



2005 Tri-Service Infrastructure Systems Conference & Exhibition

St. Louis, MO

“Re-Energizing Engineering Excellence”

2-4 August 2005

Agenda

Panel: The Future of Engineering and Construction

- LTG Carl A. Strock, Commander, USACE
- Dr. James Wright, Chief Engineer, NAVFAC

Panel: USACE Engineering and Construction

- Dr. Michael J. O'Connor, Director, R&D

Panel: Navy General Session

- Mr. Steve Geusic, Engineering Criteria & Programs NAVFAC Atlantic

Introduction to Multi-Disciplinary Tracks, by Mr. Gregory W. Hughes

Engineering Circular: Engineering Reliability Guidance for Existing USACE Civil Works Infrastructure, by Mr. David M. Schaaf, PE, LRD Regional Technical Specialist, Navigation Engineering Louisville District

MILCON S&A Account Study, by Mr. J. Joseph Tyler, PE, Chief, Programs Integration Division, Directorate of Military Programs HQUSACE

Financial Justification on Bentley Enterprise License Agreement (ELA)

Track 1

- The Chicago Shoreline Storm Damage Reduction Project, by Andrew Benziger
- Protecting the NJ Coast Using Large Stone Seawalls, by Cameron Chasten
- Cascade: An Integrated Coastal Regional Model for Decision Support and Engineering Design, by Nicholas C. Kraus and Kenneth J. Connell
- Modeling Sediment Transport Along the Upper Texas Coast, by David B. King Jr., Jeffery P. Waters and William R. Curtis
- Sediment Compatibility for Beach Nourishment in North Carolina, by Gregory L. Williams
- Evaluating Beachfill Project Performance in the USACE Philadelphia District, by Monica Chasten and Harry Friebe
- US Army Corps of Engineers' National Coastal Mapping Program, by Jennifer Wozencraft
- Flood Damage Reduction Project Using Structural and Non-Structural Measures, by Stacey Underwood
- Shore Protection Project Performance Improvement Initiative (S3P2I), by Susan Durden
- Hurricane Isabel Post-Storm Assessment, by Jane Jablonski
- US Army Corps of Engineers Response to the Hurricanes of 2004, by Rick McMillen and Daniel R. Haubner
- Increased Bed Erosion Due to Increased Bed Erosion Due to Ice, by Decker B. Hains, John I. Remus, and Leonard J. Zabilansky
- Mississippi Valley Division, by James D. Gutshall
- Impacts to Ice Regime Resulting from Removal of Milltown Dam, Clark Fork River, Montana, by Andrew M. Tuthill and Kathleen D. White, and Lynn A. Daniels
- Carroll Island Micromodel Study: River Miles 273.0-263.0, by Jasen Brown
- Monitoring the Effects of Sedimentation from Mount St. Helens, by Alan Donner, Patrick O'Brien and David Biedenharn
- Watershed Approach to Stream Stability and Benefits Related to the Reduction of Nutrients, by John B. Smith
- A Lake Tap for Water Temperature Control Tower Construction at Cougar Dam, Oregon, by Stephen Schlenker, Nathan Higa and Brad Bird
- San Francisco Bay Mercury TMDL – Implications for Constructed Wetlands, by Herbert Fredrickson, Elly Best and Dave Soballe
- Abandoned Mine Lands: Eastern and Western Perspectives, by Kate White and Kim Mulhern
- Translating the Hydrologic Tower of Babel, by Dan Crawford
- Demonstrating Innovative River Restoration Technologies: Truckee River, Nevada, by Chris Dunn
- System-Wide Water Resource Management – Tools of the Trade

Track 2

- Ecological and Engineering Considerations for Dam Decommissioning, Retrofits, and Reoperations, by Jock Conyngham
- Hydraulic Design of tidegates and other Water Control structures for Ecosystem Restoration projects on the Columbia River estuary, by Patrick S. O'Brien
- Surface Bypass & Removable Spillway Weirs, by Lynn Reese
- Impacts of using a spillway for juvenile fish passage on typical design criteria, by Bob Buchholz
- Howard Hanson Dam: Hydraulic Design of Juvenile Fish Passage Facility in Reservoir with Wide Pool Fluctuation, by Dennis Mekkers and Daniel M. Katz
- Current Research in Fate Current Research in Fate & Transport of Chemical and Biological Contaminants in Water Distribution Systems, by Vincent F. Hock
- Regional Modeling Requirements, by Maged Hussein
- Tools for Wetlands Permit Evaluation: Modeling Groundwater and Surface Water Interaction, by Cary Talbot
- Ecosystem Restoration for Fish and Wildlife Habitat on the UMRS, by Jon Hendrickson
- Missouri River Shallow Water Habitat Creation, by Dan Pridal
- Aquatic Habitat Restoration in the Lower Missouri River, by Chance Bitner
- Transition to an Oracle Based Data System (Corps Water Management System, CWMS), by Joel Asunskis
- RiverGages.com: The Mississippi Valley Division Water Control Website, by Rich Engstrom
- HEC-ResSim 3.0: Enhancements and New Capabilities, by Fauwaz Hanbali
- Hurricane Season 2004 – Not to Be Forgotten, by Jacob Davis
- Re-Evaluation of a Flood Control Project, by Ferris W. Chamberlin
- Helmand Valley Water Management Plan, by Jason Needham
- A New Approach to Water Management Decision Making, by James D. Barton
- Developing Reservoir Operational Plans to Manage Erosion and Sedimentation during Construction – Willamette Temperature
- Control, Cougar Reservoir 2002-2005, by Patrick S. O'Brien
- Improved Water Supply Forecasts for the Kootenay Basin, by Randal T. Wortman
- ResSIM Model Development for Columbia River System, by Arun Mylvahanan
- Prescriptive Reservoir Modeling and the ROPE, by Jason Needham
- Missouri River Basin Water Management, by Larry Murphy

Track 3

- Corps Involvement in FEMA's Map Modernization Program, by Kate White, John Hunter and Mark Flick
- Innovative Approximate Study Method for FEMA Map Modernization Program, by John Hunter
- Flood Fighting Structures Demonstration and Evaluation Program (FFSD), by Fred Pinkard
- Integrating Climate Dynamics Into Water Resources Planning and Management, by Kate White
- Hydrologic and Hydraulic Contributions to Risk and Uncertainty Propagation Studies, by Robert Moyer
- Uncertainty Analysis: Parameter Estimation, by Jackie P. Hallberg
- Geomorphology Study of the Middle Mississippi River, by Eddie Brauer
- Bank Erosion and Morphology of the Kaskaskia River, by Michael T. Rodgers
- Degradation of the Kansas City Reach of the Missouri River, by Alan Tool
- Sediment Impact Assessment Model (SIAM), by David S. Biedenham and Meg Jonas
- Mississippi River Sedimentation Study, by Basil Arthur
- Sediment Model of Rivers, by Charlie Berger
- East Grand Forks, MN and Grand Forks, ND Local Flood Damage Reduction Project, by Michael Leshner
- Hydrologic and Hydraulic Analyses, by Thomas R. Brown
- Hydrologic and Hydraulic Modeling of the McCook and Thornton Tunnel and Reservoir Plans, by David Kiel
- Ala Wai Canal Project, by Lynnette F. Schaper
- Missouri River Geospatial Decision Support Framework, by Bryan Baker and Martha Bullock
- Systemic Analysis of the Mississippi & Illinois Rivers Upper Mississippi River Comprehensive Plan, by Dennis L. Stephens

Section 227: National Shoreline Erosion Control Demonstration and Development Program Annual Workshop

- Workshop Objectives
- Section 227: Oil Piers, Ventura County, CA, by Heather Schlosser
- An Evaluation of Performance Measures for Prefabricated Submerged Concrete Breakwaters: Section 227 Cape May Point, New Jersey Demonstration Project, by Donald K Stauble, J.B. Smith and Randall A. Wise
- Bluff Stabilization along Lake Michigan, using Active and Passive Dewatering Techniques, by Rennie Kaunda, Eileen Glynn, Ron Chase, Alan Kehew, Amanda Brotz and Jim Selegean
- Storm Damage at Cape Lookout
- Branchbox Breakwater Design at Pickleweed Trail, Martinez, CA
- Section 227: Miami, FL
- Section 227: Sheldon Marsh Nature Preserve
- Section 227: Seabrook, New Hampshire
- Jefferson County, TX – Low Volume Beach Fill
- Sacred Falls, Oahsacred Falls, Oahu Section 227 Demonstration Project

Track 4

- Fern Ridge LakFern Ridge Lake Hydrologic Aspects of Operation during Failure, by Bruce J Duffe
- A Dam Safety Study Involving Cascading Dam Failures, by Gordon Lance
- Spillway Adequacy Analysis of Rough River Lake Louisville District, by Richard Pruitt
- Water Management in Iraq: Capability and Marsh Restoration, by Fauwaz Hanbali
- Iraq Ministry of Water Resources Capacity Building, by Michael J. Bishop, John W. Hunter, Jeffrey D. Jorgeson, Matthew M. McPherson, Edwin A. Theriot, Jerry W. Webb, Kathleen D. White, and Steven C. Wilhelms

- HEC Support of the CMEP Program, by Mark Jensen
- Geospatial Integration of Hydrology & Hydraulics Tools for Multi-Purpose, Multi-Agency Decision Support, by Timothy Pangburn, Joel Schlager, Martha Bullock, Michael Smith, and Bryan Baker
- GIS & Surveying to Support FEMA Map Modernization and Example Bridge Report, by Mark Flick
- High Resolution Bathymetry and Fly-Through Visualization, by Paul Clouse
- Using GIS and HEC-RAS for Flood Emergency Plans, by Stephen Stello
- High Resolution Visualizations of Multibeam Data of the Lower Mississippi River, by Tom Tobin and Heath Jones
- System Wide Water Resources Program Unifying Technologies Geospatial Applications, by Andrew J. Bruzewicz
- Raystown Plate Locations
- Hydrologic Engineering Center: HEC-HMS Version 3.0 New Features, by Jeff Harris
- SEEP2D & GMS: Simple Tools for Solving a Variety of Seepage Problems, by Clarissa Hansen, Fred Tracy, Eileen Glynn, Cary Talbot and Earl Edris
- Sediment and Water Quality in HEC-RAS, by Mark Jensen
- Advances to the GSSHA Model, by Aaron Byrd and Cary Talbot
- Watershed Analysis Tool: HEC-WAT Program, by Chris Dunn
- Little Calumet River UnsteadLittle Calumet River Unsteady Flow Model Conversion UNET to HEC-RAS, by Rick D. Ackerson
- Kansas River Basin Model, by Edward Parker
- Design Guidance for Breakup Ice Control Structures, by Andrew M. Tuthill
- Computational Hydraulic Model of the Lower Monumental Dam Forebay, by Richard Stockstill, Charlie Berger, John Hite, Alex Carrillo, and Jane Vaughan
- Use of Regularization as a Method for Watershed Model Calibration, by Brian Skahill
- Demonstration Program Urban Flooding and Channel Restoration in Arid and Semi-Arid Regions (UFDP), by Joan Pope, Jack Davis, Ed Sing, John Warwick, Meg Jonas

Track 5

- Walla Walla District Northwestern Division, by Robert Berger
- Best Practices for Conduits through Embankment Dams, by Chuck R. Cooper
- Design, Construction Design, Construction and Seepage at Prado Dam, by Douglas E. Chitwood
- 2-D Liquefaction Evaluation with Q4Mesh, by David C. Serafini
- Unlined Spillway Erosion Risk Assessment, by Johannes Wibowo, Don Yule, Evelyn Villanueva and Darrel Temple
- Seismic Remediation of the Clemson Upper and Lower Diversion Dams; Evaluation, Conceptual Design and Design, by Lee Wooten and Ben Foreman
- Seismic Remediation of the Clemson Upper and Lower Diversion Dams; Deep Soil Mix Construction, by Lee Wooten and Ben Foreman
- Historical Changes in the State of the Art of Seismic Engineering and Effects of those changes on the Seismic Response Studies of Large Embankment Dams, by Sam Stacy
- Iwakuni Runway Relocation Project, by Vincent R. Donnally
- Internal Erosion & Piping at Fern Ridge Dam, by Jeremy Britton
- Rough River Dam Safety Assurance Project, by Timothy M. O'Leary
- Seepage Collection & Control Systems: The Devil is in the Details , by John W. France
- Dewey Dam Seismic Assessment, by Greg Yankey
- Seismic Stability Evaluation for Ute Dam, New Mexico, by John W. France
- An Overview of Criteria Used by Various Organizations for Assessment and Seismic Remediation of Earth Dams, by Jeffrey S. Dingrando
- A Review of Corps of Engineers Levee Seepage Practices and Proposed Future Changes, by George Sills
- Ground-Penetrating Radar Applications for the Assessment of Pavements, by Lulu Edwards and Don R. Alexander
- Peru Road Upgrade Project, by Michael P. Wielputz
- Slope Stability Evaluation of the Baldhill Dam Right Abutment, by Neil T. Schwanz
- Design and Construction of Anchored Bulkheads with Synthetic Sheet Piles Seabrook, New Hampshire, by Siamac Vaghar and Francis Fung
- Characterization of Soft Claya Case Study at Craney Island, by Aaron L. Zdinak
- Dispersive ClayDispersive Clays – Experience andHistory of the NRCS (Formerly SCS), by Danny McCook
- Post-Tensioning Institute, by Michael McCray
- Demonstration Program Urban Flooding and Channel Restoration in Arid and Semi-Arid Regions (UFDP), by Joan Pope, Jack Davis, Ed Sing, John Warwick, Meg Jonas

Track 6

- State of the Art in Grouting: Dams on Solution Susceptible or Fractured Rock Foundations, by Arthur H. Walz
- Specialty Drilling, Testing, and Grouting Techniques for Remediation of Embankment Dams, by Douglas M. Heenan
- Composite Cut-Offs for Dams, by Dr. Donald A. Bruce and Trent L. Dreese
- State of the Art in Grout Mixes, by James A. Davies
- State of the Art in Computer Monitoring and Analysis of Grouting, by Trent L. Dreese and David B. Wilson
- Quantitatively Engineered Grout Curtains, by David B. Wilson and Trent L. Dreese
- Grout Curtains at Arkabutla Dam: Outlet Monolith Joints and Cracks using Chemical Grout, Arkabutla Lake, MS, by Dale A. Goss
- Chicago Underflow Plan – CUP: McCook Reservoir Test Grout Program, by Joseph A. Kissane
- Clearwater Dam: Sinkhole Repair Foundation Investigation and Grouting Project, by Mark Harris
- Update on the Investigation of the Effects of Boring Sample Size (3" vs 5") on Measured Cohesion in Soft Clays, by Richard Pinner and Chad M. Rachel
- Soil-Bentonite Cutoff Wall Through Free-Product at Indiana Harbor CDF, by Joe Schulenberg and John Breslin
- Soil-Bentonite Cutoff Wall Through Dense Alluvium with Boulders into Bedrock, McCook Reservoir, by William A. Rochford
- Small Project, Big Stability Problem the Block Church Road Experience, by Jonathan E. Kolber
- Determination of Foundation Rock Properties Beneath Folsom Dam, by Michael K. Sharp, José L. Llopis and Enrique E. Matheu
- Waterbury Dam Mitigation, by Bethany Bearmore
- Armor Stone Durability in the Great Lakes Environment, by Joseph A. Kissane
- Mill Creek - An Urban Flood Control Challenge, by Monica B. Greenwell
- Next Stop, The Twilight Zone, by Troy S. O'Neal
- Limitations in the Back Analysis of Shear Strength from Failures, by Rick Deschamps and Greg Yankey
- Reconstruction of Deteriorated Concrete Lock Walls After Blasting and Other Demolition Removal Techniques, by Stephen G. O'Connor

- Flood Fighting Structures Demonstration and Evaluation Program (FFSD), by George Sills
- Innovative Design Concepts Incorporated into a Landfill Closure and Reuse Design Portsmouth Naval Shipyard, Kittery, Maine, by Dave Ray and Kevin Pavlik
- Laboratory Testing of Flood Fighting Structures, by Johannes L. Wibowo, Donald L. Ward and Perry A. Taylor
- Bluff Stabilization Along Lake Michigan, Using Active and Passive Dewatering Techniques, Allegan Co. Michigan, by Rennie Kaunda, Eileen Glynn, Ron Chase, Alan Kehew and Jim Selegean

Track 7

- Case History: Multiple Axial Static Tests on a Drilled Shaft Embedded in Shale, by Paul J. Axtell, J. Erik Loehr, Daniel L. Jones
- The Sliding Failure of Austin Dam Pennsylvania - Revisited, by Brian H. Greene
- M3 –Modeling, Monitoring and Managing: A Comprehensive Approach to Controlling Ground Movements for Protection of Existing Structures and Facilities, by Francis D. Leathers and Michael P. Walker
- Time-Dependent Reliability Modeling for Use in Major Rehabilitation of Embankment Dams and Foundation, by Robert C. Patev
- Lateral Pile Load Test Results Within a Soft Cohesive Foundation, by Richard J. Varuso
- Engineering Geology Challenge Engineering Geology Challenges During Design and Construction of the Marmet Lock Project, by Ron Adams and Mike Nield
- Mill Creek Deep Tunnel Geologic Conditions and Potential Impacts on Design/Construction, by Kenneth E. Henn III
- McAlpine Lock Replacement Instrumentation: Design, Construction, Monitoring, and Interpretation, by Troy S. O’Neal
- Geosynthetics and Construction of the Second Powerhouse Corner Collector Surface Flow Bypass Project, Bonneville Lock and Dam Project, Oregon and Washington, by Art Fong
- McAlpine Lock Replacement Project Foundation Characteristics and Excavation, by Kenneth E. Henn III
- Structural and Geotechnical Issues Impacting The Dalles Spillwall Construction and Bay 1 Erosion Repair, by Jeffrey M. Ament
- Rock Anchor Design and Construction: The Dalles Dam Spillwalls, by Kristie M. Hartfeil
- The Future of the Discrete Element Method in Infrastructure Analysis, by Raju Kala, Johannes L. Wibowo and John F. Peters
- Sensitive Infrastructure Sites - Sonic Drilling Offers Quality Control and Non-Destructive Advantages to Geotechnical Construction Drilling, by John P. Davis

Track 8

- Evaluation of The Use of Lithium Compounds in Controlling ASR in Concrete Pavement, by Mike Kelly
- Roller Compacted Concrete for McAlpine Lock Replacement, by David E. Kiefer
- Soil-Cement for Stream Bank Stabilization, by Wayne Adaska
- Using Cement to Reclaim Asphalt Pavements, by David R. Luhr
- Valley Park 100-Yr Flood Protection Project: Use of ‘Engineered Fill’ in the Item IV-B Levee Core, by Patrick J. Conroy
- Bluestone Dam: AAR –A Case Study, by Greg Yankey
- USDA Forest Service: Unpaved Road Stabilization with Chlorides, by Michael R. Mitchell
- Use of Ultra-Fine Amorphous Colloidal Silica to Produce a High-Density, High-Strength Grout, by Brian H. Green
- Modular Gabion Systems, by George Ragazzo
- Addressing Cold Regions Issues in Pavement Engineering, by Edel R. Cortez and Lynette Barna
- Geology of New York Harbor: Geological and Geophysical Methods of Characterizing the Stratigraphy for Dredging Contracts, by Ben Baker, Kristen Van Horn and Marty Goff
- Rubblization of Airfield Concrete Pavements, by Eileen M. Vélez-Vega
- US Army Airfield Pavement Assessment Program, by Haley Parsons, Lulu Edwards, Eileen Velez-Vega and Chad Gartrell
- Critical State for Probabilistic Analysis of Levee Underseepage, by Douglas Crum,
- Curing Practices for Modern Concrete Production, by Toy Poole
- AAR at Carters Dam: Different Approaches, by James Sanders
- Concrete Damage at Carters Dam, by Toy Poole
- Damaging Interactions Among Concrete Materials, by Toy Poole
- Economic Effects on Construction of Uncertainty in Test Methods, by Toy Poole
- Trends in Concrete Materials Specifications, by Toy Poole
- Spall and Intermediate-Sized Repairs for PCC Pavements, by Reed Freeman and Travis Mann
- Acceptance Criteria for Unbonded Aggregate Road Surfacing Materials, by Reed Freeman, Toy Poole, Joe Tom and Dale Goss
- Effective Partnering to Overcome an Interruption In the Supply of Portland Cement During Construction at Marmet Lock and Dam, by Billy D. Neeley, Toy S. Poole and Anthony A. Bombich

Track 10

- Marmet Lock & Dam: Automated Instrumentation Assessment, Summer/Fall 2004, by Jeff Rakes and Ron Adams
- Success Dam Seismic Remediation

Track 9

- Fern Ridge Dam, Oregon: Seepage and Piping Concerns (Internal Erosion)

Track 11

- Canton Dam Spillway Stability: Is a Test Anchor Program Necessary?, by Randy Mead
- Dynamic Testing and Numerical Correlation Studies for Folsom Dam, by Ziyad Duron, Enrique E. Matheu, Vincent P. Chiarito, Michael K. Sharp and Rick L. Poepelman

Status of Portfolio Risk Assessment, by Eric Halpin

- Mississinewa Dam Foundation Rehabilitation, by Jeff Schaefer
- Wolf Creek Dam Seepage Major Rehabilitation Evaluation, by Michael F. Zoccola
- Bluestone Dam DSA Anchor Challenges, by Michael McCray
- Clearwater Dam Major Rehab Project, by Bobby Van Cleave
- Design, Construction and Seepage at Prado Dam, by Douglas E. Chitwood
- Seven Oaks Dam: Outlet Tunnel Invert Damage, by Robert Kwan
- An Overview of An Overview of the Dam Safety Program Management Tools (DSPMT), by Tommy Schmidt

Track 12

- Greenup L&D Miter Gate Repair and Instrumentation, by Joseph Padula, Bruce Barker and Doug Kish
- Marmet Locks and Dam Lock Replacement Project, by Jeffrey S. Maynard,
- Status of HSS Inspections in The Portland District, by Travis Adams
- Kansas City District: Perry Lake Project Gate Repair, by Marvin Parks
- Mel Price – Auxiliary Lock Downstream Miter Gate Repair, by Thomas J. Quigley, Brian K. Kleber and Thomas R. Ruf
- J.T. Myers Lock Improvements Project Infrastructure Conference, by David Schaaf and Greg Werncke
- J.T. Myers Dam Major Rehab, by David Schaaf, Greg Werncke and Randy James
- Greenup L&D, by Rodney Cremeans
- McAlpine Lock Replacement Project, by Kathy Feger
- Roller Compacted Concrete Placement at McAlpine Lock, by Larry Dalton
- Kentucky Lock Addition Downstream Middle Wall Monolith Design, by Scott A. Wheeler
- London Locks and Dam Major Rehabilitation Project, by David P. Sullivan
- Replacing Existing Lock 4: Innovative Designs for Charleroi Lock, by Lisa R. Pierce, Dave A. Stensby and Steve R. Stoltz
- Olmsted L&D, Dam In-the-wet Construction, by Byron McClellan, Dale Berner and Kenneth Burg
- Olmsted Floating Approach Walls, by Terry Sullivan
- John Day Navigation Lock Monolith Repair, by Matthew D. Hanson
- Inner Harbor Navigation Canal (IHNC) Lock Replacement, by Mark Gonski
- Comite River Diversion Project, by Christopher Dunn
- Waterline Support Failure: A Case Study, by Angela DeSoto Duncan
- Public Appeal of Major Civil Projects: The Good, the Bad and the Ugly, by Kevin Holden and Kirk Sunderman
- Chickamauga Lock and Dam Lock Addition Cofferdam Height Optimization Study, by Leon A. Schieber
- Des Moines Riverwalk, by Thomas D. Heinold

Track 13

- Folsom Dam Evaluation of Stilling Basin Performance for Uplift Loading for Historic Flows and Modification of Folsom Dam
- Stilling Basin for Hydrodynamic Loading, by Rick L. Poepelman, Yunjing (Vicky) Zhang, and Peter J. Hradilek
- Seismic Stress Analysis of Folsom Dam, by Enrique E. Matheu
- Barge Impact Analysis for Rigid Lock Walls ETL 1110-2-563, by John D. Clarkson and Robert C. Patev
- Belleville Locks & Dam Barge Accident on 6 Jan 05, by John Clarkson
- Portugues Dam Project Update, by Alberto Gonzalez, Jim Mangold and Dave Dollar
- Portugues Dam: RCC Materials Investigation, by Jim Hinds
- Nonlinear Incremental Thermal Stress Strain Analysis Portugues Dam, by David Dollar, Ahmed Nisar, Paul Jacob and Charles Logie
- Seismic Isolation of Mission-Critical Infrastructure to Resist Earthquake Ground Shaking or Explosion Effects, by Harold O. Sprague, Andrew Whitaker and Michael Constantino
- Obermeyer Gated Spillway S381, by Michael Rannie
- Design of High Pressure Vertical Steel Gates Chicago Land Underflow Plan McCook Reservoir, by Henry W. Stewart, Hassan Tondravi, Lue Tekola,
- Development of Design Criteria for the Rio Puerto Nuevo Contract 2D/2E Channel Walls, by Janna Tanner, David Shiver, and Daniel Russell
- Indianapolis North Indianapolis North Phase 3A Warfleigh Section
- Design of Concrete Lined Tunnels in Rock CUP McCook Reservoir Distribution Tunnels Contract, by David Force

Track 14

- GSA Progressive Collapse Design Guidelines Applied to Concrete Moment-Resisting Frame Buildings, by David N. Bilow and Mahmoud E. Kamara,
- UFC 4-023-02 Retrofit of Existing Buildings to Resist Explosive Effects, by Jim Caulder
- Summit Bridge Fatigue Study, by Jim Chu
- Quality Assurance for Seismic Resisting Systems, by John Connor
- Seismic Requirements for Arch, Mech, and Elec. Components, by John Connor
- SBEDS - (Single degree of freedom Blast Effects Design Spreadsheets), by Dale Nebuda,
- Design of Buildings to Resist Progressive Collapse UFC 4-023-03, by Bernie Deneke,
- Fatigue and Fracture Assessment, by Jesse Stuart
- Unified Facilities Criteria: Seismic Design for Buildings, by Jack Hayes
- Evaluation and Repair Of Blast Damaged Reinforced Concrete Beams, by MAJ John L. Hudson
- Building an In-house Bridge Inspection Program
- United Facilities Criteria United Facilities Criteria Masonry Design for Buildings, by Tom Wright
- USACE Homeland Security Portal, by Michael Pace
- Database Tools for Civil Works Projects

- Standard Procedure for Fatigue Evaluation of Bridges, by Phil Sauser
- Consolidation of Structural Criteria for Military Construction, by Steven Sweeney
- Cathodic Protection for the South Power Plant Reinforcing Steel, Diego Garcia, BIOT, by Thomas Tehada and Miki Funahashi

Track 15

- Engineering Analysis of Airfield Lighting System Lightning Protection, by Dr. Vladimir A. Rakov and Dr. Martin A. Uman
- Dr. Martin A. Uman
- Charleston AFB Airfield Lighting Vault
- UNIFIED FACILITIES CRITERIA (UFC) UFC 3-530-01 Design: Interior, Exterior Lighting and Controls, by Nancy Clanton and Richard Cofer
- Electronic Keycard Access Locks, by Fred A Crum
- Unified Facilities Criteria (UFC) 3-560-02, Electrical Safety, by John Peltz and Eddie Davis
- Electronic Security System Electronic Security Systems Process Overview
- Lightning Protection Standards
- Electrical Military Workshop
- Information Technology Systems Criteria, by Fred Skroban and John Peltz
- Electrical Military Workshop
- Electrical Infrastructure in Iraq- Restore Iraqi Electricity, by Joseph Swiniarski

Track 16

- BACnet® Technology Update, by Dave Schwenk
- The Infrastructure Conference 2005, by Steven M. Carter Sr. and Mitch Duke
- Design Consideration for the Prevention of Mold, by K. Quinn Hart
- COMMISSIONING, by Jim Snyder
- New Building Commissioning , by Gary Bauer
- Ventilation and IAQ The New ASHRAE Std 62.1, by Davor Novosel
- Basic Design Considerations for Geothermal Heat Pump Systems, by Gary Phetteplace
- Packaged Central Plants
- Effective Use Of Evaporative Cooling For Industrial And Institutional/Office Facilities, by Leon E. Shapiro
- Seismic Protection For Mechanical Equipment
- Non Hazardous Chemical Treatments for Heating and Cooling Systems, by Vincent F. Hock and Susan A. Drozd
- Trane Government Systems & Services
- LONWORKS Technology Update, by Dave Schwenk
- Implementation of Lon-Based Specifications by Will White and Chris Newman

Track 17

- Utility System Security and Fort Future, by Vicki Van Blaricum, Tom Bozada, Tim Perkins, and Vince Hock
- Festus/Crystal City Levee and Pump Station
- Chicago Underflow Plan McCook Reservoir (CUP) Construction of Distribution Tunnel and Pumps Installation
- Technological Advances in Lock Control Systems, by Andy Schimpf and Mike Maher
- Corps of Engineers in Iraq Rebuilding Electrical Infrastructure, by Hugh Lowe
- Red River of the North at East Grand Forks, MN & Grand Forks, ND: Flood Control Project – Armada of Pump Stations Protect Both Cities, by Timothy Paulus
- Lessons Learned for Axial/Mixed Flow Propeller Pumps, by Mark A. Robertson
- Creek Automated Gate Considerations, by Mark A. Robertson
- HydroAMP: Hydropower Asset Management, by Lori Rux
- Acoustic Leak Detection for Water Distribution Systems, by Sean Morefield, Vincent F. Hock and John Carlyle
- Remote Operation System, Kaskaskia Dam Design, Certification, & Accreditation, by Shane M. Nieukirk
- Lock Gate Replacement System, by Shaun A. Sipe and Will Smith

Track 20

- “Re-Energizing Medical Facility Excellence”, by COL Rick Bond
- Rebuilding and Renovating The Pentagon , by Brian T. Dziekonski,
- Resident Management System
- Design-Build and Army Military Construction, by Mark Grammer
- Defense Acquisition Workforce Improvements Act - Update, by Mark Grammer
- Construction Management @ Risk: Incentive Price Revision – Successive Targets, by Christine Hendzlik
- Construction Reserve Matrix, by Christine Hendzlik
- Award contingent on several factors..., by Christine Hendzlik
- 52.216-17 Incentive Price Revision--Successive Targets (Oct 1997) - Alt I (Apr 1984), by Christine Hendzlik
- Preconstruction Services, by Christine Hendzlik
- Proposal Evaluation Factors, by Christine Hendzlik
- MILCON Transformation in Support of Army Transformation, by Claude Matsui
- Construction Practices in Russia, by Lance T. Lawton

- Partnering as a Best Practice, by Ray Dupont
- USACE Tsunami Reconstruction for USAID, by Andy Constantaras

Track 21

- Dredging Worldwide, by Don Carmen
- SpecsIntact Editor, by Steven Freitas
- SpecsIntact Explorer, by Steven Freitas
- American River Watershed Project, by Steven Freitas
- Unified Facilities Guide Specifications (UFGS) Conversion To MasterFormat 2004, by Carl Kersten
- Unified Facilities Guide Specifications (UFGS) Status and Direction , by Jim Quinn

Workshops

- Design of Buildings to Resist Progressive Collapse UFC 4-023-03, by Bernie Deneke
- Security Engineering and at Unified Facility Criteria (UFC), by Bernie Deneke, Richard Cofer, John Lynch and Rudy Perkey
- Packaged Central Plants, by Trey Austin



2005 Tri-Service Infrastructure Systems Conference & Exhibition

*“Re-Energizing Engineering
Excellence”*

ON-SITE AGENDA

*The America's Center
St. Louis Convention Center
St. Louis, MO
August 2-4, 2005
Event # 5150*



AGENDA

Monday, August 1, 2005

8:00 AM-9:00 PM Exhibit Move-In

12 Noon-5:00 PM Registration

Tuesday, August 2, 2005

7:00 AM-8:00 AM Registration and Continental Breakfast

8:00 AM-8:15 AM Welcome and Introduction
Ferrara Theatre

8:15 AM-9:00 AM The Future of Engineering and Construction Panel
Ferrara Theatre
Moderator:
Mr. Don Basham, Chief, Engineering & Construction, USACE
Panelists:
LTG Carl A. Strock, Commander, USACE
Dr. James Wright, Chief Engineer NAVFAC

9:00 AM-9:45 AM Keynote Address
Ferrara Theater
The Lord of the Things: The Future of Infrastructure Technologies
Mr. Paul Doherty, AIA, Managing Director,
General Land Corporation

9:45 AM-10:15 AM Break

10:15 AM-11:15 AM USACE Engineering and Construction Panel
Ferrara Theatre
Moderator:
Mr. Don Basham, Chief, Engineering & Construction, USACE
Panelists:
MG Donald T. Riley, Director, Civil Works, USACE
BG Bo M. Temple, Director, Military Programs, USACE
Dr. Michael J. O'Connor, Director, R&D

10:15 AM-11:15 AM Navy General Session
Room 225

11:00 AM - 7:00 PM Exhibits Open

11:15 AM-1:00 PM Lunch in Exhibit Hall (on your own)

11:15 AM-1:00 PM Women's Career Lunch Session (Bring your lunch from Exhibit Hall)
Washington G
Moderator:
Ms. Demi Syriopoulou, HQ USACE
Opening Remarks:
LTG Carl A. Strock, Commander, USACE
Presentations & Discussion:
Dwight Beranek, Kristine Allaman, Donald Basham, HQ USACE

1:00 PM-1:55 PM Introduction to Multi-Disciplinary Tracks
Ferrara Theatre

- Track 1: Acquisition Strategies for Civil Works
Room 230 *Walt Norko*
- Track 2: Risk and Reliability Engineering
Room 231 *Anjana Chudgar*
David Schaaf
- Track 3: Portfolio Risk Assessment
Room 232 *Eric Halpin*
- Track 4: Hydrology, Hydraulics and Coastal Engineering
Support for USACE
Room 240 *Jerry Webb*
Darryl Davis
- Track 5: Civil Works R&D Forum
Room 241 *Joan Pope*
- Track 6: Civil Works Security Engineering
Room 242 *Joe Hartman*
Bryan Cisar
- Track 7: Building Information Model Applications
Room 226 *Brian Huston*
Daniel Hawk
- Track 8: Design Build for Military Projects
Room 220 *Mark Grammer*
- Track 9: Army Transformation/Global Posture Initiative/
Force Modernization
Room 221 *Al Young*
Claude Matsui
- Track 10: Force Protection - Army Access Control Points
Room 222 *John Trout*
- Track 11: Cost Engineering Forum on Government Estimates
vs. Actual Costs
Room 227 *Ray Lynn* *Jack Shelton* *Kim Callan*
Miguel Jumilla *Ami Ghosh* *Joe Bonaparte*
- Track 12: Engineering & Construction Information Technology
Room 228 *MK Miles*
- Track 13: Sustainable Design
Room 223 *Harry Goradia*
- Track 14: ACASS/CCASS/CPARS
Room 224 *Ed Marceau*
Marilyn Nedell
- Track 15: Whole Building Design Guide
Room 229 *Earle Kennett*

Tuesday, August 2, 2005

2:50 PM-3:30 PM	Break in Exhibit Hall
3:30 PM-4:20 PM	2 nd Round of Multi-Disciplinary Sessions
4:30 PM-5:20 PM	3 rd Round of Multi-Disciplinary Sessions
5:30 PM-7:00 PM	Ice Breaker Reception in Exhibit Hall

Wednesday, August 3, 2005

7:00 AM-8:00 AM	Registration and Continental Breakfast
8:00 AM-9:30 AM	Concurrent Sessions (Please Refer to Concurrent Session Schedule on the Following Pages)
9:00 AM	Exhibit Hall Opens
9:30 AM-10:30 AM	Break in Exhibit Hall
10:30 AM-12:00 Noon	Concurrent Sessions (Please Refer to Concurrent Session Schedule on the Following Pages)
12:00 Noon-1:30 PM	Lunch in Exhibit Hall
1:30 PM-3:00 PM	Concurrent Sessions (Please Refer to Concurrent Session Schedule on the Following Pages)
3:00 PM-4:00 PM	Break in Exhibit Hall
4:00 PM-5:30 PM	Concurrent Sessions
5:00 PM	Exhibit Hall Closes

Thursday, August 4, 2005

7:00 AM-8:00 AM	Registration and Continental Breakfast
8:00 AM-9:30 AM	Concurrent Sessions (Please Refer to Concurrent Session Schedule on Following Pages)
9:30 AM-10:30 AM	Break in Exhibit Hall (Last Chance to view Exhibits)
10:30 AM-12:00 Noon	Concurrent Sessions (Please Refer to Concurrent Session Schedule on Following Pages)
12:00 Noon-1:30 PM	Lunch (On your own)
12:00 Noon-6:00 PM	Exhibits Move-Out
1:30 PM-3:00 PM	Concurrent Sessions (Please Refer to Concurrent Session Schedule on Following Pages)
3:00 PM-3:30 PM	Break
3:30 PM-5:00 PM	Concurrent Sessions (Please Refer to Concurrent Session Schedule on following pages)

Wednesday, August 3, 2005 Concurrent Sessions

HH&C Track

	8:00 AM	8:30 AM	9:00 AM	9:30 AM	10:30 AM	11:00 AM	11:30 AM
Room 220	TRACK 1 Coastal Structures Protecting the NJ Coast using large stone seawalls	Session 1A Chicago shoreline storm damage reduction project	Session 1A Cameron Chasten	Session 1A Jeffrey Malby	TRACK 1 Coastal Regional Management Session 1B Cascade: An integrated regional model for decision support	Session 1B Nicholas Kraus	Session 1B Gregory Williams
Room 221	TRACK 2 Ecological Engineering & Design Ecological and engineering considerations for dam decommissioning, retrofits and operations	Session 2A Hydraulic design of tidegates and other water control structures for ecosystem restoration on the Columbia Estuary	Session 2A Jock Conyngham	Session 2A Andrew Goodwin	TRACK 2 Ecological Engineering & Design Innovative hydraulic structure design at Lower Granite Dam: design that saves water and salmon	Session 2B Lynn Reese	Session 2B Dennis Mekkers
Room 222	TRACK 3 Modeling Corps involvement in the FEMA map modernization program	Session 3A Innovative approximate study method for FEMA map modernization program	Session 3A John Hunter	Session 3A Fred Pinkard	TRACK 3 Modeling Integrating climate dynamics into water resources planning and management	Session 3B Kate White	Session 3B Rob Moyer
Room 223	TRACK 4 H&H Aspects of Dam Safety Hydrologic aspects of operating in failure mode: Fern Lake	Session 4A Dam safety study with cascading failures	Session 4A Gordon Lance	Session 4A Richard Pruitt	TRACK 4 International/Military H&H Capability restoration and historic marsh restoration	Session 4B Faauwaz Hanbali	Session 4B Mark Jensen

Break in Exhibit Hall

12 Noon

Lunch in Exhibit Hall

	1:30 PM	2:00 PM	2:30 PM	3:00 PM	4:00 PM	4:30 PM	5:00 PM
Room 220	TRACK 1 Coastal Sediments Evaluating beachfill project performance in the NAP	Session 1C USACE's regional coastal mapping program	Session 1C Stacey Underwood	TRACK 1 Shore Protection Projects Hurricane Isabel effects on communities	Session 1D Jane Jablonski	Session 1D Rick McMillen	Session 1D Sharon Haggatt
Room 221	TRACK 2 Ecological Restoration/Systems Assessment Session 2C Regional modeling requirements for ecosystem restoration	Session 2C Tools for wetlands permit evaluation: Modeling groundwater and surface water distribution systems	Session 2C Mark Ginsberg	TRACK 2 Ecosystem Habitat Restoration Aquatic habitat restoration in the lower Missouri River	Session 2D Chance Bittner	Session 2D Daniel Pridal	Session 2D Jon Hendrickson
Room 222	TRACK 3 River Morphology Geomorphology study of the Mississippi river	Session 3C Bank erosion and morphology of the Kaskaskia river	Session 3C Alan Tool	TRACK 3 Modeling River Sedimentation Sediment impact assessment model (SIAM) MS River, Cairo to Gulf	Session 3D David Biedenbarn	Session 3D Basil Arthur	Session 3D Charlie Berger
Room 223	TRACK 4 GIS and Surveying GIS tools available now to support HHC	Session 4C High resolution bathymetry and fly-through visualization	Session 4C Mark Flick	TRACK 4 GIS and Surveying Update flood emergency plans with GIS and HEC-RAS	Session 4D Stephen Stello	Session 4D Thomas Tobin	Session 4D Andrew Bruzewicz

Break in Exhibit Hall

Wednesday, August 3, 2005 Concurrent Sessions

Geotechnical Track

	8:00 AM	8:30 AM	9:00 AM	9:30 AM	10:30 AM	11:00 AM	11:30 AM
TRACK 5	Levee lowering for the Lewis & Clark bi-centennial celebration Robert Berger	Conduits through embankment dams - best practices for design, construction, problem id and evaluation, inspection, maintenance, renovation & repair Dave Pezza	Design, construction and seepage at Prado Dam, CA Douglas Chitwood	Break in Exhibit Hall	2-D liquefaction evaluation with q4MESH David Serafini	Unlined spillway erosion risk assessment Johannes Wibowo	Seismic remediation of the Clemson upper and lower diversion dams: evaluation, conceptual design and design (P1) Ben Foreman
Session 5A	USACE dams on solution susceptible or highly fractured rock foundations Art Walz	Special drilling and grouting techniques for remedial work in embankment dams Doug Heenan	Composite grouting & cutoff wall solutions Donald Bruce	TRACK 6	Session 5B	TRACK 6	Session 5B
Session 6A	Case history: multiple axial static test on a drilled shaft embedded in shale Paul Axtell	Austin Dam, Pennsylvania: the sliding failure of a concrete gravity dam revisited Brian Greene	M ³ (Modeling, Monitoring and Manufacturing) - a comprehensive approach to controlling ground movements for protecting existing structures and facilities Michael Walker	Session 6B	Session 6B	Session 6B	Session 6B
TRACK 7	Evaluation of the use of lithium nitrate in controlling alkali-silica reactivity in an existing concrete pavement Mike Kelly	Use of self-consolidating concrete in the installation of bulbhead slots - Lessons learned in the use of this innovative concrete material Darrell Morey	Roller compacted concrete for McAlpine lock walls David Kiefer	TRACK 7	Session 7A	Session 7A	Session 7A
Session 8A	Seismic remediation of the Clemson upper and lower diversion dams: deep soil mix construction Ben Foreman	Historical changes in the state-of-the-art of seismic engineering & effects of those changes on the seismic response studies of large embankment dams Samuel Stacy	New Iwakuni runway Vincent Donnelly	Session 7B	Session 7B	Session 7B	Session 7B
Session 5C	Grount courtyains at Arkabutla Dam outlet monolith joints using chemical grout to seal joints, Arkabutla, MS Dale Goss	Mill Creek deep tunnel - Geological affects on proposed structures and construction techniques Joseph Kissane	Earth pressure loads behind the new McAlpine Lock replacement project Mark Harris	TRACK 8	Session 8A	Session 8A	Session 8A
Session 6C	Engineering geology during design and construction of the Marmet lock project Michael Nield	Unpaved road stabilization with chlorides Tres Henn	Use of ultra-fine amorphous colloidal silica to produce a high-density, high-strength rock-matching grout for instrumentation grouting Brian Green	Session 7C	Session 7C	Session 7C	Session 7C
Session 7C	What to do if your dam is expanding: a case study Greg Yankey	Session 8B	Session 8B	Session 7D	Session 7D	Session 7D	Session 7D
Session 8C	Internal erosion and piping at Fern Ridge dam: Problems and solutions Jeremy Britton, Ph.D.	Session 8B	Session 8B	Session 8B	Session 8B	Session 8B	Session 8B
Session 8C	Update on the investigation of the effects of boring sample size (3' vs 5'') on measured cohesion in soft clays Richard Pinner	Session 8B	Session 8B	Session 8B	Session 8B	Session 8B	Session 8B
Session 8C	Geosynthetics and construction of the Bonneville lock and dam second powerhouse corner collector surface flow bypass project Art Fong	Session 8B	Session 8B	Session 8B	Session 8B	Session 8B	Session 8B
Session 8C	Innovative techniques in the Gabion system George Ragazzo	Session 8B	Session 8B	Session 8B	Session 8B	Session 8B	Session 8B
Session 8C	Addressing cold regions issues in pavement engineering Lynette Barria	Session 8B	Session 8B	Session 8B	Session 8B	Session 8B	Session 8B
Session 8C	Geology of New York Harbor - geological and geophysical methods of characterizing the stratigraphy for dredging contracts Ben Baker	Session 8B	Session 8B	Session 8B	Session 8B	Session 8B	Session 8B

12 Noon

Lunch in Exhibit Hall

Break in Exhibit Hall

Wednesday, August 3, 2005 Concurrent Sessions

Structural Engineering Track

	8:00 AM	8:30 AM	9:00 AM	9:30 AM	10:30 AM	11:00 AM	11:30 AM
Room 240	TRACK 12 Civil Works Structural	Recent changes to Corps guidance on steel hydraulic structures	Crack repairs and instrumentation of Greenup L&D miter gate	Recent hydraulic steel structures findings in the Portland district	TRACK 12 Civil Works Structural	Perry Lake gate repair	Mel Price auxiliary lock gate repair (Continued)
	Session 12A	<i>Joe Padula</i>	<i>Doug Kish</i>	<i>Travis Adams</i>	Session 12B	<i>Marvin Parks</i>	<i>Andrew Schimpf</i>
Room 241	TRACK 13 Civil Works Structural	Folsom Dam evaluation of stilling basin performance for uplift loading for historic flows	Rehabilitation of Folsom Dam stilling basin	Seismic stability evaluation of Folsom Dam	TRACK 13 Civil Works Structural	Barge impact guidance for rigid lock walls, ETL 110-2-563 and probabilistic barge impact analysis	Belleville barge accident
	Session 13A	<i>Rick Poepelman</i>	<i>Rick Poepelman</i>	<i>Enrique Matheu</i>	Session 13B	<i>John Clarkson</i>	<i>John Clarkson</i>
Room 242	TRACK 14 Bridges/ Buildings	The USACE bridge management system	Standard procedures for fatigue evaluation of bridges	Fatigue and fracture assessment of Jesse Stuart Highway Bridge	TRACK 14 Bridges/ Buildings	Fatigue analysis of Summit bridge program	Consolidation of Structural criteria for military construction
	Session 14A	<i>Phil Sauser</i>	<i>Phil Sauser</i>	<i>John Jaeger</i>	Session 14B	<i>Jennifer Laning</i>	<i>Jim Chu</i>
							<i>Steve Sweeney</i>

Break in Exhibit Hall

12 Noon

Lunch in Exhibit Hall

	1:30 PM	2:00 PM	2:30 PM	3:00 PM	4:00 PM	4:30 PM	5:00 PM
Room 240	TRACK 12 Civil Works Structural	Overview of John T. Myers locks improvements project	John T. Myers rehabilitation study	Ohio River Greenup Lock extension	TRACK 12 Civil Works Structural	McAlpine lock replacement project, project summary and status of construction	Results of Roller Compacted concrete placement at the McAlpine lock replacement project
	Session 12C	<i>Greg Wernecke</i>	<i>Greg Wernecke</i>	<i>Rodney Cremeans</i>	Session 12D	<i>Kathleen Feger</i>	<i>Scott Wheeler</i>
Room 241	TRACK 13 Civil Works Structural	Portugues Dam, Ponce, Puerto Rico project update	Portugues Dam, Ponce, Puerto Rico, RCC design and testing program	Thermal analysis of hydration and subsequent cooling of RCC	TRACK 13 Civil Works Structural	Miter gate anchorage design	Obermeyer gated spillway project - S381 high pressure steel gates
	Session 13C	<i>Jim Mangold</i>	<i>Jim Hinds</i>	<i>Ahmed Nisar</i>	Session 13D	<i>Andy Harkness</i>	<i>Michael Rannie</i>
Room 242	TRACK 14 Bridges/ Buildings	Unified facilities criteria seismic design for buildings	Seismic requirements for architectural, mechanical and electrical components	Quality assurance for seismic resisting systems	TRACK 14 Bridges/ Buildings	Unified facilities criteria masonry structural design for buildings	Catholic protection of building reinforcing steel web portal (in Diego Garcia)
	Session 14C	<i>Jack Hayes</i>	<i>John Connor</i>	<i>John Connor</i>	Session 14D	<i>Tom Wright</i>	<i>Thomas Tehada</i>
							<i>Mike Pace</i>

Break in Exhibit Hall

Wednesday, August 3, 2005 Concurrent Sessions

Dam Safety Track & Construction Track

	8:00 AM	8:30 AM	9:00 AM	9:30 AM	10:30 AM	11:00 AM	11:30 AM	
Room 224	TRACK 10 Dam Safety Tuttle Creek warning and alert systems	Session 10A Lessons from the dam failure warning system exercise - Tuttle Creek	Session 10A Tuttle Creek ground modification treatability program	Break in Exhibit Hall				TRACK 10 Dam Safety Dam safety analysis of Cannelton Dam
Room 225	TRACK 11 Dam Safety Canton lake spillway stabilization project: IS a test anchor program NECESSARY?	Session 11A Dynamic testing and numerical correlation studies for Folsom dam	Session 11A Status of portfolio risk assessment	Session 10B Mississinewa Dam remediation	Session 10B Wolf creek seepage history	Session 10B Blue dam major rehabilitation	Session 10B Blue dam major rehabilitation	
Room 230	TRACK 19 Construction RMS Update	Session 11A RMS Update (Continued)	Session 11A Updated CQM for Contractors Course	Session 11B Lessons learned on major construction projects	Session 11B Update on safety issues - Safety manual 385-1-1 (continued)	Session 11B Update on safety issues - safety manual 385-1-1 (continued)	Session 11B Update on safety issues - safety manual 385-1-1 (continued)	
Room 231	Session 19A Construction methods in Russia	Session 19A Construction methods in Russia (Continued)	Session 19A Renovating the Pentagon using Design/Build delivery	Session 19B Completion of the Olmsted approach walls (Continued)	Session 19B Completion of the Olmsted approach walls (Continued)	Session 19B Construction management at risk	Session 19B Construction management at risk	
	Session 20A Construction methods in Russia	Session 20A Construction methods in Russia (Continued)	Session 20A Renovating the Pentagon using Design/Build delivery	Session 20B Completion of the Olmsted approach walls (Continued)	Session 20B Completion of the Olmsted approach walls (Continued)	Session 20B Construction management at risk	Session 20B Construction management at risk	

12 Noon

Lunch in Exhibit Hall

	1:30 PM	2:00 PM	2:30 PM	3:00 PM	4:00 PM	4:30 PM	5:00 PM
Room 224	TRACK 10 Dam Safety Project specific risk analysis - Success Dam	Session 10C Clearwater Dam major rehabilitation	Session 10C Dam safety lessons learned, Winter storm 2005, Muskingum & Scioto Basins	TRACK 10 Dam Safety Prompton Dam hydrologic deficiency and spillway modification	Session 10D Problems on the Santa Ana River - Seven Oaks Dam	TRACK 11 Dam Safety "Well, that's water over the dam" - Rough River spillway adequacy design	Session 10D Dam safety program management tools
Room 225	Session 11C 3D Modeling and impact on constructability	Session 11C 3D Modeling and impact on constructability (Continued)	Session 11C 3D Modeling and impact on constructability (Continued)	Session 11D Air Force streamlining Design/Build	Session 11D Air Force streamlining Design/Build (Continued)	Session 11D Sustainable design requirements & construction implementation	Session 11D Sustainable design requirements & construction implementation
Room 230	TRACK 20 Construction Tsunami reconstruction (Continued)	Session 19C Tsunami reconstruction (Continued)	Session 19C Military construction transformation in support of Army transformation	Session 19D MEDCOM Construction Issues	Session 19D MEDCOM Construction Issues (Continued)	Session 19D MEDCOM Construction Issues (Continued)	Session 19D MEDCOM Construction Issues (Continued)
Room 231	Session 20C Tsunami reconstruction (Continued)	Session 20C Tsunami reconstruction (Continued)	Session 20C Military construction transformation in support of Army transformation	Session 20D MEDCOM Construction Issues	Session 20D MEDCOM Construction Issues (Continued)	Session 20D MEDCOM Construction Issues (Continued)	Session 20D MEDCOM Construction Issues (Continued)

Break in Exhibit Hall

Rick Bond

Rick Bond

Rick Bond

Sally Parsons

Andy Constantaras

Andy Constantaras

Andy Constantaras

Andy Constantaras

Fares Abdo

Richard Pruitt

Tommy Schmidt

Robert Kwan

Joel Hoffman

Joel Hoffman

Joel Hoffman

Joel Hoffman

Joel Hoffman

Harry Gioradia

Joel Hoffman

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Wednesday, August 3, 2005 Concurrent Sessions

Electrical & Mechanical Engineering Track

	8:00 AM	8:30 AM	9:00 AM	9:30 AM	10:30 AM	11:00 AM	11:30 AM
Room A	TRACK 15 Military Electrical Tri-Service Electrical Criteria Overview - (Continued)	Tri-Service Panel Tri-Service Electrical Criteria Overview - (Continued)	Tri-Service Panel Tri-Service Electrical Criteria Overview - (Continued)		TRACK 15 Military Electrical Interior/Exterior and security lighting criteria	Tri-Service Panel Information technology systems criteria (Continued)	Tri-Service Panel Information technology systems criteria (Continued)
Room B	Session 15A Tri-Service Panel Building Commissioning	Tri-Service Panel HVAC Commissioning	Tri-Service Panel Ventilation and indoor air quality		Session 15B Tri-Service Panel Ventilation and indoor air quality (Continued)	Tri-Service Panel Refrigerant implications for HVAC specifications, selection, and o&m - now and future (Continued)	Tri-Service Panel Refrigerant implications for HVAC specifications, selection, and o&m - now and future (Continued)
Room D	Session 16A Dale Herron Sustainable design update	Dale Herron HVAC Commissioning	Davor Novosel Ventilation and indoor air quality		Session 16B Davor Novosel Utility systems security and fort future	Mike Thompson Acoustic leak detection for utilities distribution systems (Continued)	Mike Thompson Acoustic leak detection for utilities distribution systems (Continued)
Room E	Session 17A Harry Goradia Emsworth Dam vertical lift gate hoist replacement	Janine Krempa Hydraulic drive for Braddock Dam	Ronald Wridge John Day navigation lock upstream lift gate wire rope failure		Session 17B Vicki L. Van Blaricum Overhead bulkhead at Olmstead Lock	Sean Morefield Replacement of gate # 5 intermediate gear and pinion at RC Byrd Lock and Dam	Richard Nichols Mechanical design issues during construction of McAlpine Lock
Room F	Session 18A John Nites Mass notification system	Janine Krempa Mass notification system (Continued)	Ronald Wridge Electronic card access locks		Session 18B Rick Schultz Lightning protection standards	Brenden McKinley Lightning and surge protection (Continued)	Richard Nichols Lightning and surge protection (Continued)

12 Noon

Lunch in Exhibit Hall

	2:00 PM	2:30 PM	3:00 PM	3:30 PM	4:00 PM	4:30 PM	5:00 PM
Room A	TRACK 15 Military Electrical Mass notification system	Tri-Service Panel Mass notification system (Continued)	Ernesto Go Synchronous condensing with large Kaplan turbine - A case study		TRACK 15 Military Electrical Lightning protection standards	Tri-Service Panel Lightning and surge protection (Continued)	Ernesto Go Synchronous condensing with large Kaplan turbine - A case study
Room B	Session 15C Tri-Service Panel Basic design considerations for geothermal heat pump systems	Tri-Service Panel Basic design considerations for geothermal heat pump systems (Continued)	Ernesto Go Synchronous condensing with large Kaplan turbine - A case study		Session 15D Richard Bouchard Effective use of evaporative cooling for industrial and institutional/office facilities (Continued)	Tri-Service Panel Effective use of evaporative cooling for industrial and institutional/office facilities (Continued)	Ernesto Go Synchronous condensing with large Kaplan turbine - A case study
Room D	Session 16C Gary Phetteplace Hydropower asset management partnership (hydroAMP)	Gary Phetteplace New gas fueled/diesel fueled turbine powered electrical generating station in Iraq	Ernesto Go Synchronous condensing with large Kaplan turbine - A case study		Session 16D Leon Shapiro The Festus/Crystal City levee and pump station project	Leon Shapiro Remote operations for Kaskaskia Dam	Vincent Hoek Technological advances in lock control systems
Room E	Session 17C Lori Rux New coating products for civil works structures	Lester Lowe New guide specification for procurement of turbine oils	Brian Moentenich Synchronous condensing with large Kaplan turbine - A case study		Session 17D Stephen Farkas Acquifer storage and recovery (ASR) system	Shane Nieuirkirk Wastewater infrastructure improvements in Appalachia	Andy Schimpf Storm water pumps
Room F	Session 18C Al Bettelman Mass notification system	John Micetic Mass notification system (Continued)	Brian Moentenich Synchronous condensing with large Kaplan turbine - A case study		Session 18D Gerald Deloach Lightning protection standards	James Sadler Lightning and surge protection (Continued)	Thomas Jamieson Lightning and surge protection (Continued)

Thursday, August 4, 2005 Concurrent Sessions

HH&C Track

	8:00 AM	8:30 AM	9:00 AM	9:30 AM	10:30 AM	11:00 AM	11:30 AM
Room 220	TRACK 1 Sedimentation & New Concepts Session 1E Ice jams, contaminated sediment and structures Clark Fork River, MT <i>Andrew Tuthill</i>	Increased bed erosion due to ice <i>John Hains</i>	Monitoring the Mississippi River using GPS coordinated video <i>James Gushall</i>		Watershed approach to stream stability the reduction of nutrients <i>John B. Smith</i>	Monitoring the effects of sedimentation from Mount St. Helen <i>Alan Donner</i>	Navigation and environmental interests in alleviating repetitive dredging <i>Jason Brown</i>
Room 221	TRACK 2 Water Management Enhancements and new capabilities of HEC-ResSim 3.0 <i>Fauwaz Hambali</i>	Transition to Oracle based data system <i>Joel Asunskis</i>	Accessing real time Mississippi Valley water level data <i>Rich Engstrom</i>		Hurricane Season 2004 <i>Susan Sylvestre</i>	Reevaluation of a project's flood control benefits <i>Ferris Chamberlin</i>	Helmand Valley water management plan <i>Jason Needham</i>
Room 222	Session 2E Red River of the north flood protection project <i>Michael Lesher</i>	Southeast Arkansas flood control & water supply feasibility study <i>Thomas Brown</i>	McCook and Thornton tunnel and reservoir modeling <i>David Kiel</i>		Ala Wai Canal Project, Honolulu, Oahu, Hawaii <i>Lynnette Schapers</i>	Missouri River geospatial decision support framework <i>Brian Baker</i>	Systemic analysis of the Mississippi & Illinois Rivers <i>Dennis Stephens</i>
Room 223	TRACK 4 Modeling Hydrologic models supported by ERDC <i>Robert Wallace</i>	HEC-HMS Version 3.0 new features <i>Jeff Harris</i>	SEEP2D & GMS: Simple tools for solving a variety of seepage problems <i>Clarissa Hansen</i>		Water quality and sediment transport in HEC-RAS <i>Mark Jensen</i>	Advances to the GSSHA program <i>Aaron Byrd</i>	Software integration for watershed studies HEC-WAT <i>Chris Dunn</i>

12 Noon

Lunch

	1:30 PM	2:00 PM	2:30 PM	3:00 PM	3:30 PM	4:00 PM	4:30 PM
Room 220	TRACK 1 Water Quality Management San Francisco Bay Mercury TMDL- Implications for constructed wetlands <i>Herb Fredrickson</i>	Abandoned mine land: Eastern and Western perspectives <i>Kate White</i>	A lake tap for temperature control tower construction at Cougar Dam <i>Steve Schlenker</i>		Demonstrating innovative river restoration technologies: Truckee River, NV <i>Chris Dunn</i>	Comprehensive watershed restoration in the Buffalo district <i>Anthony Friona</i>	Translating the hydrologic tower of Babel <i>Dan Crawford</i>
Room 221	TRACK 2 Water Management Developing reservoir operation plans to manage erosion <i>Patrick O'Brien</i>	New approaches to water management decision making <i>James Barton</i>	Improved water supply forecasts for Kootenay basin using principal components regression <i>Randal Wortman</i>		Prescriptive reservoir modeling and ROPE study <i>Jason Needham</i>	Missouri River mainstem operations <i>Larry Murphy</i>	Res-Sim model for the Columbia River <i>Arun Mybathanan</i>
Room 222	TRACK 3 Section 227 Section 227 Workshop/Program Review <i>William Curtis</i>	Section 227 Workshop/Program Review (Continued) <i>William Curtis</i>	Section 227 Workshop/Program Review (Continued) <i>William Curtis</i>		Section 227 Workshop/Program Review (Continued) <i>William Curtis</i>	Section 227 Workshop/Program Review (Continued) <i>William Curtis</i>	Section 227 Workshop/Program Review (Continued) <i>William Curtis</i>
Room 223	TRACK 4 Modeling Little Calumet River unsteady flow model conversion <i>Rick Ackerson</i>	Kansas City River basin model <i>Edward Parker</i>	Design guidance for breakup ice control <i>Andrew Tuthill</i>		Forebay flow simulations using Navier-Stokes code <i>Charlie Berger</i>	Use of regularizatio as a method for watershed model calibration <i>Brian Skahill</i>	Demonstration program in the arid southwest <i>Margaret Jonas</i>

Break

Thursday, August 4, 2005 Concurrent Sessions

Geotechnical Track

	8:00 AM	8:30 AM	9:00 AM	9:30 AM	10:30 AM	11:00 AM	11:30 AM
TRACK 5	Dynamic deformation analyses Dewey Dam Huntington District Corps of Engineers	Seismic stability evaluation for Ute Dam, NM	An overview of criteria used by various organizations for assessments and seismic remediation of earth dams		USACE seepage berm design criteria and district practices	Ground penetrating radar applications for the assessment of airfield pavements - Peru road project	
Session 5E	<i>Greg Yankey</i>	<i>John France</i>	<i>Sean Carter</i>		<i>George Sills</i>	<i>Lulu Edwards</i>	<i>Michael Wielputz</i>
TRACK 6	Small geotechnical project, big stability problem - The Block Church Road experience	Geophysical investigation of foundation conditions beneath Folsom Dam	Bioengineering slope stabilization techniques coupled with traditional engineering applications - The result a stable slope		Shoreline armor stone quality issues	Mt Mill Creek - An urban flood control challenge	Next stop, The Twilight Zone
Session 6E	<i>Jonathan Kolber</i>	<i>Jose Llopis</i>	<i>Bethany Bearmore</i>		<i>Joseph Kissame</i>	<i>Monica Greenwell</i>	<i>Troy O'Neal</i>
TRACK 7	The geotechnical and structural issues impacting the Dalles spillway construction	The Dalles spillway engineering and design	The future of the discrete element method in infrastructure analysis		Evaluating the portable falling weight deflectometer as a low-cost technique for post-seasonal load restrictions on low volume pavements	Soil structure interaction effects in the seismic evaluation of success dam control tower	Olmsted locks and Dam project geotechnical/construction issues
Session 7E	<i>Kristie Harfai</i>	<i>Kristie Harfai</i>	<i>Raju Kala</i>		<i>Maureen Kestler</i>	<i>Michael Sharp</i>	<i>Jeff Schaefer</i>
TRACK 8	Rubblization of airfield concrete pavement	US Army airfield pavement assessment program	Critical state for probabilistic analysis of levee underseepage		Curing practices for modern concrete construction	AAR at Carters Dam, a different approach	Concrete damage at Carters Dam, GA
Session 8E	<i>Eileen Velez-Vega</i>	<i>Haley Parsons</i>	<i>Douglas Crum</i>		<i>Toy Poole</i>	<i>James Sanders</i>	<i>Toy Poole</i>

Break in Exhibit Hall

Lunch

12 Noon

	1:30 PM	2:00 PM	2:30 PM	3:00 PM	3:30 PM	4:00 PM	4:30 PM
TRACK 5	Slope stability evaluation of the Baldhill Dam right abutment	Lateral pile load test results within a soft cohesive foundation	Design and construction of anchored bulbheads for river diversion, Seabrook, NH		Characterization of soft marine clays - A case study at Craney Island	50 years of NRSC experience with engineering problems caused by dispersive clays	Changes in the post-tensioning institutes new (4th Ed. 2004) "Recommendations for prestressed rock and soil anchors"
Session 5G	<i>Neil Schwanz</i>	<i>Richard Varuso</i>	<i>Siamaq Vaghar</i>		<i>Aaron Zdinak</i>	<i>Danny McCook</i>	<i>Michael McCray</i>
TRACK 6	Perils in back analysis failures	Reconstruction of deteriorated lock walls concrete after blasting and other demolition removal techniques	Flood fighting structures demonstrations and evaluation program		Innovative design concepts incorporated into a landfill closure and reuse design	Laboratory testing of flood fighting structures	Bluff stabilization along Lake Michigan using active and passive dewatering techniques
Session 6G	<i>Greg Yankey</i>	<i>Steve O'Connor</i>	<i>George Sills</i>		<i>Dave Ray</i>	<i>Johannes Wibowo</i>	<i>Eileen Glynn</i>
TRACK 7	Geotechnical instrumentation and foundation re-evaluation of John Day lock and Dam, Columbia River, Oregon-Washington	A study of the long term performance of seepage cutoff barriers in dams	Design, construction, and performance of seepage barriers for Guanella Dam, near Empire, CO		Sensitive infrastructure sites and structures - Sonic drilling offers quality control and non-destructive advantages to geotechnical construction drilling	Subgrade failure criteria according to soil type and moisture condition	The automated stability monitoring of the Mississippi River levees using the range scan system
Session 7G	<i>David Scofield</i>	<i>John Rice</i>	<i>John France</i>		<i>John Davis</i>	<i>Edel Cortez</i>	<i>Robert Jolisian</i>
TRACK 8	Damaging interactions among concrete materials	Economic effects on construction of uncertainty in test methods	Major issues in materials specifications		Spall and intermediate-sized repairs for PCC pavements	Acceptance criteria for unbonded aggregate road surfacing materials	Effective partnering to overcome an interruption in the supply of Portland cement during construction of Marmet lock and Dam
Session 8G	<i>Toy Poole</i>	<i>Toy Poole</i>	<i>Toy Poole</i>		<i>Reed Freeman</i>	<i>Reed Freeman</i>	<i>Billy Nealey</i>

Break

Thursday, August 4, 2005 Concurrent Sessions

Geotechnical, Specifications, Electrical & Mechanical Engineering & Construction Tracks

	8:00 AM	8:30 AM	9:00 AM	9:30 AM	10:30 AM	11:00 AM	11:30 AM
Room 225	TRACK 9 Geotechnical Seepage Committee Meeting (Continued)	Session 9E GROUP DISCUSSION Seepage Committee Meeting (Continued)	TRACK 21 Specifications SpecInact-Demonstration of the SI explorer, publishing to PDF and Word	Session 9F GROUP DISCUSSION UFGS transmittal to Master-Format 2004	TRACK 9 Geotechnical GMCoP Forum (Continued)	Session 9E GROUP DISCUSSION SpecInact - Demonstration of the SI explorer, publishing to PDF and Word	TRACK 21 Specifications Project specifications for the upper tier Folsom outlet works modifications
Room 232	Session 21E <i>Patricia Robinson</i>	Session 21E <i>Patricia Robinson</i>	Session 21E <i>Patricia Robinson</i>	Session 21F <i>Carl Kersten</i>	Session 21F <i>Carl Kersten</i>	Session 21F <i>Steve Freitas</i>	Session 21F <i>Steve Freitas</i>
Room A	TRACK 15 Military Electrical Electronic Security (Continued)	TRACK 15 Military Electrical Electronic Security (Continued)	TRACK 15 Military Electrical Electronic Security (Continued)	TRACK 15 Military Electrical Electrical safety and arc flash UFC	TRACK 15 Military Electrical Electrical safety and arc flash UFC (Continued)	TRACK 15 Military Electrical Electrical safety and arc flash UFC (Continued)	TRACK 15 Military Electrical Electrical infrastructure in Iraq - Restore Iraqi electricity
Room B	Session 15E <i>Tri-Service Panel</i>	Session 15E <i>Tri-Service Panel</i>	Session 15E <i>Tri-Service Panel</i>	Session 15F <i>Tri-Service Panel</i>	Session 15F <i>Tri-Service Panel</i>	Session 15F <i>Tri-Service Panel</i>	Session 15F <i>Tri-Service Panel</i>
	TRACK 16 Military Mechanical Lon works technology update	TRACK 16 Military Mechanical Lon works technology update	TRACK 16 Military Mechanical Lon works technology update	TRACK 16 Military Mechanical Prelabricated Chiller Plants	TRACK 16 Military Mechanical Prelabricated Chiller Plants	TRACK 16 Military Mechanical Prelabricated Chiller Plants	TRACK 16 Military Mechanical Design considerations for the prevention of mold
Room D	Session 16E <i>David Schwenk</i>	Session 16E <i>David Schwenk</i>	Session 16E <i>David Schwenk</i>	Session 16F <i>Trey Austin</i>	Session 16F <i>Trey Austin</i>	Session 16F <i>Trey Austin</i>	Session 16F <i>Trey Austin</i>
	TRACK 17 Civil Mechanical Lessons learned on flood water pump stations	TRACK 17 Civil Mechanical Lessons learned on flood water pump stations	TRACK 17 Civil Mechanical Lessons learned on flood water pump stations	TRACK 17 Civil Mechanical Lock gate replacement system	TRACK 17 Civil Mechanical Lock gate replacement system	TRACK 17 Civil Mechanical Lock gate replacement system	TRACK 17 Civil Mechanical Automated closure gate design for Duck creek flood control
Room 230	Session 17E <i>Mark Robertson</i>	Session 17E <i>Mark Robertson</i>	Session 17E <i>Mark Robertson</i>	Session 17F <i>Will Smith</i>	Session 17F <i>Will Smith</i>	Session 17F <i>Will Smith</i>	Session 17F <i>Will Smith</i>
	TRACK 19 Construction NAVFAC Construction scheduling	TRACK 19 Construction NAVFAC Construction scheduling	TRACK 19 Construction NAVFAC Construction scheduling	TRACK 19 Construction Self-consolidating concrete	TRACK 19 Construction Self-consolidating concrete	TRACK 19 Construction Self-consolidating concrete	TRACK 19 Construction Self-consolidating concrete
Room 231	Session 19E <i>Glenn Saito</i>	Session 19E <i>Glenn Saito</i>	Session 19E <i>Glenn Saito</i>	Session 19F <i>Beatrix Kerhoff</i>	Session 19F <i>Beatrix Kerhoff</i>	Session 19F <i>Beatrix Kerhoff</i>	Session 19F <i>Beatrix Kerhoff</i>
	TRACK 20 Construction Update on DAWIA and Facilities Engineering	TRACK 20 Construction Update on DAWIA and Facilities Engineering	TRACK 20 Construction Update on DAWIA and Facilities Engineering	TRACK 20 Construction S&A Update	TRACK 20 Construction S&A Update	TRACK 20 Construction S&A Update	TRACK 20 Construction S&A Update
	Session 20E <i>Mark Grammer</i>	Session 20E <i>Mark Grammer</i>	Session 20E <i>Mark Grammer</i>	Session 20F <i>Harry Jones</i>	Session 20F <i>Harry Jones</i>	Session 20F <i>Harry Jones</i>	Session 20F <i>Harry Jones</i>
Room 225	TRACK 9 Geotechnical Seismic Manual (Continued)	TRACK 9 Geotechnical Seismic Manual (Continued)	TRACK 9 Geotechnical Seismic Manual (Continued)				
	Session 9G GROUP DISCUSSION	Session 9G GROUP DISCUSSION	Session 9G GROUP DISCUSSION				

Break in Exhibit Hall

Lunch

12 Noon

	1:30 PM	2:00 PM	2:30 PM	3:00 PM	3:30 PM	4:00 PM	4:30 PM
Room 225	TRACK 9 Geotechnical Seismic Manual (Continued)	TRACK 9 Geotechnical Seismic Manual (Continued)	TRACK 9 Geotechnical Seismic Manual (Continued)				
	Session 9G GROUP DISCUSSION	Session 9G GROUP DISCUSSION	Session 9G GROUP DISCUSSION				

Thursday, August 4, 2005 Concurrent Sessions

Dam Safety Track & Structural Engineering Track

	8:00 AM	8:30 AM	9:00 AM	9:30 AM	10:30 AM	11:00 AM	11:30 AM
Room 224	TRACK 10 Dam Safety	Seepage and stability, final evaluation for reservoir pool raising project, Terminus Dam, Kaweah River, CA <i>Michael Ramsotham</i>	Initial filling plan, Terminus dam spillway enlargement, Terminus Dam, Kaweah River, CA <i>Michael Ramsotham</i>	Hydrologic aspects of operating in a "failure mode" - Fern Ridge Lake, OR <i>Bruce Duffe</i>	TRACK 10 Dam Safety	A dam safety study involving cascading dam failures <i>Gordon Lance</i>	The relationship of seismic velocity to the erodibility index <i>Joseph Topi</i>
Room 240	Session 10E	London lock and dam, West Virginia major rehabilitation project <i>David Sullivan</i>	Replacing existing lock 4-Innovative designs for Charleroi lock <i>Steveb Stoltz</i>	Use of non-linear incremental structural analysis in the design of the Charleroi lock <i>Randy James</i>	Session 10F	Olmsted dam in-the-wet construction methods <i>Lynn Raque</i>	John Day lock monolith repair <i>Mathew Hanson</i>
Room 241	TRACK 13 Civil Works Structural	Chicago shoreline project <i>Jan Plachia</i>	Structural assessment of Bluestone Dam <i>Robert Reed</i>	Duck Creek, OH local flood protection projection phase III Culvert damage <i>Jeremy Nichols</i>	TRACK 13 Civil Works Structural	Design of concrete lined tunnels in rock <i>David Force</i>	Indianapolis north phase IIIA project <i>Gene Hoard</i>
Room 242	TRACK 14 Bridges/Buildings	Urban search & rescue program overview <i>Tom Niedernhofer</i>	Evaluation and repair of blast damaged reinforced concrete beams <i>John Hudson</i>	Single degree of freedom blast effects spreadsheets <i>Dale Nebuda</i>	Session 13F	Development of design criteria for the Rio Puerto Nuevo contract 2D/2E channel wall <i>Jana Tanner</i>	U.S. general services administrative progressive collapse design guidelines applied to concrete moment-resisting frame buildings <i>David Billow</i>

Break in Exhibit Hall

12 Noon

Lunch

	1:30 PM	2:00 PM	2:30 PM	3:00 PM	3:30 PM	4:00 PM	4:30 PM
Room 224	TRACK 10 Dam Safety	Dam safety instrumentation data management utilizing WinIDP to aid data collection and evaluation <i>Travis Tutka</i>	Automated instrumentation assessments at Marmet lock & Dam <i>Ronald Rakes</i>	Potential failure mode analysis of Eau Claire Dam <i>David Rydeen</i>	TRACK 10 Dam Safety	Dam safety officers panel - The Good <i>Bruce Murray</i>	Dam safety officers panel - The Bad <i>Bruce Murray</i>
Room 240	Session 10G	Inner Harbor navigation canal and lock structure <i>Mark Gonski</i>	Design features and challenges of the Comite River diversion project <i>Christopher Dunn</i>	Waterline support failure on the Harvey canal: A case study <i>Angela DeSoto Duncan</i>	Session 10H	Public appeal of major civil projects- The good, the bad and the ugly <i>Bruce Murray</i>	Chickamauga lock and Dam height optimization study using Monte Carlo simulation <i>Leon Schieber</i>

Break



2005 Tri-Service Infrastructure Systems Conference & Exhibition
“Re-Energizing Engineering Excellence”
August 2-4, 2005
St. Louis, MO



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Critical State for Probabilistic Analysis of Levee Underseepage

Douglas Crum, P. E.





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1. Failure Prediction
2. Reliability
3. Levee Underseepage
4. Surcharge Factor
5. Evidence (Case Histories)
6. Recommendations





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Levee Consequences & Damages

- Impending Failure Mechanism
- Prediction of Limit State (Collapse)
- Not Design Criteria





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Reliability Criteria

- PGL No. 26 (1991)
 - Requires reliability approach for levees
 - Mentions PFP/PNP
- ETL 1110-2-328 (1993)
 - Template Method
- ER 1105-2-101 (1996)
 - Requires risk analysis for flood damage reduction studies
- EM 1110-2-1619 (1996)
 - Economics
- ETL 1110-2-556 (1999)
 - Geotechnical risk analysis for planning studies
 - Appendix B, “Evaluating the Reliability of Existing Levees”





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Reliability

Methods

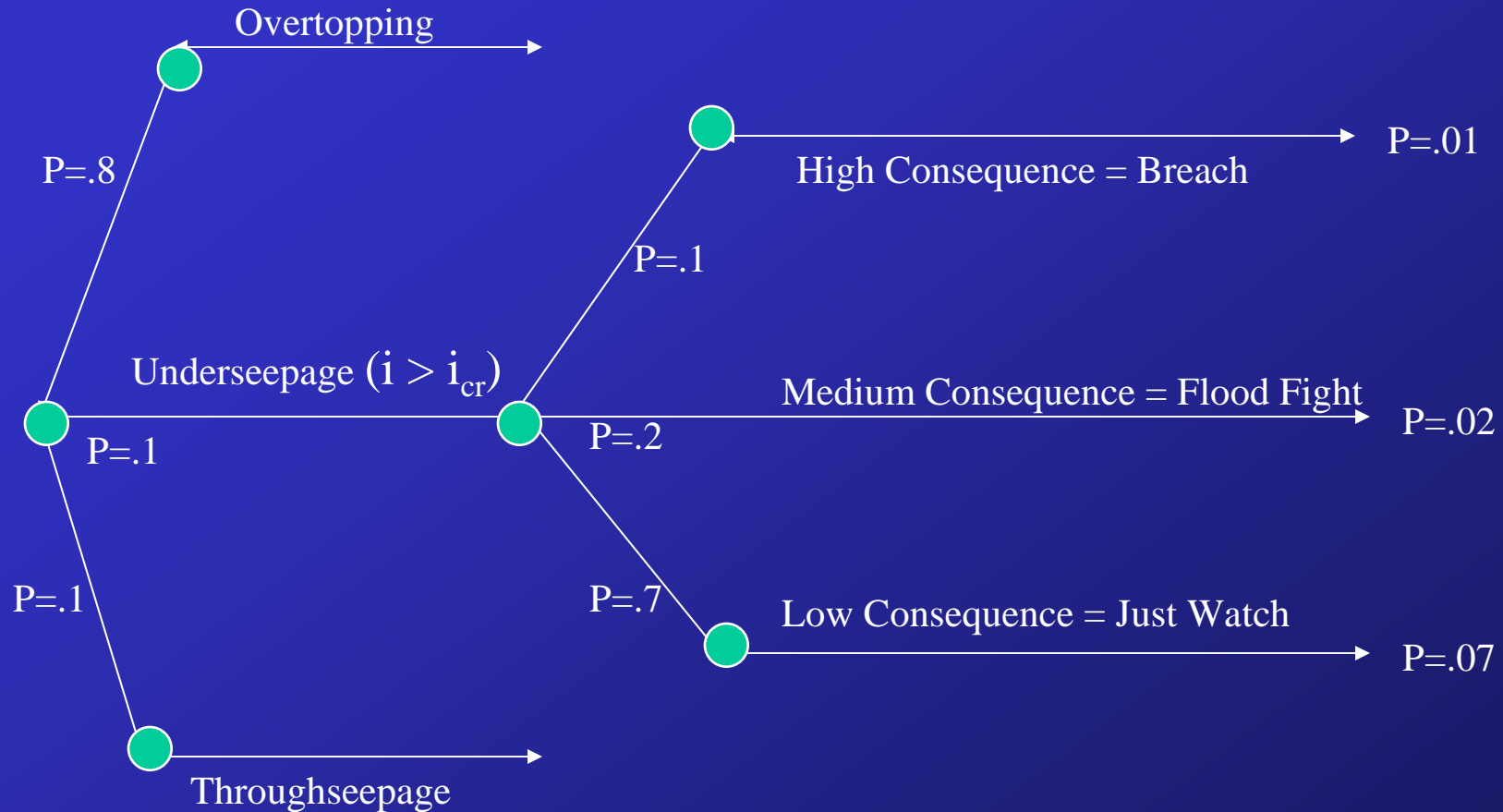
- Taylor's Series (first order – second moment)
- Point Estimate
- Advanced Method (Hasofer & Lind)
- Monte Carlo





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Reliability





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LEVEE FAILURE MODES

- Overtopping



- Other (Scour, Trees, etc.)



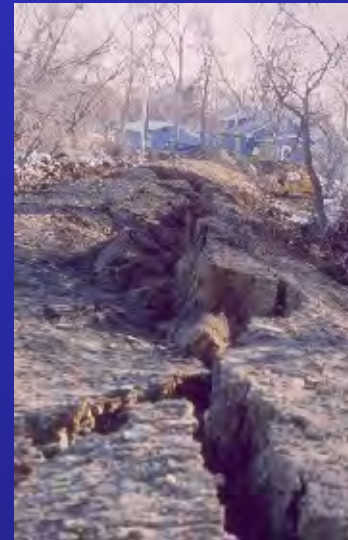


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LEVEE FAILURE MODES

Slides

- End of Construction
- **Steady State Seepage**
- Rapid Drawdown
- Seepage



Under-seepage



Through-seepage



Pipes/Structures





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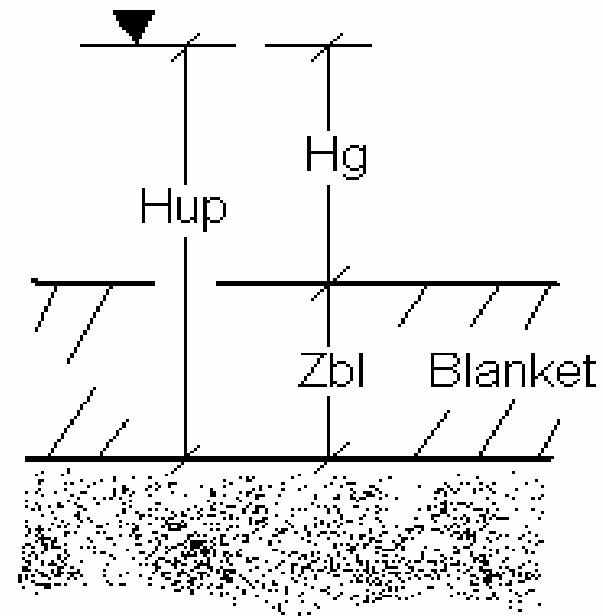
Levee Underseepage: Piping and Heave

$$FS_g = \frac{\gamma_{sat} \cdot Z_{bl} - \gamma_w \cdot Z_{bl}}{\gamma_w \cdot H_g}$$

$$FS_{up} = \frac{\gamma_{sat} \cdot Z_{bl}}{\gamma_w \cdot H_g + \gamma_w \cdot Z_{bl}}$$

At critical state:

$$FS_{up} = FS_g = 1$$





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Performance Function

$$FS_g = i_{cr}/i$$

Critical state at “quick conditions” is when effective stress throughout layer is reduced to zero.

$$i_{cr} = \gamma_b / \gamma_{h20} = (G_s - 1) / (1 + e)$$





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Unsatisfactory Performance at the Critical Gradient

$$FS_g = i_{cr}/i$$

Capacity (C) = i_{cr} = critical gradient

Demand (D) = i = calculated gradient

Normally distributed, uncorrelated:

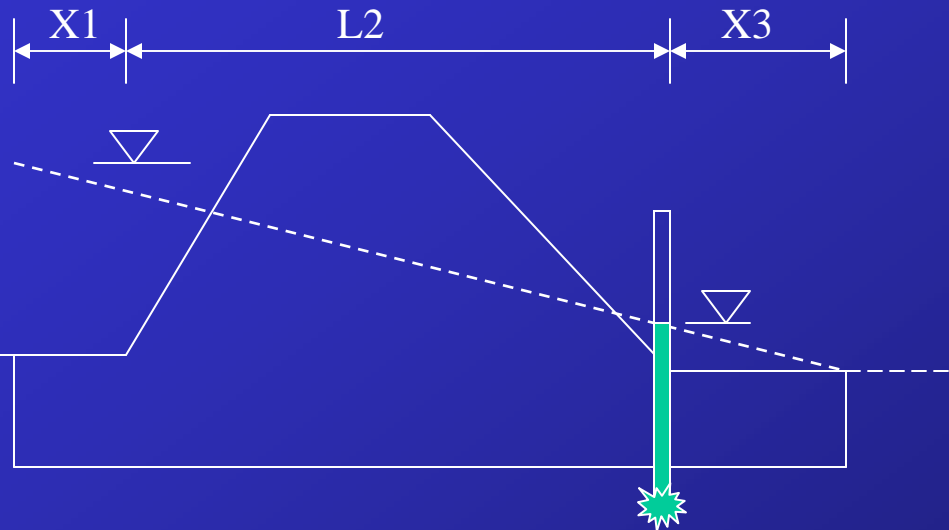
$$\beta = \frac{E(C) - E(D)}{\sqrt{\sigma_C^2 + \sigma_D^2}}$$





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Levee Underseepage



$$m_{\text{toe}} = \frac{X3}{X1 + L2 + X3}$$



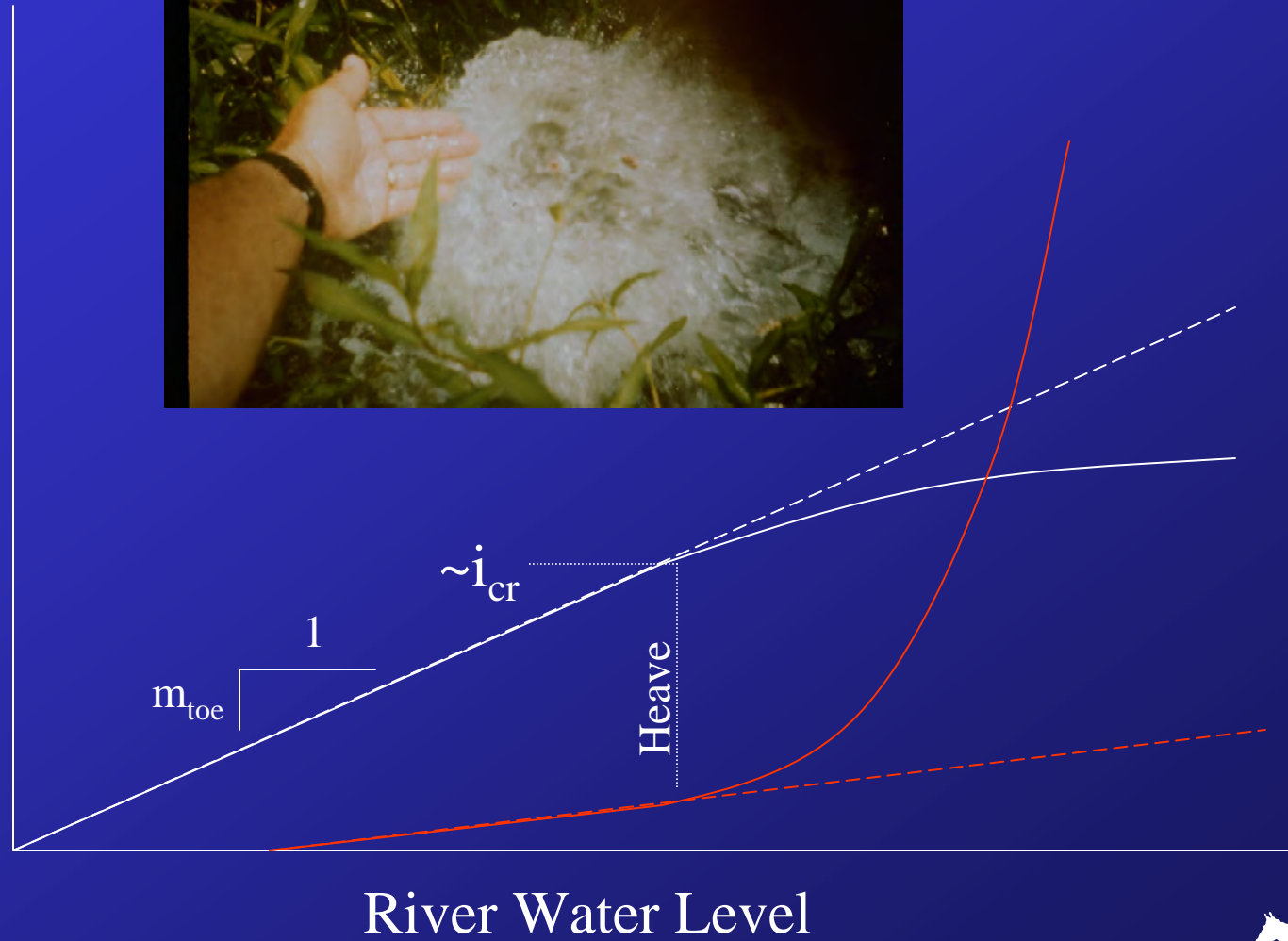


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Levee Underseepage: Extrapolated Gradient

Flow, Q (gpm/lf)

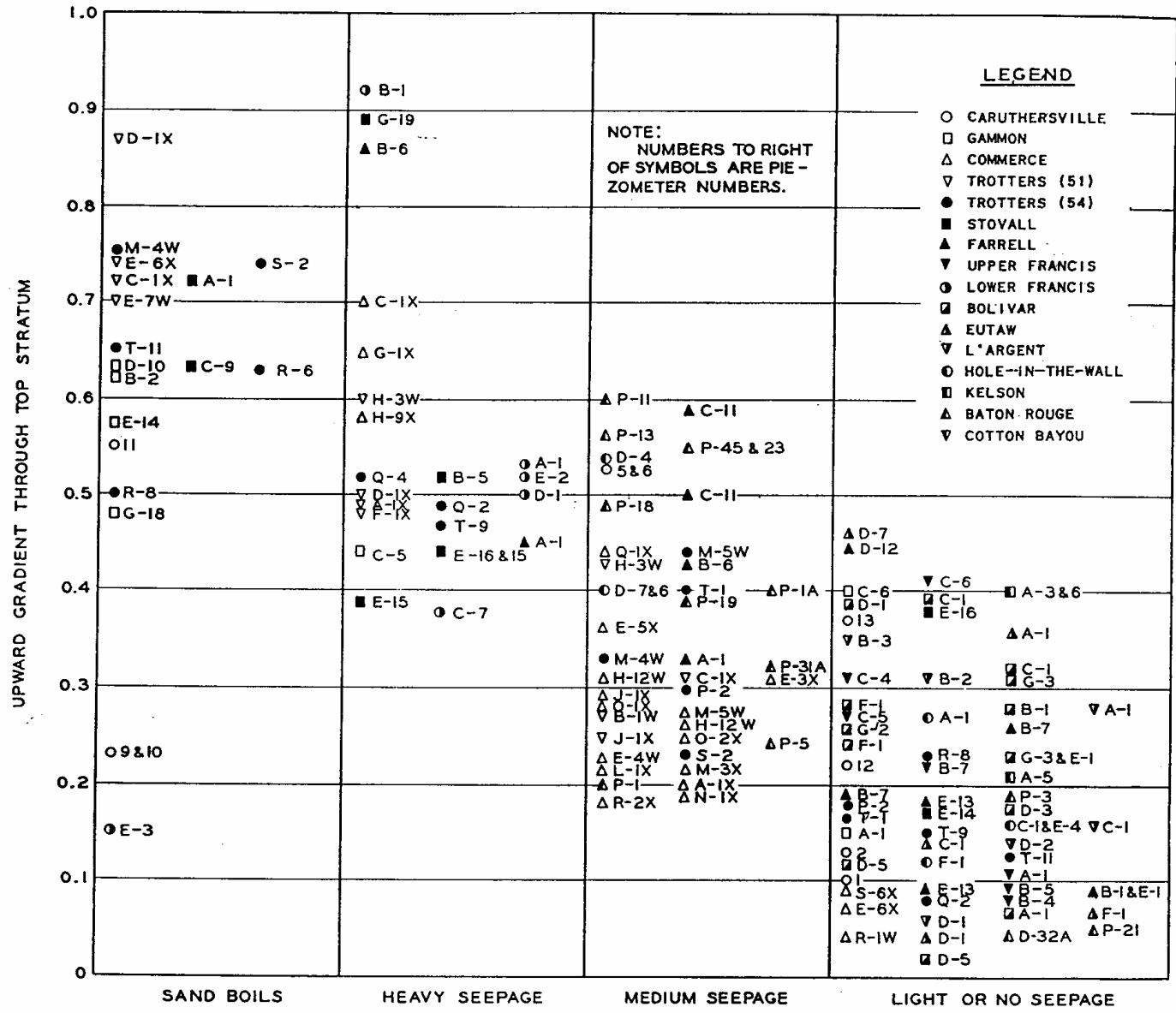
Piezometer level at toe





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WES
Technical
Memorandum
3-424
Figure 47
(1956)





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CASE 1: Kansas City District, Historic Design Criteria for Agricultural Levees

- No past boil activity, $FS_g = 1$
- Minor boil or heavy seepage, $FS_g = 1.25$
- Major boil activity, $FS_g = 1.5$

The ratio 1:1.5 approximates
(Critical State : Failure State).

$$\rightarrow (i_{cr}/i_f) = 1/1.5 = 0.67 \cong 0.7$$

References:

Design memorandum no. 1 – underseepage control – levee unit 400-L, 20 Nov. 1953

Design memorandum no. 1 – underseepage control – levee unit 406-L, revised 24 mar 1953





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CASE 2: Rock Island District, Historic Design Criteria for Agricultural Levees

- “The Rock Island District has a philosophy..... to organize the necessary men and equipment to put up a flood fight. ...they feel justified in allowing major boils to develop...”
- Design criteria at toe: $FS_g > 0.7$
Assuming a necessary flood fight to prevent a breach is tantamount to failure, $i = i_f$
 $\rightarrow (i_{cr}/i_f) = FS_g = 0.7$

Reference:

Rock Island District Levee Practices, MRKED-F Memorandum for Branch File,
25 October 1962.





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CASE 3: Kansas City District, Back Calculation from 1952 Flood

Computed FS_g at flood crest	Seepage Conditions during flood crest
< 0.55	Objectionable seepage, major flood fight, boils requiring sandbagging
> 0.8	Tolerable Seepage, distributed seepage, pin boils

$$\rightarrow (i_{cr}/i_f) = (.55/.8) = 0.6875 \cong 0.7$$

Reference:

Meeting at MRD on Underseepage Control on Agricultural Levees, 27 November 1962.





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CASE 4: St. Louis District, Back Calculation from 1993 Flood

Bois Brule & Kaskaskia Island levee failures

- Both failures were due to underseepage and resulted in an actual breach of the levee.
- Back calculated gradient = 1.35
- Assume $i_{cr} \cong 0.85$

$$\rightarrow (i_{cr}/i_f) = (.85/1.35) = 0.63$$

Reference:

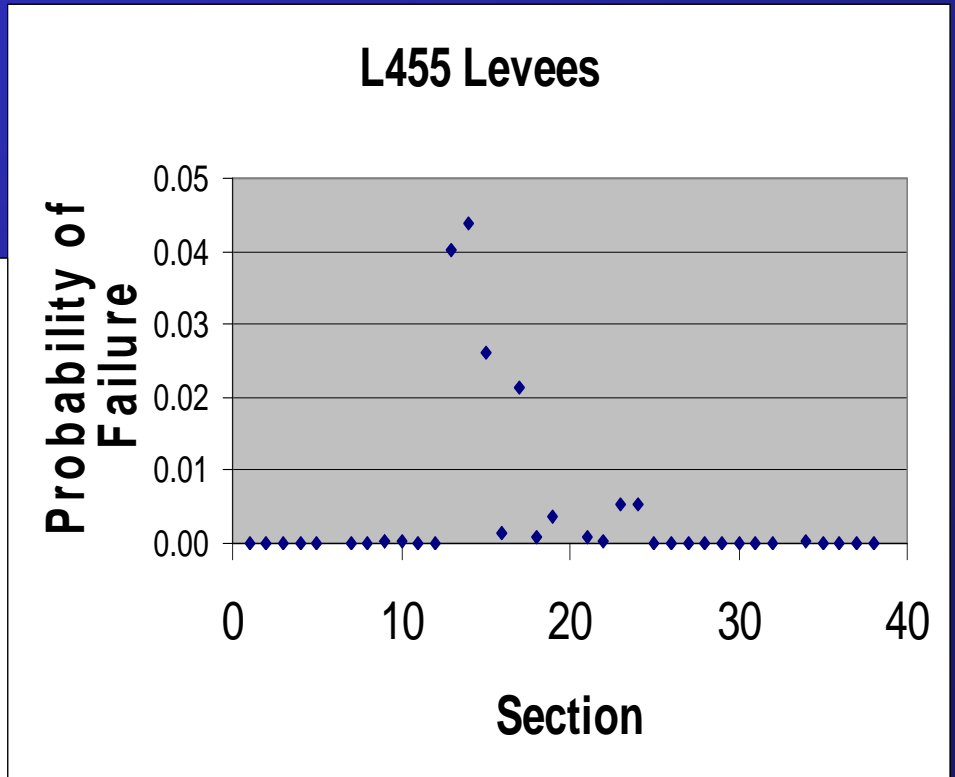
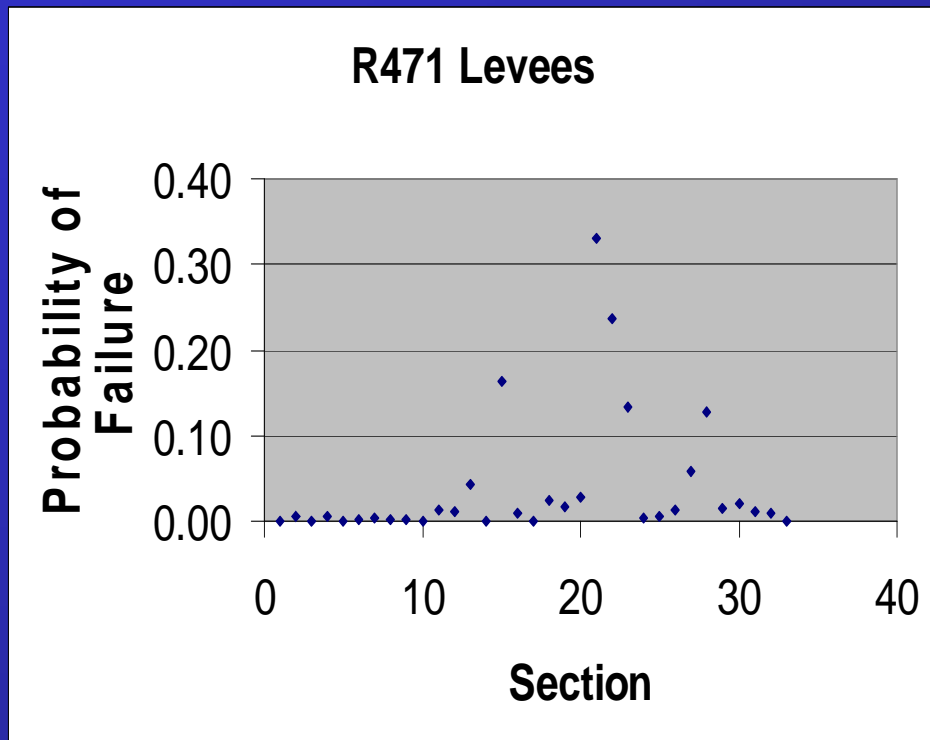
Communication with Mr. Edward Demsky, CEMVS, 19 July 2004





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CASE 5: 1993 Flood Calibrations for Existing Projects





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Unsatisfactory Performance at the Critical Gradient

$$FS_g = i_{cr}/i$$

Capacity (C) = i_{cr} = critical gradient

Demand (D) = i = calculated gradient

Normally distributed, uncorrelated:

$$\beta = \frac{E(C) - E(D)}{\sqrt{\sigma_C^2 + \sigma_D^2}}$$





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Unsatisfactory Performance at Impending Failure

$$FS = i_f / i$$

Surcharge Factor = $(i_{cr} / i_f) \cong 0.7$

Capacity (C) = $i_f = i_{cr} / (i_{cr} / i_f)$ = “failure” gradient

Demand (D) = i = calculated gradient (extrapolated)

Normally distributed, uncorrelated:

$$\beta = \frac{E(C) - E(D)}{\sqrt{\sigma_C^2 + \sigma_D^2}}$$





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Recommendations

- Rational methods are necessary for deriving the Limit State from design criteria
- A consistent methodology should be adopted
- Impending Levee Breaches Occur near a Surcharge Factor of $(i_{cr}/i_f) = 0.7$





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Design Criteria Concerns

- Deterioration of Levee from Past Seepage Distress
- Flood Fight Capability
- Managing Risk & Consequences (Urban/Rural/Agricultural)
- Affect on B/C ratio





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From executive summary, “*Risk Analysis and Uncertainty in Flood Damage Reduction Studies*”, National Academy Press, (2000).

“The committee recommends that the Corps undertake statistical ex post studies to compare predictions of geotechnical levee failure probabilities made by the reliability model against frequencies of actual levee failures during floods.”





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~~Questions~~ Comments

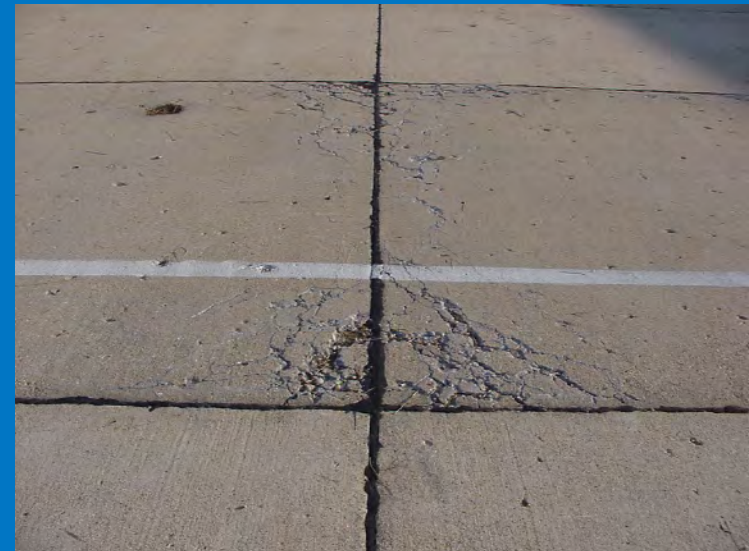
Doug Crum

816/983-3604

douglas.a.crum@usace.army.mil



Spall and Intermediate-Sized Repairs for PCC Pavements



Reed Freeman and Travis Mann

US Army Engineer Research and Development Center



Joint Rapid Airfield Construction (JRAC) Program

- Site Selection
- Enhanced Construction Technology
- Rapid Stabilization

... develop materials and techniques for rapidly upgrading existing or constructing new contingency airfields in-theater with a low logistical footprint.



Problem Statement

- Existing airfields are typically in poor shape. However, they are essential to operations
 - strategic locations
 - better than starting from scratch
- Military demands extremely fast “return to service” time
 - Rapid Repair – 24 hours
 - Very Rapid Repair – 3 hours



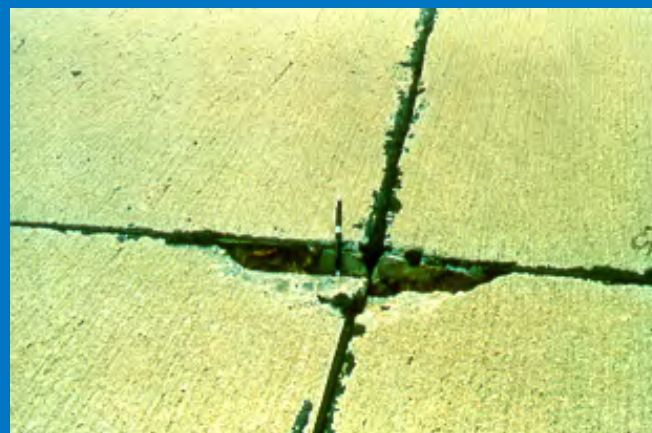
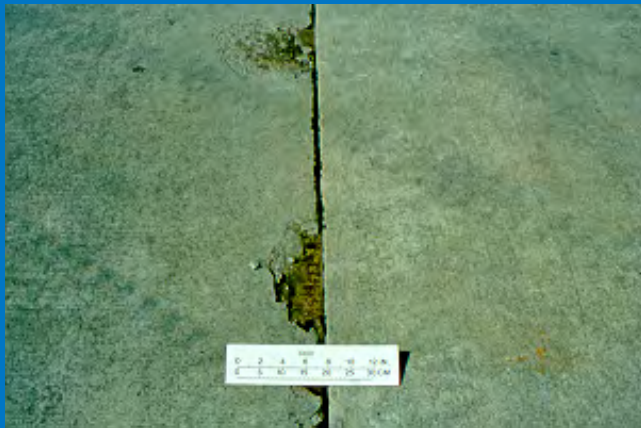
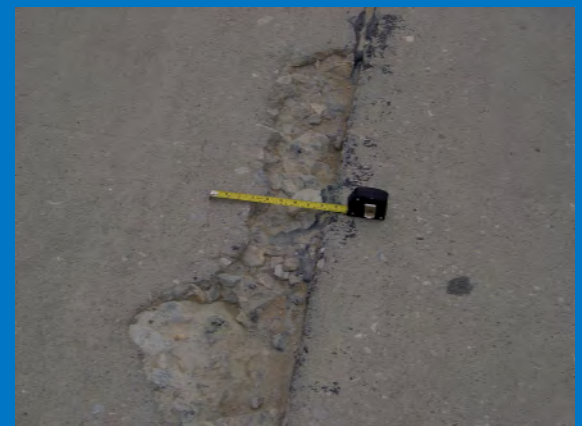
Project Plan

- **FY04: partial-depth spall repair**
 - PCC-surfaced and AC-surfaced
- **FY05: partial replacement of PCC slabs**
 - 1 cu.ft. < size of repair < 1 cu.yd.
- **FY06: secure cracked surfaces**
 - reduce FOD potential
- **FY07: repair structurally deteriorated AC surfaces**
 - also, program-wide demonstration for C-17



FY04 – Spall Repair

- **Specific Problem:**
 - many materials on the market
 - wide range of performances
 - need to define when to use what



FY04 – Scope

- **Spalls**
 - Surficial, not structural
 - Size that can be handled by a portable mixer
- **Asphalt and concrete surfaces**
- **Products**
 - Recommendations for materials and procedures
 - Establish material approval process
 - physical and mechanical requirements

Repair Requirements

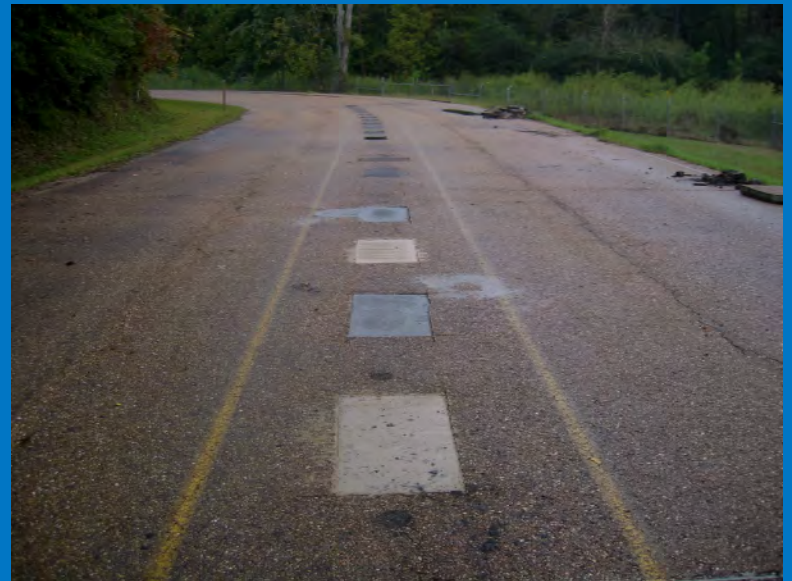
- Ready for C-17 in less than 1 day (“rapid repairs”) or 3 hours (“very rapid repairs”)
 - Consistent with ASTM C 928
- Simple procedures and little equipment
- Should last a couple of years and sustain several thousand aircraft operations

Materials

- **Polymeric**
 - Delcrete
- **Asphaltic**
 - Quality Pavement Repair
 - Instant Road Repair
- **Cementitious**
 - Set-45
 - PaveMend
- **Aggregate**
 - Pea gravel



'Field' Placements



'Field' Placements

Load Cart

HVS



'Field' Placements



Field Placements – Findings

- **Delcrete**
 - Resists cracking
 - No rutting
 - Abraded by dozer blade
 - Not for use on asphalt concrete
 - Cumbersome
 - Expensive



Field Placements – Findings

- **Asphaltic materials**
 - Difficult to compact adequately
 - Couldn't conform to irregularities
 - Both QPR and IRR rutted
 - QPR remained soft
 - Cheap



Field Placements – Findings

- **Set 45**
 - Mortar mixer required
 - Vibration and floating required
 - Particularly for “extended” mix
 - Good bond
 - Good color match for PCC
 - No cracking



Field Placements – Findings

- **PaveMend**
 - Drill and paddle mixer
 - Self-leveling
 - Excellent bond
 - Conformed to irregularities
 - No cracking
 - Technicians' favorite



Field Placements – Findings

- **PaveMend**
 - Used successfully as a leveling material



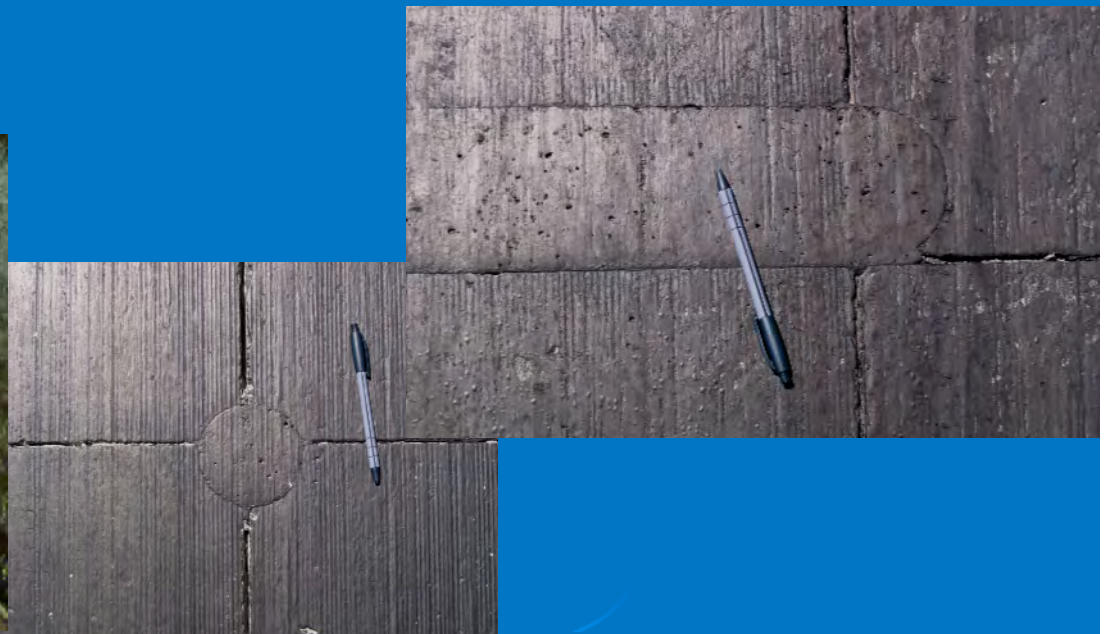
Field Placements – Findings

- **Feathering**
 - Works for:
 - neat Set 45
 - and PaveMend
 - PCC pavement
 - No good for:
 - Delcrete
 - mixes extended with aggregate
 - AC pavement



Field Placements – Findings

- **Repairs at Joints**
 - Delcrete – can place through joint
 - Cementitious – place against joint filler



Field Placements – Findings

- **Accounting for climate**

- PaveMend and Set45

- > 85 °F

- PM30 and Set45-HW

- cool materials, water, and repair surface

- extend with rounded gravel (max. particle size = ½ in.)

- < 45 °F

- PM5 or PM15 and Set45

- warm materials, water, and repair surface

- Delcrete NG > 95 °F

- Asphaltic materials NG < °32

Material Approval Process

- Cementitious Materials Only
- Include physical and mechanical considerations
- Use standard test procedures
- Learn from REMR study by ERDC (mid-1990's)

Physical Property Requirements (1 of 2)

- **Flow (for grouts)**
 - Maximum = 80 sec
 - 'self-leveling'
- **Coefficient of thermal expansion**
 - Maximum = $7 \times 10^{-6} / ^\circ\text{F}$
- **Freeze-thaw resistance**
 - Maximum loss in dynamic modulus = 50% after 50 cycles



Physical Property Requirements (2 of 2)

- **Restraining Ring Shrinkage Test**
 - 14 days
 - 50 microstrain max.
 - No cracks



Mechanical Property Requirements

- **Chord modulus**
 - Max. = 3.5×10^6 psi
- **Compressive strength**
 - 3000 psi (3 hours) or
 - 3000 psi (1 day)
- **Bond strength (1 day)**
 - 500 psi (to opc mortar) *and*
 - 1000 psi (to self)



Material Approval Process

- **Test Summary**

- Flow (for grouts)(ASTM C 939)
- Coefficient of thermal expansion(ASTM C 531)
- Freeze-thaw resistance(ASTM C 666, Method A)
- Restraining Ring Shrinkage(AASHTO PP34)
- Chord modulus(ASTM C 469)
- Compressive strength(ASTM C 109, ASTM C 39)
- Bond strength(ASTM C 882)

- **Additional Important Considerations**

- Shelf life
- Simplicity
- Safety / non-hazardous
- Effects of using non-potable water

Project Plan

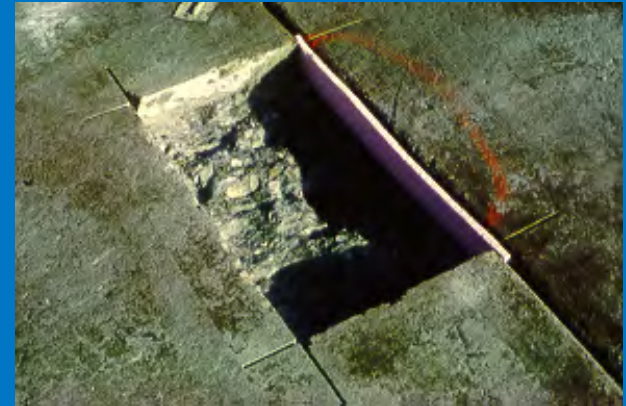
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 - 1 cu.ft. < size of repair < 1 cu.yd.
- **FY06: secure cracked surfaces**
 - reduce FOD potential
- **FY07: repair structurally deteriorated AC surfaces**
 - also, program-wide demonstration for C-17



Categories of Repair

- **Spalls**

- < 1 cu.ft.
- partial depth



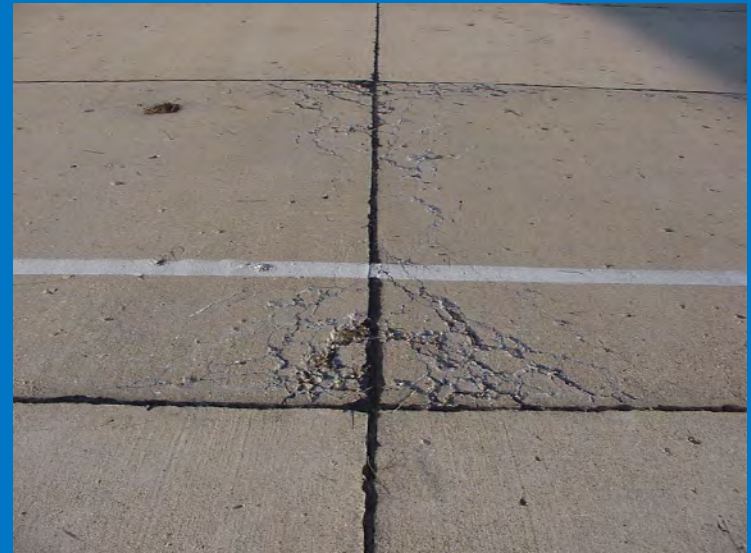
- **Airfield Damage Repair (ADR)**

- 'crater repair'
- surface area > 50 sq.ft. (typ.)
- damage well into subgrade



Categories of Repair

- Intermediate-Sized Repairs
 - up to partial slab replacement, < 1 cu.yd. (typ.)
 - full-depth concrete
 - minimal work on base course

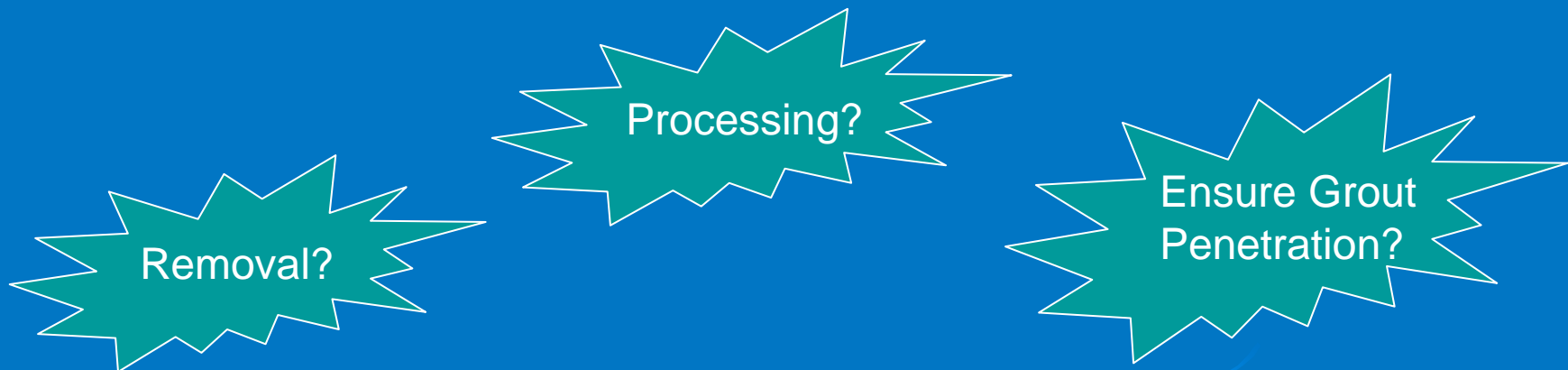


Intermediate Repairs

- **Requirements for Proposed Repair Method**
 - minimize requirement for transported materials
 - meet 'rapid' and/or 'very rapid' repair requirements
 - use only equipment accessed easily by military construction units

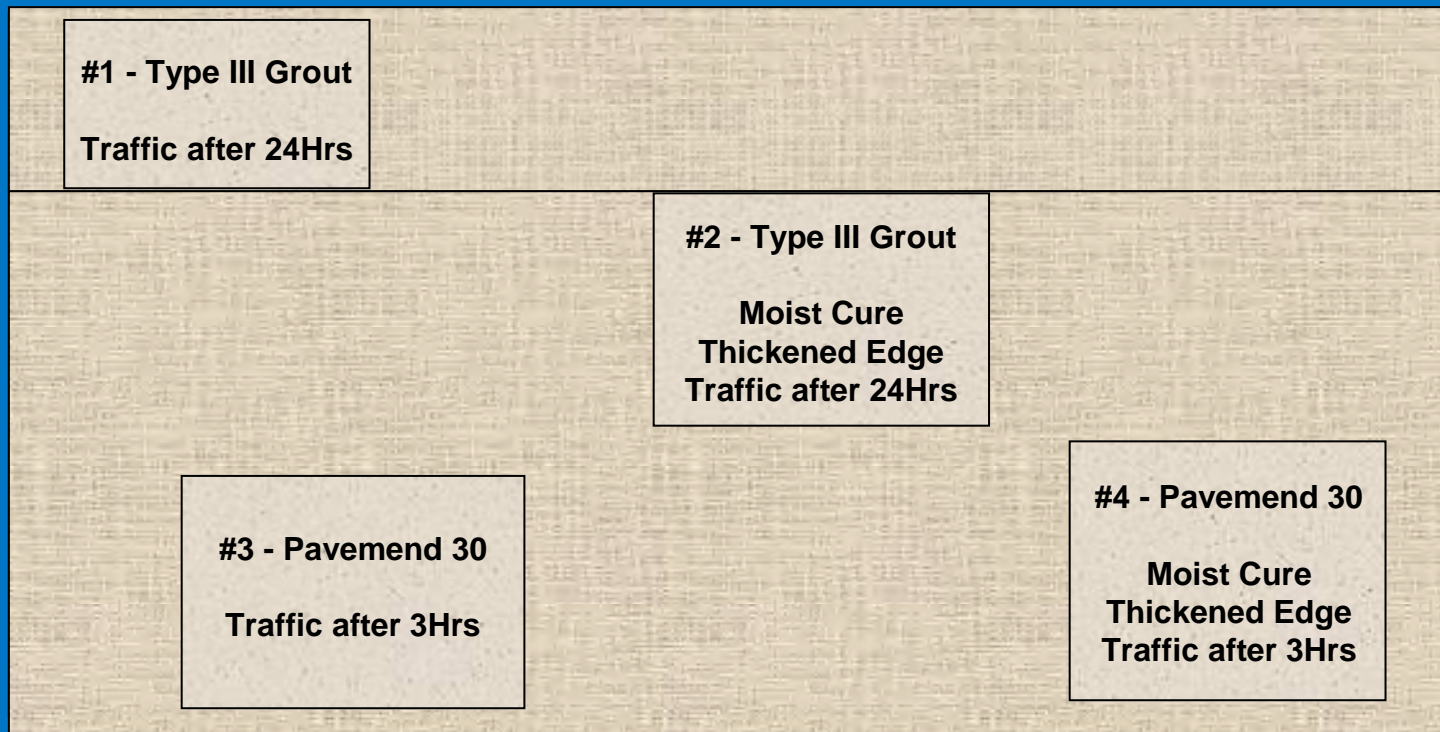
Intermediate Repairs

- **Description of Proposed Repair Method**
 - remove unsound concrete
 - place debris back in the hole
 - pour in grout that can penetrate to the bottom of the hole
 - ensure level, smooth pavement surface



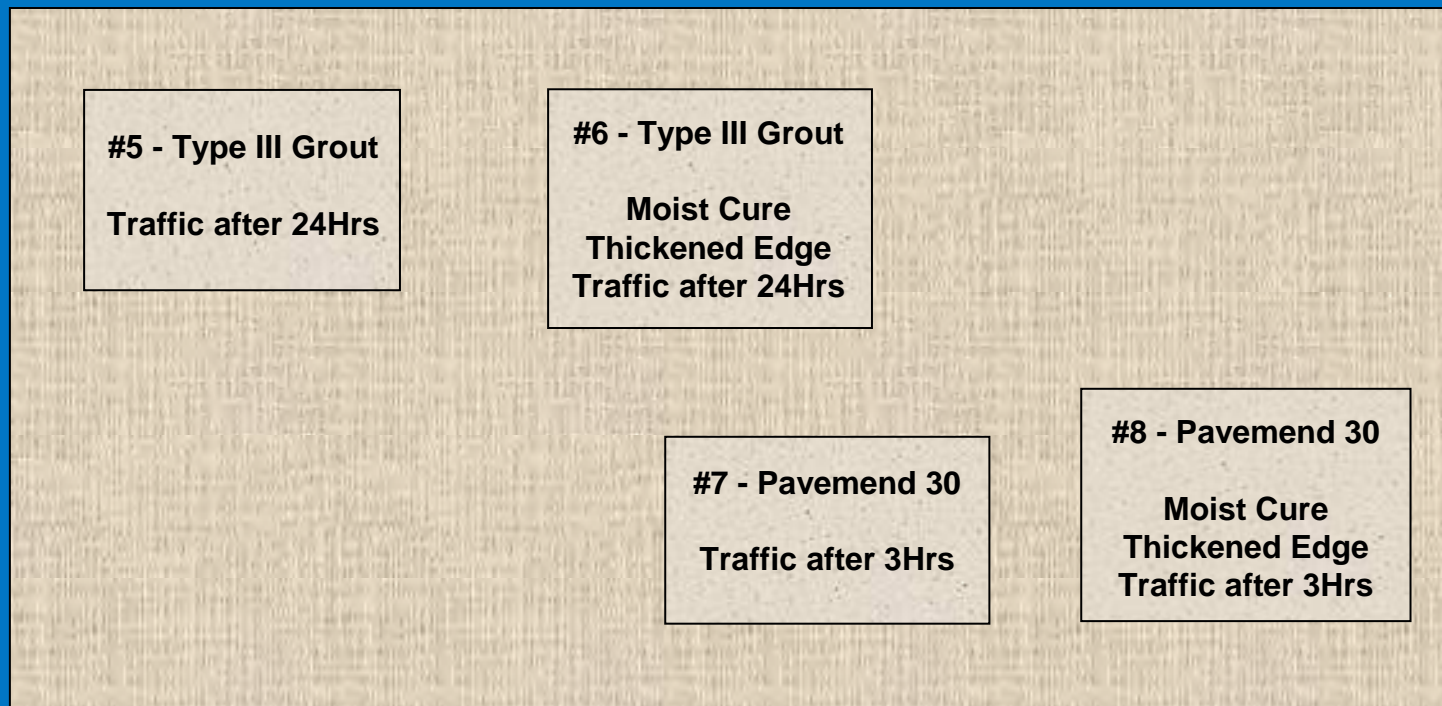
Field Placements

- **Slab No. 1**
 - Repairs 1 through 4
 - Slab = 18 in. thick



Field Placements

- **Slab No. 2**
 - Repairs 5 through 8
 - Slab = 9.5 in. thick



Develop Method of Removal



Characterize Debris



Ensure Grouts Could Penetrate



Ensure Grouts Could Penetrate



Ensure Grouts Could Penetrate



Field Placements



Field Placements



Field Placements



Field Placements



44,000 lb, 50 passes



Field Placements - Findings

- Wheel saw + hammer attachments make the technique viable
- Type of concrete affects debris gradation
- No load-related distresses
- No evidence of thermal distress
- Type III grout had shrinkage cracks if not moist-cured
- Type III repair - \$200 / cu.yd.
- PaveMend repair - \$2000 / cu.yd.

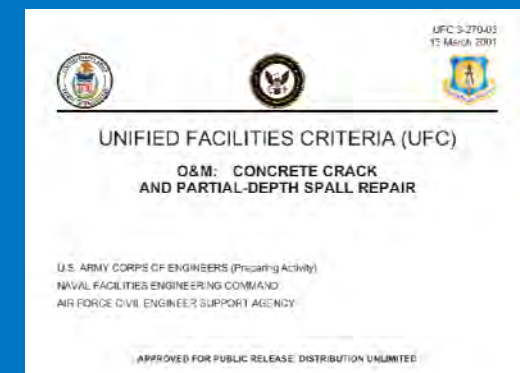
Conclusions

- Recommend military units purchase wheel saw and hammer attachments
- Sieve debris over 2 in. screen
- Thickened edge not needed for short-term, but is good practice
- Place larger debris near bottom, smaller near top of repair
- Curing advisable for Type III grout if possible
- Type III grout = rapid repair (24 hr),
- PaveMend = very rapid repair (3 hr)
- Type III grout – cheaper and consistent over time
- PaveMend requires special care
 - Reduced set time when placing layer on top of hot (setting) material
 - Should use PM-TR as a cap

Where to Publish?

- **Airfield Damage Repair (craters)**
 - UFC 3-270-07, "Airfield Damage Repair"
- **Spall Repair**
 - UFC 3-270-07 only provides expert contacts
 - Could incorporate modern (non-PCC) materials into
 - UFC 3-270-03, "Concrete Crack and Partial-Depth Spall Repair"
 - UFGS 02980, "Patching of Rigid Pavements"
 - Recommend posting material assessments on the Triservice Transportation website

<http://www.triservicetransportation.com>



Where to Publish?

- **Intermediate-Sized Repairs**

- Could incorporate into:
 - UFC 3-270-07, “Airfield Damage Repair”
- Could produce a flip-book manual similar to:
 - UFC 3-270-03, “Concrete Crack and Partial-Depth Spall Repair”
- Could produce a new guide specification such as:
 - UFGS 02980, “Patching of Rigid Pavements” and
 - UFGS 03372, “Preplaced Aggregate Concrete”



Thanks



Acceptance Criteria for Unbonded Aggregate Road Surfacing Materials

Dale Goss

Vicksburg District, Mississippi
Valley Division

Reed Freeman
Toy Poole
Joe Tom

Engineer Research and
Development Center

US Army Corps of Engineers



Problem

- Good sand clay gravel sources nearly depleted
- Crushed aggregates provide various levels of performance
- Need to update/improve UFGS 02731A, "Aggregate Surface Course"

Objective

- Update UFGS 02731A to allow the use of various types of unbound materials
 - Well-defined limits used to accept or reject proposed material sources
 - Differentiate between construction and maintenance situations

Current UFGS 02731A

- 4 grading options
 - Natural or crushed

USACE Grading Requirements for Surface Aggregate				
Sieve Size	No. 1	No. 2	No. 3	No. 4
1 in.	100	100	100	100
3/8 in.	50 – 85	60 – 100	---	---
No. 4	35 – 65	50 – 85	55 – 100	70 – 100
No. 10	25 – 50	40 – 70	40 – 100	55 – 100
No. 40	15 – 30	24 – 45	20 – 50	30 – 70
No. 200	8 – 15	8 – 15	8 – 15	8 – 15

- Coarse fraction
 - LA abrasion $\leq 50\%$
 - Flat/elongated $\leq 20\%$
- Fine fraction
 - LL $\leq 35\%$
 - PI = 4 to 9

MVD Specifications

- 3 material options →
 - 1 grading each
- Coarse fraction
 - LA abrasion $\leq 40\%$
 - MgSO₄ soundness $< 15\%$

MVK Grading Requirements for Surface Aggregate			
Sieve Size	Sand Clay Gravel	Crushed Stone	Crushed Stone with Binder
2 in.	100	No data	No data
1-1/2 in.	95 – 100	100	100
1 in.	75 – 100	No data	No data
3/4 in.	No data	50 – 95	50 – 100
1/2 in.	45 – 90	42 – 85	42 – 85
No. 4	30 – 65	25 – 65	25 – 65
No. 10	20 – 50	No data	20 – 50
No. 40	10 – 30	10 – 32	10 – 32
No. 200	5 – 15	3 – 12	3 – 12

- Fine fraction
 - LL $\leq 30\%$
 - PI = 5 to 15%

- Fine fraction
 - LL $\leq 30\%$
 - PI = 4 to 9%

Compaction Requirements

- **UFGS 02731A**
 - 100% modified Proctor
- **MVD**
 - "... compacted as evenly and densely as practicable by the controlled movement of the hauling equipment over the entire area."
 - Dress with a motor grader

Review of Other Agencies

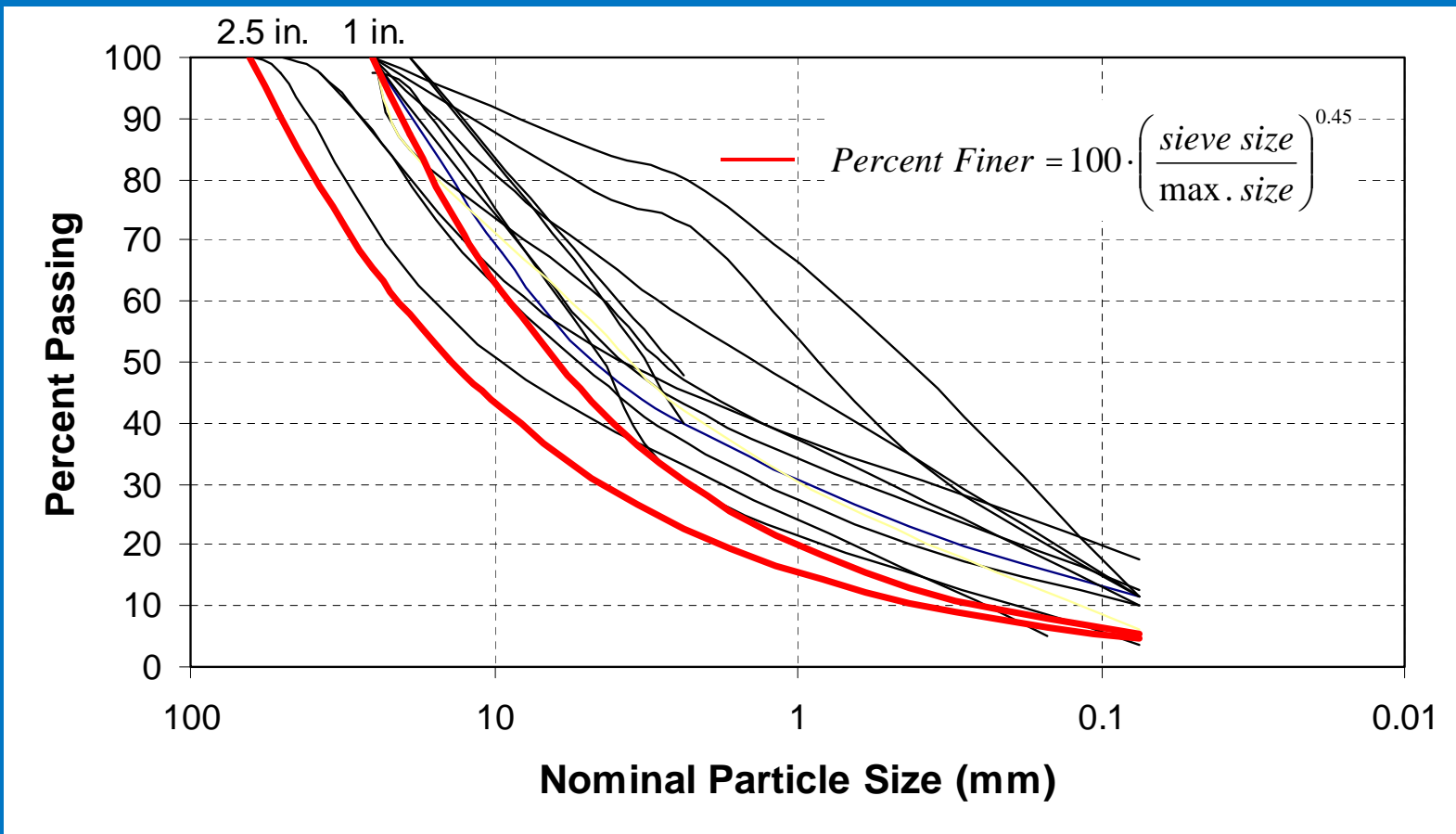
- 9 state DOTs
- US Forest Service
- FHWA
- South Africa, SRA and CSIR
- Popular specification tests:
 - gradation
 - LA abrasion
 - flat / elongated
 - fractured face counts
 - LL and/or PI
 - sulfate soundness
 - sand equivalent
 - % passing No. 200
 - No. 200 / No. 40

Popular Specification Tests

Test	Limit(s)	Note
Gradation	next slide	
LA Abrasion	35 to 50% max.	% loss
Flat / Elongated	10 to 20% max.	3 to 1 ratio
Fractured Face Counts	50 to 75% min.	at least one face
LL	25 to 40% max.	
PI	8 to 15% max. 0 to 5% min.	
Sulfate Soundness	12 to 15% max.	Na or Mg
Sand Equivalent	40 to 45% min.	
% Passing No. 200	10 to 20% max. 0 to 10% min.	
No. 200 / No. 40	67% max.	

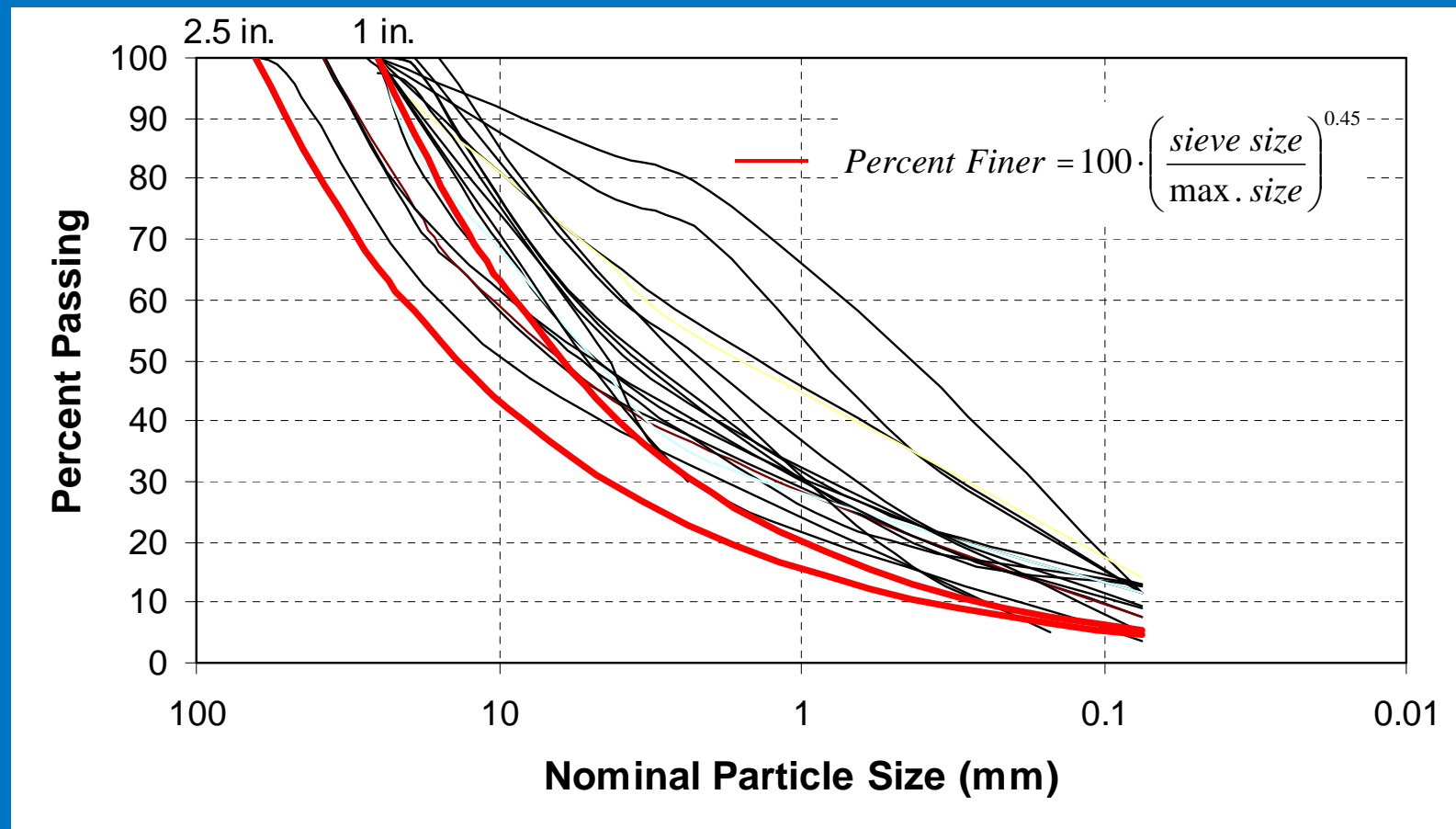
Target Gradations - Literature

Natural Aggregate



Target Gradations - Literature

Crushed Aggregate



This Study - 5 Aggregate Sources

1) Sand clay gravel, SCG

SC



Greenwood Hill Gravel in Greenwood, MS

5 Aggregate Sources

2) Crushed limestone, LS

GW-GM



Vulcan Materials Co., Reed Quarry, Gilbertsville, KY

5 Aggregate Sources

3) Sandstone, SS

GP-GM



Pine Bluff Sand and Gravel, River Mountain Quarry, Delaware, AR

5 Aggregate Sources

4) Igneous, IGN

GP



McGeorge Corp., Granite Mountain Quarries, Little Rock, AR

5 Aggregate Sources

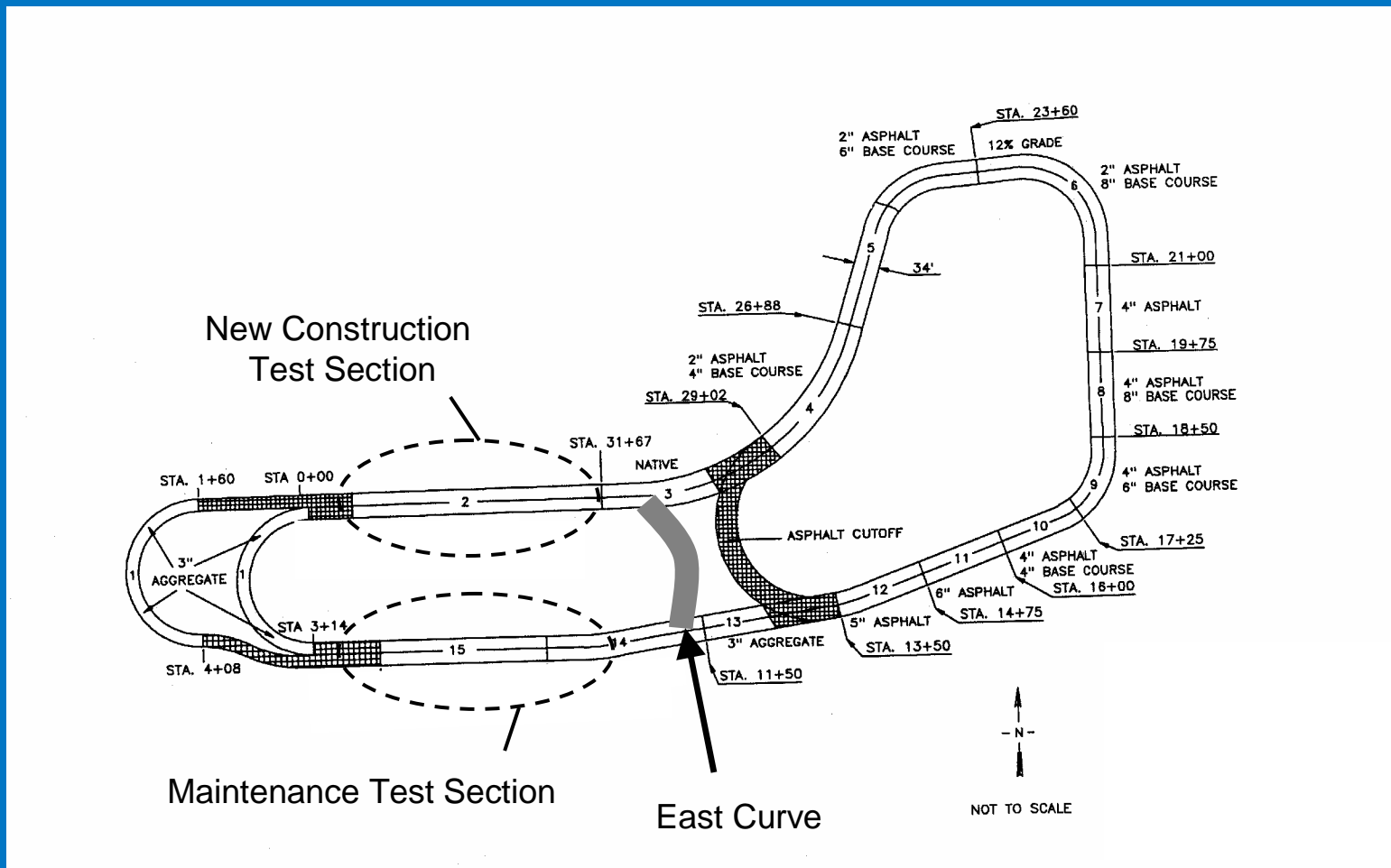
5) Sandstone with binder, SSB

GC



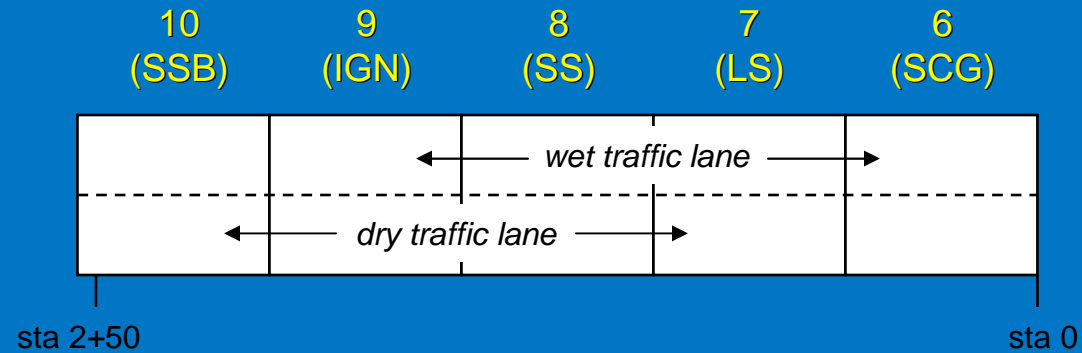
Martin Marietta Aggregates, Sawyer Quarry, Sawyer, OK

Experimental Approach

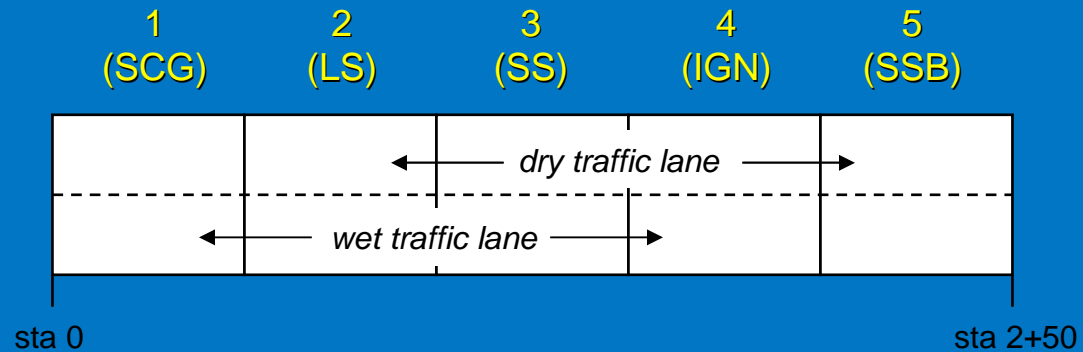


Experimental Approach

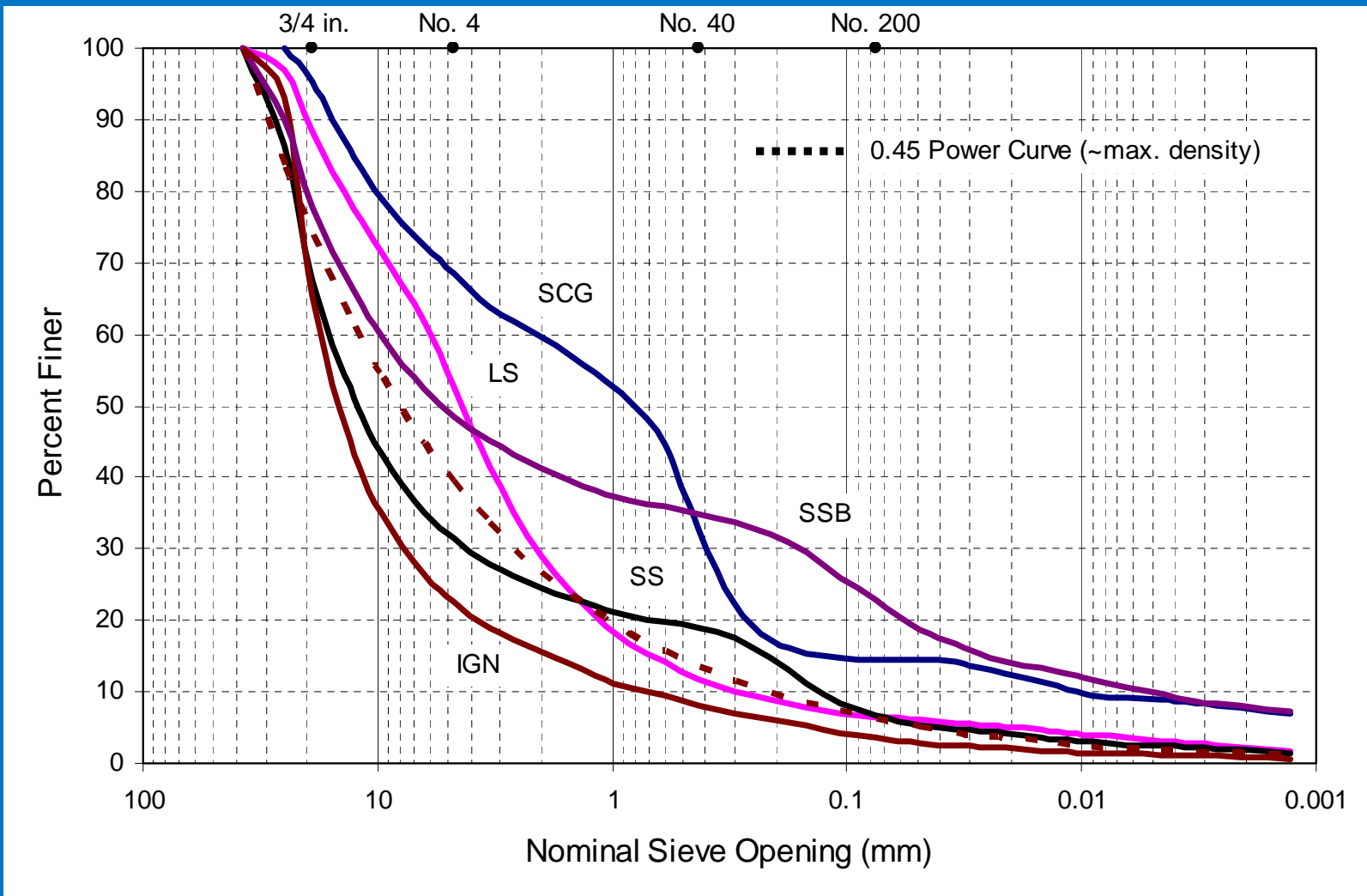
New
Construction



Maintenance

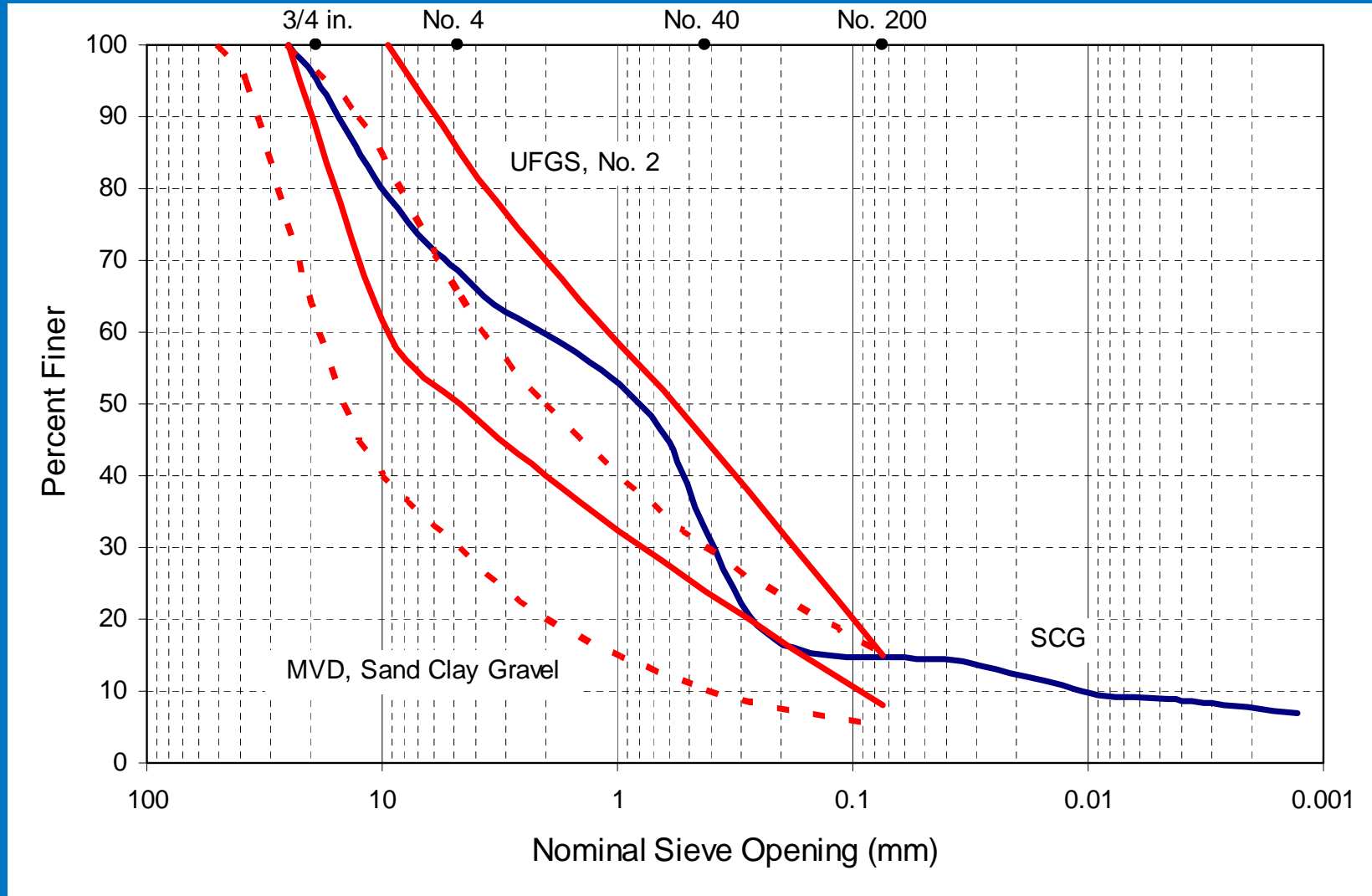


Particle Size Distribution



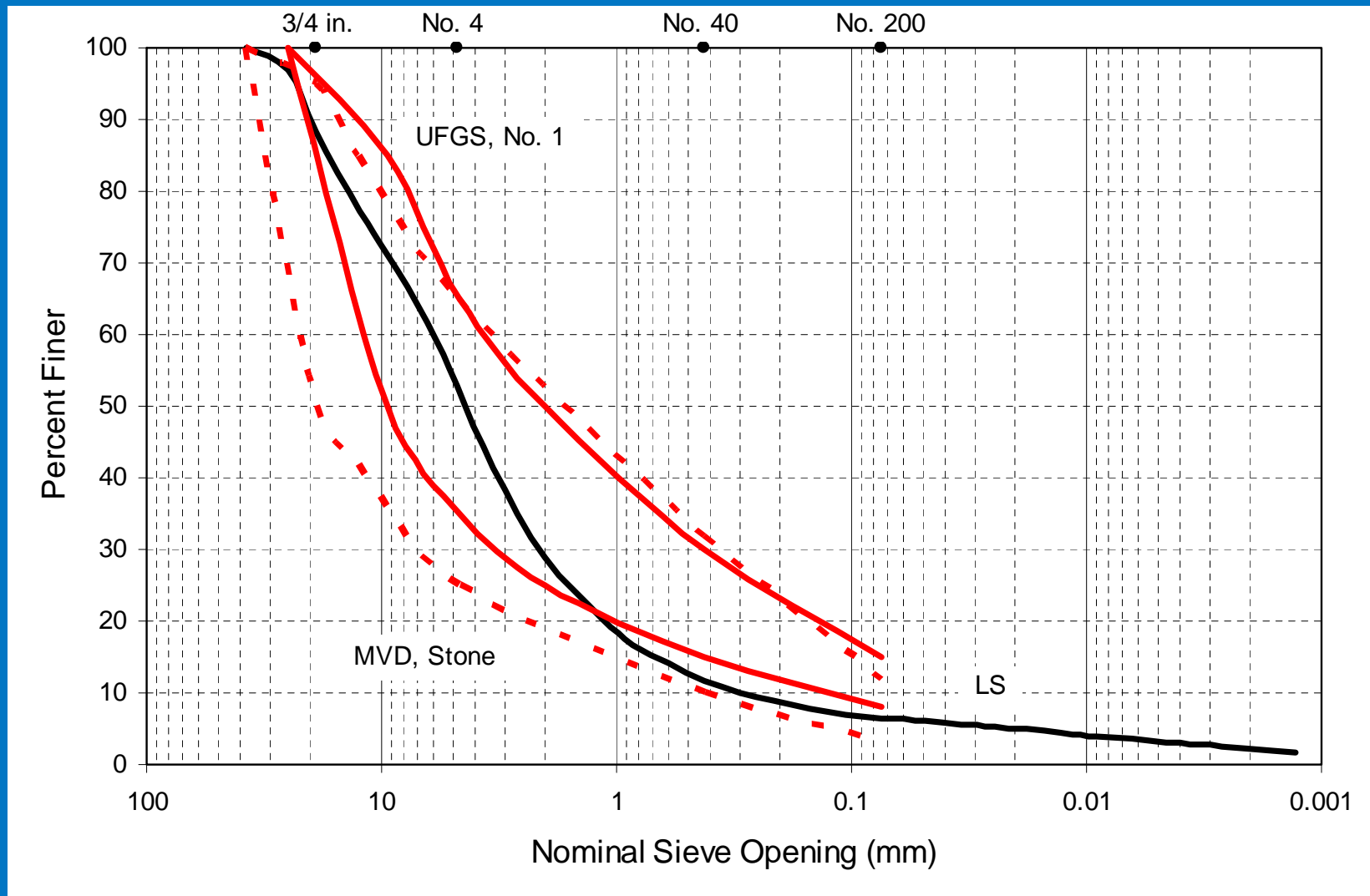
Particle Size Distribution

sand clay gravel



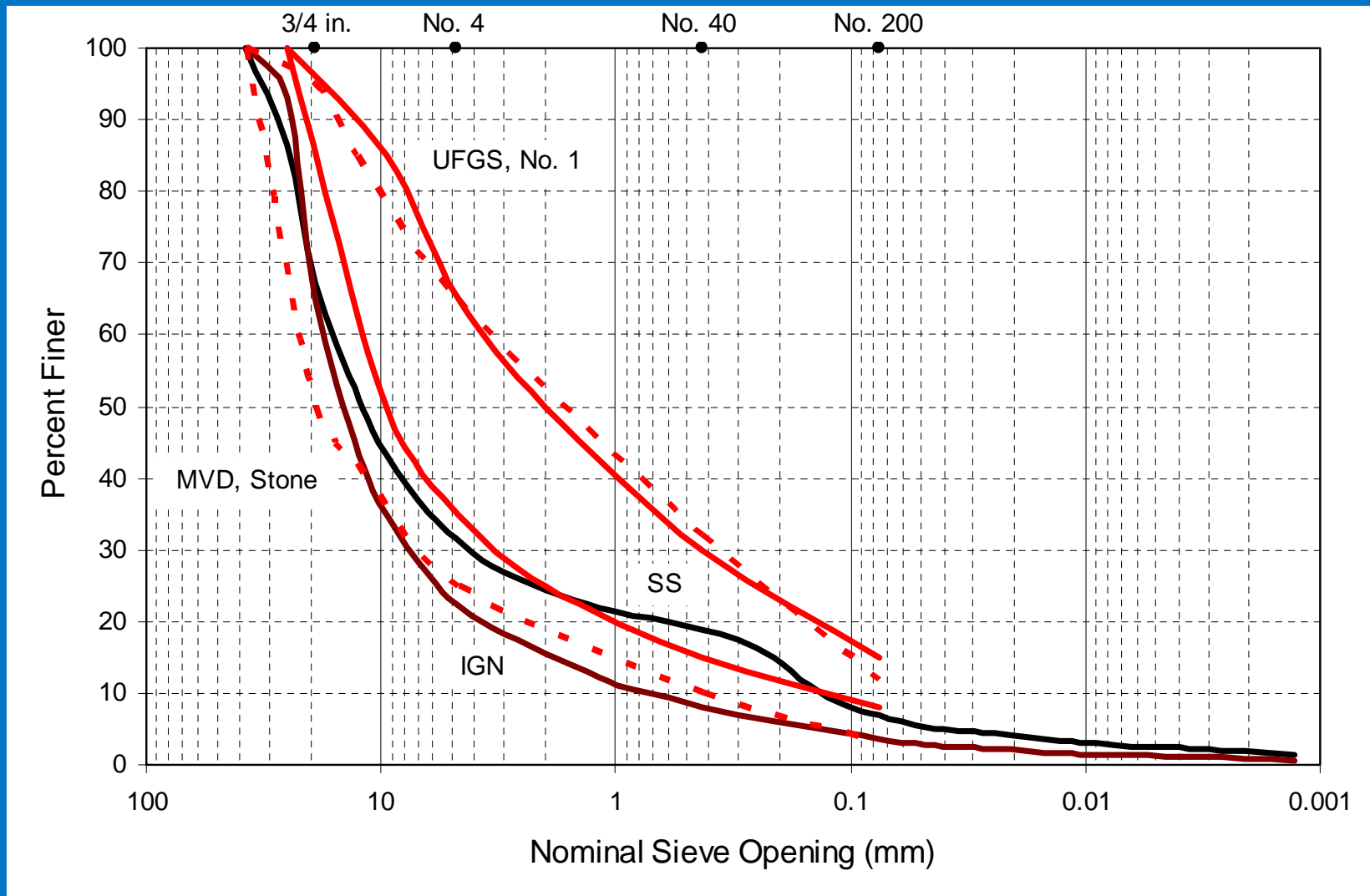
Particle Size Distribution

limestone



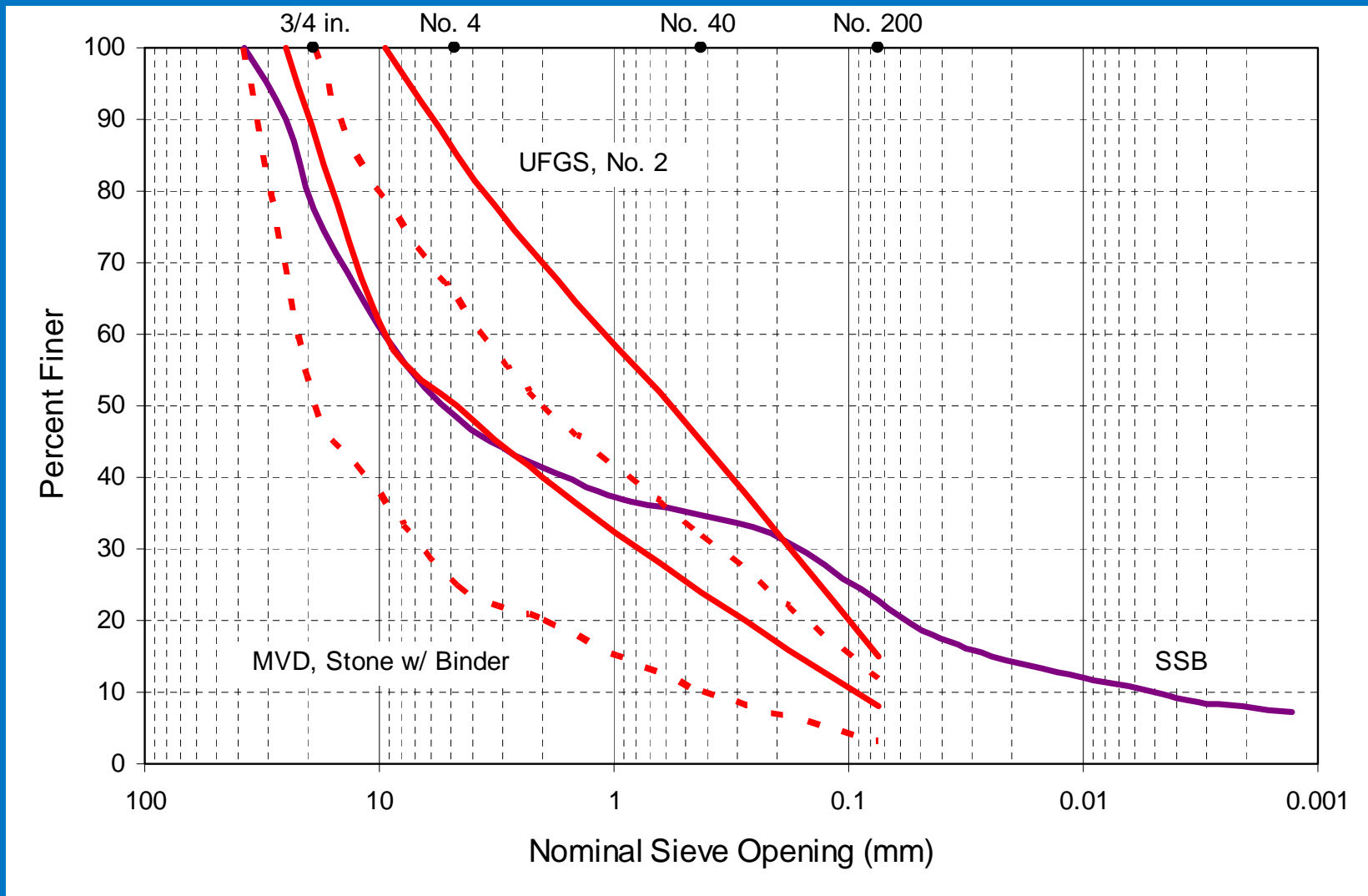
Particle Size Distribution

igneous and sandstone



Particle Size Distribution

sandstone with binder



Material Characteristics

Test		SCG	LS	SS	IGN	SSB
LA Abrasion	35 to 50% max.	18.2	18.8	33.5	27.3	27.8
Flat / Elongated	10 to 20% max.	4.2	5.8	5.5	5.8	10.8
LL	25 to 40% max.	31	NP	NP	NP	28
PI	8 to 15% max. 0 to 5% min.	18	NP	NP	NP	14
Sulfate Soundness	12 to 15% max.	1.0	0.3	4.2	0.4	6.4
Sand Equivalent	40 to 45% min.	20	73	23	61	10
Linear Shrinkage	So. Africa	6.1	1.1	0.2	0.5	6.4
% Passing No. 200	10 to 20% max. 0 to 10% min.	14.4	6.3	6.8	3.6	22.8
No. 200 / No. 40	67% max.	44	53	36	28	66

- Targets
 - Subgrade CBR = 5 to 10%
 - Surface to receive maintenance layer to have dry unit weight = 130 pcf
 - Compaction of surface layers to be similar to field

New Construction Test Section



Initial buildup
CBR = 4 to 25%

After reworking top 6 in.
Moisture = 13 to 19%
CBR = 5 to 15%

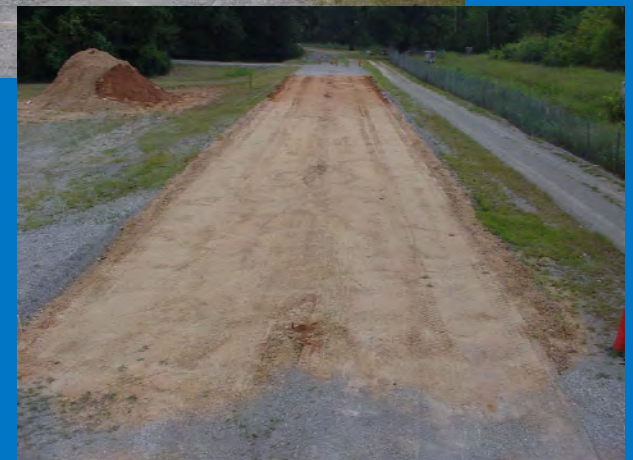
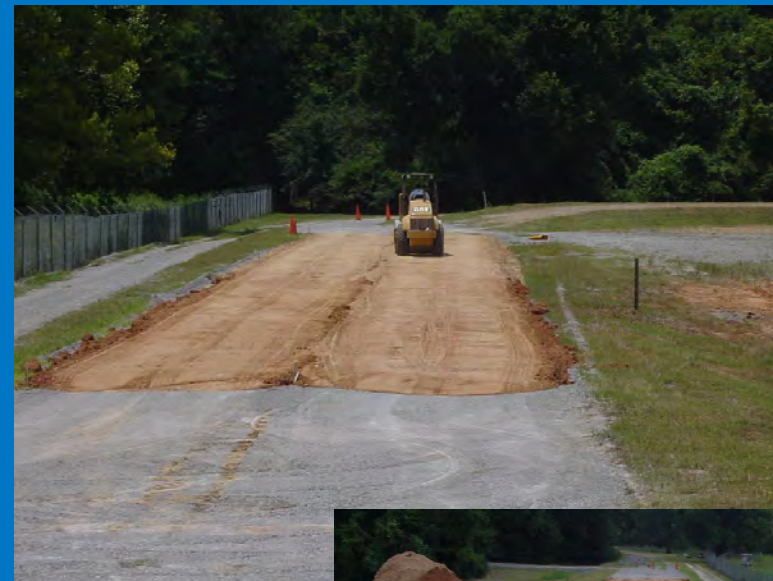


Maintenance Test Section



3 to 5 in. clay-limestone
mix remains
CBR = 50 to 100% over
CBR ~ 10% at 10 in.

Placed 6 in. of SCG at
6 to 8% moisture
Dry unit wt. = 128 to 130 pcf



Placing Surface Materials

Spread with John Deere 550G track dozer

Add 16 coverages with dozer

Smooth with static steel drum



Placing Surface Materials



Maintenance Test Section

New Construction Test Section



15 to 20 mph

Trafficking



pickup w/ 500 lb



small empty dump truck



flatbed w/ 2000 lb

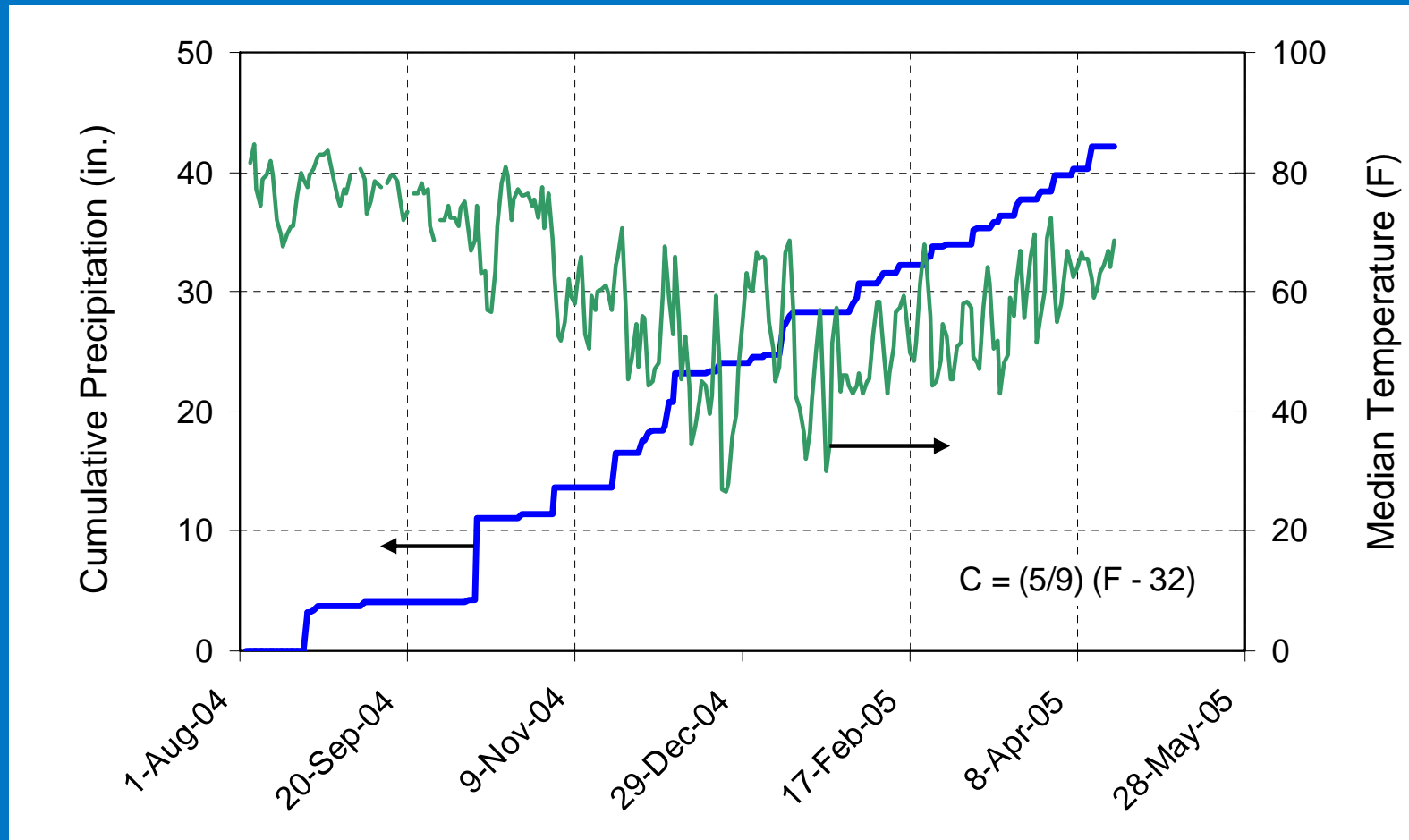


emulsion truck w/ 750 gal

Trafficking

Vehicle	Front Axle, lb	Rear Axle, lb	Inflation Pressure, psi
Pickup Truck	2600	2400	40
Dump Truck	6800	7500	110
Flatbed	5500	11000	80
Emulsion	5700	21800	80

Climate





SCG

CEMVK
UNBONDED
AGGREGATE
SURFACE
INVEST.
ITE
MA
150
13A



LS

CEMVK
UNBONDED
AGGREGATE
SURFACE
INVEST.
ITEM 7
NEW CONST.
150PASSES
13 AUG 04

August 2004
Pickup Truck
150 passes



SSB

CEMVK
UNBONDED
AGGREGATE
SURFACE
INVEST.
ITEM 10
NEW CONST.
150PASSES
13 AUG 04

dry conditions



SS

CEMVK
UNBONDED
AGGREGATE
SURFACE
INVEST.
ITEM 8
NEW CONST.
150PASSES
13 AUG 04



IGN

CEMVK
UNBONDED
AGGREGATE
SURFACE
INVEST.
ITEM 4
MAINT.
150PASSES
13 AUG 04



SCG



LS

October 2004
Pickup Truck
2500 passes

dry conditions



SSB



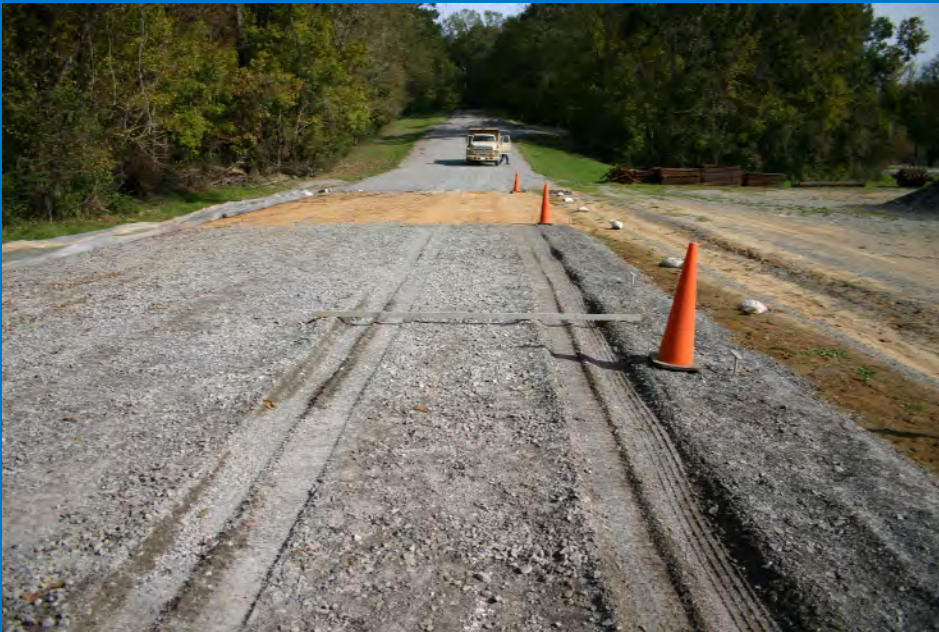
SS



IGN

After Rainy Oct./Nov. (> 10 in.)

17 November 2004
Dump Truck, 10 passes
dry surface – wet subgrade



Only LS on New Construction Rutted:
- 4 to 6 in.
- both wheelpaths



All other items had no distress.



SCG



LS



SSB

05 April 2005
Emulsion Truck
200 passes

relatively dry conditions
maintenance section



SS



IGN



16 Feb 2005
Dump Truck
200 passes



very wet conditions
maintenance section





SCG

CEMVK
UNBONDED
AGGREGATE
SURFACE
INVEST.
ITEM 1 WET
150 SINGLE
1.25" RAIN
7 MAR 05



LS

CEMVK
UNBONDED
AGGREGATE
SURFACE
INVEST.
ITEM 2 WET
150 SINGLE
1.25" RAIN
7 MAR 05

08 Mar 2005
Pickup Truck
150 passes



SSB

CEMVK
UNBONDED
AGGREGATE
SURFACE
INVEST.
ITEM 5 WET
150 SINGLE
1.25" RAIN
7 MAR 05

very wet surface
1.25 in. rain event
maintenance section



SS

CEMVK
UNBONDED
AGGREGATE
SURFACE
INVEST.
ITEM 3 WET
150 SINGLE
1.25" RAIN
7 MAR 05



IGN

CEMVK
UNBONDED
AGGREGATE
SURFACE
INVEST.
ITEM 4 WET
150 SINGLE
1.25" RAIN
7 MAR 05



SCG



LS

01 April 2005
Flatbed Truck
50 passes



SSB

wet conditions
maintenance section



SS



IGN



SCG

15 April 2005
Flatbed Truck
25 passes



LS

wet subgrade
new construction section



SSB



SS



IGN

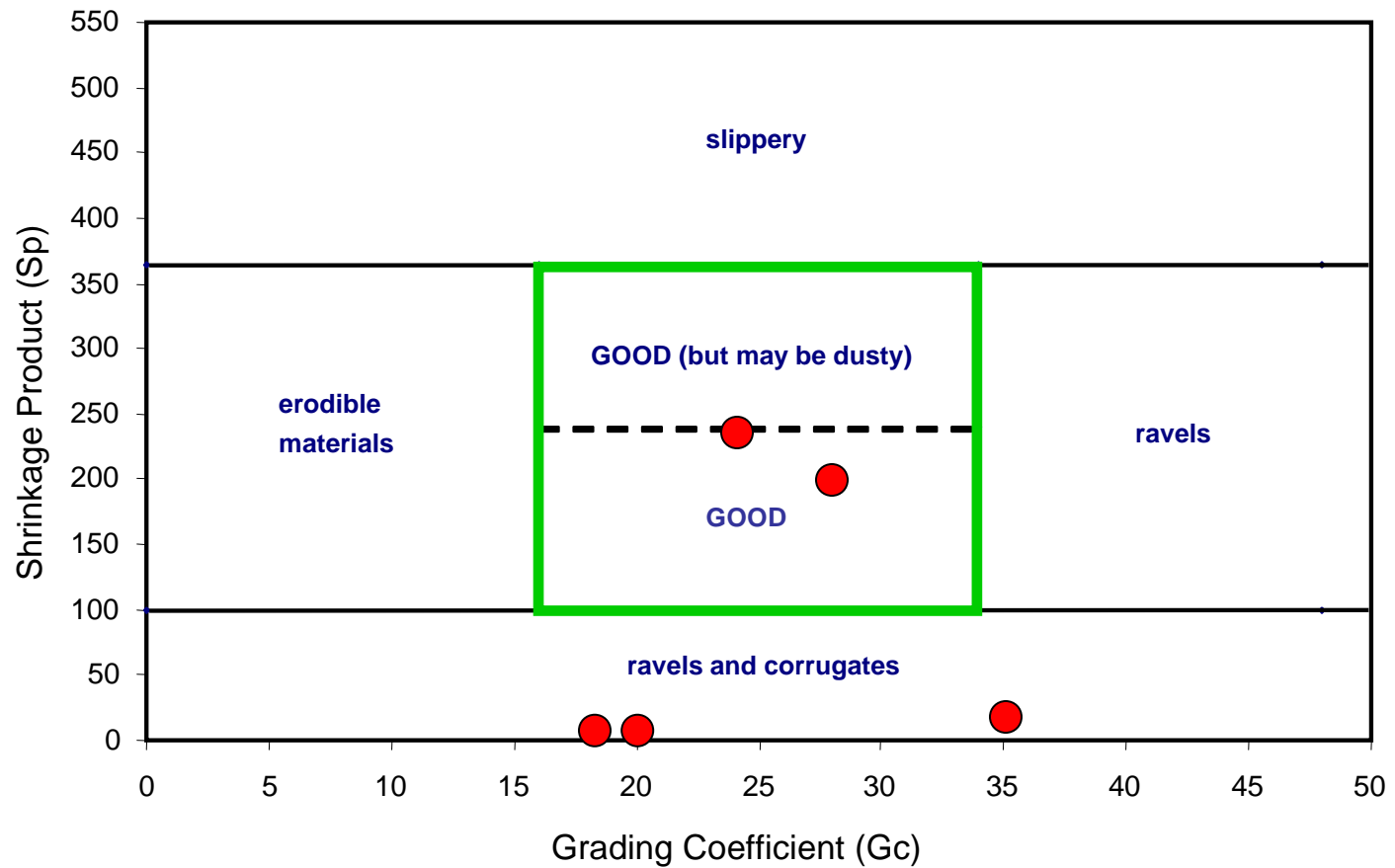
↑
least
subgrade
rutting

Summary

- **New Construction (no subbase)**
 - All materials could support light traffic adequately in dry conditions
 - SCG had surface rutting when wet, even under light traffic
 - Aggregates with high fines and plasticity partially protected subgrade from rain, thus prolonging life of road
 - SSB performed best under heavy traffic
 - If heavy traffic is possible, road should include a subbase

- **Maintenance (SCG subbase)**
 - o All materials, except SCG, could support light traffic adequately in dry or wet conditions
 - o SCG had surface rutting when wet, even under light traffic
 - o SS and IGN performed best under medium and heavy traffic in wet conditions

South African Approach



$$Sp = \text{linear shrinkage (\%)} \times \text{No. 40}$$

$$G_c = \frac{(1 \text{ in.} - \text{No. 10}) \cdot \text{No. 4}}{100}$$

Conclusions

- Subbase layer is recommended if heavy traffic is possible
 - If no subbase, criteria for surface aggregate will be different than for the case of aggregate on top of subbase
- Key components of new specification:
 - overall gradation
 - minus No. 200
 - No. 200 / No. 40
 - plasticity of fines
 - linear shrinkage?
- Apply concept similar to South Africans' but adjust for higher precipitation

Thanks



Use of Ultra-Fine Amorphous Colloidal Silica to Produce a High-Density, High-Strength Grout

Brian H. Green, R.P.G.

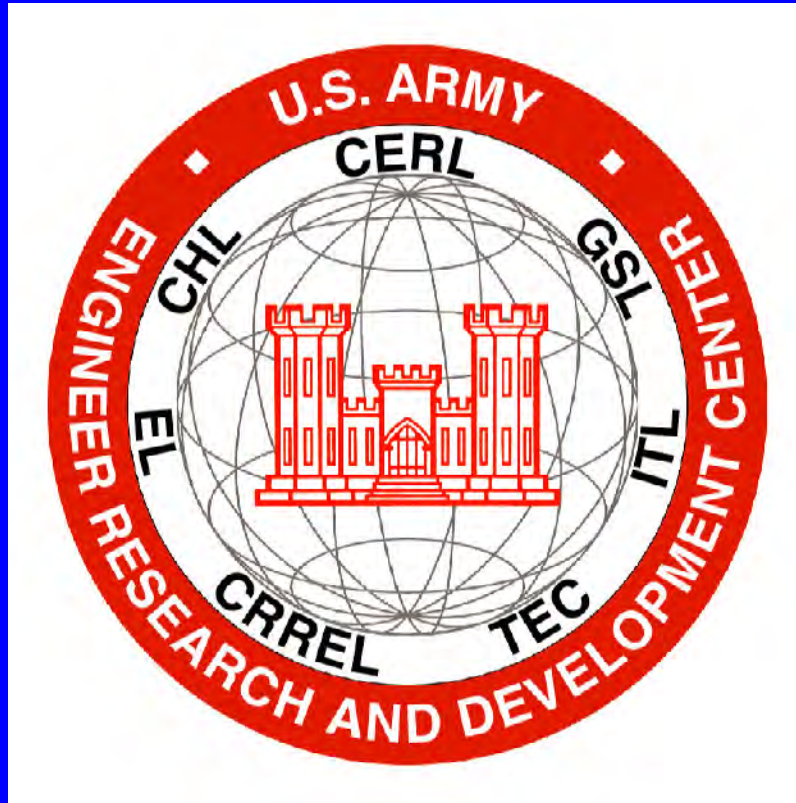
Research Geologist

Concrete and Materials Branch

Geotechnical and Structures Laboratory



Research



- Performed by:
- Concrete and Materials Branch
- Geotechnical and Structures Laboratory

- **Grout Requirements**

- **High Density: $> 2.6 \text{ Mg/m}^3$ (162.3 lb/cu ft)**
- **High Strength: $> 70 \text{ MPa}$ (10,150 psi)**
- **Ultra-Sonic Pulse Velocity: $> 3.65 \text{ km/sec}$ (11,975 ft/sec)**

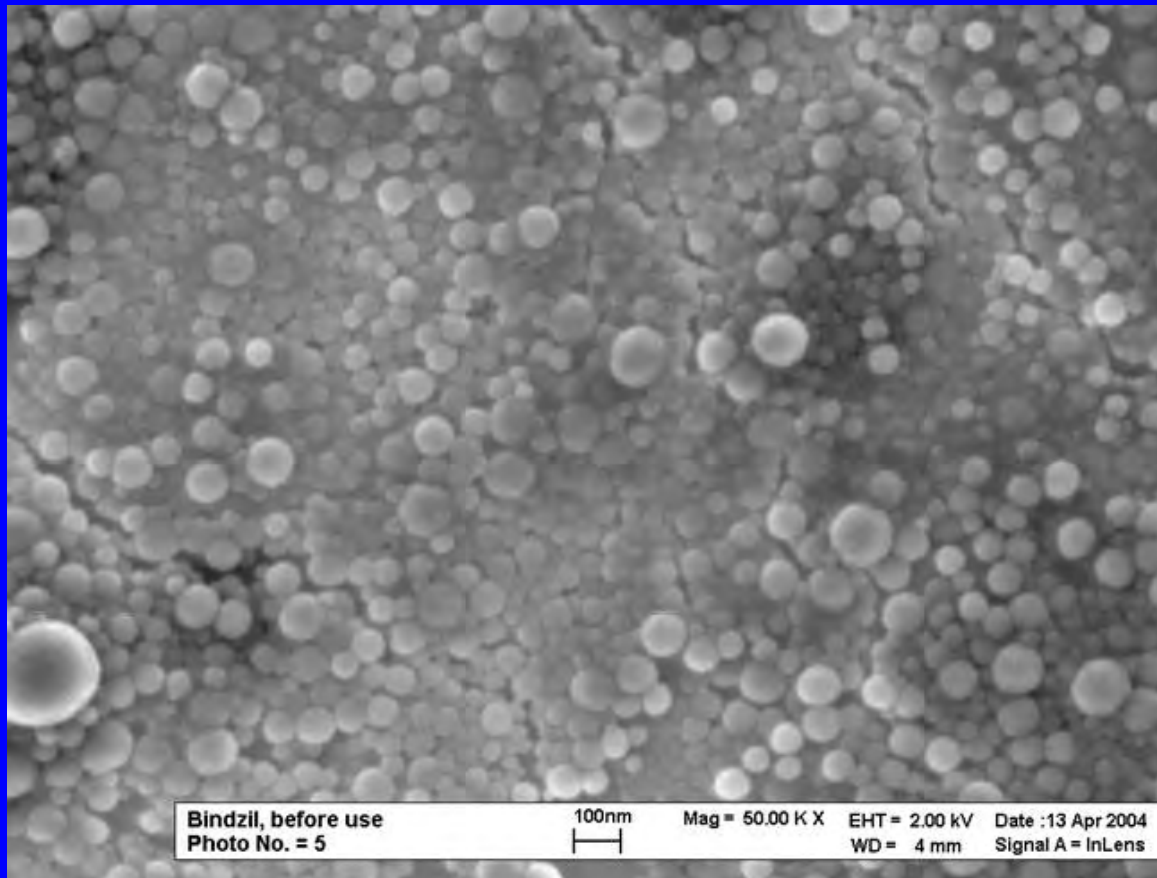
Materials for Grout Mixture

- **Portland Cement - ASTM C 150, Type I/II**
 - **Lehigh Portland Cement**
- **Hematite Fine Aggregate – ASTM C 637, Grading 1**
 - **Nuclear Shielding Supplies and Service**
- **Silica Fine Aggregate – # 20 to # 40 Sieve Size**
 - **Oglebay Norton**
- **Silica Fume – Low-Carbon, from Production of Zirconia**
 - **Elkem Materials**

Chemical Admixtures for Grout Mixture

- **High-Range Water Reducing Admixture**
 - Glenium 3030 NS, Degussa Admixtures, Inc.
- **Air *Detraining* Admixture**
 - D7 Defoamer, Amber Chemical
- **Ultra-fine Amorphous Colloidal Silica (UFACS)**
 - Cembinder 8, Eka Chemical, Akzo Nobel

Ultra-Fine Amorphous Colloidal Silica



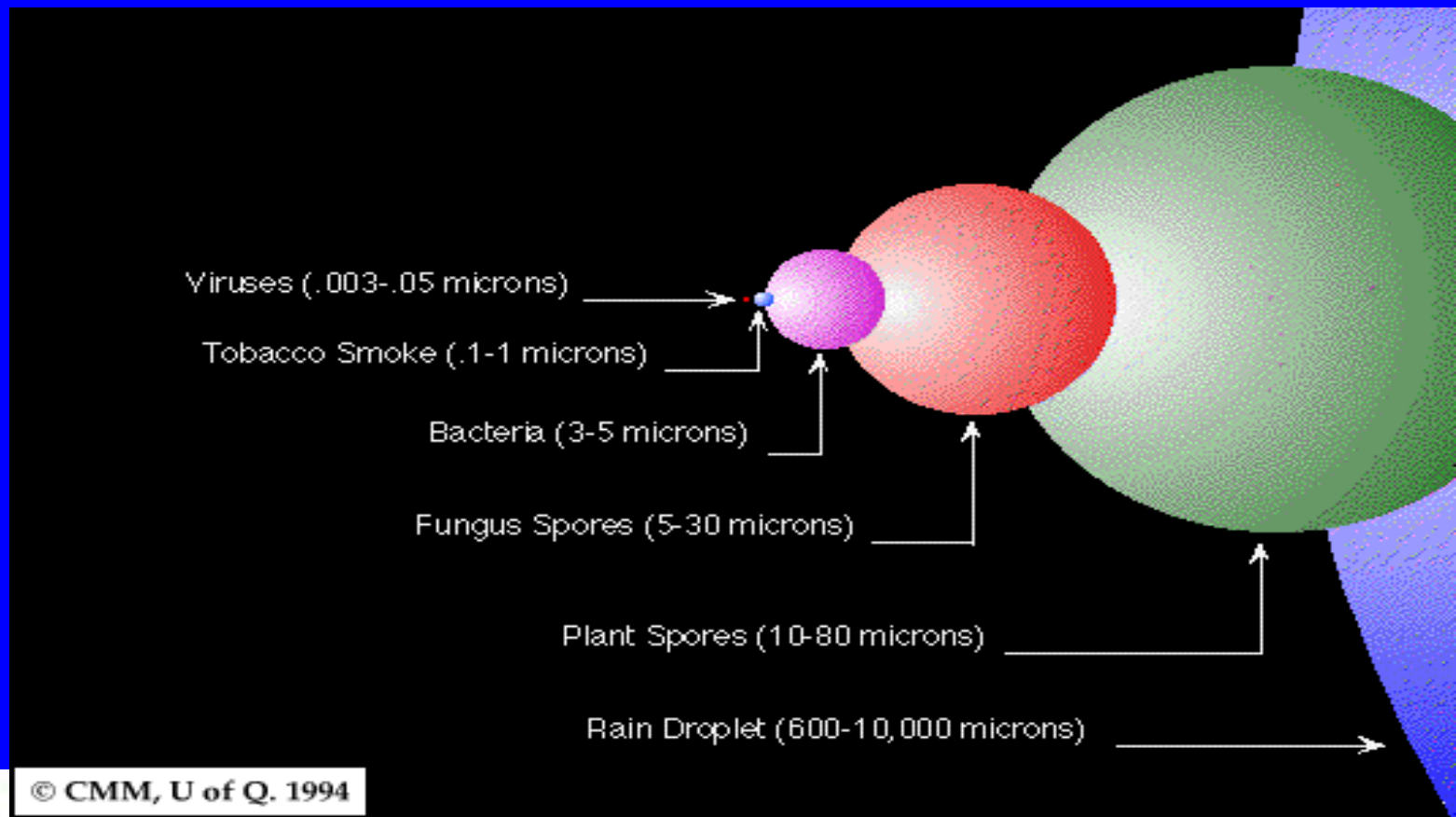
Ultra-Fine Amorphous Colloidal Silica (UFACS)

- Nano-Silica
- Nano-SiO₂

Viscosity Modifier

Definitions - Ultra-Fine Amorphous Colloidal Silica

- Nano – From the Greek *Nanos* – Meaning “Dwarf”
 - 10^{-9} Meter or One *Billionth* of a Meter
 - Nanoscience - 1 to 100 Nanometer Scale

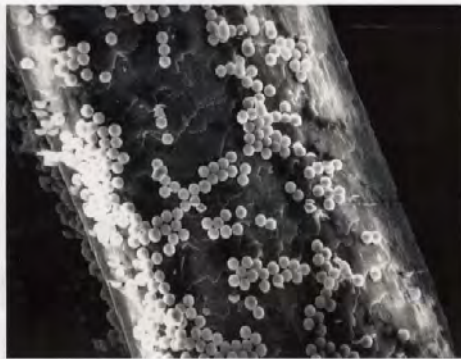
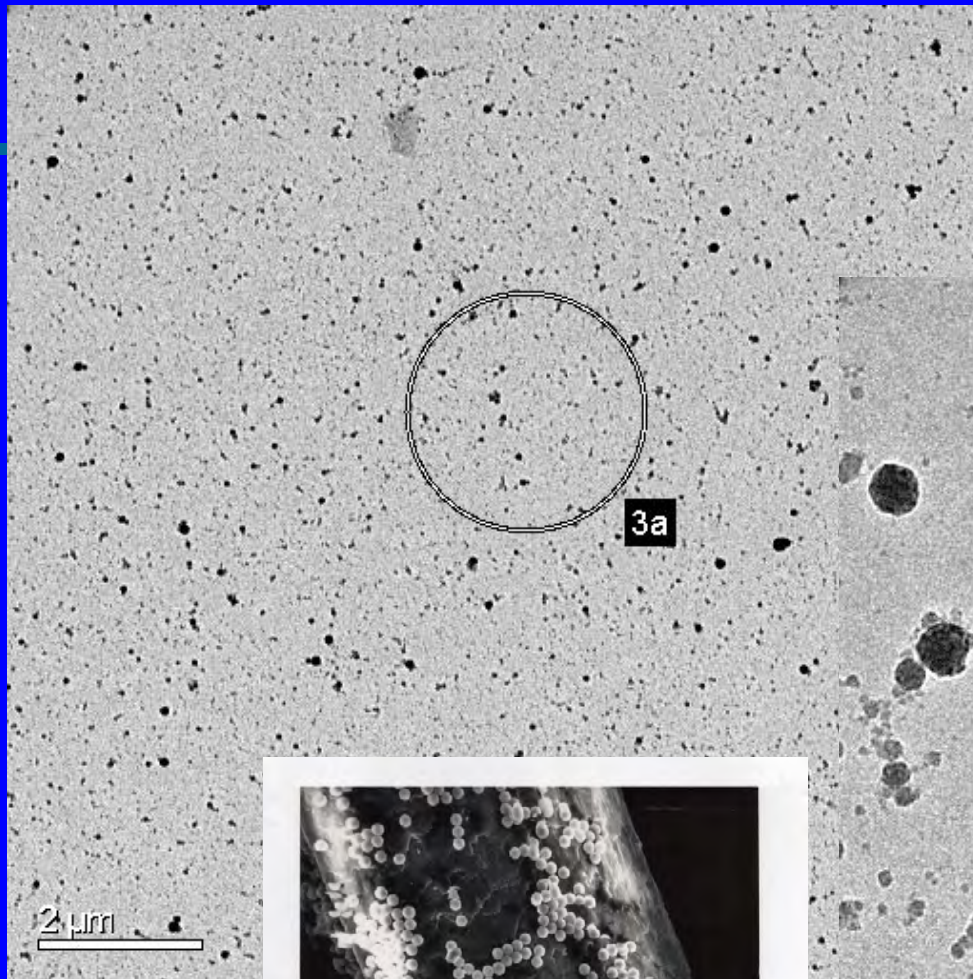


3.4 μ m = 3,400 Nanometers!

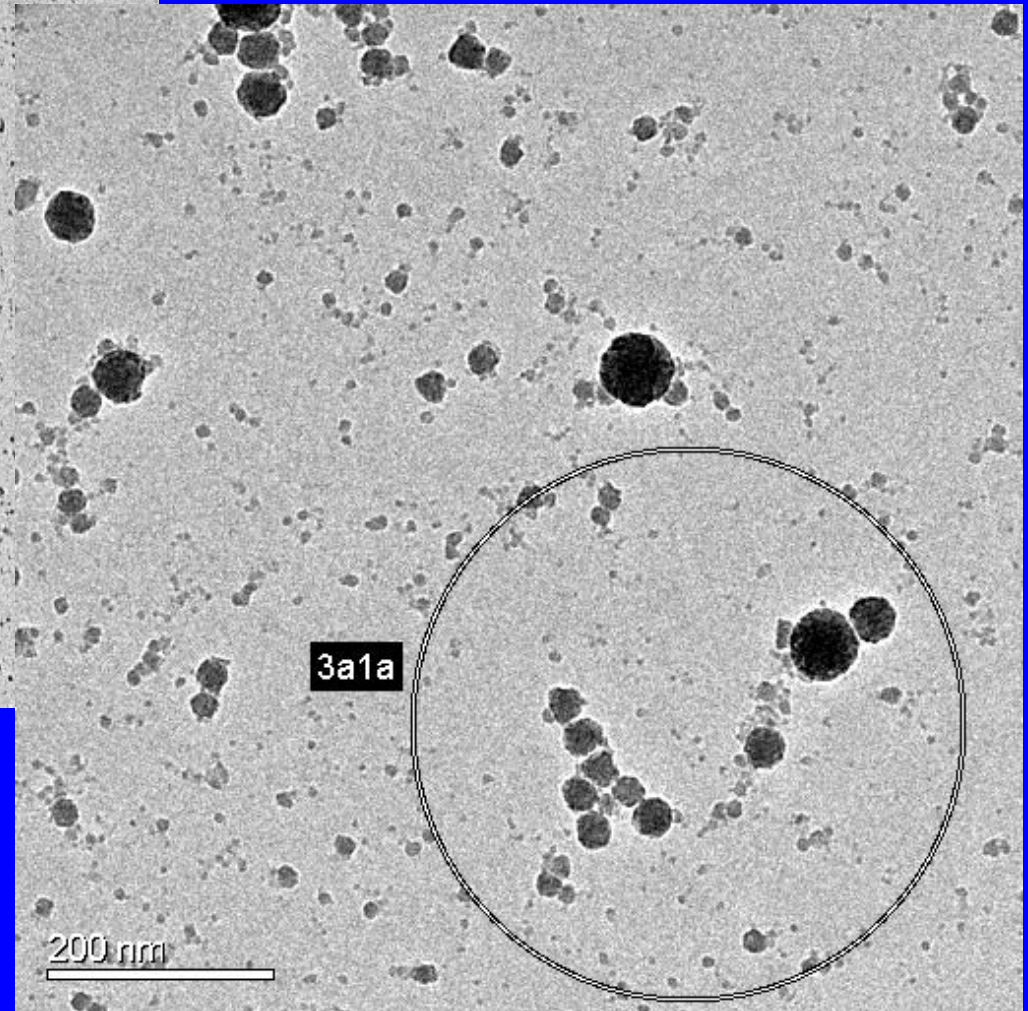


SEM PHOTO of 3.4 μ m PARTICLES ON HUMAN HAIR, 1000X

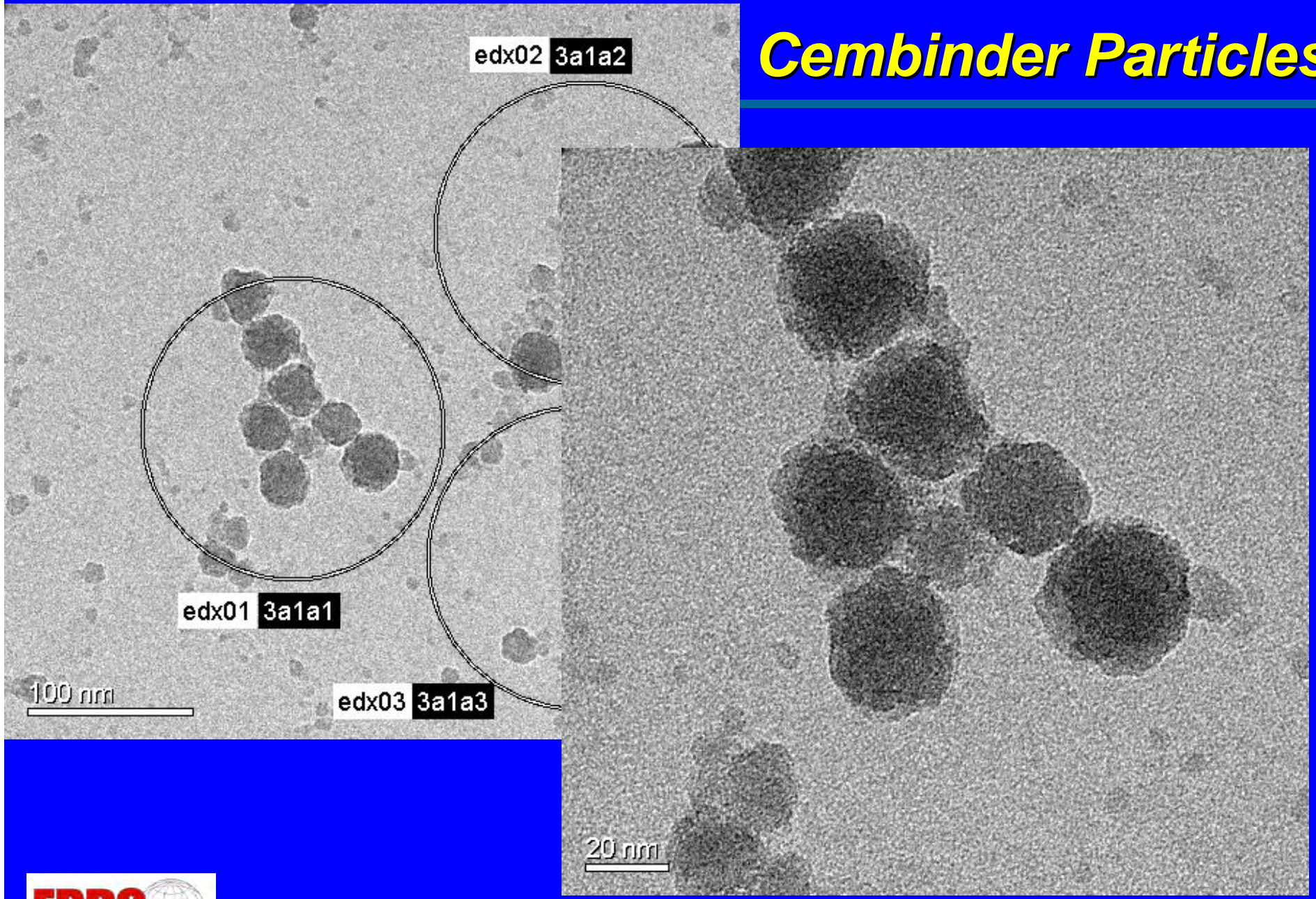
Cembinder Particles



SEM PHOTO of 3.4 μm PARTICLES ON HUMAN HAIR, 1000X



Cembinder Particles



Definitions - Ultra-Fine Amorphous Colloidal Silica

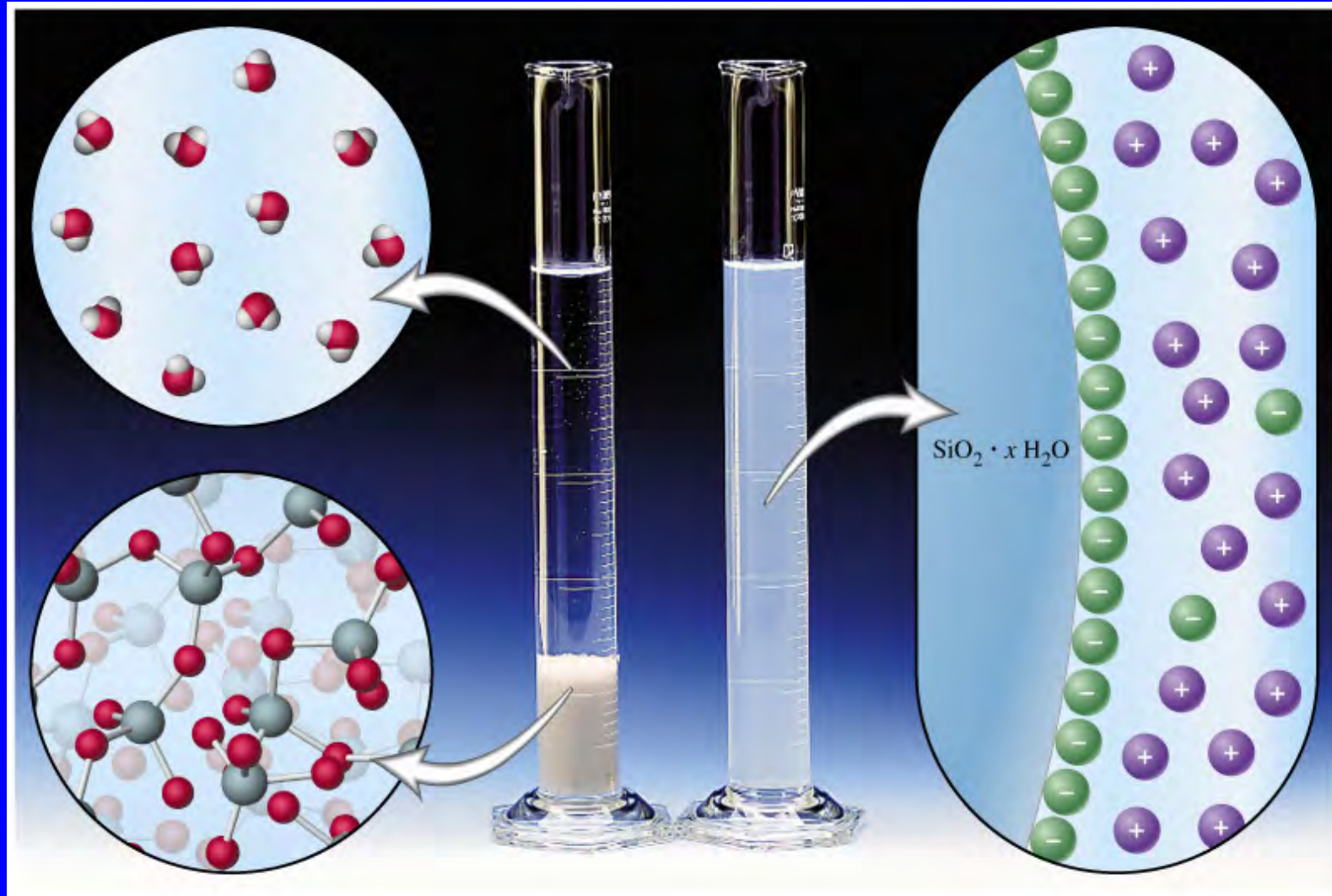
- **Amorphous**
 - Non-Crystalline Silica
 - Random Distribution of $[\text{SiO}_4]^{4-}$ tetrahedra
 - Glass is a Common Amorphous Material

Definitions - Ultra-Fine Amorphous Colloidal Silica

- **Colloid**

- - Stable dispersion of particles in a medium
 - No settling out!
- Small – Can't be seen with light optics
 - >1 nm to < 100 nm
- Can't pass through a membrane

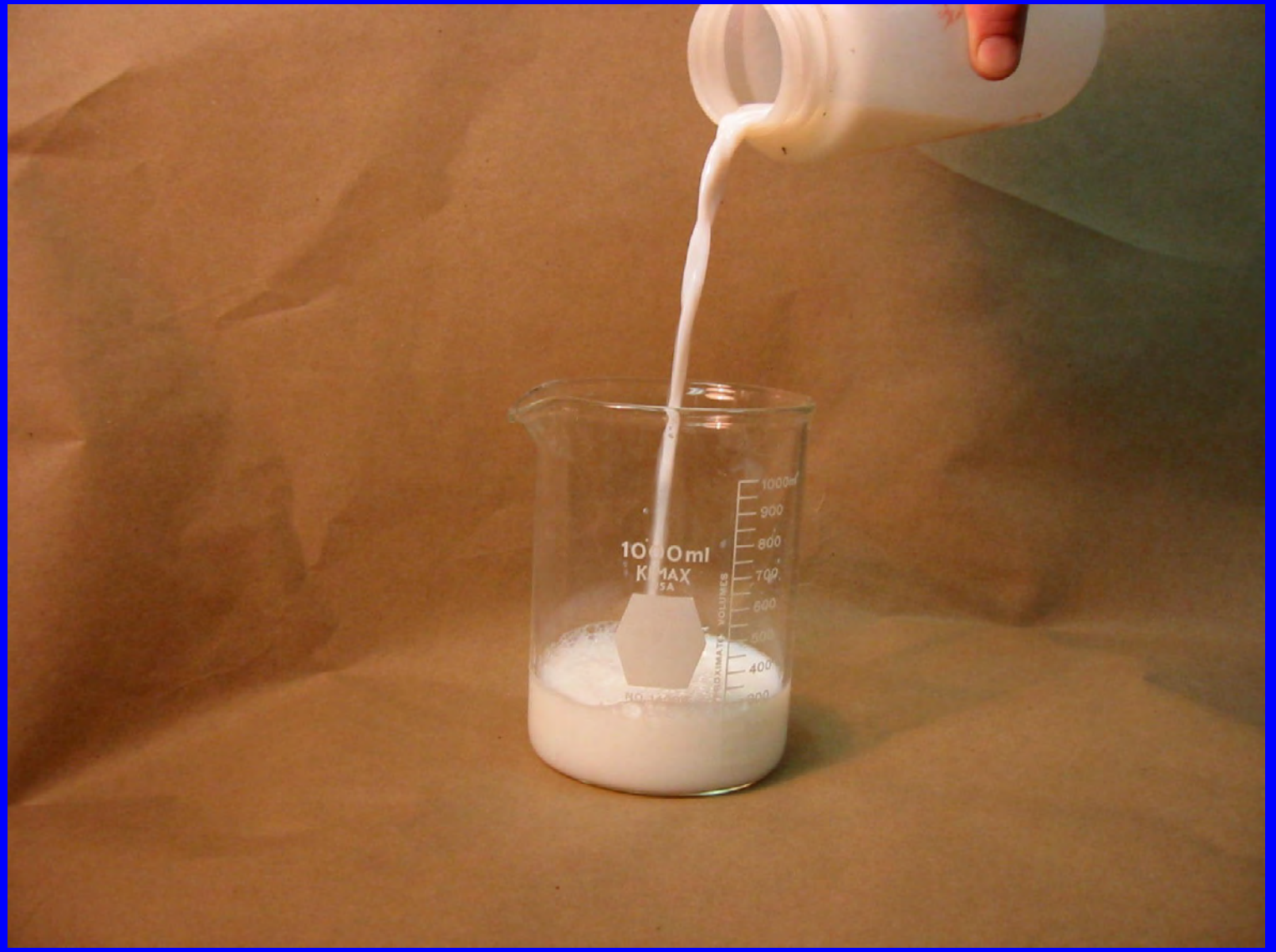
Suspension of Silica Vs. Colloidal Silica



Definitions - Ultra-Fine Amorphous Colloidal Silica

- **Ultra-Fine Amorphous Colloidal Silica (UFACS)**
 - **Industrially Manufactured**
 - **Liquid Form**
 - **Resembles Skim Milk**

Ultra-Fine Amorphous Colloidal Silica



Ultra-Fine Amorphous Colloidal Silica

- **Developed for Drilling Applications**
- **Keep Solid Particles in Grout Mixture from Segregating or “Falling Out”**



Grout Mixer and Pump



Grout Consistency



Grout - Fresh Properties

- **ASTM C 939 (Flow Cone Method)**
 - 20- 30 Second, Flow Time
- **Wet Density:**
 - ASTM C 938, Section 9.5.1
(Proportioning Grout Mixtures for Preplaced-Aggregate Concrete)
 - 2.7-2.76 Mg/m³ (168-172 lbs/ft³)

Grout –Hardened Results

- **Hardened Density: 2.68 Mg/m³
(167.4 lb/cu ft)**
- **High Strength: 71.2 MPa
(13,230 psi)**
- **Ultra-Sonic Pulse Velocity: 4.40 km/sec
(14,435 ft/sec)**

- **New Chemical Admixture**
 - **Viscosity Modifying Admixture (VMA)**
 - **Keeps Solids in Suspension**
 - **Does not Decrease Strength**
 - **Reduces Bleed**

Questions?



Contact Information

Brian H. Green, R.P.G.

Research Geologist (CEERD-GM-C)

Engineer Research and Development Center

3909 Halls Ferry Road

Vicksburg, MS 39180-6199

(601) 634-3216

Brian.H.Green@erdc.usace.army.mil





US Army Corps
of Engineers
Omaha District

Evaluation of The Use of Lithium Compounds in Controlling ASR in Concrete Pavement

Tri-Service Infrastructure Conference

3 August 2005

UNIVERSITY OF
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Department of Roads





US Army Corps
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Omaha District

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ASR Distress



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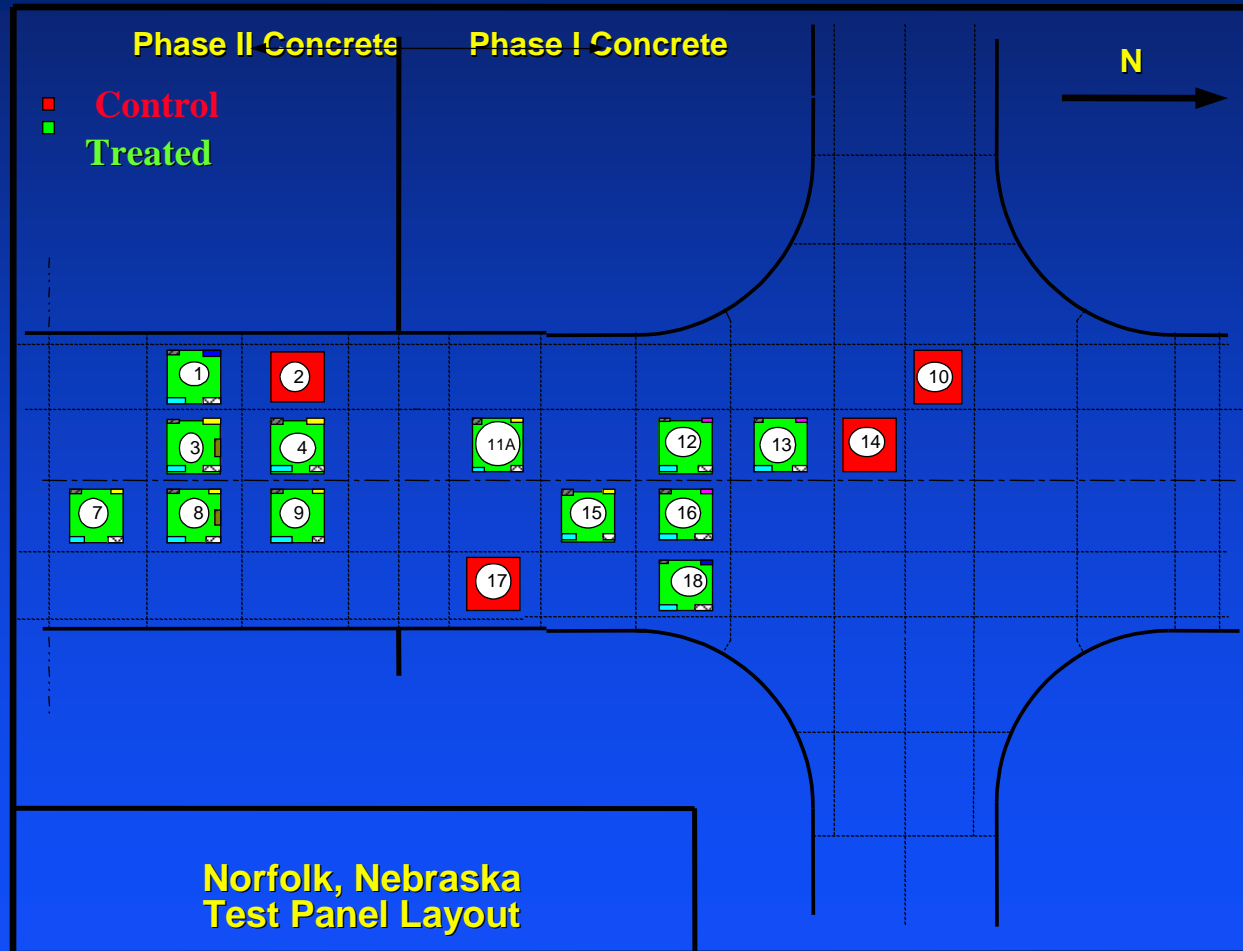
Department of Roads





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of Engineers
Omaha District

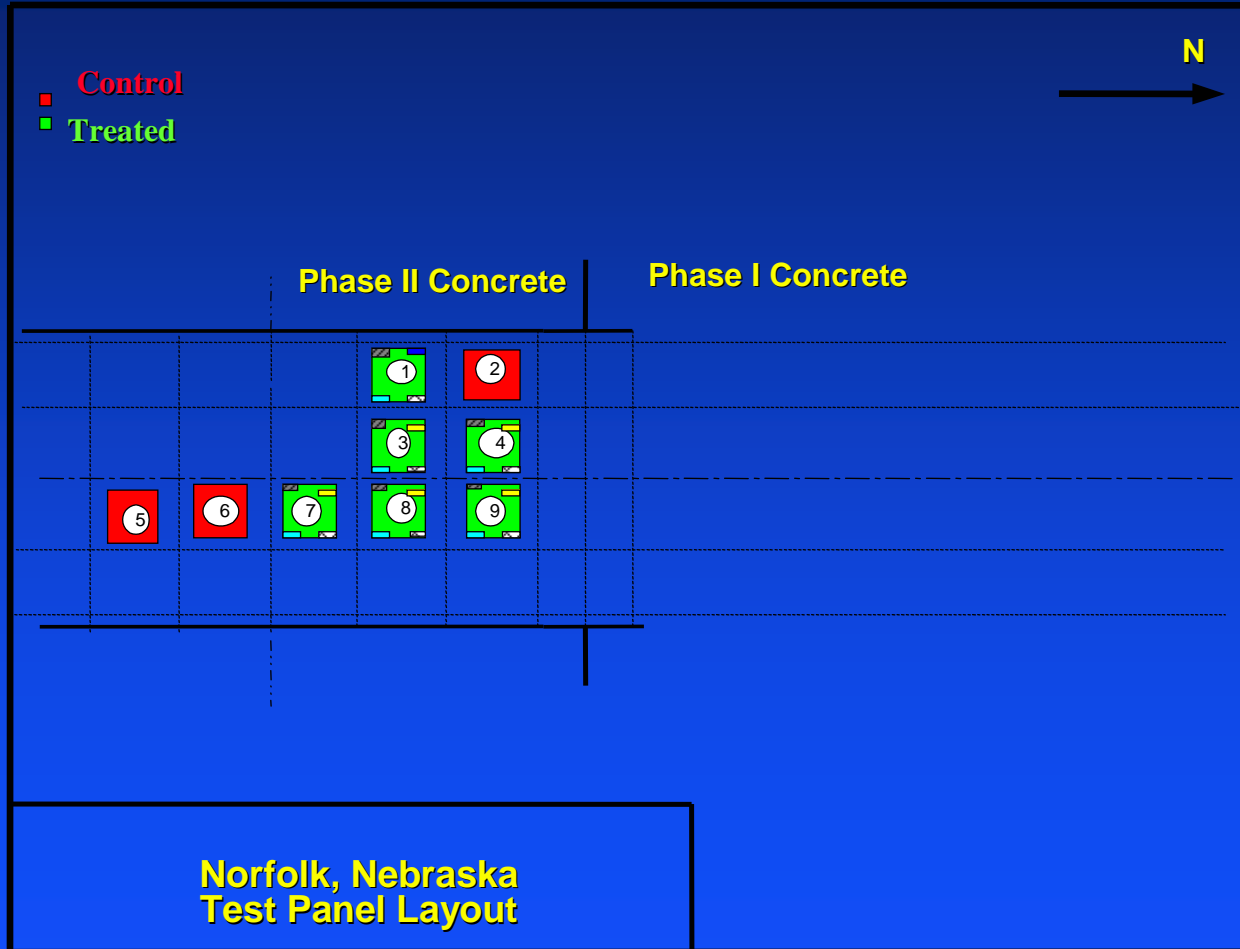
Site Layout





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of Engineers
Omaha District

Site Layout





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of Engineers
Omaha District

Site Characterization

- Petrographic Examination
- Map Cracking
- “V” Meter
- Schmidt Hammer
- Impact Echo
- Expansion / Contraction Measurements

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Core Samples



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Core Samples



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Petrographic Examination



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Crack Mapping



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Omaha District

Crack Mapping



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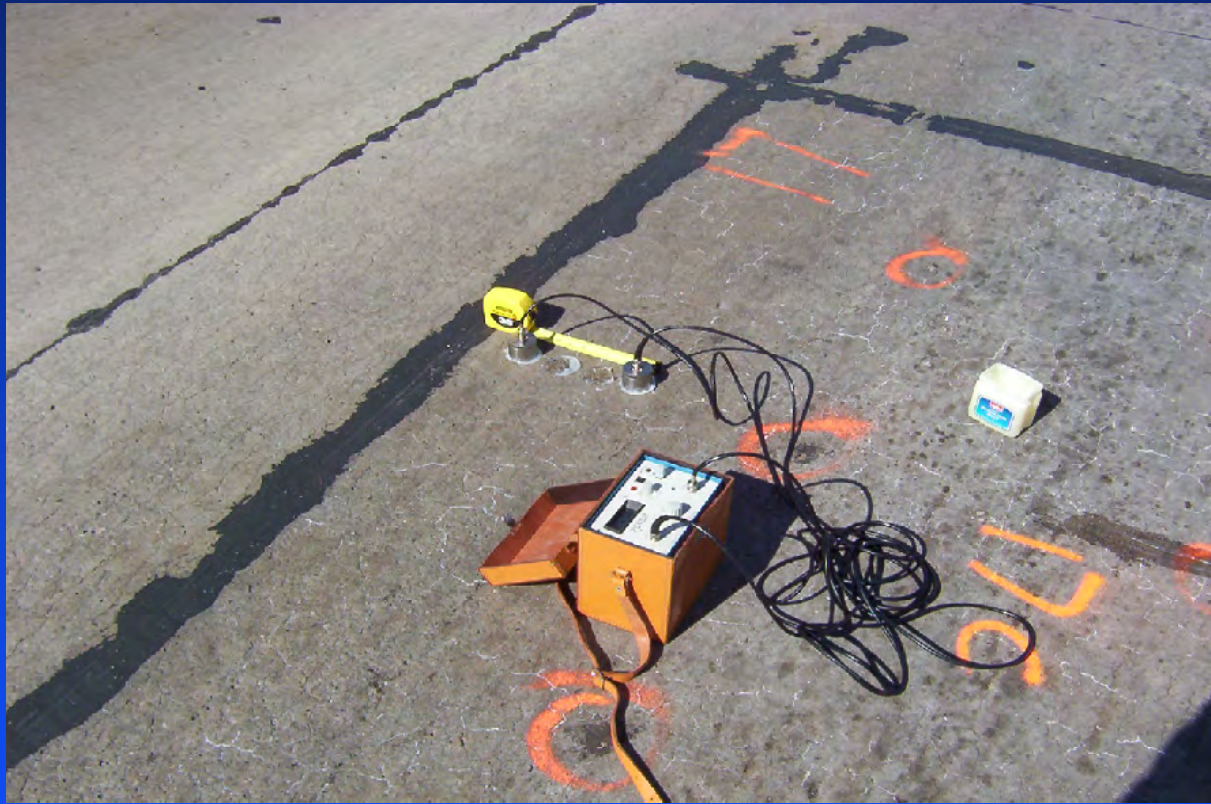
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Omaha District

“V”-Meter



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US Army Corps
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Omaha District

Schmidt Hammer



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Impact Echo



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Expansion / Contraction Demac Points



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Demac Point



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Demac Points



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Expansion / Contraction Measurements



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Expansion / Contraction Measurements



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Saw Cut Operation



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Full Depth Saw Cut



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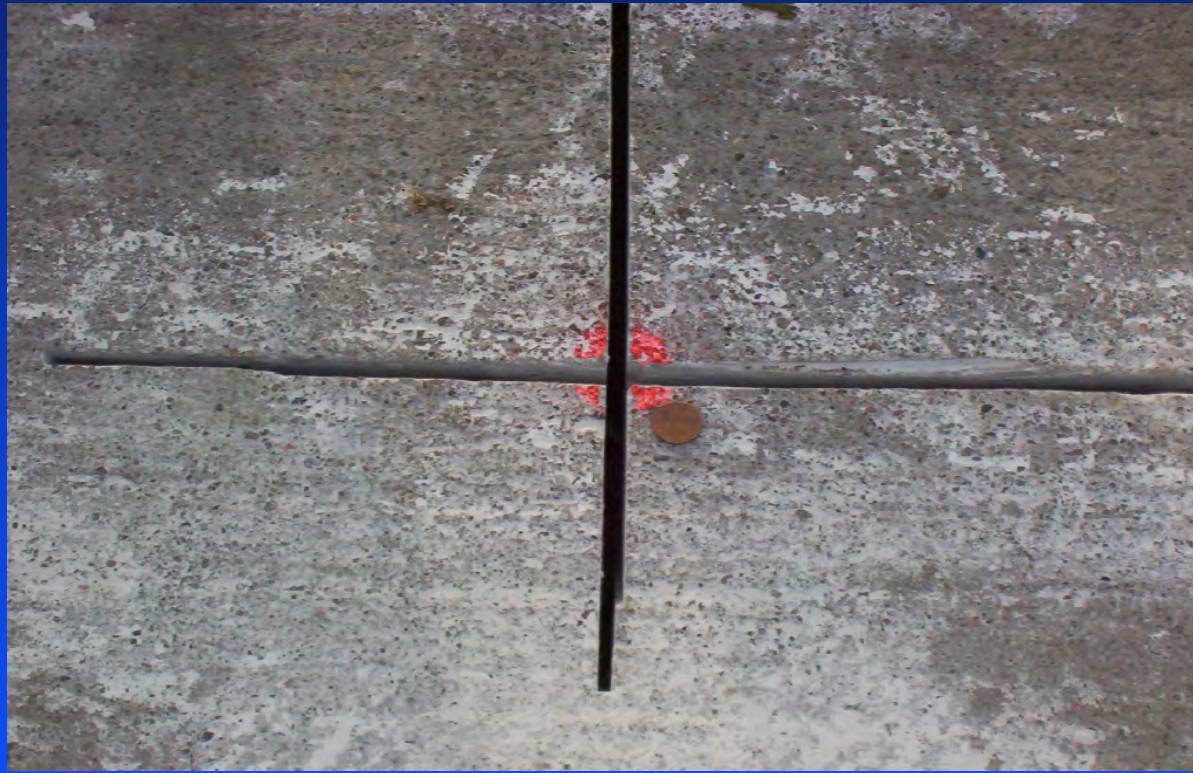
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Saw Cut



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Treated / Control Panels



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Lithium Application



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Lithium Application

Dates & Application Rates:

- Nov 2002: 0.006 – 0.012 gal/s.f.
- Dec 2002: 0.012 gal/s.f.
- May 2003: 0.006 gal/s.f.
- Oct 2003: 0.006 gal/s.f.
- May 2004: 0.006 gal/s.f.
- Oct 2004: 0.012 gal/s.f.

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Lithium Material



Lithium Nitrate

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Salt Residue



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Addition of Water



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Powder Samples



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Powder Samples



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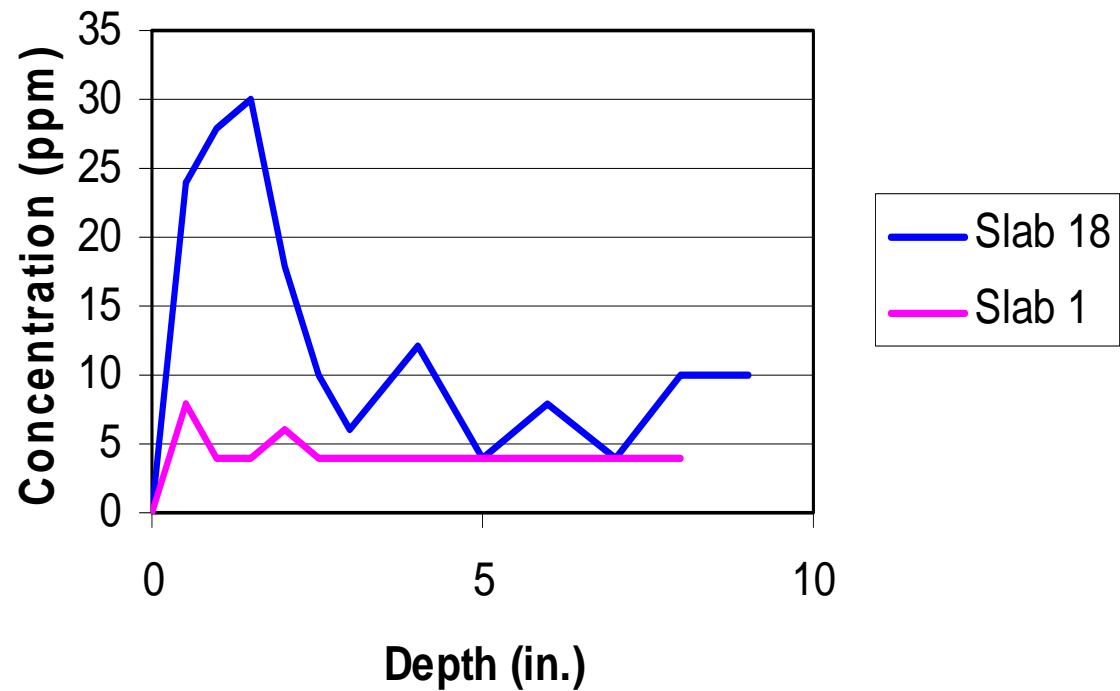
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Lithium Contents



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Pressure Injection



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Pressure Cell



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Pressure Cell



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Pressure Cell



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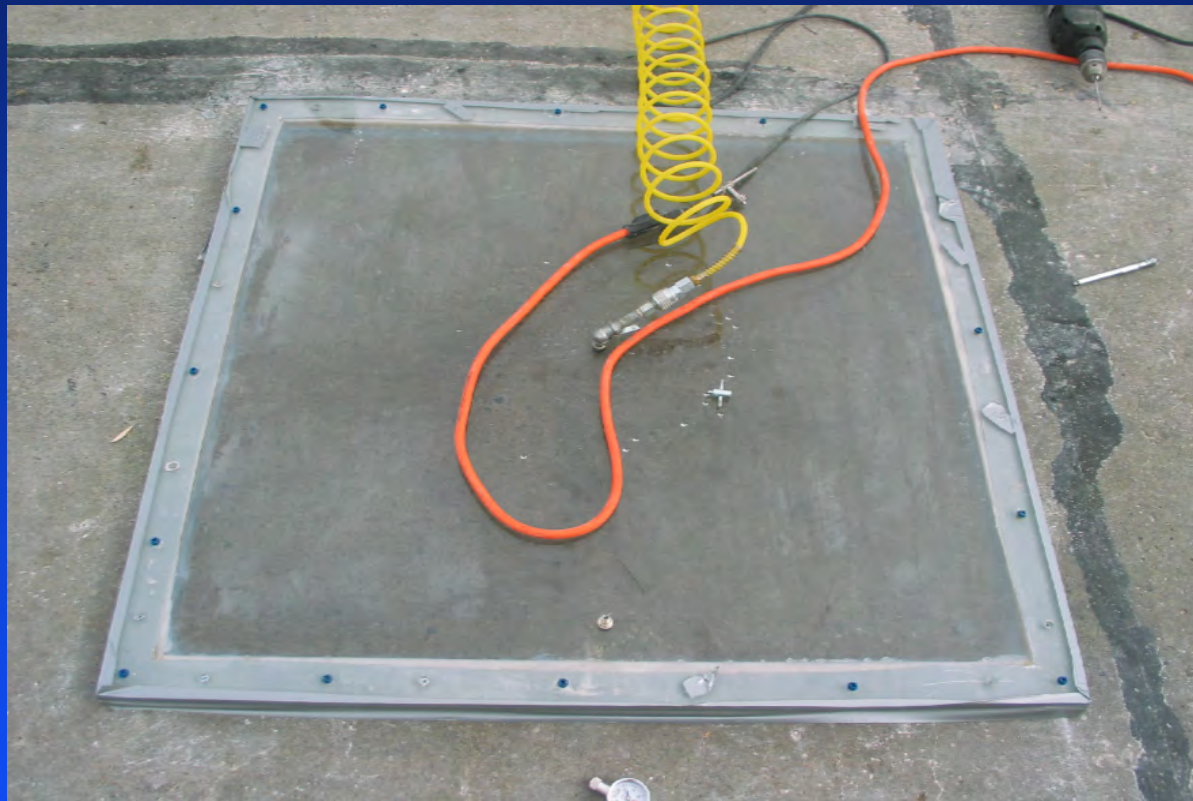
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Pressure Injection



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Pressure Application



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Vacuum Impregnation



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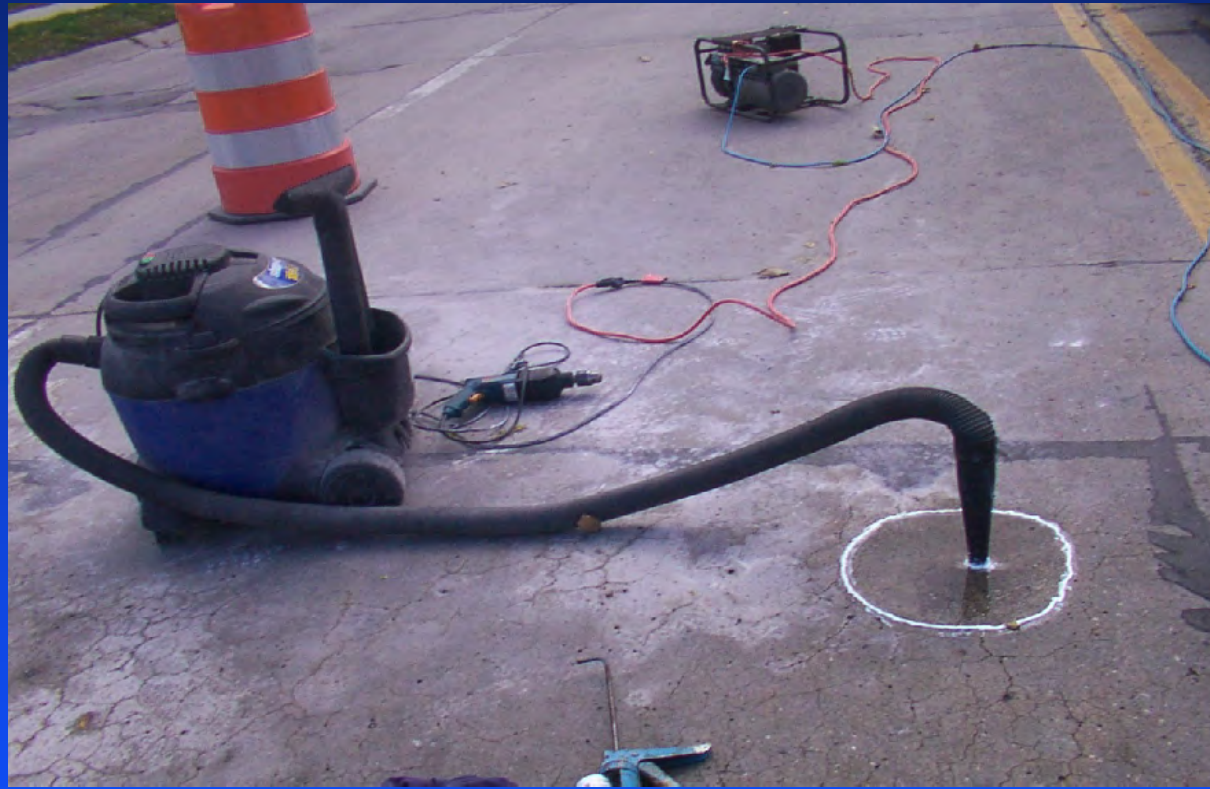
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Vacuum Impregnation



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Powder Samples



Lithium Content ????

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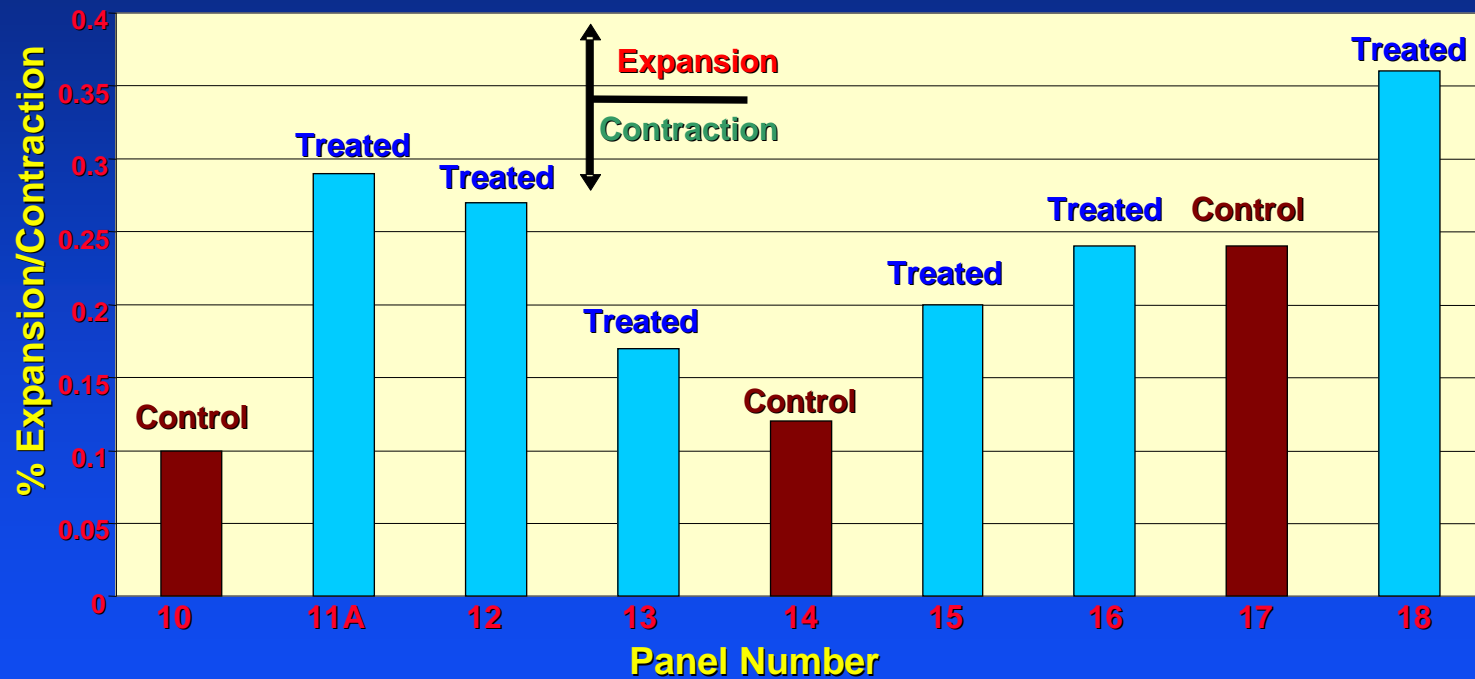
Data Analysis





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N-S Phase I Concrete



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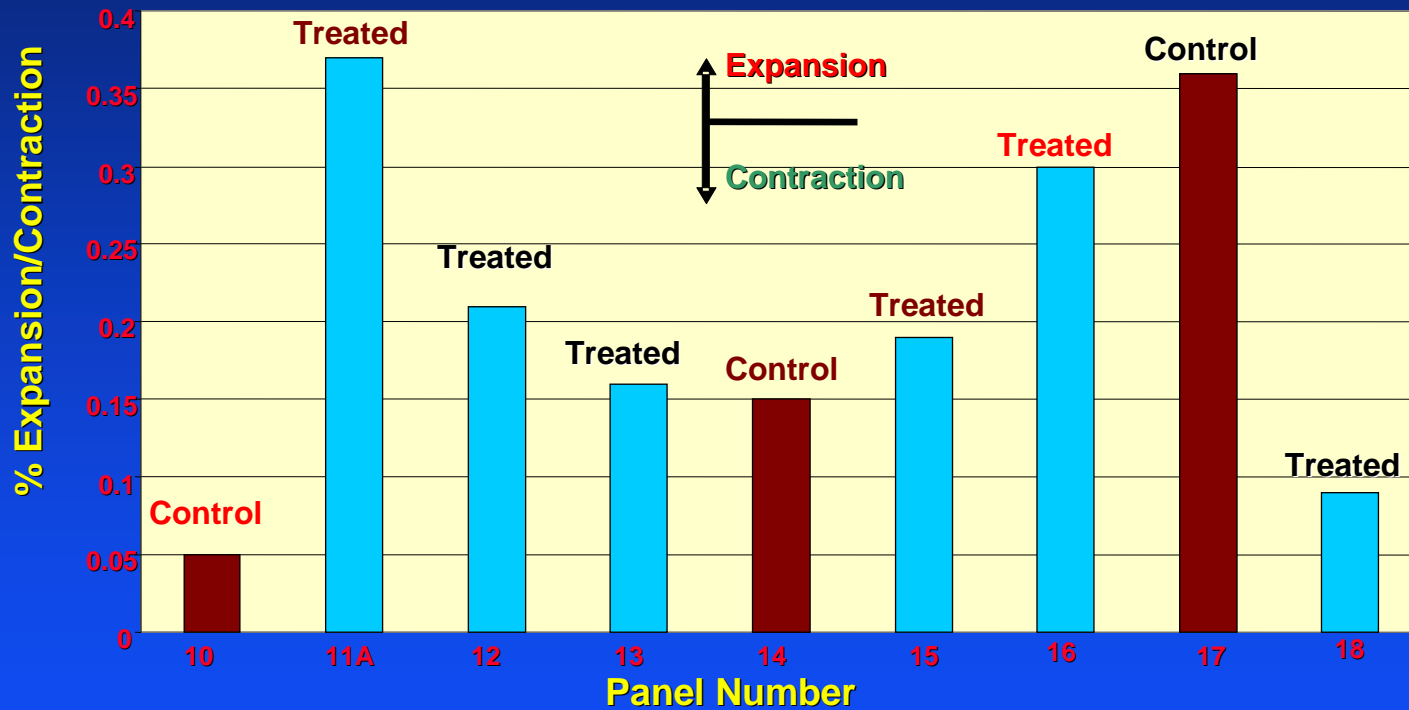
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E-W Phase I Concrete



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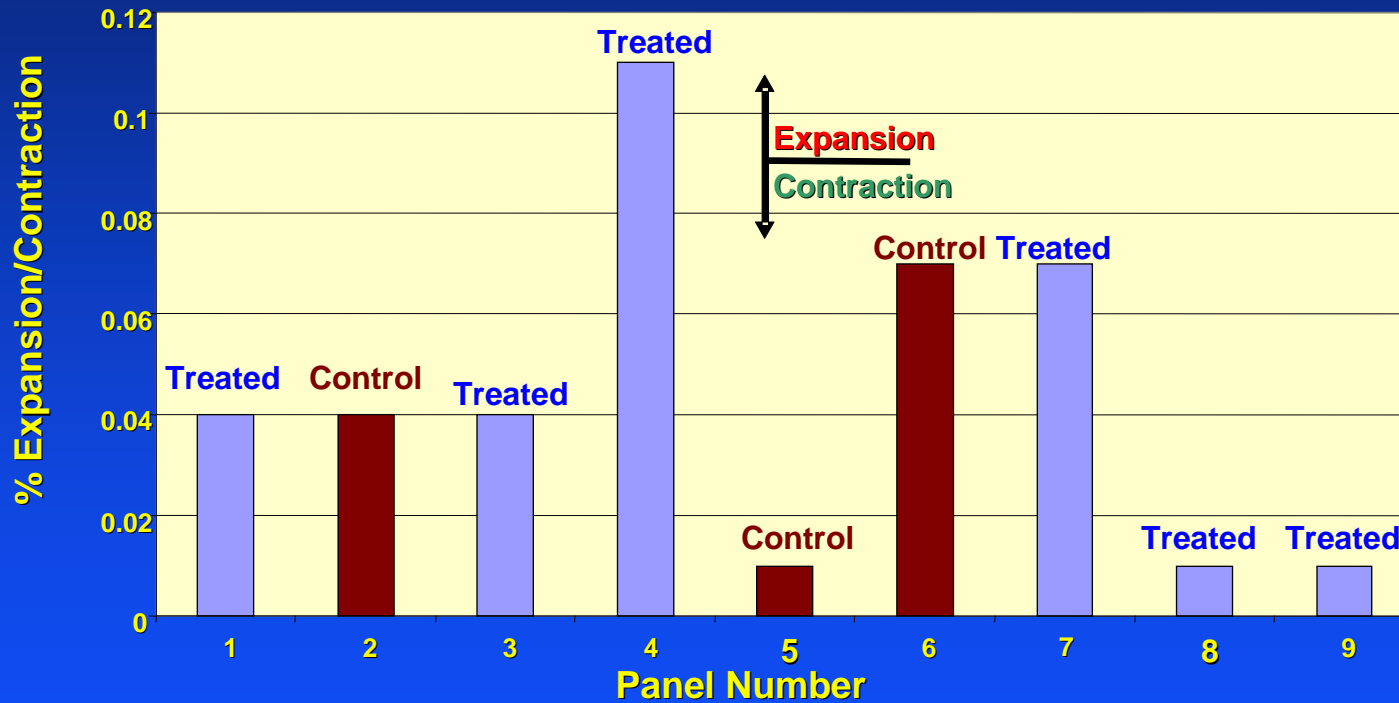
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N-S Phase II Concrete



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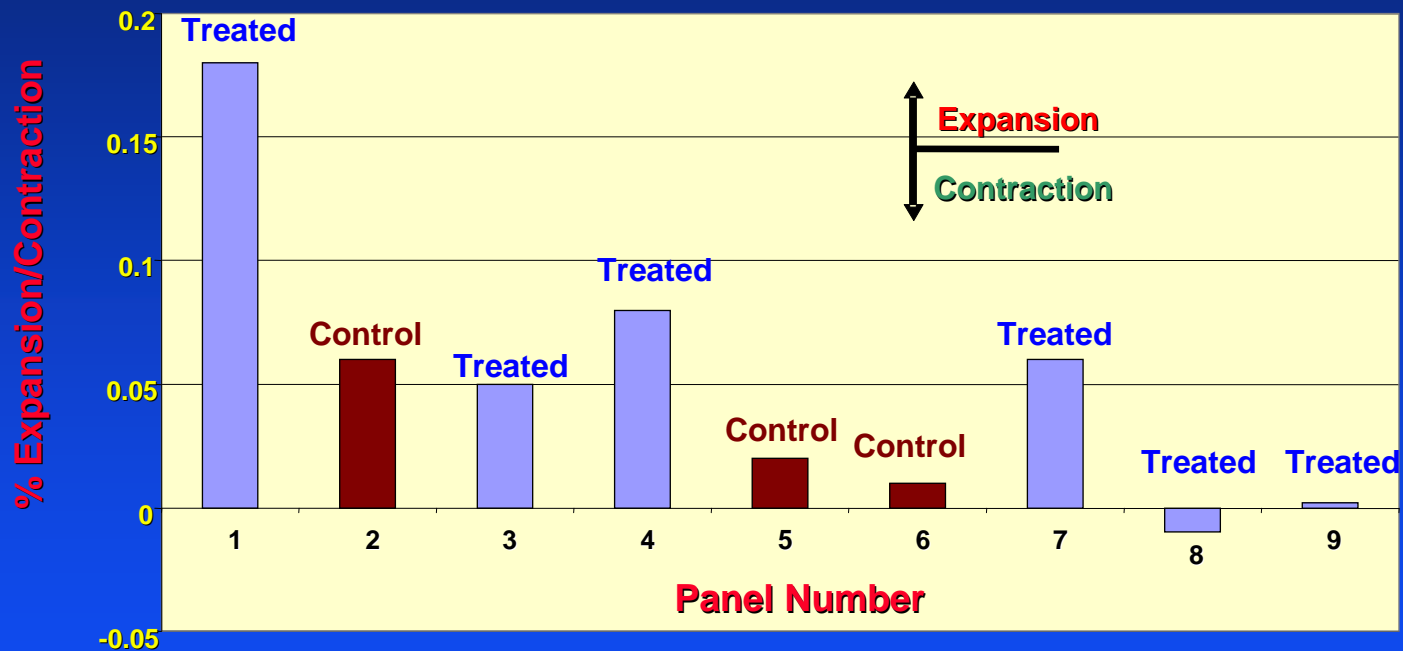
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E-W Phase II Concrete



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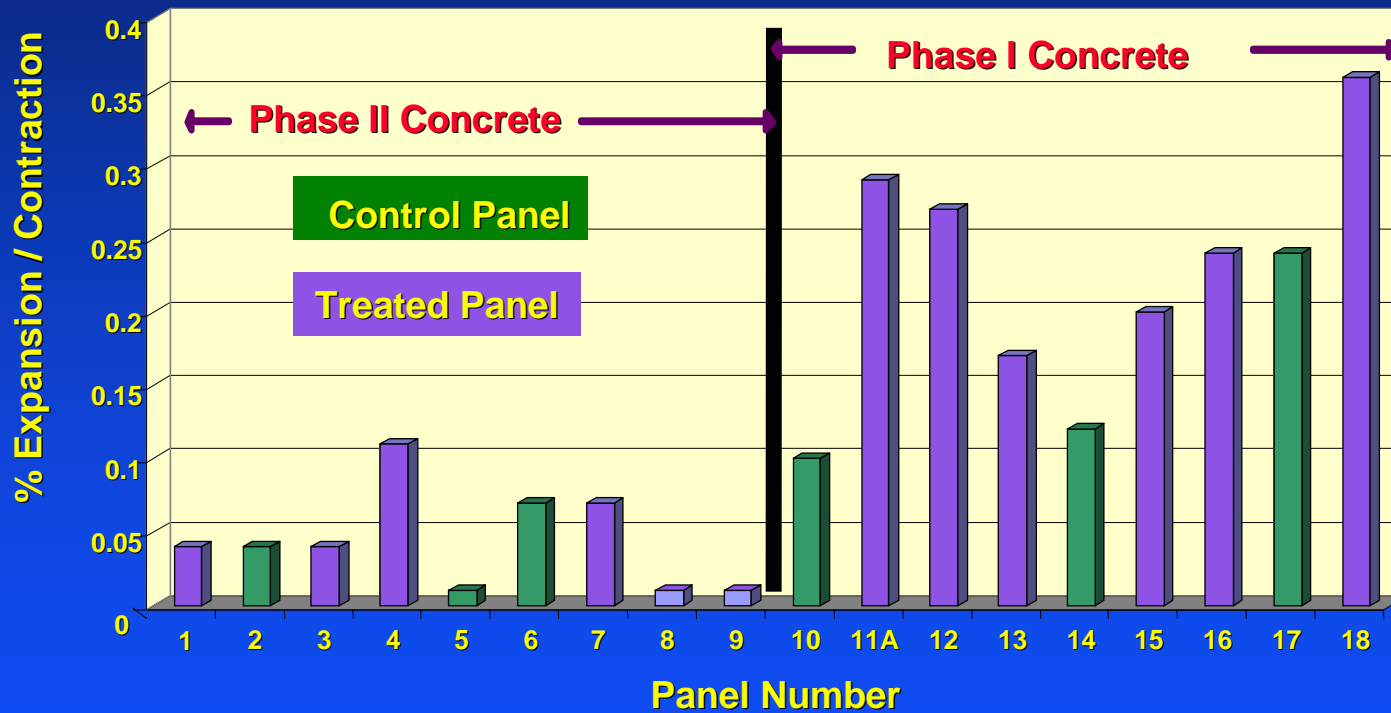
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N-S Expansion / Contraction



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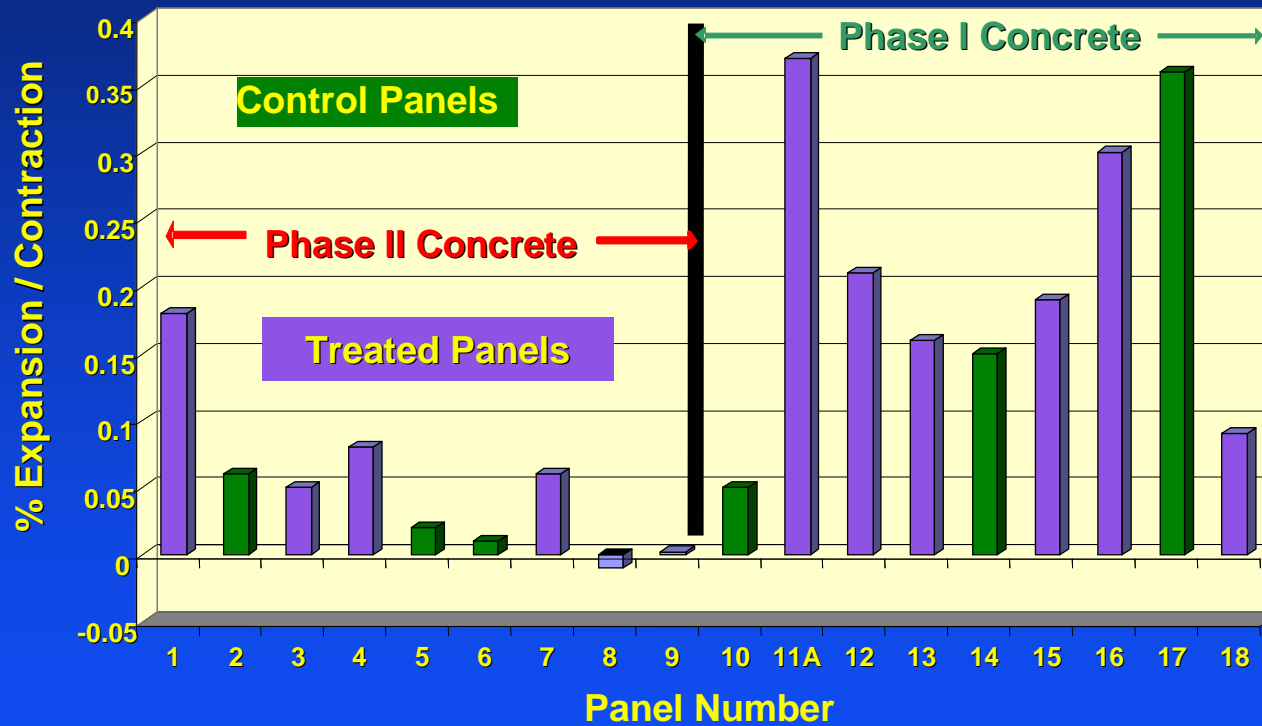
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E-W Expansion / Contraction



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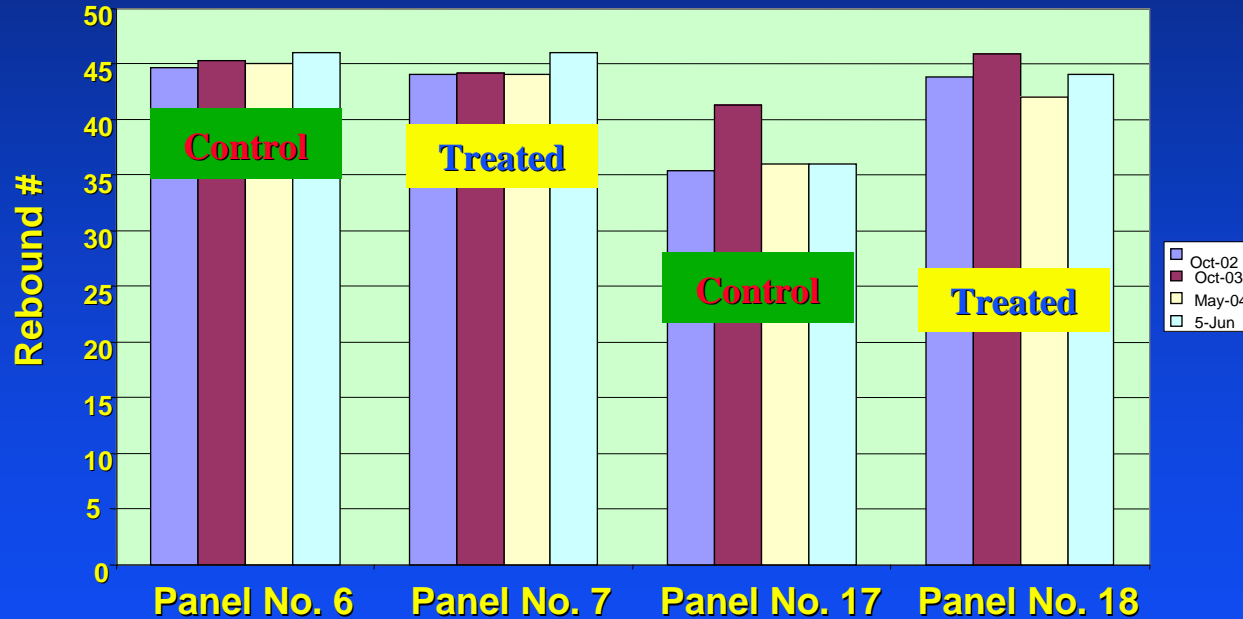
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Schmidt Hammer Evaluation



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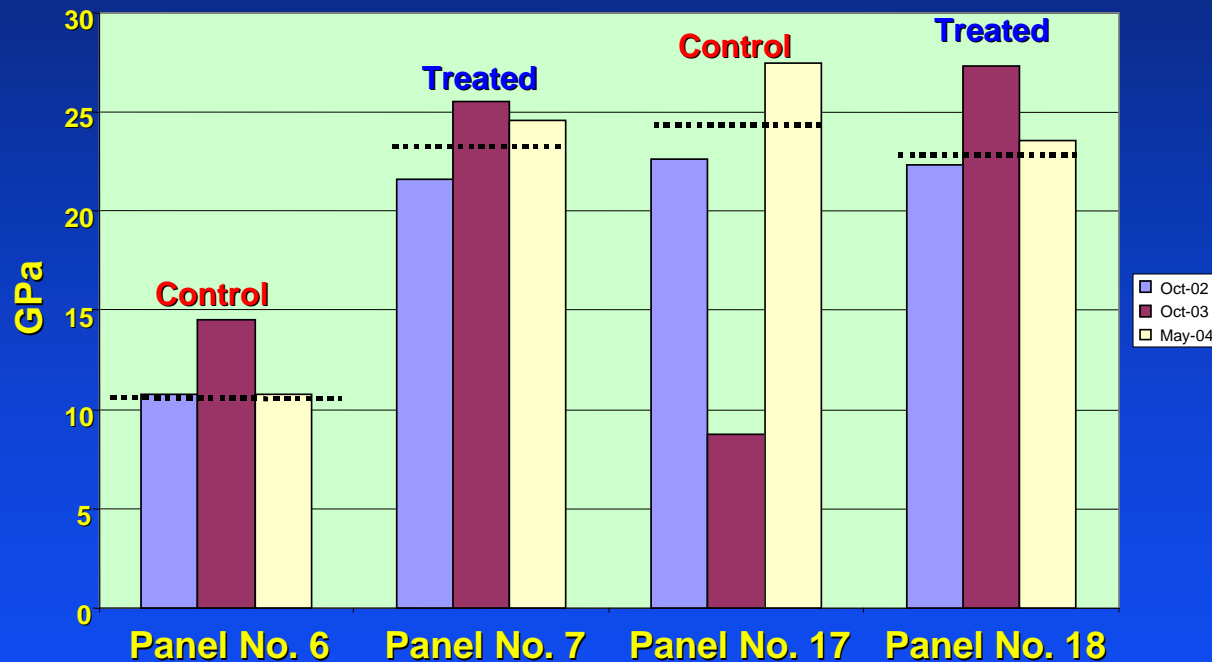
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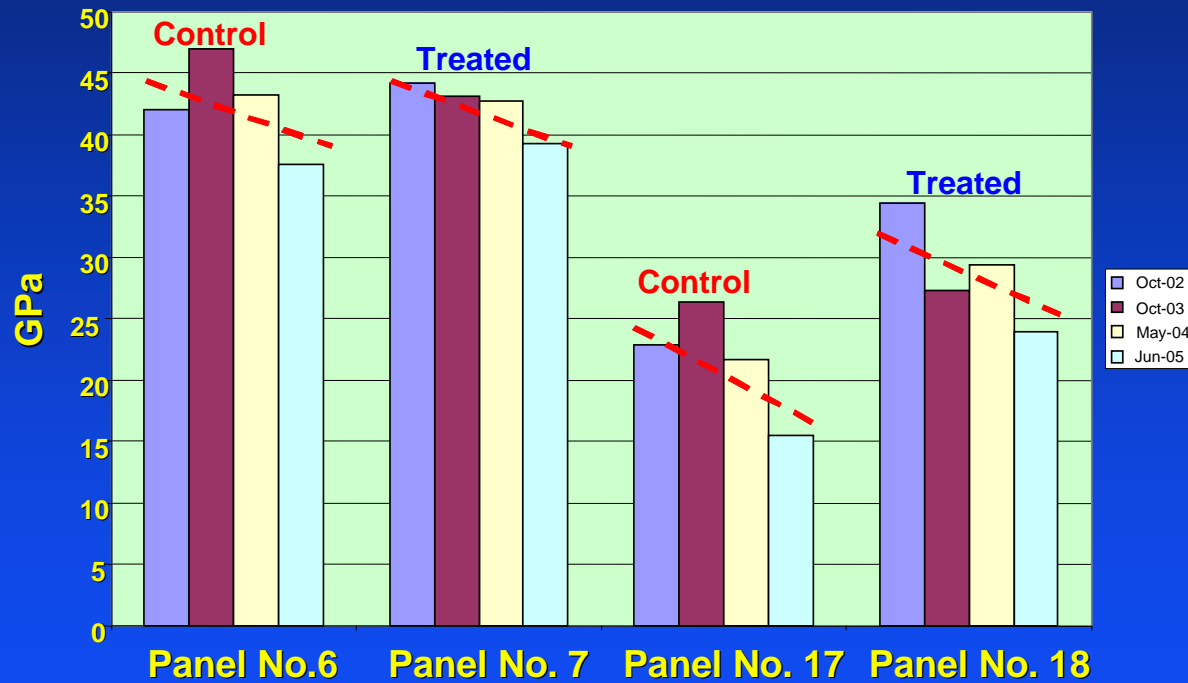
Impact Echo Evaluation





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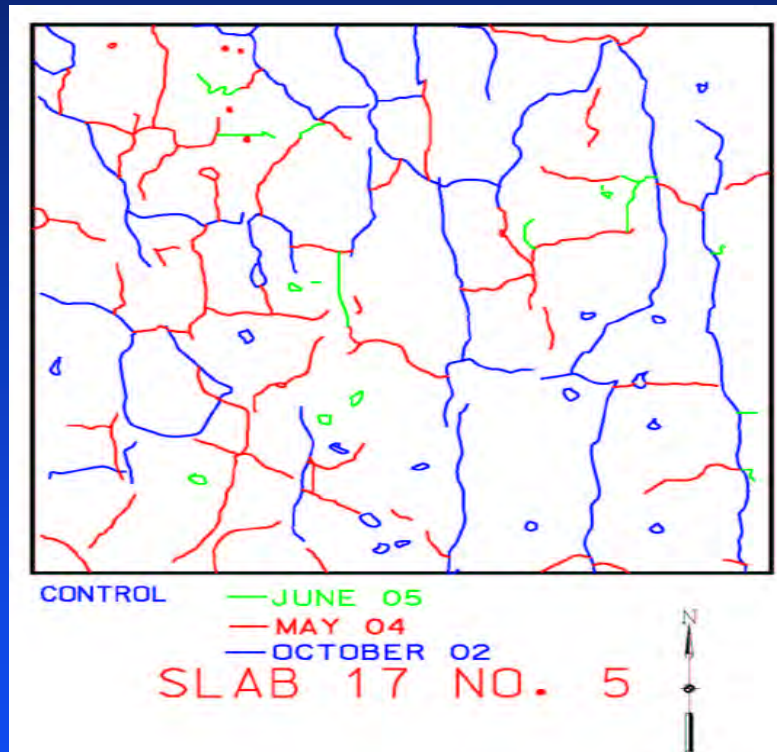
“V”- Meter Evaluation





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Map Cracking



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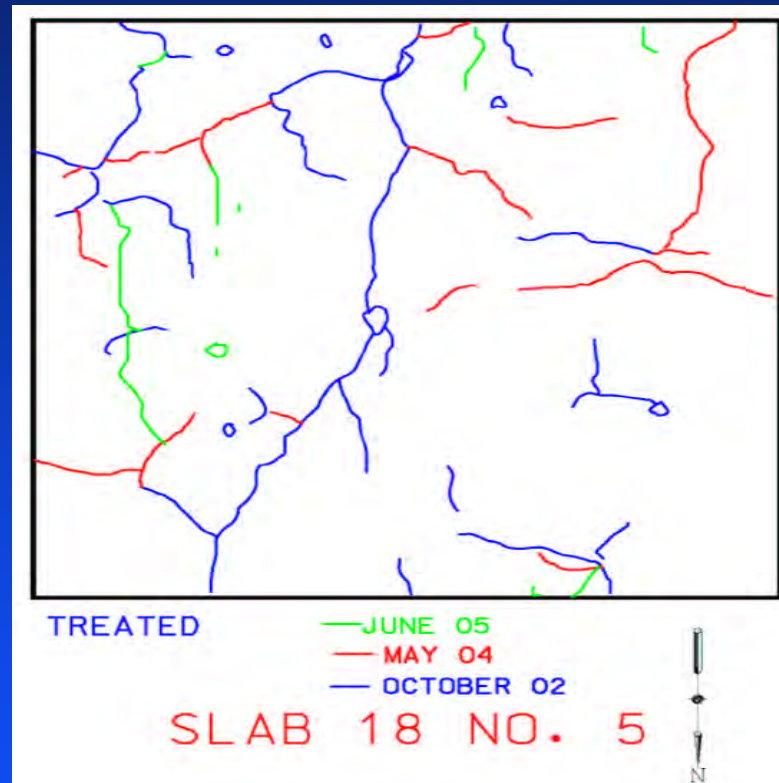
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Map Cracking



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QUESTIONS PP

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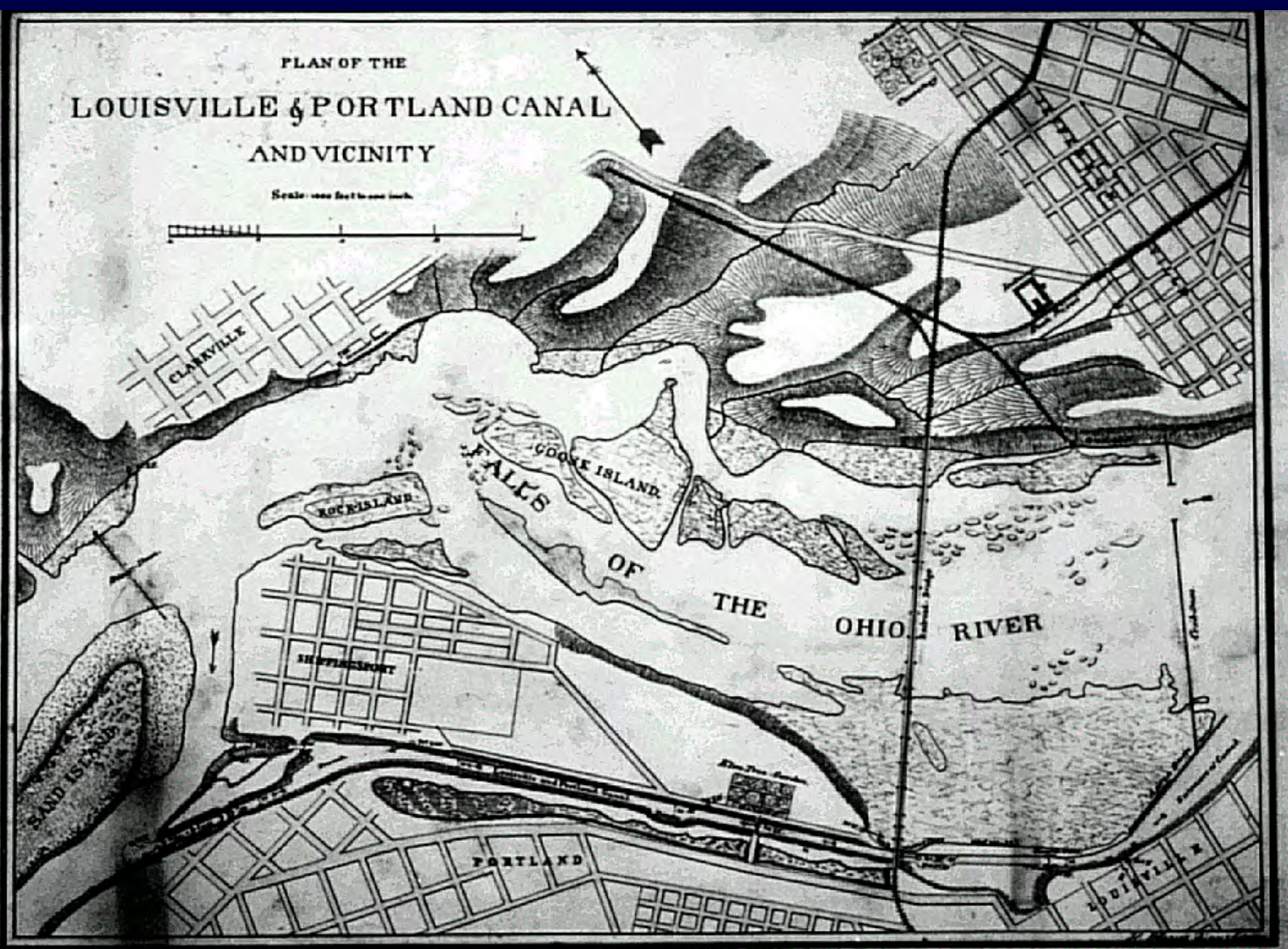
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Louisville District

**ROLLER COMPACTED CONCRETE
FOR
McALPINE LOCK
REPLACEMENT:
BY
DAVID E. KIEFER P.E.**



PLAN OF THE
LOUISVILLE & PORTLAND CANAL
AND VICINITY

Scale: one foot to one inch.



NOTE: The red arrows show the direction and points from which the views were taken.



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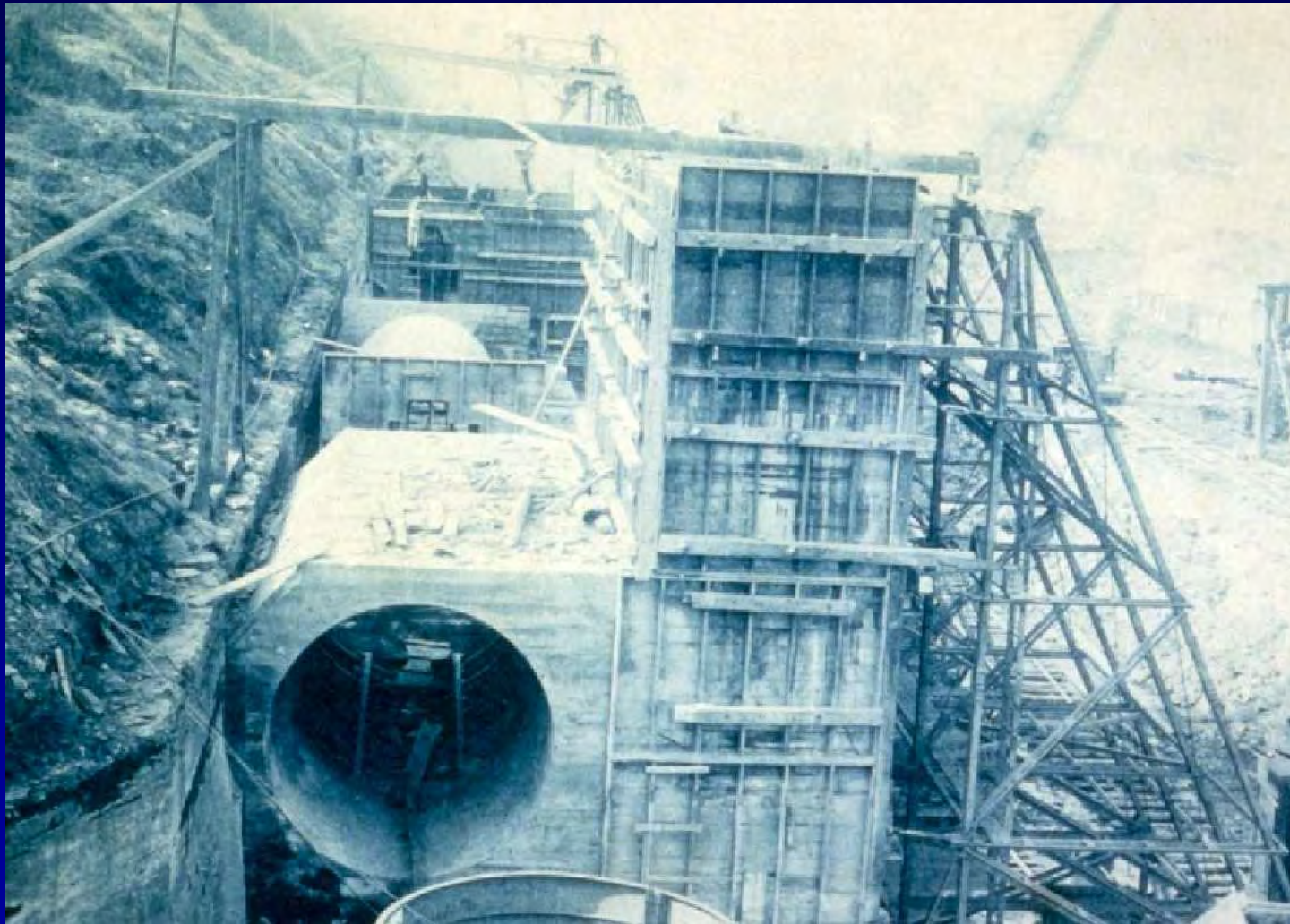
CONSTRUCTION OF 360' 2-STAGE LOCK, 1870





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CONSTRUCTION OF 600' LOCK, 1900





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CONSTRUCTION OF EXISTING 1200' LOCK, 1960







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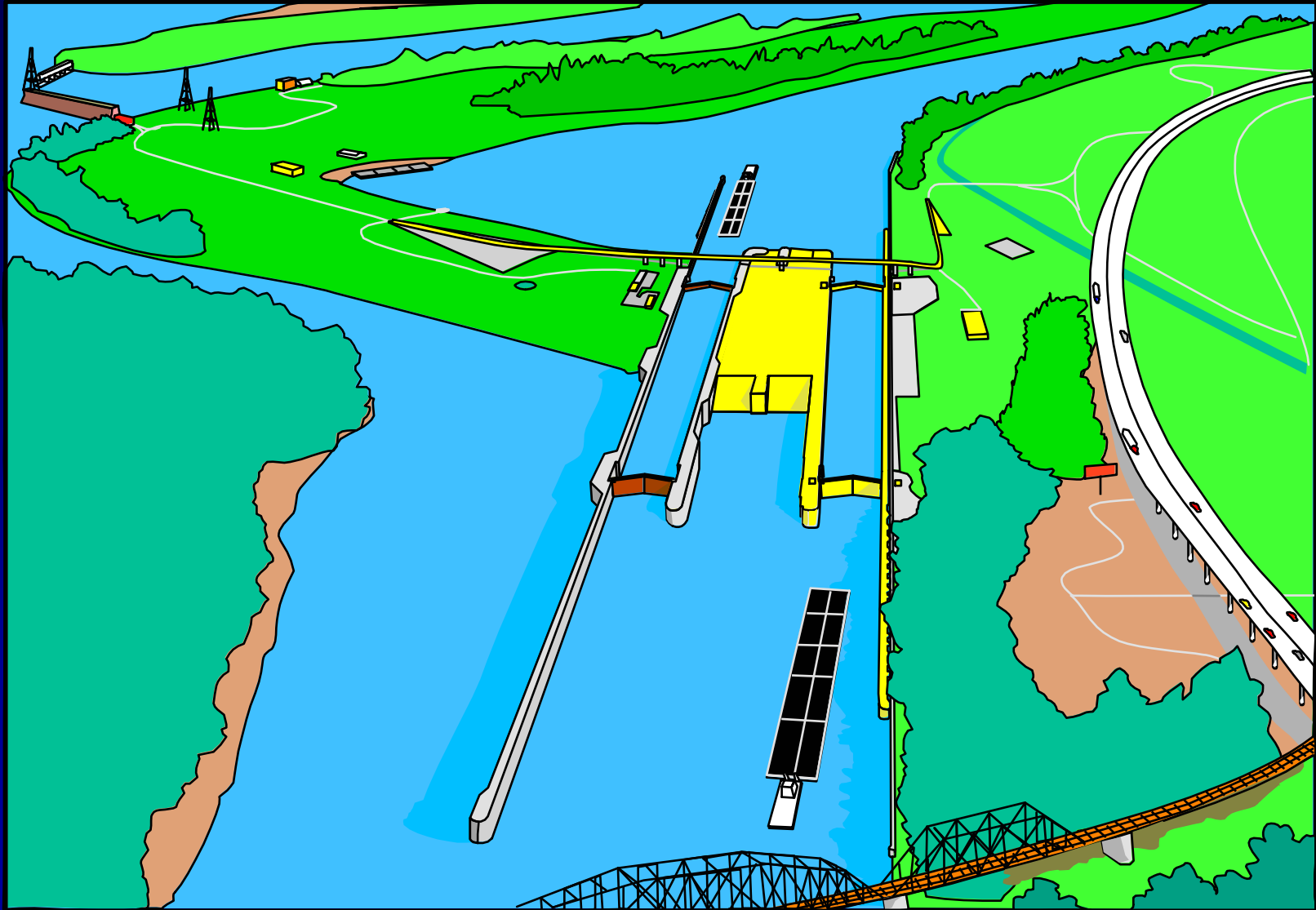
McALPINE LOCK REPLACEMENT PROJECT

- *360' lock deactivated due to miter gate failure
- *600' lock used only as back-up (slow and unreliable)
- *New 1200' lock will add capacity and reliability
- *New lock will be located south of existing 1200' lock



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NEW 1200' LOCK





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Downstream Cell Construction





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Downstream Cofferdam Cells





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Upstream Cofferdam Cells







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Demolition and Foundation Excavation





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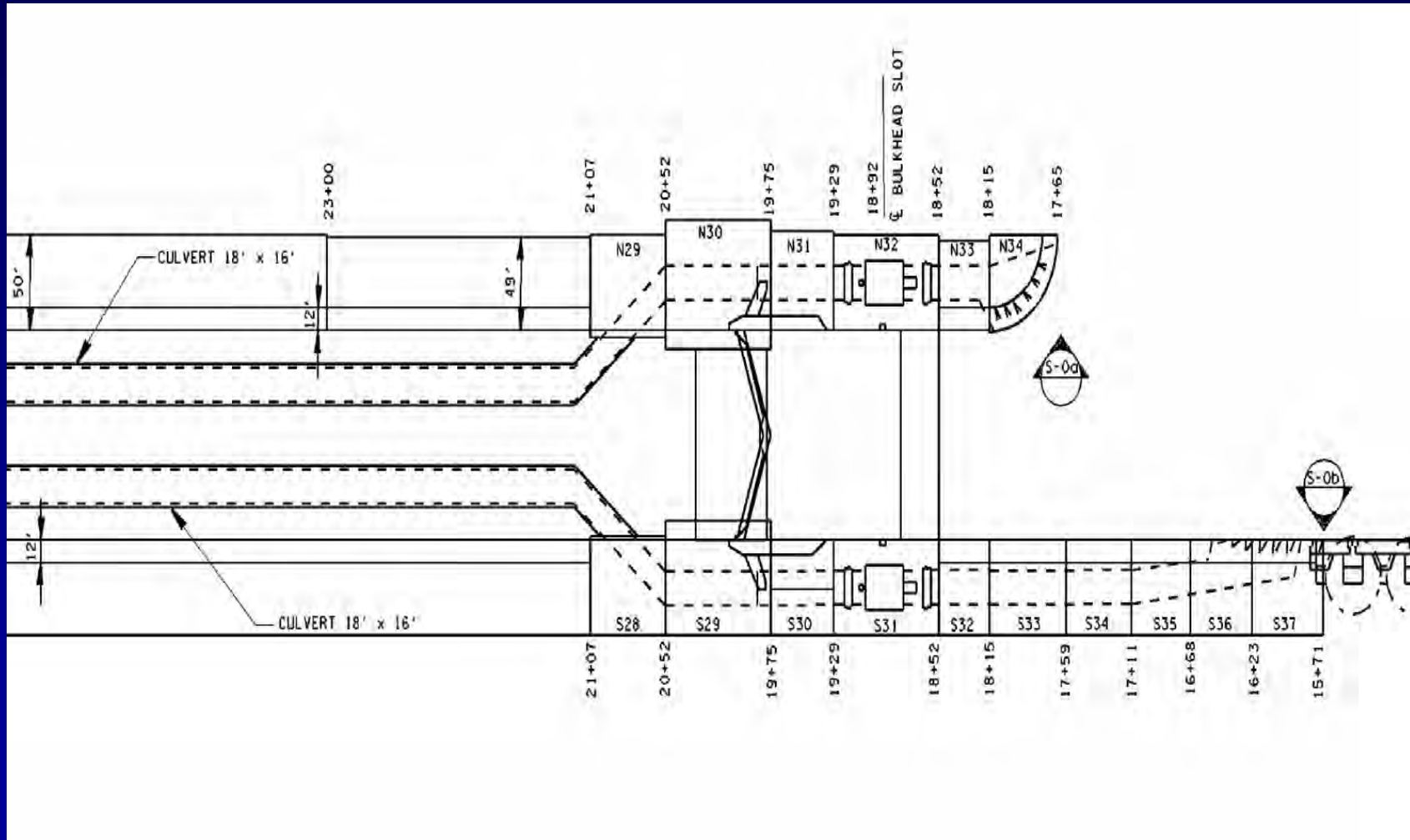
ENGINEERING AND DESIGN OF NEW LOCK

- *Evaluate Alternative/Innovative Emptying and Filling Systems
- *Evaluate Alternative Lock Wall Designs
- *Perform Hydraulic Model Studies
- *Select Best Alternative for Hydraulic and Wall Construction Considerations.



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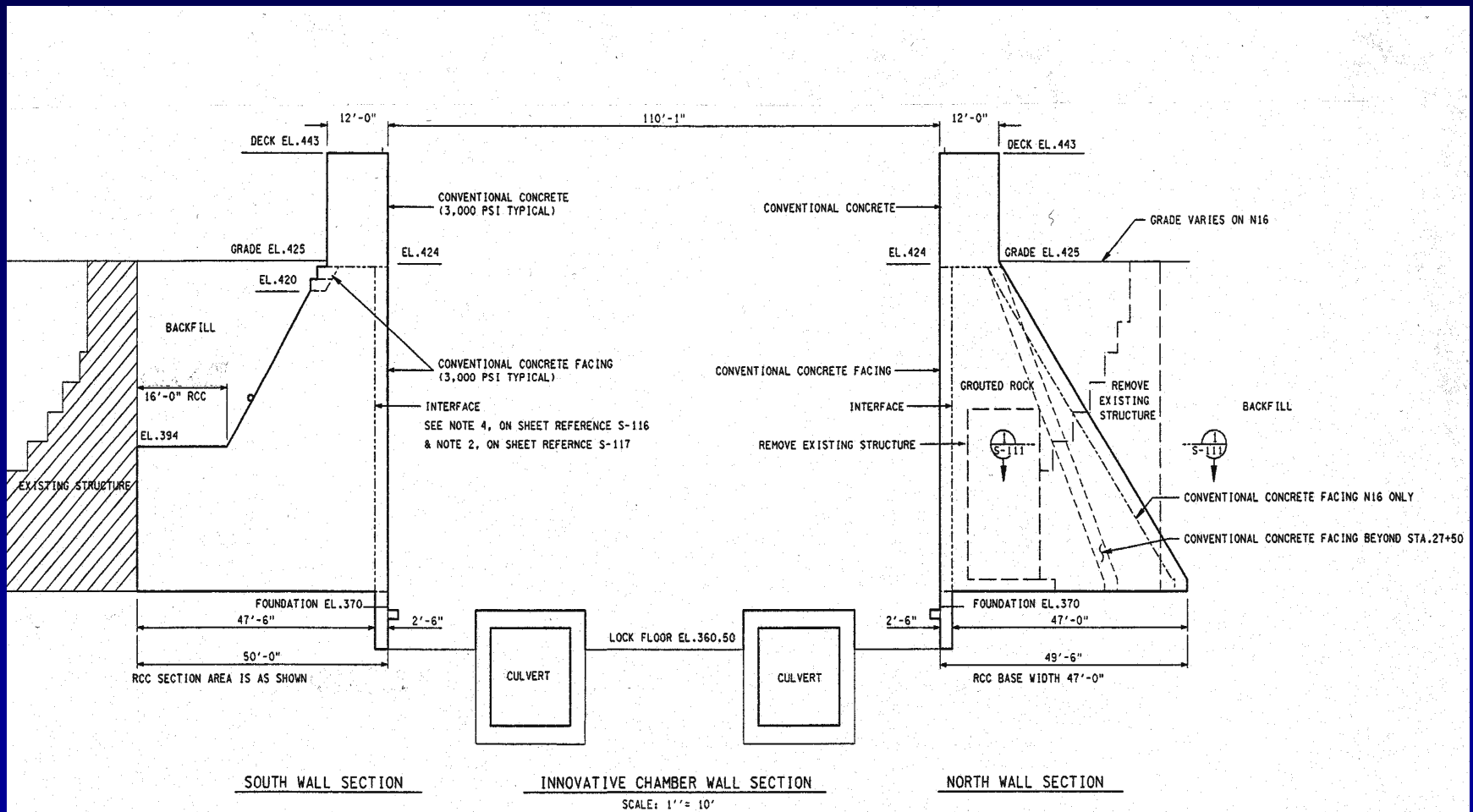
CONVENTIONAL INTAKE SYSTEM W/LOCK FLOOR CULVERTS





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NEW 1200' LOCK CROSS SECTION





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LOCK WALL OPTIONS

- * Thin-wall design with tie-back anchors
- * Reinforced Earth type wall
- * Thin-wall design with deadmen
- * Grouted Stone Fill
- * Roller Compacted Concrete (RCC)
Selected as Preferred Option



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ROLLER COMPACTED CONCRETE

- * ACI 207; Concrete of no-slump consistency in its unhardened state that is transported, placed, and compacted using earth and rockfill construction equipment.
- * A well graded aggregate mixture with a little bit of cement, fly ash and water thrown in for good measure.
- * Looks like a pile of wet rock.
- * Work it like dirt/soil, core it like concrete.



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RCC CONSTRUCTION





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McALPINE

LOCK CONSTRUCTION

- * 150,000 cubic yards rock excavation
- * 400,000 cubic yards concrete
- * Access Bridge: 42 drilled shafts,
6' diameter, 45' to 100' long
- * 165,000 cubic yards backfill
- * Traylor Bros, Granite, Massman (TGM)



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CONCRETE MATERIALS FOR MASS AND RCC

- * Crushed Limestone Coarse Aggregate,
2” NMSA
- * Natural, River Dredged Fine Aggregate
- * Class F Fly Ash
- * Type II, max 80 cal/g cement



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BATCH PLANT

- Twin 6-yard Besser compulsory mixers
- ASTM #3 (2-inch) and #57 (1-inch) coarse aggregate.
- Coarse aggregate wet belt and liquid nitrogen for temperature control.
- 70 Degree (Mass) and 80 Degree (RCC) temperature requirements.



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BATCH PLANT





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BATCH PLANT





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WET-CHILL BELT





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LIQUID NITROGEN





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TEST SECTION

- Constructed to demonstrate suitability of Contractor's equipment, methods and personnel.
- 50' long by 30' wide at top, (5) 1-foot lifts.
- Test section saw cut and inspected after placement for evaluation of RCC placement procedures.



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TEST SECTION





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TEST SECTION





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TEST SECTION





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McALPINE RCC CONSTRUCTION

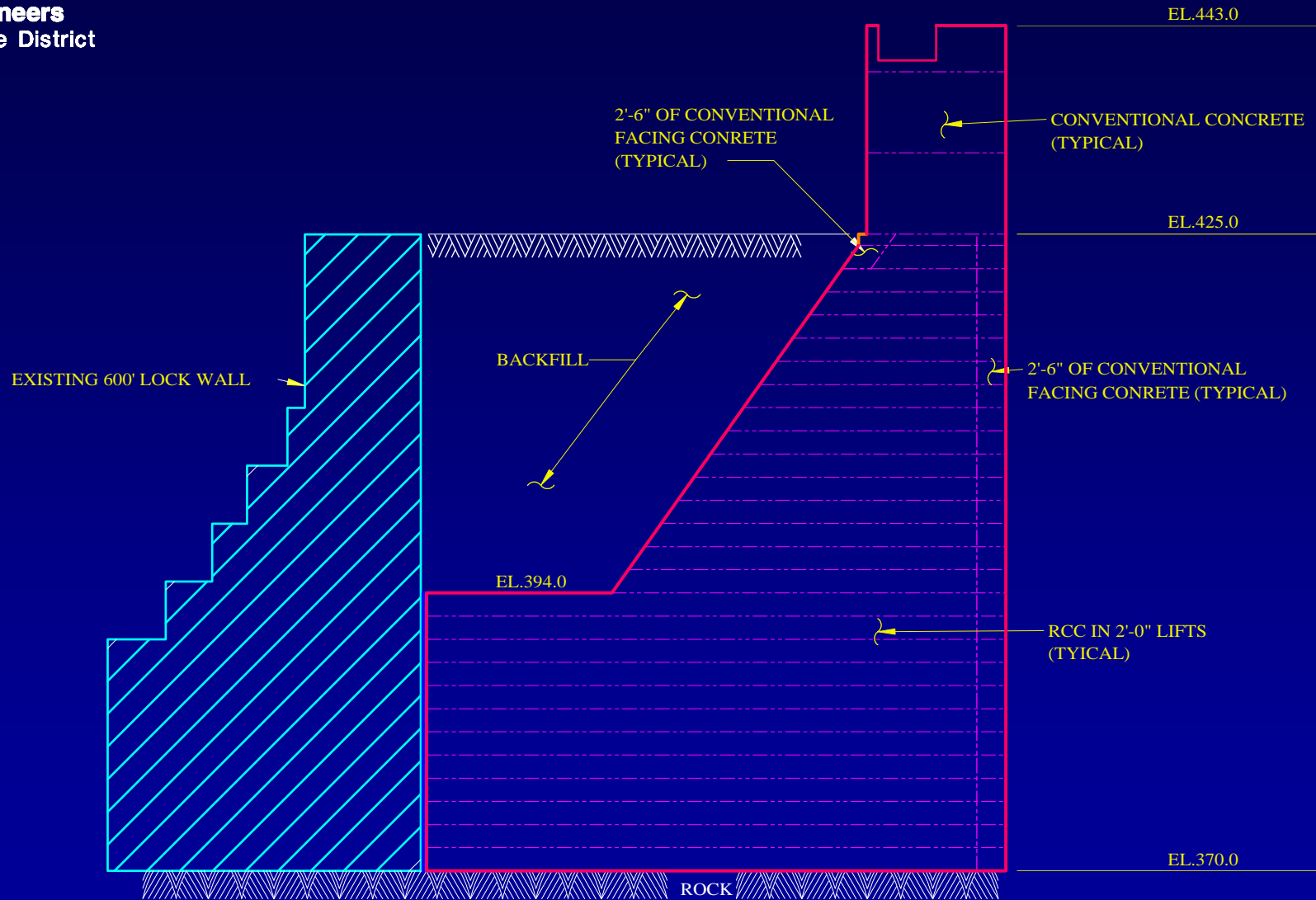
RCC and conventional concrete transported from batch plant using Maxon Agitor trucks.

- Rotec creter-crane primarily used for concrete placement.
- Buckets and creter-crane used for RCC facing concrete
- Large and small rollers used for compaction



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SOUTH LOCK WALL





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RCC CONSTRUCTION





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RCC CONSTRUCTION





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FACING CONCRETE





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BEDDING MORTAR





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CONSOLIDATION OF INTERFACE





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CONSOLIDATION OF INTERFACE





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PRIMARY ROLLER







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SECONDARY ROLLER







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SEGREGATION





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QC – NUCLEAR DENSITY TESTING





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INSERTING MONOLITH JOINT





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SLOPING BACKFACE







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LOCK WALL FACE





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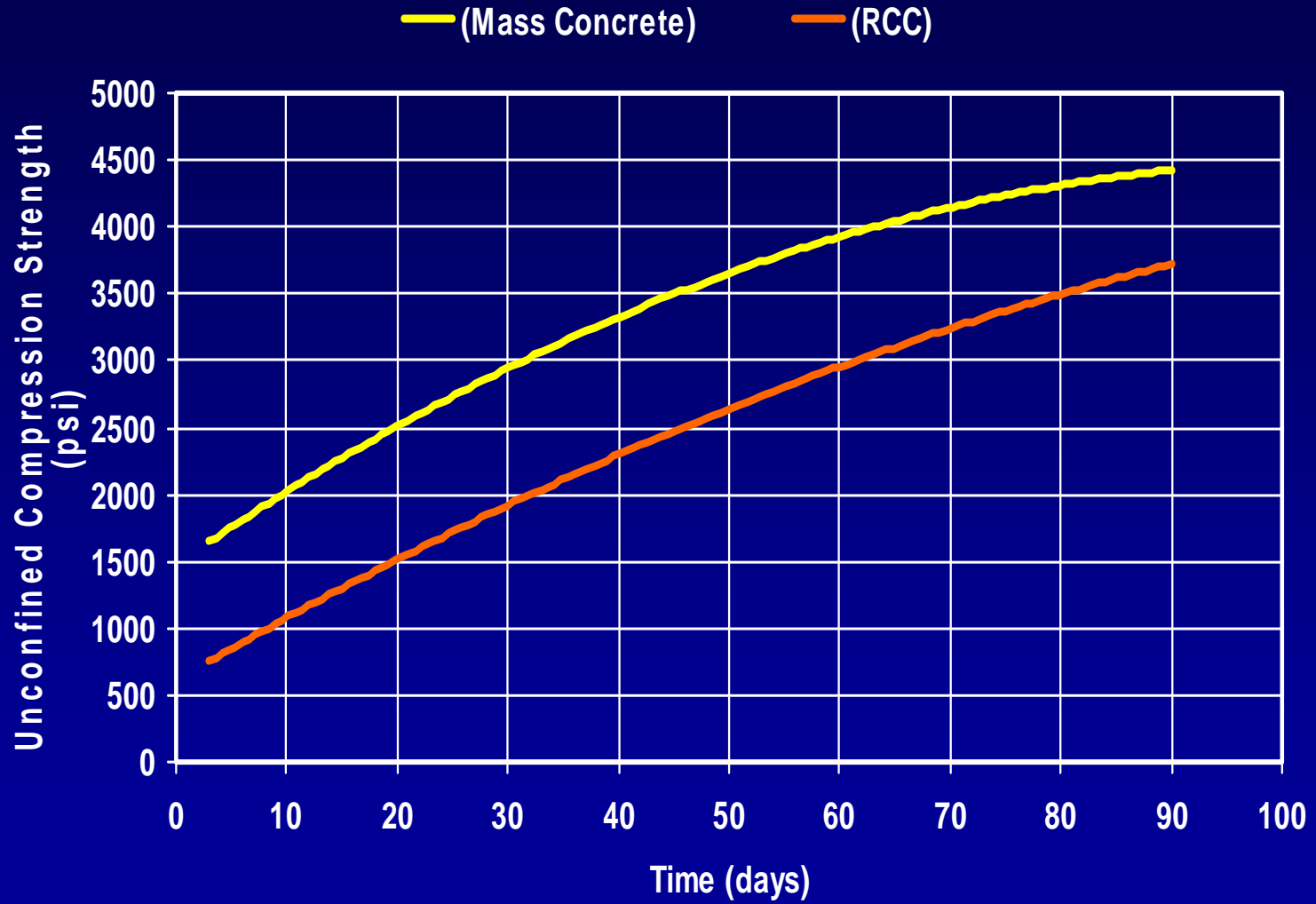
MIX PROPORTIONS

	<u>MASS</u>	<u>RCC</u>
Cement	259	120
Fly Ash	187	156
Coarse Agg.	2350	2440
Fine Agg.	1070	1132
Water	187	174



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Strength Gain versus Time





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Louisville District

JULY 2005





**McAlpine Locks and Dam
Completed Project
2007**



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Louisville District

QUESTIONS ???

Using Cement to Reclaim Asphalt Pavements

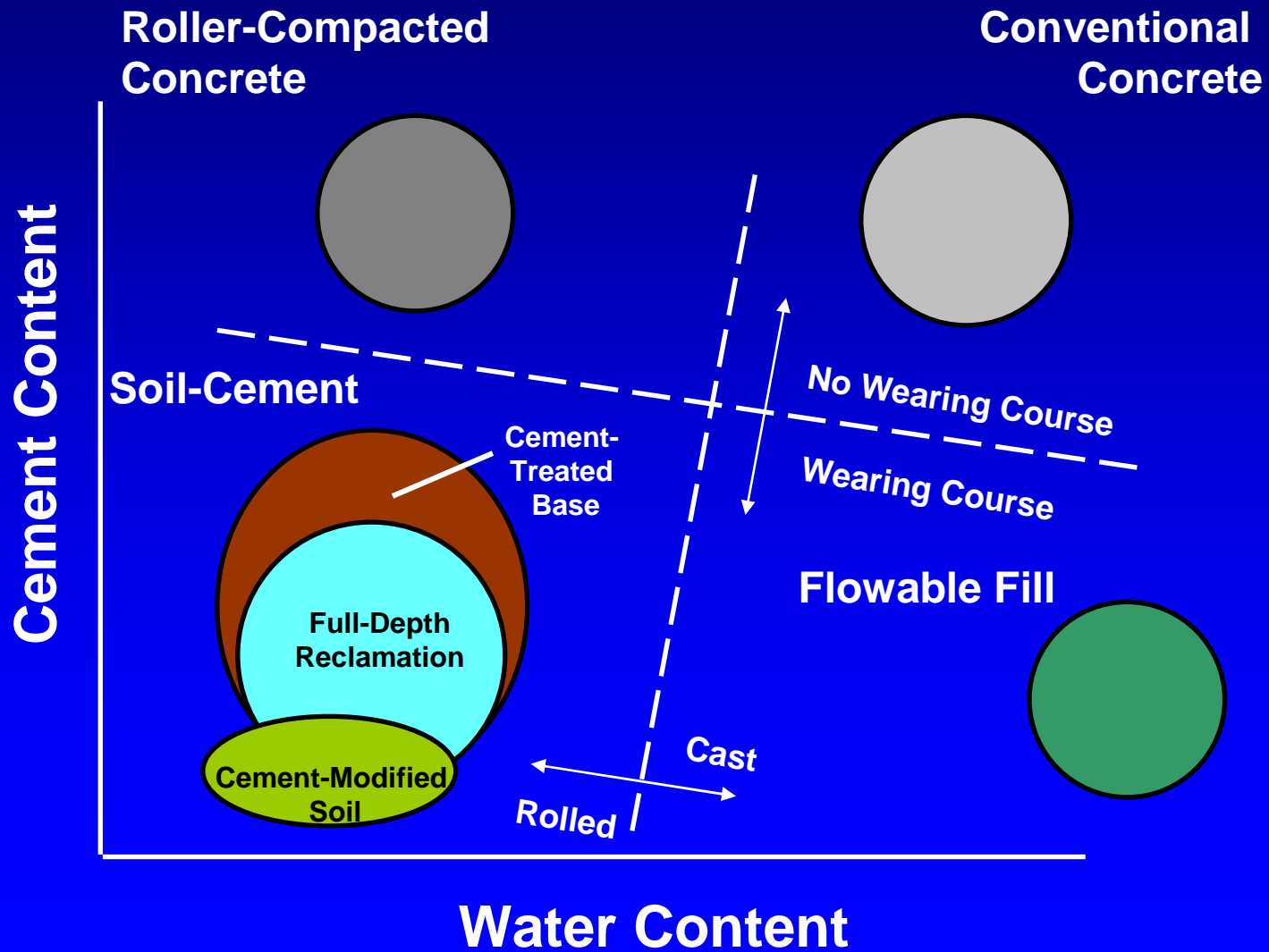
David R. Luhr PhD PE
Pavements Program Manager
Portland Cement Association
(919) 462-0840

What is Cement Stabilization?

- Mixture of portland cement, soil/aggregate and water
- Pulverized, mixed, compacted to high density



Cement-Based Pavement Materials



Full-Depth Reclamation (FDR)

- Pulverization and recycling of asphalt and base
- Utilizes existing materials
- Fast and convenient
- Eliminates new base
- Environmentally friendly



Pavement Distress



Alligator Cracking



Base Failure

Pavement Distress



Excessive Patching

Advantages of FDR

- **Use of in-situ materials**
- **Little or no material hauled off and dumped**
- **Conserves virgin material**
- **Saves cost by using in-place “investment”**
- **Saves energy by reducing mining, hauls**

Benefits

Full Depth Reclamation

Surface Course

6-10" FDR

Subgrade

Mill & Fill

1.5" Mill & Fill

HMA

Base/Sub-base

Subgrade

Overlay

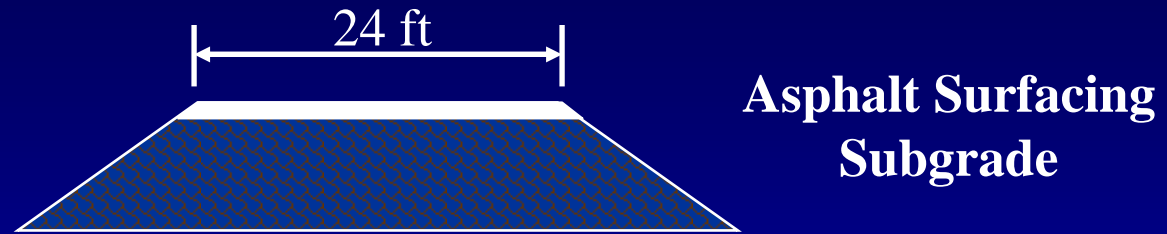
1.5" Overlay

HMA

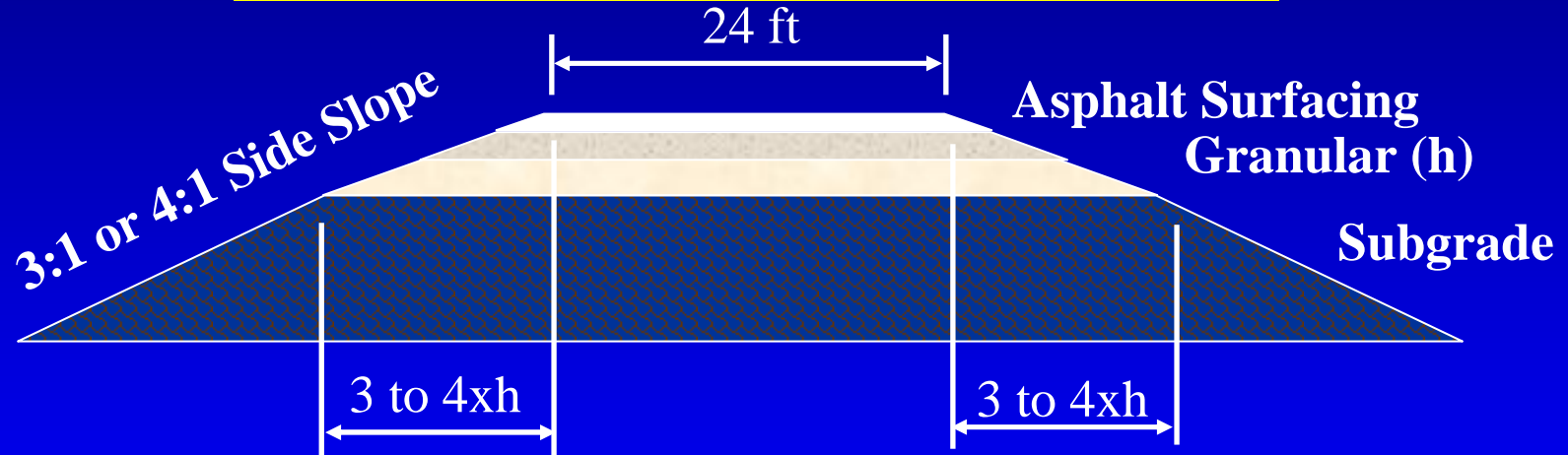
Base/Sub-base

Subgrade

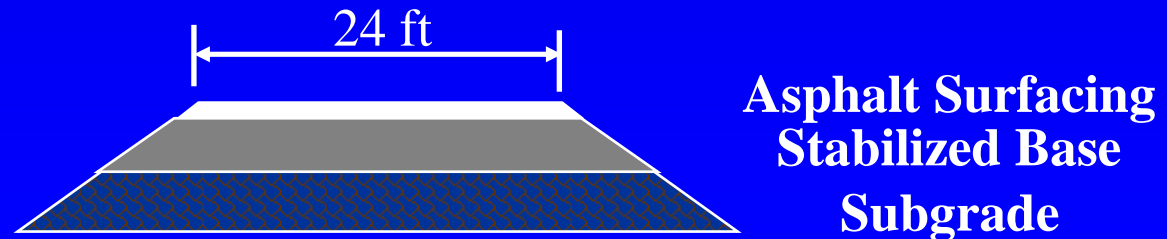
Existing Thin Paved Structure



Conventional Build Up Granular Structure



Full-Depth Recycled Structure



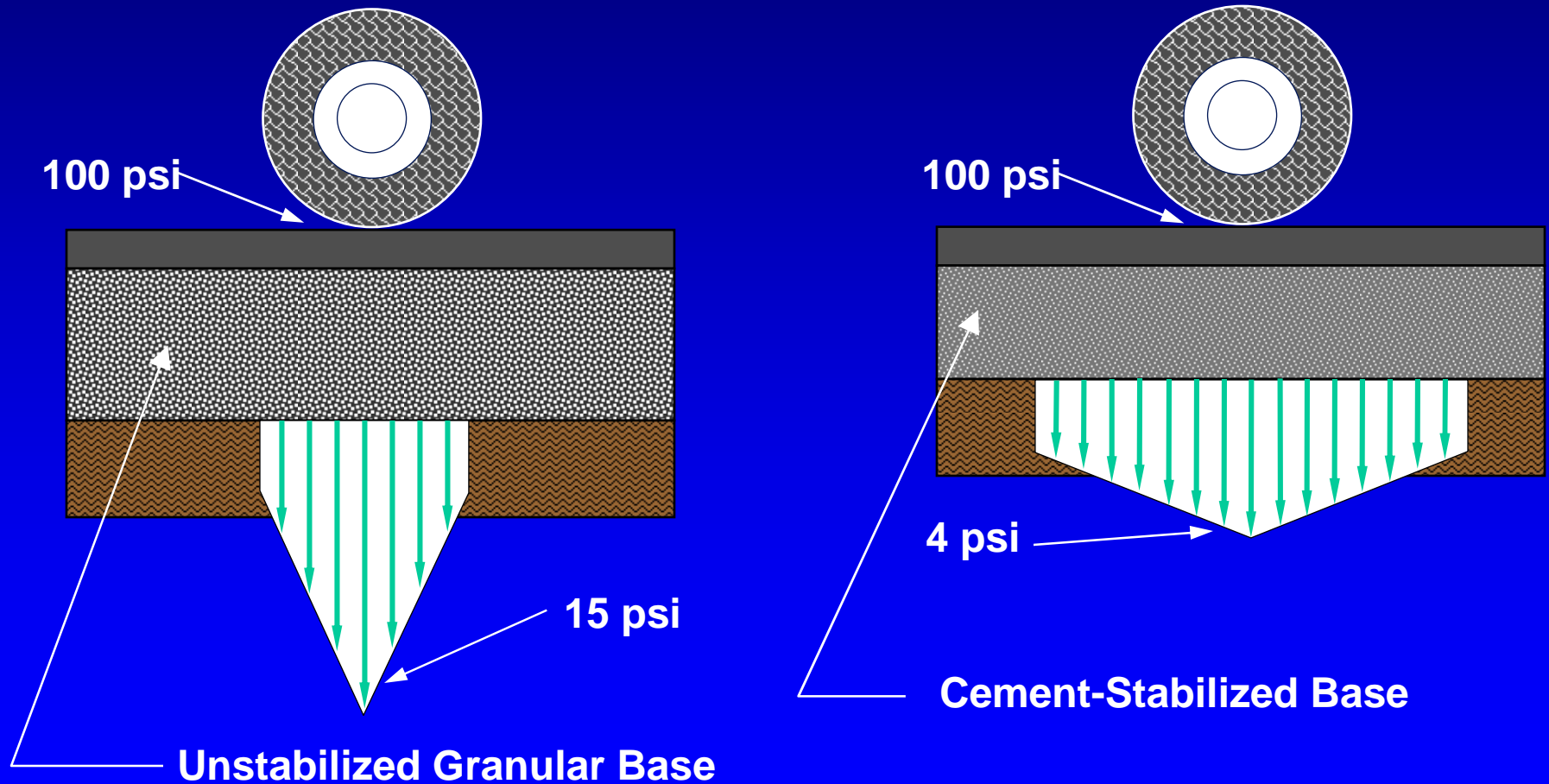
Cement Stabilization History

- 70 years of successful pavements
- Diverse geographic areas (Texas, Florida, California, Montana, Michigan, Canada)
- Wide variety of soil types
 - Gravels
 - Sands
 - Silts
 - Clays

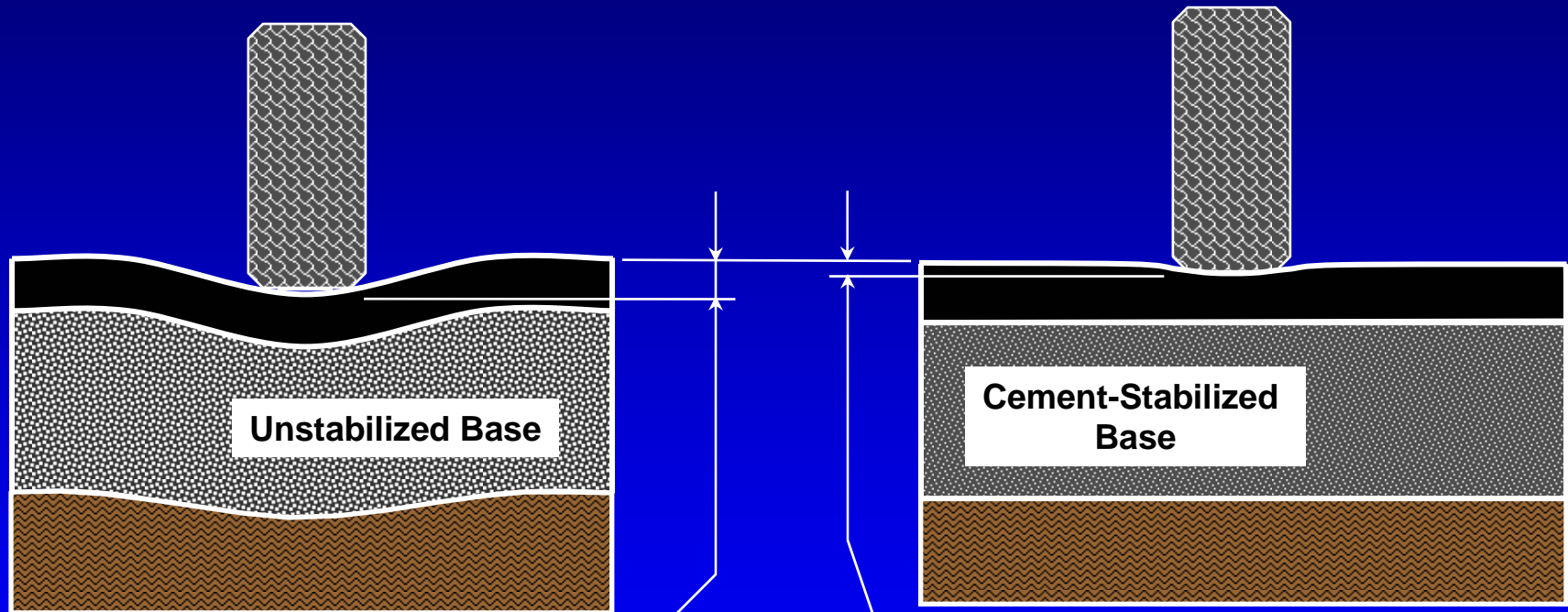
“Portland Cement is probably the closest thing we have to a universal stabilizer.”

From U.S. Army Corps of Engineers report
“Chemical Stabilization Technology for
Cold Weather”, Sept. 2002

Increased Rigidity Spreads Loads



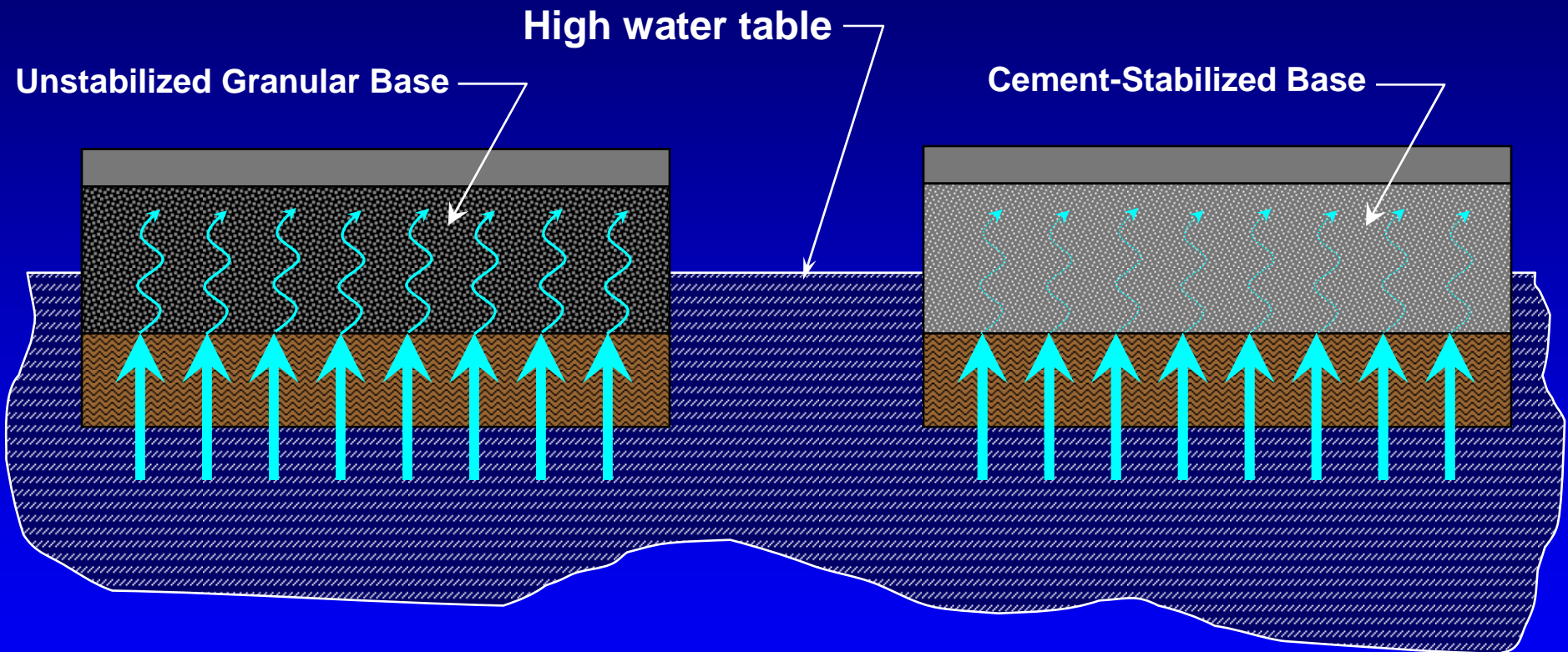
Eliminates Rutting Below Surface



Rutting can occur in surface, base and subgrade of unstabilized bases due to repeated wheel loading

Cement-stabilized bases resist consolidation and movement, thus virtually eliminating rutting in all layers but the asphalt surface.

Reduced Moisture Susceptibility



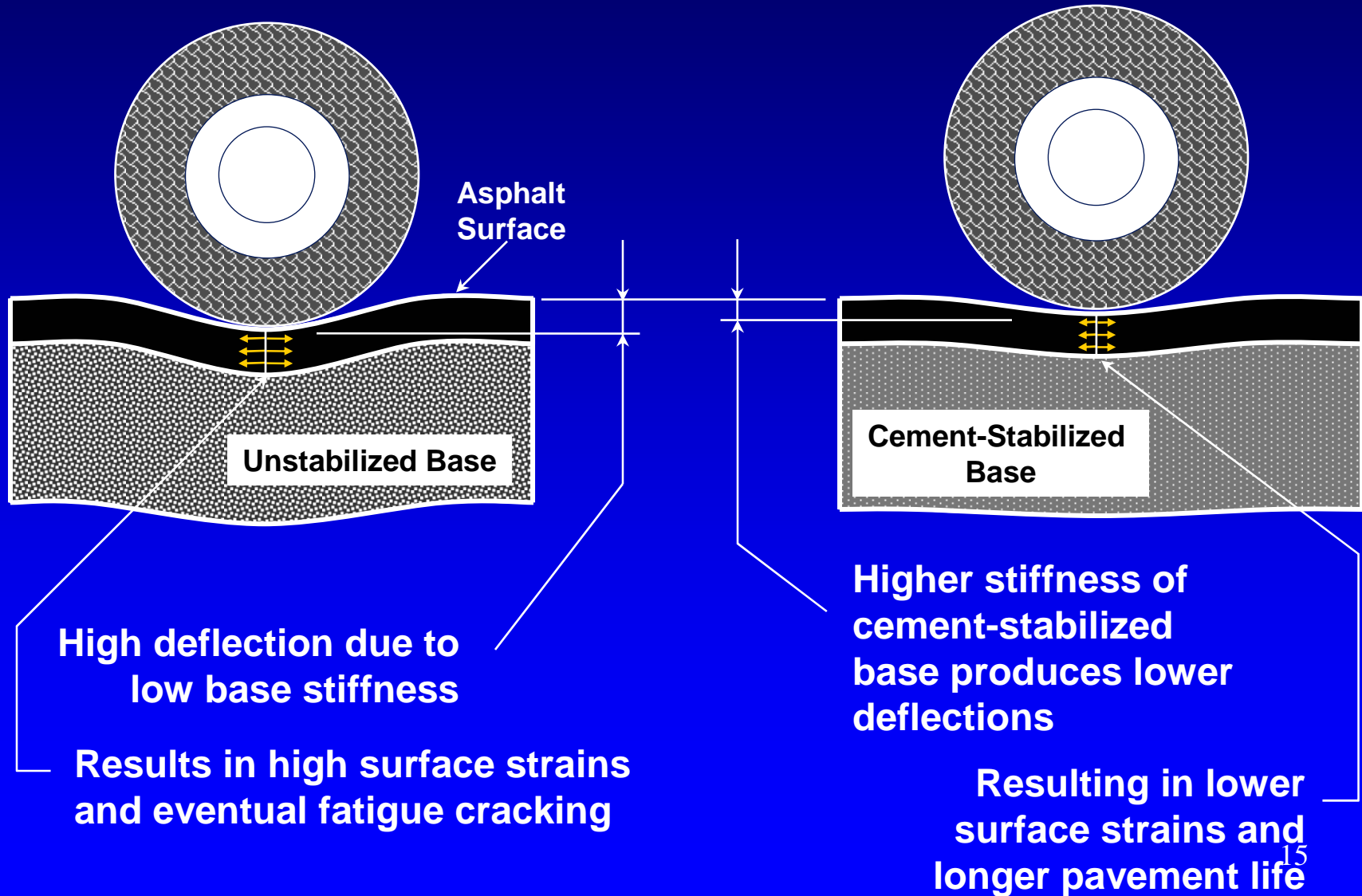
Moisture infiltrates base

- Through high water table
- Capillary action
- Causing softening, lower strength, and reduced modulus

Cement stabilization:

- Reduces permeability
- Helps keep moisture out
- Maintains high level of strength and stiffness even when saturated

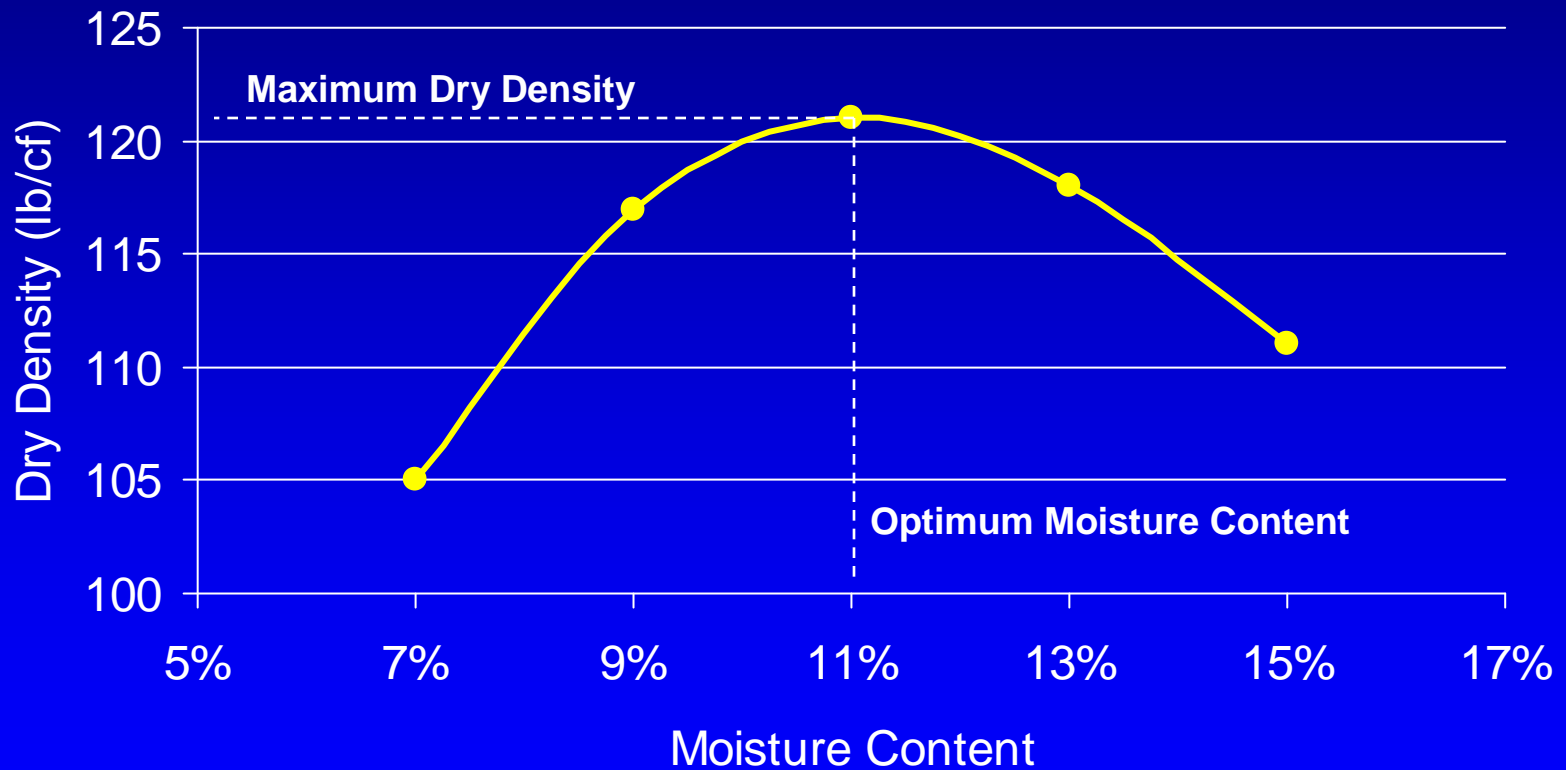
Reduced Fatigue Cracking



FDR Engineering

- Evaluation of existing materials
- Design of stabilized mix
- Thickness design
- Construction procedures
- Quality control

Moisture/Density Relationship



ASTM D558

Unconfined Compressive Strength



Typical Recycled Base and Surface Thickness

Road Function	Typical Thickness	Recommended Surface
Residential	5 in	0.75 – 1.5 in
Secondary	8 in	1.5 – 2.5 in
Highway	10 in	2 – 3+ in

Recycling Process

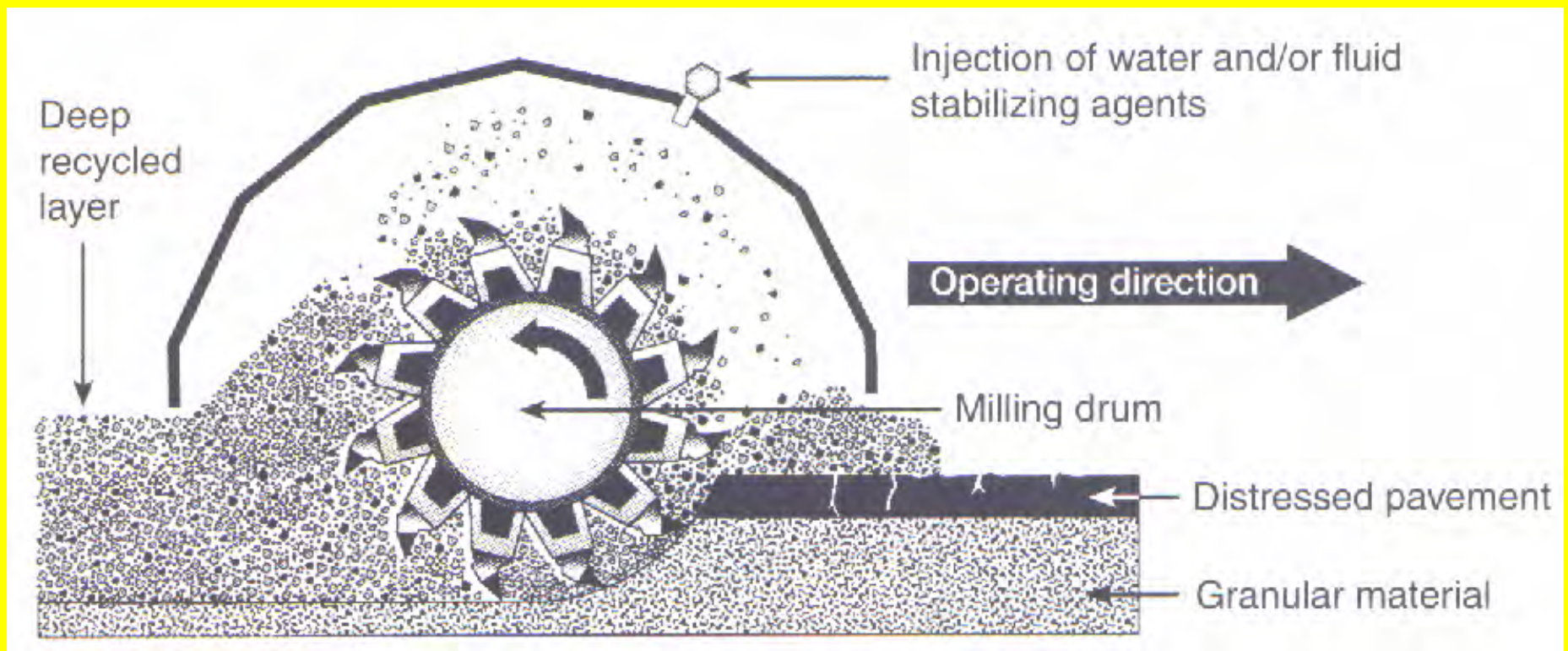
- **Simple process**
 - **Cement Spreader**
 - **Motor Grader**
 - **Pulverizer/reclaimer**
 - **Water truck**
 - **Roller/compactor**
- **Fast**

Pulverization

- Pulverize mat to appropriate gradation
- Typically 1-2 passes



Inside a Reclaimer



Aggregate Adjustment



Cement Spreading

- Cement is spread on top in measured amount



Blending and Moisture Addition

- Cement is blended into pulverized, recycled material
- Water is added to optimum moisture



Grading

- Material is graded
- Excess removed



Excellent Time for Widening!!



Example:

**Montgomery County,
NY**



Compaction

- Material is compacted
- 95% Proctor density minimum



Curing



Water

**Prime
Coat**



Surfacing

- Surface course applied
 - Chip seal
 - Asphalt
 - Concrete



Thank You!

**[www.cement.org/
pavements](http://www.cement.org/pavements)**



Portland Cement Association

USDA Forest Service



**San Dimas Technology and
Development Center**

Unpaved Road Stabilization with Chlorides



Unpaved Road Stabilization with Chlorides

- 3 Year Project, FY 2002 - 2004
- Completion Date: 9/2004
- The goal of this project is to evaluate different chloride products, applied at different application rates, using different construction methods as stabilizing agents for aggregate surfaced roads.

Project Details

- 12 Project Sites
 - ◆ Each project site has 4 to 12 test sections, 800 feet long
 - ◆ Minimum of 2" of crushed aggregate surfacing
- 39 Treated Sections
 - ◆ 4 chloride products
 - ◆ Liquid Magnesium Chloride & Calcium Chloride
 - ◆ Solid Calcium Chloride, flakes and pellets
 - ◆ 2 chloride application rates, 1.5% and 2.0%
 - ◆ 2 different types of mixing, blade and tilling
 - ◆ Chloride mixed with the top 2" of surfacing
- 40 Untreated Sections
 - ◆ 18 normally bladed and 22 untreated control sections

Project Site Locations

- Oregon 4 Projects
- Washington 1 Projects
- Idaho 4 Projects
- Montana 3 Projects

Map of Project Area



Project Construction

- Construction on all 12 projects was completed by 7/15/2003
- Construction and materials cost (cost per mile for 22 foot wide road)
 - ◆ \$8000 to \$10000 per mile

Project Construction Sequence

- Road Preparation
- Chloride Application
- Mixing
- Quality Assurance
- Compaction
- Chloride Surface Application

Road Preparation - Watering



Road Preparation - Blading and Shaping



Chloride Application - Dry Product



Chloride Application - Liquid Product



Tiller Mixing Dry Chloride



Blade Mixing Dry Chloride



Tiller Mixing - Liquid Chloride



Blade Mixing Liquid Chloride



Quality Assurance - Tiller Mixing Depth Checks



Quality Assurance - Windrow Sizing During Blade Mixing



Quality Assurance - Windrow Measurement & Mixing Consistency



Compaction - Watering



Compaction with Water Truck



Chloride Surface Application



Test Section Photos



Test Section Photos



Monitoring Items

- Performance – Dust, Loose Aggregate, Washboards, Rutting, Potholes and Speed
- Weather – Temperature, Humidity, Rainfall
- Traffic
- Testing of Aggregate & Chlorides
- Vegetation Damage, Stream Water Contamination, Migration in Soil
- Costs – Construction, Maintenance, User Costs, Aggregate Loss

Performance Rating System

- US Army Corps of Engineers “Rating Unsurfaced Roads”
- Measurement intensive process for 100 foot long segment of each test section
- Measured defects are converted to deducts, which are subtracted from 100 to get Condition Index
- Some system modifications made to improve process

Loose Aggregate & Washboards – Untreated Section



Loose Aggregate – Treated Section



Rutting

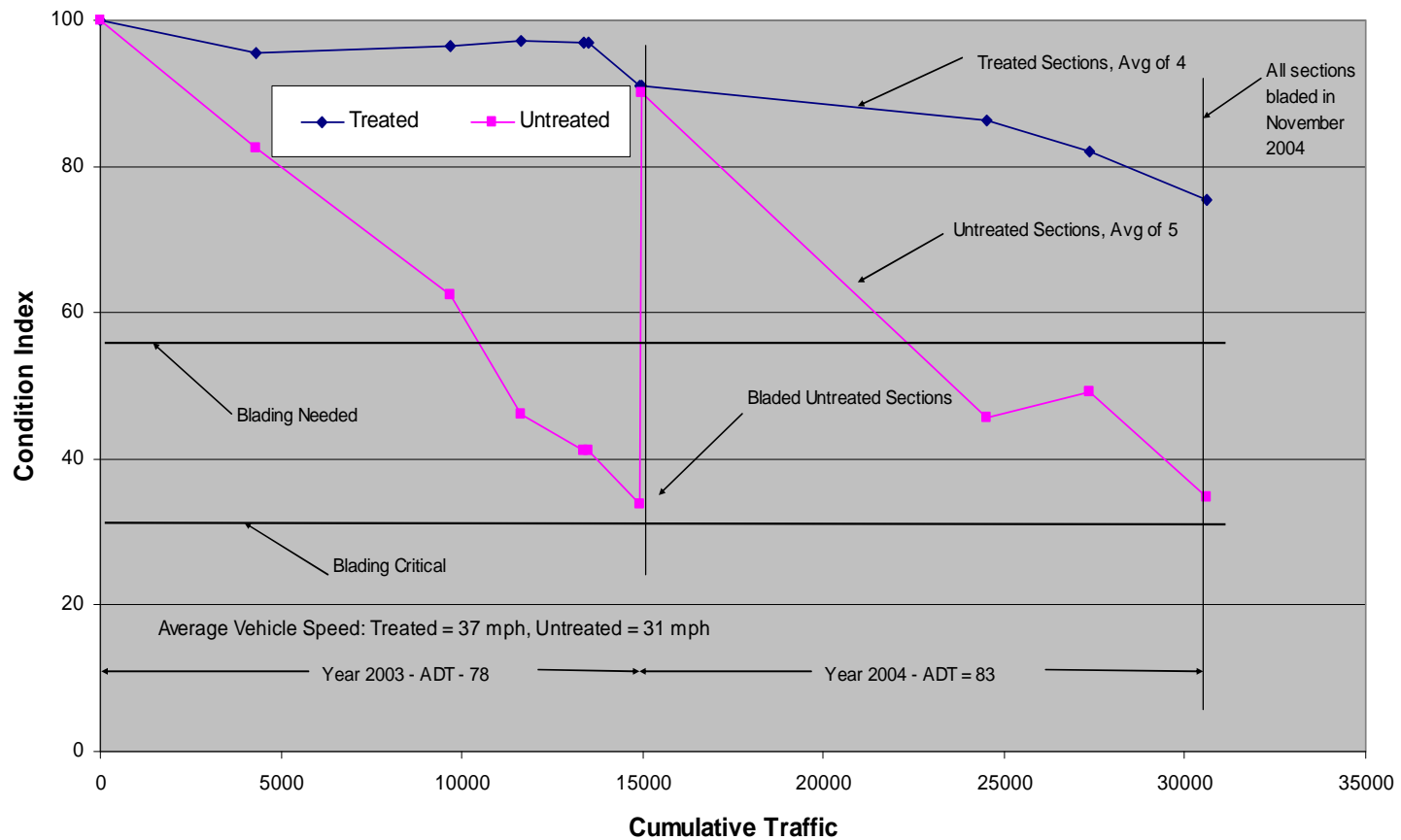


Potholes



Performance Curves

Tucannon River Road Surfacing Performance 2003-2004



General Observations

- All 40 untreated sections needed blading 95% of the time during the first season
- 13 of 39 treated sections needed blading once during the first two seasons
- Dry chloride has advantages over liquid chloride
- Tiller mixing has advantages over blade mixing
- Projects using dry chloride that are tiller mixed had the lowest construction cost

Report - Performance

- Treated segments
 - ◆ Needed blading after 22000 vehicles (About 2 to 3 years)
 - ◆ Very few defects - potholes, loose aggregate
- Untreated segments
 - ◆ Needed blading after 3000 vehicles (About 1 month)
 - ◆ Numerous defects most of the time

Report - Environmental Impacts (Before and After Samples)

- Vegetation - 200 samples on 4 projects, no significant impacts
- Migration in Soil - 96 samples on 12 projects, no significant impacts
- Stream Water Contamination - 8 composite samples on one project, no increase in chloride levels

Final Report - Costs

- ◆ Construction Costs: \$8,000 to \$10,000 per mile
 - ◆ Costs are recovered by savings during first 3 years
 - ◆ Annual spring blading with water truck and roller extends effective life to 10 years.
- ◆ Maintenance Savings: \$500/mile/year
- ◆ User Costs Savings: \$900/mile/year
- ◆ Aggregate Loss Savings: \$1900/mile/year

Report - Intangible Benefits

- Sedimentation - significantly reduced
- Aggregate Resource - conserved
- Road User Safety - improved
- Dust Health Hazard - significantly reduced
- Public Relations - improved

Michael R. Mitchell, PE

909-599-1267 ext 246

US Forest Service

San Dimas Technology and
Development Center

mrmitchell@fs.fed.us

Effective Partnering to Overcome an Interruption In the Supply of Portland Cement During Construction at Marmet Lock and Dam

**Billy D. Neeley
Toy S. Poole
Anthony A. Bombich**

**Concrete and Materials Branch
Geotechnical and Structures Laboratory**

Joan B. Stclair

U. S. Army Engineer District, Huntington



**US Army Corps
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Engineer Research and Development Center

12 October, 2004

We have a problem !



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The Problem

- ❖ A full silo of type II, HH portland cement at the Armstrong Cement facility in Cabot, PA was ruined by rising flood waters in October 2004.
- ❖ The loss occurred approximately 1 to 2 weeks before the cement was scheduled to be delivered to the Marmet construction site



The Time Crunch

- ❖ The supply of type II, HH cement remaining at the construction site would be exhausted within 2 weeks, or less
- ❖ Armstrong Cement would require approximately 4 to 5 weeks to produce and deliver another shipment of type II, HH cement
- ❖ Concrete placements would be halted within approximately 2 weeks unless a suitable alternative could be found



The Challenge

- ❖ Find an acceptable solution within less than 2 weeks that would allow concrete placements to continue uninterrupted, while maintaining the integrity and quality of the concrete construction

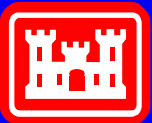


The Team

❖ **Huntington District**

❖ **Kokosing / Fru-Con**

❖ **ERDC**



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Available Options

- ❖ Use type II portland cement, without the HH restrictions, from Armstrong (proposed by Kokosing / Fru-Con; preferred by ERDC)
- ❖ Procure type II, HH portland cement from another source
- ❖ Discontinue concrete placements until a new shipment of type II, HH portland cement could be delivered from Armstrong (last resort)



The BIG Question

- ❖ Determine whether a mixture with type II portland cement, without the heat of hydration restriction, and a modest increase in fly ash content will have an acceptably low adiabatic temperature rise comparable to a similar mixture using type II, HH portland cement and a lower amount of fly ash



The Dilemma

- ❖ Ongoing placements were guide-wall cells being filled with a high-slump tremie mixture for which no temperature rise data existed
- ❖ Temperature rise data existed only on two 3-in. NMSA mass mixtures with type II HH cement
- ❖ Not enough time to measure actual temperature rise in the laboratory on any mixtures using type II cement without the HH restriction



A Multi-Pronged Approach

- ❖ **Kokosing / Fru-Con to cast 2 well-insulated and instrumented test cells of concrete, with the portland cement being the only variable**
 - **Armstrong type II, HH**
 - **Armstrong type II**
- ❖ **Kokosing / Fru-Con to review construction schedule looking for ways to**
 - **Slow demand for concrete, and**
 - **Move less critical placements forward without severely hindering overall schedule**



A Multi-Pronged Approach

- ❖ ERDC to conduct a review of literature to estimate potential temperature difference based upon heat of hydration of cement and fly ash content
- ❖ ERDC to conduct a review of available project data to estimate potential temperature difference based upon mixture proportions
- ❖ ERDC to analyze all available data and make final recommendation on mixtures



A Multi-Pronged Approach

- ❖ **Huntington to coordinate efforts between Kokosing / Fru-Con and ERDC**
- ❖ **Huntington to make final decision to use of type II portland cement, without the HH restriction, or to terminate concrete placements until type II, HH available again**



Test Cells



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Mixture 348 Used in Test Cells

- ❖ Portland cement – 70% by volume
- ❖ Fly ash – 30% by volume
- ❖ $w/(c+m) = 0.485$
- ❖ Type portland cement
 - Type II, HH
 - Type II



Test Cell



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Test Cell



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Test Cell



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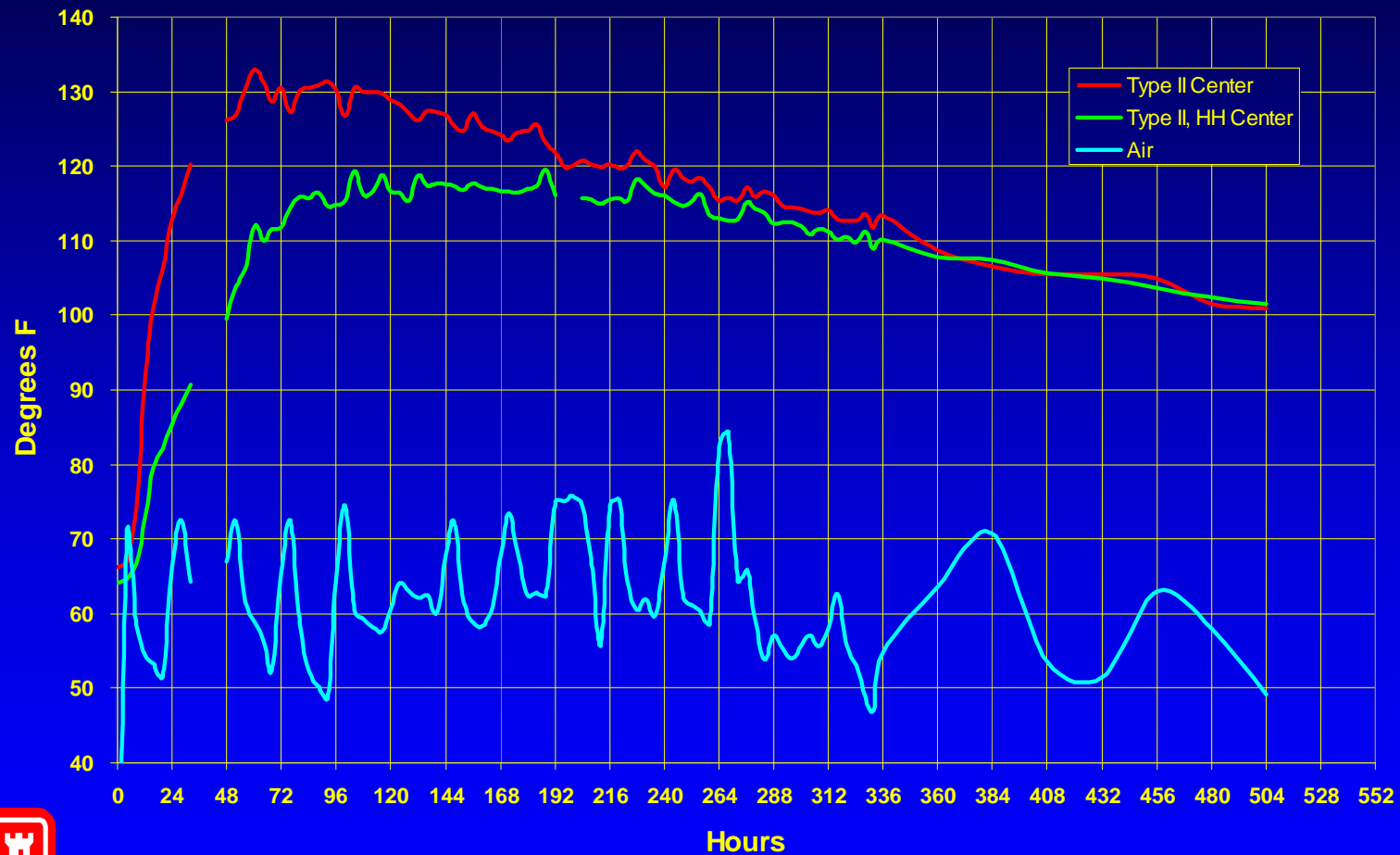
Test Cell



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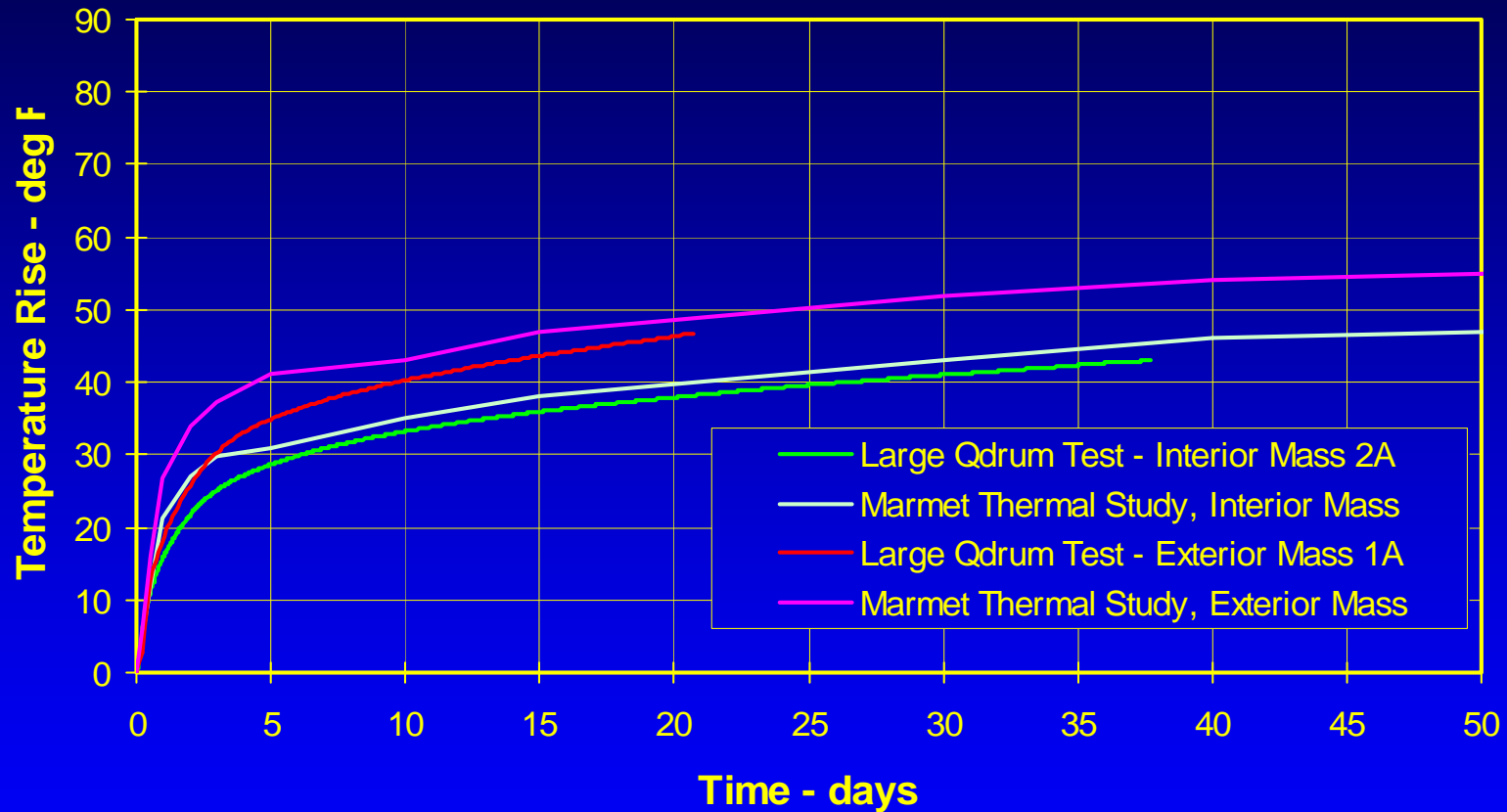
Test Cell Temperatures



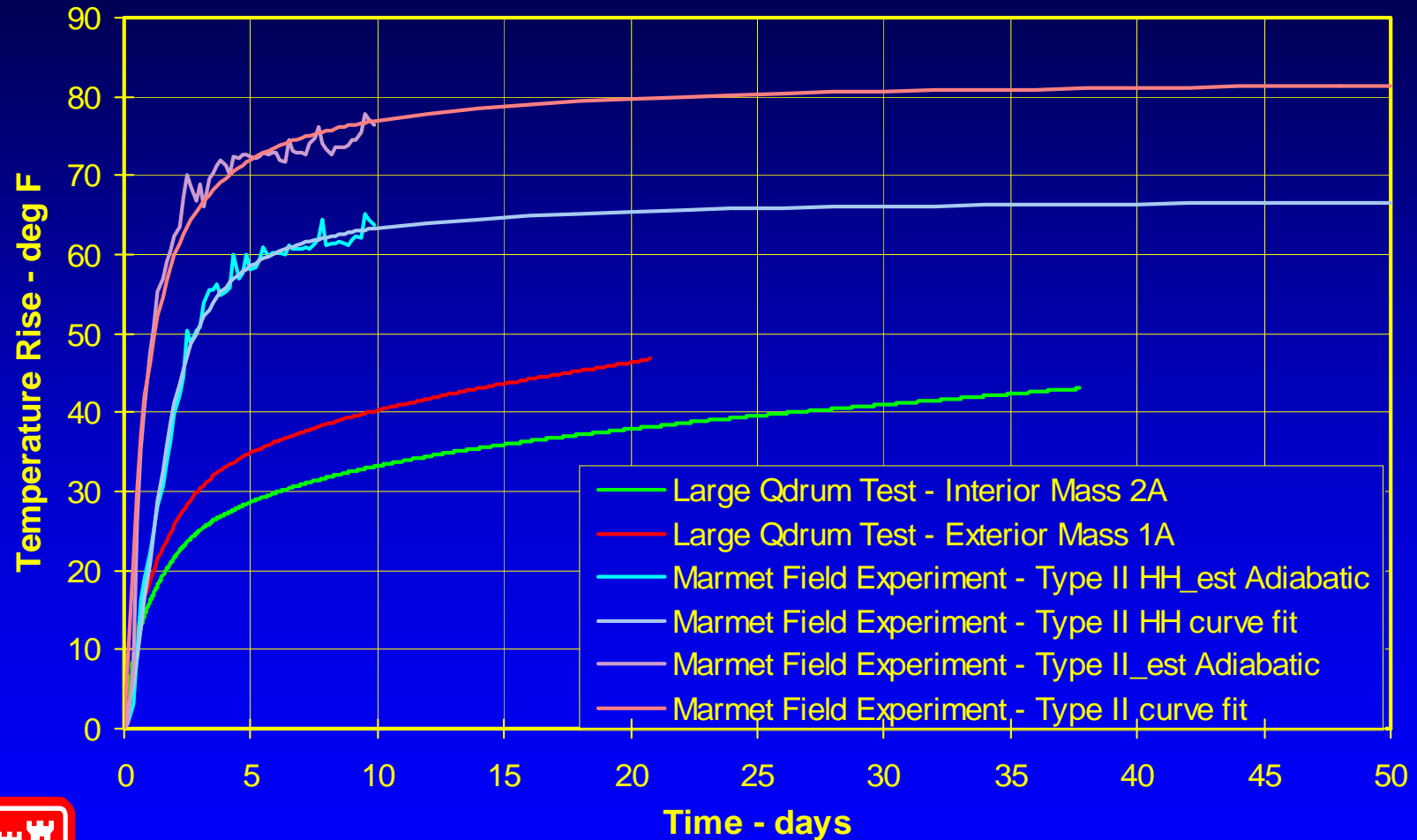
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Baseline Mass Mixtures with Type II, HH



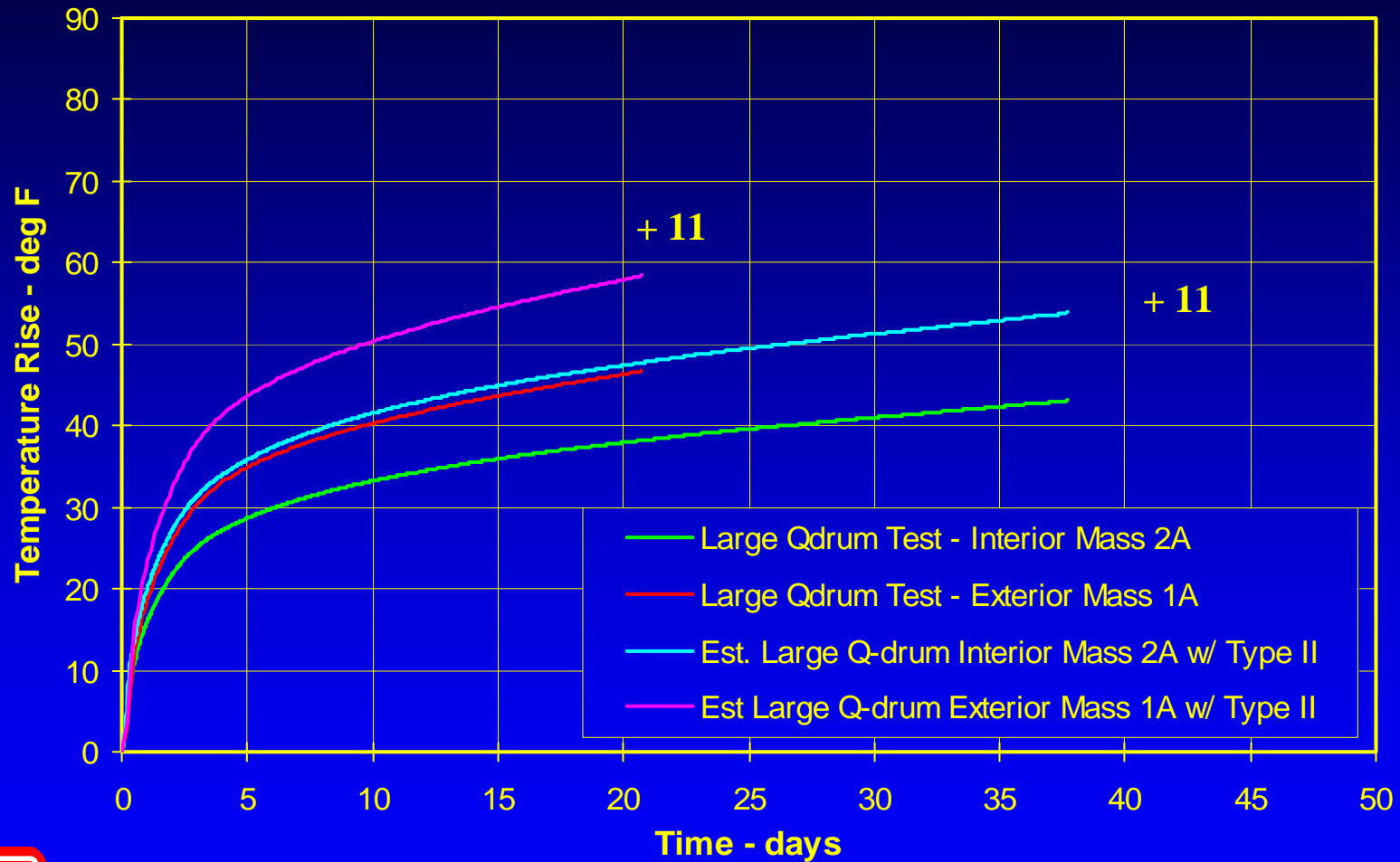
Test Cells



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Type II, HH versus Type II



Analysis of Mixture Proportions

Mix No.	w/(c+m)	Fly Ash %	Quantity per cubic yd, lb				Water	Estimated maximum temp rise	w/ LH cement	Estimated maximum temp rise	w/ MH cement	Estimated increase in temp
			PC	Total Cementitious	FA							
						w/ MH ceme						
1A	0.49	20	281	329	48	171	51		60		9	
1B	0.49	25	263	323	60	172	49		58		9	
1C	0.49	30	243	315	72	170	47		55		8	
1D	0.46	25	286	351	65	175	52		61		9	
2A	0.55	30	223	289	66	175	45		52		7	
2B	0.60	30	199	257	58	170	43		50		6	
2C	0.60	25	215	264	49	172	44		51		7	
2D	0.65	25	202	248	46	175	43		49		7	
348	0.435	30	461	596	135	286	66		81		15	
347	0.495	30	392	507	115	277	60		73		13	



Heat of Hydration Analysis

❖ Thesis

- Temperature rise = $\frac{\text{HH of cement} \times \text{cement fraction}}{\text{heat capacity of concrete}}$
- Adjust cement fraction for % fly ash
- $dT = \frac{(1.3 \text{ HH} + ((1.3 - 0.51(\% \text{ fly ash}))) \times \% \text{ cement}}{\text{heat capacity of concrete}}$



Heat of Hydration Analysis

❖ Example calculations

$$dT = \frac{(1.3 \text{ HH} + (1.3 - 0.51(\% \text{ fly ash}))) \times \% \text{ cement}}{\text{heat capacity of concrete}}$$

$$dT = \frac{((1.3)(79) + (1.3 - (0.51)(30))) \times 0.1231}{0.24} = 45^\circ \text{ C} \\ = 82^\circ \text{ F}$$

$$dT = \frac{((1.3)(68) + (1.3 - (0.51)(30))) \times 0.1231}{0.24} = 38^\circ \text{ C} \\ = 69^\circ \text{ F}$$



Heat of Hydration Analysis

❖ Example calculations

$$dT = \frac{(1.3 \text{ HH} + (1.3 - 0.51(\% \text{ fly ash}))) \times \% \text{ cement}}{\text{heat capacity of concrete}}$$

$$dT = \frac{((1.3)(79) + (1.3 - (0.51)(30))) \times 0.1231}{0.24} = 45^\circ \text{ C} = 82^\circ \text{ F}$$

$$dT = \frac{((1.3)(79) + (1.3 - (0.51)(45))) \times 0.1231}{0.24} = 42^\circ \text{ C} \quad 38^\circ \text{ C} = 75^\circ \text{ F} \quad 69^\circ \text{ F}$$



The Conclusion

- ❖ Mixtures comprised of type II portland cement, without the HH restriction, combined with a modest increase in fly ash to 40 to 45 % will result in a mixture that has a significantly higher temperature rise than the mixture it would be replacing
- ❖ A significantly higher fly ash content will be required to adequately reduce the temperature rise
- ❖ The required fly ash content would be higher than anything the Corps had a ready history of using



What's the Bottom Line?

- ❖ **The required fly ash content appeared to be approximately 60%, by volume**
 - Would Huntington District be willing to use mixtures with 60% fly ash?
 - Would Kokosing / Fru-Con be willing to use mixtures with 60% fly ash?



Brave Souls

(or was it desperation)

- ❖ Huntington District said YES
- ❖ Kokosing / Fruj-Con said YES
- ❖ ERDC provided a tentative substitute for use in the guide-wall cells



The Result

- ❖ Starting on 6 Nov 2004, the mixture with 60% fly ash was used to fill 2-1/2 cells

	<u>7-day</u>	<u>28-day</u>	<u>90-day</u>
30% ash + HH	1,300	4,000	5,500
60% ash + reg II	1,300	3,000	4,800

- ❖ Fewer cracks noted on these 2 cells than on previous cells cast with the original mixture
- ❖ Armstrong Cement delivered a new shipment of HH portland cement on 13 Nov 04



Summary

- ❖ A bizarre problem developed out of the blue that was completely out of everyone's control
- ❖ Effective and cooperative partnering was key to finding a workable solution in a very short period of time
- ❖ Even though a degree of estimating was involved, the solution was based upon sound engineering principles



Summary

❖ The interim solution was successful

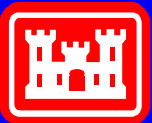
❖ You can do it, ERDC can help!



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Questions?



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US Army Airfield Pavement Assessment Program

Geotechnical and Structures Laboratory
Vicksburg, MS

Haley Parsons

Lulu Edwards

Eileen Velez-Vega

Chad Gartrell



US Army Corps of Engineers

Background

- Initiated in May 1982 by the Department of the Army
- Requested by FORSCOM, TRADOC, and AMC
- Army Airfields (AAFs) last evaluated in the 1960s
- Pavements designed for WWII and Korean War era aircraft
- Now required to support heavier and larger aircraft



1941-1993 AAF Mission Aircraft



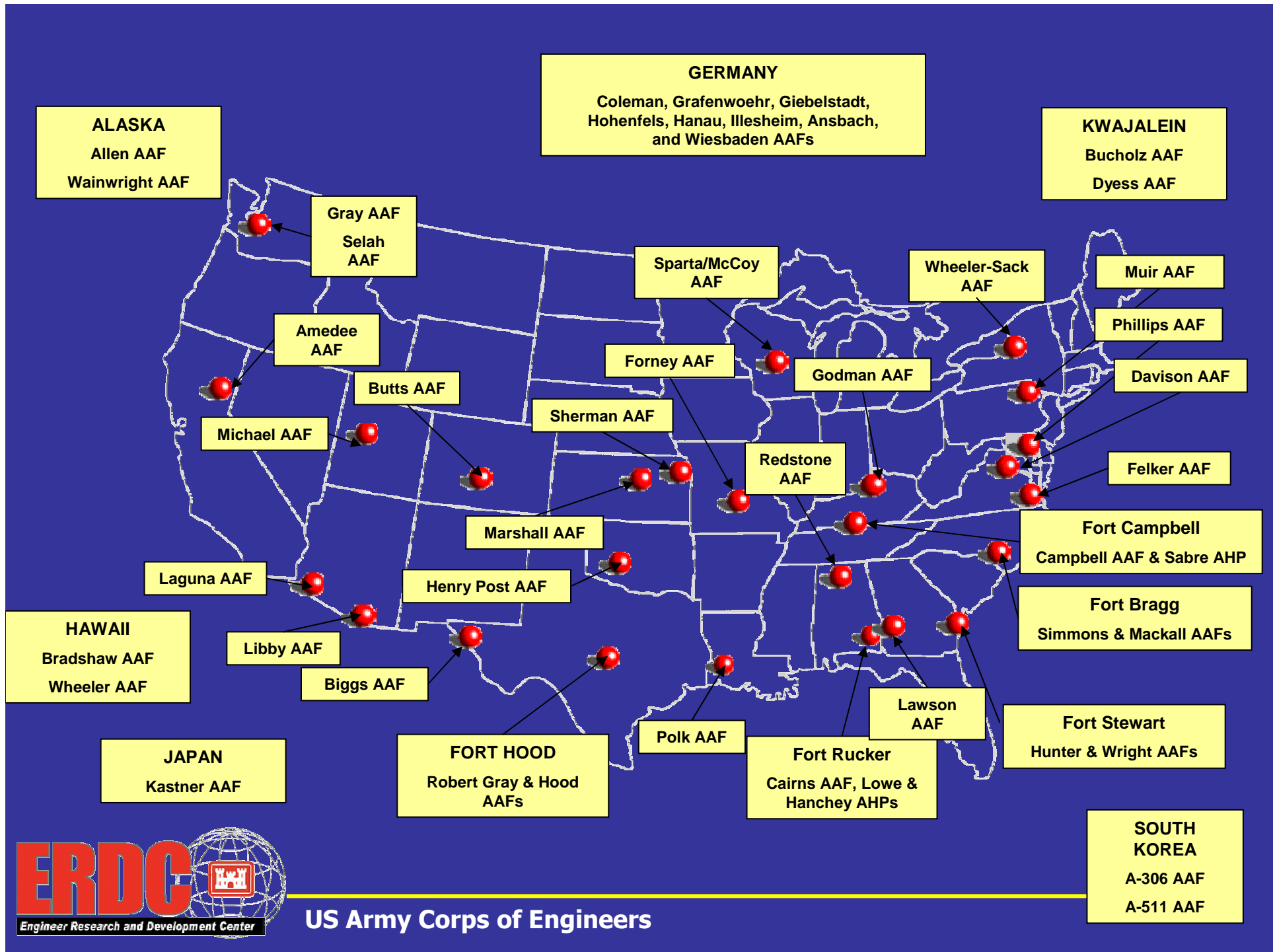
Significance

- Determines the overall mission readiness of the AAFs in support of the Army's force projection mission
- Provides technical data required to quantify airfield pavement maintenance, construction, and repair needs
- Data assists in optimal use of available funding for maintenance and repair (M&R)
- Provides information for establishing work plans necessary to reach and maintain AR 420-72 facility condition requirements
- Provides data for runway-bearing strengths

Why ERDC?

- Leadership in pavement design, evaluation, and research
- Expertise
- Military and security issues
- Database expansion and research validation
- Consistency
- Equipment
 - Dynatest heavy weight deflectometer (HWD)
 - 2 Dynatest falling weight deflectometers (FWDs)
 - dynamic cone penetrometer (DCP)
- State of the art equipment implementation
 - ground-penetrating radar (GPR)
 - portable seismic pavement analyzer (PSPA)





Inspection Intervals

- Critical Category I airfields
 - structural evaluation including nondestructive testing (NDT) every 5 years
 - pavement condition survey to determine the pavement condition index (PCI) every 5 years
- Category I airfields and instrumented heliports
 - structural evaluation including NDT every 8 years
 - pavement condition survey to determine the PCI every 4 years

Objectives

- Structural evaluation
 - determines allowable aircraft loads and design traffic
 - ◆ FWD/HWD
 - ◆ DCP
- Visual evaluation
 - pavement condition survey
 - identify M&R
- Test new technologies
 - PSPA
 - GPR

FWD/HWD

- Trailer mounted, nondestructive, impact load device
- Dynamic force applied to the pavement
 - drop height of 0-15.7 in
 - 0-50,000 lbs
 - 25-30 ms duration
- Applied force and pavement deflections are measured



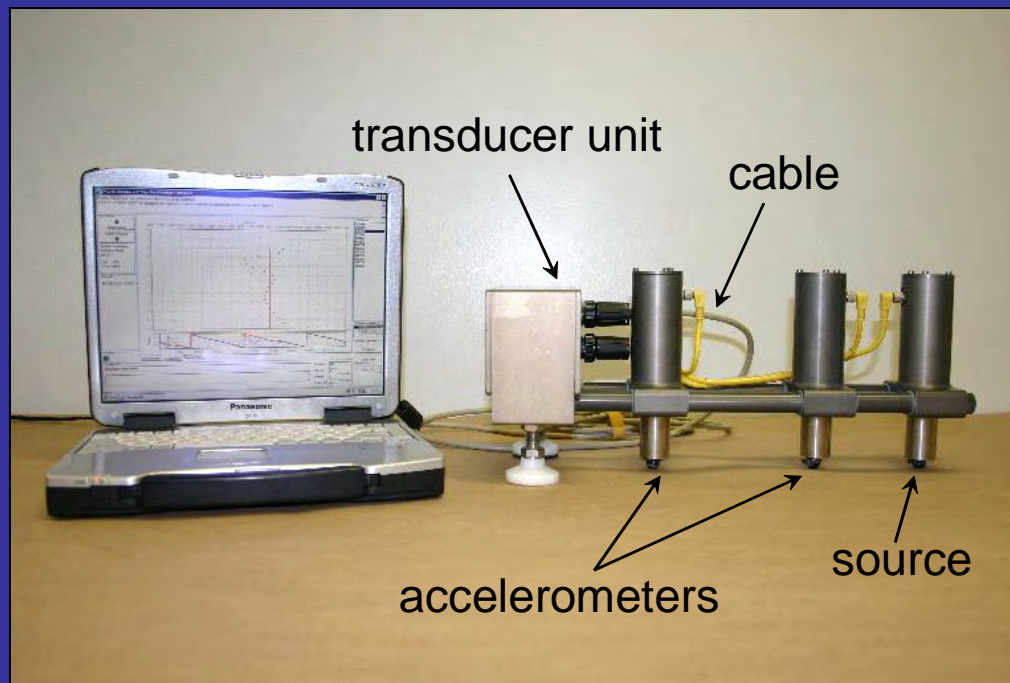
DCP

- Determines strength (CBR) of underlying soil layers
- Thickness is delineated from changes in strength
- 4 main components
 - cone, rod, anvil, hammer
- Procedure:
 - 1-in drilled hole
 - drop hammer until penetration depth is 20-30 mm
 - record number of blows and depth
 - penetration/mm is correlated to CBR



PSPA

- Measures seismic modulus of concrete pavements
- Quick, simple, nondestructive
- Measurements taken from near surface pavements



GPR

- GPR is used to non-invasively determine thickness of pavements
- Two radar antennas are usually used
 - 1 GHz – penetrates pavements up to 3 ft
 - 500 MHz – penetrates pavements up to 6 ft
- Depth of penetration is dependent on the material type and the dielectric constants



Pavement Condition Survey

- Visual inspection to determine present surface condition
 - types of distress
 - severity of distress
 - quantity of distress
- Airfield broken into features and sample units
- Estimated quantities and severity of distresses are used to compute the PCI for each feature



Micro PAVER

- Developed by USACE, Champaign, IL
- Aids pavement managers in:
 - developing and organizing the pavement inventory
 - assessing the current conditions of pavements
 - developing models to predict future conditions
 - reporting on past and future pavement performance
 - developing scenarios for pavement M&R based on budget or condition requirements



NDT Analysis

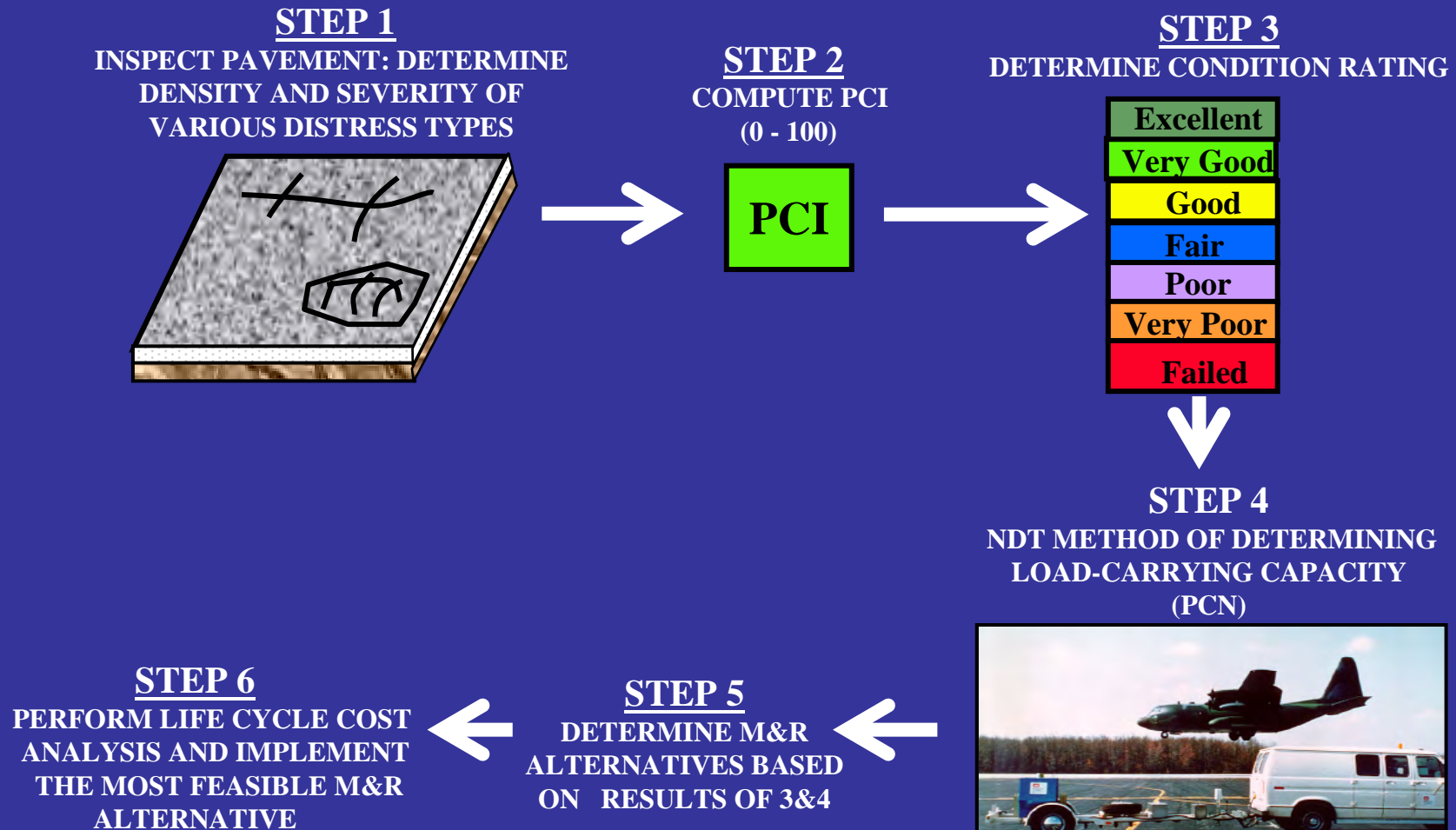
- Pre-evaluation
 - climatological data
 - traffic data (critical aircraft and maximum number of passes)
- Load-carrying capacity
 - strength of the pavement
 - gross weight of the aircraft
 - number of applications of the load
- ACN/PCN method is used to report pavement load-carrying capacity
 - ACN – structural effect of an aircraft (single wheel load)
 - PCN – load-carrying capacity in terms (single wheel load)
 - ACN/PCN ratio
 - ◆ should be < 1
 - ◆ pavement life is greater than the design life

PCASE

- Developed by USACE, Vicksburg, MS
- Aids in the design and evaluation of transportation systems
- Some capabilities:
 - generate ACN curves for any vehicle
 - analyze DCP data with DCP module
 - generate a design curve for any aircraft
 - determine the load-carrying capacity for any airfield using modulus values
 - backcalculate the modulus using the FWD/HWD data
 - percent-life curves can tell how much damage an aircraft will do to an airfield
 - use the NDT module to analyze deflection data



Determination of M&R Recommendations



Northwest (NWRO)

Claud Reindl

309-782-8264

Claud.reindl@us.army.mil

Northeast (NERO)

Joe Fuller

757-788-4405

Joe.s.fuller@us.army.mil

Southwest (SWRO)

Ali Achmar

210-295-2177

Ali.achmar@samhouston.army.mil

(ACSIM)

Mike Dean

703-601-0703

Mike.dean@us.army.mil

Southeast (SERO)

Stacy Holcomb

404-464-4207

Stacy.holcomb@forscom.army.mil

Pacific Area (PARO)

William Wong

808-438-1162

William.wong@shafter.army.mil

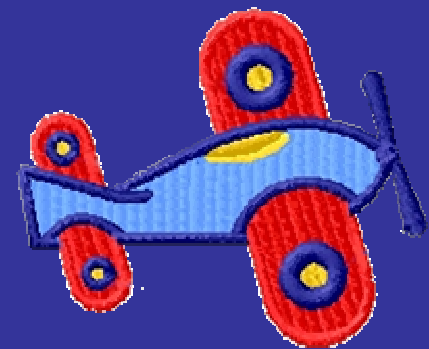
Overseas: **Europe**
Korea



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Airfield Evaluation Summary

- Review previous reports
- Brief installation personnel
- Get necessary data
- Drive over and identify overall visual condition
- Mark features and sample units
- Survey, NDT
- Review PCI sheets and NDT data
- Enter all information into PAVER, PCASE
- Analyze data
- Generate report



US Army Corps of Engineers

Haley Parsons

601-634-3602

US Army Corps of Engineers

Engineer Research and Development Center (ERDC)

Vicksburg, MS

Haley.M.Parsons@erdc.usace.army.mil

Contributing authors:

Lulu Edwards (Lulu.Edwards@erdc.usace.army.mil)

Eileen Velez-Vega (Eileen.M.Velez-Vega@erdc.usace.army.mil)

Chad Gartrell (Chad.A.Gartrell@erdc.usace.army.mil)



US Army Corps of Engineers

Curing Practices for Modern Concrete Production

Toy Poole

U.S. Army Corps of Engineers

August 2005



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Problems with Curing?



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Curing Practices - Need for Revisions??

- Review major points of current practice
- Discuss effects of newer concrete practice



Purpose of Curing

- Conserve water
- Maintain favorable temperatures



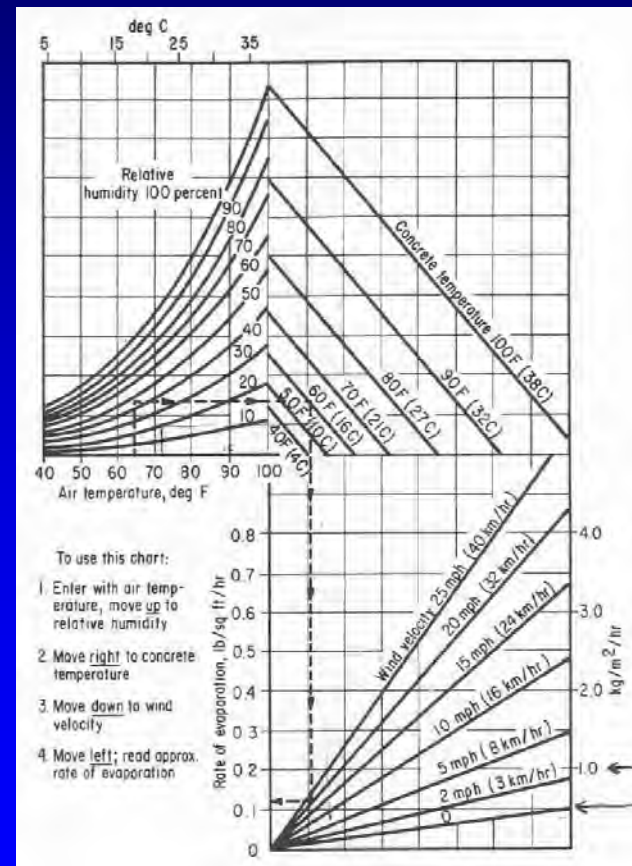
Current Practice

- Protect fresh concrete
- Apply final curing
 - After finishing
 - After sheen gone
- Duration of Curing
- Curing materials specs



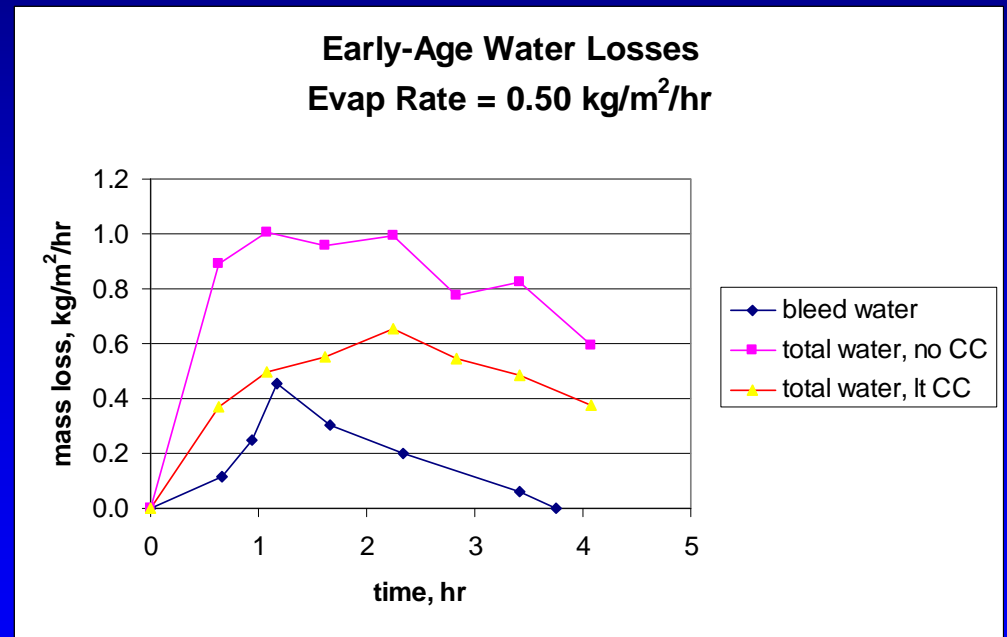
Protect Fresh Concrete

- Critical evap rate
 - 0.5, 1.0 kg/m²/h
- Based on “old time” bleeding rates



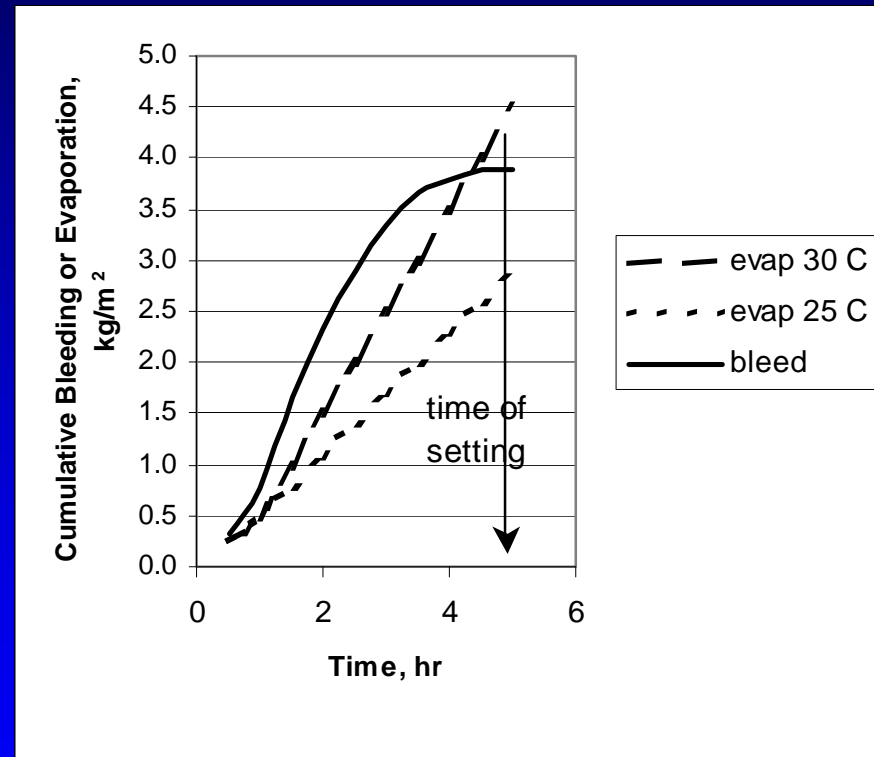
Low w/c Concrete

- Low w/c concretes
 - Evap rates <0.5 $\text{kg/m}^2/\text{h}$
- Action: More care to reduce drying
- Cool concrete
- Evap reducers
- Misting



Action

- Action: reduce evaporation
- Cool concrete



Current Practice

- Protect fresh concrete
- **Apply final curing**
 - After finishing
 - After sheen gone
- Duration of Curing
- Curing materials specs



Apply Final Finishing

- After finishing
- After sheen disappears



Problem

- Pavements
 - Little bleed
 - Finishing ~ placing
- Curing compounds
 - Applied soon after placing
 - May not perform



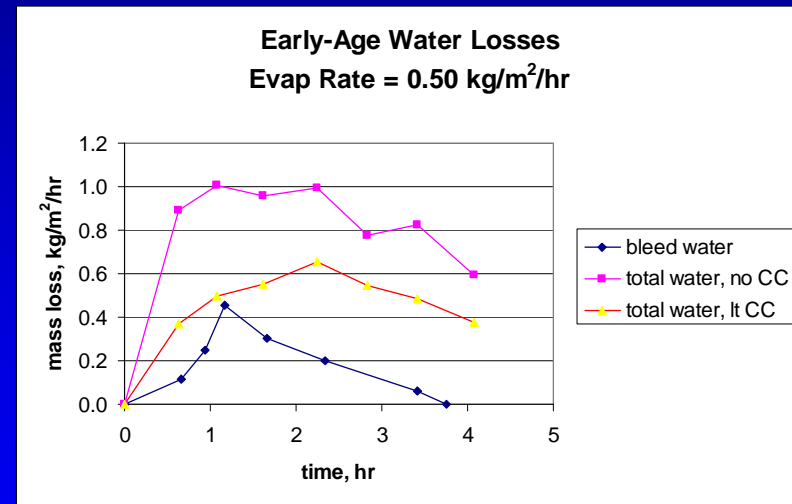
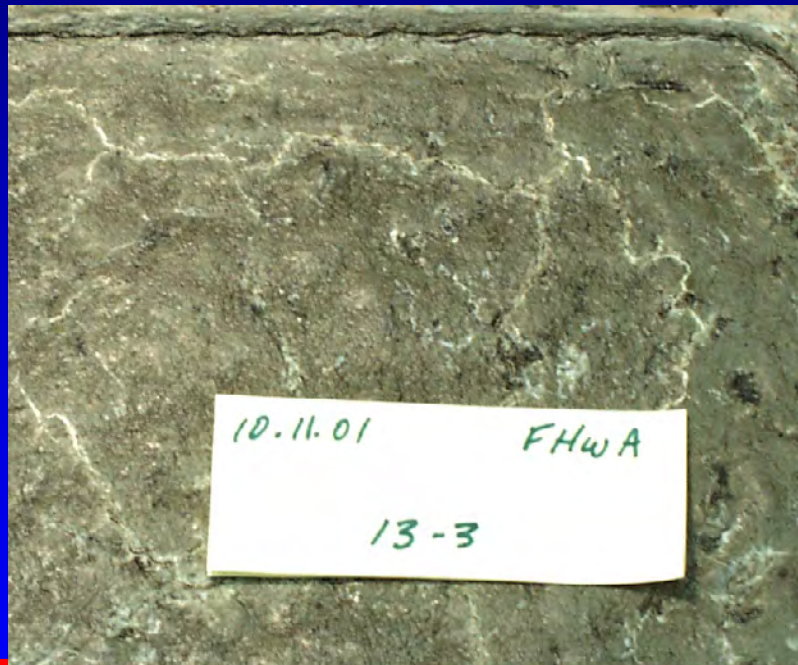
Uniformity of Application



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Early Application of Curing Compound



Early Application of Water, Mats

- If before TOS
 - Erosion
 - Marring



Resolution

- Delay application???!!!
- Live with consequences



Current Practice

- Protect fresh concrete
- Apply final curing
 - After finishing
 - After sheen gone
- **Duration of Curing**
- Curing materials specs



Duration of Curing

- Corps of Engineers - prescriptive
 - Based on cement type
 - Presence of pozzolan
- State DoT's – prescriptive
 - Based on time – 3 – 10 days
- ACI – mixed spec
 - Time
 - % f'c



Emerging Technologies

- Maturity
 - ASTM C 1074 based
- NDT
 - ultrasonic



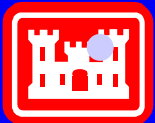
Current Practice

- Protect fresh concrete
- Apply final curing
 - After finishing
 - After sheen gone
- Duration of Curing
- Curing materials specs



Curing Materials – Curing Compounds

- Water Retention
 - CE: 0.31 kg/m² @ 7 days
 - Old Bu Rec: 0.86 kg/m² @ 7 days
 - ASTM:
 - C 309: 0.55 kg/m² @ 3 days
 - C 1315: 0.40 kg/m² @ 3 days
 - State DoT's: <0.3 kg/m² @ 3 days



Drying Time – 4 hours

Water Retention (?, Loss?) Requirements

- True value??
 - Some early work – 0.7 kg/m²
 - Other work - 1.0 kg/m² in several days
- Major problems with testing
 - Often not done
 - Precision of TM (C 156)
 - d2s = 0.20 kg/m²



Drying Time Problems Low VOC Materials



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Evaporation Reducers

- No Specs
- No TM's
- ASTM C 9.22



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The End!



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Concrete Damage at Carters Dam

January 2005



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Reregulation Dam – Downstream View



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Reregulation Dam – Downstream View



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Downstream D-2, Lifts 23, 24?



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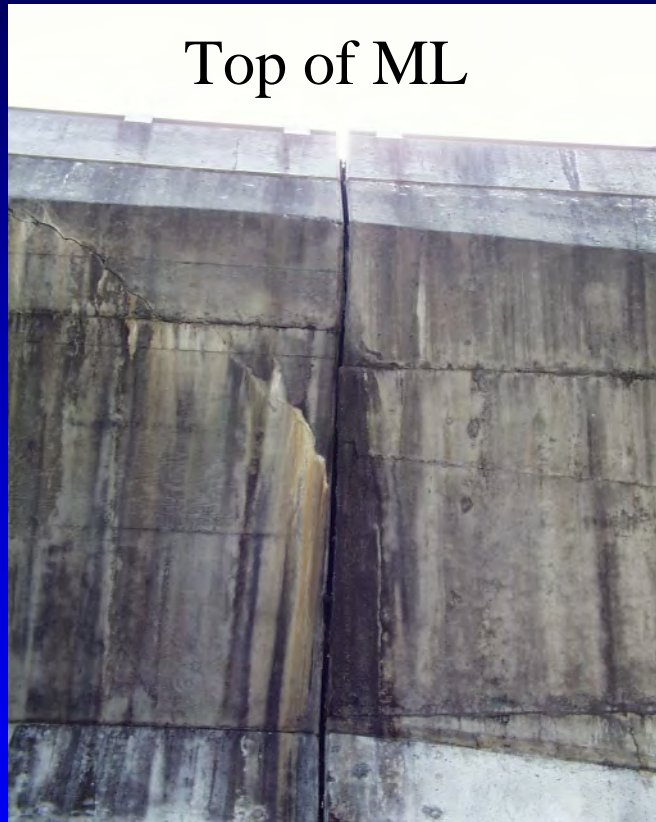
Upstream D-2



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Downstream Joint - D-8 and D-9



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Upstream D-9



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Upstream D-1



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Trunnion Block, ML D-8



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Emergency Spillway



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Shaft in ML 11



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Aggregate

- Single source – Dalton Quarry
- At least 3 distinct types in the 1.5 and 3 inch sizes
 - One suspected of ACR
 - Problems with ACR rock: Sep – Nov 71

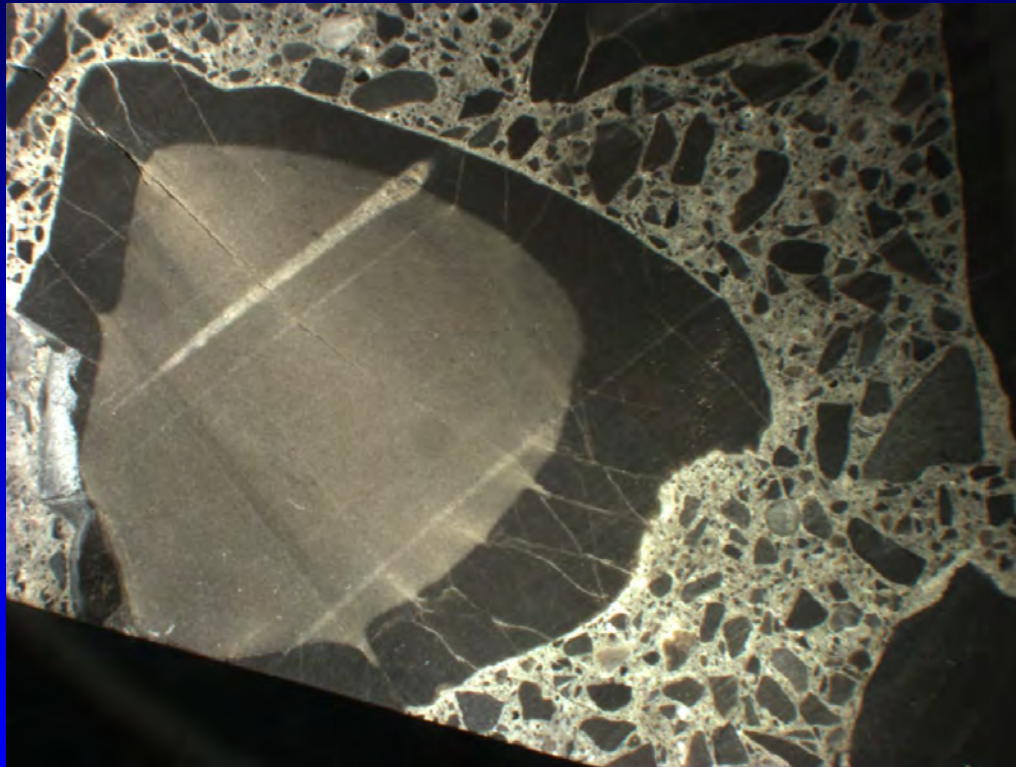


Portland Cement

- 3 sources
 - All low alkali
 - 0.45 – 0.55 $\text{Na}_2\text{O}_{\text{eq}}$
- Pozzolan
 - Probably not

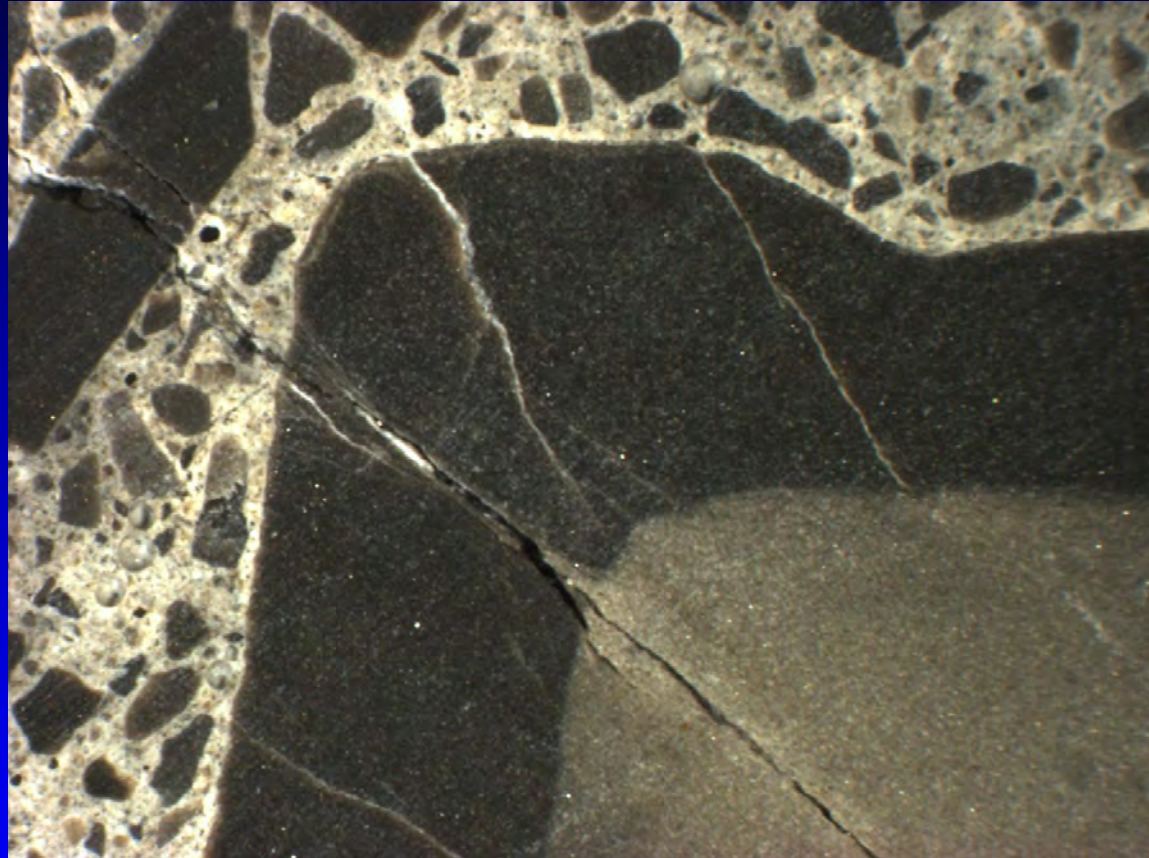


Reactive Pieces



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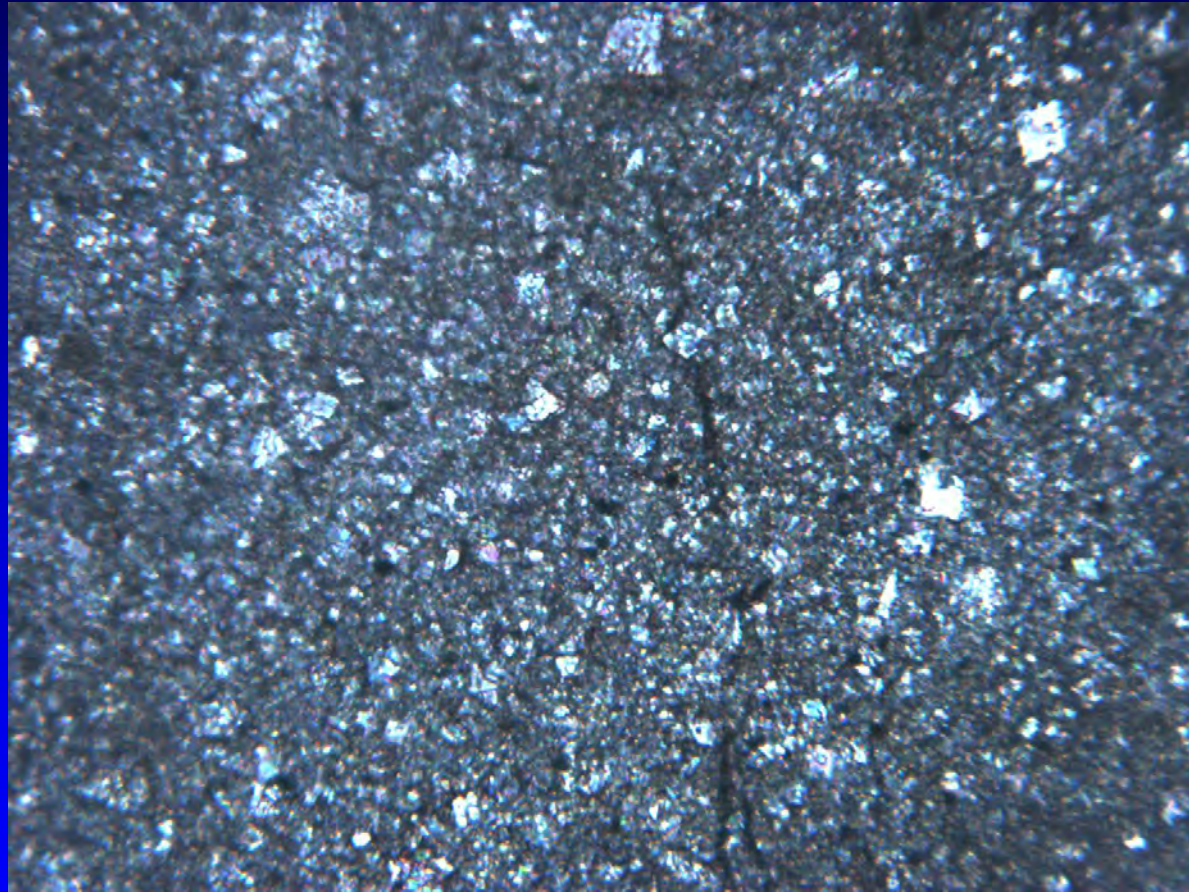
Engineer Research and Development Center



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Engineer Research and Development Center

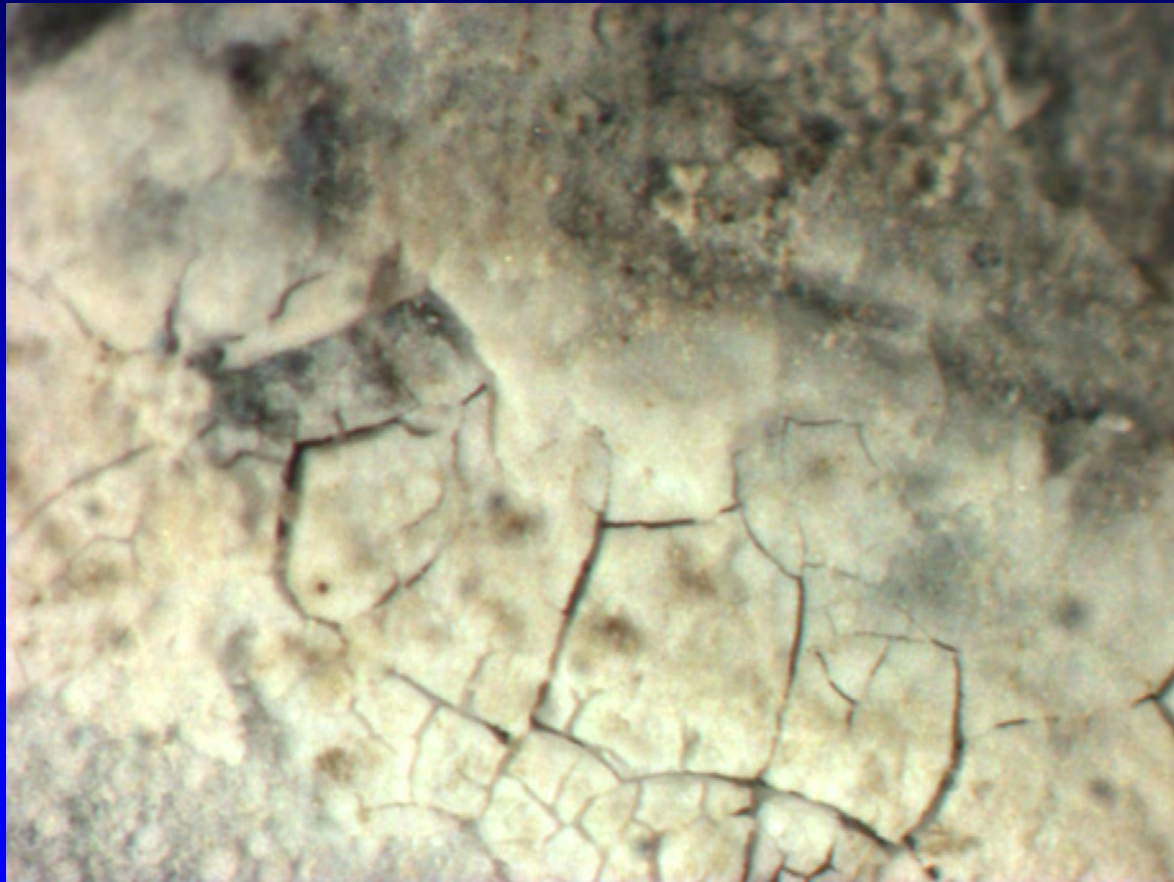
Rhombs



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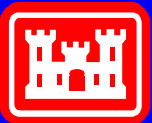
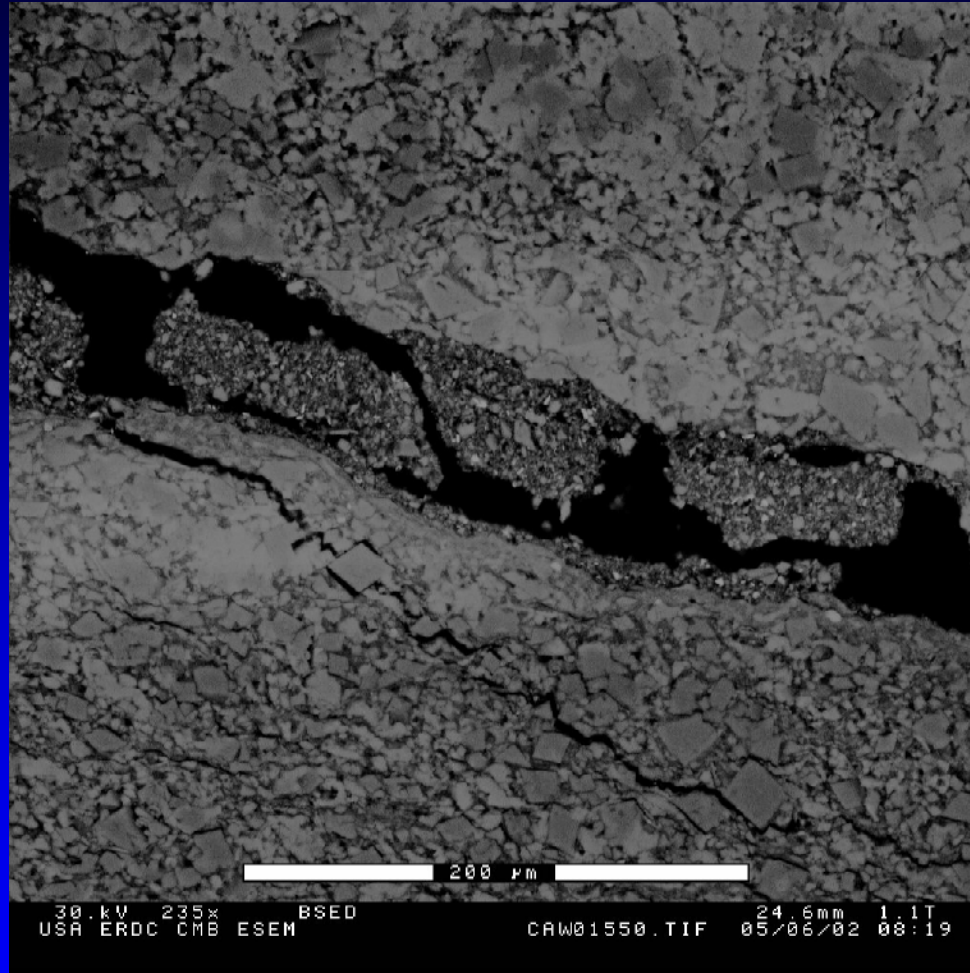
Engineer Research and Development Center

Reaction Products



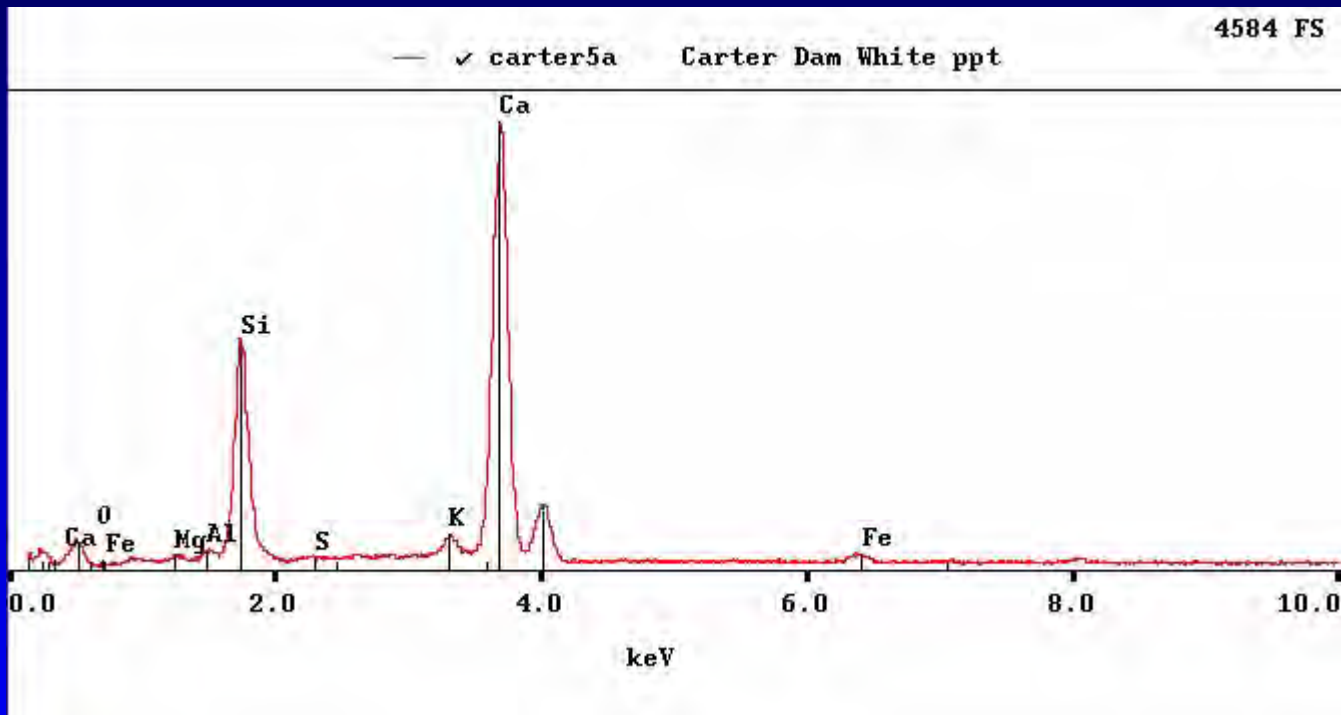
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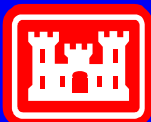
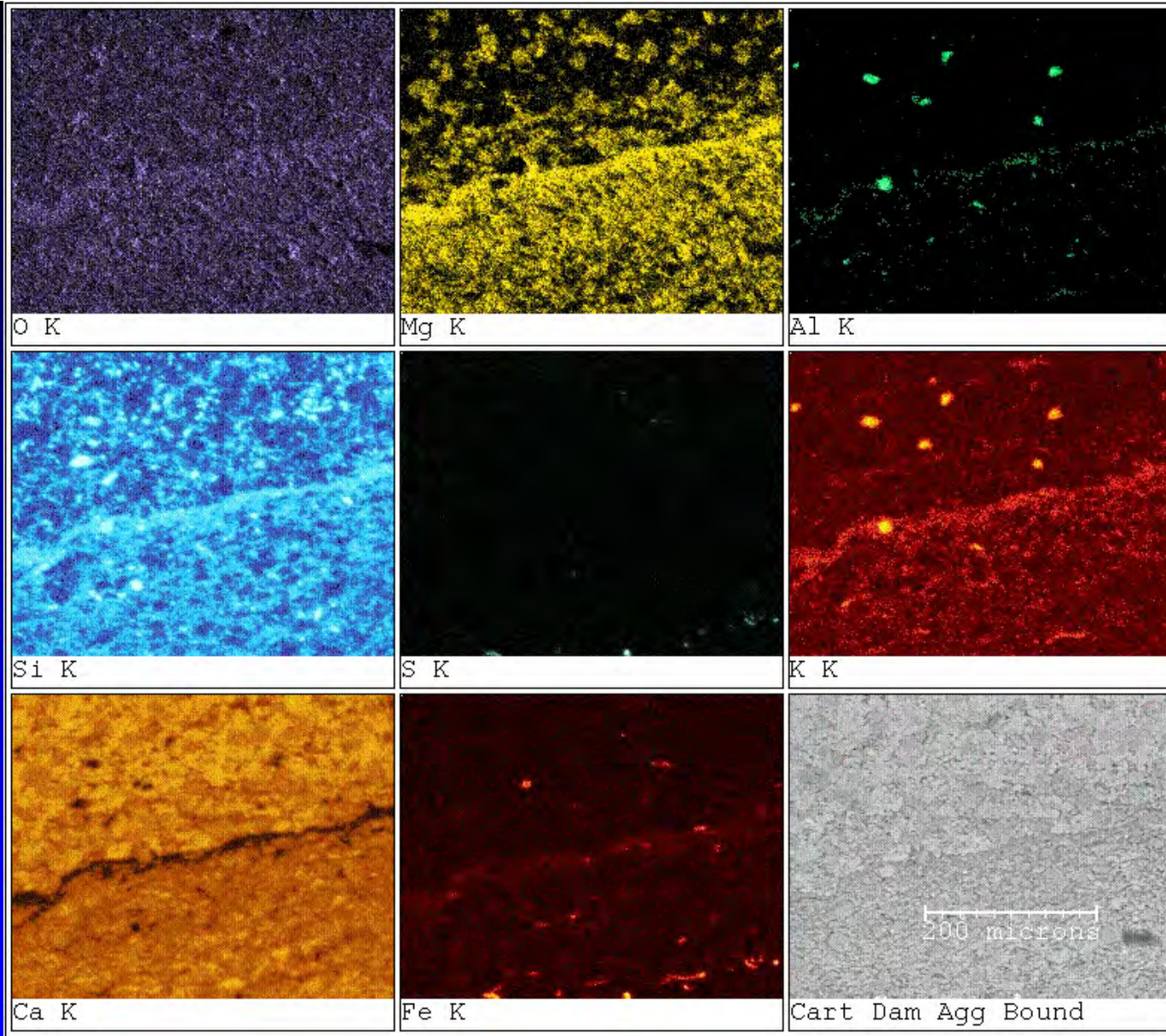
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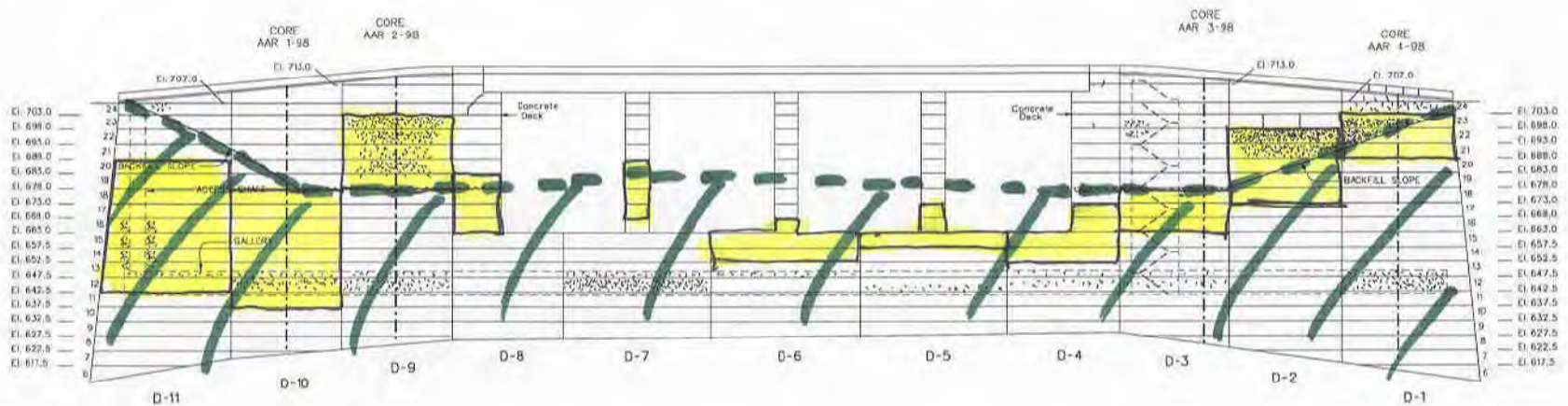


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Expected Damage – Upstream Face



CARTERS REREGULATION DAM - UPSTREAM ELEVATION -

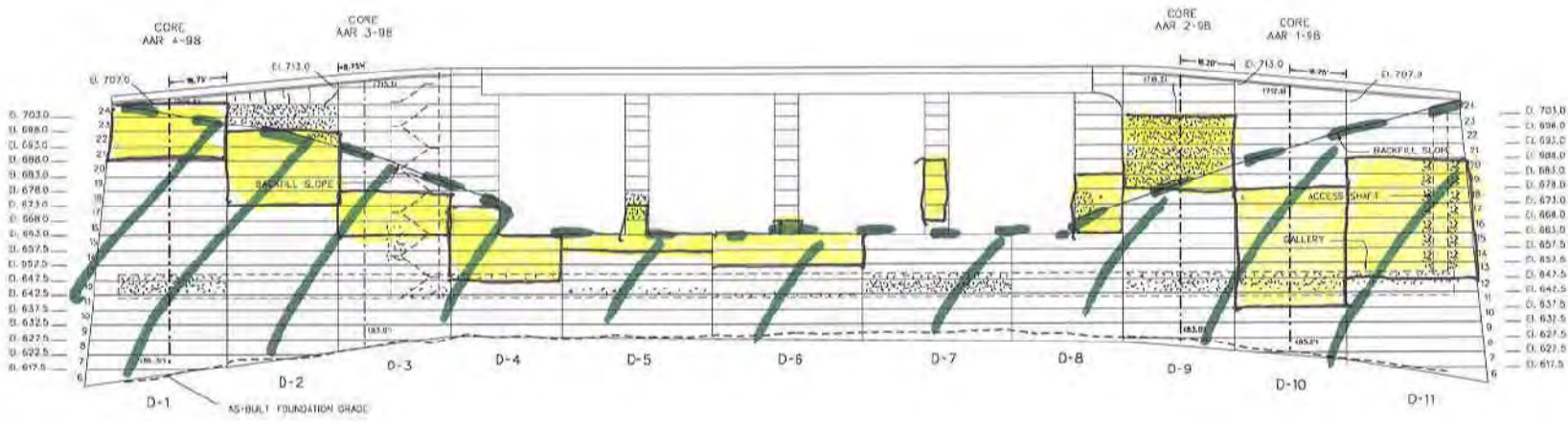
 - Not visible (back filled or water covered)
 - Expected Damage based on placing date

LEGEND
 OBSERVED MAP CRACKING
 STRUCTURAL CRACKS
 PG. CORE HOLE

FIGURE



Expected Damage - Downstream Face



CARTERS REREGULATION DAM - DOWNSTREAM ELEVATION

/// Not visible (back-filled)
[Yellow Box] Expected damage based on placing date

LEGEND	
	OBSERVED MAP CRACKING
	STRUCTURAL CRACKS
	PG CORE HOLE

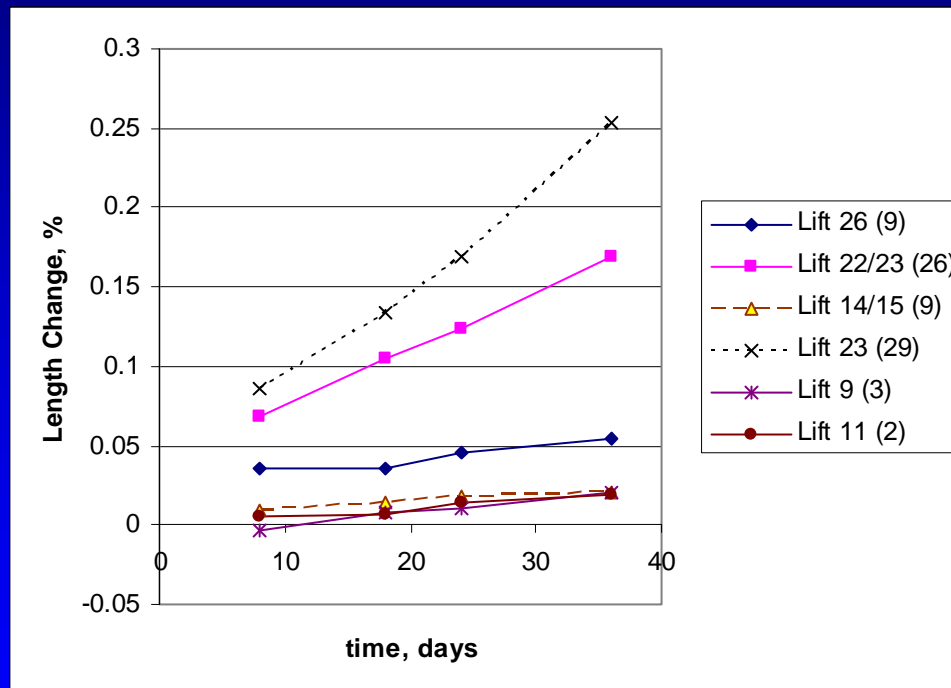


Strength

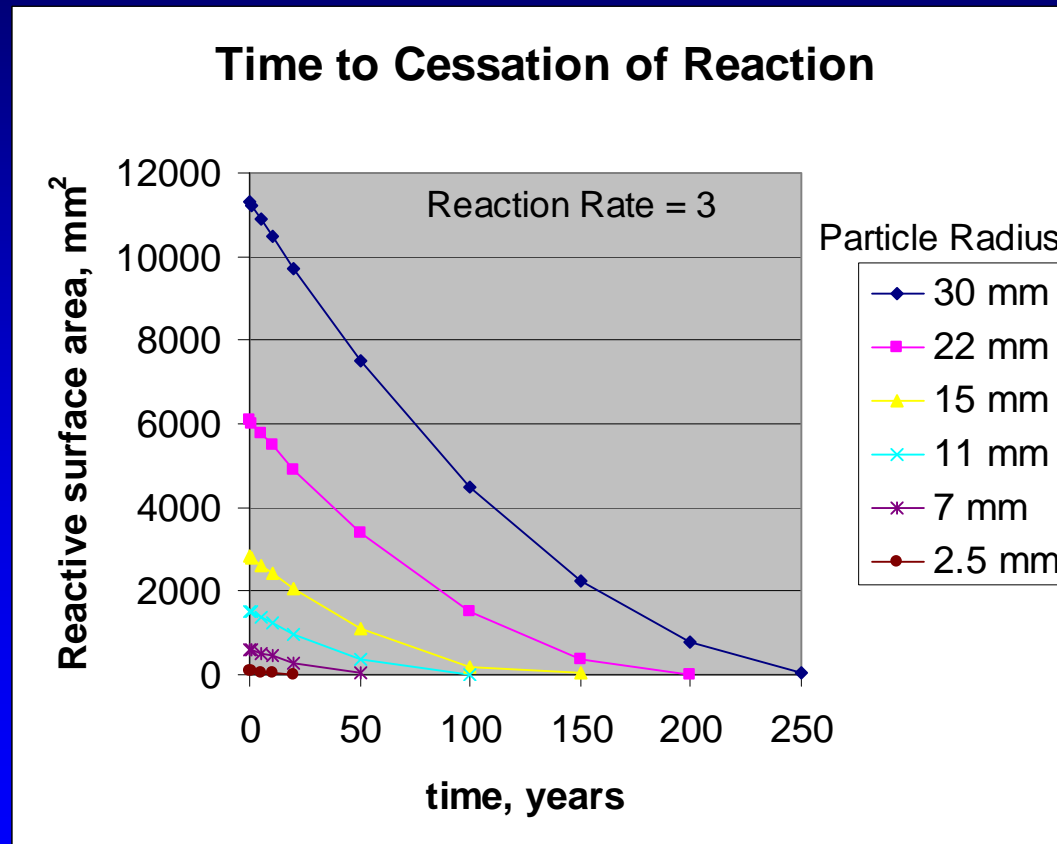
- Strength ~ number reactive particles/ft
 - Low counts: 3935 psi
 - Moderate counts: 3357 psi
 - High counts: 2884 psi (best of the worst!!)



Residual Expansion



Remaining Reaction



Similar Structures

- Chickamauga
 - Lock soon to be replaced
- Center Hill



Major Materials Issue

- Aggregate QC
- Alkali Carbonate Reaction
 - First analysis suggests
 - Bad news for aggregate sources
- Alkali Silica Reaction
 - Similar in some features
 - Better news for aggregate sources
- AAR - Do we really know what we're doing?



Damaging Interactions Among Concrete Materials

Toy Poole

U.S. Army Corps of Engineers

August 2005



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Interactive Effect

- Ad hoc definition: Effect of two or more materials acting on each other in unexpected ways.
- Focus on the negative
- Usually are problematic because of lack of understanding of mechanism
- Tend to defy specifications



AAR: One of the Older Ones

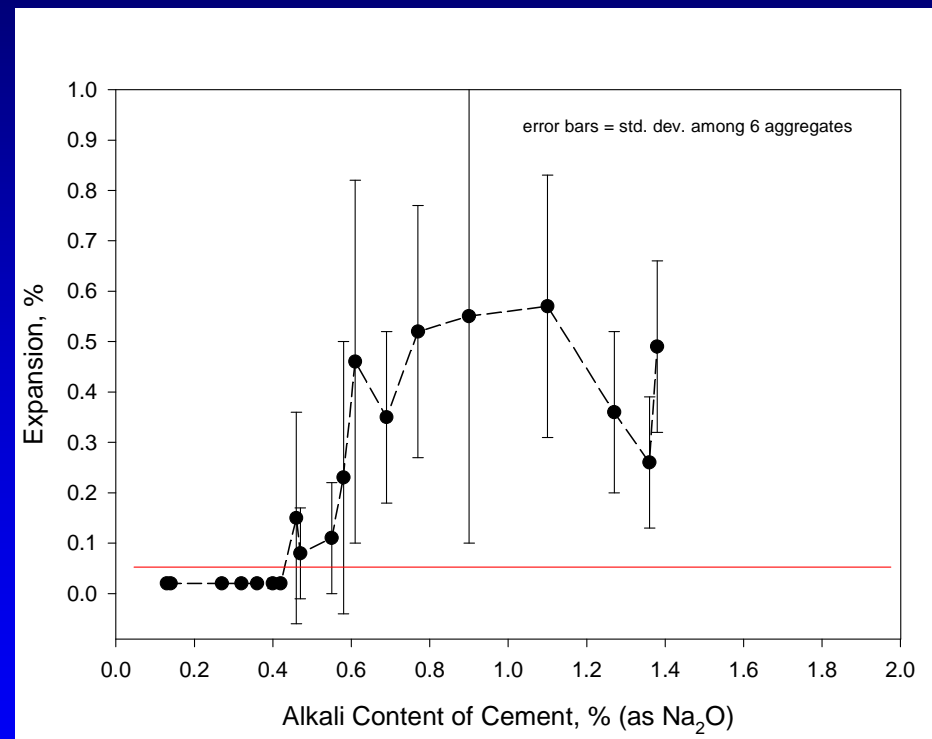


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AAR

- Cement alkalis
 - Solution: Total alkalis < 0.60%
- Reactive Constituents



Low Alkali Didn't Work!

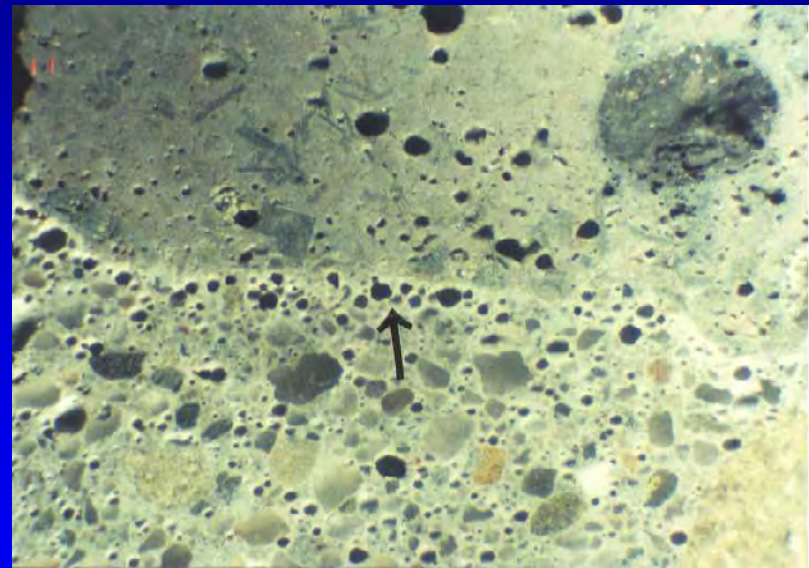


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Cement – Air Entraining Admixture (relatively new)

- Some AEA's?
- Some concrete materials?
- Some conditions?
- Air voids collapse around aggregate



Failure of Air Void Systems



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Early Stiffening Reactions

- Portland Cement –WRA Reactions
- Portland Cement – Fly Ash Reactions
- Vary from mild to severe
 - Mild – nuisance
 - Intermediate – often most problem
 - Severe – total show stopper!

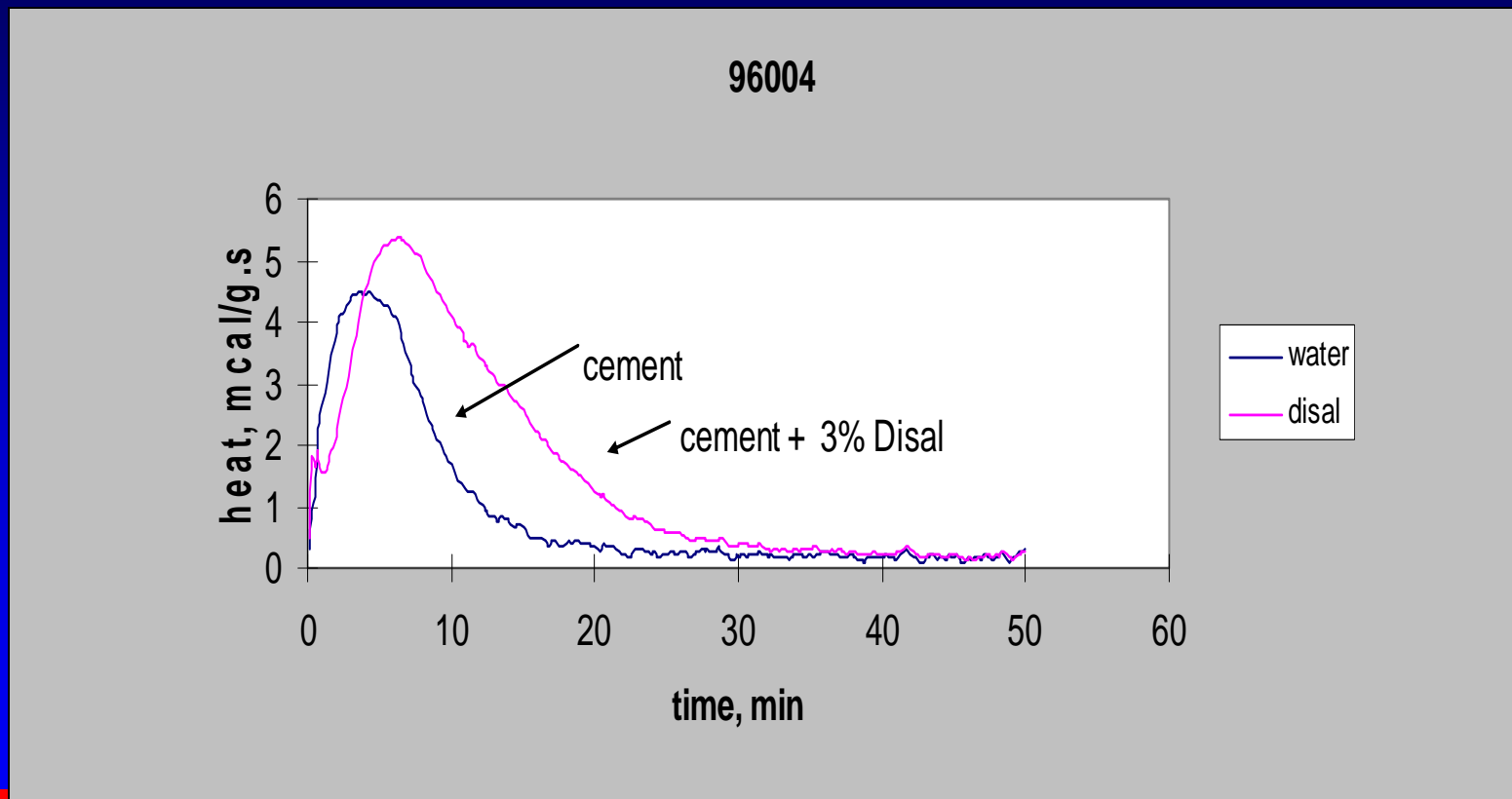


Flash Setting vs False Setting

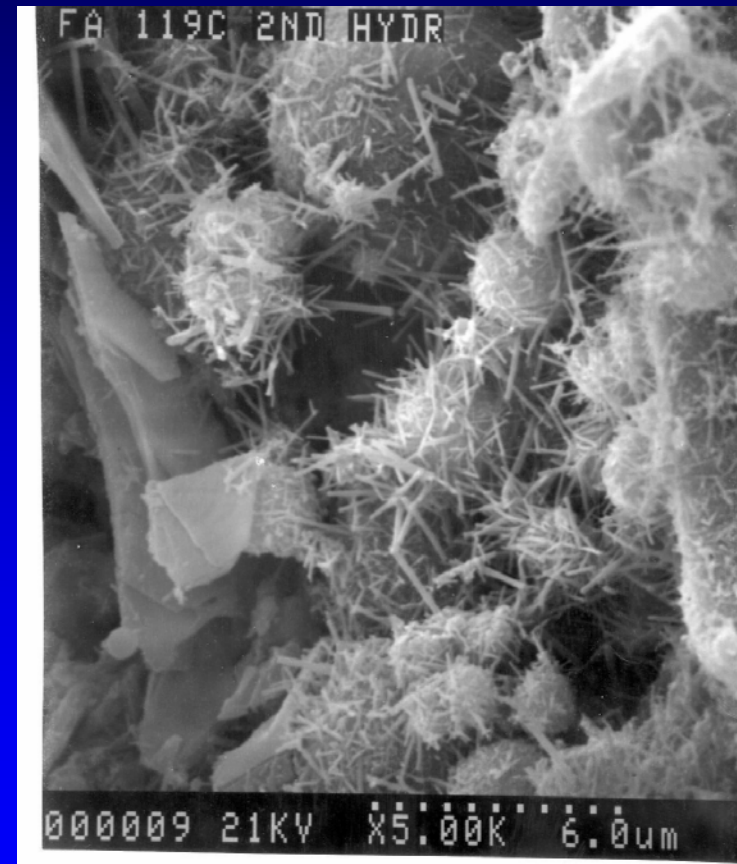
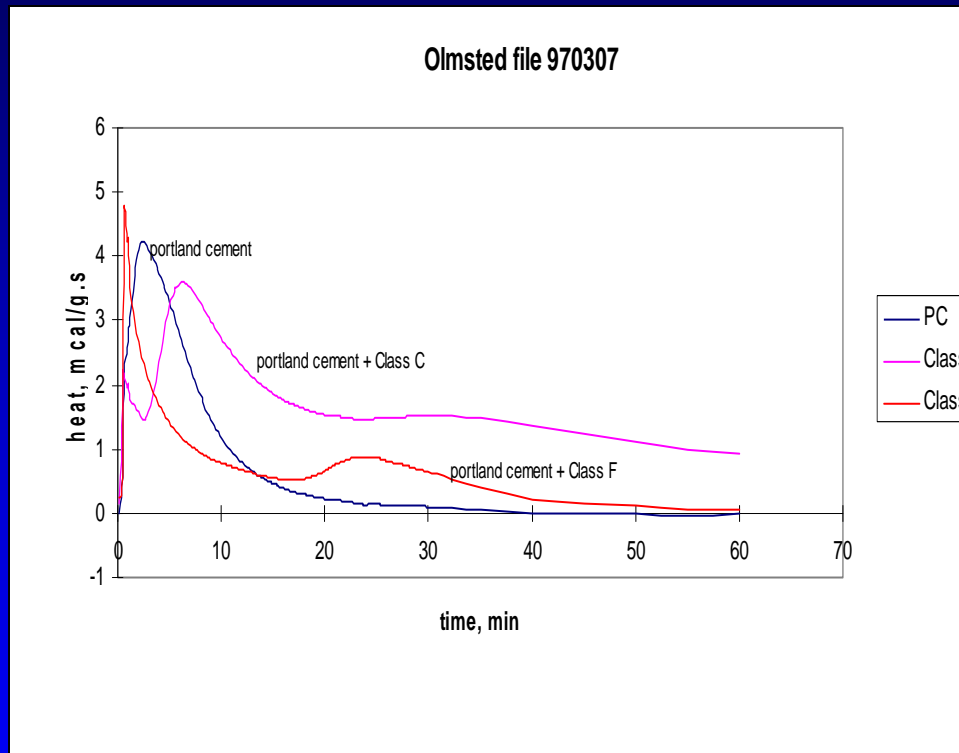
- Flash setting – doesn't disappear on extended mixing – usually caused by accelerated cement hydration
- False setting – disappears with extended mixing – caused by plaster in cement



Cement – WRA: Flash Setting



Cement – Fly Ash Reaction



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Damage Factors

- Poor compaction
- Temptation to add water
- Economic - Lost productivity



Poor Compaction



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Extra Water



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Lost Productivity



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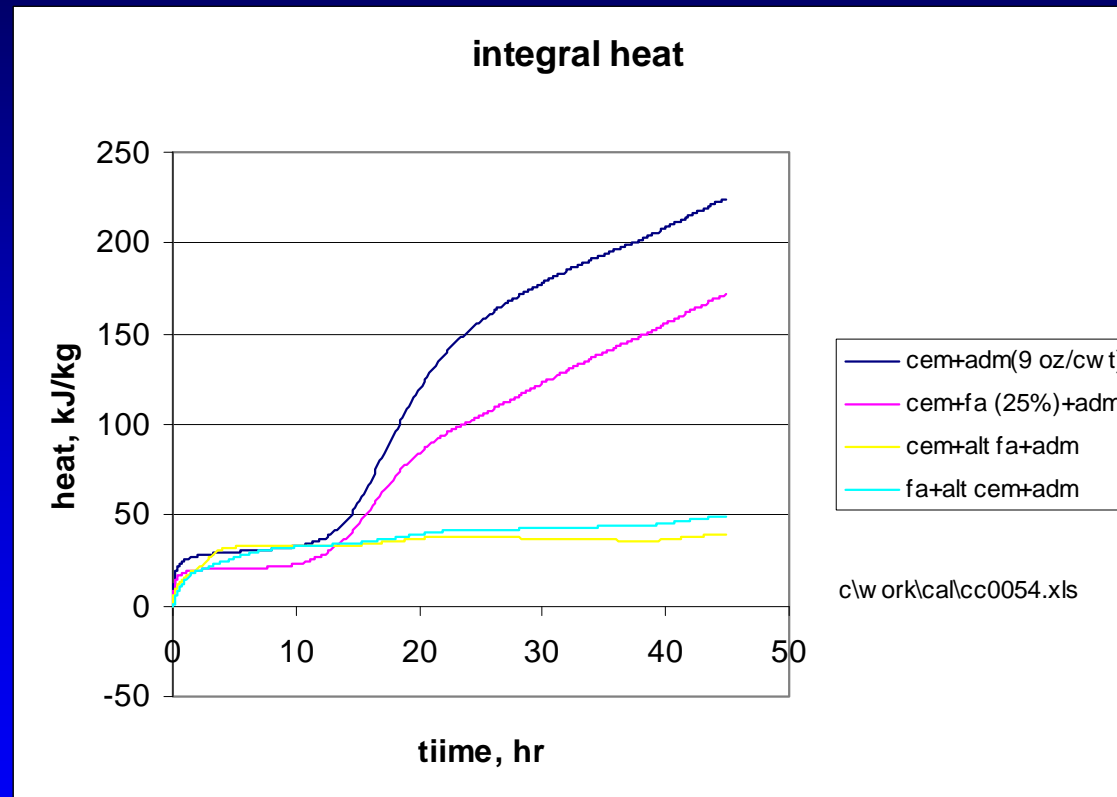
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Extreme Retardation

- Cement – WRA Reactions
- Cement – Fly Ash – WRA Reactions



Inhibition of C_3S Hydration



Damage Factors

- Plastic Shrinkage Cracking
- Economic – Lost Productivity



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ASTM Task Group on Interactions

- Developing test methods
 - Early stiffening
 - Delayed setting
- No specification activity
 - Plausible with fly ashes
 - No clear responsibility tag with admixtures



The End



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Economic Effects on Construction of Uncertainty in Test Methods

Toy Poole

U.S. Army Corps of Engineers

August 2005



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Selected Examples

- CRD-C 114 - F/T dur of aggregates
- ASTM C 78 – flex beam
- ASR testing
- Curing compound testing
- Heat of hydration testing



Test Method Uncertainty

- Within-laboratory variation
 - Operator
 - Equipment
- Between-laboratory variation
- Simple bias
- Material-dependent bias



ASTM

- Requires precision and bias statement
 - Within laboratory - repeatability
 - Between laboratory – reproducibility
- d2s – based on std dev
- d2s% - based on CV



d2s

- Maximum difference among a set of determinations in 95% of cases
- For duplicate determinations,
 - $d2s = 2.8*s$, or $2.8*CV$
- For triplicate determinations,
 - $d2s = 3.3*s$, or $3.3*CV$
- Multipliers for larger sets in ASTM C



670

Example – ASTM C 138

Density of Concrete

- Within-lab std dev = 0.65 lb/ft^3
 - $d_{2s} (n=2) = 1.85 \text{ lb/ft}^3$
 - $d_{2s} (n=3) = 2.15 \text{ lb/ft}^3$
- Between-lab std dev = 0.82 lb/ft^3
 - $d_{2s} (n=2) = 2.31 \text{ lb/ft}^3$



CRD-C 114

Durability of Aggregates to Cycles of Freezing and Thawing

- Acceptance testing of concrete aggregate
- Based on ASTM C 666
 - Air-entrained concrete
 - Results reported as a Durability Factor 0 – 100%
 - 100% Specifications typically 50 – 75%
- No reported precision estimate



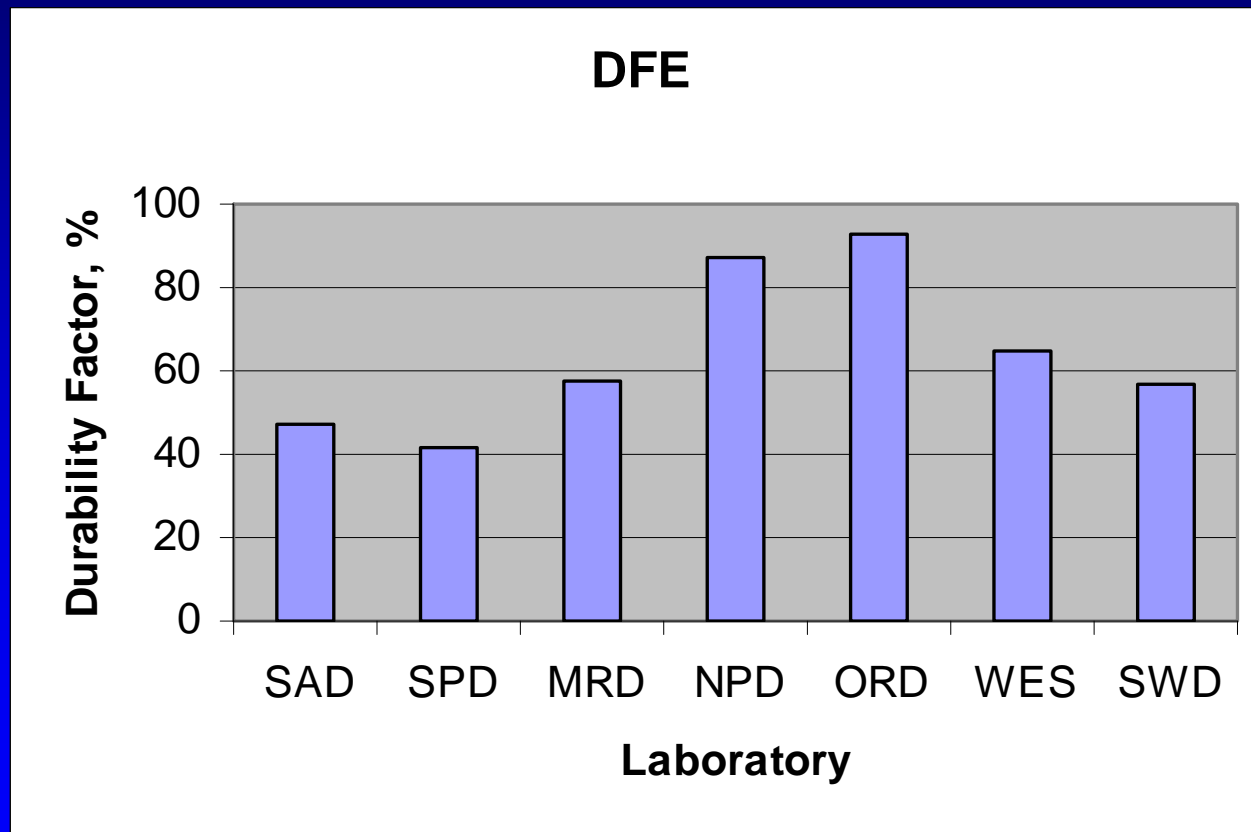
CRD-C 114

Durability of Aggregates to Cycles of Freezing and Thawing

- Significant between-laboratory disagreements
- Changes in use of durability factor specifications



Mather 1954



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Precision CRD-C 114

- Standard deviation among labs
 - 19.3%
- d2s among labs
 - 54%



Economic Consequences of Rejection

- Hauling distance to secondary source
- 10 mi of 4 lane highway
 - 120,000 yd³ of concrete at \$0.15/ton/mi
 - 25 mi haul = \$450,000
 - 50 mi haul = \$900,000



ASTM C 78

Flexural Strength

- Basis for acceptance of mix design
- CV = 7% between laboratory
- At 650 psi
 - d2s ~ 125 psi



Economic Consequences

- Delays over mixture acceptance
- Add extra 100 lb/yd³ to insure compliance
- 10 mi of 4 Lane
- ~\$1,000,000 in cement cost



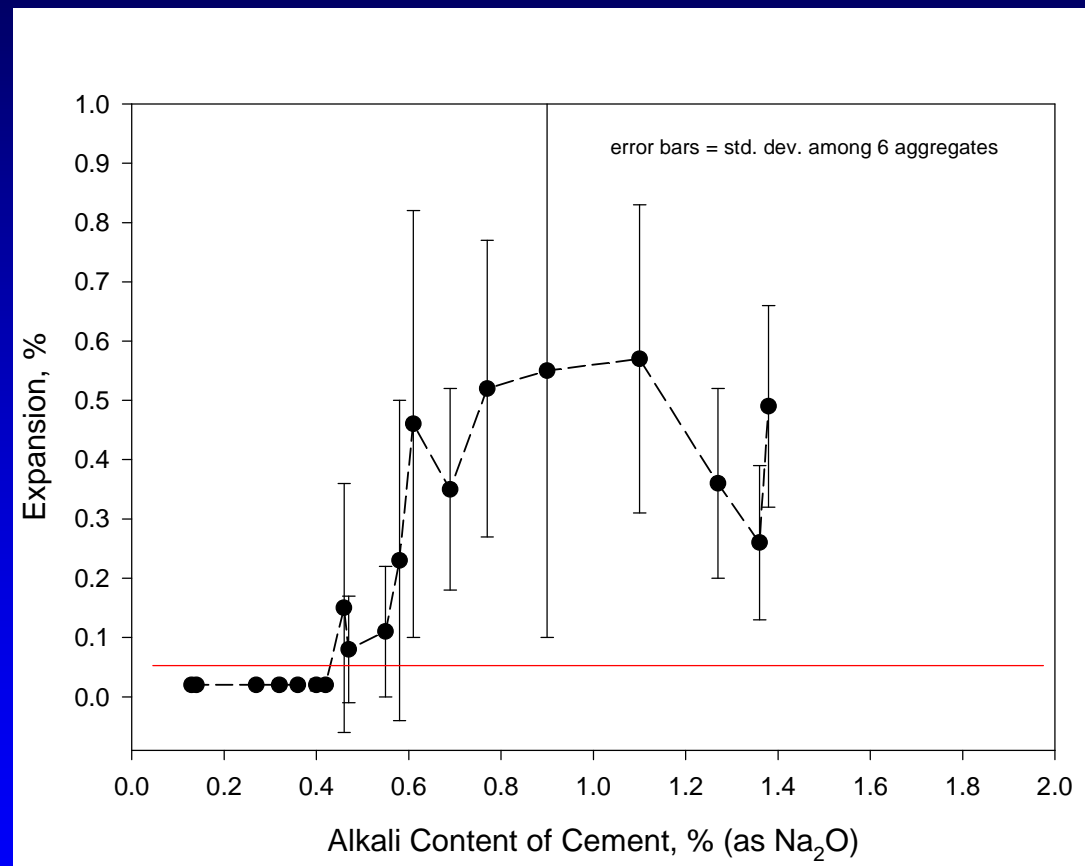
AAR



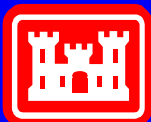
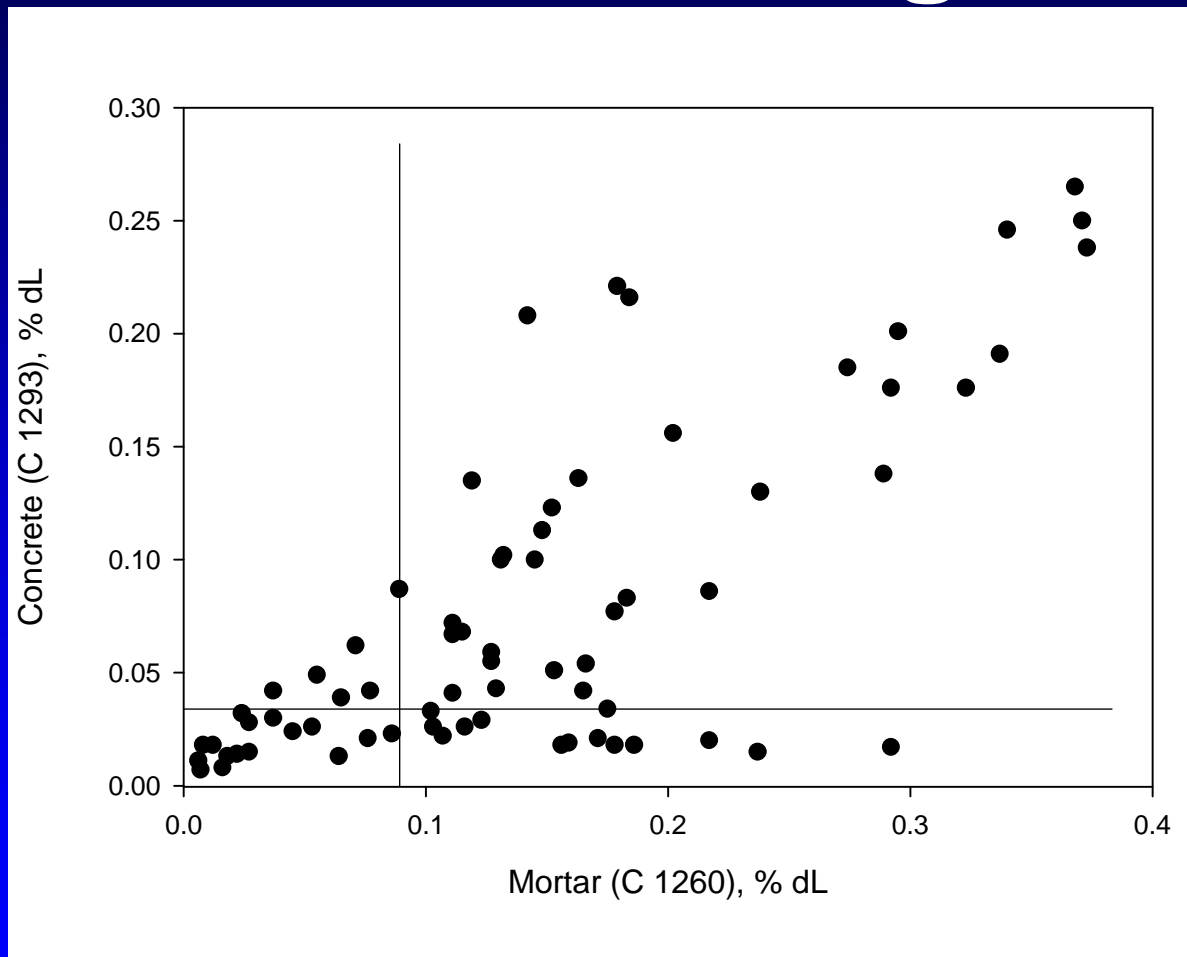
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ASTM C 150 – Low Alkali



ASR Testing



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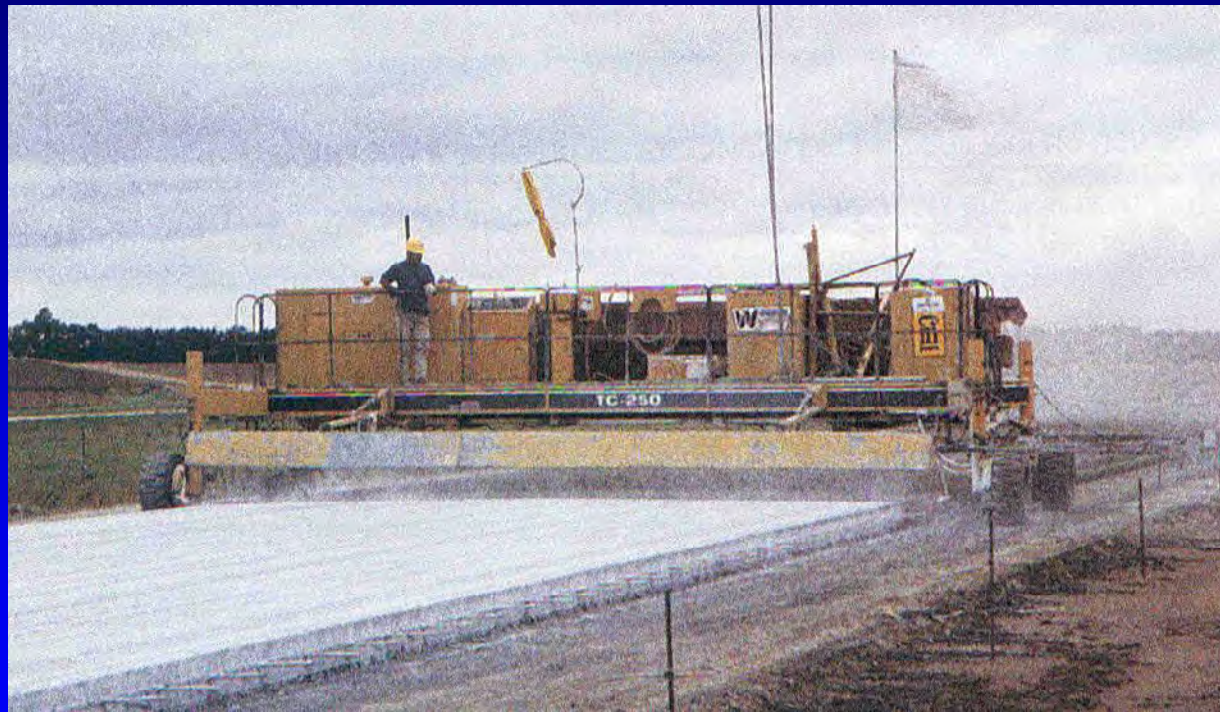
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AAR Cost Factors

- Rejection of acceptable aggregate
 - Short term \$\$
- Acceptance of inadequate aggregate
 - Long term \$\$



ASTM C 156 – TM for Curing Compounds



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ASTM C 156

- Typical limit: 0.55 kg/m²
- Typical production: 0.45 - 50 kg/m²
- Between Lab Std dev = 0.07 kg/m²
- Between Lab d2s = 0.20 kg/m²

Error > Safety Margin!!



C 156 Cost Factors

- User – producer disputes
- Over conservative specification
 - High solids materials
 - Difficult to apply
- May not perform
- Little testing by Federal Gov't



ASTM C 186

Heat of Hydration of Hydraulic Cement



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ASTM C 186

Heat of Hydration of Hydraulic Cement

- Between Lab std dev = 4 cal/g
- d2s = 11 cal/g
- Represents ~1,000 psi strength difference
- Target strength = 1500 psi, 3 days
- Specification limit = 1000 psi, 3 days



Cost Issues

- Uniformity in Strength Gain
 - Weekly variation ~1,000 psi
- Uncertainty in Form Removal



Trends in Concrete Materials Specifications

Toy Poole

U.S. Army Corps of Engineers

August 2005



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Hydraulic Cement



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Hydraulic Cement Portland Cement

- Type I – general purpose → Increasing strength
 - Type II – mod SO_4 , mod heat → Increasing heat
 - Type III – high early
 - Type IV – low heat → FAPP doesn't exist
 - Type V – high SO_4
- Diagram: A vertical arrow points from "Increasing heat" to "Increasing strength".



AASHTO – ASTM Harmonization

- Current Activity
- Develop a common PC spec
- Major revision of Type II
 - Limit on heat of hydration
 - Limit on fineness



Hydraulic Cement P2P

- C 150 – Portland Cement
- C 595 – Blended Cement
- C 1157 – Hydraulic Cement

Prescriptive



Performance



Major Industry Trends

- Strength
 - Increasing 1970 - 1995
- Fuel costs
 - Waste fuel initiatives
- Waste management
 - Dust recycling – high alkali levels
- CO₂ Emissions
 - Non PC additions

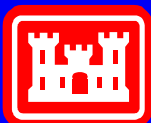


Additions

- Carbonate rock dust - 2004
- Slag – as a processing addition
- CKD - ???



Pozzolan



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Major Industry Trends

- Increasing Class C
- “Spot Market” coal supplies
- SO₂ emissions
- Ash from alternative fuels
- Development of **Performance** stds



Slag



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Industry Trends

- Increased marketing
- Shifting emphasis to finer materials
 - Grade 80 uncommon
 - Grades 100 & 120
- Name: GGBFS Slag Cement



Aggregate



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Industry Trends

- ASR testing

- Mortar bar →

- C 1260 – accel mortar ←
 - C 1293 – concrete prism

- Manufactured Fine Aggregate

- High fines concrete

- Appendix to ASTM C 33



Admixtures



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Industry Trends

- New Products, new versions of old products
 - SCC
 - Antiwashout
 - Antifreezing
 - Anticorrosion
- Cement – Admixture Interaction
 - Early stiffening
 - Delayed setting HRWRA
 - polycarboxylate



Repair Materials

- Historically: few or no spec's
- Rapid-strength-gaining cements
- Corps of Engineers – REMR
 - Focus on compatibility
 - Modulus
 - Thermal expansion
 - Volume stability



The End



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PRESENTER:

GEORGE RAGAZZO

PHONE: (251) 380-0332

CELL: (251) 422-3536

MODULAR GABION SYSTEMS

gragazzo@gabions.net

“GABION”

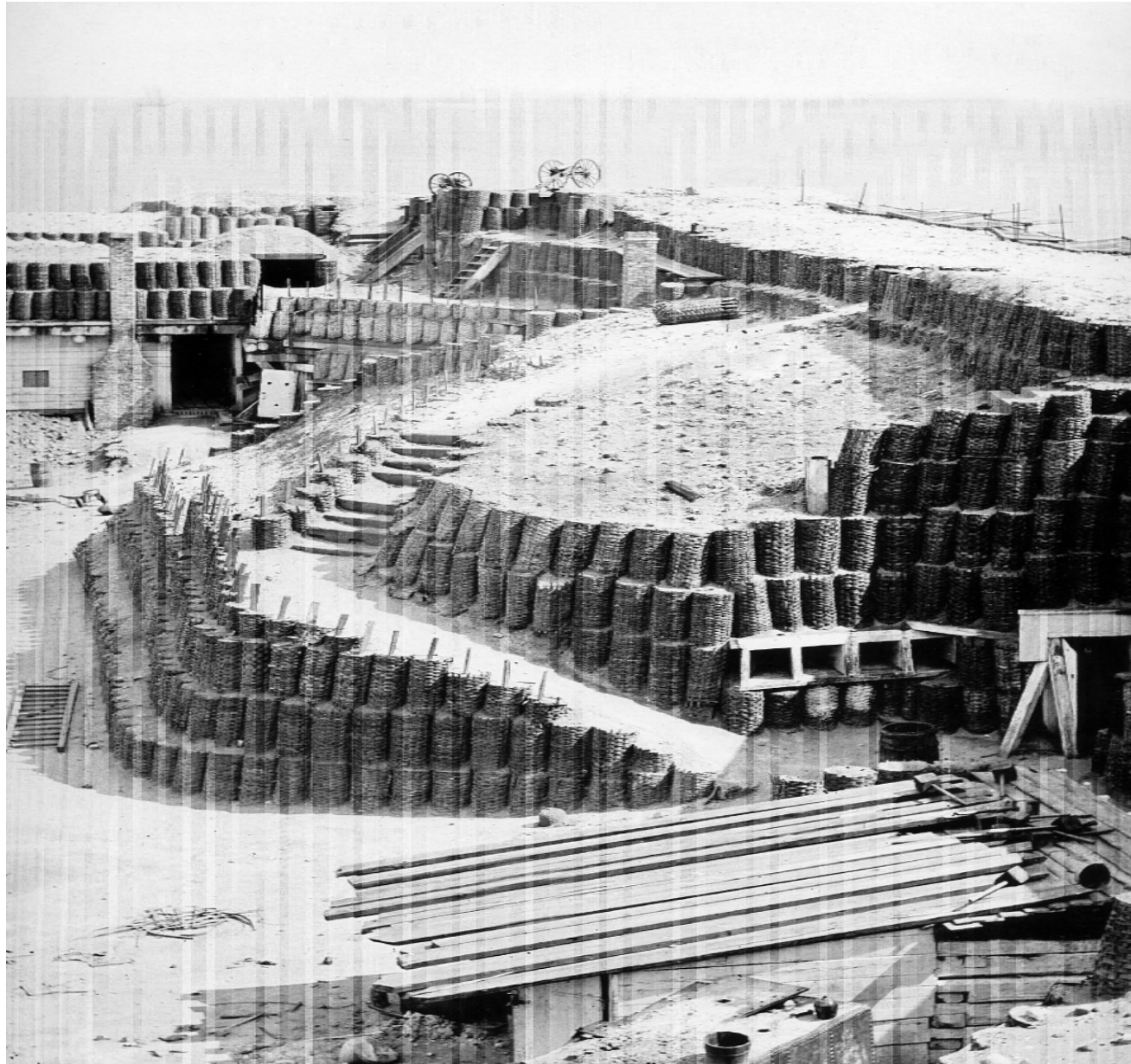
THE WORD ORIGINATED FROM:

- **LATIN - “CAVEA” = CAGE**
- **ITALIAN - “GABBIA” = CAGE**
- **ITALIAN - “GABBIONE” = LARGE CAGE**
- **ENGLISH - “GABION” = LARGE CAGE**

“GABION”

WEBSTER’S DEFINITION:

- 1. A cylinder of wicker filled with earth or stones, formerly used in building fortifications.**
- 2. A similar cylinder of metal, used in building dams, dikes, etc.**



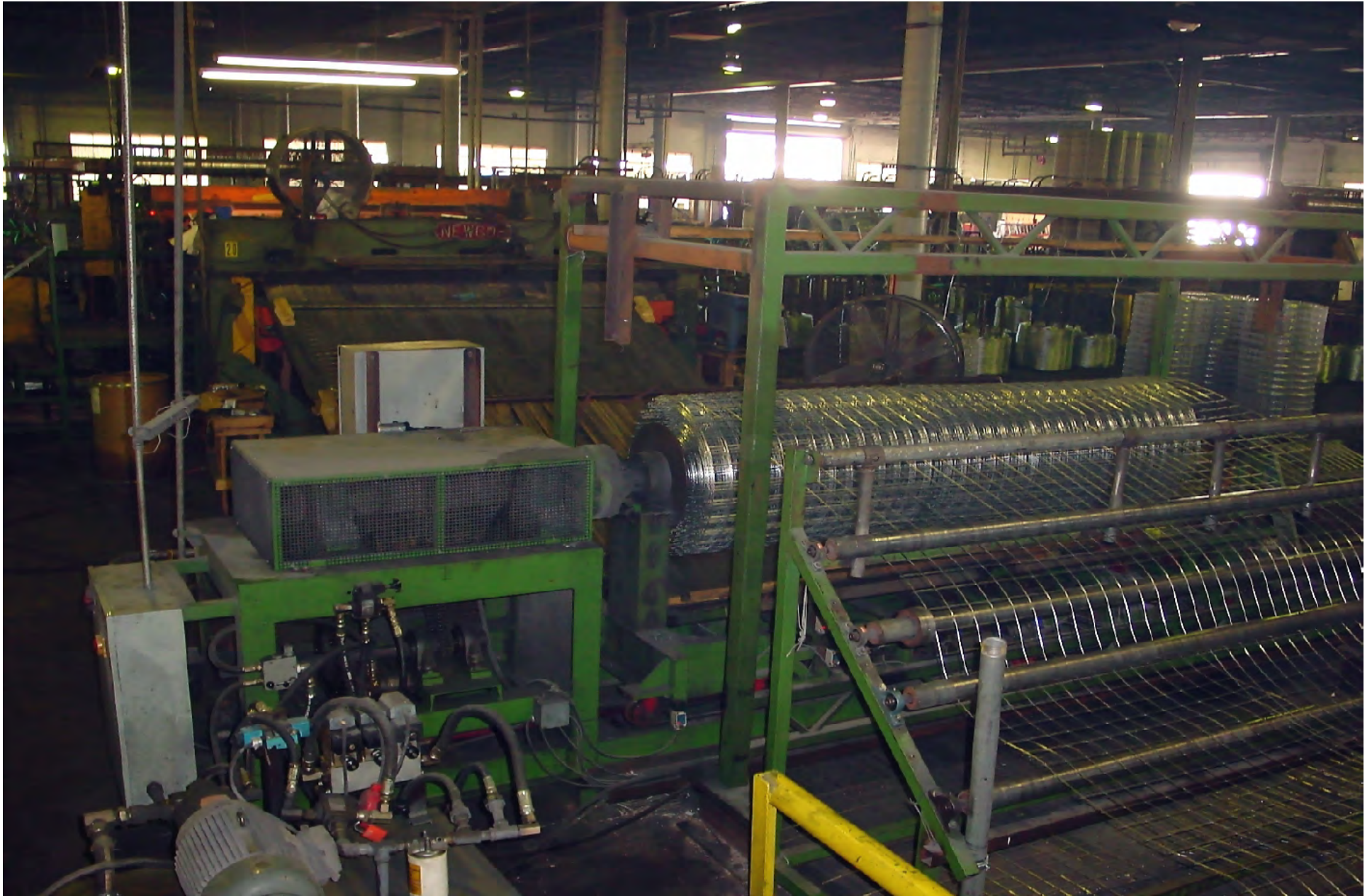
**GABION FORTIFICATION – FT. SUMTER, SC
CIVIL WAR 1865**

GABIONS are steel wire mesh “large cages”, “baskets” or “containers”, which when **interconnected and rock-filled form monolithic, flexible, permeable structures** unique to solve the complex problems of erosion control, flood control, earth retention, bank stabilization, etc. at relatively low cost.

GABION WIRE

TYPES AVAILABLE

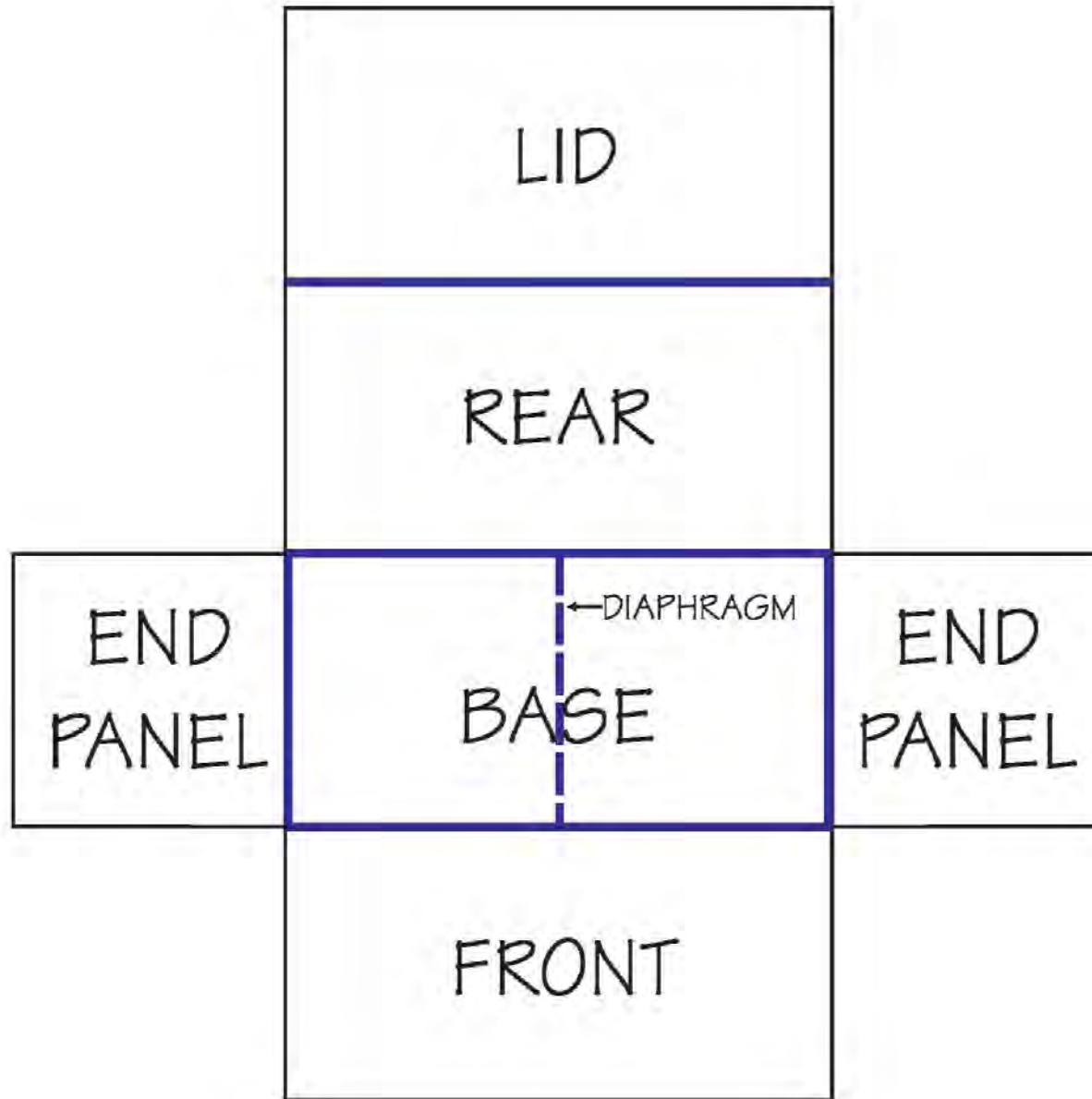
- **Galvanized wire – class 3 zinc coated**
- **Bezinal coated wire – 95% zinc
± 5% aluminum**
- **PVC coated wire – zinc or bezinal & PVC**
- **Stainless steel wire**



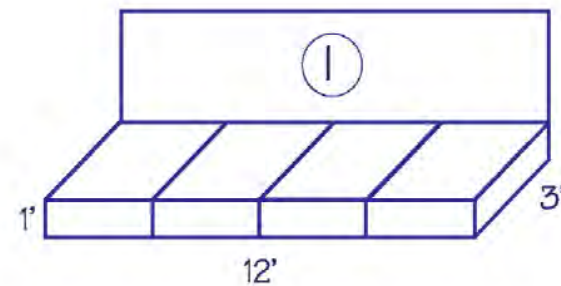
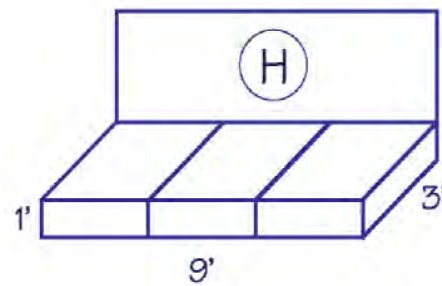
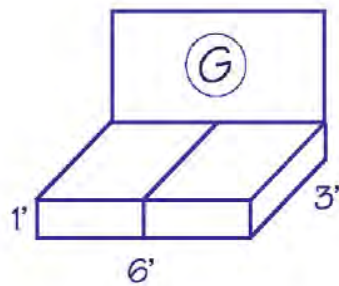
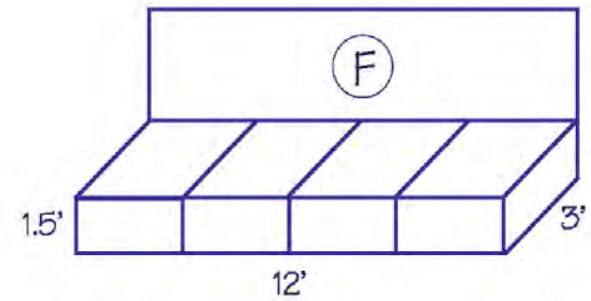
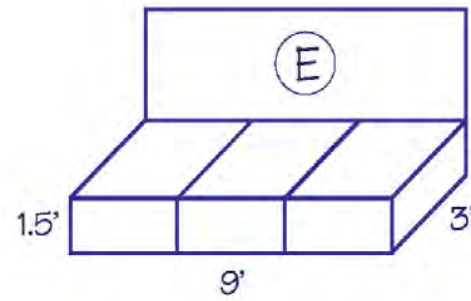
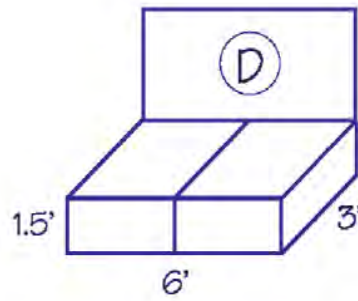
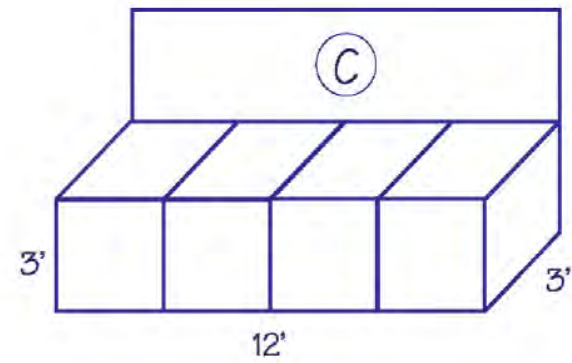
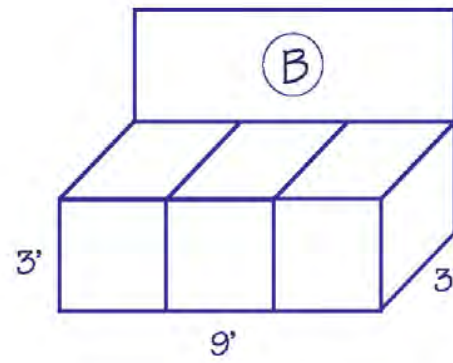
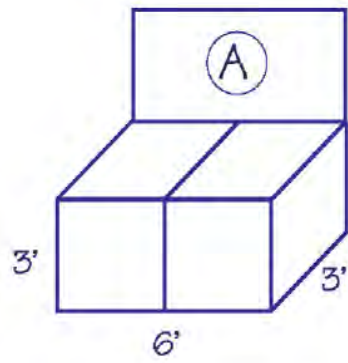
WELDED WIRE MESH GABION MACHINE
MESH IS PRODUCED IN ROLLS



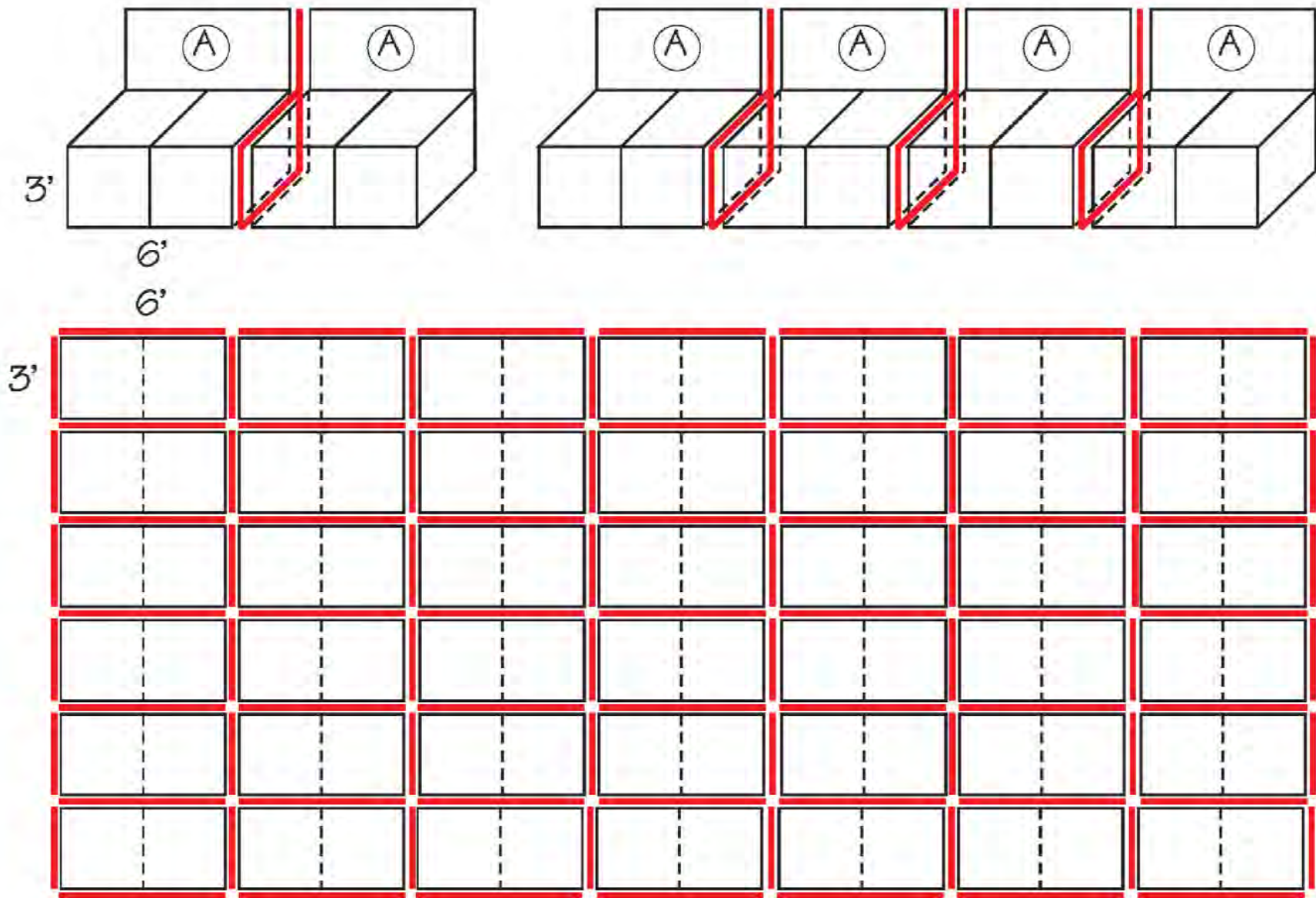
**TWISTED WIRE MESH GABION MACHINE
MESH IS PRODUCED IN ROLLS**



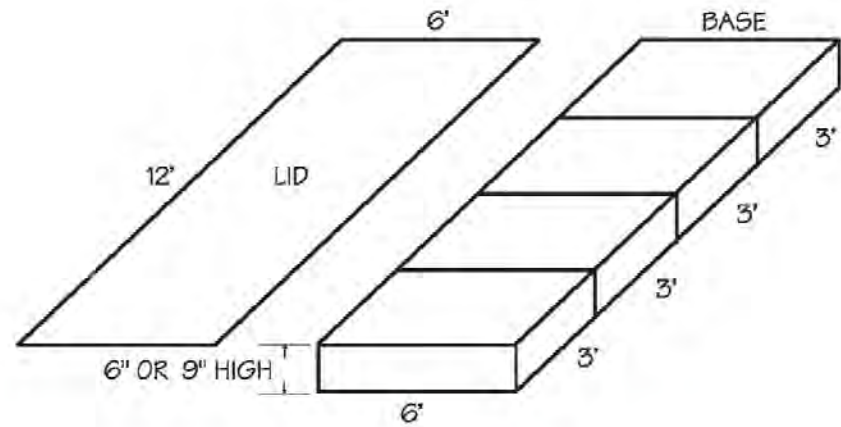
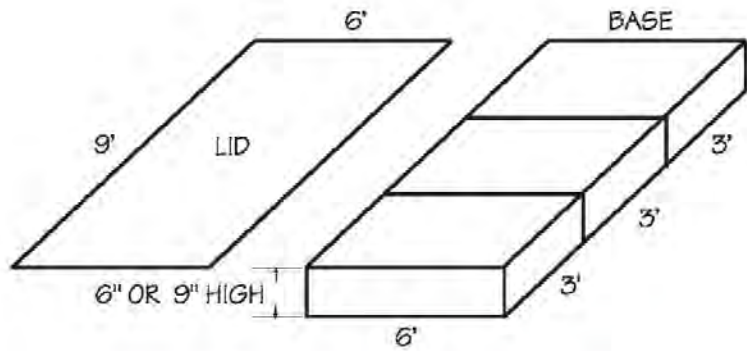
UNFOLDED-UNASSEMBLED GABION



STANDARD GABION SIZES



GABION LAYOUT - ISOMETRIC & PLAN VIEW



GABION MATTRESS LAYOUT ISOMETRIC & PLAN VIEW

JOINTLESS GABIONS

**Trapezoidal channel revetment
constructed with
PVC coated Gabion Mattress
utilizing jointless gabions
from “Roll-Stock” material.**



TRAPEZOIDAL CHANNEL REVETMENT - COMPLETED



**“ROLL-STOCK” GABION MATERIAL
DELIVERED TO JOBSITE**



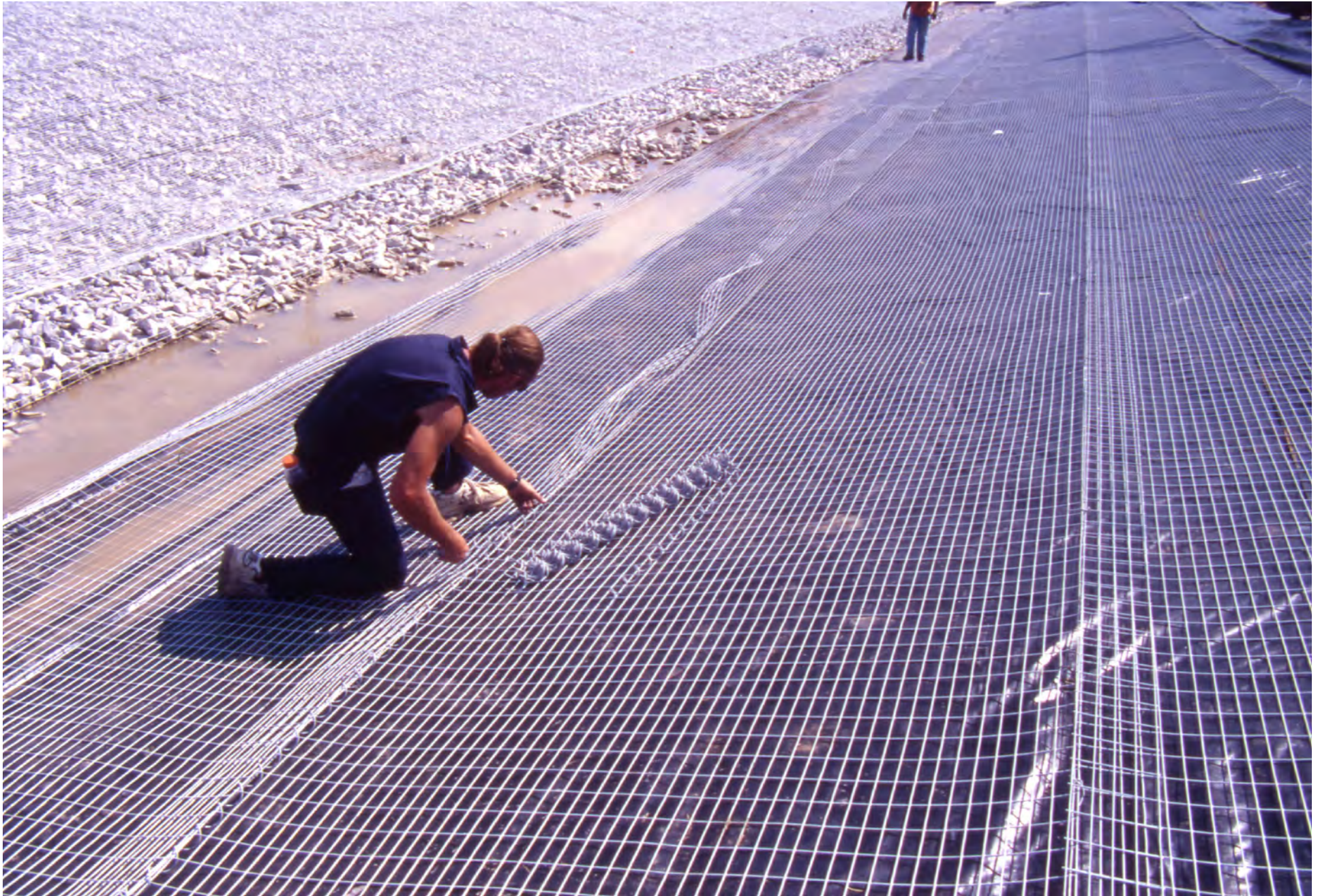
GABION MESH BEING UNROLLED OVER GEOTEXTILE



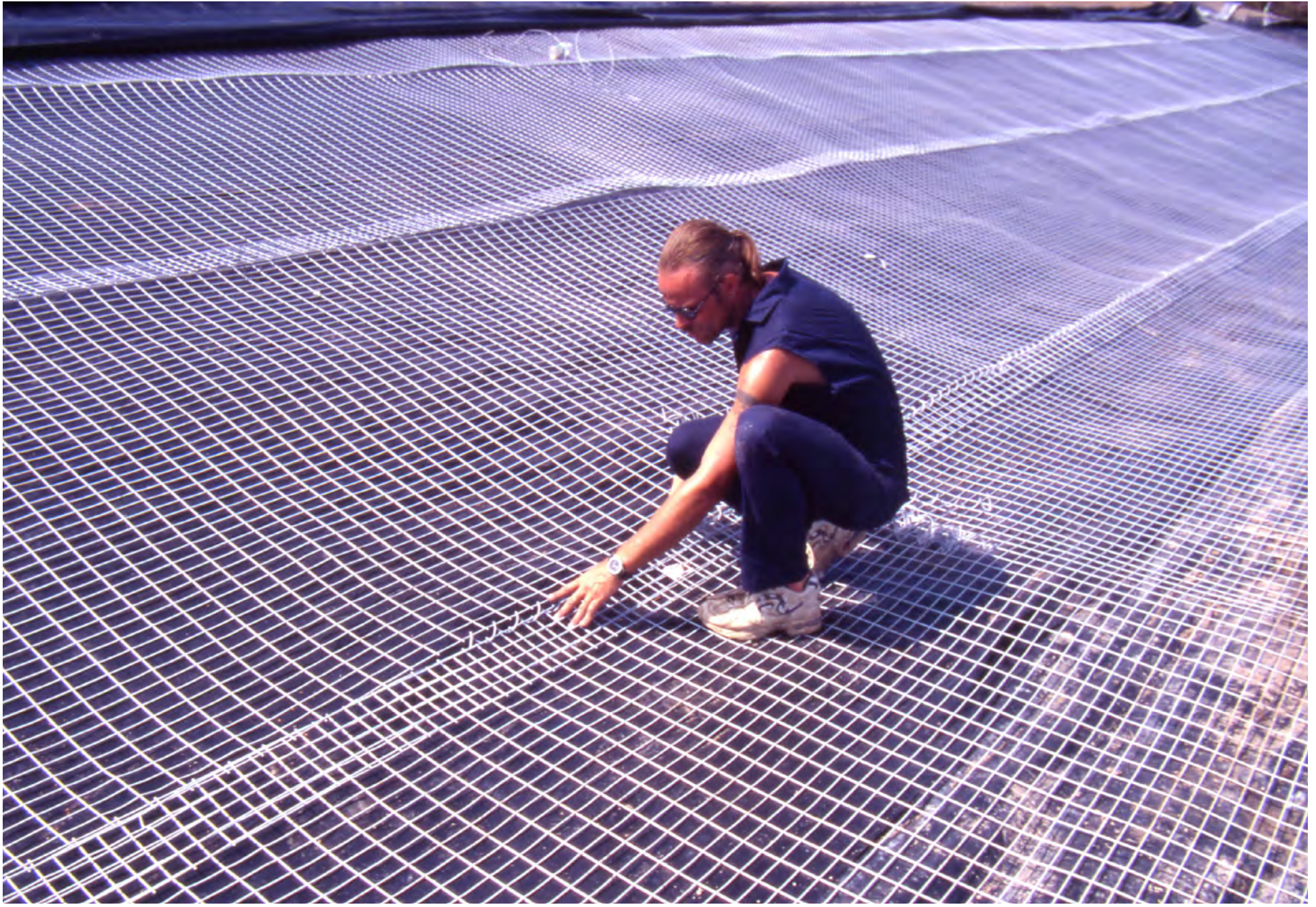
UNROLLING CONTINUOUS DIVIDER PANEL



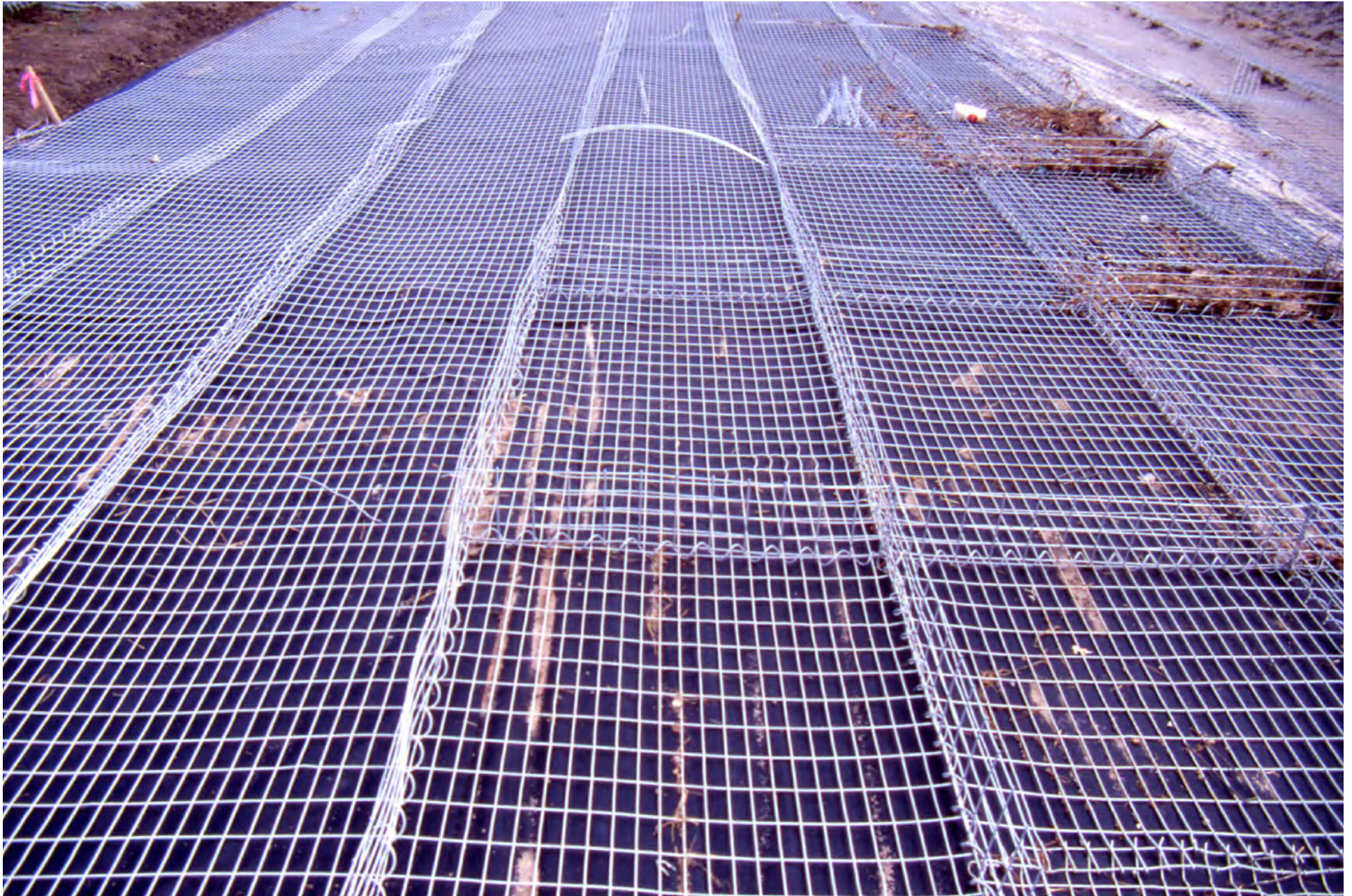
UNROLLING CONTINUOUS EDGE PANEL



SPIRAL CONNECTING DIVIDER TO BASE PANELS



DETAIL OF SPIRAL CONNECTION



SUBDIVIDING BASE INTO 6' X 3' COMPARTMENTS



DIAPHRAGMS ARE CUT FROM “ROLL-STOCK”



ROCK-FILLING THE GABION MATTRESS



WOOD FORMS PROTECT TOP OF DIAPHRAGMS



LEVELING ROCK-FILL & LID CLOSING



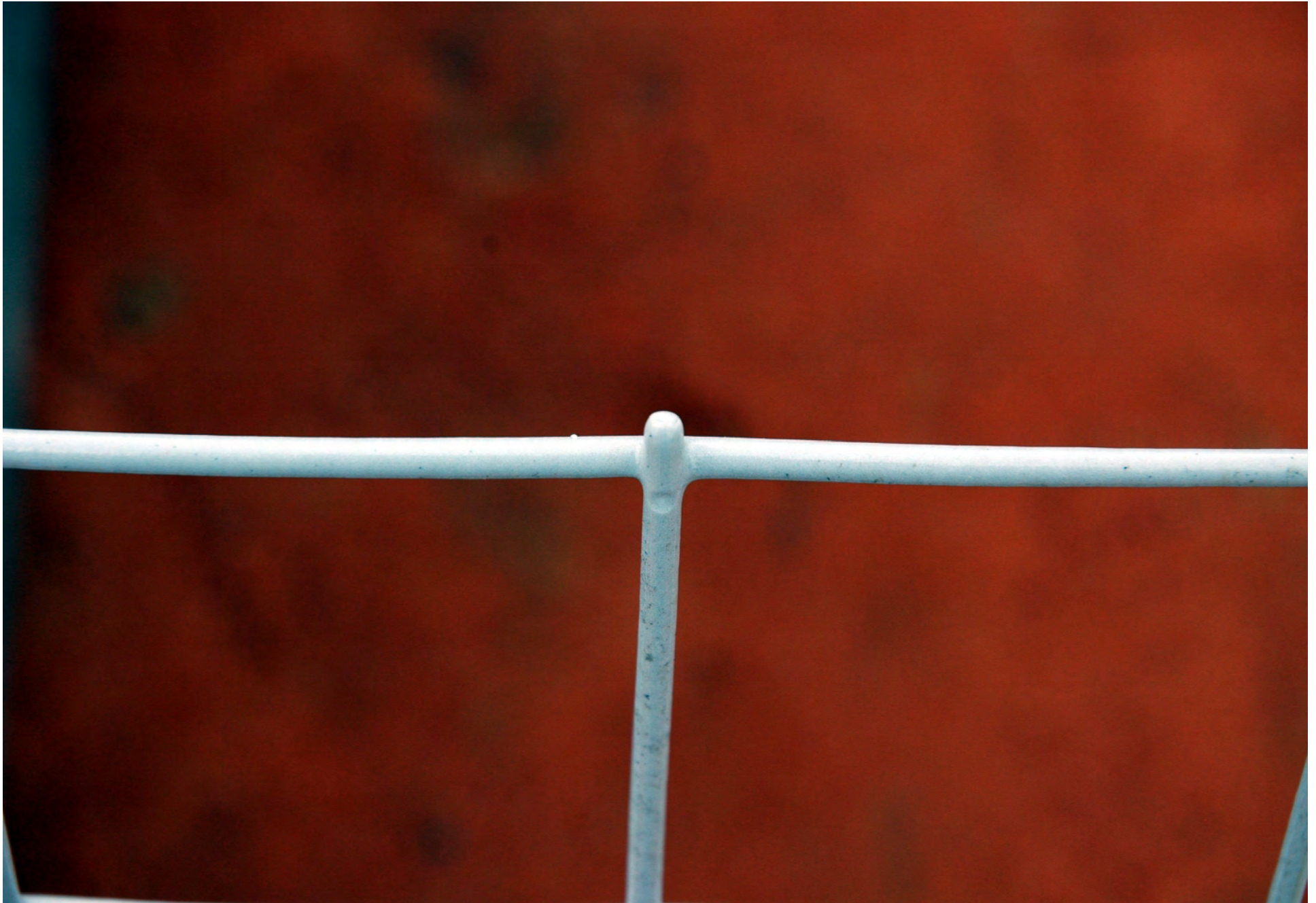
SPIRAL CONNECTING LIDS TO DIAPHRAGMS



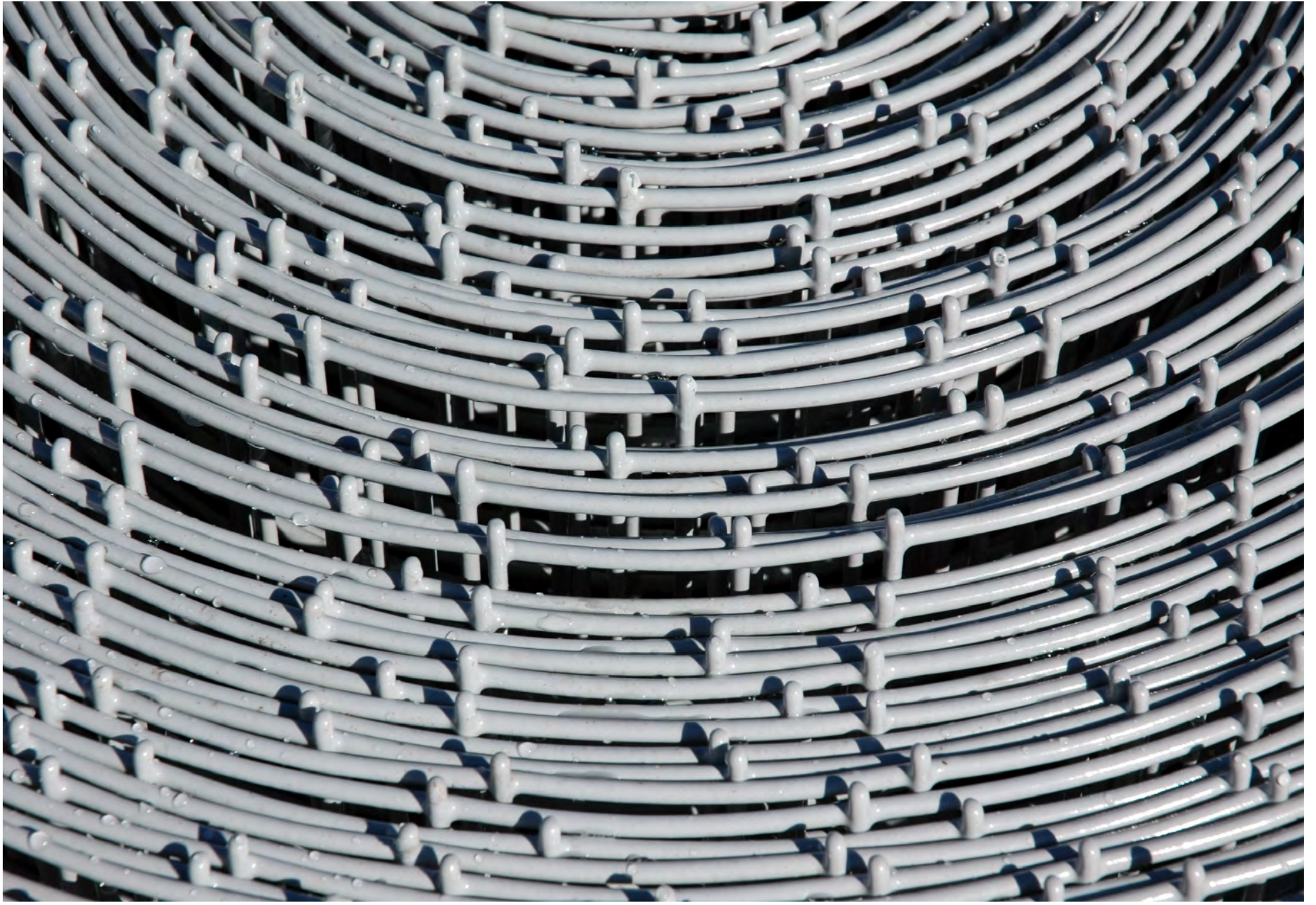
JOINTLESS LIDS FROM “ROLL-STOCK”



COMPLETED SECTION OF JOINTLESS GABIONS



ALL WIRE TERMINALS PROTECTED WITH PVC



ALL WIRE TERMINALS PROTECTED WITH PVC



**PRE-CUT PANELS
TERMINALS PROTECTED WITH PVC**

MECHANICALLY STABILIZED EARTH (MSE) GABION WALLS

**48 ft. high MSE wall, constructed
from PVC coated Gabion-Faced
Welded Wire Reinforced Soil
Wall, supporting a new building.**



MSE GABION WALL COMPLETED MARCH 1998



SITE EXCAVATED-DRAIN PIPE-GRAVEL BEDDING



6' WIDE PVC "ROLL-STOCK" UTILIZED FOR SOIL REINFORCING – 3" X 3" MESH – 12 GAUGE WIRE



33' LONG X 6' WIDE PANELS CUT FROM "ROLL-STOCK" FOR BASE COURSE SOIL REINFORCING



**JOINTLESS GABION BASE COURSE ASSEMBLED
OVER SOIL REINFORCEMENT PANELS**



18" WIDE X 300' LONG "ROLL-STOCK" UTILIZED FOR JOINTLESS GABIONS CONSTRUCTION



**SPIRALS CONNECTING GABION DIAPHRAGMS
TO SOIL REINFORCEMENT GRID**



ROCK-FILLING GABIONS WITH 4" TO 8" STONE



TYPICAL MSE GABION WALL CONSTRUCTION



SOIL BACKFILL COMPACTION TO 98% PROCTOR



**WELDED WIRE MESH SOIL REINFORCING
EXTENDED TO FRONT OF GABIONS**



MSE GABION WALL ABOUT 1/2 COMPLETED



**ONE STORY BUILDING ADDITION CONSTRUCTED
TO WITHIN 6' FROM EDGE OF WALL**



48' HIGH MSE GABION WALL COMPLETED 03/1998



AERIAL VIEW OF MSE GABION WALL & BLDGS.



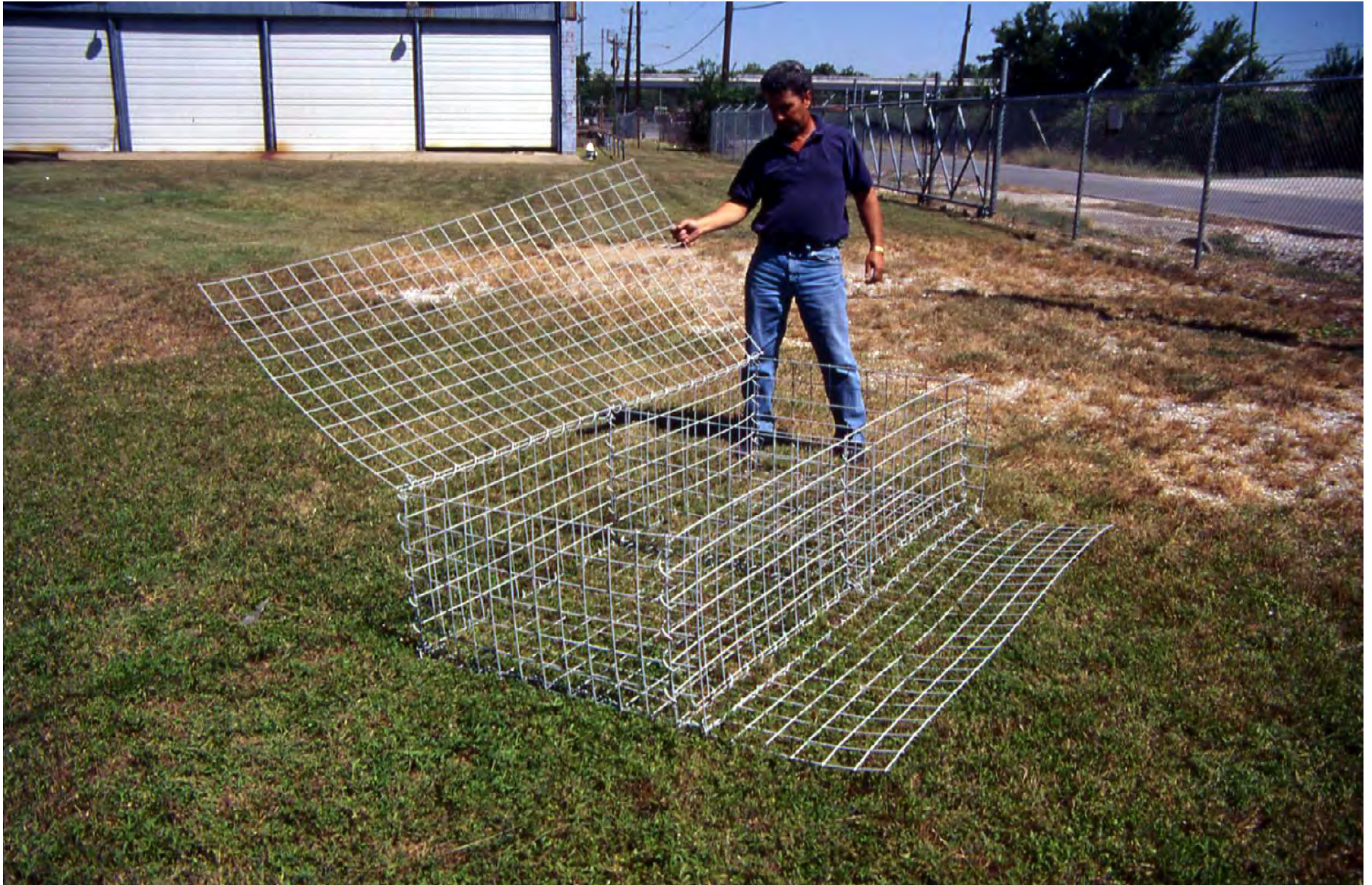
**MSE GABION WALL AS SEEN IN JUNE 2005,
SEVEN YEARS AFTER COMPLETION**

CONCRETE BLOCKS FACED GABION WALLS

**6 ft. high Gabion Walls faced with
“Ragazzo Blocks” supported by a
conventional 12 in. thick Gabion
Mattress. All Gabion material is
PVC coated after galvanizing.**



CONCRETE "RAGAZZO" BLOCKS FACED GABION WALLS – QUICK CONSTRUCTION DEMO



EMPTY GABION – NOTICE THE DOUBLE WIRE MESH FACING TO HOLD THE BLOCKS



**BLOCKS PLACED BETWEEN THE TWO FRONTAL
GABION MESH PANELS**



GABIONS ARE ROCK-FILLED BEHIND THE CONCRETE "RAGAZZO" BLOCKS FACING



**COMPLETED GABION WITH CONCRETE
BLOCKS FACING**



GABION LIDS ARE SECURELY CLOSED



**CONCRETE BLOCKS FACED GABION WALLS
PROJECT AT THE U.S.A. CAMPUS**



**UNIVERSITY of SOUTH ALABAMA
RESEARCH AND TECHNOLOGY PARK**

**STREET, DRAINAGE, WATER & SEWER DISTRIBUTION IMPROVEMENTS
CONSTRUCTED BY: G.A. WEST & CO., INC.**

DESIGNED BY:

**SPEAKS & ASSOCIATES
CONSULTING ENGINEERS, INC.**

**CSA GROUP, INC.
LANDSCAPE ARCHITECTS-PLANNERS**

CONCRETE BLOCKS FACED GABION WALLS



**CONCRETE “RAGAZZO” BLOCKS AND GABION
“ROLL-STOCK” MATERIAL AT JOBSITE**



**CONCRETE “RAGAZZO” BLOCK DETAIL
MEASURING 6” W. X 12” L. X 3” DEEP**



**PVC COATED GABION MATERIAL IN
“ROLL-STOCK” FORM**



**12" THICK GABION MATTRESS SUPPORT FOR THE
CONCRETE BLOCKS FACED GABION WALLS**



ROCK-FILLING THE 12" GABION MATTRESS



12" THICK MATTRESS READIED FOR WALL BASE



TWO 3' HIGH GABION PANELS, 3" APART, TIED TO THE MATTRESS & READY FOR CONCRETE BLOCKS



FIRST TWO CONCRETE BLOCKS PLACED



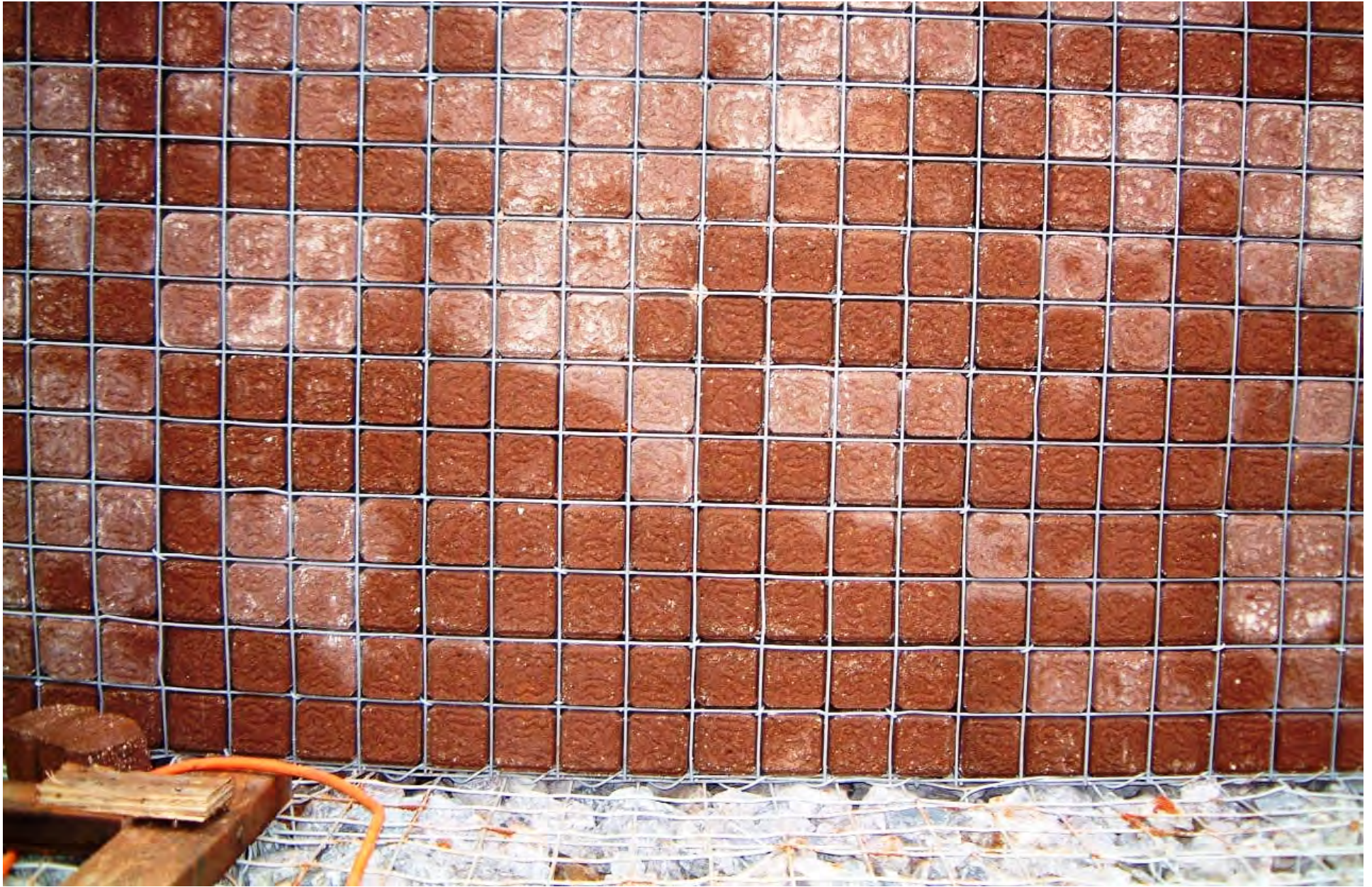
LEVELING THE CONCRETE BLOCKS FACING



CONCRETE BLOCKS PLACEMENT



CONCRETE “RAGAZZO” BLOCKS WALL FACING



**DETAIL OF GABION MESH RECESSED INTO
CONCRETE BLOCKS GROOVES**



BLOCKS CUT TO FIT CORNERS



BLOCKS CUT & SHAPED TO FIT CORNERS



**BASE COURSE GABIONS ARE ROCK-FILLED &
READY FOR HORIZONTAL BLOCKS LAYER**



**BLOCKS PLACED HORIZONTALLY
ON GABION WALL SETBACK**



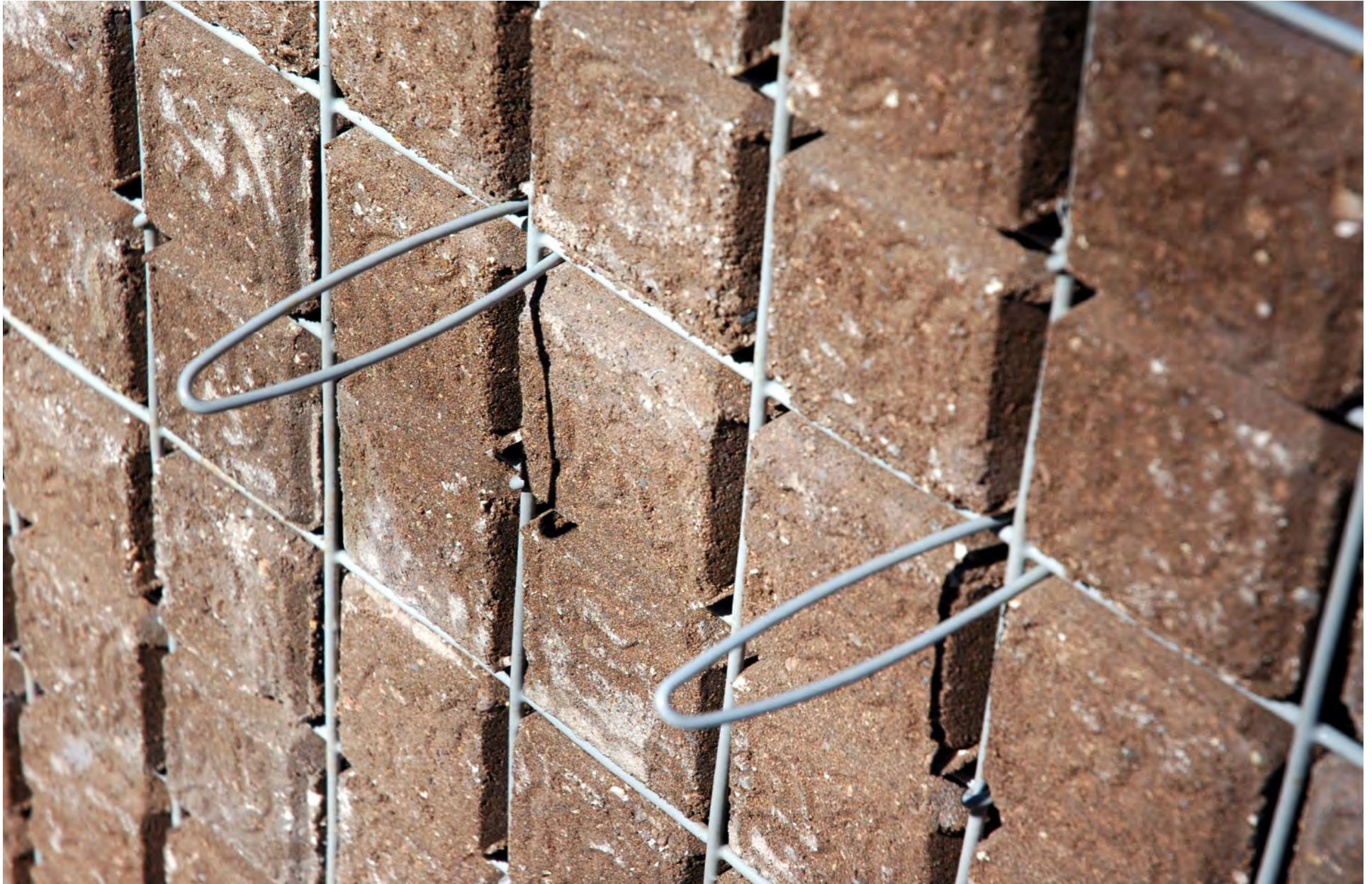
**GABION MESH SECURES HORIZONTALLY PLACED
BLOCKS IN THEIR POSITION**



**TOOL DESIGNED TO RECESS GABION MESH INTO
BLOCK GROOVES**



**SHAPED WIRE CONNECTS FRONT & REAR MESH
PANELS THROUGH BLOCKS DRAINAGE HOLES**



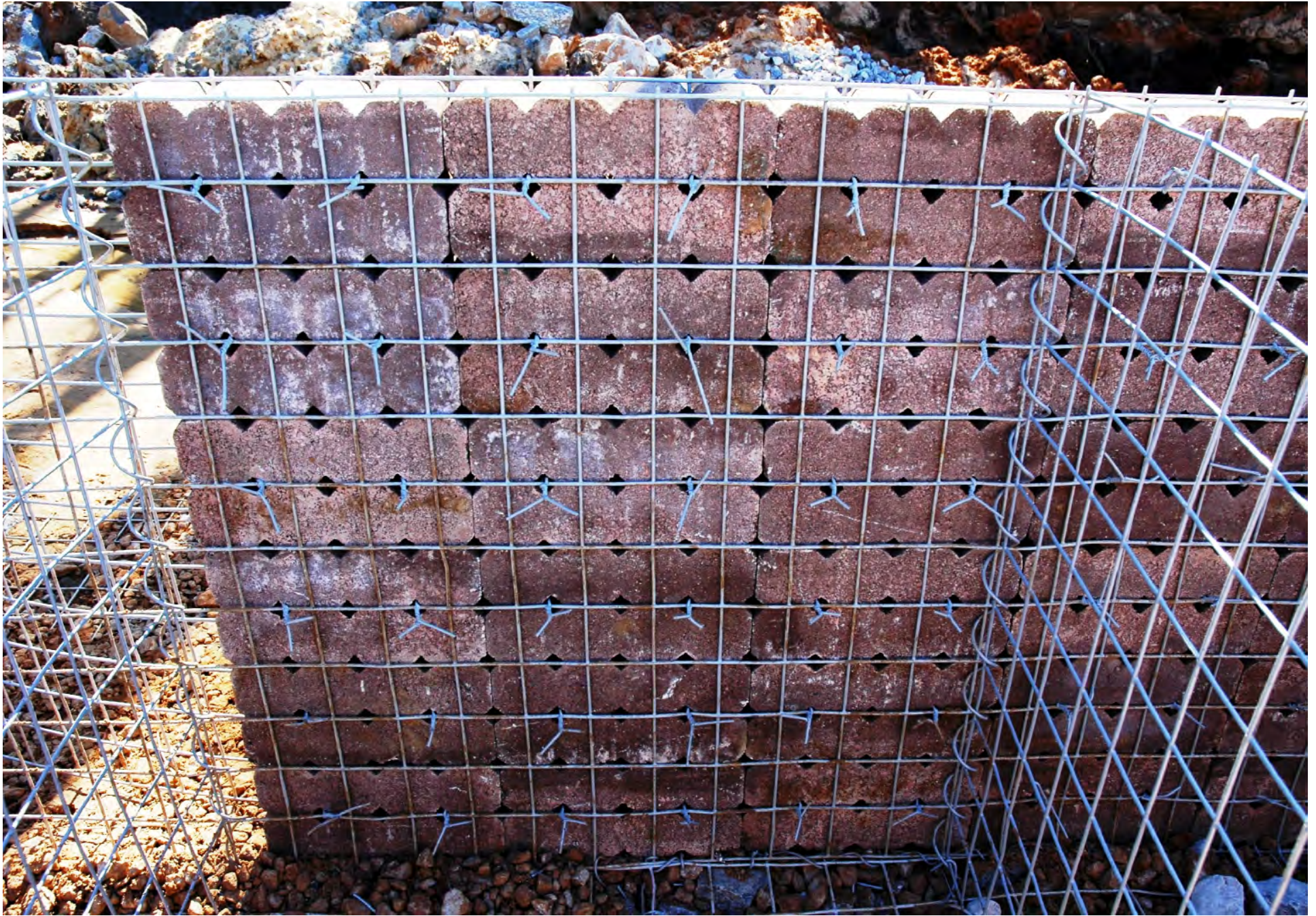
**SHAPED WIRES PLACED AT 6" O.C. THROUGH
CONCRETE BLOCKS DRAINAGE HOLES**



**SHAPED WIRE TIES FASTENED TO
REAR MESH PANEL**



TWO MAN CREW SECURES BLOCKS IN PLACE



REAR VIEW OF CONCRETE BLOCKS INSTALLED



**CORNER DETAIL OF CONCRETE “RAGAZZO”
BLOCKS FACED GABION WALL**



**SECOND TIER BLOCKS FACED GABION WALL
BEING INSTALLED**



**DETAIL OF A CONCRETE BLOCK
NOTICE THE DRAINAGE HOLES**



SPIRAL BINDERS – VERTICAL JOINTS



COMPLETED WALL SECTION



**SECTION OF CONCRETE “RAGAZZO” BLOCKS
FACED GABION WALL NEAR COMPLETION**



**SECTION OF CONCRETE “RAGAZZO” BLOCKS
FACED GABION WALL COMPLETED**

ECOMATTRESS

**12 in. thick PVC coated Gabion
Mattress partially rock filled,
saturated with top soil, seeded
and covered with a coconut
fiber mat before closing with
Gabion mesh lid.**



**GRADING THE SLOPE FOR A 12" HIGH
"ECOMATTRESS"**



PLACING GEOTEXTILE & “ROLL-STOCK” PVC MESH FOR THE ECOMATTRESS BASE



**FORMING JOINTLESS ECOMATTRESS WITH BASE
& LONGITUDINAL DIVIDER PANELS**



**3' LONG PVC SPIRAL BINDERS FASTEN
DIVIDER PANELS TO BASE PANELS**



**3' LONG PVC SPIRAL BINDERS FASTEN
TRANSVERSE DIVIDER PANELS TO BASE PANEL**



**PVC SPIRAL BINDERS FASTEN LONGITUDINAL
DIVIDERS TO TRANSVERSE PANELS**



**GEOTEXTILE PREVENTS SOIL MIGRATION
ECOMATTRESS IS PARTIALLY ROCK-FILLED**



**ECOMATTRESS IS SATURATED
& LEVELED WITH TOP SOIL**



TOP SOIL SEEDED WITH SELECTED GRASS SEED



**TOP SOIL IS IRRIGATED FOR COMPACTION
TOP SOIL IS ADDED AS REQUIRED**



**WIRE TIES ARE PLACED ALONG TOP OF DIVIDERS
FOR FASTENING TO ECOMATTRESS MESH LIDS**



**COCONUT FIBER BLANKET PLACED OVER
TOP SOIL FOR GRASS GROWTH SUPPORT**



**PVC GABION MESH
SECURES TOP OF ECOMATTRESS**



ECOMATTRESS LID FASTENED TO DIVIDER'S TOP



**ECOMATTRESS IRRIGATION
HELPS GRASS SEED GERMINATION**



GRASS GROWTH BEGINS IN TWO WEEKS TIME



ECOMATTRESS GRASS CONTINUES TO GROW



A VIEW OF THE ECOMATTRESS OVER THE CONCRETE BLOCKS FACED GABION WALL



**ECOMATTRESS GIVES THE ENGINEER HIS
CHOICE OF VEGETATION GROWTH DESIRED**

STAINLESS STEEL WIRE MESH GABIONS

**UTILIZED IN MARINE WORKS,
COASTAL PROTECTION, SEA
WALLS, HEAVILY POLLUTED
WATERS AND WHEREVER
HEAVY ABRASION IS
PREVALENT**

**FAMILY CAMP SHORELINE
STABILIZATION
PATRICK AFB, FLORIDA**

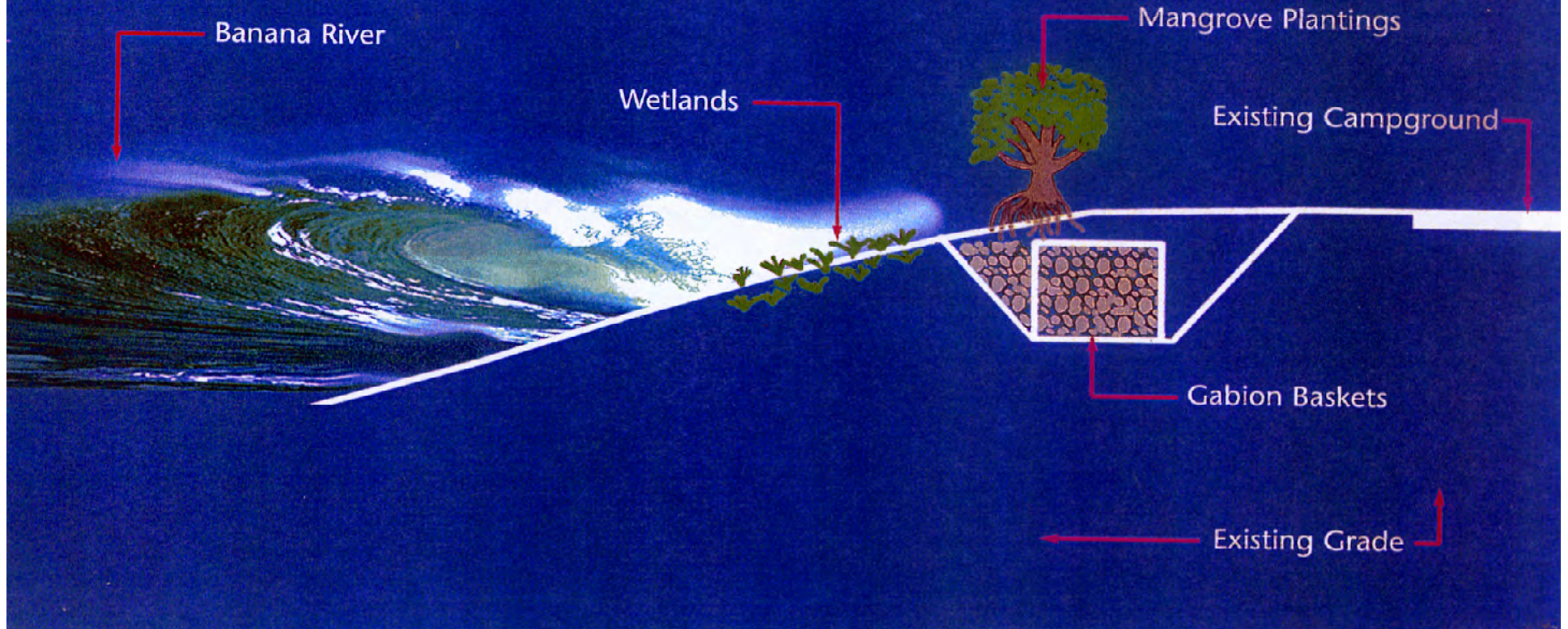
Client:

**45CES/CECC, U.S. Air Force
Patrick AFB, Florida
Completed: June 30, 2001**

Value of work for which AMEC was responsible: \$500,000

SHORELINE EROSION CONTROL

Typical Cross-Section



GABIONS ARE PLACED BELOW BEACH LEVEL



**DETAIL OF STAINLESS STEEL GABIONS PLACED
BELOW THE WATER TABLE**



GABION ROCK-FILLING



**GABION WALL WRAPPED IN GEOTEXTILE AND
PLACED BELOW THE BEACH LEVEL**



TREES TO BE PLANTED INSIDE THE SONOTUBES



**PROJECT COMPLETED – SHORELINE AND
WETLANDS PROTECTED WITH GABIONS**



STAINLESS STEEL WIRE GABION SEA WALL



**GABION SEA WALL SURVIVED CATEGORY 3
HURRICANES: IVAN 9-04 & DENNIS 7-05**



**STAINLESS STEEL WIRE GABIONS AT M.I.T.
CAMPUS LANDSCAPING STRUCTURES**



**M.I.T. CAMPUS - S. S. GABION WALLS
CONSTRUCTION DETAIL**



**M.I.T. CAMPUS ARCHITECTURAL LANDSCAPING
S. S. GABIONS DETAIL**



**M.I.T. CAMPUS CAMBRIDGE, MA
S. S. WIRE GABIONS LANDSCAPING DETAIL**

LAND RECLAMATION PROJECT
MISSOURI DEPARTMENT
OF
NATURAL RESOURCES
LAND RECLAMATION COMMISSION





**GABION WEIR STEPS REPLACED WITH STAINLESS
STEEL WIRE MESH DUE TO SOIL ABRASION**



NEW S. S. WIRE GABION WEIR REPLACING THE PREVIOUS ONE FAILED DUE TO SOIL ABRASION



**DOWNSTREAM VIEW OF THE NEW STAINLESS
STEEL WIRE GABION WEIR**



**AUDUBON LAKE BIRD SANCTUARY
GABION BREAKWATERS BUILT AROUND ISLANDS**

**OTHER GABION PROJECTS
CONSTRUCTED WITH
“ROLL-STOCK”
CONTINUOUS
JOINTLESS
GABIONS**

BANK STABILIZATION - CAPE MAY CANAL, NEW JERSEY USA – U.S. ARMY CORPS OF ENGINEERS







GABION MATTRESS UNDERWATER PLACEMENT



MEMPHIS AIRPORT – HURRICANE CREEK



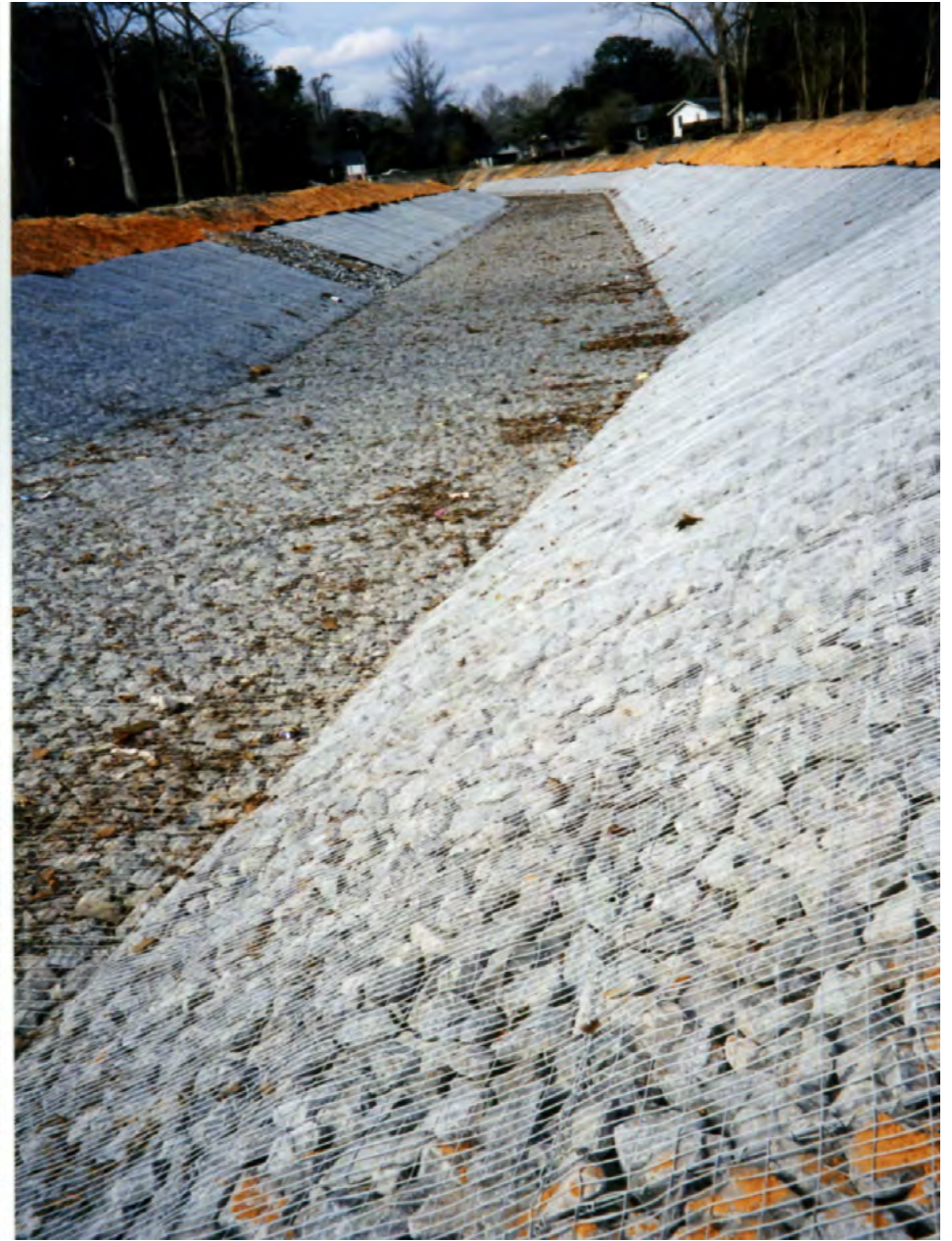
TYPICAL GABION WALL



TOMBIGBEE RIVER – BANK PROTECTION



DIVERSION DAM PECOS RIVER



NALL STREET - GABION CHANNEL LINING



MOUNTAIN BROOK – GOLF COURSE



SPRING CREEK - FLOOD CONTROL



SAN MARCOS RIVER – LULING, TX

PRESENTER:

GEORGE RAGAZZO

PHONE: (251) 380-0332

CELL: (251) 422-3536

MODULAR GABION SYSTEMS

gragazzo@gabions.net

AAR AT CARTERS DAM

DIFFERENT APPROACHES
(ONE OLD, ONE NEW)

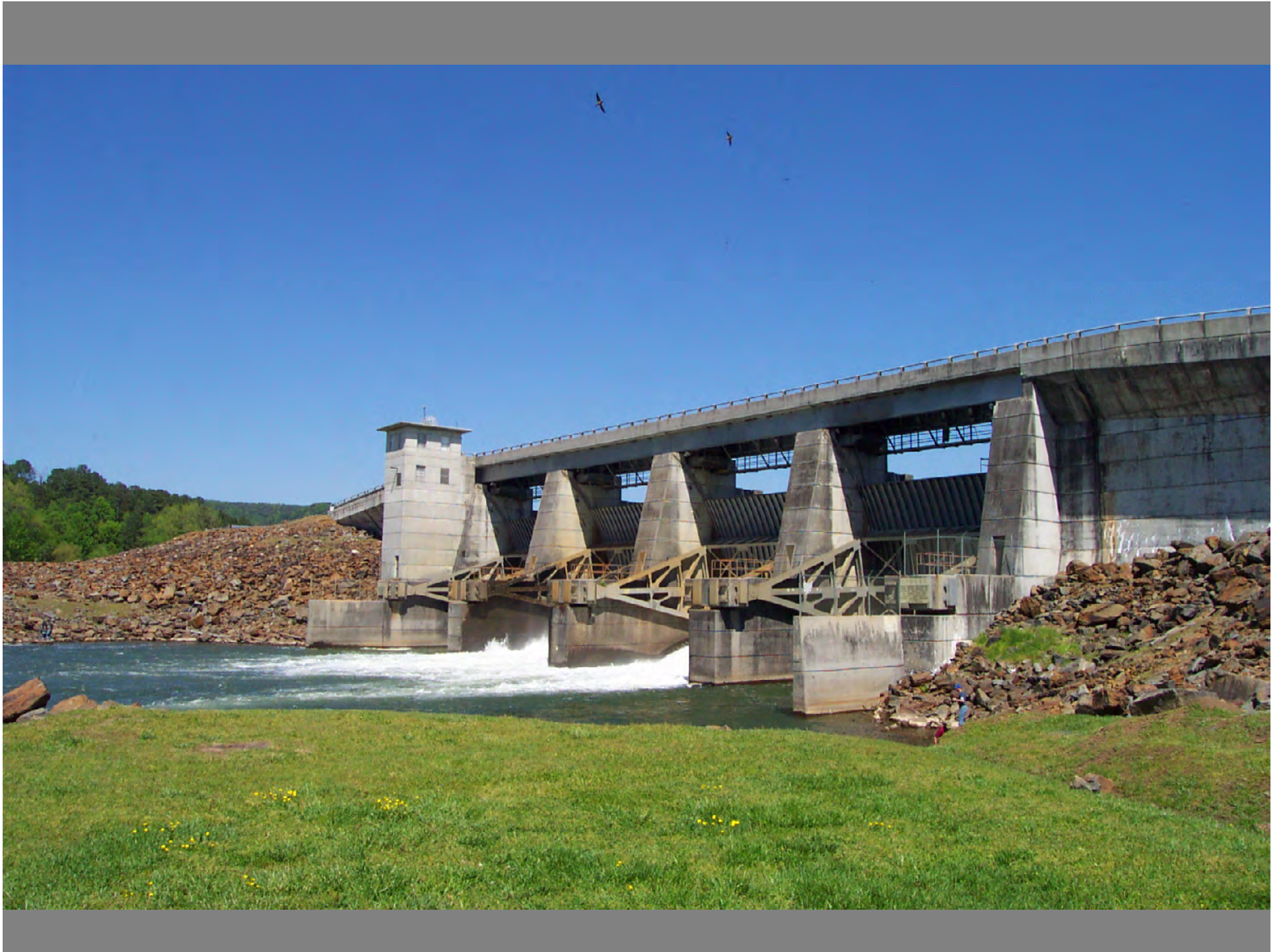




Carters Main and Reregulation Dams







A large, light-colored stone sign with the word "Vulcan" in large, blue, 3D block letters. Below it, "Materials Company" and "Dalton Quarry" are written in smaller blue letters. The sign is set against a background of trees and a stone wall.

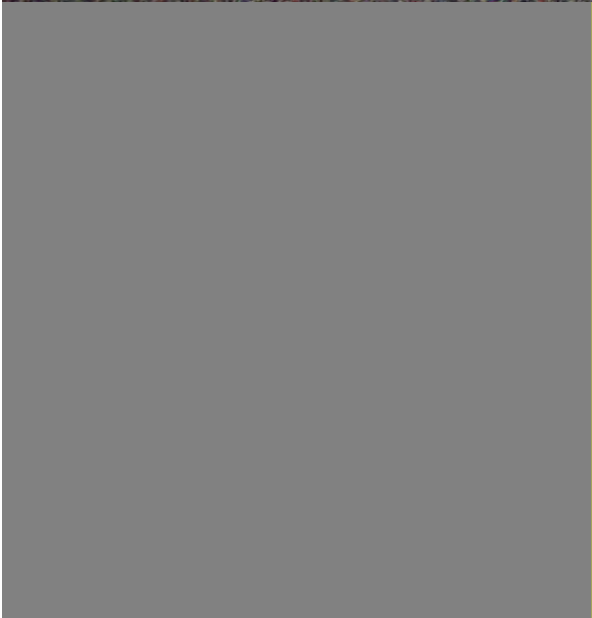
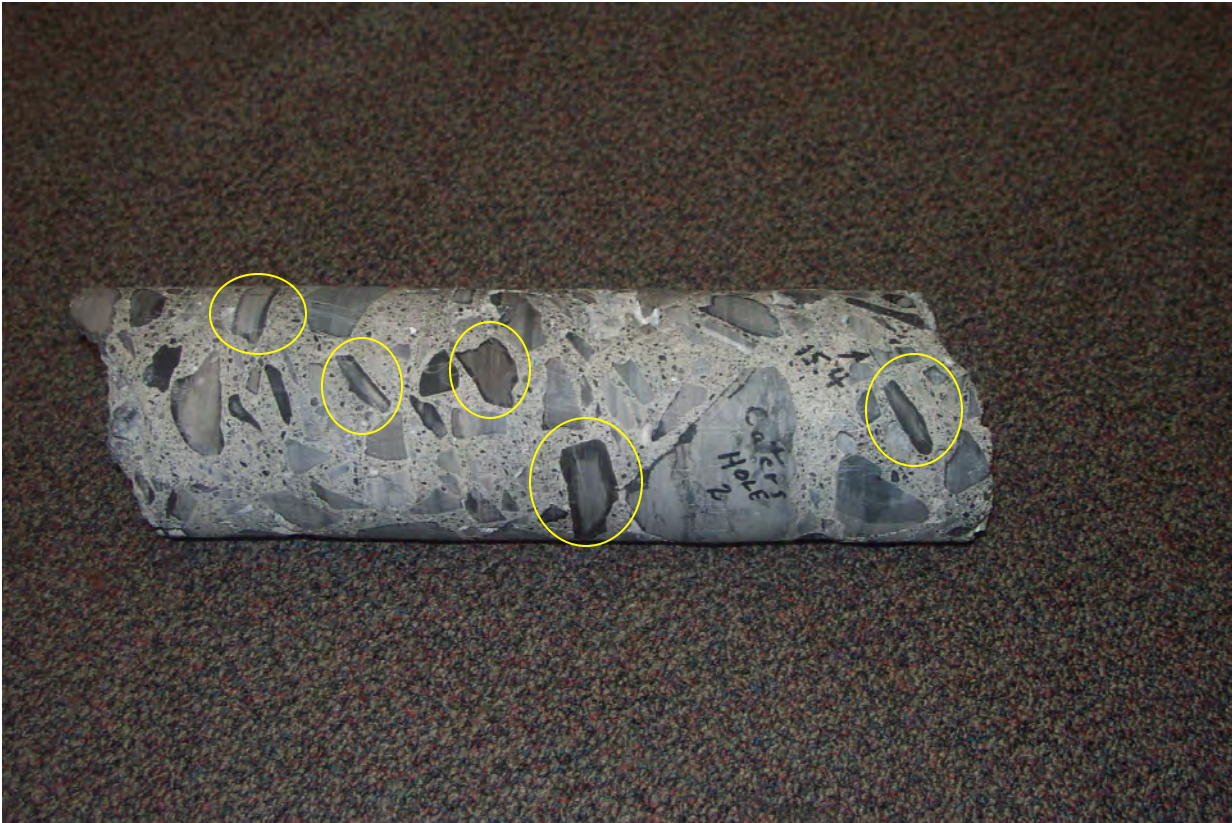
Vulcan

Materials Company

Dalton Quarry





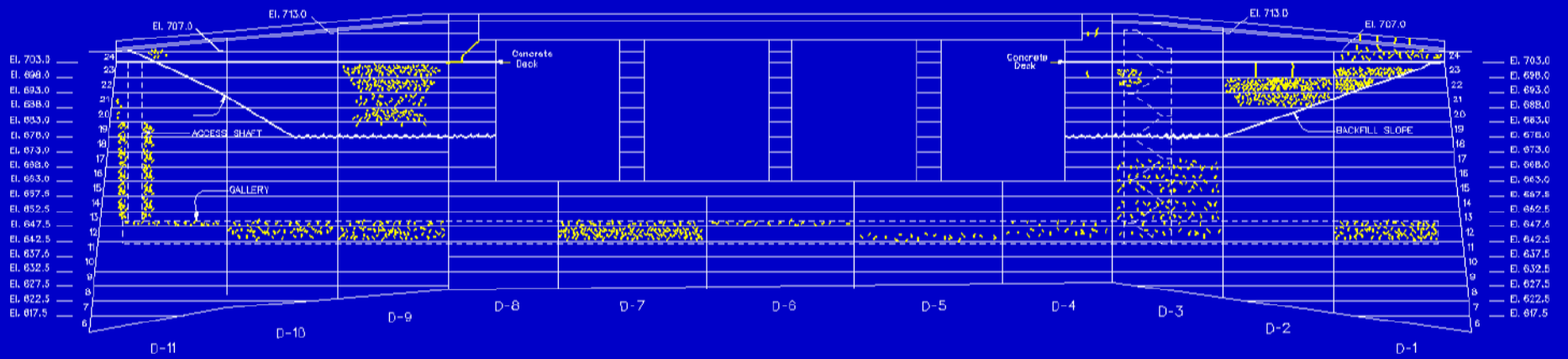












CARTERS REREGULATION DAM - UPSTREAM ELEVATION

LEGEND

 OBSERVED MAP CRACKING





Weather <u>FAIR</u>		U. S. ARMY ENGINEER DISTRICT, MOBILE			
Air <u>70</u> °F to <u>76</u> °F		CONCRETE PLACING CARD		DATE: <u>APRIL 21, 1971</u>	
Conc <u>72</u> °F to <u>77</u> °F					
CONCRETE PLACED			LOCATION		
TYPE	1ST SHIFT RECEIVED	2ND SHIFT RECEIVED	3RD SHIFT RECEIVED	<u>MONO D-9, LIET 9</u>	
GROUT	<u>2</u>			PAY ITEM NO.	
3, 4" 3000				<u>15, 11, 19, 12B</u>	
1 1/2" 3000				TIME	
1 1/2" 4000				STARTED	COMPLETED
3" 3000 <u>2000</u> <u>353</u> ⁵				<u>1245</u>	<u>2020</u>
3" 4000				ELEVATION	
6" EXTERIOR				BOTTOM	TOP
6" INTERIOR				<u>ROCK</u>	<u>632 5</u>
REJECTED MATERIAL				<u>CHAS MORGAN</u> INSP	
TIME	TYPE	QUANTITY	REASON	_____ INSP	
		<u>0</u>		_____ INSP	
				_____ INSP	

MOB FORM **710**
APR 67

Use other side for remarks or sketch

Weather P. Cloudy's Warm
 Air 80 °F to 85 °F
 Conc 79 °F to 82 °F

U. S. ARMY ENGINEER DISTRICT, MOBILE

CONCRETE PLACING CARD

DATE: MAY 19, 1971

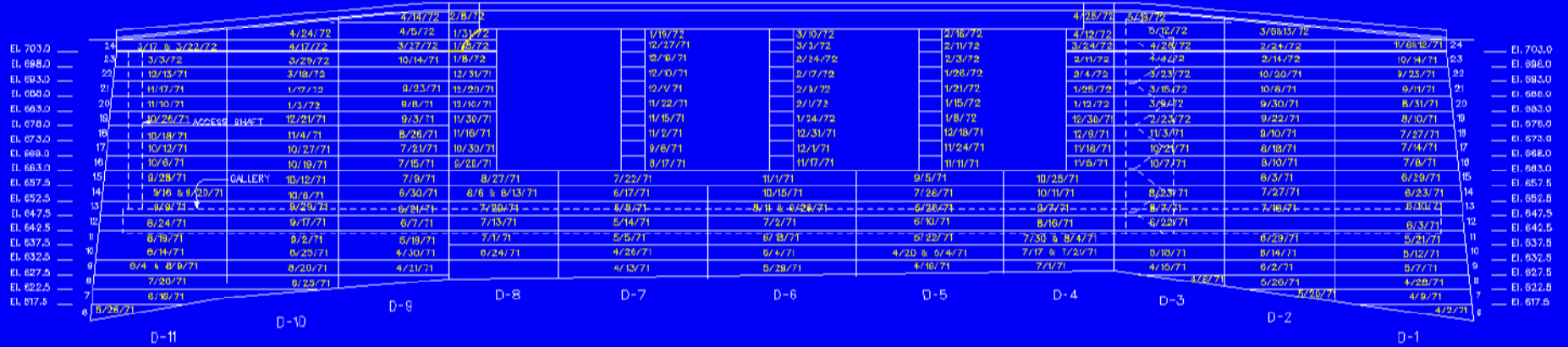
CONCRETE PLACED				LOCATION	
TYPE	1ST SHIFT RECEIVED	2ND SHIFT RECEIVED	3RD SHIFT RECEIVED	MONO D-9, LIFT II	
GROUT	<u>1cy</u>	<u>0</u>	<u>0</u>	PAY ITEM NO.	
3/4" 3000	<u>0</u>	<u>0</u>	<u>0</u>	<u>15 4 19</u>	
1 1/2" 3000	<u>13cy</u>	<u>0</u>	<u>0</u>	TIME	
1 1/2" 4000	<u>0</u>	<u>0</u>	<u>0</u>	STARTED	COMPLETED
3" 3000	<u>80cy</u>	<u>0</u>	<u>0</u>	<u>0115</u>	<u>1625</u>
3" 4000 2000	<u>196</u>	<u>0</u>	<u>0</u>	ELEVATION	
6" EXTERIOR				BOTTOM	TOP
6" INTERIOR				<u>6315</u>	<u>6425</u>

REJECTED MATERIAL			
TIME	TYPE	QUANTITY	REASON
	<u>NONE</u>		

CHARLES MORGAN INSP
KANON INC, DAKW01-71-C-0014
 _____ INSP
 _____ INSP

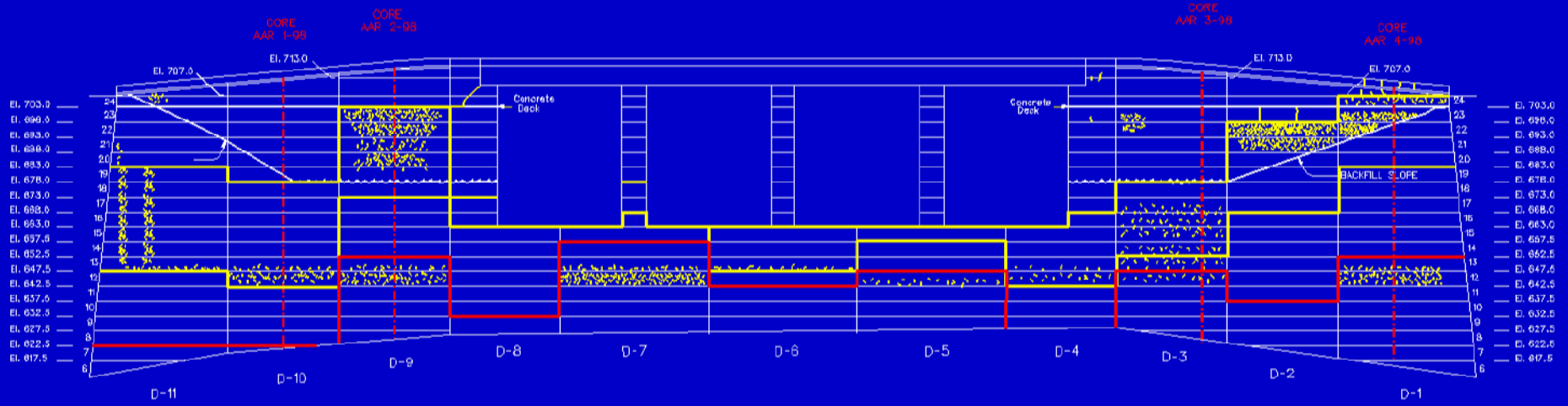
MOB FORM 710
 APR 67

Use other side for remarks or sketch



CARTERS REREGULATION DAM - UPSTREAM ELEVATION

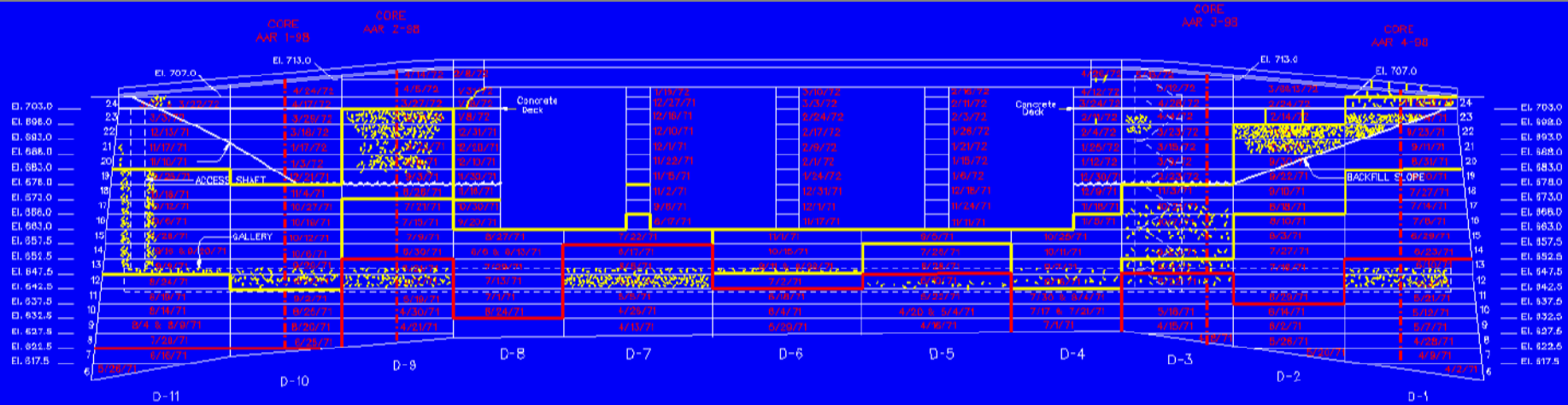
LEGEND



CARTERS REREGULATION DAM - UPSTREAM ELEVATION

LEGEND

- █ 11/9/71 Zone of most intense AAR
- █ OBSERVED MAP CRACKING



CARTERS REREGULATION DAM - UPSTREAM ELEVATION

LEGEND

8/4/71
 Zone of most intense AAR
 8/3/71 8/4/71

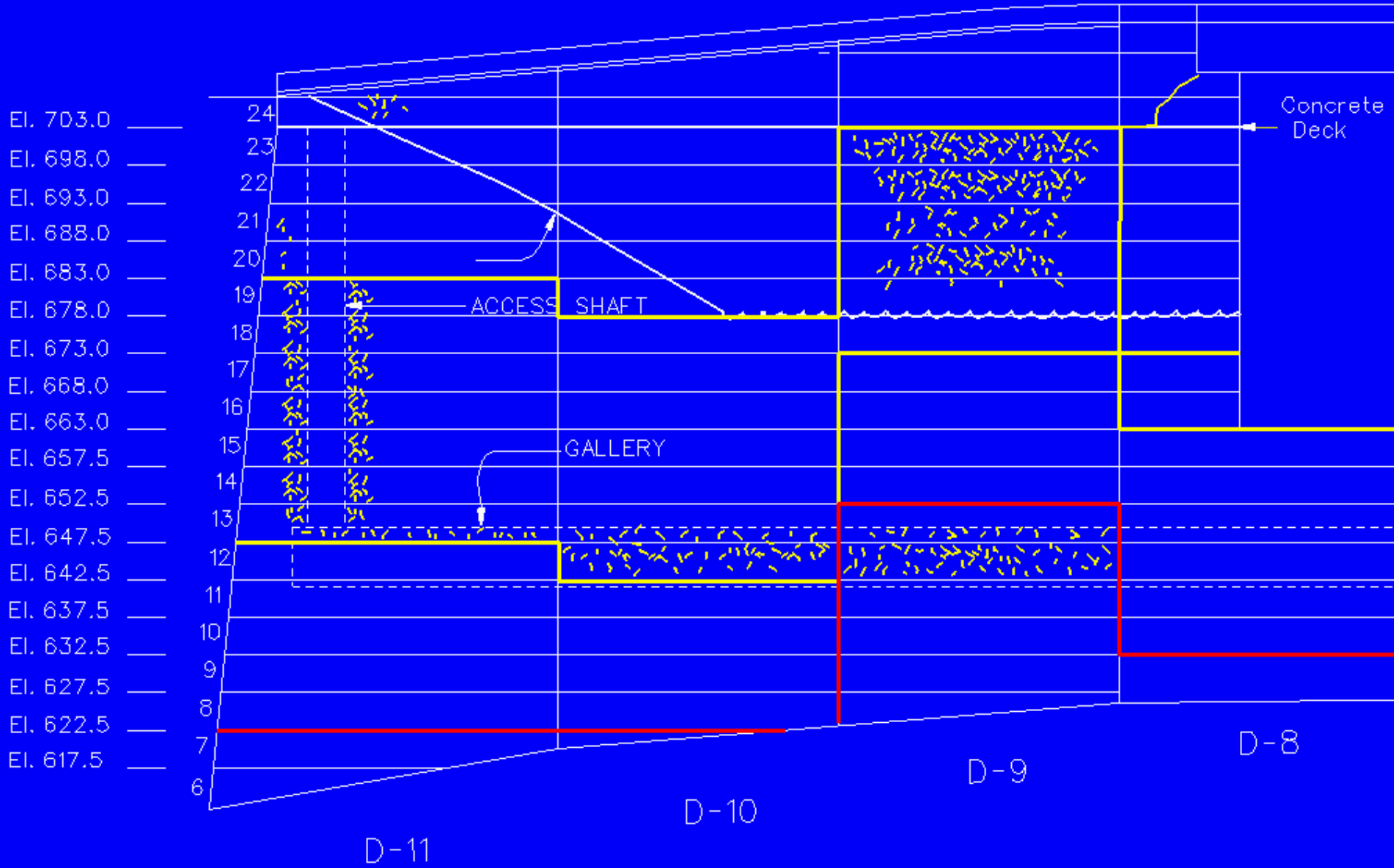
8/4/71 CONCRETE PLACEMENT DATE
 OBSERVED MAP CRACKING

STRUCTURAL CRACKS
 REPAIR CONCRETE

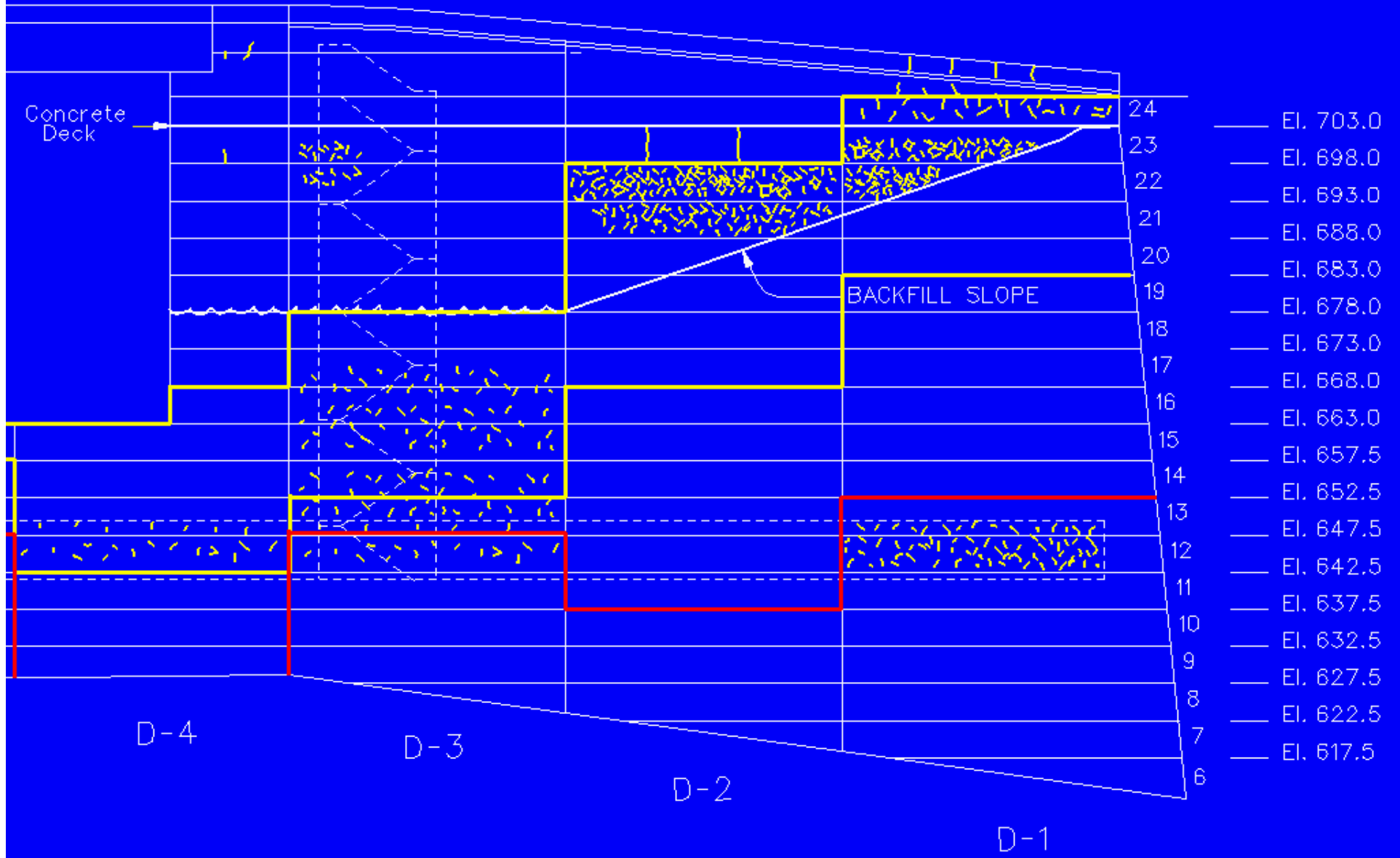
Note: AAR data points in concrete placed prior to 8/22/71

3" AGGREGATE, 3000 PSI CONCRETE
 PG CORE HOLE
 3" AGGREGATE, 2000 PSI CONCRETE

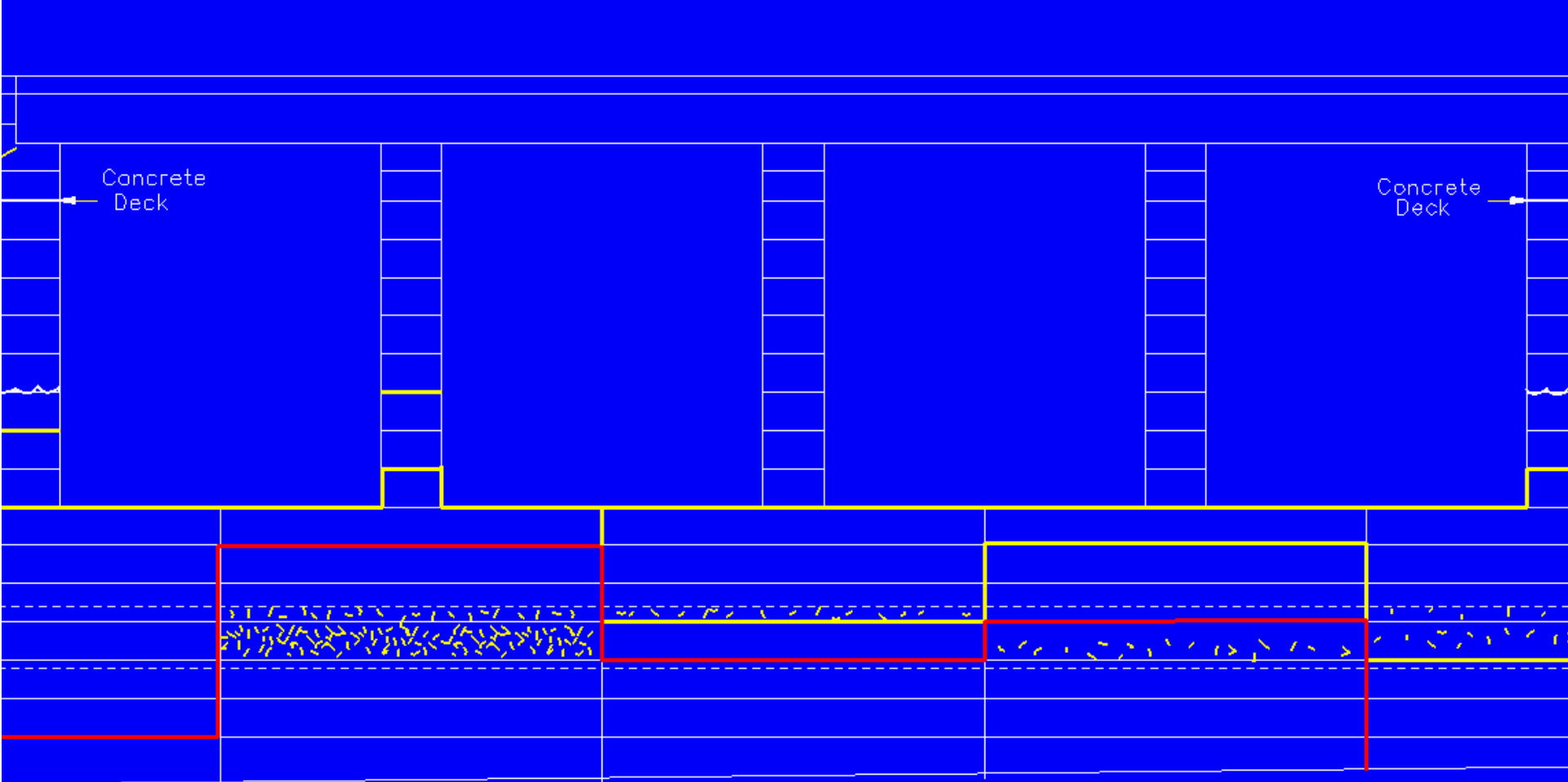
Left Abutment



Right Abutment



Gate Area



D-8

D-7

D-6

D-5

D-4

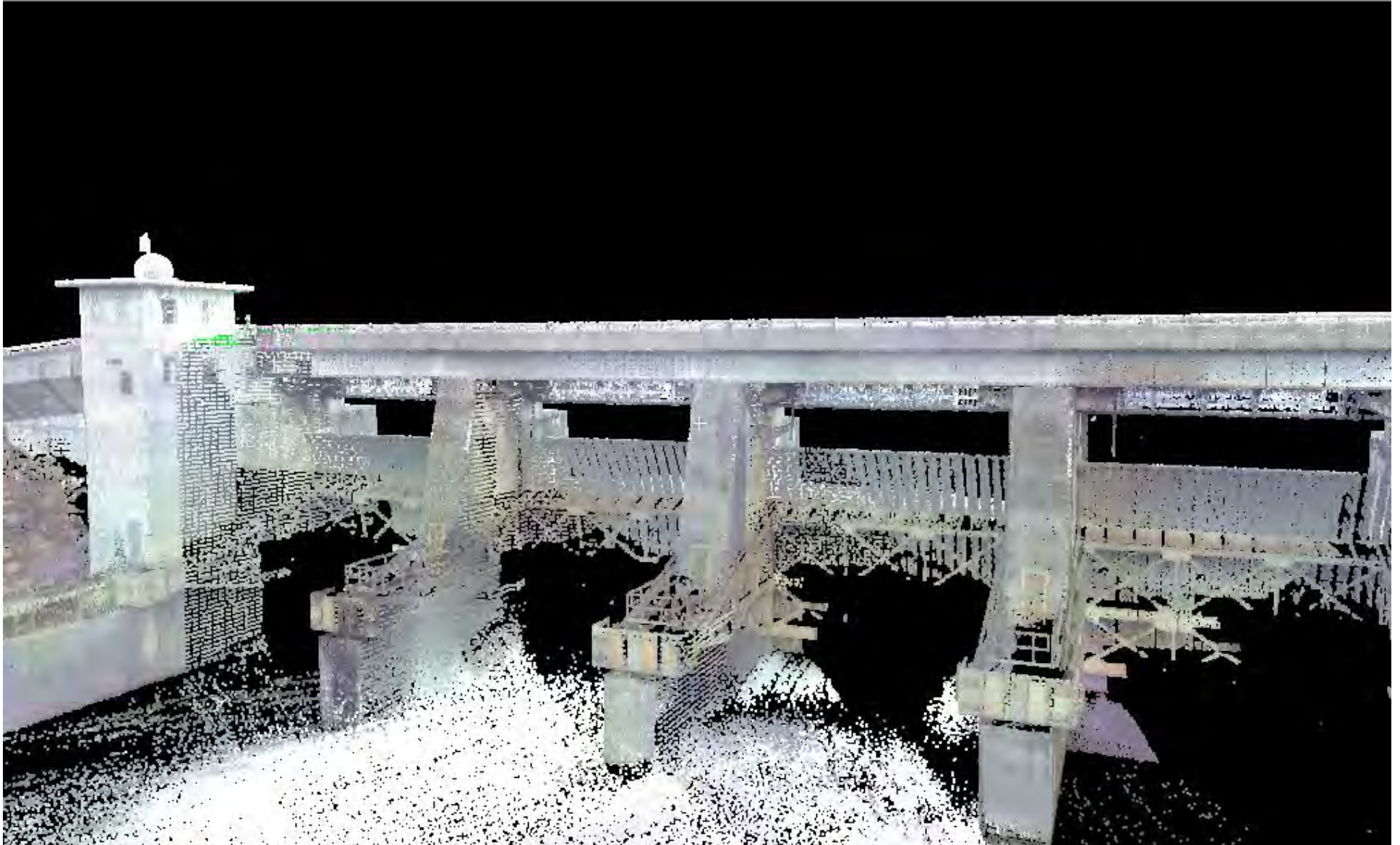
Lidar Survey



Lidar Survey

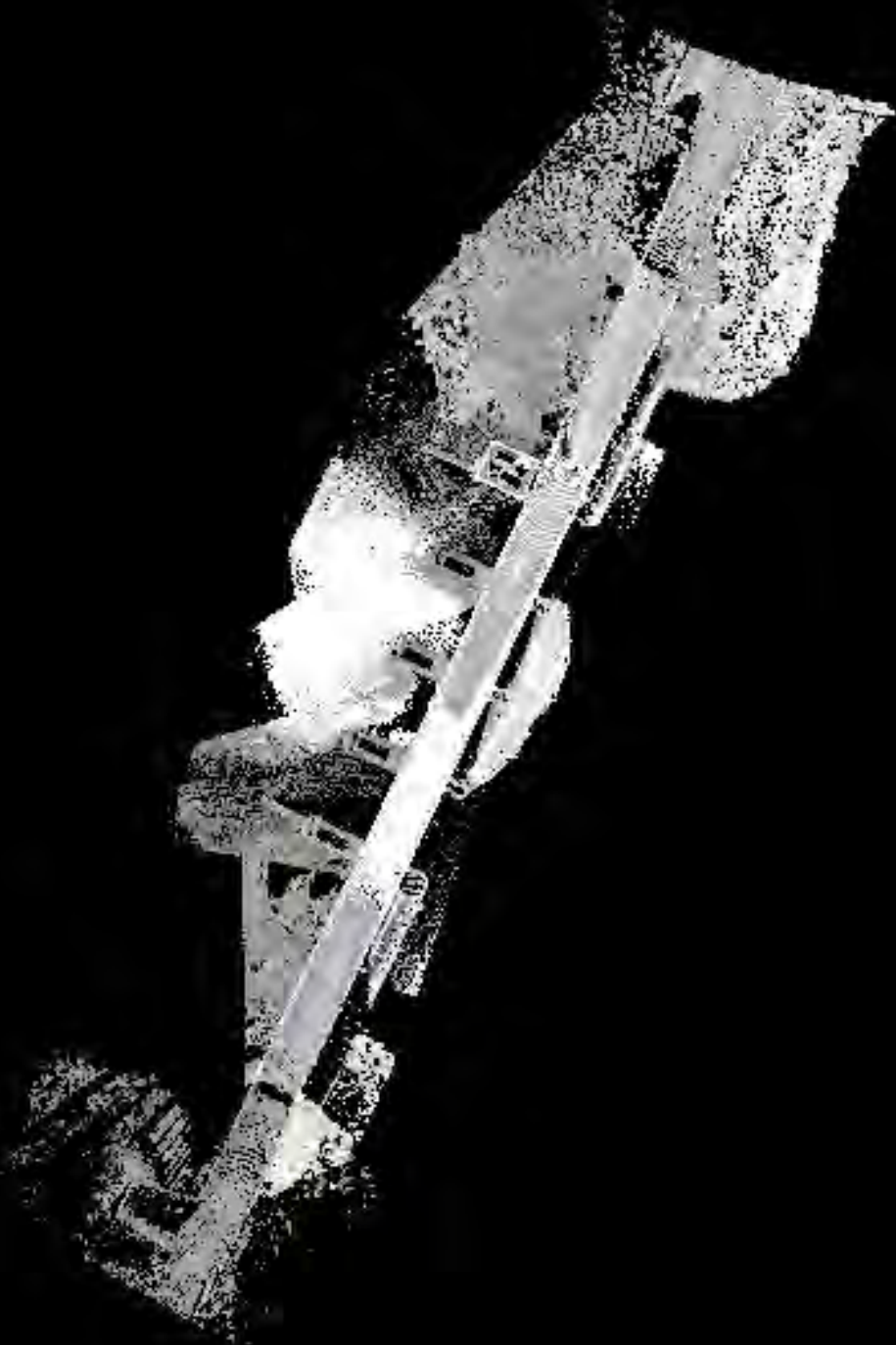


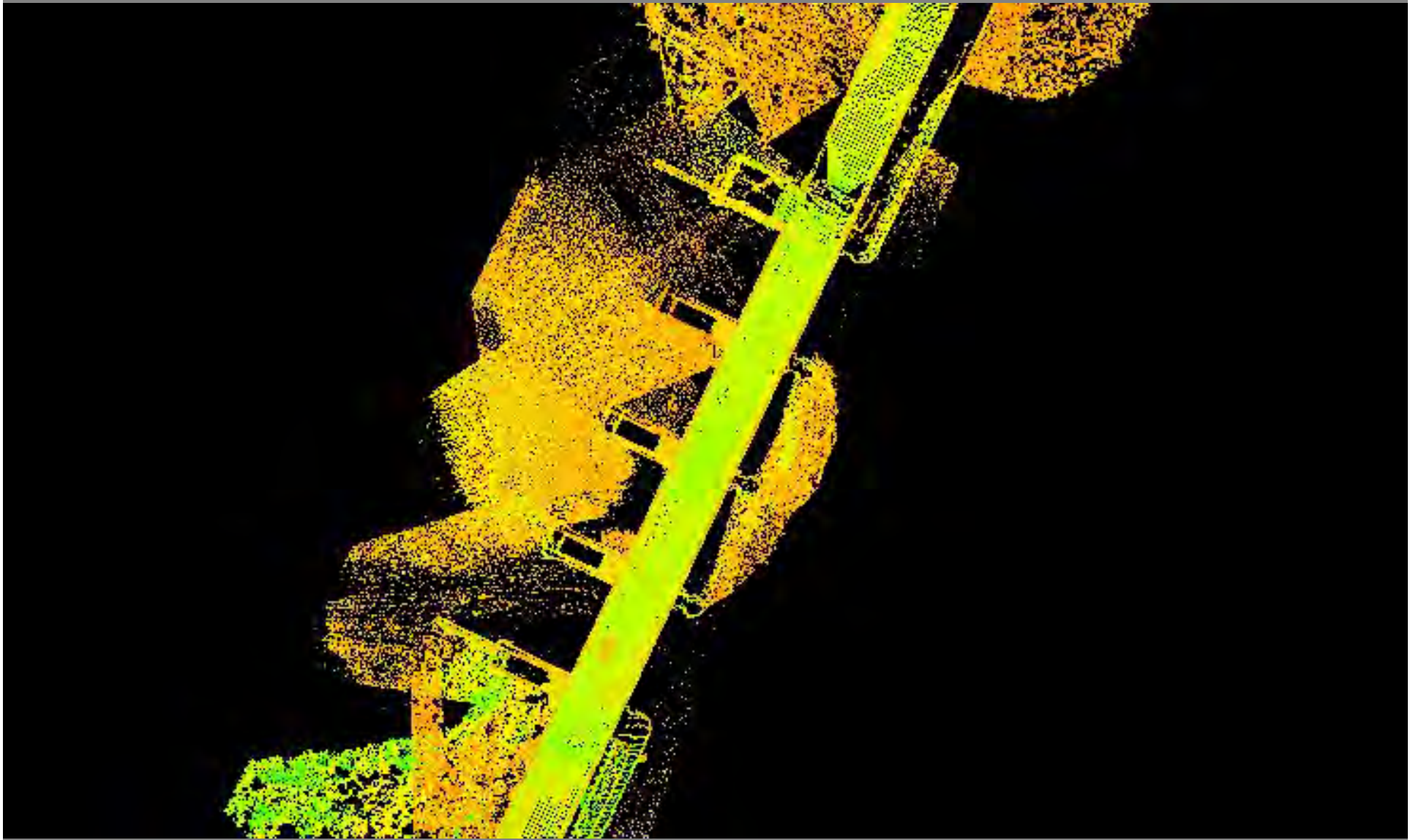
Cyclone oblique view



CloudWorx

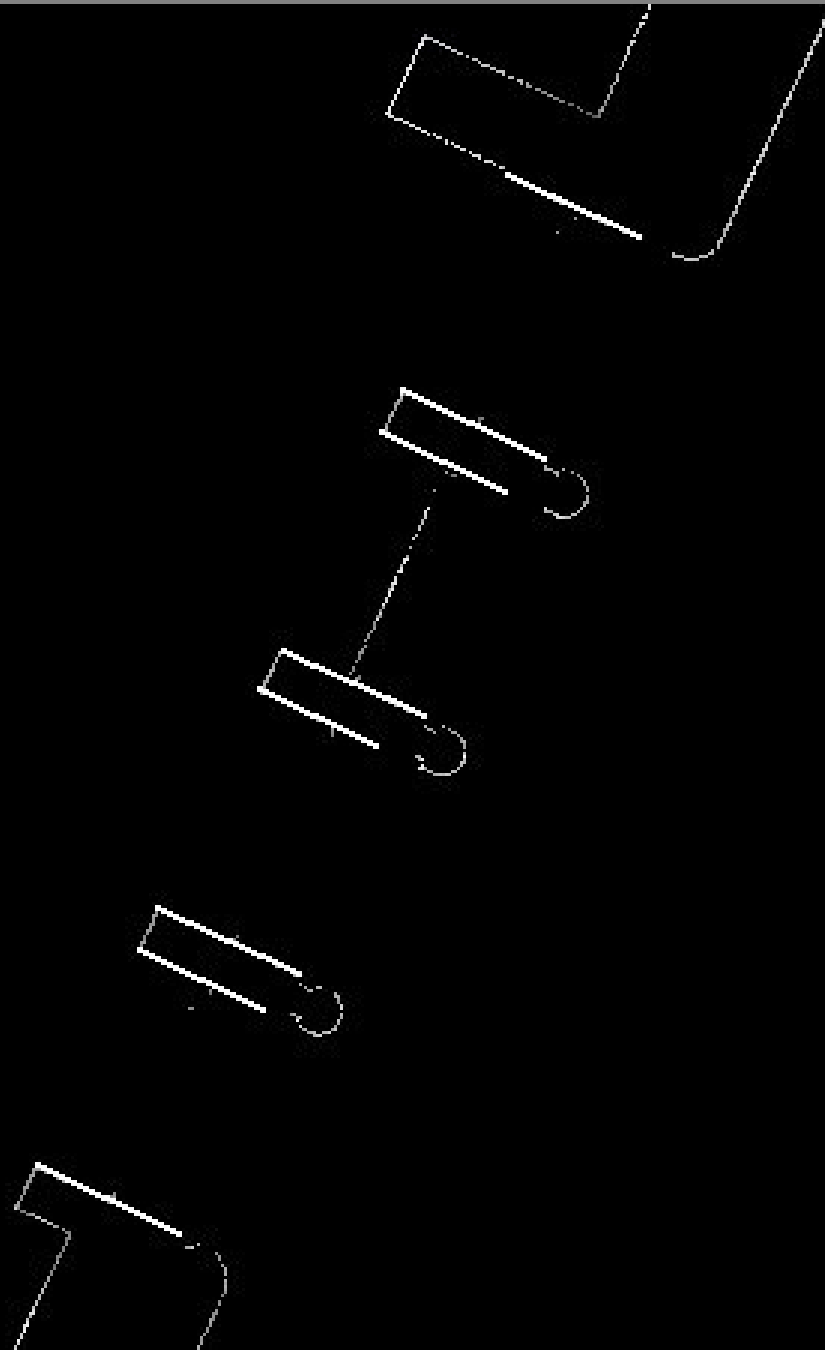
Plan View





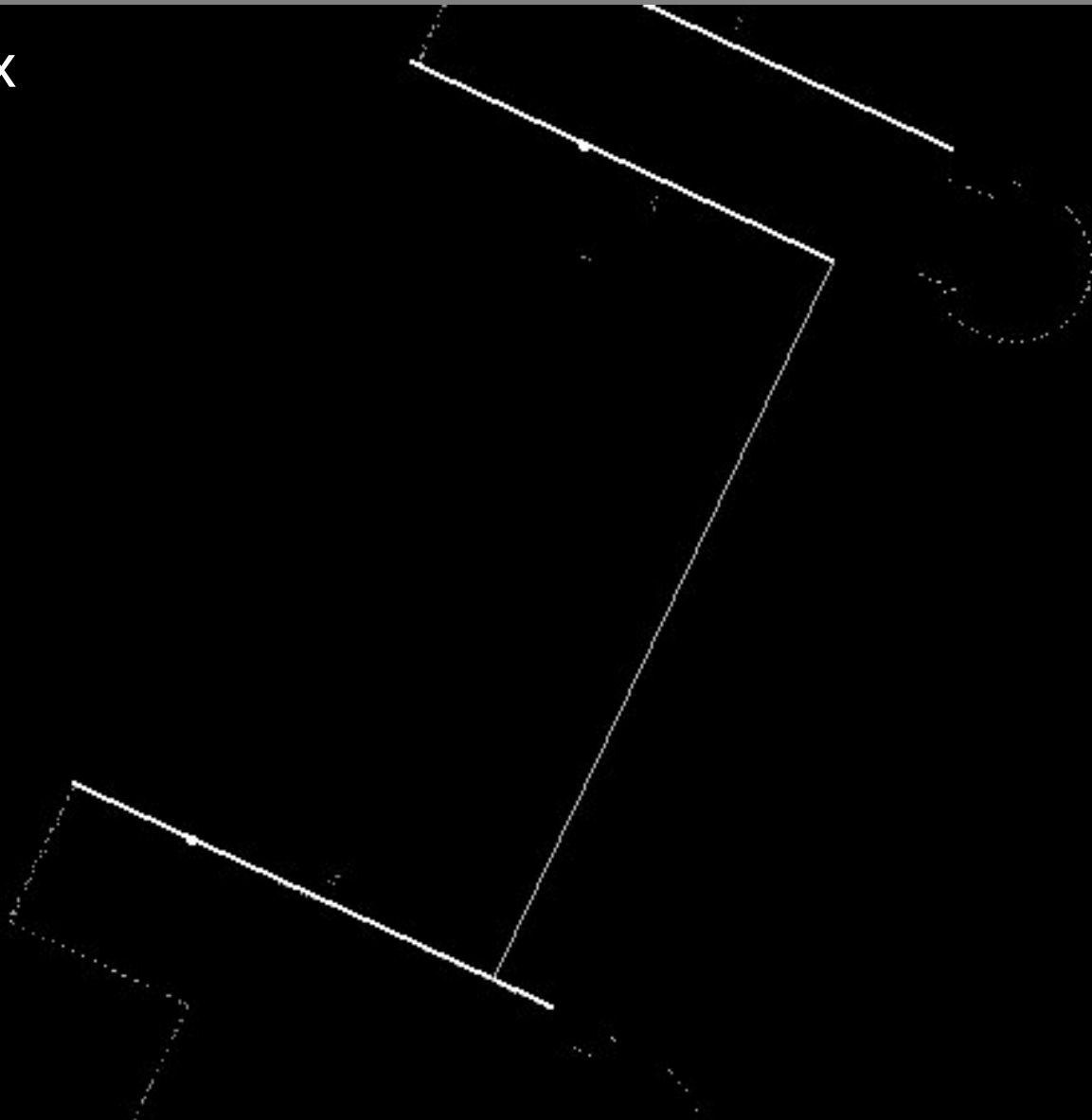
CloudWorx

Horizontal Slice




CloudWorx

Fit Line



Horizontal Slice

Elevation 664-665



Gate 1 = 42.000'

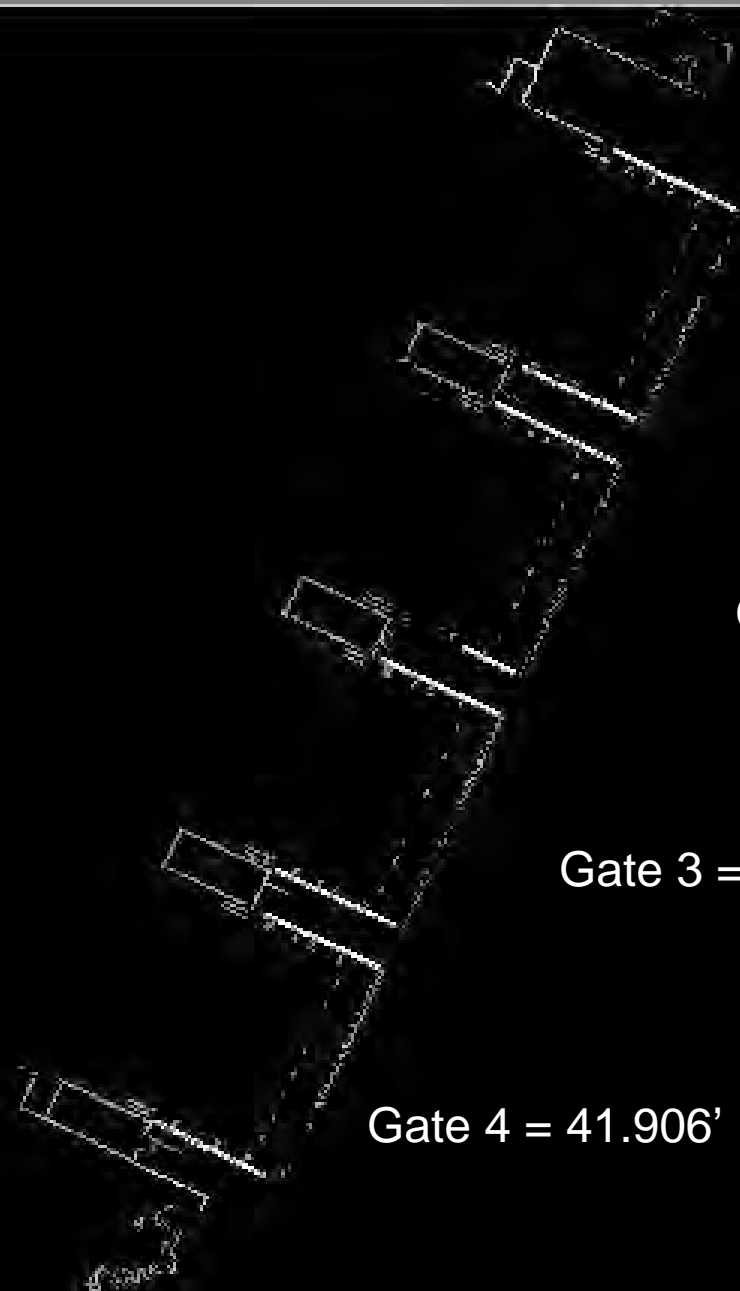
Gate 2 = 42.004'

Gate 3 = 42.000'

Gate 4 = 41.909'

Horizontal Slice

Elevation 679-680



Gate 1 = 41.945'

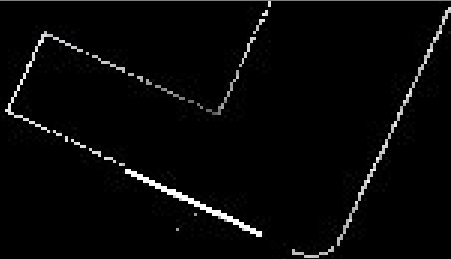
Gate 2 = 41.914'

Gate 3 = 41.969'

Gate 4 = 41.906'

Horizontal Slice

Elevation 699-700



Gate 1 = 41.875'



Gate 2 = 41.942'



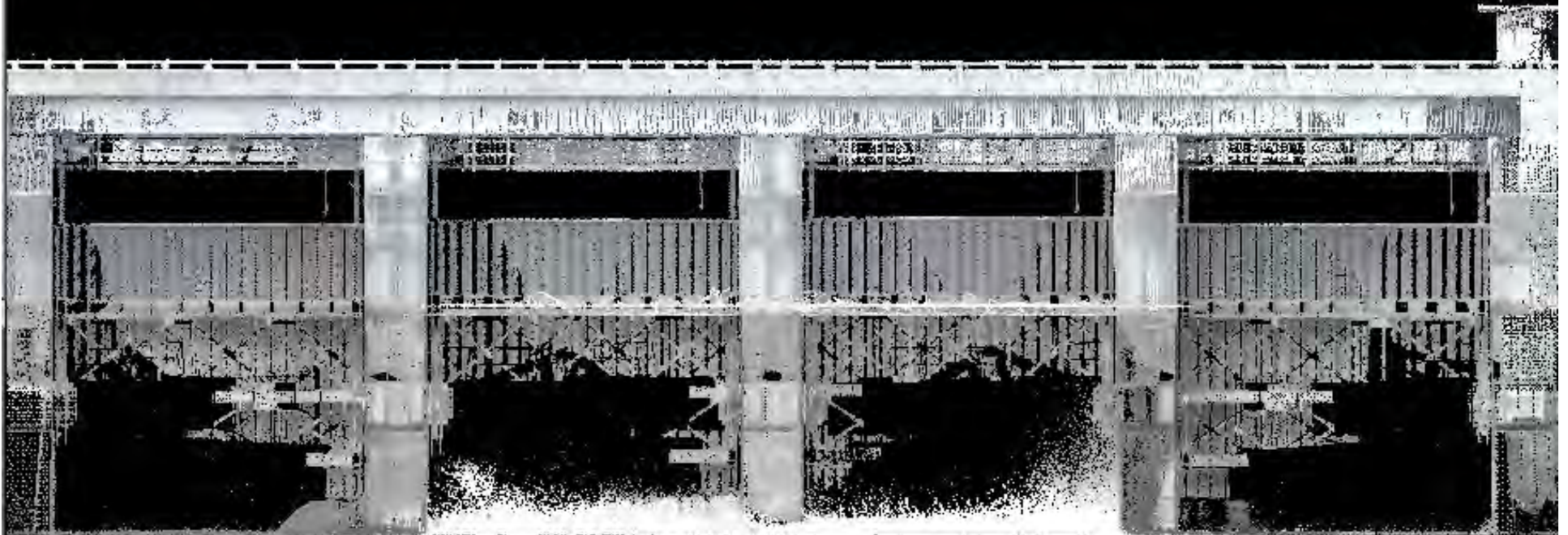
Gate 3 = 41.962'



Gate 4 = 41.862'

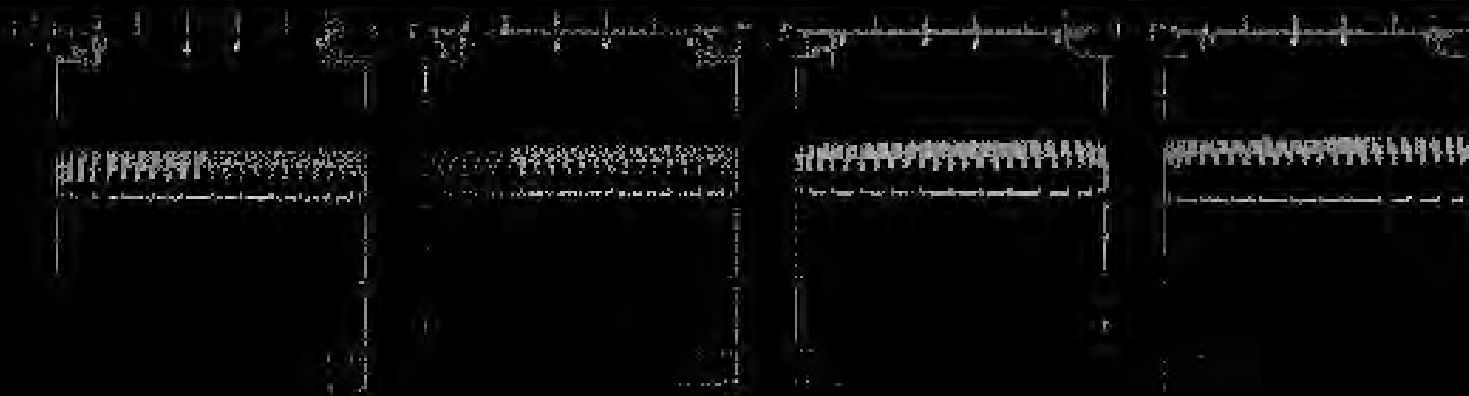
CloudWorx

Front View



CloudWorx

Vertical Slice



Vertical Slice - downstream piers

GATE 4

GATE 3

GATE 2

GATE 1

41.843

41.974

41.985

41.883

41.855

41.973

41.950

41.914

41.886

41.922

41.930

42.012

41.926

41.924

41.881

41.896

41.876

41.867

41.867

41.885

41.892

41.893

41.911

Vertical Slice - upstream piers

Gate 4

Gate 3

Gate 2

Gate 1



41.851

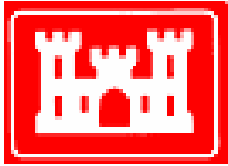
42.018

41.939

41.929







**US Army Corps of Engineers
Engineer Research and Development Center
Vicksburg, Mississippi**



**2005 Tri-Service Infrastructure Systems Conference & Exhibition
August 1-4, 2005**

Rubblization of Airfield Concrete Pavements

By

Eileen M. Vélez-Vega

Research Civil Engineer

Airfields and Pavements Branch



Overview



- **Introduction**
 - FY 03-04 AFCESA Research
 - FY 05 AMC Research
- **FY 03-05 Research Approach**
 - Phase 1
 - Equipment & Procedure
 - Phase 2
 - Highway and Airfield Rubblization Evaluations
 - Cost Analysis
 - Grand Forks Air Force Base Study
 - GF AFB Guidelines and Specifications
 - Runway Reconstruction Project
- **Results and Conclusions**
- **Future Research Studies**
- **Questions**



Rubblization



- **Main Objective:**

- Develop a design procedure and criteria for the design of asphalt overlays over rubblized, and crack and seat PCC pavements.

- Project History:

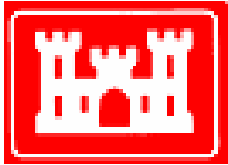
- FY 03-04 AFCESA: Rubblization Design Procedure
- FY 05 AMC: Grand Forks AFB Runway Reconstruction Project

- ***Rubblization...***

- ...is a relatively “new” rigid pavement rehabilitation technique.
- ...eliminates existing slab action by breaking the PCC pavement into small particles ranging from:

- sand size to 75 mm (3 in) at the surface,
- 150 to 230 mm (6-9 in) on the top half,
- 305 to 380 mm (12-15 in) at the bottom half of the PCC layer.

- **Crack and Seat** has almost been replaced with Rubblization due to the significant advantages that it proves to have in the rehabilitation of PCC pavements.

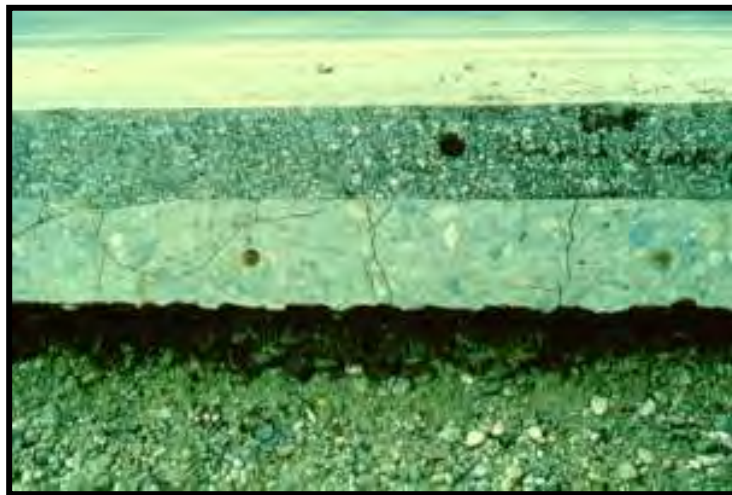


Why Rubblization?



- ***Pavement Distresses***

- Reflective Cracking
- Severe Joint Deterioration
- Slab Settlement
- Excessive Patching
- “Pop-outs”, etc.



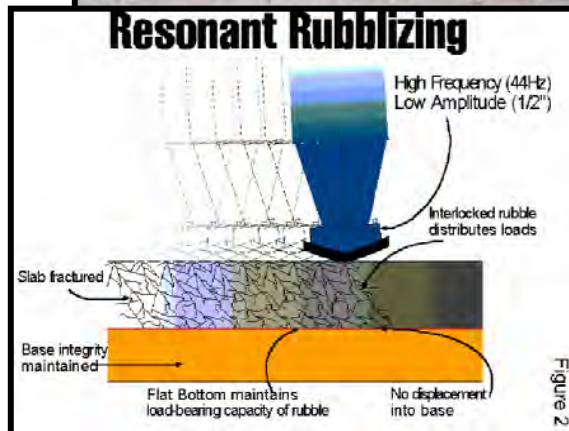


Rubblization Equipment *

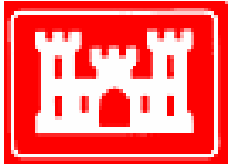


• Current U.S. major contractors:

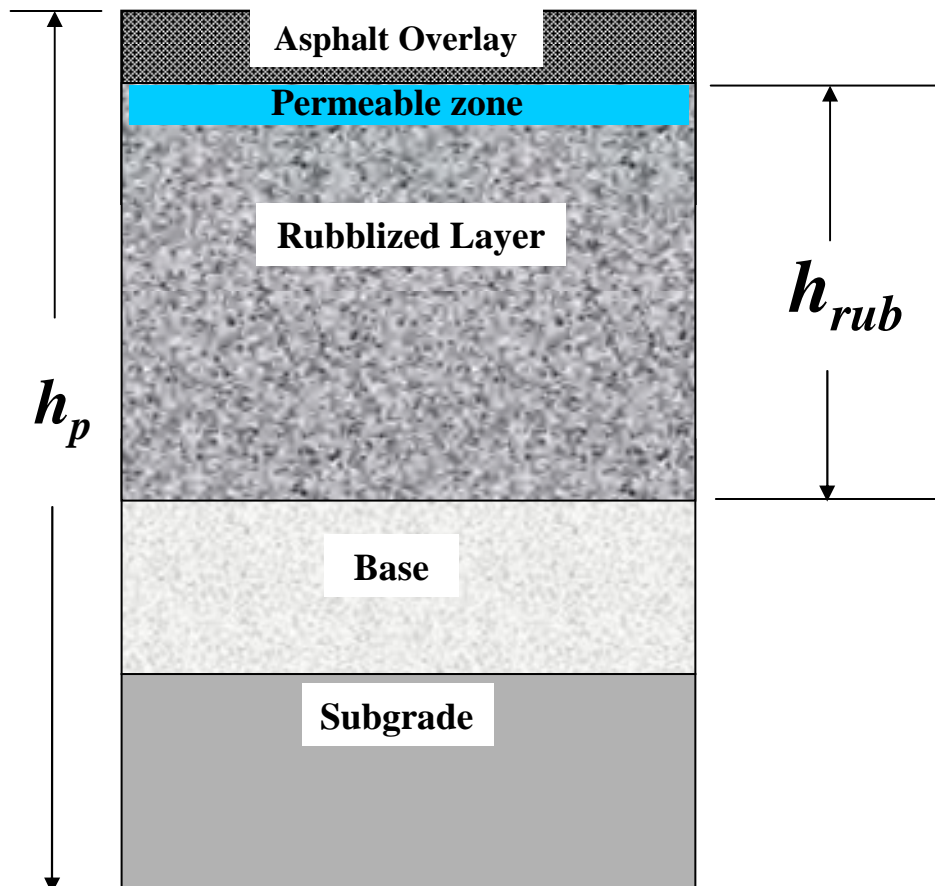
- Resonant Machines Inc. (RMI)
 - Resonant Breaker, RB-500
 - Low Amplitude
 - » 12 to 20 mm (1/2-3/4 in)
 - High Frequency Hammer
 - » 44-47 Hz
- Antigo Construction, Inc.
 - Guillotine Type Breaker
 - 5,440 kg (12,000 lb), 2.4 m (8 ft) hammer
 - Multi-Head Badger Breaker®
 - 16-450 kg (1,000 lb) hammers
 - 4 m (13 ft) wide
 - 1.5 m (5 ft) individual drops



* Pictures from Antigo and RMI Website



Particle Size Distribution



h_{rub} = maximum depth of the slab

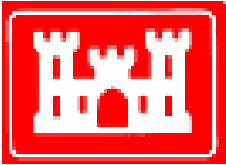
h_p = pavement thickness

RMI Particle Size Specifications:

- Particle Size Range:
Sand size to 6 inches not greater than 1.25 times h_{rub}
- Majority of the pieces:
Sand size to 0.75 times h_{rub}
- For reinforced PCC:
Larger pieces are accepted and reduced to the best possible size.

Antigo Construction Inc. Particle Size Specifications:

- Size Range:
Sand size to 3 inches or less in the top half of the slab.
9 inches or less in the bottom half of the slab.
- For reinforced PCC:
Similar to the RMI Specifications



Highway Rubblization Projects



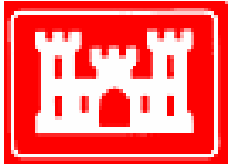
- **I-10 Louisiana Rehabilitation Project**
 - 11.0 km (7-mi) pavement rubblization
 - Contractor: Resonant Machines, Inc.
 - Pavement Structure:
 - 250 mm (10 in) AC O/L
 - 230 mm (9 in) Rubblized PCC
 - Subgrade: Sandy Soil
- **I-65 Alabama Rehabilitation Project**
 - Contractor: Antigo Construction, Inc.
 - Pavement Structure:
 - 280 mm (11 in) AC O/L
 - 250 mm (10 in) Rubblized PCC
 - Subgrade unknown
 - Test Pits required every 305 m (1000 ft)



I-10 Louisiana



I-65 Montgomery



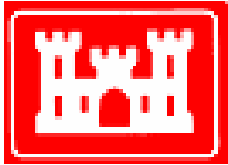
Airfield Rubblization Projects



- **Hunter Army Airfield, Savannah, GA**
 - East Taxiway Rubblized in 2003
 - Equipment (Antigo Construction Inc.):
 - Guillotine type breaker
 - Multi-Head Badger Breaker
 - Pavement Structure
 - 250 mm (10 in) AC O/L
 - 11,000 m² (13,167 yd²) of 200 mm (8 in) Rubblized PCC
 - Subgrade: Poorly Graded Sand
- **Selfridge Air National Guard Base, MI**
 - Runway Reconstruction, Summer 2002
 - Equipment (Antigo Construction Inc.):
 - Guillotine type breaker
 - Multi-Head Badger Breaker
 - Pavement Structure
 - 180 mm (7 in) AC O/L
 - 115 mm (4.5 in) Crushed Concrete Base Course (leveling course)
 - Rubblized PCC thicknesses varied from 330 to 530 mm (13-21 in)
 - Subgrade: Silty Sand soils



**Selfridge ANG Base
Rubblization Project**



Rubblization Evaluation Results



- **Pavement Structural Evaluation**

- Collect and analyze HWD data
 - Maximum load: 114,400 kg (52,000 lb)
 - Data analyzed in the PCASE program
 - Back-calculate Modulus values using WESDEF

- **Airfield Evaluation Results**

- Hunter Army Airfield
 - Average Rubblized PCC Modulus values:
 - 4,070 MPa (590 ksi)
- Selfridge ANG Base
 - 530 mm (21 in) Rubblized PCC Modulus values:
 - 8,700 MPa (1,260 ksi)

- **Additional FWD data:**

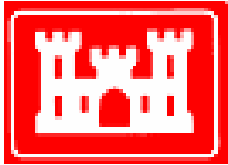
- Niagara Falls Joint Air Reserve Station
 - Data provided by AFCESA
 - Runway Pavement Structure:
 - 130 mm (5.0) AC O/L
 - 240 mm (9 in) Rubblized PCC
 - Subgrade: Silty Gravelly Sand



Heavy Weight Deflectometer

- Average Rubblized PCC Modulus values:

- 700 to 1,080 MPa (100-157 ksi)
- Variations:
 - High Water Table
 - Shallow Depth to Bedrock



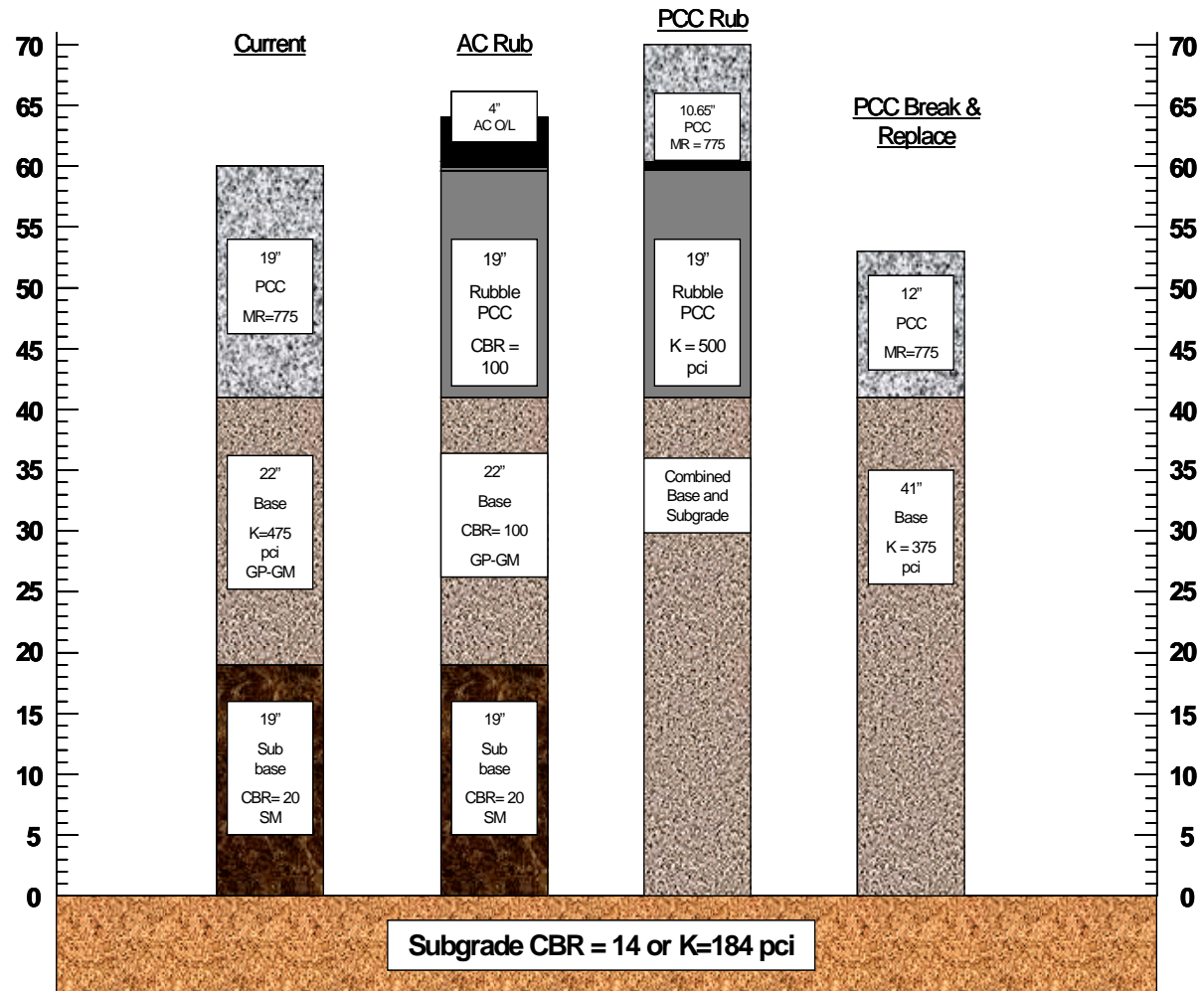
Grand Forks AFB Cost Analysis

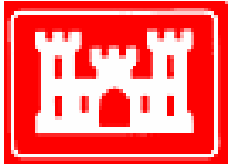


Based on the rehabilitation of a 480 mm (19 in) PCC pavement:

- **Grand Forks Air Force Base pavement design:**
 - Air Force Medium Traffic
 - 400 passes B-52
 - 400,000 passes C-17
 - 100,000 passes F-15E
- **Costs:**
 - Rubblization:
 - \$1.15 - \$5.50 per square meter (\$0.95- \$4.50 per square yard)
 - Break & Remove:
 - \$3.95 - \$7.50 per square meter (\$3.30 - \$6.50 per square yard)
 - Rubblization cost is approximately 40% of the cost of break and removal.

Grand Forks Air Force Base, North Dakota
Pavement Rehabilitation Options (Traffic = Air Force Medium)





Grand Forks AFB Runway Reconstruction Project



- **Monitor Ongoing Rehabilitation Project in Grand Forks Air Force Base, North Dakota**
 - Interesting Facts:
 - 250,000 sq. yards of PCC Rubblization
 - Average PCC layer thickness = 16-19 inches
 - Rubblization contract
 - Replaced RMI for Antigo Construction Inc.
 - New pavement will consist of AC and PCC overlays
 - Measure pavement response (HWD/FWD):
 - Before rubblization
 - After rubblization, before seating
 - After seating/ before AC/PCC overlay
 - After AC/PCC Overlay
 - Material characterization
 - Particle size distribution
 - Test pit particle sampling
 - Verify existing Rubblization guidelines and specifications



GF AFB Rub. Phase 1





Grand Forks AFB Rubblization Process



Step 1

Step 2

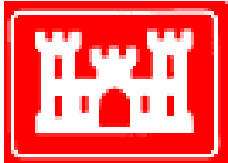
Ru

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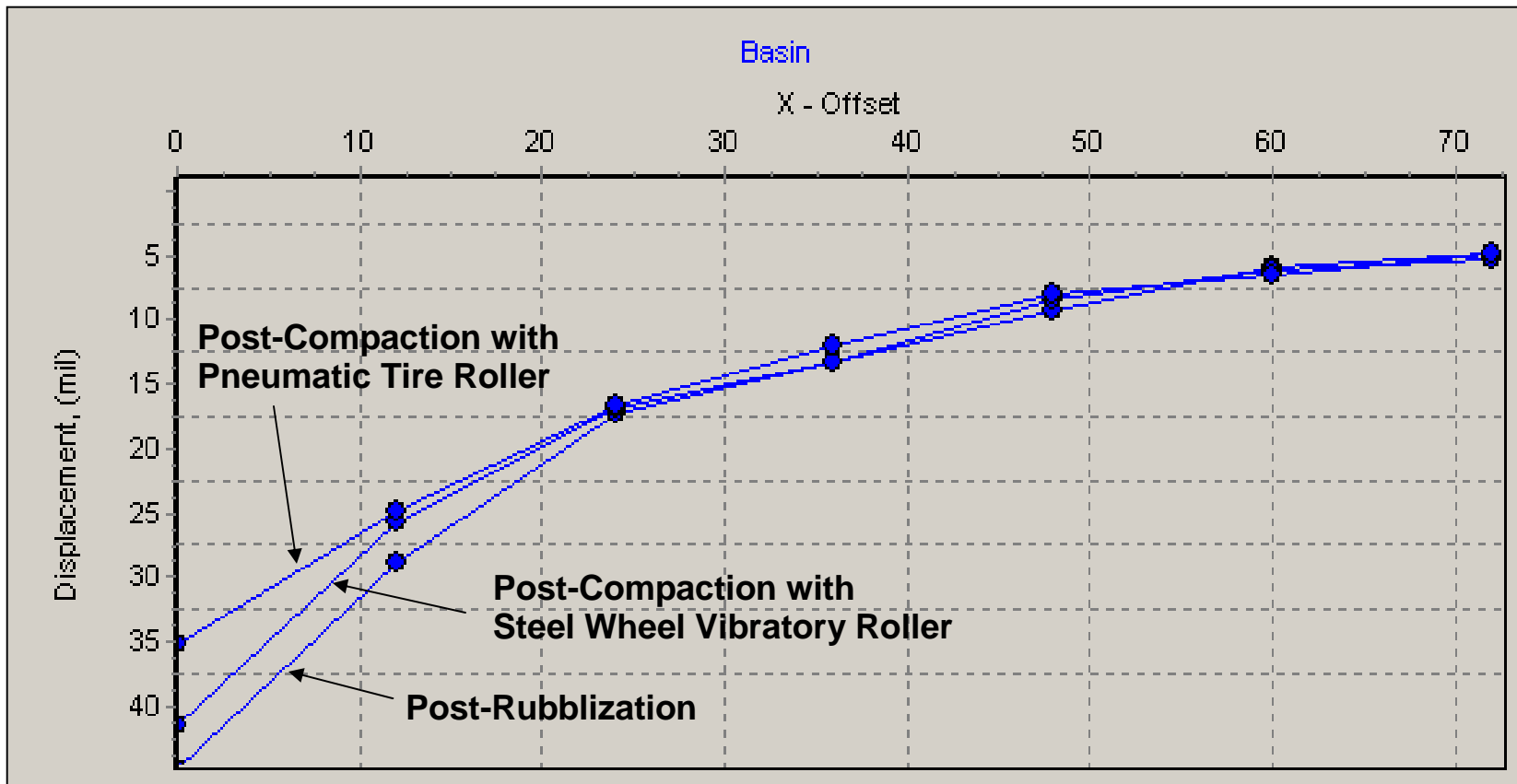
3
Overlay

6-12 in crushed PCC (leveling course)

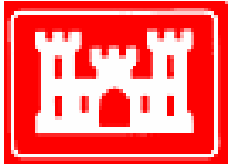




Grand Forks AFB - FWD Test Results



- GF AFB Phase I Runway Rubblization: 14-inch PCC pavement



Results and Conclusions



- Without proper guidance rubblization may not be considered a practical solution and there is substantial risk of premature failures.
- Overall cost of rubblization represents a 10% cost savings.
- Important Considerations:
 - Concrete slab
 - Thickness
 - Reinforcement type (if any)
 - Underground utilities
 - Base and Subgrade Strength
 - Soil moisture
 - Type of material
 - Subgrade Modulus >15,000 psi.
 - Proper drainage system
- The engineer may require more roller passes to achieve proper compaction. Over-compaction will break particle interlock.



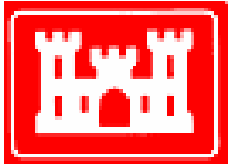
Proper drainage is required



Test Pits – Verify Cracked Pattern



Traffic Control



Future Research Studies



- **FAA Pavement Test Facility, New Jersey**
 - Load/Rolling tests
 - HVS
 - Aircraft loading
- **Monitor Long-term Rubblization Projects**
 - Existing condition evaluations
 - Non destructive testing:
 - HWD/FWD
 - Evaluate “old” crack & seal projects
 - Aberdeen Proving Grounds
 - Traffic responses
 - 5 (+) year term
 - HVS-A
 - Full-Scale Accelerated Pavement Testing
 - Other projects:
 - USAF – Elimination of Alkali-silica Reaction (ASR)
 - Travis AFB, California



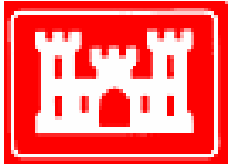
HWD



FAA Pavement Test Facility



HVS-A



Acknowledgements



- This past and ongoing research is sponsored by the Air Force Civil Engineering Support Agency (AFCESA) and conducted by the Geotechnical and Structures Laboratory in Vicksburg, Mississippi.
- For additional information on rubblization specifications:
 - Asphalt Institute Website, www.asphaltinstitute.org
 - Engineering Brief No.66 *Rubblized Portland Cement Concrete Base Course*, February 13, 2004 Federal Aviation Administration
- US Army Corps of Engineers Rubblization Specifications are currently under development. For more information please contact Eileen M. Vélez-Vega at **Eileen.M.Velez-Vega@erdc.usace.army.mil**

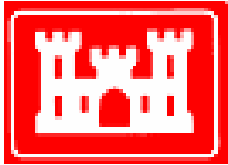


Thank you for your time!



QUESTIONS?





Contact Information



Eileen M. Vélez-Vega

**US Army Research and Development Center
Geotechnical and Structures Laboratory
Airfields and Pavements Branch**

Telephone number: 601-634-2717

Email: Eileen.m.velez-vega@erdc.usace.army.mil

Bluestone Dam

AAR – A Case Study



Huntington District

Fuller
Mossbarger
Scott &
May





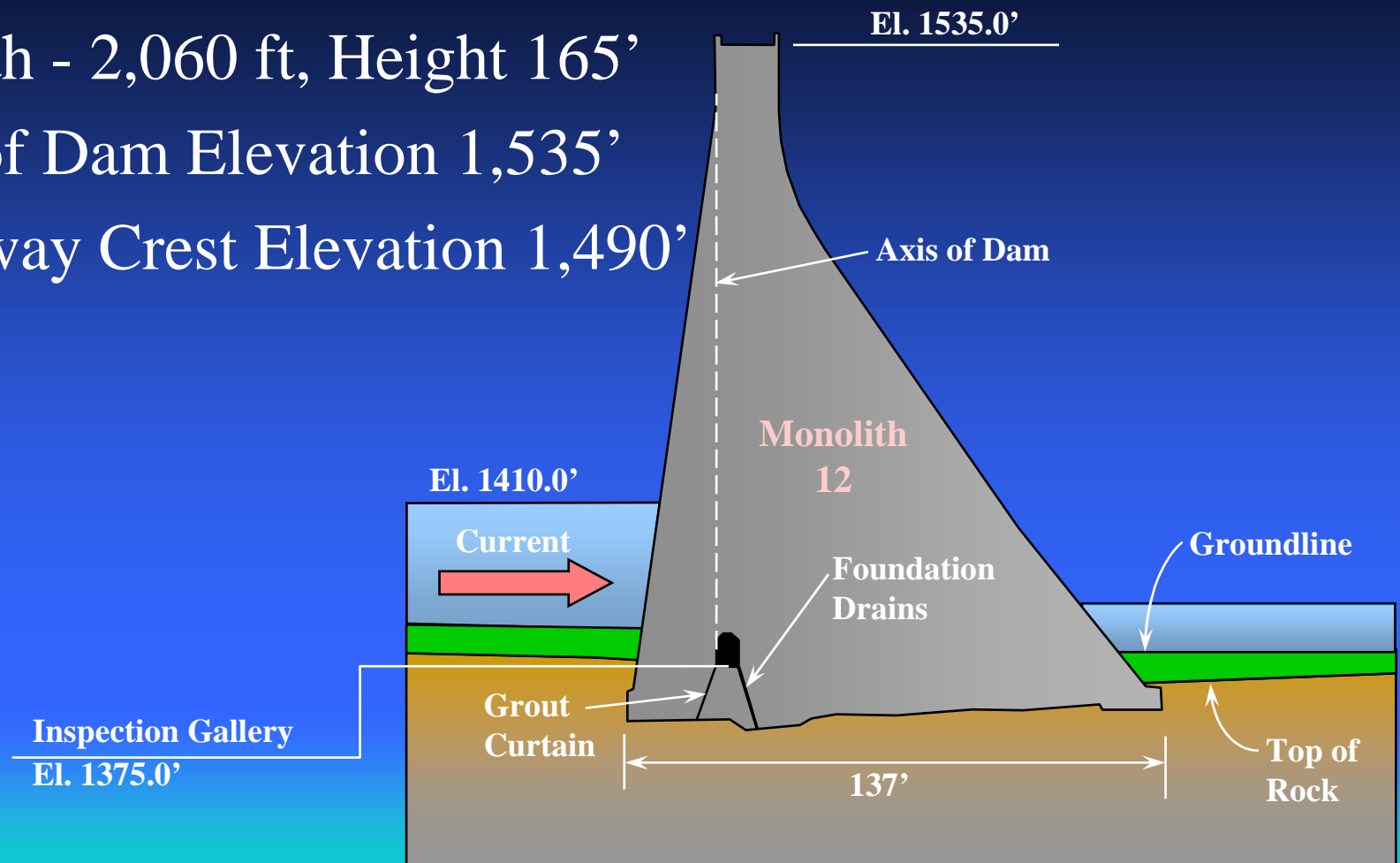
Presentation Overview

- Site Overview
- Ongoing DSA Projects
- AAR Project Issues
- Sample Retrieval
- Laboratory Testing
- Conclusions



Bluestone Dam – Existing Project

- Concrete Gravity Dam - 1940's
- Length - 2,060 ft, Height 165'
- Top of Dam Elevation 1,535'
- Spillway Crest Elevation 1,490'







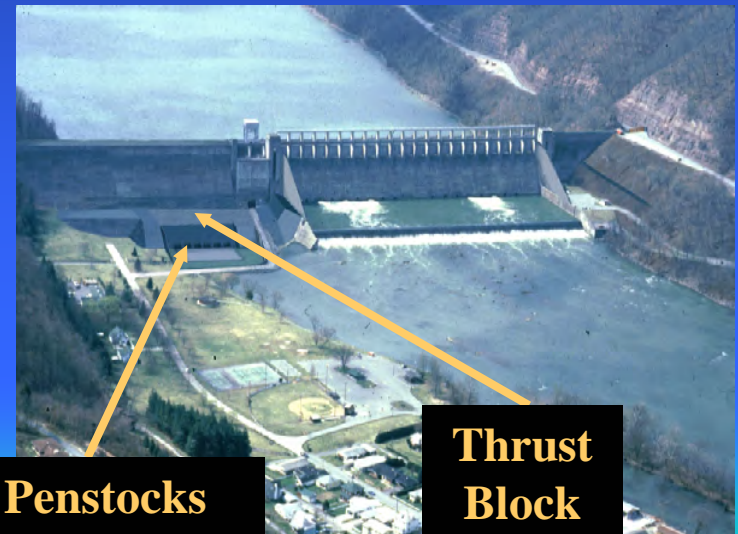


Bluestone DSA Phase I

- Project Features
 - 2 Lane Bridge
 - Thrust Blocks
 - Extending Penstocks
 - Sacrificial Bulkheads



Bridge

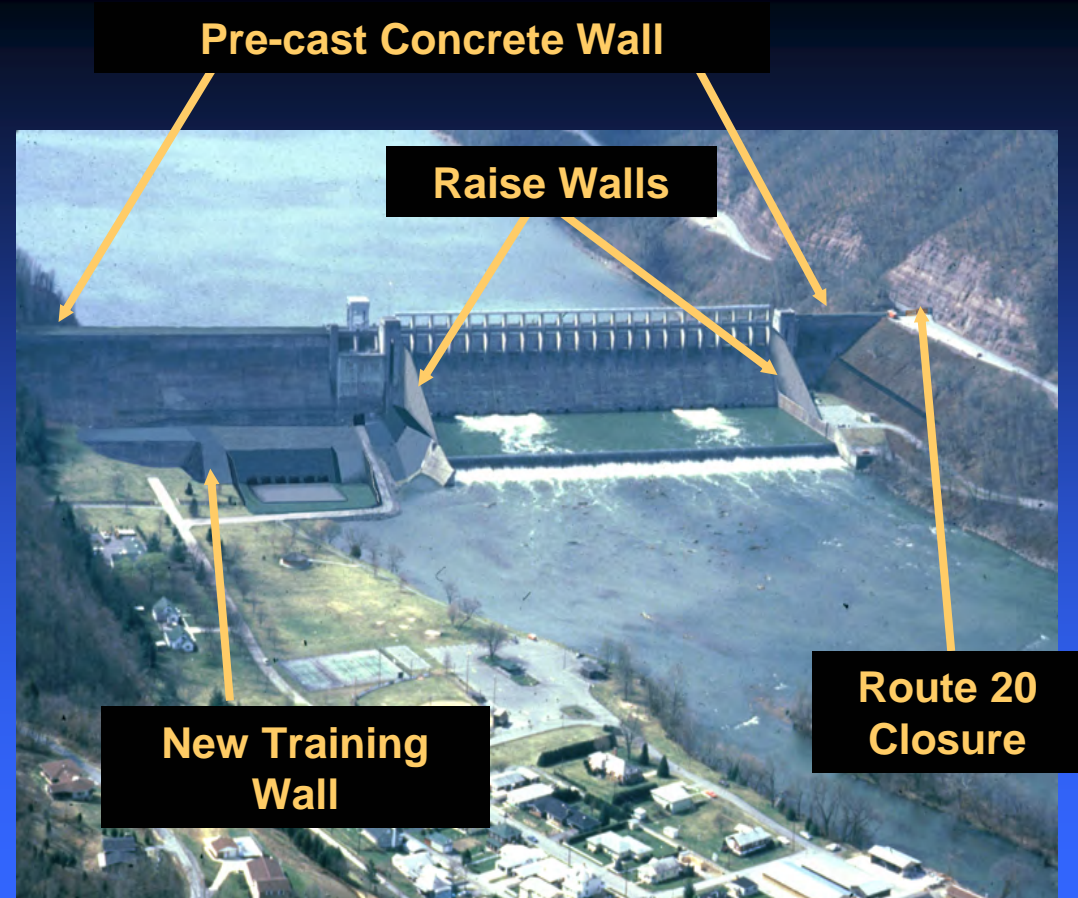


Penstocks

Thrust Block

Bluestone DSA Phase II

- Project Features
 - Rock Anchors
 - Parapet Wall
 - Rt 20 Gate Closure
 - New and Modified Training Walls



What is AAR?

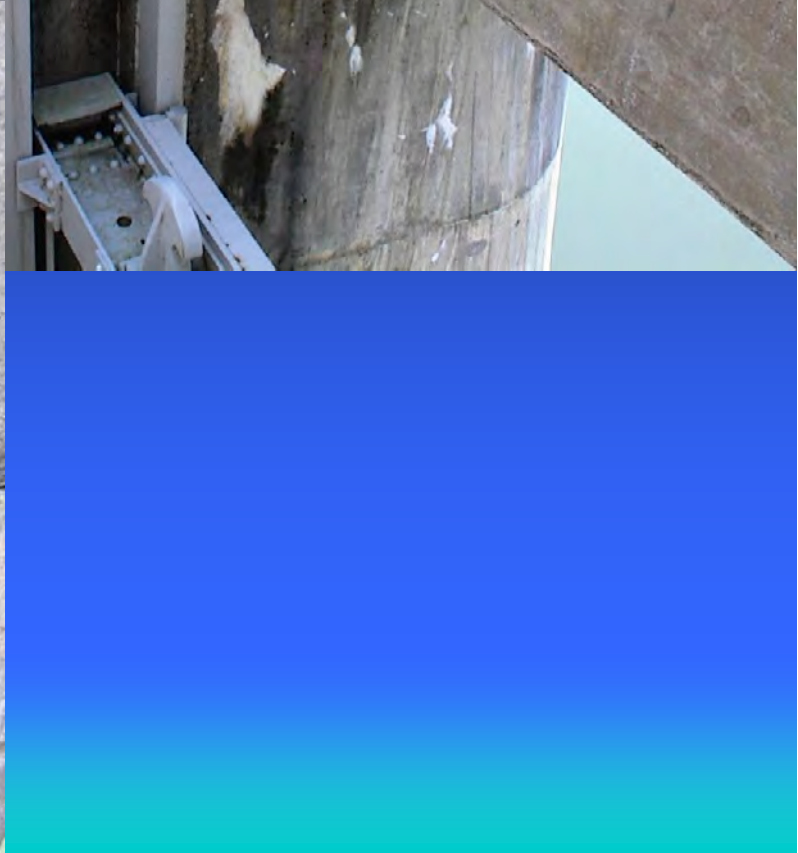
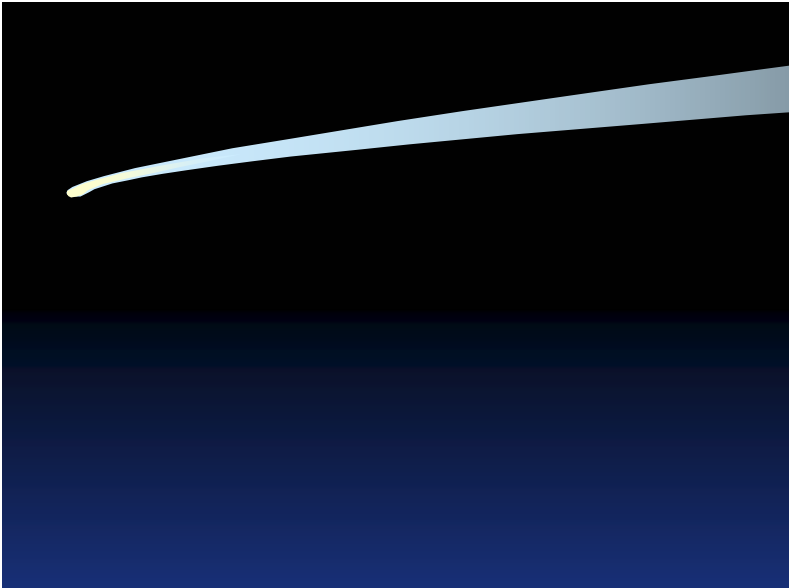
- Alkali Reaction with Silica (ASR)
- Alkali Reaction with Carbonates (ACR)
- Severity Influenced by:
 - Aggregate
 - Cement – Alkali
 - Humidity
 - Temperature
 - Stress Level
 - Time
- Decreased Serviceability and Design Life

Issues for Bluestone Dam

- Growth Mechanism – ASR or ACR?
- Growth Rate
- Impacted Areas of the Dam
- Compressive Strengths
- Influence on Planned Construction
- Same Quarry OK?





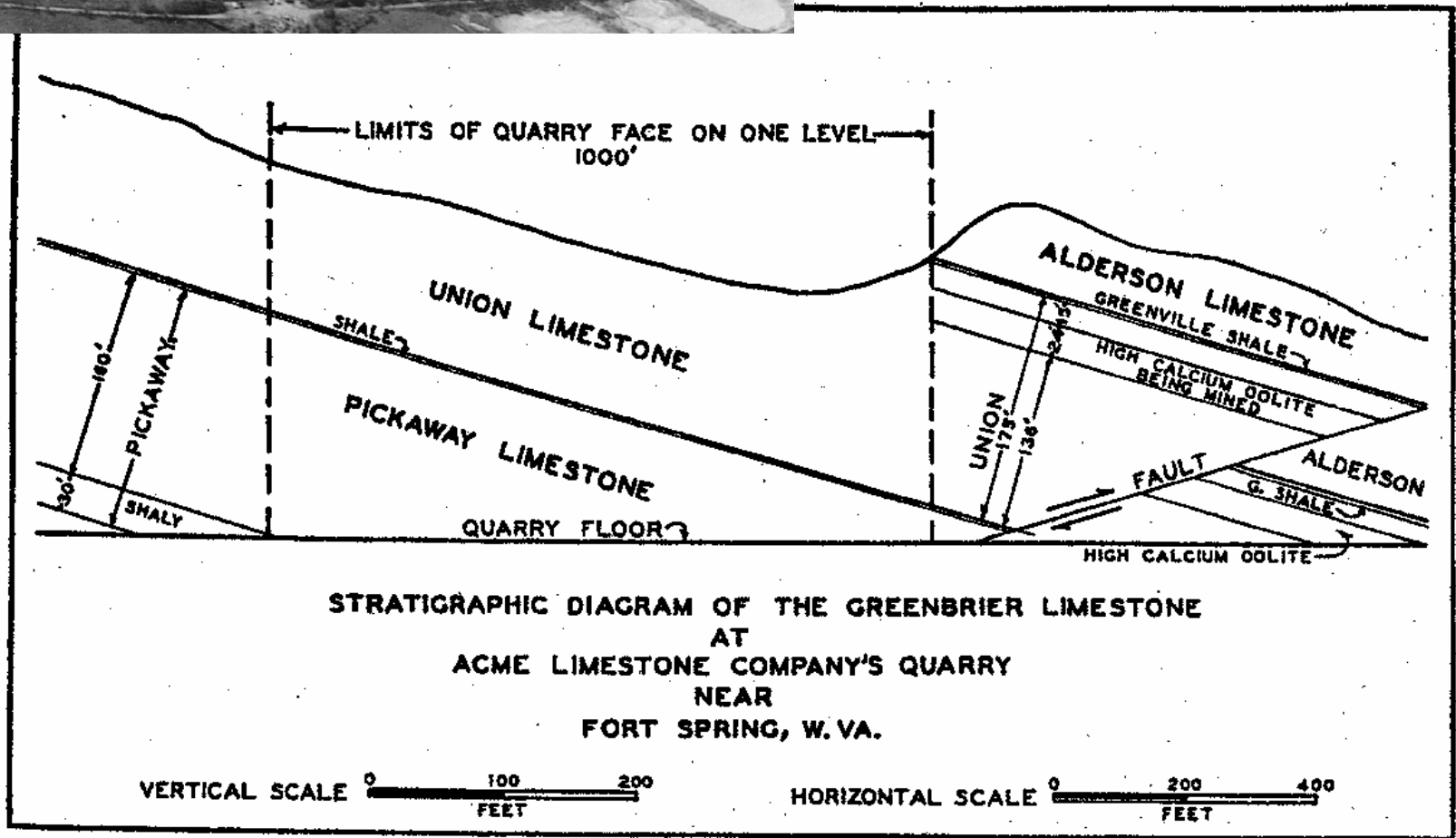








Snowflake Quarry - Potentially ASR Reactive



Sample Retrieval from Dam

- Roughly 30 Sample Locations
- 4” and 6” Thin Wall
- NQ, PQ and 3”
- Positioned Primarily in Spillway Bridge
- Selected other Locations
 - Galleries
 - Abutments

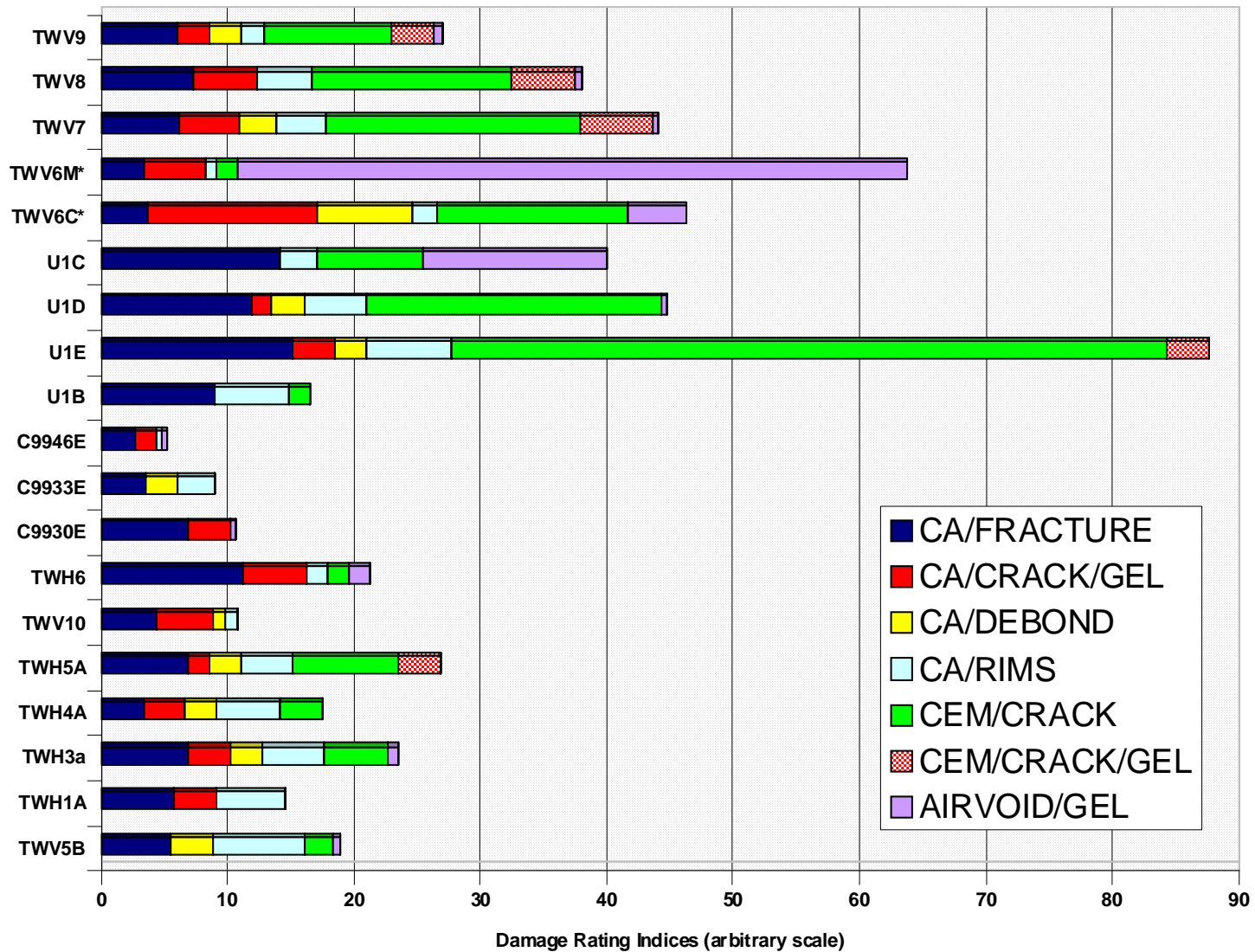
Damage Rating Indices

- Stereobinocular MS
- Mag = 16x
- Natural and UV Light
- Uranyl Acetate
- Gel Fluoresces
- DRI ~ 30

Weighting Factors for Determination of DRI	
Feature measured	Factor
Cracks in coarse aggregate	X 0.25
Cracks in coarse aggregate + gel	X 2.0
Open cracks in coarse aggregate	X 4.0
Coarse aggregate debonded	X 3.0
Reaction rims	X 0.5
Paste with cracks	X 2.0
Paste with cracks + gel	X 4.0
Gel in air voids	X 0.5

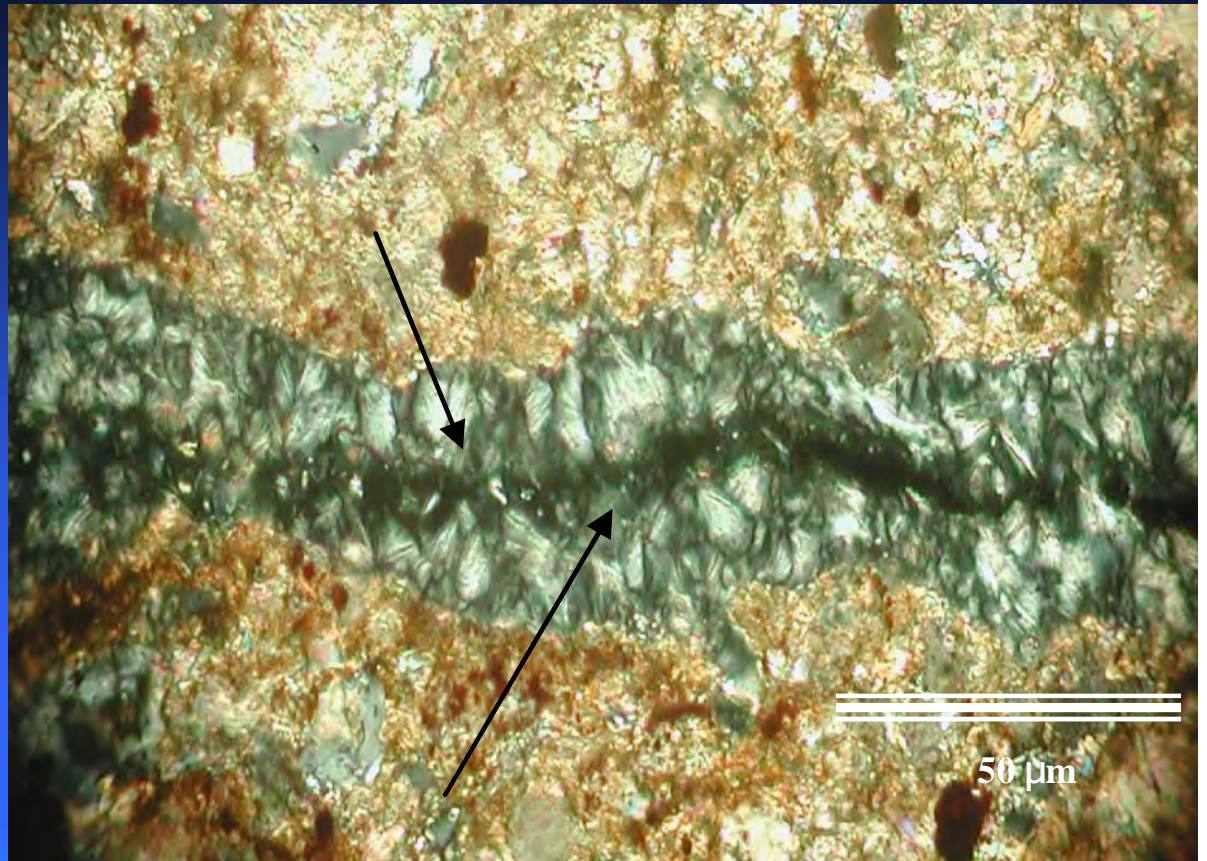
DRI Results

BLUESTONE DAM CORES



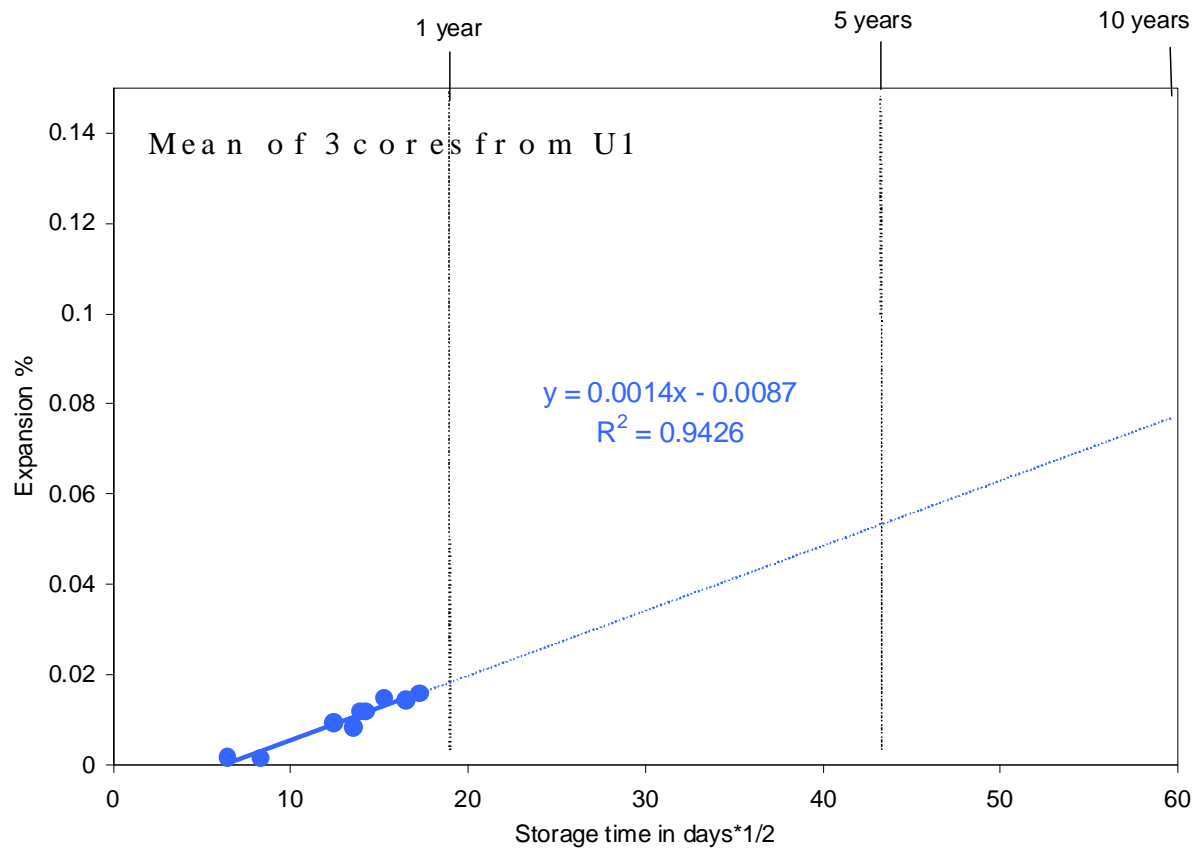
Petrography

- Alkali Silica Gel Observed
- Chert
- Chalcedony
- Greywacke
- Alkali Contents $< 2 \text{ kg/m}^3$

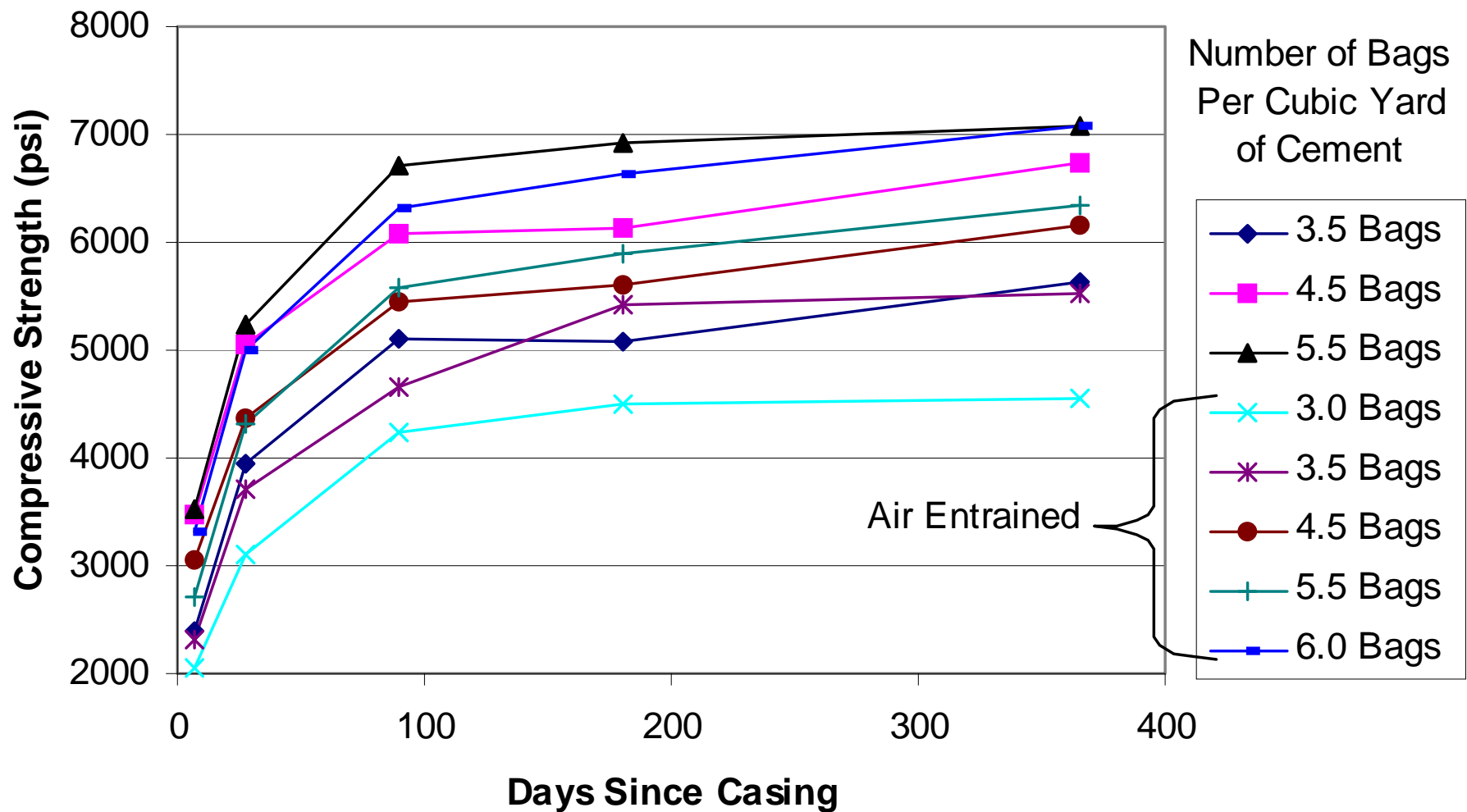


Expansion Tests

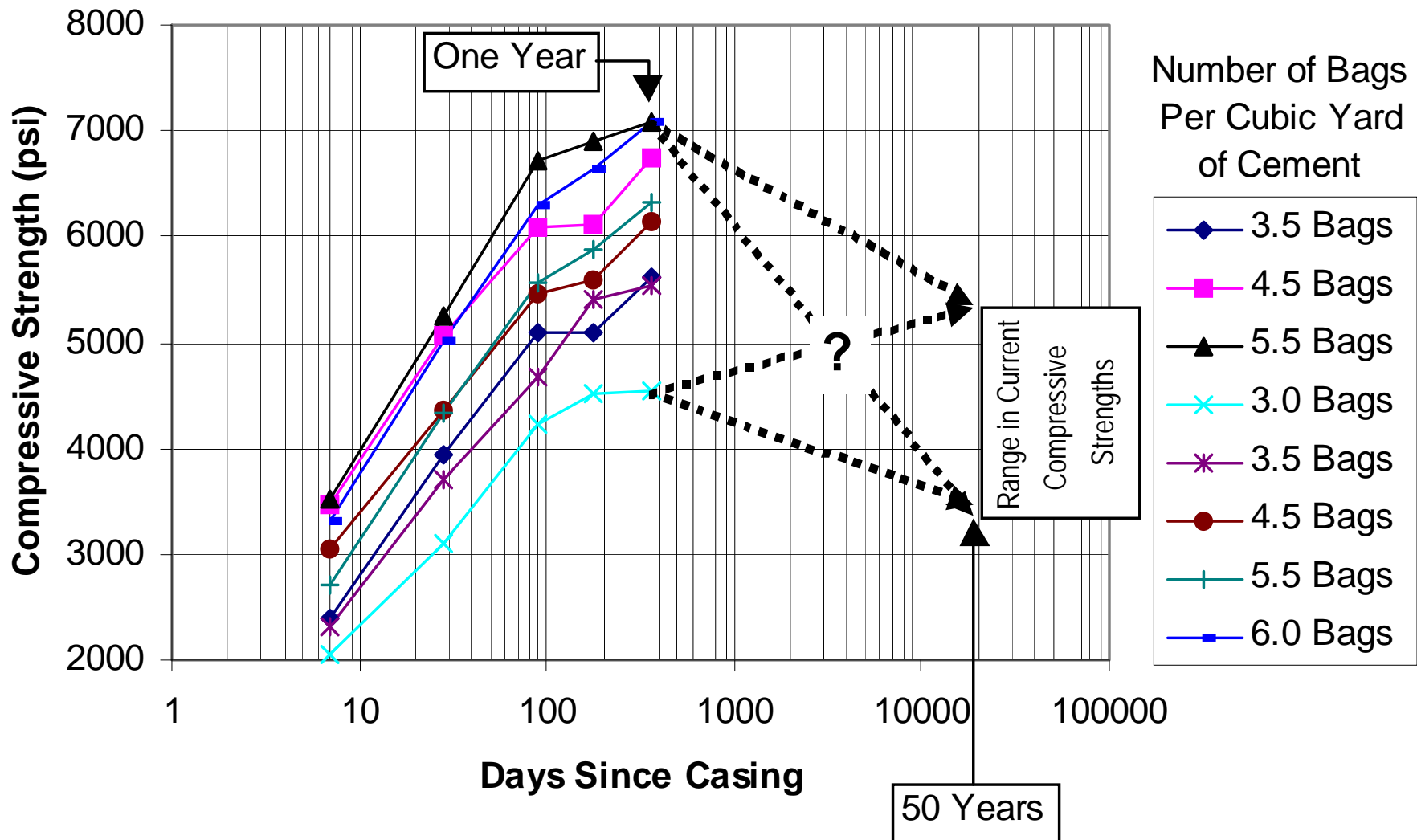
- On Cores, CSA A864-00
- 100% Relative Humidity, 38 C
- Over Water and w/NaOH Added – Insufficient Alkalis



Compressive Strengths – 1940s



Compressive Strengths – 2000



Conclusions

- Growth Mechanism – ASR
- Growth Rate ~ Very Small
- Insufficient Alkalis to Support any Further Significant Expansion
- Compressive Strengths Decreased – Consider in Future Designs
- Spillway Bridge Capacity

Fuller
Mossbarger
Scott &
May

FMSM
ENGINEERS

Questions ??



Greg Yankey

- 859/422-3000 (office)
- 859/619-8951 (cell)
- FMSM Engineers
- gyankey@fmsm.com

Fuller
Mossbarger
Scott &
May

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