27.5 | 285.0 | | 305.0 | 305.0 | 24.5 | 88.0(11 | U-Frame | 220.0 | 220.0 | 0.79 | 95.0 | 244.0 | 269.3 | 0.6 | 7.0 | 7.0 | 3.0 |
| 120 | 009X4 | 258.91 | 250.0 | 270.0 | 7.5 | 285.0 | | 267.0 | 287.0 | 17.0 | 70.07 | | 220.0 | 255.0 | 53.0 | 10.0 | 268.0 | 207.0 | 5.0 | 17.0 | 0.5 | 0.0 |
| ω, | 4x500 | 274.51 | 270.0 | 284.0 | 14.0 | 286.0 | | 294.0 | 305.0 | 10.0 | 42.0 | Piling | 255.0 | 569.0 | 32.0 | 18.0 | 294.0 | 294.0 | 54.0 | 10.0 | 0.6 | 0.0 |
| (4,) | 009% | 286.64 | 284.0 | 308.0 | 24.0 | 309.0 | | 315.0 | 325.0 | 7.0 | 0.64 | = | 269.0 | 293.0 | 15.0 | 18.0 | 315.0 | 315.0 | 31.0 | 7.0 | 18.0 | 0.0 |
| 7,44 | 009X1 | 298,38 | 308.0 | 326.0 | 18.0 | 327.0 | | 333.0 | 348.0 | 7.0 | 43.0 | - | 293.0 | 311.0 | 36.0 | 19.0 | 333.0 | 333.0 | 25.0 | 7.0 | 12.0 | 0.0 |
| 8 | 4x500 | 306.31 | 326.0 | 344.0 | 18.0 | 345.0 | | 351.0 | 361.0 | 7.0 | 43.0 | | 311.0 | 329.0 | 36.0 | 18.0 | 351.0 | 351.0 | 25.0 | 7.0 | 13.0 | 0.0 |
| 30 | 42500 | 311.25 | 344.0 | 356.0 | 12.0 | 357.0 | | 363.0 | 369.0 | 7.0 | 37.0 | | 329.0 | 341.0 | 30.0 | 18.0 | 363.0 | 363.0 | 19.0 | 7.0 | 12.5 | 6.0 |
| 100 | 009X1 | 317.31 | 356.0 | 372.0 | 16.0 | 373.0 | | 379.0 | 394.0 | 2.0 | 41.0 | 11 | 341,0 | 357.0 | 34.0 | 16.0 | 379.0 | 379.0 | 23.0 | 7.0 | 15.5 | 0.0 |
| 200 | SXLOO | 331.31 | 372.0 | 306.0 | 24.0 | 397.0 | | 403.0 | 419.0 | 7.0 | 0.64 | - 11 | 357.0 | 381.0 | 42.0 | 18.0 | 403.0 | 403.0 | 31.0 | 7.0 | 17.5 | 0.0 |
| | SXIAOC | 342.51 | 396.0 | 1,24.0 | 28.0 | 425.0 | | 431.0 | 433.0 | 7.0 | 53.0 | | 381.0 | 0.604 | 0.04 | 18.0 | 431.0 | 431.0 | 35.0 | 7.0 | 25.0 | 0.0 |
| | College | 341.01 | 10 404 | 0.654 | O.R.C | 453.0 | | 459.0 | 163.0 | 7.0 | 0.25 | | 0.604 | 437.0 | 146.0 | 18.0 | 0.654 | 459.0 | 35.0 | 7.0 | 29.0 | 0.0 |
| | | 27445 | 0.00 | 2000 | 2000 | | | | 0.000 | | 2000 | | | | | 0.000 | 1 | | | | | |

(1) When ses level elevation under maximum operation discharge.

(2) Then are level tallwater elevation repression operation discharge.

(3) The new level tallwater elevation operation operation operation and the second of the set of long of the set of the first flow.

(4) Measured free top of long flow of flow.

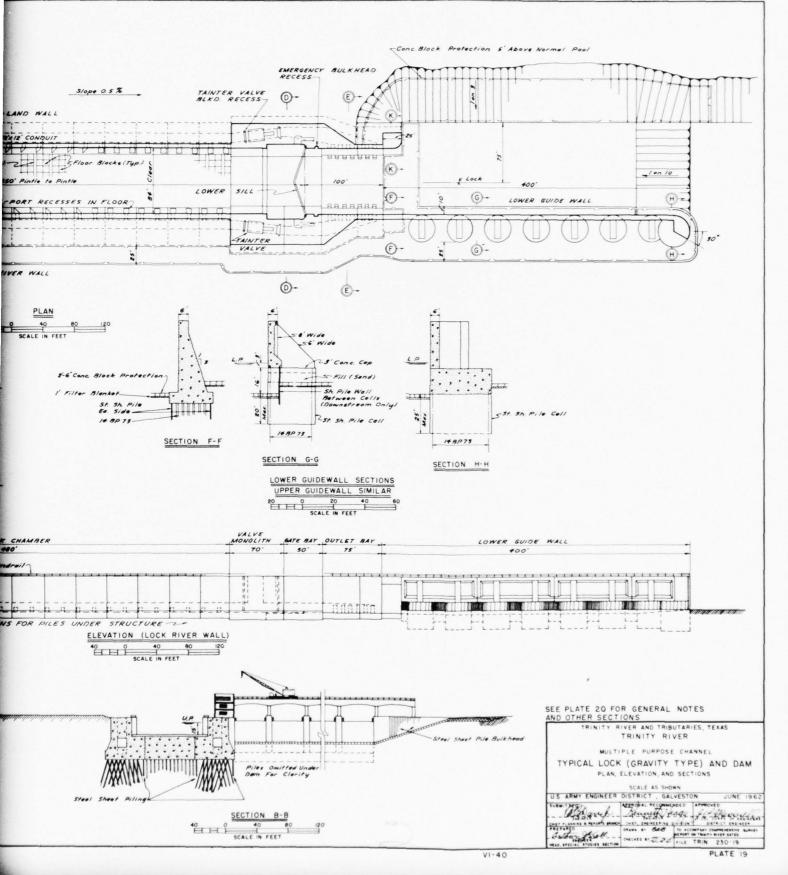
(5) Long No. 1 and 2 provide for maxigation through latingston reservoir between pool elevation 131.0 to 101.0.

(6) Longs No. 108 in and 12 provide for maxigation through latingston reservoir between pool elevation (23.0.0).

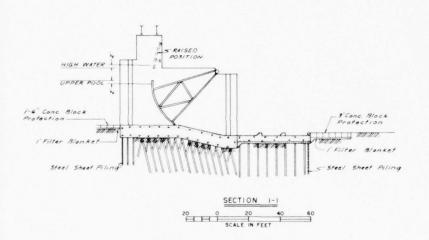
(7) Longs No. 108 in and 12 provide for maxigation through Permessee Oldony reservoir benefic of 101.0.

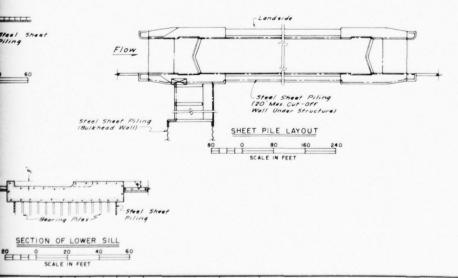
(8) Longs No. 108 in and 12 provide for maxigation through Permessee Oldony reservoir Wen floot control pool 1s full to elevation 13.0.

(9) Freeboard in feet between top of gaideenly and maximum design maxigation stage equal to talizater elevation under operating discharge (11) At upper gate boy and raive moneilth only.



GATE RECESS 26:6" I Filter Blanket SECTION D-D 20 0 20 40 H H I SCALE IN FEET





GENERAL NOTES LOCKS

- 1 THE 84' 600' LOCK SHOWN ON PLATES 17 AND 18 IS REPRESENTATIVE OF LOCKS 2, 3, 6, 7, 8, 9, 104, 12,13, 14, 15, 16, AND 17.

- 1 THE 84 KEDÓ LOCK SHOWN ON PLATES IT AND 18 IS REPRESENTATIVE OF LOCKS 2, 3, 6, 7, 8, 9, 104, 12, 13, 14, 15, 16, 400 17

 2 OTHER LOCKS RESIMILAR EXCEPT FOR THE FOLLOWING FERTURES:

 LOCKS SA, 6, 108, 400 11 ARE U-FRAME
 LOCKS SA, 6, 108, 400 11 ARE U-FRAME
 LOCKS SA, 400, 201
 LOCKS SA, 400, 201
 LOCKS SA, 400 201
 REPUBLIC AT LAM SECTION ASE EXTENDED
 TO PROVIDE RETAINING WALLS UPPER GATE
 HEIGHTS ARE ADJUSTED ACCORDINGLY
 LOCKS SA, 19, 20, AND 21 SMALLER LOCKS
 (SG'S 400' CHAMBER IN LIEU OF 812 600'
 AND 300-FOOT GUIDEWALL LENGTHS IN
 LIEU OF 400-FOOT).

 3 THE FOLLOWING CRITERIA APRIES TO LOCKS 2
 THROUGH 21, EXCEPT AS NOTEO
 DEPTH AT SILLS 15 FEET BELOW RESPECTIVE
 POOL OR 2 FEET ABOVE MAXIMUM MAVIGATION
 POOL WHICHEVER IS HIGHER, EXCEPT FOR SB & 108
 BOTTOM OF CHAMBER FLOOR 18 FEET
 BELOW LOWER POOL
 BOTTOM OF CULVERTS 12 12
 HEIGHT OF GATES 1 FOOT BELOW TOP OF
 SILL OF 2 FEET ABOVE UPPER POOL
 EXCEPT UPPER GATES AT LOCKS SB AND 10B
 A THE FOLLOWING CRITERIA IS USED WHERE
 APPLICABLE:
 STEEL BERNING PILE CAPACITY 90 TONS MAX,
 UNDER EXTREME LOADING
 B HALLS AND AT BASE OF WALLS AS
 SHOWN
 EMBRINGMENT PROTECTION 5 FEET ABOVE
 NORMAL POOL ON EARTH SLOPES OPPOSITE
 GUIDE WALLS AND AT BASE OF WALLS AS
 SHOWN
 SUMMAL AND GUARD WALL SECTIONS APE
 TYPICAL FOR LOCKS 2 THROUGH 201
 EXCEPT UPPER SALES AT LOCKS 2 THROUGH 21
 FOR FAIL AND GUARD WALL SECTIONS APE
 TYPICAL FOR LOCKS 2 THROUGH 201
 EXCEPT UPPER SALES AND SALES AS
 SHOWN
 SUMMAL AND GUARD WALL SECTIONS APE
 TYPICAL FOR LOCKS 2 THROUGH 201
 EXCEPT UPPER SALES AND SALES AS
 SHOWN
 SUMMAL AND GUARD WALL SECTIONS APE
 TYPICAL FOR LOCKS 2 THROUGH 201

- SHOWN

 5 GUIDE WALL AND GUARD WALL SECTIONS ARE
 TYPICAL FOR LOCKS 2 THROUGH 21

 6 GRAVITY TYPE WALLS ON BEARING PILES ARE
 DESIGNED TO BE INDEPENDENTLY STABLE.

DAMS

- 1. THE CONTROL DAM SHOWN IS REPRESENTATIVE OF DAMS 2, 3, 4,6,7,8,4,12,13,14,15,16,17,18,19,20, AND 21.
 2 CONTROL DAMS ARE NOT REQUIRED AT SITES 54,58,10A, AND 10B
 3 A CONCRETE SPILLWAY IS PROVIDED FOR DAM 11.

- 11.

 2 THE FOLLOWING CRITERIA APPLIES TO ALL CONTROL DAMS

 a WINTH OF GATES 40 FEET

 b WINTH OF PIERS 8 FEET

 C WINTH OF MONOLITHS 48 FEET

 1 TOP OF GATES 2 FEET ABOVE UPPER POOL

 1 TOP OF PIERS, ABUTMENTS AND STORAGE

 TARD 2 FEET ABOVE HIF WATER

 F SILL ELEVATION 12 FEET BELOW UPPER POOL

 ELEVATION OF OSSION APPROACH CHANNEL

 BOTTOM, WHICHEVER IS THE LOWER ELEVATION

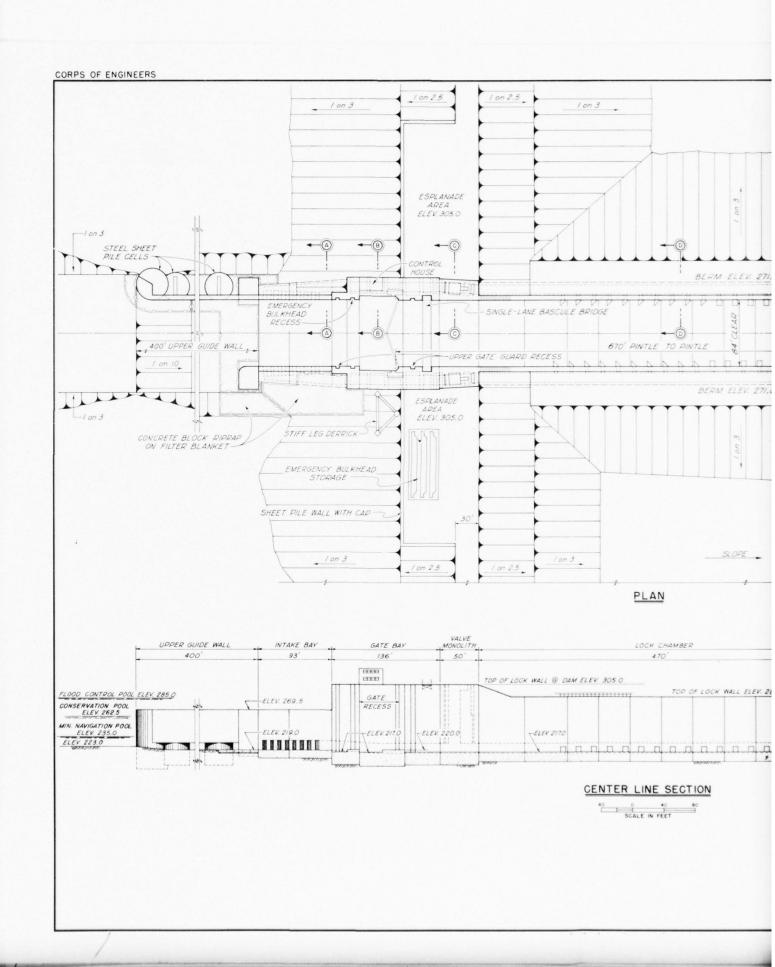
TRINITY RIVER MULTIPLE PURPOSE CHANNEL TYPICAL LOCK (GRAVITY TYPE) AND DAM SECTIONS SCALE AS SHOWN

U.S. ARMY ENGINEER DISTRICT, GALVESTON

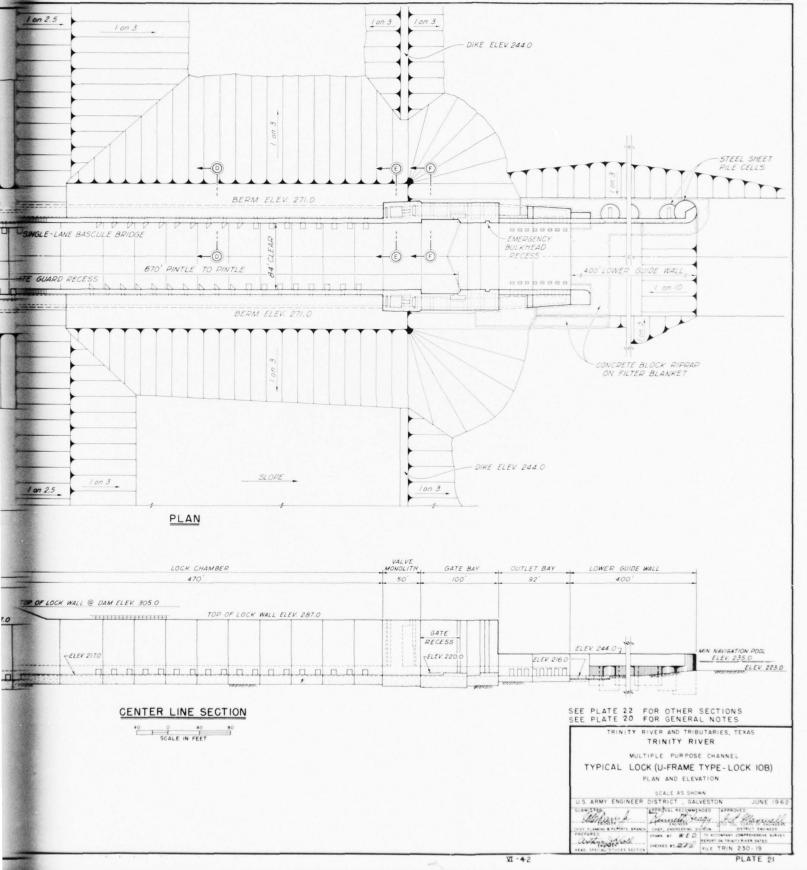
U.S. ARMY ENGINEER DISTRICT, GALVESTON

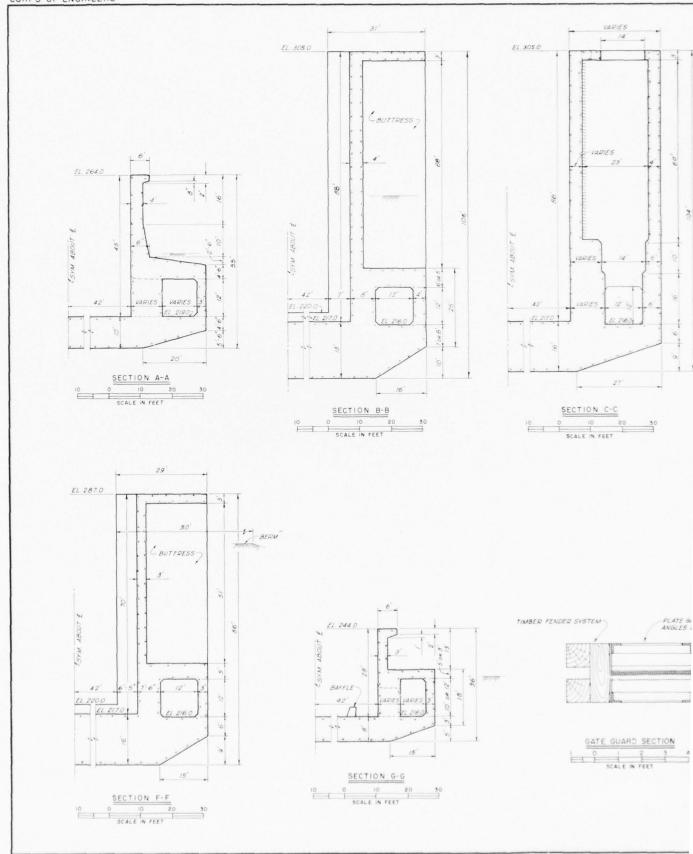
SUBJECT OF THE STAND STAND CHEEF CHE

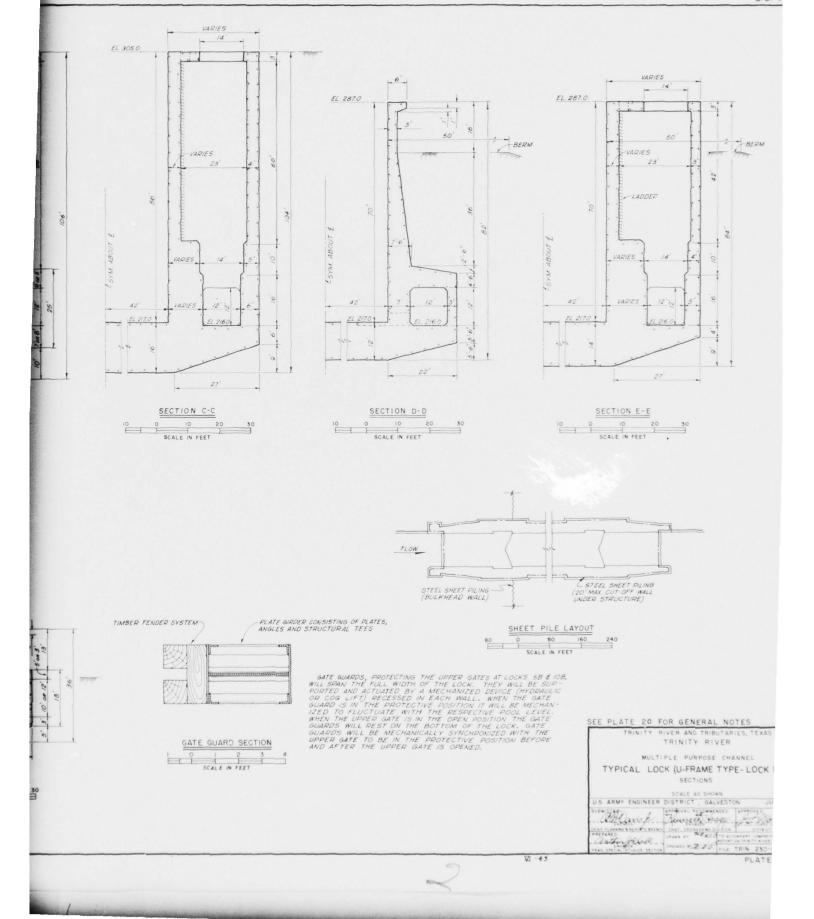
TRINITY RIVER AND TRIBUTARIES, TEXAS







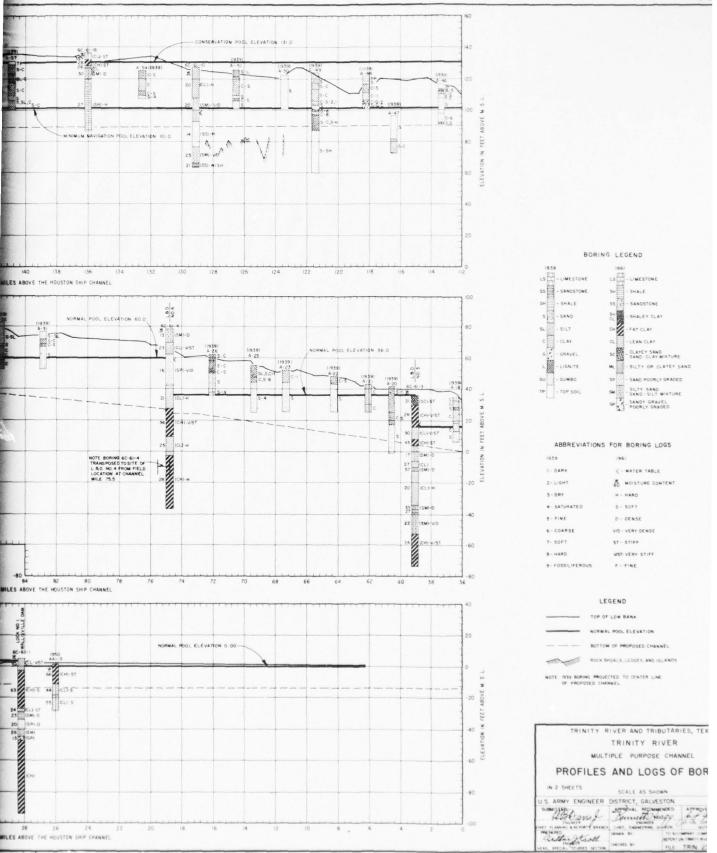




(B)

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J.S. ARMY 480 NORMAL POOL ELEVATION 396.0 SL-5-5 S-C (1939) C-S A-170 C-5-5 S-G-5 C NORMAL POOL ELEVATION 372.0 NORMAL POOL ELEVATION 326.0 332 330 DISTANCE IN CHANNEL MILES ABOVE THE HOUSTON SHIP CHANNEL (SH) 306 S-G SL-C-8 S-G-1-C-S-G C-S-7 CONSERVATION POOL ELEVATION 262.5 (CL) A-98 ABOVE THE HOUSTON SHIP CHANNEL (CL)-ST 180 7 168.0 252 6C 61-24 EL 123.9 (1939) A-77 C-8 C-9 S-C-7 TRINITY RIVER AND TRIBUTARIES, TEXAS TRINITY RIVER MULTIPLE PURPOSE CHANNEL PROFILES AND LOGS OF BORINGS

26 (SH-CL)-

BOVE THE HOUSTON SHIP CHANNEL

100

U.S. ARMY ENGINEER DISTRICT, GALVESTON JUNE 1962

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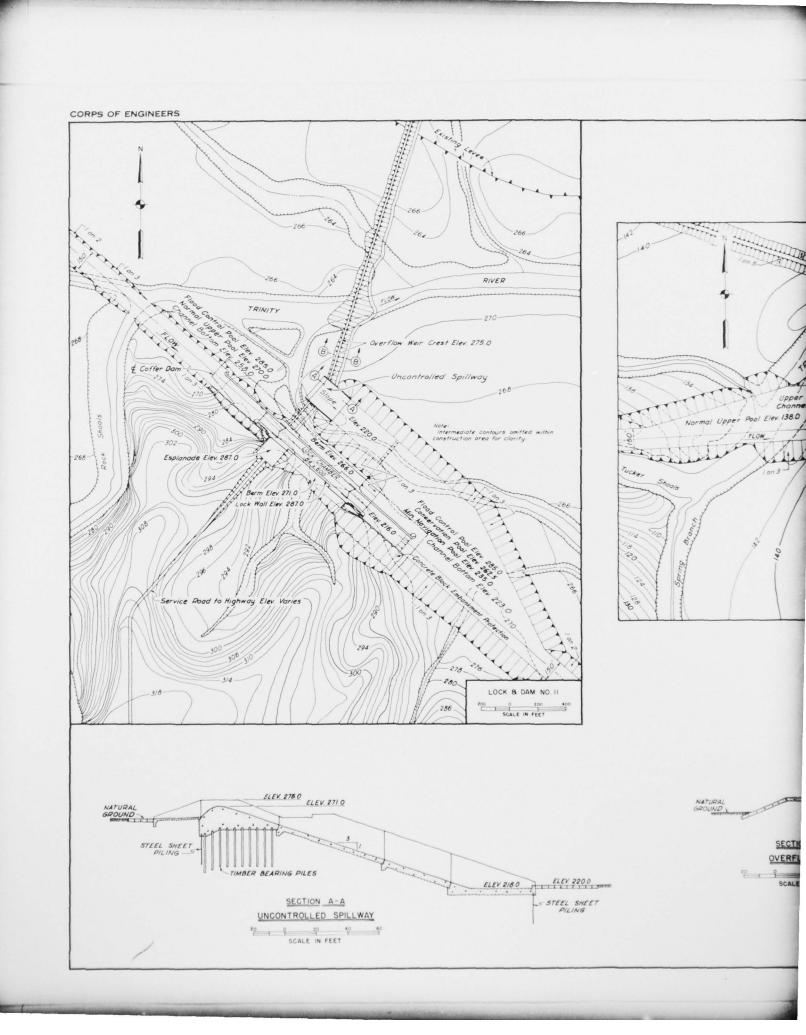
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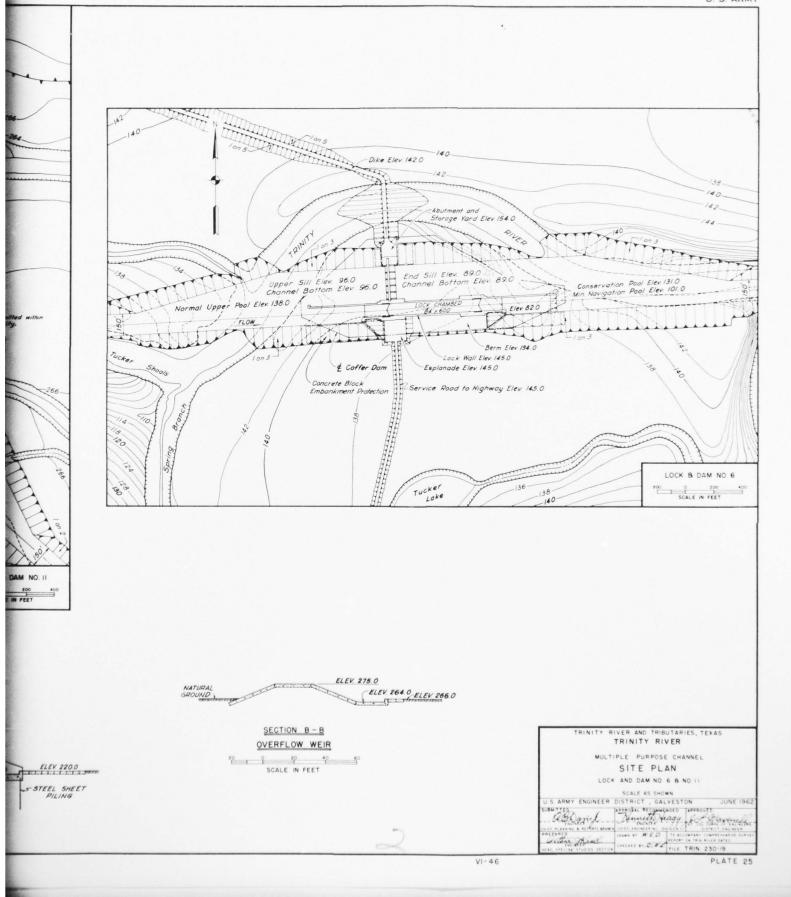
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atther that one or





- 47. The estimate of cofferdam cost is also based on the proposal that one cofferdam be provided for construction of both lock and dam with cost of cofferdam construction and dewatering proportionally to the lock and dam on the following basis:
- a. Allocate to lock cost, the estimated cost of providing a separate cofferdam and dewatering required for construction of the proposed lock.
- <u>b.</u> Allocate to dam cost, the difference in estimated cost of providing a cofferdam and dewatering required for construction of the proposed lock and dam compared to the cost of providing the cofferdam and dewatering determined in "a" above for lock construction.
- 48. The estimates of costs for diversion of river flows during construction are based on available topographic maps. An outline of the lock and dam unit was located at the proposed lock site on the alignment of the proposed multiple purpose channel shown on the topographic map. Where it appeared that the existing river channel would provide for river flows during construction of the lock and dam, no costs were provided in the estimates for such purpose. Where a river diversion channel was required, the cost of providing the channel was charged to construction of the lock. The care of water during construction of the locks and dams was based on the cost of pumpage required to remove the estimated amount of seepage from the subsurface earth materials.
- 49. The plan of improvement provides for both upstream and downstream guide walls to be 400 feet long for the 84 x 600-foot locks and 300 feet long for the 56 x 400-foot locks. Concrete block or riprap paving on filter blanket would be provided at the base of the guide walls to prevent erosion and scour, also on the sloping banks extending both upstream and downstream of the landward lock wall to the ends of the guide walls to a height of 5 feet above the respective pools. The guide walls for all locks would be of similar design as proposed for the Arkansas River locks. Plate 19 shows a plan and section of the proposed guide walls.
- 50. The upper guide wall of lock 5B would extend to elevation 138.0 or 7 feet above maximum conservation storage in the Livingston reservoir. The upper guide wall of lock 10B would extend to elevation 269.5, or 7 feet above maximum conservation storage in the Tennessee Colony reservoir. The upper guide walls would be inundated by floods having a frequency of once in six years. Flood storage to elevation 285.0 would occur once in 100 years, and it was considered uneconomical to provide guide walls to elevation 285.0.

- 51. Upstream and downstream guide walls would be of similar construction, the downstream wall differing from upstream wall in that it is a larger wall and would have a sheet pile cutoff wall provided between the cells. The first monolith of the guide walls adjacent to lock structure would be a conventional gravity type. Remainder of the wall would be of the cellular type having a reinforced concrete wall supported by bearing piles contained in steel sheet pile cell spaced at 50-foot intervals. The concrete walls would be continuous with keys spaced at 100-foot intervals through which the impact loads would be distributed to adjacent supports. Each key serves as an expansion joint. Cells would be filled with sand and capped with concrete and would protect the bearing piles from being damaged by debris and to dampen the vibrations from impact loads.
- 52. Both the upper and lower miter sills of the proposed 22 locks excluding lock No. 1 in the Wallisville dam, would be set a minimum of 15 feet below normal pool elevations. The length of lock between pintles are as follows:
- a. Six hundred and fifty (650) feet for the 84×600 -foot U-frame type locks 5A, 6 and 11 and the gravity type locks, excepting lock No. 5B.
- b. Six hundred and seventy (670) feet for the 84×600 -foot gravity type lock No. 5B in the Livingston dam and the U-frame lock No. 10B in the Tennessee Colony dam to provide for the proposed prtective gate guards at these locks, as subsequently described.
- c. Four hundred and thirty-five (435) feet for the 56 x 400-foot gravity type locks at and upstream of Dallas, Texas.
- 53. In connection with the design of locks No. 5B and 10B in the Livingston and Tennessee Colony dams, respectively, consideration was given to the advisability of hinged gates or special designed bulkhead gate which could be installed at the upper end of the locks to prevent loss of conservation storage from the reservoirs in the event that the upper gates were damaged or sprung apart by accidental collision of a barge tow. Investigation reveals that the cost of providing a set of sector gates upstream of the lock gates would be almost prohibitive and that special designed bulkheads would be less expensive but could not be easily installed under high head flow conditions. As a protective measure to prevent collision of barge tow and gate, the plan provides that a specially designed gate guard be provided on both sides of the upper gate.
- 54. Gate guards would be trussed structures, approximately 2.5 feet deep and designed to withstand an impact load of 250,000 pounds. The guards would be located approximately 20 feet clear on each side of the upper gate and would span the full width of the locks. The gate guards would be supported and actuated by a mechanized device recessed 10 inches in each wall. When the gate guards are in protective position, they would be mechanized to fluctuate with the

respective pool levels. When the upper gate is in open position, the gate guards would rest on the lock floor. The gate guards would be synchronized with the upper gate. The plan also provides for a complete set of stop logs and stiff leg derrick at each lock for use in maintenance of the upper gate, which could also be used in an extreme emergency in the event of a gate guard failure and subsequent damage to the upper gate.

- 55. All miter gates would be horizontally framed. For estimating purposes the heights of the gates were considered from one foot below the top of the sills to two feet above the upper pools. The majority of upper gates are identical in height and would be interchangeable with locks of corresponding widths. Spare lower gates may be sectionalized to permit later assembly into complete gates of heights required. This planning is desirable to provide spare gates to shorten the time navigation would be stopped in the event of serious damage to a set of gates. While it is not practical to provide spare gates for each set of service gates, standardizing the upper gates and sectionalizing the lower gates would permit a limited number of spare gates for the navigation system. Hydraulic equipment is used to operate tainter valves and miter gates with electric motors providing power.
- of closure equipment to be used at the locks. Excepting locks 5B and loB, closure systems for Trinity River locks are not required to prevent loss of pools. It is therefore, not essential that closure equipment be available at each lock site. To meet this requirement it is proposed to provide each lock with the necessary fixed equipment and to provide closure equipment at selected storage points. This plan anticipates division of the system into groups of locks. Central storage yards would provide transporting and lifting equipment, and a sufficient number of stop logs to close one end of any lock in the group of locks served by that storage point. Spare gates, valves, valve bulkheads, pintle, hinges, etc., may also be located at central storage yards. Guide walls on the land side would be gravity walls similar to the first monoliths of the guide walls beyond the ends of the intake and outlet monoliths.
- 57. The plans for lock Nos. 5B and 10B, located in the Livingston and Tennessee Colony Reservoir dams, respectively, provide for extending the upper gate bays and a portion of the lock walls to the top of respective dams at elevation 140 and 305, generally as shown on plate 21. The top of the dams would be widened to form esplanade, 100 feet wide by 200 feet long, on both sides of the lock structure. A single lane bascule bridge would be provided at top of the gate bay walls to provide a crossing of the locks for such purposes, as operation and maintenance of the lock structures, and inspection, maintenance and repair of the earth dams, as may be required.

- 58. Excluding locks Nos. 5B and 10B, the other locks would have a 150-foot square esplanade adjoining the upper gate bay land-wall which would have a top elevation equal to or greater than the lock wall elevation and would connect with the access road to the lock sites.
- 59. The plan provides for minimum berm filling behind the land wall of the gravity type locks from the esplanade on a gradient of 0.5 percent to an elevation three feet above the tailwater elevation of the design operating discharge of the multiple purpose channel at the downstream end of the landwall. Typical illustrations of the berm behind the land lockwall are indicated on site maps shown on plates 16, 17, 18 and 25.
- 60. ESTIMATED COST OF LOCKS. Detailed estimates of first costs of a 84 x 600-foot gravity type lock No. 4, a 84 X 600-foot U-frame type lock No. 6, and a 56 x 400-foot gravity type lock No. 19 are shown in table 8. The gate and valve machinery and electrical costs were established by application of the Engineering News Record Cost index factors to costs of similar items of construction of prior projects. The detailed estimates of costs shown in table 8 are typical illustrations of determing the estimated first cost of the other proposed locks, copies of which are excluded from this report to eliminate more or less repetitious material. A summary of the estimated first costs for each of the 22 proposed locks is given in table 6, page 36.

THAIR STIMEN PART CAST OF INVICATION OF SALE, 4, 6, M. STIMEN OF SALE OF SALE

| | | | XITO | 500 Gravity | 7 type : | X 168 | 500 "U Frame type | ane type | 04 % 95 : | 56 % 400 Gravity type | type |
|-------|--|-------|---------------|-------------------------|-----------|---------|-------------------|-----------|----------------|-----------------------|-----------|
| ic it | Designation | Unit: | Unit Price | Unit : Price : Quantity | : Amount | Unit | Quantity | : Amount | Unit Price: | Quantity | : Amount |
| | Coffer dam (fill) | 3 | \$0.25 | 117,000 | \$29,250 | \$ 0.25 | 106,300 | \$ 26,580 | | 31,300 | \$ 7,83 |
| | Chief dam, suce, sneet piling Clearing | Acre | 3.1 | None | 100,000 | 200.00 | 000 5 | 1,600 | 150.00 | 20,000 | 150 |
| | Excavation, common | CX | 0.50 | 50,000 | 25,000 | 0.50 | 530,000 | | | 438,000 | 240,900 |
| | Excavation, rock | Z C | 0.0 | None | | | None | | | 19,800 | 39,000 |
| | gill, (lott, espianae) | 200 | 0.0 | 20,900 | 7,320 | 0.35 | 207,270 | | 0.35 | 26,300 | 9,330 |
| | Titty of this Sticewist Chis | MEDM | 60.00 | 2, 100 | | 0.32 | 3,050 | 1,350 | | 2,500 | 910 |
| | perfect result R(1485-731 | TE OF | 8.8 | 375 000 | c | 300 | 2.53.0 | | | E.) 1 | 10,740 |
| | Piling, steel-H(1457-73)Cellular stideWells | 3 12 | 8.00 | 97.900 | 200,000 | 88 | 10,10 | 155 200 | 9.8 | 130,000 | 100,000 |
| | Pollunateel sheer | 1 20 | 3.00 | 64,100 | 192,300 | 3.00 | 43.560 | | 8.8 | 15,000 | 117 700 |
| | Filtes, seed smeet, cellular muidewalls | 594 | 3,00 | 45,550 | 136,650 | 90.6 | 33,300 | | 38 | 30,100 | 38.6 |
| | Charter aloca, ripros | CY | 30.00 | 14,730 | 1443,400 | 30.00 | 12,640 | | 30.00 | 13.040 | 301.140 |
| | Willer Finalso | CY | 9.50 | 5,960 | 32,730 | 5.50 | 6,100 | | | 6,200 | 200 |
| | Conditions, wills | 30 | 26.00 | 99,000 | 2,314,000 | 28.00 | 46,500 | 1,302,000 | | 79,000 | 2.054,000 |
| | Concrete floors, struts and sills | CY | 38.00 | 17,400 | 382,800 | 55.00 | 57,000 | 1,254,000 | | 11,400 | 250,300 |
| | Comprete, relitar guidewall | CY | 45.00 | 8,300 | 373,500 | 35.00 | 9,300 | 325,500 | 45.00 | 6,500 | 297,000 |
| | Cement | Buls | | 145,500 | 727,500 | 2.00 | 169,200 | 346,000 | | 100,300 | 501,500 |
| | Syeel, str., wall track | Lo. | 0.50 | 250,000 | 50,000 | 0.20 | 304,000 | 00,00 | 0.20 | 257,000 | 51,400 |
| | Steel, str., will armor, cellular guidewell, | 10. | 0.50 | 509,000 | 41,800 | 0.20 | 110,000 | 22,000 | 0.20 | 166,000 | 33,200 |
| | Steel, sir, miler gates (MF) | ġ, | 0.42 | 450,000 | 202,500 | 0.45 | 240,000 | 243,000 | 0.45 | 300,000 | 135,000 |
| | Leel, Str., Thinter valves (Mr) | 9 | 0.43 | 73,230 | 32,950 | 0.45 | 73,230 | 32,350 | 64.0 | 73,830 | 32,950 |
| | Stock, Sor., Miscell neugh | | 0.44 | 307,000 | 046,70 | 0.33 | 324,000 | 71,230 | 0.22 | 195,600 | 43,030 |
| | Steel countries rein. | | 28 | 6,000 | 131,090 | 0.00 | 12,000,000 | 2,340,900 | 0.13 | 930,000 | 120,030 |
| | Greel, Sir. chronium | 10.0 | 0.90 | 0,040 | 2010 | 88 | 0,000 | 2,430 | 900 | 0,00 | 20,40 |
| | Forgings, steel curbon | 10. | 0.80 | 80,000 | 64,000 | 86.0 | 98,000 | 70,000 | 9.0 | 0,000 | 2,700 |
| | Forgings, Alloy steel | Lo. | 0,80 | 5,110 | 060,4 | 0.30 | 5,110 | 060.4 | | 4,110 | 000.4 |
| | Castings, steel, grade "C" | 10. | 0.00 | 19,200 | 11,520 | 0.00 | 19,200 | 11,520 | | 14,700 | 0.00 |
| | Castings, steel, grade 1,2,8 3 | Lo. | 00.00 | 40,610 | 24,370 | 0.60 | 40,610 | 24,370 | | 40,610 | 24,370 |
| | Chattags, alloy steel | 70. | 8 | 5,570 | 3,940 | 8.0 | 0,570 | 3,940 | | 6,570 | 3,940 |
| | Castings, tron | CD. | 0.00 | 5,970 | 3,500 | 0.00 | 5,570 | 3,500 | | 5,970 | 3,530 |
| | Bronze and Orses | .03 | 8.50 | 0000 | 2,530 | 2.50 | 2,230 | 5,530 | | 2,230 | 5,530 |
| | FLECT, TON-THINDS | 10. | 35 | 0,00 | 1,000 | 8 | 2,070 | 1,060 | | 2,070 | 1,660 |
| | Condition tiers 3 D | 4 : | 1.00 | 30,7 | 74,400 | T. 20 | 9,600 | 14,400 | 1.50 | 0,490 | 10,340 |
| | Condoit more; 2" | 4 . | 0.00 | 156 | 130 | 0.0 | 150 | 130 | 0.05 | 140 | 120 |
| | When went to | Tol | 0.0 | C(1) | 200 | 2.(2 | 125 | 200 | 3.75 | 145 | 040 |
| | Windrell, steel plos | 200 | 100.4 | 5 700 | 20,000 | 1 1 | E SON | 200,000 | | 100 | 37,000 |
| | Whor plating steel | Di Ci | 2.00 | 80.5 | 020 | 88 | 3,000 | 2000 | 38 | 4,400 | 2000 |
| | Floor grating, steel | 100 | 4.65 | 3,310 | 40.970 | 4.65 | 2 210 | 070 070 | 39.7 | 2 810 | 26 200 |
| | Robber, sesis, noulded | Tp. | 2.50 | 1,100 | 2.750 | 05.6 | 1 100 | 0320 | | 040 | 20,000 |
| | The gages | LF | 14.00 | 08 | 340 | 14.00 | 4,100 | 1,000 | 1000 | 80 | 0000 |
| | Operating machines, miter gates | Els. | 27,500.0 | 4 | 110,000 | | 7.7 | 110,000 | 27 | 3 | 310.000 |
| | Double 4-way velves | Eu. | 900.00 | CU CO | 1,300 | | 0.0 | 1,300 | 900.00 | e. | 1,300 |
| | Operating rachine teinter valve | E0. | 17,500.00 | 7 0 | 70,000 | | -2 | 70,000 | 17,500.00 | 77 | 70,000 |
| | 3-say control valve | Es. | 700.0 | 77 | 2,300 | | 47 | 2,300 | 700.00 | 7 | 2,300 |
| | 3-8717 AUTOFICTIO VELVO | E. 1. | 500.00 | 77 | 5,000 | | 4 | 8,000 | 500.00 | -7 | 2,000 |
| | Oll piging system | Top | | Sum | 000,00 | | Sum | 000,00 | | Sum | 96,000 |
| | Pold office | Job | | Sum | 2,000 | t | Sum | 3,000 | | Shurn | 5,000 |
| | Central control structure | Jop | ı | Stan | 38,000 | 1 | Sum | 30,000 | × | Stura | 36,000 |
| | | | | | 200 | | | 0.00 | | | |

61. SUMMARY OF NAVIGATION DAMS (INCLUDING COSTS).- The plan of improvement for the multiple purpose channel provides for the construction of 17 movable dams and one overflow dam, to serve in conjunction with the proposed navigation locks and the Wallisville, Livingston and Tennessee Colony Reservoir Dams, to form and maintain navigation pools that would overcome the fall of the river to provide for navigation to Fort Worth and facilitate the passage of river flows and drift The locations of the proposed dams are shown on the general plans, plates 4 through 11, and in profile on plates 12 through 15, pages 1 to 29. The first costs of the proposed 17 movable dams and the overflow spillway dam No. 11 is estimated at \$45,491,000, as shown in taken the content of the proposed of the proposed 19 movable dams and the overflow spillway dam No. 11 is estimated at \$45,491,000, as shown in taken the content of the proposed of the proposed 19 movable dams and the overflow spillway dam No. 11 is estimated at \$45,491,000, as shown in taken the content of the proposed of the proposed of the proposed 19 movable dams and the overflow spillway dam No. 11 is estimated at \$45,491,000, as shown in taken the proposed of the proposed

TABLE 9
SUMMARY OF FIRST COSTS OF NAVIGATION DAMS REQUIRED FOR
THE MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS

| Dam | : | | Number and | l size | : | | |
|----------------------------------|-------------|-------------------|--|--------------|-------------|--------------------------------------|--|
| No. | : | | of tainter ga | ates (ft.) | : | Cos | t |
| | | | el to upper rea | ach of Walli | isville Res | servoir | (Mile |
| 1 | 35. | | 4 - 40×200 | | | (2 | • |
| | | | Lisville Reserv | voir to head | d of author | rized ch | annel |
| to | Lib | erty (Mile | 35.5 to 47.4) | | | | None |
| | | | channel to Lik | perty to Ter | nnessee Col | lony dan | n (Mile |
| 3 4 | 233 & 5B | .5) Livingston | 7 - 40×31 6 - 40×34 6 - 40×36 Dem (3) 5 - 40×44 5 - 40×42 | .5 | | 2,62 3,56 3,00 2,67 | 7,280 23,340 55,170 - 93,580 72,950 |
| 9 | | Total | 6 - 40x46 | | | 3,11 | 20,250 1,780 74,350 |
| Tenr | ness | ee Colony D | am to lock & da | am No. 12 (1 | Mile 233.5 | to 274. | 4) |
| | | | e Colony Dam (1 (5) | | | | 2,930 2,930 |
| Loci | k & | dam No. 12 | to Five-mile C | reek (Mile 2 | 274.4 to 32 | 22.0) | |
| 12 13 14 15 16 17 | | Total | 5 - 40x28 6 - 40x32 5 - 40x26 5 - 40x24 5 - 40x27 5 - 40x30 | | | 2,75 2,04 2,07 1,85 2,17 | 30,710 58,760 42,640 71,750 59,300 77,930 41,090 |

TABLE 9 (CONT'D) SUMMARY OF FIRST COSTS OF NAVIGATION DAMS REQUIRED FOR THE MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS

| Dam | • | and an in- | | and size | 2 | | |
|--------|----------|---|-------------|-------------|----------|--------|-----------|
| No. | 0 | | of tainte | er gates (1 | ft.) : | | Cost |
| Dallas | Terminus | to Dall | as Floodway | (Mile 326 | 6.7 to 3 | 31.1) | - None |
| | Floodway | (Mile 3 | 31.1 to 338 | | | 4 | 0.056.000 |
| 18 | | | 5 - 40x3 | 54.5 | | ф | 2,356,080 |
| Dallas | Floodway | to Fort | Worth Term | inus (Mile | 338.8 | to 362 | .8)_ |
| 19 | | and story and a second story and a second | 6 - 40x2 | 4 | | | 2,813,170 |
| 20 | | | 6 - 40x2 | 18 | | | 2,802,610 |
| 21. | | | 6 - 40x3 | 1 | | | 2,690,770 |
| | Total | L | | | | | 8,306,550 |
| | | | | | | | |

Fort Worth Terminus to and including Riverside bridges (Mile 362.8 369.8) -

Total Mile 0.0 to 369.8

le 9.

7.4

\$45,491,000

Note: Estimated first costs are based on prices as of January 196 (1) In addition, the recommended Wallisville Reservoir Dam would provide an overflow spillway about 20,100 feet long having a crest at elevation 4.0.

(2) Costs are included in the Wallisville Reservoir report.

(3) Gated spillway of Livingston Reservoir would control the rived discharges passing lock 5A & 5B.

(4) Cated spillway of Tennessee Colony Reservoir would control th

river discharges passing lock 10A & 10B.

(5) Dam No. 11 would consist of an overflow spillway without moval gates having a length of about 5,500 feet with crest at eleva-275.0 and a low flow spillway 200 feet long with crest at ele-271.0.

62. DESIGN INFORMATION ON DAMS .- A preliminary study movable dams with non-submersible tainter gates 80, 60, and 40 feet 1 indicates that the 40-foot gates provide more desirable proportion offers greater economy in bulkheads, hoisting equipment and bandling bulkheads. Accordingly, for the purpose of this report, the plan for non-submersible tainter gates, 40 feet long, for all movable d It also provides for the tainter gate sills to be set at the desig tom elevation of the multiple purpose approach channel at each site prevent silting of the upstream pool. The top of gates are set two higher than the upper pool elevation to provide one foot of freebox an additional one foot of storage over and above the normal pools variations in water demand. The number of gates at each dam were by the criterion that the swell head should not exceed one foot who charging the capacity of the channel at that point. Pertinent data cerning design elevations, type of foundation and the number and s tainter gates for each dam are given in table 10.

TABLE 10

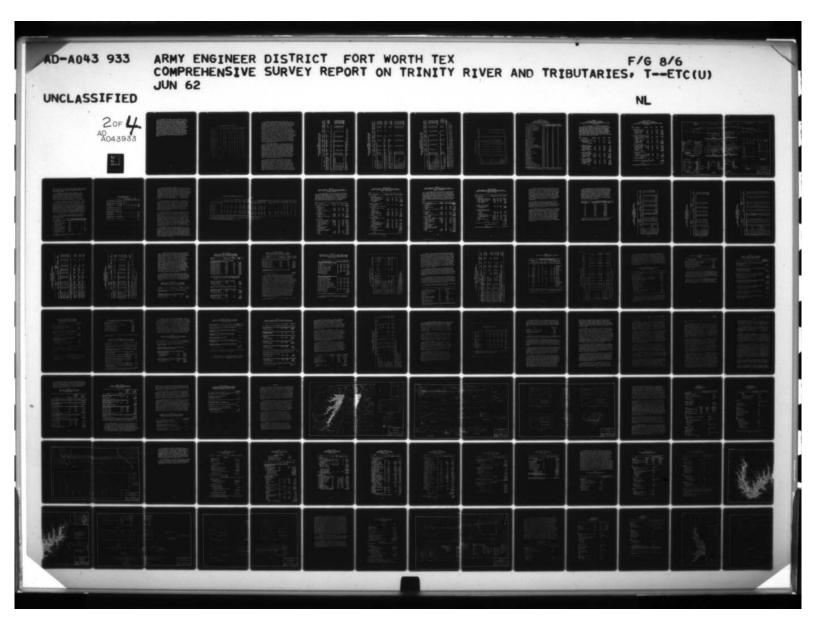
PERTINENT DATA - NAVIGATION DAMS MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

| Dem No. : : 1(2) 2 3 4 4 (3) 6 | Channel mile 28.30 47.45 59.08 74.85 147.92 183.92 207.55 | : Pool el : Lower 0.0 4.0 16.0 36.0 | ool elevation : | in | : maximum | storage yard: | of gate: | Taint | Tainter gates |
|--------------------------------|---|--|-----------------|------|-----------------|---------------|----------|-------|---------------|
| Dem No. : 1(2) 2 3 4 4 (3) 6 | mile 28.30 47.45 59.08 74.85 147.92 183.92 207.55 | : Lower 0.0 4.0 16.0 36.0 | : Upper : | | () + | |) | N | CC 500 |
| 1(2) 3 (3) | 28.30 47.45 59.08 74.85 147.92 183.92 207.55 | 0.0 4.0 16.0 36.0 | 0.47 | reet | :navigation(1): | & abutment: | sill : | NO. | :Size(ft.) |
| 100 tm 10 | 147.45 59.08 74.85 147.92 183.92 207.55 | 4.0 16.0 36.0 | > - | 4.0 | 5.0 | 8.0 | -16.0 | 7 | 40x21 |
| 100 tm | 59.08 74.85 147.92 183.92 207.55 | 36.0 | 16.0 | 12.0 | 17.0 | 30.0 | -13.0 | _ | 40x31 |
| 1034 | 74.85 147.92 183.92 207.55 | 36.0 | 36.0 | 20.0 | 37.0 | 45.0 | 3.5 | 0 | 40x34.5 |
| (E) 0 1 | 147.92 | 131.0 | 0.09 | 24.0 | 61.0 | 73.0 | 26.0 | 9 | 40x36 |
| 101 | 147.92 | 131.0 | | | | | | | |
| | 207.55 | 1.101 | 138.0 | 7.0 | 139.0 | 154.0 | 0.96 | 7 | 40×44 |
| | 207.55 | 138.0 | 168.0 | 30.0 | 169.0 | 180.0 | 126.0 | 5 | 40×44 |
| 8 | 1000 | 168.0 | 192.0 | 24.0 | 193.0 | 207.0 | 152.0 | 5 | 40×42 |
| 6 | CT-1-22 | 192.0 | 210.0 | 18.0 | 211.0 | 224.0 | 166.0 | 9 | 94x04 |
| (†) | | | | | | | | | |
| 11(5) | 258.91 | | | | | | | | |
| 12 | 274.51 | 270.0 | 284.0 | 14.0 | 286.0 | 305.0 | 258.0 | 5 | 40x28 |
| 13 | 286.64 | 284.0 | 308.0 | 24.0 | 309.0 | 323.0 | 278.0 | 0 | 40x32 |
| 14 | 298.38 | 308.0 | 326.0 | 18.0 | 327.0 | 346.0 | 302.0 | 7 | 40x26 |
| 15 | 306.31 | 326.0 | 344.0 | 18.0 | 345.0 | 359.0 | 322.0 | 5 | 40x54 |
| 16 | 311.25 | 344.0 | 350.0 | 12.0 | 357.0 | 368.0 | 331.0 | N | 40x27 |
| 17 | 317.81 | 356.0 | 372.0 | 16.0 | 373.0 | 391.0 | 344.0 | 1 | 40x30 |
| 1.8 | 331.31 | 372.0 | 396.0 | 24.0 | 397.0 | 417.0 | 363.5 | 5 | 40x34.5 |
| 19 | 342.51 | 396.0 | 454.0 | 28.0 | 425.0 | 432.0 | 402.0 | 9 | 40x04 |
| 20 | 351.91 | 424.0 | 452.0 | 28.0 | 453.0 | 462.0 | 426.0 | 9 | 40x28 |
| 21 | 360.17 | 452.0 | 480.0 | 28.0 | 481.0 | 0.784 | 451.0 | 9 | 40x31 |

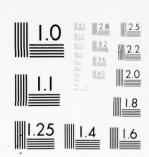
(1) Assumed to be elevation of maximum operating discharge of the multiple purpose channel at navigation dams.

- 63. Based on preliminary subsurface investigation at lock site, consisting of one core boring, it was concluded all dams be founded on bearing piles battered in two direct to withstand lateral loads. Steel bearing piles (14BP73) selected as foundation piling because of the anticipated ! driving conditions at many sites. The foundation bearing were considered to be driven to a sufficient depth to deve friction to resist the imposed loads. Steel sheet pile cu walls would be provided at the upstream edge of the gate : and the downstream edge of the stilling basin, with wall e to the steel piling wall beneath the lock structure and the off wall beneath the storage yard. Information concerning diversion and care of water during construction of the dar given in paragraph 48 of this appendix. Plates 23 and 24 the log of borings made at lock sites which were considere connection with the foundation requirements for the severe navigation dams.
- 64. The plan proposes that gate sill monoliths be do not the center line of gate width to provide 48-foot sill reliths to serve as foundation for each pier. All piers works feet wide and their lengths would provide for both upstrand downstream bulkhead recesses, service bridge supports gate trunnion anchorages. The trunnions are set in horizon position above the regulated two percent flood discharge of tion at the dam and the radius of the gate is long enough permit the bottom of the skin plate to meet the horizontal at an angle of not less than 40 degrees and provide 5 feet clearance above maximum high water. The top elevation of stream nose of the pier and the abutment would be at least above highwater at the dam site.
- 65. The service bridge would be centered on a vertice to the arc of the gate and set high enough so that low standard the arc of the gate in raised position. Electric he raising the gates would be mounted between the service brigirders. The crane provided for handling emergency bulkhe been estimated as a "locomotive crane" on rails, but future should include consideration of mobile cranes which could at more than one dam. Upstream bulkheads were estimated as as of providing each dam with bulkheads to close one gate one foot above the maximum navigation stage. Downstream are estimated on the basis that one complete set of downst bulkheads (for one gate only) would be provided to serve twith closure of one gate bay being provided for a height in between the low pool and the maximum navigation stage.

- 66. Excavation for the dams includes the excavation required to provide both an upstream approach channel and a downstream dischar channel. The upstream approach channel would have a width equal to the width of the dam extending upstream to 200 feet above the upstream and of the riverside lock guidewall, then a 1000-foot transition to join with the multiple purpose channel, all with channel bottom at the gate sill elevation. The discharge channel would have a width equal to the width of the dam, extending to 200 feet below the downstream end of the guidewall then a 1000-fcot transition to join with the multiple purpose channel. Downstream from the dam the channel bottom elevation would be the same as the stilling basin end sill elevation to near the end of the lock wall, then sloped to the multiple purpose channel bottom elevation at the junction of the two channels. All permanent earth slopes were assumed to be 1 on 3. Precast concrete blocks on a gravel filter blanket would be provided to protect earth slopes adjacent to the storage yard, the approach channel for 15 feet upstream from the gate sill, and the discharge channel for 25 feet downstream from the stilling basin, as shown on plates 19 and 21, pages 40 and 42.
- 67. The plan of improvement proposes that dam No. 11 be located within the Tennessee Colony Reservoir upstream of the mouth of Cedar Creek to form and maintain pool No. 11 at normal elevation of 270 feet, generally as shown on plate 25, page 46. When conservation storage in Tennessee Colony Reservoir is fully depleted, the top of lower navigation pool below dam No. 11 would be at elevation 235.0 with bottom at elevation 223. Under maximum flood storage conditions in Tennessee Colony Reservoir to elevation 285, pool No. 1 would be inundated to a depth of 15 feet. Preliminary investigations indicate that an overflow spillway dam at this location would be more economical than a movable dam with either submersible or nonsubmersible tainter gates. Accordingly, the plan of improvement proposes that dam No. 11 consist of an overflow dam about 5,700 feet long extending from lock No. 11 across the Trinity River flood plain. The dam would consist of an uncontrolled spillway 5,500 feet long of paved earth embankment with crest at elevation 275.0, and a low-water uncontrolled concrete spillway 200 feet long with crest at elevation 271.0, or one foot above proposed normal elevation of pool No. 11, and a stilling basin at elavation 218. The lowwater spillway would be founded on timber piling and would be of design similar to that shown on plates 4 and 5 accompanying engineering manual 1110-2-2400.



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MICROCOPY RESOLUTION TEST CHART NATIONAL BUREAU OF STANDARDS 1963

ESTIMATED COST OF DAMS. - Detailed estimates of first cost for dams Nos. 4, 6 and 19 are shown in table 11. The estimated quantities of construction materials were determined for about 20 items that varied with site conditions and design requirements. The other items of costs were based on curves or formulae which were either derived from similar items of dam construction considered in the 1938-39 Trinity River studies made in connection with House Document No. 403, 77th Congress, 1st session, or directly from the cost items for the Mississippi River dam No. 24, with appropriate adjustments made to reflect the quantities required for the 40-foot gates in lieu of the 80 foot gates used at dam No. 24 and in the 1938-39 Trinity River studies. The detailed estimates shown in table 11 are considered to be typical illustration for determining the estimated first cost of the other proposed movable dams, copies of which are excluded from this report to eliminate repetitious material. The estimated cost of dam No. 11 is based on a detailed quantity survey of the construction items required for the proposed dam. Cost for dam No. 11 and other dams are shown in table 9, page 52 .

TABLE 11
ESTIMATED FIRST COST OF NAVICATION DAMS NOS. 4, 6 % 19
REQUIRED FOR MULTIPLE PURPOSE TRINTTY RIVER CHANNEL TO FORT WORTH, TEXAS

| - | | | 1) | um No. 4 | | Dam | No. b | | Dom | No. 19 | |
|-------|---|----------|-----------|---------------|-----------|-----------|--|-----------|-----------|------------|---------|
| | | | Unit (S | ix gates, | 101 x361 | Unit (Fi | ve gates, 40's | : | (Si | x gates 40 | 1241) |
| tem | | | Price : | Quantity | | | mentity : | Amount : | Price : Q | mantity : | Amount |
| | : Designation : U | 12.0 | Price . | 16.1011.07.07 | - Amorro | | | | | | |
| | | CY | \$ 0.25 | 5,220 | \$ 1.310 | 0.25 | 27,100 \$ | 6,750 | 0.25 | 460 | |
| | offer Dum (Fill) | Job | 0.00 | Suzi | 25,000 | - | Sum | 25,000 | - | Sum | 2:,00 |
| | e-watering | SF | 3.00 | 12,000 | 36,000 | 3.00 | 15,000 | 45,000 | 3.00 | 8,000 | 24,00 |
| | teel akeet piling, Coffer dam | | 3.00 | 10,000 | 30,000 | 200,00 | 3 | 1,000 | 150.00 | 1 | 15 |
| | learing | Acre | | | 495,000 | 0.55 | 1.104.000 | 607,400 | | 1,501,000 | 353.05 |
| | meavation, Common | CY | 0.50 | 990,000 | | | 10,700 | 3,750 | 0.35 | 13,700 | 6,55 |
| . 3 | III (Incl.Storage Yard) | CY | 0.35 | 8,700 | 3,050 | 0.39 | | 3,700 | 0.37 | 10,100 | ~,,,,, |
| . 8 | mbanument, Dike | CY | - | - | | 0.2 | 180,000 | 45,000 | | | |
| | ling (14 BP-73) S111 | LF | 8.00 | 52,000 | 416,000 | 8.00 | 30,900 | 247,200 | 3.00 | 30,100 | |
| | iling (14 BF-73) Apron | LF | 5.0G | 5,130 | 41,440 | 8.00 | 2,200 | 17,600 | 8.00 | 4,610 | 36,00 |
| | lling, Steel Sheet, Type"Mk-31" | SF | 3.00 | 0,560 | 19,680 | 3.00 | 2,300 | 3,400 | 3.00 | 3,=00 | 9,84 |
| | iling, Steel Sheet, (MA-ce) | SF | 2.35 | 3,250 | 23,510 | 2.89 | 3,650 | 10,400 | 2.85 | 4,130 | 11,77 |
| | ting, seed siece, (with) | SF | 4.00 | 16,800 | 07,200 | 4.00 | 25,700 | 114,300 | 4.00 | 11,000 | 44,00 |
| | lling, Steel Sheet, Type "Z-38" | MEBM | 600.00 | 3.2 | 1,930 | 600.00 | 3.9 | 2,340 | 600.00 | 2.32 | 1,39 |
| | Imber, Soft Wood Frested | | | | 93,000 | 30.00 | 3,500 | 105,000 | 30.00 | 3,060 | 94,43 |
| | tiprap, Conc. Block | CY | 30.00 | 3,100 | | 3.50 | 1,200 | 6,600 | 5.50 | 1,320 | 7,26 |
| 3. 0 | mivel, Filter Planket | CY | 5.50 | 1,020 | 6,910 | | | | 22.00 | 1,170 | 25,74 |
| 1. 0 | oncrete, Apron | CY | 22.00 | 1,710 | 37,020 | 22.00 | 974 | 21,430 | | | poli le |
| | Oncrete, Sill | CY | 22.00 | 0,450 | 141,900 | 22.00 | 6,300 | 149,600 | 22.00 | 10,200 | |
| | oncrete, Pier | CY | 26.00 | 7,150 | 185,320 | 26.00 | 3,770 | 220,020 | 26.00 | 4,340 | 114,14 |
| | Concrete, Abutment Ftg. & Walls | CY | 26.00 | 3,340 | 36,840 | 26.00 | 3,800 | 98,800 | 26.00 | 2,950 | 75,70 |
| | oncrete, 6" Stor.Yd.Paving & Slope Prot. | CY | 20.00 | 234 | 4,680 | 20.00 | 258 | 5,160 | 20.00 | 1.83 | 3,66 |
| | | Bbls. | 5.00 | 154,000 | 770,000 | 5.00 | 30,100 | 150,500 | 5.00 | 27,400 | 137,00 |
| | ement | | | | | 0.26 | 172,800 | 44,930 | 0.26 | 201,000 | 52,20 |
| | teel, Structural, Service Briage | LB | 0.26 | 202,000 | 54,520 | 0.20 | 112,000 | 4.1230 | 3.00 | , | ,-,- |
| | Anchoruge & Head Flate | Lb | 0.22 | 190,000 | 41,800 | 0.22 | 193,600 | 42,590 | 0.22 | 127,000 | 27,94 |
| . S | teel, Structural Phos. Chromium Tainter | | | | | | 216 | and the | | ici w now | ~+D D |
| | getes (Movable parts) | Lo. | 0.40 | 321,000 | 328,400 | 0.40 | 836,000 | 334,400 | 0.40 | 547,000 | |
| | teel, Structural, Miscellaneous | i.b. | 0.22 | 53,200 | 11,700 | 0.22 | 54,000 | 11,830 | 0.22 | 39,000 | 8,50 |
| | iteel, Structural High Tensile | Do. | 0.22 | 149,000 | 32,780 | 0.23 | 211,000 | 46,420 | 0.28 | 63,000 | 13,30 |
| | | Lo. | 0.40 | 92,200 | 36,330 | 0.40 | 91,200 | 36,480 | 0.40 | 66,200 | 26,48 |
| | teel, Structural Phos. Chromium Misch. | | | | 73,530 | 0.13 | 618,100 | 30,350 | 0.13 | 471,600 | |
| 9. 3 | Steel, Concrete Reinf. | Lb. | 0.13 | 566,400 | | | | | 0.80 | 7,500 | |
| 0. 5 | teel, Corrusion Resistant | Lb. | 0.60 | 10,600 | 3,480 | | 10,500 | 8,400 | | | 0,0 |
| 1. 3 | teel, Cold Rolled | Lo. | 0.20 | 1,400 | 280 | 0.20 | 1,200 | 240 | 0.20 | 1,400 | |
| | Forgings, Steel | Lb. | 0.30 | 141,000 | 112,800 | 0.30 | 145,000 | 116,000 | 0.30 | | |
| | Furgings, 41loy Steel | 56. | 0.80 | 1,980 | 1,580 | 0.80 | 2,300 | 1,640 | 0.80 | 1,500 | |
| 2. | orgings, alloy ducer | Lo. | 0.80 | 2,930 | 2,340 | 0.80 | 2,960 | 2,370 | 0.80 | 2,400 | 1,9 |
| * . ! | Pargings, Phos. Chromium Steel | Lo. | 1.50 | 150 | 230 | 1.50 | 150 | 230 | 1.50 | 250 | |
| 5. | Orgings, Aluminum Alloy Estings, Steel, Class "O" | | 0.60 | 4,750 | 2,550 | | | 6,300 | 0.60 | 3,170 | 1,90 |
| A | Astings, Steel, Glass "O" | La. | | | 20 1.00 | | | 39,000 | 0.60 | | |
| 7. 0 | Distings, Steel, Class 1, 2 & 3 | Lo. | 0.60 | 64,000 | 38,400 | | | 300 | 0.50 | | |
| d. 6 | astings, alloy Steel | Lo. | 0.00 | 600 | 360 | | | | | | |
| 0. / | Luminum Alloy, Structurel (Eigh Strength) | Lo. | 1,20 | 7,000 | 8,420 | | | 23,040 | 1.20 | | |
| 1. | luminum Alloy, Structural (Med. Strength) | Ub. | 1,40 | 350 | 420 | | | 1,150 | 1.0 | | |
| 5 | ronze, (Incl. other non-ferrous metals) | 1.5. | 6,50 | 7,200 | 18,000 | 2.50 | | 15,000 | 2.50 | | |
| | | 1.6. | 2,50 | 1,200 | 3,000 | 2.50 | 1,000 | 2,500 | 2.50 | 1,200 | |
| | lopper | Joo | 6.000 | L | 30,600 | | Li. | 30,000 | | Le | 30,0 |
| | electrical system | | | 335 | 5,040 | | | 4,350 | 15.00 | | |
| U. 1 | Jesso Truck - 80/ Rull | TR & Pt | 15.00 | | 3,040 | | | 1,320 | 100 | | 1,3 |
| | Storige Yari Track-60/3.11 | TR & Ft | | 140 | 1,320 | 0.00 | | | 4.00 | | |
| 8. 1 | Smartillag, Comel pipe | 1.6 | 4.00 | 300 | 3,600 | 4.00 | | 3,-00 | | | |
| | Toor grating, steel | 38 | 4.00 | 970 | 4,510 | | 330 | 3,550 | 4.65 | | |
| | loor place | SF | 5.00 | 4,000 | 23,400 | | | 13, 300 | 2.00 | | 23,4 |
| | phoer C la, Mouleed | La. | 0.00 | 0,000 | 15,600 | 2.30 | 9,480 | 13,700 | 2.50 | | 0,7 |
| 4. 1 | perside, menince, T. inter Gite | | | 0 | | 25,000.00 | | 1.0,000 | 25,000.00 | | 150,0 |
| | communical equipment Operating michinery, Balater Gate | 120 | 25,600.00 | | | | | | | | |
| | electrical equipment Parter Gase operation machinery space | 20 | 00.00 | 0 | 39,000 | 5,500.00 | | 32,500 | 6,500.00 | | 30,0 |
| | | Lot | 3.00 | Lis | 10,000 | - | 1.0 | 20,000 | - | 1.5 | 10,0 |
| | parts & mise. equipment | | | | 39,750 | | Lis | 39.750 | - | L | 39.7 |
| | Di-Ton Track Grane (Installed) | Job | | 1 | | 500.00 | | 3,000 | 500.00 | | 3,0 |
| 6. 1 | Sporage Yard Cara | 1311 4 | 500,00 | 0 | 3,000 | | | | 300.00 | | 3,0 |
| | Plex Up Device | Ea. | 300,00 | 1 | 300 | 300.00 | The state of the s | 300 | 300.00 | | |
| | Storage yard pier super structure | 3(1) | | IO | 6,500 | - | 46 | _8,500 | | i.e | _0,5 |
| | Total estimated Hirst cost (Eschudi | iz conti | ngencies) | | 3,563,170 | | | 3,003,500 | | | 2,52,1 |

VI - 58

- 69. SUMMARY OF HIGHWAY RELOCATIONS (INCLUDING COSTS) .- The estimated cost of highway relocations within Tennessee Colony Reservoir are included in the cost estimate of Tennessee Colony Reservoir given in this appendix. The construction of the multiple-purpose channel would require 48 bridge relocations of which 20 would be partial modification of existing bridges, 10 would be complete replacement of existing bridges and 18 would be new bridges over land-cut channels. The relocation cost for individual alterations varies from \$1,866,000 to \$116,000 and totals \$44,024,000. Data and cost thereon are shown in table 12. The Texas Highway Department reports that its present longrange plan includes construction of up to 12 additional highway crossings of the Trinity River within the next 15 years. The Highway Department states that two bridges on the planned Interstate Highway 635 loop crossing at Dallas have been authorized and funded for preliminary engineering this year and one bridge on Farm to Market Road 162 in Liberty County has been authorized. The other nine crossings are in various stages of program planning. Definite information as to the exact location, size and type of construction of these crossings was not available at the time this report was prepared and costs were not estimated for any modification that might be required for project purposes. However, the contingency item in the cost estimate is considered adequate to cover such modification of additional bridges that might be built prior to construction of the multiple-purpose channel.
- 70. HIGHWAY DESIGN INFORMATION .- All highway bridges over the proposed multiple purpose channel including the highway bridge in the Tennessee Colony reservoir are based on providing a minimum vertical clearance of 50 feet above maximum navigation elevation and a minimum horizontal clearance of 250 feet between fenders for bridges located between the mouth of the Houston Ship Channel and the Dallas Terminus. A minimum horizontal clearance of 225 feet between fenders would be provided for bridges located between the Dallas and Fort Worth Termini. A side slope of two horizontal to one vertical was assumed for embankments. Based on these clearance requirements, the length of span required for the channel would be 300 feet for bridges located below Dallas and 274 feet for bridges located above Dallas. In general, the design loads used for highway bridges were H2O-S-16 for interstate highways and H2O-S12 for state highway bridges. Maximum grades used were 3.5% for interstate highway bridges and 5.0% for state highway bridges. A typical bridge is shown on plate 26.
- 71. Earth fill approaches to bridges would be provided to a maximum height of 20 feet above natural ground. Earth embankments will not be used in floodways. Trestles consisting of concrete girder-spans on concrete piles would be used in floodway in place of earth embankments. Approach spans would consist of prestressed concrete beams and concrete deck. The center span of a three-span continuous plate girder with concrete deck would span the project channel. Timber pile fender systems would be provided on the channel side of bridge piers adjacent to the navigation channel. Table 13 gives information concerning design data considered in determining the estimated first cost of the hgihway bridges for the multiple-purpose Trinity River channel to Fort Worth, Texas. Table 14 gives information concerning the

TABLE 12
SUMMARY OF ESTIMATED FIRST COST AND COMPONENT LENGTH
OF HIGHWAY BRIDGES REQUIRED FOR MULTIPLE PURPOSE
TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

| Total Stimated irst cost | 963,000 963,000 1,926,000 | | 1,083,000 1,086,000 1,093,000 874,000 874,000 1,071,000 876,000 876,000 |
|--|---|--|--|
| :Total : Total : Length: Stimmted):(feet):first cos | 2,320 \$ | (h.74 o | 0,0,0,1,0,0,0,0,0,0,0,0,0,0,0,0,0,0,0,0 |
| : Right : embank-): ment(ft. | 000 | 4ile 35.5 t | 250 250 400 1400 1400 200 200 |
| Right approach spans(ft. | 0 to 35.5) 770 770 | authorized channel to Liberty (Mile 35.5 to 47.4) | (Mile 47.4 to 233.5) 780 950 780 950 780 605 780 550 715 900 780 770 770 780 770 780 770 |
| : Span : (feet) : | 30.37 600 770 780 770 770 30.37 600 770 780 770 770 780 770 770 780 770 77 | channel to | Dem (Mile 4 780 780 780 780 780 780 780 780 780 780 |
| : Left : Left :Channe:embank- :approach : span :ment(ft,):(feet) | le Reservo 770 770 | authorized | Collony 900 900 940 940 950 950 950 950 950 |
| : Left :embank- :ment(ft. | Wallisvil 600 600 | head of | to Tennessee 140 |
| Channel mile | reach of 30.36 30.37 | servoir to | Liberty 47.5 47.5 47.9 91.6 98.9 111.5 136.1 171.6 196.6 |
| Name of bridge | Houston Ship Channel to upper Interstate Hwy 10 (Eastbound) Interstate Hwy 10 (Westbound) Total - Mile 0.0 to 35.5 | Upper Reach of Wallisville Reservoir to head of None | Head of authorized channel to U. S. Hwy No. 90 (Eastbound) U. S. Hwy No. 90 (Westbound) State Hwy No. 105 U. S. Hwy No. 59 County Road U. S. Hwy No. 190 State Hwy No. 19 State Hwy No. 21 State Hwy No. 21 State Hwy No. 7 U.S. Hwy 79 & 84 Total - Mile 47.4 to 233. |

Tennessee Colony Dam to Lock & dam No. 12 (Mile 233.5 to 274.4)

U.S. Hwy No. 287

249.99

See cost estimate for Tennessee Colony Reservoir.

State Hwy No. 31

264.52

See cost estimate for Tennessee Colony Reservoir.

TABLE 12
SUMMARY OF ESTIMATED FIRST COST AND COMPONENT LENGTH
OF HIGHWAY BRIDGES REQUIRED FOR MULTIPLE PURPOSE
TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

| Total sstinated first Cost: | \$861,000 890,000 909,000 884,000 848,000 | 1,037,000 1,042,000 2,079,000 | 857,000 794,000 1,113,000 2,764,000 | 1,214,000 1,163,000 1,325,000 1,189,000 2,151,000 1,607,000 1,226,000 982,000 1,226,000 |
|--|---|--|---|--|
| <pre>:Right :Total : Total :embank-:length:estinated :ment :(feet):First Cos</pre> | 1,780 2,030 2,130 1,990 1,780 | 2,680 | 1,980 | 2,065 1,965 1,365 1,315 1,615 1,615 |
| 1 ~ | 200 | 200 | | |
| Right :Approach :Spans(Ft. | 600 650 505 300 | 7) 1,400 1,400 | 700 550 350 | 700 650 700 700 550 550 1,000 |
| :Channel:Right:span :appro):(feet) :spans | 780 780 780 780 780 780 | 0 to 326.7 780 780 | to 331.1) 780 780 715 | 715 715 715 715 715 715 715 |
| :Left :approach .):s_ans(ft. | Mile 274.4 to 322.0 400 400 50 650 260 700 100 605 | (Mile 322.0 500 500 | (Mile 326.7 t 500 400 350 | 650 650 650 650 650 650 650 650 650 650 |
| :Left :enbank- :ment(ft. | (Mile 27 ^b 400 50 260 100 100 | Terminus 100 100 | | 111150 |
| :Channel :mile | e-mile Creek 285.60 298.04 312.84 315.57 319.92 | g Dallas 326.19 326.20 | ing Dallas Floodway ound) 328.46 - ound) 328.47 - 330.65 200 | to 338.8) 331.41 331.41 332.28 338.28 338.50 333.50 333.50 334.39 336.33 337.26 |
| : Name of bridge | Lock & Dam No. 12 to Five-mil State Hwy No. 1129 State Hwy No. 34 Malloy County Road Belt Line Road Dowdy Ferry Total - Mile 274.4 to 322.0 | Five-mile Creek to & including State Hwy Loop 12 (Westbound) State Hwy Loop 12 (Westbound) Total - Mile 322.0 to 326.7 | Dallas Terminus to existing D Interstate No. 45(Northbound) Interstate No. 45(Southbound) Forest Avenue Total-Mile 326.7 to 331.1 | Dallas Floodway (Mile 331.1 to Corinth St. Cadiz St. Interstate Hwy 35E Houston St. Dallas Ft.Worth Turnpike Commerce St. Continental St. Sylvan Avenue Hampton Road Westmoreland Road Total-Mile 331.1 to 338.8 |
| N | State State State Mallo Belt Dowdy | State State | Dallas Interi Fores Tots | Dallar Corin Cadiz Interr Houst Dallar Comme |

SUMMARY OF ESTIMATED FIRST COST AND COMPONENT LENGTH OF HIGHWAY BRIDGES REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS TABLE 12 (CONT'D)

| | | Left | : Left | 1 | Right | : Right | :Total | Total |
|--|-------------------------------------|-------------------------|---------------------------|---------------|---------------------------|------------------------|-----------------------|-------------------------|
| Name of bridge | :Channel: embank : mile :ment(ft | : embank- :ment(ft.) | : approach : spans(ft.):(| spen feet) | : approach :spans(ft.) | : embank- :ment(ft. | :length:):(feet): | estimated first cost |
| Dallas Floodway to & including | Fort Worth | th Terminus | (Mile | 338.8 to 36 | 362.8) | | | |
| State Hwy Loop 12 | 1 | | 750 | 715 | 650 | • | 2,115 | \$ 860,000 |
| Meyers Road | 342.94 | 1,00 | 550 | 715 | 1,040 | 160 | 2,305 | 892,000 |
| Belt Line Road | 345.25 | 250 | 650 | 715 | 650 | 250 | 2,015 | 823,000 |
| State Hwy No. 360 | 350.75 | 250 | 550 | 71.5 | 350 | 700 | 1,615 | 708,000 |
| F. M. Rd. Lo. 157 | 354.00 | 700 | 350 | 715 | 850 | 175 | 1,915 | 831,000 |
| Arlington-Bedford Road | | 300 | 1,150 | 715 | 1,150 | 300 | 3,015 | 1,075,000 |
| Arlington-Smithfield Rd. | | 200 | 1,330 | 715 | 950 | 200 | 2,995 | 000,066 |
| U. S. Hwy Loop 820 (Northbound) | _ | ı | 700 | 71.5 | 550 | 200 | 1,965 | 734,000 |
| U. S. Hwy Loop 820 (Southbound) | 362.12 | 1 | 700 | 715 | 550 | 200 | 1,965 | 734,000 |
| Handley-Ederville Rd. | 362.70 | 400 | 009 | 715 | 550 | 300 | 1,760 | 784,000 |
| Total - Mile 338.8 to 362. | Φ. | | | | | | | \$8,431,000 |
| Fort Worth Terminus to and including Riverside | luding Ri | verside I | Drive bridges | (Mile | 362.8 to 369.8 | (8) | | |
| First St. | 366.80 | 350 | 7480 | 325 | 330 | 350 | 1,135 | 247,000 |
| Beach St. | 368.60 | 250 | 200 | 230 | 8 | 275 | 1,200 | 129,000(1) |
| Riverside Drive (Northbound) | 369.40 | 1 | 210 | 333 | 510 | 200 | 1,053 | 170,000(2) |
| Riverside Drive (Southbound) | 369.41 | • | 210 | 292 | 570 | 200 | 1,072 | 116,000(3) |
| Total - Mile 362.8 to 369. | ω. | | | | | | | 262,000 |

\$44,024,000 Estimated cost of adding 3-30' concrete girder spans on left approach and 23-30' concrete girder spans on right approach plus earth fill approaches. Total - Mile 0.0 to 369.8

Estimated cost of adding seven 30-foot concrete girder spans on left approach and nineteen 30-foot (5)

concrete girder spans on right approach plus earth fill approach at the right end of bridge. Estimated cost of adding seven 30-foot concrete girder spans on left approach and nineteen 30-foot concrete girder spans on the right approach plus earth embankment at the right end of bridge. (3)

TWALE 13
PERFINENT DESIGN DATA OF PROPOSED HIGHMY BRIDGES REQUIRED
FOR MILTIPLE PURDESE PRINTY RIVER CHANNEL TO FORM WORTH, TEACH

| (401) (402 | | | Proposed | Proposed design of: Normal | Normal | Slevation : of design :Maximum | :Maximum | | . Design | : Leveed floodway (4) | odway (4) |
|---|--|----------|-----------------|----------------------------|------------------|-----------------------------------|--------------------|----------------|-------------|-----------------------|-------------------------|
| Marchellone | Name of Bridge | Channel | Width (Feet) | : Grade | elevation: (MSL) | | elevation (MSL) | 2 | : low steel | | feet c. to of levees |
| 10 cm 11 c | Houston Ship Channel to Dallas | Perminus | | | | | | | | | |
| 17.1 | Interstate Hwy 73(Westbound) | 30,36 | | -11.0 | 4.0 | 4.1 | 0.0 | 1.0 to 11.1 | 54.1 | | |
| Virgin V | Interstate Hwy 73(Eastbound) | 30.37 | | -11.0 | 0.4 | 4.4 | 9.0 | 1.0 to 11.1 | 74.1 | | |
| 1,1,1,2,1,2,1,3,1,4,4,4,4,4,4,4,4,4,4,4,4,4,4,4,4,4 | U.S. Hwy 90(Eastbound) | 47.6 | | -13.0 | 10.0 | 17.1 | 23.0 | 16.0 to 24.1 | 67.1 | | |
| 10 | U.S. Ewy 90(westbound) | 26.74 | | -13.0 | 10.0 | 17.1 | 58.0 | 16.0 to 24.1 | 67.1 | | |
| 1,000,000 1,00 | STREE UNI TO | 2.60 | | 700 | 200 | 01.3 | 0.50 | 00.0 to 00.3 | 111.3 | | |
| The column The | Occupation Spans (14) | 24,00 | | · · · | 0.00 | 2.50 | 77.0 | 00.0 to 01.0 | 0.427 | | |
| 15 17 17 18 18 18 18 18 18 | Column Road (*) | 20.00 | | 0.00 | 131.0 | 103.0 | 131 | 101.0 to 110.0 | 153.0 | | |
| The column The | 10 10 10 10 10 10 10 10 10 10 10 10 10 1 | 136.11 | | 80.0 | 131.0 | 131.0 | 133 | 101.0 10 138.0 | 183.0 | | |
| 10, 10, 10, 10, 10, 10, 10, 10, 10, 10, | CO C | 171.63 | | 136.0 | 138.0 | 140.7 | 167.7 | 138 0 10 156 7 | 100.7 | | |
| ### ### ### ### ### ### ### ### ### ## | 0 | 196.6 | | 141.0 | 168.0 | 175.0 | 178.1 | 168 0 to 189 0 | 255.0 | | |
| Continue | Internation Hwy 352 | 220.55 | | 170.0 | 210.0 | 211.8 | 244.0 | 210.0 to 218.5 | 26.03 | | |
| 286-58 190 200 200 200 200 200 200 200 200 200 2 | 0.0. Hwy 207 | 249.99 | | 223.0 | 285.0 | 285.0 | 2000 | 235.0 to 292.0 | 335.0 | | |
| Sec. 6, 6, 70, 20, 20, 20, 20, 20, 20, 20, 20, 20, 2 | State Hwy 31 | 264.58 | | 258.0 | 270.0 | 285.0 | 289.5 | 270.0 to 292.0 | 335.0 | | |
| Second | State Bwy 1129 | 265.50 | | 272.0 | 284.0 | 295.2 | 319.5 | 284.0 to 302.2 | 345.2 | | |
| State Stat | State Hwy 34 | 298.04 | | 296.0 | 308.0 | 320.5 | 343.0 | 308.0 to 327.5 | 370.2 | 345.0 | 1800 |
| The contribution of the | Malloy County Road | 312.04 | | 334.1 | 356.0 | 357.6 | 370.0 | 356 to 364.8 | 407.8 | 376.0 | 2000 |
| The state of the | Belt Line Road | 315.57 | | 339.5 | 356.0 | 350.0 | 377.8 | 350 to 303.0 | 0.904 | 385.0 | 2500 |
| Example Section Sect | DOWN FERRY ROBG (4) | 319.95 | | 347.0 | 372.0 | 373.5 | 390.0 | 372 to 360.0 | 423.0 | | · |
| Experimentary Sec. 20 Sec. 2 Se | Rear How look 10 Eact bound | 326.10 | | 0.038 | 0.075 | 370.0 | A08 c | 3 | 0 001 | ling a | 03160 |
| | State New Loop 12 (Westbound) | 326.20 | | 360.0 | 372.0 | 379.0 | 405 | 3 5 | 420.0 | 409 | 2150 |
| #\$\(\text{A}\) \$\(\text{S}\) \$\(\text{A}\) \$\(\text{S}\) \$\(\text{A}\) \$ | | 325.46 | | 360.0 | 372.0 | 300.0 | 100 | 000 | 430.8 | 412.7 | 2000 |
| 335. | Interstate Hyr 45(Southbound) | 328.47 | | 360.0 | 372.0 | 380.8 | 408.7 | 0 | 430.8 | 412.7 | 2000 |
| 332 332 4 1 10 364.2 350.0 370.1 413.3 350.0 to 00.0 10.0 10.0 10.0 10.0 10.0 10.0 10. | Forrest Avenue | 330.65 | | 360.0 | 372.0 | 354.3 | 411.6 | 03 | 432.3 | 415.6 | 2400 |
| 335 357-26 150 556-1 357-1 414-1 356-1 to 404.0 447-1 147-1 | Corinth Street | 331.41 | | 364.2 | 330.0 | 396.7 | 413.3 | 02 | 446.7 | 425.7 | 2300 |
| The color of the | Cadiz street | 25.000 | | 300.0 | 390.0 | 376.0 | 414.7 | 3 | 0.747 | 45/25 | 1950 |
| National Sistem 150, 250, 250, 250, 250, 250, 250, 250, 2 | Euston Street | 330.65 | | 368.1 | 0.00 | 397.2 | 415.3 | 3 5 | 0.74 | 1.075 | 2000 |
| 1333-50 150 371-2 336-5 141-5 366-6 141-7 141- | TOLL | 333.12 | | 369.0 | 396.0 | 397.3 | 416.3 | 0 | 447.3 | 153 | 1950 |
| The contribution of the co | | 333.50 | | 371.8 | 396.0 | 397.3 | 416.8 | 0 | 447.3 | 430.7 | 1800 |
| 39-33 150 37-7 38-0 39-2 448.7 38-0 43.14 12 37-2 150 38-0 39-2 448.7 38-0 49.2 13 37-2 150 38-0 39-0 49.1 14 37-2 150 38-0 39-0 49.1 15 38-2 39-0 40.0 40.2 15 38-2 39-0 40.0 40.2 15 38-2 39-0 40.0 15 38-2 39-0 40.0 16 38-2 49-2 49.1 17 49-2 49-3 18 49-2 49-3 18 49-2 49-3 18 49-2 49-3 18 49-2 49-3 18 49-2 49-3 18 49-2 49-3 18 49-2 49-3 18 49-2 49-3 18 49-2 49-3 18 49-3 | Continental Street | 333.93 | | 372.4 | 396.0 | 397.8 | 417.5 | 03 | 6.744 | 430.3 | 1900 |
| March Marc | Sylvan Moad | 334.00 | | 30. | 330.0 | 390.8 | 416.7 | 00 | 440.5 | 431.4 | 2000 |
| 10 10 10 10 10 10 10 10 | Manageton none Road (4) | 200.30 | | 39T. 0 | 306.0 | 10000 | 1000 | 3 . | 7.00 | 1.22 | 3000 |
| 34.5 | State Hwy Loop 12 | 340.30 | | 384.0 | 396.0 | 408.8 | 429.1 | 0 0 | 6.55 | 431.1 | 2001 |
| 18-5 200 19-5 1 | Meyers Road | 342.94 | | 402.8 | 424.0 | 424.9 | 438.0 | 100 | 474.9 | 442.0 | 2100 |
| 35%-00 12.0 | Belt Line Road | 345.25 | - | 409.7 | 424.0 | 425.1 | 6.444 | 03 | 475.1 | 448.9 | 2000 |
| 357-00 800 43.7 4-52.0 4770.1 452.0 to 459.9 502.9 474.1 470.1 452.0 to 459.9 502.9 474.1 470.1 450.0 to 459.9 502.9 474.1 457.0 to 450.0 | State Hwy 360 | 350.75 | | 412.0 | 454.0 | 426.1 | 459.5 | 0 | 476.1 | 463.5 | 3200 |
| 355,95 R00 thun, this this this this this this this this | FM Road 157 | 354.00 | | 431.7 | 452.0 | 452.9 | 470.1 | 03 | 505.9 | 474.1 | 1000 |
| 757-99 800 140-10 493-77 493-8 490-7 503-7 497-8 7 10 493-8 490-7 10 493-8 490-7 503-7 497-8 7 10 4 | Arlington-Sedford Road | 357.00 | | 0.044 | 455.0 | 453.8 | 477.0 | 00 | 503.2 | 4.62.3 | 3000 |
| 750, 100 150 150 150 150 150 150 150 150 150 | Arlington-Smithfleid Road | | | 140.0 | 450.0 | 453.7 | 403.0 | 0 | 503.7 | 467.0 | 3000 |
| 366.70 150 472.0 430.0 180.0 1 | The Boy Long Book (Court thround) | | | 450.0 | 1,400.0 | 0.101 | 173.0 | 0 | 331.0 | | None |
| 3966.00 120 472.0 L30.0 Rone 510.4 514.4 5 | Handley-Rie and 11e Road | | | 458.3 | 480.0 | 400.0 | 1000 B | 3 9 | 530.0 | 8 005 | None |
| 366.70 130 472.0 430.0 None 510.4 514.4 514.4 514.4 514.4 514.4 514.4 514.4 514.4 514.4 514.4 514.4 514.4 514.4 514.4 514.4 514.6 510.6 150.4 514.4 51 | Proposed Fort Worth Terminus | | | | | | 2.00 | 3 | 6.000 | 2 | 2007 |
| 369,40 800 488.0 5547.6 584.2 584.2 584.2 369.42 369.42 584.2 584.2 580.2 | Sast First Street | 366.90 | | 472.0 | 400.0 | None | 510.4 | | 514.4 | 514.4 | 1050 |
| 369,40 800 482.0 560.2 564.2 564.2 564.2 564.2 | Beach Street | 366,60 | | 480. | | | 517.6 | | 521.6 | 521.6 | 1700 |
| 569,41 200 402.0 524,2 524,2 | Riverside Drive (Northbound) | 369,40 | | 462.0 | | | 520.2 | | 554.2 | 554.5 | 1120 |
| | diverside Drive (bouthbound) | 203.44 | ACA. | 406.0 | | | 250.5 | | 2.426 | 254.6 | 0211 |

(1) Terform to staps of river which is equalled or exceeded 2 percent of time providing for mavigation 96 percent of time.

(2) Merers to elevation of maximan high stars of records in any below channel mile 286.5 and the standard project flood design water surface in proposed or existing levent floodways upstream of channel mile 286.5 and the standard project flood design water surface in proposed or existing levent floodways upstream of channel mile 286.5.

(3) Encode monotonistion are of immunery 1980.

(4) Ender construction are of immunery 1980 requires new bridge on land-cut channel for both the north- and southbound lanes.

TABLE 14

DATA RELATIVE TO INCREASED

HEIGHT OF HIGHWAY BRIDGES REQUIRED

FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL

| | | | | : Increased |
|------------------------------------|----------|-----------|----------|----------------|
| Name of bridge | : mile : | Existing: | Proposed | :bridge heigh |
| Interestate Here 10 (Feathound) | 30.36 | 14 | 58 | 2424 |
| Interstate Hwy 10 (Eastbound) | | 14 | 20 | 44 |
| Interstate Hwy 10 (Westbound) | 30.37 | | 58 | |
| J. S. Hwy No. 90 (Eastbound) | 47.84 | 29 | 77 | 48 |
| J. S. Hwy No. 90 (Westbound) | 47.90 | 29 | 77 | 48 |
| State Hwy No. 105 | 75.78 | 72 | 121 | 49 |
| J. S. Hwy No. 59 | 91.86 | 98 | 134 | 36 |
| county Road | 98.90 | 103 | 163 | 60 |
| J. S. Hwy No. 190 | 111.54 | 140 | 191 | 51 48 |
| State Hwy No. 19 | 136.15 | 148 | 196 | 48 |
| State Hwy No. 21 | 171.63 | 176 | 210 | 34 |
| State Hwy No. 7 | 196.68 | 202 | 235 | 33 |
| J. S. Hwy No. 79 & 84 | 220.55 | 239 | 271 | 32 45 45 |
| J. S. Hwy No. 287 (1) | 249.99 | 294 | 349 | 45 |
| State Hwy No. 31 (1) | 264.52 | 294 | 349 | 45 |
| tate Hwy No. 1129 | 285.60 | 322 | 355 | 33 |
| tate Hwy No. 34 | 298.04 | 349 | 381 | 32 |
| alloy County Road | 312.84 | 391 | 418 | 27 |
| elt Line Road | 315.57 | 396 | 420 | 24 |
| | | 400 | 434 | 34 |
| ordy Ferry Road | 319.92 | | | |
| tate Hwy Loop 12 (Eastbound) | 326.19 | 410 | 439 | 29 |
| tate Hwy Loop 12 (Westbound) | 326.20 | 410 | 439 | 29 |
| interstate Hwy No. 45 (Northbound) | 328.46 | 429 | 441 | 12 |
| nterstate Hwy No. 45 (Southbound) | 328.47 | 432 | 441 | 9 |
| 'orest Avenue | 330.65 | 425 | 442 | 17 |
| orinth St. | 331.41 | 429 | 457 | 28 |
| adiz Street | 332.22 | 429 | 457 | 28 |
| nterstate Hwy 35 E | 332.28 | 432 | 457 | 25 |
| ouston St. | 332.61 | 433 | 457 | 24 |
| allas-Ft. Worth Turnpike | 333.12 | 436 | 457 | 21 |
| ommerce St. | 333.50 | 434 | 457 | 23 |
| ontinental St. | 333.93 | 435 | 458 | 23 |
| ylvan Ave. | 334.89 | 408 | 458 | 50 |
| ampton Road | 336.33 | 440 | 460 | 20 |
| estmoreland Road | 337.26 | 415 | 460 | 45 |
| tate Hwy Loop 12 | 340.39 | 445 | 462 | 17 |
| eyers Road | 342.94 | 440 | 485 | 45 |
| elt Line Road | 345.25 | 454 | 485 | 31 |
| tate Hwy No. 360 | 350.75 | 450 | 486 | 36 |
| .M. Road No. 157 | 354.00 | 474 | 513 | 39 |
| rlington-Bedford Road | 357.00 | 481 | 513 | 32 |
| | | 487 | 514 | 27 |
| rlington-Smithfield Road | 359.95 | | 541 | |
| S. Hwy Loop 820 (Northbound) | 362.11 | 510 | | 31 |
| J. S. Hwy Loop 820 (Southbound) | 362.12 | 510 | 541 | 31 |
| andley-Ederville Road | 362.70 | 500 | 541 | 41 |

(1) Based on highway bridge required for single purpose Tennessee Colony flood control reservoir and its modification to provide clearance for navigation.

TABLE 15

DETAILED ESTIMATES OF FIRST COST FOR U.S. HIGHWAY BRIDGE NO. 90(WESTBOUND)

AT CHANNEL MILE 47.88 OVER MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH

The U. S. Highway No. 90 (westbound) new bridge would be constructed over cutoff channel at mile 47.88 consisting of 780-foot continuous plate girder unit (240-300-240), thirty-seven 50-foot prestressed concrete beam approach spans and earth fill approaches to a maximum height of 20' above natural ground. The bridge was designed for H20-S16 loading with a 28-foot roadway on 3.5% grade. Fender system would be provided for pier protection. Approximately 850' of approach spans on west end of existing bridge over Trinity River and entire relief structure located 0.5 miles west of the river would be removed. Eastbound bridge would be used for maintaining traffic during construction of new bridge.

| Item No. | Description | Quantity | Unit | Price | Amount |
|----------|---|-------------------------------|------------------------------|---------------------------------------|--|
| 8 | Earth fill approaches (1 a. Earth fill b. Approach Rd(Conc pymt. | 8,880 | CY | \$ 0.85 | \$7, 550 |
| | and sub-base) Subtotal | 250 | LF | 18.85 | 4,710 12,260 |
| 8 | Abutment Bents (1) a. Structural excavation b. Class "A" concrete | 25 30 | CY CY | 2.50 40.00 | 60 1,200 |
| ć | c. Reinforcing steel d. Concrete piling | 4,550 440 | Lbs LF | 0.13 7.50 | 590 3,300 |
| | e. Conc. riprap(Class"B" f. Cement Subtotal |) 115 45 | CY Bbl | 30.00 5.00 | 3,450 230 8,830 |
| 8 | Interior Bents (35) a. Structural excavation | 1,225 | CY | 2.50 | 3,060 |
| (| o. Class "A" concrete c. Reinforcing steel d. Concrete piling | 1,505 240,000 17,325 | CY Lbs LF | 40.00 0.13 7.50 | 60,200 31,200 129,940 |
| • | e. Cement Subtotal | 2,258 | Bbl | 5.00 | 11,290 235,690 |
| 1 | Pransition Bents (2) a. Structural excavation b. Class "A" concrete c. Reinforcing Steel d. Concrete piling | 100 197 31,200 2,310 | CY CY Lbs LF Bbl | 2.50 40.00 0.13 7.50 5.00 | 250 7,880 4,060 17,320 1,480 |
| | Subtotal | 296 | DUI | 9.00 | 30,990 |

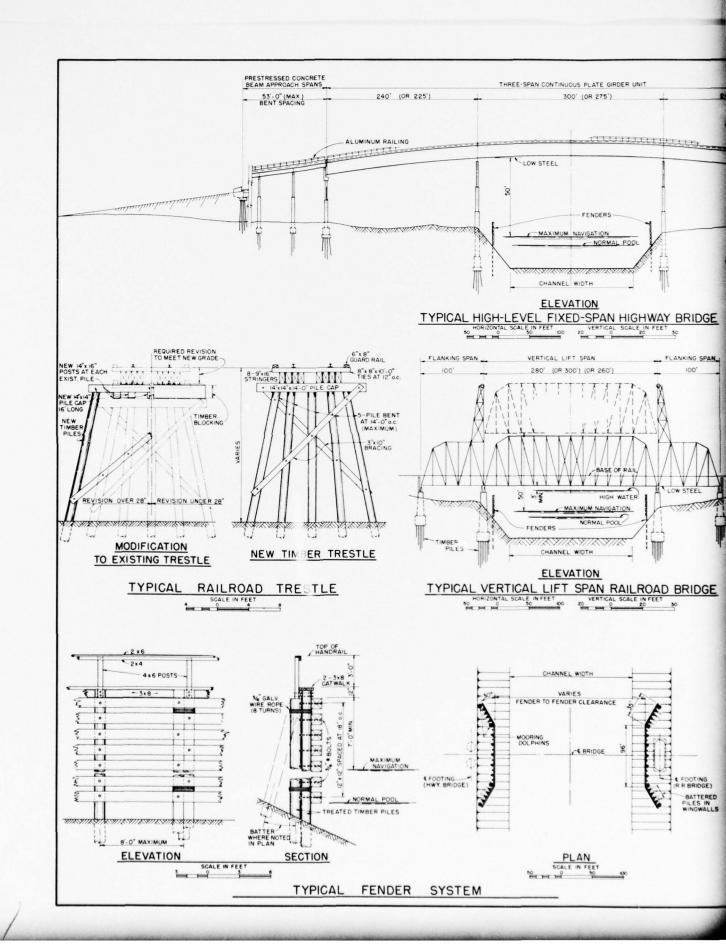
TABLE 15 (CONT'D)

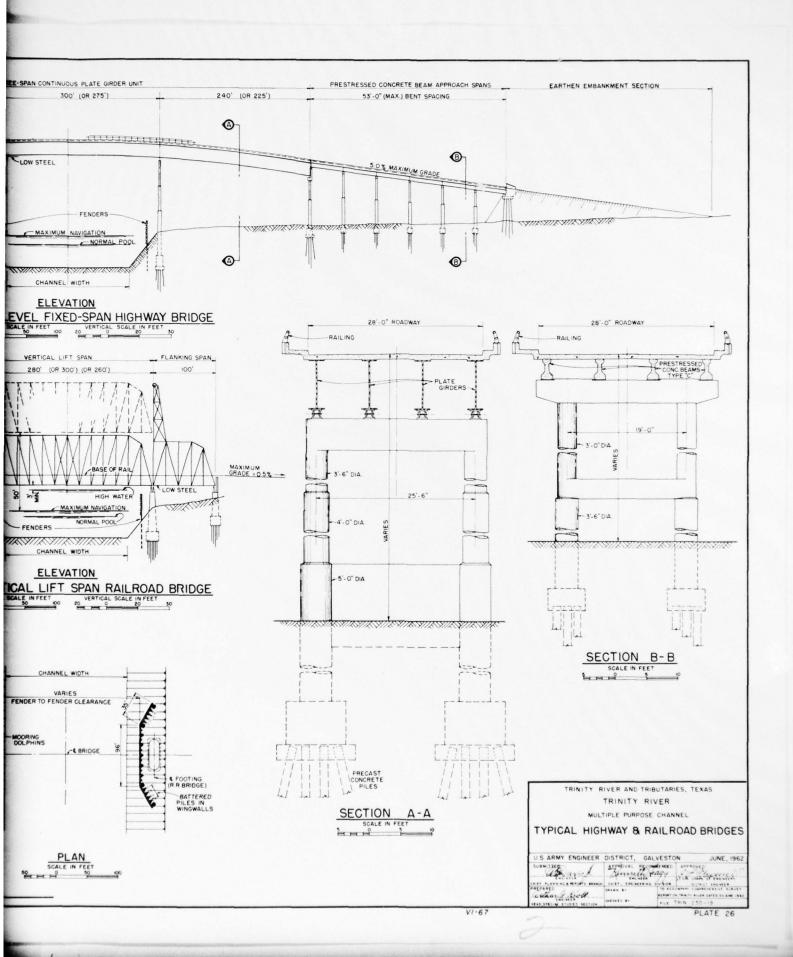
DETAILED ESTIMATES OF FIRST COST

FOR U.S. HIGHWAY BRIDGE NO. 90 (WESTBOUND)
AT CHANNEL MILE 47.88 OVER MULTIPLE PURPOSE TRINITY RIVER CHANNEL
TO FORT WORTH

| No. | | Quantity | <u>Unit</u> | Price | Amount |
|-----|--------------------------------------|----------------|-------------|--------|-----------|
| 5. | Main piers (2) | | | | |
| | a. Structural excavation | on 315 | CY | \$2.50 | \$ 790 |
| | b. Class "A" concrete | 456 | CY | 42.50 | 19,380 |
| | c. Reinforcing steel | 62,000 | Lbs | 0.13 | 8,060 |
| | d. Concrete piling | 4,620 | LF | 7.50 | 34,650 |
| | e. Cement | 684 | Bbl | 5.00 | 3,420 |
| , | Subtotal | () | | | 66,300 |
| 6. | Prestressed conc. beam | | ~ | 1.0.50 | (7 590 |
| | a. Class "A" concrete | | CY | 42.50 | 67,580 |
| | b. Reinforcing steel | 307,000 | Lbs | 0.13 | 39,910 |
| | c. Struct. steel (Armo: plates) | | Lbs | 0.14 | 710 |
| | d. Type "C" prestressed | 5,100 | TOP | 0.14 | 110 |
| | concrete beams | 7,400 | LF | 13.50 | 99,900 |
| | e. Aluminum railing | 3,700 | LF | 5.00 | 18,500 |
| | f. Cement | 2,385 | Bbl | 5.00 | 11,920 |
| | Subtotal | -,5-, | | | 238,520 |
| 7. | Plate girder spans (3) | | | | |
| | a. Class "A" concrete | (Slab) 610 | CY | 42.50 | 25,930 |
| | b. Reinforcing steel | 121,800 | Lbs | 0.13 | 15,830 |
| | c. Structural steel | 1,747,200 | Lbs | 0.22 | 384,380 |
| | d. Aluminum railing | 1,560 | LF | 5.00 | 7,800 |
| | e. Cement | 915 | Bbl | 5.00 | 4,580 |
| | f. Navigation light | | LS | | 2,500 |
| 0 | Subtota | | | | 441,020 |
| 8. | Fender system a. Structural steel (G | na ma | | | |
| | plates) | 2,250 | Lbs | 0.14 | 320 |
| | b. Treated timber (Creo | | FBM | 0.60 | 9,330 |
| | c. Treated piling(Creo | | LF | 3.25 | 21,120 |
| | Subtota | | | 3/ | 30,770 |
| 9. | Removal of existing br | | | | • , |
| | slab & girder spans | | LF | 15.00 | 21,300 |
| | | | | | |
| 10. | Salvage value, existing | | | | None |
| | Total c | onstruction co | ost | | 1,085,680 |
| | | | | | 0/ |
| | Rounded | | | | 1,086,000 |

Note: Prices are as of January 1962





pertinent data relative to increased height of highway bridges. Plate 26 shows typical views of the high level fixed bridge and timber pile fender system proposed for bridges crossing the navigable reaches of the multiple-purpose channel.

- 72. ESTIMATE OF FIRST COSTS OF HIGHWAY RELOCATIONS. The estimated material quantities for the proposed highway bridges over the project channel were determined from curves based on the newly constructed Aransas Pass, Texas, two-lane bridge across the Gulf Intracoastal Waterway. A detailed estimate of first cost for U. S. Highway Bridge No. 90 (westbound) at channel mile 47.88 is given in table 15. This estimate is a typical example of cost of similar type high level fixed highway bridges required for the project channel.
- 73. SUMMARY OF RAILROAD RELOCATIONS (INCLUDING COSTS).- Existing railroads cross the multiple purpose channel at 13 locations. Eight of the existing bridge crossing would be affected to some degree by the multiple purpose channel and would have to be modified to provide the clearances considered necessary for navigation on the channel. Four other bridges would not be affected as the project channel crosses the railroad embankments on cut-off alignment; however, new bridges providing necessary clearances would be required at the cut-off crossings. The estimated cost for modification of the St. Louis Southwestern Railroad crossing the project channel at mile 264.14 near Trinidad, Texas, is considered in the cost estimate for the proposed Tennessee Colony reservoir. Pertinent data and cost of the required railroad bridge alterations, estimated at a first cost of \$25,398,000, are shown in table 16.

74. New bridges required for the project channel are listed as item No. 1, 5, 6, and 13 in the following tabulation, which shows the average daily (1961) traffic crossing the existing bridges over the multiple purpose channel:

| Ite | m : Railroad | | | | existing | |
|-----|---------------------------|-------|----------|-----------|-------------|-----------|
| No. | : owner | :mile | Passenge | er:Freigh | t:Switching | ng:Total: |
| 1 | Texas & New Orleans(SP) | 47.9 | 4 4 | 6 | - | 10 |
| 2 | Missouri Pacific | 52.5 | 7 4 | 6 | - | 10 |
| 3 | Gulf, Colorado & Santa Fe | 77.2 | 3 - | - | - | 3(1) |
| 4 | Texas & New Orleans(SP) | 91.9 | 3 - | 7+ | - | 4 |
| 5 | Missouri Pacific | 136.0 | 3 4 | 3 | - | 7 |
| 6 | Missouri Pacific | 219.7 |) 4 | 4 | - | 8 |
| 7 | St. Louis & Southwestern | 264.1 | + - | 9 | - | 9 |
| 8 | Texas & New Orleans(SP) | 328.3 |) - | 6 | 12 | 16 |
| 9 | Missouri-Kansas-Texas | 330.2 | 3 8 | 6 | 4 | 18 |
| 10 | Gulf, Colorado & Santa Fe | 331.0 | 9 - | 4 | 6 | 10 |
| 11 | Texas & Pacific | 333.6 | 5 - | - | - | 42(1) |
| 12 | Gifford Hill Gravel Co. | 341.8 | 5 - | - | - | (2) |
| 13 | Chicago, Rock Island & | 350.5 | + 1 | - | - | 1 |
| | Pacific | | | | | |

⁽¹⁾ Composition of total traffic not reported.

(2) Traffic not reported.

TABLE 16
SUMMARY OF ESTIMATE PIST COSTS AND PERTINENT DATA
CONCERNING MATLEAGAD BRIDGES REQUIRED FOR MULTIPLE PURPOSE
TRINITY RIVER CHANNEL TO FORT WORTH, TEXAL

| | : | : | | lengths of | | | | | |
|--|---|--|-------------------------------------|-------------------|-------------------|------------------------------|------------------------|-------------------------------|--|
| | : | : | : Left | :Lift bridge | | | | Lineal | : Estimated |
| | | : Bridgework | | :& flanking | | | | | : total |
| Name of railroad | :Mile | : Required | : approach | : spans | : approac | h: length: | raise : | revision(1) | : cost |
| ouston Ship Channel to Upper Reach of Wa | llisville Re | servoir (Mile 0.0 | 0 to 35.5) | | | | | | None |
| one . | | | | | | | | | |
| per Reach of Wallisville Reservoir to h | ead of autho | rized channel to | Liberty (Mi | le 35.5 to 4 | 7.4) | | | | |
| ne | | | | | | | | | None |
| ad of authorized channel to Liberty to | Monneagen Co | long Dom (Mile le | 7 1 +0 222 6 | | | | | | |
| xas & New Orleans RR (SP) | 47.94 | New bridge | None | 500 | None | 500 | None | None | \$2,043,000 |
| ssouri Pacific | 52.57 | Modification | ~ | 500 | - | 500 | None | None | 2,028,000 |
| lf. Colorado & Santa Fe RR (AT & SF) | 77.28 | Modification | None | 480 | None | 480 | None | None | 1,971,00 |
| xas & New Orleans RR (SP) | 91.93 | Modification | None | 480 | None | 480 | 1.0(ft.) | 1,550 | 1,976,000 |
| ssouri Pacific | 136.08 | New bridge | None | 500 | None | 500 | None | None | 2,056,00 |
| ssouri Pacific | 219.70 | New bridge | None | 500 | None | 500 | None | None | 2,053,00 |
| Total - Mile 47.4 to 233.5 | | men errage | | , | | , | | | 12,127,00 |
| 10.00 11.00 25.50 | | | | | | | | | -,,- |
| nnessee Colony dam to Lock & Dam No. 12 | (mile 233.5 | to 274.4) | | | | | | | |
| . Louis Southwestern of Texas | 264.14 | Considered in | the plan of | improvement | for the To | ennessee C | Colony rese | rvoir. | |
| . Dours Southwestern of Texas | | | | | | | | | |
| | | | | | | | | | |
| ock & Dam No. 12 to Five-mile creek (Mil | e 274.4 to 3 | 22.0) | | | | | | | |
| ock & Dam No. 12 to Five-mile creek (Mil | e 274.4 to 3 | 22.0) | | | | | | | None |
| ock & Dam No. 12 to Five-mile creek (Mil | | | | | | | | | None |
| ock & Dam No. 12 to Five-mile creek (Mil one ive-mile Creek to & including Dallas Ter | | | | | | | | | |
| ock & Dam No. 12 to Five-mile creek (Mil one | | | | | | | | | None |
| ock & Dam No. 12 to Five-mile creek (Mil one ive-mile Creek to & including Dallas Ter one | mninus (Mile | 322.0 to 326.7) | | | | | | | |
| ock & Dam No. 12 to Five-mile creek (Mil one ove-mile Creek to & including Dallas Ter one | minus (Mile | 322.0 to 326.7) | 1.275 | 480 | 1 090 | 2.845 | 8.8(+,) | 5.710 | None |
| ock & Dam No. 12 to Five-mile creek (Mil ne ve-mile Creek to & including Dallas Ter ne llas Terminus to existing Dallas Floodw exas & New Orleans RR (SP) | minus (Mile | 322.0 to 326.7) | 1,275 | 480 460 | 1,090 | 2,845 | 8.8(ft.) | 5,710 6.640 | None 2,109,000 |
| ck & Dam No. 12 to Five-mile creek (Milne ve-mile Creek to & including Dallas Terme llas Terminus to existing Dallas Floodw xas & New Orleans RR (SP) ssouri-Kansas-Texas RR (MKT) | minus (Mile | 322.0 to 326.7) | 1,275 1,570 | 480 460 | 1,090 270 | 2,845 | 8.8(ft.) 13.4(ft.) | 5,710 6,640 | None 2,109,00 2,010,00 |
| cck & Dam No. 12 to Five-mile creek (Mil ne ve-mile Creek to & including Dallas Ter ne ne llas Terminus to existing Dallas Floodw xas & New Orleans RR (SP) | minus (Mile | 322.0 to 326.7) | | | | | | | None 2,109,00 2,010,00 |
| ck & Dam No. 12 to Five-mile creek (Milne ve-mile Creek to & including Dallas Terme llas Terminus to existing Dallas Floodw xas & New Orleans RR (SP) souri-Kansas-Texas RR (MKT) Total - Mile 326.7 to 331.1 | minus (Mile | 322.0 to 326.7) | | | | 2,300 | | | None 2,109,00 2,010,00 |
| cck & Dam No. 12 to Five-mile creek (Milme ve-mile Creek to & including Dallas Termine llas Terminus to existing Dallas Floodw xas & New Orleans RR (SP) ssouri-Kansas-Texas RR (MKT) Total - Mile 326.7 to 331.1 illas Floodway (Mile 331.1 to 338.6) | minus (Mile way (Mile 326 328.30 330.28 | 322.0 to 326.7) | | | | 2,300 | | 6,640 | None 2,109,000 2,010,000 4,119,000 |
| ck & Dam No. 12 to Five-mile creek (Milne ve-mile Creek to & including Dallas Ter ne llas Terminus to existing Dallas Floodw xas & New Orleans RR (SP) ssouri-Kansas-Texas RR (MKT) Total - Mile 326.7 to 331.1 to 338.8) lf, Colorado & Santa Fe RR (AT & SF) | minus (Mile 328.30 330.28 | 322.0 to 326.7) 7.7 to 331.1) Modification Modification | 1,570 | 460 | 270 | 2,300 | 13.4(ft.) | 6,640 | None 2,109,000 2,010,000 4,119,000 |
| ck & Dam No. 12 to Five-mile creek (Milne ve-mile Creek to & including Dallas Ter me lilas Terminus to existing Dallas Floodw txas & New Orleans RR (SP) ssouri-Kansas-Texas RR (MKT) Total - Mile 326.7 to 331.1 lilas Floodway (Mile 331.1 to 338.6) lif, Colorado & Santa Fe RR (AT & SF) | minus (Mile way (Mile 326 328.30 330.28 | 322.0 to 326.7) 7 to 331.1) Modification Modification | 1,570 | 460 | 270 | 2,300 | 13.4(ft.) | 6,640 540 | None 2,109,000 2,010,000 4,119,000 1,916,000 3,662,000 |
| cok & Dam No. 12 to Five-mile creek (Miles one compared to the creek to & including Dallas Termine compared to the compared to | minus (Mile 326 328.30 330.28 331.09 333.66 | 322.0 to 326.7) 7.7 to 331.1) Modification Modification Modification Modification | 1,570 40 N one | 460 | 270 | 2,300 | 13.4(ft.) | 6,640 540 | None 2,109,000 2,010,000 4,119,000 1,916,000 3,662,000 |
| cck & Dam No. 12 to Five-mile creek (Mil- ne ve-mile Creek to & including Dallas Ter- ne llias Terminus to existing Dallas Floodw xxas & New Orleans RR (SP) ssouri-Kansas-Texas RR (MKT) Total - Mile 326.7 to 331.1 llias Floodway (Mile 331.1 to 338.8) lif, Colorado & Santa Pe RR (AT & SF) xxas and Pacific RR (TP) Total - Mile 331.1 to 338.8 llias Floodway to & including Fort Worth | minus (Mile 326 328.30 330.28 331.09 333.66 | 322.0 to 326.7) 7.7 to 331.1) Modification Modification Modification Modification Modification | 1,570 40 None | 460 480 | 270 40 None | 2,300 460 480 | 13.4(ft.) .2(ft.) None | 6,640 540 None | None 2,109,000 2,010,000 4,119,000 1,916,000 3,662,000 5,575,000 |
| ck & Dam No. 12 to Five-mile creek (Milne ve-mile Creek to & including Dallas Ter me llas Terminus to existing Dallas Floode xas & New Orleans RR (SP) soouri-Kansas-Texas RR (MKT) Total - Mile 326.7 to 331.1 lilas Floodway (Mile 331.1 to 338.8) lif, Colorado & Santa Fe RR (AT & SF) xas and Pacific RR (TP) Total - Mile 331.1 to 338.8 illas Floodway to & including Fort Worth fford Bill Gravel Co. RR | minus (Mile 326 328.30 330.28 331.09 333.66 a Terminus (Mile 341.36 | 322.0 to 326.7) i.7 to 331.1) Modification Modification Modification Modification Modification Modification Modification | 1,570 40 None | 460 480 460 | 270 40 None | 2,300 460 480 2,000 | .2(ft.) None | 6,640 540 None 6,220 | None 2,109,000 2,010,000 4,119,000 1,916,000 3,662,000 5,575,000 |
| ck & Dam No. 12 to Five-mile creek (Milne ve-mile Creek to & including Dallas Ter ne llas Terminus to existing Dallas Floodw xas & New Orleans RR (SF) ssouri-Kansas-Texas RR (MKT) Total - Mile 326.7 to 331.1 llas Floodway (Mile 331.1 to 338.8) lf, Colorado & Santa Fe RR (AT & SF) xas and Pacific RR (TF) Total - Mile 331.1 to 338.8 llas Floodway to & including Fort Worth fford Hill Gravel Co. RR icago, Rock Island & Pacific RR | minus (Mile 326 328.30 330.28 331.09 333.66 | 322.0 to 326.7) 7.7 to 331.1) Modification Modification Modification Modification Modification | 1,570 40 None | 460 480 | 270 40 None | 2,300 460 480 | 13.4(ft.) .2(ft.) None | 6,640 540 None 6,220 | None 2,109,000 2,010,000 4,119,000 1,916,000 3,662,000 5,578,000 1,834,000 1,740,000 |
| ck & Dam No. 12 to Five-mile creek (Milne ve-mile Creek to & including Dallas Terme llas Terminus to existing Dallas Floodw xas & New Orleans RR (SP) soouri-Kannas-Texas RR (MKT) Total - Mile 326.7 to 331.1 llas Floodway (Mile 331.1 to 338.8) lf, Colorado & Santa Fe RR (AT & SF) xas and Pacific RR (TP) Total - Mile 331.1 to 338.8 llas Floodway to & including Fort Worth fford Bill Gravel Co. RR | minus (Mile 326 328.30 330.28 331.09 333.66 a Terminus (Mile 341.36 | 322.0 to 326.7) i.7 to 331.1) Modification Modification Modification Modification Modification Modification Modification | 1,570 40 None | 460 480 460 | 270 40 None | 2,300 460 480 2,000 | .2(ft.) None | 6,640 540 None 6,220 | None 2,109,000 2,010,000 4,119,000 1,916,000 3,662,000 5,578,000 1,834,000 1,740,000 |
| cek & Dam No. 12 to Five-mile creek (Milme ve-mile Creek to & including Dallas Termine llas Terminus to existing Dallas Floodw xas & New Orleans RR (SP) ssouri-Kansas-Texas RR (MKT) Total - Mile 326.7 to 331.1 llias Floodway (Mile 331.1 to 338.8) llf, Colorado & Santa Fe RR (NT & SF) xas and Pacific RR (TP) Total - Mile 331.1 to 338.8 llias Floodway to & including Fort Worth fford Bill Gravel Co. RR incago, Rock Island & Pacific RR Total Mile 338.8 to 362.8 | minus (Mile 326 328.30 330.28 331.09 333.66 1 Terminus (M 341.36 350.54 | 322.0 to 326.7) 7.7 to 331.1) Modification Modification Modification Modification Modification Modification Modification Modification Modification New bridge | 1,570 40 None 1,080 570 | 460 480 460 | 270 40 None | 2,300 460 480 2,000 | .2(ft.) None | 6,640 540 None 6,220 | None 2,109,000 2,010,000 4,119,000 1,916,000 3,662,000 5,578,000 1,834,000 1,740,000 |
| ck & Dam No. 12 to Five-mile creek (Milne ve-mile Creek to & including Dallas Ter me lilas Terminus to existing Dallas Floodw xxas & New Orleans RR (SP) ssouri-Kanasa-Texas RR (MKT) Total - Mile 326.7 to 331.1 lilas Floodway (Mile 331.1 to 338.8) lif, Colorado & Santa Pe RR (AT & SP) xxas and Pacific RR (TP) Total - Mile 331.1 to 338.8 lilas Floodway to & including Fort Worth fford Hill Gravel Co. RR total Mile 338.2 to 362.8 ort Worth Terminus to & including Rivers | minus (Mile 326 328.30 330.28 331.09 333.66 1 Terminus (M 341.36 350.54 | 322.0 to 326.7) 7.7 to 331.1) Modification Modification Modification Modification Modification Modification Modification Modification Modification New bridge | 1,570 40 None 1,080 570 | 460 480 460 | 270 40 None | 2,300 460 480 2,000 | .2(ft.) None | 6,640 540 None 6,220 | None 2,109,000 2,010,000 4,119,000 1,916,000 3,662,000 5,578,000 1,834,000 1,740,000 |
| ck & Dam No. 12 to Five-mile creek (Milne ve-mile Creek to & including Dallas Termine llas Terminus to existing Dallas Floodw xas & New Orleans RR (SP) ssouri-Kansas-Texas RR (MKT) Total - Mile 326.7 to 331.1 llas Floodway (Mile 331.1 to 338.8) lf, Colorado & Santa Pe RR (NT & 3F) xas and Pacific RR (TP) Total - Mile 331.1 to 338.8 llas Floodway to & including Fort Worth fford Bill Gravel Co. RR icago, Rock Island & Pacific RR Total Mile 338.8 to 362.8 | minus (Mile 326 328.30 330.28 331.09 333.66 1 Terminus (M 341.36 350.54 | 322.0 to 326.7) 7.7 to 331.1) Modification Modification Modification Modification Modification Modification Modification Modification Modification New bridge | 1,570 40 None 1,080 570 | 460 480 460 | 270 40 None | 2,300 460 480 2,000 | .2(ft.) None | 6,640 540 None 6,220 | None 2,109,000 2,010,000 4,119,000 1,915,000 3,662,000 5,575,000 1,834,000 1,740,000 3,576,000 |

- 75. RATLROAD BRIDGE DESIGN INFORMATION The plan of improvement provides for all railroad bridges over the project channel to be of the vertical lift type, with 100-foot flanking spans designed for Coopers E-65 loading. Timber trestle and/or earth embankment approaches to the flanking spans would be provided where necessary for new bridge crossings. Modification of existing bridges provides for the removal of the portion of bridge affected by the lift bridge and flanking spans and modifying the remaining portion of the existing bridge to meet the lift bridge requirements. A timber pile fender system would be provided on the channel side of the lift bridge piers as shown on plate 26, page 67. A timber trestle and/or earth embankment by-pass would be provided to maintain rail traffic during construction of the required bridge work.
- 76. The lift bridge would provide a minimum vertical clearance of 50 feet in open position above the stage that is equalled or exceeded 2 percent of time and a minimum vertical clearance of three feet in closed position above elevation of high water below channel mile 286.5 and three feet above standard project flood design water surface elevation in leveed floodways at the bridge site. The plans provide for minimum horizontal clearance between bridge fenders of 250 feet for bridges below the Dallas terminus and 225 feet for bridges between the Dallas and Fort Worth termini. Plate 26 shows typical views of the vertical lift and trestle bridges proposed for the multiple purpose channel.
- 77. Table 17 gives pertinent design data considered in determining the estimated first costs of the required railroad bridges. The data shows that grade modification is required for seven lift bridges to provide three feet of vertical clearance above the elevation of maximum highwater or standard project flood design water surface in leveed floodways as may be applicable. The grade revisions are based on 0.5 percent grades. Earth fill approaches are provided to a maximum height of 20 feet above natural ground with side slopes of two horizontal to one vertical with timber approach trestles to the flanking spans of the lift bridge as may be required, excepting floodways. Where railroads cross existing or proposed floodways, the plan provides for a vertical lift bridge over the project channel and the modification of the existing trestles or new trestles as may be required. All trestles across floodways provide a minimum of four feet of vertical clearance between bottom of stringers and the standard project design water surface elevation.
- 78. ESTIMATES OF FIRST COST OF RAILROAD RELOCATIONS. The estimated material quantities for the proposed railroad lift bridges over the project channel are based on "Estimating Data for Bridges and Miscellaneous Structures," prepared in 1932 for Calument-Sag Board, by First Chicago District, United States Engineers. The material quantities for all other bridge work were based on available plans of similar railroad bridges as modified and site-adapted. Detailed estimates of first cost for the new Texas and New Orleans Railroad bridge at channel mile 91.93 and for the modification of the Gulf, Colorado and Santa Fe Railroad bridge at channel mile 331.09 are given in tables 18 and 19, respectively, as typical examples for estimating the cost of all railroad bridges required for the project channel.

TABLE 17
PERTINENT DESIGN DATA OF PROPOSED RAILROAD BRIDGES REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

| Name of Bridge | : Channel: | channel | | : Normal : pool : elevation : (MSL) | : Elevation : : of design : : navigation: : stage : : (MSL)(1) : | Maximum high water elevation(| : Fenders : required 2): between : elevation(3 | : Des: :Lift :over |
|---|------------------|---------|-------|--|--|-------------------------------------|--|--------------------------|
| Houston Ship Channel to Dallas | s Terminus | | | | | | | |
| T. & N. O. RR | 47.94 | 250 | -13.0 | 16.0 | 17.1 | 26.3 | 16.0 to 24.1 | 29 |
| Missouri Pacific RR | 52.57 | 250 | 5.6 | 16.0 | 20.2 | 34.5 | 16.0 to 27.2 | 38 |
| G. C. & S. F. RR | 77.28 | 150 | 28.3 | 60.0 | 62.7 | 73.8 | 60.0 to 69.7 | 8 |
| T. & N.O. RR | 91.93 | 150 | 42.8 | 60.0 | 73.9 | 91.0 | 60.0 to 80.9 | 91 |
| Missouri Pacific RR | 136.08 | 150 | 89.0 | 131.0(7) | 131.0 | | 101.0 to 138.9 | 142 |
| Missouri Pacific RR S. L. & S. W. RR | 219.70 264.14 | 150 | 167.0 | 210.0 | 211.1 | | 210.0 to 218.1 270.0 to 282.0 | 228 |
| Proposed Dallas Terminus | 204.14 | 150 | 258.0 | 210.0 | 20).0 | 209.0 | 270.0 66 202.0 | 292 |
| T. & N. O. RR | 328.30 | 150 | 360.0 | 372 | 380.8 | 408.2 | 372.0 to 387.8 | 41: |
| M. K. & T. RR | 330.28 | 150 | 360.0 | 372 | 382.2 | | 372.0 to 389.2 | 41 |
| G. C. & S. F. RR | 331.09 | 150 | 360.0 | 372 | 382.5 | | 372.0 to 389.5 | 416 |
| T. & P. RR | 333.66 | 150 | 371.5 | 396 | 397.6 | | 396.0 to 404.6 | 428 |
| Gifford Hill Gravel Co. RR | 341.86 | 150 | 384.0 | 396 | 402.2 | 433.3 | 396.0 to 409.2 | 430 |
| C. R. I. & P. RR | 350.54 | 200 | 412.0 | 424 | 426.1 | 459.3 | 424.0 to 433.1 | 46 |

Refers to stage of river which is equalled or exceeded 2 percent of time providing for navigation 98 percent of time. Refers to elevation of maximum highwater of record at and below channel mile 322.0, and the standard project flood des in proposed or existing leveed floodways upstream of channel mile 322.0.

Based on providing bridge fenders extending from normal pool elevation to 7 feet above elevation of design navigation Elevation of base of rail on existing bridge or of railroad at proposed location of bridge over land cut channel. Total length of grade revision including lift bridge and flanking spans.

Based on maintaining existing grade of railroad at bridge crossing.

Top of conservation storage in Livingston Rexervoir which may be depleted to elevation 101.0.

TEXAS

| | | | | | | | - | | | | | |
|-----|-----------------------------|------------------------------|-------|------------------------------|-----------------------------|------------------|-------------------|----------|------------------------|-----------------------|-------------------------|-------------|
| | : : Fenders | | | l elevation (MSL) bridges | Leveed floodway | approsc | h bridge | : | Proposed r | ailroad grad | e modificat | ion |
| ter | : required | :Lift span | : | Flanking | Design | : Length | of appro | aches : | Elevation | base of rail | : Grade : | Lineal feet |
| | : between : elevation(3) | :over channe :Closed : Or | | spans (fixed) | Low-steel elevation(MSL) | : Left : bank | : Right : bank | : Total: | Proposed : bridge : | Existing track (4) | : raise : : (feet) : | 0 |
| | | | | | | | | | | | | |
| 16 | 6.0 to 24.1 | 29.3 | 67.1 | 29.3 | - | - | - | - | 37.3 | 33.5 | 3.8 | 1100 |
| 16 | 6.0 to 27.2 | 38.2(6) | 70.2 | 38.2 | - | - | - | - | 44.5 | 44.5 | 0.0 | 0 |
| 60 | 0.0 to 69.7 | 83.5(6) | 112.7 | 83.5 | - | - | - | - | 89.8 | 89.8 | 0.0 | 0 |
| 60 | 0.0 to 80.9 | 94.0 | 124.0 | 94.0 | - | - | - | - | 100.3 | 99.3 | 1.0 | 1070 |
| 10 | 1.0 to 138.9 | 142.7 | 183.0 | 142.7 | - | - | - | - | 149.0 | 149.0 | 0.0 | 0 |
| 210 | 0.0 to 218.1 | 228.9(6) | 261.1 | 228.9 | - | - | - | - | 235.2 | 235.2 | 0.0 | 0 |
| 270 | 0.0 to 282.0 | 292.0 | 335.0 | 292.0 | • | - | - | • | 298.3 | 296.1 | 2.2 | 880 |
| 372 | 2.0 to 387.8 | 411.2 | 430.8 | 411.2 | 412.2 | 720 | 900 | 1620 | 417.6 | 408.8 | 8.8 | 4000 |
| 372 | 2.0 to 389.2 | 414.0 | 432.2 | | 415.0 | 1570 | 270 | 1840 | | 407.3 | 13.0 | 6640 |
| 372 | 2.0 to 389.5 | 416.6 | 432.0 | 416.6 | 416.0 | 1270 | 1190 | 2460 | 421.3 | 421.1 | 0.2 | 80 |
| 396 | 6.0 to 404.6 | 428.8 | 447.6 | 428.8 | 428.0 | 980 | 540 | 1520 | 435.1 | 435.1 | 0.0 | 0 |
| 396 | 6.0 to 409.2 | 436.3 | 452.2 | 436.3 | 437.3 | 1080 | 460 | 1540 | 442.6 | 430.3 | 12.3 | 6220 |
| | 4.0 to 433.1 | 462.3 | 476.1 | 462.3 | 463.3 | 570 | 570 | 1140 | 468.6 | 457.0 | 11.6 | 5100 |

gation 98 percent of time. standard project flood design water surface

ration of design navigation stage. over land cut channel.

0.

VI-7I

TABLE 18

DETAILED ESTIMATES OF FIRST COSTS FOR THE T.& N.O.RR(SP) BRIDGE AT CHANNEL MILE 91.93 OVER MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH

A new bridge would be constructed over cutoff channel at mile 91.93 replacing approximately 500 feet of timber trestle and earthen embankment. The new bridge would consist of 280-foot vertical lift span with two 100-foot approach spans and is designed for single track E-65 loading. Base of rail would be raised approximately one foot. Fender system would be provided for pier protection. A temporary bypass of timber trestle and earth fill would be constructed for maintaining traffic during construction of the new bridge.

| Item | • | | | |
|----------------------------------|--|--|--|---|
| No. | Description | :Quantity: | Unit : | Price : Amount : |
| | xcavation (for all piers) Subtotal butment piers(2) | 34,000 | CY | \$2.50 <u>\$85,000</u> |
| 3. Ma a. b. | Concrete Reinforcing steel Cement Timber piles,untr.45'cl."B" Subtotal ain piers (2) Concrete Reinforcing steel Cement Timber,piles, untr.45'cl"B" Subtotal | 820 41,000 1,025 5,310 2,360 118,000 2,950 20,430 | CY Lbs Bbl LF CY Lb. Bbl. | 42.50 34,850 0.13 5,330 5.00 5,130 1.85 9,820 55,130 42.50 100,300 0.13 15,340 5.00 14,750 1.85 37,800 168,190 |
| a. b. c. d. e. f. g. h. i. j. k. | uperstructure Struct. steel in spans Struct. st. in towers & cwt Counterweight chains Ropes & machinery Conc. in counterweights Reinforcing Cement Electrical equipment Signal system Auxiliary power Hsg. (For pr. & machinery) Shoes, cast. & sheaves Ballast Ties Rail Subtotal | 2,125,560 863,372 46,489 358,631 404 20,900 520 284 75,000 21,300 | Lb. Lb. Lb. CY Lb. Bbl. LS LS LS LS LS LS LS LS LS | 0.30 637,670 0.35 302,170 0.35 16,270 1.00 358,630 40.00 16,160 0.13 2,720 5.00 2,600 75,000 13,000 7,500 12,000 24,000 5.00 1,420 0.15 11,250 0.15 3,200 1,483,590 |

TABLE 18 (CONT'D)

DETAILED ESTIMATES OF FIRST COSTS FOR THE T.& N.O. RR(SP)

BRIDGE AT CHANNEL MILE 91.93 OVER MULTIPLE PURPOSE TRINITY RIVER

CHANNEL TO FORT WORTH

| No.: Description 5. Fender system a. Timber piles, treat. cl."B" b. Guard timbers (Wales) Brac. & catwalk - treated c. Steel tangent plates d. Bolts & spikes | 8,500 61,500 11,000 7,200 | Unit: LF FBM Lb Lb | \$3.25 0.60 0.14 | #27,620 36,900 |
|--|------------------------------------|----------------------|------------------------|-------------------|
| a. Timber piles, treat. cl."B" b. Guard timbers (Wales) Brac. & catwalk - treated c. Steel tangent plates d. Bolts & spikes | 61,500 11,000 | FBM Lb | 0.60 | 36,900 |
| b. Guard timbers (Wales) Brac. & catwalk - treated c. Steel tangent plates d. Bolts & spikes | 61,500 11,000 | FBM Lb | 0.60 | 36,900 |
| Brac. & catwalk - treated c. Steel tangent plates d. Bolts & spikes | 11,000 | Lb | | |
| c. Steel tangent platesd. Bolts & spikes | 11,000 | Lb | | |
| d. Bolts & spikes | | | 0.14 | 7 -1 - |
| | 7,200 | 1.0 | 0 00 | 1,540 |
| Coltant | | 110 | 0.20 | 1,440 |
| Subtotal | | | | 67,500 |
| 6. Removal of exist. structure | | | | |
| Remove pile trestle | 320 | LF | 10.00 | 3,200 |
| Subtotal | | | | 3,200 |
| 7. Rev. elev. of exist. track to m | neet | | | |
| new bridge | | | | |
| a. Earthen embankment (770 LF) | | | | 60 - |
| 1.Fill | 2,700 | CY | 0.25 | 680 |
| 2.Ballast | 380 | CY | 5.00 | 4,400 |
| b. Trestle (300 LF) | 21,000 | FBM | 0.40 | 9,600 |
| Timber blocking c. Hardware (Galvanized) | 900 | Lb | 0.40 | 270 |
| d. Spikes, plates, bolts | 8,000 | Lb | 0.20 | 1,600 |
| e. Ties | 20,800 | FBM | 0.10 | 2,080 |
| f. Reinstall exist. track | 1,070 | LF | 1.00 | 1,070 |
| Subtotal | | | | 19,700 |
| 8. Temporary by-pass | | | | |
| a. Earthen embankment (421 LF) | | | | |
| 1. Fill | 9,000 | CY | 0.25 | 2,250 |
| 2. Ballast | 760 | CY | 5.00 | 3,800 |
| 3. Ties | 20,300 | FBM | 0.10 | 2,030 |
| b. Trestle(1,100 LF) | n!! 0.010 | | 2.05 | 7 100 |
| 1. Timber, piles, treat.cl." | | LF Lb | 3.25 0.40 | 7,180 |
| 2. Timber (Dimensional) c. Hardware (Galvanized) | 89,700 7,900 | Lb | 0.40 | 35,880 2,370 |
| d. Rail (New) | 116,800 | Гр | 0.15 | 17,520 |
| e. Spikes, plates, bolts | 12,200 | Lb | 0.20 | 2,440 |
| f. Ties (Switch) | 136,200 | FBM | 0.15 | 20,430 |
| Subtotal | -3-,-00 | | / | 93,900 |
| 9. Salvage value | | | | None |
| Total construction cost | | | 1 | ,976,210 |
| Rounded | | | 1, | ,976,000 |

Note: Prices are as of January 1962

TABLE 19

DETAILED ESTIMATES OF FIRST COSTS FOR THE G. C.& S.F. RR BRIDGE AT CHANNEL MILE 331.09 OVER MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH

A new bridge would be constructed over the proposed channel at mile 331.09 replacing the existing 198-foot thru truss span and about 280 feet of open deck pile trestle. The new bridge would consist of a 260-foot vertical lift span and two 100-foot flanking spans. The new bridge would be designed for single track E-65 loading. Existing approaches would be raised 0.2-foot on 0.5% grades to meet the new bridge elevation. A fender system would be provided for pier protection. Temporary by-pass of earth fill and timber trestle would be constructed for maintaining traffic during construction of the new bridge.

| Item | : | | : : | : | |
|------|--|------------|------|-------|-------------------|
| No. | : Description | : Quantity | | | Amount |
| 1. E | Excavation (for all piers) | 20,000 | CY | 2.50 | |
| | Subtotal | | | | 50,000 |
| | Abutment piers (2) | 0 | | 1 | 12 560 |
| | Concrete | 978 | CY | 42.50 | |
| | Reinforcing steel | 48,900 | Lbs. | 0.13 | |
| | Cement | 1,220 | Bbl | 5.00 | |
| d. | Timber piles, untreat.45'cl." | в" 6,210 | LF | 1.85 | 11,490 |
| | ain piers (2) | | | | |
| | Concrete | 2,764 | CY | 42.50 | |
| | Reinforcing steel | 138,200 | Lb | 0.13 | |
| | Cement | 3,455 | Bb1. | 5.00 | |
| d. | Timber piles, untr. 45' cl. "B" Subtotal | 22,140 | LF | 1.85 | 40,960 193,670 |
| . Su | perstructure | | | | |
| | Struct. steel in spans | 1,931,976 | Lb | 0.30 | |
| | Struct. st. in towers & cwt. | 742,210 | Lb | 0.35 | |
| | Counterweight chains | 40,000 | Lb | 0.35 | |
| | Ropes & machinery | 296,883 | Lb | 1.00 | , , |
| | Conc. in counterweight | 348 | CY | 40.00 | -0,, |
| | Reinforcing | 17,400 | Lb | 0.13 | |
| | Cement | 435 | Bbl | 5.00 | |
| | Electrical equipment | | LS | | 75,000 |
| | Signal system | | LS | | 13,000 |
| | Auxiliary power | | LS | | 7,500 |
| | Hsg. (Operator & machinery) | | LS | | 12,000 |
| | Shoes, castings & sheaves | -/- | LS | | 24,000 |
| | Ballast | 265 | CY | 5.00 | 1,330 |
| | Ties | 70,000 | FBM | 0.15 | 10,500 |
| 0. | Rail | 19,800 | Lb | 0.15 | 2,970 |
| | Subtotal | | | | L,314,900 |
| | | | | | |

TABLE 19 (CONT'D)

DETAILED ESTIMATES OF FIRST COSTS FOR THE G. C. & S.F. RR

BRIDGE AT CHANNEL MILE 331.09 OVER MULTIPLE PURPOSE TRINITY RIVER

CHANNEL TO FORT WORTH

| Ite No. | | : Quantity: | Unit | : Price: | : Amount |
|------------|-----------------------------------|--------------|------|----------|---------------|
| | | | | | |
| 5• | Fender system | | | 4 | |
| | a. Timber piles, treat.cl."B" | 6,600 | LF | \$ 3.25 | \$21,450 |
| | b. Guard timbers (Wales) treat. | 38,400 | FBM | 0.60 | 23,040 |
| | c. Brac. & Catwalk - treat | 3,900 | FBM | 0.60 | 2,340 |
| | d. Steel tangent plates | 6,930 | Lb | 0.14 | 970 |
| | e. Bolts & spikes | 4,800 | Lb | 0.20 | 960 48,760 |
| | Removal of exist. structure | | | | |
| | a. Remove thru truss span | 200 | LF | 18.50 | 3,700 |
| | b. Remove pile trestle | 280 | LF | 10.00 | 2,800 |
| | Subtotal | | | | 6,500 |
| | Temporary by-pass 2,116 LF | | | | |
| | a. Earthen embankment 335 LF | | | | |
| | 1. Fill | 18,000 | CY | 0.25 | 4,500 |
| | 2. Ballast | 350 | CY | 5.00 | 1,750 |
| | 3. Ties | 9,100 | FBM | 0.10 | 910 |
| | b. Trestle (1,781 LF) | | 122 | | |
| | 1. Timber piles treat. cl."B" | | LF | 3.25 | 109,690 |
| | 2. Timber (Dimensional) | 217,900 | FBM | 0.40 | 87,160 |
| | . Hardware (Galv) | 19,000 | Lb | 0.30 | 5,700 |
| | i. Rail | 162,000 | Lb | 0.15 | 24,300 |
| | e. Spikes, plates, bolts | 16,900 | Lb | 0.20 | 3,380 |
| 1 | f. Ties | 57,800 | FBM | 0.15 | 8,670 |
| | Subtotal | | | | 246,060 |
| 3. 5 | Salvage value thru truss span, es | stimated \$9 | ,000 | | -9,000 |
| | Total construct | tion cost | | \$: | 1,916,400 |
| | Rounded | | | | 1,916,000 |

- 79. SUMMARY OF PIPELINE RELOCATIONS (INCLUDING COSTS). The estimated costs of pipeline relocations required for the multiple purpose channel are based on providing the same number and size of pipes which the respective companies installed at their pipeline crossings of the Trinity River. Accordingly, a total of lll pipeline crossings would be required for the multiple purpose channel between the Houston Ship Channel and Fort Worth, Texas, of which 37 are required below Liberty and 74 are required above Liberty. Four gathering lines of 2.5-inch size cross the completed portion of the channel to Liberty in the vicinity of Double Bayou, channel mile 15.7. These lines would not require relocation to provide for deepening to 12 feet the existing 9 x 150-foot channel to Liberty project.
- 80. Fifteen of the 37 pipeline crossings required below Liberty are located in the vicinity of channel mile 30.6 on the multiple purpose cut-off channel alinement proposed in this report near Wallisville, Texas. The cost of relocating these lines are assigned to the multiple purpose project because these lines need not be relocated at the river crossing to provide for the authorized channel to Liberty project.
- 81. From channel mile 30.6 to channel mile 47.4, the multiple purpose channel alinement generally coincides with the authorized alinement of the uncompleted channel to Liberty project. In this reach, 22 pipe lines would be relocated to provide for either the multiple purpose channel or the authorized channel to Liberty. Since the authorized channel to Liberty project provides for relocating the pipelines affected by the 9 x 150-foot channel, the cost of any additional length of pipeline relocation over that required for the 9 x 150-foot channel is assigned to the multiple purpose channel. The cost of pipeline relocations upstream of Liberty are assigned to the multiple purpose channel project. Alteration costs for the pipeline relocations in the Tennessee Colony Reservoir are contained in the estimate of cost of Tennessee Colony Reservoir.

82. The estimated costs of pipeline relocations are based on the pipelines extending 25 feet beyond the bottom width of the channel at a depth of either 25 feet below normal pool to provide for probable deepening of the navigation sections of the channel, or five feet below the bottom of the multiple purpose channel, whichever is deeper. The estimate of costs also provides for a pipeline valve to be installed on each side of the channel, with manifold headers if required, and the pipelines to be coated or wrapped and secured with sufficient weights. The unit cost of excavation and backfill is estimated at 55 cents per cubic yard. The unit installed costs for the various size of pipes and valves are shown in the following tabulation.

| Size of pipe, | : Unit cos | sts (Installed) |
|------------------|-----------------|-----------------|
| diameter(inches) | : Pipeline (LF) | : Valves (each) |
| 3" | \$ 7.10 | \$ 250.00 |
| 4 " | 8.00 | 290.00 |
| 6" | 12.00 | 360.00 |
| 7" | 13.50 | 400.00 |
| 8" | 15.00 | 430.00 |
| 10" | 18.00 | 500.00 |
| 12" | 24.00 | 610.00 |
| 14" | 24.30 | 1,160.00 |
| 16" | 30.00 | 1,510.00 |
| 18" | 39.00 | 1,880.00 |
| 20" | 46.00 | 2,260.00 |
| 24" | 58.00 | 3,520.00 |
| 26" | 66.00 | 4,180.00 |
| 30" | 84.00 | 5,480.00 |

A summary of the cost of pipeline relocations required for the multiple purpose channel to Fort Worth is given in table 20.

TABLE 20
SUMMARY OF COST OF PIPELINE RELOCATIONS
REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL
TO FORT WORTH, TEXAS

| | Total Costs | | \$99,700 | 22,990 | 63,090 | 16,460 | 56,500 | 8,910 | 10,440 | 14,960 | 35,580 | 7,590 | 54,180 | 15,740 | 13,820 | | | | 419,960 |
|----------|-----------------------|----------------------|---------------|---------------|----------|------------------|----------|----------|----------|----------|---------------------|--------------------|--------------------|-------------|----------------|-------------------|-----------------|--------------------|---------------|
| • 0 • 0 | Valves : Cost : | | \$1.0,960 | 3,020 | 0,040 | 860 | 10,960 | 500 | 960 | 1,000 | 4,520 | 720 | 10,960 | 860 | 860 | l project | project | project | |
| ts: | No. | 0.0 to 35.5) | 2,200 2 | 4,245 2 | 7 000 7 | 5,060 2 | 5,440 2 | 1,500 2 | 2,321 2 | 3,948 2 | 2 000 5 | 1,770 2 | 3,440 2 | 5,955 2 | 3,341 2 | authorized | authorized | authorized | |
| Weights | 4 | e e | \$71,400 \$10 | 2,750 | 9,100 | 6,375 | 35,700 6 | 3,018 | 210 | 550 | - | 5,100 | 5,700 | 6,375 | 6,075 | included in a | included in a | included in a | |
| | Pipeline LF : Cost | Reservoir (Mile | | 1,25 | | | | | | | 405 | | | | | charge-cost inc | charge-cost inc | | |
| | Excavation | Wallisville | \$7,140 | 2,975 | 5,950 | 4,165 | 3,400 | 3,892 | 1,184 | 1,462 | 2,430 | (1) | 4,080 | 2,550 | 3,54 | No charg | No charg | No charg | |
| Number : | tty: | upper reach of Wa | -30" Gas | -16" Prod. | -20" Gas | 1-8" Prod. | 30" Gas | 3" Gas | -8" Ges | -10" Gas | 1-20" Gas | 6" 'Prod. | 30" Gas | -8"Ethylene | | 1-12" 011 | -20" Oil | 2-10" L.P. | |
| . Nu | Channel size & | \$ | 30° | 30.6 | 30.6 | 30°6 | 30,6 | 30.6 | 30.6 | 30.6 | 30.6 | 0.30°6 | co .30.6 1 | 30.6 | 30.6 | ထ့ | 34.8 | .co35.0 2 | 0 to 35.5 |
| | Name of owner | Houston Ship Channel | sterm | stern | stern | Service P.L. Co. | L. Co. | L. Co. | L. Co. | L. Co. | United Cas P.L. Co. | Tennessee Gas P.L. | Pennessee Gas P.L. | et Co. | Corp. | Sinclair P.L. Co. | L. Co. | Frans.Southern P.L | Total-Mile 0. |
| | Name o | Houston | Texas Ea | Texas Eastern | Texas Ea | Service | Humble P | Humble P | Humble P | Humble P | United G | Tennesse | Tennesse | Warren P | Gulf Oil Corp. | Sinclair | Texas P.L. Co. | Trans.So | Tot |

TABLE 20 (CONT'D)
SUMMARY OF COST OF PIPELINE RELOCATIONS
REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL
TO FORT WORTH, TEXAS

| Total | 47.4) | 6,290 | 7,690 | 4,210 | 5,840 | 2,510 | 2,790 | 3,780 | 3,960 | 2,790 | 4,080 | 2,790 | 4,140 | 010 120 |
|-------------------------------------|---|-------------------|----------------|--------|----------------|------------|----------------|---------------------|---------------|----------------|----------------------|----------------|----------------|----------------|
| es Cost | 1 | • | 0 | ě | ı | , | i | í | ı | ı | 1 | 1 | 1 | |
| Valves | cty (| 8 | 1 | , | 6 | 9 | • | 0 | 1 | 1 | • | 1 | 1 | |
| :Weights: :costing: :& misc.: | to Liberty (Mile | 1,650 | 290 | 550 | 1,630 | 535 | 250 | 1,070 | 1,000 | 250 | 280 | 250 | 250 | |
| + | d channel | 2,250 | 3,600 | 1,800 | 3,000 | 1,350 | 800 | 1,200 | 1,800 | 1,200 | 1,800 | 1,200 | 1,500 | |
| Pipeline Th | thorize | 150 | 150 | 150 | 200 | 100 | 1.00 | 100 | 100 | 100 | 100 | 100 | 100 | |
| Excavation | Reservoir to Read of authorized | 2,390 | 800 | 1,860 | 1,210 | 625 | 1,740 | 1,510 | 1,160 | 1,340 | 2,000 | 1,340 | 2,390 | |
| | Prod. | 26.53 | 011 | 011 | 011 | 688 | 011 | Gas | Gas | Oil | Gas | 111 | 011 | |
| :Number :of pipe :size & | Reserve | - | - | 1-6" | | | | | - | | 1-10" | 1-6" | 1-8" | 1.7 |
| : :Channe] :mile | 41.0 | 41.0 | 42.9 | 42.1 | 44.5 | 9.77 | 45.9 | 5. 45.7 | 7. 45.7 | 45.7 | D.Co46.4 | 4.74 | 4°24 | 25 5 40 |
| Мете от очиви | Upper reach of Walli Magnolia P.L. Co. | Magnolia P.L. Co. | Cities Service | L. Co. | Gulf Oil Corp. | e P.L. Co. | Gulf Oil Corp. | United Gas P.L. Co. | d Gas P.L. Co | Gulf Oil Corp. | Industrial Gas Sup.C | Gulf Oil Corp. | Humble Oil Co. | TO OF TO TO TO |
| E SA | Upper | Magno | Citie | Sun F | Gulf | Humb] | Gulf | Unite | Unite | Gulf | Indus | Gulf | Humb1 | |

TABLE 20 (CONT'D)
SUMMARY OF COST OF PIPELINE RELOCATIONS
REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL
TO FORT WORTH, TEXAS

| | ••• | : Mumber | ber : | | | •0 | Weights | | •• | |
|------------------------|----------|----------|-------------|------------|------------|----------|---------|------|-----------|---------|
| | | ;of | bibe : | | | •• | Coating | : | •• | |
| | :Channel | Sig | & | | : Pipeline | | & misc. | Va | valves : | Total |
| Name of owner | : mile | :com | nodity: F | Excavation | H | Cost | f.tems | No | Cost: | costs |
| | 1 | | 1 | | | | | | | |
| Liberty to Tennessee | Colony | n n | Wile 4'(of | . ! | (| 0,00 | 4 | (| 0,00 | 0.10 |
| United Gas P.L. Co. | 47.8 | φ - | Gas | \$6,488 | 1,140 | \$34,260 | \$1,442 | N | 200 | 440,020 |
| Sun P.L. Co. | 1,64 | 1-6" | 01.1 | 11,530 | 040 | 7,680 | 1,600 | CV | 720 | 21,530 |
| Sinclair P.L.Co. | 49.1 | 1-12" | 011 | 9,973 | 959 | 15,024 | 1,873 | Q. | 1,220 | 28,090 |
| Sun P. L. Co. | 49.3 | 1-8 | 011 | 11,520 | 640 | 009'6 | 1,760 | N | 860 | 23,740 |
| Magnolia P.L. Co. | 51.9 | 2-8 | 011 | 8,698 | 1,216 | 18,240 | 2,132 | 7 | 1,720 | 30,790 |
| Trans. Cont. Gas F.L.C | 1052.4 | 2-24" | (Fees | 9,417 | 1,232 | 71,456 | 5,557 | _+ | 14,080 | 100,510 |
| Trans.Cont. Gas P.L.C | ,052,4 | 1-30" | Gas | 9,417 | 616 | 51,744 | 3,079 | cu | 10,960 | 75,200 |
| Magnolia P.L. Co. | 55.2 | 2-10" | LPG | 126.6 | 929 | 11,268 | 1,565 | N | 1,000 | 23,810 |
| Magnolia P.L. Co. | 55.5 | 2 8 | 011 | 9,975 | 929 | 9,390 | 1,565 | CJ. | 960 | 21,790 |
| Houston P.L. Co. | 9°69 | 1-18" | Gas | 80,000(1 | 900 | 31,200 | 1,200 | 0 | 10,000(4) | 122,400 |
| Houston P.L. Co. | 9°69 | 1-12" | Gas | 5,011 | 514 | 12,336 | 1,543 | CJ. | 1,220 | 20,110 |
| Atlantic P.L. Co. | 9.49 | 2-10" | 011 | 8,751 | 1,156 | 35,258(| 2)2,021 | - † | 2,000 | 48,030 |
| Trunkline Gas Sup.Co | 6.69.6 | 2-24" | | 9,719 | 1,156 | 67,04.3 | 4,913 | 7 | 14,080 | 95,760 |
| Gulf Oil Corp. | 6.11 | 1-10" | | 7,526 | 528 | 405,6 | 1,320 | N | 1,000 | 19,350 |
| Tennessee Gas P.L.Co | .85.0 | 1-6" | (7) | 11,428 | 728 | 17,472 | 1,820 | C) | 1,220 | 31,940 |
| Tennessee Gas P.L.Co | ,86,3 | 1-30" | Ge 53 | | 728 | 61,152 | 3,640 | cv | 10,960 | 87,180 |
| Tennessee Gas P.L.Co | ,86,3 | 1-26" | S. 65 | | 128 | 48,048 | 3,274 | cu : | 8,360 | 71,110 |
| Tennessee Gas P.L.Co | ,86,3 | 3-16" | Cass | 14,800 | 2,184 | 65,520 | 7,640 | 9. | 090,6 | 97,020 |
| Natural Cas F.L.Co. | 90,2 | 2-24" | Ses. | | 1,204 | 69,832 | 5,113 | | 14,080 | 103,320 |
| United Cas F.L. CO. | 91.8 | 5-8" | Ce, a | | 2,990 | 44,850 | 2,475 | 10 | 4,300 | 68,160 |
| United Gas P.L. Co. | 91.8 | 3~20" | Gas | | 1,794 | 82,524 | 7,173 | 0 | 13,560 | 119,150 |
| Service P.L. Co. | 93.0 | 1-12" | 011 | 10,937 | 572 | 13,728 | 1,715 | cv. | 1,820 | 27,600 |
| Shell P.L. Co. | 93.0 | 2-10" | 011 | 11,287 | 1,156 | 20,808 | 2,895 | 4 | 2,000 | 36,990 |
| Shell P.L. Co. | 93.0 | 1-6" | 011 | 11,039 | 578 | 6,936 | 1,445 | cu | 720 | 20,140 |
| United Gas P.L. Co | 135.7 | 2-6" | 608 | 26,374 | 1,668 | 20,016 | 4,170 | CJ. | 720 | 51,280 |
| Gulf Oil Corp. | 9°061 | 1-26" | 011 | 13,331 | 1,216 | 80,256 | 5,473 | .+ | 16,720 | 115,780 |

TABLE 20 (CONT'D)
SUMMARY OF COST OF PIPELINE RELOCATIONS
REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL
TO FORT WORTH, TEXAS

| | | · Mumber | 1. | | | 3 | Weights: | | 1. | |
|-----------------------------|---------------|-----------------------|-------|-----------------------|-----------------|----------------|------------|--------|-----------|------------|
| | | of pipe | | | | | coating: | | | |
| | :Channe | :Channel:size & | •4 | | : Pipeline | | & Misc.: | Valves | | Total |
| Name of Owner | : Mile | :commodity | ty: | Excavation: | T. | :Cost: 1 | items : | No. | Cost: | Costs |
| Liberty to Tennesse | ee Colony | Dam (Mile | | 47.4 to 233.5 |)(Cont'd | d) | | | | |
| Lone Star Gas Co. | 205.9 | 2-6" Gas | S S | \$11,950 | 608 | \$9,120 | \$1,510 | N | \$ 860 | \$23,440 |
| Humble P.L.Co. | 205.9 | | - | 12,338 | 709 | 4,832 | 1,510 | N | | 19,260 |
| Humble P.L. Co. | 207.8 | 1-4" 011 | Ч | 12,338 | 1 09 | 4,832 | 1,510 | α. | 280 | 19,260 |
| Magnolia P.L. Co. | 209.8 | 1-20" 011 | Ч | 12,750 | 1,156 | 53,176 | 4,624 | 4 | 0,040 | 79,590 |
| Lone Star Gas Co. | 220.8 | 1-12" Gas | ĽΩ | 4,520 | 740 | 11,280 | 1,410 | a | 1,220 | 18,430 |
| Gulf Oil Corp. | 221.4 | 2-10" LPG | Ç | 2,922 | 1,196 | 21,528 | 2,990 | 4 | 2,000 | 59,440 |
| Humble P.L. Co. | 230.5 | 1-10" 01-1 | ٦ | 10,548 | 578 | 10,404 | 1,448 | N | 1,000 | 23,400 |
| Humble P.L. Co. | 230.5 | 2-8" LPG | Ü | 10,548 | 1,156 | 17,340 | 2,532 | 4 | 1,720 | 32,140 |
| Sinclair P.L. Co. | 231.5 | 2-12" 011 | ٦ | 9,752 | 1,104 | 36,496 | 3,312 | 4 | 2,440 | 45,000 |
| Total - Mile 47 | .4 to 233 | 3.5 | | | | | | | | 1,799,790 |
| Tennessee Colony Dam | m & Reservoir | roir | le 2 | (Mile 233.5 to 274,4) | (1, | | | | | |
| Note: Estimated cost of all | st of al | pipe | e re | relocations i | in this | this reach are | e included | d in | estimated | ed cost of |
| Tennessee Colony reservoir | lony res | ervoir. | | | | | | | | |
| Tennessee Colony Re | servoir | eservoir to Five-mile | ile | Creek (Mile 274.4 | 274.4 | to 322.0 | _ | | | |
| Humble P.L. Co. | 276.2 | 1-4" 011 | | 7,491 | 558 | 494,4 | 1,395 | 0 0 | 580 | 13,930 |
| Total - Mile 274 | .4 to 322.0 | 3 | 4 | | | | 1 | 1 | 2 | 55,000 |
| | % includ | including Dallas | | Terminus (Mile | e 322.0 | to 326. | () | | | |
| United Gas P.L. Co. | 322.8 | 1-18" Gas | 8 | 6,355 | 502 | 19,578 | .] | cu | 3,760 | 31,450 |
| United Gas P.L. Co. | 324.6 | -50" | 02 | 8,660 | 540 | 54,840 | 3,160 | cu | 4,520 | 41,180 |
| CU | | 7.0 | , | | 7 - 20 | | | | | 72,630 |
| | | FLOODWay (Mile | Mile | | 331.1) | | | | , | |
| Ione Star Gas Co. | 328.1 | 3-12" Cas | to tr | 14,490 | 678 | 96,000 | 3,695 | 000 | 3,660 | 124,540 |
| Total - Mile 326.8 | | , , | , | 10.6. | , | | 11260 | | Ĺ | 147,130 |

TABLE 20 (CONT'D)
SUMMARY OF COST OF PIPELINE RELOCATIONS
REQUIRED FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

| | | Total | COSTS | \$31,540 21,340 52,880 | | 19,870 | 49,820 | 20,610 | 14,060 | 22,470 | 18,130 | 30,970 | 26,520 | | 24,690,828,770 |
|----------|-----|-----------|----------------------------------|--|--|---------|-----------------------------------|---------|---------------------------------|-----------------------------------|-------------------------------------|------------------------------|--|--|--|
| •• | • • | - 1 | COST | \$3,020 1,220 | | 720 | 4,520 | 860 | 860 | 1,000 | 1,000 | 3,020 | 3,020 | | 1,000 |
| | , | Valves | NO | a a | | N | N | CU | a | a | CU | N | CI | 369.8) | cu |
| Weights: | | ••! | rems : | \$4,105 3,442 | | 2,655 | 1,000 | 1,460 | 1,325 | 2,630 | 1,450 | 1,910 | 1,526 | 362.8 to | 1,658 |
| M : | •• | | :Cost: | \$18,060 11,568 | 8) | 7,752 | 33,580 | 8,760 | 6,420 | 11,736 | 9,720 | 16,380 | 15,240 | (Mile | 11,952 |
| | •• | | 4 | 602 | to 362,8 | 949 | 730 | 584 | 1,28 | 652 | 540 | 246 | 508 | bridges | 1799 |
| | | 1 | EXCAVATION: | \$ 6,355 5,110 | Mile 338.8 | 8,743 | 10,720 | 9,530 | 5,455 | 7,104 | 5,960 | 099,6 | 6,734 | Drive | 10,080 |
| : Number | •• | :Channel: | Name of owner : mile : commodity | Dallas Floodway (Mile 331.1 to 338.8) Lone Star Gas Co. 332.7 1-16" Gas United Gas P.L.Co. 333.1 1-12" Gas Total - (Mile 331.1 to 338.8) | Dallas Floodway to Fort Worth Terminus (| 339.0 1 | Lone Star Gas Co. 340.0 1-20" Gas | 340.1 1 | Texas P.I. Co. 341.8 1-8" Prod. | Lone Star Gas Co. 353.3 1-10" Gas | Sinclair P.L. Co. 354.5 1-10" Prod. | lia P.L. Co. 360.9 1-16" 0il | United Gas P.L. Co. 362.0 1-16" Gas Total - (Mile 338.8 to 362.8) | Fort Worth Terminus to & Including Riverside | Lone Star Gas Co. 364.2 1-10" Gas Total - Mile 0.0 to 369.8 |
| | | : | Name | Dalla Ione Unite | Dalla | Lone | Lone | Humb] | Texas | Lone | Sincl | Magno | Unite | Fort | Lone |

Prices are as of January 1962 Note:

Estimate covers 2-100-foot towers and suspension bridges to carry elevated pipeline providing 50-foot clearance above pose.

Estimate provides for installing pipelines in 14-inch casings. Includes 520° of 12" sleeve @ 48.00 per LF. Includes costs of valves and fitting for everhead line. £396

- 83. SUMMARY OF POWER TRANSMISSION LINE RELOCATIONS (INCLUDING COSTS) .- This item includes the alteration of all power transmission lines crossing the navigable portion of the multiple purpose channel between the Houston Ship Channel and Fort Worth with the exception of the reach located in Tennessee Colony reservoir. Alteration costs for the power transmission lines crossing the Tennessee Colony reservoir are contained in the estimate of cost for the Tennessee Colony reservoir. Powerlines crossing the navigable portion of the channel would be raised to conform with minimum clearance requirements given in information pamphlet entitled "Permits for Work in Navigable Waterways of the United States." In general all transmission lines crossing the multiple purpose channel, having voltages of 115 KV or less would have a minimum vertical clearance of 70 feet, and lines having voltages greater than 115 KV and not more than 138 KV would have minimum vertical clearances of 75 feet above the backwater elevation of the operating discharge at the crossing. Where the transmission lines cross the multiple purpose channel in river cut-off alinement, and extends across the Trinity River, the lines crossing the Trinity River would have a minimum clearance of 25 feet above the 50-year flood elevation at the river crossing.
- 84. FIRST COST. Power line alteration costs total \$710,800. The costs for alteration of power transmission lines include in general the cost of constructing towers on each side of the channel and installing new cable between towers. Salvage value of old cable has been subtracted from the construction cost. The estimated first costs and other data for relocation of the individual power transmission lines required for the multiple purpose channel are given in table 21.

TABLE 21 SUMMARY OF FIRST COSTS OF MODIFICATION TO ELECTRIC-POWER TRANSMISSION LINES TO PROVIDE FOR THE MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

| Owner of | :Iceation : | Voltage | : Total |
|--|------------------------|----------------|------------------|
| power line | :(Channel mile): | (K.V.) | : costs |
| Houston Ship Channel to upp | er Reach of Wallist | ville Reservoi | <u>.r-</u> |
| (Mile 0.0 to 35.5) None | | | None |
| Upper reach of Wallisville | | of authorized | |
| Channel to Liberty (Mile Gulf States Utilities | 35.5 to 4(.4) 47.30 | 69 | \$22,000 |
| Gulf States Utilities Total - Mile 35.5 to 47.4 | 47.30 | 138 | 47,800 69,800 |

TABLE 21 (CONT'D)
SUMMARY OF FIRST COSTS OF MODIFICATION TO ELECTRICPOWER TRANSMISSION LINES TO PROVIDE FOR THE MULTIPLE
PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

| Owner of | :Location : | | Total | : |
|---|-------------------------|--------------|-----------------|----|
| power line | :(Channel mile): | (K.V.) | costs | _: |
| Head of authorized channel Colony Dam (Mile 47.4 to 2 | | essee | | |
| Gulf States Utilities | 47.66 | 69 | \$22,000 | |
| Gulf States Utilities | 76.70 | 38.5 | 21,800 | |
| Sam Houston Electric Co. | 77.27 | 12.5 | 19,800 | |
| Gulf States Utilities | 91.91 | 34.5 | 16,800 | |
| Sam Houston Electric Co. | 114.5 | 12.5 | 31,400 | |
| Gulf States Utilities | 127.72 | 138 | 38,800 | |
| Gulf States Utilities | 136.05 | 69 | 34,600 | |
| Gulf States Utilities | 137.28 | 33 | 34,200 | |
| | 196.69 | 138 | 34,000 | |
| Texas Power & Light Co. | | 138 | 31,800 | |
| Texas Power & Light Co. | 216.19 | 12.5 | 27,000 | |
| Texas Power & Light Co. | 219.70 | 7.5 | 8,000 | |
| Texas Power & Light Co. | 220.57 | | 12,000 | |
| Texas Power & Light Co. Total-Mile 47.4 to 233. | 220.57 | 12.5 | 332,200 | |
| Total-Mile 4(.4 to 233. | • > | | 332,200 | |
| Note: Estimated cost of recent are included in Reservoir. | elocation of power | lines within | n this | |
| Lock No. 12 to Five-mile Co | | | 10,000 | |
| Texas Power & Light Co. | 299.70 | 12.5 | 12,000 | |
| Texas Power & Light Co. | 312.91 | 2.4 | 12,000 8,000 | |
| Texas Power & Light Co. | 320.0 | 2.4 | | |
| Total-Mile 274.4 to 322 | 2.0 | | 32,000 | |
| Five-Mile Creek to Dallas ? | Terminus (Mile 322. | 0 to 326.7) | None | |
| Dellas Terminus to Dallas B | 71 - oderner (M47 o 206 | 7 +0 221 11 | | |
| Dallas Power & Light Co. | 326.7 | 60 | \$10,600 | |
| Dallas Power & Light Co. | 328.8 | 60 | 34,600 | |
| Dallas Power & Light Co. | 331.1 | 60 | 7,000 | |
| Total-Mile 326.7 to 331 | | 00 | 52,200 | |
| 10001-0110 9200 00 991 | - • - | | ,_,_ | |
| Dallas Floodway (Mile 331.1 | to 338.8) | | | |
| Dellas Power & Light Co. | 332.6 | 13 | 6,600 | |
| Dallas Power & Light Co. | 333.5 | 13 | 6,600 | |
| Dallas Power & Light Co. | 334.0 | 60 | 11,000 | |
| Total Mile 331.1 to 338 | | | 24,200 | |
| | | | | |

TABLE 21 (CONT'D)
SUMMARY OF FIRST COSTS OF MODIFICATION TO ELECTRICPOWER TRANSMISSION LINES TO PROVIDE FOR THE MULTIPLE
PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

| Owner of power line | : Location : (channel mile) | | : Total : costs |
|--|---|--|---|
| Dallas Floodway to Fort Worth Te | rminus (Mile 338.8 | | |
| Dallas Power & Light Co. Dallas Power & Light Co. Texas Power & Light Co. Texas Electric Service Texas Power & Light Co. Texas Electric Service Co. Texas Electric Service Co. Total Mile 338.8 to 362.8 | 339.0 340.0 342.9 345.2 348.1 351.4 362.8 | 60 138 12.5 12.5 138 12.5 | 12,000 32,000 12,000 21,000 29,000 48,600 45,800 200,400 |
| Fort Worth Terminus to and inclu (Mile 362.8 to 369.8) | ding Riverside Dri | ve bridges | |

None

Total - Mile 0.0 to 362.8

None \$710,800

85. SUMMARY OF COMMUNICATION LINE RELOCATIONS (INCLUDING COSTS).-The plan of improvement for the multiple purpose channel provides for modification of 17 existing communication lines, one of which is a proposed crossing. The proposed crossing at Westmoreland Road, channel mile 338.5, is to be installed in 1962 and for this report is considered as existing. The proposed crossing along with 14 other existing crossings are owned by Southwestern Bell Telephone Company, seven of which lie below Dallas. The other two existing crossings on the Trinity River are the 7-wire telephone crossings at channel mile 45.6 owned by the Gulf Oil Corporation and the 2-wire telephone crossing at channel mile 64.6 owned by Atlantic Pipe Line Co. Of the total 17 crossings, 5 are aerial crossings attached to existing bridges, 11 are aerial lines on poles and one is a buried armoured cable at channel mile 326.6. The existing aerial lines on poles are on 35 to 40-foot poles and are to be modified by installing taller poles of sufficient height to provide a vertical clearance of 70 feet above normal pool elevation of the proposed channel at maximum sag. The existing aerial lines attached to bridges will be modified and attached to new or modified bridges at the same location. The buried cable crossing will be modified to provide a minimum depth below the normal pool elevation of 25 feet, or 5 feet below the bottom of the proposed channel, whichever is deeper. Alteration costs for communication relocations in the Tennessee Colony Reservoir are contained in the estimate of cost for the Tennessee Colony Reservoir.

86. A detailed estimate of two typical communication line crossings are given in table 22.

TABLE 22

ESTIMATED FIRST COST FOR RELOCATION OF BELL TELEPHONE COMPANY'S SIX CABLE CROSSING AT MODIFIED CADIZ STREET BRIDGE AT CHANNEL MILE 332.2 and NINE CABLE CROSSING AT MODIFIED HOUSTON STREET BRIDGE, MILE 332.6

| | | | ** ** | m-1-2 |
|--|-------|--------|---|---------|
| T1 0 1 1 | : :: | | :Unit: | |
| Item of construction | | uantit | y:ost: | dost |
| Cadiz Street bridge (Channel mile 332.2 | | | | |
| One cable, 200 pair 19 gauge and 500 pa | | 0000 | 471.00 | 409 000 |
| 22 gauge | LF | 2000 | \$14.00 | |
| One cable, 1400 pair 24 gauge and | LF | 2000 | 30.00 | |
| 2 pair video | LF | 2000 | 5.00 | |
| ne cable, 900 pair 22 gauge | LF | 2000 | | 36,000 |
| ne cable, 455 pair 19 gauge | LF | 2000 | 8.10 | 16,200 |
| me cable, 200 pair 19 gauge | | | | 1 |
| and 800 pair 24 gauge | LF | 2000 | | 40,000 |
| ne cable 900 pair 27 gauge | LF | 2000 | | |
| plicing and checking | M.Hr. | 1080 | 10.00 | |
| discellaneous items | LS | | | 6,400 |
| ransportation & plant | LS | | | 11,500 |
| Total | | | 5 | 254,900 |
| | | | | |
| Houston Street bridge, (Channel mile 332 | .6) | | | |
| One cable, 200 pair 19 gauge | | | | |
| and 400 pair 24 gauge | LF | 2000 | 100000000000000000000000000000000000000 | |
| ne cable, 400 pair 22 gauge | LF | 2000 | 8.00 | 16,000 |
| me cable, 25 pair 16 gauge | | | | |
| and 250 pair 19 gauge | LF | 2000 | 5.50 | |
| me cable, 900 pair 22 gauge | LF | 2000 | 18.00 | 36,000 |
| me cable, 54 pair 19 gauge (toll) | LF | 2000 | 1.10 | 2,200 |
| me cable, 64 pair 16 gauge (toll) | LF | 2000 | 1.30 | 2,600 |
| me cable, 6 pr 16 gauge, 296 pair | | | | |
| 19 gauge and 2 pr. 22 gauge (toll) | LF | 2000 | 6.30 | 12,600 |
| me cable, 168 pr. 19 gauge (toll) | LF | 2000 | 3.40 | |
| me cable, 8 pr. 16 gauge, 8 pr 19 gaug | 9 | | | |
| and 8 co-axials (toll) | LF | 2000 | 1.00 | 2,000 |
| plicing and checking | M Hr. | | 10.00 | |
| Hiscellaneous items | IS | , | | 5,000 |
| Pransportation & plant | IS | | | 10,000 |
| Total | | | | 133,800 |
| 10.000 | | | | |

Note: Prices are as of January 1962.

^{87.} Table 23 gives pertinent data and costs of communication lines required to be alterated or relocated. The estimate first cost is \$1,176,950.

SPECIAL HEISON, LIDAR OL TEMBRES MEANTH MAINTH STORMAN STREAM AND ASSIST HOLLYCHINGHOD SERVOTERS OF THEM.

CONTROL THE STREAM ST

| Houston Shin Channel to Horar Res | Mile) | : Type of crossing : | : Poles | : Miscel. : (2) | Poles : Miscel. : Wire (1) : (2) : (L.F.) | : Cost | Cable | . L.F. | : Cost : | . & . Checking | | & : Total Plant: Cost |
|---|----------------|---|-------------|--------------------|---|----------|---|--------|----------|----------------|--------|---------------------------|
| None | ich of Wall | Ship Channel to Upper Reach of Wallisville Reservoir (Mile 0.0 to 35.2) | Mile 0.0 t | 0.35.51 | | | | | | | | None |
| Upper Reach of Wallieville Reservoir to bend of Authorized channel to Liberty (ails 35.5 to 47.4). 15.6 7-wire, merial 31,300 \$1,000 \$7,000 \$ | olr to beach | of Authorized chan 7-wire, aerial | \$1,800 | \$1,000 | 35.5 to 47 | ·+) | | | | \$300 | \$500 | \$5,870 |
| Co. | Liberty to Ter | (2) | Mile 47.4 | 12 | | | 24qD-19 Ga. | 1,500 | \$3,000 | 500 | 3,000 | 8,90 |
| S. W. Bell Telephone Co. Atlantic P. L. Co. | 6.45 | 2-vire, serial | 1,800 | 1,000 | 3,600 | 3,000 | 76Pr-19 Ga. | 2,000 | 9,000 | 888 | 2,300 | 5,610 |
| | 77.3 | 30-wire, serial | 1,800 | 1,890 | 000,000 | 0 | | | | 700 | 8,50 | 8,6 |
| W. Bell Telephone Co. | 136.1 | 26-wire aerial | 800 | 1,000 | 33.800 | 1.000 | 75Pr-19 Ga. | 1,000 | 3,000 | 520 | 88 | 2,35 |
| | 219.0 | 2-vire, aerial | 1,800 | 2,000 | 3,600 | 110 % | | | | 88 | 888 | 2,610 |
| Tennessee Colony Dam to Lock & Dam No. 12 (Mile 233.5 to $ZT^{4}, 4$) Worse | M No. 12 () | tile 233.5 to 274.4 | , | | | | | | | | | None |
| Look & Dum No. 12 to Five-mile Greek (Mile 274.4 to 322.0) None | eek (Mile | 774.4 to 322.0) | | | | | | | | | | None |
| Five-mile Greek to Dallas Terminus (Mile 322,0 to 326.7) S. 4. Dell Telephone Co. 326.6 2-cables, buried | 326.6 | 2.0 to 326.7) 2-cables, buried | , | 22,250 | | | (3) | 900 | 26,400 | 2,600 | 2,000 | 59,250 |
| Dallas Terminus to Dallas Floodway (Mile 326.7 to 331.1) None | y (Mile 326 | 5.7 to 331.1) | | | | | | | | | | None |
| llas Floodway (Mile 331.1 to 33 | 18.8) | And blac service | | 9 | | | (4) | | 98 | of or | 85 | 001 |
| W. Bell Telephone Co. | 332.6 | 9-cables.aerial | | 5,000 | | | (5) | | 113,200 | 5.600 | 10.000 | 133.80 |
| S. W. Bell Telephone Co. S. W. Bell Telephone Co. 3 Total - Mile 331,1 to 338.8 | 333.5 | 4-cables, aerial | 1 1 | 3,500 | | | 393 | | 163,000 | 7,540 | 2,000 | 202 900 202 800 110 |
| llss Floodway to Fort Worth Ter | minus (Mile | 338.8 to 362.8) | | , | | | | | | | | |
| 8. W. Bell Telephone Co. 345.7 S. W. Bell Telephone Co. 345.3 S. W. Bell Telephone Co. 354.1 Total - Mile 338.6 to 362.8 | 345.3 354.1 | l-cable, serial l-cable, serial l-cable, serial | 1,200 | 1,200 | | 238 | 200pr22 ga. 400pr24 ga. 300pr22 ga. | 1,380 | 7,800 | 988 | 888 | 38,130 |
| Fort Worth Terminus to & including Riverside Drive Bridges (Mile 362.8 to 369.9) None | g Riverside | Drive Bridges (Mil | le 362.8 to | 369.9) | | | | | | | | None |
| Total - Mile 0.0 to 369.8 | | | | | | | | | | | | 941,560 |

Note: Prices are as of January 1962

308380 E

Installed cost of timber pile "f" frumes and bracing.

Menclascows teems such as insulation X-terms increase; bangers and excavation.

One cable 200 pair, 19 gauge, and one cable 500 pair, 22 gauge.

Oables to be suspended from modified Gairs Street Bridge, see detailed estimate Pable 27.

Cables to be suspended from modified Gameroe Street Bridge, see detailed estimate Rable 27.

Cables to be suspended from modified Gameroe Street Bridge constating of 1-1500 pair, 24 gauge, 1-500 pair, 22 gauge, and 1-1500 pair, 22 gauge, and 1-150 pair, 19 gauge (toil) cables.

Cables to be suspended from modified Westsoreland bridge, consisting of 1-2100 pair, 26 gauge, 1-500 pair, 22 gauge and 1-1500 pair, 24 gauge cables.

- 88. SUMMARY OF WATER AND SEWER LINE RELOCATION (INCLUDING COSTS).Modification of six existing water lines and seven sewer lines crossing
 the multiple-purpose channel to Fort Worth would be required. Twelve of
 the lines are owned by the City of Dallas and one water line is owned by
 the City of Grand Prairie. All lines are single lines except at channel
 mile 328.92, a 24" and 36" sewer line crossing the river joins into one
 84" line on each side of the river and would require replacement of 25'
 of 84" line beyond the junction of 24" and 36" lines in addition to the
 modification of the two lines crossing the river. The cost of modifying
 the water and sewer lines is based on placing each line at a minimum elevation of 25 feet below the normal pool elevation or 5 feet below the
 bottom of the proposed channel, whichever is deeper, and extending the
 line 25 feet beyond the bottom width of the proposed channel. Table 24
 gives data and cost on required alteration of water and sewer lines. The
 estimated first cost is \$588,290.
- 89. SUMMARY OF ACCESS ROADS TO LOCKS AND DAMS (INCLUDING COSTS).—
 The proposed alignments of the access roads were selected after field
 reconnaissance and consultation with local personnel familiar with each
 lock and dam location. The proposed 21 access roads would connect with
 the nearest improved all weather road and would have a 20-foot wide double
 bituminous type pavement and 6-foot wide flexible base shoulders. The
 alignments of existing graded roads would be used whenever they are favorably located.
- 90. Road embankments within the Trinity River flood plain would have a top elevation 5 feet above maximum highwater of record. At locations where road alignments cross perpendicular to the direction of flood flows approximately one-fourth of the roadway would be made of timber trestles to provide relief during flooding. The creosoted timber trestles with asphaltic concrete covered decks would have a 22-foot wide roadway. Pertinent data for each access road are given in table 25.
- 91. Estimates of first cost to provide the proposed access roads are based on unit costs as shown in the following tabulation:

| | : | | |
|---------------------------------|----------|------|-----------------|
| Item | <u>:</u> | Unit | : Unit cost |
| Clearing and grubbing (light) | | Ac | \$150.00 |
| Clearing and grubbing (heavy) | | Ac | 300.00 |
| Excavation | | CY | 0.60 |
| Select material base | | CY | 2.75 to 3.00 |
| Flexible base | | CY | 3.50 |
| Prime coat | | Gal. | 0.20 |
| Double bituminous treatment | | SY | 0.85 |
| Timber trestle (relief bridges) |) | LF | 150.00 |
| Small drainage structures | | DA | 500.00 |
| Fencing | | LF | 0.80 |
| Marking traffic signs | | Sum | 50.00 to 350.00 |
| Rework existing road crown | | SY | 0.15 |
| Bridge approach fill | | CY | 0.85 |

Table 26, that follows, shows cost of access roads to lock and dam sites estimated at \$5,018,990.

TABLE 24
SUMMARY OF FIRST COSTS OF WATER AND SEWER LINE
MODIFICATIONS REQUIRED FOR MULTIPLE FURPOSE TRINITY RIVER
CHANNEL TO FORT WORTH, TEXAS

| hts: | & misc.: Total | s costs | | | | | | \$146,820 | | | | 75 86,900 | | 1 | | | | 22,580 | | | • | | 24,460 | | COC BBIG |
|--------------|----------------|---------------|--|----------------------|--------------------------------|--------------------|--------------------|---------------------------|----------------------|--------------------------------|--------------------|--------------------|--------------------|---------------------------|-----------------------------|-------------------------|-------------------------|-------------------------|--------------------|-------------------------|---------------------------|-------------------------------|-----------------------|--|---------------------|
| tt : Weights | . & mi | 00 | The second secon | | | 1,000 | | | | 00 | 00 | 22,0 | 5 1,250 | | | 788 8,2 | 2,000 | | | 00 1,250 | | | • | None | |
| : Coat | جة د. د. | t : wrap | | | | 50 \$855 | | | | 009 00 | 009 00 | - 00 | 201,115 | | | | | 84 1,520 | | ٦ | | | 8 | | - |
| 00 | . Pipe | : cost | | | \$67,8 | 25,560 | 18,4 | | | 17,4 | 30,08 | 5,0 | 66,900 | | | 8,55 | 38,4 | 17,184 | 39,4 | 50,2 | | | | to 369.76) | |
| 60 | | H | one | | | 5 426 | | | | 300 | 300 | | 5 892 | | | | | 6 358 | | | | | | 36 | - |
| | :Excavation: | cost | 2.0) - None | | \$12,900 | | 8,740 | | | 3 | • | 12,22 | 13,195 | | | 6,60 | 8,00 | 3,876 | | | | 52.8) | 10,96 | Bridges (Mile | |
| e and | :kind of | pipe | 0 to 322.0 | 0 326.7 | 21 RCP | 4" Steel | 8" CI | | (Mile 325.7 to 331.1 | TO | 5" CI | 4" RCP | | | | 15 " t | O" Cone, | h" CI | 6" Steel | 8" Steel | | 338.8 to 362.8) | 1-36" Steel 10,960 | | |
| Size | kin | 60 | Creek (Mile 0.0 | (Mile 322.0 to 326.7 | | 1-24" | | | 326°7 t | 1-24" | | | | | | | 1-60" | | 1-36" | | | tile 338 | | ide Drive | - |
| 0.6 | | : Utility | Creek (| | Water | Sewer | Sewer | | | 1 | Sewer | | | | 3.8) | Water | Sewer | | Water | Water | | Terminus (Mile | Water | Rivers | 76 076 07 |
| 6.0 | :Channel | ; mile | Five-mile | as Perminus | 322.85 | 324.27 | 326.2 | 326.7 | as Floodway | 328,92 | 328.92 | 328.92 | 329.9 | 331.1 | 31.1 to 336 | 331.43 | | 332° | | | | | 345.21 | & including | |
| | | Name of owner | Houston Ship Channel to Five- | mile Creek to Dalla | Dallas City Water Works 322.85 | s City Water Works | s City Water Works | Total-Mile 322.0 to 326.7 | s Terminus to Dalla | Dallas City Water Works 328,92 | s City Water Works | s City Water Works | s City Water Works | Total Mile 326.7 to 331.1 | Dallas Floodway (Mile 331.1 | Dallas City Water Works | Dallas City Water Works | Dallas City Water Works | s City Water Works | Dallas City Water Works | Total-Mile 331.1 to 338.8 | Dallas Floodway to Fort Worth | City of Grand Frairie | Fort Worth Terminus to & including Riverside | 0 0 - 1234 - 1-4-10 |
| | | Na | Houst | Five- | Dalls | Dalla | Dalla | L | Dalla | Dalla | Dalla | Dalls | Dalls | - | Dalla | Dalla | Dalla | Dalla | Dalla | Dalla | 1 | Dalla | City | Fort | |

Note: Prices are as of January 1962,

TABLE 25
PERTINENT DATA CONCERNING PROPOSED ACCESS ROADS
TO LOCK SITES

| | : | | | | of access ro | | _: |
|----------|----------|---------|---------|---------|--------------|--------|-------------------------|
| | | | | | t: Improve | : Tota | l: Access |
| | ocation: | | new | : new | | | h; road to |
| | channel: | | road | :bridge | : road | :(mile | s) connect |
| No. : | mile):(| acres): | (miles) | :(feet) | : (miles) | : | : with |
| 1 | 28.30(1) | | | | | | |
| 2 | 47.45 | 6 | .5 | 0 | | • 5 | Washington St. |
| 2 | 41.47 | , | ., | · | | •) | (Liberty) |
| 3 | 59.08 | 83 | 5.7 | - | - | 5.7 | FM 1008 |
| 4 | 74.85 | 15 | 1.0 | 24 | - | 1.0 | Tex. 105 |
| 5A | 98.00 | 21 | • 3 | - | 1.7 | 2.0 | FM 222 |
| 53 | 99.20 | - | • 5 | - | <u>-</u> | 0.5 | Access Rd 5A |
| 5 | 147.92 | 67 | 1.5 | 1500 | 4.0 | 5.5 | FM 980 |
| 7 8 | 183.92 | 79 | 2.5 | 1500 | 4.0 | 6.5 | FM 811 |
| 3 | 207.55 | 27 | | 24 | 2.2 | 2.2 | FM 227 |
| 9 | 217.95 | 72 | - | 3275 | 5.9 | 5.9 | FM 542 |
| 10A | 233.00 | 122 | 1.0 | - | 8.1 | 9.1 | St.Hwy 84 |
| 108(2) | 233.61 | - | - | - | - | - | |
| 11 | 258.91 | 44 | 1.6 | - | 2.0 | 3.6 | FM 635 |
| 12 | 274.51 | 61 | 2.0 | 550 | 3.0 | 5.0 | FM 636 |
| 13 | 286.54 | 23 | 1.2 | 4000 | - | 1.2 | FM 1129 |
| 14 | 298.38 | 2 | .2 | - | - | .2 | St. Hwy 34 |
| 15 16 | 306.31 | 46 | .8 | 24 | 3.0 | 3.8 | FM 780 |
| | 311.25 | 24 | 1.6 | - | .4 | 2.0 | Malloy Rd. |
| 2.7 | 317.81 | 6 | .5 | - | - | • 5 | Post Oak Rd. |
| 1.8 | 331.31 | 2 | .2 | 700 | | .2 | Montgomery St. (Dallas) |
| 19 | 342.51 | 7 | .6 | | | .6 | Meyers Rd. |
| 20 | 351.91 | 22 | 1.2 | 3000 | .6 | 1.8 | FM 157 |
| 21 | 360.17 | 2 | .2 | | 4 - 1 - 1 | .2 | Randol Mill Rd |

⁽¹⁾ New road from Wallisville to lock No. 1 provided for in Wallisville reservoir project recommended in Chief of Engineers report dated April 18, 1961.

(2) Lock No. LOB to be served by road extending from lock No. 10A.

TABLE 26 SUPMARY OF ESTIMATED FIRST COST OF ACCESS ROADS TO LACKS & DAKS REQUIRED FOR MULTIPLE-PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH

| Note: Beach of Wallisville Reservoir to head of Authorized 1 1 Head of Authorized Channel to Liberty to Tennessee Colony | Material : Base : Reservoir (Mile 0.0 to | 35.5) | Treatment | s: :Structure | Approach | Drainage :Structure: | Fencing | & Traffic: | : Exist. | Total |
|--|---|------------|-------------------|------------------|------------|----------------------|----------|------------|----------|-----------|
| Tennessee | Authorized Channel to Liberty (Mile 35.5 to 47.4) | erty (Mile | 35.5 to 4 | (4-7 | | | | | | |
| A to the same of the same of the | Mile 47.4 | to 233.5) | | | | | | | | |
| \$2,220 | \$4,310 | \$390 | | | 0 | \$1,000 | 047,440 | \$200 | \$60 | |
| 24,000 | | 4,810 | 20,000 | \$3.500 | o c | 2,500 | 9,190 | 300 | 00 | |
| 7,180 | | 1,260 | 16,960 | 0 | 00 | 3,500 | 14,370 | 320 | 240 | |
| 0 | | 390 | | 0 | 0 | 0 | 0 | 20 | 0 | |
| | | 2,960 | 39,910 | 225,000 | 0 000 | 10,000 | 46,430 | 280 | 5,630 | |
| | | 1,630 | | | 2000 | 4.500 | 18,580 | 350 | 200 | |
| | | 1,080 | | | 18,920 | 11,000 | 0.86, 64 | 920 | 780 | |
| 34, 900 116,600 | 295,390 | 26,550 | | | 27,880 | 74,000 | 352,030 | 3,650 | 7,270 | 6.0 |
| 2 (Mile 233.5 to 274.4 | 74.47 | | | | | | | | | |
| 15,210 41,310 | 27,130 | 2,660 | 35,900 | 0 | 10 | 3,500 | 30,400 | 8 4 | 280 | |
| 7 | ~ | 3,700 | | 98,500 | 6,480 | 10,000 | 43,120 | 200 | 420 | |
| 4,800 16,020 8 120 2,670 | 3,550 | 040 | 11,320 | 000,000 | 00 | 2,500 | 16,000 | 000 | 00 | |
| | | 2,180 | 37,900 | 3,600 | 00 | 7,500 | 32,100 | 200 | 4,220 | |
| | | 1,470 | | 0 0 | 00 | 2,000 | 1000 | 200 | 230 | |
| - | | 9,310 | | 999,100 | 6,480 | 25,500 | 113,620 | 1,950 | 5,170 | |
| 322.0 to 326.7) | | | | | | | | | | |
| Floodway [Mile 326.7 to 331.1] | 331.1) | | | | | | | | | |
| | | | | | | | | | | |
| 810 140 | 06 000 000 | 10 | 130 | 150,000 | 0 | 0 | 0 | 500 | | |
| 2,490 8,280 | 4.450 | 430 | 5,860 | 0 | 0 | 1,500 | 4,960 | 500 | | |
| 5,230 | | 910 | | 450,000 | 00 | 3,500 | 15,200 | 300 | 850 | |
| Total-Mile 338.8 to 362.8 7,950 5,360 25,020 | 10-1 | 1,450 | 20,330 | 450,000 | | 5,500 | 21,440 | 188 | 850 | |
| ide Bridges (Mile | 362.8 to 369.8 | 7 | | | | | | | | |
| 317,430 663,730 | 432,770 | 39,930 | 540,940 2,189,400 | 189,400 | 34,360 | 108,500 | 045,784 | 006,9 | 13,570 | 5,018,990 |

Note: Friess are as of January 1962. (1) Costs for access read to lock No. 1 are contained in estimated cost of recommended Wallisville Reservoir, (2) Access read to lock 10A passes through lock site 10B - All cost of read has been assigned to lock 10A. 92. SUMMARY OF PERMANENT OPERATING EQUIPMENT (INCLUDING COST).- The estimated first cost of permanent operating equipment for the canalized multiple purpose channel includes the cost of 20 sets of lock and dam equipment, four sets of heavy duty maintenance equipment and a system of river observation gages and recorders. The estimate of cost excludes the operating equipment required at lock No. 1, which is provided for in the recommended Wallisville reservoir project. One set of operating equipment would be provided to serve both locks Nos. 5A and 5B, also both locks Nos. 10A and 10B. It is proposed that each set of operating equipment to be provided at the lock sites consist of the equipment listed in table 27, which also shows the estimated unit costs and first cost of the equipment for each installation.

TABLE 27
ESTIMATED FIRST COST OF ONE SET OF LOCK AND DAM
PERMANENT OPERATING EQUIPMENT

| Item of equipment | Unit cost |
|--|-----------|
| Fruck (1/2 ton pick-up) | \$ 2,000 |
| Fruck (Carry-all) | 2,500 |
| Lawn mowers, 2 @ \$150 | 300 |
| Small derrick barge for removal of logs and debris | 12,000 |
| Tractor with mowing equipment, and blade scrapper | 2,000 |
| Work boat, capable of moving small derrick barge | 10,000 |
| Skiffs, survey type, 2 @ \$700 | 1,400 |
| Outboard motor | 300 |
| Portable radio, 2 @ \$500 | 1,000 |
| Office equipment | |
| Desk, 3 @ \$96.50 | 300 |
| Map case (5 drawers) | 100 |
| File cabinets, 3 @ \$104 | 300 |
| Chairs 6 @ \$37 | 200 |
| Adding machine and/or calculator | 800 |
| Typewriter | 300 |
| Wind direction, velocity indicator and rain gage | 500 |
| Radio transmitter and receiver | 900 |
| Misc. supplies and equipment | 1,000 |
| Total - one set | 35,900 |

^{93.} It is proposed that a set of heavy duty maintenance equipment be provided at four lock sites along the multiple purpose channel to be used by permanent maintenance crews performing ordinary maintenance work on the locks and dams as may be required. The heavy duty maintenance equipment to be provided in each set would consist of the equipment listed in table 28, which also shows the unit and total cost of the equipment.

TABLE 28
ESTIMATED FIRST COST OF ONE SET OF HEAVY DUTY
EQUIPMENT

| Item of equipment | Unit cost |
|---|-------------------|
| ump trucks 2 @ \$3,575 (1 radio equipped) | \$ 7,150 2,800 |
| take body truck with 1,000 gal. water tank and pumps umps (3 in. gasoline operated) 2 @ \$175 | 350 |
| obile Crane (15 ton capacity) | 26,000 |
| ozer, with winch and hydraulic blade | 18,000 |
| ow-boy equip., trailer and cab | 10,000 |
| ombination road maintainer and heavy-duty front end load Total - one set | der 6,200 |
| Total - one set | 70,500 |

- 94. The proposed river observation system is required for the collation of basic precipitation and river stage data required to forecast the operation of the navigation dams on the multiple purpose channel to pass flood rises and reservoir releases and at the same time maintain normal pool elevation for navigation. The existing precipitation and stream recording gages on the watershed would be augmented by 26 standard precipitation gages and 40 waterstage recorders.
- 95 The unit cost of the new precipitation gages and waterstage recorders in place is estimated at \$500 and \$2,500, respectively. The total first costs of the 26 gages and 40 recorders are estimated at \$113,000 excluding contingencies. A summary of the estimated first cost of the proposed permanent operating equipment is given in table 29.

TABLE 29 SUMMARY OF ESTIMATED FIRST COST OF PERMANENT

OPERATING EQUIPMENT REQUIRED FOR MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS

| | Estimated |
|--|---|
| Item of equipment : | first cost |
| Houston Ship Channel to upper reach of Wallisville Reservoir (Mile 0.0 to 35.5) Operation equipment for lock No. 1 Heavy duty maintenance equipment River observation system | None(1) None(1) \$ 2,950 |
| Upper Reach of Wallisville Reservoir to head of authorized ch | nannel |
| to Liberty (Mile 35.5 to 47.4) Operation equipment for lock No. 1 Heavy duty maintenance equipment River observation system | None(1) None(1) 5,000 |
| Head of authorized channel to Tennessee Colony Dam (Mile 47.1 | <u>+</u> |
| to 233.5) Operation equipment for locks No. 2,3,4,5A,6,7,8 & 9 Heavy duty maintenance equipment (2 sets) River observation system Total-Mile 47.4 to 233.5 | 287,200 141,000 60,640 488,840 |
| Tennessee Colony Dam to Lock & Dam No. 12 (Mile 233.4 to 274 Operation equipment for locks No. 10B & 11 Heavy duty maintenance equipment (1/2 set)(2) River observation system Total-Mile 233.5 to 274.4 | 71,800 35,250 13,330 120,380 |
| Lock & Dam No. 12 to Five-mile Creek (Mile 274.4 to 322.0) Operation equipment for locks No. 12 thru 17 Heavy duty maintenance equipment (1/2 set)(2) River observation system Total-Mile 274.4 to 322.0 | 215,400 35,250 15,510 266,160 |
| Five-mile Creek to Dellas Terminus (Mile 322.0 to 326.7) Operation equipment for locks Heavy duty maintenance equipment River observation system | None None 1,530 |
| Dallas Terminus to Dallas Floodway (Mile 326.7 to 331.1) Operation equipment for locks Heavy duty maintenance equipment River observation system | None None 1,430 |

TABLE 29 (Cont'd)

SUMMARY OF ESTIMATED FIRST COST OF PERMANENT OPERATING EQUIPMENT REQUIRED FOR MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS

| Item of equipment | : Estimated : first cost |
|--|--|
| Dallas Floodway (Mile 331.1 to 338.8) Operation equipment for lock No. 18 Heavy duty maintenance equipment (1/4 set)(2) River observation system Total-Mile 331.1 to 338.8 | \$ 35,900 17,630 2,480 56,010 |
| Dallas Floodway to Fort Worth Terminus (Mile 338.8) Operation equipment for locks 19, 20 & 21 Heavy duty maintenance equipment (3/4 set)(2) River observation system Total-Mile 338.8 to 362.8) | 107,700 52,870 7,850 168,420 |
| Fort Worth Terminus to & including Riverside Drive bridges (Mile 362.8 to 369.8) Operation equipment for locks Heavy duty maintenance equipment River observation system Total cost permanent operation equipment Houston Ship Channel to Fort Worth - Mile 0.0 to 369.8 | None None 2,280 |

⁽¹⁾ Cost of operating equipment for Lock No. 1 is included in cost of Wallisville Reservoir project.

MOTE: Prices are as of January 1962.

96. SUMMARY OF BUILDINGS, GROUNDS AND UTILITIES. - Each lock of the multiple purpose channel to Fort Worth would be provided, lockmaster's quarters, garage, storage shed, boathouse, fencing and lighting facilities in addition to the necessary utility distribution systems. A detailed estimate of first cost for each lock installation is given in the following tabulation:

⁽²⁾ Costs of unit assigned for economic analysis of project.

COST OF BUILDINGS, GROUNDS & UTILITIES AT EACH LOCK SITE

| Lockmaster's quarters 1 @ 13,000 Garage & storage shed 1 @ 7,800 | \$13,000 7,800 |
|--|-------------------|
| Utilities | |
| (1) Electrical distribution & lighting system | 50,000 |
| (2) Heating gas - butane | 5,000 |
| (3) Water supply & distribution | 10,800 |
| (4) Sanitary sewer system(5) Telephone facilities | 12,500 |
| (5) Telephone facilities | 6,000 |
| Boathouse & appurtenances | 19,200 |
| Fence 4 ft. stock, chain link | 3,300 |
| Unit cost for one lock site | 127,600 |

97. A summary of first costs for buildings, grounds, and utilities is given in table 30.

TABLE 30 ESTIMATED FIRST COST OF BUILDINGS, GROUNDS, & UTILITIES REQUIRED FOR MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS

| o. of units | : Unit | : |
|---|-------------------------------------|----------------------|
| required | : cost | : Cost |
| Tourton Ohio Ohomol to more man | and the Walliamilla was a | (M: 1 - 0 0 |
| <pre>(ouston Ship Channel to upper res to 35.5)</pre> | ach of Wallisville reser | voir (Mile 0.0 |
| 1 | 127,600 | (1) |
| pper reach of Wallisville reserv | | |
| Liberty (Mile 35.5 to 47.4) - | | None |
| (and an outlessined absumpl to Iii | | Daw |
| ead of authorized channel to Lik (Mile 47.4 to 233.5) | berty to Tennessee Colon | y Dem |
| 10 | 127,600 | 1,276,000 |
| ennessee Colony Dam to Lock & Da | am No. 12 (Mile 233.5 to | 274.4) |
| 2 | 127,600 | 255,200 |
| ock & Dam No. 12 to Five-mile Cr | reek (Mile 274.4 to 322. 127,600 | <u>0)</u> 765,600 |
| ive-mile Creek to Dallas Termin | | |
| TVC-millo Oloch to Dolland Icamilla | 10 (1110 522.0 00 520.1) | 110110 |
| allas Terminus to Dallas Floodwa | ay (Mile 326.7 to 331.1) | - None |
| allas Floodway (Mile 331.1 to 33 | 28 8) | |
| 1 | 127,600 | 127,600 |
| allas Floodway to Fort Worth Ter | | |
| 3 | 127,600 | 382,800 |
| ort Worth Terminus to and include | ding Riverside Drive Bri | |
| (Mile 362.8 to 369.8) - | | None |
| Total - Mile 0.0 to 369.8 | | 2,807,200 |

98. SUMMARY OF AIDS TO NAVIGATION (INCLUDING COSTS).~ Estimates of unit costs and total first cost of aids to navigation for the section of the multiple purpose Trinity River channel from Liberty to Fort Worth, Texas, was furnished by letter dated October 30, 1961, from the Commander, Eighth Coast Guard District, New Orleans, Louisiana. The aids to navigation would consist of single pile daybeacons installed on the channel bank at the lower ends of river cut-offs and along the channel route as may be necessary, and 3rd class radar reflecting buoys in Livingston and Tennessee Colony reservoirs. In addition it is proposed that a light attendant station complete with wharf, vehicles, and 45-foot buoy boat be provided at each reservoir. Two sites of approximately 2 acres each would be required for the light attendant stations. The total cost of the required aids to navigation is estimated at \$800,000. Table 31 shows a summary of the estimated first cost of the required aids to navigation for the multiple purpose channel, excluding land costs for the light attendant station which would be located within the reservoir limits.

TABLE 31 SUMMARY OF ESTIMATED FIRST COST OF AIDS TO NAVIGATION REQUIRED FOR THE MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

| Section of channel and : | | | | : Estimated |
|--|----------|------------|----------------|--------------|
| | Tīma + a | 0 | · | |
| aids to navigation : | UIII U | Quantity | COST(I) | : first cost |
| Houston Ship Channel to up | oper r | each of Wa | allisville Re | eservoir - |
| (Mile 0.0 to 35.5) None | - | - | - | None |
| Upper reach of Wallisville | | | nead of author | orized |
| channel to Liberty (Mile None | - 32.2 | - 4(.4) | - | None |
| Head of authorized channel | | iberty to | Tennessee Co | olony |
| Dam (Mile 47.4 to 233.5) Single pile daybeacons | | 285 | 500 | \$142,500 |
| Third Class (RR) buoys | | 40 | 650 | 26,000 |
| Light attendant station | | 1 | 100,000 | 100,000 |
| Buoy boat, 45-foot Total | ea. | 1 | 150,000 | 150,000 |
| Tennessee Colony dam to Lo | ock & | | | to 274.4) |
| Third class (RR) buoys | ea. | 68 | 650 | 44,000 |
| Light attendant station | ea. | 1 | 100,000 | 100,000 |
| Buoy boat-45-foot Total | ea. | 1 | 150,000 | 150,000 |

TABLE 31 (Cont'd) SUMMARY OF ESTIMATED FIRST COST OF AIDS TO NAVIGATION REQUIRED FOR THE MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

| Unit: Qu | antity: Unit | cost (1): fi | timated rst cost (1) |
|-----------|--|--|---|
| -mile Cre | eek (Mile 274 94 | .4 to 322.0) 500 | 47,000 |
| cluding I | Dallas Termin | us (Mile 322.0 | |
| ea. | 9 | 500 | 4,500 |
| ng Dallas | Floodway (M | lle 326.7 | |
| ea. | 9 | 500 | 4,500 |
| .1 to 338 | 15 | 500 | 7,500 |
| uding Ft. | Worth Termin | nus (Mile 338. | 8 |
| ea. | 48 | 500 | 24,000 |
| including | g Riverside D | rive bridges - | |
| - | _ | - | None |
| 369.8 | | | \$800,000 |
| | ea. cluding I ea. ng Dallas ea1 to 336 ea. uding Ft. ea. including | ea. 94 cluding Dallas Terming ea. 9 ng Dallas Floodway (M: ea. 9 .1 to 338.8) ea. 15 uding Ft. Worth Terming ea. 48 including Riverside Di | ea. 9 500 ng Dallas Floodway (Mile 326.7 ea. 9 500 1 to 338.8) ea. 15 500 uding Ft. Worth Terminus (Mile 338.6) ea. 48 500 including Riverside Drive bridges |

⁽¹⁾ Cost furnished by Commander, U. S. Coast Guard.

^{99.} SUMMARY OF RECREATION FACILITIES (INCLUDING COST).- A summary of first cost of recreation facilities provided in connection with the multiple purpose channel estimated at \$2,866,200 is shown in table 32.

TABLE 32
SUMMARY OF ESTIMATED FIRST COST OF RECREATIONAL FACILITIES
FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO FORT WORTH, TEXAS

| | : | : | : Unit : | Total |
|-----------------------------|----------------|----------|-------------|--------------------|
| Item | :Quantity | : Unit | : Cost : | Cost |
| | | | (2017 | - 0 0 to 25 5) |
| Souston Ship Channel to hea | | Miles | \$35,000 | \$ 87,500 |
| Access roads | 2.5 | Each | 66,600 | 199,800 |
| Recreation facilities | 3 | Lacn | 00,000 | 287,300 |
| Total construction | | | | 201,300 |
| lead of Wallisville Reserve | oir to head of | author | ized channe | l to Liberty |
| (Mile 35.5 to 47.4) | | | (((00 | (((00 |
| Recreation facilities | 1. | Each | 66,600 | 66 ,60 0 |
| Mead of authorized channel | to Liberty to | Tennes | see Colony | Dem (Mile 47.4 |
| to 233.5) | | | | |
| Access roads | 7 | Miles | 35,000 | 245,000 |
| Recreation facilities | 1.4 | Each | 66,600 | 932,400 |
| Total construction | | | | 1,177,400 |
| | | / | | orb. I.V |
| Cennessee Colony Dam to Loc | ck & Dam No. | 12 (Mile | 233.5 to 2 | 35,000 |
| Access road | 1.0 | Mile | 35,000 | |
| Recreation facilities | 2 | Each | 66,600 | 133,200 168,200 |
| Total construction | | | | 100,200 |
| lock & Dam No. 12 to Five-r | mile Creek (M | ile 274. | 4 to 322.0) | |
| Access roads | 7 | Miles | 35,000 | 245,000 |
| Recreation facilities | ė. | Each | 66,600 | 532,800 777,800 |
| Total construction | | | | 777,800 |
| | | | | (-) |
| live-mile Creek to and inc | luding Dallas | Terminu | s (Mile 322 | 2.0 to 326.7) - N |
| allas Terminus to existing | g Dallas Floor | dwav (Mi | le 326.7 to | 331.1) - N |
| | | | | |
| Dallas Floodway (Mile 331. | 1 to 338.8) | | | |
| Access | 1.0 | Mile | 35,000 | 35,000 |
| Recreation facilities | 1 | Each | 66,600 | 66,600 |
| Total construction | | | | 101,600 |
| | 7 | the Mean | (Mila | 228 8 +0 362 81 |
| Dallas Floodway to & inclu | ding Fort Wor | Miles | 35,000 | 87,500 |
| Access roads | 2.5 | | | 199,800 |
| Recreation facilities | 3 | Each | 00,000 | 287,300 |
| Total Construction | | | | 201,300 |
| Fort Worth Terminus to Riv | verside bridge | es (Mile | 362.8 to 3 | 69.8) None |
| | | | | |
| Total (Mile 0.0 to 369.8 | 3) | | | \$2,866,200 |

- 100. SUMMARY OF ANNUAL MAINTENANCE, OPERATION AND MAJOR RE-PLACEMENT COSTS. The following paragraphs give information and supporting data considered in determining the annual costs of maintenance, operation and major replacements, based on a 100-year amortization period. The total annual cost of maintenance operation and major replacements for various features of the multiple purpose channel is estimated at \$4,438,200, as summarized in table 33, which excludes an estimated cost of \$272,000 annually, for maintenance and operation of recreational facilities proposed for the multiple purpose channel.
- 101. LOCKS, DAMS, AND RIVER OBSERVATION SYSTEM. The annual maintenance, operation and replacement cost for lock No. 1, is provided for in the Wallisville Reservoir project as recommended in prior report and accordingly is excluded from further consideration in this report. Also excluded from consideration are the costs of maintenance and operation of the spillway gates and outlet works at the Livingston and Tennessee Colony reservoirs dams, which costs are considered as a function of the respective reservoirs. The annual costs of operating personnel, maintenance supplies, major repair and painting, and major replacements for the proposed 22 locks and 18 navigation dams and river observation system, are set forth in the following paragraphs.
- 102. OPERATING PERSONNEL.— The yearly operation of each lock and dam would be accomplished with a crew of 9 men consisting of lock master and an assistant lock master on a yearly basis and seven lockmen working 40 hours per week. Lockmasters would reside in houses at the lock sites. Grounds maintenance would be performed by lock personnel when operation of the locks is not pending. Ordinary maintenance of all locks and navigation dams would be performed by four crews consisting of 8 men each. These maintenance crews would also maintain the river observation system. Personnel required for maintenance and operation of locks, dams and river observation system is proposed as follows:

| Personnel | Number required | Average yearly cost | Total yearly cost |
|-------------------------------|--------------------|------------------------|----------------------|
| Lockmaster, S-06 | 2.2 | \$ 8,090 | \$177,980 |
| Asst. lockmaster, W-08 | 22 | 6.340 | 139,480 |
| Lockmen, W-07 | 154 | 6.050 | 931,700 |
| Maintenance foreman, S-06 | 4 | 8,090 | 32,360 |
| Heavy equip, operator, W-12 | 12 | 6,220 | 74,640 |
| Laborers, W-02 | 16 | 4,160 | 66,560 |
| Sub-total | | \$ | 1,422,720 |
| Supervision and overhead, 12% | | | 170,780 |
| Total | | \$ | 1,593,500 |

TABLE 33
ESTIMATED ANNUAL COST OF MAINTENANCE, OPERATION AND REPLACEMENT
FOR DESIGNATED REACHES OF MULTIPLE PURPOSE CHANNEL
FROM HOUSTON SHIP CHANNEL TO FORT WORTH, TEXAS

| 1 | Estimated 0.0 to : | 35.5 47.4 | cost for 17.4 to 233.5 | : 233.5 to | designated reaches of multiple purpose channel : 233.5 to : 274.4 to: 322.0 to: 326.7 to: 331.1 : 3 | 1tiple pur 322.0 to: | 331.1 : 338. | 1.1 to: | 338.7 to : | 362.8 to | |
|-----------------------------------|--------------------|--------------|------------------------|------------|---|----------------------|--------------|-----------|------------|----------|-------------|
| Locks, dams and river observation | (7) | (5) | : 137 | 1+1 | : (0) | : 70) | : ()) | : (0) | : (8) | (10) | Total |
| system | | | | | | | | | | | |
| Operating personnel | 1 | ı | \$724,300 | \$144,900 | \$434,600 | ı | • | \$ 72,400 | 217,300 | 1 | \$1,593,500 |
| Ordinary maintenance supplies | | , | 76,200 | 15,500 | 46,300 | • | | 7,700 | 23,200 | • | 163,900 |
| Major rapair and painting | 1 | ı | 165,500 | 33,100 | 99,300 | | ı | 16,500 | 76,600 | • | 364,000 |
| Major replacements: | | | | | | | | | | | |
| Dams | 1 | 1 | 39,200 | , | 33,400 | • | 1 | 2,600 | 16,800 | 1 | 95,000 |
| Locks | 1 | 1 | 64,200 | 13,500 | 38,500 | , | 1 | 6,300 | 19,200 | | 141,700 |
| Permanent operating equipment | 1 | | 42,700 | 8,600 | 25,600 | • | | 4,300 | 12,900 | , | 94,100 |
| Access roads | | | | | | | | | | | |
| Maintenance | , | 1 | 29,000 | 13,000 | 13,000 | | | | 3,000 | 1 | 58,000 |
| Major replacement | • | ı | 61,100 | 26,300 | 26,100 | • | 1 | 1 | 5,700 | 1 | 119,200 |
| Railroad bridges | | | | | | | | | | | |
| Maintenance & operation | , | ı | 126,000 | 21,000 | 1 | | 45,000 | 42,000 | 45,000 | • | 273,000 |
| Major replacement | ı | • | 19,000 | 3,200 | ı | 1 | 6,300 | 6,300 | 6,300 | 1 | 41,100 |
| Channel dredging | 1 | 30,000 | 577,800 | 232,600 | 224,200 | 28,800 | 33,000 | 35,600 | 153,400 | 17,500 | 1,332,900 |
| Channel revetment | 1 | 8,000 | 45,000 | , | 1 | | 1 | | 1 | 1 | 20,000 |
| Aids to navigation | | | | | | | | | | | |
| Maintenance | 1 | , | 42,400 | 42,400 | ť | 1 | • | | ı | | 84,800 |
| Major replacement | | , | 12,900 | 3,000 | 3,300 | 300 | 300 | 500 | 1,700 | 1 | 22,000 |
| Total | 1 | 38,000 | 2,022,300 | 557,100 | 944,300 | 29,100 | 81,600 | 197,200 | 551,100 | 17,500 | 4,438,200 |

(1) Houston Ship Channel to upper reach of Wallisville reservoir.
(2) Upper reach of Wallisville reservoir to head of authorized channel to Liberty.
(3) Head of authorized channel to Liberty to Tennessee Colony reservoir dam.
(4) Tennessee Colony Dam to Lock and Dam No. 12.
(5) Lock and Dam No. 12 to Five-mile Greek.
(6) Five-mile Creek to Dallas Terminus
(7) Dallas terminus to Dallas Floodway.
(9) Dallas Floodway to Fort Worth Terminus.
(9) Dallas Floodway to and including Riveredde Drive bridges.

- 103. ORDINARY MAINTENANCE SUPPLIES.— Based on experienced cost of ordinary maintenance supplies for existing locks and dams in the Galveston and Mobile Districts, it is estimated that ordinary maintenance supplies would cost \$4000 per year for each lock and dam. The cost includes \$1000 per year for electric power. The total annyal cost of ordinary maintenance supplies for the 22 locks and 18 navigation dams is estimated at \$160,000. Cost of ordinary maintenance supplies for the river observation system is estimated at \$10,000 annually. The total annual cost of ordinary maintenance supplies for the lock, dams, and river observation system is estimated at \$170,000.
- 104. MAJOR REPAIR AND PAINTING.- It is estimated that major repair and complete painting of lock and dam gates would be performed on a contract basis every 10 years. The average cost of major repair and painting of the gates at each lock and the gates of each dam is estimated at \$150,000 and \$50,000, respectively. The total cost for major repair and painting of gates at the 22 locks and 18 dams is estimated at \$4,200,000. Average annual cost of major repairs and painting is estimated at \$364,000.
- 105. MAJOR REPLACEMENTS. In determining the cost of major replacements, it was considered that the replacement costs of specific items in the future would equal the present construction first cost less salvage. The average annual replacement cost was computed by discounting to present worth all replacement costs and miltiplying the present worth value by the capital recovery factor for a uniform annual series having a period of 100 years. The estimated annual replacement costs for operating equipment and machinery for the 18 navigation dams, 22 locks and permanent operating equipment are given in table 34.
- 106. ACCESS ROADS.- Maintenance of access roads would include patching of bituminous surfaces, policing of rights-of-way, mowing of rights-of-way, grading of shoulders, repainting center strip, repairing signs, and cleaning of drainage structures. The annual cost of access road maintenance is estimated at \$1,000 per mile of road, or a total of \$58,000 for the proposed 21 access roads. The annual maintenance cost is influenced by the consideration that the crown of the road would be replaced every 10 years. The cost of crown replacement on the combined total of 58 miles of access roads is estimated at \$119,000 annually as shown in table 34.

TABLE 34
ESTIMATED ANNUAL COST OF MAJOR REPLACEMENTS
FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL
(100 YR. PROJECT LIFE 2-7/8% INTEREST RATE)

| Designation | : Estimat : life : in : years | ed: Estimated : single : replacement : cost (1) | :Estimated: :salvage : | replacement: cost less : | annual : | Estimated Annual replacement cost(3) |
|--|--|---|---------------------------|-----------------------------|----------|---|
| Dems | | | 1 | | | |
| Tainter gates | 40 | \$9,723,000 | \$4,834,000 | \$3,889,000 | 0.01209 | \$47,000 |
| Operating machinery (mech.) | 40 | 3,243,000 | 1,946,000 | 1,297,000 | 0.01209 | 16,000 |
| Crane | 25 | 933,000 | 560,000 | 373,000 | 0.02609 | 10,000 |
| Pier, storage yard | 25 | 152,000 | 91,000 | 61,000 | 0.02609 | 2,000 |
| Electrical system | 25 | 704,000 | 422,000 | 282,000 | 0.02609 | 7,000 |
| Operating machinery (elect.) | 25 | 840,000 | 504,000 | 336,000 | 0.02609 | 9,000 |
| Stop logs | 40 | 883,000 | 530,000 | 353,000 | 0.01209 | 4,000 |
| Total Dams | | 14 | | | | 95,000 |
| ocks | | | | | | |
| Miter gates and tainter valves | 40 | 11,041,000 | 6,626,000 | 4,415,000 | 0.01209 | 53,000 |
| Operating machinery | 40 | 5,465,000 | 3,279,000 | 2,186,000 | 0.01209 | 26,000 |
| Valves | 40 | 200,000 | 120,000 | 80,000 | 0.01209 | 1,000 |
| Oil piping system | 40 | 2,732,000 | 1,639,000 | 1,093,000 | 0.01209 | 13,000 |
| Stop logs | 40 | 2,318,000 | 1,391,000 | 927,000 | 0.01209 | 11,000 |
| Electrical system | 25 | 3,574,000 | 2,144,000 | 1,430,000 | 0.02609 | 37,000 |
| Total locks | | 2,7,1., | -,, | | | 141,000 |
| ermanent operating equipment | 10 | 1,220,000 | 122,000 | 1,098,000 | 0.08594 | 94,000 |
| Mailroad bridges (machinery) | 140 | 8,405,000 | 5,043,000 | 3,362,000 | 0.01209 | 41,000 |
| ccess roads to lock sites | 10 | 6,876,000 | 5,488,000 | 1,388,000 | 0.08594 | 119,000 |
| ids to navigation | | 7-1-7 | | ., | | |
| Buoys, radar reflecting | 20 | 70,000 | None | 70,000 | 0.03590 | 3,000 |
| Day beacons | 12 | 230,000 | None | 230,000 | 0.06924 | 16,000 |
| Buoy boats Total aids to navigation | 25 | 300,000 | 180,000 | 120,000 | 0.02609 | 3,000 |
| Total | | | | | | 512,000 |

Based on assumption that replacement costs will equal construction first costs - Estimates contains 25% contingencies and engineering, design, supervision and administration costs.
 Accumulated present worth factor multiplied by capital recovery factor.
 Replacement cost less salvage multiplied by annual replacement cost factor.

107. RAILROAD BRIDGES.- The plan of improvement provides for the construction of a vertical lift railroad bridge at each of the 13 railroad crossings of the multiple purpose channel. The total annual maintenance and operation cost for the 13 bridges is estimated at \$273,000, based on the estimated cost of \$21,000 per year for 24 hour daily operation, utilities, painting and repairs for single lift bridge as shown below.

| Item | Annual Cost |
|---|--------------------|
| Bridge tenders - 4 @ \$4,000 a year Utilities | \$ 16,000 1,000 |
| Painting and miscellaneous repairs Operating equipment repairs | 2,000 2,000 |
| Total | \$ 21,000 |

It is further considered that the machinery for each lift bridge would have an average life of about 40 years. The total annual replacement costs for the 13 lift bridges machinery is estimated at \$41,000.

- 108. CHANNEL DREDGING.- In determining the estimated annual cost of maintaining project depths in the proposed multiple purpose channel from the Houston Ship Channel to Fort Worth, Texas, consideration was given to the proposed enlargement of the authorized channel to Liberty project, the sediment load that would enter the channel from various sources and the requirements to maintain project depths for navigation and regulated flood releases.
- 109. With respect to the enlargement of the channel to Liberty project, the plan provides for deepening to 12 feet the 9 x 150-foot channel from the Houston Ship Channel to the upper end of the Wallisville Reservoir at channel mile 35.5 in the interest of navigation. Thence, for deepening and widening of the channel to the head of the authorized project at channel mile 47.4 in the interest of both navigation and flood control. The following concerns maintenance dredging of the channel from the Houston Ship Channel to channel mile 35.5.
- 110. The plan of improvement for the Wallisville Reservoir project (House Document No. 215, 87th Congress, 1st Session) provides for diversion of all Trinity River flows to a new outlet on the north shore of Trinity Bay. Since sediment inflow into the channel below the Wallisville dam would be eliminated, maintenance dredging of the project channel below the dam would be largely confined to the reach from the Houston Ship Channel to near Smith Point, a distance of 6.5 to 9 miles depending on tidal influences on the channel. Some maintenance dredging would also be required in the vicinity of the Double Bayou channel crossing of the project channel. It is considered that maintenance of the proposed 12 x 150-foot channel below the Wallisville dam would not increase materially the maintenance of the 9 x 150-foot project channel. The cost of maintaining the 9 x 150-foot project channel from the Houston

Ship Channel to the Anahuac Channel, a distance of 24.3 miles, is estimated at \$114,000 annually as given in appendix II accompanying the District Engineer's report in House Document No. 215, 87th Congress, 1st Session. It is considered that the presently estimated annual cost of \$114,000 for the project channel to the Anahuac Channel is also adequate to maintain the proposed 12 x 150-foot channel below the Wallisville Dam including maintenance of the adjacent protective (spoil) embankment below the Anahuac Channel.

- 111. Maintenance of the authorized 9 x 150-foot channel to Liberty project from Lock No. 1, in the Wallisville Dam to the Texas Gulf Sulphur Company's channel at channel mile 33.8 is provided for in the Wallisville Reservoir project. Appendix II accompanying the project document (HD 215, 87th Congress, 1st Session) reports that "Maintenance dredging of the upper lock forebay, the navigation channel in the reservoir area, and some maintenance and snagging of the river diversion channel would average about \$20,000 per year". In addition to deepening of the project channel through the Wallisville Reservoir, the plan of improvement for the multiple purpose channel provides for the channel to be located in a river cut-off alignment at Wallisville, Texas. A review of the probable shoaling in the 12 x 150foot channel from lock No. 1, to channel mile 35.5 indicates that the required annual dredging in this reach would be approximately equal to the dredging required for the 9 x 150-foot channel terminating at mile 33.8. Accordingly, no increase in annual cost of channel maintenance for the 12 x 150-foot channel to channel mile 35.5 is proposed.
- 112. Information concerning the sediment inflow into the multiple purpose channel from the areas above the Tennessee Colony, Livingston and Wallisville Reservoirs is given in appendix II of this report. The study indicates that sediment deposition in the 95.27-mile reach of multiple purpose channel upstream of lock and dam No. 12, located at the upper end of the Tennessee Colony reservoir, would be about 425 acre feet per annum (685,000 cubic yards) and that the net inflow and deposition in the channel in the vicinity of Tennessee Colony reservoir would be 1,100 acrefeet per annum (1,775,000 cubic yards).
- 113. The sediment inflow in the 85.7-mile reach of channel between the Tennessee Colony dam and lock and dam No. 6, at the upper end of the Livingston Reservoir, is estimated at 522 acre feet per annum (845,000 cubic yards). It was considered that flood release discharge from the Tennessee Colony reservoir and flood flows from the uncontrolled drainage area in this reach would be largely contained in the enlarged multiple purpose channel. The sediment load in this reach would be transported into the Livingston Reservoir during the periods of high flood release from the Tennessee Colony Reservoir.
- 114. The sediment inflow in the 51.75-mile of channel from the Livingston Dam to the upper end of the Wallisville Reservoir, is estimated at 277 acre-feet per annum (447,000 cubic yards). It was considered that all of the sediment produced in this reach would be transported to the Wallisville reservoir because the channel conditions in

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this reach are similar to those in the Tennesses Colony - Livingston reach.

- 115. Quantities of sediment resulting from channel bank erosion or slides and degradation of channel bed cannot be precisely determined because of unknown factors at this time. Such factors include: (a) The ability of the bank materials to withstand the erosion action of stream flow, wave and propeller wash from passing waterborne craft, rainfall and run-off from small areas along the route of the channel, and (b) the formation of shoals or bars on the channel bed by sediment deposition and degradation of the channel bed whereby the flows would be diverted to the channel banks causing erosion and/or large bank slides to occur.
- 116. The entire Trinity River valley is composed of alluvial materials of Quaternary origin consisting of sands, clays, silts or their mixtures. In the reach of the channel upstream of the Livingston reservoir the alluvial material is of moderate depth to bed rock, and are considered to be quite firm and resistant to stream flow. In the lower reaches of the river the alluvial material deepens to depths of 30 feet and greater. From about channel mile 87.0 to 35.5 the channel banks are relatively unstable within comparatively narrow widths in certain reaches resulting generally from natural cut-offs. The character of the bank materials along the Trinity River is shown on plates 23 and 24. The plan for the multiple purpose channel provides for bank stabilization works on the outer curves of the channel in the reach from about the Livingston Dam to the upper reach of the Wallisville Reservoir.
- 117. In view of the foregoing, it appears reasonable to consider that maintenance dreiging of the several channel reaches providing a project depth of 12 feet below normal pool elevation be expected on an annual basic to assure adequate depth for navigation, and that maintenance dredging of the channel reaches having depths greater than 12 feet be anticipated on a three-year basis to prevent serious encroachment of the channel capacity for flood storage releases and meandering of stream flow within the channel resulting from the formation of large bars or shoals.
- 118. It is further anticipated that during flood rises on the channel, a considerable amount of sediment may be deposited in the upper and lower foreboys at each lock. The removal of such sediment deposits would probably be on an expedited basis to prevent serious delays to mavigation. Accordingly, for estimating purposes, it is considered that such dredging would be required at each lock on an annual basis.
- 119 During infrequent periods of drought, conservation storage in the Livingston and Tennessee Colony reservoirs would be withdrawn. During these drought periods, condition surveys would be made of the 12 x 150-foot navigation channels located at the bottom of the conservation pools. When full depletion of the conservation storage appeared imminent, expedited dreaging of the navigation channel may be required. The occurrence of the severe droughts would be very infrequent, possibly not more often than once in 50 years. For estimating purposes, it is proposed that dreaging of the navigation channels below the conservation pools in

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the Livingston and Tennessee Colony Reservoirs be envisioned on an annual basis.

of the Wallisville Reservoir would not be required based on study given in Appendix I accompanying the District Engineer's report contained in House Document No. 215, 87th Congress, lst Session. The study of probable sediment storage in the Wallisville Reservoir concluded that practically all of the sediment load entering the Wallisville Reservoir would be passed out of the reservoir by high flood flows. The multiple purpose channel would carry regulated discharges of 35,000 second-feet for extended periods, and these flows with their accompanying silt loads would require the same operation of the Wallisville gated river diversion channel proposed in the design of the Wallisville Reservoir project. Accordingly, it is considered that practically all of the presently estimated sediment load of 277 acre-feet produced in the Livingston-Wallisville reach would be passed out of the Wallisville reservoir and only a small amount of channel maintenance dredging would be required, annually.

121. The sediment load entering the Livingston Reservoir, estimated at 845,000 cubic yards, annually, would pass lock and dam No. 6, located at channel mile 127.92, and enter into the deeply entrenched navigation pool No. 58. Pool 58 would have a bottom width of 150 feet at a constant elevation 89 and would be about 18 miles long to the upper reach of inundated lands of the Livingston Reservoir. During high flood releases from the Tennessee Colony Reservoir and/or flood flows from the uncontrolled area above the Livingston Reservoir, the conservation storage in Livingston Reservoir would be full to elevation 131, which would extend to lock and fam No. 6. In view thereof all sediment load passing lock and dam No. 6 would enter into pool No. 58 having a depth of 42 feet and would be trapped in its upper reaches. In order to provide an adequate channel for flood releases and navigation, it is proposed that channel dredging involving 845,000 cubic yards, annually, be anticipated for in the upper reaches of pool 58.

122. Similar conditions would exist below lock and dam No. 12 located at channel mile 274.51 in the upper reach of the flood control storage pool of the Tennessee Colony Reservoir, as shown on plate 13. Pool No. 11 would have a bottom width of 150 feet at a constant elevation of 158 throughout its length of 15.6 miles, and would be entrenched generally in the upper 8 miles. However, pool No. 11 would ordinarily have a 12-foot depth of water below normal pool elevation 270 and it would be necessary to provide that depth constantly for navigation. The sediment inflow passing lock and dam No. 12 would be deposited in the navigation channels, pools No. 11 and 108, in the allasent overbank area in the lower portion of pool No. 11, and the conservation pool of the Tennessee Colony Reservoir, depending on the largeness of the flood inflow. It is considered condition surveys of pools No. 11 and 10B would be required after recession of the flood inflows and that emergency dredging be performed to maintain adequate depth for navigation in these pools. The distribution, pattern, frequency, and quantity of sediment deposition in the

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pools are not precisely known at this time. However, because of their possibility, the unit cost of maintenance dredging may be higher than regular maintenance dredging costs. To allow for these unknown factors, it is proposed that the annual cost of channel dredging in pools 10B and 11 be based on the removal of 1,500,000 cubic yards, annually.

- 123. The materials to be encountered in the maintenance dredging of the multiple purpose channel would consist of sand, sandy clay, clay, silt and some gravel. These materials should offer no unusual dredging difficulties, however, to overcome the higher banks of the deeply entrenched channel in certain reaches, it may be necessary to provide booster pumps for disposal of the earth materials on lands adjacent to the channel.
- ment load of 1,500,000 cubic yards from pools No. 10B and 11, in the Tennessee Colony reservoir and the 845,000 cubic yards from pool No. 5B in the Livingston reservoir is based on the use of a modern 27-inch pipeline dredge with an average daily output of 22,000 cubic yards of material from a 20-hour dredging day. The cost of operating a large dredge is estimated at \$3,000 per day, which includes contingencies, engineering and design, and supervision and inspection. The annual cost of dredging the 1,500,000 and 845,000 cubic yards is estimated at \$205,000 and \$115,000, respectively.
- 125. The cost of maintenance dredging of the multiple purpose channel upstream from channel mile 35.5 to 362.8. excluding dredging in the Livingston and Tennessee Colony reservoirs and at lock sites, is based on the consideration that the sediment deposits in the channel would not be sufficient to delay a steady upstream advance of the dredge and therefore the cost of dredging the channel would depend largely on the amount of time required for the dredge to move through the reach. The 27-inch dredge has a maximum rate of progress of about 2,500 to 3,000 feet per day. However, for the purpose of this report, it was estimated that the dredge would advance about 2,100 feet per day, or at a cost of \$7,500 per mile of channel. The estimated annual cost of dredging channel having a 12-foot project depth below normal pool is based on \$7,500 per mile of channel assuming the channel to be dredged yearly. For channel depths greater than 12 feet, the estimated annual cost of dredging is based on \$2,500 per mile of channel, assuming the channels to be dredged every three years.
- 125. It is considered that expedited dredging at the lock forebays would not involve a large amount of dredging and that such dredging could be performed efficiently by small dredges designed for such purposes. The cost of dredging both forebays of each lock is estimated at \$10,000 annually.
- 127. The cost of expedited dredging of the 12 x 150-foot channels below the conservation pools in the Livingston and Tennessee Colony reservoirs is estimated at \$300 per mile on an annual basis. Maintenance dredging in the reach of flood control channel between mile 362.8 and

369.8, would probably be performed by land based equipment because the channel diminishes from a depth of about 21 feet at mile 362.8 to zero at about mile 368. For the purpose of this report, the cost of maintaining project depth in this reach is estimated at \$2,500 per mile. The total annual cost of maintenance dredging of the multiple purpose channel upstream of lock No. 1 in the Wallisville reservoir is estimated at \$1,332,900, a summary of which is given in whole 35.

TABLE 35
ESTIMATE OF ANNUAL MAINTENANCE DREDGING COSTS
FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO
FORT WORTH, TEXAS

| Reach | : : : Miles of : channel | : Estimated : annual : dredging : cost per : mile | Estimated annual dredging cost |
|-------------------------------------|-----------------------------------|---|--------------------------------|
| Houston Ship Channel to upper reach | | lle reservoir | |
| Mile 0.0 to 6.5 | 6.5 | - | None (1) |
| Mile 6.5 to 35.5 | 28.9 | | None (1) |
| Upper reach of Wallisville reservoi | r to head of | authorized c | hannel to Liberty |
| (Mile 35.5 to 47.4) | | 40 000 | 4 00 000 |
| Mile 35.5 to 47.4 | 12.0 | \$2,500 | \$ 30,000 |
| Total mile 35.5 to 47.4 | | - / 1 | 30,000 |
| Head of authorized channel to Tenne | | | |
| Mile 47.4 to 73.1 | 25.7 | 2,500 | 64,300 |
| Mile 73.1 to 74.9 | 1.8 (2) | | 13,500 |
| Mile 74.9 to 99.1 | 24.2 | 2,500 | 60,500 |
| Mile 99.1 to 134.8 | 35.7 | 300 | 10,700 |
| Mile 134.8 to 148.0 | 13.2 | 8,700 | 115,000 |
| Mile 148.0 to 233.5 | 85.5 | 2,500 | 213,800 |
| Emergency dredging at locks No. 2 t | hru 10A | | 100,000 |
| Total Mile 47.4 to 233.5 | | | 577,800 |
| Tennessee Colony Dam to lock and da | | le 233.5 to 2 | 274.4) |
| Mile 233.5 to 258.9 | 25.4 | 300 | 7,600 |
| Mile 258.9 to 274.4 | 15.5 | | 205,000 |
| Emergency aredging at locks No. 10B | & 11 | | 20,000 |
| Total - Mile 233.5 to 274.4 | | | 232,600 |
| Lock and dam No. 12 to Five-mile Cr | eek (Mile 27 | 74.4 to 322.0) | |
| Mile 274.4 to 283.3 | 8.9 | 2,500 | 22,300 |
| Mile 283.3 to 286.6 | 3.3 (2) | 7,500 | 24,800 |
| Mile 286.6 to 295.0 | 8.4 | 2,500 | 21,000 |
| Mile 295.0 to 298.3 | 3.3 (2 | 7,500 | 24,800 |
| Mile 298.3 to 303.9 | 5.6 | 2,500 | 14,000 |
| Mile 303.9 to 306.3 | 2.4 (2 | 7,500 | 18,000 |
| Mile 306.3 to 322.0 | 15.7 | 2,500 | 39,300 |
| Emergency dredging at locks No. 12 | thru 17 | | 60,000 |
| Total - Mile 274.4 to 322.0 | | | 224,200 |

TABLE 35 (Cont'd)
ESTIMATE OF ANNUAL MAINTENANCE DREDGING COSTS
FOR MULTIPLE PURPOSE TRINITY RIVER CHANNEL TO
FORT WORTH, TEXAS

| Re ac h | : : Miles of : channel | : | Estimated annual dredging cost per mile | | Estimated annual dredging cost |
|--------------------------------------|------------------------------|---|---|-------|---|
| Five-mile Creek to Dallas Terminus | (322.0 to 3 | 26.7 | 7) | | |
| Mile 322.0 to 326.0 | 4.0 | | 2,500 | \$ | 10,000 |
| Mile 326.0 to 326.7 | 0.7 (2 |) | 7,500 | | 5,300 |
| Dallas Terminus | _ ` ` | • | - | | 13,500 |
| Total - Mile 322.0 to 326.7 | | | | | 28,800 |
| Dallas Terminus to Dallas Floodway | (Mile 326.7 | to | 331.1) | | |
| Mile 326.7 to 331.1 | 4.4 (2 | | 7,500 | | 33,000 |
| Dallas Floodway (Mile 331.1 to 338 | | • | .,, | | |
| Mile 331.1 to 337.4 | 6.3 | | 2,500 | | 15,800 |
| Mile 337.4 to 338.8 | 1.3 | | 7,500 | | 9,800 |
| Emergency dredging at Lock No. 18 | | | .,, | | 10,000 |
| Total - Mile 331.1 to 338.8 | | | | | 35,600 |
| Dallas Floodway to Fort Worth Term | inus (Mile 3 | 38.8 | 3 to 362.8) | | |
| Mile 338.8 to 342.5 | 3.8 (2 | | 7,500 | | 28,500 |
| Mile 342.5 to 346.0 | 3.5 | | 2,500 | | 8,800 |
| Mile 346.0 to 351.9 | 5.9 (2 |) | 7,500 | | 44,300 |
| Mile 351.9 to 357.3 | 5.4 | • | 2,500 | | 13,500 |
| Mile 357.3 to 360.2 | 2.9 (2 |) | 7,500 | | 21,800 |
| Mile 360.2 to 362.8 | 2.6 | • | 2,500 | | 6,500 |
| Emergency dredging at locks No. 19 | , 20 & 21 | | | | 30,000 |
| Total - Mile 338.8 to 362.8 | | | | | 153,400 |
| Fort Worth Terminus to and including | ng Riverside | Dri | ive bridges | (Mile | |
| 369.8) | | | | | |
| Mile 362.8 to 369.8 | 7.0 | | 2,500 | | 17,500 |
| Total - Mile 0.0 to 369.8 | | | | \$ | 1,332,900 |

(1) Maintenance costs for the proposed 12 x 150-foot channel to Liberty in this reach considered to be the same as for the 9 x 150-foot channel.

(2) Reach of channel having 12-foot project depth below normal pool elevation.

128. CHANNEL REVETMENT. The plan for the multiple purpose channel includes bank revetment works at 34 separate curves totalling about 57,000 lineal feet in a total channel length of about 52 miles. The proposed revetment works would consist of 24-inch thickness of quarry run stone ranging from 1/2 inch to 200-pound stone placed on the outerbanks of the curves from two feet above normal pool elevation to the channel bottom gradient. Maintenance of the stone revetment works would consist of occasional replacing and/or reshaping the stone blanket along the top of the revetment works or at the up and downstream ends of the

blanket disturbed by erosion and wave action. Maintenance of the stone blanket below the waterline would be to replace the stone blanket in the event of a "washout" or damage caused by floating craft. For the purpose of this report the cost of maintaining the 57,000 lineal feet of channel revetment works is estimated at \$50,000 annually.

- 129. AIDS TO NAVIGATION. The Commander, Eighth Coast Guard District, New Orleans, Louisiana, by letter dated October 30, 1961, file H2/L-1 Ser 1557, advises that annual maintenance of the aids to navigation proposed for the multiple purpose channel is estimated at \$84,800, annually, and that the estimated life of daybeacons and buoys is estimated to be 12 years and 20 years, respectively. The life of the buoy boats is estimated to be 25 years. The annual cost of replacing the aids to navigation required for the multiple purpose channel is estimated at \$22,000.
- of maintenance and operation of the recreational facilities proposed for the multiple purpose channel is estimated at \$272,000 annually. The annual cost is based on the proposal that general supervision and maintenance of four recreational areas be accomplished by a ranger and a fourman crew, working 40-hour week, performing maintenance of access and park roads, signs and public-use facilities including rental of maintenance and transportation equipment. The cost also provides for contract work for mowing, clean up and repair of public-use buildings and facilities, as may be necessary, resulting from damage through general use, vandalism, storm or other causes. For purposes of economic analyses, the total annual cost of \$272,000 is allocated to various reaches of the channel on the basis of first costs of the construction items shown in table 36.

TABLE 36
ESTIMATED ANNUAL COST OF MAINTENANCE AND OPERATION
OF RECREATIONAL FACILITIES IN DESIGNATED REACHES OF
THE MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS

| Designated reach of channel | : Estimated :annual cost |
|--|--------------------------|
| Houston Ship Channel to head of Wallisville Reservoir (Mile 0.0 to 35.5) Maintenance & operation - 3 recreational areas | \$ 29,000 |
| Head of Wallisville Reservoir to head of authorized channel to Liberty (Mile 35.5 to 47.4) Maintenance & operation - 1 recreational area | 7,000 |
| Head of authorized channel to Liberty & Tennessee Colony Dam (Mile 47.4 to 233.5) Maintenance & operation - 14 recreational areas | 118,500 |

TABLE 36 (Cont'd) ESTIMATED ANNUAL COST OF MAINTENANCE AND OPERATION OF RECREATIONAL FACILITIES IN DESIGNATED REACHES OF THE MULTIPLE PURPOSE CHANNEL TO FORT WORTH, TEXAS

| Designated reach of channel | : Estimated :annual cost |
|--|--------------------------|
| Lock & Dam No. 12 to Five-mile Creek (Mile 274.4 to 322.0) Maintenance & operation - 8 recreational areas | \$ 78,500 |
| Five-mile Creek to and including Dallas Terminus (Mile 322.0 to 326.7) Maintenance & operation | None |
| Dallas Terminus to existing Dallas Floodway (Mile 326.7 to | 331.1)None |
| Dallas Floodway (Mile 331.1 to 338.8) Maintenance & operation - 1 recreational area | 10,000 |
| Dallas Floodway to & including Fort Worth Terminus, (Mile 3 to 362.8) Maintenance & operation - 3 recreational areas | 38.8 29,000 |
| Fort Worth Terminus to & including Riverside Drive bridges (Mile 362.8 to 369.8) Maintenance & operation | None |
| Total - Maintenance & operation | \$272,000 |

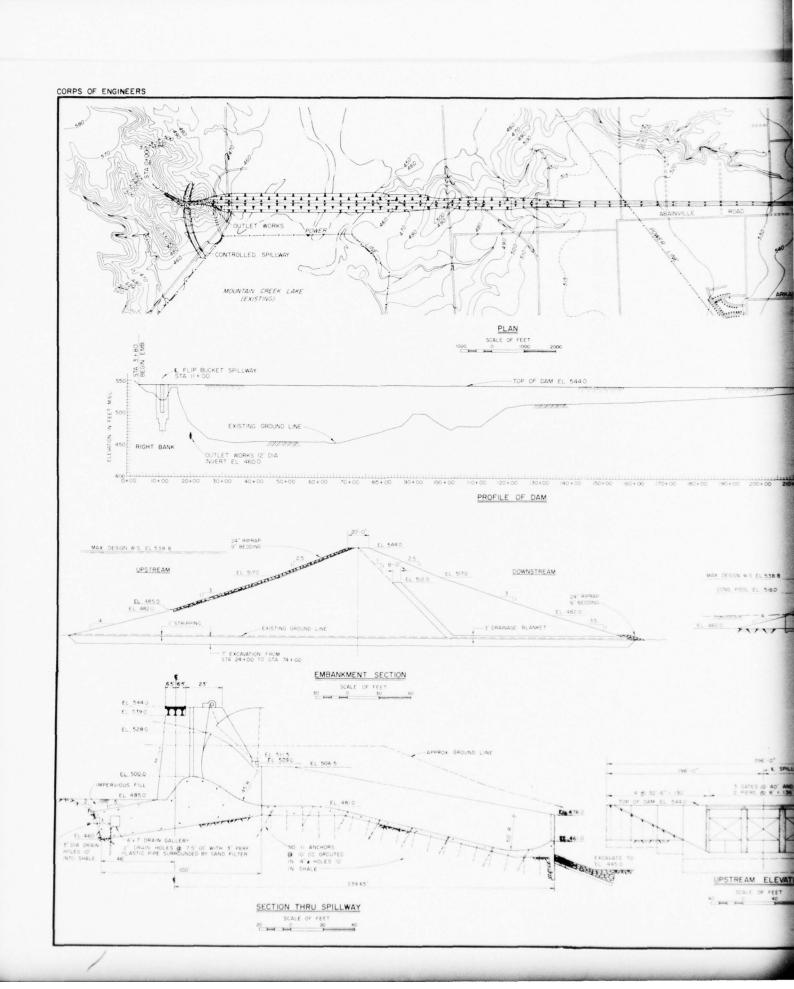
RESERVOIRS

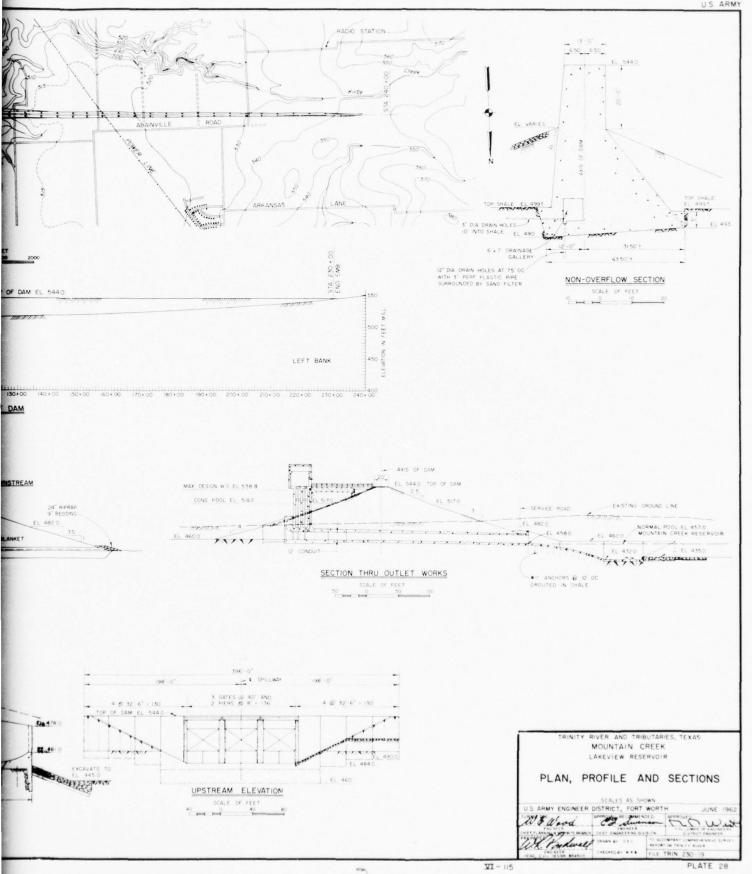
- 131. LAND ACQUISITION CRITERIA. The criteria for land acquisition in reservoirs as contained herein is based on the "Joint Policies of the Departments of the Interior and of the Army Relative to Reservoir Project Lands" which was published in the Federal Register, Volume 27, Page 1734, dated 22 February 1962 and Change 9 of EM 405-2-150 dated 9 March 1962. The fee acquisition line is based on blocking-out the area beneath the upper guide contour or a strip of land 300 feet horizontally from the static full pool elevation, whichever is the greatest, except in remote areas. The fee acquisition line also includes remainders of ownerships having no access or remainders of ownerships where provision of access would cost more than the value of the land. It further includes lands required for public access. Flowage easements are proposed in small remote areas of tributary streams which sever ownerships and are subject to occasional floods.
- 132. LAKEVIEW RESERVOIR PROJECT. The Lakeview dam site is located at river mile 7.2 on Mountain Creek in Dallas County about 3.1 miles above the existing Mountain Creek Dam. The Lakeview Dam site is just south of the city limit of Grand Prairie, about 14 miles from downtown Dallas and 22 miles from downtown Fort Worth. The proposed reservoir would be constructed for flood control, water conservation, fish and wildlife, and general recreation purposes. Subsequent to completion of project formulation studies, local interests proposed a minor revision in the site selection made by the Corps of Engineers for the Lakeview Reservoir project. This revision will be considered in the preconstruction planning of the Lakeview Reservoir project.
- 133. DAM.- The Lakeview Reservoir would be formed by a main earth dam with a maximum height above streambed of about 91 feet and a total length of about 22,620 feet including a 136-foot concrete spillway. The spillway located in a saddle on the right abutment, would be a gate-controlled ogee flip-bucket type with three 40 by 28-foot tainter gates. The net opening of the spillway would be 120 feet. The outlet works would consist of one 12-foot diameter conduit controlled by two 5-foot 6-inch by 12-foot gates. The location and area are shown on plate 27, and the typical sections and detailed layout are shown on plate 28. The general hydraulic design data are shown on plate 29.
- 134. RESERVOIR.- The Lakeview Reservoir will have a total controlled storage of 488,700 acre-feet and an area of 15,650 acres at elevation 528.0, top of spillway gates or top of flood control pool. At top of conservation pool, elevation 518.0, the reservoir will have an area of 12,300 acres and a storage capacity of 349,500 acre-feet, including a portion of the sediment reserve. Lands required for reservoir operation, construction of the proposed dam, and for fish, wildlife, and recreation purposes will be

U.S. DALLAS VICINITY MAP AREA IN THOUSANDS OF ACRES 540 ABOVE CAPACITY AREA 480 CAPACITY IN THOUSANDS OF ACRE-FEET AREA - CAPACITY CURVES - 32° 35' 00" LEGEND U.S. HIGHWAY AND ROUTE NUMBER STATE HIGHWAY AND ROUTE NUMBER FARM TO MARKET HIGHWAY AND ROUTE NUMBER COUNTY ROAD DALLAS 7.2 RIVER MILE (MILE ABOVE MOUTH) RAILROAD PIPE LINE POWER TRANSMISSION LINE TOP OF CONSERVATION POOL - EL 518.0 TOP OF FLOOD CONTROL - EL 5280 MOUNTAIN CREEK LAKEVIEW RESERVOIR RESERVOIR MAP 96* 57' 30" 96* 55' 00" 96*52'50" to Hawley

0

VI - 114





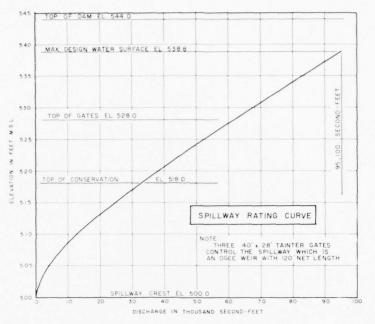


FIG -1

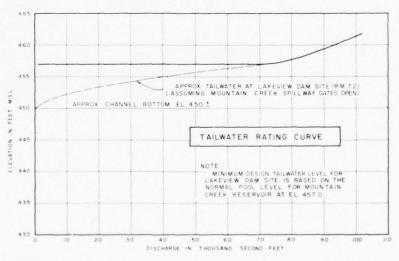


FIG - 3







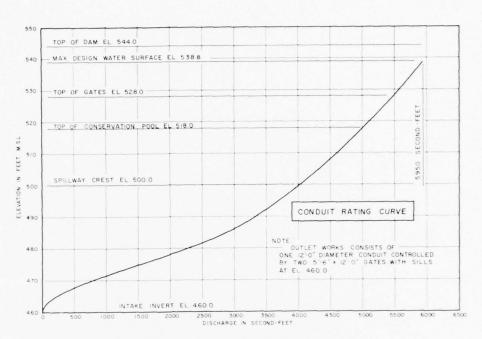


FIG. 2



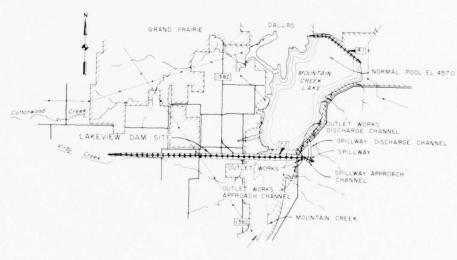


FIG - 4

MOUNTAIN CREEK LAKEVIEW RESERVOIR GENERAL HYDRAULIC DATA PLATE 29

about 20,360 acres in fee simple and 800 acres in flowage easements. This land is classified as 9 percent home site, acreage tracts, and potential subdivisions; 43 percent cropland; 40 percent pasture; and 8 percent as floodwater storage for Dallas Power and Light Company. Construction of Iakeview Reservoir would require relocation of 9.7 miles of farm to market highways, 12.0 miles of county roads, 1.2 miles of railroad, 1.5 miles of pipeline, 33 miles of telephone lines, 33 miles of REA (138KV) distribution lines, and 7.4 miles of electrical transmission and distribution lines. There are no known cemeteries in the reservoir area. No oil or mineral activity of any kind is in evidence anywhere in the reservoir area. Two recognized, but unincorporated, subdivisions are located within the proposed reservoir area. Other pertinent data for the proposed Iakeview Reservoir are shown in table 37 while the location of the appurtenant channel improvement works is shown on plate 43 on page 178.

- 135. GEOLOGY .- The dam site was investigated by drilling and augering seven borings as shown on plate 30. Ten feet of residual clay were penetrated by the boring located in a natural spillway saddle immediately to the east of the crest of the right abutment. The boring, on the slope of the right abutment, encountered 10.6 feet of residual clay overburden overlying 3.6 feet of weathered primary strata. The contact indicated at 10.6 feet is transitional between the residuum and the underlying bedrock. Three hand auger borings were drilled along the axis where it crosses the swampy flood plain area at the upper end of Mountain Creek Lake. None of the three borings reached primary strata although one of the holes was augered to a depth of 20.7 feet. The three hand auger borings penetrated an average of about 6 feet of organic, mucky, silty clay overlying a soft and variable silty and sandy clay which, in places, contained traces of gravel. The clay overburden varies between 20 feet and 23-1/2 feet in thickness beneath the proposed embankment area along the relatively gentle slopes of the left abutment west of the alluvial filled bottom lands.
- 136. Primary strata, immediately underlying the overburden at the Lakeview dam site, are represented by the Eagle Ford shale formation of Upper Cretaceous age. In an unweathered condition, the Eagle Ford is a dark gray, or black, firm laminated shale. The formation contains occasional thin lenses of flaggy limestone and concretionary septeria, but apparently none of the borings were drilled deep enough to reach any of these characteristic components which normally occur in the lower section of the formation.
- 137. FOUNDATION CONDITIONS. Overburden and primary strata consist of impervious materials which will inhibit leakage of impounded water beneath an embankment. Unweathered Eagle Ford shale strata have a bearing capacity adequate to support the required appurtenant structures. At the upper end of Mountain Creek Lake, a significant quantity of organic, silty, clayey muck should be removed from an embankment foundation in the existing flood plain area. This may necessitate the construction of a dike, which can be incorporated into the downstream toe of the embankment. Dewatering provisions should be employed during the foundation excavation in the bottom land area and while placing embankment fill back to, or above, the elevation of the original ground surface.

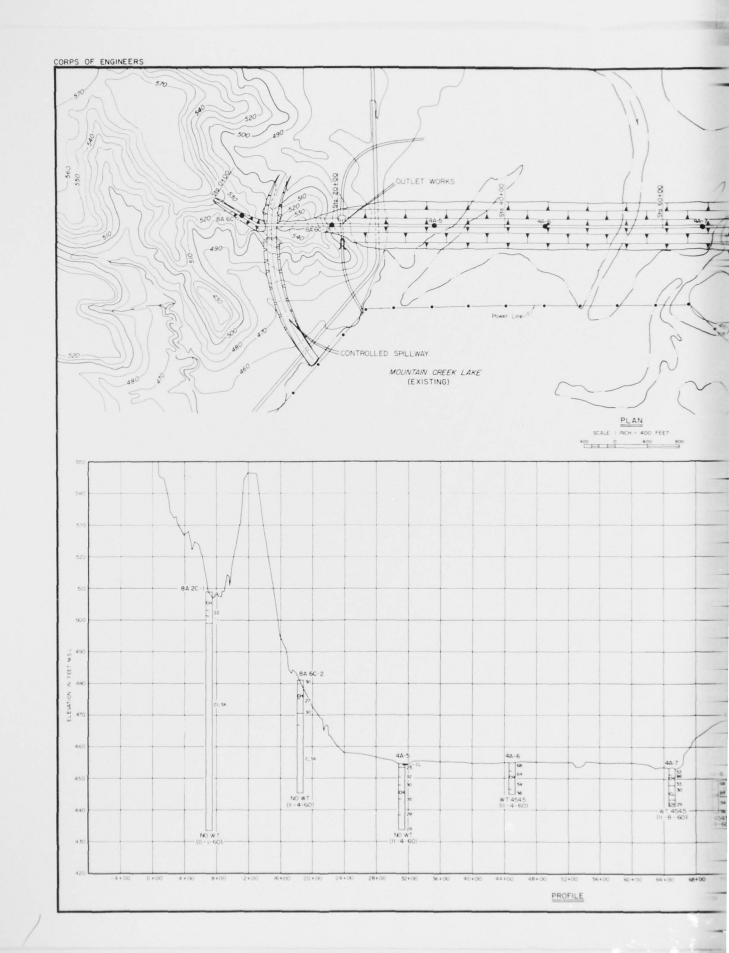
TABLE 37 PERTINENT DATA PROPOSED LAKEVIEW RESERVOIR TRINITY RIVER

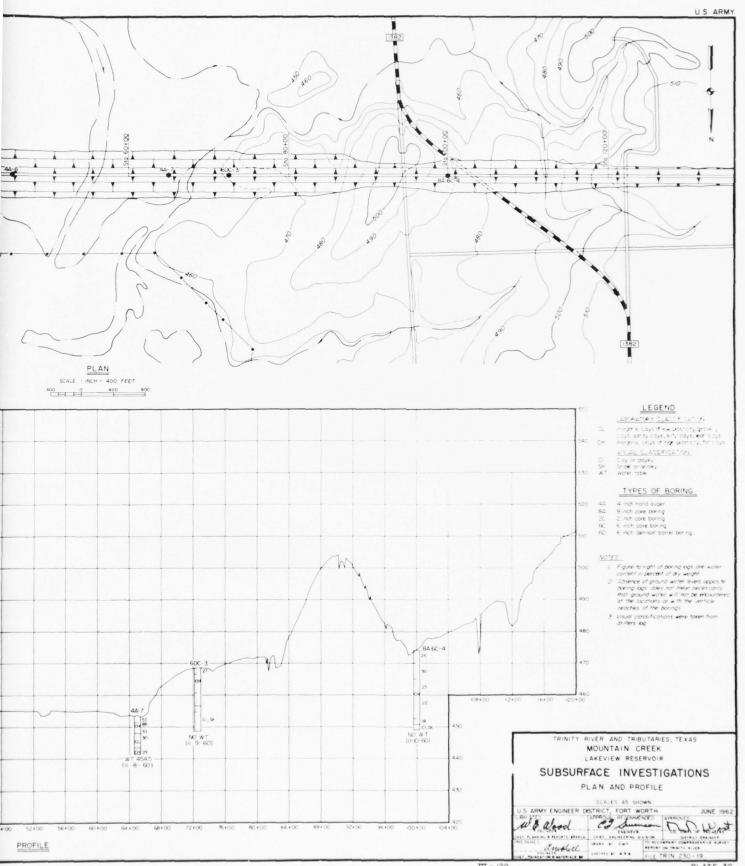
| Item | | control, wat | er conservation, |
|-------------------------------|-----------|--------------|------------------|
| Miscellaneous | | | |
| Dam location, river mile | | | 7.2 |
| Drainage area, square miles | | | 272 |
| Flood control storage, acre-f | eet | | 136,700 |
| Water conservation storage, a | | | 306,400 |
| Sediment storage | | | 45,600 |
| Yield, CFS | | | 47 |
| Million gallons daily | | | 30 |
| Spillway design flood | | | |
| Peak inflow, CFS | | | 372,400 |
| Volume, acre-feet | | | 413,400 |
| Volume, inches | | | 28.50 |
| Peak outflow, CFS | | | 101,000(1 |
| | Elevation | Area | Capacity |
| Reservoir | (feet) | (Acres) | (Acre-feet) |
| Spillway crest | 500.0 | 7,910 | 170,700 |
| Top of conservation storage | 518.0 | 12,300 | 349,500 |
| Top of gates | 528.0 | 15,650 | 488,700 |
| Maximum design water surface | 538.8 | 21,210 | 686,300 |
| Top of dam | 544.0 | | - |
| Maximum tailwater at dam | 462.6 | - | |
| | | | |
| Dam | | | |
| Type of dam | | | h fill |
| Total length, feet (including | spillway) | 22,6 | 20 |
| Embankment section: | | | |
| Туре | | | acted earth fill |
| Total length, feet (minus s | pillway) | 22,2 | 24 |
| Height above streambed, fee | t | 91 | |
| Freeboard, feet | | 5.2 | |
| Crown width, feet | | 20 | |
| Side slopes: | | | |
| Upstream | | 2-1/ | 2:1, 3:1, 4:1 |
| Downstream | | | 2:1, 3:1, 3-1/2 |
| Non-overflow sections: | | | |
| Туре | | Conc | rete gravity |
| Total length, feet | | 260 | |
| Height above end sill, feet | | 83 | |
| Top width, feet | | 13 | |
| Spillway section: | | | |
| Type | | Cong | . ogee, flip bu |
| Gross length, feet | | 136 | . oboo, rrip ou |
| Net length, feet | | 120 | |
| Crest height above apron, fee | t. | 39 | |
| ores derem above apron, ree | | 39 | |
| | | | |

TABLE 37 (CONT'D) PERTINENT DATA PROPOSED LAKEVIEW RESERVOIR TRINITY RIVER

| T+om | Flood control, water conservation |
|------------------------------|-----------------------------------|
| Item | and recreation |
| Gates: | Mointer |
| Type | Tainter |
| Number | 3 feet 40x28 |
| Size (Width x height), | |
| Spillway discharge, CFS | |
| Top of gates | 61,600 |
| Maximum water surface | 95,100 |
| Outlet works | |
| Type | Conduit with 2 gated |
| | inlets |
| Number of conduits | 1 |
| Diameter, feet | 12 |
| Invert elevations, feet | 460.0 |
| Conduit control | Two 5'6"x12' gates |
| Relocations | |
| Highways, miles | 9.7 |
| County roads, miles | 12.0 |
| Railroads, miles | 1.2 |
| Power lines (138KV), miles | 7.4 |
| REA distribution line, miles | 33.0 |
| Telephone lines, miles | 33.0 |
| Pipelines, miles | 1.5 |
| Cemeteries, number | |
| Towns, number | None |
| Lands | |
| Dam and reservoir: | |
| Clearing, acres | 2,765 |
| Land acquisition: | |
| Fee simple, acres | 19,600 |
| (Top control elevation) | 531.0 |
| Flood easement, acres | 800 |
| (Top control elevation) | 531.0 |
| Recreation: | |
| Clearing, acres | 8,500 |
| Land acquisition: | |
| Fee simple, acres | 760 |
| * | |
| | |

⁽¹⁾ Includes 5,900 CFS discharge through the outlet works.





- 138. AVAILABILITY OF CONSTRUCTION MATERIALS. Adequate quantities of impervious materials for the embankment are available in the immediate vicinity of the dam site. Pervious materials which can be used for a drainage blanket, are being produced from pits along the Trinity River within about 15 miles of the dam site. Fine aggregate can be obtained from the same sources. Coarse aggregate and riprap are not available locally and will have to be shipped by railroad or truck. Four sources of acceptable coarse aggregate and riprap are near Bridgeport, Texas, and an acceptable riprap source is near Tehuacana, Texas. Railroad facilities exist within approximately three miles of the dam site.
- 139. COST ESTIMATE. The detailed estimate shows the separation of first costs for the dam and reservoir and for recreation included in the project. A summary of first costs and annual charges for the Lakeview Reservoir project are shown in table 38. The detailed cost estimate for the Lakeview Reservoir project is shown in table 39.

TABLE 38 SUMMARY OF FIRST COST AND ANNUAL CHARGES PROPOSED LAKEVIEW RESERVOIR MOUNTAIN CREEK

| | Item | Costs |
|----|--|--|
| | FIRST COST | |
| | Federal First Cost | |
| • | Iands and damages | \$14,354,000 |
| | Relocations | 2,038,000 |
| | Reservoir (clearing) | 493,000 |
| | Dam | .,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, |
| | a. Earth embankment | 5,396,000 |
| | b. Concrete, dam and spillway | 2,168,000 |
| | c. Outlet works | 1,201,000 |
| | Access roads | 244,000 |
| | Recreation facilities | 3,250,000 |
| | Buildings, grounds, and utilities | 200,000 |
| | Permanent operating equipment | 91,000 |
| | Engineering and design | 716,000 |
| | Supervision and administration | 1,029,000 |
| | Total estimated Federal first cost | 31,180,000 |
| | Total estimated rederal filst cost | 31,100,000 |
| | Non-Federal First Cost | None |
| | Total Estimated First Cost of Project | 31,180,000 |
| Cc | onstruction period-5 years) (100 yr. amortizat | ion)(2-7/8% interest rat |
| | Federal Investment a. Federal first cost b. Interest during construction | \$31,180,000 2,241,100 |
| | a. Federal first cost | 2,241,100 |
| | a. Federal first cost b. Interest during construction Total Federal investment | 2,241,100 33,421,100 |
| | a. Federal first costb. Interest during construction | 2,241,100 |
| | a. Federal first cost b. Interest during construction Total Federal investment | 2,241,100 33,421,100 None |
| | a. Federal first cost b. Interest during construction Total Federal investment Non-Federal Investment | 2,241,100 33,421,100 |
| | a. Federal first cost b. Interest during construction Total Federal investment Non-Federal Investment Federal Annual Charges | 2,241,100 33,421,100 None |
| | a. Federal first cost b. Interest during construction Total Federal investment Non-Federal Investment Federal Annual Charges a. Interest on investment b. Amortization charge | 2,241,100 33,421,100 None |
| | a. Federal first cost b. Interest during construction Total Federal investment Non-Federal Investment Federal Annual Charges a. Interest on investment | 2,241,100 33,421,100 None |
| | a. Federal first cost b. Interest during construction Total Federal investment Non-Federal Investment Federal Annual Charges a. Interest on investment b. Amortization charge c. Operation and maintenance (including | 2,241,100 33,421,100 None 960,900 59,800 |
| | a. Federal first cost b. Interest during construction Total Federal investment Non-Federal Investment Federal Annual Charges a. Interest on investment b. Amortization charge c. Operation and maintenance (including replacement of parts) | 2,241,100 33,421,100 None 960,900 59,800 351,300 |
| | a. Federal first cost b. Interest during construction Total Federal investment Non-Federal Investment Federal Annual Charges a. Interest on investment b. Amortization charge c. Operation and maintenance (including replacement of parts) Total-Federal annual charges | 2,241,100 33,421,100 None 960,900 59,800 351,300 1,372,000 |
| | a. Federal first cost b. Interest during construction Total Federal investment Non-Federal Investment Federal Annual Charges a. Interest on investment b. Amortization charge c. Operation and maintenance (including replacement of parts) Total-Federal annual charges Total-Non-Federal Annual Charges | 2,241,100 33,421,100 None 960,900 59,800 351,300 1,372,000 None |

| PERTINENT INFORMATI | ON |
|--------------------------------|---------|
| Top of dam, elevation | 544.0 |
| Top of gates, elevation | 528.0 |
| Spillway crest, elevation | 500.0 |
| Storage capacity (top of gates | 443,100 |
| less sediment) acre-feet | |

| Item | | : Unit | : | : |
|------------------------------|------------|-------------|-----------|-------------|
| | :Unit: | cost | :Quantity | : Cost |
| DEPART OF TAXABLE OF TAXABLE | GOOM DAI | A NID DECEM | WATE: | |
| DETAILED ESTIMATE OF FIRST | COST-DAM | M AND RESER | VOIR | |
| a. Land costs | | | | |
| (1) Fee simple lands | Acre | | 19,600 | \$7,733,875 |
| (2) Flood easement lands | Acre | | 800 | 256,500 |
| (3) Severance damage | LS | | 000 | 310,000 |
| (4) Improvements | IS | | | 2,280,000 |
| (5) Resettlement reimburseme | | | | 42,000 |
| Subtotal - land costs | C.II (1.10 | | | 10,622,375 |
| Contingencies, 25% ± | | | | 2,655,625 |
| Total - land costs | | | | 13,278,000 |
| b. Land acquisition expense | | | | 271,000 |
| Total - lands and damage | 9.0 | | | 13,549,000 |
| (02.0) Relocations | -5 | | | 25,7.7,000 |
| a. Roads and railroad | | | | |
| (1) F.M. Highway 661 | | | | |
| Embankment | CY | \$ 0.32 | 22,000 | 7.040 |
| Flexible base | CY | 4.50 | 4,400 | 19,800 |
| Surfacing | SY | 0.40 | 9,900 | 3,960 |
| Bridge (28' roadway) | LF | 150.00 | 202 | 30,300 |
| Riprap | CY | 6.00 | 2,500 | 15,000 |
| Bedding | CY | 3.00 | 1,300 | 3,900 |
| Subtotal - F.M. Highway | | | -,5 | 80,000 |
| (2) F.M. Highway 1382 | 00.2 | | | |
| New road | Mile | 50,000.00 | 9 | 450,000 |
| Spillway channel bridge | LF | 150.00 | 152 | 22,800 |
| Embankment | CY | 0.32 | 85,000 | 27,200 |
| Subtotal - F.M. Highway | 1382 | | | 500,000 |
| (3) County roads | Mile | 12,500.00 | 12 | 150,000 |
| (4) T. &N. O. Railroad | | | | |
| Embankment | CY | 0.32 | 125,000 | 40,00 |
| Ballast | CY | 6.00 | 4,400 | 26,40 |
| Cross ties | Each | 4.00 | 4,000 | 16,00 |
| Tracklaying and surfacing | Mile | 6,600.00 | 1.2 | 3 8,11 |
| Relocate signal system | LS | | | 7,50 |
| Ballast deck trestle | LF | 150.00 | 1,336 | 200,40 |
| Bridge | LF | 125.00 | 100 | 12,50 |
| Riprap | CY | 6.00 | 14,000 | 84,00 |
| Bedding | CY | 3.00 | 5,000 | 15,00 |
| Subtotal - T. & N.O. R. | R. | | | 409,91 |
| Subtotal - roads and ra | ilroad | | | 1,139,91 |
| VI- | 123 | | | |

| Item : | | 0 0 | : : | |
|---|-------|------------|------------|-----------|
| | Unit: | cost | :Quantity: | Cost |
| b. Utilities | | | | |
| (1) Electric lines | | 1 | _ , | 4000 000 |
| 138 K.V. transmission lines | | | | \$222,000 |
| R.E.A. distribution lines | Mile | 2,000.00 | 33 | 66,000 |
| Subtotal electric lines | | | 20 | 288,000 |
| (2) Telephone lines(3) Pipelines | Mile | 1,000.00 | 33 | 33,000 |
| Lone Star Gas Co. 18" line | Mile | 120,000.00 | 1 | 120,000 |
| Magnolia 16" line | | 100,000.00 | | 50,000 |
| Subtotal - pipelines | | | | 170,000 |
| Subtotal - utilities | | | | 491,000 |
| Subtotal - relocations | | | | 1,630,918 |
| Contingencies, 25% ± | | | | 407,082 |
| Total - relocations | | | | 2,038,000 |
| 03.0) Reservoir clearing | Acre | 35.00 | 2,765 | 96,775 |
| Contingencies, 25% + | | | | 24,225 |
| Total-reservoir clearing | | | | \$121,000 |
| 04.0) Dams | | | | |
| a. Earth embankment | | | | |
| (1) Diversion and care of wat | er LS | | | 29,800 |
| (2) Clearing and grubbing | Acre | 200.00 | | 29,600 |
| (3) Excavation, stripping | CY | 0.25 | | 55,500 |
| (4) Excavation, common | CY | 0.25 | | 174,500 |
| (5) Excavation, borrow | CY | | 7130,000 | 1,711,200 |
| (6) Compacted fill | CY | | 6560,000 | 459,200 |
| (7) Drainage blanket | CY | 2.50 | | 798,625 |
| (8) Riprap | CY | 6.00 | | 823,290 |
| (9) Bedding | CY | 3.00 | | 154,365 |
| (10)Flexible base | CY | 4.50 | | 38,115 |
| (11)Aggregate | CY | 6.00 | | 4,140 |
| (12)Asphalt treatment | Gal. | 0.16 | | 3,952 |
| (13) Timber guide posts | Each | 5.00 | | 8,720 |
| (14)Slope protection, turfing | | 400.00 | 64 | 25,600 |
| Subtotal - earth embank | ment | | | 4,316,607 |

| Item | : : | Unit | : : | |
|-------------------------------|--------|-------------|-----------|-----------|
| | :Unit: | cost | Quantity: | Cost |
| b. Concrete dam and spillway | v | | | |
| (1) Care of water during | LS | | | 30,000 |
| construction | ш | | | 50,000 |
| (2) Clearing | Aana | \$150.00 | 3 | 450 |
| (3) Excavation, common | CY | 0.30 | 95,000 | |
| (4) Excavation, shale | CY | 0.60 | 330,000 | |
| (5) Structural backfill | CY | 1.00 | 43,000 | |
| (6) Drilling and grouting | LF | 2.25 | 2,530 | |
| anchor holes | 1.1 | 2.2) | 2,550 | 7,093 |
| | LF | 2.00 | 3,060 | 6,120 |
| (7) Drilling drain holes | | | | |
| (8) Asphalt, shale treatmen | | 0.90 | 2,240 | |
| (9) Concrete, weir | CY | 18.00 | 11,340 | |
| (10)Concrete, non-overflow | CY | 20.00 | 15,120 | |
| (11)Concrete, pier | CY | 25.00 | 3,335 | |
| (12)Concrete,slab | CY | 20.00 | 3,980 | |
| (13)Concrete, wall | CY | 35.00 | 780 | |
| (14)Concrete, bridge deck | CY | 60.00 | 35 | |
| (15)Cement | Bbl | 5.00 | 35,000 | |
| (16)Steel, reinforcing | Lb | 0.13 | 1,397,000 | |
| (17)Steel, structural | Lb | 0.20 | 37,900 | |
| (18) Pipe railing (aluminum) | Lb | 1.50 | 2,870 | 4,305 |
| (19)Pipe railing, bridge " | Lb | 1.50 | 4,500 | |
| (20) Metals, miscellaneous | Lb | 0.40 | 5,000 | |
| (21) Ladders, gratings, &gril | lls Lb | 1.50 | 4,000 | |
| (22) Water stop, copper | Lb | 1.75 | 860 | ,,,, |
| (23) Water gages, tile | LF | 14.00 | 60 | 840 |
| (24) Tainter gates | Lb | 0.30 | 239,000 | 71,700 |
| (25) Tainter gate hoists, | Lb | 1.00 | 58,500 | 58,500 |
| shafts, & hangers | | | | |
| (26)Trunnion anchorages | Lb | 1.00 | 73,000 | 73,000 |
| & seals | | | | |
| (27) Emergency bulkheads | Lb | 0.32 | 63,000 | 20,160 |
| (28)Precast bridge girders | Each | 700.00 | 9 | 6,300 |
| (29)Crane | | 75,000.00 | ĺ | 75,000 |
| (30)Electrical facilities | LS | | | 7,000 |
| (31)Standby power unit | Each | 5,000.00 | 1 | 5,000 |
| (32)Riprap | CY | 7.00 | 1,316 | 9,212 |
| (33)Bedding | CY | 4.50 | 475 | 2,138 |
| (34)12"drain holes 3" plast | | ,- | .12 | 8,500 |
| pipe & sand filter | | | | 0,,00 |
| | | and spillwa | | 1,734,774 |

| Item | | Unit | : | |
|--------------------------------|--------|-----------|-------------|-----------|
| | :Unit: | cost | :Quantity : | Cost |
| c. Outlet works | | | | |
| (1) Care of water during | LS | | | 20,000 |
| construction | | | | 1 |
| (2) Clearing | Acre | 150.00 | 1; | 600 |
| (3) Excavation, common | CY | 0.50 | 18,700 | 9,350 |
| (4) Excavation, shale | CY | 0.90 | 48,000 | 43,200 |
| (5) Backfill, structural | CY | 1.00 | 11,000 | 11,000 |
| (6) Drilling & grouting | LF | 2.25 | 790 | 1,778 |
| anchor holes | LI | 2027 | 170 | -, 11 |
| (7) Drilling drain holes | LF | 2.00 | 790 | 1,580 |
| | | 1.00 | 170 | 170 |
| (8) Asphalt, shale treatmen | LS | 1.00 | 110 | 50,000 |
| (9) Operating house | | 1,0,00 | 495 | 19,800 |
| (10)Concrete, control tower | CY | 40.00 | | |
| (11)Concrete, tower base | CY | 38.00 | 1,625 | 61,750 |
| & transition | ~~~ | 22.00 | 1 (10 | 1.9 200 |
| (12)Concrete, conduit | CY | 30.00 | 1,610 | 48,300 |
| (13)Concrete, slab | CY | 24.00 | 955 | 22,920 |
| (14)Concrete, walls | CY | 35.00 | 2,470 | 86,450 |
| (15)Concrete, bridge | CY | 60.00 | 95 | 5,700 |
| (16)Cement | Bbl | 5.00 | 9,080 | 45,400 |
| (17)Steel, reinforcing | Lb | 0.13 | 937,000 | 121,810 |
| (18)Steel, structural | Lb | 0.22 | 46,000 | 10,120 |
| (19) Metal, miscellaneous | Lb | 0.60 | 1,180 | 708 |
| (20) Ladders, gratings, & gril | | 0,,60 | 1,680 | 1,008 |
| (21)Spiral stairs | Set | 1,530.00 | 1 | 1,530 |
| (22)Conduit liner | Lb | 0.30 | 529,000 | 158,700 |
| (23) Rubber water stop | LF | 3.00 | 440 | 1,320 |
| (24) Water gages, tile | LF | 14.00 | 25 | 350 |
| (25) Gates & operating | IS | | | 185,000 |
| equipment | | | | |
| (26)Bulkhead | Lb | 0.25 | 56,000 | 14,000 |
| (27) Overhead crane | Each | 16,000.00 | 1 | 16,000 |
| (28)Electrical facilities | IS | | | 4,000 |
| (29)Riprap | CY | 7.00 | 2,120 | 14,840 |
| (30)Bedding | CY | 4.50 | 750 | 3,375 |
| Subtotal - outlet | works | | | 960,759 |
| Subtotal - Dams | | | | 7,012,140 |
| Contingencies, 25% | + | | | 1,752,860 |
| Total - Dams | | | | 8,765,000 |
| 8.0) Access road | Mile | 40,000 | 1 | 40,00 |
| | | , , , , , | | |
| Contingencies, 25% | 4. | | | 10,00 |

| | 0 0 | Unit | 2 . | |
|--|--|-----------------------|-------------|--------------|
| Marie San Control of C | :Unit: | cost | :Quantity: | Cost |
| (20 0) Designation | | | | |
| (19.0) Buildings, grounds and | IS | 65 | | 100,000 |
| (1) Maintenance facilities | LS | | | 30,000 |
| (2) Water supply (3) Powerline & substation | IS | | | 10,000 |
| (4) Visitors overlock | IS | | | 20,000 |
| facilities | 710 | | | 20,000 |
| Subtotal-Buildings, | and and a | c & 2:++ | littes | 160,000 |
| Contingencies, 25% | | s, was | 110169 | 40,000 |
| Total-Buildings, gr | | | ac | 200,000 |
| (20.0) Permanent operating equ | | the water hands for a | .00 | 200,000 |
| (1) Radio-telephone equipmen | + TS | | | 5,000 |
| (2) Boats | IS | | | 9,200 |
| (3) Miscellaneous furniture | LS | | | 23,800 |
| & equipment | charter. | | | -5, |
| (4) Stream gages | LS | | | 8,000 |
| (5) Evaporation & rain gages | | | | 2,000 |
| (6) Sediment & degradation | LS | | | 25,000 |
| ranges | | | | |
| Subtotal-Permanent | ccerati | ng equip | ment | 73,000 |
| Contingencies, 25%; | | | | 18,000 |
| Total-Permanent ope | | equipmen | it | 91,000 |
| (30.0) Engineering and design | | | | 428,000 |
| (31.0) Supervision and adminis | stration | | | 676,000 |
| Subtotal-estimated | THE RESERVE AND LOSS OF THE PARTY AND LOSS O | ost-dam | and reservo | r 25,918,000 |
| | | | | -///- / |
| B. DETAILED ESITMATE OF FIRST O | OST - R | ECREATIO | ON | |
| (Ol.O) lands and damages | THE RESERVE AND ADDRESS OF THE PERSON NAMED IN | | | |
| (i) Fee simple lands | Acre | | 760 | \$494,000 |
| (2) Severance damage | LS | | | 63,000 |
| (3) Diprovementa | LS | | | 75,000 |
| Subtotal | | | | 632,000 |
| Contingencies. 25%; | | | | 158,000 |
| (4) Acquisition expense | | | | 15,000 |
| Total lands and dan | B -8703 | | | 805,000 |
| (03.0) Reservoir clearing | Arra | 35.00 | 8,500 | 297,500 |
| Contingencies, 25% | | | | 74,500 |
| Total - Reservoir c | Learing | | | 372,000 |
| | | | | |

| Item | :Unit: | Unit cost | : :Quantity: | Cost |
|---|-----------------|--------------|-----------------|---|
| (08.0) Access roads Contingencies, 25 Total-access road (14.0) Facilities | | 31,000 | 5 | \$ 155,000 39,000 194,000 |
| (1) Gravel roads (2) Paved roads (3) Parking areas (4) Boat launching ramps (5) Sanitary facilities (6) Water supply system (7) Picnic facilities (8) Site improvements (9) Signs and buoys Subtotal Contingencies, 25 Total-facilities (30.0) Engineering and desig (31.0) Supervision and admin | n distration | | | 728,000 364,000 260,000 52,000 234,000 260,000 546,000 104,000 52,000 2,600,000 650,000 288,000 353,000 |
| recreation | | | | 5,262,000 |
| TOTAL - ESTIMATED PROJECT FIR Note: Prices are as of Janua | | | | \$31,180,000 |

140. AUBREY RESERVOIR. - The Aubrey dam site is at mile 60.0 on the Elm Fork of the Trinity River in northeast Denton County. The site is between the towns of Sanger and Aubrey and is 30 river miles upstream from the Corps of Engineers' Lewisvill Dam (Garza-Little Elm Reservoir). The Aubrey Reservoir would contain 899,900 acre-feet of controlled storage capacity of which 258,300 acre-feet would be for flood-control, 603,800 acre-feet would be for water conservation, and 37,800 acre-feet for sediment reserve. The flood-control storage in Aubrey Reservoir would permit a reallocation of storage in the Garza-Little Elm Reservoir and increase that storage presently allocated to water conservation purposes. Under 2020 conditions of watershed development and a recurrence of the 1950-1957 drought period, the dependable yield from the proposed Aubrey-Garza-Little Elm Reservoir system is estimated to be 151.3 million gallons per day. This system will produce an increased dependable yield of 65.3 MGD over that which would be produced by Garza-Little Elm Reservoir alone. State Highway 10, FM Highways 372 and 455, county roads, the Texas and Pacific Railroad, and a pipe line would require relocation or alteration. The estimated first cost of this project is \$34,073,000. Pertinent data on the Aubrey Dam and Reservoir are contained in table 40. Plates 31 thru 33 show the reservoir area; plan, profile and sections; subsurface investigations and general hydraulic data for Aubrey Reservoir.

TABLE 40
PERTINENT DATA PROPOSED AUBREY RESERVOIR
ELM FORK OF TRINITY RIVER

| 3 0 | Water conservation, and |
|---------------------------------------|-------------------------|
| Item : | recreation (1) |
| Miscellaneous | |
| Dam location, river mile | 60.0 |
| Drainage area, square miles | 682 |
| Flood control storage, acre-feet | 258,300 |
| Water conservation storage, acre feet | 603,800 |
| Sediment storage, acre-feet | 37,800 |
| Yield, CFS | 75 |
| Million gallons daily | 116 |
| Spillway design flood | |
| Peak inflow, CFS | 483,100 |
| Volume, acre-feet | 952,000 |
| Volume, inches | 26.18 |
| Peak outflow, CFS | 350,800 |

TABLE 40 (CONT'D) PERTINENT DATA PROPOSED AUBREY RESERVOIR ELM FORK OF TRINITY RIVER

| | 0 | Water conse | ervation, and | | |
|---|-------------|--------------------|----------------|--|--|
| Item | 9 | recreat | recreation (1) | | |
| E | levation | Area | Capacity | | |
| Reservoir | (feet) | (acres) | (acre-ft.) | | |
| Spillway crest | 600.0 | 10,800 | 203,300 | | |
| Top of conservation storage | 625.5 | 24,340 | 639,000 | | |
| Top of gates | 635.0 | 30,750 | 899,900 | | |
| Maximum design water surface | 640.3 | 34,530 | 1,072,800 | | |
| Top of dam | 646.0 | 38,800 | 1,281,800 | | |
| D am | | | | | |
| Type of dam | | Earth | fill | | |
| Total length, feet(including | spillway) | 13,660 | | | |
| Embankment section: | 1 | -5, | | | |
| Туре | | Compacted | earth fill | | |
| Total length, feet (minus | spillway) | 12,972 | | | |
| Height above streambed, f | | 116 | | | |
| Freeboard, feet | | 5.7 | | | |
| Crown width, feet | | 20 | | | |
| Side slopes: | | 20 | | | |
| Upstream | | 1 on 3 | | | |
| Downstream | | 1 on 3 | | | |
| Downs of eath | | 1 011 5 | | | |
| Non-overflow sections: | | | | | |
| Type | | Concrete & | gravity | | |
| Total length, feet | | 328 | , | | |
| Height above apron, feet | | 122 | | | |
| Top width, feet | | 13 | | | |
| Spillway section: | | | | | |
| Type | | Concrete o | gee | | |
| Gross length, feet | | 688 | | | |
| Net length, feet | | 360 | | | |
| Crest height above apron, fe | et | 76 | | | |
| Gates: | | | | | |
| Туре | | Tainter | | | |
| Number | THE RESERVE | 9 | | | |
| Size (width x height), fe | et | 40 x 35 | | | |
| Chillman diachamas COC. | | | | | |
| Spillway discharge, CFS: | | | | | |
| Top of gates Maximum design water surf | | 278,000 350,800 | | | |

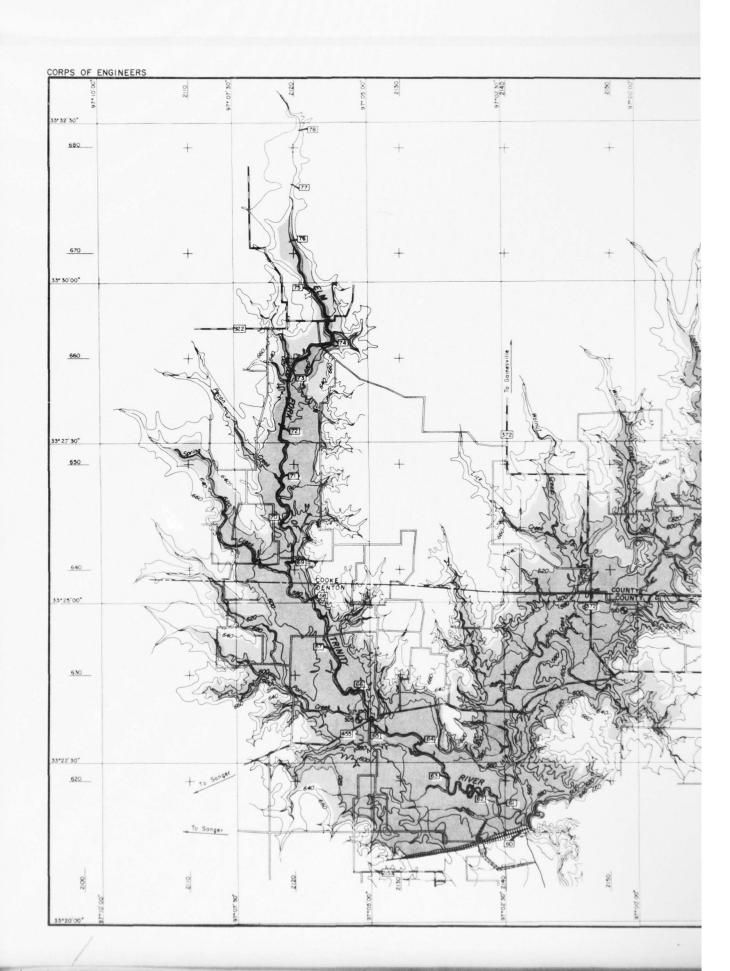
TABLE 40 (CONT'D) PERTINENT DATA PROPOSED AUBREY RESERVOIR ELM FORK OF TRINITY RIVER

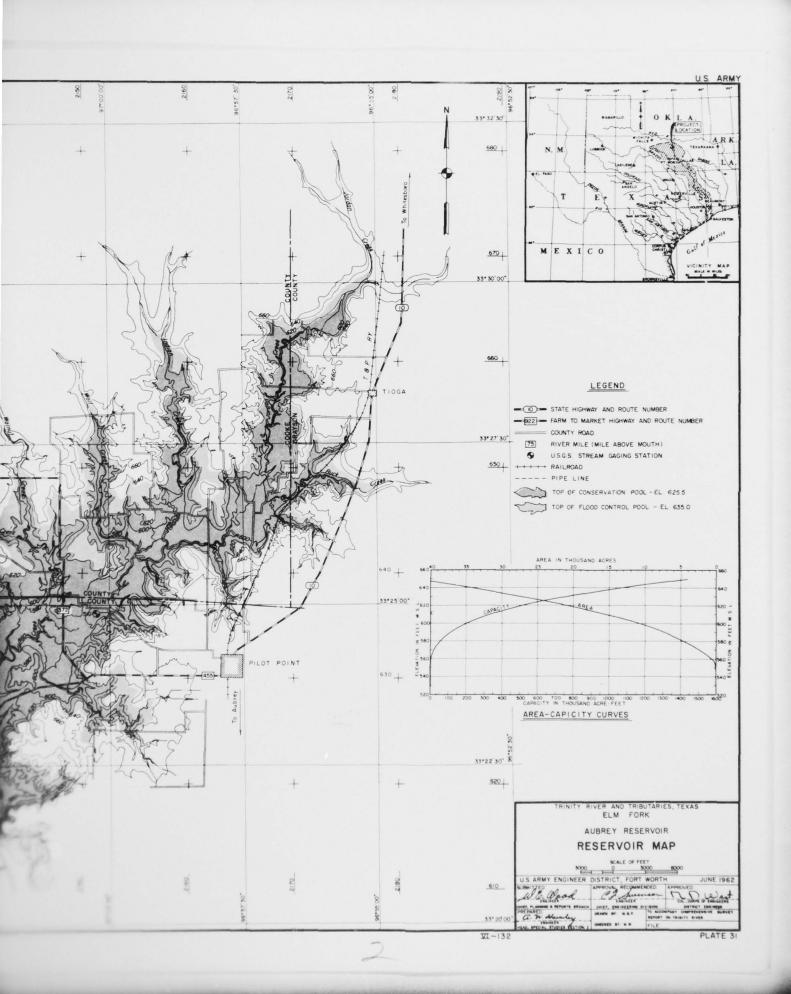
| Item | Water conservation, and recreation (1) |
|-----------------------------|--|
| Outlet works | |
| Туре | Gate-controlled sluices through |
| | spillway piers |
| Number of sluices | 2 |
| Diameter, inches | 36 |
| Invert elevations, feet | 550.0 |
| Sluice control | Power operated slide gates |
| Relocations | |
| Highways, miles | 16.0 |
| County roads, miles | 6.0 |
| Railroads, miles | 5.0 |
| Power lines, miles | 12 |
| Telephone lines, miles | 10 |
| Pipelines, miles | 1.0 |
| Cemeteries, number | 0 |
| Towns, number | 0 |
| Lands | |
| Dam and reservoir: | |
| Clearing, acres | 5,900 |
| Land acquisition: | |
| Fee simple, acres | 37,700 |
| (top control elevation) | 638.0 |
| Flood easement, acres | 1,500 |
| Recreation: | |
| Clearing, acres (2) | 16,000 |
| Land acquisition: | |
| Fee simple, acres (2) | 1,400 |
| Conversion, easement to fee | , |
| acres (3) | 2,800 |

⁽¹⁾ Aubrey Reservoir has a primary purpose of water conservation; however, the reservoir contains flood-control storage to enable a comparable amount of flood-control storage in the downstream reservoir, Garza-Little Elm, to be reallocated to water conservation.

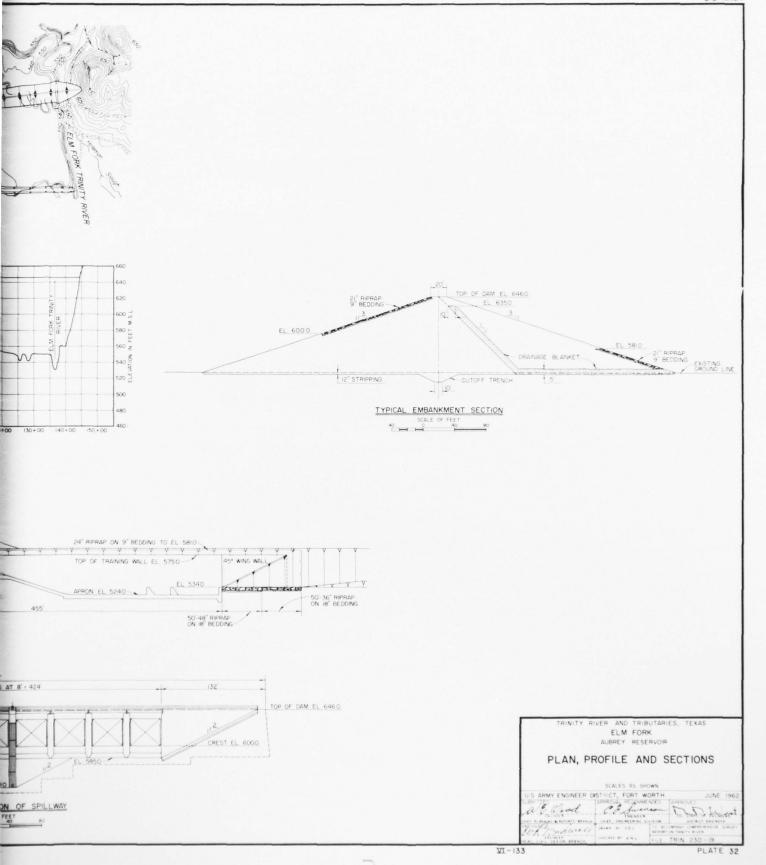
⁽²⁾ Includes requirements for additional general recreation at Garza-Little Elm Reservoir.

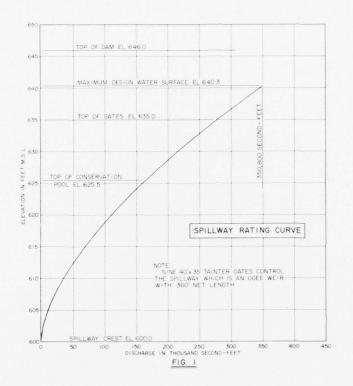
⁽³⁾ In Garza-Little Elm Reservoir.



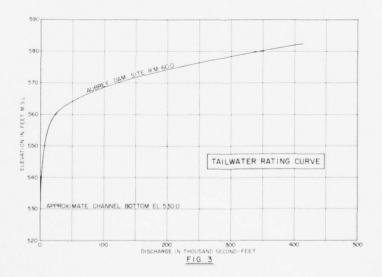


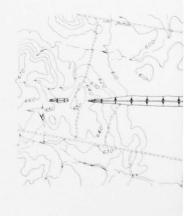


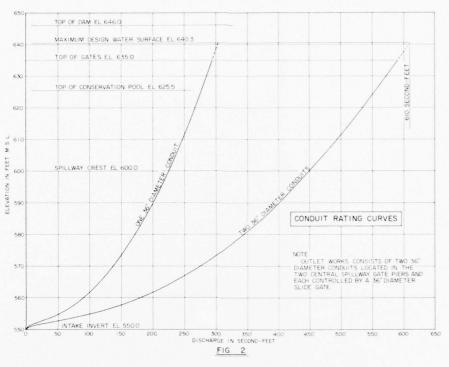


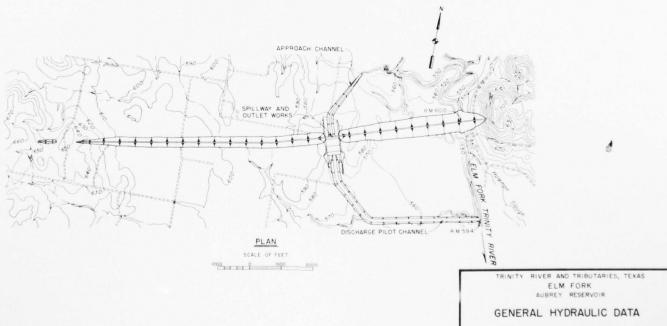












VI - 134

SCALES AS SHOWN
S ARMY ENGINEER DISTRICT, FORT WORTH

W & Wood

PLATE 33

141. GEOLOGY .- The general upland area in the vicinity of the Aubrey Dam site is overlain with a thin mentle of residual material originating from the underlying Washita group and the Woodbine sand. Foundations in the reservoir area are made up of alternating layers of limestone, marl, and shale. Because these materials are impervious practically no water would be lost from the reservoir by percolation. Strata of the Upper and Lower Cretaceous divisions occur in the vicinity of the dam site. The only formation outcropping in this area is the Upper Cretaceous, Woodbine sand. The Elm Fork follows very closely the contact between the Upper and Lower Cretaceous rocks, thus the east bank of the proposed dam site is composed of Woodbine sand while the west bank is composed of the Grayson formation. As in the reservoir area, foundation materials at the site are composed of alternating layers of shale, marl, and soft limestone. The shale and marl disintegrate easily when exposed to the atmosphere and, although these materials are watertight and structurally sound, they erode rapidly. A layer of sand and gravel lies on the top of the primary rock below the valley floor. No geological structures such as faults, caverns, or folds are present either at the dam site or in the reservoir. Subsurface investigation locations, plan and profile are shown in plate 34.

142. AVAILABILITY OF CONSTRUCTION MATERIALS. Four stone quarries in the Bridgeport-Chico area are suitable sources for riprap and coarse concrete aggregate. Fine concrete aggregate may be obtained from any one of five natural sand and gravel pits in the Dallas area. Embankment material should be available near the dam site.

143. COST ESTIMATE. - A summary of the estimated first cost and annual charges of the Aubrey Reservoir project is shown in table 41. This summary includes the cost of recreation in Aubrey Reservoir, additional recreation in Garza-Little Elm Reservoir, and alteration of existing recreation facilities in Garza-Little Elm Reservoir, particularly as the exchange in storage effects the real estate acquisition and the existing recreation facilities.

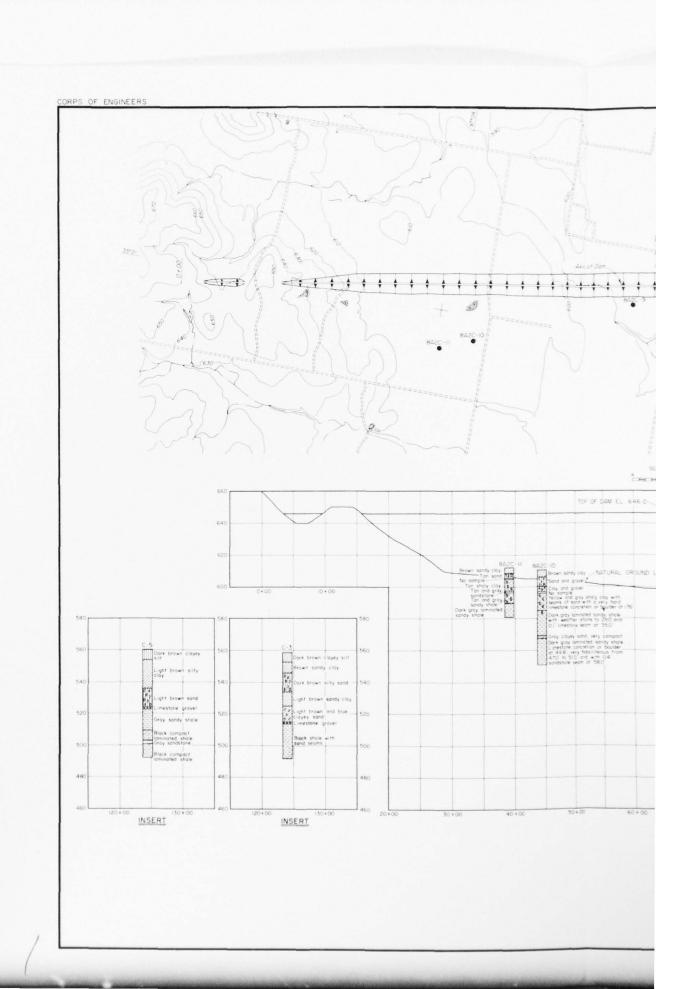
TABLE 41 SUMMARY OF FIRST COST AND ANNUAL CHARGES PROPOSED AUBREY RESERVOIR ELM FORK

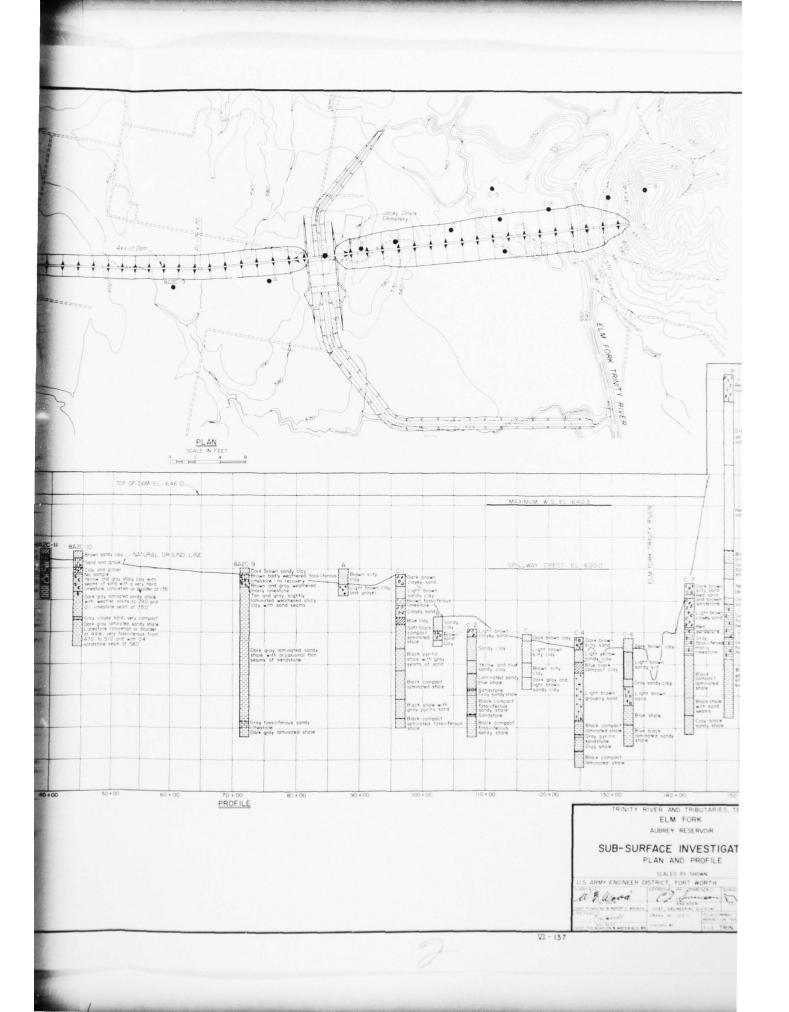
| | Item FIRST COST | Costs |
|----|---|----------------------|
| 1. | Federal first cost | |
| | Lands and damages | \$12,047,000 |
| | Relocations (1) | 2,351,000 |
| | Reservoir (clearing) | 1,647,000 |
| | Dem | |
| | a. Embankment | 4,850,000 |
| | b. Spillway and outlet facilities | 3,900,000 |
| | Access roads | 323,000 |
| | Recreation facilities | 6,250,000 |
| | Buildings, grounds and utilities | 163,000 |
| | Permanent operating equipment | 100,000 |
| | Engineering and design | 939,000 |
| | Supervision and administration | 1,503,000 |
| | Total estimated Federal first cost | 34,073,000 (1) |
| 2. | Non-Federal first cost | None |
| 3. | Total estimated first cost of project | 34,073,000 |
| ir | astruction period - 5 years) (100 yr. amortizat | ion) (2-7/8% |
| 1. | Federal investment | 21, 272, 222 |
| | a. Federal first cost | 34,073,000 |
| | b. Interest during construction | 2,449,000 |
| 0 | Total Federal investment | 36,522,000 None |
| 2. | Non-Federal investment | None |
| 3. | Federal annual charges | 1 050 000 |
| | a. Interest on investment | 1,050,000 |
| | b. Amortization charge | 65,400 |
| | c. Operation and maintenance (including | 500,000 |
| | replacement of parts) | 509,000 1,624,400 |
| 4. | Net Federal annual charges Net non-Federal annual charges | None |
| 5. | | 1,624,400 |
|)• | Total estimated annual charges Preauthorization cost = \$60,000 | 1,024,400 |
| | | |

Note: Prices are as of January 1962

(1) Includes \$101,000, alterations to existing recreation facilities in Garza-Little Elm Reservoir.

(2) Includes additional recreation in Garza-Little Elm Reservoir.





144. ROANOKE RESERVOIR .- The Roanoke dam site is located at rive mile 32.0 on Denton Creek, a tributary to the Elm Fork of the Trinity River. The dam site is located in Denton County, Texas, and the right abutment of the dam would be about one-half mile northwest of the town of Roanoke. The dam site crosses Denton Creek about 4,000 feet upstre along the axis of the Denton Creek Valley, from the Texas and Pacific Railroad. The dam site is near the upstream limits of the Corps of Engineers' Grapevine Reservoir. Roanoke Reservoir would have a total storage capacity of 249,900 acre-feet, of which 223,700 acre-feet are for flood control and 26,200 acre-feet are for sediment reserve. The flood-control storage provided in the Roanoke Reservoir would permit a reallocation of the storage in the Grapevine Reservoir and increase th storage presently allocated to water conservation purposes. The depen yield from the Grapevine Reservoir, with the reallocation made possibl by the flood-control storage in Roanoke Reservoir, under 2020 conditio of watershed development and a recurrence of the 1950-1957 drought per is 42 million gallons per day. This is an increase of 23.9 MGD over that which would be produced by Grapevine Reservoir. Requiring reloca tion or alteration would be State Highway 114, FM Highways 156 and 407 county roads, and the Gulf-Colorado and Santa Fe Railroad. The estima first cost of this project is \$16,900,000 as shown in table 43. Other pertinent data on the Roanoke Dam and Reservoir are contained in the following table 42. Plates 35, 36 and 37 show the reservoir area; pla profile and sections; and general hydraulic data for Roanoke Reservoir

TABLE 42 PERTINENT DATA PROPOSED ROANOKE RESERVOIR DENTON CREEK

| Item | Flood control |
|---------------------------------------|---------------|
| Miscellaneous | |
| Dam location, river mile | 32.0 |
| Drainage area, square miles | 604 |
| Flood control storage, acre-feet | 223,700 |
| Water conservation storage, acre-feet | None |
| Sediment storage | 26,200 |
| Yield, CFS | |
| Million gallons daily | • |
| Spillway design flood | |
| Peak inflow, CFS | 325,600 |
| Volume, acre-feet | 780,000 |
| Volume, inches | 24.21 |
| Peak outflow, CFS | 297,000 |
| | |

TABLE 42 (Cont'd) PERTINENT DATA PROPOSED ROANOKE RESERVOIR DENTON CREEK

| Item | | Flood control (1) | |
|---|-----------|-------------------|-----------|
| | Elevation | Area | Capaci |
| Reservoir | (Feet) | (Acres) | (Acre- |
| Spillway crest | 584.0 | 3,180 | 29,60 |
| Top of gates | 619.0 | 9,720 | 249,90 |
| Maximum design water surface | 625.7 | 11,420 | 320,60 |
| Top of dam | 631.0 | 12,570 | 384,20 |
| Dam | | | |
| Type of dam | | Earth fi | 11 |
| Total length, feet (including spillway) Embankment section: | | 15,200 | |
| Туре | | Compacte | d earth f |
| Total length, feet (minus spi | illway) | 14,600 | |
| Height above streambed, feet | | 97 | |
| Freeboard, feet | | 5.3 | |
| Crown width, feet | | 20 | |
| Side slopes: | | | |
| Upstream | | 1 on 3 | |
| Downstream | | 1 on 3 | |
| Non-overflow sections: | | | |
| Туре | | | e gravit; |
| Total length, feet | | 272 | |
| Height above apron, feet | | 111 | |
| Top width, feet | | 13 | |
| Spillway section: | | | |
| Type | | Concrete | ogee |
| Gross length, feet | | 328 | |
| Net length, feet | | 280 | |
| Crest height above apron, feet | | 64 | |
| Gates: | | | |
| Type | | Tainter | |
| Number | | 7 | |
| Size (width x height), feet | | 40 x 35 | |
| Discharge, CFS: | | | |
| Top of gates | | 217,000 | |
| Maximum design water surface | | 287,900 | |

t able

s od,

ed

TABLE 42 (Cont°d) PERTINENT DATA PROPOSED ROANOKE RESERVOIR DENTON CREEK

| Item | Flood control |
|--|-------------------------------|
| Outlet works | |
| Type | Gate-controlle |
| Number of sluices | 1 |
| Diameter, feet | 15 |
| Invert elevations, feet | 560.0 |
| Sluice control | 3 - 4.5' × 15 operated sl: |
| Relocations | |
| Highway, miles | 12.1 |
| County roads, miles | 1.0 |
| Railroads, miles | 4.5 |
| Power lines; miles | 15 |
| Telephone lines, miles | 10 |
| Pipelines, miles | 5 |
| Cemeteries, number | - |
| Towns, number | 0 |
| Lands | |
| Dam and reservoir: | |
| Clearing, acres (3) | 4800 |
| Land acquisition: | |
| Fee simple, acres (3) | 1210 |
| Flood easement, acres | 11,990 |
| (Top control elevation) | 624.0 |
| Conversion, easement to fee, acres | (4) 600.0 |
| Roanoke Reservoir contains flood-concontrol storage in Grapevine Reservoir conservation. | ntrol storage to enable |
| (2) Includes 9,100 CFS discharge through | n the outlet works. |
| (3) Includes requirements for additional | l general recreation at |

Reservoir.

(4) In Grapevine Reservoir.

(1)

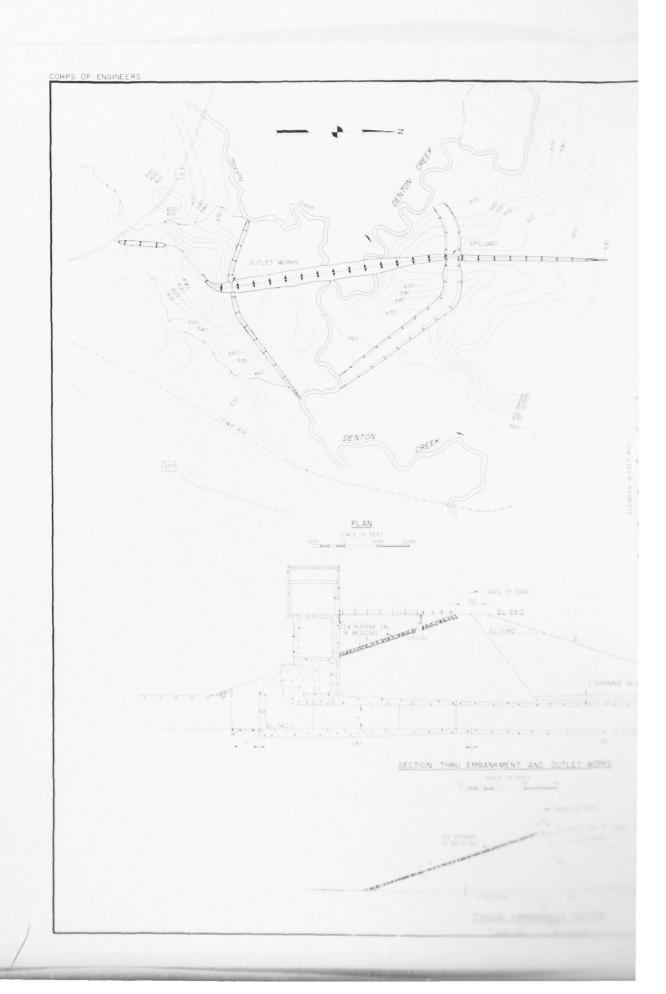
d conduit

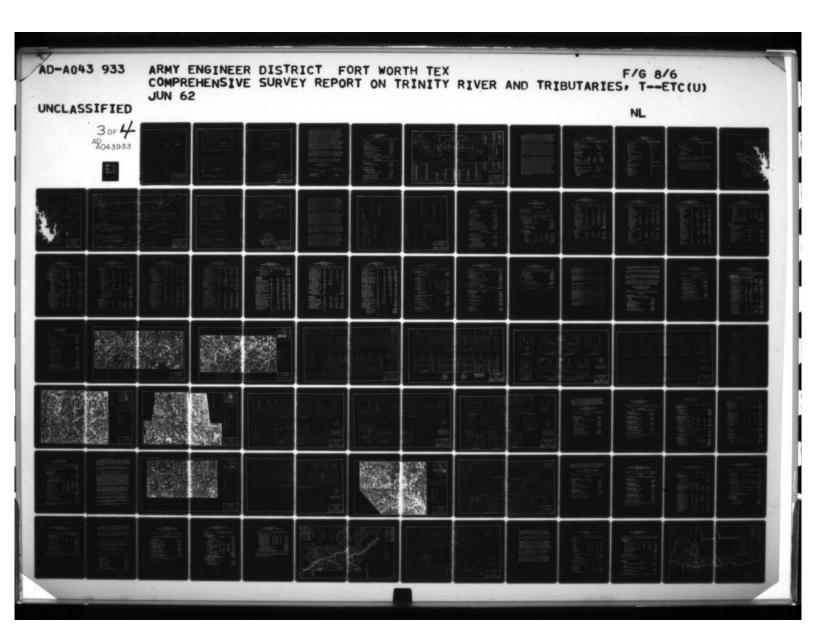
power de gates

flood water

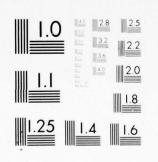
Grapevine





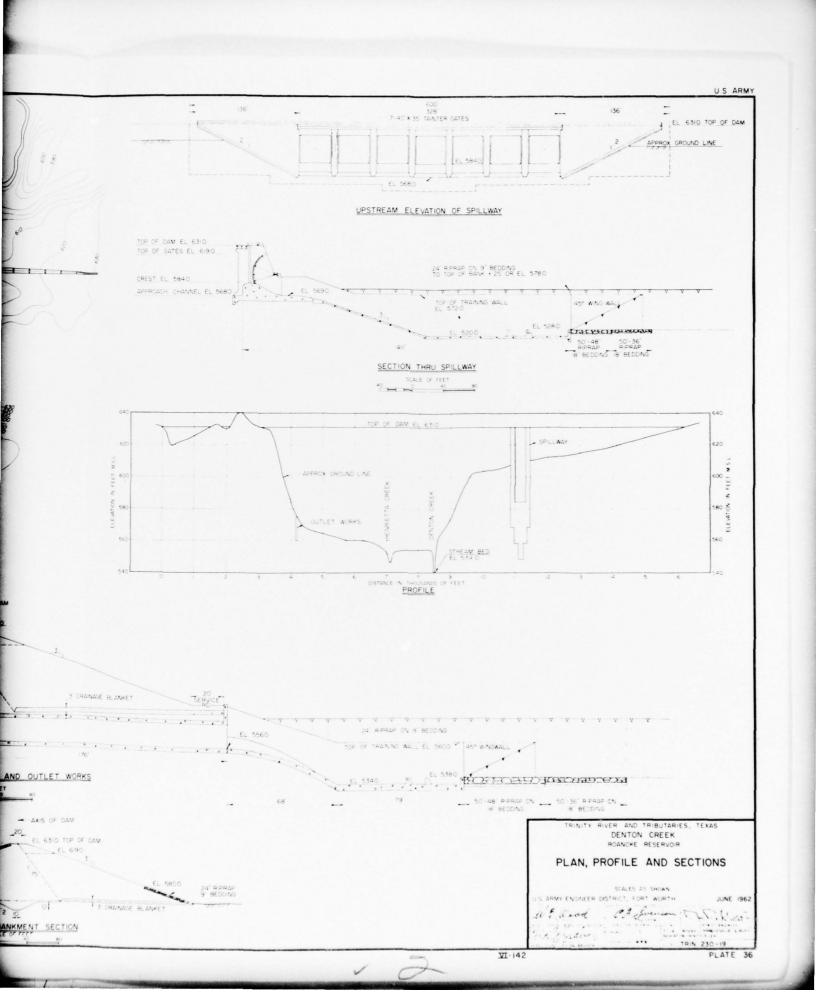


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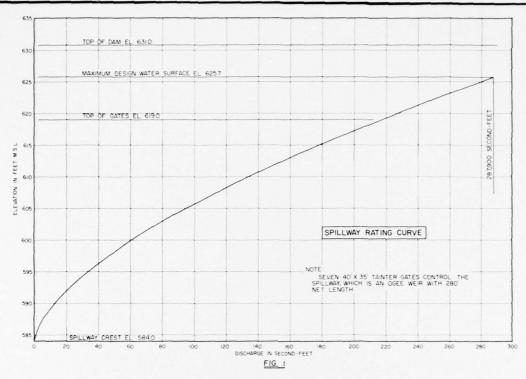


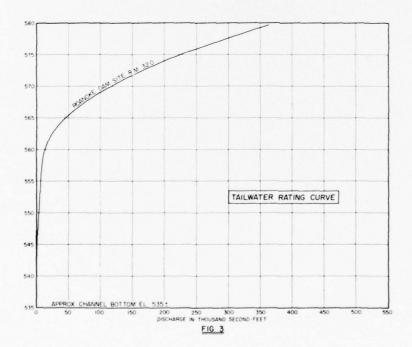
MICROCOPY RESOLUTION TEST CHART NATIONAL BUREAU OF STANDARDS 1963

NEMENT SECTIO





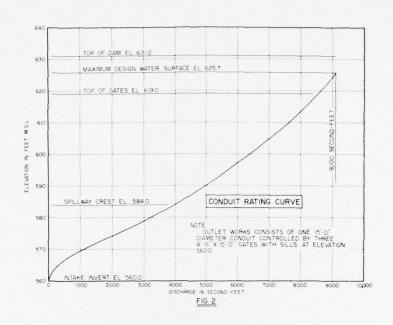


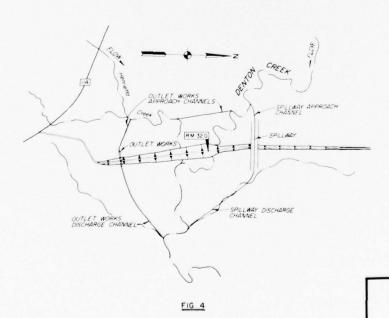




ELEVATION IN FEET M S.L.







TRINITY RIVER AND TRIBUTARIES, TEXAS
DENTON CREEK
ROANOKE RESERVOIR

GENERAL HYDRAULIC DATA

SCALES AS SHOWN

S ARMY ENGINEER DISTRICT, FORT WORTH

JUNE 1962

S ARMY ENGINEER DISTRICT, FORT WORTH

JUNE 1962

S ARMY ENGINEER DISTRICT, FORT WORTH

JUNE 1962

S ARMY ENGINEER DISTRICT

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VI-143

145. GEOLOGY .- Formations of the Washita series of Lower Cretaceous age underlie the reservoir area of the Roanoke Dam site. The specific formations, listed in ascending order (oldest to youngest) are: Duck Creek, Fort Worth, Denton, Weno, Paw Paw, Main Street, and Grayson. The Duck Creek and the Fort Worth formations consist of alternating beds of argillaceous limestone and marl. The Denton is predominantly marl with occasional bands of shelly limestone. The Weno is composed of a section of marly clay overlain by a zone characterized by marl and bands of soft limestone. The Paw Paw is a calcareous clay containing occasional ironstone concretions and thin, lenticular bands of sandstone. The Main Street is a bedded limestone, and the overlying Grayson formation is predominantly marly clay. The Main Street limestone provides the principal outcrop at the Roanoke Dam site. Erosional outliers of the overlying Grayson formation are found capping isolated knobs in the vicinity of the site, but primary strata of the Grayson have been removed by erosion on the abutments at the location of the proposed embankment. Consequently, foundation materials underlying the dam will consist of a thin section (approximately 10 to 15 feet) of Main Street limestone on the crest of each abutment underlain, in the following order, by the Paw Paw, Weno, Denton, Fort Worth, and Duck Creek formations. It is probable that the foundation for the spillway will extend into the Weno formation to about 40 foot depth. The Weno is about 60 feet thick. Plate 38 shows information on subsurface investigations.

146. AVAILABILITY OF CONSTRUCTION MATERIALS. - Riprap, coarse aggregate, drainage blanket material, and bedding material could be produced at any of four operating commercial quarries in the Bridgeport-Chico area. Drainage blanket material, bedding material, and fine aggregate could be produced at any one of several operating sources in the Fort Worth-Dallas area. It is expected that embankment material can be obtained near the dam site.

147. COST ESTIMATE. - A summary of the estimated first cost and annual charges for the Roanoke Reservoir project is shown in table 43. The summary includes the cost of additional recreation and alteration to existing recreation facilities in Grapevine Reservoir.

TABLE 43
SUMMARY OF FIRST COST AND ANNUAL CHARGES
PROPOSED ROANOKE RESERVOIR
DENTON CREEK

Item Cost

FIRST COST

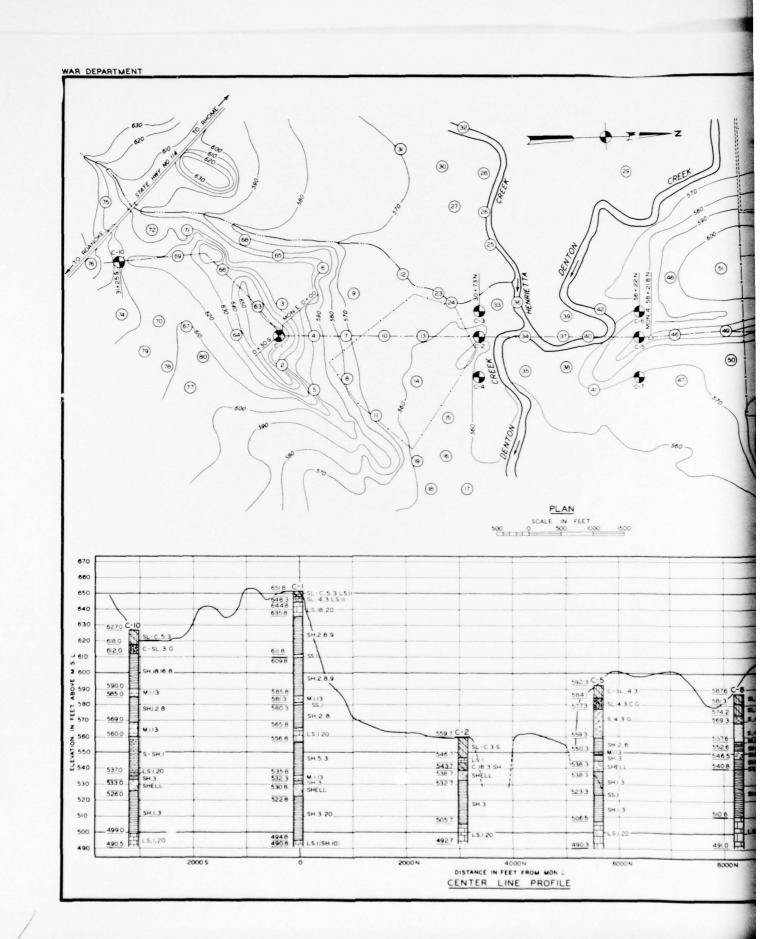
1. Federal first cost
Tands and damages \$ 3,496,000
Relocations (1) \$ 3,370,000
Reservoir (clearing) 450,000

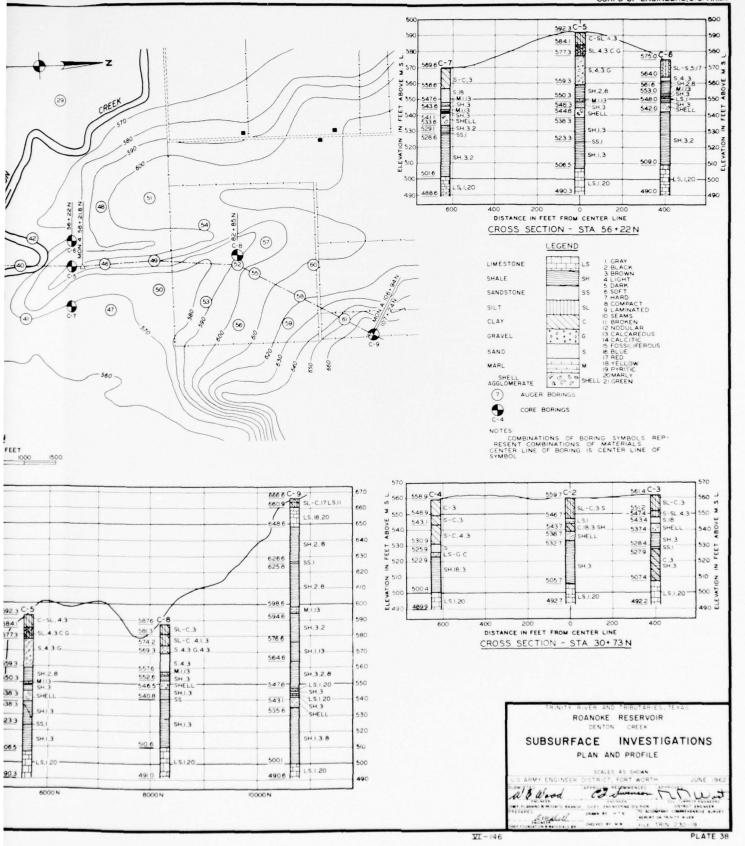
TABLE 43 (Cont'd) SUMMARY OF FIRST COST AND ANNUAL CHARGES PROPOSED ROANOKE RESERVOIR DENTON CREEK

| Item | : : Cost |
|---|----------------------------------|
| 1.0911 | . 000 |
| Dam | |
| a. Embankment | \$3,165,000 |
| b. Spillway | 3,250,000 |
| c. Outlet works | 1,300,000 |
| Access road | 13,000 |
| Recreation facilities | 375,000 |
| Buildings, grounds, and utilities | 163,000 |
| Permanent operating equipment | 80,000 |
| Engineering and design | 482,000 |
| Supervision and administration | 756,000 |
| Total estimated Federal first cost | 16,900,000 (2) |
| 2. Non-Federal first cost | None |
| 3. Total estimated first cost of project | \$16,900,000 |
| (1) Includes \$170,000 alterations to exis | ting recreation facilities |
| in Grapevine Reservoir. (2) Includes additional recreation in Gra | nevine Reservoir. |
| (2) Includes additional recreation in ora | pevine negervoir. |
| (Construction period - 4 years)(100 yr. amort | zization)(2-7/8% interest rate) |
| | |
| 1. Federal investment | |
| a. Federal first cost | 16,900,000 |
| b. Interest during construction | 971,800 |
| Total Federal investment | 17,871,800 |
| 2. Non-Federal investment | None |
| 3. Federal annual charges | |
| a. Interest on investment | 513,800 |
| b. Amortization charge | 32,000 |
| c. Operation and maintenance (including | |
| replacement of parts) | 78,000 |
| Net Federal annual charges | 623,800 |
| 4. Net Non-Federal annual charges | None |
| 5. Total estimated annual charges | \$ 623,800 |

Preauthorization cost = \$40,000

Prices are as of January 1962





2

- 148. TENNESSEE COLONY RESERVOTR PROJECT. The Tennessee Colony Dam site is located at river mile 339.2 on the Trinity River about 16 miles west of Palestine, Texas, and 7 miles southwest of Tennessee Colony. The proposed dam crosses the Trinity River with its east abutment in Anderson County and its west abutment in Freestone County. The proposed reservoir for navigation, flood control, water conservation, fish and wildlife, and recreation purposes will be located in Anderson, Freestone, Henderson and Navarro Counties. The total contributing drainage area above Tennessee Colony Dam is 12,687 square miles.
- 149. DAM. The structure consists of an earth fill dam with a maximum height of 114 feet above streambed and a total length of 29,500 feet including a 520-foot controlled concrete spillway. The concrete ogee spillway located in a natural saddle near the left abutment is controlled by 11 tainter gates, each 40 feet wide and 35 feet high, and has a net opening of 440 feet. Low flow releases are made through four 3-foot by 6-foot sluices located in the spillway piers and controlled by power operated slide gates. Navigation locks numbers 10A and 10B will be located near the right abutment of the dam. The locations and details of these locks are shown on plates 17, 21 and 22 on pages 34, 42 and 43, respectively. Other pertinent data for Tennessee Colony are shown in table 44, and the detailed layout and typical sections of the dam are shown on plate 40. The general hydraulic design data are shown on plate 41.
- 150. RESERVOIR. Tennessee Colony Reservoir will have a total controlled storage of 3,366,800 acre-feet and an area of 119,500 acres at elevation 285.0, top of spillway gates or top of flood control pool. At top of conservation pool, elevation 262.5, the reservoir will have an area of 73,540 acres and a storage capacity of 1,193,000 acre-feet, including a portion of the sediment reserve. Lands required for reservoir operation, construction of the proposed dam, and for recreation purposes will be about 168,151 acres in fee simple and 7,000 acres in flowage easements. Construction of Tennessee Colony Reservoir would require relocation of 21 miles of highway, 10 miles of county roads, 1.8 miles of railroads, 44 miles of pipelines, and 21 miles of electrical transmission and distribution lines. There are no known cemeteries in the reservoir area. Relocations would extend into the proposed Richland Creek and Tehuacana Reservoirs, anticipating that Tennessee Colony Reservoir would be built first. However, the runoff from the drainage areas above the Richland Creek and Tehuacana Reservoirs have not been included in estimating the yield of Tennessee Colony Reservoir. Other pertinent data on the proposed reservoir are listed in table 44 and the reservoir area is shown on plate 39.

TABLE 44

PERTINENT DATA PROPOSED TENNESSEE COLONY RESERVOIR TRINITY RIVER

| 9 | | ntrol, water conserva |
|---|----------------------|-----------------------|
| Item | tion and r | ecreation |
| Miscellaneous | | |
| Dam location, river mile | 339.2 | |
| Drainage area, square miles | 12,687 | |
| Flood control storage, acre-feet | 2,1 | 44,300 |
| Water conservation storage, acre-feet | | 32,500 |
| Sediment storage | 1 | 90,000 |
| Yield, CFS | | 450(1) |
| Million gallons daily | | 291 |
| Spillway design flood | | |
| Peak inflow, CFS | | 51,800 |
| Volume, acre-feet | | 33,400 |
| Volume, inches | 14.8 | |
| Peak outflow, CFS | 5 | 556,000(2) |
| Elevation | Area | Capacity |
| Reservoir (Feet) | (Acres) | (Acre-ft) |
| Spillway crest 250.0 | 42,190 | 470,200 |
| Top of conservation storage 262.5 | 73,540 | 1,193,000 |
| Top of gates 285.0 | 119,500 | 3,366,800 |
| Maximum design water surface 297.8 | 151,920 | 5,109,200 |
| Top of dam 305.0 Maximum tailwater at dam 250.9 | | |
| Maximum tailwater at dam 250.9 | | |
| Dam | 77 | 0:33 |
| Type of dam | Earth fill 29,500 | |
| Total length, feet (including spillway) | 29, | 500 |
| Embankment section: | Compo | cted earth fill |
| Type Total length, feet (minus spillway) | | 736 |
| Height above streambed, feet | 20, | 113 |
| Freeboard, feet | 7.2 | 11) |
| Crown width, feet | 1.2 | 20 |
| Side slopes: | | |
| Upstream | | |
| Downstream | 2-1/2:1, 3: | 1, 3-1/2:1 |
| A CHANGE OF COMM | 2-1/2:1, 3: | |
| | 2-1/2:1, 3: | 1 |

TABLE 44 (Cont'd) PERTINENT DATA PROPOSED TENNESSEE COLONY RESERVOIR TRINITY RIVER

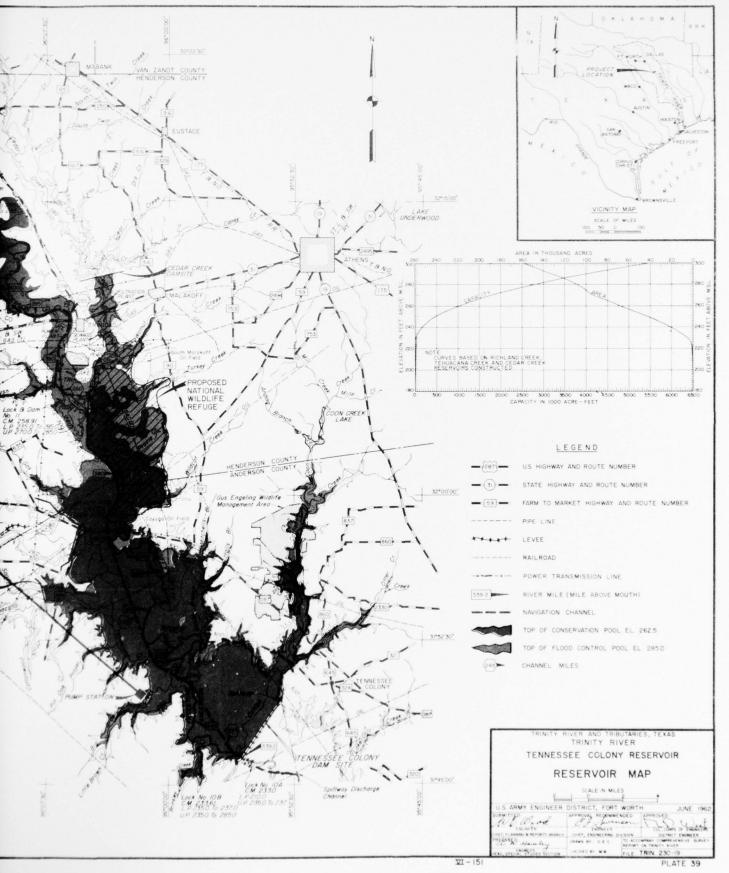
| | Navigation, flood control, water | |
|-----------------------------------|----------------------------------|--|
| Item | conservation, and recreation | |
| Non-overflow sections: | | |
| Type | Concrete gravity | |
| Total length, feet | 244 | |
| Height above apron, feet | 112 | |
| Top width, feet | 13 | |
| Spillway section: | 13 | |
| | Concrete ogee | |
| Type Gross length, feet | 520 | |
| | 440 | |
| Net length, feet | | |
| Crest height above apron, feet | 57 | |
| 34101 | m - 1 - 1 | |
| Туре | Tainter | |
| Number | 11 | |
| Size (width x height), feet | 40 x 35 | |
| Spillway discharge CFS: | | |
| Top of gates | 330,000 | |
| Maximum design water surface | 552,600 | |
| Outlet works | | |
| Туре | Gate-controlled sluices | |
| | through spillway piers | |
| Number of sluices | 4 | |
| Dimensions (width x height), feet | 3 x 6 | |
| Invert elevation, fee | 225.0 | |
| Sluice control | Power operated slide gate | |
| Relocations | | |
| Highways, miles | 21.0 | |
| County roads, miles | 10.0 | |
| Railroads, miles | 1.8 | |
| Power lines, miles | 21.0 | |
| Telephone lines, miles | | |
| Pipelines, miles | 44.0 | |
| | None | |
| Cemeteries, number | | |

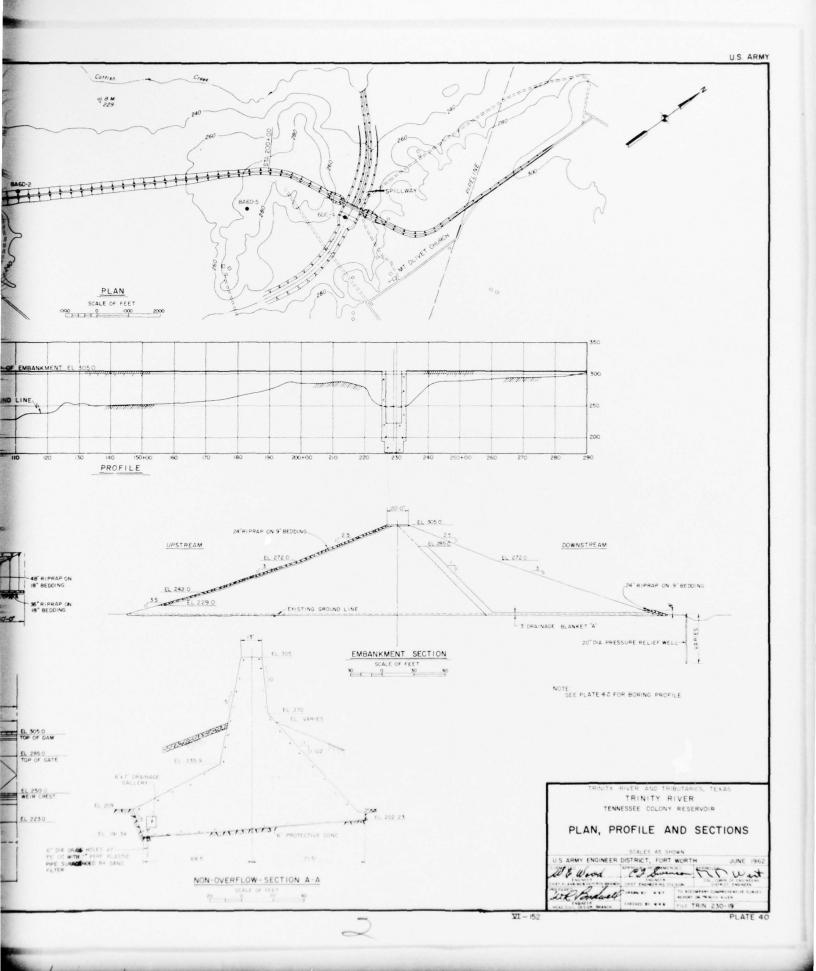
TABLE 44 (Cont'd)

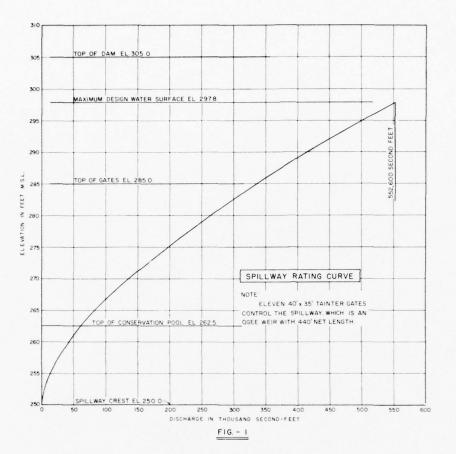
PERTINENT DATA PROPOSED TENNESSEE COLONY RESERVOIR TRINITY RIVER

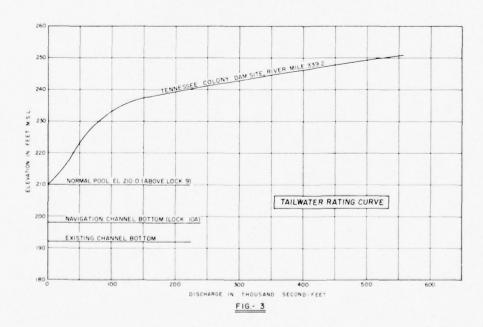
| | :Navigation, flood control, water | |
|-------------------------|-----------------------------------|--|
| Item | conservation, and recreation | |
| Lands | | |
| Dam and reservoir: | | |
| Clearing, acres | 6,209 | |
| Land acquisition: | | |
| Fee simple, acres | 166,244 | |
| (top control elevation) | 288.0 | |
| Flood easement, acres | 7,000 | |
| (top control elevation) | 288.0 | |
| Recreation: | | |
| Clearing, acres | 20,000 | |
| Land acquisition: | | |
| Fee simple, acres | 1,907 | |

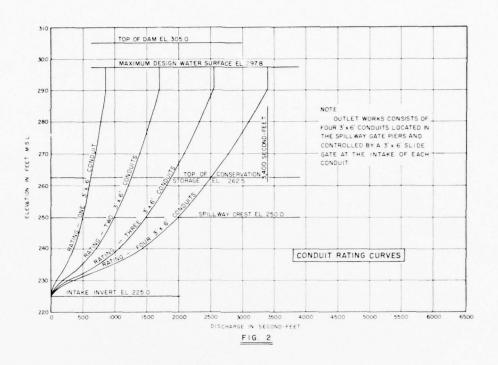
- (1) Of the 450 c.f.s. yield from the reservoir, 124 c.f.s. initially has been assigned for the purpose of water quality control and 326 c.f.s. has been assigned for municipal and industrial use.
- (2) Includes 3400 c.f.s. discharge through the outlet works.
- 151. GEOLOGY. The subsurface investigations at river mile 340.2 consisted of seven borings as shown on plate 40. Two were drilled in the proposed spillway area; three were located between the spillway and the left bank of the river; one on the right bank of the stream and one boring was drilled at a navigation lock site between the river and the right abutment. Two additional borings were made at the selected dam site river mile 339.2. One boring is located at the top of the right bank abutment, and one is located at or near the location of lock No. 108.

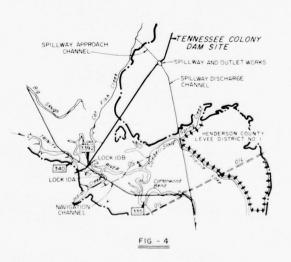












TRINITY RIVER AND TRIBUTARIES, TEXAS
TRINITY RIVER
TENNESSEE COLONY RESERVOIR

GENERAL HYDRAULIC DATA

US ARMY ENGINEER DISTRICT, FORT WORTH

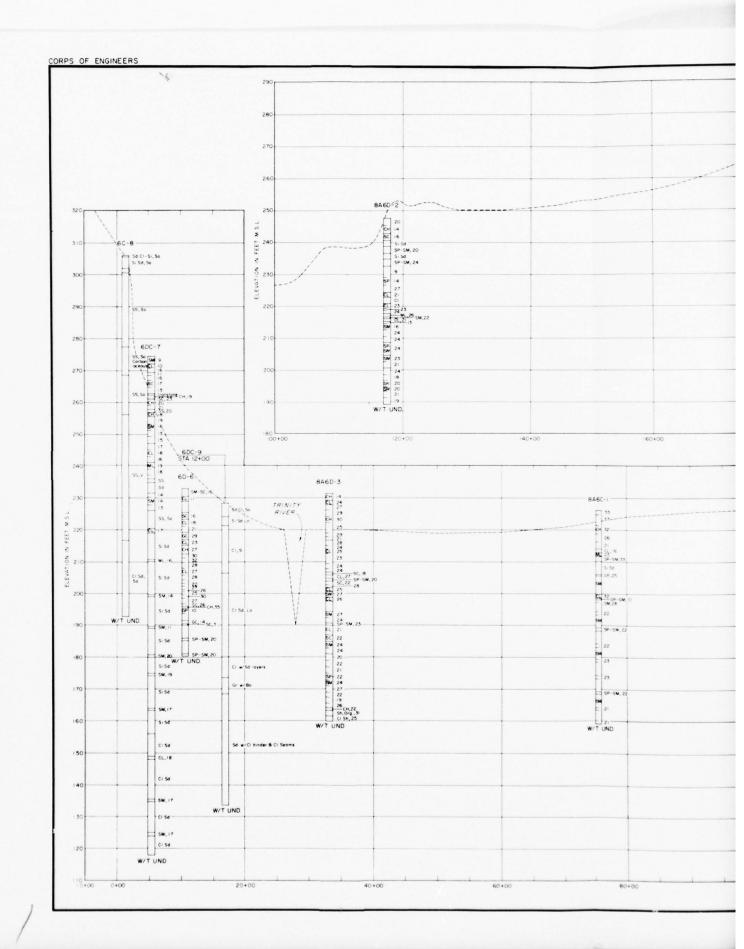
US ARMY ENGINEER DISTRICT, FORT WORTH

APPROVED TO COMMENTED AND STREET COMMENT

OUT A STREET STREET COMMENTS OF THE STRE



- 152. The alluvial overburden varies in thickness from about 24 feet to 28 feet in the wide flood plain area to the left of the river and between 55 and 56 feet in depth beneath the crest of the left abutment. The thickness of the overburden approximates 22-1/2 feet in the spillway saddle. At the boring location on the right bank of the river, the overburden extends to a depth of about 42-1/2 feet. The boring at the lock site, located on a promontory between the right bank of the river and the crest of the right abutment, penetrated only a few feet of residual overburden which is almost indistinguishable from the underlying primary strata. All of the overburden, with the exception of that at the lock site and on the crest of the right abutment, appears to be alluvium and consists of silty and sandy clays and sand with some gravel with the latter generally being found immediately overlying primary strata. No fault zones were located in the area.
- 153. All primary sediments in the area of the dam site were deposited during the Eocene Epoch of the Tertiary Period and are represented by the Reklaw formation to the left of the river and by the Reklaw conformably overlying the Carrizo sand formation in the right abutment. The Reklaw is predominantly clay and sandy clay with occasional thin lentils of ferruginous sandstone, whereas the underlying Carrizo is composed principally of silty or clayey sands and lesser amounts of clay. The foundation for that portion of the embankment on and along both abutments can be prepared with a minimum amount of stripping. Foundation design for appurtenant facilities will provide for placing such structures on uncemented materials. The distinct stratification of impervious and underlying pervious foundation materials will require pressure relief wells at the toe of the embankment and in the spillway area. Subsurface investigations are shown on plate 42.
- 154. AVAILABILITY OF CONSTRUCTION MATERIALS. Impervious material required for the embankment is available in the immediate vicinity of the dam site. Pervious materials, penetrated by the borings, can be used in the drainage blanket. Fine aggregate is not available locally and probably would be shipped by rail from Waco to Palestine, Texas, and trucked the remaining approximate 16 miles to the project area. Coarse aggregate and riprap are available at the Blue Mountain quarry, less than 20 miles from the dam site.
- 155. COST ESTIMATE. A summary of first costs and annual charges for the Tennessee Colony Reservoir is shown in table 45. The detailed cost estimate for the Tennessee Colony Reservoir project is shown in table 46. The detailed estimate shows the separation of costs for the dam and reservoir and for recreation included in the project. Included in the estimate is the cost of lock No. 10B and all other navigation costs up to Lock and Dam No. 12, which is at the upstream end of the reservoir. The estimate also includes a portion of land and relocation costs in the proposed Richland Creek and Tehuacana Reservoirs as well as provision for protecting these dams should they be constructed. Protection of the Cedar Creek Dam, presently under construction is also provided. Summary and detailed estimates of first cost and annual charges for Tennessee Colony Reservoir are shown in tables 45 and 46, respectively.



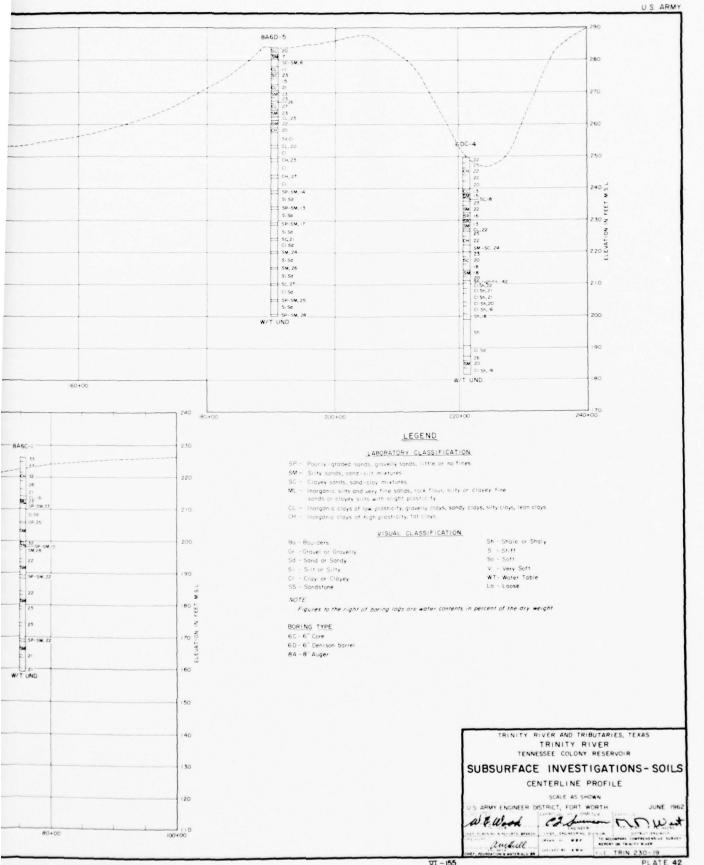


TABLE 45 SUMMARY OF FIRST COST AND ANNUAL CHARGES PROPOSED TENNESSEE COLONEY RESERVOIR TRINITY RIVER

| Item : | Costs |
|--|--------------------------|
| | |
| First cost | |
| . Federal first cost | |
| Lands and damages | \$ 22,505,000 |
| Relocations | 27,283,000 |
| Reservoir (clearing) | 2,457,000 |
| Dams | |
| a. Earth embankment | 10,349,000 |
| b. Concrete dam and spillway | 16,414,000 |
| c. Navigation dam No. 11 | 2,641,000 |
| Locks a. Lock No. 10B | 13,030,000 |
| b. Lock No. 11 | 11,902,000 |
| Access roads | 775,000 |
| Channel | 10,359,000 |
| Recreation facilities | 7,666,000 |
| Buildings, grounds, and utilities | 548,000 |
| Permanent operating equipment | 345,000 |
| Engineering and design | 4,113,000 |
| Supervision and administration | 6,457,000 |
| U. S. Coast Guard-aids to navigation | 294,000 |
| Total estimated Federal first cost (1) | 137,138,000 |
| . Non-Federal first cost | None |
| . Total estimated first cost of project | 137,138,000 |
| Annual charges | |
| (Construction period-8 years)(100 yr. amortizat: | ion)(2-7/8% interest rat |
| . Federal investment | |
| a. Federal first cost | 137,138,000 |
| b. Interest during construction | 15,770,900 |
| Total-Federal investment | 152,908,900 |
| . Non-Federal investment | None |
| . Federal annual charges | |
| a. Interest on investment | 4,396,100 |
| b. Amortization charge | 273,700 |
| c. Operation and maintenance (including | |
| replacement of parts) | 1,367,400 |
| Total Federal annual charges | 6,037,200 |
| . Total Non-Federal annual charges | None |
| . Total estimated annual charges | 6,037,200 |
| Preauthorization cost \$183,000 | |
| Prices are as of January 1962 | |
| | |

VI-156

TABLE 46

DETAILED ESTIMATE OF FIRST COST TENNESSEE COLONY RESERVOIR MULTIPLE PURPOSE TRINITY RIVER

PERTINENT INFORMATION

| Top of dam, elevation | 305.0 |
|---|-----------|
| Top of gates, elevation | 285.0 |
| Spillway crest, elevation | 250.0 |
| Storage capacity (top of gates less sediment) acre-feet | 3,176,800 |

| | | Unit | | |
|-------------------------------|--------|-------------------------------|-------------------|------------------------------|
| Item | Unit | cost | Quantity | Cost |
| A. DETAILED ESTIMATE OF FIRST | COST - | DAM AND RESI | ERVOTR | |
| (01.0) Lands and damages | | | | |
| (1) Fee simple lands | Acre | | 166,244 | \$13,428,600 |
| (2) Flood easement lands | Acre | | 7,000 | 175,000 |
| (3) Severance damage | L.S. | | | 250,000 |
| (4) Improvements | L.S. | | | 150,000 |
| (5) Mineral subordination | L.S. | | | 2,493,000 |
| (6) Resettlement reimburse | - | | | 28,000 |
| ment | L.S. | | | |
| Subtotal | | | | 16,524,600 |
| Contingencies, 25%+ | | | | 4,131,400 |
| (7) Land acquisition expens | se | | | 1,398,000 |
| Total - lands and dar | mages | | | \$22,054,000 |
| (02.0) Relocations | | | | |
| a. Roads and railroad | | | | |
| (1) U. S. Highway No. 287 | | | | |
| Reservoir arms | | | | |
| Embankment | C.Y. | \$ 0.75 | 1,200,000 | 900,000 |
| Base | C.Y. | 4.50 | 32,000 | 144,000 |
| Surfacing | Mile | 10,000.00 | 3.6 | 36,000 |
| Riprap | C.Y. | 6.00 | 104,000 | 624,000 |
| Bedding | C.Y. | 4.50 | 39,000 | 175,500 |
| | L.F. | 2.50 | 36,000 | 90,000 |
| Guard rali | | E 0 10 | 3000 | ,0,000 |
| Guard rail Bridge | | 250.00 | 400 | 100,000 |
| Bridge | L.F. | 250.00 378.00 | 400 | |
| | | 250.00 378.00 80,000.00 | 400 119 4.2 | 100,000 44,982 336,000 |

| | | Unit | | |
|--------------------------|-------------|-----------|------------|-----------|
| Item | Unit | cost | Quantity | Cost |
| Main stem | | | | |
| Embankment | C.Y. | 0.85 | 213,000 \$ | 181,050 |
| Approach road | | | , | |
| flex pavement | L.F. | 12.10 | 4,000 | 48,400 |
| Riprap, 12" conc. | | | | |
| blocks | C.Y. | 30.00 | 3,310 | 99,300 |
| Gravel blanket | | | | |
| under riprap | C.Y. | 5.50 | 1,650 | 9,080 |
| Structural excavati | ion C.Y. | 2.50 | 8,240 | 20,600 |
| Class "A" concrete | C.Y. | 41.20 | 24,566 | 1,012,120 |
| Reinforcing steel | Lbs. | 0.13 | 4,050,640 | 526,580 |
| Conc. piling 14" so | q. L.F. | 7.50 | 115,340 | 865,05 |
| Cement | Bbl. | 5.00 | 36,650 | 184,25 |
| Structural steel | Lbs. | 0.14 | 38,430 | 5,38 |
| Structural steel | Lbs. | 0.22 | 1,747,200 | 384,38 |
| Type 'C" conc. beam | ns L.F | 13.50 | 49,700 | 670,95 |
| Aluminum railing | L.F. | 5.00 | 26,400 | 132,00 |
| Treated timbers | FBM | 0.60 | 77,800 | 46,68 |
| Treated piling | L.F. | 3.25 | 9,750 | 31,69 |
| Navigation lights | Ea. | 2,500.00 | 1 | 2,50 |
| Detour road | L.F. | 1.25 | 4,000 | 5,00 |
| Remove existing bri | | - | | 12,99 |
| Subtotal - main | | | \$ | 4,238,00 |
| Subtotal - U.S. | Highway No. | 287 | | 6,688,48 |
| (2) State Highway No. 31 | | | | |
| Reservoir arms | | | | |
| Embankment | C.Y. | 0.75 | 112,000 | 84,00 |
| Base | C.Y. | 4.50 | 11,000 | 49,50 |
| Surfacing | Mile | 10,000.00 | 1.3 | 13,00 |
| Riprap | C.Y. | 6.00 | 25,000 | 150,00 |
| Bedding | C.Y. | 4.50 | 8,000 | 36,00 |
| Guard rail | L.F. | 2.50 | 11,000 | 27,50 |
| Bridge | L.F. | 250.00 | 2,140 | 535,00 |
| Subtotal - reservoi | r orme | | | 895,00 |

TABLE 46 (Cont'd)

DETAILED ESTIMATE OF FIRST COST
TENNESSEE COLONY RESERVOIR MULTIPLE PURPOSE
TRINITY RIVER

| | | Unit | | |
|------------------------|----------|-----------|-----------|---|
| Item | Unit | cost | Quantity | Cost |
| Main stem | | | | |
| Embankment | C.Y. | 0.85 | 15,700 \$ | 13,340 |
| Approach road | | | -/// 1 | -3,3 |
| flex base | L.F. | 12.10 | 400 | 4,840 |
| Structural excavation | C.Y. | 2.50 | 1,760 | 4,400 |
| Class "A" concrete | C.Y. | 41.20 | 5,220 | 215,06 |
| Reinforcing steel | Lb. | 0.13 | 809,544 | 105,24 |
| Piling conc. 14" sq. | L.F. | 7.50 | 21,130 | 158,47 |
| Concrete riprap | | | | |
| (Class B) | C.Y. | 30.00 | 166 | 4,98 |
| Cement | Bbl. | 5.00 | 7,836 | 39,18 |
| Structural steel | Lbs. | 0.14 | 11,010 | 1,54 |
| Structural steel | Lbs. | 0.22 | 1,747,200 | 384,38 |
| Aluminum railing | L.F. | 5.00 | 5,260 | 26,30 |
| Type "C" conc. beams | L.F. | 13.50 | 7,400 | 99,90 |
| Treated timber | FBM | 0.60 | 46,700 | 28,02 |
| Treated piling | L.F. | 3.25 | 6,500 | 21,12 |
| Navigation light | Each | 2,500.00 | 1 | 2,50 |
| Detour road | L.F. | 1.25 | 4,400 | 4,50 |
| Timber trestle, | | | | |
| detour road | L.F. | 150.00 | 300 | 45,00 |
| Remove existing | | | | |
| bridge | L.S. | | | 21,23 |
| Subtotal - main ste | m | | \$ | 1,180,00 |
| Subtotal - State Hi | ghway No | . 31 | | 2,075,00 |
| (3) F. M. Highway 488 | | | | |
| Embankment | C.Y. | 0.75 | 970,000 | 727,50 |
| Base | C.Y. | 4.50 | 11,000 | 49,50 |
| Surfacing | Mile | 7,040.00 | 2.0 | 14,08 |
| Bridge | L.F. | 200.00 | 3,370 | 674,00 |
| Riprap | C.Y. | 6.00 | 80,000 | 480,00 |
| Bedding | C.Y. | 4.50 | 30,000 | 135,00 |
| Guard rail | L.F. | 2.50 | 19,000 | 47,50 |
| Subtotal - F. M. Highw | | | | 2,127,58 |
| (4) F.M. Highway 321 | | | | |
| New road, complete | Mile | 50,000.00 | 4,4 | 220,00 |
| New road, comprete | MILLE | 0,000.00 | 7.7 | 220,00 |
| (5) County roads | Mile | 12,500.00 | 10.0 | 125,00 |
| | | , , | | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, |

TABLE 46 (Cont'd)

DETAILED ESTIMATE OF FIRST COST
TENNESSEE COLONY RESERVOIR MULTIPLE PURPOSE
TRINITY RIVER

| | | Unit | | |
|--|------------|--|-----------|---------------|
| Item | Unit | cost | Quantity | Cost |
| (6) St. Javis & Southwestern | D D | | | |
| (6) St. Louis & Southwestern Reservoir arms | n.n. | | | |
| Embankment | C.Y. | 0.75 | 115,000 | 86,25 |
| Ballast | C.Y. | 3.50 | 2,300 | 8,05 |
| Cross ties | Each | 4,00 | 2,000 | 8,00 |
| Track laying and | Lach | 4,00 | 2,000 | 0,00 |
| | Mile | 7,920.00 | 0.62 | 4,91 |
| surfacing Track hardware | Mile | 2,640.00 | 0.62 | 1,63 |
| | L.F. | · Committee of the comm | 700 | 350.00 |
| Bridge | L.F. | 500.00 | | 487,50 |
| Trestle | C.Y. | 125.00 6.00 | 3,900 | |
| Riprap | | | 6,000 | 36,00 9,00 |
| Bedding | C.Y. | 4.50 | 2,000 | 9,00 |
| Communication line, | Man. | 70 500 00 | 1 5 | 15 75 |
| relocations | Mile | 10,500.00 | 1.5 | 15,75 |
| Subtotal - reservoir a | rms | | | 1,007,09 |
| Main stem | 0.12 | 0.50 | 700 | 7 77 |
| Structural excavation | C.Y. | 2.50 | 700 | 1,7 |
| Concrete | C.Y. | 42.50 | 1,900 | 80,7 |
| Reinforcing steel | Lb. | 0.13 | 94,850 | 12,3 |
| Cement | Bbl. | 5.00 | 2,370 | 11,8 |
| Treated piles | T T3 | 2 05 | 05 050 | 07 1 |
| (Class B) | L.F. | 3.25 | 25,050 | 81,4: |
| Structural steel in | Th | 0.20 | 2,202,620 | 660 7 |
| spans | Lb. | 0.30 | 2,202,020 | 660,7 |
| Structural steel in tower & cwt. | Lb. | 0.35 | 1,098,200 | 384,3 |
| | | 0.14 | 8,600 | 1,2 |
| Steel tangent plate Rails | Lb. Lb. | 0.14 | 24,430 | 3,4 |
| | Lb. | 1.00 | 396,900 | |
| Ropes & machinery Electrical equipment | | 1.00 | 390,900 | 396,9 75,0 |
| Signal system | L.S. | | - | |
| | L.S. | | - | 13,0 |
| Auxiliary power | | • | | 7,5 |
| Housing (oper & mach) | L.S. | | - | 12,0 |
| Shoes, castings & | T C | | | 0), 0 |
| sheaves | L.S. | - 00 | 206 | 24,0 |
| Ballast | C.Y. | 5.00 | 306 | 1,5 |
| Ties | FBM | 0.15 | 81,000 | 12,1 |
| Bolts & spikes | Lb, | 0.20 | 5,600 | 1,1 |
| Treated timbers | FBM | 0.60 | 48,700 | 29,22 |
| Removal of existing bridge | T 0 | | | 6,30 |
| nri age | L.S. | - | - | 0.30 |

| 0.20 5.00 0.40 3.25 0.30 0.15 0.20 0.15 | 97,400 13,800 8,600 110,000 11,500 | 1,040 3,270 38,960 44,850 |
|--|--|--|
| 5.00 0.40 3.25 0.30 0.15 0.20 | 655 97,400 13,800 8,600 110,000 | 3,270 38,960 |
| 5.00 0.40 3.25 0.30 0.15 0.20 | 655 97,400 13,800 8,600 110,000 | 3,270 38,960 |
| 5.00 0.40 3.25 0.30 0.15 0.20 | 655 97,400 13,800 8,600 110,000 | 3,270 38,960 |
| 0.40 3.25 0.30 0.15 0.20 | 97,400 13,800 8,600 110,000 | 38,960 |
| 3.25 0.30 0.15 0.20 | 13,800 8,600 110,000 | |
| 0.30 0.15 0.20 | 8,600 110,000 | 44.850 |
| 0.30 0.15 0.20 | 8,600 110,000 | 44.850 |
| 0.15 | 110,000 | |
| 0.20 | | 2,580 |
| | 11.500 | 16,500 |
| 0.15 | | 2,300 |
| | 26,100 | 3,910 |
| | | 113,410 |
| | \$ | 1,930,000 |
| n & arms | | 2,937,09 |
| 3 | | 14,173,15 |
| 420,000.00 | 1.5 | 630,000 |
| | | 2,033,000 |
| 230,000.00 | 1.3 | 299,000 |
| 70 000 00 | 0.00 | 2,332,000 |
| 70,000.00 | 0.90 | 68,600 |
| 260,000.00 | 1.75 | 455,000 |
| 210,000.00 | 0.34 | 71,400 |
| | | 526,400 |
| | | |
| | | |
| 12,000.00 | 1.5 | 18,000 |
| 12,000.00 | 1.5 | 18,000 3,000 21,000 |
| | 190,000.00 230,000.00 70,000.00 | 190,000.00 10.7 230,000.00 10.7 70,000.00 0.98 |

| | | Unit | | |
|---|---------------|---|------------|-----------------|
| Item | Unit | cost | Quantity | Cost |
| /() I | | | | |
| (6) Lone Star Gas Co. 20" line | е | | | |
| S.E. of Cayuga Field | 11: 7 - | 120 000 60 | 2.0 | 200 00 |
| New line outside reservoi: | | 130,000.00 | 3.0 | 390,00 |
| Reservoir crossings | Mile | 160,000.00 | 1.5 | 240,00 |
| Subtotal | M: 1 o | 40,000.00 | 1 7 | 630,00 68,00 |
| (7) Texas Co. 8" oil line (8) Texas Co. 16" oil line | Mile Mile | 124,000.00 | 1.7 | 558,00 |
| (9) West Texas Gulf 20" oil | MITE | 124,000.00 | 4.) | 2,00,00 |
| line | Mile | 190,000.00 | 9.3 | 1,767,00 |
| 10) Magnolia 20" oil line | Mile | 50,000.00 | 0.5 | 25,00 |
| 11) Magnolia 12" 0.1 line | Mile | 75,000.00 | 5.1 | 382,50 |
| 12) Pipelines crossing | TILLO | 17,000.00 | 7.0 ± | 502,70 |
| spillway outlet channel | | | | |
| 1-10" & 2-8" oil lines | | | | |
| Excavation | C.Y. | 0.50 | 36,000 | 18,00 |
| 10" pipe) Coated, wrapp | | 12,00 | 320 | 3,84 |
| 8" pipe & weighted | L.F. | 7.00 | 640 | 4,48 |
| 10" valves | Each | 750.00 | 2 | 1,50 |
| 8" valves | Each | 600.00 | L | 2,40 |
| 2-12" oil lines | | | | |
| Excavation | C.Y. | 0.50 | 34,000 | 17,00 |
| 12" pipe, coated, | | | | |
| wrapped & weighted | L.F. | 28.00 | 320 | 8,96 |
| 12" valves | Each | 900.00 | 2 | 1,80 |
| Subtotal - pipelines | crossing | spillway char | mel | 57,98 |
| Subtotal - pipelines | | | | 7,066,48 |
| (Electric lines) | 0 1117 | 0 200 00 | , | 0 20 |
| (1) T.P.&L. 69 KV line @ RM 39 | 38 Mile | 8,300.00 | 1 | 8,30 |
| (2) T.P.&L. 69 KV line E.S.E. | Mile | 8,300.00 | 1.1 | 0.15 |
| from Trinidad plant (3) T.P.&L. 69 KV & 138 KV | MITTE | 0,300.00 | 1 | 9,13 |
| lines @ RM 389.9 | Mile | 46,500.00 | 2.0 | 93,00 |
| (4) T.P.&L. 138 KV line @ | MILLO | +0,00000 | 2.0 |)), |
| Rm 390.2 | Mile | 48,750.00 | 1.4 | 68,25 |
| (5) T.P.&L. 138 KV line @ | ritte | 10,170.00 | 2 | 00,00, |
| RM 389.6 | Mile | 48,750.00 | 1.7 | 82,8 |
| (6) Chambers & Tehuacana Cr. | | ,1,,,,,, | | ,- |
| crossing | Mile | 48,750.00 | 1.3 | 63,3 |
| Relocate H-frame line | Mile | 20,000.00 | 6.9 | 138,00 |
| New H-frame line | Mile | 15,000.00 | 2.5 | 37,50 |
| T.P.&L. 7.2 KV line @ | | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | | 0.,, |
| | Milo | 2,500.00 | 3.1 | 7,75 |
| Hwy. 287 | MITE | 2,000,000 | J 0 1 | 191 |
| Hwy. 287 Subtotal - electric | Mile lines | 2,000.00 | . . | 508,18 |

| | | Unit | | _ |
|--|-------------|--------------|------------|-------------|
| Item | Unit | cost | Quantity | Cost |
| The state of the s | | | | |
| c. Other items | | | | |
| (1) Project dam - T.P. & L | | | | |
| plant @ Trinidad | 0.35 | (00 | 900 | 1. 000 |
| Riprap | C.Y. | 6.00 4.5 | 800 | 4,800 |
| Bedding | C.Y. | 85.00 | 300 | 1,350 |
| Slope preparation | Sta. | 05.00 | 0 | 6,830 |
| Subtotal - dam prot (2) Trinidad sewage plant | eccion | | | 0,030 |
| protection | | | | |
| Levee embankment | C.Y. | 0.50 | 16,000 | 8,000 |
| Riprap | C.Y. | 6.00 | 2,300 | 13,800 |
| Bedding | C.Y. | 4.50 | 800 | 3,600 |
| Pumping facilities | L.S. | 4.00 | 000 | 10,000 |
| Subtotal - sewage p | | tion | | 35,400 |
| (3) Trinidad water filtration | | 01011 | | 37, .00 |
| plant protection | | | | |
| Levee embankment | C.Y. | 0.50 | 33,000 | 16,500 |
| 24" CMP | L.F. | 6.50 | 76 | 494 |
| Flap gate, 24" | Each | 150.00 | 1 | 150 |
| Metal headwall, 24" | Each | 38.00 | 1 | 38 |
| Riprap | C.Y. | 6.00 | 2,500 | 15,000 |
| Bedding | C.Y. | 4.50 | 800 | 3,600 |
| Subtotal water filt | ration plan | t protection | | 35,782 |
| Subtotal - other it | | | | 78,012 |
| | | | | 23 005 023 |
| Subtotal - relocati | ons | | | 21,825,831 |
| Contingencies 25% | | | | 5,457,169 |
| Total - relocations | | | | 27,283,000 |
| (03.0) Reservoir - clearing | Acre | 75.00 | 6,209 | 465,675 |
| Contingencies, 25% | | | , | 116,325 |
| Total - reservoir | - | | | 582,000 |
| | | | | , , , , , , |
| (04.0) Dams | | | | |
| a. Earth embankment | | | | |
| (1) Diversion and care of wa | ter L.S. | | | 30,000 |
| (2) Clearing and grubbing | Acre | 250.00 | 215 | 53,750 |
| (3) Excavation, stripping | C.Y. | 0.25 | 161,000 | 40,250 |
| (4) Excavation, borrow | С.Ү. | 0.26 | 3,080,000 | 800,800 |
| (5) Compacted fill | С.Ү. | 0.07 | 10,091,000 | 706,370 |
| (6) Overhaul | C .Y. | 0.05 | 6,000,000 | 300,000 |
| (7) Drainage blanket | C.Y. | 3.50 | 580,700 | 2,032,450 |
| (8) Riprap | C.Y. | 6.00 | 316,350 | 1,898,100 |
| | | | | |

| Item | Unit | Unit | Quantity | Cost |
|--------------------------------|--------|--------|-------------|----------------------|
| A. UVIII | 3111 | | 3,434,64,53 | |
| (9) Bedding | C.Y. | 3.50 | 118,630 | 415,205 |
| (10) Flexible base | C.Y. | 4.50 | 9,800 | 44,100 |
| (11) Aggregate | C.Y. | 6.00 | 560 | 3,360 |
| (12) Asphalt treatment | Gal. | 0.20 | 30,000 | 6,000 |
| (13) Timer guard posts | Each | 5.00 | 2,090 | 10,450 |
| (14) Slope protection, turfing | Acre | 500.00 | 100 | 50,000 |
| (15) Relief well and paved | | | | |
| ditch | L.S. | | | 530,000 |
| (16) Downstream riprap on | | | | |
| Tehuacana, Richland Cr., | | | | |
| & Cedar Cr. dams | C.Y. | 6.00 | 185,710 | 1,114,260 |
| (17) Downstream bedding on | | | | |
| Tehuacana, Richland Cr. | | | | |
| & Cedar Cr. dams | C.Y. | 3.50 | 69,640 | 243,740 8,278,835 |
| Subtotal - earth embar | nkment | | | 8,278,835 |
| . Concrete dam and spillway | | | | |
| (1) Care of water during | | | | |
| construction | L.S. | | | 20,000 |
| (2) Clearing | Acre | 175.00 | 8 | 1,400 |
| (3) Excavation, common | C.Y. | 0.25 | 606,000 | 151,500 |
| (4) Excavation, shale | C.Y. | 0.60 | 148,400 | 89,040 |
| (5) Structural backfill | C.Y. | 1.00 | 95,000 | 95,000 |
| (6) Drilling and grouting | | | | |
| anchor holes | L.F. | 2.25 | 10,250 | 23,063 |
| (7) Drilling drain holes | L.F. | 2.00 | 11,270 | 22,540 |
| (8) Line drilling | S.F. | 1.50 | 35,000 | 52,500 |
| (9) Asphalt shale treatment | S.Y· | 0.75 | 4,550 | 3,413 |
| (10) Concrete, weir | C.Y. | 22.00 | 163,820 | 3,604,040 |
| (11) Concrete, non-overflow | C.Y. | 25.00 | 63,340 | 1,583,500 |
| (12) Concrete, pier | C.Y. | 27.00 | 14,890 | 402,030 |
| (13) Concrete, slab | C.Y. | 20.00 | 17,010 | 340,200 |
| (14) Concrete, wall | C.Y. | 35.00 | 9,420 | 329,700 |
| (15) Concrete, bridge deck | C.Y. | 60.00 | 160 | 9,600 |
| (16) Cement | Bbl. | 5.00 | 335,800 | 1,679,000 |
| (17) Steel, reinforcing | Lb. | 0.13 | 8,510,000 | 1,106,300 |
| (18) Steel, structural | Lb. | 0.30 | 162,000 | 48,600 |
| (19) Pipe railing, aluminum | Lb. | 1.50 | 8,400 | 12,600 |
| (20) Pipe railing bridge, | | | | |
| aluminum | Lb. | 1.50 | 10,800 | 16,200 |
| (21) Metals, miscellaneous | Lb. | 0.40 | 18,300 | 7,320 |
| (22) Ladders, gratings, and | | | | |
| | | | 14,700 | |

| THE RESIDENCE OF THE CONTROL OF THE | | Unit | | |
|---|-------|--------------|------------|-----------|
| I tem | Unit | cost | Quantity | Cost |
| (23) Walkways | Lb, | 0.50 | 9,600 | 4,800 |
| (24) Waterstop, copper | Lb | 1.75 | 2,330 | 4,078 |
| (25) Water gages, tile | L.F. | 14.00 | 100 | 1,400 |
| (26) Tainter gates | Lb. | 0.26 | 1,096,000 | 284,960 |
| (27) Tainter gate hoists, | 2.0. | 0 120 | 1,0,0,000 | 20.,,00 |
| shafts, & hangers | Lb. | 1.00 | 253,000 | 253,000 |
| (28) Trunnion anchorages and | 13.00 | 1.00 | 2)),000 | 2)3,000 |
| seals | Lb | 0.26 | 326,000 | 84,760 |
| (29) Sluice gates & operating | LD | 0.20 | 520,000 | 0.,,00 |
| equipment | Each | 5,700.00 | 8 | 45,600 |
| (30) Trash racks and guides | Lb. | 0.30 | 15,400 | 4,620 |
| (31) Emergency bulkheads | Lb. | 0.30 | 42,600 | 12,780 |
| (32) Precast bridge girders | Each | 528.00 | | 17,424 |
| (33) Crane | | 85,000.00 | 33 | 85,000 |
| | Each | 05,000.00 | 1 | 15,000 |
| (34) Electrical facilities | L.S. | 7,000.00 | 1 | |
| (35) Standby power unit | Each | | 8 530 | 7,000 |
| (36) Riprap | C.Y. | 6.00 4.50 | 8,530 | 16,56 |
| (37) Bedding | | | 3,680 | 2,618,20 |
| (38) Channel excavation (39) 12" drain holes, plastic | C.Y. | 0.26 | 10,070,000 | 2,010,20 |
| | TE | | | 16,700 |
| pipe, filter Subtotal - concrete | L.F. | | | 13,131,63 |
| Navigation dam No. 11 | | | | 25,252,05 |
| (1) Care of water | L.S. | | | 25,00 |
| (2) Clearing | Acre | 200.00 | 22 | 4,40 |
| (3) Excavation, common | C.Y. | 0.60 | 394,000 | 236,4 |
| (4) Stripping for dike | C.Y. | 2,00 | 14,500 | 29,0 |
| (5) Fill (incl. Str. backfill) | C.Y. | 0.35 | 2,000 | 7 |
| (6) Embankment, dike | C.Y. | 0.25 | 122,000 | 30,5 |
| (7) Piling, timber, treated | L.F. | 3.25 | 26,200 | 85,1 |
| (8) Piling, timber, untreated | L.F. | 2.85 | 5,000 | 14,2 |
| (9) Pipe, 6" V.C., perforated | L.F. | 2.00 | 1,080 | 2,1 |
| (10) Piling, steel sheet, MA-31 | S.F. | 3,00 | 5,600 | 16,8 |
| (11) Piling, steel sheet, MA-22 | S.F. | 2.85 | 5,200 | 14,8 |
| (12) Riprap, concrete block | C Y | 30.00 | 5,040 | 151,2 |
| (13) Gravel, filter blanket | C.Y. | 5.50 | 7,760 | 42,6 |
| (14) Concrete, apron | C.Y. | 22,00 | 2,730 | 60,0 |
| (15) Concrete, spillway sill | C.Y. | 22.00 | 30,000 | 660,0 |
| (16) Concrete, 2" stabilizer | | | | |
| | C.Y. | 26.00 | 200 | 5,20 |
| (17) Concrete, walls (18) Concrete, 6" paving | C.Y. | 26.00 | 500 | 13,0 |
| I IOI CONCRETE D DOUING | C Y | 20.00 | 11,900 | 238,0 |
| (19) Cement | Bbl. | 5.00 | 63,000 | 315,0 |

| Unit | | | | |
|--|------------------|---------------|----------------|---------------------------------------|
| Item | Unit | cost | Quantity | Cost |
| (20) Steel, reinforcing Subtotal - navigation | Lb. dam No. 1 | 0.13 | 1,297,000 | 168,610 2,112,930 |
| Subtotal - dams Contingencies, 25% + Total - dams | | | | 23,523,398 5,880,602 29,404,000 |
| (05.0) Locks | | | | |
| Lock No. 10B (1) Cofferdam | C.Y. | 0.25 | 64,400 | 16,100 |
| (2) Clearing | Acre | 300.00 | 10 | 3,000 |
| (3) Excavation, common | C.Y. | 0.50 | 210,200 | 105,100 |
| (4) Fill (incl. esplanade) (5) Fill, cellular guidewall | C.Y. | 0.35 | 191,800 | 67,130 |
| cells | C.Y. | 0.35 | 3,070 | 1,075 |
| (6) Timber, Y.P., creosoted | MFBM | 600.00 | 45.6 | 27,360 |
| (7) Piling, steel H, 14 BP-73 | L.F. | 8.00 | 27,500 | 220,000 |
| (8) Piling, steel sheet | S.F. | 3.00 | 90,460 | 271,380 |
| (9) Riprap, concrete block (10) Filter blanket | C.Y. | 30.00 | 5,789 | 173,670 |
| (11) Concrete, walls and floor | C.Y. | 5.50 20.00 | 2,260 1,372 | 12,430 27,440 |
| (12) Concrete, walls | C.Y. | 28.00 | 58,300 | 1,632,400 |
| (13) Concrete, floors and sills | C.Y. | 22.00 | 64,800 | 1,425,600 |
| (14) Concrete, cellular guide- | | | | |
| wall | C.Y. | 45.00 | 8,100 | 364,500 |
| (15) Cement | Bbl. | 5.00 | 198,515 | 992,575 |
| (16) Steel, str., wall armor | Lb. | 0.20 | 459,140 | 91,828 |
| (17) Steel, str., miter gates | Lb. | 0.45 | 1,080,000 | 486,000 |
| (18) Upper gate guards (19) Steel, str., tainter valves | L.S. | 0.45 | 73,230 | 250,000 32,954 |
| (20) Steel, str., miscellaneous | Lb. | 0.22 | 314,000 | 69,080 |
| (21) Steel, reinforcing | Lb. | 0.13 | 82,300 | 10,699 |
| (22) Steel, reinforcing | Lb. | 0.15 | 18,206,000 | 2,730,900 |
| (23) Steel, corosion resistant | Lb. | 0.80 | 6,850 | 5,480 |
| (24) Steel, str., chromium | Lb. | 0.80 | 4,940 | 3,952 |
| (25) Forgings, steel, carbon | Lb. | 0.80 | 197,000 | 157,600 |
| (26) Forgings, steel, alloy | Lb. | 0.80 | 5,110 | 4,088 |
| (27) Castings, steel, grade "0" | Lb. | 0.60 | 19,200 | 11,520 |
| (28) Castings, steel, grade 1, 2 & 3 | Th | 0.60 | 40,610 | 24,366 |
| (29) Castings, steel, alloy | Lb. | 0.60 | 6,570 | 3,942 |
| (30) Castings, iron | Lb. | 0.60 | 5,970 | 3,582 |
| (31) Bronze and brass | Lb. | 2.50 | 2,230 | 5,575 |

| | | Unit | | |
|---------------------------------|-------|-----------|----------|------------|
| Item | Unit | cost | Quantity | Cost |
| (20) 44 4 2 | | | | |
| (32) Metals, miscellaneous, | | 0.00 | | . (-(|
| non-ferrous | Lb. | 0.80 | 2,070 | 1,656 |
| (32) Conduit, fibre, 3" D | L.F. | 1.50 | 9,600 | 14,400 |
| (34) Conduit, metal, 1" D | L.F. | 0.85 | 150 | 128 |
| (35) Conduit, metal, 3" D | L.F. | 3.75 | 155 | 581 |
| (36) Electrical facilities | L.S. | | | 120,000 |
| (37) Pipe railing, steel | L.F. | 4.00 | 5,800 | 23,200 |
| (38) Floor plating, steel | S.F. | 5.00 | 90 | 450 |
| (39) Floor grating, steel | S.F. | 4.65 | 8,810 | 40,967 |
| (40) Rubber seals, moulded | Lb. | 2.50 | 1,100 | 2,750 |
| (41) Gages, tile | L.F. | 14.00 | 148 | 2,072 |
| (42) Operating machines, | | | | -, |
| miter gates | Each | 27,500.00 | 14 | 110,000 |
| (43) Valves, double 4-way | Each | 900.00 | 2 | 1,800 |
| (44) Operating machine, | Dacii | ,00.00 | _ | 1,000 |
| tainter valve | Each | 17,500.00 | 24 | 70,000 |
| (45) Valve, control, 3-way | Each | 700.00 | 4 | 2,800 |
| (46) Valve, automatic, 3-way | Each | 500.00 | 4 | 2,000 |
| (47) Oil piping system | L.S. | ,00.00 | | 90,000 |
| (48) Field office | L.S. | | | 5,000 |
| (49) Central control structure | | | | 36,000 |
| (50) Stop logs | L.S. | | | |
| | L.S. | | | 150,000 |
| (51) Care of water during | | | | 050 000 |
| construction | L.S. | | | 250,000 |
| (52) Derrick, stiff leg | L.S. | | | 37,500 |
| (53) Bascule bridge | L.S. | 1. 00 | 202 20 | 120,000 |
| (54) Piling, steel sheet | S.F. | 4.00 | 27,800 | 111,200 |
| Subtotal - lock No. 10 | В | | | 10,423,830 |
| b. Lock No. 11 | | | | |
| (1) Cofferdam | C.Y. | 0.25 | 132,000 | 33,000 |
| (2) Clearing | | 200.00 | 10 | |
| | Acre | | 698,100 | 2,000 |
| (3) Excavation, common | C.Y. | 0.50 | | 349,050 |
| (4) Fill (incl. esplanade) | C.Y. | 0.35 | 213,700 | 74,795 |
| (5) Fill cellular guidewall | a 17 | 0.25 | 7 1.50 | 0 600 |
| cells | C.Y. | 0.35 | 7,450 | 2,608 |
| (6) Timber, Y.P. creosoted | MFBM | 600.00 | 29 | 17,400 |
| (7) Piling, steel-H, (14BP-73) | L.F. | 8.00 | 29,110 | 232,880 |
| (8) Piling, steel sheet | S.F. | 3.00 | 89,660 | 268,980 |
| (9) Riprap, concrete block | C.Y. | 30.00 | 14,780 | 443,400 |
| (10) Filter blanket | C.Y. | 5.50 | 5,960 | 32,780 |
| (11) Concrete, walls | C.Y. | 28.00 | 55,500 | 1,554,000 |
| (12) Concrete, floors and sills | C.Y. | 22.00 | 63,000 | 1,386,000 |
| | | | | |

| | | Unit | | |
|---------------------------------|------|-----------|------------|------------|
| Item | Unit | cost | Quantity | Cost |
| (12) g | | | | |
| (13) Concrete, cellular | a w | 25.00 | 0 (00 | 203 000 |
| guidewall | C.Y. | 35.00 | 8,600 | 301,000 |
| (14) Cement | Bbl. | 5.00 | 189,600 | 948,000 |
| (15) Steel, str., wall armor | Lb. | 0.20 | 339,500 | 67,900 |
| (16) Steel, str., miter gates | Lb. | 0.45 | 432,000 | 194,400 |
| (17) Steel, str., tainter | | . 1 . | | 1 |
| values | Lb. | 0.45 | 73,230 | 32,954 |
| (18) Steel, str., miscellaneous | Lb. | 0.22 | 366,100 | 80,542 |
| (19) Steel, reinforcing | Lb. | 0.15 | 17,622,000 | 2,643,300 |
| (20) Steel, corrosion resistant | Lb. | 0.80 | 6,850 | 5,480 |
| (21) Steel, str., chromium | Lb. | 0.80 | 4,940 | 3,952 |
| (22) Forgings, steel, carbon | Lb. | 0.80 | 96,500 | 77,200 |
| (23) Forgings, steel, alloy | Lb. | 0.80 | 5,110 | 4,088 |
| (24) Castings, steel, grade "0" | Lb. | 0.60 | 19,200 | 11,520 |
| (25) Castings, steel, grades 1, | | | | |
| 2, & 3 | Lb. | 0.60 | 40,610 | 24,366 |
| (26) Castings, steel, alloy | Lb. | 0.60 | 6,570 | 3,942 |
| (27) Castings, iron | Lb. | 0.60 | 5,970 | 3,582 |
| (28) Bronze and brass | Lb. | 2.50 | 2,230 | 5,575 |
| (29) Metal, miscellaneous, | | | | |
| non-ferrous | Lb. | 0.80 | 2,070 | 1,656 |
| (30) Conduit, fibre, 3" D | L.F. | 1.50 | 9,600 | 14,400 |
| (31) Conduit, metal, 1" D | L.F. | 0.85 | 150 | 128 |
| (32) Conduit, metal, 3" D | L.F. | 3.75 | 155 | 581 |
| (33) Electrical facilities | L.S. | | | 120,000 |
| (34) Pipe railing, steel | L.F. | 4.00 | 5,700 | 22,800 |
| (35) Floor plating, steel | S.F. | 5.00 | 90 | 450 |
| (36) Floor grating, steel | S.F. | 4.65 | 8,810 | 40,967 |
| (37) Rubber seals, moulded | Lb. | 2.50 | 1,100 | 2,750 |
| (38) Gages, tile | L.F. | 14.00 | 69 | 966 |
| (39) Operating machines, miter | | | | |
| gates | Each | 27,500.00 | 4 | 110,000 |
| (40) Valve, double, 4-way | Each | 900.00 | 2 | 1,800 |
| (41) Operating machine, tainter | | | | |
| valve | Each | 17,500.00 | 4 | 70,000 |
| (42) Valve, control, 3-way | Each | 700.00 | 4 | 2,800 |
| (43) Valve, automatic, 3-way | Each | 500.00 | 4 | 2,000 |
| (44) Oil piping system | L.S. | | | 90,000 |
| (45) Field office | L.S. | | | 5,000 |
| (46) Central control structure | L.S. | | | 36,000 |
| (47) Stop logs | L.S. | | | 45,000 |
| (48) Care of water during | | | | |
| construction | L.S. | | | 150,000 |
| Subtotal - lock No. 11 | | | | 9,521,992 |
| | | | | |
| Subtotal - locks | | | | 19,945,822 |
| Contingencies, 25% + | | | | 4,986,178 |
| Total - locks | | | | 24,932,000 |
| | 60 | | | |

| | | Unit | | |
|----------------------------------|---------------------------|-----------|------------|------------|
| Item | Unit | cost | Quantity | Cost |
| (08.0) Access roads | | | | |
| (1) Access road to main dam | L S | | | 112,000 |
| (2) Access road to lock No. 11 | | | | 163,000 |
| Subtotal - access roads | | | | 275,000 |
| Contingencies, 25% + | | | | 69,000 |
| Total - access roads | | | | 344,000 |
| (09.0) Channel | | | | |
| (1) Clearing and grubbing, | | | | |
| pool 10-B | Acre | 350.00 | 887 | 310,450 |
| (2) Clearing and grubbing, | | | | |
| pool 11 | Acre | 200.00 | 330 | 66,000 |
| (3) Clearing spoil area, | | | | |
| pool 10-B | Acre | 200.00 | 2,234 | 446,800 |
| (4) Clearing spoil area, | | | | |
| pool 11 | Acre | 100,00 | 344 | 34,400 |
| (5) Excavation, common, | | | | |
| pool 10-B | C.Y. | 0.27 | 18,017,300 | 4,864,671 |
| (6) Excavation, common, | | | 0 - 1 | |
| pool 11 | C.Y. | 0.30 | 8,334,900 | 2,500,470 |
| (7) River diversion dams, | | | | |
| pool 11 | L.S. | | | 64,500 |
| Subtotal - channel | | | | 8,287,291 |
| Contingencies, 25% + | | | | 2,071,709 |
| Total - channel | | | | 10,359,000 |
| (19.0) Buildings, grounds, and w | tilities | | | |
| (1) Maintenance facilties @ | The Company of the law of | | | |
| main dam | L.S. | | | 100,000 |
| (2) Water supply | L.S. | | | 30,000 |
| (3) Power line and substation | L.S. | | | 25,000 |
| (4) Visitors overlook | | | | |
| facilties | L.S. | | | 28,000 |
| (5) Facilties @ lock 10B | L.S. | | | 127,600 |
| (6) Facilties @ lock ll | L.S. | | | 127,600 |
| Subtotal - buildings | | | | 438,200 |
| Contingencies, 25% + | | | | 109,800 |
| Total - buildings, groun | nds, and | utilities | | 548,000 |
| 20.0) Permanent operating equip | | | | |
| (1) Radio-telephone equipment | L.S. | | | 5,000 |
| | | | | 0.000 |
| (2) Boats | L.S. | | | 9,200 |
| | L.S. | | | 9,200 |

| | | Unit | | |
|--|--------------------------|--------------|-------------|--|
| Item | Unit | cost | Quantity | Cost |
| (4) Stream gages | L.S. | | | \$ 21,000 |
| (5) Evaporation and rain gages | | | | 2,000 |
| (6) Sedimentation and degrada- | LI.D. | | | 2,000 |
| tion ranges | L.S. | | | 95,000 |
| (7) Equipment @ lock 10B | L.S. | | | 35,900 |
| (8) Equipment @ lock 11 | L.S. | | | 35,900 |
| | | | | 35,250 |
| (9) Heavy duty equipment | L.S. | | | |
| (10) River observation system | | | | 13,330 276,380 |
| Subtotal - permanent op | erating e | equipment | | 69,500 |
| Contingencies, 25% + | | | | 68,620 345,000 |
| Total - permanent opera | ting equi | ipment | | 345,000 |
| (30.0) Engineering and design | | | | 3,480,000 |
| (31.0) Supervision and administra | ation | | | 5,430,000 |
| U. S. Coast Guard - aids to navi | gation | | | 294,000 |
| | | | | |
| Subtotal - estimated fi | rst cost | - dam and re | eservoir \$ | 125.055.000 |
| Subtotal - estimated fi | rst cost | - dam and re | eservoir \$ | 125,055,000 |
| | | | eservoir \$ | 125,055,000 |
| DETAILED ESTIMATE OF FIRST COS | | | | |
| DETAILED ESTIMATE OF FIRST COST | r - recr | | 1,907 | |
| DETAILED ESTIMATE OF FIRST COST (01.0) Lands and damages (1) Fee simple lands | P - RECRI | | | \$ 243,050 |
| DETAILED ESTIMATE OF FIRST COST (01.0) Lands and damages (1) Fee simple lands (2) Severance damage | Acre | | | \$ 243,050 27,150 |
| DETAILED ESTIMATE OF FIRST COST (01.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements | P - RECRI | | | \$243,050 27,150 70,000 |
| DETAILED ESTIMATE OF FIRST COST (01.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements Subtotal | Acre | | | \$ 243,050 27,150 70,000 340,200 |
| DETAILED ESTIMATE OF FIRST COS (01.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements Subtotal Contingencies, 25% + | Acre | | | \$243,050 27,150 70,000 340,200 84,800 |
| DETAILED ESTIMATE OF FIRST COS (01.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements Subtotal Contingencies, 25% + (4) Acquisition expense | Acre L.S. L.S. | | | \$ 243,050 27,150 70,000 340,200 84,800 26,000 |
| DETAILED ESTIMATE OF FIRST COS (01.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements Subtotal Contingencies, 25% + | Acre L.S. L.S. | | | \$ 243,050 27,150 70,000 340,200 84,800 26,000 |
| DETAILED ESTIMATE OF FIRST COS (01.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements Subtotal Contingencies, 25% + (4) Acquisition expense Total - lands and damage | Acre L.S. L.S. | | | \$243,050 27,150 70,000 340,200 84,800 26,000 451,000 |
| DETAILED ESTIMATE OF FIRST COS (01.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements Subtotal Contingencies, 25% + (4) Acquisition expense Total - lands and damage (03.0) Reservoir clearing | Acre L.S. L.S. | EATION | 1,907 | \$243,050 27,150 70,000 340,200 84,800 26,000 451,000 |
| DETAILED ESTIMATE OF FIRST COS (01.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements Subtotal Contingencies, 25% + (4) Acquisition expense Total - lands and damage | Acre L.S. L.S. | EATION | 1,907 | \$243,050 27,150 70,000 340,200 84,800 26,000 451,000 |
| DETAILED ESTIMATE OF FIRST COS (01.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements | Acre L.S. L.S. Acre ing | EATION | 1,907 | \$ 243,050 27,150 70,000 340,200 84,800 26,000 451,000 1,500,000 375,000 1,875,000 |
| DETAILED ESTIMATE OF FIRST COS (Ol.O) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements | Acre L.S. L.S. | EATION | 1,907 | \$ 243,050 27,150 70,000 340,200 84,800 26,000 451,000 1,500,000 375,000 1,875,000 |
| DETAILED ESTIMATE OF FIRST COS (Ol.O) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements | Acre L.S. L.S. Acre ing | EATION | 1,907 | \$243,050 27,150 70,000 340,200 84,800 26,000 451,000 1,500,000 375,000 1,875,000 345,000 86,000 |
| DETAILED ESTIMATE OF FIRST COS (Ol.O) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements | Acre L.S. L.S. Acre ing | EATION | 1,907 | \$243,050 27,150 70,000 340,200 84,800 26,000 451,000 1,500,000 375,000 1,875,000 345,000 431,000 |
| DETAILED ESTIMATE OF FIRST COS (ol.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements | Acre L.S. L.S. Acre ing | EATION | 1,907 | \$ 243,050 27,150 70,000 340,200 84,800 26,000 451,000 1,500,000 375,000 1,875,000 345,000 86,000 |
| DETAILED ESTIMATE OF FIRST COS (ol.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements | Acre L.S. L.S. Acre ing | EATION | 1,907 | \$ 243,050 27,150 70,000 340,200 84,800 26,000 451,000 1,500,000 375,000 1,875,000 86,000 431,000 |
| DETAILED ESTIMATE OF FIRST COS (ol.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements | Acre L.S. L.S. Acre ing | EATION | 1,907 | \$ 243,050 27,150 70,000 340,200 84,800 26,000 451,000 1,500,000 375,000 1,875,000 86,000 431,000 |
| DETAILED ESTIMATE OF FIRST COS (01.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements | Acre L.S. L.S. Acre ing | EATION | 1,907 | \$243,050 27,150 70,000 340,200 84,800 26,000 451,000 1,500,000 375,000 345,000 86,000 431,000 |
| DETAILED ESTIMATE OF FIRST COS (ol.0) Lands and damages (1) Fee simple lands (2) Severance damage (3) Improvements | Acre L.S. L.S. Acre ing | EATION | 1,907 | \$ 243,050 27,150 70,000 340,200 84,800 26,000 451,000 1,500,000 375,000 1,875,000 345,000 86,000 |

| | 9 | | : | Unit | | | : | |
|-------------------------------|-----|--------|---|--------|-----|----------|------|------------|
| Item | • | Unit | : | cost | : | Quantity | : | Cost |
| (5) Sanitary facilities | | | | | | | | \$ 552,000 |
| (6) Water supply system | | | | | | | | 613,000 |
| (7) Picnic facilities | | | | | | | | 1,288,000 |
| (8) Site improvements | | | | | | | | 245,000 |
| (9) Signs and buoys | | | | | | | | 123,000 |
| Subtotal - facilities | | | | | | | | 6,133,000 |
| Contingencies, 25% + | | | | | | | | 1,533,000 |
| Total - facilities | | | | | | | | 7,666,000 |
| 30.0) Engineering and design | | | | | | | | 633,000 |
| 31.0) Supervision and adminis | tra | ation | | | | | | 1,027,000 |
| Total - estimated first | C | ost of | r | ecreat | ion | | \$1 | 2,083,000 |
| Total - Estimated project | ct | first | C | ost | | | \$13 | 7,138,000 |

Prices are as of January 1962.

LOCAL PROTECTION UNITS

- 156. GENERAL. The local flood protection projects proposed in this report are designed to supplement and operate in conjunction with the existing and authorized local flood protection projects. These projects are the existing Fort Worth and Dallas Floodways and the authorized Big Fossil channel improvement and Fort Worth Floodway extension (West Fork) and the Liberty levee project.
- 157. WEST FORK. The plan of improvement for the West Fort of the Trinity River consists of a multiple purpose channel and floodway for navigation and flood control from the mouth of the West Fork in the Dallas Floodway upstream, a distance of about 31 miles, to the end of the existing Fort Worth Floodway. The navigation features of the multiple-purpose channel are presented in the first part of this appendix. The flood control features of the West Fork project include the following items:
- (a) Sufficient realignment and enlargement of the West Fork channel to provide within-bank capacity of at least 15,000 cubic feet per second in this reach of the river.
- (b) The enlargement and realignment or diversion of about 16.74 miles of tributary channels through the leveed areas.
- (c) The construction of a parallel levee system through the low areas consisting of about 34.0 miles of levee along the left bank, 9.1 miles of new levee and the rehabilitation of 1.5 miles of existing levee on the right bank of the proposed enlarged and realigned channel of the West Fork and tributary channels. The floodway along the West Fork would be a dedicated floodway maintained free of encroachments and would vary in width from 1,000 feet to 3,000 feet between centerline of levees. It would have sufficient capacity of contain the standard project flood which varies from about 95,000 cubic feet per second at Fort Worth to about 160,000 cubic feet per second at the mouth of West Fork. The levees would be constructed with 1 on 2.5 side slopes, with a 10-foot crown width and have 4 feet of freeboard above the design discharge water surface.
- (d) The provision of appurtenant interior-drainage facilities consisting of permanent sump areas to provide an aggregate storage capacity of 9,900 acre-feet below the damaging stage elevations of the various locations through the levee system, and low interior areas to be filled with excess excavated material to raise damaging elevations.

- (e) The modification of 14 main stem and 5 tributary channel bridges and the construction of one new road in connection with the proposed improvements along the West Fork.
- (f) The filling of eight areas outside the proposed leveed areas to elevations about 2 feet, and one area about four feet, above the design water surface by utilizing the excess material from the channel-excavation work.
- (g) The acquisition of rights-of-way consisting of about 8,430 acres of land in fee simple for the construction of the excavated channels, levees, floodway, and permanent sump areas. The proposed plan for the West Fork is shown on plates 43 and 44 and the detailed profiles are shown on plates 45 through 48.
- 158. COST ESTIMATES. A summary of first costs and annual charges for the West Fork Floodway project is shown in table 47. The detailed cost estimate for the West Fork Floodway project is shown in table 48. The estimate shows the separation of Federal and non-Federal costs and does not include the flood control portion of the multiple-purpose channel.

TABLE 47 SUMMARY OF FIRST COST AND ANNUAL CHARGES WEST FORK FLOOD PROTECTION PROJECT WEST FORK TRINITY RIVER

Tt.em

| | 1 OCM | COST |
|----|--|--------------|
| | First cost | |
| 1. | | |
| | Railroad alterations (none on diversion channels |) - |
| | Levees, sluices, sumps, and diversion channels | |
| | a. Levees | \$ 1,676,000 |
| | b. Sluices | 732,000 |
| | c. Sumps | 4,763,000 |
| | d. Diversion channels | 2,596,000 |
| | Engineering and design | 371,000 |
| | Supervision and administration | 581,000 |
| | Total estimated Federal first cost (1) | 10,719,000 |
| 2. | Non-Federal first cost | |
| | Lands and damages | 6,570,000 |
| | Relocations and alterations | 470,000 |
| | Engineering and design | 18,000 |
| | Supervision and administration | 32,000 |
| | Total estimated non-Federal first cost | 7,090,000 |
| 3. | Total estimated first cost of project | 17,809,000 |
| | TO THE RESIDENCE OF THE PARTY O | |

TABLE 47 (CONT'D) SUMMARY OF FIRST COST AND ANNUAL CHARGES WEST FORK FLOOD PROTECTION PROJECT WEST FORK TRINITY RIVER

| | | tem | Cost |
|----|--------------|--|--------------------------------|
| | | Annual charges | |
| (| Cons 2-7/ | truction period-4 years) (100 yr. amort 8% Federal interest rate) (3% Non-Feder | cization) ral interest rate |
| 1. | Fed | eral investment | |
| | a. | Federal first cost | \$10,719,000 |
| | b. | Interest during construction | 616,300 |
| | C. | Total Federal investment | 11,335,300 |
| 2. | | -Federal investment | |
| | a. | Non-Federal first cost | 7,090,000 |
| | b. | Interest during construction | 425,400 |
| | C. | Total non-Federal investment | 7,515,400 |
| 3. | Fed | eral annual charges | |
| | a | Interest on Federal investment | 325,900 |
| | b. | Amortization charge | 20,300 |
| | | Operation and maintenance | * |
| | | Total Federal annual charge | 346,200 |
| 4. | Non | -Federal annual charges | |
| | | Interest on non-Federal investment | 225,500 |
| | b. | Amortization charge | 12,400 |
| | | Operation and maintenance | 224,000 |
| | | Total non-Federal annual charges | 461,900 |
| 5. | Tot | al estimated annual charges | 808,100 |

(1) \$70,000 preauthorization cost excluded.

Prices are as of January 1962.

TABLE 48 DETAILED ESTIMATE OF FIRST COST WEST FORK FLOOD PROTECTION PROJECT WEST FORK TRINITY RIVER

| - | THE PROPERTY OF THE PROPERTY O | ; ; | Unit : | | |
|----|--|---------|----------------|------------|------------|
| - | Item | :Unit: | cost : | Quantity: | Cost |
| 7 | EEDERAL ETECH COCH | | | | |
| 10 | FEDERAL FIRST COST 02.0 Railroad alterations (None o | n diver | sion channe | 10) | |
| | (11.0) Levees, sluices, sumps and | | | | |
| | a. Levees | arvers | TOTT CITATRICE | | |
| | (1) Clearing main stem berms | Acre | \$ 100.00 | 3,500 | \$ 350,000 |
| | (2) Clearing & grubbing | Acre | 150.00 | 525 | 78,750 |
| | (3) Excavation, stripping | C.Y. | 0.25 | 734,000 | 183,500 |
| | (4) Compacted fill | C.Y. | 0.07 | 7,140,000 | 499,800 |
| | (5) Slope protection | Acre | 500.00 | 457 | 228,500 |
| | Subtotal - levees | | , | | 1,340,550 |
| | b. Sluices | | | | -,5.0,77 |
| | (1) Care of water | L.S. | | | 20,000 |
| | (2) Excavation, structural | C.Y. | 1.50 | 68,000 | 102,000 |
| | (3) Excavation, discharge | | | | |
| | channels | C.Y. | 0.30 | 495,000 | 148,500 |
| | (4) Structural backfill | C.Y. | 0.50 | 62,000 | 31,000 |
| | (5) Concrete (Incl. cement) | C.Y. | 50.00 | 2,950 | 147,500 |
| | (6) Steel, reinforcing | Lb. | 0.13 | 295,000 | 38,350 |
| | (7) Flap gates - 4'x4' | Ea. | 800.00 | 3 | 2,400 |
| | (8) Flap gates - 5'x5' | Ea. | 1,250.00 | 3 | 3,750 |
| | (9) Flap gates - 6'x6' | Ea. | 1,800.00 | 7 | 12,600 |
| | (10) Sluice gates, shafts, & | | | | |
| | stands - 4'x4' | Ea. | 1,600.00 | 6 | 9,600 |
| | (11) Sluice gates, shafts, & | | | | |
| | stands - 5'x5' | Ea. | 2,500.00 | 6 | 15,000 |
| | (12) Sluice gates, shafts, & | | | | |
| | stands - 6'x6' | Ea. | 3,600.00 | 14 | 50,400 |
| | (13) Miscellaneous metals | Lb. | 0.40 | 5,330 | 2,132 |
| | (14) Riprap | C.Y. | 6.00 | 290 | 1,740 |
| | (15) Bedding | C.Y. | 5.00 | 120 | 600 |
| | Subtotal - sluices | | | | 585,572 |
| | c. Sumps | | | | |
| | (1) Care of water | L.S. | | | 15,000 |
| | (2) Clearing | Acre | 100.00 | 453 | 45,300 |
| | (3) Excavation, common | C.Y. | 0.25 | 15,000,000 | 3,750,000 |
| | Subtotal - Sumps | | | | 3,810,300 |
| | d. Diversion channels | | | | |
| | (1) Care of water | L.S. | | | 15,000 |
| | (2) Clearing | Acre | 100.00 | 1,185 | 118,500 |
| | (3) Excavation, common | C.Y. | 0.25 | 4,315,000 | 1,078,750 |
| | (4) Excavation, shale | C.Y. | 0.75 | 1,080,000 | 810,000 |
| | (5) Slope protection | Acre | 500.00 | 110 | 55,000 |
| | Subtotal - diversion chann | nels | | | 2,077,250 |
| | | | | | |

TABLE 48 (CONT'D) DETAILED ESTIMATE OF FIRST COST WEST FORK FLOOD PROTECTION PROJECT WEST FORK TRINITY RIVER

| Item | | Jnit: | Unit cost | ; | Quantity: | Cost |
|------------------|--|-----------------|-------------------|-----|-----------|---|
| Cor | ctotal - levees, sluices, channels stingencies 25% ± cal - levees, sluices, sur | | | | on | \$7,813,672 1,953,328 |
| (30.0) | channels Engineering and design Supervision and administr | ration | | | | 9,767,000 371,000 581,000 |
| a. Lar | Total - estimated Federal eral first cost ids and damages Fee simple lands and | l Hirst | cost (| 1) | | 10,719,000 |
| | improvements Contingencies, 25% + Subtotal - lands and dan Acquisition expense Total - lands and damage | | | | 8,430 | 5,119,000 1,281,000 6,400,000 170,000 6,570,000 |
| | ocations and alterations Reidges and roads Connection E. end Ellio Reeder Rd. to Midway Rd | | 25,000 | .00 | 0.5 | 12,500 |
| | Hurst Sewage Plant Rd., bridge Trammel-Davis Rd.,bridge Sulpher Branch, bridge Jefferson Ave., bridge | L.F. L.F. | 175 175 175 | .00 | | 43,750 52,500 87,500 175,000 |
| (2) | Utilities Fort Worth Sewage plant protection Subtotal - relocations a Contingencies, 25% ± Total - relocations and | L.S. and alt | | S | | 5,000 376,250 93,750 470,000 |
| c. Eng d. Sur | tineering and design ervision and administrati Total - estimated non-R | ion | | ost | | 18,000 32,000 7,090,000 |
| 3. Total e | stimated first cost of pr | | | | | \$17,809,000 |

^{(1) \$70,000} preauthorization cost excluded.

Prices are as of January 1962



LEGEND



NAVIGATION AND FLOOD CONTROL CHANNEL DESIGN WATER SURFACE HIGH WATER LINE (MAXIMUM OF RECORD) NAVIGATION LOCK AND DAM RIVER MILEAGE CHANNEL MILEAGE CORE BORINGS AUGER BORINGS FILL AREA SUMP AREA NEW EARTH LEVEES

EXISTING LEVEES

STRENDTHEN EXISTING LEVEE

FEDERAL HIGHMAY

STATE HIGHMAY

ATTERSTATE HIGHMAY

EXISTING BHODE

COUNTY LINE

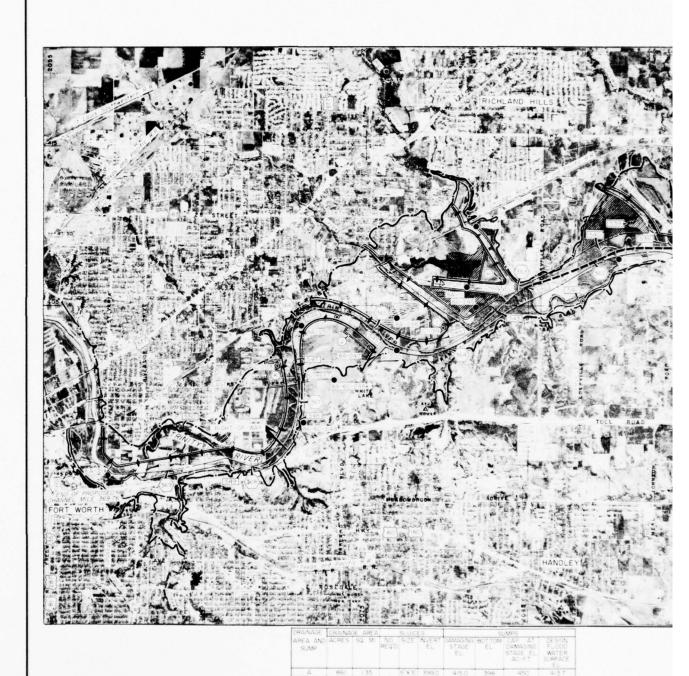
GRAVITY FLOW DRAINAGE STR

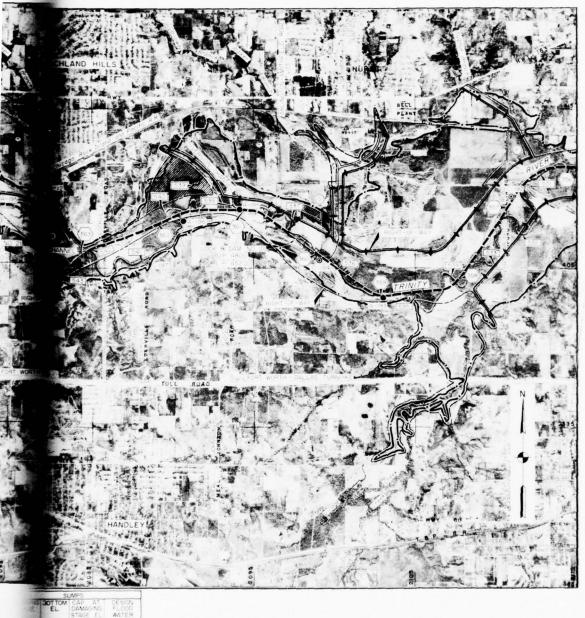


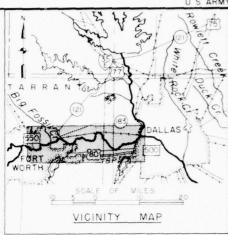
EXISTING LEVEES
STRENGTHEN EXISTING LEVEES FEDERAL HIGHWAY INTERSTATE HIGHWAY
FARM TO MARKET ROAD EXISTING BRIDGE GRAVITY FLOW ORAINAGE STRUCTURE

RINITY RIVER AND TRIBUTARIES, TEXAS
WEST FORK AND TRIBUTARIES
MULTIPLE PURPOSE CHANNEL
AND FLOODWAY PLAN OF IMPROVEMENT

| IN 2 SHEETS 2 | SCALE (| PEET 2000 | SHEET NO. |
|--|------------------------|------------------------|---------------------------|
| U.S. ARMY ENGINEE | | T WORTH | JUNE 196 |
| W. C. Cood | 031 | WALL BALLOW | To lan or towner! |
| The Hamley | (8184 5) (1) | TO ACCUSA WE FORT I | STANCTONING SENSOR SURVEY |
| THE STATE OF THE S | TON THE REAL PROPERTY. | FILE T | RIN 230-19 |







LEGEND

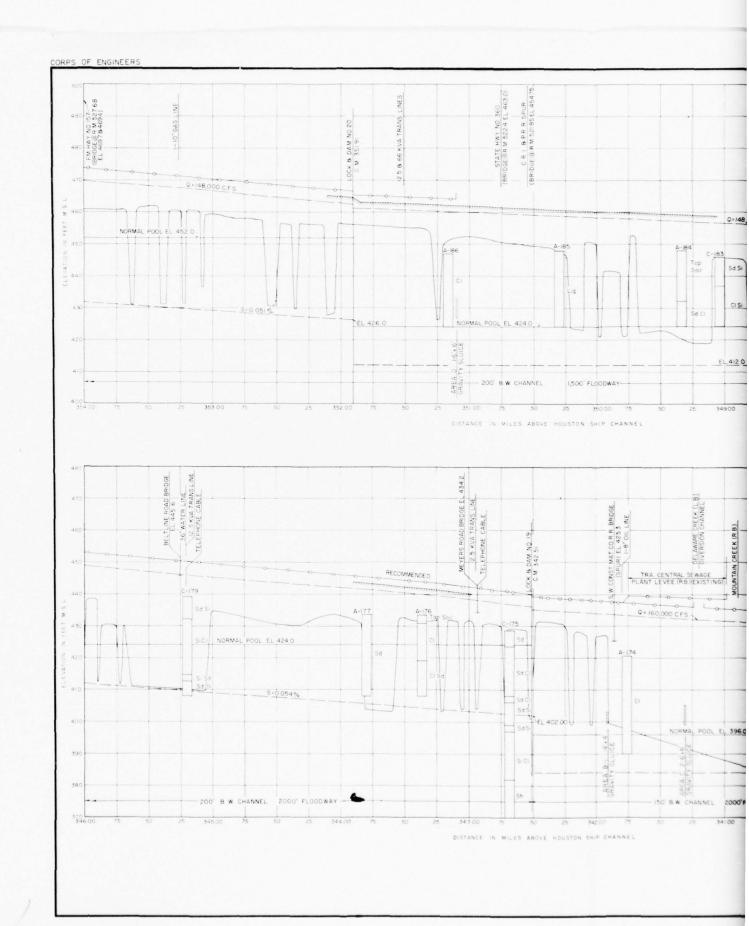
HIGH WATER LINE (MAXIMUM NAVIGATION LOOK AND DAM RIVER MILEAGE CHANNEL MILEAGE CORE BORINGS SUMP AREA NEW EARTH LEVEES FEDERAL HIGHWAY STATE HIGHWAY 1.1

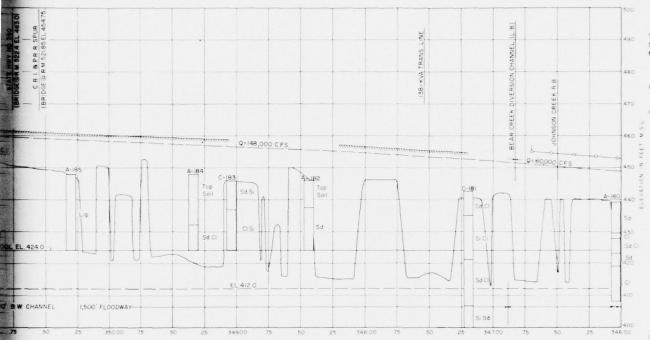
COUNTY LINE

RINITY RIVER AND TRIBUTARIES, TEXA WEST FORK AND TRIBUTARIES MULTIPLE PURPOSE CHANNEL AND FLOODWAY

PLAN OF IMPROVEMENT

| IN 2 SHEETS 2000 | SCALE OF | FEET 2000 4000 | SHEET NO |
|--------------------------------|-----------------|-----------------|----------|
| U.S. ARMY ENGINEER D | | | JUNE 196 |
| Al Callood | LA Ser | Comment Land | S total |
| Co W Warneley | (REN TO REL | TO ACCOMPANY DO | |
| CHEAD, SPECIAL STUDIES DECTION | Descript At the | TILE TRIN 2 | 30-19 |





MILES ABOVE HOUSTON SHIP CHANNEL



HOUSTON SHIP CHANNEL

BORING LEGEND

LABORATORY CLASSIFICATION
M Silty Sands, Sand-Silt mixtures

SM Silty Sands, Sand-Silt mixtures.
SC Clovey Sand, Sand-Clay mixtures.
ML Inorganic Silts and very fine.
Sands, Rock Flour, Silty or.
Clayer fine Sands or Clayery.
Silts with slight plasticity.
CL Inorganic Clays of law plasticity.
Gravelly Clays, Sandy Clays,
Silty Clays, Lean Clays.

Inarganic Clays of high plasticity, Fat Clays

VISUAL CLASSIFICATION
Gr Gravel or Gravelly
Sd Sand or Sandy
Si Silt or Silty

Clay or Clayey Fuller's Earth

SS Sandstone SiS Siltstone

CS Claystone

Sh Shale or Shaly

Ls Limestone H Hard BA Machine Auger Boring,

B" diameter 6C Core Boring, 6" diameter

WT Water Table

HD Texas Highway Dept boring RD Dallas County Road District No / Borings

Absence of ground water levels opposite boring logs does not mean necessarily that ground water will not be encountered at the locations or within the vertical reaches of the borings

Figures to the right of boring logs are water contents in percent of the dry

NOTES

Floodway dimensions shown are minimum requirem measured from centerline of levees, or from centerline of levee to natural bank or fill area.

Chamber and sink or mit great on 2 horizontal.

Chamber and slopes are I vertical on 2 horizontal.

Levee side slopes are I vertical on 2½ horizontal.

All bridges shown are existing structures. Elevation of Bridges refer to existing low steel elevations.

Refer to Plate 56 for typical channel and levee sections

LEGEND

DESIGN WATER SURFACE

LEVEE, RIGHT BANK

LEVEE LEFT BANK

CENTERLINE PROFILE BOTTOM GRADE

RECOMMENDED FILL AREAS LEFT BANK

RECOMMENDED FILL AREAS, RIGHT BANK

воттом міртн B W. L.B. LEFT BANK

RIGHT BANK

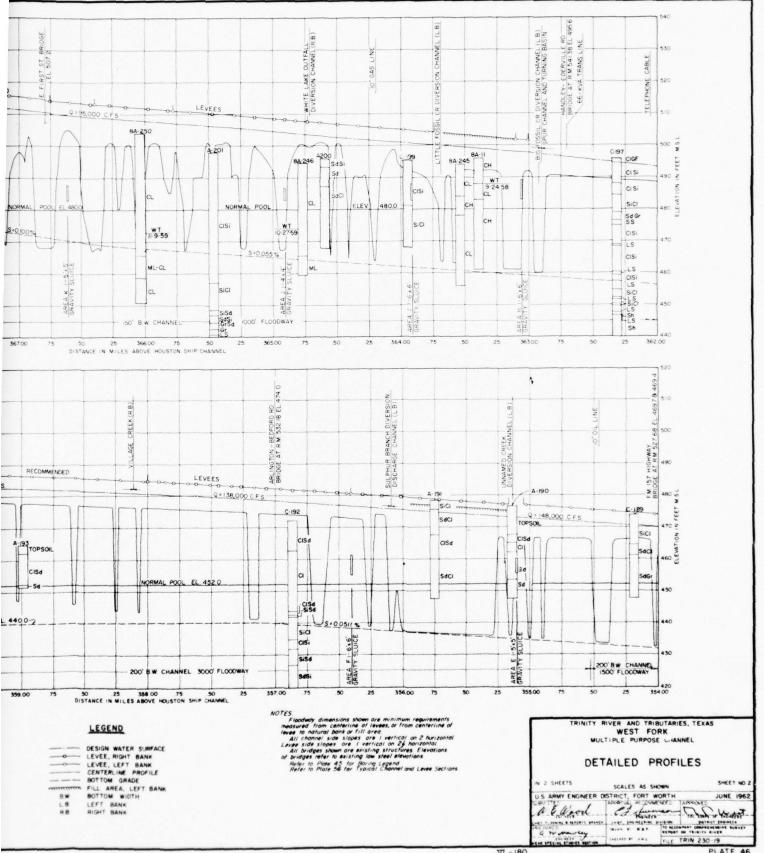
TRINITY RIVER AND TRIBUTARIES, TEXAS WEST FORK MULTIPLE PURPOSE CHANNEL

DETAILED PROFILES

US ARMY ENGINEER DISTRICT, FORT WORTH

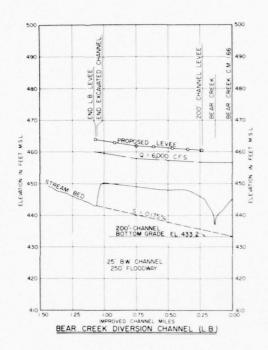
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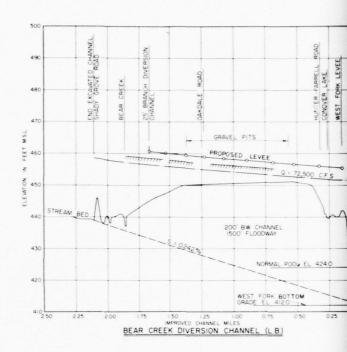
BW LB RB

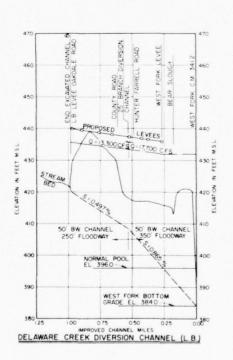


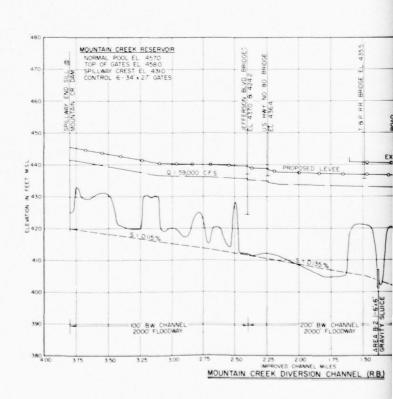
AI -180

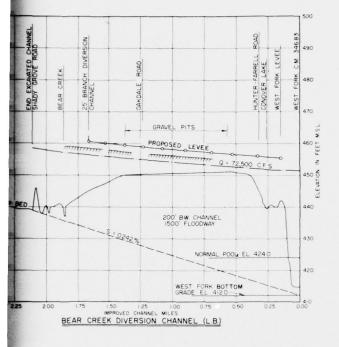
PLATE 46

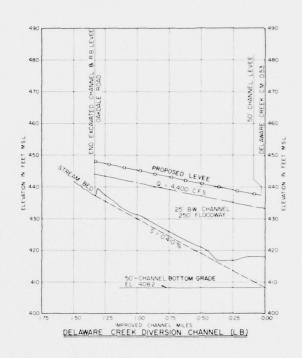


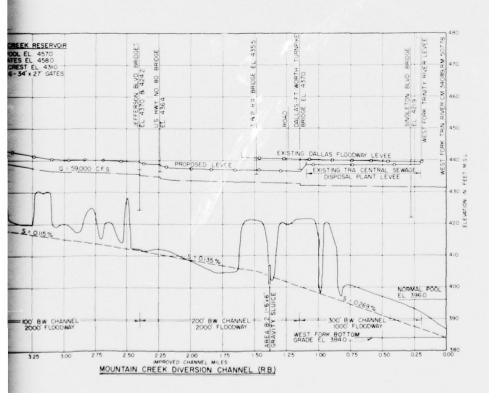












LEGEND

DESIGN WATER SURFACE
LEVEE, RIGHT BANK
LEVEE, LEFT BANK
CENTERLINE PROFILE
BOTTOM GRADE
FILL AREA, RIGHT BANK
BOTTOM WIDTH
LEFT BANK
RIGHT BANK

NOTES:
Floodway dimensions shown are minimum requirements measured from centerline of levees, or from centerline of levee to natural bank or full area.
All channel side stopes are I vertical on 2 horizontal. Levee side slopes are I vertical on 2 horizontal. All bridges shown are existing structures. Elevations of bridges refer to existing low steel elevations. Refer to plate \$46 for typical channel and levee sections.

TRINITY RIVER AND TRIBUTARIES, TEXAS
WEST FORK TRIBUTARIES
FLOOD CONTROL CHANNELS AND FLOODWAYS

DETAILED PROFILES

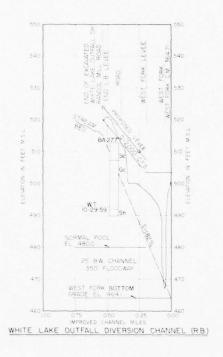
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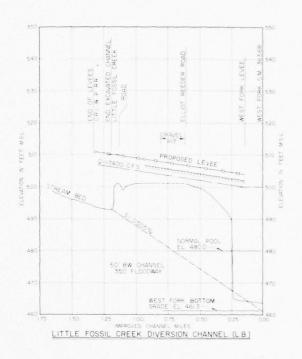
ARMY ENGINEER DISTRICT, FORT WORTH JUNE 1962

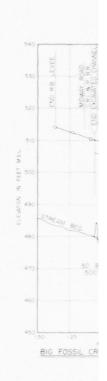
ARMY ENGINEER DISTRICT, FORT WORTH

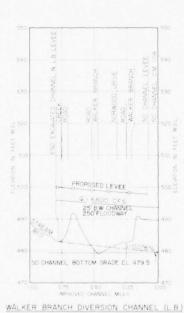
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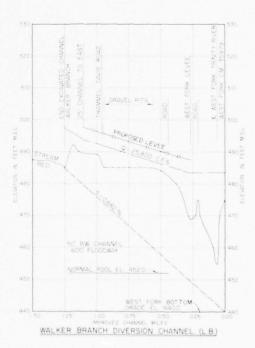


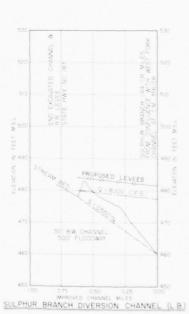




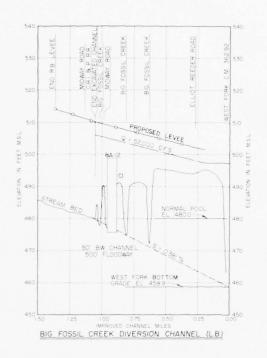


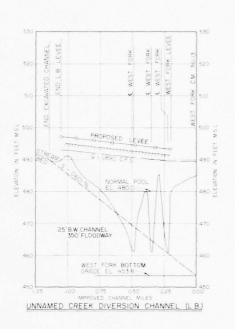


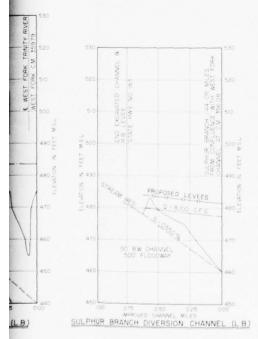


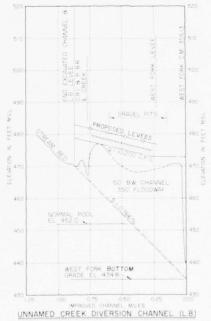












DESIGN WATER SURFACE
LEVEE, RIGHT BANK
LEVEE, LEFT BANK
CENTERLINE PROFILE
BOTTOM GRADE
FILL AREA, RIGHT BANK
FILL AREA, LEFT BANK
BOTTOM WIDTH
LEFT BANK
RIGHT BANK

LEGEND

NOTES

Floodway dimensions shawn are minimum requirements measured from centerline of levees, or from centerline of leve to natural bank or fill area or from centerline of All channel side sloges are I vertical on 2 horizontal ceve side sloges are I vertical on 2 horizontal All bridges shawn are existing structures. Elevations of bridges refer to existing low steel elevations. New or modified bridge openings are to provide 3 feel minimum vertical clearance above design water surface, and unabstructed horizontal clearance in dedicates floodway exclusive of bridge piers.

Refer to plate 36 for typical channel and levee sections.

TRINITY RIVER AND TRIBUTARIES, TEXAS
WEST FORK TRIBUTARIES
FLOOD CONTROL CHANNELS AND FLOODWAYS

DETAILED PROFILES

| IN 2 SHEETS | | OWN | SHEET NO |
|--------------------|------------------|------------------|----------|
| U.S. ARMY ENGINEER | DISTRICT, FORT W | ORTH | JUNE 196 |
| US ARMY ENGINEER | THE STATE OF | 50 177 20 177 | The sat |
| | | ACRES IN THE | |
| 4 h diantry | | FILE TRIN 23 | 0-19 |

159. EIM FORK. - The plan of improvement in the Elm Fork watershed extends along the main channel from its mouth upstream to Grapevine and Lewisville Dams as shown on plates 49 and 50 and on detailed profiles, plates 51 through 53. The project includes the following principal features:

(a) The realignment and enlargement of the Kim Fork channel from Lewisville Dem outlet works channel downstream to the mouth of Denton Creek, a distance of about 7.5 miles, to provide a within-banks channel capacity discharge of at least

10,000 cubic feet per second.

(b) Clearing the existing Denton Greek channel and flood plain to a width of 200 feet from Grapevine Lam outlet works channel downstream to river mile 3.7, a distance of about 7.0 miles, and realignment and enlargement of the channel from river mile 3.7 downstream to the Elm Fork, about 1.9 miles, to provide a within-banks channel capacity discharge of at least 7000 cubic feet per second.

(c) The realignment and enlargement of the ELM Fork channel from Denton Creek downstream to its confluence with the West Fork, about 14.3 miles, to provide a within-banks channel capacity discharge of at least 15,000 cubic feet per second.

(d) The enlargement and realignment or diversion of about 11.2 miles of tributary channels through the leveed areas

between channel miles 4.0 and 12.8 along the Elm Fork.

- through the low areas consisting of about 24.8 miles of levee along the left bank, 14.1 miles of new levee and the rehabilitation of 1.8 miles of existing leves on the right bank of the proposed enlarged and realigned channel of the Elm Fork and tributary channels between channel miles 2.7 and 15.2. The floodway along the main stem would be a dedicated floodway maintained free of encroachments and would have a minimum width of 1,100 feet between centerline of levees and have sufficient capacity to contain the standard project flood which varies from about 50,000 cubic feet per second at the Carrollton gage to about 61,000 cubic feet per second at the mouth of Elm Fork. The levees would be constructed with 1 on 2.5 side slopes with a 10-foot crown width and have 4 feet of freeboard above the design discharge water surface.
- (f) The provision of appartment interior-drainage facilities consisting of permanent sump areas to provide an aggregate storage capacity of 5,260 acre-feet below the damaging-stage elevations of the various interior-drainage areas, gate-controlled gravity sluices at various locations through the levee system, and low interior areas to be filled with excess material to raise damaging elevations.

(g) The modification of 18 bridges and the construction of two new streets in connection with improvements in the Elm Fork

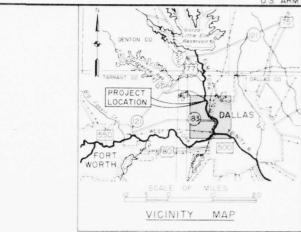
Watershed.

(h) The filling of four areas outside the proposed leveed areas to elevations about 2 feet above design water surface by utilizing the excess material from the channel excavation work.

(i) The relocation and alteration of various urban utilities and of oil, gas, and pover lines of private companies.

(j) The acquisition of rights-of-way, consisting of about 3,294 acres of land in fee simple and 129 acres in clearing easement, for the construction of the excavated and cleared channels, levees, floodway, and permanent sump areas.





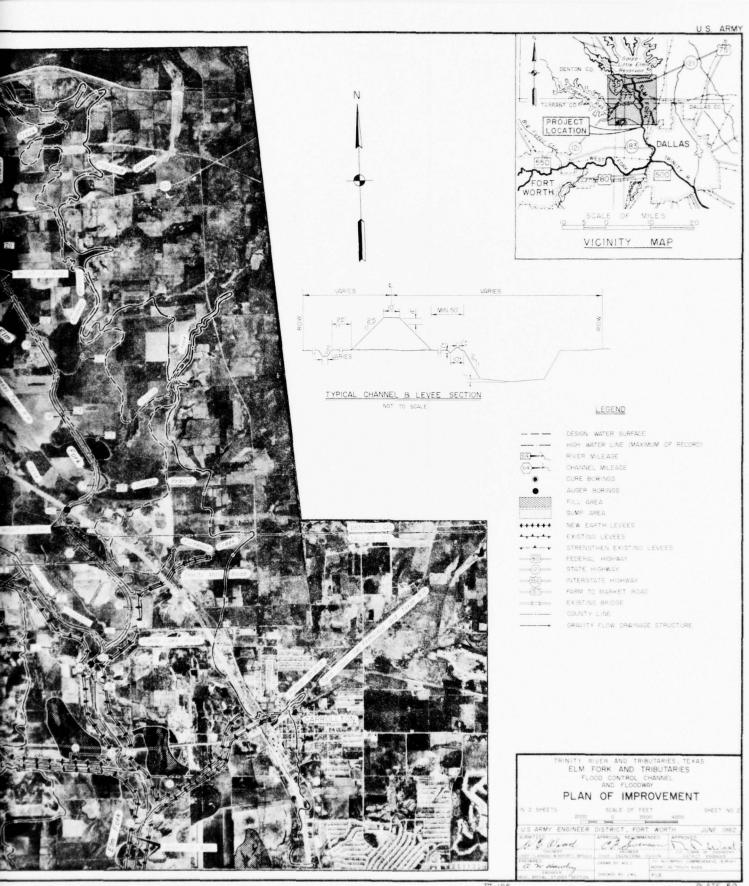
| DRAINAGE | DRAINA | GE AREA | | SLUICE | ES | | St | JMPS | |
|------------------|--------|---------|-------------|---------|--------|-------------------------|--------|---|---|
| AREA AND SUMP | ACRES | SQ. MI. | NO REQ'D | SIZE | INVERT | DAMAGING STAGE EL | BOTTOM | CAP AT DAMAGING STAGE EL AC - FT | DESIGN FLOOD WATER SURFACE EL |
| Д | 905 | 141 | 3 | 5' X 5' | 40 | 415 | 400 | 250 | 414.2 |
| 8 | 2,952 | 4.61 | 6 | 5' X 5 | 401 | 416 | 400 | 800 | 415.9 |
| C | 1,273 | 199 | 3 | 5 X5 | 405 | 421 | 404 | 350 | 421.0 |
| D | 2,381 | 3.72 | 3 | 6' X 6' | 406 | 423 | 405 | 650 | 422.8 |
| E | 3,738 | 5.84 | 5 | 6'X6' | 408 | 425 | 407 | | 424.4 |
| F | 3.724 | 5.82 | 5 | 6' X 6' | 411 | 430 | 410 | | 4286 |
| | 2,720 | 4.25 | 5 | 5' x 5' | 417 | 435 | 416 | 750 | 434.8 |
| H | 1,700 | 2.66 | 3 | 6'X6' | 418 | 437 | 417 | 460 | 435.2 |

HIGH WATER LINE (MAXIMUM OF RECORD) NAVIGATION LOCK AND DAM CORE BORINGS SUMP AREA NEW EARTH LEVEES **** ·--- 60 - 00 - 00 - 00 FEDERAL HIGHWAY STATE HIGHWAY EXISTING BRIDGE GRAVITY FLOW ORAINAGE STRUCTURE

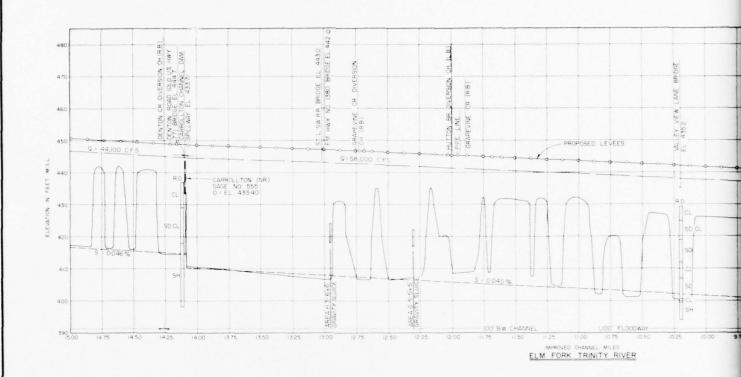
ELM FORK AND TRIBUTARIES, TE ELM FORK AND TRIBUTARIES FLOOD CONTROL CHANNEL AND FLOODWAY

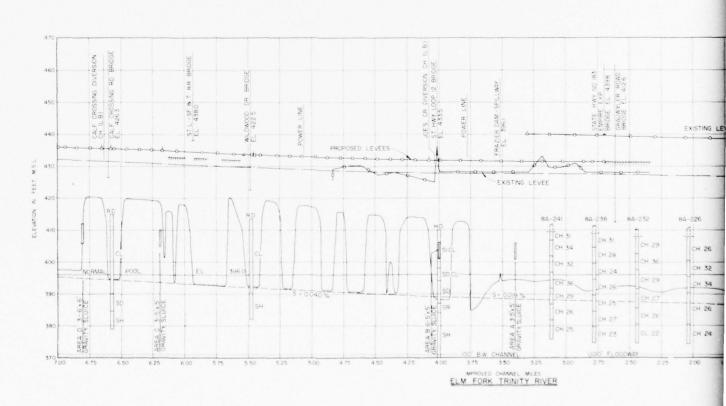
PLAN OF IMPROVEMENT

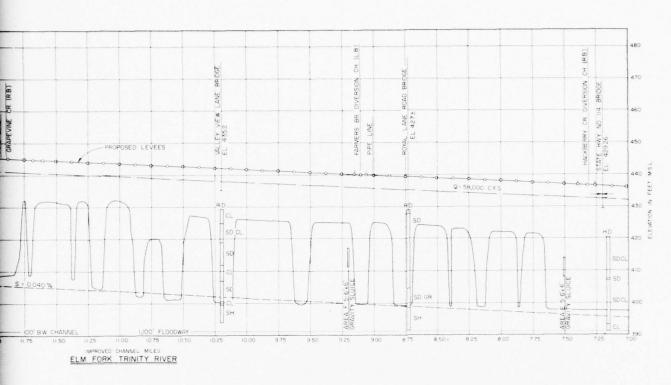
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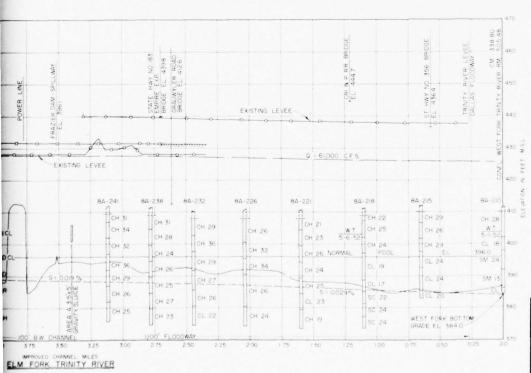












DESIGN WATER SURFACE
LEVEE, RIGHT BANK
LEVEE, LEFT BANK
CENTERLINE PROFILE
BOTTOM GRADE
FILL AREA, RIGHT BANK
BOTTOM WIDTH
LEFT BANK
RIGHT BANK BW LB RB

NOTES cloudway dimensions shown are minimum requirements measured from centerline of levees, or from centerline of levee to natural bank or fill area.

All channel side slopes are I vertical on 2 horizontal. Levee side slopes are I vertical on 2 horizontal. Levee side slopes are I vertical on 2 horizontal. All bridges shown are existing structures Elevations of bridges refer to existing low steel elevations. New or modified bridge openings are to provide 3 feet minimum vertical clearance above design water surface, and unabstructed horizontal clearance in dedicated floodway exclusive of bridge piers.

Refer to plate 55 for bring legend.

Refer to plate 55 for bring legend.

INITY RIVER AND TRIBUTARIES, TEXAS ELM FORK AND TRIBUTARIES D CONTROL CHANNELS AND FLOODWAYS

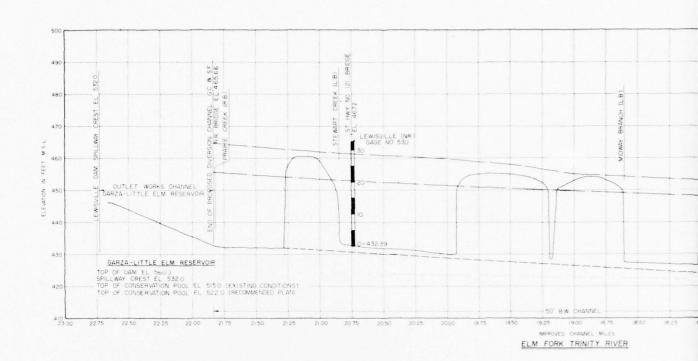
DETAILED PROFILES

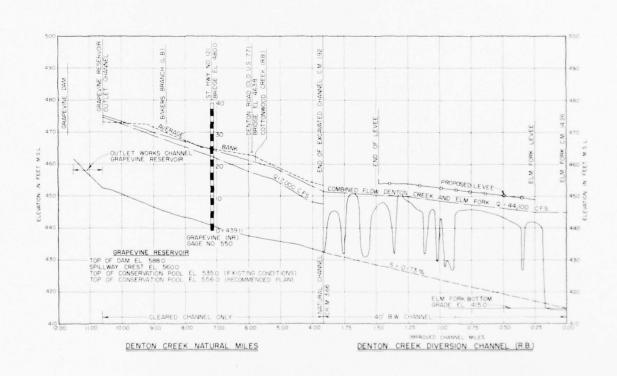
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U.S. ARMY ENGINEER DISTRICT, FORT WORTH JUNE 1962 DISTRICT, FURL WORLH

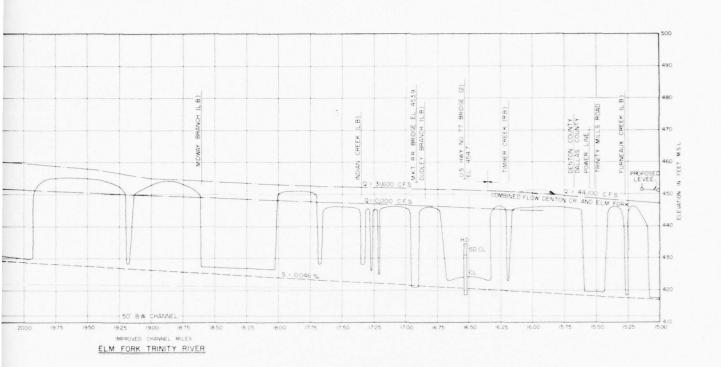
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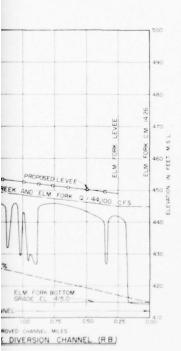
APPROXIMETER TOWN NOD

ONE OF THE THE TOWN TOWN TO ADDRESS TO ADDR WE Wood









DESIGN WATER SURFACE DESIGN WATER SURFACE LEVEE, RIGHT BANK LEVEE, LEFT BANK CENTERLINE PROFILE BOTTOM GRADE FILL AREA, RIGHT BANK BOTTOM WIDTH LEFT BANK RIGHT BANK BW LB RB

NOTES:

NOTES:

Inadway dimensions shown are minimum requirements measured from centerline of levees, or from centerline of levee to natural bank or fill area.

All channel side slopes are I vertical on 2 horizontal Levee side slopes are I vertical on 2½ horizontal Levee side slopes are I vertical on 2½ horizontal All bridges sheller on are existing structures Elevations of bridges refer to existing low siteal elevations. New or modified bridge apenings are to provide 3 feel minimum vertical clearance above design water surface, and unobstructed horizontal clearance in dedicated floodway exclusive of bridge piers.

Refer to plate 55 for boring legend.

Refer to plate 56 for typical channel and levee sections.

TRINITY RIVER AND TRIBUTARIES, TEXAS

ELM FORK AND TRIBUTARIES
FLOOD CONTROL CHANNELS AND FLOODWAYS

DETAILED PROFILES

IN 3 SHEETS SCALES AS SHOWN SHEET NO

US ARMY ENGINEER DISTRICT, FORT WORTH

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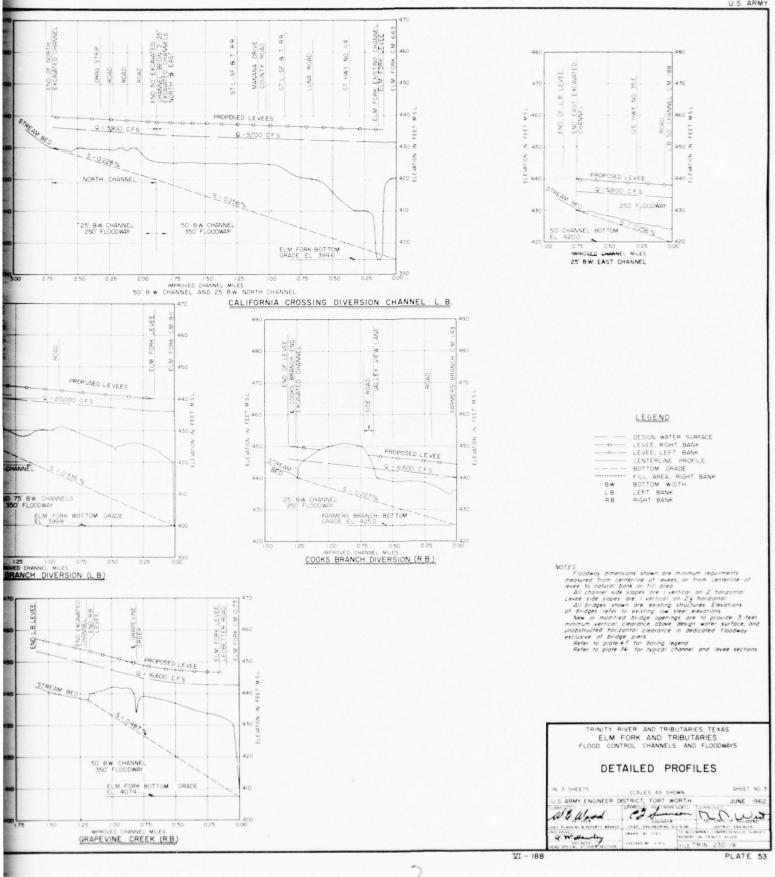
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160. COST ESTIMATE. A summary of first cost and annual charges for the Elm Fork Local Protection project is shown in table 49. The detailed estimate for the Elm Fork Local Protection project is shown in table 50. The estimate shows the separation of Federal and non-Federal costs. The cost of constructing the main item channel of the Elm Fork from Lewisville Dam downstream and on Denton Creek from Grapevine downstream is a Federal cost since it is essential for the proper functioning of these two reservoirs.

TABLE 49 SUMMARY OF FIRST COST AND ANNUAL CHARGES ELM FORK FLOOD PROTECTION PROJECT ELM FORK TRINITY RIVER

| | Iten | : : Cost |
|-----------|---|---|
| ON BLUBON | First cost | |
| 1. | Federal first cost Lands and damages Relocations and alterations Main channel Levees, sluices, sumps, and diversion channels a. Levees b. Sluices c. Sumps d. Diversion channels Engineering and design Supervision and administration Total estimated Federal first cost (1) | 600,000 536,000 3,163,000 1,332,000 885,000 2,416,000 1,317,000 367,000 575,000 11,191,000 |
| 2. | Non-Federal first cost Lands and damages Relocations and alterations Engineering and design Supervision and administration Total estimated non-Federal first cost | 3,203,000 2,211,000 84,000 134,000 5,632,000 |
| 3. | Total estimated first cost of project | 16,823,000 |

TABLE 49 (CONT'D) SUMMARY OF FIRST COST AND ANNUAL CHARGES ELM FORK FLOOD PROTECTION PROJECT ELM FORK TRINITY RIVER

| | Item | : Cost |
|----|--|---|
| | Annual charges (Construction period - 4 years)(10) (2-7/8% Federal interest rate)(3% no | 0 yr. amortization) n-Federal interest rate) |
| 1. | Federal investment a. Federal first cost b. Interest during construction c. Total Federal investment | \$11,191,000 643,500 11,834,500 |
| 2. | Non-Federal investment a. Non-Federal first cost b. Interest during construction c. Total non-Federal investment | 5,632,000 337,900 5,969,900 |
| 3. | Federal annual charges a. Interest on Federal investment b. Amortization charge c. Operation and maintenance Total Federal annual charge | 340,200 21,200 70,000 431,400 |
| 4. | Non-Federal annual charges a. Interest on non-Federal investment b. Amortization charge c. Operation and maintenance Total non-Federal annual charges | 179,100 9,900 103,000 292,000 |
| 5. | Total estimated annual charges | \$ 723,400 |

(1) \$50,000 preauthorization cost excluded

TABLE 50 DETAILED ESTIMATE OF FIRST COST ELM FORK FLOOD PROTECTION PROJECT ETM FORK TRINITY RIVER

| | | | 0 | : Unit | 0 6 | 0 | |
|----|--|------------------------------|---------|------------|------------|--------|---------|
| | | Item | : Unit | : cost | : Quantity | 0 | Cost |
| , | Tindama | 1 Street cont | | | | | |
| Τ. | | l first cost | | | | | |
| | Designation of the last of the | Lands and damages | | | | | |
| | | d costs | | | 01.0 | do | 1.20 0 |
| | | Fee lands | Acre | | 840 | \$ | 420,0 |
| | , , | Clearing easement lands | Acre | | 129 | | 29,0 |
| | (3) | Severance damage | L.S. | | | Cheers | 19,1 |
| | | Subtotal | | | | | 468,1 |
| | h Dee | Contingencies, 25% ± | | | | | 118,3 |
| | | uisition expense | | | | _ | 13,5 |
| | Tota | al - Lands and damages | | | | | 600,0 |
| | (02.0) | Relocations and alterations | | | | | |
| | | ds and railroads | | | | | |
| | (1) | Gravwyler Road | Mile | 150,000.00 | 0.23 | | 31,5 |
| | (2) | Wildwood Drive, bridge | L.F. | 175.00 | | | 8,7 |
| | (3) | | L.F. | 175.00 | 200 | | 35,0 |
| | | Trestle - St.LS.W. R.R. | L.F. | 140.00 | 900 | | 126,0 |
| | (5) | | L.F. | 130.00 | | | 45,5 |
| | (6) | Trestle - spur - St.L | | | | | |
| | | S.F.&T. R.R. | L.F. | 130.00 | 600 | | 78,0 |
| | (7) | Trestle - St.LS.F.&T. R.R. | L.F. | 1.40.00 | 400 | | 56,0 |
| | | Subtotal - roads and railroa | d.s | | | | 380,7 |
| 7 | h II+i | lities and channel dam. | | | | | |
| | | Lone Star Gas Co pipeline | Mile | 100 000 00 | 0.1 | | 10,0 |
| | | Power lines | Mile | 100,000.00 | | | 2,0 |
| | | Remove and rebuild Frazier | MILLE | 10,000.00 | 0.2 | | 2,0 |
| | (3) | channel dam | L.S. | | | | 26 N |
| | | Subtotal - utilities and cha | | *** | | - | 1,8 0 |
| | | bublocal - utilities and tha | iner de | 411 | | | 40,0 |
| | | Subtotal - relocations and a | lterati | ons | | | 428,7 |
| | | Contingencies, 25% | | | | | 107,2 |
| | | Total - relocations and alte | rations | | | Manner | 536,0 |
| | | | | | | | |
| | CONTRACTOR OF THE PARTY OF THE | Main channel | | | | | 1 |
| | | Care of water | L.S. | 700 00 | -0- | | 41,0 |
| | (2) | Clearing (Including berms) | Acre | 100.00 | 785 | | 78,5 |
| | | Excavation, common | C .Y. | 0.25 | | | 1,335,2 |
| | | Excavation, shale | C.Y. | 0.75 | 1,335,500 |] | 1,001,6 |
| | (5) | Slope protection | Acre | 500.00 | 147 | | 73,5 |
| | | Subtotal - main channel | | | | 2 | 2,529,8 |
| | | Contingencies, 25% | | | | | 633,1 |
| | | Total - main channel | | | | | 3,163,0 |

TABLE 50 (Cont'à) DETAILED ESTIMATE OF FIRST COST ELM FORK FLOOD PROTECTION PROJECT ELM FORK TRINITY RIVER

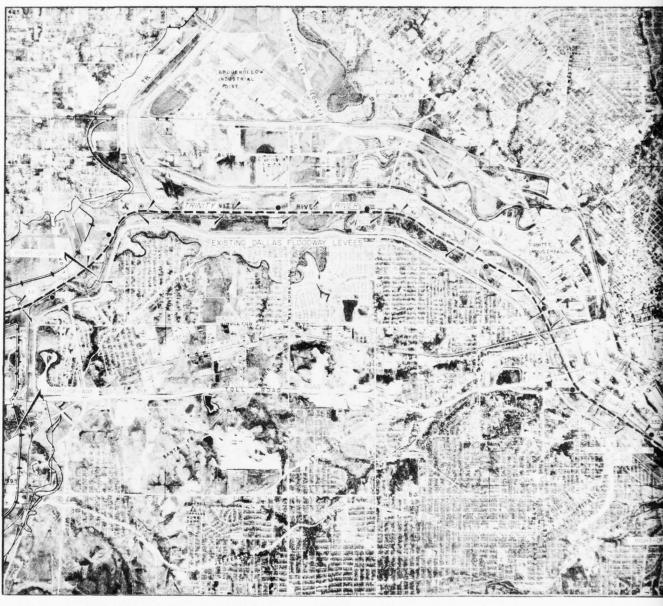
| | 0 0 | Unit | 0 | |
|---|-------------|------------|-------------|------------------|
| Item | : Unit : | Cost | : Quantity | : Cost |
| | | | | |
| (11.0) Levees, sluices, sumps, and | d diversion | 1 channels | | |
| a. Levees | Anna | 100.00 | 840 | \$ 84,0 |
| (1) Clearing berms | Acre | 150.00 | 438 | 65, |
| (2) Clearing and grubbing | Acre | 0.25 | 686,500 | 171,6 |
| (3) Excavation, stripping | C.Y. | 0.07 | 7,492,000 | 524,1 |
| (4) Compacted fill | C.Y. | 500.00 | 439 | 219, |
| (5) Slope protection | Acre | 300.00 | 437 | 1,065,2 |
| Subtotal - levees | | | | 1,00/92 |
| b. Sluices | | | | |
| (1) Care of water | L.S. | | | 10, |
| (2) Excavation, structural | C.Y. | 1.50 | 55,000 | 82, |
| (3) Excavation, channel | C.Y. | 0.30 | 125,000 | 37, |
| (4) Structural backfill | C.T. | 0.50 | 40,000 | 20, |
| (5) Concrete (Incl. cement) | C.Y. | 50.00 | 4,750 | 237, |
| (6) Steel, reinforcing | Ib. | 0.13 | 475,000 | 61, |
| (7) Flapgates, 5'x5' | Ea. | 1,250.00 | 17 | 21, |
| (8) Flapgates, 6'x6' | Ea. | 1,800.00 | 16 | 28, |
| (9) Sluice gates, shafts, & | | | | |
| stands, 5'x5' | Es. | 2,500.00 | 34 | 85, |
| (10) Sluice gates, shafts, & | | | | |
| stands, 6°x6° | Ea. | 3,600.00 | 32 | 115, |
| (11) Miscellaneous metals | Lb. | 0.40 | | 5, |
| (12) Riprap | C.Y. | 5.00 | | 2, |
| (13) Bedding | C.Y. | 5.00 | 117 | |
| Subtotal - sluices | | | | 707, |
| 500000000000000000000000000000000000000 | | | | |
| c. Sumps | | | | 10 |
| (1) Care of water | L.S. | | 000 | 10, |
| (2) Clearing | Acre | 100.00 | | 35, |
| (3) Excavation, common | C.Y. | 0.25 | 7,547,000 | 1,886, |
| Subtotal - sumps | | | | 1,932, |
| d. Diversion channels | | | | |
| (1) Care of water | L.S. | | | 10, |
| (2) Clearing | Acre | 100.00 | | 60, |
| (3) Excavation, common | C.Y. | | 2,140,000 | 535, |
| (4) Excavation, shale | C.Y. | 0.75 | | 401, |
| (5) Slope protection | Acre | 500.00 | 94 | 47, |
| Subtotal - diversion chann | | | | 1,053, |
| | | | | 1 |
| Subtotal - levees, sluices | , sumps, s | na diversi | on channels | 4,758, |
| Contingencies, 25% | | | | 1,191, 5,950, |
| Total - levees, sluices, s | | | | |

TABLE 50 (CONT'D) DETAILED ESTIMATE OF FIRST COST ELM FORK FLOOD PROTECTION PROJECT ELM FORK TRINITY RIVER

| Item | : : Unit | * | Unit: cost: | Quantit | y: Cost | t |
|---|--|-----|--|---|---|--|
| (30.0) Engineering and design (31.0) Supervision and administration Total - estimated Federal first | | (1) | | | \$367,00 575,00 11,191,00 | 00 |
| 2. Non-Federal first cost a. Land and damages (1) Fee simple lands (2) Severance damage (3) Improvements Contingencies, 25% Acquisition expense Total - lands and damages | Acre L.S. L.S. | | | 2,454 | 2,208,60 55,90 255,00 644,00 39,50 3,203,00 | 00 |
| b. Relocations and alterations bridge roads, and power lines Gravwyler Road, new street Wildwood Dr., bridge California crossing, bridge State Hwy. 14, bridge Royal Lane, bridge Valley View Lane, bridge Belt Line Road, bridge Sandy Lake Road, bridge Ledbetter Road, new road Ledbetter Road, bridge Luna Road, bridge Power lines Subtotal - relocations and alteration Contingencies, 25% Total - relocations and alteration | Mile L.F. L.F. L.F. L.F. L.F. Mile L.F. Mile | 80 | ,000.00 175.00 175.00 250.00 175.00 175.00 175.00 ,000.00 175.00 175.00 | 750 1,250 1,600 1,150 1,300 1,100 1,200 0.5 350 400 | 131,25 218,75 400,00 201,25 227,50 192,50 210,00 40,00 | 50 50 50 50 60 60 60 60 60 60 60 60 60 60 60 60 60 |
| c. Engineering and design | | | | | 84,00 | 00 |
| d. Supervision and administration | | | | | 134,00 | |
| Total - estimated non-Federal fire | st cos | Ü | | | 5,632,00 | 00 |
| 3. Total - Estimated first cost of proje | ect | | | \$ | 16,823,00 | 00 |

^{(1) \$50,000} preauthorization cost excluded. Prices are as of January 1962.

- 161. EXTENSION OF DALIAS FLOODWAY. An element of the multiple purpose channel and floodway for navigation and flood control in the Dallas area is the extension of the existing Dallas Floodway downstream to Five Mile Creek, including the lower end of White Rock Creek. The navigation features of the multiple purpose channel are covered in the first paragraph of the appendix. The flood control features of this project include the following items:
- (a) Sufficient enlargement and realignment of the Trinity River in this area, from about river mile 496.3 downstream to river mile 487.7, to provide within-banks channel capacity of at least 27,000 cubic feet per second.
- (b) The enlargement and realignment or diversion of about 10.5 miles of tributary channels through the leveed areas.
- (c) The construction of a parallel levee system through the low areas consisting of about 10.2 miles of levee on the left bank, 14.6 miles of new levee and the rehabilitation of 0.6 mile of existing levee on the right bank of the proposed enlarged and realigned channel of the main stem and tributary channels. The plan provides for a dedicated floodway maintained free of encroachments. The floodway along the main stem would have a minimum width of 2,000 feet between centerline of levees and have sufficient capacity to contain the standard project flood which varies from about 163,000 cubic feet per second at Dallas to about 171,000 cubic feet per second in the lower reach just upstream from the mouth of Five Mile Creek. The levees would be constructed with 1 on 2.5 side slopes, with a 10-foot crown width and have 4 feet of free-board above the design discharge water surface.
- (d) The provision of appurtenant interior drainage facilities consisting of permanent sump areas to provide an aggregate storage capacity of 6,820 acre-feet below the damaging stage elevations of the various interior-drainage areas, gate controlled gravity sluices at various locations through the levee system, and low interior areas to be filled with excess material to raise damaging elevations.
- (e) The modification of 13 bridges in connection with the proposed improvements for the extension of the Dallas Floodway.
- (f) The filling of one area outside the proposed leveed areas to elevations about 4 feet above the design water surface by utilizing the excess material from the channel excavation work.
- (g) The acquisition of rights-of-way consisting of about 4,030 acres of land in fee simple for the construction of the excavated channels, levees, floodway, and permanent sump area.
- (h) The plan of improvement is shown on plates 54 and 56, and the detailed profiles are shown on plates 55 and 57.







NAVIGATION AND FLOOD CONTROL CHANNEL SESSEN WATER SURFACE
HIGH WATER LINE LIMIXIMUM OF RECORD NAVIGATION LOCK AND DAM

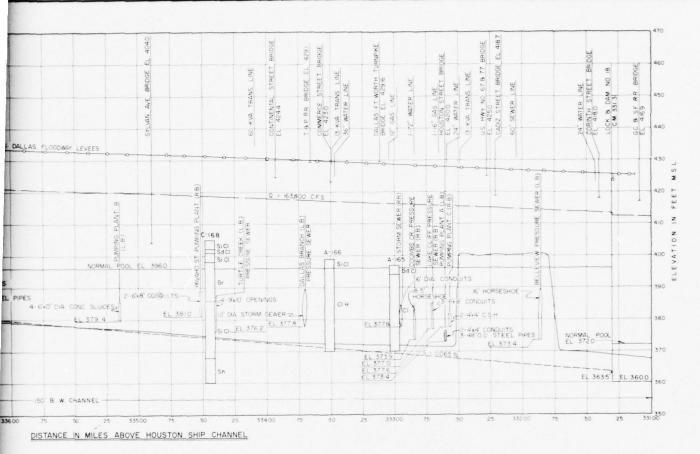
EAT RIVER WILEAGE
CHANNEL WILEAGE
CHANNEL WILEAGE
CHANNEL WILEAGE
CORE BORINGS
FILL AREA
SUMP AREA
SUMP AREA
SUMP AREA
EXISTING LEVEES
EXISTING LEVEES
FECERAL HIGHWAY
WITERSTATE HIGHWAY
STATE HIGHWAY
EATTH LEVEES
EXISTING BRIDGE
COUNTY LINE
GRAVITY FLOW DRAINAGE STRUCTURE

TRINITY RIVER AND TRIBUTARIES, TEXAS DALLAS FLOODWAY MULTIPLE PURPOSE CHANNEL

PLAN OF IMPROVEMENT

US ARMY ENGINEER DESTRICT, FORT WORTH

A MANUAL PROPERTY OF THE PR



NOTES
All channel side slopes are I vertical on 2 horizontal.
All bridges shown are existing structures. Elevations of bridges refer to existing low steel elevations.
Pumping plants and drainage structures shown are those of the existing Dallas Floodway.
Refer to plate 45 for boring legend

TRINITY RIVER AND TRIBUTARIES, TEXAS TRINITY RIVER MULTIPLE PURPOSE CHANNEL

DETAILED PROFILE

DALLAS FLOODWAY

SCALES AS SHOWN

U.S. ARMY ENGINEER DISTRICT, FORT WORTH

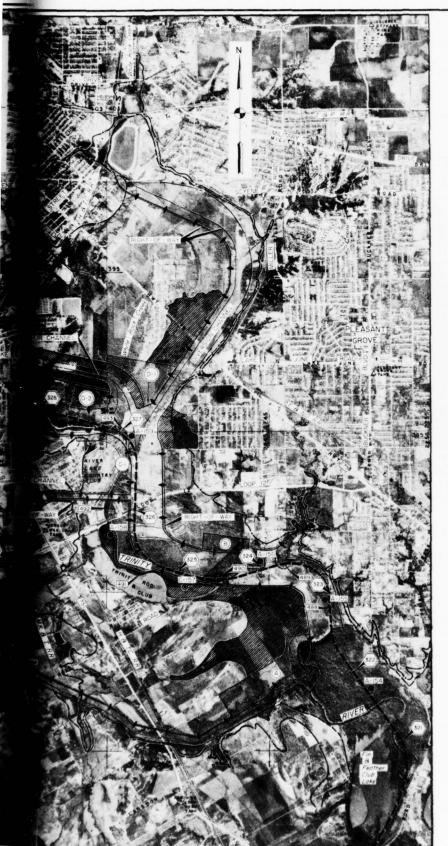
U.S. ARMY ENGINEER DISTRICT

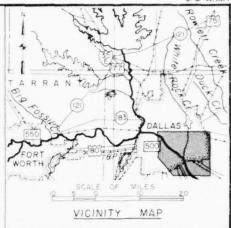
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VI - 196

PLATE 55



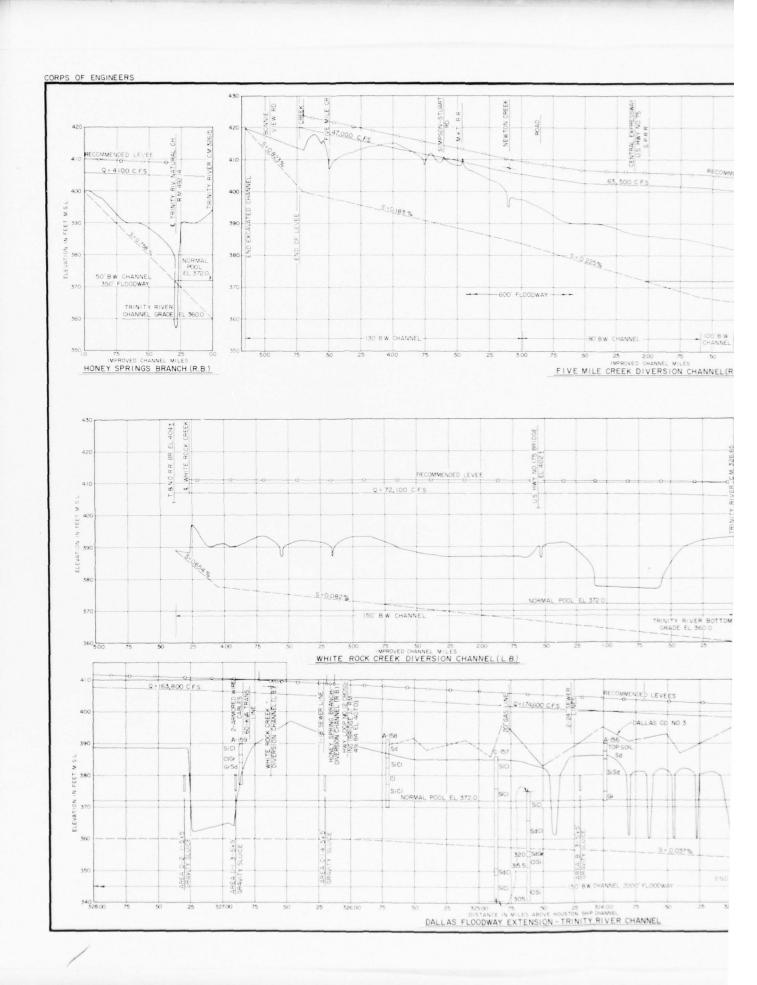


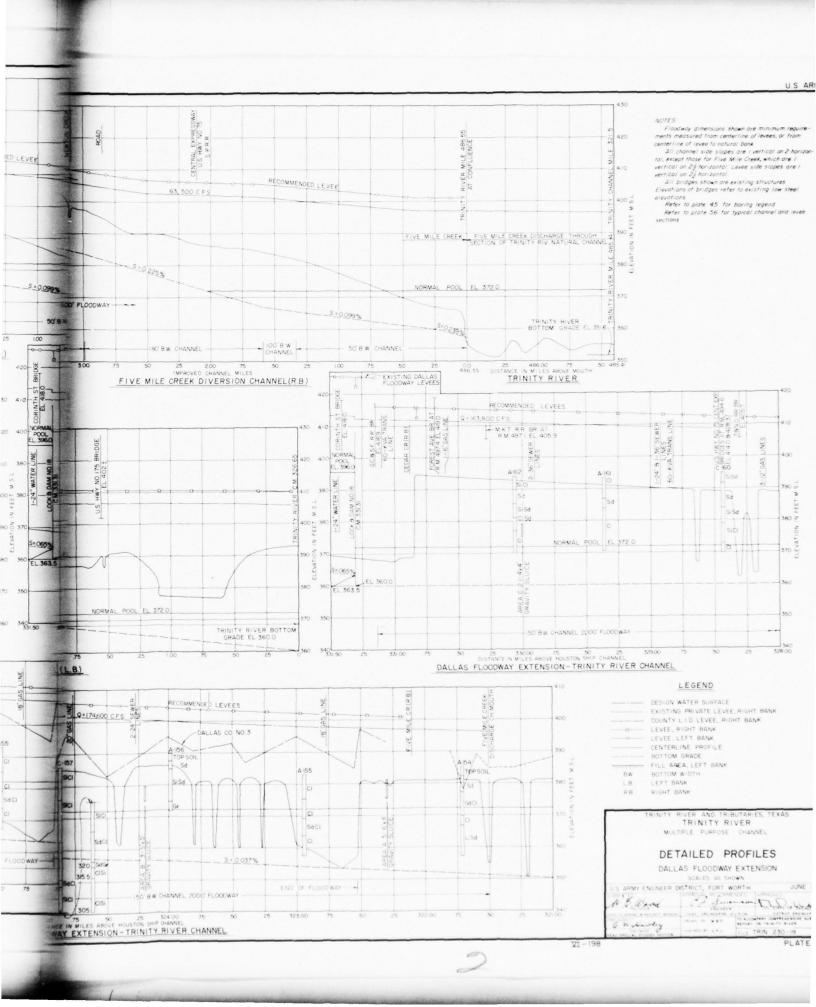
| DRAINAGE | | GE AREA | | | | | | SU | MPS | | |
|------------------|-------|---------|-------------|------|---|--------------|-------------------------|---------------|---|---|--|
| AREA AND SUMP | ACRES | SQ MI | NO REQ'D | Siz | | INVERT EL | DAMAGING STAGE EL | BOT TOM EL | CAP AT DAMAGING STAGE EL AC-FT | DESIGN FLOOD WATER SURFACE EL | |
| A | 5,114 | 799 | 5 | 6 X | 6 | 375 | 385 | 374 | 2,250 | 385.0 | |
| 8 | 1,376 | 215 | 3 | 5 X | 5 | 375 | 385 | 374 | 620 | 3845 | |
| | 2,411 | 3.77 | 4. | 5 X | 5 | 375 | 390 | 374 | | 389.0 | |
| C-2 | 535 | 0.84 | | 4 X | 4 | 375 | 400 | 374 | 250 | 3932 | |
| | 3,736 | 584 | 3 | 5' X | 5 | 375 | 390 | 374 | 1,800 | 3897 | |
| 0-2 | 1,634 | 2.55 | | 5 X | 5 | 375 | 390 | 374 | 800 | 388.0 | |

NAVIGATION AND FLOOD CONTROL CHANNEL DESIGN WATER SURFACE HIGH WATER LINE (MAXIMUM OF RECORD) NAVIGATION LOCK AND DAM - FF RIVER MILEAGE CHANNEL MILEAGE CORE BORINGS AUGER BORINGS SUMP AREA NEW EARTH LEVEES EXISTING LEVEES ----(80)-FEDERAL HIGHWAY STATE HIGHWAY INTERSTATE HIGHWAY FARM TO MARKET ROAD EXISTING BRIDGE COUNTY LINE GRAVITY FLOW DRAINAGE STRUCTURE

> RINITY RIVER AND TRIBUTARIES, TEXAS DALLAS FLOODWAY EXTENSION MULTIPLE PURPOSE CHANNEL

PLAN OF IMPROVEMENT





162. COST ESTIMATE. A summary of first cost and annual charges for the Dallas Floodway Extension project is shown in table 51. The detailed cost estimate for the Dallas Floodway Extension project is shown in table 52. The estimate shows the separation of Federal and non-Federal costs and does not include the flood-control portion of the multiple purpose channel.

TABLE 51 SUMMARY OF FIRST COSTS AND ANNUAL CHARGES DALLAS FLOODWAY EXTENSION FLOOD PROTECTION PROJECT TRINITY RIVER

| | Item | : | Cost | |
|----|--|----|--|--|
| | First cost | | | |
| 1. | Federal first cost Railroad alterations (On diversion channels) Levees, sluices, sumps, and diversion channels a. Levees b. Sluices c. Sumps d. Diversion channels Engineering and design Supervision and administration Total estimated Federal first cost (1) | \$ | 195,000 1,137,000 645,000 3,511,000 2,665,000 310,000 486,000 8,949,000 | |
| 2. | Non-Federal first cost Lands and damages Relocations and alterations Engineering and design Supervision and administration | _ | 4,000,000 1,256,000 48,000 74,000 | |
| | Total estimated non-Federal first cost | | 5,378,000 | |
| 3. | Total estimated first cost of project | \$ | 14,327,000 | |

TABLE 51 (CONT'D) SUMMARY OF FIRST COSTS AND ANNUAL CHARGES DALLAS FLOODWAY EXTENSION FLOOD PROTECTION PROJECT TRINITY RIVER

| | Item | : : Cost |
|----|--|--|
| | Annual charges | |
| | (Construction period - 4 years)(100 yr. amorti (2-7/8% Federal interest rate)(3% non-Federal in | zation) terest rate) |
| 1. | Federal investment a. Federal first cost b. Interest during construction c. Total Federal investment | \$8,949,000 514,600 9,463,600 |
| 2. | Non-Federal investment a. Non-Federal first cost b. Interest during construction c. Total non-Federal investment | 5,378,000 322,700 5,700,700 |
| 3• | a. Interest on Federal investment b. Amortization charge c. Operation and maintenance | 272,100 16,900 |
| 4. | Non-Federal annual charges a. Interest on non-Federal investment b. Amortization on charge c. Operation and maintenance | 289,000 171,000 9,400 135,500 |
| 5. | Total non-Federal annual charges Total estimated annual charges | 315,900 604,900 |

(1) \$30,000 preauthorization cost excluded.

TABLE 52

DETAILED ESTIMATE OF FIRST COST
DALLAS FLOODWAY EXTENSION FLOOD PROTECTION PROJECT
TRINITY RIVER

| | | | 6 | | : | Unit | : | | : | |
|----|-------|-------------------------------------|-----|--------|-----|----------|-----|-----------|------|------|
| | | Item | : | Unit | : | cost | : | Quantity | : | nem. |
| | | | | | | | | | | |
| 1. | | al first cost | | | | | | | | |
| | | Railroad alterations | | | | 4.00 | | | | |
| | | M.K.TR.R., trestle | | L.F. | | \$130.00 | | 1,000 | \$ | |
| | (2) | S.PR.R., trestle | | L.F. | | 130.00 | | 200 | | - |
| | | Subtotal-railroad alterat | io | ns | | | | | | |
| | | Contingencies, 25% | 010 | 110 | | | | | | |
| | | contingencies, 276 | | | | | | | **** | - |
| | | Total - railroad alterati | on | S | | | | | | |
| | (11.0 |) Levees, sluices, sumps, | an | d dive | ers | ion char | ine | ls | | |
| | a. L | | | | | | | - | | |
| | (1) | Clearing main stem berms | | Acre | | 100.00 | | 1,500 | | |
| | | Clearing and grubbing | | Acre | | 150.00 | | 378 | | |
| | | Excavation, stripping | | C.Y. | | 0.25 | | 51.1,000 | | |
| | | Compacted fill | | C.Y. | | 0.07 | | 5,805,000 | | |
| | | Slope protection | | Acre | | 500.00 | | 338 | | |
| | , , , | | | | | | | | **** | |
| | | Subtotal - levees | | | | | | | | |
| | | | | | | | | | | |
| | | luices | | T 0 | | | | | | |
| | | Care of water | | L.S. | | 1 50 | | 27 000 | | |
| | | Excavation, structural | | C.Y. | | 1.50 | | 37,000 | | |
| | (3) | Excavation, discharge | | G 37 | | 0.20 | | lal coo | | |
| | (1.) | channels | | C.Y. | | 0.30 | | 424,000 | | |
| | | Structural backfill | | C.Y. | | 0.50 | | 31,000 | | |
| | | Concrete (Incl. cement) | | C.Y. | | 50.00 | | 2,960 | | |
| | | Steel, reinforcing | | Lb. | | 0.13 | | 296,000 | | |
| | | Flap gates, 4'x4' | | Ea. | - | 800.00 | | 1 | | |
| | | Flap gates, 5'x5' | | Ea. | | ,250.00 | | 11 | | |
| | | Flap gates, 6'x6' | | Ea. | 1 | ,800.00 | | 5 | | |
| | (10) | Sluice gates, shafts & | | 77- | - | (00.00 | | | | |
| | (22) | stands -4'x4' | | Ea. | 1 | ,600.00 | | 5 | | |
| | (TT) | Sluice gates, shafts & | | | | F00 00 | | 00 | | |
| | (10) | stands-5'x5' | | Ea. | C | ,500.00 | | 55 | | |
| | (12) | Sluice gates, shafts & stands-6'x6' | | 77- | - | 600 00 | | 3.0 | | |
| | (12) | | | Ea. | 3 | ,600.00 | | 10 | | |
| | | Miscellaneous metals | | Lb. | | 0.40 | | 5,300 | | |
| | | Riprap | | C.Y. | | 6.00 | | 170 | | |
| | (15) | Bedding | | C.Y. | | 5.00 | | 70 | *** | |
| | | Subtotal - sluices | | | | | | | | |
| | | | | VI-20 |)] | | | | | |
| | | | | - | | | | | | |

TABLE 52 (Cont'd) DETAILED ESTIMATE OF FIRST COST DALLAS FLOODWAY EXTENSION FLOOD PROTECTION PROJECT TRINITY RIVER

| | | | 6 | - | | Unit | 0 | | : |
|----|-----------|---|--------|--------|-----|----------|-----|--------------|----|
| | | Item | 0 a | Unit | 0 | cost | : | Quantity | :_ |
| | c. | Sumps | | | | | | | |
| | | (1) Care of water | | L.S. | | | | | |
| | | (2) Clearing | | Acre | | 100.00 | | 489 | |
| | | (3) Excavation, common | | C.Y. | | 0.25 | | 11,010,000 | _2 |
| | | Subtotal - sumps | | | | | | | 2 |
| | d. | Diversion channels | | | | | | | |
| | | (1) Care of water | | L.S. | | | | | |
| | | (2) Clearing | | Acre | | 100.00 | | 345 | |
| | | (3) Excavation, common | | C.Y. | | 0.25 | | 4,030,000 | 1 |
| | | (4) Excavation, shale | | C.Y. | | 0.75 | | 1,345,000 | 1 |
| | | (5) Slope protection | | Acre | | 500.00 | | 149 | |
| | | Subtotal-diversion | char | nels | | | | | 2 |
| | | Subtotal-levees, sl Contingencies, 25% | | es, sw | nps | and div | ers | ion channels | 6 |
| | | Total-levees, sluid | es, | sumps | an | d divers | ion | channels | 7 |
| | (30 | .0) Engineering and desig | n | | | | | | |
| | (31 | .0)Supervision and admin | istı | ation | | | | | |
| | | Subtotal-estimated Fe | dera | l fir | st | cost (1) | | | 8 |
| 2. | Non a. | -Federal first cost Lands and damages (1) Fee simple lands an | d | | | | | | |
| | | improvements Contingencies, 25% | + | Acre | | | | 4,032 | 3 |
| | | Subtotal-lands and | | ages | | | | | 3 |
| | | (2) Acquisition expense Total-lands and da | | S | | | | | -1 |

TABLE 52 (CONT'D) DETAILED ESTIMATE OF FIRST COST DALLAS FLOODWAY EXTENSION FLOOD PROTECTION PROJECT TRINITY RIVER

| | : | 0 | Unit | : | | 0 | |
|--|---|--|--|---|--|--|---|
| Item | :Unit | 0 0 0000-Minus | cost | | Quantity | | Cost |
| Relocations and alterations Bridges | | | | | | | |
| Simpson-Stuart Rd. bridge Interstate Hwy #45, | L.F. | | 175.0 | 0 | 600 | \$ | 105,000 |
| bridge | L.F. | | 500.0 | 0 | 800 | | 400,000 |
| U.S. Hwy #75, bridge | L.F. | | 500.0 | 0 | 1,000 | No. Proper | 500,000 |
| Subtotal - bridges | | | | | | | 1,005,000 |
| Contingencies, 25% | | | | | | man | 251,000 |
| Total - relocations and alter | rations | | | | | | 1,256,000 |
| Engineering and design | | | | | | | 48,000 |
| Supervision and administration | n | | | | | | 74,000 |
| Total estimated non-Federal f | Pirst c | os | t | | | | 5,378,000 |
| al estimated first cost of pro | ject | | | | | | 14,327,000 |
| | Relocations and alterations Bridges Simpson-Stuart Rd. bridge Interstate Hwy #45, bridge U.S. Hwy #75, bridge Subtotal - bridges Contingencies, 25% Total - relocations and alter Engineering and design Supervision and administration Total estimated non-Federal for | Relocations and alterations Bridges Simpson-Stuart Rd. bridge L.F. Interstate Hwy #45, bridge L.F. U.S. Hwy #75, bridge L.F. Subtotal - bridges Contingencies, 25% Total - relocations and alterations Engineering and design Supervision and administration | Relocations and alterations Bridges Simpson-Stuart Rd. bridge L.F. Interstate Hwy #45, bridge L.F. U.S. Hwy #75, bridge L.F. Subtotal - bridges Contingencies, 25% Total - relocations and alterations Engineering and design Supervision and administration Total estimated non-Federal first cos | Relocations and alterations Bridges Simpson-Stuart Rd. bridge L.F. 175.0 Interstate Hwy #45, bridge L.F. 500.0 U.S. Hwy #75, bridge L.F. 500.0 Subtotal - bridges Contingencies, 25% Total - relocations and alterations Engineering and design Supervision and administration Total estimated non-Federal first cost | Relocations and alterations Bridges Simpson-Stuart Rd. bridge L.F. 175.00 Interstate Hwy #45, bridge L.F. 500.00 U.S. Hwy #75, bridge L.F. 500.00 Subtotal - bridges Contingencies, 25% Total - relocations and alterations Engineering and design Supervision and administration Total estimated non-Federal first cost | Relocations and alterations Bridges Simpson-Stuart Rd. bridge L.F. 175.00 600 Interstate Hwy #45, bridge L.F. 500.00 800 U.S. Hwy #75, bridge L.F. 500.00 1,000 Subtotal - bridges Contingencies, 25% Total - relocations and alterations Engineering and design Supervision and administration Total estimated non-Federal first cost | Relocations and alterations Bridges Simpson-Stuart Rd. bridge L.F. 175.00 600 \$ Interstate Hwy #45, bridge L.F. 500.00 800 U.S. Hwy #75, bridge L.F. 500.00 1,000 Subtotal - bridges Contingencies, 25% Total - relocations and alterations Engineering and design Supervision and administration Total estimated non-Federal first cost |

^{(1) \$30,000} preauthorization cost excluded.

- 163. DUCK CREEK. The plan of improvement for Duck Creek consists of a channel improvement project along the main channel in the vicinity of Garland, Texas, from river mile 10.38 upstream to river mile 17.53 as shown on plate 58 and the detailed profile, plate 59. The project includes the following principal features:
- (1) The realignment and enlargement of 6.6 miles of Duck Creek channel including 0.58 mile of concrete gravity wall section, to provide sufficient within-banks capacity to contain the standard project flood which varies from about 21,500 cubic feet per second at the upstream end near Buckingham Road to 40,700 cubic feet per second at the downstream end near Oates Drive.
 - (2) The modification of 10 bridges.
- (3) The acquisition of rights-of-way for the construction of the excavated channel and for disposal of excess material.
- 164. COST ESTIMATES. A summary of first cost and annual charges for the Duck Creek Local Protection project is shown in table 53. The detailed estimate for the project is shown in table 54. The estimate shows the separation of Federal and non-Federal costs as generally specified for such projects.

TABLE 53 SUMMARY OF FIRST COSTS AND ANNUAL CHARGES LOCAL FLOOD PROTECTION PROJECT - GARLAND, TEXAS DUCK CREEK

| Item | Cost |
|---------------------------------------|------------------|
| First c | ost |
| 1. Federal first cost | |
| Railroad alterations | \$ 236,000 |
| Channel | 3,568,000 |
| Engineering and design | 145,000 |
| Supervision and administration | 227,000 |
| Total - estimated Federal first co | st (1) 4,176,000 |
| 2. Non-Federal first cost | |
| Lands and damages | 443,000 |
| Relocations and alterations | 369,000 |
| Engineering and design | 14,000 |
| Supervision and administration | 22,000 |
| Total - estimated non-Federal firs | t cost 848,000 |
| 3. Total estimated first cost of proj | ect \$5,024,000 |

TABLE 53 (CONT'D) SUMMARY OF FIRST COSTS AND ANNUAL CHARGES LOCAL FLOOD PROTECTION PROJECT - GARLAND, TEXAS DUCK CREEK

| and the American | Item | Cost |
|------------------|---|----------------------|
| | Annual charges | |
| | (Construction period - 2 years) (100 yr. (2-7/8% Federal interest rate) (3% non-F | amortization) |
| | (2-7/8% Federal interest rate) (3% non-F | ederal interest rate |
| . Fe | ederal investment | |
| a | . Federal first cost | \$4,176,000 |
| | . Interest during construction | None |
| | . Total - Federal investment | 4,176,000 |
| . No | on-Federal investment | |
| a | Non-Federal first cost | 848,000 |
| b. | Interest during construction | None |
| C. | . Total - non-Federal investment | 848,000 |
| . Fe | ederal annual charges | |
| | Interest on Federal investment | 120,100 |
| ъ. | . Amortization charge | 7,500 |
| C. | Operation and maintenance | None |
| | Total - Federal annual charge | 127,600 |
| . No | on-Federal annual charges | |
| | Interest on non-Federal investment | 25,400 |
| b. | · Amortization charge | 1,400 |
| C. | Operation and maintenance | 6,000 |
| | Total - non-Federal annual charges | 32,800 |
| . To | otal - Estimated annual charges | \$ 160,400 |

(1) \$10,000 preauthorization cost excluded

3

TABLE 54 DETAILED ESTIMATE OF FIRST COST LOCAL FLOOD PROTECTION PROJECT - GARLAND, TEXAS DUCK CREEK

| | : | | Unit : | | - |
|----------------------------|----------|---------|----------|-----------|-----------|
| Item | : 1 | Unit: | cost : | Quantity: | Cost |
| l. Federal first cost | | | | | |
| (02.0) Railroad alteration | ans | | | | |
| (1) GC&SF RR | | T. F. | \$350.00 | 240 | \$ 84,000 |
| (2) MKT RR | | | 350.00 | 300 | 105,000 |
| Subtotal-rail: | road | 201 0 | 370.00 | 300 | =0),00 |
| alterations | Loud | | | | 189,000 |
| Contingencies | 25% + | | | | 47,000 |
| Total - railro | | eration | ns | | 236,000 |
| (09.0) Channel | | | | | 3-, |
| (1) Care of water | | L.S. | | | 30,00 |
| (2) Clearing | | Acre | 150.00 | 185 | 27,75 |
| (3) Excavation, co | ommon | C.Y. | 0.50 | 868,000 | 434,00 |
| (4) Excavation, re | | C.Y. | 2.25 | 790,000 | 1,777,50 |
| (5) Drilling & gro | outing | | | | |
| anchor holes | | L.F. | 1.60 | 10,200 | 16,32 |
| (6) Line drilling | | S.F. | 1.25 | 66,300 | 82,87 |
| (7) Concrete | | C.Y. | 35.00 | 9,170 | 320,95 |
| (8) Cement | | Bbl. | 5.00 | 11,460 | 57,30 |
| (9) Steel, reinfor | | Lb. | 0.13 | 755,000 | 98,150 |
| (10) Slope protect: | | Acre | 300.00 | 33 | 9,90 |
| Subtotal - cha | | | | | 2,854,74 |
| Contingencies | | | | | 713,25 |
| Total - channe | | | | | 3,568,00 |
| (30.0) Engineering and de | | | | | 145,00 |
| (31.0) Supervision and ac | | | | | 227,000 |
| Subtotal - estimat | ted Fede | eral fi | rst cost | | 4,176,00 |
| Non-Federal first cost | | | | | |
| a. Lands and damages | , | | | | |
| (1) Fee simple lands | and | Acres | | 190 | 250 (0 |
| improvements | 350,60 | | | | |
| Subtotal - lands | 350,60 | | | | |
| Contingencies, 259 | | | | | 87,70 |
| (2) Acquisition exper | | | | | 4,70 |
| Total - lands and | damage | es | | | 443,000 |

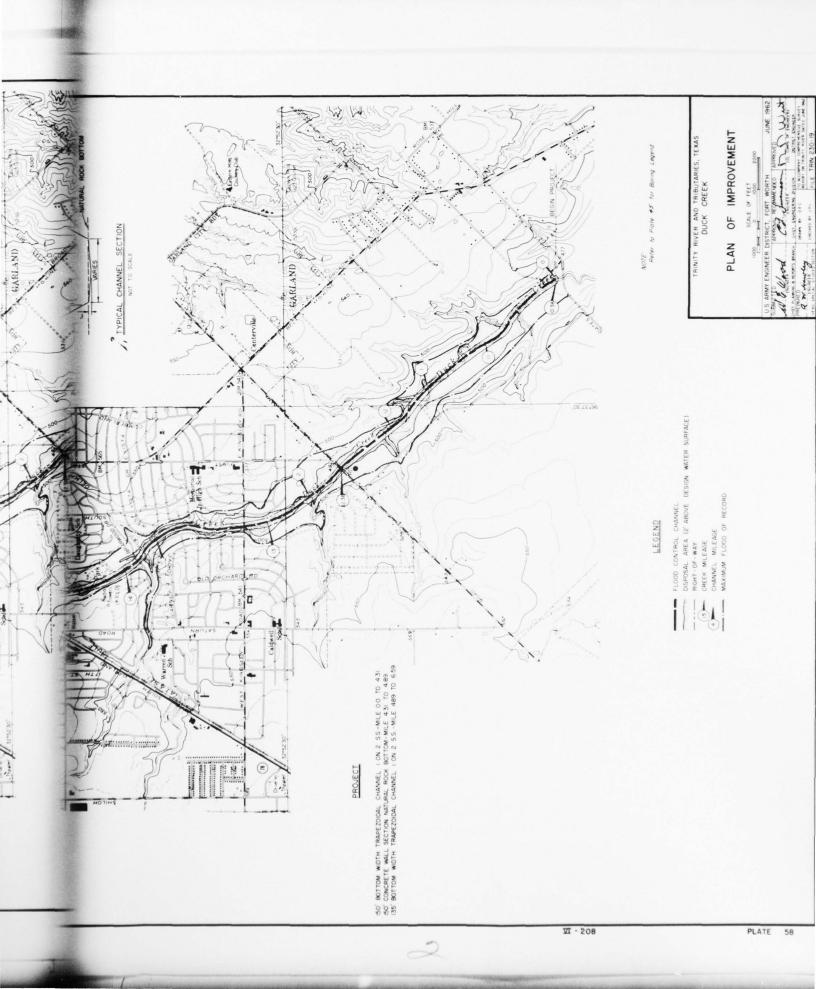
TABLE 54 (Cont'd) DETAILED ESTIMATE OF FIRST COST LOCAL FLOOD PROTECTION PROJECT - GARLAND, TEXAS DUCK CREEK

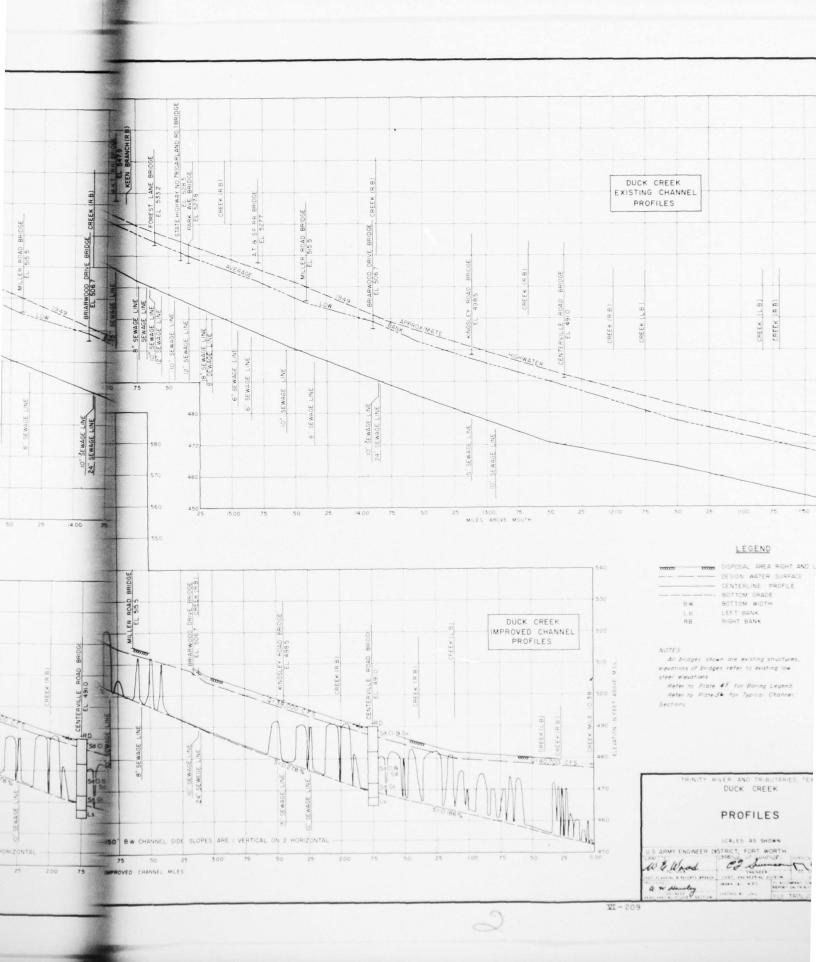
| | : | Unit | | 0 | |
|---|--------|----------|----------|----|----------|
| Item | :Unit: | cost | :Quantit | у: | Cost |
| b. Relocations and alterations | | | | | |
| Bridges and utilities | | | | | |
| (1) Centerville Rd, bridge | L.F. | \$200.00 | \$ 210 | \$ | 42,00 |
| (2) Kingsley Rd, bridge | L.F. | 200.00 | 200 | | 40,00 |
| (3) Briarwood Dr, bridge | L.F. | 175.00 | 210 | | 36,75 |
| (4) Miller Rd, bridge | L.F. | 200.00 | 145 | | 29,00 |
| (5) Park Ave, bridge | L.F. | 300.00 | 80 | | 24,00 |
| (6) Carland Rd, bridge | L.F. | 200.00 | 210 | | 42,00 |
| (7) Forest Lane, bridge | L.F. | 350.00 | 75 | | 26,25 |
| (8) Walnut St, bridge | L.F. | 200.00 | 130 | | 26,00 |
| (9) Telephone cable | L.F. | 32.00 | 250 | | 8,00 |
| (10) Rearrange sewer laterals | L.S. | | | | 21,00 |
| Subtotal - | | | | | 295,00 |
| Contingencies, 25% ± | | | | | 74,00 |
| Total - relocations and a | | 369,00 | | | |
| c. Engineering and design | | | | | 14,00 |
| d. Supervision and administration | | | | | 22,00 |
| Subtotal - estimated non-Federal first cost | | | | | 848,00 |
| Total - Estimated first cost of | projec | t | | \$ | 5,024,00 |

Note: Unit prices for excavation sufficient for disposal of excavated material and placement of all riprap required. It is proposed to supply riprap from rock excavation.

Prices are as of January 1962







165. LIBERTY.- The plan of improvement in the vicinity of Liberty consists of levees with appurtenant interior drainage facilities extending along the left bank of the Trinity River as shown on plate 60. The water surface and levee profiles are shown on plate 61. The project includes the following principal features.

60

50

530

490

480

460

FT BANK

- (1) Enlargement and realignment of the Trinity River to provide within-banks channel capacity of 45,000 cubic feet per sefor the multiple purpose channel project.
- (2) Construction of about 53,500 feet of levee of whice approximately 31,400 feet would be constructed by controlled spot of material hydraulically dredged from the multiple purpose channed the levees constructed with land based equipment would have side slopes of 1 vertical on 3 horizontal and a minimum crown width of 10 feet. The hydraulically constructed levees would have side sloverying from 1 vertical on 20 horizontal to 1 vertical on 3 horizontal depending upon the natural repose of the hydraulically placed material aminimum crown width of 20 feet. All levees would have a minimum of 4 feet of freeboard above the design discharge water states.
- (3) The provision of appurtenant interior drainage facilities consisting of permanent sump areas, gated gravity drainand two pump stations having capacities of 40,000 and 150,000 gallons per minute for the Big Bayou (north) and Clayton Bayou (south) drainage areas, respectively.
- 166. COST ESTIMATE. The cost estimate for the Liberty Local Protection project is shown in table 55. The following tabulation does not include costs for spoil embankment rights-of-way or cost of placing spoil in spoil embankments. These costs were included in costs for the multiple purpose channel. Costs of clearing and grubbing the spoil embankment areas and final shaping of embankment are included in the costs for the local protection project.

TABLE 55 SUMMARY OF FIRST COSTS AND ANNUAL CHARGES FLOOD PROTECTION LIBERTY, TEXAS

1

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n ling

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urface.

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its

es:

| | : : | | • | |
|---|-----------------|---------------|-------------------------|-------|
| Item | : Unit : | Quantity | :Unit cost: | |
| Federal first cost: | | | | |
| 09.0 Channels | | | | |
| North diversion ditch | | | | |
| Excavation | CY | 44,444 | \$0.35 | \$ |
| Clearing and grubbing | Ac | 482 | 450.00 | |
| ll.O Levees | | | | |
| Embankment | CY | 671,230 | | 5 |
| Clearing and grubbing | Ac | 534.4 | 450.00 | 2 |
| Stripping | CY | 79,430 | | |
| Sprigging | A.c | 460.9 | 300.00 | 1 |
| Shaping spoil embankment Pumping plants | LF | 31,400 | 1.00 | |
| North plant | MGP. | M 40 | 1500.00 | |
| South plant | MGP. | | | 2 |
| Drain structures thru levees North sluice South sluice Subtotal Contingencies, 25% Total - Construction cos | 2" pipe | w/gate 4 7 | 6667.50 6667.50 1 | ,3 |
| 30.0 Engineering and design | | | | |
| 31.0 Supervision and administration | on | | | |
| Total estimated Federal first | t cost | Rounde | | .7 |
| Non-Federal first cost: | | 11000100 | | . , 1 |
| Ol.O Lands and damages Rights-of-way Ponding areas Contingencies Acquisition cost Total - lands and damages | Ac Ac L.S | 556 493 | | 1 |

TABLE 55 (CONT'D) SUMMARY OF FIRST COSTS AND ANNUAL CHARGES LOCAL FLOOD PROTECTION PROJECT LIBERTY, TEXAS

| Item | : : : : : : : : : : : : : : : : : : : | Quantity : | Unit cos |
|-------------------------------|---------------------------------------|------------|----------|
| Non-Federal costs, (Cont'd) | | | • |
| 02.0 Relocations | | | |
| Pipelines | | | |
| Gulf Gil Corp. 4" oil line | LF | 380 | \$ 8.00 |
| 3 Humble Oil & Refg. Corp 8" | | | |
| oil lines | LF | 1050 | 15.00 |
| 2 Gulf Oil Corp. 8" oil lines | LF | 700 | 15.00 |
| Gulf Oil Corp. 6" oil lines | LF | 700 | 12.00 |
| Texas Co. 6" oil line | LF | 380 | 12.00 |
| Texas Co. 4" oil line | LF | 380 | 8.00 |
| Liberty sewage effluent line | LF | 200 | 50.00 |
| Gulf Oil Corp. telephone line | LF | 500 | 1.00 |
| Subtotal - Relocations | | | |
| Contingencies | | | |
| Total - Relocations | | | |

30.0 Engineering and design

31.0 Supervision and administration

Total estimated non-Federal first cost

Total estimated project cost

Annual charges

Federal

Interest and amortization (2-7/8% interest rate & 100 yr. amort.)

Total Federal annual charges

Non-Federal

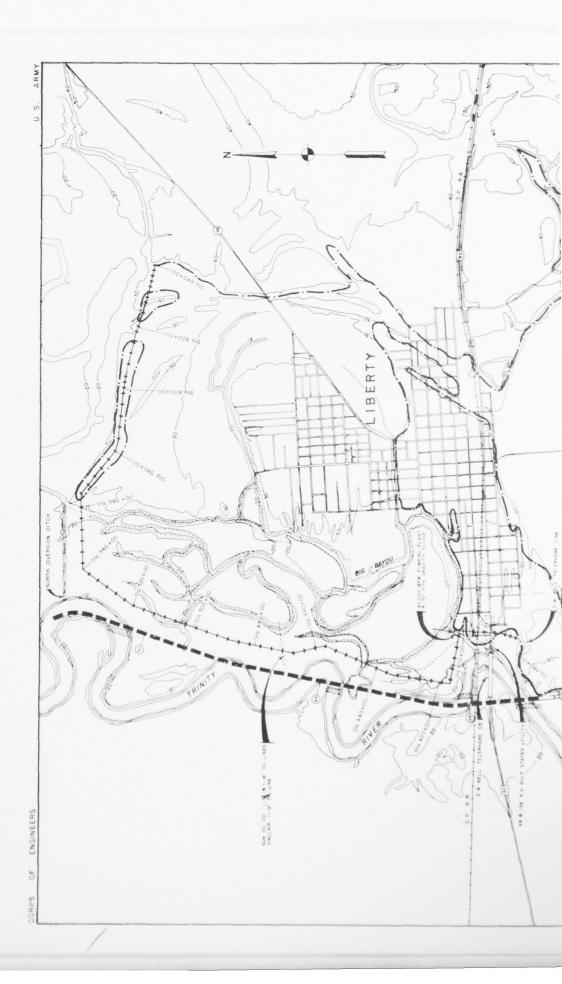
Interest and amortization (3% interest rate & 100 yr. amort.)

Operation and maintenance Major replacement Total non-Federal annual charges

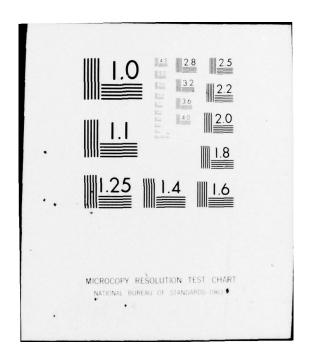
Total annual charges

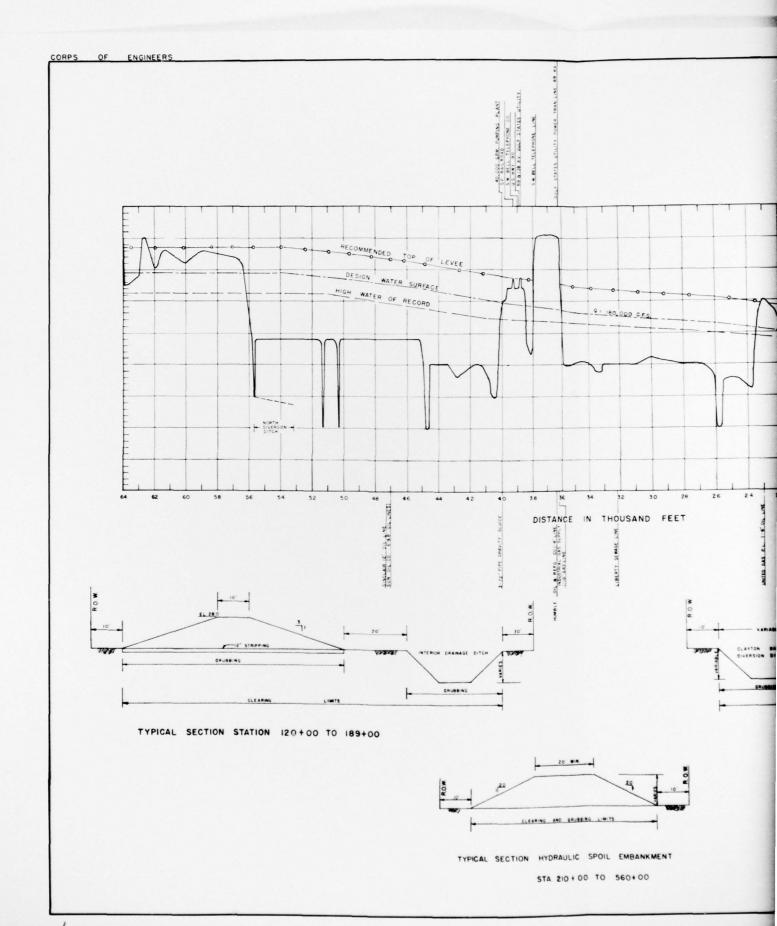
Note: Frices are as of January 1962.

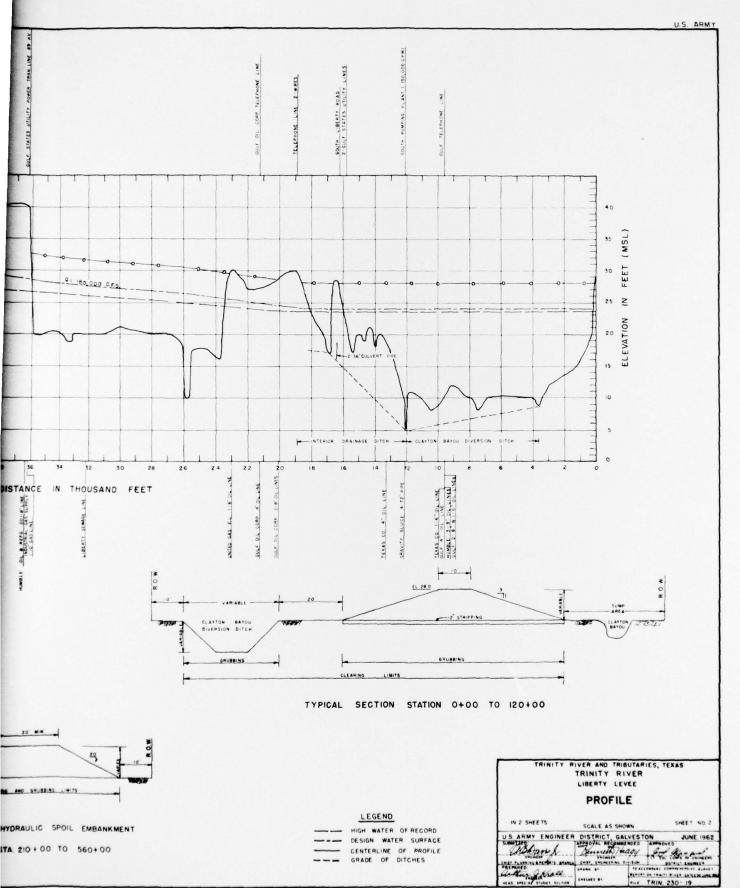
| : | | Tota | 1 |
|---|-----|--|--------|
| : | | cost | |
| | | | CARON |
| | \$ | 3,04 | 10 |
| | | 15,75 10,50 8,40 4,56 3,04 10,00 50 55,79 13,95 69,74 | 000000 |
| | | 2,65 | 0 |
| | | 4,04 | 0 |
| | 2 | 296,66 | 0 |
| | 2,0 | 90,67 | 0 |
| | | 54,80 54,80 | 000 |
| | | 9,40 13,00 2,00 24,50 | 0 |
| | | 79,30 | 0 |











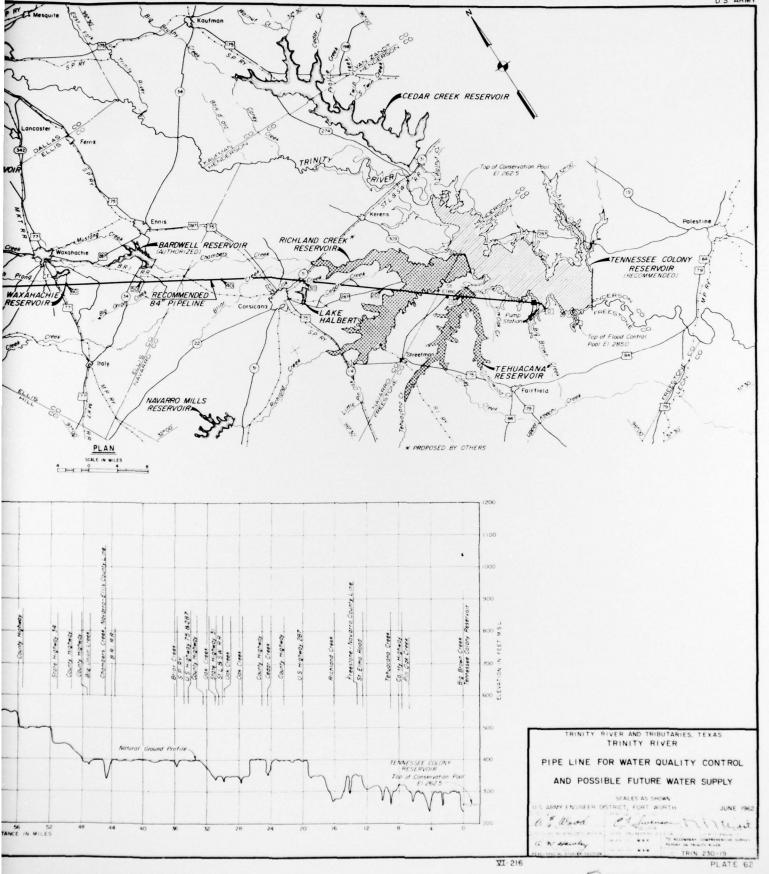
VI-214

PLATE 61

TRANSMISSION AND PUMPING FACILITIES

- 167. GENERAL.- In order to facilitate pollution abatement along the Trinity River from Fort Worth downstream to the vicinity of Rosser, Texas, a supply of about 80 million gallons per day of fresh water of acceptable quality would be necessary at Fort Worth. This water is available from Tennessee Colony Reservoir and will be delivered by pipeline to Benbrook Reservoir, 14 miles upstream from Fort Worth on the Clear Fork of the Trinity River. Plate 62 shows a preliminary location and profile of the proposed pipeline. The use of Benbrook Reservoir for terminal storage will enable the pipeline to be sized for average demand rather than for a higher peak demand and will also enable the regulation of the flows in the river in contrasting periods of drought and excessive runoff.
- 168. PIFELINE. The water from Tennessee Colony Reservoir would be conveyed to Benbrook Reservoir through an 84-inch pipeline sized to carry the average demand of 80 million gallons per day. This pipeline would be approximately 98 miles long and would cross three major watershed divides. Two of the divides crossed by the pipelines, one near Midlothian and one near Burleson, Texas, are about elevations 800 feet and 750 feet, respectively. The highest of the three divides at elevation 840 is the Clear Fork of the Trinity River eastern watershed divide. The pipeline would terminate just past this divide and would discharge into an unnamed tributary of the Clear Fork that enters Benbrook Reservoir from the east about one mile upstream from the dam. The static head will vary with the level of Tennessee Colony Reservoir from 577.5 feet with a full conservation pool in the reservoir to 615 feet with maximum drawdown to elevation 225.0. The static head with the conservation pool half full would be about 596 feet. The friction head developed in the pipeline while pumping 80 MGD would be about 229 feet. Therefore, the total pumping head would be about 825 feet when the conservation pool in Tennessee Colony Reservoir was half full.
- 169. PUMPING PLANTS... The Tennessee Colony pumping plant would be located about 6 miles above the dam and near Big Brown Creek, sufficiently in the main body of the reservoir where the silt load from the river and its major tributaries will be considerably less by the time the water reaches the pump station. Because of the length of the pipeline and the large pumping head, three additional booster pumping stations will be required. The plants will be equipped with pumping facilities having a capacity of 80 million gallons per day.





170. ESTIMATED COST. - Based on January 1962 price level, the total capital investment for the pipeline and the four pumping stations is estimated to be \$56,644,000 as shown in the following tabulation:

SUMMARY OF FIRST COSTS AND ANNUAL CHARGES FOR TRANSMISSION AND PUMPING FACILITIES

| Item | : | Unit | : | Quantity | : | Unit cost | : | Total cost |
|----------------------|-----|--------|-----|----------|---|---|---|---------------|
| Pipeline | | Mile | | 98 | | \$427,700 | | \$41,914,600 |
| Pumping plant | | Each | | 4 | | 850,000 | | 3,400,000 |
| Subtotal | | | | | | -,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | | 45,314,600 |
| Contingencies | | | | | | | | 11,329,400 |
| Total first cost | | | | | | | | 56,644,000 |
| Interest during cons | tr | uction | | | | | | None |
| Total investment | | | | | | | | 56,644,000 |
| Annual charges | | | | | | | | |
| Interest on investme | | (2-7/ | 8%) | | | | | 1,628,500 |
| Amortization (100 yr | | | | | | | | 101,400 |
| Power costs 89,346,0 | 000 | KWH @ | | | | | | |
| 9 mills/KWH | | | | | | | | 804,100 |
| OM&R costs | | | | | | | | |
| Pipelines (1% x f | | | | | | | | 523,900 |
| Pumping plant (2- | 1/2 | 2% x f | irs | t cost) | | | | 106,300 |
| Total annual charges | | | | | | | | 3,164,200 |

The total annual charges are estimated to be \$3,164,200. This includes interest and amortization at the rate of 2-7/8 percent for 100 years; the estimated power costs based on a rate of 9 mills per kilowatt hour; and the estimated annual operation, maintenance and replacement costs. The transmission costs for delivery of 80 million gallons per day from Tennessee Colony Reservoir to Benbrook Reservoir are estimated to be \$35.24 per acrefoot or 10.8 cents per 1000 gallons.

