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Galveston Island, Texas, Sand Management Strategies

Ashley E. Frey, Andrew Morang, and David B. King

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Galveston Island, Texas, Sand Management Strategies

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Abstract

The Coastal and Hydraulics Laboratory, Engineer Research and Development Center, and the U.S. Army Engineer District, Galveston, conducted a study to support a sand management strategy for the Galveston Park Board of Trustees of Galveston, TX. The long-term management strategy encompasses not only Galveston Park Board of Trustees managed areas, but the entire shoreline of Galveston Island. In the first phase of the project, a sediment budget was recomputed and GenCade, a numerical model, was calibrated for Galveston Island. After discussing potential solutions and actions with the Park Board, engineering analyses and numerical modeling were conducted to quantify the performance of each selected alternative. The long-term solution is a wide beach along Galveston Island that is filled through beach nourishment and backpassing plants on both ends of the island. This solution will require a large volume of sand for the initial construction; therefore, sand management solutions were identified and potential offshore sand sources were identified. Shorter-term and smaller-scale beach nourishment activities were also provided as options within the strategy.

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Preface

This study was conducted for the Headquarters, U.S. Army Corps of Engineers (HQUSACE), Washington, DC, under the USACE Regional Sediment Management (RSM) Program, Project 454632, “Galveston Sand Management Strategies” Project and the Galveston Park Board of Trustees. The HQUSACE RSM Program Manager was Linda S. Lillycrop, CEERD-HN-C. Jeffrey A. McKee was the HQUSACE Navigation Business Line Manager overseeing the RSM Program.

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The Commander of ERDC was COL Bryan S. Green, and the Director was Dr. Jeffery P. Holland.

Unit Conversion Factors

In this document, most units are reported as U.S. customary units, in keeping with common usage. Some of the analyses were performed in metric units, but the results are reported in U.S. customary units.

Multiply	By	To Obtain
cubic yards	0.7645549	cubic meters
feet	0.3048	meters
miles (U.S. statute)	1,609.347	meters
miles per hour	0.44704	meters per second
square feet	0.09290304	square meters
square miles	2.589998 E+06	square meters
square yards	0.8361274	square meters
yards	0.9144	meters

1 Introduction

1.1 Background

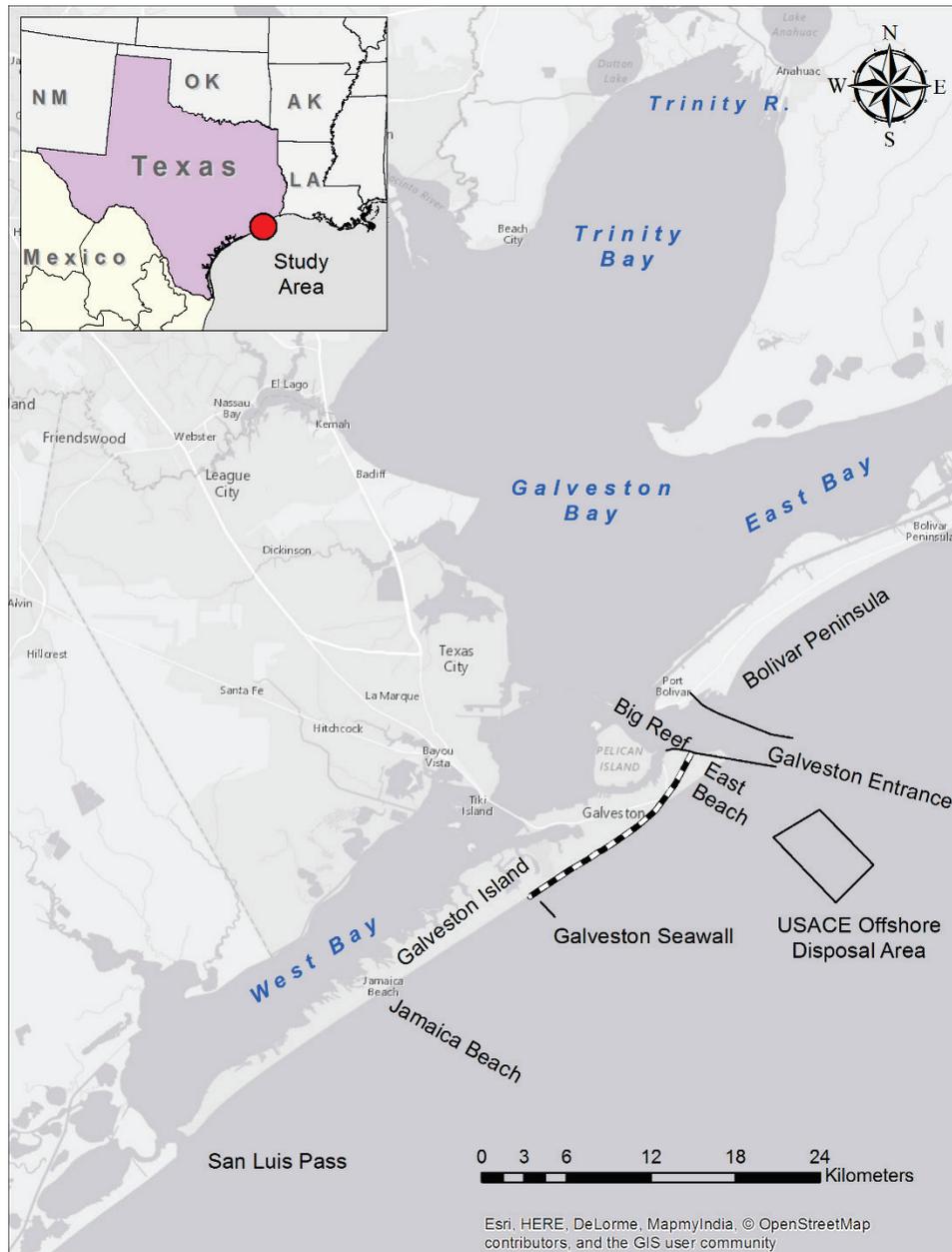
The Galveston Park Board of Trustees (GPB) enlisted the aid of the U.S. Army Engineer District, Galveston (SWG), and the U.S. Army Engineer Research and Development Center's Coastal and Hydraulics Laboratory (ERDC/CHL) in developing a long-term sand management strategy for Galveston Island. Presently, sand is placed on Galveston Island at a cost of up to \$40/yd³, with no clear plan for long-term sustainability. . By combining a review of the technical literature with a suite of engineering analyses and numerical modeling, this report presents a broad look at alternatives that are technically realistic and have the potential for long-term sustainability. Economic, environmental, and societal issues will require extensive additional analyses and deliberation prior to refinement, selection, and implementation of a chosen alternative.

1.1.1 Geographic setting

Galveston Island is a 29-mile long sandy barrier island along the upper Texas coast (Figure 1), located approximately 45 miles south-southeast of Houston and approximately 70 miles west-southwest of the Texas border with Louisiana. The island varies from approximately 0.6 mile to 3 miles in width and is oriented with the long axis running from east-northeast to west-southwest at an angle of approximately 235°.

To the southeast, the island faces the Gulf of Mexico. The island is separated from the Bolivar Peninsula to the northeast by the Galveston Entrance channel, which is the main navigation channel into Galveston Bay. This jettied, deep-draft channel, which provides access to the ports of Galveston and Houston, is one of the busiest shipping entrances in the United States. The U.S. Army Corps of Engineers (USACE), SWG, maintains the jetties and navigation channels. To the northwest, the island is separated from the mainland by an arm of Galveston Bay known as West Bay. To the southwest, Galveston Island is separated from the adjacent coast (Follets Island and the mainland) by San Luis Pass, a modest-sized, (Jarrett 1978) downdrift-offset, natural (undredged and unjettied) inlet. While the pass is a fairly typical nonmigratory Texas inlet (Price 1951), the shoals and inlet margins are dynamic.

Figure 1. Study area, Galveston Island, TX.



The entire island, other than the town of Jamaica Beach, is incorporated within the city of Galveston (population approximately 49,000). The eastern third of the island, which contains the residential and commercial heart of the city, is protected by a seawall and raised land that was initiated following the devastation caused by the Galveston Hurricane of 1900 (Larson 1999; Roker 2015). The western portion of the island has limited development of single-family beach homes, subdivisions, and condominiums, interspersed with undeveloped areas.

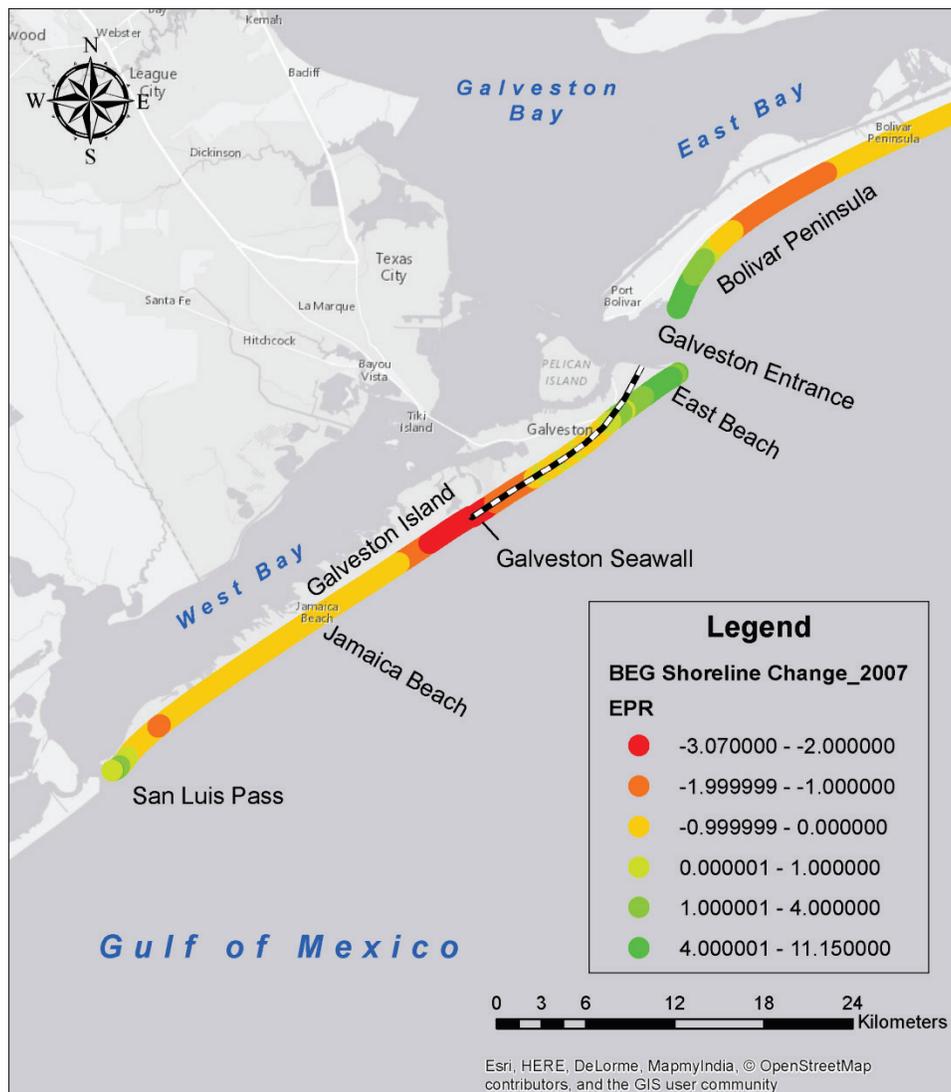
1.1.2 Sediment transport in the study area

Along most of the northern and central Texas coastline, the direction of the net longshore sediment transport is to the southwest (Hall 1975; Mason 1981; USACE 1983). However, there is a reversal in the net direction (a divergent nodal zone) along the western portion of the Galveston seawall, and east of this region the direction of net sediment transport is towards the Galveston Entrance Channel to the northeast (King 2007; Morang 2006). To the west of this zone, the net sediment transport direction is southwestward towards the portion of the island lacking a seawall and eventually towards San Luis Pass. The primary reason for this reversal is due to changes in wave refraction over the complex offshore bathymetry.

Note that on any given day, the transport direction can be in either direction along any section of the coastline, depending upon wave conditions at the time. The net direction is the predominant direction of transport averaged over intervals of a year or longer. The location of the reversal should be thought of as a zone, not a point, because there are year-to-year, as well as seasonal, variations in the wave field as it approaches the island. Also note that throughout most of the study area, daily variations in the wave conditions make the potential gross transport an order of magnitude greater than the net transport (King 2007). Most of West Galveston Island is experiencing long-term erosion as seen in Figure 2; however, both tips of the island are accreting.

Another hallmark of the region is the lack of rivers that deliver sand to the coast. East Galveston Bay was formed at a lower sea level stand as the Trinity River valley. Today, most sandy material carried by the Trinity River is deposited in Lake Livingston (Phillips and Musselman 2003). Sediment from the lower Trinity River is derived from scour and bank erosion and is largely stored in the delta in Trinity Bay (Phillips et al. 2004). Sand that is currently on the beaches within the study area is either reworked from relic deposits or remains with the shoreline as it retreated landward at the end of the last ice age (see King (2007) for further discussion). Thus, the coastal zone of Galveston Island and nearby barrier islands is sand limited, consisting of a sand veneer perched on a mud substrate, with minimal new supply entering the system. Mud outcrops can be occasionally seen on Bolivar Peninsula and even (rarely) on Galveston Island. The sand in this system is very fine, with typical median grain sizes in the range of 0.15 mm (2.75 phi).

Figure 2. Net long-term shoreline change rate on the Upper Texas coast calculated through 2007. Data values are end point rate in meters/year; from Paine et al. (2011).



1.2 Objective

The GPB initiated a planning process that developed from a concept with the Beach Maintenance Advisory Committee to lead to the development of a comprehensive and sustainable long-term, science-based sand management strategy, establishing a holistic approach to the management of sediment on Galveston Island. This planning process was implemented through the partnership between GPB, SWG, and ERDC. This report is intended to present a number of alternatives and recommendations that the GPB should execute in order to better manage sands on Galveston Island to help reduce the long-term cost of beach maintenance.

1.3 Approach

To achieve the goal of developing a sand management strategy that reduces the long-term cost of beach maintenance, the following approach was applied. First, it was necessary to identify present and future GPB needs, plans, and constraints. This meeting was conducted at the same time as a site visit to Galveston Island. Following the meeting, ERDC needed to understand the physical processes of Galveston Island. ERDC has conducted many previous studies on Galveston Island and at the Galveston Entrance Channel. All previous reports were reviewed to better understand the movement of sediment, placement of sediment, dredging requirements, and previous modeling studies. During this phase of the project, it was determined that instead of updating the previous GENESIS model, a newer model called GenCade would be used. Historical dredging and placement were evaluated, and an updated sediment budget was calculated. In conjunction with the GPB, ERDC and SWG identified and developed potential solutions and actions for the long-term sand management strategy. To better understand how each alternative would perform, the GenCade model was used to qualitatively evaluate shoreline change and sand transport for up to 50 years. The review of the physical processes, the sediment budget, the GenCade calibration and alternatives, and the discussion of the long-term plan are documented in this report.

1.4 Report organization

This report is organized into nine chapters. Chapter 1 discusses the geographic setting of the study area. Chapter 2 is a summary of the sand management alternatives that are supported by the engineering analyses. Chapter 3 describes the sediment budget while Chapter 4 provides background on the numerical modeling with GenCade. Chapter 5 lists sand management options near East Beach, and Chapter 6 discusses the sand management options near San Luis Pass. Chapter 7 describes the large-scale beach fill and covers the reach alternatives. Chapter 8 discusses the GenCade alternatives. Conclusions are summarized in Chapter 9.

2 Sand Management Alternatives

2.1 Objectives of this sand management study

Beach restoration alternatives for Galveston Island will require substantial quantities of sand, both for initial fill and for periodic renourishment. Renourishment will be required on a continuing basis because, once placed on the beach, portions of the fill will be carried along the shore by wave-generated longshore currents away from the placement sites. The main focus of this study has been to use engineering approaches to better understand the details of the physical processes at work on the Galveston beaches to quantify the sediment needs for various alternatives. This has been done through updating an existing sediment budget along Galveston Island and developing the GenCade numerical model for the island.

Based on discussions with the GPB, Galveston Island was divided into six reaches (Figure 3 and Table 1) as follows:

- Reach 1: ~14th Street to 61st Street. This reach covers the eastern portion of the Galveston seawall that contains the groin field. Several small beach fills have been placed in this location in the past two decades. Net sediment transport along this reach is to the east (toward East Beach). Along the seawall, a nodal zone exists where net longshore sediment transport diverges, some moving east and some moving west. The divergence zone cannot be precisely defined and moves within Reaches 1 and 2.
- Reach 2: 61st Street to 103rd Street. This reach covers the western portion of the Galveston seawall. Currently there is no exposed beach along most of this reach.
- Reach 3: 103rd Street to State Park. This reach extends from the western end of the seawall to the Galveston Island State Park. The region is experiencing erosion, and net sediment transport is to the west.
- Reach 4: State Park to Pointe San Luis. This reach is further west along west Galveston Island. As with Reach 3, this reach is experiencing long-term erosion, and net transport is to the west.
- Reach 5: Pointe San Luis to San Luis Pass. This reach covers the western tip of the island and experiences strong episodes of erosion

and accretion related to the inlet dynamics of San Luis Pass. Net transport is westward into the pass.

- Reach 6: South Jetty to ~14th Street. This reach covers the eastern tip of the island, including East Beach. This region has experienced long-term accretion.

Figure 3. Study area, Galveston Island, TX. Green boxes are sediment budget cells used in the study.



Table 1. Location of reaches along Galveston Island.

	Park Board Reach	Location	Sediment Budget Cell	Latitude	Longitude	Distance (ft)
1	East End	14th Street	E end Cell 1_6	29.2965°	-94.7795°	18,400
	West End	61st Street		29.2664°	-94.8262°	
2	East End	61st Street		29.2664°	-94.8262°	16,200
	West End	103rd Street	W end 1_6	29.2419°	-94.8686°	
3	East End	103rd Street	E end 1_7	29.2419°	-94.8686°	38,600
	West End	State Park	W end 1_7	29.183°	-94.9695°	
4	East End	State Park	E end 1_8	29.183°	-94.9695°	52,900
	West End	Pt. San Luis	Central 1_9	29.0948°	-95.1015°	
5	East End	Pt. San Luis	Central 1_9	29.0948°	-95.1015°	6,300
	West End	San Luis Pass	W end 1_10	29.083°	-95.1159°	
6	East End	South Jetty	E end 1_5	29.3316°	-94.7253°	21,700
	West End	14th Street	W end 1_5	29.2965°	-94.7795°	

Although some of the alternatives described will be specific to a reach, it is assumed that in many cases, alternatives will involve a combination of reaches. Additionally, the sand management strategy must be adaptive; for example, if erosion rates change in the future, there must be options to adjust the beach fill volumes and renourishment intervals.

2.2 Sand management and placement alternatives

2.2.1 Overview

Table 2 lists alternatives from most to least comprehensive. These alternatives are reviewed in the following sections.

Table 2. Overview of Galveston Island sand management and placement alternatives listed from most (1) to least (5) comprehensive.

Plan	Coverage (reaches)	New Material (Offshore or Other Sources)	Management and Recycling of Existing Sand Sources and Dredge Material	Performance Monitoring	Notes
1. Comprehensive beach fill	1 - 5	√	√	√	Beach revitalization plan
2. Limited area beach fill	1, 2, 3(?)	√	√	√	Most critical areas only
3. Systematic recycle	1, 2		√	√	Reuse existing sediment in system without external new sediment
4. Present action plan/ existing	1, 2		√		Reacts to storms or emergencies
5. No action					Baseline

2.2.2 Plan 1. Reach 1 through 5 comprehensive alternative

Plan 1 is the comprehensive beach fill along Galveston Island over Reaches 1–5, with systematic (semicontinuous) maintenance (renourishment) and beach monitoring. This includes improved management of existing sand accumulations at East Beach, Big Reef, and San Luis Pass and recycling (beneficial use) of dredge material.

Plan 1 represents the most extensive nourishment area and expensive alternative. This project would be expected to have backpassing plants at East Beach and possibly at San Luis Pass, with sediment pipelines to redistribute sand onto the beaches. From a regional sediment management perspective, Galveston Island is a logical, relatively isolated unit and would gain advantages from management of the entire island's beach as a unit.

Note that a 25-mile long project on Galveston Island would be the longest beach fill project ever constructed in the United States. For comparison, the initial placement of the Miami Beach Restoration Project between 1976 and 1981 nourished 10 miles of beach at a cost of \$64 million (in 1981 dollars; Frohling 1986; Dean and Dalrymple 2004). The Sea Bright to Manasquan Inlet, NJ, Beach Erosion Project (currently the largest) nourished 21 miles of beaches and was constructed between 1994 and 2001 at a cost of \$195 million (USACE 2015) (<http://www.nan.usace.army.mil/Media/FactSheets/FactSheetArticleView/tabid/11241/Article/487661/sea-bright-to-manasquan-nj-beach.aspx>).

2.2.3 Plan 2. Beach fill in limited areas

Plan 2 is similar to the comprehensive Plan 1 except that a beach fill will only be placed in the most vulnerable areas along the island. Reach priority is related to the historical rates of erosion and accretion, amount of infrastructure, and the likelihood that beach fills will be appropriate in the future. Reaches 1 and 2 are the most likely candidates for a fill while Reaches 5 and 6 are currently accreting and are not expected to require any fill unless conditions change.

2.2.3.1 Reach 1 Alternative

Placing a beach along the eastern end of the seawall should be a component of any final design strategy. Not only is construction of a dry berm along the eastern end of the seawall a high priority, but it is assumed that it will have the highest visitor use and the lowest construction and maintenance costs. The material placed on Reach 1 must be compatible with the sand presently on the beach, particularly the grain size, sorting and color (Gravens et al. 2008). This alternative requires use of sediment sources discussed in Section 2.3.

2.2.3.2 Reach 1 and 2 Alternative

The Reach 1 and 2 alternative involves placing a beach fill in front of both Reaches 1 and 2 and is described in detail in Chapter 7. Sand placed in Reach 2 needs to be tapered into the beach beyond the end of the seawall to mitigate the erosional end effects caused by the present right-angle structure.

2.2.3.3 Reach 1 through 3 Alternative

The Reach 1 through 3 alternative is a beach fill in front of the entire seawall and the unarmored beach in Reach 3 to mitigate the erosion now present at the end of the seawall using sediment backpassed from East Beach and/or sediment dredged from the navigation channel. Pre- and post-construction monitoring is required to monitor fill stability. A disadvantage of this alternative is that backpassing distances from East Beach are large (approximately 10 miles).

2.2.3.4 *Reach 1, 2, and 4 Alternative*

This alternative includes beach fill along the seawall (Reaches 1 and 2) plus the vulnerable Reach 4 along the west end of the island.

2.2.3.5 *Reach 5 and 6 Alternative*

Reaches 5 and 6 currently have wide beaches and therefore only need volume for dune construction. They are currently accretive, and it is not expected that they will experience significant long-term erosion in the future.

2.2.4 Plan 3. Recycle existing sediment without addition of new material

Plan 3 involves recycling and managing the sediment that currently accumulates at East Beach/Big Reef and at San Luis Pass. It also involves beneficially using dredge material for beach use but does not include using new material from offshore sources for beach placement. The GenCade modeling (Chapter 8) demonstrated that this alternative will result in shoreline advance, but the time scale is over decades. For a beach revitalization plan, it will be more effective to add new fill (Plans 1 and 2) and recycle existing material for maintenance.

2.2.5 Plan 4. Continue existing management

Plan 4 continues the present practice of reacting to storms and emergency conditions on an as-needed basis. This normally consists of truck-hauling sand from East Beach to the beaches along the base of the seawall. Truck-hauling is inefficient and disruptive, does not effectively use sand already in the system, and has not stabilized the beaches along much of Galveston Island. In addition, material dredged from the Federal navigation project is not beneficially used on beaches. Nourishment on an emergency basis is not recommended as a long-term strategy for Galveston Island.

2.2.6 Plan 5. No-action plan

Plan 5 is the no-action plan used as a baseline for the GenCade modeling (Chapter 8). It is not recommended as a strategy for Galveston Island.

2.2.7 Fundamental constraints

Two important constraints need to be appreciated in considering any major beach fill project on Galveston Island. The first is that sediment

supplies are limited compared with other large constructed beaches around the country. The beach sand now on Galveston Island needs to be acknowledged as a precious commodity, and creative ways must be considered in its optimal use and conservation.

The second is that there is a divergent nodal zone along the Galveston Seawall approximately in Reaches 1 and 2. The nodal point moves with wave and meteorological conditions. In Reach 6, the net direction of transport is to the east while in Reaches 3 and 4, it is to the west. This has several subtle impacts. For example, if a beach fill is placed along the entire seawall and a backpassing plant is only built at the east end, then a portion of that fill will be transported west beyond the west end of seawall. This sand will have to be replaced from other sources rather than just backpassing.

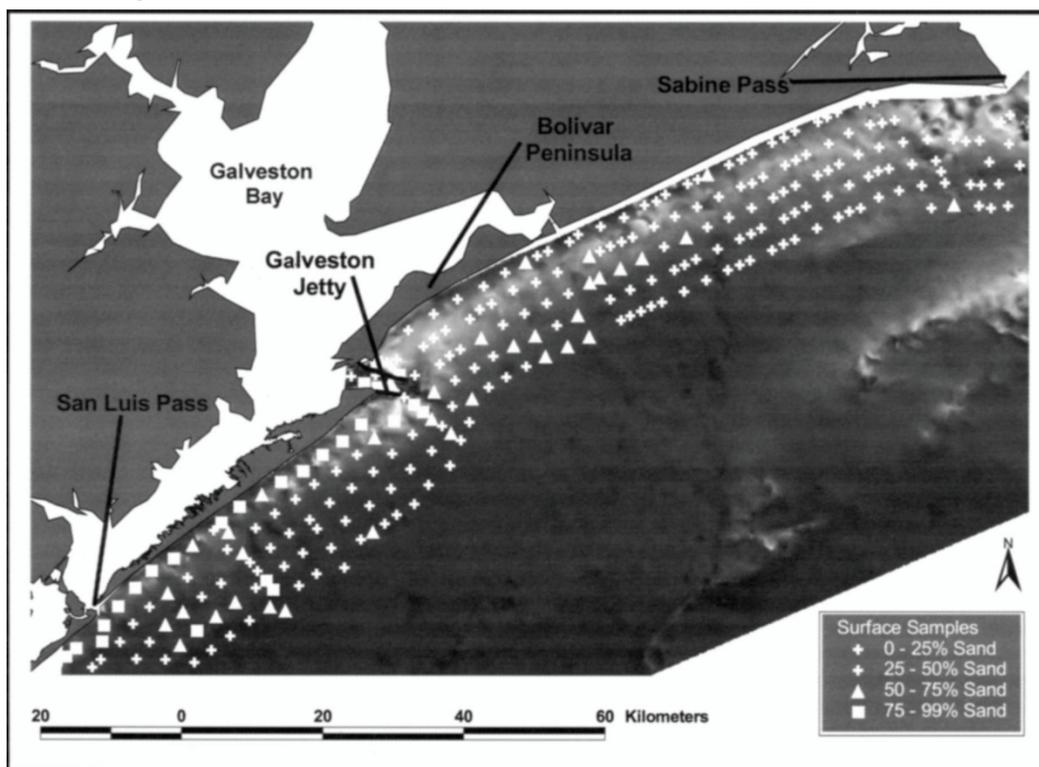
2.3 Sand source and delivery system alternatives

An integral part of developing alternatives for where sediment is to be placed is the sediment source location and the options for transport. The most expensive component of most beach fill projects is the cost of the sediment, which is greatly influenced by the distance it must be transported. Thus, the closest source or sources of beach-quality sediment are frequently the least expensive and the first to be utilized.

2.3.1 Offshore dredging

Offshore deposits are the most common source of sediment for large beach renourishment projects in the United States. However, offshore sediments along the Texas coastline are largely fine grained (silts and clays) and unsuitable for recreational beaches. Researchers (White et al. 1985; Siringan and Anderson 1994; Anderson and Wellner 2002; Finkl et al. 2004; Williams et al. 2012) who have investigated offshore sediments near Galveston Island have generally identified only limited pockets of sand. Most of the usable material was too close to shore (within the active surf zone) to be usable for beach fill. However, potential offshore areas remain that require further study (Finkl et al. 2004). TxSed maintains an active database and a map viewer of offshore sand resources that can be accessed at <http://gisweb.glo.texas.gov/txsed/index.html?config=config-Corp.xml>. Figure 4 shows surface sample percent sand distribution near Galveston Island.

Figure 4. Percent surface sand in offshore sediment samples (Finkl 2004).



Sediment reserves exist farther offshore. Morton and Gibeaut (1995) estimate that Heald Bank, approximately 35 miles offshore, contains roughly 585 million m³ of sand. They estimate that Sabine Bank, which is twice as far away, holds approximately 1.2 billion m³ of beach quality sand. However, Texas projects to date have not utilized these sources because of transportation costs.

The lack of nearby sources of beach-quality sediments imposes limitations on the design of economically viable alternatives. For most of the large-scale alternatives considered in this study, it is likely that some amount of material will be required from Heald Bank or some other offshore source over the project lifetime.

2.3.2 Channel dredging

The Galveston Entrance Channel is dredged on a regular basis by the USACE to maintain the navigation channel and is discussed separately from offshore dredging. Sediment is currently deposited offshore near the tip of the South Jetty. It is a sustainable regional sediment management strategy to use this shoal material for beach placement. However, quantities available are dictated by channel navigation requirements not by the

amounts necessary for initial construction or maintenance of the beach fills. Also, the fine-grained portion of the dredged material is not suitable for beach placement (see Chapter 3).

The analysis of dredging records indicates that on average, 1.098 million yd³ of beach quality sediment is dredged from the Galveston Entrance channel, anchorage area, and inner and outer bar channels every year (Appendix A, Tables A1, A2, and A4). However, if the South Jetty were sand tightened, some of this material would accumulate on East Beach and be available for backpassing.

2.3.3 Truck-haul backpassing

Most previous beach fills along the eastern portion of the seawall involved truck hauls of sediment from East Beach (see Appendix B). This is proven technology and costs per yard of placed material are generally understood. However, limitations of truck capacity, highway noise, fumes, and congestion make this methodology practical only for small fills.

2.3.4 Pipeline backpassing

A pipeline backpassing system is discussed in Chapter 5. It is assumed that this will be a key component of any alternative. If a fill is only placed in front of the seawall, then only a backpassing system on the east end of the island would be needed. However, if the fill is placed the length of the island, then backpassing plants at both ends are envisioned. When compared with an alternative truck-haul system, the substantially higher initial costs in infrastructure are expected to be outweighed by lower operating costs and higher transport capacity. However, a detailed cost analysis will be required. On East Beach, it is expected that the backpassing system will be developed in concert with sand tightening the South Jetty.

Moving forward with pipeline backpassing will require additional design, permitting, and economic and environmental analyses. The first step is to gain support of the residents of Galveston. If the city accepts the strategy, it will be easier to obtain permits and receiving funding. If pipeline backpassing is chosen for Galveston, one option is for the operation to only run at night so that tourists and residents will be minimally bothered by noise or pumping. Additionally, video cameras should be mounted near the discharge locations. These cameras should have a live feed available online so residents and others can see the work in progress. Telephones

could be set up along the seawall and west end so that concerned residents or tourists may inform of a possible issue with the operation. Finally, it is recommended that signs be placed along the seawall and west end to inform the public about the process and operations of backpassing as a part of basic policy management.

2.3.5 Alternative sources

Following the 1900 Galveston Hurricane, the first segment of the Galveston Seawall was constructed, and the elevation of the town was raised using sediments dredged from West Bay (Alperin 1977). However, these sediments were generally silts and muds and are not considered usable as beach-quality sediment today. The authors have not located any reports that indicate an availability of beach-quality sediments in either local interior water bodies such as West Bay or in local upland locations. Other potential sources include

- the fillet at the north side of the north jetty (requiring barge transport)
- portions of Big Reef, especially the underwater accumulation that slopes down into the Federal channel
- the sand accumulation north of the south jetty.

For future investigation, the practicality of using recycled glass as a potential sediment source could be investigated. See Finkl and Kerwin (1997), Edge et al. (2002), and Makowski et al. (2011, 2013) for its experimental use in limited trials.

2.3.6 Combination of sources

Regardless of the alternative chosen, the final solution is likely to be a creative mix of most or all of these alternatives.

2.4 Estimated sediment volumes for different combinations of reaches

Values in this section should be considered order of magnitude estimates, which are subject to substantial modification and refinement based upon additional studies. They are presented with the intent that they represent a starting point in the discussion leading to the development of a preferred alternative. Additionally, these volumes represent a wide beach along Galveston Island. Because of the difficulty in locating enough sand to

construct a single wide beach along the entire island at one time, smaller beach nourishment options and other more short-term alternatives are also presented.

The discussion below is divided into three parts: volumes of sediment needed for an initial fill, volumes moved by longshore transport which need to be backpassed, and volumes which can be considered as lost to the system and will need to be replaced over the lifetime of the project.

2.4.1 Initial fill volumes

Calculated initial fill volumes are given in Table 3. This includes a 200 ft wide berm/beach along the seawall and a dune with beach/berm along the remainder of the island. Chapter 7 details the derivation of these volumes.

Table 3. Initial fill volumes by reach (200 ft beach at seawall, 300 ft remainder).

Scenario	Length of Reach (ft)	Initial Fill Volume (yd ³)	Initial Fill Volume with Advanced Nourishment (yd ³)
Reach 1	18,400	560,000	840,000
Reach 2	16,200	2,131,600	3,197,000
Reach 3	30,000	2,518,800	3,778,200
Reach 4	51,700	4,407,900	6,611,850
Reach 5	12,300	471,800	707,700
Total		10,000,000	15,000,000

The most convenient locations to obtain initial fill are from the two ends of the island. A crude estimate of all the beach-quality sediment available from the east end of the island (the sandy portions of Big Reef and of East Beach east of Boddeker Road) is 3 to 6 million yd³. Much of this volume is along East Beach, so it is unlikely this total volume could be mined due to recreation and environmental restrictions. If only the areas that meet present-day requirements to mine are included, the volume available drops to approximately 1.8 million yd³. Based upon field work of Israel et al. (1987), Wallace et al. (2010) estimate that the San Luis Pass flood shoal contains approximately 11.8 million yd³ of beach quality sand. However, it is expected that if permits are issued that allow any of the flood shoal to be mined, this sediment will need to be shared with Brazoria County, and some amount will be reserved as ecological habitat. Thus a reasonable estimate is that a maximum of 4 million yd³ would be available for beach fills on Galveston Island. Thus, these sources are expected to provide less

than half of the needed initial fill volumes for the island. Presumably, the rest of the material would need to be obtained from Heald Bank, unless closer sources are located.

Note that backpassing plants at each end of the island would only need to backpass on the order of 250,000 yd³/year after the initial fills are placed (discussed in Chapter 8). It is unlikely that these plants would be used as the sole transport mechanisms for the initial fill as it would take multiple years to transport the needed initial fill volumes. Instead, much of the initial fill will likely be placed by dredges.

2.4.2 Backpassing volume rates

The backpassing volume rates are derived from the sediment budget. Because only the two ends of the island are accreting, the amount of material that needs to be backpassed yearly is the amount that is transported into the two end reaches. The sediment budget shows that approximately 41,000 yd³/year of sediment move from Reach 1 to Reach 6. Additionally, the budget indicates that up to 356,000 yd³/year moves into the Reach 6 cell from offshore. Approximately 251,700 yd³/year moves from Reach 4 to Reach 5. However, all but 11,600 yd³/year moves into San Luis Pass or is bypassed to the west side of the pass. The GenCade alternatives (Chapter 8) include a variety of backpassing rates and discharge locations along the reaches.

2.4.3 Sediment losses and gains over the project lifetime

While much of the sediment that is in motion in the surf zone stays in the surf zone, the system is not entirely closed, and sediment can leak out of (and into) the system. Other processes, such as sea level rise, can be treated as a sediment loss. Because some of these processes are nonlinear, it is easiest to treat them all on a project lifetime basis. These processes must be specifically accounted if the beaches are to maintain a long-term stability.

2.4.3.1 Storm losses

Hurricanes and other storms are thought to generally alter the beach profile by flattening it. That is, they erode sand from the upper part of the beach (the part that is normally dry: the dune, the berm and the foreshore slope) and deposit it within the surf zone. However, some beach sediment is carried inland by storm overwash, and there is mounting evidence that

large storms also carry sand offshore, seaward of the surf zone, where it is lost to the beach system. Note that this type of transport is fundamentally different from the day-to-day longshore transport within the surf zone. While longshore transport can erode a beach, the sediment is not lost from the nearshore. With the type of system being proposed for Galveston Island, the sand that is moved by longshore transport can be captured at a downstream location and backpassed to where it is needed. However, offshore, storm-deposited sediments will be mixed with the prevailing mud substrate and become unrecoverable. Thus, they represent a permanent loss to the system. Over long timescales (centuries), significant sediment can move onshore, but this process is not applicable to beach nourishment projects.

Numerous authors have made reference to this type of sediment behavior along the Texas coast during storm events. The 1915 hurricane was the first major hurricane to hit Galveston following the construction of the seawall. Qualitative reports indicate that after the storm, the beach never recovered its pre-storm width (USACE 1981, p. 13). Hayes (1967) reports on movement of offshore sand lenses following Hurricanes Carla (1961, Port O'Conner) and Cindy (1963, High Island), and Morton et al. (1994) discuss beach recovery in the years following Hurricane Alicia (1983, San Luis Pass). Gibeaut et al. (2002) discuss episodic beach erosion caused by Tropical Storms Josephine (1996, Galveston County) and Francis (1998, Galveston County). Several authors discuss long-term beach impacts following Hurricane Ike (2008, Galveston Entrance Channel), including Watson (2009), Dellapenna and Johnson (2010), Goff et al. (2010), Hawkes and Horton (2012), and HDR (2014).

From a geologic perspective, Wallace et al. (2010) used sediment cores collected at depths of 4 to 8 m (13 to 26 ft) off Galveston Island to calculate that over the last 2660 years, beach sediment has been transported seaward of the surf zone from the whole island length at an average rate of 150,000 yd³/year. He also concluded that washover rates have been minimal on Galveston Island.

Offshore sediment transport requires further study. However, this mechanism does represent a permanent sediment loss to the system that will need to be accounted for if long-term beach stability is to be achieved. In this report, the Wallace et al. (2010) average storm loss rate will be used. The island is approximately 150,000 ft long, and the loss rate

conveniently works out to a cubic yard of sediment per foot of beach per year. The lifetime amounts needed are listed in Table 4 for each of the reaches. These values are primarily intended for planning purposes. While the erosive events are episodic, appropriate renourishment, which introduces new sand to the system, should be planned on a regular basis.

Table 4. Storm and sea level rise project lifetime sediment losses (yd³).

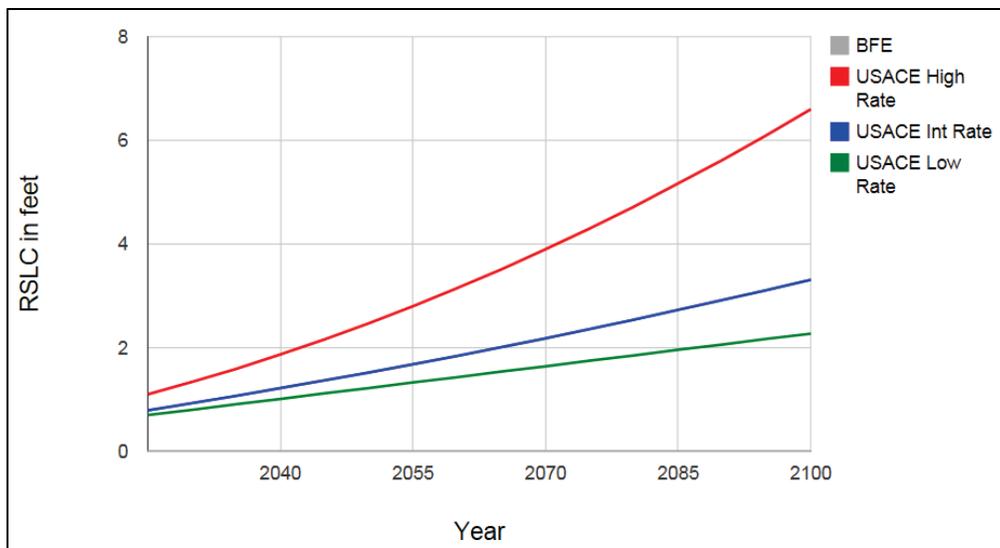
Scenario	Storm	Sea Level Rise		Maximum
		Minimum	Medium	
Reach 1	920,000	4,770,400	6,433,200	11,721,500
Reach 2	810,000	4,200,000	5,664,000	10,320,000
Reach 3	1,500,000	7,777,800	10,488,900	19,111,100
Reach 4	2,585,000	13,403,700	18,075,900	32,934,800
Reach 5	615,000	3,188,900	4,300,400	7,835,600
Reach 6	1,085,000	5,625,900	7,587,000	13,823,700
Total	7,515,000	38,966,700	52,549,400	95,746,700

2.4.3.2 Sea level rise

It is USACE policy that all coastal projects consider ramifications of sea level rise and have issued specific guidance for doing so. Figure 5 shows the predicted low, medium, and high curves for sea level rise at Galveston over the expected project lifetime (2025–2075); the information was obtained using the sea level rise calculator available at the website: <http://www.corpsclimate.us/ccaceslcurves.cfm>. This calculator incorporates the Corps of Engineers guidance found in USACE (2011) as updated by USACE (2014a).

The procedure used in this report is to add sufficient sediment to raise the entire beach profile (from the dune crest to the depth of closure; ~ 4000 ft in the cross-shore direction) by the same amount as sea level rises. The project lifetime amounts needed are shown in Table 4. The conservative curve should be used for planning purposes, but the amount of new material introduced into the system should be based upon the sea level rise curve as it actually happens in this location. Renourishment should occur at regularly scheduled intervals.

Figure 5. Relative low, medium, and high sea level curves (RSLC) based upon USACE guidance for the Galveston, TX, Pier 21 National Oceanic and Atmospheric Administration (NOAA) Tide Gauge: 8771450.



2.4.3.3 Sediment compatibility

Additional issues must be addressed if the grain size distribution of the fill material does not closely match the native sediment at the site. This is not likely to be an issue if material is taken off the beach and backpassed along the island to where it is needed. However, sediments dredged from the channel or offshore may contain more fines (silts and clays) than the native beach sediment. In this case, allowances must be made for a certain percentage of the fill material being winnowed by wave action from the beach face. This analysis must wait until the source of the fill is finalized. For further discussion, see National Research Council (1995), Gravens et al. (2008), and Dean (2002).

2.4.3.4 Longshore transport sediment gains

The only outside source of sediment made available by longshore transport is the amount transported from the Bolivar Peninsula through the North Jetty into the Galveston Entrance Channel, estimated at 110,000 yd³/year from Figure 10. This material could be used on Galveston beaches whenever channel maintenance dredging occurs.

2.4.3.5 Onshore transport sediment gains

Based on balancing the sediment budget, onshore movement of a substantial volume of sediment occurs at East Beach (Figure 10;

355,700 yd³/year). This sediment may be derived from the offshore dredge disposal site (Morang 2006) or from the collapse of the ebb delta following jetty construction (Morton 1977; Siringan and Anderson 1994). Regardless of the source, the material will accumulate on East Beach, particularly if the South Jetty is sand tightened (sand tightening refers to filling voids in the jetty to reduce sand transmission). Rather than allowing the accumulation, it would be preferable to include this material with the amount that is to be backpassed. GenCade modeling indicates that most of the material arrives on the beach near the South Jetty at the eastern tip of East Beach. While the source may be poorly understood, this is a significant addition of new sediment to the system.

2.5 Short-term alternatives

The sand placement alternatives presented in Section 2.4 assume unlimited funding and sand. However, both of these factors will constrain the type of beach that can be built along Galveston Island. Therefore, this study includes a number of smaller-scale alternatives that can be implemented in the short term.

At present, beach placement on Galveston Island only occurs after a major event like Hurricane Ike, when sufficient funds are available to correct hot spots or build small beach fills. The beaches now are relatively narrow, and they are not nourished at any set interval. While this process can be continued into the future, it is not a sand management strategy. Additionally, the present actions do little to recycle the existing sand on the island. Sand is a precious commodity in Galveston. There is a limited volume of beach-ready sand, and once it moves into the Galveston Entrance Channel or into San Luis Pass or farther down the coast, that sand is lost unless it is replaced through beneficial use dredging, mining, or backpassing. Because the coast is one of the main reasons tourists visit the area, there should be a better policy in place than emergency and spot placements.

In the short term, Reaches 1 and 2 are the highest priorities. They are the “Face of Galveston” and the main attractions for tourists. If funding and sediment supply are limited, the beach along Reach 2 should be nourished prior to nourishing any of the other beaches. Even small beach fills of 100,000 yd³ renourished every 5 years could help keep the beach from retreating to the seawall. This option will not widen the beach to a desired

200 ft width but at the minimum, will provide future generations with a beach similar to the present conditions.

Additionally, if there is not enough funding or sand for a large-scale beach nourishment project immediately, another option would be to place approximately 250,000 yd³ of sand along the beach and then renourish the same volume every 5 to 10 years. While more material would be lost over time than an initial large-scale placement, a specified renourishment interval would protect the existing beach and provide a slightly wider beach for recreation.

2.6 Other possible alternatives

In addition to the large-scale beach fill, backpassing, and low-level, short-term alternatives, other alternatives were considered during this study. One of these alternatives looks at the big picture while others are alternatives geared to research.

The Houston area has been growing constantly. In 2012, the greater Houston area became the fifth largest metropolitan area with 6.22 million people (Pulsinelli 2012). It is expected that under a moderate growth scenario, the Houston metropolitan area will grow to 10.27 million by 2050 (Greater Houston Partnership Research Department [GHPRD] 2014). The population could reach 14.41 million by 2050 under a fast-growth scenario (GHPRD 2014).

If the population in and around Houston doubles over the next 50 years, then the number of tourists who visit Galveston and the population of Galveston will substantially increase. At present, most of the hotels are located along the seawall east of 61st Street. If tourism doubles or triples, more hotels will need to be built farther to the west along the seawall or even along the west end. Additionally, as the population of Galveston Island increases, high-rise condominiums could be built to meet the increased demand for real estate and property. If the population increases significantly, the west end of Galveston Island could look more like present day Galveston Island behind the seawall. One idea to protect the entire island is to extend the seawall. It would be costly, require many permits, and take years, but it is an option when considering the long-term, multicentury strategy of population growth in the face of rising sea level. It would drastically change the shoreline of the island and require periodic nourishment in front of the seawall. At the time of this study, this proposal

was being considered in the Coastal Texas Protection and Restoration Reconnaissance Study (late 2014).

Another option along the west end of Galveston Island is nearshore berm placement. During Fiscal Year (FY) 2015, the SWG plans to place material dredged from the entrance channel in the nearshore along the west end of the seawall. As of early 2015, plans were still in progress. Nearshore placement is a way to use wave action to move sand onshore without placing it directly (mechanically) on the beaches. The berm at Ft. Myers, FL, was constructed with dredged material from Matanzas Pass. The dredged material contained greater than 10% fines, which Florida law does not allow for subaerial placement (Beck et al. 2012). Therefore, not only could a nearshore berm be placed in shallow water along the west end, a demonstration could be conducted to determine how finer material would react. Both short- and long-term beach fill performance monitoring of the placement would be necessary. The CHL is conducting research in this area under the Coastal Inlets Research Program (CIRP).

This study does not include engineering calculations or analyses for a nearshore berm placement; if a decision is made to move forward with a nearshore berm, significant engineering analyses and design would need to be conducted prior to construction.

2.7 Additional considerations

2.7.1 Need for phased development of the selected alternative

Regardless of the alternative(s) chosen, the Galveston Island restoration will be a large project with many unique features. No amount of predictive modeling can provide all the answers. It will be important to implement an adaptive management strategy and to seek appropriately flexible design strategies. If a partial fill is constructed first, it should be monitored to track performance over time. Based on the findings, construction may continue along the rest of the reaches. If the beach is not responding as expected through studies and modeling, the design should be modified before continuing to other reaches. If backpassing is chosen in addition to initial large-scale beach fills, the rate of backpassing should start low to ensure the system is functioning properly and the plant does not have an adverse effect near the jetty. Once it is confirmed that the system is working, the rate of backpassing may be increased.

2.7.2 Additional studies

This report should be considered a preliminary examination of the project. More detailed investigations will be required to guide and refine the selection of a final alternative. The application of an advanced economic model such as Beach-FX will provide a detailed analysis of projected costs and benefits of different alternatives. Benefits can include storm damage reduction with tourism as a secondary benefit. Other types of analyses will include the detailed design of the components of the bypassing system and geotechnical exploration and laboratory testing with sediment compatibility analysis.

2.7.3 Data collection needs for monitoring

A well-developed monitoring program will be a key component of this project and a requirement for an adaptive management strategy. Certain data should be collected for the life of the project and continuously analyzed to monitor trends and issues as they arise. For certain data sets, the collection process should start well before construction begins. Environmental monitoring will be needed during preconstruction engineering and design (PED) as well as evaluation of endangered species and other nearshore habitat constraints (essential fish habitat, sea grass, etc.).

Following construction, beach survey data should be collected quarterly for the first year and thereafter annually and possibly after severe storms. Georeferenced vertical aerial photography should be taken at the same intervals. The profiles and aerial photography will be used to analyze shoreline change and movement of the sand. If the shoreline responds as expected, the next phase of implementation of a wide beach along Galveston Island may proceed. Along with physical process monitoring, wave data may be needed from a nearshore wave gauge. The closest National Data Buoy Center buoy is Station 42035, 22 nautical miles (NM) east of Galveston (29°13'54" N 94°24'46" W).

2.7.4 Funding

Any option which develops a wider beach along part or all of Galveston Island will require a number of funding streams to help cost share. First, the GPB receives funding for beach nourishment. One dollar for each vehicle that enters any of the beach parks goes to beach nourishment

funding. Approximately \$200,000 is collected annually. Additionally, 0.5667 cent out of the state hotel occupancy tax and umbrella and chair concessions at the parks goes toward nourishing the beaches. The tax provides approximately \$600,000 annually, and the account has not been used in approximately 3 years¹. This means the GPB has close to \$2,000,000 available for beach nourishment. As of late 2014, the Galveston Industrial Development Corporation (IDC) has a fund balance of \$3,000,000 for beach nourishment. Any future beach nourishment projects should involve cooperation with the IDC.

Evaluation of Coastal Storm Risk Management for Galveston beaches has been included in the Coastal Texas Feasibility Study, scheduled to start in FY 2017. It is unknown how this will affect Galveston Island beach projects. For near-term projects that involve the beneficial use of dredge material from the Galveston Entrance Channel, the GPB can coordinate with the USACE SWG and serve as a non-Federal sponsor for cost sharing material for placement on the beaches of Reach 2. Other non-Federal sponsors could include the Texas General Land Office (GLO) and the IDC.

The GLO also provides funding through the Coastal Erosion Planning and Response Act (CEPRA) Program. The main purpose of CEPRA is to implement coastal erosion response projects and studies to reduce the effects of erosion and better understand the coastal processes. Typical projects considered for funding include beach nourishment, dune restoration, demonstration projects, and shoreline stabilization. Any of the options discussed so far fall under the typical projects. A nearshore berm placement or placement of finer material on the beach might be reasonable projects to request funding. Applications for the last CEPRA funding cycle were due 1 July 2013 for FY 2014–2015. Based on those dates, the next applications will probably be due in the summer of 2015 for FY 2016–2017.

Another possible source of funding is the Federal Emergency Management Agency (FEMA). However, emergency FEMA funds are normally only available when a beach has eroded to a state of emergency and provides no protection for upland structures or when facilities have been damaged during a storm. Even if one of these requirements is met, the applicant must also provide design studies, plans, and documents for the original nourishment and any renourishments, details and documentation for the

¹ Kelly De Shaun, Director, Galveston Park Board of Trustees, personal communication, 14 Oct 2014.

maintenance plan, and pre- and post-storm profiles. Based on these requirements, FEMA funding may only be available for the Galveston beaches after the beach is nourished and a storm event causes significant damage afterwards.

In summary, funding for the beach fill may be available from GPB revenues, the Galveston IDC, the GLO CEPR program, and the Park Board can partner with the SWG to beneficially use dredge material. In addition, under some circumstances, FEMA funds may be available after major storms.

2.7.5 Additional considerations

Several additional considerations deserve mention. It is fundamentally important to have community involvement and support. This should be started as early as practical for citizens to have the opportunity to voice their concerns. Public meetings/public comment periods are generally done as part of the National Environmental Policy Act (NEPA) scoping process.

Environmental concerns will be a major factor. Environmental partners and agencies can help promote ecotourism and opportunities for habitat mitigation, restoration, and enhancement.

Engineering-type activities should be included in any final plan. For example, the 100+ year-old Galveston Seawall should be analyzed in light of the expected rise in sea level as well as the vulnerability of the low bay side of the island.

2.7.6 Basic beach management policy

The GPB's mission statement is to position Galveston Island among the top five tourist destinations in Texas by accepting responsibility for tourism, Galveston's number-one industry. As part of a beach management strategy, there are certain steps the GPB should take to meet their mission. By widening the beaches, Galveston could become a prime vacation destination for many Texans.

First, with an improved beach and the potential for more tourists due to population growth, GPB can make the beach experience more interactive and friendly. The Galveston Pleasure Pier reopened a few years ago with

new dining, shops, and rides and is now a main attraction for visitors. Expanding the boardwalk is one idea to cater to larger crowds of tourists. Another important part of revitalizing the beach is adding signs. Many beach communities have signs that explain the processes along the beach, the need for beach nourishment, and the nourishment policies in place. Sand backpassing is still a relatively rare activity, so including public information about it would be very beneficial for visitors.

In order to accommodate additional tourists, a number of management and maintenance policies should be in place. First, the GPB and the City of Galveston must continue to maintain the beaches and dunes. If a dune is damaged due to human alterations or storms, there must be a system in place to restore the dune. At present, the GPB's beach maintenance policy includes wrack maintenance and trash removal from its parks and at the Seawall Urban Park (10th Street to 103rd Street). If all of the beaches along Galveston Island are nourished and widened, they should all be maintained by the GPB or another group. In addition, sand from the beaches typically deposits along Seawall Boulevard or in parking lots across the street. The GPB and the City of Galveston collect the sand and deposit it back onto the beaches. This maintenance activity should continue in the future either by removing debris and placing the material back on the beaches or storing it for future use.

When the dunes are constructed, beach grass should be planted, and sand fences should be erected to minimize erosion or possibly encourage growth. Dune walkovers should be constructed and located at every parking lot or access point along the west end and at every major street intersection along the seawall. These dune walkovers should be angled to reduce waves and surge from storms. It is very important that visitors understand that they may not walk across the dunes. Therefore, signs stating not to walk on dunes and the reason why should be placed at strategic locations along the dune system.

Finally, an increase in the number of visitors to the Galveston beaches could cause parking problems. At present, visitors park along the seawall, at one of the parks, or on the beach. In the future, additional or larger parking lots should be added so that visitors do not park on side or residential streets. The GPB should limit the number of vehicles, enforce speed limits, and specify locations on the beaches that allow driving.

2.7.7 Additional information

Members of the GPB may find the following references helpful making informed decisions.

- Dean, R. G. 1988. Realistic economic benefits from beach nourishment. In *Twenty-first International Conference on Coastal Engineering, ASCE*, Chapter 116, 1558–1572. Malaga, Spain, June.
- Dean, R. G., R. A. Davis, and K. M. Erickson. 2016. Beach nourishment: A guide for local government officials. Accessed 24 May 2016.
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3 Galveston Island Sediment Budget

3.1 Background

Morang (2006) calculated a sediment budget for the north Texas shore between Sabine Pass and San Luis Pass as part of the Sabine Pass to Galveston Bay, Texas–Shoreline Erosion Feasibility Study. Using additional and more recent data, the sediment budget has been revised for Galveston Island. The mechanics of the budget process will not be described herein but can be reviewed in Morang (2006). The present study includes 11 budget cells, extending from just north of the north jetty to San Luis Pass (Figure 3). This new budget represents average pre-Hurricane Ike conditions covering the mid-1980s to mid-2000s. Hurricane Ike was a powerful and destructive storm event, and the sediment effects are still being evaluated. Post-Ike beach fills (when data were available) have not been included in the placement averages.

The sediment budget in the entrance channels was based on two sources of dredging data:

- Mid-1980s to approximately 2010: internal dredging database supplied by SWG
- 2010–2013: spreadsheet based on SWG RMS data¹.

Table A1 in Appendix A lists dredging volumes used in this analysis. The values used in the 2006 analysis are included for comparison. Units in the table are cubic yards per dredging contract or event.

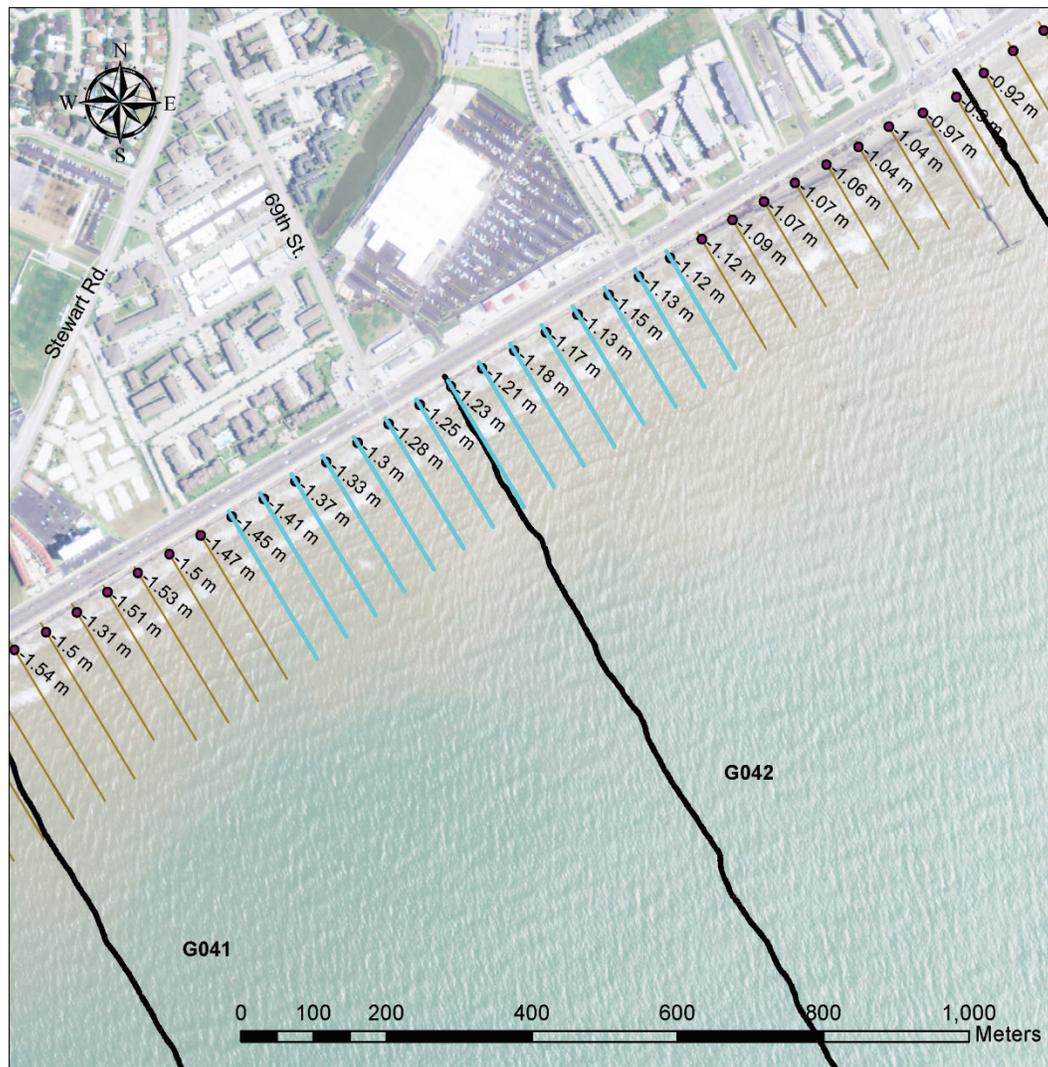
Table B1 in Appendix B lists Gulf-side beach fills or sand placements. These data were originally supplied by Shiner Moseley and Associates and updated by Coastal Strategies Group, LLC, in 2014². As stated above, post-Hurricane Ike beach fills have not been included in the placement averages. West Bay dredging was tabulated by Atkins Global (2012), but these events did not affect the ocean side beaches.

¹ Tricia Campbell, Operations Manager, SWG, personal communication, 21 Feb 2014.

² John Lee, Coastal Strategies Group, LLC, personal communication, 18 Feb 2014.

Beach volume changes were based on shoreline change statistics and cross-shore beach profiles. Texas Bureau of Economic Geology (BEG) supplies shoreline shapefiles via its web page (<http://www.beg.utexas.edu/coastal/>). The Texas shoreline change project has been ongoing since the late 1990s and is the definitive source of shoreline change statistics for the Texas coast. The statistics used in this study cover historical change through 2007, and in many areas include over 100 years of data (Paine et al. 2011; 2012) (Figure 6). Short-term, post-2010 shoreline change rates were not used because of the extreme influence of Hurricane Ike (Paine et al. 2013). In addition, the authors are not aware of repetitive cross-shore profiles for the post-2008 period to evaluate post-storm recovery.

Figure 6. Example of shoreline change transects from Texas BEG and 2002 Texas A&M cross-shore beach profiles. Highlighted transects were averaged to compute a single retreat rate for profile G042.



Texas A&M University conducted cross-shore beach surveys during August and September of 2002 under contract to SWG. Profiles were spaced at half-mile intervals (approximately 1000 yd) and were oriented perpendicular to the local shoreline (Figure 6). They extended from the dune or a prominent man-made feature (e.g., seawall or building) to approximately the 10 m water depth. These profiles were also used in the 2006 study. Profiles from 2006 to 2007 were available for some areas, but their origins (end points on the beach) were different, and effort is required to make them directly comparable, which is outside of the scope of this work.

Computing volume change required a series of steps:

1. Import the 2002 profiles into the Beach Morphology and Analysis Package (BMAP) software (Sommerfeld et al. 1994).
2. Using the Texas BEG average shoreline change values from each 50 m transect, compute a simple arithmetic mean for the shoreline change for each budget cell along the coast.
3. Obtain an average profile based on all cross-shore profiles within each cell.
4. With the translation tool in BMAP, translate the average profile the appropriate distance seaward (shoreline advance) or landward (retreat).
5. Using the area under the curve (computed by BMAP), multiply by the length of the cell to obtain a total sediment volume in cubic yards.

For this study, a depth of 10 ft was used as the active zone (the black box in Figure 7). The active zone is the part of the shoreface where ordinary day-to-day fair weather waves move sand onshore and offshore between the beach and offshore sand bars.

The SWG collected sediment samples in shoaled areas of the navigation channels and in the previously used upland placement areas for sediment characterization. Sediment types were tabulated and plotted in Esri ArcMap Geographic Information System (GIS) software for the 2006 study, providing a convenient way to visualize the sediments in different portions of the channels (Figure 8; Figure 9). Based on the samples located landward of the tip of the south jetty, an average sand content of 86% has been used in the budget computations.

Figure 7. Example of procedure used to compute area under curve for two profiles using BMAP software. The profile shown is an average profile for an individual cell. The average profile has been translated an amount equal to the average shoreline change (either retreat or advance) for that cell. Volume under each curve is supplied in units of cubic yards/foot.

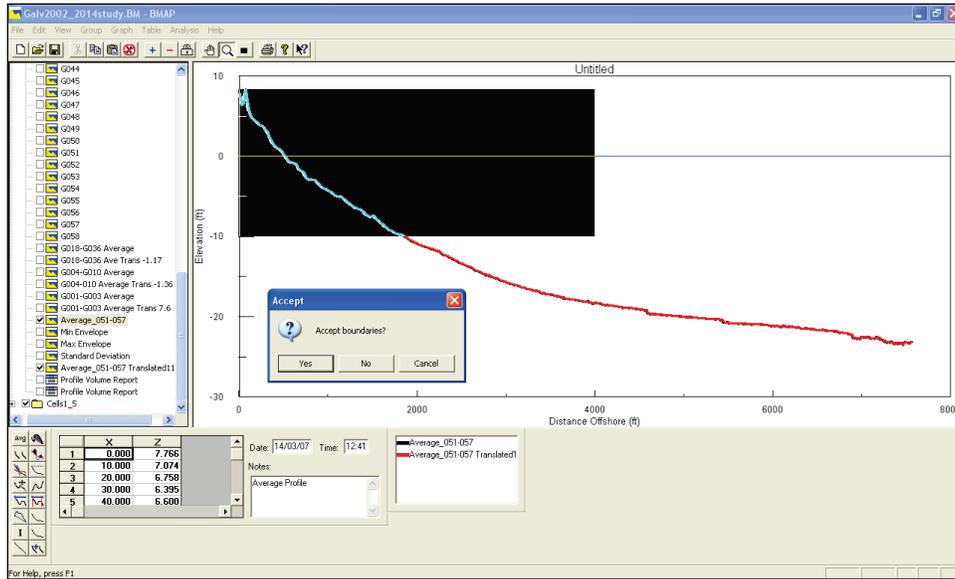


Figure 8. Gravel, sand, silt, and clay distributions of sediment samples from Galveston inner bar and anchorage areas. Numbers represent sand percentage.

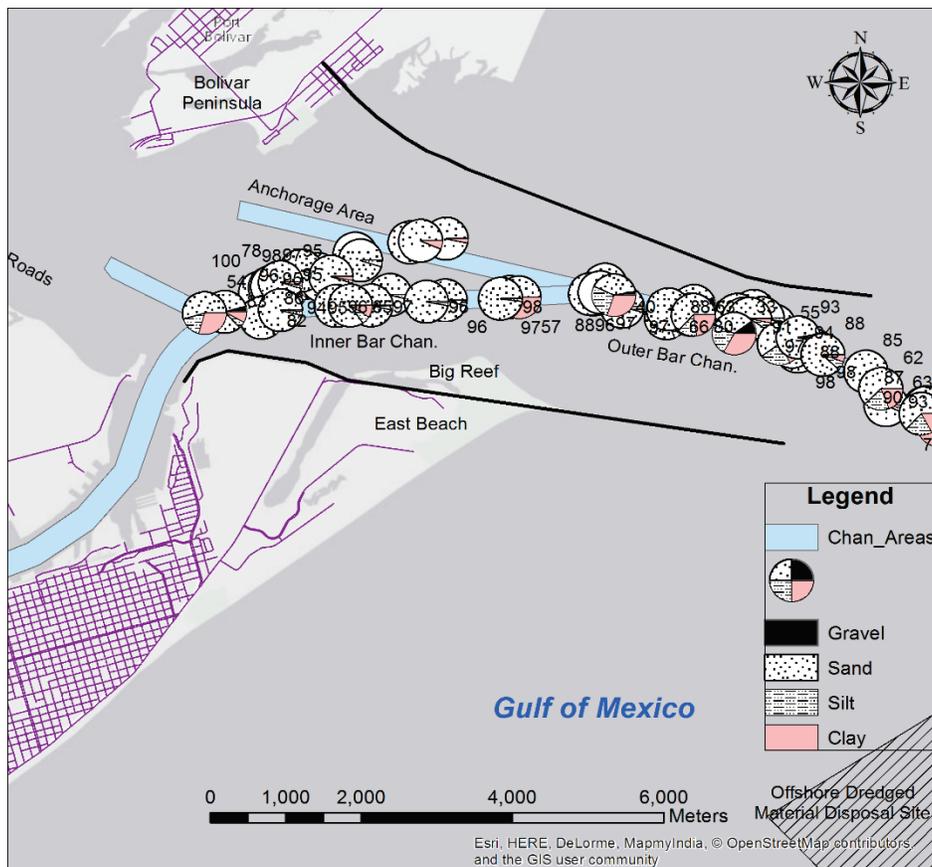
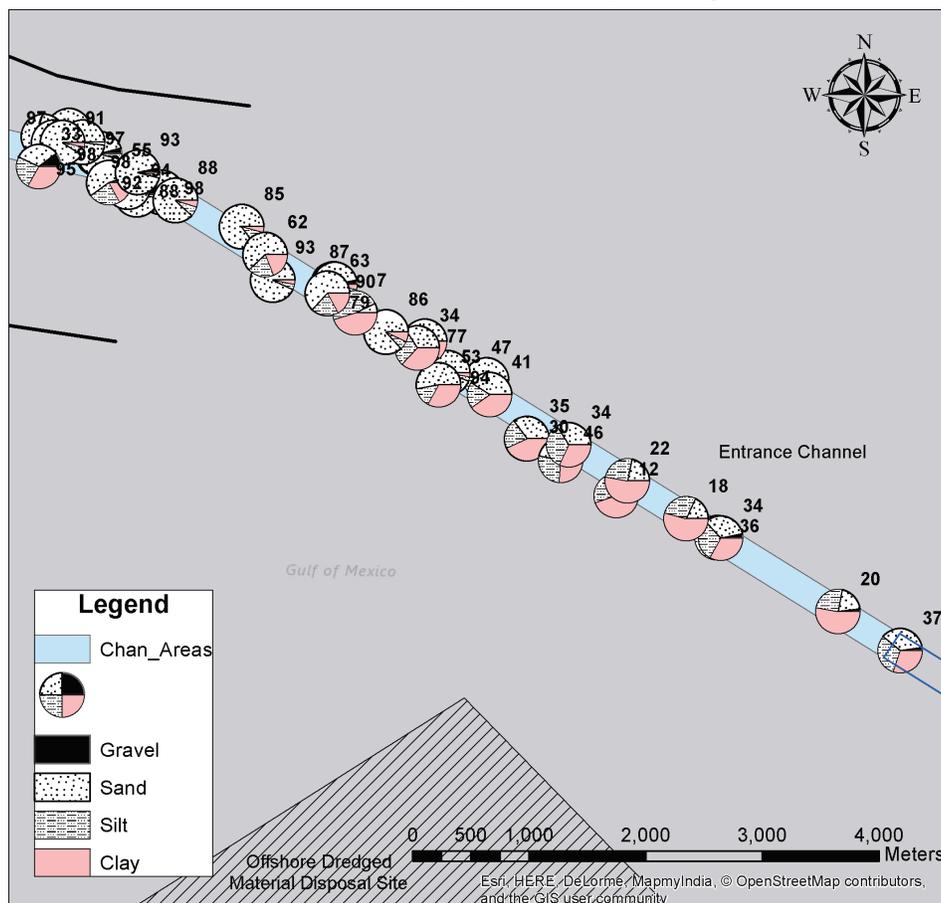


Figure 9. Gravel, sand, silt, and clay distribution for samples along the entrance channel. Numbers represent sand percentage.



3.2 Results

Table C1 in Appendix C summarizes the fluxes, placements, and volume changes in each cell. The second column lists the parameters used in the budget such as sediment volume change (ΔV). These are the same parameters used in the SBAS software. The third column contains the expected values for the parameter (multiyear average) used for the sediment budget in cubic yards/year. Note that some parameters do not apply for some cells

Standard error estimates are difficult to predict. Estimates of error, or \pm values, had to be computed for each parameter. In Table C1, column 4 contains the minimum (-) values, and column 5 contains the maximum (+) values.

- Sediment volume (ΔV) was the term with greatest variability over time. Beach retreat was computed from the BEG shoreline change statistics.

However, during a mild winter, a beach may experience negligible retreat while during a harsh winter, retreat may be much greater than the multiyear average. For this study, the minimum value of ΔV was assumed to be 50% of the average while maximum was two times or 200%.

- Placement (P) was reliable for most sites along Galveston Island because truck loads are tabulated. However, some nourishments may not have been recorded or were performed unofficially. The maximum P value is assumed to be 25% greater or 1.25 P.
- Dredge volume (R) for the navigation channels was based on spreadsheets from SWG. The volumes reported for each contract are accurate representations of the volume actually removed from the channel, and shoaling during the production did not significantly change the reported volumes¹. Therefore, \pm values are the same as the reported volume.

3.2.1 Cell 1_1: Galveston north fillet

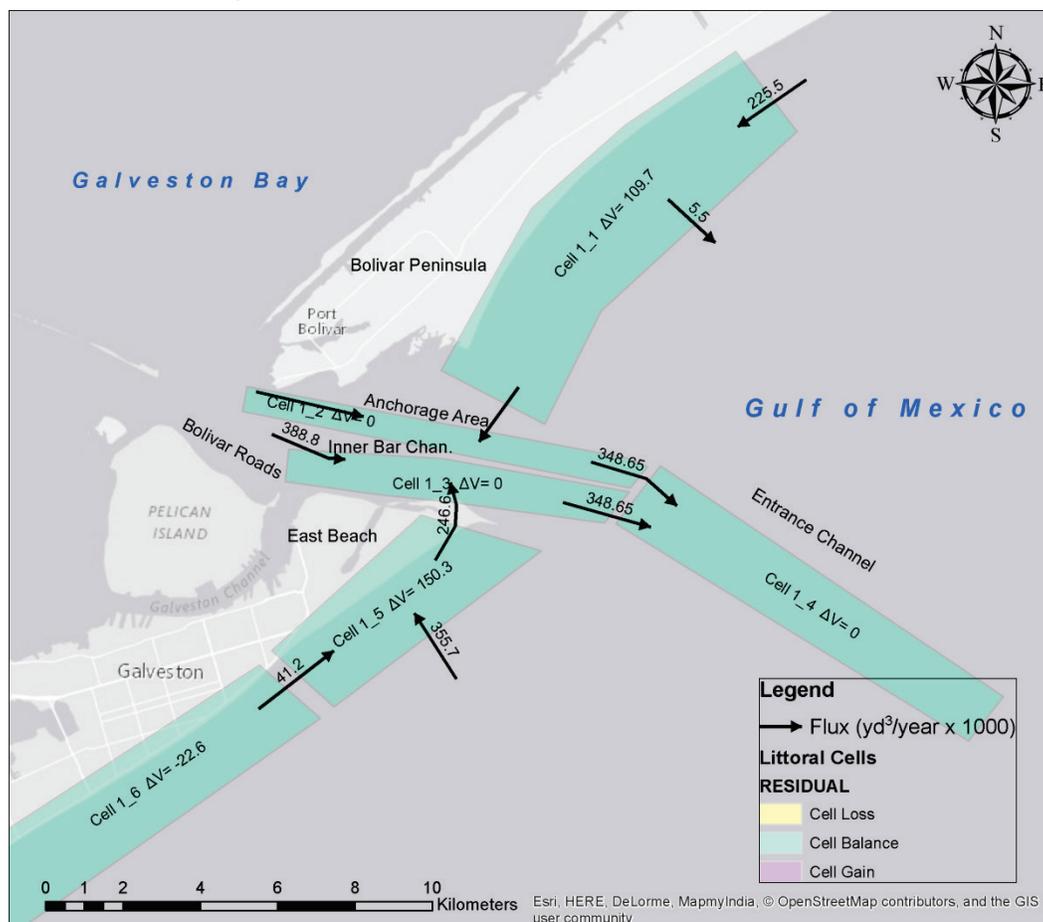
Cell 1_1 is north of the Galveston north jetty. This cell, Cells 1_2, 1_3 and 1_4 in the channel, and Cell 1_5 (East Beach) are interconnected with a complex pattern of sediment exchange (Figure 10).

Cell 1_1 has accumulated a significant quantity of sand since the jetties were built in the 1880s. The beach has advanced since the 1970s, indicating that sand input exceeds sand losses through the porous jetty. Whether sand passes south through the north jetty is not immediately obvious based on the beach morphology. Just north of the jetty is a marshy, open water area rather than a traditional fillet built up against the structure. A comparison of the 1956 shoreline and the 2012 aerial photography shows that the open water area has remained, although has diminished (Figure 11). Two reasons may account for the marshy zone:

1. Wave energy in this area is low because of the shadow effect of the jetty. Therefore, littoral currents lose most of their sediment load farther north of the jetty.
2. Sand moves through the porous jetty at a rate sufficient to prevent the accumulation of a fillet directly against the structure.

¹ Tricia C. Campbell, Operations Manager, USACE, Galveston District, personal communication, 5 Mar 2014.

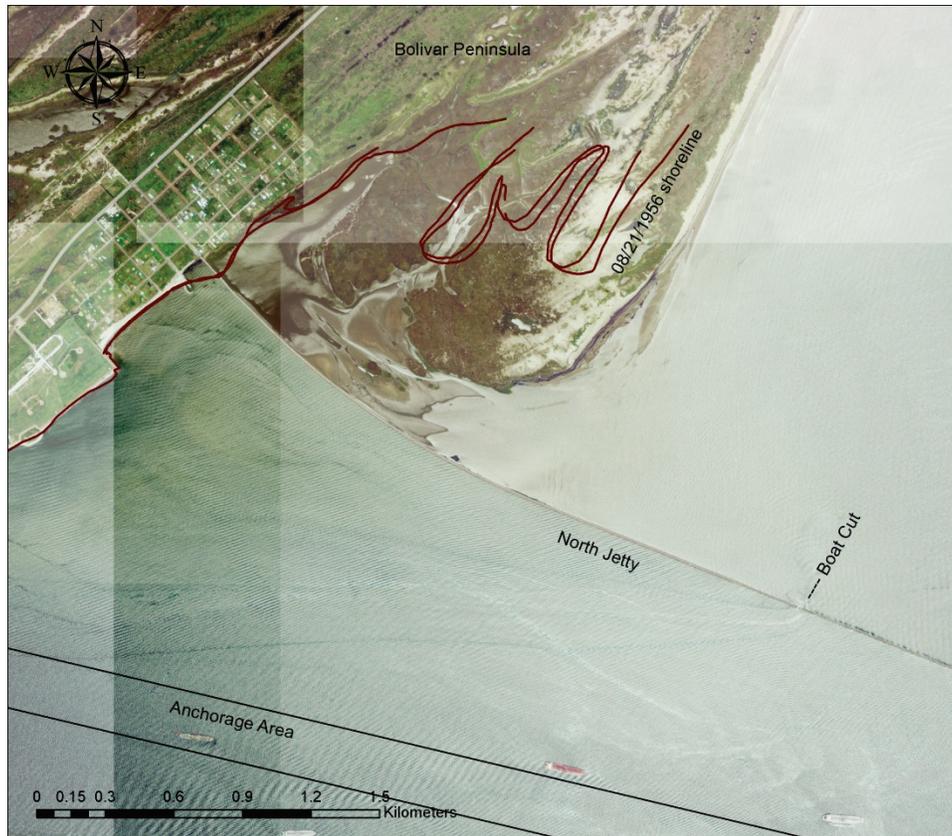
Figure 10. Sediment budget cells in the Galveston Entrance area. Units are in cubic yards/year \times 1000. ΔV represents sediment volume change in each cell (beach accretion or erosion). The flux arrow shows the direction of sediment movement.



An opening was cut in the north jetty in 1964 to allow the passage of small boats. Cross-shore profiles from the 1960s and 1970s show that the seafloor became deeper in a semicircle around the cut over the following years, indicating sand loss into the channel¹. Whether sand passes through the jetty away from the cut is less clear. Radioactive tracer studies in the 1960s showed that following release of material at 3 and 6 ft water depth north of the jetty, tracer was detected south of the structure. The authors concluded that some material may pass through the porous structure, but the patterns indicated that most of the material passed through the small boat opening (Ingram et al. 1965).

¹ Robert C. Thomas, SWG, personal communication, 10 Mar 2014.

Figure 11. Bolivar Peninsula, 22 May 2012. Despite growth of the beach updrift of the jetty, there is still a marshy, open water area immediately north of the structure. The boat cut is at the right side of the jetty (photograph from the Texas Natural Resources Information System, 1956 shoreline from Texas BEG).



Based on translating the 2002 profiles by 1.12 m/year end-point rate, $\Delta V = 109,700 \text{ yd}^3/\text{year}$.

Assumption 1. Sand movement through the jetty into the Galveston anchorage area equals approximately $110,000 \text{ yd}^3/\text{year}$. This is based on the total volume dredged from the anchorage area (Cell 1_2) multiplied by the average sand percentage of sediment samples from the inner and outer bar channels (86%).

Assumption 2. Sand loss offshore is minor, approximately $5,500 \text{ yd}^3/\text{year}$. This term is necessary to balance the cell considering fillet growth, loss through the jetty, and littoral transport.

3.2.2 Cell 1_4: Galveston entrance channel

Cell 1_4 covers the portion of the channel from Sta 31+000 seaward past the ends of the jetties (Figure 10). Average 1979–2013 maintenance dredging was 697,000 yd³/year.

Assumption: The majority of the material that fills the dredged channel comes from Galveston Bay and the inner bar channels. Some material may come from the ebb shoal during storms. Tests of radioactive tracers in the 1960s showed that material released near the outer end of the north jetty moved quickly around the north jetty and thence into the navigation channel (Ingram et al. 1965). Surveys using sidescan sonar might resolve bedform patterns to test whether the ebb shoal contributes material to the channel.

3.2.3 Cell 1_2: Anchorage area

Cell 1_2 captures the anchorage area north of the inner and outer bar channel (Figure 10). Dredging from 1978 to 1997 averaged 128,000 yd³/year.

Assumption 1: Half of the material that moves out to Cell 1_4 (entrance channel) passes through this cell. The volume out is 349,000 yd³/year.

Assumption 2: Littoral input from Cell 1_1 (north of the north jetty) is 110,000 yd³/year. Based on over 60 samples (Figure 8), the average sand percentage of bottom samples equals 86%. It is possible that these samples, taken during dredging operations from inside the hopper are biased towards coarser grain sizes. One reason is that the cutter head, as it moves across the bottom, stirs up the fines, which dissipate. Also, the overflow from the hopper carries away many fines. However, without some other geotechnical data, such as a sampling grid consisting of bottom grabs and/or cores, the authors used the percentages as stated in the 1950s to 1990s USACE dredge reports. For Cell 1_2, the only source of sand is littoral material passing through the north jetty. There is no sand source in Galveston Bay. Sand is carried down the Trinity River and deposited in Livingston Lake, creating a delta in upper Trinity Bay. However, it is too far from the Galveston entrance to serve as a sediment source for the anchorage area (Phillips and Musselman 2003).

Assumption 3. To balance the cell, the remaining material that fills the anchorage area consists of silt and mud from Galveston Bay (approximately 367,000 yd³/year).

3.2.4 Cell 1_3: Inner and outer bar channel

Cell 1_3, the inner and outer bar channel follows the south jetty and provides access to Galveston Harbor and the Houston ship channel. In the period 1980–1999, dredging averaged 273,000 yd³/year.

Assumption 1: This cell is the source of half of the material that moves out to Cell 1_4 (entrance channel), 249,000 yd³/year.

Assumption 2: Littoral input is approximately 247,000 yd³/year. Based on over 60 sediment grab samples, the material removed from the channels averages 86% sand (see discussion in Cell 1_2 paragraphs). The 21 westernmost samples had higher sand content, averaging 93% (Figure 8). The high sand content is unexpected, considering that most of Galveston Bay is a muddy environment. The only source of sand is littoral material passing through the south jetty via Big Reef or wind-blown sand from East Beach.

Assumption 3. To balance the cell, the remaining material that enters (and passes through) the inner and outer bar channels consists of silt and mud from Galveston Bay (approximately 389,000 yd³/year).

3.2.5 Cell 1_5: East Beach (Park Board Reach 6)

Cell 1_5, East Beach, located south of the south jetty, has grown steadily in the 120 years since the jetty was built. Unlike the fillet to the north, here sand has accumulated directly against the jetty. The jetty is porous, as shown by the steady growth of Big Reef, a sand body that projects northward into the navigation channel (Figure 11). The reef is occasionally mined for sand for beach nourishment (Appendix B).

Based on translating 2002 profiles by +11.25 ft/year, with an active depth of -10 ft, $\Delta V = +150,000$ yd³/year.

Assumption 1. Sand movement through the jetty into the inner and outer bar channel (with temporary storage in Big Reef) area equals 247,000 yd³/year. This value is based on dredging of the bar channels multiplied by the average sand percentage of sediment samples (86%).

Assumption 2. Onshore sand movement equals 356,000 yd³/year. This is the only way to balance the cell considering fillet growth, loss through the jetty, and minor littoral input from the south.

Determining exactly how much material moves onshore near East Beach needs to be evaluated in greater detail. Seismic studies conducted by Texas A&M University detected sandy facies offshore south of the jetty, making an offshore sand source feasible¹. Dellapenna and Johnson (2012; their Figures 15 and 16) show sand and coarse sand facies off East Beach based on side-scan sonar surveys and vibracores.

The 1960s movable bed model studies also demonstrated bed movement onshore. With wave direction of S 29° E and S 37° E, bed movement was divided, some material moving north of the south jetty but most onto East Beach. With wave direction of S 37° E and S 66° E, bed movement was exclusively onto East Beach (Simmons and Boland 1969, Plates 59–62). Based on 1960s model studies (unspecified), USACE (1993a) also concluded that some of the material deposited in the USACE offshore disposal area (Figures 8 and 9) could feed Big Reef. Exact quantities disposed in offshore disposal area are not available, but during the mid-to-late twentieth century, much of the material dredged from the entrance channel was placed here.

However, as a contrary hypothesis, Hall (1975) wrote that the net sediment transport in the lower shoreface was towards the southwest and parallel to Galveston Island. Principal sediment transport agents were near-bottom currents generated by tides, which were superimposed on a semipermanent current flowing toward the southwest. He concluded, “Sandy material placed in the dredged material disposal area has little chance to ever return to the channel and will probably enter the longshore transport system and nourish beaches farther down the Texas coast” (Hall 1975, p. vi). Hall based his conclusions on theoretical considerations of bed shear calculated from monthly vertical current profiles. It is unclear if he considered large-scale morphological factors such as the growth of East Beach over time.

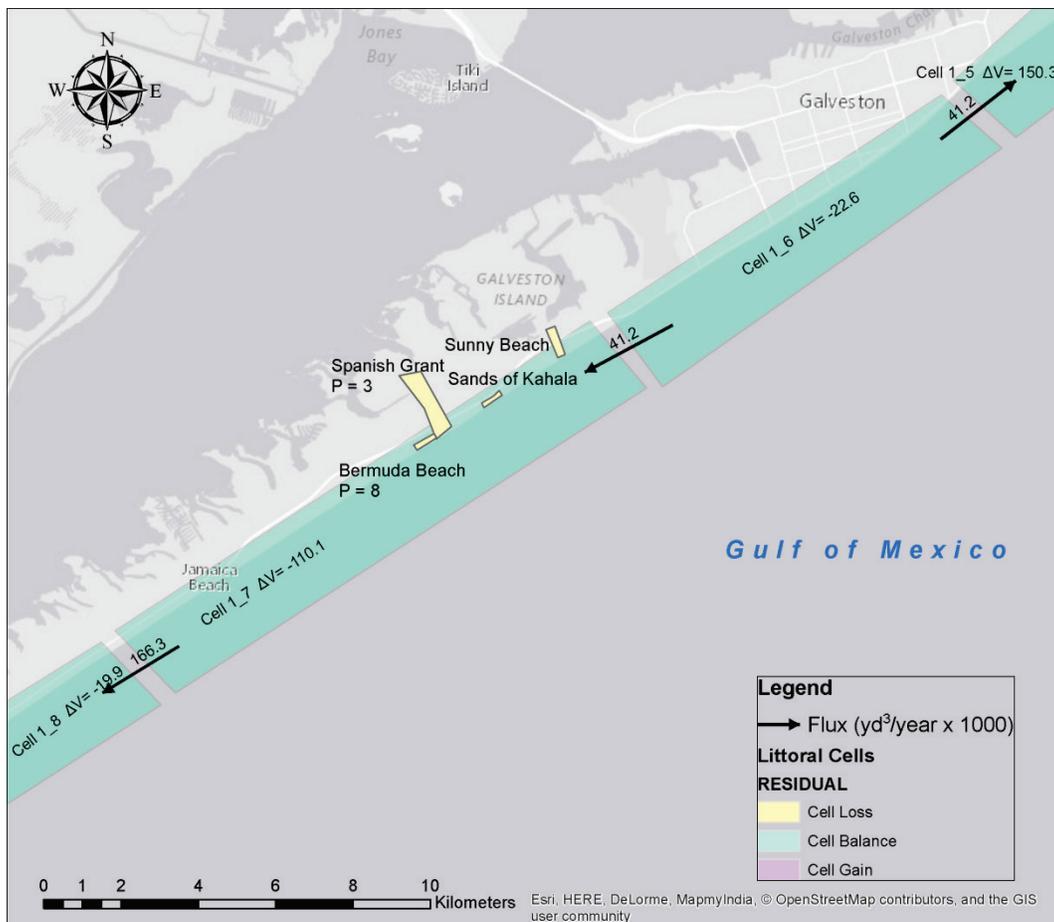
¹ Timothy Dellapenna, Assistant Professor, Department of Oceanography, Texas A&M University, personal communication, 12 Nov 2003.

3.2.6 Cell 1_6: Galveston Seawall (Park Board Reaches 1 and 2)

Cell 1_6, the Galveston Seawall protects the Gulf shore of Galveston Island for a total distance of 9.76 miles. The east portion of the wall is now inland because of the growth of East Beach, leaving the western 6.82 miles with direct Gulf exposure. The City of Galveston has historically been concerned about retaining a beach at the foot of the wall to provide recreation and protect the structure. As a result, a series of groins was built in the mid-twentieth century to trap or retain sand, and the City and private interests have placed sand on the beach at various times. Based on 1985 to 2008 records, average annual placement is approximately 60,000 yd³ (excluding the 2009 post-Ike fill).

Assumption 1. Because of the rigid seawall, the shore is essentially fixed, although the beach at the base of the wall has retreated over the years. Average shoreline change has been -0.72 ft/year, resulting in $\Delta V = -22,600 \text{ yd}^3/\text{year}$ (Figure 12).

Figure 12. Sediment budget for central Galveston Island. Units are in cubic yards/year \times 1000. *P* represents annual sand placement at beach communities in cubic yards/year \times 1000.



Assumption 2. This cell is a divergence zone. At the west end of the seawall, there is clear morphological evidence that net drift is to the west because the shore has cut back (Figure 13). This budget study assigned 50% of the littoral transport at each end, or 41,000 yd³/year to the east and the same amount to the west.

Figure 13. Eroded beach just beyond the west end of the Galveston seawall (19 February 2003). This is the border between Cells 1_6 and 1_7. Sand has been placed on the beach to protect the dune in front of the hotel.



3.2.7 Cell 1_7 (Park Board Reach 3 and east part of 4)

From Cell 1_7 and continuing west, net littoral sediment transport is to the west according to King's (2007) wave modeling studies. This agrees with most of the published literature for this part of Texas coast. The beach in Cell 1_7 has retreated 3.39 ft/year, resulting in $\Delta V = -110,000$ yd³/year.

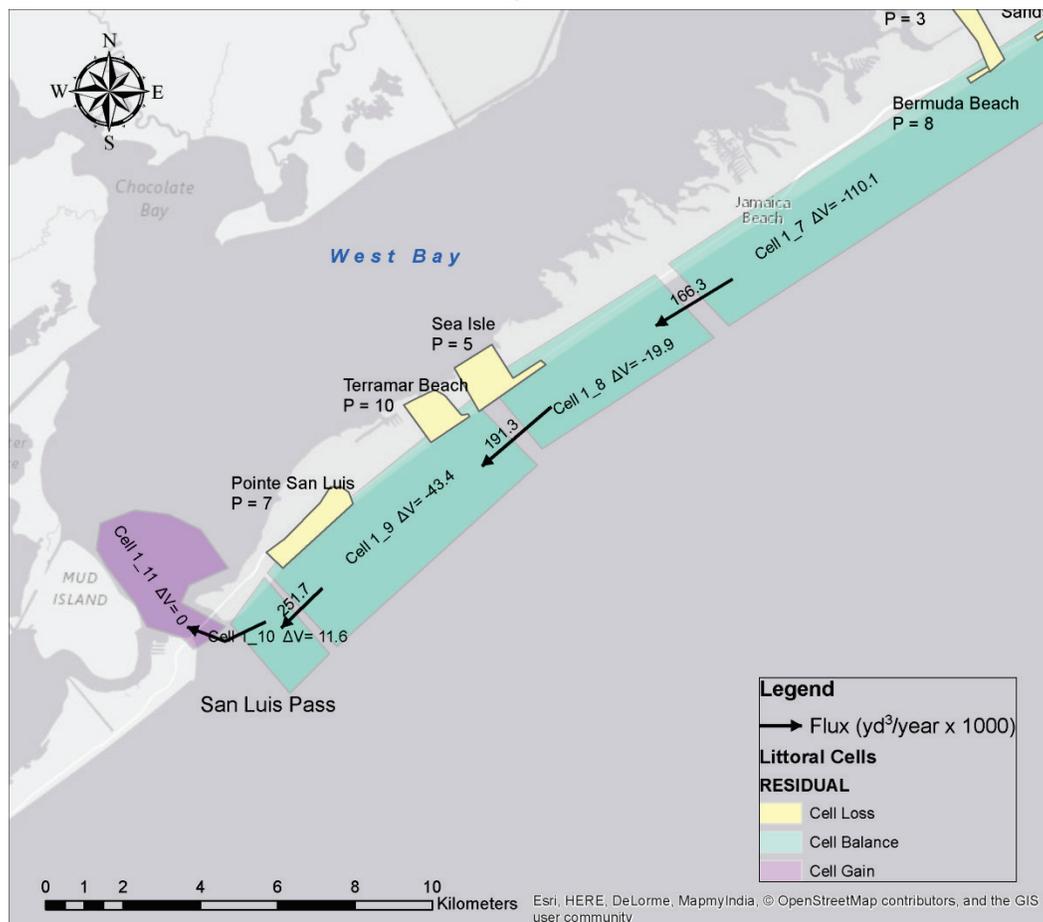
Assumption 1. Beach placement is approximately 15,000 yd³/year. Beach nourishments have been reported at Sunny Beach, Sands of Kahala, Spanish Grant, and Bermuda Beach, but records are incomplete.

Assumption 2. All the material removed from the beach moves west, with $Q_{LST2} \approx 166,000$ yd³/year.

3.2.8 Cell 1_8 (Park Board Reach 4)

Cell 1_8 is a semi-stable section of Galveston Island, with ΔV of -19,900 yd³/year (Figure 14).

Figure 14. Sediment budget along southwest end of Galveston Island. Units are cubic yards/year \times 1000. Annual volumetric change in the San Luis Pass flood shoal is unknown.



Assumption 1. Minor placements at Sea Isle equal 5,000 yd^3/year .

Assumption 2. Littoral material entering the east side of the cell continues on out the west side, with $Q_{LST} \approx 191,000 \text{ yd}^3/\text{year}$.

3.2.9 Cell 1_9: West Beach (Park Board Reach 5)

Cell 1_9 includes an eroding section of Galveston Island, with average retreat of 2.74 ft/year (Figure 14). This results in ΔV of -43,400 yd^3/year .

Assumption. With the addition of approximately 17,000 yd^3/year of beach material to the incoming littoral drift, littoral transport out the west side of the cell increases to 251,000 yd^3/year . This is a large net transport and needs to be verified with field studies.

3.2.10 Cell 1_10: San Luis Pass East (Park Board Reach 5)

Cell 1_10 includes the dynamic section of shore on the east side of the mouth of San Luis Pass (Figure 14). The pass has been in approximately this location since before 1853 (Mason 1981). Because the pass is unstructured, the marginal flood and ebb channels have migrated back and forth over time. The shoreline east of the mouth has advanced 3.68 ft/year in the last 25 years, resulting in $\Delta V = 11,600 \text{ yd}^3/\text{year}$.

Assumption. All remaining littoral material not accounted for in beach growth enters San Luis Pass (Figure 14). Therefore, $Q_{\text{sink}3} = 240,000 \text{ yd}^3/\text{year}$. It is possible that some material bypasses the mouth, but the west side of the pass does not have an obvious attachment bar, as is common at inlets with ebb shoals that bypass littoral material.

3.2.11 Cell 1_11: San Luis Flood Shoal

Most researchers believe San Luis Pass is a major sediment sink (Figure 14). Water depths range from -22 ft at the Gulf end of the Pass to only zero or -1 ft at the distal portions of the flood shoal (Atkins Global 2002). The ebb shoal may contain 4 million yd^3 of sand, but growth or loss rates are unavailable¹. Bathymetry coverage is insufficient to determine the quantities of sand involved. Some material may enter from the west side of the pass, but there was insufficient data to compute volume.

Assumption 1. The flood shoal is a sink for all littoral material entering the pass. Sediment input is at least $240,000 \text{ yd}^3/\text{year}$. The numbers cannot be refined until complete bathymetry surveys are available to document growth of the shoal.

Assumption 2. Some sediment may be moving from the flood shoal to the deeper portions of West Galveston Bay West Basin, but quantities were not reported by Atkins Global (2012). This flux will be treated as zero.

Mason (1981) discusses the inlet's history and stability and suggests that it is a significant sediment sink as well as a potential sand source for beach renourishment.

¹ James Gibeaut, Bureau of Economic Geology, University of Texas, personal communication, 12 Nov 2003.

4 GenCade Calibration and Alternatives

GenCade (Frey et al. 2012; Frey et al. 2014) was applied to Galveston Island to model shoreline change and longshore transport and to evaluate structural and engineering alternatives. This chapter describes the process used to set up and calibrate the model.

4.1 GenCade background

GenCade is a one-line shoreline change, sand transport, and inlet sand-sharing numerical model (Frey et al. 2012) based on the synthesis of Cascade (Larson et al. 2003), a design-scale, planning-level model, and GENESIS (Hanson and Kraus 1989), a project-scale, engineering design-level numerical model. GenCade calculates shoreline change in response to coastal structures and engineering activities such as groins, jetties, inlet dredging, seawalls, breakwaters, beach fills, and bypassing. The model was developed by updating the existing GENESIS code and incorporating the capabilities of Cascade. Development of GenCade began in 2009, and the first official release occurred in 2012. The newest version of the model (GenCade_v1r6.exe) was used for this study.

As a one-line model, GenCade is constrained by a number of standard assumptions consistent with the type of model. These assumptions are as follows and are described in more detail in Frey et al. (2012, 2014).

- The beach profile shape remains constant.
- The shoreline and seaward depth limits of the profile are constant.
- Sand is transported alongshore by the action of breaking waves and longshore currents.
- The detailed structure of the nearshore circulation is ignored.
- There is a long-term trend in shoreline evolution.

Some of the assumptions stated above have been stretched due to the processes on and around Galveston Island and the proximity to the Galveston Entrance Channel. Additionally, wind-blown transport and the possible movement of sand placed in the Ocean Dredged Material Disposal Site (ODMDS; Figure 1) to Galveston Island are not well known and should be researched. Wind driven surf-zone currents are also not included in the model. For these reasons, it is important to consider the

GenCade model results qualitatively and note that many of the alternatives modeled will greatly affect the coastal processes, so additional studies and analyses should be conducted before moving forward with any alternative.

4.2 Numerical modeling overview and approach

Two separate GenCade grids were developed for Galveston Island. The first grid model extends from the jetty at the Galveston Entrance Channel to the end of the seawall. The second GenCade grid model covers the extent from the end of the seawall to just north of San Luis Pass. Two separate grids were developed since it is difficult to resolve the significant erosion directly downdrift of the end of the seawall. Because there is little sand in front of the seawall south of 61st Street, the end of the seawall effectively blocks most sand from moving north to south or south to north. Since very little sand moves from the seawall portion of Galveston to the west end, it was reasonable to develop two separate grids.

The United States Customary System was applied in both models. The horizontal coordinate system is State Plane, Texas South Central (FIPS 4204). The horizontal datum is NAD83, and the vertical datum is NAVD88.

The GenCade grid alignment is such that the water is to the left when facing in a positive direction along the grid. The seawall grid is oriented at a 237° angle from north while the west end grid is at an angle of 236° . The grid angle should match the angle of the shoreline as closely as possible, which is why the angles of the two grids are slightly different. When standing along the grid and facing the water, transport is negative to the left and positive to the right. Waves were imported in meteorological convention, but GenCade converted them to shore normal.

The GenCade model termed the “Seawall grid” extends from the south jetty at the Galveston Entrance Channel to the end of the seawall (Figure 15). The grid consists of 685 cells ranging from 50 to 200 ft. The smallest cells are along the seawall near the groin field. The length of the grid is approximately 10.4 miles.

The west end grid extends from the end of the Galveston seawall to just north of San Luis Pass (Figure 16). There are 470 cells along the west end grid. Constant cell spacing of 200 ft was used. The grid is approximately 17.8 miles long.

Figure 15. GenCade setup for seawall grid.



Figure 16. GenCade setup for west end grid.



4.3 Data inputs used in both grids

4.3.1 Initial shoreline position

The 1995 shoreline position from the BEG (BEG 2014) was used as the initial shoreline position for calibration. The shoreline was smoothed to remove any extreme undulations in shoreline position. In order to use the smoothed shoreline for both the seawall grid and the west end grid, the shoreline was split into two segments at the end of the seawall. Although more recent shoreline positions were available from BEG, these possible calibration periods would have included Hurricane Ike. Because Hurricane Ike made such a large impact on Galveston Island, it would not be reasonable to include it in the calibration process. Chapter 3 describes in more detail why Hurricane Ike should not be included in modeling and analyses.

4.3.2 Regional contour

The regional contour was developed from the existing shorelines provided by BEG. The 1995, 2000, 2001, 2002, 2010, 2011, and 2012 shorelines were analyzed. Previous shorelines were also analyzed, but some of the shoreline trends seen over the last 20 years did not occur further in the past, most likely due to the shoreline continuing to respond to the extended seawall constructed in the early 1960s. The seven shorelines were smoothed to eliminate extreme fluctuations in the shoreline position and later were averaged to develop the regional contour.

4.3.3 Waves

Hindcast waves from WIS (Wave Information Study) were used to drive the models. A single wave station was used as input for each grid model. Station 73070 was used to drive the seawall grid model. The wave gauge was positioned at cell number 655, and the water depth was 32.8 ft. For the west end grid, WIS Station 73067 was used. It has a water depth of 42.65 ft and was positioned at cell 276. A sensitivity analysis was conducted with many other wave stations. The hindcast waves in the Gulf of Mexico were recently updated to include 1980 to 2012. These waves were analyzed to determine which years were the most representative of typical conditions. It was determined that 1985, 1986, 1992, 1995, and 2012 were the years which produced calculated net transport rates nearest to the average net transport from 1980 to 2012. The waves from these 5 years were used as the wave input for GenCade. The hindcast waves from

1995 to 2000 were also tested as the wave input for the model, and the results were very similar to those with the representative waves.

4.4 Seawall grid domain

In addition to the information provided above, there are also some inputs and parameters unique to the seawall grid domain.

4.4.1 Beach fills

From 1995 to 2000, only one beach fill was constructed along the length of the seawall grid. In 1995, a 710,000 yd³ fill was placed along 19,000 ft of the seawall. In GenCade, beach fills are represented by average added berm width, date, and location. The 710,000 yd³ beach fill is equivalent to an added berm width of 40.54 ft based on estimated berm height and depth of closure values.

A second beach fill was constructed in 1999. Although the volume is recorded as 9,613 yd³, the approximate length and location are unknown. Since the beach fill was relatively small and the location was not known, it was not included within the model.

4.4.2 Groins

Each of the groins along the seawall was added to the model. An aerial photograph was used to map the locations of each groin. A seaward depth of 6 ft was specified for all of the groins. Initially, a permeability of 0.3 was chosen for all of the groins due to the age (construction was completed in 1939 [USACE 2009]), the low elevation, and the assumption that they are largely permeable. Previous studies (King 2007; Brown and Kraus 1994) used a permeability of 0.2. However, the permeability of each groin was adjusted between 0 and 0.99 during the calibration process.

4.4.3 Seawall

The seawall was included in the model by drawing a line on top of an aerial photograph. This seawall line was then converted to an arc before being added to the model. Some manual adjustments to the seawall were made during the conversion from the conceptual model (real-world coordinates) to the GenCade model (grid cells) due to the cell spacing in the vicinity of the seawall.

4.4.4 Lateral boundary conditions

A gated boundary condition was specified at the left boundary of the seawall grid. First, a groin was created at the boundary. A groin was specified, because the Galveston Entrance Channel was not included in the model, so there was no inlet adjacent to the jetty. Also, only a groin can be used to specify a gated lateral boundary condition. However, shoreline change and longshore transport are calculated in the same manner at jetties and groins. The other piece of information necessary to specify a gated lateral boundary condition is the length of the groin from the shoreline to the seaward tip. In this case, this length was 7451 ft. A pinned boundary condition was chosen for the right lateral boundary condition.

4.5 West end grid

4.5.1 Beach fills

Two beach fill projects were conducted between 1995 and 2000. Both were small and occurred in 1999. The first fill was constructed in front of the Pirates' Beach West subdivisions. The fill consisted of 19,500 yd³ and extended 7,785 ft along the shore. This is equivalent to an added berm width of 3 ft for GenCade. The second beach fill was located at Beach Pocket Park Number 2. The volume of this beach fill was only 1,200 yd³ and 485 ft long (added berm width of 2.35 ft in GenCade).

4.5.2 Lateral boundary conditions

Moving boundary conditions were specified at both of the grid boundaries. The left end of the grid corresponded with the end of the seawall. Initially, a gated boundary condition with a groin was used, but a simple moving boundary condition produced more reasonable results. The shoreline change specified for the moving boundary condition was -18 ft over the entire simulation. The shoreline at the right boundary has changed significantly due to the complex processes at San Luis Pass. From 1995 to 2000, the shoreline accreted 780 ft, which was the input for shoreline change for the moving boundary condition.

4.6 Calibration summary

In addition to the setups described previously, GenCade also requires a number of parameters in order to run the model. The same parameters were used for both the seawall grid and the west end grid. The models

were run for 5 years from 1 January 1995 to 31 December 1999 with a time-step of 0.1 hr. Based on the beach profiles used for the sediment budget, a berm height of 4 ft and depth of closure of 20 ft were specified. The berm height is the elevation for the typical berm while the depth of closure is the depth of the seaward limit of sediment transport for the time period modeled. A median grain size of 0.17 mm was based on previous literature and sediment samples.

GenCade was calibrated with the measured shoreline change and transport rates. The calibration process requires iterating the calibration parameters to match the measured shoreline change and the magnitude and direction of sediment transport. The calibration parameters that produced the best match to the measured shoreline change and transport rates are shown in Table 5. In addition, a wave angle offset of 8° was specified in order to match the direction of transport east of the seawall. Without this offset, the model calculates net transport to the west all along Galveston Island. This factor is typically applied at locations with coarse wave data or very low net transport rates. K1 and K2 are the sand transport calibration coefficients which were selected from a previous GENESIS study of Galveston Island (King 2007).

Table 5. GenCade parameters for calibration.

Parameter	Value
Start Date	1/1/1995
End Date	12/31/1999
Time-Step	0.1 hr
Recording Time-Step	168 hr
Effective Grain Size, mm	0.17
Average Berm Height, ft	4
Average Depth of Closure, ft	20
K1	0.4
K2	0.2
Angle Offset	8°

Figure 17 compares the measured and calculated shoreline change rates per year along all of Galveston Island. All distances are measured in miles from the South Jetty of the Galveston Entrance Channel. Initially, GenCade did not predict accretion adjacent to the south jetty of the Galveston Entrance Channel. In order to balance the cell in the sediment budget, a term of

355,700 yd³ per year of sediment was added to the cell from offshore. It is assumed that volume comes from the ODMDS. Although the source of the sediment is unknown, material must come from a source other than longshore transport in order to advance the shoreline at the observed rates. In order to calibrate the model, a volume of 355,700 yd³/year was added to the first 3 miles of Galveston Island adjacent to the jetty. Although this volume allows the calculated results to nearly match the observed shoreline change, it is difficult to predict if or how much sand may move onshore in the future. For that reason, a number of rates of onshore sand movement are modeled in the alternatives section. The modeled shoreline change in the groin field matches fairly well with the observed rates. Along the west end of the seawall, a few feet of erosion are observed. However, the model predicts little to no change in shoreline position. The reason is most likely because the model requires the beach profile shape to be constant along the grid x-axis. The model does not allow the user to specify details regarding the lack of sand in the offshore profile along the western end of the seawall. Along the west end, GenCade predicts erosion for the first 10 miles as expected. The model calculates substantial accretion near the boundary but does not predict as much accretion farther from the boundary as observed.

Figure 17. Measured and calculated shoreline change for 1995 to 2000. The location of the added volume along the first 3 miles is highlighted in gray.

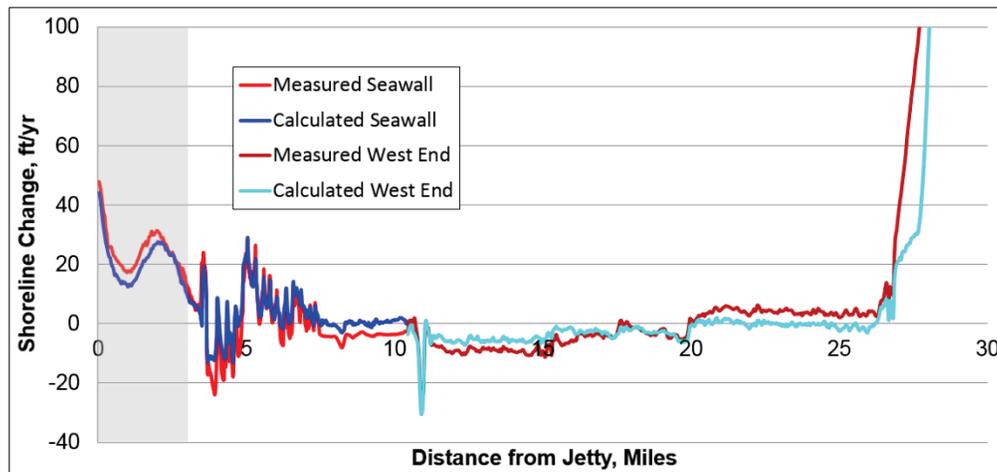


Figure 18 shows the net annual mean transport along Galveston Island. As expected from the sediment budget, the transport direction is to the northeast along East Beach and the first section of the seawall. A divergent nodal point occurs along the seawall, and then transport starts to move to the southwest. Transport along the west end is consistently to the southwest. The transport rate to the west increases at the very far end of

the west end and then promptly switches back to the east. San Luis Pass is not included in the model, so transport, sources, and sinks associated with the inlet do not exist in the model. In order for the calculated shoreline to move nearly 800 ft at the boundary in the 5-year period, the model must transport the sand from off the edge of the grid.

Figure 18. Net annual mean transport from 1995 to 2000.

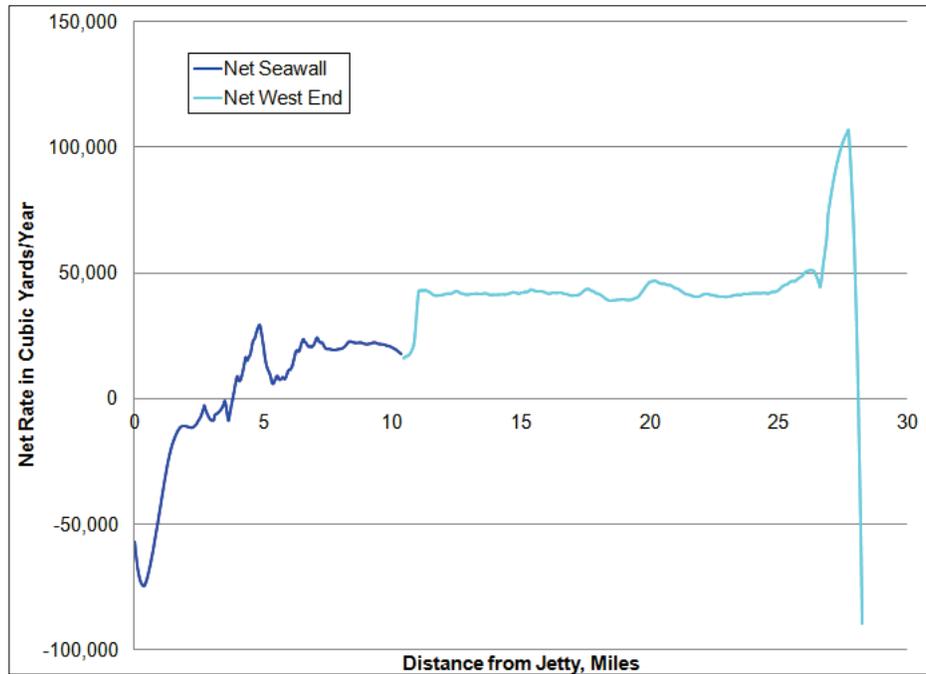


Table 6 describes the calibration statistics. The Root-Mean-Squared (RMS) Error and the Brier Skill Score provide goodness-of-fit statistics and scores for the GenCade results. The RMS Error is the difference between the measured and modeled shoreline change. The Brier Skill Score reflects the level of agreement between the measured and calculated values; a score of 1 means the measured and calculated values are in perfect agreement while a value greater than 0.8 is excellent and a value less than 0.3 is poor (USACE 2014b).

Table 6. Statistics for GenCade calibration.

Cell	Average Shoreline Change, ft/year		RMS Error, ft/year	Brier Skill Score
	Measured	Modeled		
Jetty to first groin	18.2	15.1	3.8	0.96
Groin field	1.6	5.5	5.0	0.82
Seawall west of groin field	-3.4	0.5	4.0	0.87
West end (to 13 Mile Rd)	-8.1	-5.2	3.6	0.84
13 Mile Road to Jamaica Beach	-3.3	-2.9	1.3	0.87
Jamaica Beach	-0.7	-1.5	1.1	-0.27
Jamaica Beach to Indian Beach	-3.3	-3.4	0.9	0.94
Indian Beach to Sea Isle	4.1	0.5	3.8	0.22
Sea Isle area	3.6	-0.4	4.1	-0.23
West end 1	5.7	1.2	4.7	0.54
West end 2	91.3	50.0	45.5	0.79

5 East Beach Sand Management Options

5.1 Significance of Big Reef and East Beach

Along the northeast end of Galveston Island, net transport is to the northeast, as verified by the Gulfward advance of the shoreline since the 1850s. East Beach grew from a detached separate island in 1838 to a recurved spit in 1850 (Morton 1974). The south jetty, constructed in the 1880s, trapped sand and formed a fillet. This broad, almost flat, fine-grained beach now serves as a recreational resource for Galveston.

Although millions of cubic yards of sand accumulated in East Beach, the jetty was permeable, allowing some sand to pass through into Galveston Entrance Channel. The sand accumulated north of the jetty, forming a sand body that is now known as Big Reef (Figures 18 and 19). It was first evident as early as 1919 as a body of sand extending out into the channel (USACE 1993a). Over time, the reef has expanded and contracted in response to storms and hurricanes, but surveys during the twentieth century show that it continues to rebuild after storm events.

The significance of Big Reef is that it demonstrates that sand is being recycled through the system. The 1993 Section 22 report (USACE 1993a), the previous sediment budget study (Morang 2006), and the current study conclude that sand moves onshore south of the south jetty. Much of this material passes through the jetty and into the channel, but some accumulates on the beach where aeolian processes can move it northward to Big Reef and farther into the channel. As Big Reef grows northward, it impinges on the Federal navigation channel and has to be dredged (USACE 1993a). The dredge material is subsequently placed in the USACE disposal area south of the end of the south jetty (Figure 1). Waves and currents move the material onshore, and the cycle begins again. Note that this mechanism needs to be confirmed with field studies. The continuous shoaling is a challenge in maintaining the navigation channel, and the recycling of material adds to overall costs. This chapter presents some alternatives to improve regional sediment management.

1. The reef or East Beach could be mined as a source of beach-quality sand.
2. Sand could be intercepted before it reaches the present dry beach.

3. The volume of sand reaching Big Reef could be reduced via sand-tightening, potentially reducing channel infilling and eventually reducing maintenance costs.
4. Aeolian transport could be reduced.

Figure 19. A 27 July 2003 photograph of Galveston south jetty taken after Hurricane Claudette. A fan of sand extends into the entrance channel to the right. View looking approximately northeast with the ship channel on the right and East Beach on the left (courtesy of Texas BEG).



The City of Galveston Park Board of Trustees has two permitted sand removal areas on the reef. One is at the west end near the culverts to East Lagoon. The other is on the unvegetated sand flats at the east side of the reef, an area known as R.A. Appfel Park¹. Much of the rest of Big Reef is now partially vegetated and is a nature preserve. Therefore, a large

¹ Ray Newby, Texas General Land Office, personal communication, 11 Aug 2006.

proportion of Big Reef may not be available as a sand source. However, in the listing of sand sources in Chapter 2, both permitted sand removal areas and restricted areas are included in the calculations since environmental requirements might change in the future.

Previous mining at Big Reef is documented in Appendix B. Starting in February 2003, the Board of Trustees used a jet pump and conventional dredge to remove 119,000 yd³ of sand from the submerged west portion of the reef. The reef had grown to the extent that it blocked the culverts to East Lagoon, and the project had the dual purposes of restoring circulation and mining sand. The material was placed in a temporary dredged material placement area near the west end of the jetty. In mid-April 2003, trucks moved 82,500 yd³ to the west end of the Galveston seawall and built a 2400 ft long beach approximately 50 yd wide. The remaining 35,000 yd³ were fine grain and possibly unsuitable for beach use. Regardless of its quality, it was reclaimed by the elevated seas from Hurricane Claudette, which made landfall near Port O'Conner on 15 July 2003. The storm surge level was 7.55 ft mean low low water (MLLW) at Pleasure Pier at 0554 CDT, 15 July 2003. An aerial photograph taken on July 27 shows a fan of sand pushed over the south jetty and extending out into the entrance channel (Figure 19). This example supports the hypothesis that significant sand from the open coast makes its way into the Houston-Galveston Ship Channel (Morang 2006).

In early 1995, the City of Galveston nourished the 3.7-mile-long beach at the groin field area with 710,000 yd³ of sand from an offshore source. In January 1998, 1999, and 2000, the city added material amounting to approximately 70,000 yd³ per year to supplement the initial project (Ravens and Sitanggang 2002). East Beach was scraped to provide the required sand. The Board of Trustees has continued to remove sand from East Beach/Apfel Park using trucks and place it on an as-needed basis in front of hotels to enhance tourist appeal. The quantities have not been carefully tabulated but are in the range of tens of thousands of cubic yards per year rather than hundreds of thousands. Trucks removed 25,000 yd³ of sand between May and June 2006 and carried it to Jamaica Beach at the west end of the island.

Sand mining is not possible all year due to environmental constraints. The sand flats near the south jetty are piping plover habitat but are not a nesting area. In addition, sea turtles are beginning to nest on East Beach,

which may stop mining operations from mid-March to the end of September.

5.2 Option 1: East Beach sand sources

Three areas on Big Reef and a triangular zone next to the jetty on East Beach may be suitable beach-quality sand. These are shown in Figure 20 with their approximate surface areas. The areas were traced from June 2006 aerial photographs provided by the Texas Bureau of Land Management. The volumes in Table 7 assume removing a uniform layer across each area. The 1 yd and 2 yd layers will be most economically constructed with backhoes and trucks (mechanical dredging) while the 5.5 yd layer can be constructed with mechanical or hydraulic dredging equipment. Clearly, the areas and thicknesses would be adjusted based on engineering, environmental, and economic criteria or available equipment (for example, a buffer zone must be left near the jetty and around the nesting habitat on the exposed reef), but the analysis shows that up to 2 million yd³ could be mined if a 5.5 yd thick layer were taken out while still leaving the vegetated portion of Big Reef intact.

Feasibility studies have been conducted to evaluate mining these areas. For example, PBS&J (2007) performed magnetometer and sidescan sonar surveys and identified three magnetic anomalies in a rectangular region north of the south jetty (larger than Areas 1 and 2 of this study). They interpreted them to be modern boat wrecks. In the 1993 Section 22 report, USACE (1993b) calculated that 2.4 million yd³ are available from the submerged portions of Big Reef if dredged to -20 ft MLLW, while preserving the above-water habitat areas. Mechanical dredging systems could be used, including jet pumps, a simple fixed bypassing system, and a shore-based dragscraper system, as well as a smaller cutterhead dredge.

The unvegetated area south of the jetty on East Beach could be mined as a sediment source, but the beach is a popular recreation area, and some sections are unavailable as a sand source.

Another potential source of sediment is the underwater portion of Big Reef that extends north into the channel. It consists of a high proportion of sand, similar to the samples from the navigation channel (Figure 8). Contemporary bathymetry data are needed to design a detailed dredging plan.

Figure 20. Potential sand-mining areas on Big Reef and East Beach. Units are in hectares (= 11,960 yd²). The area labeled “Unavailable” is used for recreation but could possibly be scraped occasionally.



Table 7. Potential Big Reef mining volumes.

Polygon	Area (m ²)	Vol. 1.1 yd layer (yd ³)	Vol. 2.2 yd layer (yd ³)	Vol. 5.5 yd layer (yd ³)
Big Reef Area1	231,900	255,100	510,100	1,275,300
Big Reef Area2	23,450	25,800	51,400	128,600
East Beach Area1	72,450	79,700	159,300	398,300
Total	327,800	360,600	720,800	1,802,200

5.3 Option 2: Deposition basin off East Beach

The sediment budget calculates that net northeastward-directed transport along East Beach is approximately 41,000 yd³/year while onshore transport is approximately 360,000 yd³/year. Therefore, one option to consider is trapping some of the material that moves onshore before it reaches the surf zone and the beach. This could be accomplished by dredging a temporary deposition basin offshore of East Beach parallel to the shoreline.

Dredging a deposition basin would have two benefits. First, it would yield beach-quality sand. Second, any material that the basin traps will not move onto East Beach and thereafter onto Big Reef or into the navigation channel (in turn, helping reduce dredging requirements).

Any number of configurations of basins can be designed. One example is for a rectangle 3,000 yd long × 150 yd wide. If located in water approximately 15 ft deep, the basin would be approximately 1000 yd offshore (Figure 21). This water depth is suitable for several reasons. First, it is approximately the limit of the active zone (closure depth) here and therefore would primarily trap incoming material rather than absorb sand from the beach. Second, barges and equipment can work in this depth. Final choice of a basin will be dictated by operational factors, equipment available, and bottom sediments. If the basin were excavated to a depth of 1 yd, it would yield approximately 450,000 yd³ of sand; 2 yd would supply 900,000 yd³ from initial construction.

The sediment budget calculated that onshore sediment movement along East Beach is approximately 360,000 yd³/year. If assumed that this supply is evenly distributed along East Beach, then the 3,000 yd long basin might trap approximately half of the landward-moving sediment, or approximately 180,000 yd³/year. If approximately 50% efficiency is assumed, the basin will trap approximately 90,000 yd³/year. The actual rate of infilling would have to be monitored with bathymetry surveys, but a reasonable estimate is that the basin might need to be redredged approximately every 5 years.

Table 8 lists options for different lengths of basins (all assume 150 yd wide). The volumes in the last column will vary depending on the efficiency of the basin as a trap. Environmental and permit factors will have to be investigated, and an offshore geotechnical survey will be needed to determine if the material removed during initial construction is suitable for beach placement. An analysis of wave refraction and potential effects on the south jetty will have to be numerically modeled.

Figure 21. Proposed deposition basin off East Beach. Dimensions and locations of basin could change depending on sediment needs. Profile elevations in meters, NAVD 1988. 2002 profiles courtesy of Texas A&M University. Background photograph 22 May 2012.

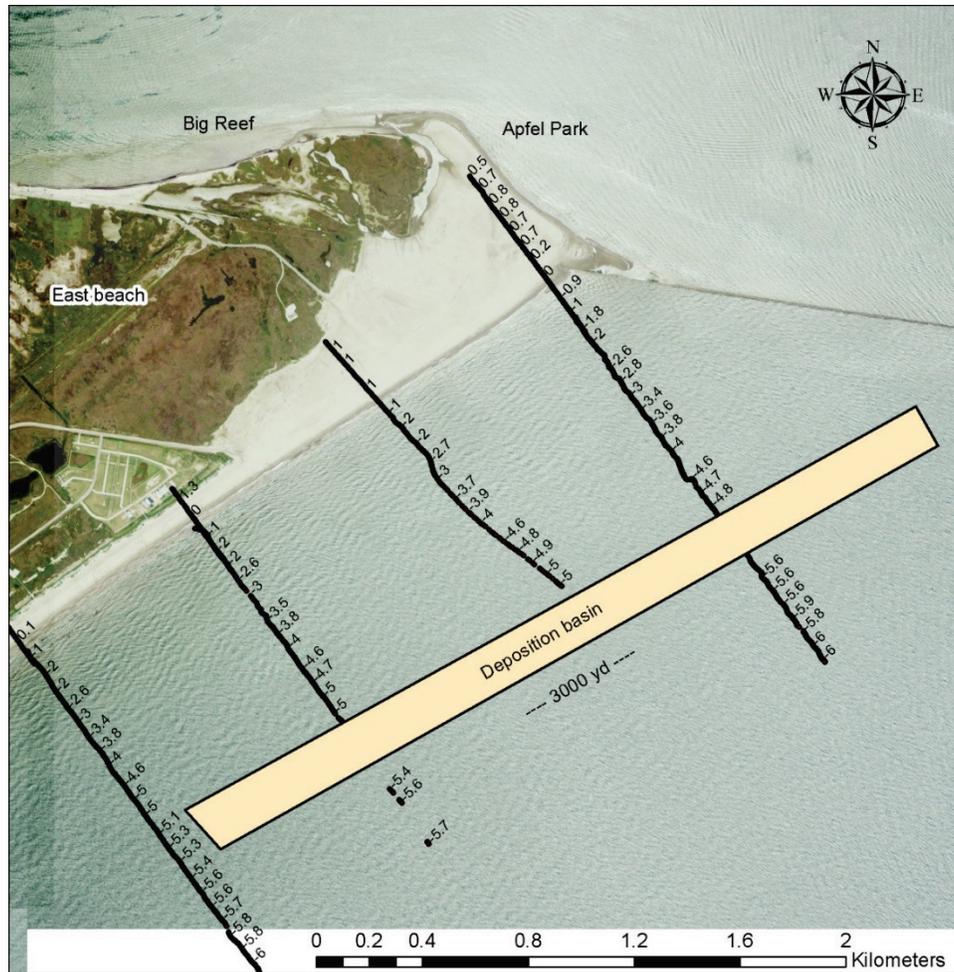


Table 8. Sediment basin parallel to East Beach.

Linear Extent of Dry East Beach (percent)	Length (yd)	1 yd Depth Initial Volume (yd ³)	2 yd Depth Initial Volume (yd ³)	Annual Volume Trapped at 50% Efficiency (yd ³)
50	3000	450,000	900,000	90,000
75	4500	675,000	1,350,000	135,000
100	6000	900,000	1,800,000	180,000

Note: Initial dredged volume based on 150 yd wide basin.

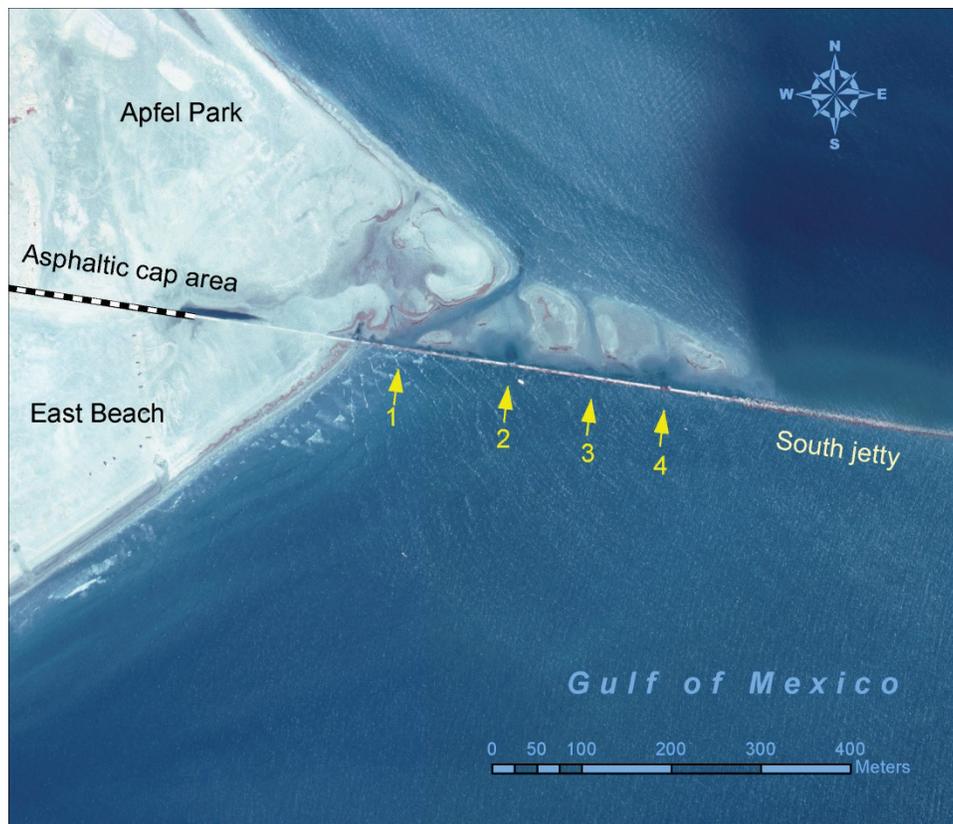
5.4 Option 3: Reduce transmission through south jetty

Over the past century, the south jetty trapped the sand that now constitutes East Beach. Despite this trapping, significant amounts of sand enter the Galveston channel, either by Aeolian movement or by

transmission through the jetty. Therefore, one option to obtain beach-quality sediment and reduce channel shoaling would be to prevent or reduce transmission through sand tightening of the jetty. The result would be increased sand accumulation on East Beach that can be mined as a sediment source.

Morphological evidence indicates that the south jetty is porous. Figure 22 is a close-up of the shoal that extends north of the south jetty (east of the main body of Big Reef). The arrows point to areas where the jetty is particularly porous and from where channels have cut across the shoal. Channel 1 (closest to the beach) appears to have enough flow to maintain an open mouth. At arrows 2 and 4, scour holes appear to have formed north of the jetty. The most likely explanation is that wave setup and the flood tide pushes water through the jetty. The post-hurricane Claudette photograph (Figure 19) shows that the water also carries a significant amount of sand over and through the jetty into the Galveston channel, at least under storm conditions.

Figure 22. Areas of the south jetty with enough permeability for significant water flow.
Aerial photograph May 2006.

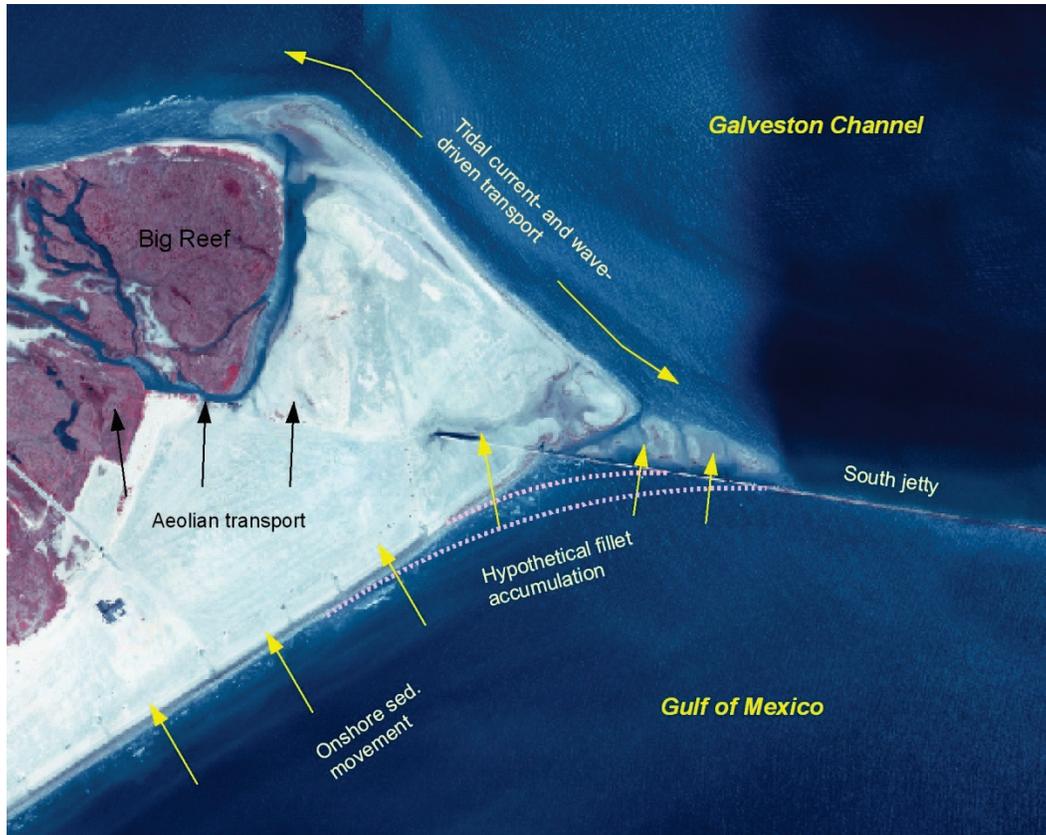


The porosity of the jetties has long been recognized, and SWG has made efforts to reduce void space. Sargent and Bottin (1989) reported that in 1935–1936, an asphaltic cap was placed on two sections of the south jetty from Sta 196+55 to Sta 230+59 and at a section near the outer end. They stated, “Prior to the cap, a seal course of asphaltic concrete was placed in the void spaces.” These sections of the jetty are now under the sand on East Beach; possibly the sealing was effective in reducing transmission. The last pre-Ike rehabilitation work was in 1962–1966, when the outer 70 m of each jetty was rebuilt as a head section and a portion of the north jetty was rehabilitated. “In many cases core stone was exposed, or cover layer stone was not tightly interlocked. Due to these conditions and use of large core stone during original construction, the jetties were considered too pervious (sic) to wave, tide, and sediment motions.” (Sargent and Bottin 1989). Background information on sealing and tightening jetties can be found in Simpson et al. (1990), and an evaluation of an example of sand tightening at the Port Everglades, FL, jetty is in Rosati and Denes (1990).

Figure 23 shows hypothetical sediment pathways through and over the south jetty. Primarily, sediment movement occurs via wave and current transport through a 500 yd section of the jetty that extends out from the current shoreline. Some sediment also moves via aeolian transport over the jetty, as discussed in section 5.5. Sealing the 500 yd portion of the jetty where the predominant sediment transport occurs would be difficult but is possible using grout or by placing a cap over the structure. Another option is to place a temporary, sand-filled geofabric groin parallel to the jetty. This geofabric groin’s effectiveness could be monitored, and it could be removed if ineffective.

Some of the material that moves shoreward to East Beach accumulates on the beach while the rest moves via littoral currents eastward toward the jetty. A reasonable assumption is that 50% moves in the littoral system. Therefore, up to 180,000 yd³/year would accumulate in a fillet after tightening the jetty (Figure 23). The dashed lines in Figure 23 are hypothetical variations of accumulation. The fillet would need to be mined regularly; otherwise, the sand fillet will reach the end of the sand-tightened portion of the structure. Thereafter, currents would move sand through the more porous jetty further seaward, negating the benefits of the sand-tightening project.

Figure 23. Hypothetical sediment pathways over and through south jetty. Arrows are symbolic, and length does not represent magnitude or rate. Dotted lines show hypothetical fillet growth if the jetty were sand tightened. Background photograph May 2006.



Currently, Big Reef contains approximately 9 million yd^3 of sand, according to calculations made at Texas A&M University¹. The feature began to develop at least before 1919, based on historical shoreline maps (USACE 1993a). An amount of 9 million yd^3 of accumulated sediment divided by 100 years results in an annual volume increase of 90,000 yd^3/year , or half the 180,000 yd^3/year hypothesized to move through the jetty. Accounting for losses into the navigation channel and occasional dredging, the growth rate of Big Reef is greater than 90,000 yd^3/year .

¹ Timothy M. Dellapenna, Assistant Professor, Dept. of Marine Sciences, Texas A&M University at Galveston, personal communication, 9 Aug 2006.

5.5 Option 4: Reduce aeolian sand transport

5.5.1 Background

Aeolian transport is a major factor along much of the south Texas coast. Persistent southeast winds along North Padre and Mustang Islands are a key agent in moving sand across the barrier islands. Unimpeded by vegetation, significant volumes of sand are transported toward Laguna Madre and Corpus Christi Bay and contribute to shoreline advance on the west sides of the islands (Morton and Paine 1984). At Mansfield Pass, wind-blown sediment is responsible for approximately 25% of the channel shoaling¹.

Anecdotal information indicates that aeolian processes move significant sand on Galveston Island. Giardino et al. (2000) reported that when San Luis Beach, in front of the San Luis Hotel, was nourished in 1985, wind transported a significant volume over the seawall. They stated, “On numerous visits to the site, the investigators observed sand blowing from the beach and over the top of the Seawall. So much sand was deposited on the road, which occupies the top of the Seawall, that patches of sand up to 10 yd² and 3 in thick were measured on one occasion. In addition, the merchants complained regularly that the doorways of their businesses were blocked by ‘sand dunes.’” On East Beach, the GPB sometimes has difficulty maintaining roads to the recreational facilities because of the constant blowing sand². Along the Seawall, work crews from both the Park Board and the City collect sand that has blown onto the road and relocate it to the beach³. Enough sand sometimes accumulated to impact drainage. USACE (1993a) considered wind-blown transport to be one of factors contributing to the growth of Big Reef.

5.5.2 Aeolian transport at East Beach–Big Reef

The wind-blown sand transport from East Beach to Big Reef can be calculated using the procedure in Chapter III-4 of the *Coastal Engineering Manual* (Hsu and Weggel 2002). Based on this procedure, annual northward transport is between 10,000 and 20,000 m³ (13,000 and 26,000 yd³; Figure 24 and Table 9). The transport is calculated crossing the

¹ Nicholas C. Kraus, Engineer Research and Development Center, Vicksburg, MS, personal communication, 17 Aug 2006.

² Ray Newby, Texas General Land Office, personal communication, 11 Aug 2006.

³ Mario Rabago, Deputy Director, Galveston Park Board of Trustees, personal communication, 14 Oct 2014.

jetty on the bare portion of East Beach. All sand moving north across the jetty is shown as red in Figure 24, while all transport moving south is green.

Figure 24. Annual aeolian transport (cubic yards) across East Beach at the south jetty. See text for more information.

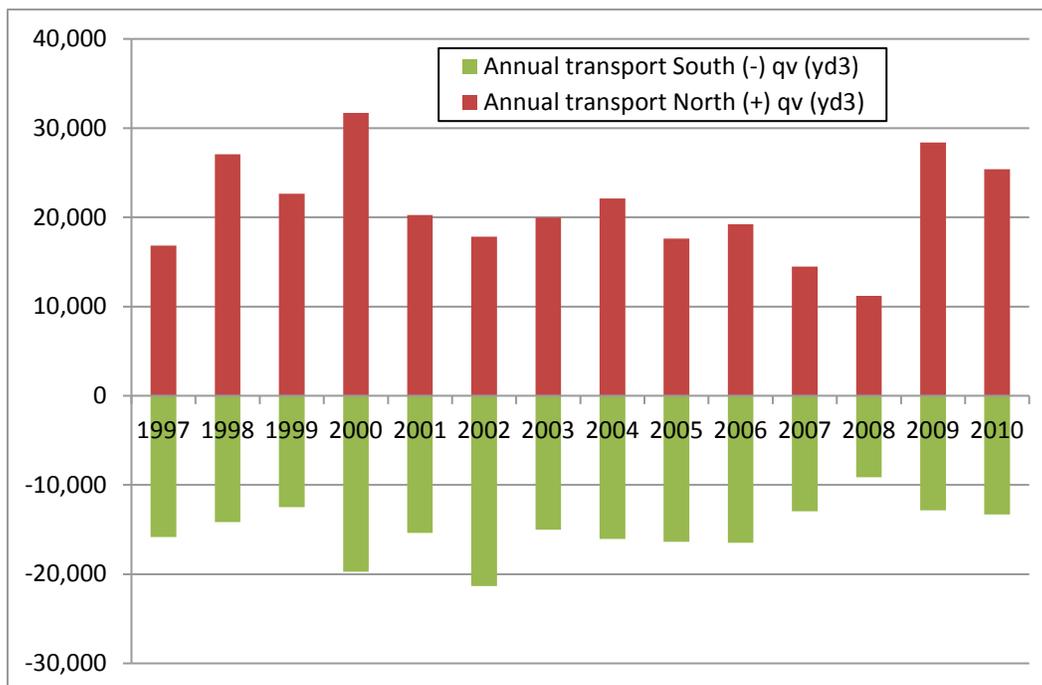


Table 9. Aeolian transport volumes crossing the jetty for the bare sand section of East Beach.

Year	Annual Transport North (+) q _v (m ³)	Annual Transport South (-) q _v (m ³)	Annual Transport North (+) q _v (yd ³)	Annual Transport South (-) q _v (yd ³)
1997	12,871	-12,104	16,840	-15,830
1998	20,703	-10,840	27,080	-14,180
1999	17,308	-9,559	22,640	-12,500
2000	24,243	-15,086	31,710	-19,730
2001	15,484	-11,755	20,250	-15,380
2002	13,633	-16,322	17,830	-21,350
2003	15,278	-11,485	19,980	-15,020
2004	16,903	-12,265	22,110	-16,040
2005	13,473	-12,506	17,620	-16,360
2006	14,702	-12,589	19,230	-16,470
2007	11,068	-9,901	14,480	-12,950
2008	8,565	-6,986	11,200	-9,140
2009	21,699	-9,825	28,380	-12,850
2010	19,403	-10,159	25,380	-13,290
Sum	225,334	-161,383	294,730	-211,090

The computation procedure is lengthy and is described in Appendix D. Wind measurements were based on the NOAA instrument at Pleasure Pier (Sta 8771510) for the period 1997 to 2010. NOAA discontinued wind measurements there in 2011, so to extend the analysis for later years, data from another site would be needed. The next most appropriate station would be Galveston Bay Entrance North Jetty (Sta 8771341). Rainfall and pan evaporation data were from the Beaumont Research Center, the closest site to Galveston with evaporation measurements. Spot comparison with Galveston rainfall shows that the rain climate at Beaumont is similar but shifted in time by hours. Beaumont data was used exclusively rather than mixing Beaumont evaporation with Galveston rainfall because there are differences in local cloud cover, wind, and rainfall.

The aeolian transport volume results are realistic on a month-to-month basis. During the fall, winter, and spring, the strongest winds usually occur when a cold front crosses the coast. During the prefrontal period, winds blow from the southeast, moving sand over East Beach and north to Big Reef. As the front passes, the winds switch abruptly to the north, often accompanied by rain. The rain typically only lasts for a day or two, but strong winds persist for days, slowly turning from north to northeast and finally to the east. As a result of these frontal passages, during many years, the greatest aeolian transport to the south was during October, December, and January (Figure 25, Figure 26; figures are in their original metric units). The fronts diminish in strength by March or April because the Gulf of Mexico's water has cooled and there is less energy transfer between the atmosphere and the surface waters. By late spring, persistent southerly winds transport sand north onto Big Reef, and summer transport is predominantly to the north (Figure 27).

Aeolian transport volume calculations lead to several conclusions. First, the approximate balance between northward and southward transport suggests that the growth of Big Reef over time has been largely due to water-borne sand carried through the jetty, with wind-blown sand as a lesser contributing factor. Certainly, some of the wind-blown sand that moves south will fall into the Gulf, where net currents will move it northeast and through the jetty. However, this gain is balanced by the loss of northerly blown sand, which falls into the Galveston channel and is carried away by tidal currents.

Figure 25. 2000 aeolian sand transport across 720 m south jetty crossing bare beach (cubic meters/month; northerly transport is positive +; note: metric units as per original calculation units).

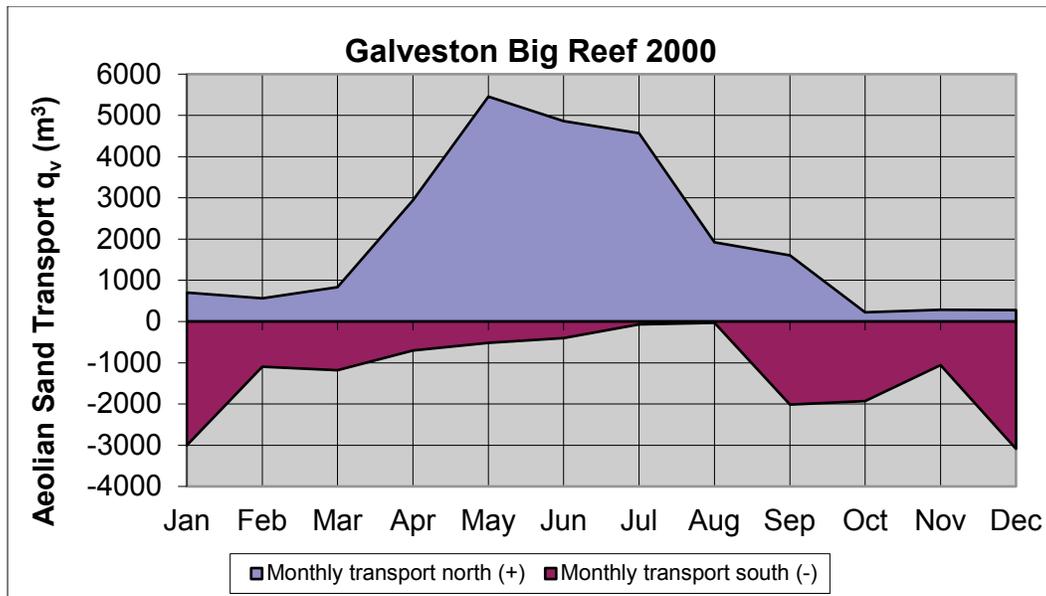


Figure 26. 2010 aeolian sand transport showing the characteristic summer-winter pattern (cubic meters/month; northerly transport is positive +).

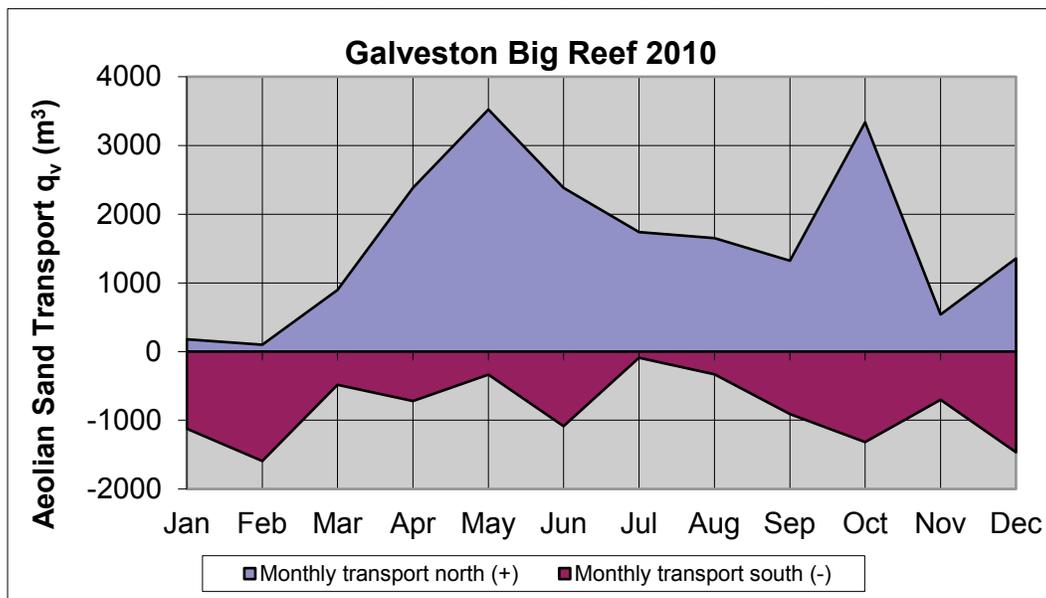
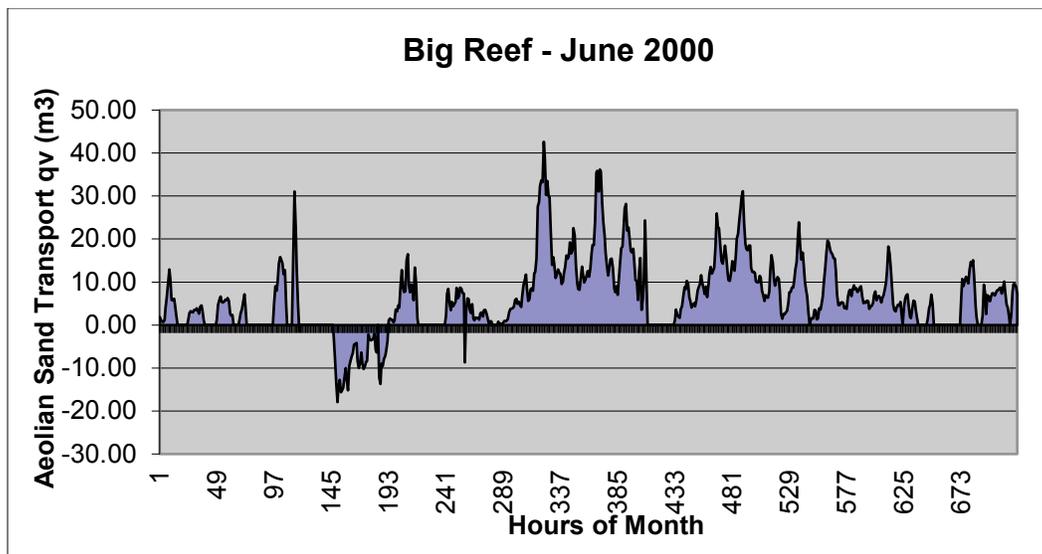


Figure 27. During summer months, the dominant sand transport is to the north (positive values). June 2000 is shown as an example.



In light of the anecdotal evidence that blowing sand can be a major factor on Galveston Island, the magnitude of aeolian transport is less than expected. There are no obvious factors to change the magnitude of the aeolian transport analyses. The computation provides an hourly transport in units of cubic meters/meter-hour. This value is multiplied by 720 m, the length of the south jetty that crosses open sand (Figure 28). More open (nonvegetated) beach exists southwest of the jetty, which provides justification to use a larger multiplier for the length of the south jetty that crosses open sand up to 1000 m. The sand diameter used in this analysis was 0.116 mm (very fine sand, passing #120 U.S. Standard sieve). Grain size selection was based on an average of six samples collected at beach profiles. The sand on the open beach may be finer, which would result in greater transport. Increasing the length multiplier and decreasing grain size might double the transport but not cause a magnitude change of aeolian sediment transport volume. In the NOAA data, some days had missing wind data, but these were not common enough to make a major change in volumes. For a future study, other computation methods should be used to compare with the procedure from the *Coastal Engineering Manual* (Hsu and Weggel 2002).

Figure 28. Jetty segment used for computation of aeolian sand transport.
Background photograph 22 May 2012.



Annual aeolian transport northward ($\approx 21,000 \text{ yd}^3$) is approximately half the annual littoral drift value of $41,000 \text{ yd}^3/\text{year}$ computed in the sediment budget but much less than the predicted onshore transport of $360,000 \text{ yd}^3/\text{year}$. With Big Reef containing approximately 9 million yd^3 of sand, it would have taken approximately 400 years to develop Big Reef by wind-blown sand. Therefore, water-borne supply must have been the prime source of sand for the growth of Big Reef.

5.5.3 Alternatives

Aeolian transport can be reduced by three practices:

1. Moisture
2. Installation of mechanical sand traps such as sand fences to build dunes
3. Vegetation (both on dunes and flats).

The first option is commonly used in the mining industry to control dust and sand. It would be impractical to set up watering systems on East Beach, although seawater is readily available. The San Jacinto Placement Area west of East Lagoon was formerly marshy but now is used by the USACE to dispose of sand from the USACE and Coast Guard docks and other areas. In the 2012 aerial photographs, at least 80% of the area within the levees was unvegetated. The vegetation grows naturally, but for weeks or months, fine sand and silt are mobile. A watering system could probably reduce wind transport until the vegetation becomes established.

A second option would be to encourage dune construction by installing sand fencing. Once new dunes formed, they would have to be vegetated to reduce their mobility. It is difficult to find quantitative data on dune growth. Woodhouse (1978) described 20 years of dune experimentation in coastal areas from the mouth of the Columbia River in Oregon through southern California and the Gulf of Mexico to Cape Cod, MA, and Brampton et al. (2000) describe techniques used in Scotland. Khalil (2008) describes sand fencing in Louisiana's barrier islands. Hsu and Weggel (2002, Table III-4-13) reproduced some measured dune growth rates from experiments conducted in the 1960s by the Coastal Engineering Research Center¹. For tests at Padre Island, TX, the annual rate of sand volume accumulation was in the range of 3.0 to 3.7 yd³/ft (Table 10). These results can be extrapolated to East Beach by drawing hypothetical sand fences across the open areas of East beach and multiplying the length of the fencing by the growth rate. Padre Island is a windier location than Galveston, so it is appropriate to use the lower volume (3.0 yd³/ft/year). The results are surprising; just placing fences on the bare areas shown in Figure 29, a length of 13,000 ft, might annually trap 39,000 yd³ of sand. Because of beach access needs, some of the area shown in Figure 29 would remain bare, but the calculation shows the volumes that potentially could be trapped.

Table 10. Measured dune growth rates at Padre Island, TX.

Type	Wood Fencing (3 fences, 3 lifts)	Wood Fencing (4 fences, 3 lifts)	Sea Oats
Average rate (m ³ /m-year)	8.7	7.61	9.2
Average rate (yd ³ /ft-year)	3.5	3.04	3.67

Source: Excerpt from *Coastal Engineering Manual* (Hsu and Weggel 2002, Table III-4-13).

Original measurements: 1960s experiments conducted by Coastal Engineering Research Center (citation unavailable but possibly Woodhouse 1978).

¹ Original citation not listed but possibly Woodhouse (1978).

Figure 29. Purple lines are hypothetical sand fences drawn over bare areas for calculating potential dune formation. Length of sand fences approximately 6,700 m (22,000 ft). Because of beach access needs, final dune lines would not cover as much of the bare beach. Background photograph 22 May 2012.



A third option to reduce wind-blown sand would be to plant the bare portions of East Beach and improve the vegetation over the areas that now have grass and other plants. It is unclear how the 1960s experiments were conducted, but if sea oats were planted in rows on East Beach similar to the sand fencing, then the trapping rate for 13,000 ft of oats would account for up to 47,000 yd³ in a year.

The San Jacinto placement area is also bare after impoundment and dewatering of dredge material. The bare zones grow vegetation within months¹, but temporary fencing or seeding might be worthwhile to reduce wind-blown transport.

¹ Tricia Campbell, Operations Manager, SWG, personal communication, 8 May 2014.

In summary, a program for installing sand fencing and planting extra vegetation on East Beach could trap 39,000 to $\pm 47,000$ yd³/year. This alternative is a relatively low-cost way to trap sand in a sand management program. In addition, fencing could also be placed in the San Jacinto placement area if needed.

5.6 Option 5: Sand backpass system

Sand backpassing is the process of mechanically moving sand away from an inlet or harbor mouth back some distance to the updrift beaches. In the case of Galveston entrance, backpassing would entail mechanically moving sand from East Beach to the west to the beaches in front of the seawall (Figure 30). As demonstrated in the sediment budget, much of the sand accumulating at East Beach has an offshore source. Therefore, a pumping plant would not only be backpassing sand but would also be adding new material not previously in the littoral zone. The purpose of a backpassing system would be the following:

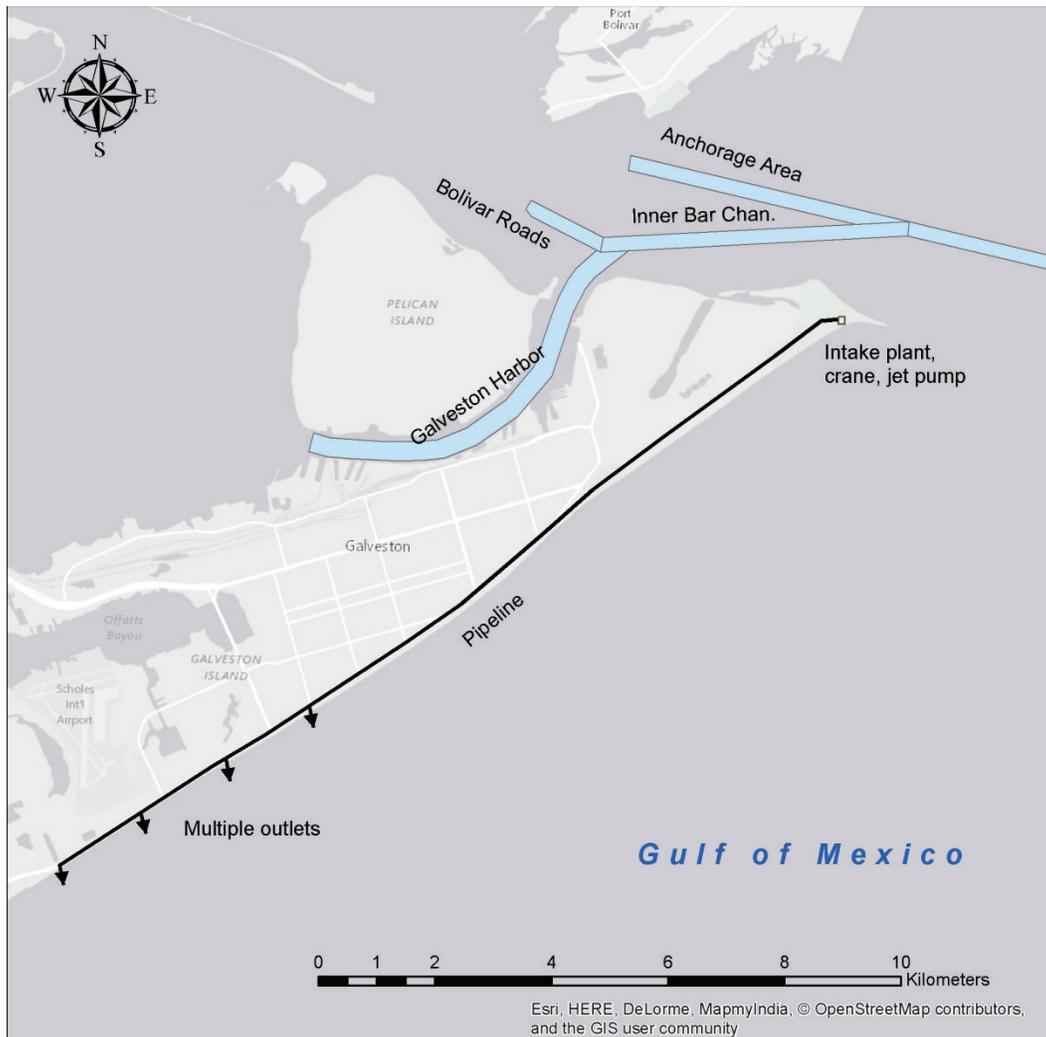
1. Intercept sand before it accumulates on East Beach and/or passes through the south jetty.
2. Move sand west along the Galveston seawall without the use of trucks.

The concept of a mechanical backpassing system is simple:

1. A fixed or semi-movable pumping/dredging plant is located in the region where significant sand accumulates. The material is entrained into a pipeline.
2. A pipeline runs along the beach to a discharge location where the sand is needed.
3. Booster pumps are located along the pipeline as needed, depending on the distance of transport.
4. Discharge is made on the beach in one or several locations.
5. If needed, sand is moved and shaped by grading equipment.

Most research on artificial sand movement at inlets has been on bypass systems (Clausner 1999; Boswood and Murray, 2001; Per Bruun 2005), but the concepts are applicable to backpassing. Backpassing has been performed at Miami Beach and St. Augustine Inlet, FL; Cape May, North Wildwood, and Avalon, NJ; Ocean Beach (San Francisco), CA; and Corpus Christi Beach, TX. BMT WTM (2010) examined the feasibility of backpassing at the Tweed River entrance, Australia.

Figure 30. Proposed backpassing system with multiple discharge points along west portion of seawall.



Another study will be needed to design the specifics of a backpassing system. The GPB, in conjunction with the SWG and state agencies, would have to decide what volume of sand to backpass annually and the total distance. The system could target 50,000, 100,000, or more yd^3/year . Then the mechanics of pipe diameter and pump capacity would be engineered. The intake plant could be a fixed unit (as at South Lake Worth Inlet, FL) or a jetpump mounted on a tracked crane, as at Indian River Inlet, DE (Figure 31). Bypassing plants can be a viable option instead of periodic dredging (Melton and Clausner 2004). One intriguing option for the intake is to use a remote-controlled jetpump such as the Punaise (Williams and Visser 1997). As another option, a small dedicated dredge could be purchased, similar to units used at Rudee Inlet, VA, or Mexico Beach, FL. These are operated by municipal employees year round.

Figure 31. Mobile bypassing system used at Indian River Inlet, DE (photograph from USACE Philadelphia District). Here, the pipe crosses the inlet via the highway bridge.



Galveston would be an excellent location for a backpass system because the pipeline does not need to cross a waterway. The pipe could be located at the base of the seawall and buried under a dune for aesthetic considerations. The location would allow for relatively easy construction and maintenance with truck-operated equipment from Seawall Boulevard above. Another design consideration is the ability to rotate the pipe to prevent the bottom from wearing out due to sand friction. Several discharge points could be installed in the pipeline to allow nourishing different parts of the beach on a scheduled basis or as needed if hot spots develop. The discharge points need to be located along the west half of the seawall to minimize sand returning towards East Beach. If booster pumps are needed, they could be disguised in buildings made to be aesthetically pleasing. Electric booster pumps have a low noise output compared to diesel. Table 11 lists advantages and disadvantages of various options for a backpassing system.

Table 11. Sand backpassing system, Galveston, Texas.

Feature	Advantages	Disadvantages
Intake at East Beach	Maximum sediment input, far from most tourist activity, limited visual impact.	Possible objection from fishermen and beach drivers.
Crane-based intake	Flexible, can be moved when pit forms. Familiar equipment; can be operated by municipal employees.	Maintenance (corrosion issues), staff needed, must be moved before storms.
Fixed intake on jetty	Reinforced for storm protection.	Initial construction costs. May not be optimum location if fillet changes. Must be removed if backpassing is discontinued.
Jet pump or Punaise intake	Submerged, minimal visual impact.	Must be manually relocated when/if needed. Electric power supply needed. Initial purchase cost.
Pipeline crossing East Beach		Need crossovers for beach traffic. Possible visual objections unless buried.
Pipeline along Seawall Blvd.	Simple construction and access for maintenance. Ability to rotate pipe.	Visual objections.
Pipeline along base of seawall	Largely out of sight, can be buried with sand dune.	Possible damage in exceptional storms. Maintenance more difficult.
Multiple discharge points	Can adjust nourishment as needed along beach.	
Booster pumps (if needed)		Noise, visual issues, diesel fumes, and maintenance costs.

6 San Luis Pass Sand Management Options

Much of west Galveston Island is experiencing erosion, but the western tip of the island, near San Luis Pass, is experiencing accretion. Net longshore transport is to the southwest along this section of coast, so sediment placed on west Galveston Island beaches will eventually end up in the unstructured San Luis Pass if not interrupted. This section addresses the location for a sediment trap and the rate of sediment delivery.

San Luis Pass is located at the southwest end of Galveston Island. Figure 1 shows the location of the natural pass that opens into West Bay. There is a small crescent-shaped ebb shoal, with several channels. The pass is dominated by an extensive flood shoal complex in West Bay (Figure 14). The inlet has been in its present position since at least 1852 and has no jetty structures for navigation purposes. The channel(s) has changed orientation and position over time at this mixed energy inlet (a mix of both wave-dominated and tide-dominated conditions throughout the year). From 1930 to 1961, a single channel migrated to the south. After 1973, the channel migrated northward until 1990, and since 1990, multiple channels have been present in the central location of the pass (Gibeaut et al. 2003).

In the larger regional sediment management picture, the flood shoal at San Luis Pass is a large sink supplied by sand from the open coast. This material is essentially lost from the littoral system due to concerns about dredging in this sensitive environment; however, in recent years the GLO has begun investigating the feasibility of using sediments from San Luis Pass. Morang (2006) and Coast & Harbor Engineering (2007), using different methods, concluded that the San Luis Pass flood delta is accreting at a rate of 100,000 yd³/year. Coastal & Harbor Engineering (2007) concluded that the flood delta is close to 100% sand. Because no sand enters West Bay from rivers, the growth of the flood shoal must be fed by sand entering from the Gulf of Mexico via tidal currents, fed by longshore transport from the north and south sides of the inlet (Atkins Global 2012).

6.1 Install groin to reduce west-moving littoral transport

Because net littoral sand movement at the west end of Galveston Island is to the southwest, one option for obtaining beach-quality sand would be to install a groin perpendicular to the shore to trap the littoral material and regularly mine the resulting fillet. A groin placed near San Luis Pass could potentially block a significant portion of the net transport (Q_{LST}) of 250,000 yd³/year (see Cells1_10 in Figure 14). Again, two assumptions need to be made:

1. If assumed that the bulk of the transport occurs in less than 6 ft water depth, a groin would need to be 500 ft long on this flat sloping shoreface.
2. A reasonable estimate of the efficiency of the groin is that 50% of the littoral material will be trapped, or 130,000 yd³/year.

Trapping some of the sediment that normally would move toward the pass would potentially reduce accumulation on the ebb shoal, in the natural channel, and on the flood shoal. This groin may be permeable to allow some passage of sediment so the impact on the pass shoreline and ebb and flood shoals would be controlled. The exact location and porosity of this jetty would need further design work, but a suggested location would be near the boundary of Cells1_10 and Cells1_9.

Following groin construction, as the new fillet filled, it would need to be dredged on a regular cycle. This work could be performed by a cutter section dredge. The sand could be carried east by barge to use as beach fill. The fillet could also be excavated by backhoe and the sand carried by truck.

6.2 Install fixed or mobile backpassing plant

Similar to the concept of the backpassing plant proposed for East Beach, two options could be used to remove sediment trapped at a groin north of San Luis Pass. First, a fixed backpassing pump system could be installed on the groin. Second, a semi-mobile, crane-based pump could be deployed on the fillet beach, similar to the unit used at Indian River Inlet, DE.

Depending on the distance to the east that the sand is to be pumped, booster pumps will need to be considered in the engineering design. For example, the distance from the proposed groin location to Jamaica Beach is 9.7 miles, much farther than sand is usually pumped in beach nourishment projects. This option may not be feasible because of construction and pumping costs.

6.3 Dredge an offshore deposition basin

Similar to the offshore deposition basin proposed for the East Beach area, a basin could be dredged near the west end of the island. The sand from the initial construction could be used as beach fill in other locations, and the periodic dredging would provide a regular source of additional sand. The exact size and location of a basin would need to be based on numerical modeling. A basin could be combined with a shore-perpendicular groin (section 6.1 above) to provide extra trapping capacity.

7 Proposed Large-Scale Beach along Seawall

This section describes one design for a 200 ft wide beach along the seawall. One of the goals of the GPB is to construct a wide beach to serve multiple purposes:

- enhance recreation opportunities
- protect the base of the seawall and provide storm damage reduction
- protect the west end of the seawall and mitigate the erosion of the present beach
- reduce the need for irregular and disruptive emergency beach fills using trucks.

This proposed beach includes the Board's Reaches 1, 2, and part of 3 (Figure 3). The beach fill should continue west beyond the end of the present seawall, adding sediment at the erosional hot spot at the seawall's right angle bend, and then taper into the existing beach in Reach 3.

7.1 General information

Beach fill is based on a designed (or desired) template and advance fill. The *design template* includes features such as dunes, beach height (above a water level datum), and dry beach width. *Advance fill* is the amount of extra fill that is installed such as beach erosion will only remove this additional material until the next planned renourishment. The amount of advance fill can be determined empirically based on wave data and budget considerations. For example, if the renourishment cycle is 5 years and the beach is expected to retreat X ft/year, then the advance fill must be adequate to build the beach out an additional 5X ft beyond the planned template. *Coastal Engineering Manual*, Chapter VI-4, provides details on of beach fill design (Gravens et al. 2008).

Sand for initial construction could come from a combination of

- offshore sand sources
- maintenance dredging of the Galveston navigation channels
- the offshore (northern) portion of Big Reef
- the USACE offshore ocean dredged material disposal sites (ODMDS).

Sediment source development includes numerous factors that must be considered:

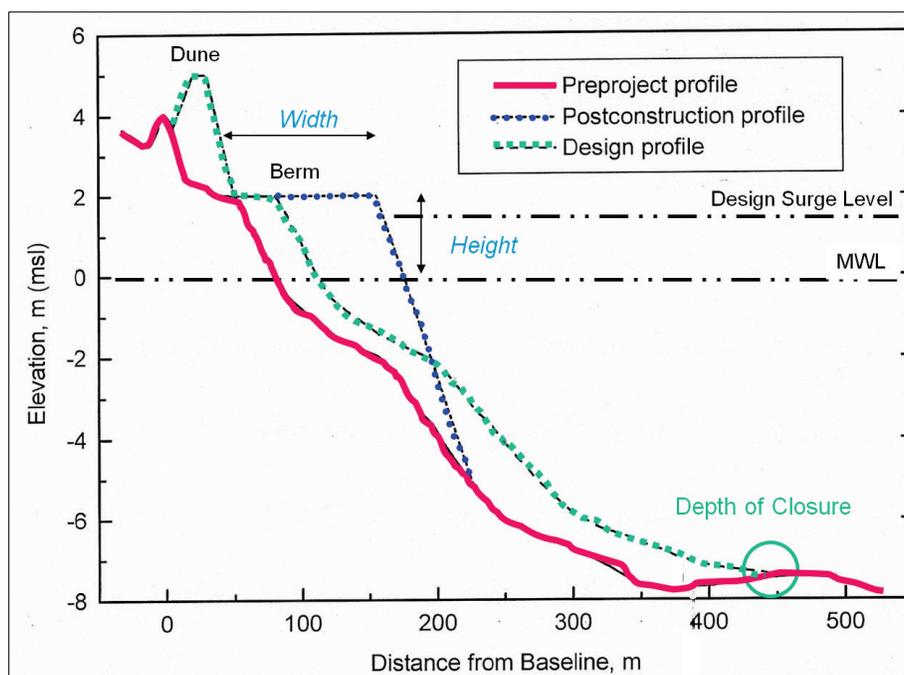
- type of site (e.g., offshore, navigation channel dredging, upland)
- distance from project
- accessibility
- morphology and stratigraphy
- sediment composition: grain size, composition, sorting, percent fines, color
- costs of mobilization/demobilization, extraction, transport, placement
- sediment source consistency and volume
- environmental factors
 - impact on habitat
 - threatened and endangered species
 - water quality
 - turbidity/suspended sediments
 - archeological sites

Hydrodynamic design parameters include

- wave climate
- tide (nonstorm) water level range
- storm surges
- tropical and extratropical storms (intensity, duration, frequency, seasonal and annual trends)
- currents, winds, drainage
- design level of protection (i.e., 100-year storm).

Public education is critical before and during a beach construction project. First, the public must understand that there will be equipment on the beach along with noise, fumes, and restricted access. Second, the beach will adjust and become narrower after construction. During placement, sand is pumped onto the upper shoreface, forming a wide beach called the construction template (blue post-construction line in Figure 32). Bulldozers move the sand and build dunes and other features specified in the contract. Over time, the sand is distributed by wave action across the shoreface, and the upper beach becomes narrower and equilibrates to the design template (green line). The public must be informed that the sand has not been *lost* but rather has been redistributed to conform to the natural shoreface profile in this region. The sand on the profile will protect the new subaerial beach.

Figure 32. Example of post-construction template versus design profile.



Construction schedule must be a consideration when contrasting a single long beach construction or smaller fills along Galveston Island. Mobilization and demobilization costs are expensive and are compounded by multiple smaller fills requiring multiple mobilizations and demobilizations. Additionally, if the project is divided into short sections, the short beaches are subject to end losses and erode more quickly than longer beach fill segments. Each successive segment construction will require effort and cost to replace sand lost during the previous winter. End losses (or beach half-life) can be calculated by a procedure in the *Coastal Engineering Manual* Part V-4 (Gravens et al. 2008). The advantages of a slow, incremental approach would outweigh the cheaper cost of constructing the project at one time. Finally, the large-scale beach fill would likely take over a year to build due to the amount of sand and environmental windows, which could require construction interruptions.

7.2 Full-scale beach fill design

The proposed all-island beach includes the following characteristics:

- length: 24.3 miles. Fill would extend from approximately 11th or 12th Streets to near San Luis Pass (Figure 33 and Figure 34).

Figure 33. Proposed comprehensive beach fill from East beach to San Luis Pass.

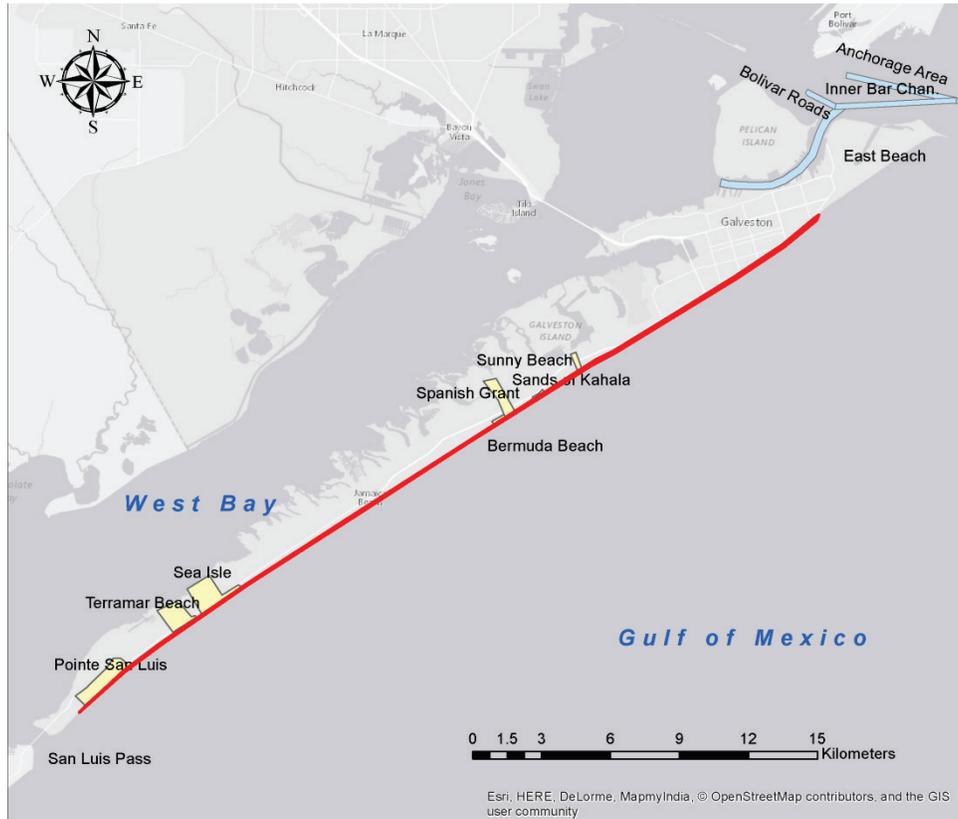
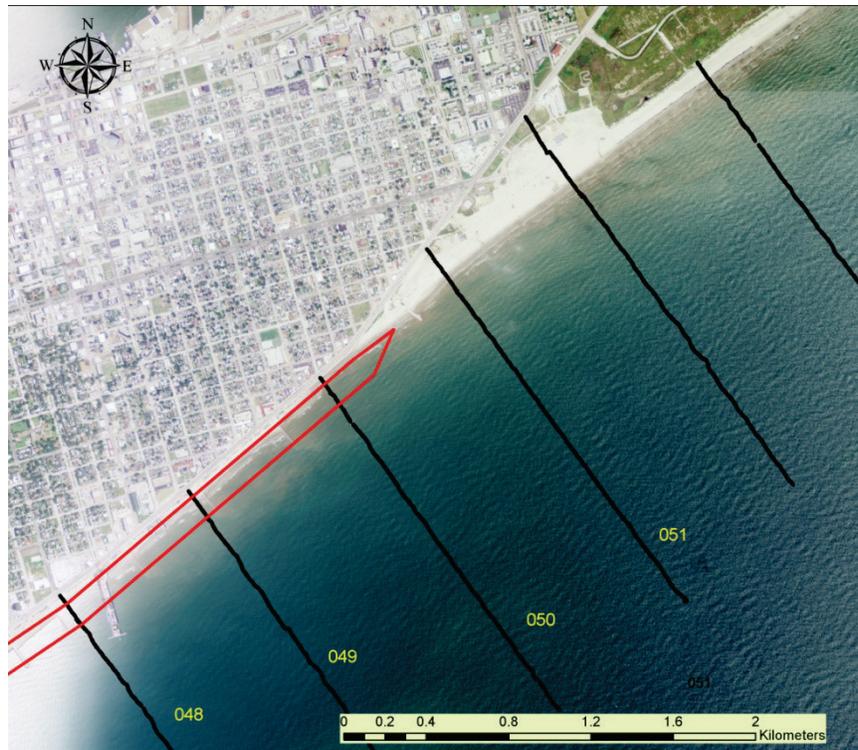


Figure 34. East end of fill begins at approximately 11th Street, to taper into East Beach.



- depth of closure: 20 ft. The most suitable way to determine the depth of closure in an area is to compare repetitive cross-shore profiles collected at the same zero origin. The seaward limit where multiple cross-shore profiles meet is known as the *depth of closure* (Morang and Birkemeier 2005). The active depth of closure is not well determined and time-sequence, cross-shore profiles are not available along Galveston Island. King (2007) used 6.0 m (19.7 ft) for his GENESIS modeling of Galveston Island. Brown and Kraus (1994) used 15 ft, while Howard (1999) used 4 m (13.1 ft). Wallace et al. (2010), looking at a geological perspective, concluded that 8 m (26 ft) was more suitable to represent the total region over which active sand movement occurred. For this study, the depth of 20 ft was used as the depth of closure to match the previous GENESIS numerical modeling and GenCade modeling presented here.
- berm elevation: 4.0 ft NAVD88 based on the 2002 profiles (Table 12).
- dune: approximately 10 ft above berm.

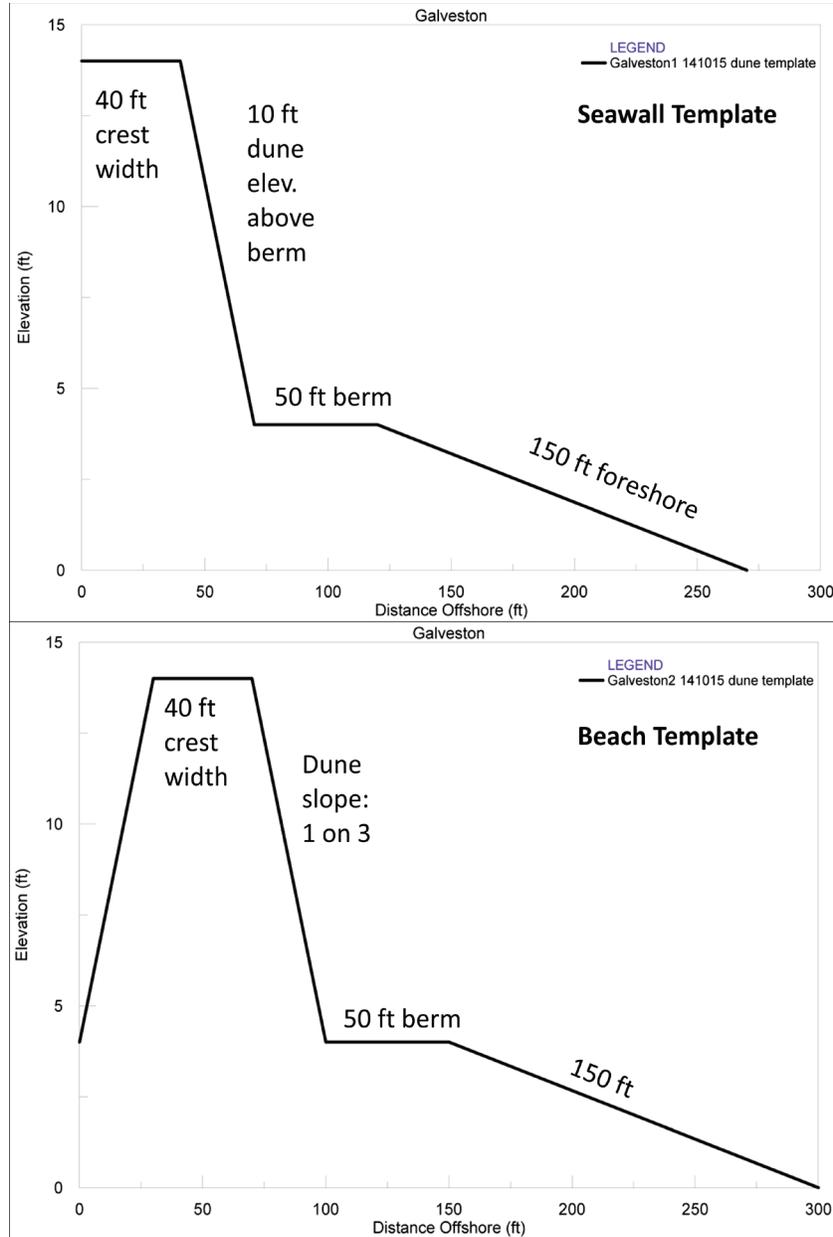
Table 12. Berm elevation measured for Galveston Island.

Sed. Budget Cell	2002 Profile	Berm elevation (ft, NAVD88)
1_5 (East Beach)	52, 55, 56, 57	4
1_6 (Seawall)		none
1_7	35	2
1_8		none
1_9	04, 05	4
1_10 (San Luis Pass)	02, 03	4

- width (as requested by Park Board, but can be adjusted depending on sediment availability, features desired, and budget)
 - Option 1: Dune: 100 ft and berm/beach; 200 ft with berm at approximately 4 ft NAVD88 elevation
 - Option 2: Same as above for the non-seawall portions of the island; no dune along the seawall and berm/beach: 200 ft wide..

shows the shape of the templates for Option 1 (note vertical exaggeration). Along the seawall (upper panel), the dune crest is 40 ft wide at an elevation of 14 ft, approximately level with the top of the seawall to provide a seamless transition. The front of the dune has a slope of 1:3, dropping down 10 ft to a 50 ft wide berm at 4 ft elevation. Then the beach foreshore slopes down to the water at an angle of 1.5°. The total beach profile width along the seawall is 270 ft; along the unstructured beach, the profile width is 300 ft.

Figure 35. Template at seawall for Option 1 (upper panel) and along western beaches (lower panel). Western beaches require a back slope for the dune because the seawall is not present to serve as a landward boundary structure.



Option 2 is a reduced sand volume design. The unstructured beach section is the same as for Option 1. However, along the seawall, the design does not include a dune. Instead, the beach consists of a flat berm 50 ft wide. Then the beach foreshore slopes down to the water at an angle of 1.5°, for a total width of 200 ft.

The volume of sand needed to build the template was computed at each cross-shore profile by superimposing the template and moving the existing profile seaward an appropriate amount to match the toe of the template at the 270 or 300 ft distance from the seawall (Figure 36 and Figure 37). Sand volumes were computed using BMAP software (Sommerfeld et al. 1994) in the synthetic profiles, beach fill module.

In Reaches 3, 4, and 5, the design was based on an average profile derived from the cross-shore profiles within the reach. Note that along the seawall, the proposed beach fill begins at the structure. But farther west, there is no obvious baseline. The analysis in this section is based on the 2002 Texas A&M profiles, which usually started at the seaward crest of the dune. The volume of sand needed for a proposed beach will vary depending on where the new dune is built or in other words, what zero position is selected. Sand needed for each reach (without advanced fill—see Tables E1 and E2) is the following:

- Reach 1 (seawall east)
 - Option 1: 1.942 million yd³
 - Option 2: 560,000 yd³
- Reach 2 (seawall west)
 - Option 1: 3.654 million yd³
 - Option 2: 2.132 million yd³
- Reach 3: 2.519 million yd³
- Reach 4: 4.408 million yd³
- Reach 5: 472,000 yd³

Figure 38 shows the fill in the western-most part of Galveston Island, Reach 5. Here, only a dune would be needed because significant sand already exists on the beach and in the offshore.

As described above, the amount of advanced fill will have to be determined based on the intended interval to renourish. If including a 50% advance, the total volume for Option 1 is 19.5 million yd³ while Option 2 is 15 million yd³. This advanced fill is an estimate only. Unlike typical open coast beaches, the seawall has modified the nearshore environment, and the beach retreat rates from BEG may not reflect the retreat that would occur along an unarmored shore.

Figure 36. Proposed Option 1 template along seawall at Profile 050. Red shows above-water fill extending to approximately the elevation of the present seawall. Blue shows the original nontranslated profile. Active depth = -20 ft NAVD88.

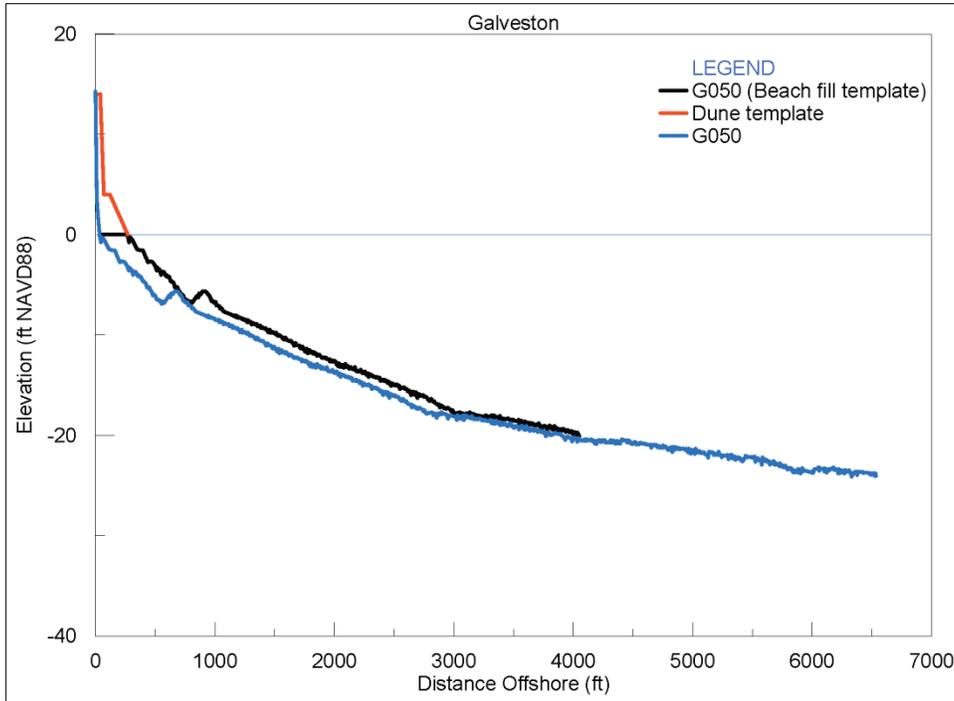


Figure 37. Proposed template along beach west of the west end of the present seawall, based on Profile G036 (full profile and offshore closure not shown).

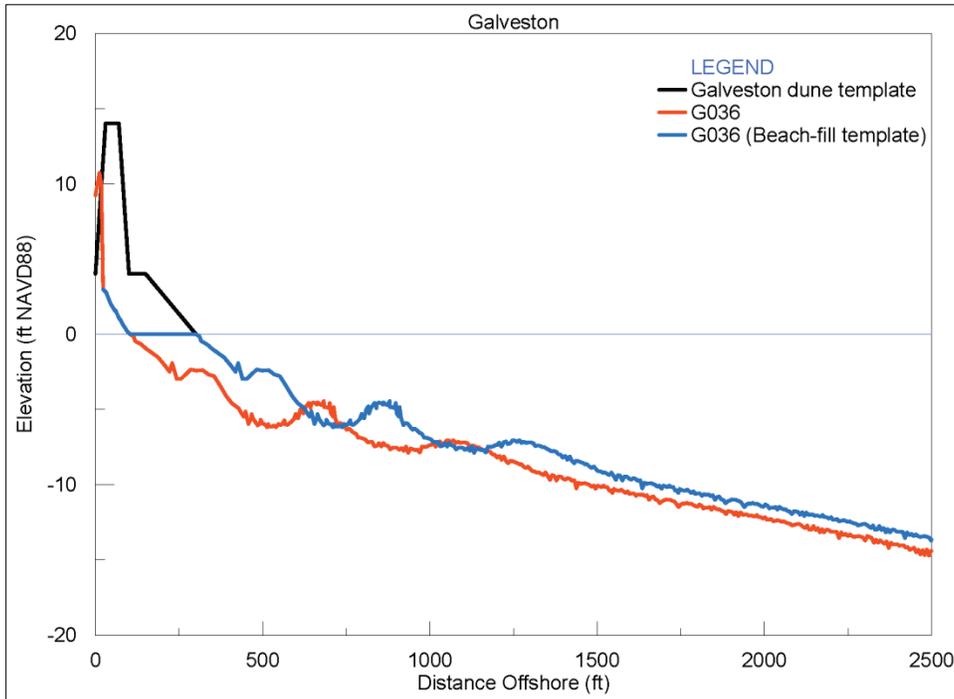
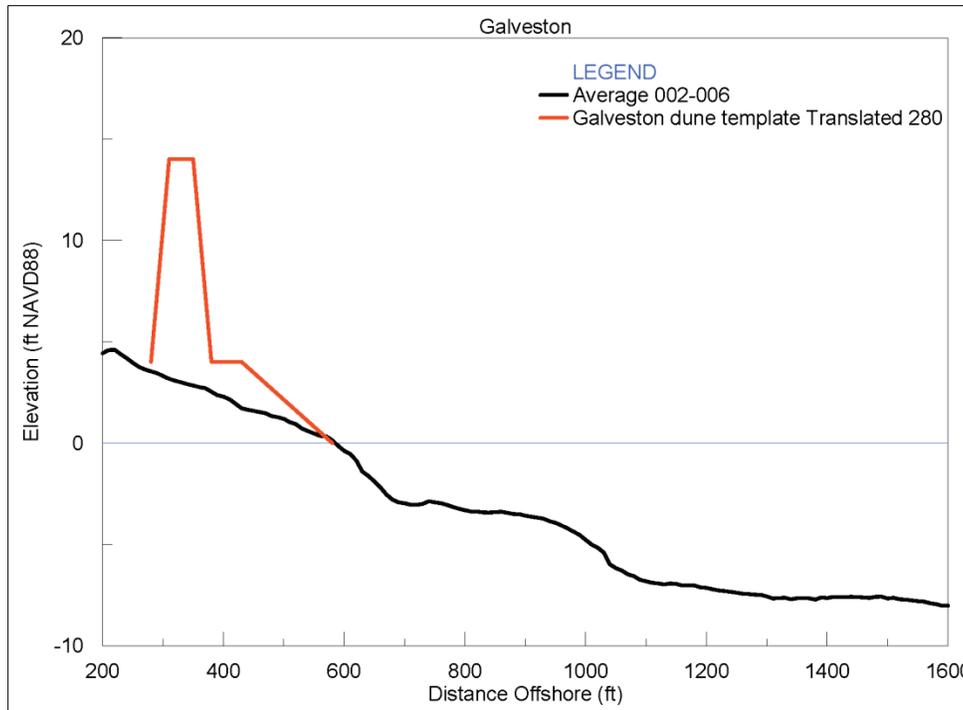


Figure 38. Example of dune constructed in Reach 5. No offshore fill would be needed. Full profiles 002-006 extend approximately 600 ft inland from the present shore (landward end not shown).



A post-construction monitoring program will be necessary to monitor fill stability, measure retreat, and check for hot spots. An emergency nourishment strategy should be developed to repair the beach rapidly following severe storm events.

	Source Term = 0K yd ³ /year	Source Term = 180K yd ³ /year	Source Term = 356K yd ³ /year	Reach 1	Reach 1 and Reach 2	GPB Properties (along Reach 3)	Reach 3 (First 1.5 miles)	Reach 3	Reach 3 and Reach 4	Groin Modifications	Large-Scale Beach Fill
5K yd ³ /year						X					
5K yd ³ /2 year						X					
5K yd ³ /5 year						X					
5K yd ³ /10 year						X					
10K yd ³ /2 year						X					
10K yd ³ /5 year						X					
20K yd ³ /year						X					
20K yd ³ /2 year						X					
20K yd ³ /5 year						X					
20K yd ³ /10 year						X					
50K yd ³ /year							X	X			
50K yd ³ /2 year							X	X			
50K yd ³ /5 year							X	X			
50K yd ³ /10 year							X	X			
100K yd ³ /year							X	X			
100K yd ³ /2 year							X	X			
100K yd ³ /5 year							X	X			
100K yd ³ /10 year							X	X			
250K yd ³ /2 year									X		
250K yd ³ /5 year									X		
250K yd ³ /10 year									X		
1 M yd ³ /2 year									X		
1 M yd ³ /5 year									X		
1 M yd ³ /10 year									X		
Large-Scale Beach Fill											
Backpassing - West End											
50K yd ³ /year							X	X			X
150K yd ³ /year									X		X
200K yd ³ /year							X	X			X
300K yd ³ /year									X		X

The No Action case was run for the 50-year time period to determine what would happen to the Galveston shoreline if no additional beach fills were added. In the GenCade calibration of the Galveston seawall grid, a source term of 355,700 yd³/year was added to the shoreline near the jetty to account for the increase in sand northeast of the 10th Street groin. Since it is unknown whether or not this rate of sand moving onshore will remain the same or decrease, rates of 0, 180,000, and 356,000 yd³/year were simulated to illustrate how much the rate of sand coming onshore affects the shoreline of Galveston Island. Figure 39 and Figure 40 compare the shoreline change along the seawall after 10 and 50 years, respectively, with different source terms. Again, all distances are measured in miles from the South Jetty of the Galveston Entrance Channel. After 10 years, when a source term of 356,000 yd³/year is added, the shoreline advances up to 225 ft in some areas northeast of the first groin. When no source term is added, the shoreline is predicted to retreat up 90 ft. After 50 years, different source terms greatly affect the final shoreline. When 356,000 yd³/year is added, the shoreline advances nearly 1,000 ft, but no addition of sand results in up to 300 ft of retreat. After 50 years, the difference in shoreline reaches 3.5 miles from the jetty, near the location of the first groin. It is important to remember that the rate of sand moving onshore will greatly impact the shoreline up to the first groin. The amount of sand available in this region of Galveston Island (previously identified as Reach 6) will dictate which of the backpassing options are available. Unless otherwise specified, all figures showing shoreline change along the seawall grid were modeled with 180,000 yd³ of material moving onshore near the jetty. Since it is not well known how much material, if any, will continue moving onshore in the future, it is recommended that a sand transport study take place before proceeding with any alternatives listed in this chapter. Along the seawall, total shoreline change is less than 50 ft in 50 years; however, the seawall restricts the shoreline from eroding much farther.

The west end grid was also analyzed after 10- and 50-year simulations to determine the effects on the shoreline if no additional sand is placed. Similar shoreline trends to the calibration were calculated. For example, just downdrift of the west end of the seawall, the shore continues to erode. After 10 years, the maximum erosion is just over 50 ft while after 50 years, the erosion increases to over 200 ft. This is to be expected based on historical shorelines in the area. Additionally, the shore accretes near San Luis Pass. Figure 41 only extends to 10 miles west of the seawall in order to

focus on the erosion near the west end. After 50 years, accretion near San Luis Pass reaches nearly 1,000 ft. In Figure 42, there is a slight dip in total shoreline change around 17 miles. The dip is due to the shape of the regional contour, which is based on the shapes of the historical shorelines.

Figure 39. Total shoreline change along seawall after 10 years.

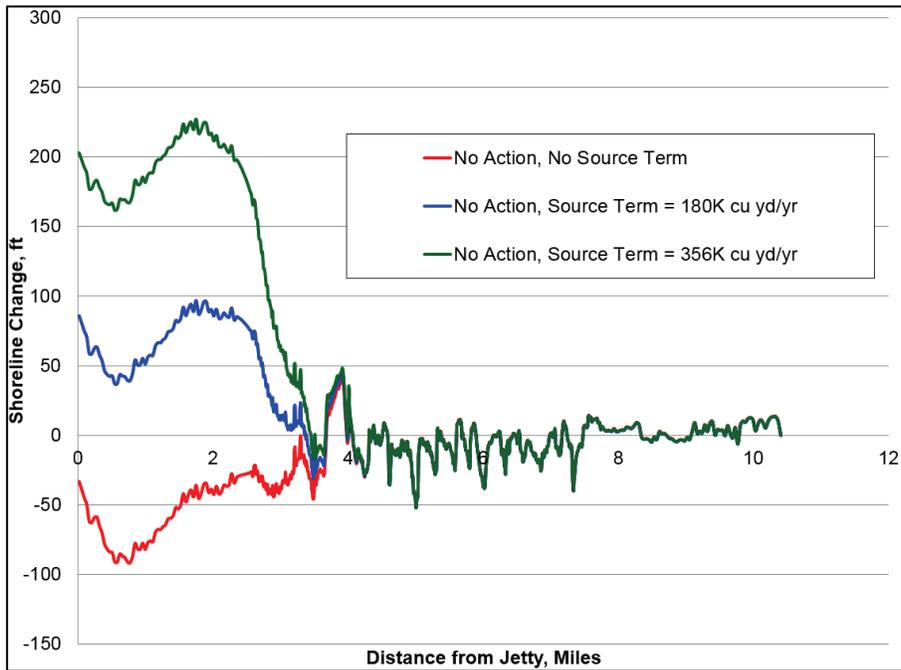


Figure 40. Total shoreline change along seawall after 50 years.

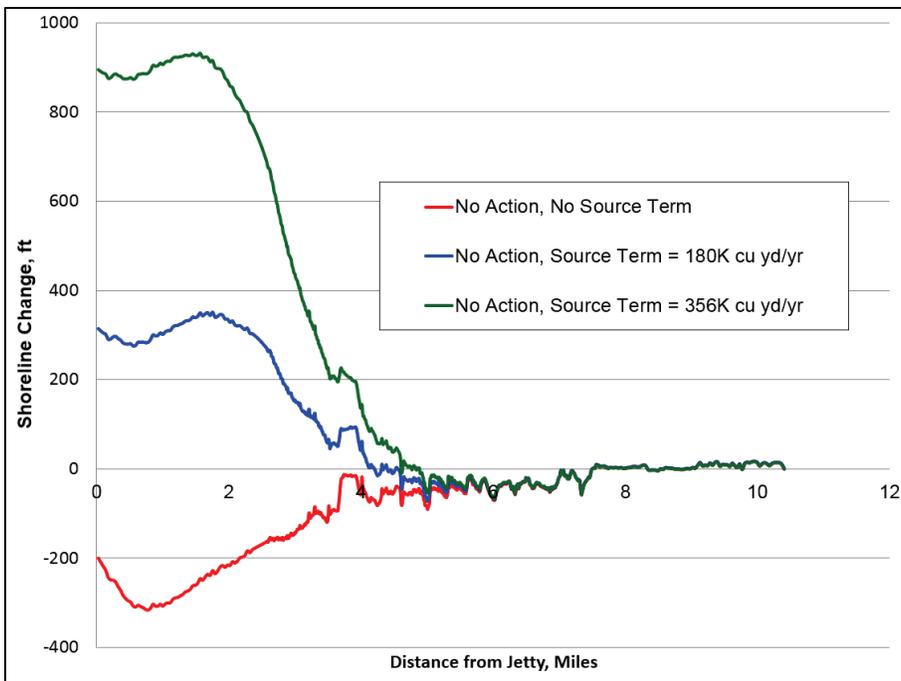


Figure 41. Total shoreline change along the west end after 10 years.

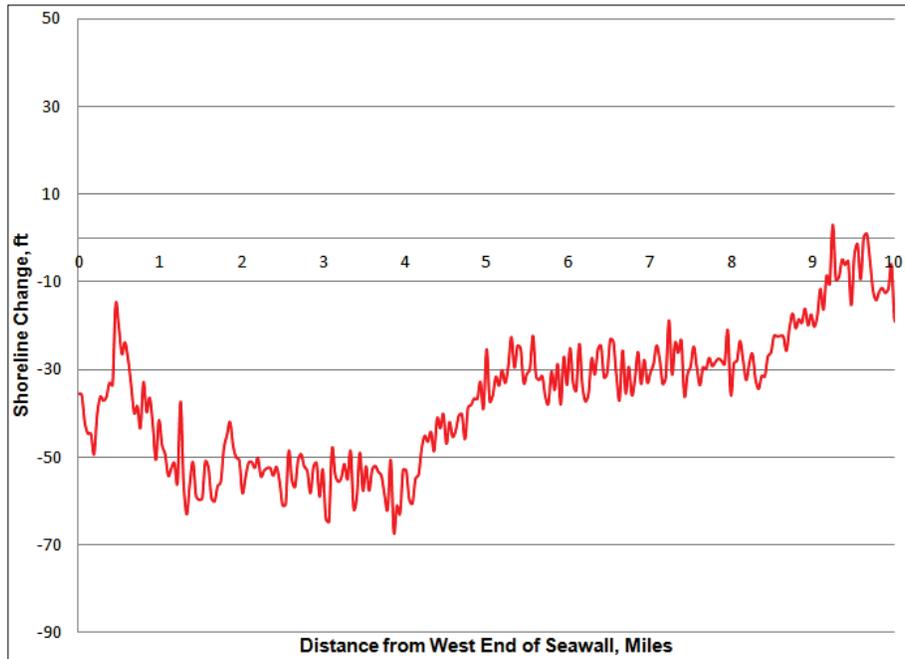
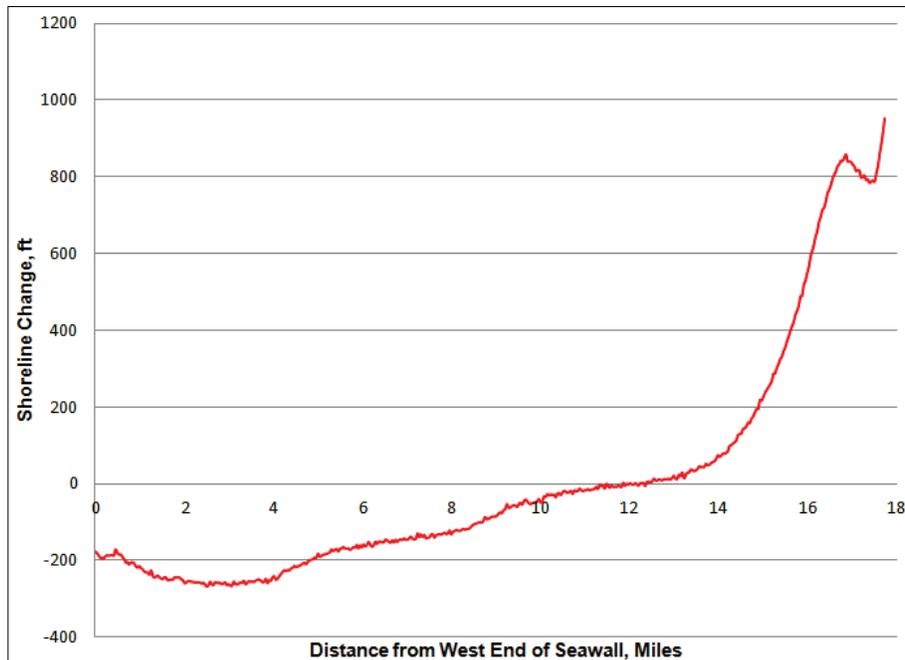


Figure 42. Total shoreline change along the west end after 50 years.



This chapter describes several different combinations of the alternatives outlined in Table 13. First, structural alternatives in front of the seawall are presented. Even if structural alternatives may not be pursued, it is important to consider all possible alternatives to provide a range of benefits for all potential actions. Then different beach fill options in front of the

seawall are also presented, including the simplest beach fill alternative consisting of a minor fill every 5 years. This alternative is representative of conditions when limited funding is available and beach fill is only placed periodically. Next, different beach fill volumes with different intervals are analyzed. Sand in these scenarios would likely be brought in by trucks. Alternatives will consist of Reach 1 only and Reach 1 and Reach 2 beach fills. Finally, a large-scale beach fill with periodic renourishment is analyzed. In addition, several alternative rates of backpassing from near the jetty to in front of the seawall are analyzed and discussed. The lower priority area for nourishment is the west end of Galveston Island; however, beach fills of various volumes and nourishment interval alternatives were modeled as well as alternatives where sand is backpassed from near San Luis Pass to Reaches 3 and 4. Note that all alternatives conducted on the west end should be considered in conjunction with alternatives in front of the seawall due to the higher priority of Reaches 1 and 2. However, it may occur that the GPB decides to nourish the west end of the seawall prior to nourishing Reach 2.

8.1 Structural alternatives in front of seawall

Numerical modeling with GenCade was conducted to determine if modifying the groins in front of the seawall would affect shoreline change. In the first alternative, all of the groins along the seawall were lengthened by 500 ft. The groins were shortened by 250 ft in the second alternative. There was also interest in the effects of the 10th Street groin, so the third model simulation was run with this particular groin removed while all of the other groins were left untouched. All of the groins were removed in the final structural alternative. Four structural alternatives are analyzed here in comparison to the no-action case.

The first alternatives compared the lengthened groins and shortened groins to the no-action case. Figure 43 shows the shoreline change for each alternative after 10 years from 2 to 8 miles west of the jetty. The shoreline change is the total over the 10-year period, so 100 ft of shoreline change in the figure is equivalent to 10 ft/year. All figures presented in this chapter show total shoreline change so that the reader can easily see how the alternatives affect the position of the shoreline in the short term (10 years) and the long term (50 years). Figure 43 shows distances of 2 to 8 miles only, because the alternatives produced nearly identical results in those areas. When the groins are lengthened, they capture more sand adjacent to each groin. Also, when the groins are shortened, they have little effect on the shoreline.

Figure 43. Total shoreline change with and without groin modifications after 10 years. The gray shading represents the alongshore distance between the 10th Street groin and the last groin in the system.

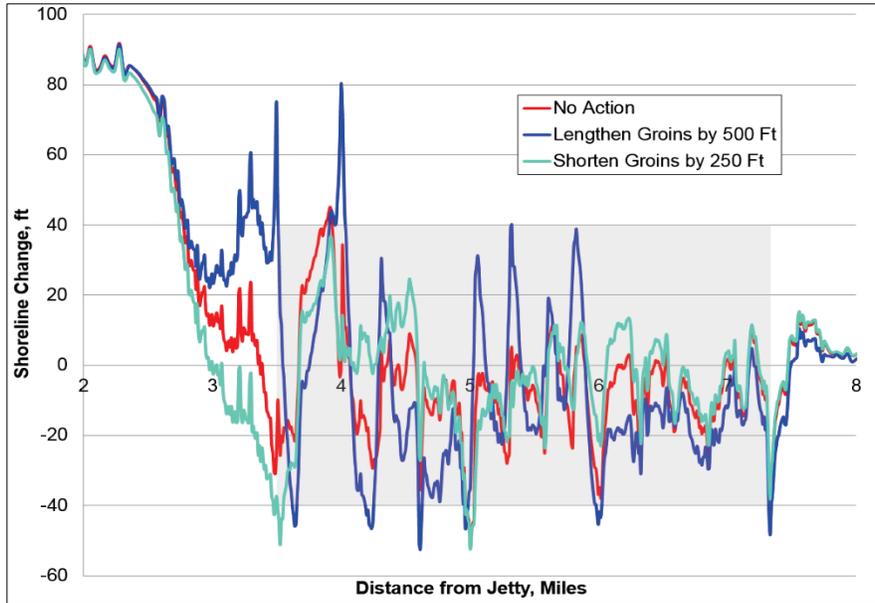
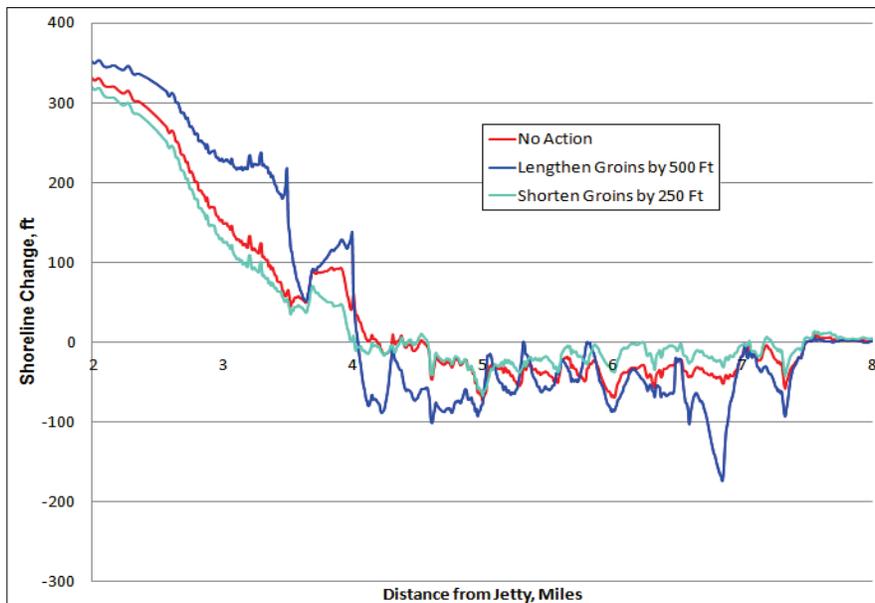


Figure 44 shows shoreline change for the no-action case, lengthened groins by 500 ft, and shortened groins by 250 ft after 50 years. The shoreline when the groins are shortened is not very different from the no-action results. When the groins are lengthened by 500 ft, the shoreline between each of the groins recedes approximately 50 ft compared to the no-action case.

Figure 44. Total shoreline change with and without groin modifications after 50 years.



The biggest impact of the length-modified groins can be seen close to the 10th Street groin (3.5 miles west of the South jetty). Northeast of the 10th Street groin, the lengthened groin alternative resulted in shoreline advance of approximately 40 ft while the shortened groin alternative produces a shoreline loss of approximately 40 ft.

It was also necessary to determine the impact of the 10th Street groin alone. Figure 45 shows shoreline change after 50 years for the no-action case, the no groins case, and the removal of the 10th Street groin case. The 10th Street groin is located 3.5 miles west of the jetty. When the 10th Street groin is removed, the shoreline advances slightly more than the no-action alternative. About 1.5 miles west of the groin, removing the single 10th Street groin has no impact on shoreline change. When all of the groins are removed, the shoreline does not erode as much as the no-action case, although the shoreline still erodes from the initial position. Figure 46 compares the no-action case to the no-groins case near the 10th Street groin. The initial shoreline is red while the calculated 50-year shoreline is green. The left image is the no-action case while the right image shows the shoreline with the groins removed. Note that both images show the location of the groin to better compare the accretion in the area. While it is difficult to see, the calculated shoreline advances more when the groins are removed.

Figure 45. Total shoreline change with and without groin after 50 years.

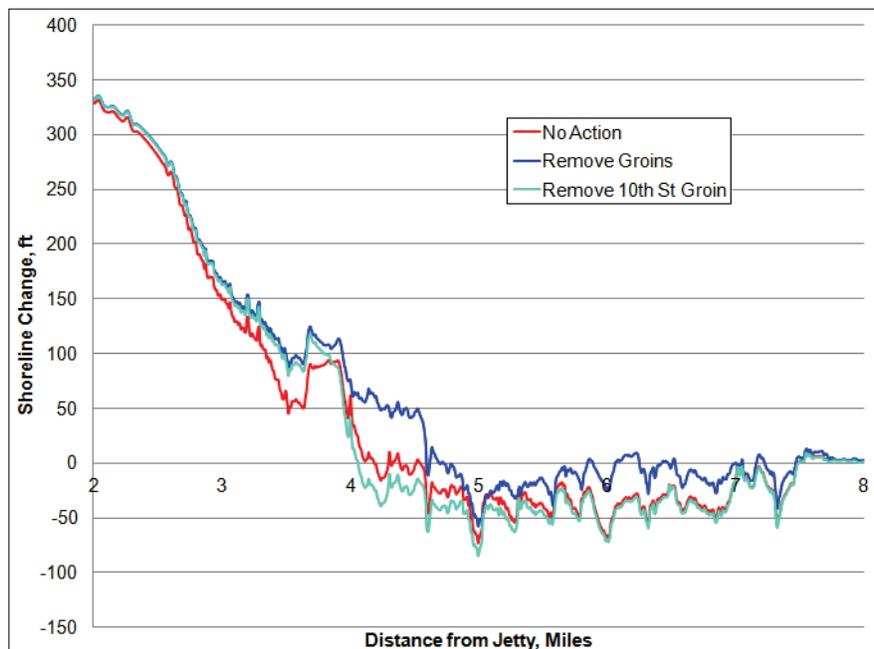


Figure 46. SMS image of the shoreline after 50 years (green) compared to initial (red) with the 10th Street groin (left) and without (right).

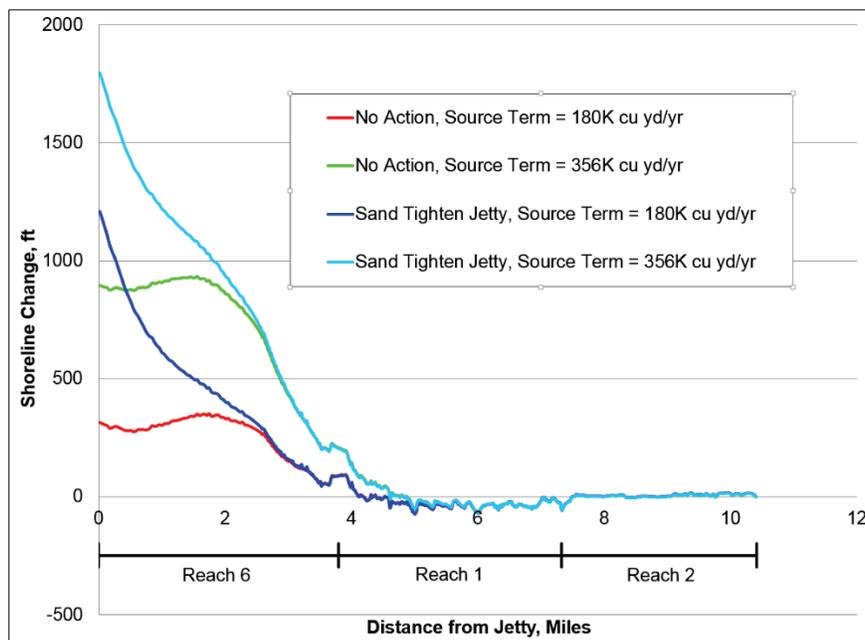


Although the structural alternatives result in slightly different calculated shorelines than the no-action alternative, none of the structural alternatives independently resulted in shoreline advance along the seawall; therefore, modifying the structures as a part of the sand management strategy is not recommended. Some of the structural alternatives were modeled in conjunction with beach fills, and those alternatives are presented later in this chapter.

8.2 Sand tightening the Galveston Entrance Channel jetty

Sand tightening of the Galveston Entrance Channel jetty was an alternative discussed in preliminary scoping. In the no-action alternative, the jetty permeability was 0.1. When the jetty is sand tightened, the permeability becomes zero so that no sand moves through the jetty. In Figure 47, the shoreline change after 50 years for the no-action case was compared to the sand tightening cases. The model was run with a source term of 180,000 yd³/year and 356,000 yd³/year. When the source term is 180,000 yd³/year, sand tightening results in shoreline advance of approximately 900 ft compared to approximately 300 ft with no action. With a source term of 356,000 yd³/year, the shoreline change near the jetty increases from 1200 ft to 1800 ft. The impacts of sand tightening only extend approximately 2.6 miles from the jetty. Although sand tightening the jetty results in further shoreline advance near the jetty compared to the no-action case, it is not recommended that this alternative be considered alone. Sand tightening could result in more sand adjacent to the jetty, so it should be considered in combination with sand backpassing.

Figure 47. Comparison of total shoreline change for no action and sand tightening with 180,000 yd³/year and 356,000 yd³/year source terms after 50 years. Reaches 6, 1, and 2 are labeled.



8.3 Beach fill alternatives along the seawall

Many beach fill alternatives were modeled with GenCade. The alternatives are classified by levels where Level 1 is considered very minor and probably most similar to the type of beach nourishment presently taking place in Galveston while Level 4 is the most significant. Level 4 represents an initial wide beach along Reaches 1 and 2 with periodic renourishment. The alternatives in this section start with Level 1 and end with Level 4.

8.3.1 Level 1 – Small beach fill in Reach 1

The beach fill alternative representing Level 1 is the smallest of the beach fills presented. It is meant to represent the type of beach nourishment that presently takes place along the seawall: small, infrequent nourishments. At this time, a beach fill in front of the seawall is considered to be purely cosmetic and for recreation instead of a necessity to protect the shoreline since a shoreline protection structure is already in place. The Level 1 beach fill consists of an initial beach fill of 100,000 yd³ and an additional 100,000 yd³ placed every 5 years. The beach fill is placed along Reach 1 only. The sand for this beach fill is likely to be brought by trucks from near the jetty. The purpose of this beach fill is to show whether or not a small beach fill in front of the seawall will widen the beach compared to a no-

action condition. This alternative is the least expensive, so if funding is a concern, it is probably the most likely alternative.

Figure 48 and Figure 49 compare the 100,000 yd³ beach fill renourished every 5 years to the no-action scenario. After 10 years, two 100,000 yd³ beach fills have been placed on Reach 1; however, there is little advance from the no-action case. In most locations along Reach 1, the advance from the no-action case is less than 10 ft. Even though a minor beach fill is placed, there are some locations along Reach 1 that experience erosion from the initial shoreline. After 50 years, a total of 1 million yd³ will be placed along Reach 1. In all locations along Reach 1, the Level 1 beach fill advances at least 50 ft from the no-action case. After 50 years, the Level 1 beach fill advances from the initial shoreline approximately 25 ft, but the future beach is not expected to look significantly wider than the present day beach. The no-action case has eroded to near the seawall, so although the Level 1 case does not widen the beach from the initial condition, it does protect the seawall.

Figure 48. Total shoreline change after 10 years for the no-action case and a 100,000 yd³ beach fill renourished every 5 years. Note vertical scale differences.

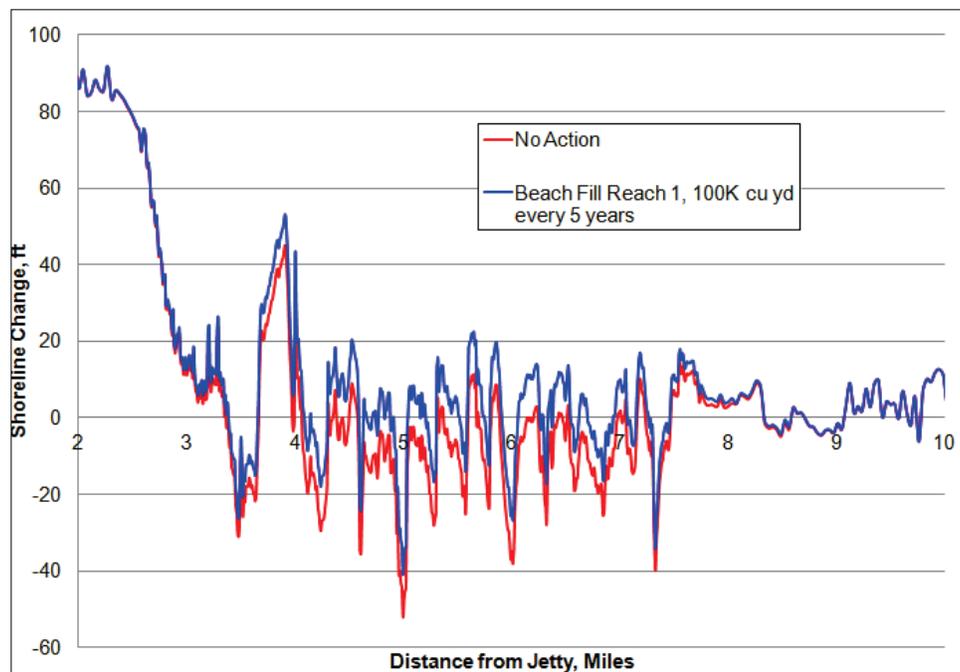
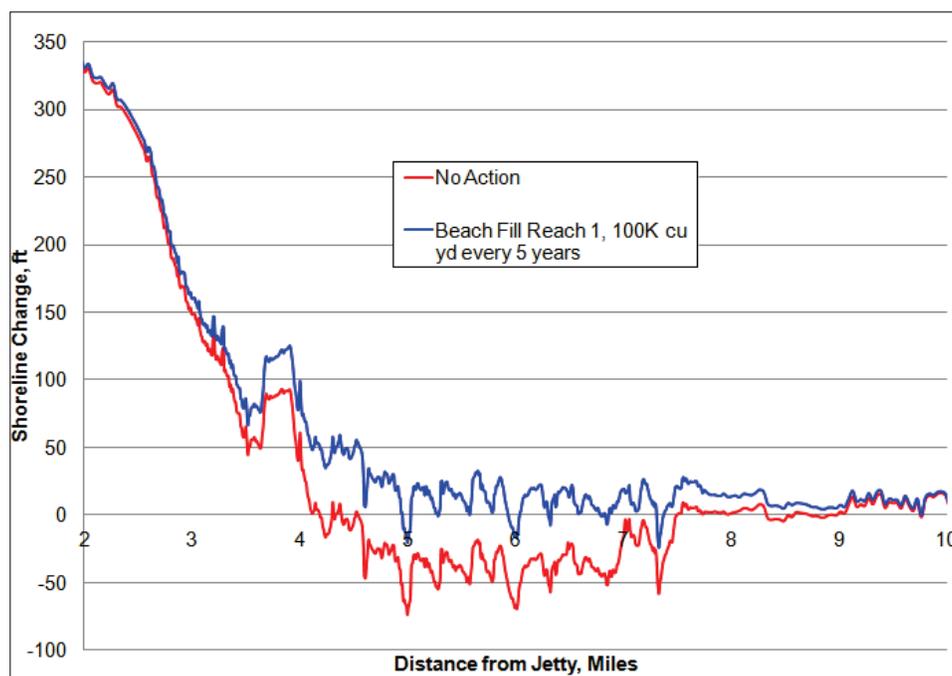


Figure 49. Total shoreline change after 50 years for the no-action case and a 100,000 yd³ beach fill renourished every 5 years (total added sand = 1 million yd³).



8.3.2 Level 2 – Beach fills on Reach 1

Level 2 beach fills consist of larger scale nourishment projects along Reach 1. Several of these alternatives would provide a wide, recreational beach along Reach 1 and would require periodic nourishment. The volumes range from approximately 250,000 yd³ to up to 2 million yd³. None of the Level 2 or 3 beach fills include a larger fill at the beginning; the same volume is placed at the beginning and during renourishment. The reason the initial placement is not larger than the volume of renourishment is because it might be difficult to find enough sand at the beginning to construct a large-scale beach fill. Level 4 alternatives include a large beach fill at the beginning of the simulation with renourishment over the rest of the simulation. Note that some of these alternatives require large volumes of sand every couple of years and that the source of sediment is not considered in the beach fill alternatives. (There is a lack of available, beach-compatible sand near to Reach 1. Some of the sand could be mined from Big Reef and trucked to Reach 1, but it is likely that other options like offshore mining would be necessary.)

The first Level 2 alternative is a 250,000 yd³ beach fill renourished every other year. The total placement after 50 years is 6.25 million yd³. Figure 50 compares the 250,000 yd³ beach fill every other year alternative to the no-

action case after 10 years with the source term near the jetty varying from 0 to 356,000 yd³/year. The purpose of showing three different source term rates is to illustrate how much that term affects the shoreline near the 10th Street groin. While the source term is not as important for the beach fill options as the backpassing options, it is still necessary to understand the sand source before constructing large scale beach fills. After 10 years, the 250,000 yd³ beach fill alternative will add 1.25 million yd³ of sand to Reach 1, and the beach will advance 50 ft. With no source of material near the jetty, GenCade predicts erosion of almost 100 ft approximately 0.75 mile from the jetty. When 356,000 yd³/year is added to the shore near the jetty, the shoreline advances more than 200 ft in some areas. After 50 years, the shoreline in Reach 1 advances more than 200 ft (Figure 51). As the sand disperses, the seawall west of 61st Street begins to receive some material. The sand on the eastern section of the seawall moves to the east, so it makes sense that the shoreline advances from the no-action case up to 2 miles east of the 10th Street groin. It is noted that the source term rate has a significant impact on the shoreline. With a 356,000 yd³/year source term rate, the shoreline advances more than 900 ft while removing the source term results in erosion of almost 300 ft. The effects of the source term can be seen 2 miles west of the 10th Street groin.

Figure 50. Total shoreline change after 10 years for the no-action case and a 250,000 yd³ beach fill nourished every other year. The gray shading represents the alongshore location of the added volume. The location of the 10th Street groin is shown.

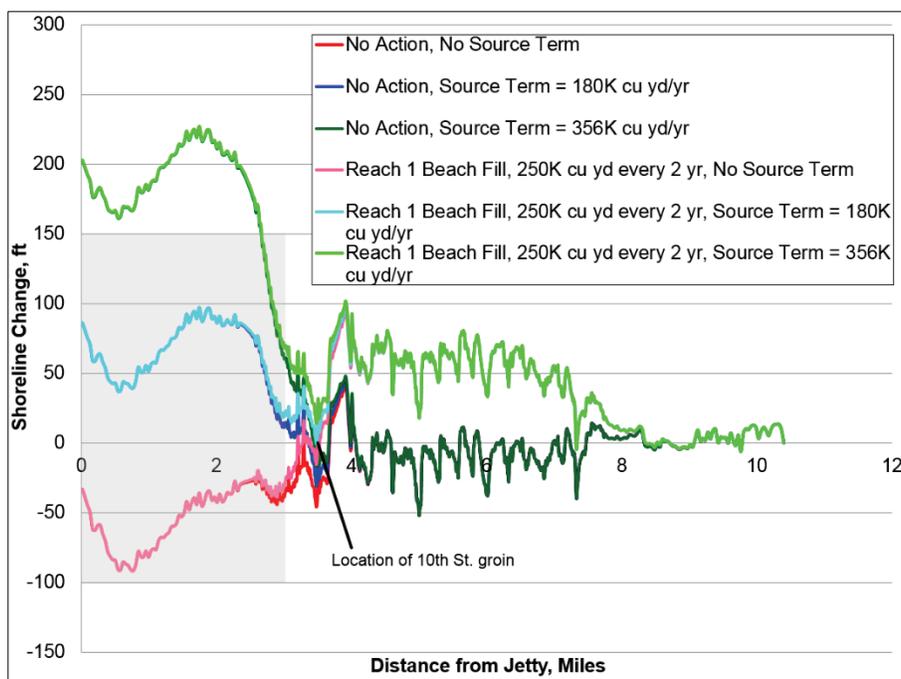
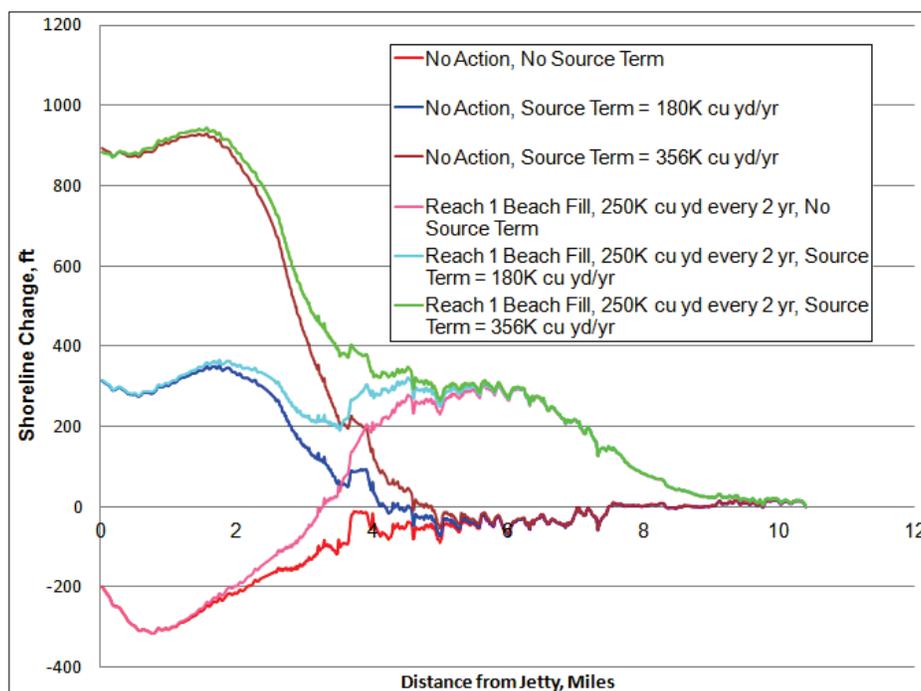
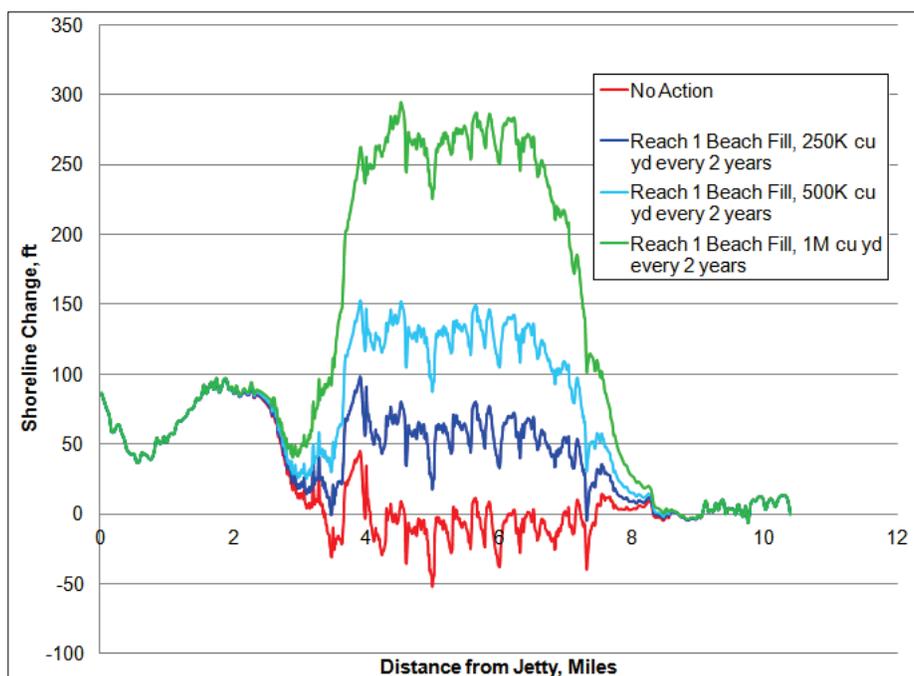


Figure 51. Total shoreline change after 50 years for the no-action case and a 250,000 yd³ beach fill renourished every other year.



Larger beach fills of 500,000 and 1 million yd³ every other year were compared with the 250,000 yd³ placement every-other-year case. The total placement along Reach 1 after 50 years is 12.5 million and 25 million for the 500,000 and 1 million yd³ cases, respectively. Figure 52 compares the total shoreline change for all three cases after 10 years. It is noted that all of the shorelines presented in subsequent figures will include a source term of 180,000 yd³/year. Although simulations with source terms of 0 and 356,000 yd³/year were modeled, it becomes repetitive to show source terms of 0, 180,000, and 356,000 yd³/year in each figure. A source term of 180,000 yd³/year is shown in the figures because it is the middle term and is probably the most representative of the rate of sand moving onshore. It is unlikely that more than 350,000 yd³/year will continue to move onshore for a total of 50 years especially if the ODMDS is the source of sand and that sand is used beneficially instead. After 10 years, the shoreline advances more than 250 ft in Reach 1 for the 1 million yd³ every-5-years case and approximately 125 ft for the 500,000 yd³ every-5-years case.

Figure 52. Total shoreline change after 10 years for the no-action case and 250,000 yd³, 500,000 yd³, and 1 million yd³ beach fills renourished every other year.



After 50 years, almost the entire shoreline from the jetty to the end of the seawall is affected by the beach fill (Figure 53). The 1 million yd³ every-other-year alternative advances the shoreline in Reach 1 by 1200 ft. This would provide an extremely wide beach in front of the seawall. However, some tourists might complain that the walking distance from the seawall to water is too far. It is also not practical to place 1 million yd³ of material along the beach every other year for 50 years since usable sand near Galveston Island is limited. The 500,000 yd³ every-other-year alternative results in shoreline advance in Reach 1 of 600 ft. The sand from these beach fills also moves east and west from Reach 1. The shoreline from 1.25 miles to 9 miles west of the jetty advances when the beach fills are in place.

Larger beach fills with longer renourishment intervals were also modeled. A 500,000 yd³ beach fill renourished every 5 years is compared with a 2 million yd³ initial beach fill that is renourished with 2 million yd³ every 5 years. After 50 years, the total volume placed for the 500,000 yd³ case is 5 million yd³ while the 2 million yd³ case placed 20 million yd³. Figure 54 compares the two cases with the no-action alternative after 10 years. After 10 years, the shoreline in Reach 1 advances about 50 ft when 250,000 yd³ is placed every 5 years. When 2 million yd³ is placed every 5 years, the shoreline advances approximately 200 ft after 10 years which is approximately GPB's preferred beach width. However, it is important to

note that if the periodic placements are discontinued, the shoreline will retreat and look similar to the no-action alternative after 50 years. After 50 years of placing 2 million yd³ on the beach every 5 years, the shoreline advances up to 1000 ft (Figure 55). The 500,000 yd³ placement every 5 years results in shoreline advance of 200 ft. Both alternatives will result in a wider beach along the eastern portion of Galveston Island and the far eastern portion of Reach 2.

Figure 53. Total shoreline change after 50 years for the no-action case and 250,000 yd³, 500,000 yd³, and 1 million yd³ beach fills renourished every other year.

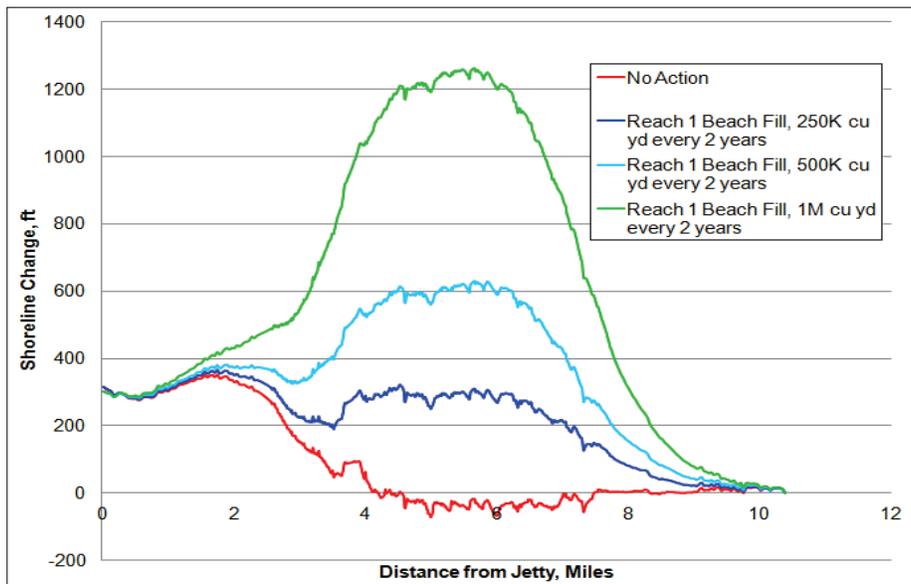


Figure 54. Total shoreline change after 10 years for the no-action case and 500,000 yd³ and 2 million yd³ cases renourished every 5 years

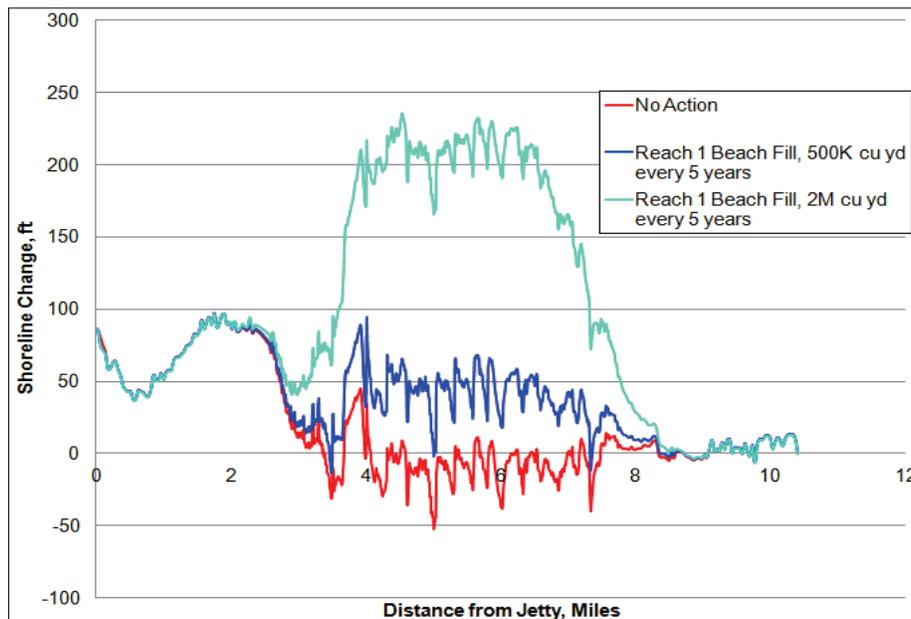
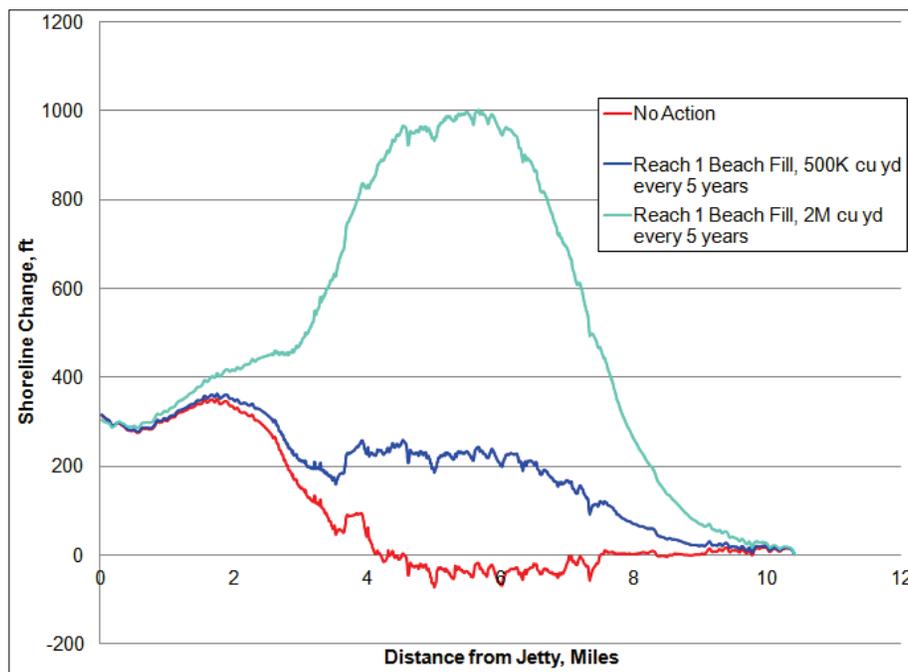


Figure 55. Total shoreline change after 50 years for the no-action case and 500,000 yd³ and 2 million yd³ cases renourished every 5 years



Another set of alternatives consists of an initial beach fill of 125,000 yd³ with renourishment of 125,000 yd³ every year. Each beach fill is small, but over 50 years, the total volume placed on the beach reaches 6.25 million yd³. Different placement options over Reach 1 were considered. First, the beach fill was placed with consistent volume over each part of Reach 1. Then, a second option placed sand in different a location along the seawall. Every other year, 125,000 yd³ were placed on the northeastern half of Reach 1 while 125,000 yd³ were placed on the southwestern half of Reach 1 during the other renourishment cycles. Finally, the material was placed in four locations along Reach 1. In year 1 and every 4 years, material was placed on the farthest northeast quarter of Reach 1. In year 2 and every 4 years, material was placed on the second quarter. Then material was placed on the third quarter in year 3 and the fourth quarter in year 4. The purpose of these alternatives was to determine if the location along Reach 1 made a difference when the same total volume was added. Figure 56 and Figure 57 show the alternatives after 10 years and 50 years. The alternatives are almost identical. The total shoreline change after 10 years is slightly different when the material is rotated between two placement locations versus four locations, but the results are so similar. Therefore, if the GPB opts to place sediment in discrete locations or along all of Reach 1, after 10 years, the results will be similar, and sand will disperse along the shore.

Figure 56. Total shoreline change after 10 years for 125,000 yd³ beach fills.

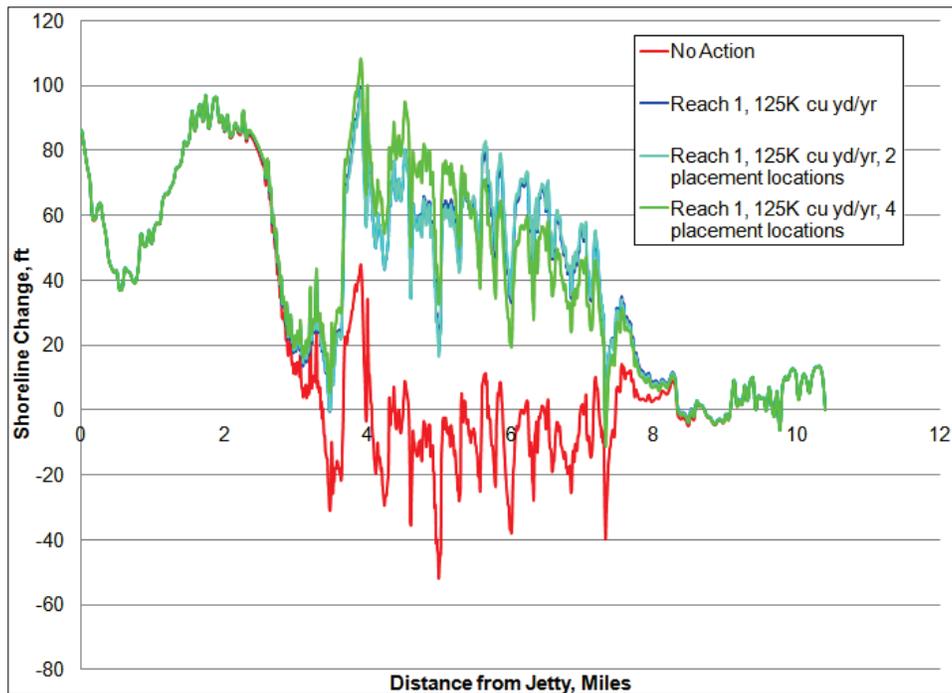


Figure 57. Total shoreline change after 50 years for 125,000 yd³ beach fills.

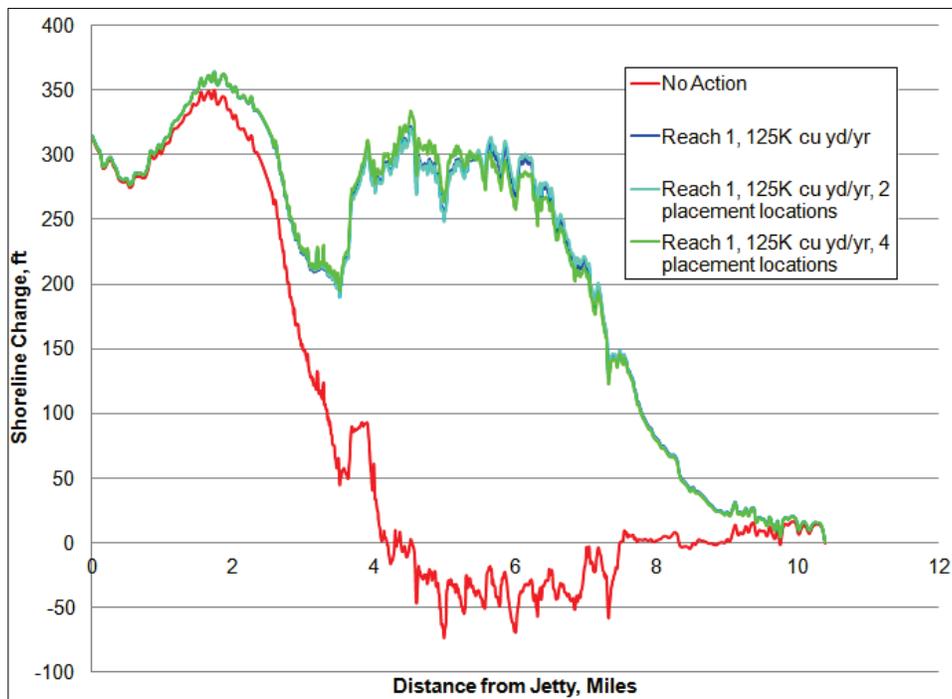


Figure 58 and Figure 59 compare the no-action alternative to adding 250,000 yd³ every other year in Reach 1 with modifications to the groins. In the first alternative, the groins are lengthened by 500 ft, and they are

removed in the second alternative. After 10 years, altering the groins does not make much difference. While lengthening the groins causes the shoreline to advance closest to the groins and recede between groins, the average shoreline advance is very similar to the alternatives where the groins are not adjusted or removed. After 50 years, changes to the structures are minimal. While the shoreline shape for each case looks slightly different, each case results in shoreline advance of approximately 300 ft. Figure 60 shows shoreline change with and without groins with a 1 million yd³ beach fill renourished every 2 years. Groin modification has an even smaller impact when paired with a larger beach fill. After 50 years with a 250,000 yd³ beach fill placed every other year, it is important to note that the existing groins are nearly buried, so they would have little effect compared to the alternative where the groins are removed. The shoreline will still respond to the lengthened groins. Based on the GenCade simulations, modifying the groins does not increase shoreline advance. If a groin modification is considered an option in the future, additional studies must be conducted prior to construction.

Figure 58. Total shoreline change after 10 years for 250,000 yd³ beach fills renourished every other year with and without modified groins.

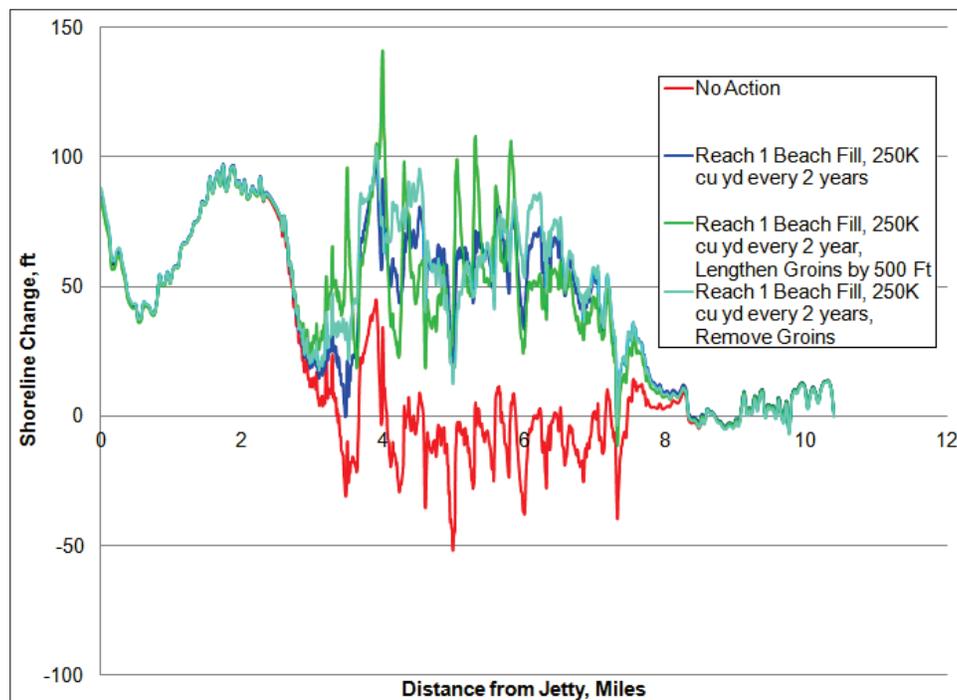


Figure 59. Total shoreline change after 50 years for 250,000 yd³ beach fills renourished every other year with and without modified groins.

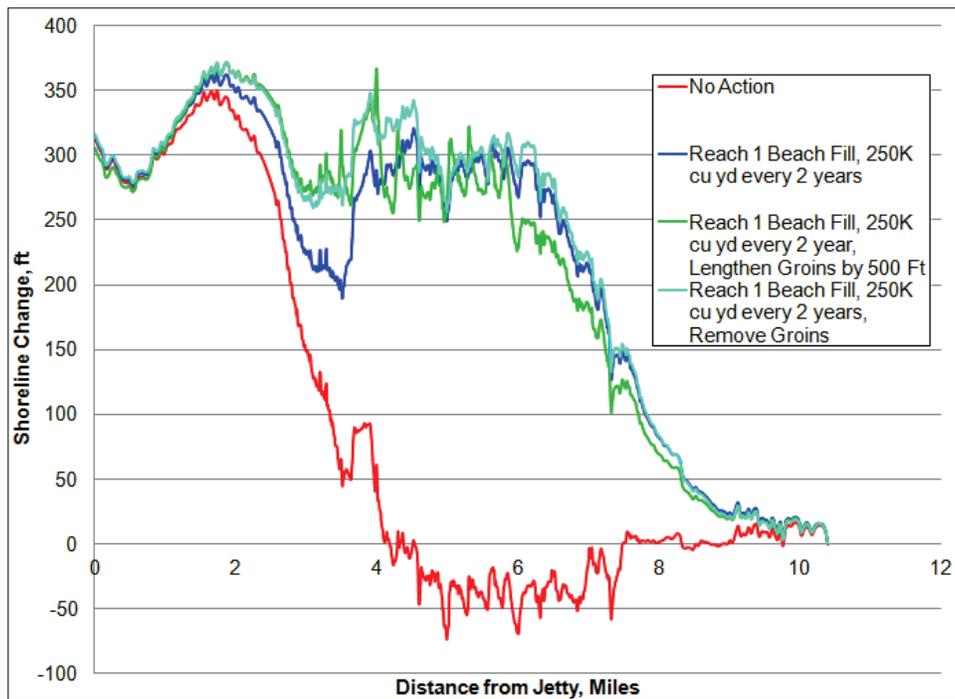
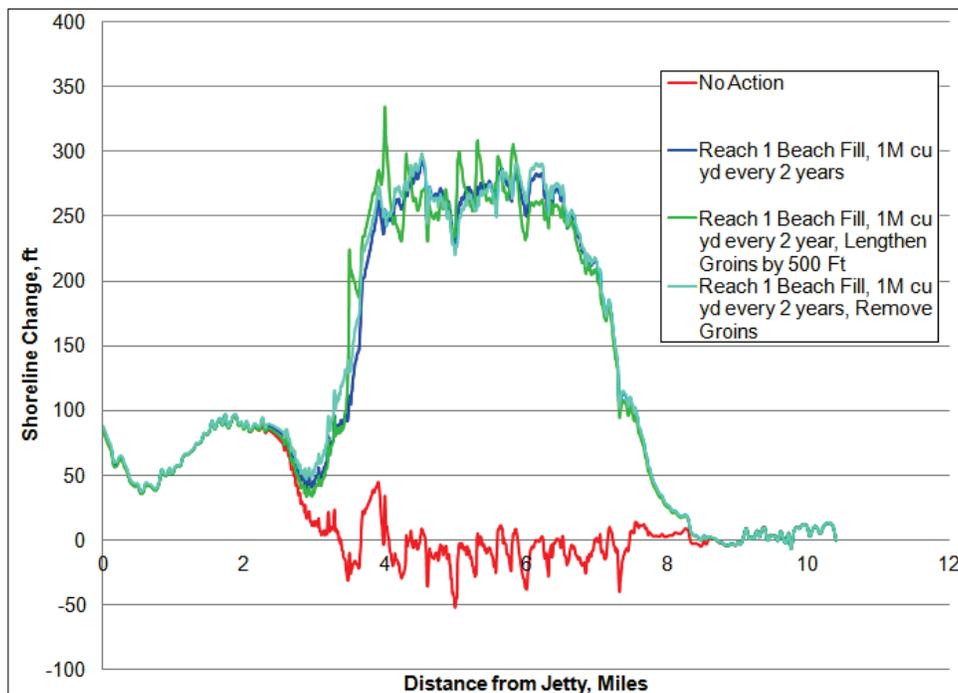


Figure 60. Total shoreline change after 10 years for 1 million yd³ beach fills renourished every other year with and without modified groins.



8.3.3 Level 3 – Beach fills on Reach 1 and Reach 2

Level 3 consists of beach fills being placed on Reaches 1 and 2. In many cases, the same volumes and renourishment intervals used in the alternatives in the previous section are shown here. The reason this is done is so the GPB can decide if it is more advantageous to place sand on Reach 1 versus Reaches 1 and 2.

The first set of alternatives compare 250,000 yd³, 500,000 yd³, and 1 million yd³ initial fills renourished with the same volumes every 2 years. Figure 61 shows total shoreline change for each alternative after 10 years. The shoreline advances approximately 40 ft when 250,000 is placed every 2 years (1.25 million yd³ total after 10 years) while the 500,000 yd³ alternative advances the beach by 75 ft. The shoreline advances 150 ft after 10 years when 1 million yd³ of material is placed every 2 years. After 50 years, the shoreline advances approximately 200 ft with placements of 250,000 yd³ every other year and up to 800 ft when 1 million yd³ is placed every other year (Figure 62).

Figure 61. Total shoreline change after 10 years for 250,000 yd³, 500,000 yd³, and 1 million yd³ beach fills renourished every other year placed on Reaches 1 and 2.

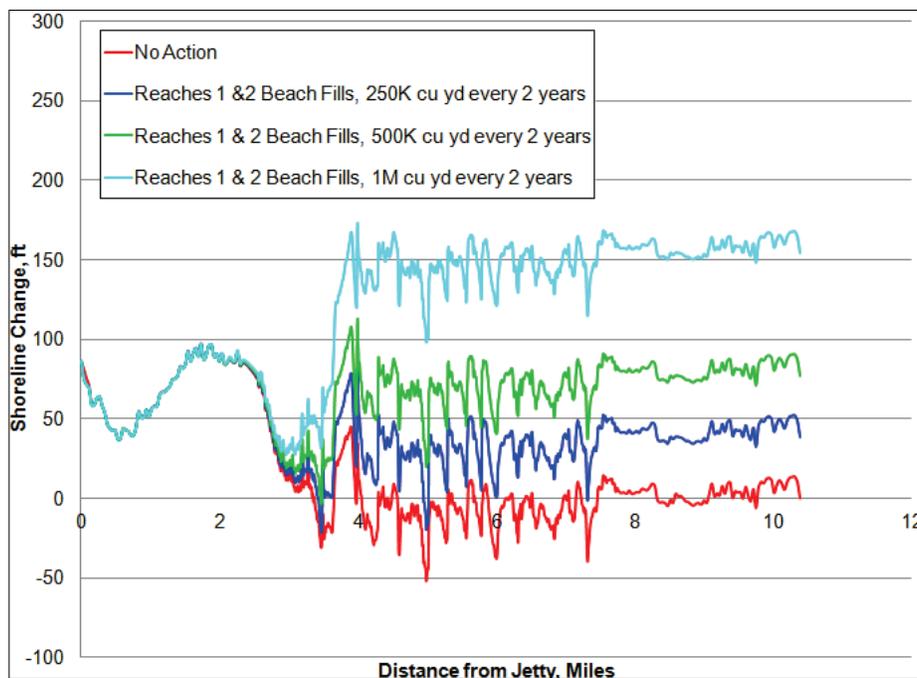
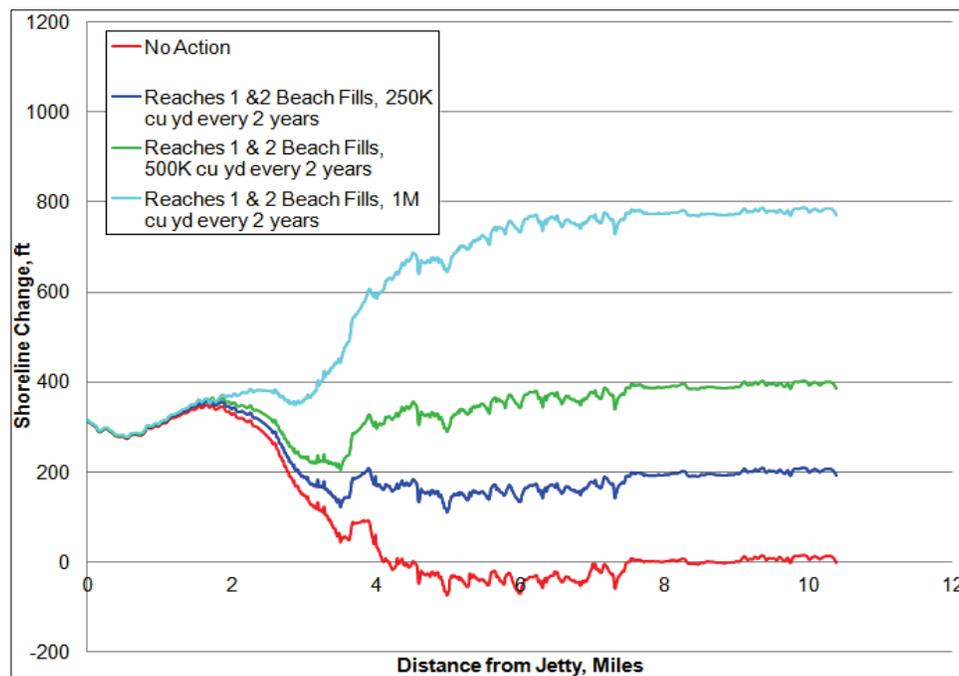


Figure 62. Total shoreline change after 50 years for 250,000 yd³, 500,000 yd³, and 1 million yd³ beach fills renourished every other year placed on Reaches 1 and 2.



It is important to remember some of the underlying parameters of GenCade. First, GenCade does not know that there is not much sand underwater in front of the seawall. Therefore, it will take less sand to build a beach in the simulations than will actually occur. Also, GenCade requires an input for berm height and depth of closure. These values remain constant across the entire grid x -axis and cannot be adjusted in specific areas. Additionally, GenCade assumes an equilibrium beach profile. Profiles in different areas along the shore indicate different offshore bathymetries out to the depth of closure. In reality, much of the sand placed in front of Reach 2 will move offshore to build the active profile, and it will take more sand than expected to build the beach. For those reasons, if a beach is to be built on Reach 2, an incremental approach with monitoring and adaptability is highly recommended.

Figure 63 and Figure 64 compare 250,000 yd³ placed every other year on Reach 1 versus Reaches 1 and 2. After 10 years, when the beach fill is only placed on Reach 1, the shoreline advances approximately 60 ft in that reach. The beach fill has not dispersed significantly by 10 years, and the shoreline change matches that of the no-action case just after 8 miles west of the jetty. The shoreline advances between 30 and 40 ft when the material is placed in Reaches 1 and 2. While the shoreline does not advance as much in Reach 1 as the case with placement only in Reach 1,

the advance in Reach 2 is very significant. After 50 years, the Reach 1 placement results in a shoreline advance in that reach of 300 ft. Reach 2, which begins approximately 7.3 miles west of the jetty, experiences slight shoreline advance from the no-action case up to approximately 9 miles west of the jetty. When the sand is placed on both reaches, the shoreline advances between 150 ft (in Reach 1) and 200 ft (in Reach 2). If sand and money were unlimited and the sand located just offshore of Reach 2 was similar to the profiles of Reach 1, the GPB would need to decide whether a 100 ft wider beach on Reach 1 were more beneficial than a slightly less wide beach that extended to the end of the seawall.

The next alternatives include placing 500,000 yd³, 1 million yd³, and 2 million yd³ on Reaches 1 and 2 every 5 years. Figure 65 compares the total shoreline change after 10 years. After 10 years, when 500,000 yd³ is placed over Reaches 1 and 2 every 5 years, the beach advances approximately 40 ft. Increasing the rate of beach fill placement to 1 million yd³ every 5 years results in an advance of 70 ft from the initial shoreline while a 2 million yd³ placement every 5 years produces a beach 120 ft wider than the initial. Figure 66 compares the results after 50 years. The beaches in Reaches 1 and 2 advance a total of 115, 275, and 600 ft from the initial shoreline for the 500,000 yd³, 1 million yd³, and 2 million yd³ placements every 5 years, respectively. The 2 million yd³ every-5-years beach fill alternative also results in an advanced shoreline more than 1.5 miles east of the 10th Street groin.

Figure 63. Total shoreline change after 10 years for 250,000 yd³ placed every other year on Reaches 1 and 2.

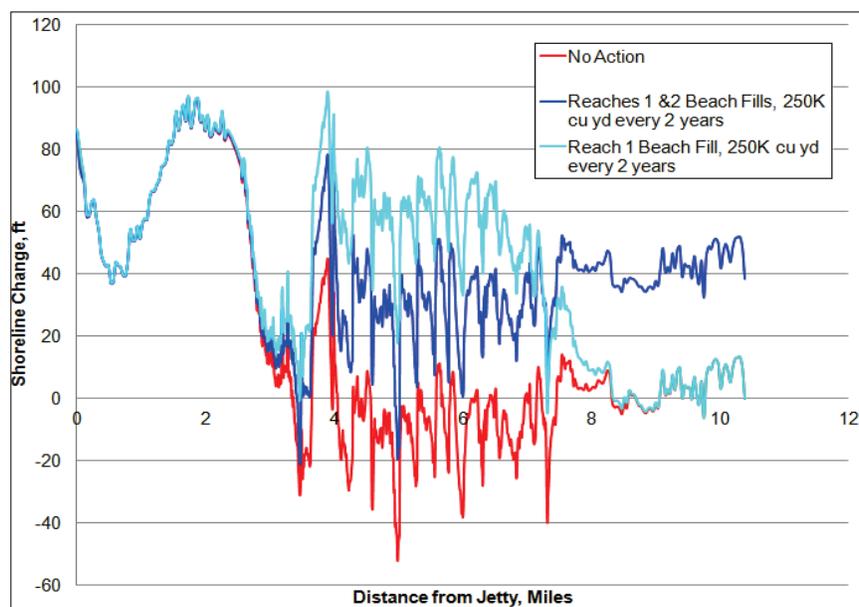


Figure 64. Total shoreline change after 50 years for 250,000 yd³ placed every other year on Reaches 1 and 2.

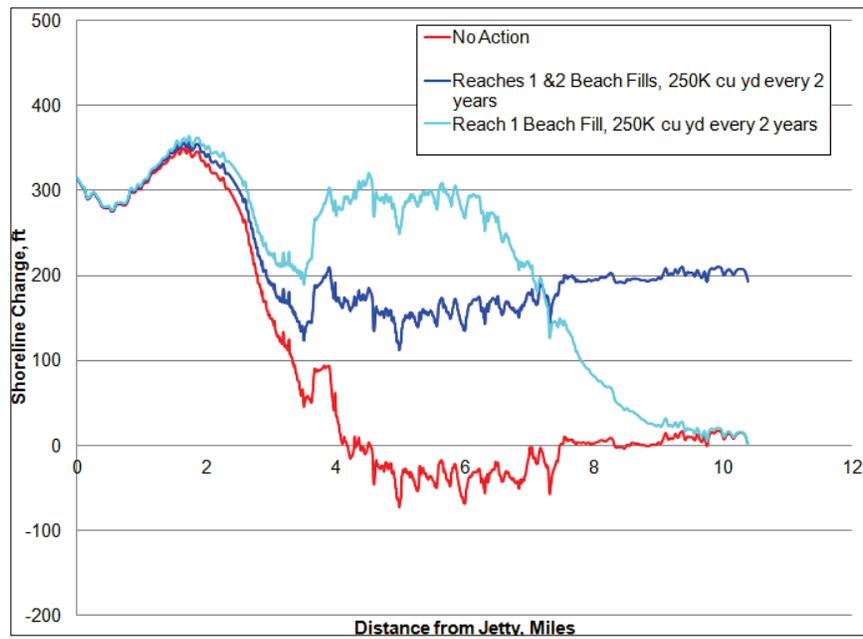


Figure 65. Total shoreline change after 10 years for 500,000 yd³, 1 million yd³, and 2 million yd³ placed on Reaches 1 and 2 every 5 years.

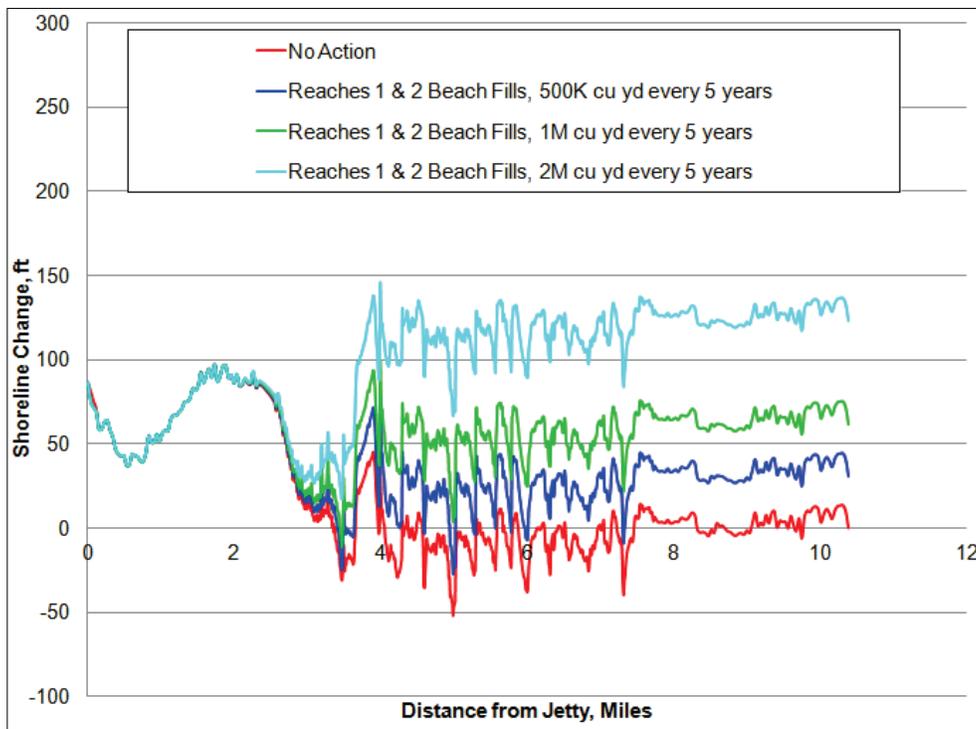
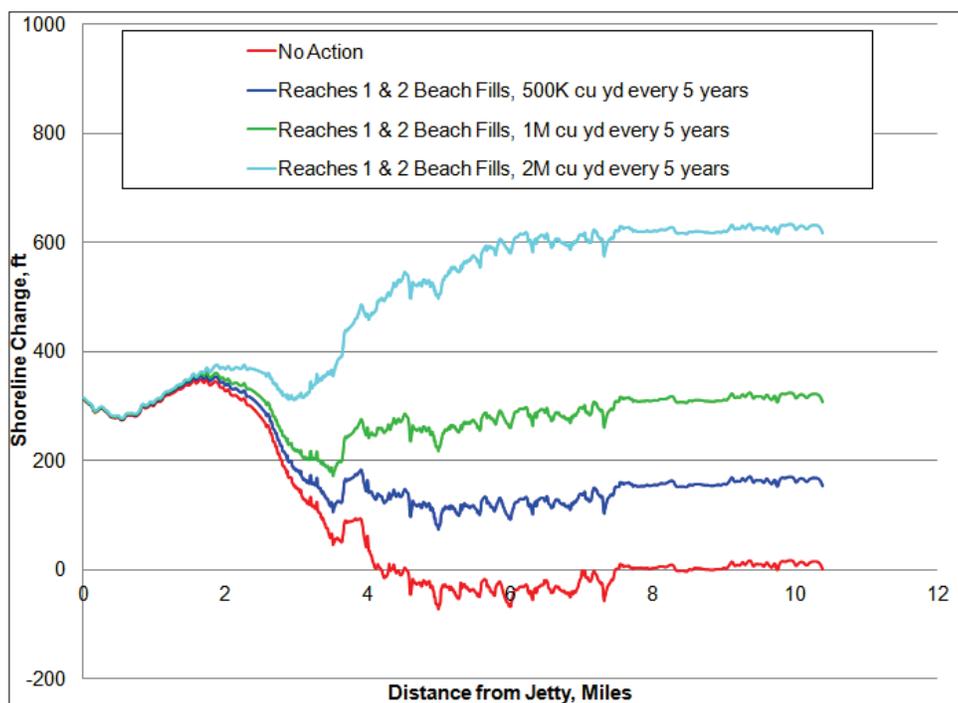


Figure 66. Total shoreline change after 50 years for 500,000 yd³, 1 million yd³, and 2 million yd³ placed on Reaches 1 and 2 every 5 years.



In addition to comparing beach fill volumes and renourishment intervals, it is also important to determine if the volume distributed on Reaches 1 and 2 makes a difference in the shoreline advance or retreat. In Figure 67 and Figure 68, all of the alternatives include a beach fill of 500,000 yd³ renourished every other year. The amount of sand placed per linear foot in Reaches 1 and 2 is identical in the first alternative. In the second alternative, two-thirds of the sand is placed on Reach 1 while three-fourths is placed on Reach 1 in the final alternative. These alternatives were simulated because the GPB rated Reach 1 as the highest priority. After 10 years, the shoreline advance for the three alternatives is similar. The alternative with 375,000 yd³ placed on Reach 1 every other year results in the widest beach in Reach 1 and the narrowest beach in Reach 2. The impact of the different distributions of volume after 50 years is more pronounced. When the fill is distributed evenly across the entire seawall, the shoreline advances 350 ft in Reach 1 and 390 ft in Reach 2. When 367,000 yd³ every other year is placed on Reach 1 and 133,000 yd³ every other year is placed on Reach 2, the beach width in Reach 1 increases by 425 ft while the beach width in Reach 2 increases 300 ft. Finally, the alternative with 375,000 yd³ and 125,000 yd³ every other year placed in Reaches 1 and 2 results in the shoreline of Reach 1 advancing 485 ft while Reach 2 only advances 225 ft. Over 50 years, the total volume placed on the beaches is 25 million yd³. This volume of

sediment placed on the beaches will advance the shoreline regardless of the distribution of placement location along the reaches. A comparison of shoreline change using smaller volumes and longer renourishment intervals results in a similar distribution of shoreline change.

Figure 67. Total shoreline change after 10 years for 500,000 yd³ placed every 2 years in different locations.

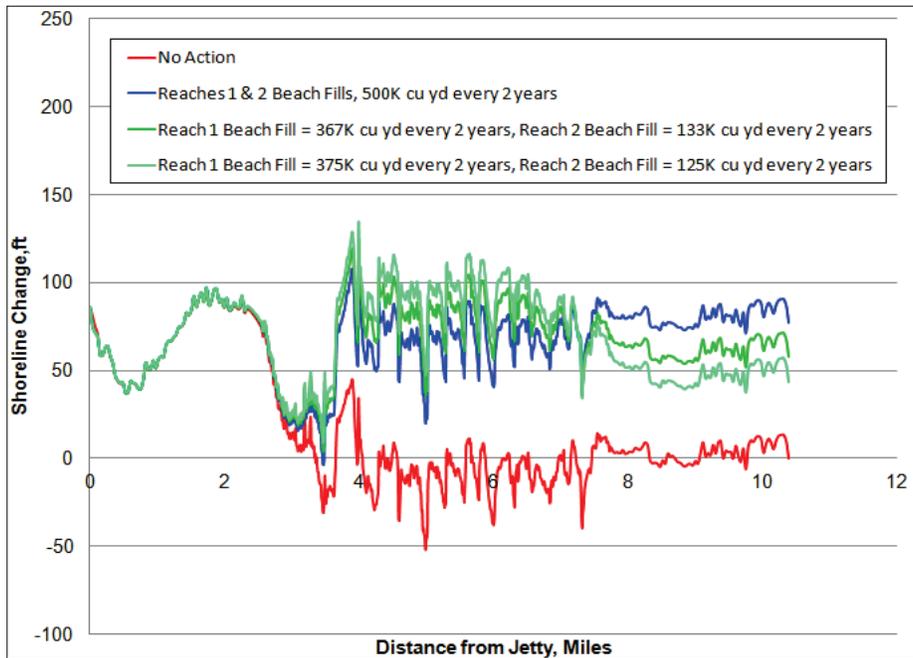
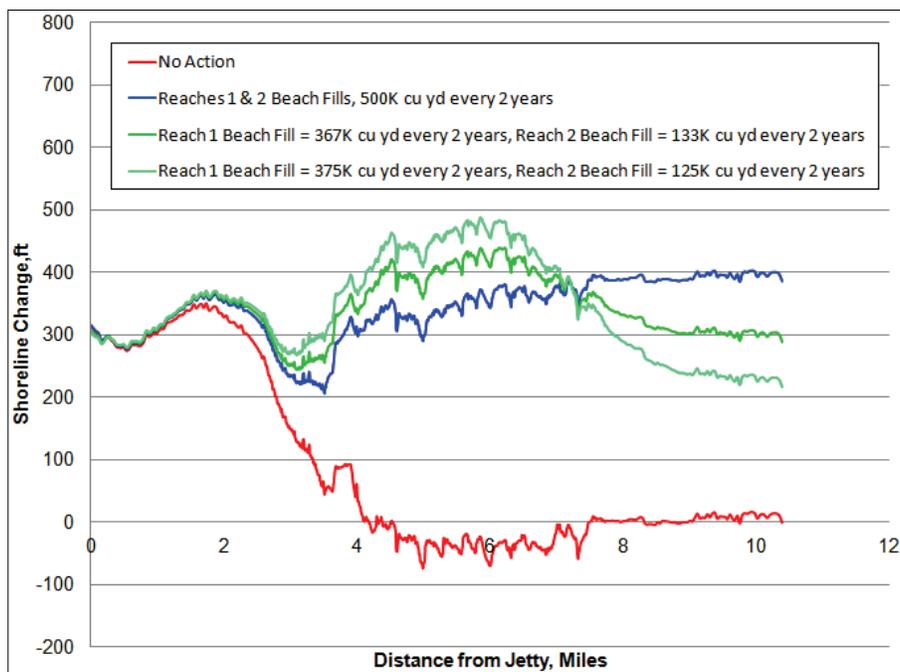


Figure 68. Total shoreline change after 50 years for 500,000 yd³ placed every 2 years in different locations.



8.3.4 Level 4 – Large-scale beach on Reach 1 and Reach 2

The final beach fill alternative for Reaches 1 and 2 is the large-scale beach fill described in Chapter 7. Initially, 1.942 million yd³ is placed on Reach 1 and 3.6542 million yd³ is placed on Reach 2. Two separate alternatives were modeled in GenCade. Since a large-scale beach fill will be extremely costly, an alternative where material is placed only along Reach 1 is compared to the full beach fill on Reaches 1 and 2. Figure 69 compares these two cases and the no-action alternative after 10 years. The beach fill cases are identical along the majority of Reach 1 where the maximum shoreline advance is approximately 110 ft. The large-scale beach fill advances the shoreline in Reach 2 by 250 ft.

Figure 70 compares the alternatives after 50 years. A percentage of the sand moves northeast towards the jetty so that the shoreline adjacent to the first groin has advanced more than the no-action case. Much of the shoreline advance seen after 10 years has eroded away along Reach 1, but the beach along Reach 2 is still very wide.

Figure 69. Total shoreline change after 10 years for the large-scale beach fills.

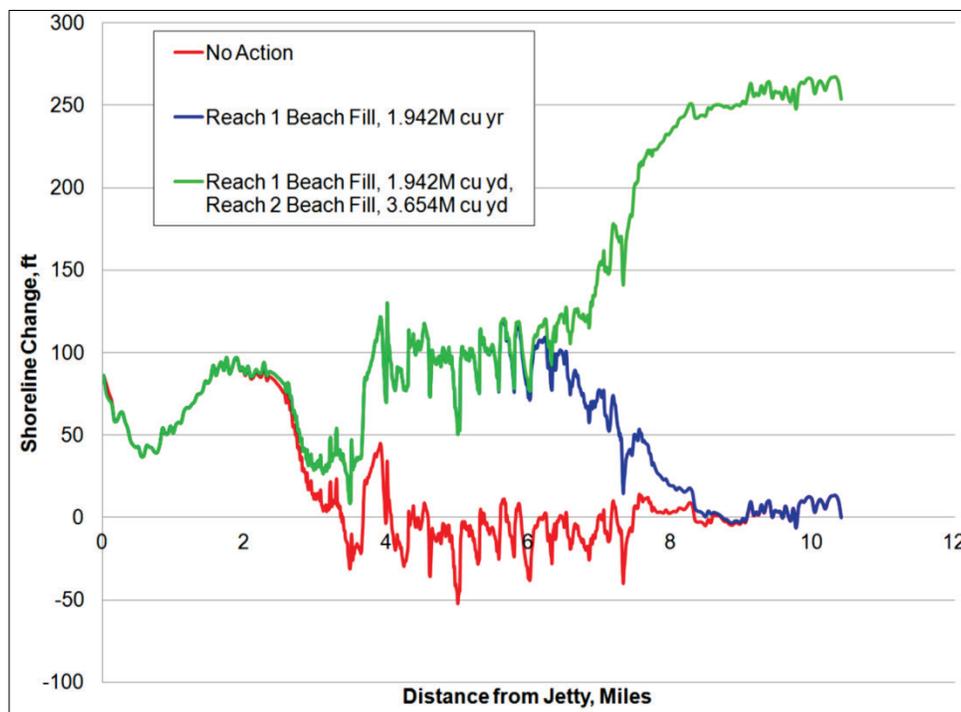
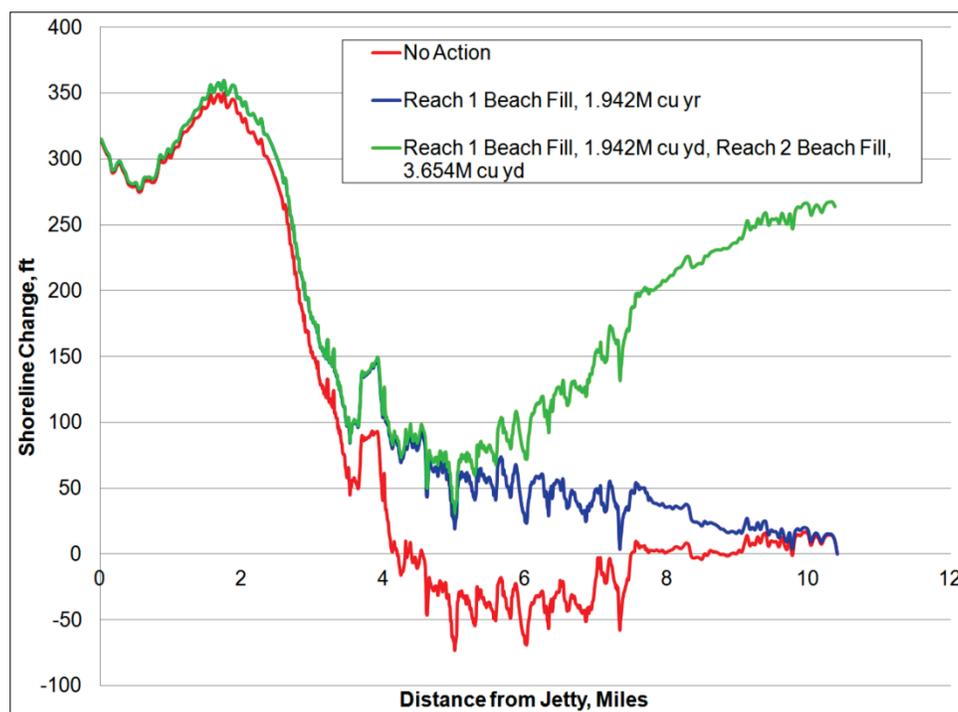
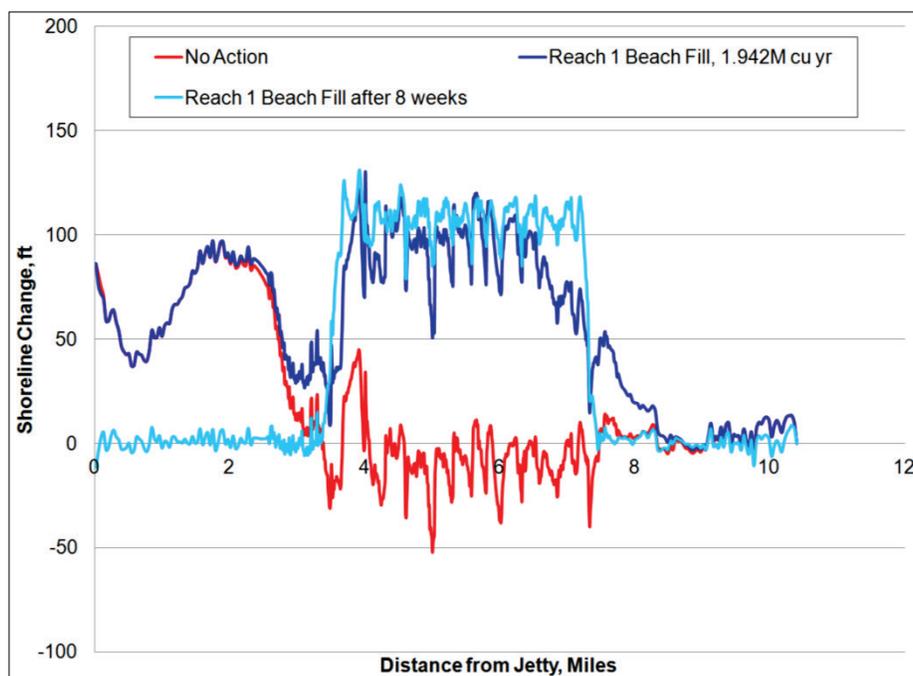


Figure 70. Total shoreline change after 50 years for the large-scale beach fills.



In addition to modeling placement alternatives, volume analysis determined the amount of material needed along each reach for different nourishment intervals in order to keep the same volume of material on the reaches. Each initial beach fill construction was completed after eight weeks within the simulation. A large-scale beach fill will take much longer to complete. When only the beach fill on Reach 1 was constructed, the reach loses a total of 113,000 yd³ after 2 years. Much of that sand is transported into Reach 2. After 5 years, a total of 195,000 yd³ is lost from Reach 1. Finally, after the 50-year simulation, the total loss of sand in Reach 1 is 741,000 yd³. If the beach is renourished every 2 years, an average of 31,000 yd³ would need to be placed each time. With a 5-year renourishment interval, the beach would need 82,000 yd³ each time. However, most of the losses occur early in the simulation. If 31,000 yd³ was placed on the beach after 2 years, it would not widen the beach enough to reach the width right after initial construction. Figure 71 compares the shoreline position after 10 years versus the position immediately after the beach fill construction.

Figure 71. Total shoreline change after beach fill construction in Reach 1 and after 10 years.

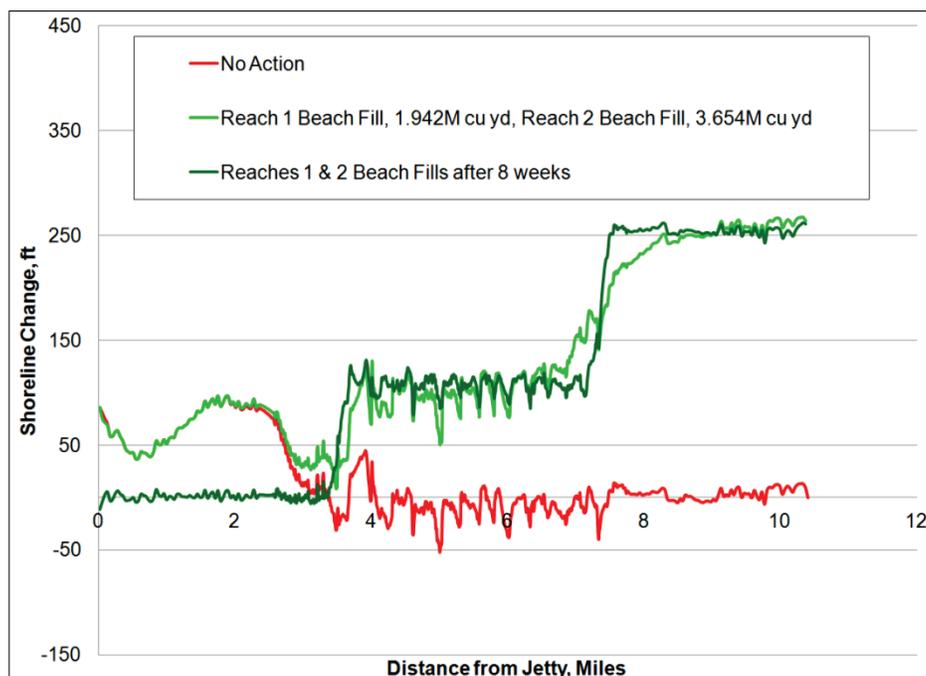


In the chapter on the large-scale beach fill, an advanced fill factor of 50% is assumed. If the initial beach fill is increased to 2.9133 million yd^3 , then the total volume added to Reach 1 after 50 years is 1.8744 million yd^3 . Therefore, if the beach is overfilled at the beginning, the beach would only lose approximately 68,000 yd^3 from the original beach fill volume without advanced fill, so the beach would be very similar to the shape of the beach after initial construction when no overfill was added.

The same volume analysis was conducted for the alternative with beach material placed along Reaches 1 and 2. When a beach fill is constructed along both Reaches 1 and 2, the total volume added to the beaches is 5.5964 million yd^3 . After 2 years, only 13,000 yd^3 is lost from Reach 1, and 41,000 yd^3 is lost from Reach 2. Reach 1 loses 16,000 yd^3 , and Reach 2 loses 73,000 yd^3 after 5 years while Reach 1 loses 96,000 yd^3 and Reach 2 loses 290,000 yd^3 after the 50-year simulation. There are two reasons that Reach 1 does not lose much sand compared to Reach 2. First, the beach is wider along Reach 2. This material begins to disperse and starts moving into Reach 1. Second, approximately 9 million yd^3 of sand moves onshore near the jetty (based on the assumption of 180,000 yd^3/year used in the model). While most of the sand accumulates near the jetty, some of it will come onshore farther to the west near Reach 1. Figure 72 shows the comparison of the shape of the beach fill immediately after construction

and after 10 years. The biggest change in the shoreline is near the transition of Reaches 1 and 2 where the shoreline at the end of Reach 1 advances and the shoreline at the beginning of Reach 2 retreats. The model does not predict erosion near the west end of the seawall because of a pinned boundary condition. In reality, a large percentage of this sand will move offshore or towards the west end of Galveston Island.

Figure 72. Total shoreline change after beach fill construction in Reaches 1 and 2 after 10 years.



An alternative with advanced fill placed in Reach 1 and Reach 2 was also modeled. The total volume of sand in this case increases to 8.3946 million yd^3 . At the end of the 50-year simulation, the volume of sand remaining in Reaches 1 and 2 is greater than the design volume without advanced fill. Since the beach fill is so large, only a small percentage of the sand volume moves off the grid towards the west end. Additionally, typical waves were used to drive the simulations. Sand will be transported out of the system if Galveston Island is impacted by large and numerous storms within the 50-year period. Sea level change is not accounted for in GenCade, so additional material will be lost (refer to 2.4.3.2).

8.4 Backpassing alternatives along seawall

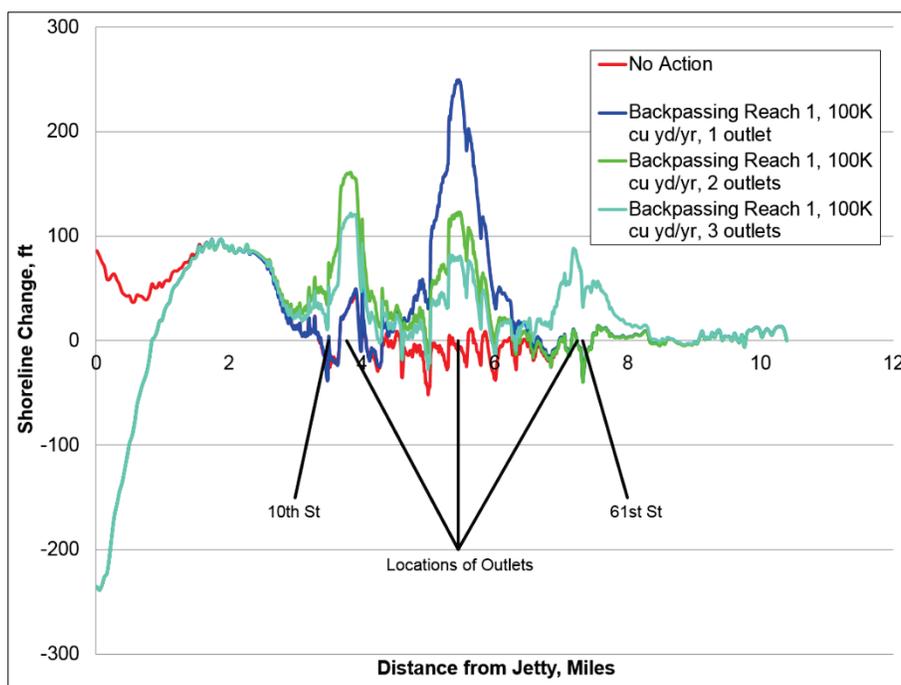
All backpassing alternatives involve removing sediment from the shoreline near the jetty and placing it on Reach 1. When a larger rate of sediment is

backpassed each year than is fed from offshore sources and longshore transport, there will be a loss of sand near the jetty. In some cases, GenCade predicts significant erosion near the jetty. Since it is unknown how much material comes from offshore sources each year, a study must be conducted before moving forward with construction on a backpassing plant. It is recommended to start backpassing yearly sediment volumes on the order of 30,000 yd³, increasing annually to 100,000 yd³ over 5 years to ensure the system is working properly and to not exceed the shoaling rate near the jetty, which may induce erosion near or in the vicinity of the jetty.

8.4.1 Level 1 – Backpassing in front of groin field

The first level of backpassing occurs near the jetty to Reach 1. Alternatives compare a permanently installed dredge pipeline with a single point of discharge of backpassed sediment along the seawall to alternatives with a permanently installed dredge pipeline with multiple discharge points that allow adjusting sediment placement locations based on sediment needs. Most likely the system will be designed with multiple discharges along the beach, but it is important to illustrate how the shoreline will respond to a single point versus multiple points. In Figure 73, total shoreline change is shown for a no-action case as well as 100,000 yd³/year of backpassing with one, two, and three discharge points along Reach 1. If a single point is placed at a central location on Reach 1, the maximum shoreline advance is approximately 250 ft; however, the effect is localized. A mile away from the discharge, the shoreline advance is almost identical to the no-action alternative. If there are two discharges, the maximum shoreline advance decreases to approximately 150 ft. There are two locations along the seawall that experience significant shoreline advance, but the shoreline recedes to near no-action conditions between the discharges. Finally, with three discharge points along the seawall, there are three distinct locations of shoreline advance of just under 100 ft. In between each discharge, the shoreline does not advance as far, but the shoreline does not recede to the no-action conditions. Although 100,000 yd³/year is backpassed, 225 ft of erosion occur near the jetty. In these cases, a source term of 180,000 yd³/year is used. If sand moves onshore at a greater rate, the erosion near the jetty will be less.

Figure 73. Total shoreline change after 10 years for 100,000 yd³ of backpassing.



After 50 years, 5 million yd³ of material will be backpassed from near the jetty. Regardless of the number of discharges, the beach will be much wider than the no-action case (Figure 74). If a single discharge is used, the beach will be more than 500 ft wide at the point but only 100 ft wide near the 10th Street groin and near 61st Street. With two discharge points, the maximum shoreline advance is approximately 350 ft. The shoreline between the points advances 200 ft while the backpassing has a limited effect more than 7 miles from the jetty. With three discharge points, the average shoreline advance along Reach 1 is approximately 200 ft. This is the only alternative where some of the sand has moved to Reach 2 and created dry beach berm. When examining three discharge points, the maximum erosion near the jetty is similar to the erosion after 10 years with no backpassing, but the impacts of backpassing are felt farther from the jetty.

Figure 74. Total shoreline change after 50 years for 100,000 yd³ of backpassing.

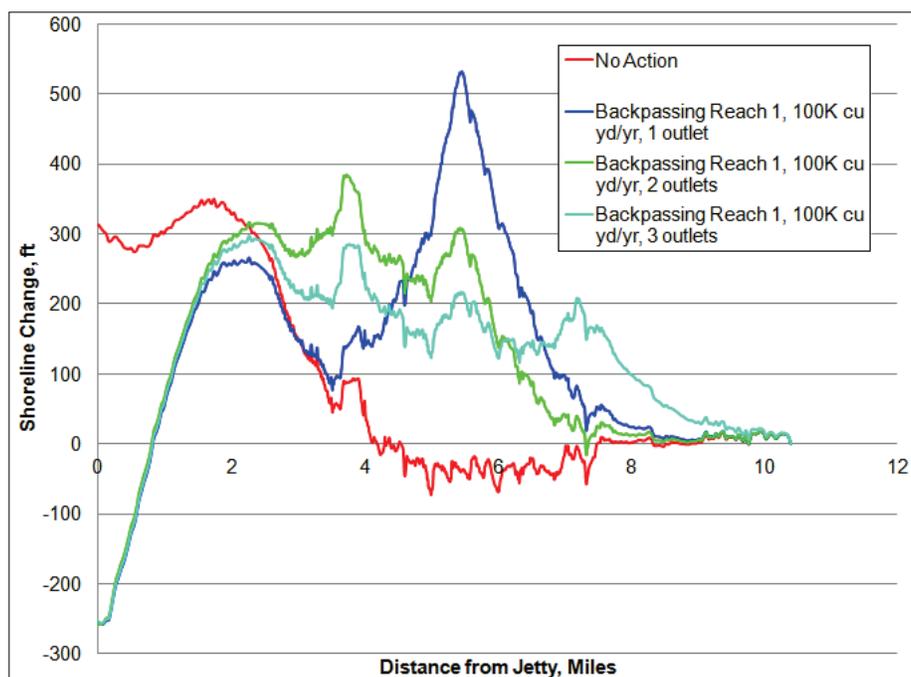


Figure 75 to Figure 80 compare different sediment input source terms and bypassing rates with three discharge points along Reach 1. Multiple discharge points are recommended to ensure a more uniform beach along the length of the project. Figure 75 compares two cases with 100,000 yd³/year backpassed onto Reach 1 with three discharge points after 10 years. One case includes a source term near the jetty of 180,000 yd³/year while the other has a source term of 356,000 yd³/year. These figures are included in this report to reiterate how much of an impact the rate of sand moving towards the jetty has on the shoreline east of the 10th Street groins and on the amount of material that can safely be backpassed to Reach 1. The no-action case in the figure includes a source term of 180,000 yd³/year. The effects of the source term are felt up to the 10th Street groin. While both source terms result in erosion near the jetty, the source term of 356,000 yd³/year advances the shoreline up to 0.5 mile east of the jetty. After 50 years (Figure 76), the source term impacts the shoreline up to 5 miles west of the jetty. If a 356,000 yd³/year source term is added to the beach, the shoreline advances up to 800 ft in some areas east of the 10th Street groin, and the erosion near the jetty is only approximately 100 ft. The 180,000 yd³/year source term results in erosion of more than 200 ft near the jetty and up to 100 ft of advance along East Beach.

Figure 75. Total shoreline change after 10 years for 100,000 yd³ of backpassing with different source terms.

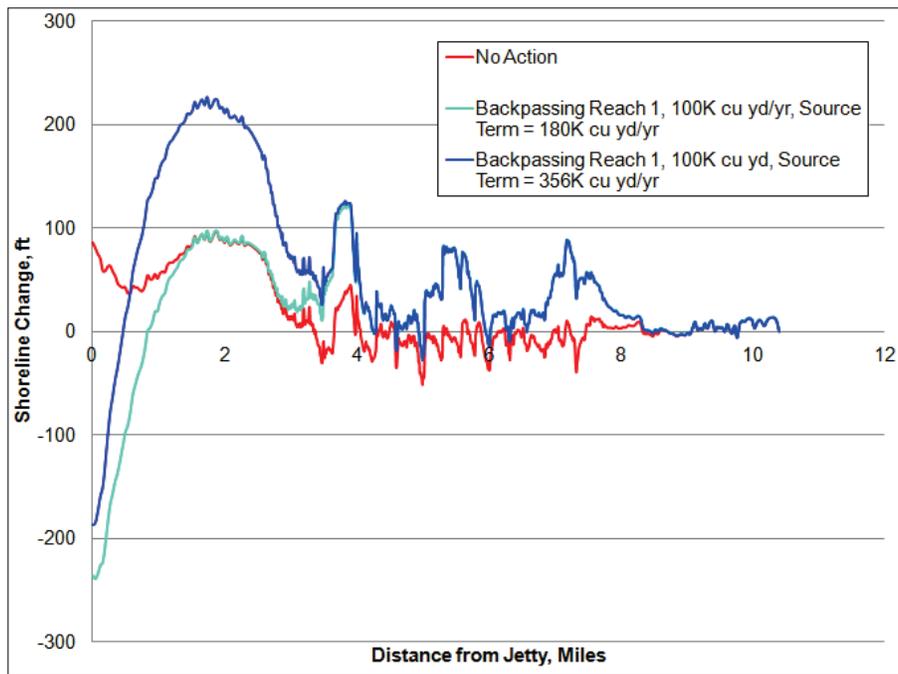


Figure 76. Total shoreline change after 50 years for 100,000 yd³ of backpassing with different source terms.

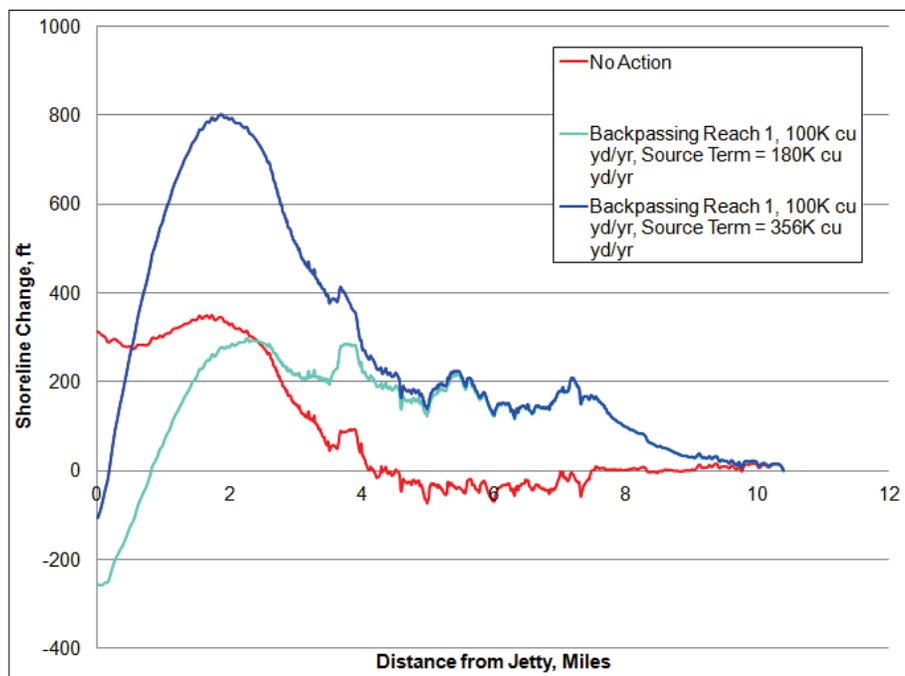


Figure 77. Total shoreline change after 10 years for 100,000, 250,000, and 356,000 yd³ of backpassing with different source terms.

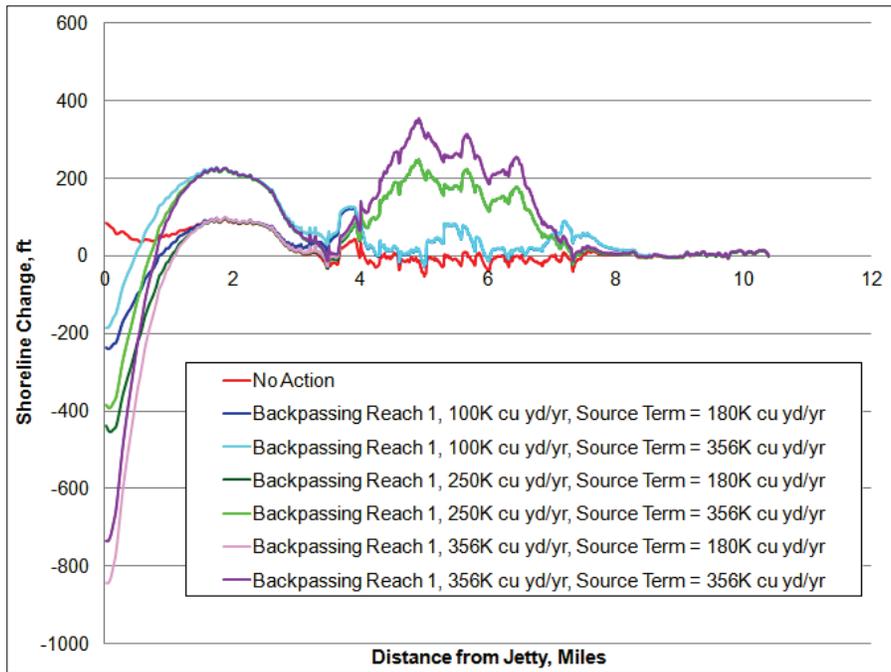


Figure 78. Total shoreline change after 50 years for 100,000, 250,000, and 356,000 yd³ of backpassing with different source terms.

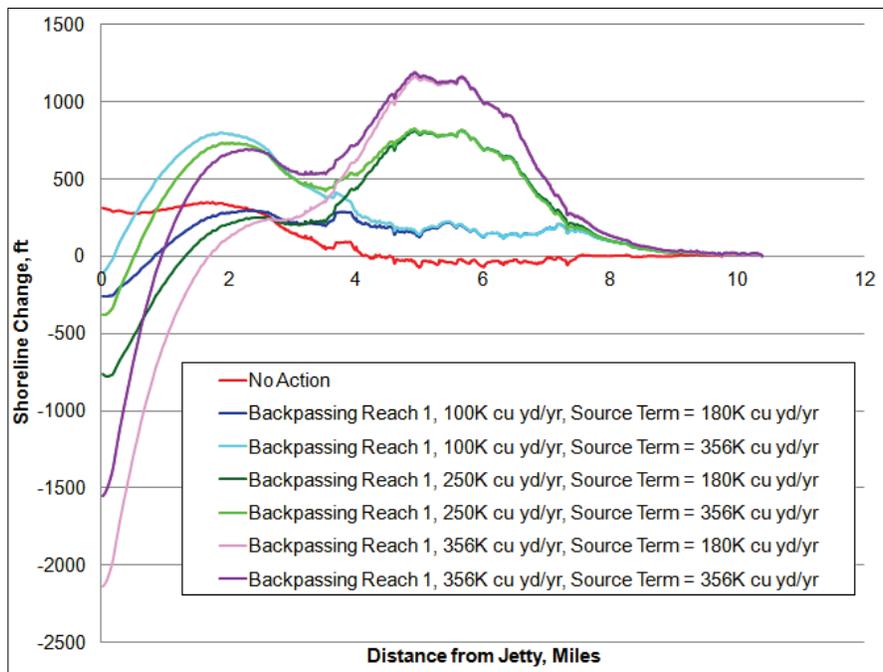


Figure 79. Total shoreline change after 10 years for 100,000 yd³ of backpassing with and without an initial beach fill.

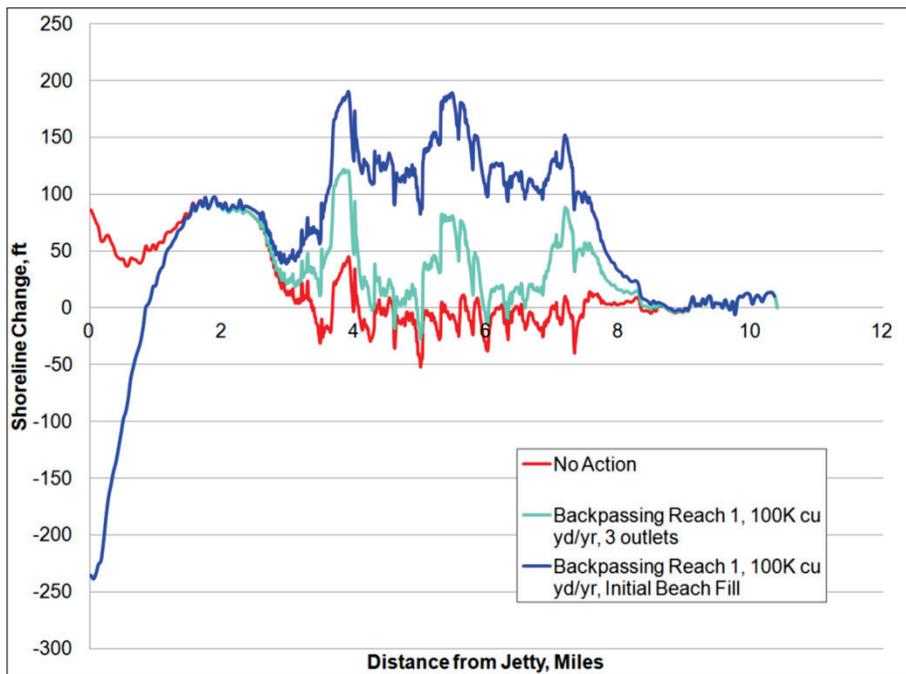


Figure 80. Total shoreline change after 50 years for 100,000 yd³ of backpassing with and without an initial beach fill.

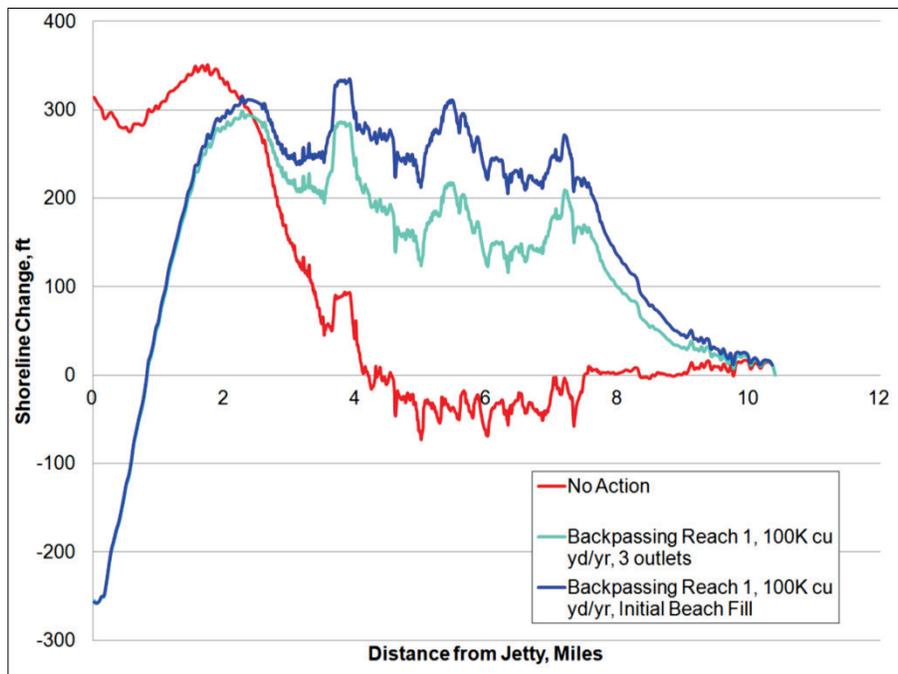


Figure 77 and Figure 78 compare total shoreline change for backpassing rates of 100,000, 250,000, and 356,000 yd³/year with different source terms for 10 and 50 years. The 356,000 yd³/year backpassing results in up

to 300 ft of advance in Reach 1. However, even when 356,000 yd³/year of sand comes from offshore, there is almost 800 ft of erosion along the jetty. Presently a fillet has formed near the jetty, and even though the shoreline will recede significantly from the present day shoreline, it will not erode enough to undermine the jetty. The 250,000 yd³/year backpassing rate advances the beach approximately 200 ft in Reach 1. The maximum erosion near the jetty for both the 180,000 and 356,000 yd³/year source term is approximately 400 ft.

After 50 years (Figure 78), the beach on Reach 1 is larger than 10 years, but the erosion near the jetty and East Beach has increased. When 356,000 yd³/year is backpassed onto Reach 1, the maximum shoreline advance from the initial shoreline is approximately 1200 ft. This option provides a very wide beach along Reach 1 and into part of Reach 2. Unfortunately, over 2000 ft of erosion occurs near the jetty when 180,000 yd³/year comes from offshore. This is unsustainable and will undermine the jetty. Some of the material is depositing a couple of miles from the jetty and has yet to be transported by longshore drift to the jetty. For that reason, other parts of the East Beach experience advance. The 250,000 yd³/year backpassing option results in a 750 ft wide beach while the 100,000 yd³/year backpassing option provides a 200 ft wide beach. The GPB only prefers a 200 ft wide beach, so the 250,000 and 356,000 yd³/year backpassing options are probably not necessary. However, the best option would be to fill the beach first and then begin backpassing since it will take a long period of time to build the beach to the required width with only backpassing.

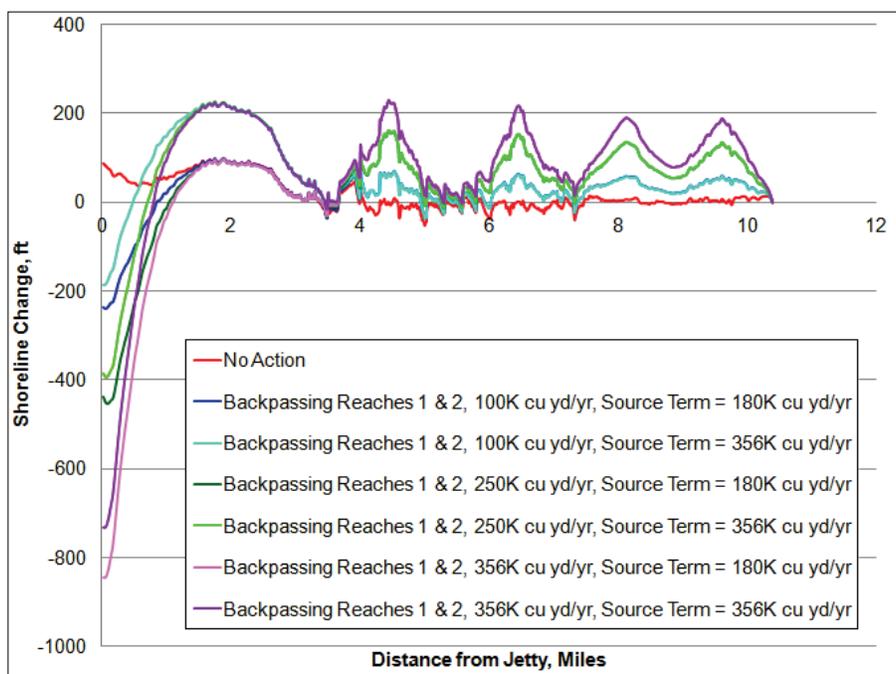
A final option is to construct the large-scale initial beach fill of 1.9422 million yd³ with backpassing. The 100,000 yd³/year backpassing rate was combined with the large-scale beach fill to show the impact the initial beach fill has on the width of the beach. Figure 79 compares the 100,000 yd³/year of backpassing with three discharge points with and without the beach fill after 10 years. Figure 80 compares the two alternatives after 50 years. After 10 years, the average beach width along Reach 1 increases from 40 ft to 130 ft. The average beach width increases from 180 ft to 260 ft after 50 years when a large-scale beach fill was placed initially.

8.4.2 Level 2 – Backpassing in front of seawall

Level 2 consists of various rates of material being backpassed on Reaches 1 and 2. In all of the alternatives, there are two discharges on Reach 1 and two discharges along Reach 2. If it is determined that backpassing is the suitable option, the locations of the discharge points can be adjusted.

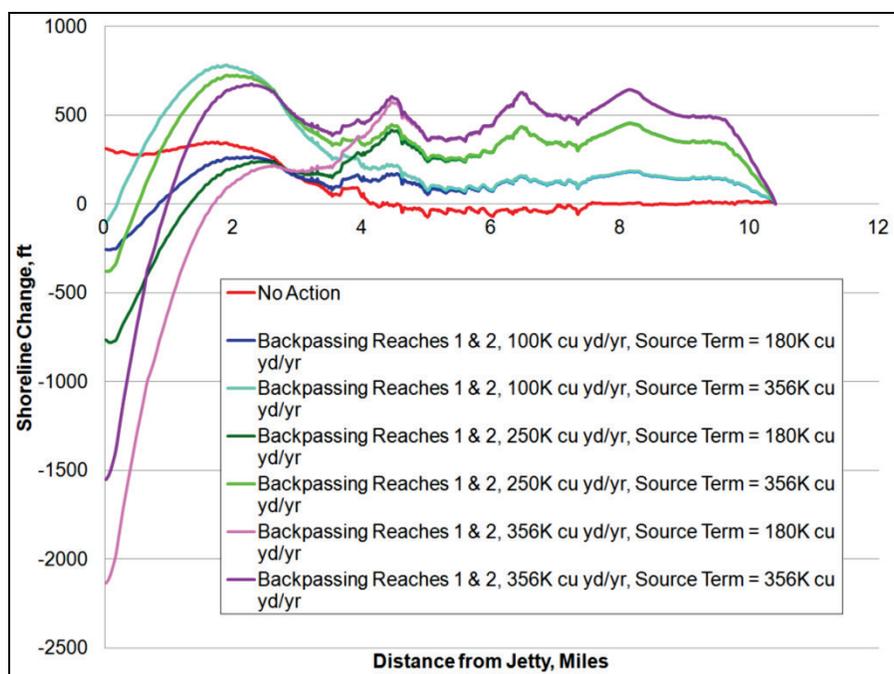
Figure 81 shows backpassing at rates of 100,000, 250,000, and 356,000 yd³/year onto Reaches 1 and 2 from near the jetty after 10 years. Source terms of 180,000 and 356,000 yd³/year are compared. The backpassing rate of 356,000 yd³/year provides a maximum advance of 200 ft along the reaches. At 100,000 yd³/year backpassing rate, the beach is only approximately 50 ft wide. With larger volumes of sand being backpassed at each discharge point, the beach width will not be uniform. When 356,000 yd³/year is backpassed, the beach between two discharge points only advances 50 ft in some areas. It is recommended to adjust the location of the point along the beach to make the beach shape more uniform. Similar to the previous alternatives, it is important to keep in mind how much backpassing affects the shoreline near the jetty. When 180,000 yd³/year moves onshore and 356,000 yd³/year is backpassed, the shore retreats more than 800 ft near the jetty, and retreat occurs compared to the initial shoreline for the first mile.

Figure 81. Total shoreline change after 10 years for 100,000, 250,000, and 356,000 yd³ of backpassing on Reaches 1 and 2 with different source terms.



The total shoreline change after 50 years for the same scenarios is shown in Figure 82. After 50 years of backpassing 356,000 yd³/year, the shoreline advances between 400 and 600 ft. The sand has dispersed more than after 10 years, so the shoreline between the discharge points experiences similar rates of advance. With 250,000 yd³/year of backpassing, the shoreline is now approximately 350 ft wider than the initial shoreline while the 100,000 yd³/year backpassing rate results in approximately 125 ft of advance. However, like the other alternatives, the material must come from near the jetty, which results in extreme erosion. When only 180,000 yd³/year comes onshore and 356,000 yd³/year is backpassed to Reaches 1 and 2, the shoreline erodes more than 2000 ft. Erosion occurs up to 1.6 miles away from the jetty. Although the 100,000 yd³/year backpassing rate does not provide a 200 ft wide beach, it is the most reasonable option since it has the least impact along the jetty. If a beach fill is placed before backpassing begins, a lower backpassing rate will be necessary to maintain the beach. Again, before any construction is started, comprehensive studies and engineering design must be conducted.

Figure 82. Total shoreline change after 50 years for 100,000, 250,000, and 356,000 yd³ of backpassing on Reaches 1 and 2 with different source terms.



In order to illustrate how constructing an initial large-scale beach fill helps the backpassing process, the large-scale initial beach fill was added to the 100,000 yd³/year backpassing alternative along Reaches 1 and 2. The results after 10 and 50 years are shown in Figure 83 and Figure 84. After 10 years, the alternative with the initial beach fill and backpassing produces a 140 ft wide beach along Reach 1 and a 280 ft wide beach along Reach 2. Without the initial beach fill, the beaches along Reaches 1 and 2 are only 25 ft and 50 ft wide, respectively. After 50 years, the average width of the beach along Reach 1 increases from 120 ft without the beach fill to 240 ft with the beach fill. Along Reach 2, constructing an initial beach fill increases the maximum beach width from 180 ft to 400 ft.

Figure 83. Total shoreline change after 10 years for 100,000 yd³ of backpassing along Reaches 1 and 2 with and without an initial beach fill.

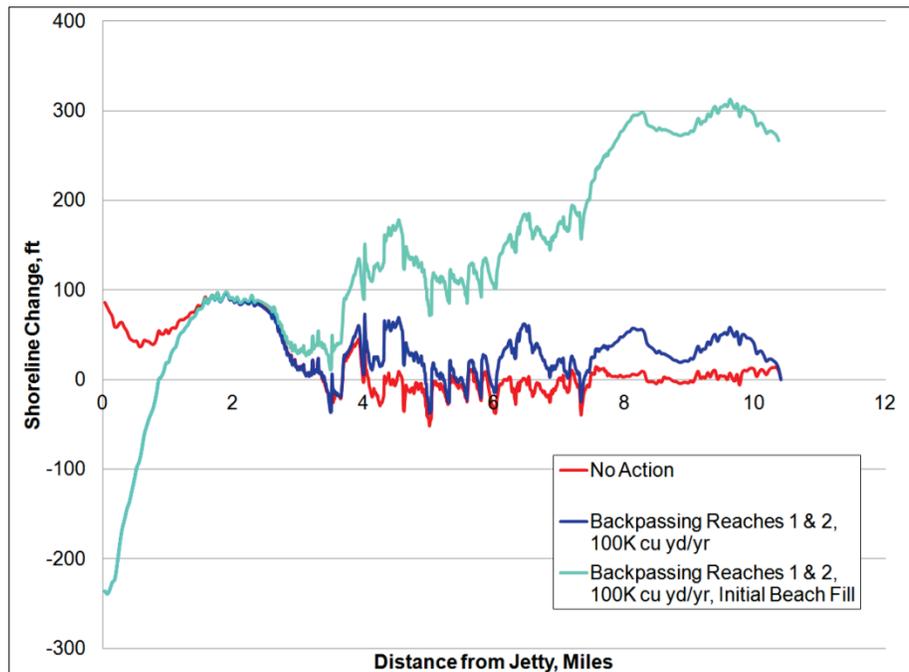
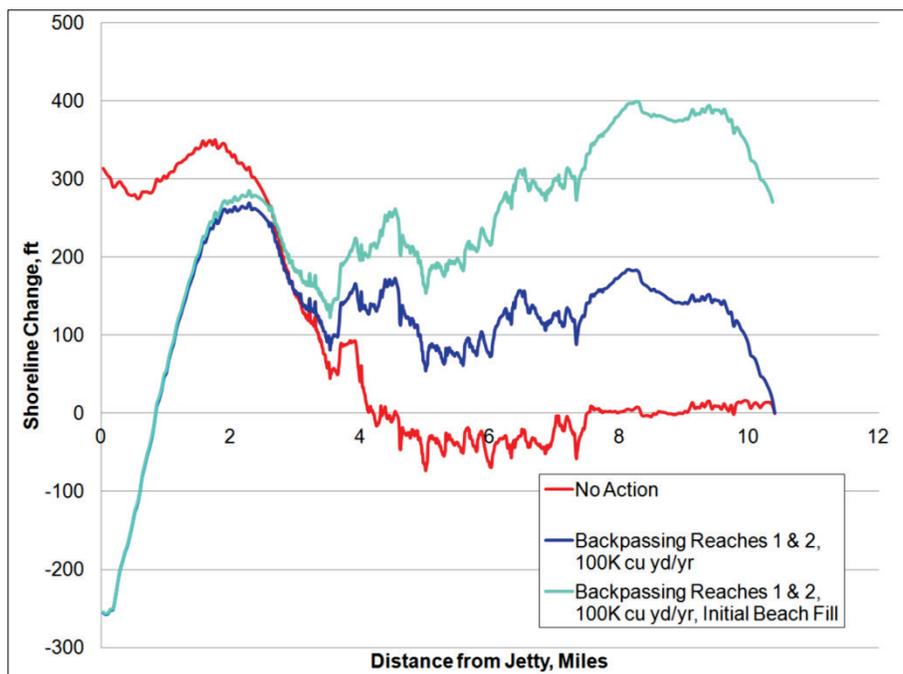


Figure 84. Total shoreline change after 50 years for 100,000 yd³ of backpassing along Reaches 1 and 2 with and without an initial beach fill.



8.5 Beach fills along west end

The GenCade alternatives focus on several levels of beach fills along Reaches 3 and 4. Level 1 consists of small beach fills along GPB property including Dellanera RV Park (0.27 mile) and Pocket Parks 1, 2, and 3 (0.23, 0.16, and 0.19 mile, respectively). Level 2 is represented by a beach fill along the first 1.5 miles to the west of the west end of the seawall. This short fill attempts to counter the end effects of the seawall. A beach fill covering all of Reach 3 refers to Level 3 while Level 4 is a beach fill in front of Reaches 3 and 4. Reach 5 is accreting, and no beach fill was considered for placement there.

8.5.1 Level 1 – Small beach fills along Galveston Park Board (GPB) property

The first alternatives involved adding beach fills at Dellanera RV Park, Pocket Park 1, Pocket Park 2, and Pocket Park 3. The rationale for this level of nourishment is the present restriction on placing material on private property. Therefore, these may be the only locations where nourishment can be placed within the foreseeable future.

In Figure 85 and Figure 86, 5,000 yd³ of material is placed on each of the four GPB properties. The renourishment interval ranges from 1 to 10 years. Small beach fills were modeled here because Reach 3 is a lower priority area. Also, each property only extends a short distance alongshore, so a large beach fill would be impractical. After 10 years, these small beach fills have little effect on the parks and Reach 3. While they do provide additional sand compared to the no-action case, the shoreline is still receding more than 50 ft. However, this rate of retreat is based on historical shorelines, so it is possible the high rate of erosion may decrease or stabilize in the future especially with beach nourishment placed along the seawall.

After 50 years, the erosion along Reach 3 is more noticeable. The no-action case results in 250 ft of erosion. Although the shoreline advances 30 ft from the no-action case, the shoreline still erodes 220 ft when 5,000 yd³ are placed at each location each year. A total of 1 million yd³ of sediment over 50 years would be placed if this alternative is selected. After 50 years, the sand disperses from each of the three small fill locations so that the entire shoreline receives some benefit. This beach fill option will not provide adequate protection along Reach 3.

Figure 85. Total shoreline change after 10 years for 5,000 yd³ beach fills on GPB property.

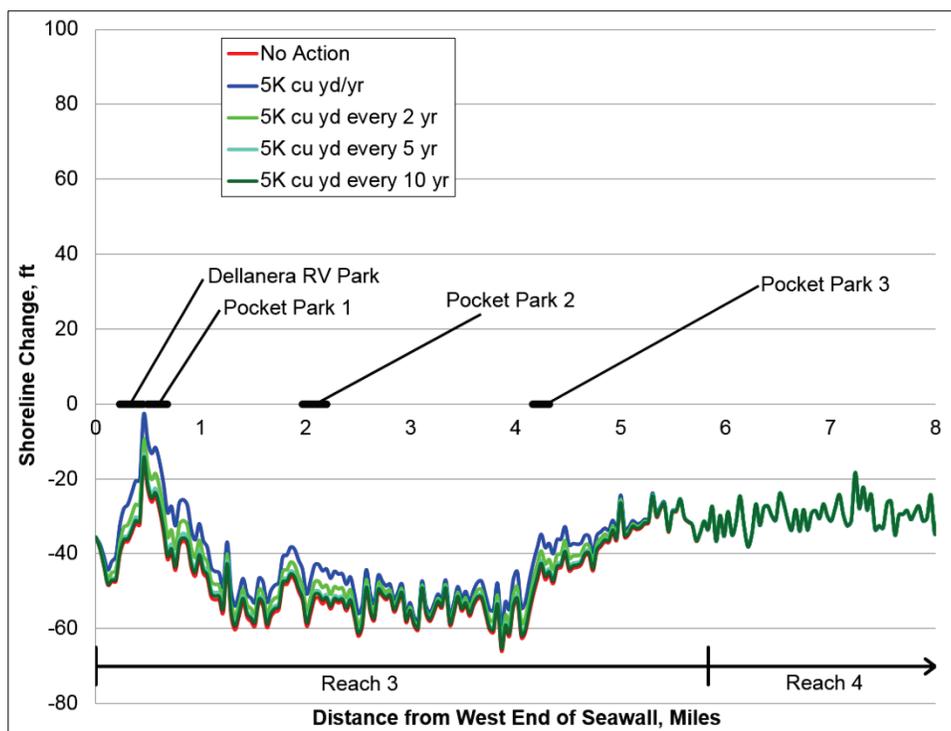
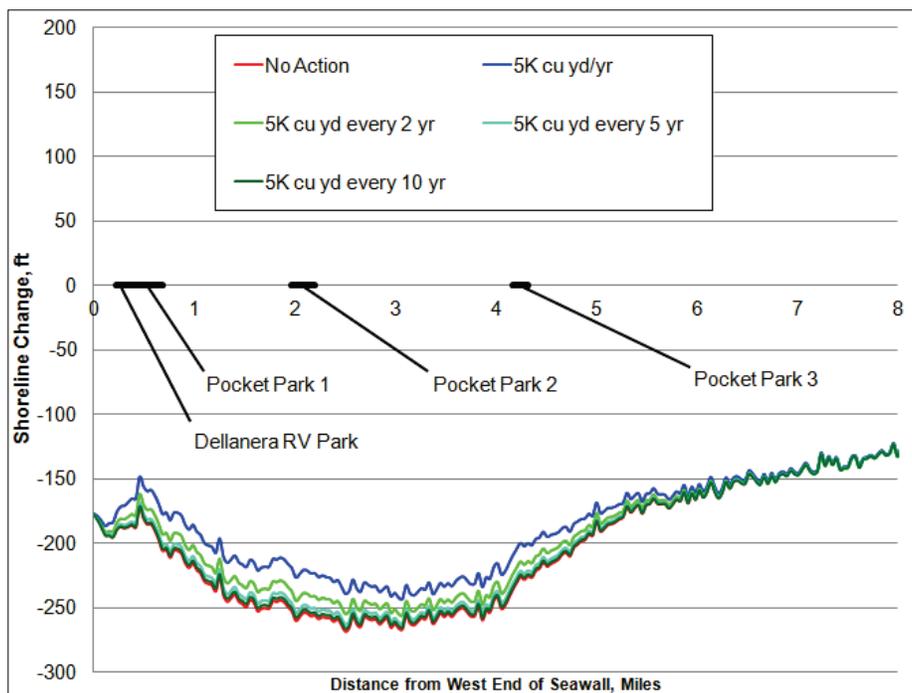


Figure 86. Total shoreline change after 50 years for 5,000 yd³ beach fills on GPB property.



Another option is to add 20,000 yd³ of sand to each GPB property with renourishment cycles every year, 2 years, 5 years, and 10 years. Figure 87 shows each 20,000 yd³ interval option after 10 years. When 20,000 yd³ is placed on each property each year, the shoreline advances near Dellanera RV Park and Pocket Park 1. Erosion occurs at all the other locations along the Reach 3 shoreline. When the renourishment interval is longer than 10 years, the entire shoreline along Reach 3 experiences erosion.

The total shoreline change for each 20,000 yd³ option after 50 years is shown in Figure 88. When 20,000 yd³ is placed on each property every year, the shoreline along Reach 3 recedes 125 ft on average. However, this option reduces erosion compared to the no-action case by 100 ft. If the proposed fill areas in Reach 3 are renourished every 5 years, erosion is reduced by approximately 20 ft compared to the erosion in the no-action case. Although 125 ft could be interpreted as severe shoreline retreat, it occurs over 50 years. The rate of erosion is equal to 2.5 ft/year, much less than historical rates of up to 10 ft/year (Figure 2).

Figure 87. Total shoreline change after 10 years for 20,000 yd³ beach fills on GPB property.

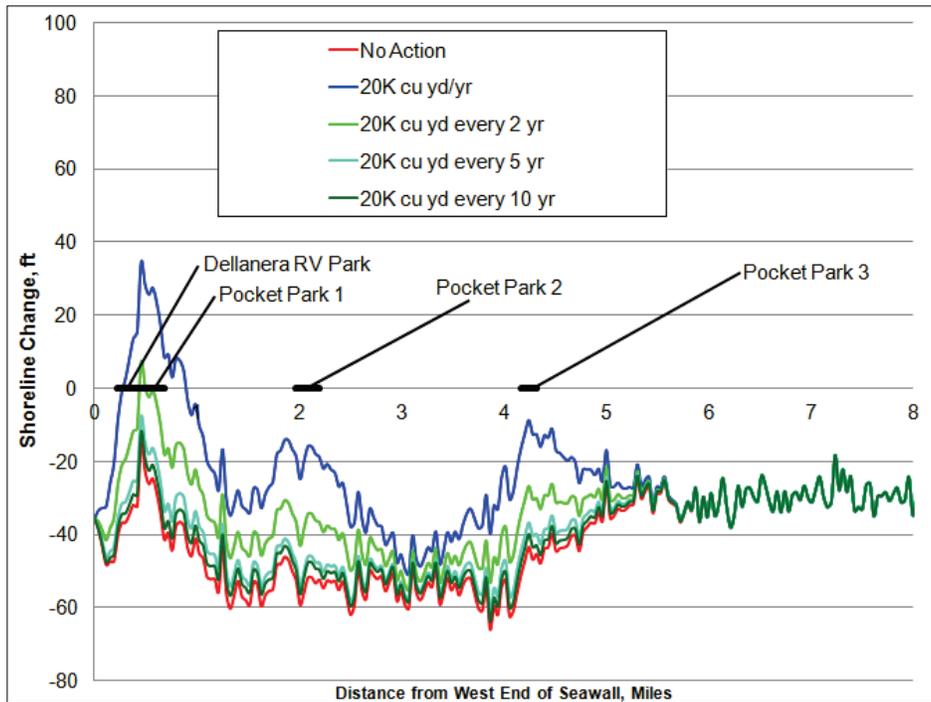


Figure 88. Total shoreline change after 50 years for 20,000 yd³ beach fills on GPB property.

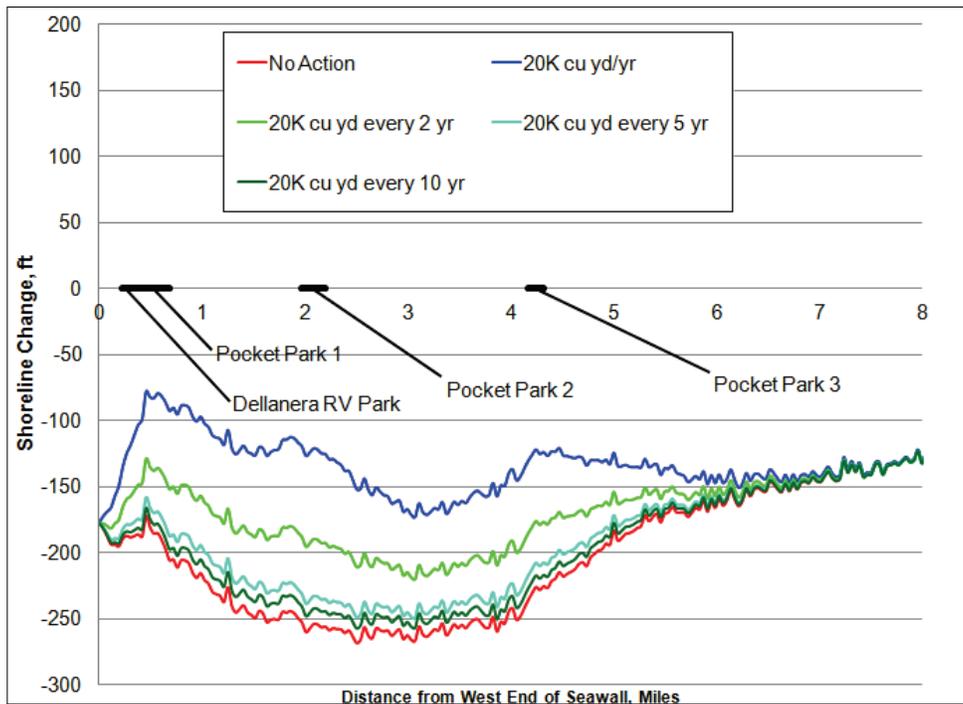


Figure 89 and Figure 90 compare 5,000, 10,000, and 20,000 yd³ placed on each of the three Galveston Board properties in Reach 3 every 2 years after 10 and 50 years. While each of these beach fills and renourishments advance the shoreline from the no-action case, they do little to advance the calculated shoreline from the initial shoreline. Figure 91 and Figure 92 compare the 5,000, 10,000, and 20,000 yd³ placements on each property every 5 years after 10 and 50 years. These less frequent nourishments have even less of an impact on the shoreline compared to the no-action case.

While these alternatives could be beneficial in the short term for each property, they will not provide a long-term benefit along Reach 3. Therefore, they should not be considered part of a long-term strategy to build a beach along all of Galveston Island but could be considered a short-term option if a fill is needed in an emergency condition.

Figure 89. Total shoreline change after 10 years for 5,000, 10,000, and 20,000 yd³ beach fills nourished every 2 years on GPB property.

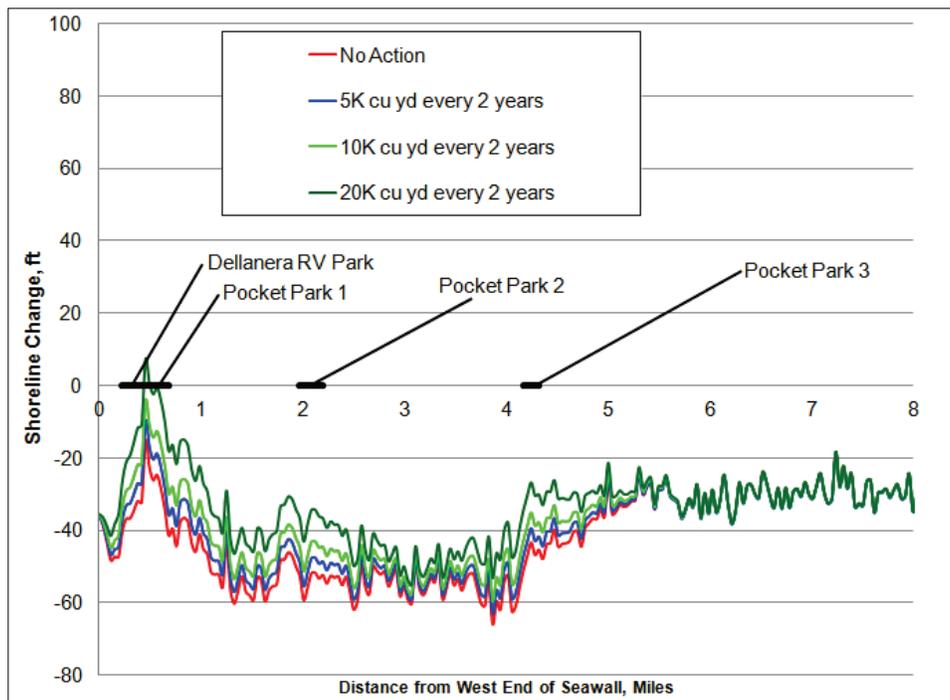


Figure 90. Total shoreline change after 50 years for 5,000, 10,000, and 20,000 yd³ beach fills nourished every 2 years on GPB property.

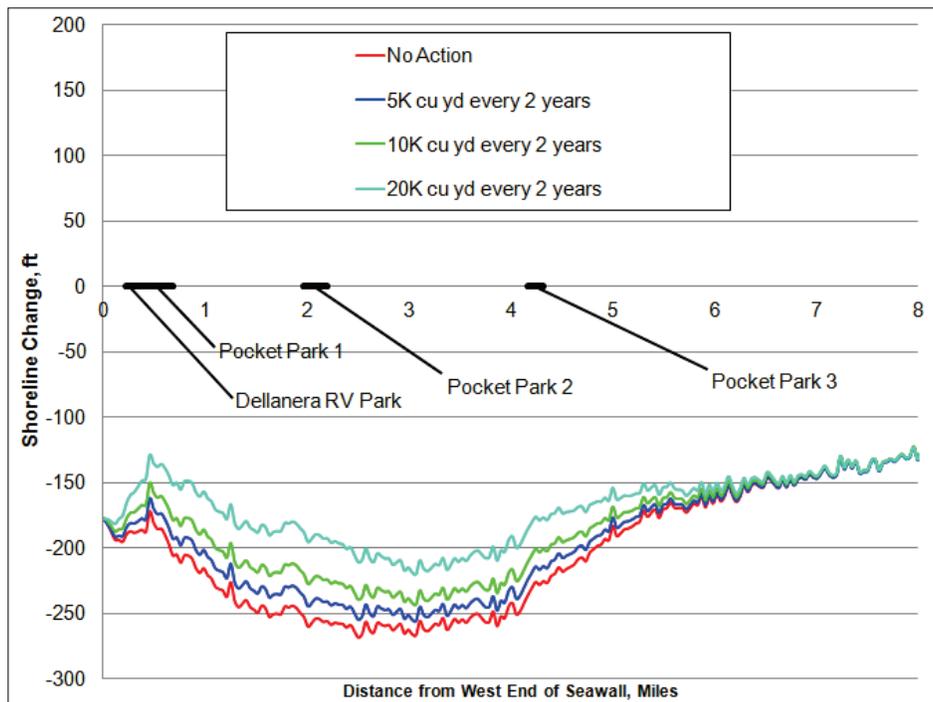


Figure 91. Total shoreline change after 10 years for 5,000, 10,000, and 20,000 yd³ beach fills nourished every 5 years on GPB property.

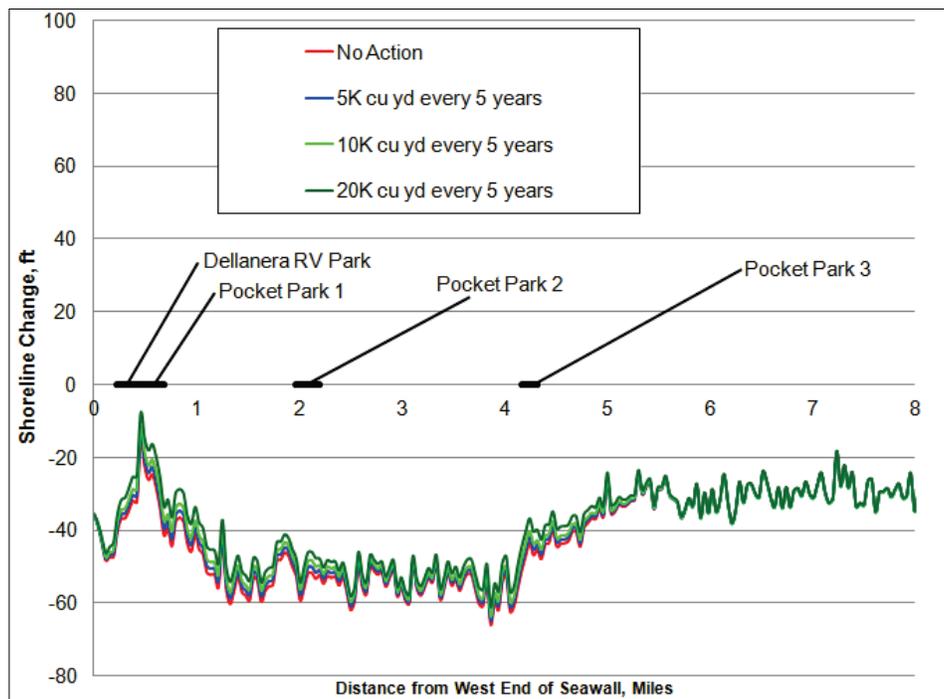
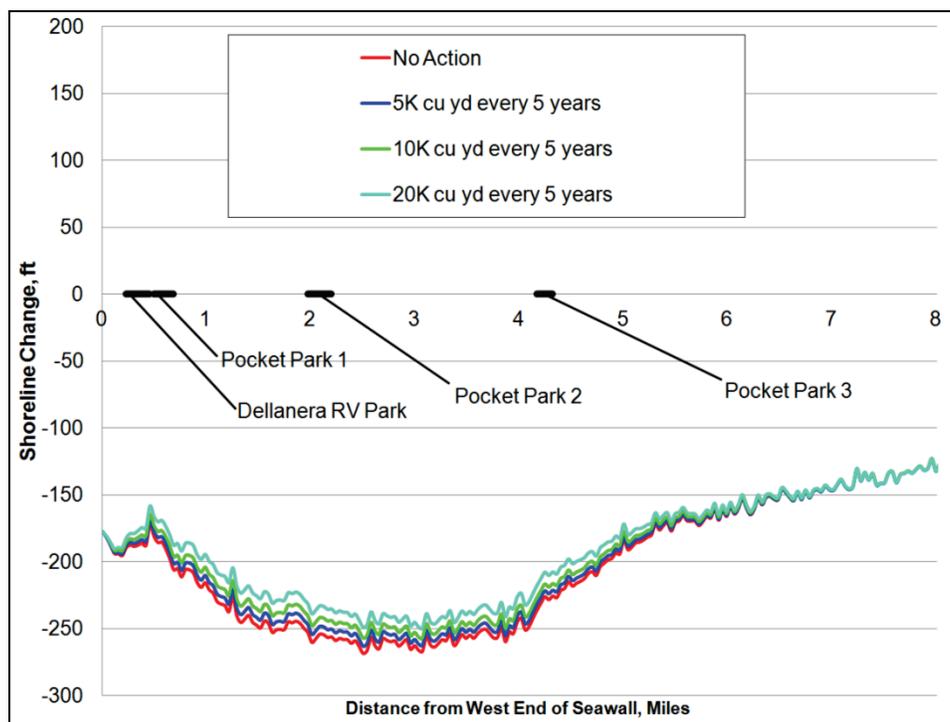


Figure 92. Total shoreline change after 50 years for 5,000, 10,000, and 20,000 yd³ beach fills nourished every 5 years on GPB property.



8.5.2 Level 2 – Beach fill along first 1.5 miles of west end

Level 2 consists of a beach fill along the first 1.5 miles of the west end of Galveston Island. The purpose of this fill is to help counter the end effects of the seawall. Beach fills of 50,000 and 100,000 yd³ were modeled with renourishment intervals of 1, 2, 5, and 10 years. Similar to the Level 1 beach fills, the initial fill volume is the same as the renourishment volume.

Figure 93 and Figure 94, compare 50,000 yd³ beach fills with renourishment cycles of 1, 2, 5, and 10 years after 10 and 50 years. After 10 years, the shoreline advances up to 40 ft near the seawall but still recedes approximately 30 ft at 1.5 miles to the west of the end of the seawall. The 50,000 yd³ beach fill renourished every year matches the no-action case at 2.5 miles west of the seawall, so the beach fill has no effect beyond that point. When 50,000 yd³ is placed on the beach every 10 years, the effects on the shoreline are minor. After 10 years, the shoreline recedes at least 55 ft and only provides approximately 5 additional feet of shoreline compared to the no-action case. After 50 years, the shoreline advances 160 ft near the seawall but recedes 100 ft at 1.5 miles west of the seawall when the fill is renourished each year. If the 50,000 yd³ beach fill has a 10-year renourishment cycle, the shoreline erodes 150 ft near the seawall and 240 ft at

1.5 miles west of the seawall. While the model shows a wide beach for the first half mile, nourishment is required every year, and the cumulative nourishment in this case is 2.5 million yd³, which is a large volume of sand for a short distance along the beach.

Figure 93. Total shoreline change after 10 years with 50,000 yd³ beach fills nourished every 1, 2, 5, and 10 years along the first 1.5 miles of Reach 3.

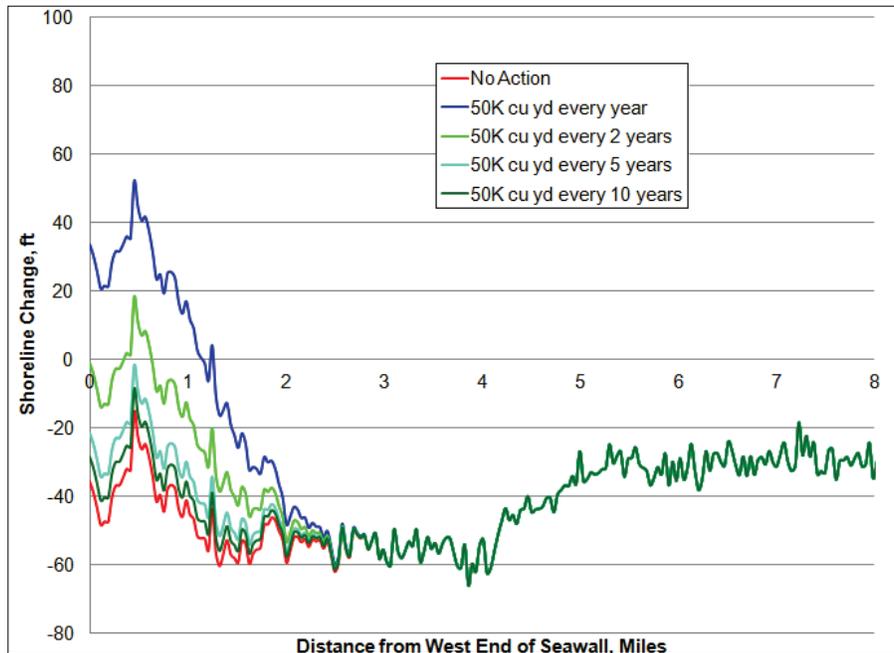
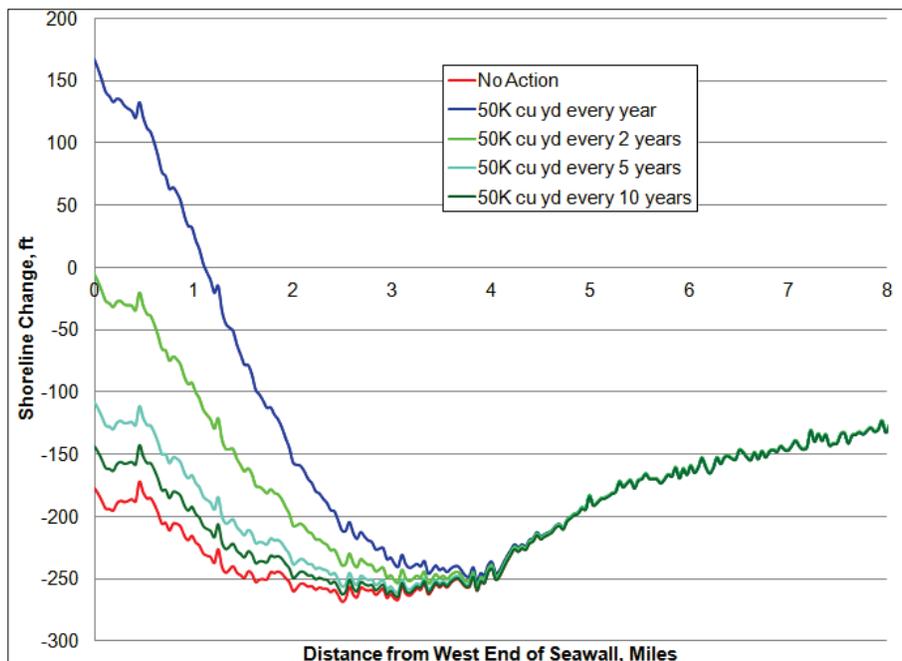


Figure 94. Total shoreline change after 50 years with 50,000 yd³ beach fills nourished every 1, 2, 5, and 10 years along the first 1.5 miles of Reach 3.



The same renourishment cycles and location were used, but the next alternatives looked at initial and renourishment volumes of 100,000 yd³ (Figure 95 and Figure 96). After 10 years, the 100,000 yd³ fill-placed-every-year option results in shoreline advance of 120 ft total. However, the shoreline 1.5 miles from the seawall is almost identical to the initial shoreline, and it matches the no-action case 2.5 miles west of the seawall. When 100,000 yd³ is placed every other year, the beach advances almost 50 ft near the seawall but erodes compared to the initial shoreline at 1.1 miles to the west. If 100,000 yd³ is placed every 5 or 10 years, the shoreline erodes compared to the initial shoreline condition along all of Reach 3.

The total shoreline change after 50 years for different renourishment cycles of 100,000 yd³ is shown in Figure 96. When 100,000 yd³ is renourished every 5 or 10 years, the entire beach recedes from initial, although the 5-year renourishment cycle provides an additional 120 ft of shoreline near the seawall compared to the no-action alternative. The GenCade model predicts that 100,000 yd³ of material placed every other year will advance the beach 150 ft near the seawall. The model also predicts a 500 ft wide beach near the seawall with 100,000 yd³ renourished every year. However, one problem is that two separate GenCade grids were used for the calibration and

Figure 95. Total shoreline change after 10 years with 100,000 yd³ beach fills nourished every 1, 2, 5, and 10 years along the first 1.5 miles of Reach 3.

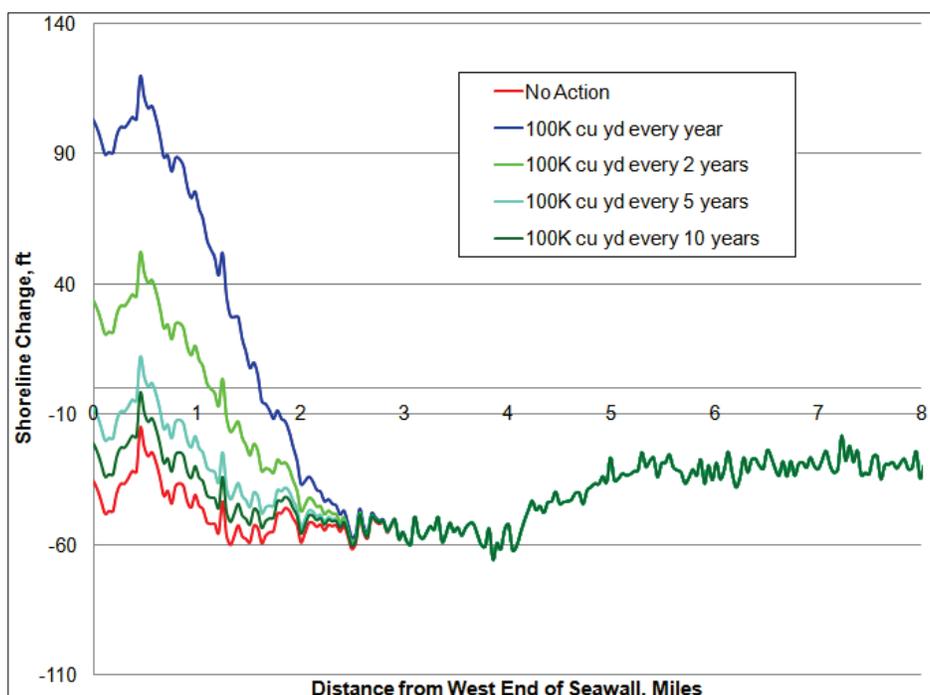
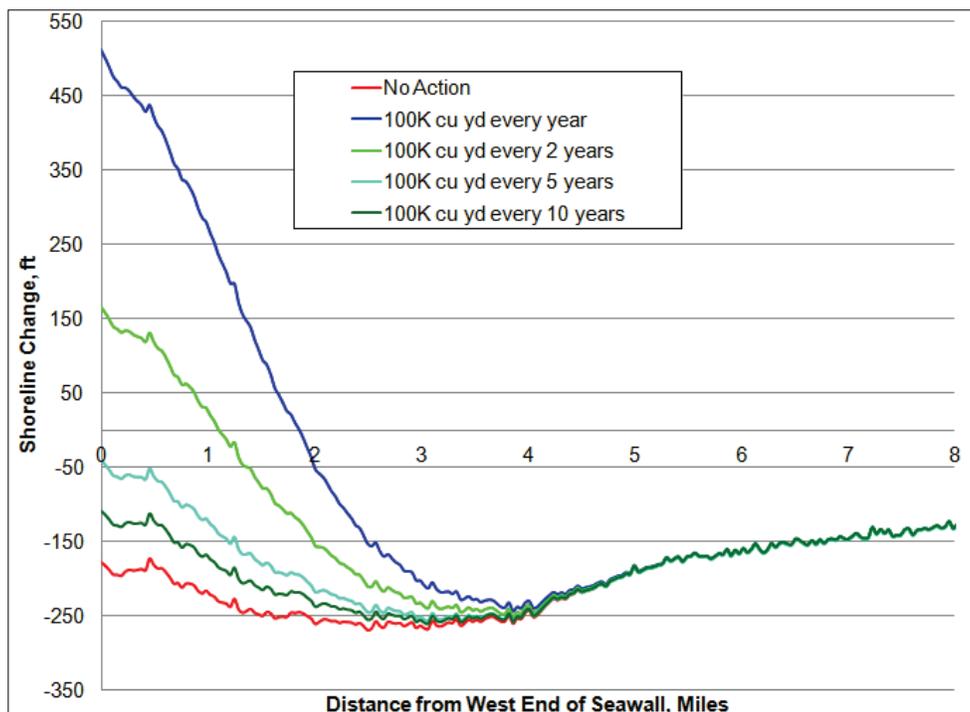


Figure 96. Total shoreline change after 50 years with 100,000 yd³ beach fills nourished every 1, 2, 5, and 10 years along the first 1.5 miles of Reach 3.



alternatives. The model does not realize that there is seawall adjacent to the grid boundary since a moving boundary condition was used to represent the seawall and the shoreline change at the boundary. It does not make sense that the shoreline could advance beyond the seaward position of the seawall. Note that placing 100,000 yd³/year along the 1.5 miles closest to the seawall will result in a wide beach adjacent to the seawall, but it is highly unlikely that a beach as wide as predicted in the model could occur.

8.5.3 Level 3 – Beach fill along Reach 3

Level 3 refers to a beach fill and renourishment along all of Reach 3. The first alternatives are 50,000 yd³ beach fills renourished over 1, 2, 5, and 10 years. Figure 97 and Figure 98 compare the 50,000 yd³ beach fills with the no-action scenario after 10 and 50 years. After 10 years, regardless of the renourishment cycle, the shoreline along Reach 3 erodes from the initial. Renourishing every year results in a total of 500,000 yd³ placed on the beach, but the shoreline still erodes 50 ft from the initial. Adding the beach fills results in an advance of 25 ft compared to the no-action alternative. After 50 years, the erosion along Reach 3 will be approximately 250 ft without action. The shoreline erosion when 50,000 yd³/year of material is placed is between 100 and 170 ft. Although small beach fills

along Reach 3 do not advance the shoreline from the initial, they do help decrease the erosion. It is possible that beach fills can be constructed on Reach 3, but they will not protect the shoreline for a long period of time nor are they part of the strategy to develop a 200 ft wide beach all along Galveston Island.

Similar alternatives along Reach 3 with 100,000 yd³ beach fills were modeled. Figure 99 and Figure 100 compare 100,000 yd³ initial beach fills with 1-, 2-, 5-, and 10-year renourishment intervals and the no-action scenario after 10 and 50 years. After 10 years of placing 100,000 yd³ of sand on the beach every year, the shoreline only erodes approximately 10 ft. This is approximately 40 ft less retreat compared to the no-action case. Note that 100,000 yd³/year is 1 million yd³ in 10 years. This is larger than the nourishments in front of the seawall over the last 20 years. If it takes 1 million yd³ to keep the shoreline almost stable, it is possible that improving other locations along the island would be more beneficial. Renourishing every 5 or 10 years has little impact on the shoreline. After 50 years, annual renourishment of Reach 3 results in shoreline erosion of approximately 50 ft, or 1 ft/year, much less than the historical retreat rate in the area. Renourishing the beach every other year for a total of 2.5 million yd³ results in erosion of 150 ft. Although erosion occurs in both of these cases, it is still less than the 250 ft eroded without action.

Figure 97. Total shoreline change after 10 years with 50,000 yd³ beach fills nourished every 1, 2, 5, and 10 years along Reach 3.

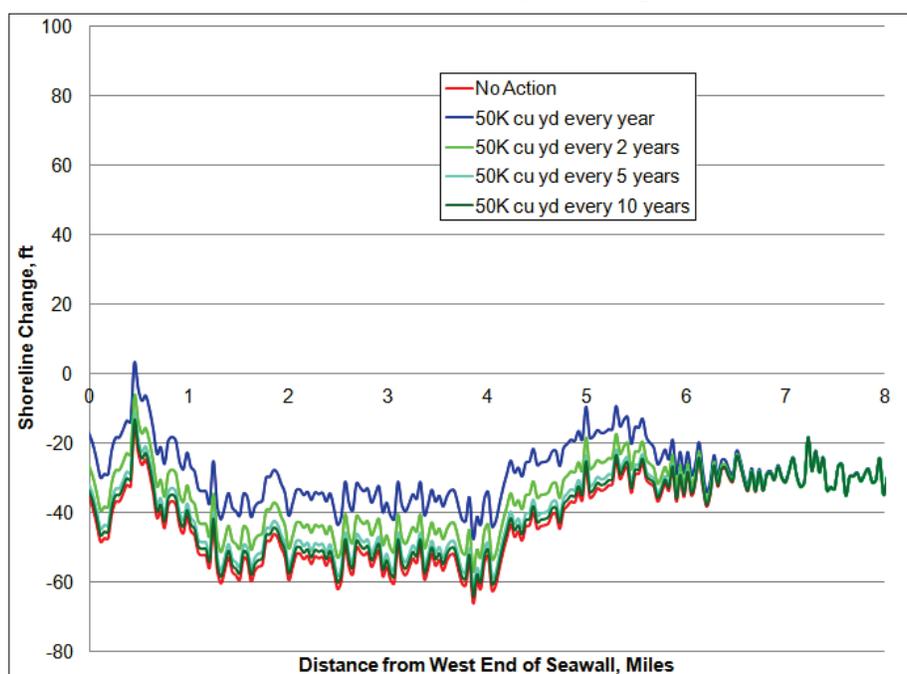


Figure 98. Total shoreline change after 50 years with 50,000 yd³ beach fills nourished every 1, 2, 5, and 10 years along Reach 3.

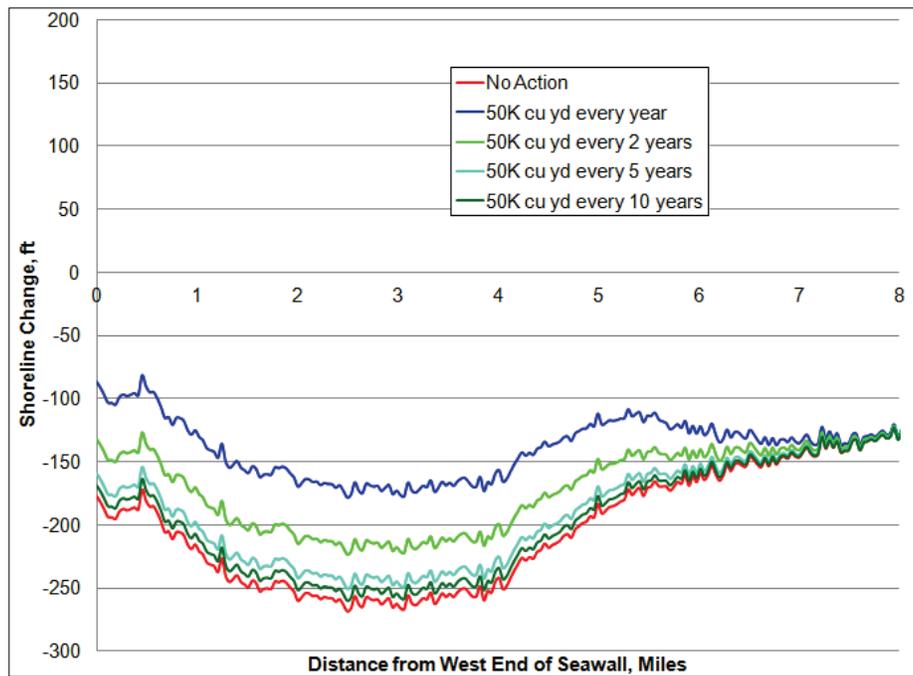


Figure 99. Total shoreline change after 10 years with 100,000 yd³ beach fills nourished every 1, 2, 5, and 10 years along Reach 3.

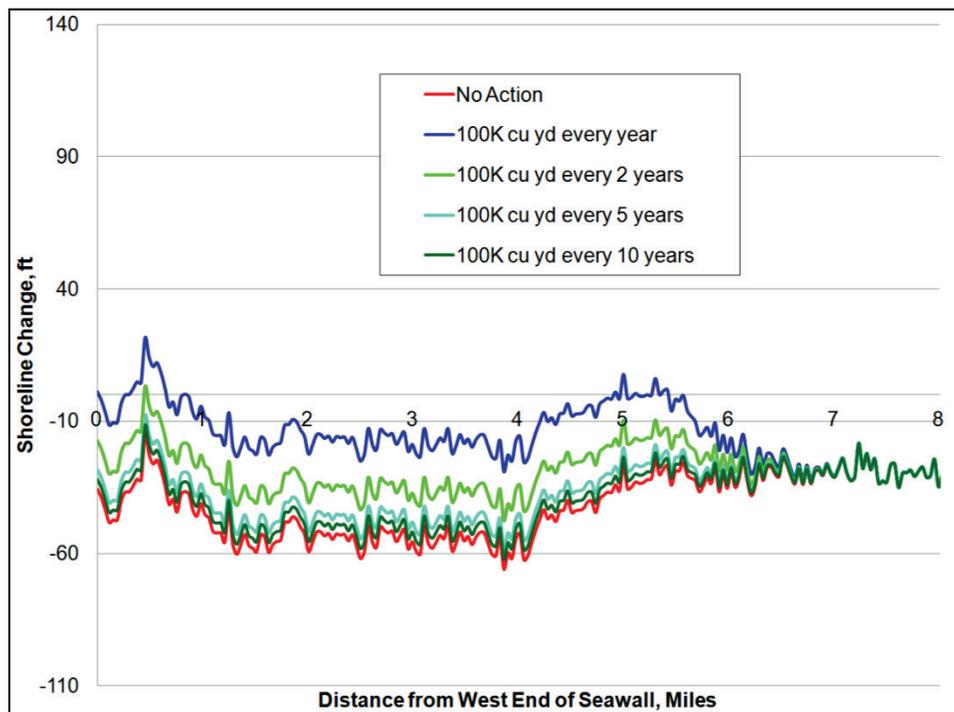


Figure 100. Total shoreline change after 50 years with 100,000 yd³ beach fills nourished every 1, 2, 5, and 10 years along Reach 3.

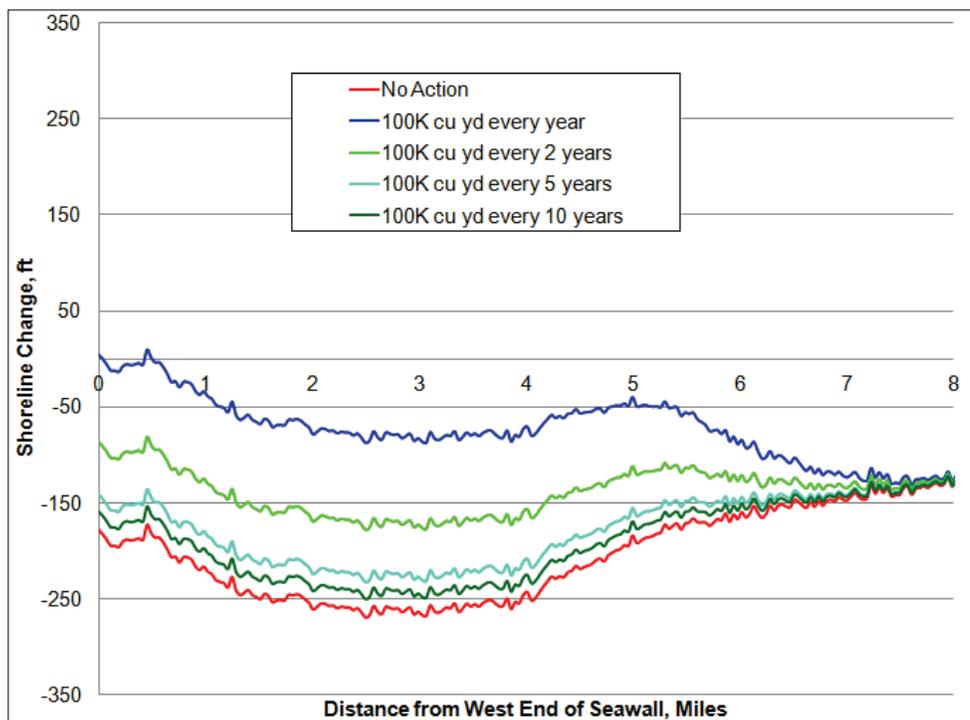


Figure 101 and Figure 102 compare 50,000 and 100,000 yd³ placements renourished every 2 years over the 1.5 miles closest to the west end of the seawall and over Reach 3 for 10 and 50 years. In Figure 101, the beach placement along the first 1.5 miles of Reach 3 has no effect after approximately 2.5 miles. While the beach fills along Reach 3 do not advance the shoreline compared to the no-action case as much as the 1.5 miles long beach fills, they do provide a little additional protection along the remainder of Reach 3. In Figure 102, the 1.5-miles-long fills result in advance near the seawall but erode more than the Reach 3 fills. Beyond 4 miles west of the seawall, the 1.5-miles-long beach fills are identical to the calculated shoreline for the no-action case. Figure 103 and Figure 104 show similar results when the 50,000 and 100,000 yd³ fills are nourished every 5 years across the first 1.5 miles of the west end and across Reach 3. It is important to show the scenarios together to see how the location and distance of the beach fill impact the shoreline response along the entire reach.

Figure 101. Comparison of total shoreline change with different beach fill volumes along the 1.5 miles west of the seawall and along Reach 3 after 10 years.

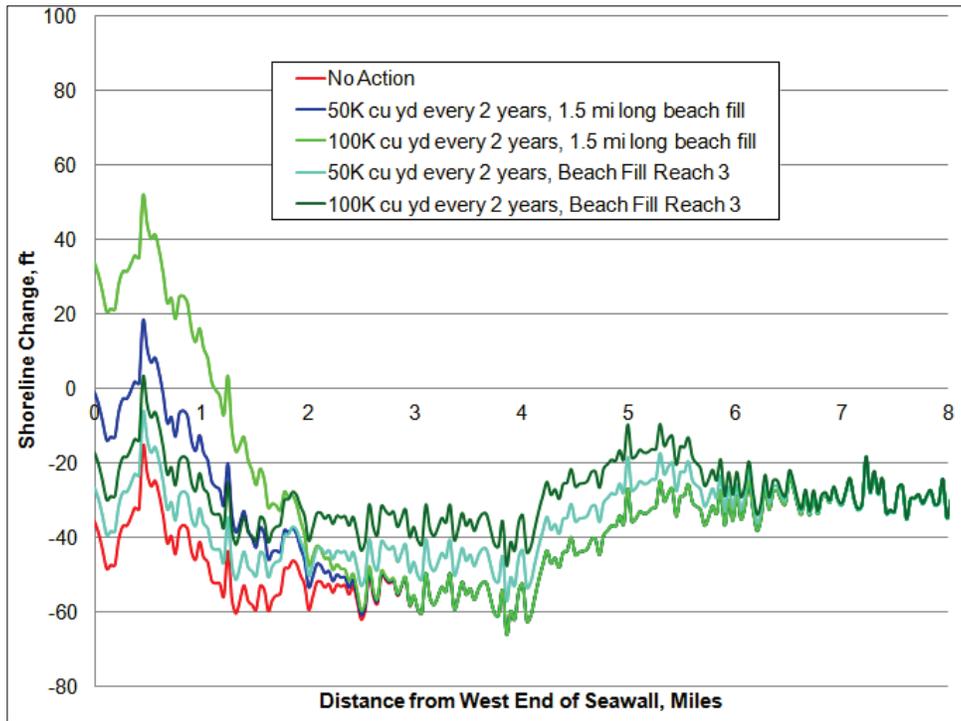


Figure 102. Comparison of total shoreline change with different beach fill volumes along the 1.5 miles west of the seawall and along Reach 3 after 50 years.

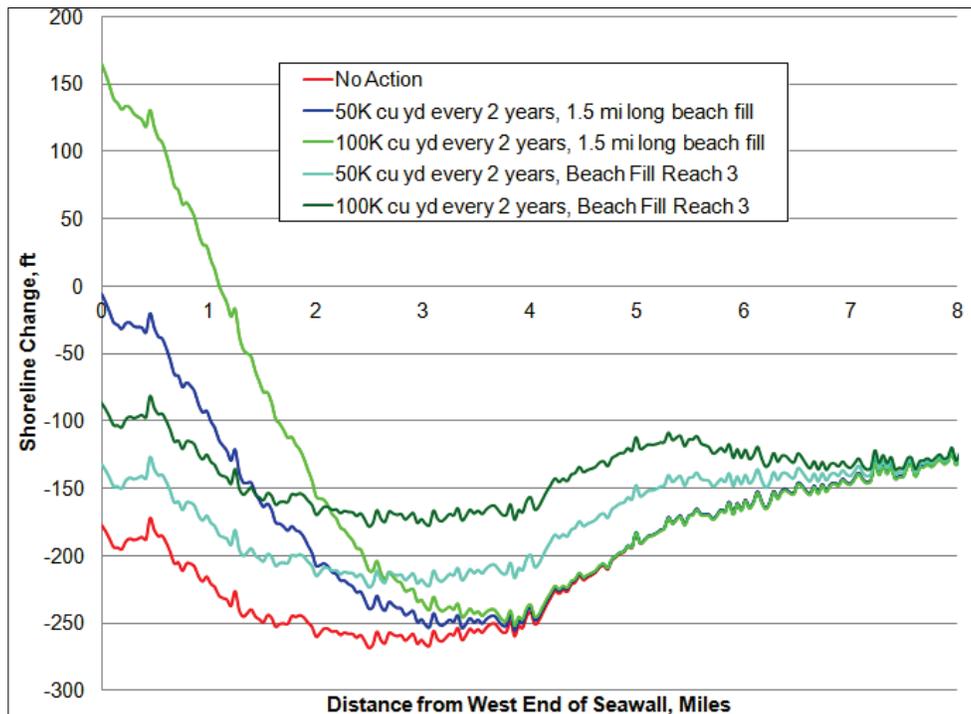


Figure 103. Comparison of total shoreline change with different beach fill volumes renourished every 5 years along the 1.5 miles west of the seawall and along Reach 3 after 10 years.

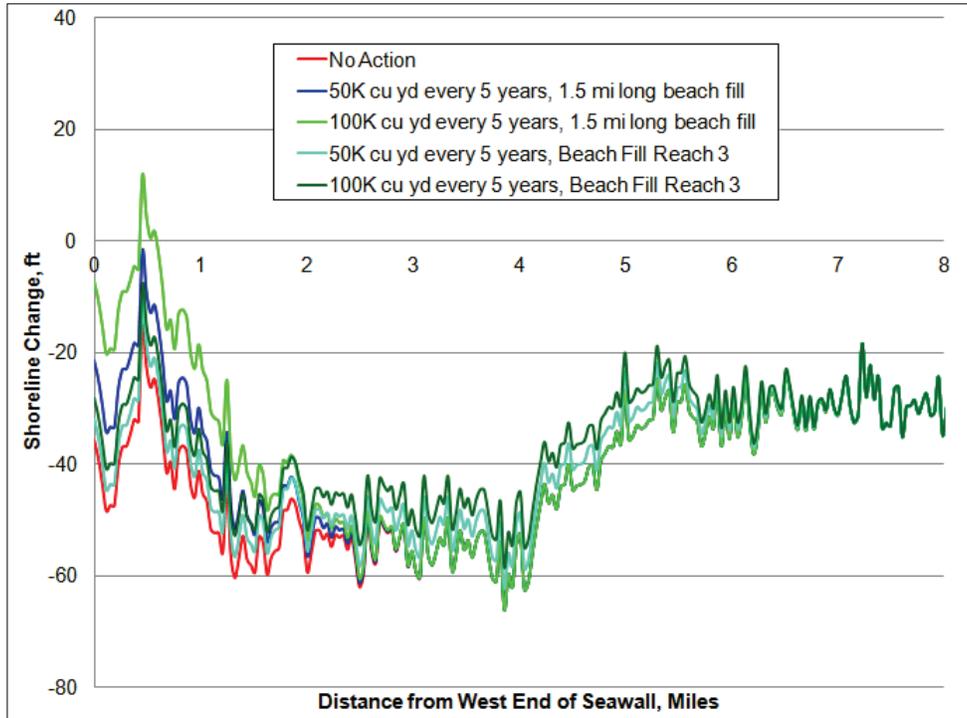
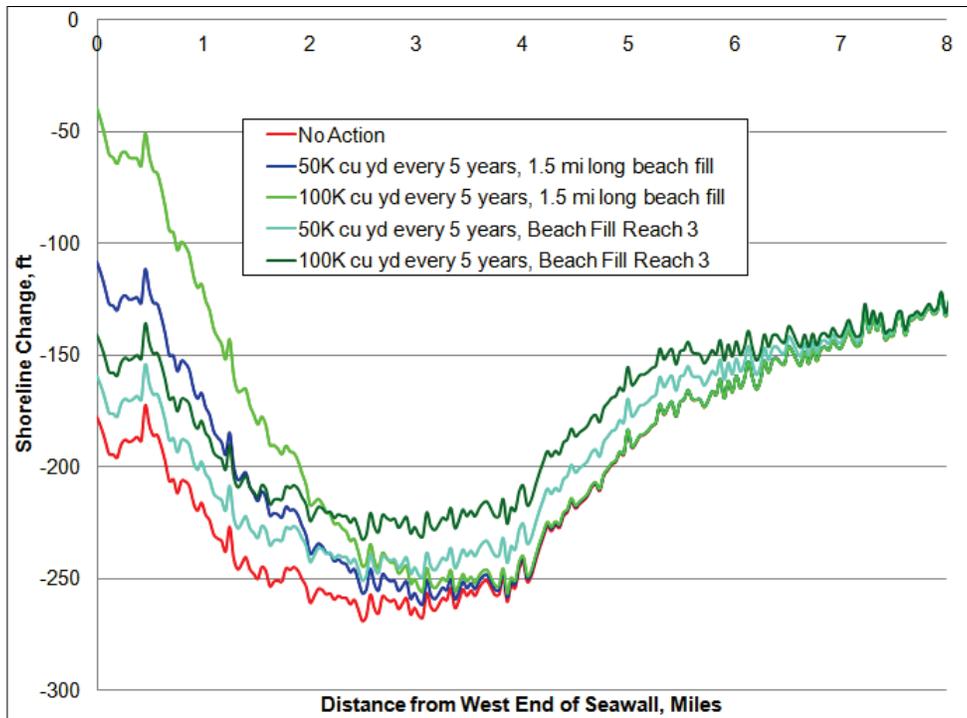


Figure 104. Comparison of total shoreline change with different beach fill volumes renourished every 5 years along the 1.5 miles west of the seawall and along Reach 3 after 50 years.



8.5.4 Level 4 – Beach fill along Reaches 3 and 4

The Level 4 beach fill expands across Reaches 3 and 4. The alternatives consist of 250,000 and 1 million yd³ beach fills renourished over 2, 5, and 10 years. The total shoreline change after 10 years is shown in Figure 105. Placing 1 million yd³ of sand across Reaches 3 and 4 every other year for 10 years is the only alternative which results in advance from the initial shoreline. The other alternatives advance the shoreline from the no-action case between 5 and 25 ft. The 1 million yd³ beach fill renourished every 5 years averages little shoreline change from the initial, but the shoreline erodes closer to the seawall and accretes more than 10 miles from the seawall. The same pattern can be seen after 50 years (Figure 106). The 1 million yd³/year beach fill results in a more than 200 ft wide beach beyond 5 miles west of the seawall. Other alternatives resulted in erosion less than 7 miles from the seawall. The 250,000 yd³ beach fill that is renourished every 10 years only results in 10 ft more beach than the no-action case. Placing 1.25 million yd³ over 50 years resulting in 10 ft of dry beach width beyond the no-action alternative will not produce a favorable benefit/cost ratio.

Figure 105. Total shoreline change with 250,000 yd³ and 1 million yd³ beach fills with different renourishment cycles after 10 years.

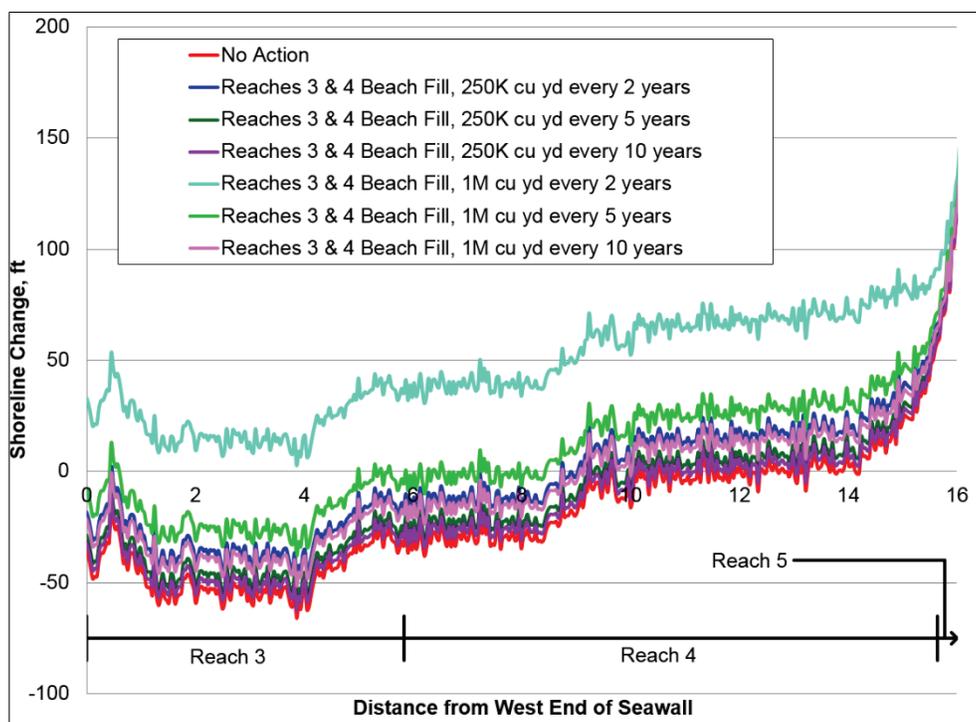
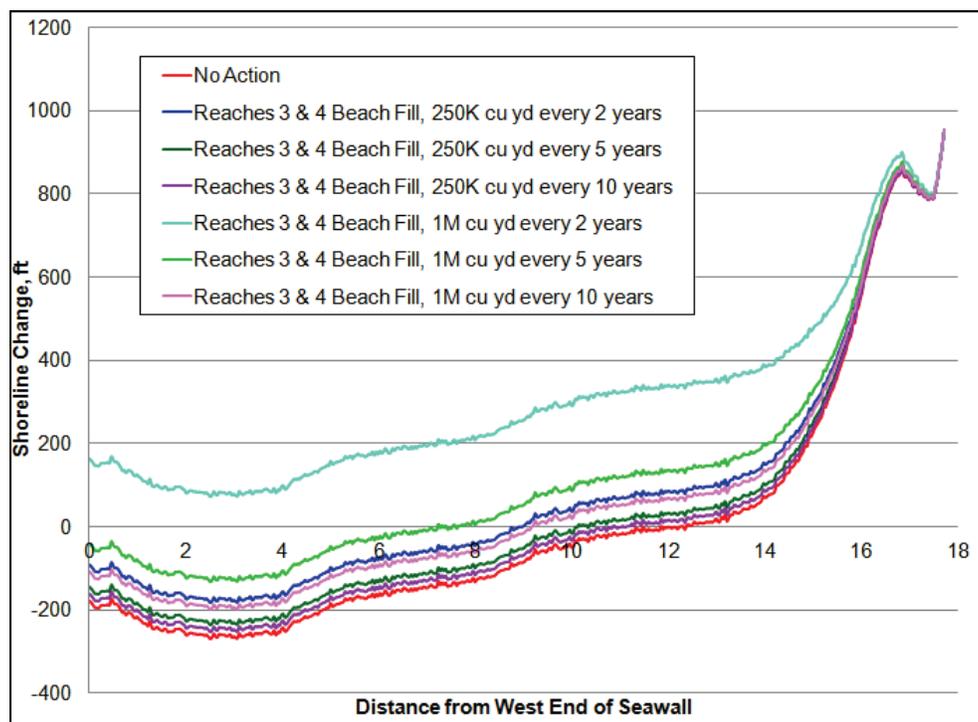


Figure 106. Total shoreline change with 250,000 yd³ and 1 million yd³ beach fills with different renourishment cycles after 50 years.



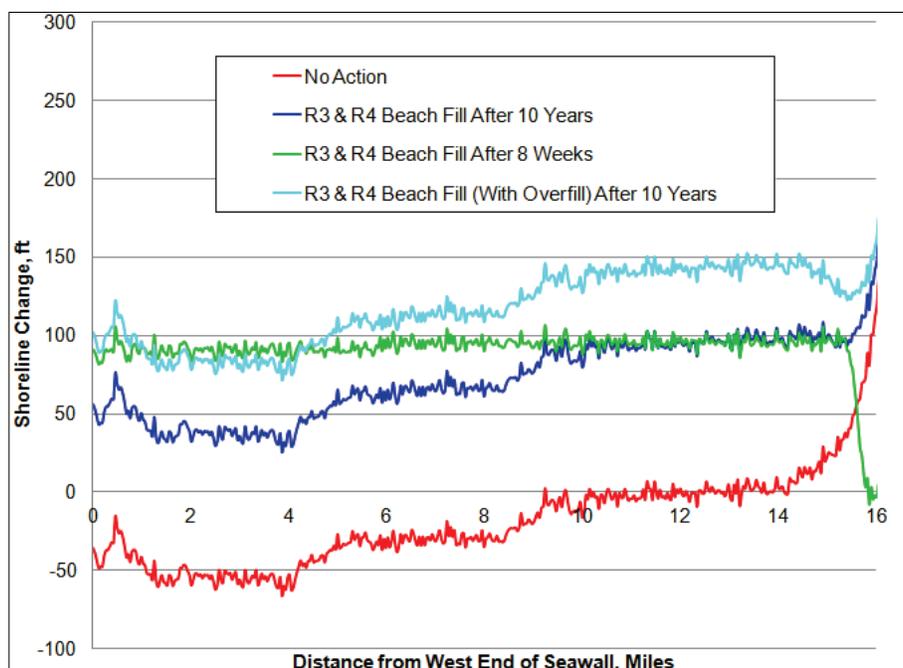
8.5.5 Level 5 – Large-scale beach fill along Reaches 3 and 4

The final alternative consists of a large-scale beach fill along Reaches 3 and 4. If a large-scale beach fill occurs on Reaches 3 and 4, Reaches 1 and 2 would have already been nourished with a large-scale beach fill or a combination of a large-scale beach fill with backpassing. Therefore, this option would be the most expensive and require the greatest volume of sand.

The beach fill is based on the volume calculated in the large-scale beach fill discussed in Section 7. Reach 3 requires an initial beach fill of 2.5188 million yd³, and Reach 4 consists of an initial beach fill of 4.4079 million yd³. Each beach fill requires a 50% advanced nourishment fill volume. Figure 107 compares the total shoreline change for the no-action alternative to the shoreline change for the large-scale beach fill with and without advanced nourishment after 10 years. The light green represents shoreline change after 8 weeks, which is when the beach fill construction is complete within the simulation. In order to keep the shoreline position similar to the position immediately after the large-scale beach fill, it is necessary to place approximately 275,000 yd³ every 2 years or 735,000 yd³ every 5 years. The majority of that material needs to be placed along Reach 3 because Figure 107 shows that there is very little change in the shoreline

position in Reach 4 from 8 weeks to 10 years. The total sand needed to advance the beaches of Reaches 3 and 4 between 100 and 150 ft from the initial shoreline is 13.5267 million yd³ if renourishing every 2 years or 13.5417 million yd³ if renourishing every 5 years. The main reason this volume is much less than the 1 million yd³/year is due to the large initial beach nourishment. The large beach fill protects the shoreline for a longer period of time and will not require large renourishments as when a smaller volume is used for the initial construction and for each subsequent nourishment.

Figure 107. Total shoreline change with a large-scale beach fill along Reaches 3 and 4 after 10 years.



8.6 Backpassing along west end

Three levels of backpassing were considered along the west end of Galveston Island. Each backpassing scenario consists of a backpassing plant near San Luis Pass and discharge points along Reaches 3 and 4.

8.6.1 Level 1 – Backpassing

Level 1 backpassing involves either 50,000 or 200,000 yd³/year from San Luis Pass to a single discharge located 0.75 miles from the west end of the seawall. Figure 108 and Figure 109 compare the total shoreline change for backpassing of 50,000 yd³/year and 200,000 yd³/year after 10 and 50 years. In addition, the initial large-scale beach fill of 2.5188 million yd³

Figure 108. Total shoreline change with backpassing along the first 1.5 miles of Reach 3 after 10 years.

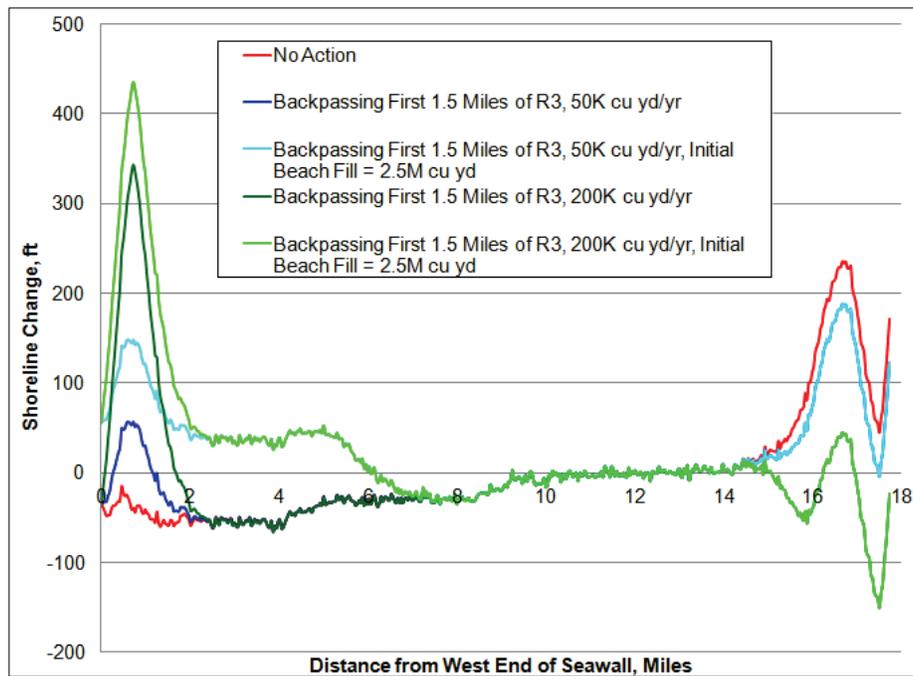
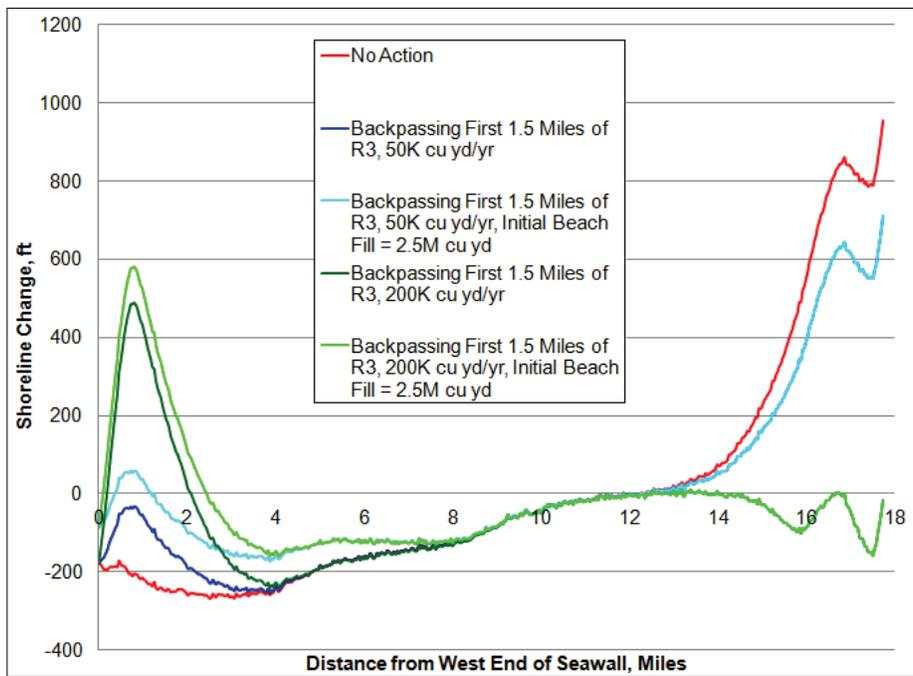


Figure 109. Total shoreline change with backpassing along the first 1.5 miles of Reach 3 after 50 years.



along Reach 3 was added to the backpassing alternatives for comparison purposes. After 10 years, the influx of sand is confined to the first 2 miles west of the seawall. With a backpassing rate of 50,000 yd³/year, the maximum shoreline advance is approximately 50 ft at the discharge point. This increases to approximately 150 ft when the large-scale beach fill is constructed at the beginning of the simulation. The large-scale beach fill combined with backpassing advances the shoreline 50 ft from the initial shoreline along Reach 3. The 200,000 yd³/year backpassing rate results in more than 300 ft of advance at the discharge without the beach fill and more than 400 ft with the beach fill. After the first 2 miles, the sand placed on the beach through backpassing has little effect on the shape of the shoreline. After 50 years, the impact of the sand from backpassing extends to approximately 4 miles west of the seawall. If only backpassing 50,000 yd³/year and constructing the initial fill, the majority of the shoreline retreats after 50 years. When 200,000 yd³/year is backpassed and the initial fill is constructed, shoreline advance only occurs up to 2.3 miles west of the seawall. Backpassing 200,000 yd³/year from near San Luis Pass equates to 10 million yd³ of backpassed sediment over a 50-year period. The shoreline near San Luis Pass no longer advances as expected and actually recedes in some locations. It will be necessary to conduct additional studies and complete detailed engineering design before developing a backpassing plant. GenCade is calibrated to past conditions, so changes to environmental forcing such as increased storminess are not considered within the model. Therefore, it is assumed that shoreline trends in the past will occur in the future. However, it is possible in the future that the shoreline along near San Luis Pass does not continue to accrete at the present rate. If the same rate of accretion does not occur, backpassing any amount of sand could erode the shoreline. Therefore, adaptive management is a key component of a backpassing plan.

8.6.2 Level 2 – Backpassing along Reach 3

The second level of backpassing along Reach 3 requires removal of 50,000 or 200,000 yd³/year of sand from San Luis Pass and placement of sediment at four discharge points along Reach 3. Figure 110 and Figure 111 show total shoreline change for backpassing with and without a large-scale initial beach fill along Reach 3 for 10 and 50 years. When only 50,000 yd³/year is backpassed, the beach fill has a much greater impact on shoreline position. The shoreline accretes by 50 ft with the beach fill but erodes approximately 30 ft without it. The four discharge points are noticeable when 200,000 yd³/year is backpassed. The shoreline change perturbation from each

discharge does not overlap perturbations from other discharges resulting in a nonuniform shoreline. The average shoreline position advance is 40 ft without the beach fill and 120 ft with the beach fill. After 50 years with 50,000 yd³/year of backpassing, both the alternatives with and without the beach fill have retreated beyond the initial shoreline. However, 250,000 yd³/year backpassing with the beach fill results in a shoreline advance of 250 ft along Reach 3. With the 250,000 yd³/year backpassing but without the beach fill, the shoreline advances approximately 190 ft. In all cases, the shoreline position matches the no-action case at approximately 7.5 miles west of the seawall. Figure 112 and Figure 113 compare the 50,000 yd³/year of backpassing case along the first 1.5 miles of Reach 3 to the Reach 3 alternatives for 10 and 50 years.

Figure 110. Total shoreline change with backpassing along Reach 3 after 10 years.

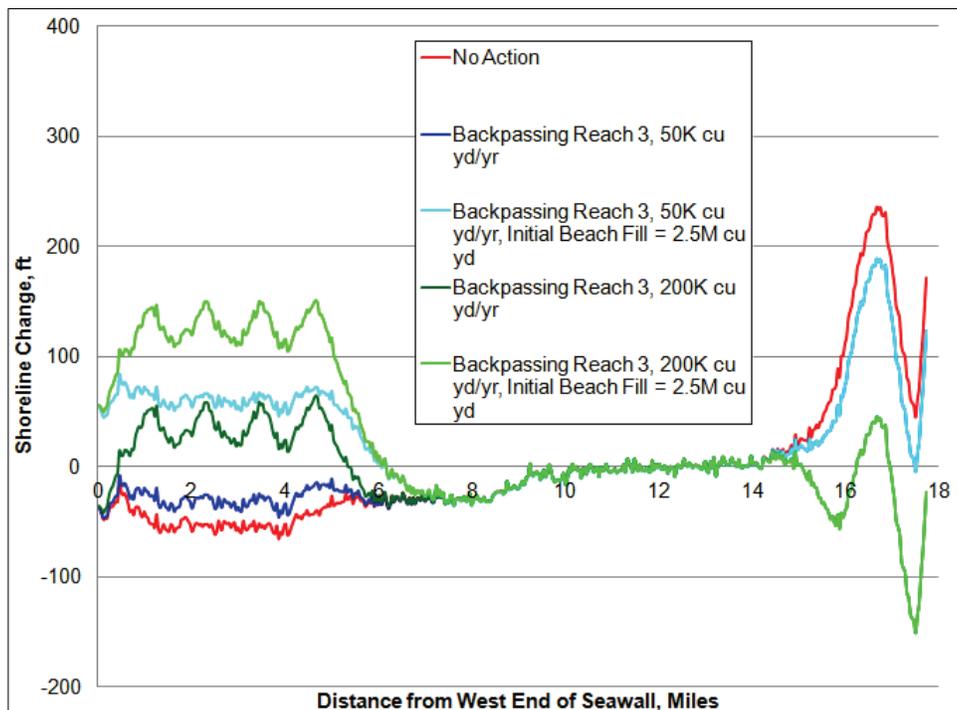


Figure 111. Total shoreline change with backpassing along Reach 3 after 50 years.

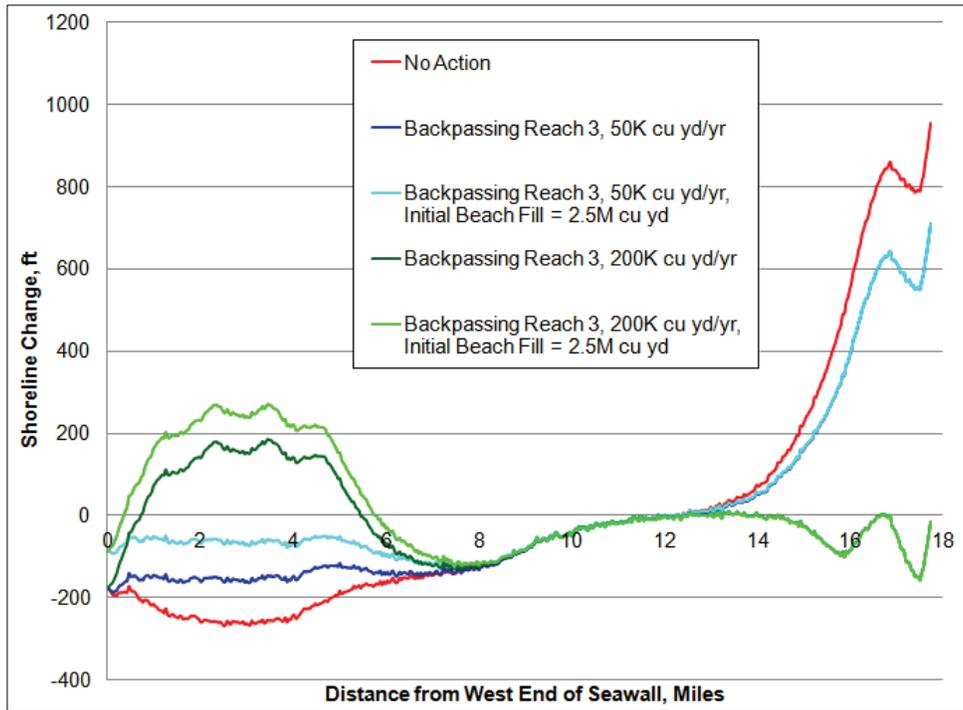


Figure 112. Total shoreline change with 50,000 yd³/year of backpassing along the first 1.5 miles of Reach 3 and along all of Reach 3 after 10 years.

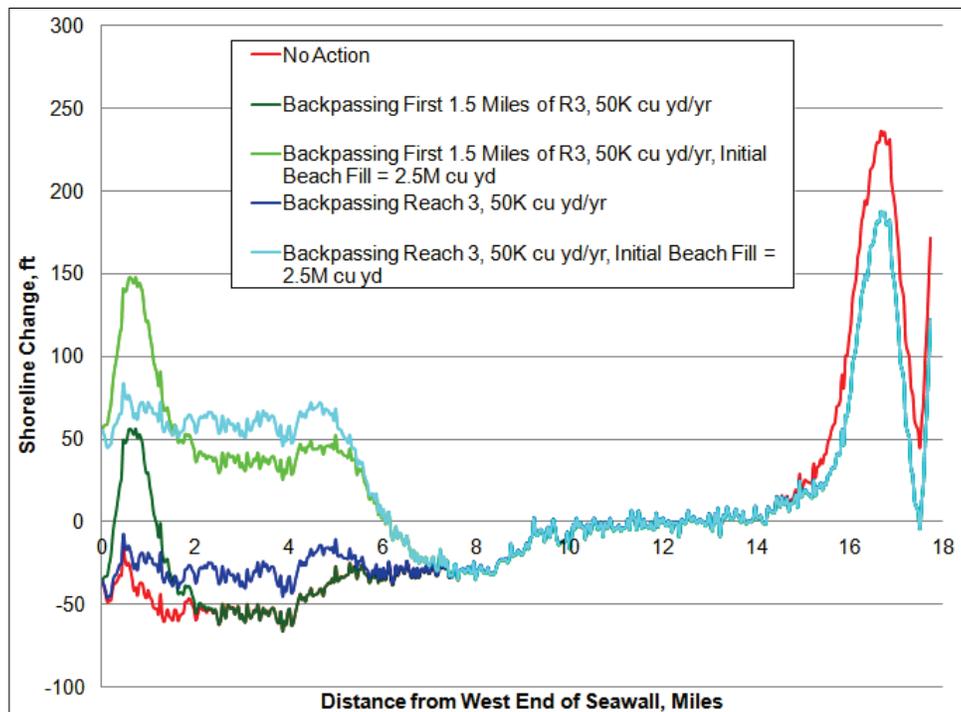
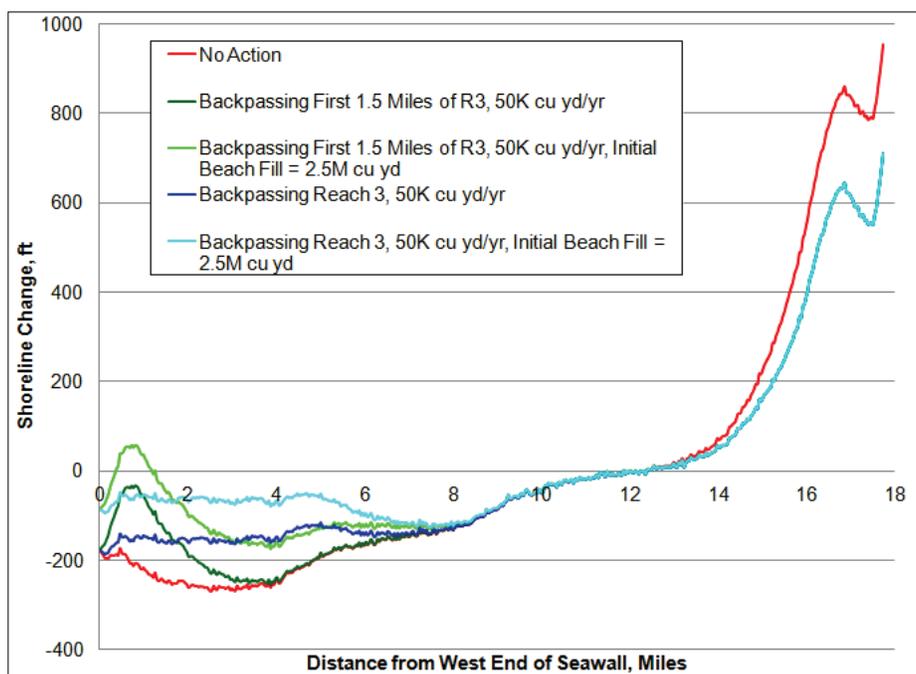


Figure 113. Total shoreline change with 50,000 yd³/year of backpassing along the first 1.5 miles of Reach 3 and along all of Reach 3 after 50 years.



8.6.3 Level 3 – Backpassing along Reaches 3 and 4

The final level of backpassing, along the west end involves backpassing 150,000 yd³/year and 300,000 yd³/year to Reaches 3 and 4. In Figure 114, backpassing 150,000 yd³/year and 300,000 yd³/year is compared to the same backpassing scenarios with the large-scale initial beach fill along Reaches 3 and 4 after 10 years. In all cases, the shoreline advances near the discharge points. However, approximately halfway between each point, the shoreline recedes to the no-action case when 150,000 yd³/year of backpassing occurs without an initial beach fill. The same type of trend occurs with backpassing of 300,000 yd³/year. The shoreline advances approximately 100 ft near the discharge points but recedes to the no-action alternative midway between the points. When the initial beach fill is included, the shoreline advances a minimum of 50 ft between the discharge points and up to 150 and 240 ft with the 150,000 yd³/year and 300,000 yd³/year of backpassing, respectively. After 50 years, the shoreline advances west of the 6 mile marker when only 150,000 yd³/year of backpassing occurs (Figure 115). Backpassing 300,000 yd³/year combined with the initial fill creates the widest beach along Reaches 3 and 4 after 50 years. If the sand is backpassed to more than four locations through adjustable discharge points, the shoreline advance will be more uniform. The shoreline still erodes less than 2 miles west of the seawall, but the rest of the shoreline

accretes between 100 and 450 ft. Although 300,000 yd³/year is backpassed from near San Luis Pass, the shoreline near the inlet does not retreat as much as the Reach 3 backpassing scenarios. The reason this happens is because the sand is backpassed to two locations along Reach 4. The final discharge point along Reach 4 is very close to Reach 5. Since longshore transport moves from east to west along the west end, the model shows that the sand moves from Reach 4 and deposits on Reach 5.

Figure 114. Total shoreline change after 10 years of backpassing along Reaches 3 and 4.

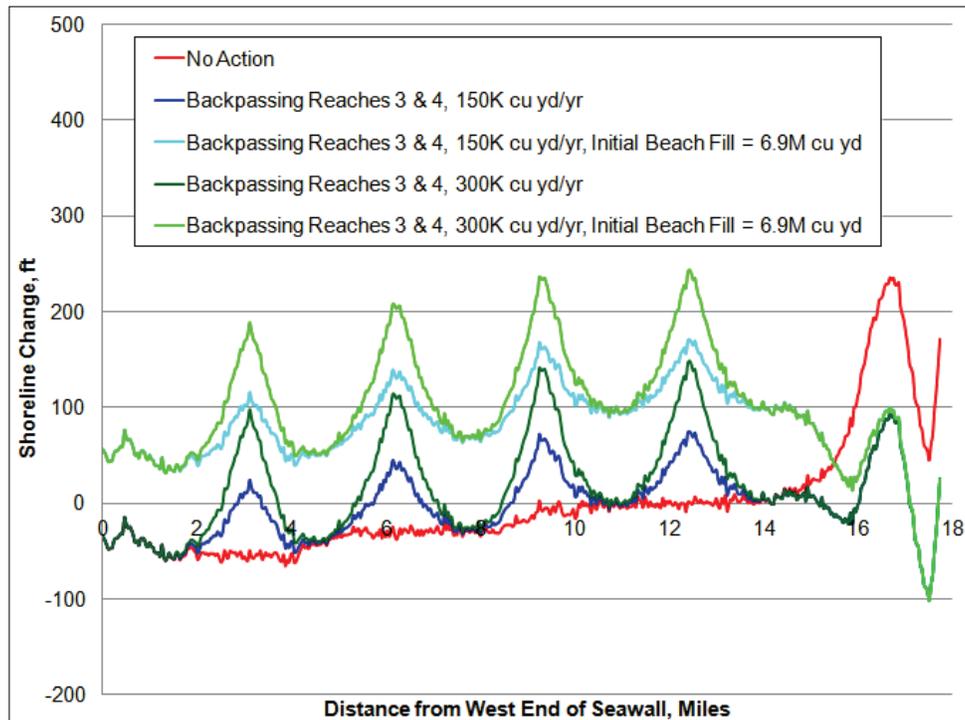
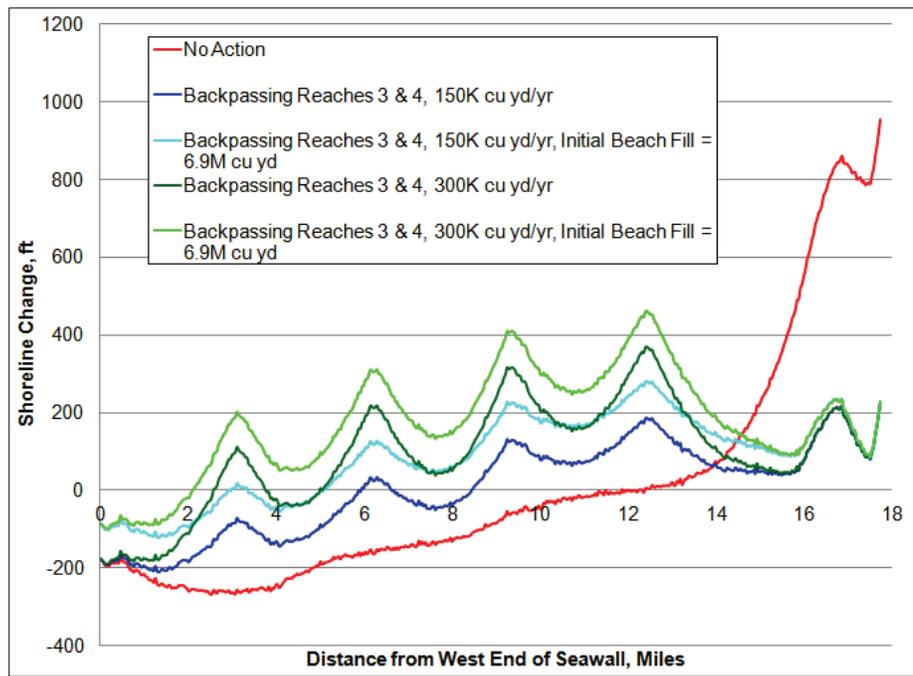


Figure 115. Total shoreline change after 50 years of backpassing along Reaches 3 and 4.



9 Conclusions

This report presents alternatives for a long-term sand management strategy for Galveston Island, TX. Widening the beaches of Galveston Island and developing a systematic strategy for managing sand resources will become the basis of a revivalism strategy to better protect the island from storms and enhance tourism.

Specific alternatives for each reach are discussed in Section 2. The main option is a large-scale initial beach fill with backpassing plants in place to renourish the beaches on a semicontinuous basis. As an alternative if there are funding issues, limited sand, or other restrictions, smaller, more localized beach fills are also included in the strategy. In order to obtain sand for beach nourishment, sediment management alternatives were investigated at East Beach and San Luis Pass.

9.1 Beach fill

A comprehensive beach fill along Reaches 1–5 (Figure 3) with a dune 100 ft wide and a beach/berm 200 ft wide will require 13 million yd³ of sand. With advance fill of 50%, the total volume would be 19.5 million yd³. The GenCade modeling (described in Section 8) demonstrated that initial fill would be maintained by backpassing. Without an initial fill, backpassing of 100,000 yd³/year would eventually result in shoreline advance but only after the passage of decades.

9.1.1 Sand sources at East Beach, Galveston Island

The first option for a sand source at East Beach is to continue mining sand from Big Reef. Big Reef has been mined for decades. Permits are already in place, and a logical procedure would be to continue mining the aerial part of the reef using trucks and land-based equipment. An analysis determined that up to 1.8 million yd³ could be available if specific areas are excavated to a depth of 5.5 yd.

A second option is to excavate an offshore deposition basin parallel to the beach in 16 ft water depth. A basin 3,000 × 150 yd and only 2 yd deep would yield 900,000 yd³ from initial construction and trap approximately 90,000 yd³ of sand annually (based on the sediment budget and assuming

50% trapping efficiency). A higher trapping efficiency would fill the basin more rapidly, allowing more frequent mining.

The third option would be to reduce sand transmission through the south jetty. If a 450 m section of the jetty extending from the current shoreline out to the east were sealed, up to 170,000 yd³/year of sand would accumulate in a fillet.

A fourth option is to reduce aeolian transport across the bare sand on East Beach, Big Reef, and the adjacent USACE dredge material disposal area. Sand fences could trap approximately 60,000–80,000 yd³ of sand in a year while planting vegetation instead could trap up to 60,000 yd³ a year.

9.1.2 Sand sources at San Luis Pass

A groin placed perpendicular to the shoreline near the opening to San Luis Pass will trap littoral material, which would then be available for use in beach fill elsewhere and would reduce sand carried into the flood shoal. A groin should be placed near the west end of the island, at the junction of littoral cells Cells1_10 and Cells1_09. Based on this study and assuming a trapping efficiency of 50%, a 500 ft groin could trap approximately 130,000 yd³ of sand annually.

9.2 Recommendations

Determining exactly how much material moves onshore near East Beach needs to be evaluated in greater detail, using the USACE's Radar Inlet Operating System (RIOS), frequent monitoring surveys, sediment tracers, side-scan sonar, bedload traps, or other methods to track seabed transport. The movement of sand from the ODMDS to Galveston Island is not well understood. These data will help verify the sediment budget and identify the source(s) of sediment volumes currently observed moving onshore.

Repetitive beach profiling and monitoring is recommended. It would be beneficial to use the profile stations established in the Texas A&M 2002 profiling effort to bolster the dataset over a longer time period

Wind-blown transport needs to be re-evaluated across East Beach and on Big Reef. A field study using sand traps will verify that the calculation procedure used in this study correctly determined the magnitude of transport.

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Appendix A: Dredging Summary, Galveston Ship Channel

Tables A1-A5 summarize dredging volumes from the Galveston Entrance Channel, Anchorage Area, boat slip, Inner and Outer Bar Channels, and Big Reef. Table A6 provides a summary of dredging volumes removed by location and year dredged. Data from before 2010 was extracted from an internal database maintained by SWG. Post-2010 data was from a spreadsheet supplied by SWG.

Table A1. Dredging volumes from Galveston entrance channel.

Channel	DSStation	USStation	Notes	EndDate	Vol (yd ³)
Entrance and extended entrance			W9126G-13-C-0041	1-Dec-13	1,269,000
Entrance and extended entrance			W912HY-12-C-0023	1-Nov-12	175,000
Entrance and extended entrance			W912HY-11-C-0016	1-Dec-11	1,758,151
Entrance channel	74+000	40+000		24-Apr-12	1,809,495
Entrance and extended entrance			W912HY-08-C-0026	1-Jan-10	2,809,652
Entrance channel	66+000	23+000		24-May-10	2,262,000
Entrance channel	82+00	32+600		22-Aug-06	2,347,991
Entrance channel	51+000	37+000		28-Jun-03	3,627,241
Entrance channel	56+000	36+000		31-Mar-97	1,457,011
Entrance channel	56+000	36+000		25-Aug-93	1,751,450
Entrance channel	56+000	35+000		5-Dec-90	1,563,000
Entrance channel	51+000	30+675		30-Sep-89	58,938
Entrance channel	56+000	30+675	Est.: 2/3 of 1,038,946 yd ³	4-Oct-88	692,631
Entrance channel	56+000	30+675		31-Jul-86	1,656,469
Entrance channel	56+000	30+675		25-Jul-84	2,909,507
Entrance channel	56+000	30+675		5-Aug-80	972,296
	56+000	30+675		7-Sep-79	2,530,951
				25-Mar-77	
Total					25,579,288
Years					36.69
Annual dredging yd ³					697,226

Notes (also applies to following Tables):

DSStation = downstream station

USStation = upstream station

EndDate = completion of dredging. First of month assumed if only month is known.

~~Strike through~~ = data from SWG dredging database superseded by newer data supplied by SWG

Table A2. Dredging volumes from anchorage area.

Channel	DSStation	USStation	Notes	EndDate	Vol (yd ³)
Anchorage area	17+400	12+000		31-Mar-97	603,695
Anchorage area	15+400	11+969		25-Aug-93	296,160
Anchorage area	19+400	11+969		5-Dec-90	1,261,000
Anchorage area	16+800	11+969.76		4-Oct-88	
Anchorage area	19+800	11+970		5-Aug-80	408,300
Anchorage area	19+800	16+000		9-Jul-78	1,130,462
				25-May-68	
Total (assume input from Bolivar Peninsula)					3,699,617
Years					28.85
Annual dredging yd ³					128,242

Table A3. Dredging volumes from boat slip.

Channel	DSStation	USStation	Notes	EndDate	Vol (yd ³)
Boat slip USACE	0+03	3+30		3-Aug-03	30,200
Boat slip USACE	0+00	3+30		6-Apr-98	19,142
Boat slip USACE	0+06	3+85		29-Jul-93	21,452
Boat slip USACE	0+30	3+30		30-Sep-89	44,502
Boat slip USACE	0+00C	3+85		7-Oct-85	40,473
Boat slip USACE	0+30	3+65		12-Apr-82	25,088
Boat slip USACE	0+55	3+80		9-Jul-78	7,213
Boat slip USACE	0+55	3+80		1-Jun-74	11,700
Boat slip USACE	0+55	3+80		11-Sep-70	7,696
				2-Jun-69	
Total					207,466
Years					34.17
Annual dredging yd ³					6,072

Table A4. Dredging volumes from inner and outer bar channel.

Channel	DSStation	USStation	Notes	EndDate	Vol (yd ³)
Inner bar channel			W9126G-13-C-0041	1-Dec-13	882,300
Outer and inner bar chan.			W912HY-12-C-0023	1-Nov-12	600,000
Outer and inner bar chan.			W912HY-11-C-0016	1-Dec-11	194,375
Inner bar channel	22+569.69	15+600.00		24-Apr-12	2,007,620
Inner bar channel	20+000	8+031		22-Jul-10	1,939,568
Outer and inner bar chan.	23+000	0+000		24-May-10	779,000
Outer and inner bar chan.			W912HY-08-C-0026	1-Jan-09	2,042,695
Inner bar channel	0+000	10+000		24-Sep-06	144,463
Outer and inner bar chan.	25+000	4+649.75		31-Mar-97	464,826
Inner bar channel	14+225	14+150		6-May-96	8,653
Inner bar channel	21+912.37	4+649.75		30-Jul-95	691,683
Outer and inner bar chan.	25+000	4+649.79		25-Aug-93	845,012
Outer and inner bar chan.*	29+400	0+200	Est.: 1/3 of 1,038,946 yd ³	5-Dec-90	792,838
Outer and inner bar chan.	30+675	4+649		4-Oct-88	345,969
Outer and inner bar chan.	30+675	4+649		31-Jul-86	556,099
Outer and inner bar chan.	30+675	4+649		25-Jul-84	1,577,898
Outer and inner bar chan.	30+675	5+600		5-Aug-80	198,282
				7-Sep-79	
Total					9,345,093
Years					34.23
Annual dredging yd ³					272,976

* Note: 1990 volumes based on total quantity dredged subdivided by the estimated portion in each reach.

Table A5. Dredging volumes from Big Reef.

Channel	DSStation	USStation	Notes	EndDate	Vol (yd ³)
Big Reef (misc. removals pre-Hurricane Ike – not listed individually)				1-Jun-08	178,731
				1-Mar-85	
Total					178,731
Years					23.25
Annual dredging yd ³					7,687

Table A6. Dredging summary.

Location	Vol (yd ³)
Sand vol.: boat slip, Inner and Outer Bar Channel, Big Reef	286,735
Percentage sand (68 samples from SWG dredge database):	86%
Sand entering via S. jetty and aeolian transport (yd ³ /year):	246,592

Appendix B: Dredging and Placement Data, Galveston Seawall and Big Reef

Table B1 lists beach fills and placements along Galveston Island and sand removal from Big Reef.

Table B1. Dredging summary.

Project	Year	Vol (yd ³)	Length (ft)	Source
Galveston Seawall - San Luis Hotel	3/1/1985	15,000	1500	Giardino et al. (1987)
Veneer and minor fills - 1999 to 2008 volume estimates based on a \$7.00/yd ³ cost estimate and was placed fronting the major hotels- undetermined length	1999	9613	unknown	Park Board
	2000	33,074	unknown	Park Board
	2001	5,823	unknown	Park Board
	2002	3,887	unknown	Park Board
	2003	32,418	unknown	Park Board
	2004	5,321	unknown	Park Board
	2005	461	unknown	Park Board
	2006	43,767	unknown	Park Board
	2007	18,499	unknown	Park Board
	6/1/2008	10,868	unknown	Park Board
Post-Ike fill (2009-2010) - Big Reef	6/1/2010	565,108	14th to 61st	HDR (via John Lee, Jr.)
Beach nourishment - Galveston Seawall	1992 nearshore berm - (dredged from channel)	500,000	6,000	Shiner Moseley
	1995 (dredged from offshore)	710,000	19,000	City of Galveston (Engineering by CPE)

Appendix C: Data Tabulation for Sediment Budget Cells

Table C1 lists the fluxes and volume changes for each cell discussed in the text. Values are in U.S. customary units (yd^3/year) $\times 1000$.

Table C1. Galveston sediment budget cell tabulation.

Sediment Budget, Galveston Island

VERSION 3b. Last update: 18 Sep. 2014

<p>Nomenclature: Units: 1000 yd^3/year Source 1 = bluffs, river influx, wind Sink 1 = wind-blown loss Source or sink 2 = offshore Source or sink 3 = other (inlet, channel, trap) LST1 = right (east) side of cell LST2 = left (west) side of cell Yellow cells = beach; blue cells = channel, inlet ΔV = beach change (erosion or growth) Placement = beach fill; Removal = dredging</p>
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Cell	Variable	Expect. value yd^3	Low value yd^3	High value yd^3	Notes, source
Galv. North Fillet	Q_{source1}				
Cell 1_1	Q_{sink1}				
34,100 ft long	Q_{source2}	-5.5	-4.0	47.8	Offshore flux - used to balance cell
	Q_{sink2}				
	Q_{source3}				
	Q_{sink3}				
	$Q_{\text{source-LST1}}$	225.5	169.1	281.9	From Cell 9 to N (from 2006 sed. budget; not updated). Assume - = 75%, + = 125%
	$Q_{\text{sink-LST1}}$				
	$Q_{\text{source-LST2}}$				
	$Q_{\text{sink-LST2}}$	110.3	110.3	110.3	Sand to anchorage area, through jetty and boat gap
	Placement				
	Removal				
	DeltaV	109.7	54.8	219.3	Based on translating 2002 profiles 1.12 m/year end-point rate. Assume - = 50%, + = 200%
	Residual	0.0	0.0	0.0	
	Q_{source1}				

Galv. Entrance Chan.	Q _{sink1}				
Cell 1_4	Q _{source2}	0.0	0.0	0.0	Assume no offshore source
35,000 ft long	Q _{sink2}				
	Q _{source3}	697.226	697.2 26	697.2 26	From Galveston Bay, Bolivar Roads. Assume + = 125%
	Q _{sink3}				
	Q _{source-LST1}				
	Q _{sink-LST1}				
	Q _{source-LST2}				
	Q _{sink-LST2}				
	Placement				
	Removal	697.226	697.2 26	697.2 26	1979-2013 maintenance dredging
	DeltaV				
	Residual	0.0	0.0	0.0	

	Q _{source1}	366.6	366.6	366.6	Fine grain sediment from Galveston Bay
Anchorage Area	Q _{sink1}				
Cell 1_2	Q _{source2}				
34,000 ft long	Q _{sink2}	348.6	348.6	348.6	To Cell 1_4 Entrance Channel
	Q _{source3}	110.3	110.3	110.3	Sand from Cell 1_1 via N. jetty (based on mean sand content of 86% for samples in inner and outer bar channels, using 1953-1997 samples)
	Q _{sink3}				
	Q _{source-LST1}				
	Q _{sink-LST1}				
	Q _{source-LST2}				
	Q _{sink-LST2}				
	Placement				
	Removal	128.2	128.2	128.2	1978-97 dredging
	DeltaV				
	Residual	0.0	0.0	0.0	

	Q _{source1}	388.8	388.8	388.8	Fine grain material from Galveston Bay
Inner and Outer Bar	Q _{sink1}				
Cell 1_3	Q _{source2}				
28,000 ft long	Q _{sink2}	348.6	348.6	348.6	To Cell 1_4 Entrance Channel
	Q _{source3}	246.6	246.6	246.6	Sand from Cell 1_5 via S. jetty and aeolian (based on mean sand content of 86% for samples in inner and outer bar channels, using 1953-1997 samples)
	Q _{sink3}				
	Q _{source-LST1}				
	Q _{sink-LST1}				
	Q _{source-LST2}				
	Q _{sink-LST2}				

	Placement				
	Removal	286.7	286.7	286.7	1980-2012 dredging from channel, boat slip (R = 6.07 yd ³ /year), Big Reef (R = 7.69 yd ³ /year)
	DeltaV				
	Residual	0.0	0.0	0.0	
East Beach	Q _{source1}				
Cell 1_5	Q _{sink1}				
20,300 ft long	Q _{source2}	355.7	286.2	487.2	From offshore - only way to balance cell and account for fillet growth
	Q _{sink2}				
	Q _{source3}				
	Q _{sink3}	246.6	246.6	246.6	Sand to Cell 1_3 via S. jetty and aeolian
	Q _{source-LST1}				
	Q _{sink-LST1}				
	Q _{source-LST2}	41.2	35.5	59.9	From Cell 1_6
	Q _{sink-LST2}	0.0	0.0	0.0	Assume all littoral transport moves to east
	Placement				
	Removal				
	DeltaV	150.3	75.1	300.5	Based on translating 2002 profiles +11.25 ft/year. (BEG long-term shoreline change through 2007). Assume - value = 50%, + = 200%
	Residual	0.0	0.0	0.0	
Galv. Sea Wall	Q _{source1}				
Cell 1_6	Q _{sink1}				
35,300 ft long	Q _{source2}				
	Q _{sink2}				
	Q _{source3}				
	Q _{sink3}				
	Q _{source-LST1}				
	Q _{sink-LST1}	41.2	35.5	59.9	Assume 50% sed. moves to east
	Q _{source-LST2}				
	Q _{sink-LST2}	41.2	35.5	59.9	Assume 50% sed moved to west
	Placement	59.7	59.7	74.7	1985-2008 placements (exclude 2009 post-Ike fill), data from John Lee., Jr., Coastal Strategies Group, LLC. Additional placements possibly not recorded, assume + value = 125%
	Removal				
	DeltaV	-22.6	-11.3	-45.2	Based on translating 2002 profiles - 0.72 ft/year. (BEG 85-year shoreline change through 2007 for original part of the seawall). Assume - value = 50%, + = 200%
	Residual	0.0	0.0	0.0	
Galv State Park	Q _{source1}				
Cell 1_7	Q _{sink1}				

49,500 ft long	Q _{source2}				
	Q _{sink2}				
	Q _{source3}				
	Q _{sink3}				
	Q _{source-LST1}	41.2	35.5	59.9	From Cell 1_6
	Q _{sink-LST1}	0.0	0.0	0.0	Assume no east movement
	Q _{source-LST2}				
	Q _{sink-LST2}	166.3	105.6	299.0	To Cell 1_8
	Placement	15.0	15.0	18.8	Placements at Sunny Beach, Sands of Kahana, Spanish grant, and Bermuda Beach. Additional placements possibly not recorded, assume + value = 125%
	Removal				
	DeltaV	-110.2	-55.1	-	Based on translating 2002 profiles - 3.39 ft/year. (BEG long-term shoreline change through 2007). Assume - value = 50%, + = 200%
	Residual	0.0	0.0	0.0	
Cell 1_8	Q _{source1}				
18,700 ft long	Q _{sink1}				
	Q _{source2}				
	Q _{sink2}				
	Q _{source3}				
	Q _{sink3}				
	Q _{source-LST1}	166.3	105.6	299.0 42	From Cell 1_7
	Q _{sink-LST1}	0.0	0.0	0.0	Assume no east movement
	Q _{source-LST2}				
	Q _{sink-LST2}	191.3	120.6	345.1 22	To cell 1_9
	Placement	5.0	5.0	6.3	Placement at Sea Isle. Assume possible unrecorded placement = 125%
	Removal				
	DeltaV	-19.9	-10.0	-39.8	Based on translating 2002 profiles - 1.59 ft/year. (BEG long-term shoreline change through 2007). Assume - value = 50%, + = 200%
	Residual	0.00	0.00	0.00	
West Beach	Q _{source1}				
Cell 1_9	Q _{sink1}				
23,800 ft long	Q _{source2}				
	Q _{sink2}				
	Q _{source3}				
	Q _{sink3}				
	Q _{source-LST1}	191.3	120.6	345.1	From Cell 1_8
	Q _{sink-LST1}	0.0	0.0	0.0	Assume no east movement
	Q _{source-LST2}				
	Q _{sink-LST2}	251.6	159.3	453.1	To cell 1_10
	Placement	17.0	17.0	21.3	Placement at Terramar Beach and

					Pointe San Luis
	Removal				
	DeltaV	-43.4	-21.7	-86.8	Based on translating 2002 profiles - 2.74 ft/year. (BEG long-term shoreline change through 2007). Assume - value = 50%, + = 200%
	Residual	0.0	0.0	0.0	
San Luis Pass E.	Q _{source1}				
Cell 1_10	Q _{sink1}				
5,000 ft long	Q _{source2}				
	Q _{sink2}				
	Q _{source3}				
	Q _{sink3}	240.1	153.5	430.0	Into San Luis Pass flood shoal
	Q _{source-LST1}	251.6	159.3	453.1	From Cell 1_9
	Q _{sink-LST1}	0.0	0.0	0.0	Assume no east movement
	Q _{source-LST2}				
	Q _{sink-LST2}	0.0	0.0	0.0	Assume no west movement (all material into inlet)
	Placement				
	Removal				
	DeltaV	11.6	5.8	23.2	2002 profile G002 translated 3.68 ft. Profile G001 not useable
	Residual	0.0	0.0	0.0	
San Luis Pass	Q _{source1}				
Flood Shoal	Q _{sink1}				
Cell 1_11	Q _{source2}				
	Q _{sink2}				
	Q _{source3}	240.1	153.5	429.9 7	From open coast Cell 1_10
	Q _{sink3}				
	Q _{source-LST1}				
	Q _{sink-LST1}				
	Q _{source-LST2}	?	?	?	Unknown input from cell south of pass
	Q _{sink-LST2}				
	Placement				
	Removal				
	DeltaV				
	Residual	240.1	153.5	430.0	INCOMPLETE, need flood shoal growth data

Appendix D: Aeolian Transport Methodology

Chapter III-4 of the *Coastal Engineering Manual* (Hsu and Weggel 2002) provides a methodology for computing wind-blown transport. The procedure is lengthy and requires building a spreadsheet to complete the calculations. A portion of the chapter is quoted in Text Box D1, and the following paragraphs describe how the procedure was applied to East Beach and Big Reef. All calculations were done in metric units.

III-4-4. Procedures for Calculating Wind-Blown Sand Transport

The steps for calculating wind-blown sand transport on beaches follow.

- a. Obtain hourly average wind speed and direction data. (Wind data tabulated at intervals less frequent than 1 hr may be used in lieu of hourly data; however, hourly data are preferable.)
- b. Obtain daily precipitation data and monthly evaporation records from a nearby National Weather Service (NWS) station. (These data are available in "Climatological Data" summaries published monthly for each state by the National Climatic Data Center (NCDC), Asheville, NC).
- c. Obtain the density and median grain size of the beach sand at the study site.
- d. Compute the critical shear velocity u_{*t} for the mean grain diameter using Equation 4-20.
- e. Compute the critical wind speed at the 2-m height U_{2mt} using Equation 4-8 with the value of u_{*t} computed under Step 4 above. (This is the wind speed measured at the 2-m height that can initiate sand transport.)
- f. Shear velocity u_* is relatively independent of height up to a height of about 50 m above ground level; therefore, Equation 4-7 can be used to compute the critical wind speed at any height above the ground using the U_{2mt} and u_{*t} . (For example, let $Z_1 = 2$ m, $U_{Z_1} = U_{2mt}$, $Z_2 =$ the height at which the available wind measurements were taken, and solve for $U_{Z_2t} =$ the critical wind velocity at the Z_2 height.) Only wind speeds in excess of the computed U_{Z_2t} will result in sand transport.
- g. If wind speeds exceed the critical value and there was no precipitation on a given day, compute the potential sand transport rate using Equation 4-16 or Equation 4-18.
- h. If there was precipitation on a given day, the amount of precipitation should be compared with the amount of evaporation. If evaporation exceeds precipitation, compute the potential sand transport rate using Equation 4-16 or 4-18. (If daily evaporation data are not available, daily evaporation can be estimated by dividing monthly evaporation by the number of days in the month.)

Step a. Hourly wind and other observations were available from two National Oceanic and Atmospheric Administration (NOAA) sites near East Beach.

- Dec. 12, 1996 - Dec. 29, 2008: Pleasure Pier at Galveston, Station 8778710, located offshore of the seawall. Before 2009, meteorology data could be downloaded from the [tidesandcurrents.noaa.gov](http://tidesandcurrents.noaa.gov/data_menu.shtml?stn=8771510%20Galveston%20Pleasure%20Pier.%20TX&type=Meteorological+Observations) web page: http://tidesandcurrents.noaa.gov/data_menu.shtml?stn=8771510%20Galveston%20Pleasure%20Pier.%20TX&type=Meteorological+Observations (accessed Jan 2009). NOAA removed the meteorology instruments on 20 Jul 2011, and the meteorology data is no longer available. In 2009, hourly data year-by-year as ASCII text files was saved and imported each year into Microsoft Office Excel 2003 spreadsheets.
- 1 Jan 2009 – 31 Dec 2010: Pleasure Pier at Galveston, Station 8778710. NOAA provided the data by special request because the NOAA web page was not operating correctly.

Step b. Precipitation and evaporation data are available online from the National Climatic Data Center at site:

<http://www.ncdc.noaa.gov/data-access/land-based-station-data/data-publications> (accessed 12 Nov 2015).

Users need to select “Climatological Data (CD)” and proceed to the next page, where they select the state of interest and then month of interest. The reports are available free to users with .gov or .mil email addresses. The climatological data reports are in the form of Adobe PDF files. Each monthly report for the State of Texas is approximately 95 pages long.

The Galveston National Weather Service (NWS) station reported data intermittently was therefore unsuitable for the analysis. The closest NWS station with both rainfall and evaporation data was the Beaumont Research Center, operated by Texas A&M University, Index 0613, Division 08, County Jefferson. It is located at 94°18' W, 27°08' N. The 24-hour day periods when rainfall exceeded evaporation were coded as “Y” in the spreadsheet while all other days were dry or “N”.

Step c. The median grain size used for the analysis was 0.116 mm, based on the average of four surface samples collected on profile lines G051 and G058 in 2002 by Texas A&M University¹.

- Line G051: B₂ = 0.119; I₂ = 0.119; W₂ = 0.119; Ave. = 0.118
- Line G058: B₂ = 0.106; I₂ = 0.12; W₂ = 0.117; Ave = 0.114

Use a density of 2.65 gm/cm³ for quartz sand.

Step d. The critical shear velocity for 0.116 mm quartz sand was 0.185 m/sec.

Step e. The critical shear at the 2 m height was 4.21 m/sec, using the results from Step d above. This is computed from the roughness relationship $U^* = 0.044 U_{2m}$ from Hsu (1977), applicable to bare or un-vegetated beach sand.

Step f. The height at which the available wind measurements were taken, Z_2 , was set at 9.0 m. The anemometer height at Pleasure Pier was 11.5 m above the water, with the assumption that this referred to MLLW. This equals approximately 11 m above mean sea level (MSL). It was then assumed that the average elevation of the dry sand on East Beach and Big Reef was 2 m above MSL. Therefore U_{z2t} , the threshold for sand movement, was 4.90 m/sec. A test was made with assumption that East Beach was only 1 m above MSL and Z_2 set at 10.0 m. Then, the resulting calculated transport for 2007 was only approximately 4% less than using $Z_2 = 9.0$ m. There is no need to recalculate the values for the 1 m MSL beach elevation.

Table D1 is a list of coefficients and calculated parameters used in the analysis.

Table D1. Galveston aeolian transport parameters.

Parameter	Value
D = median sand size, mm	0.116
U_{*1} = initiation of sand transport, m/sec	0.185
U_{2mt} = threshold wind speed at 2 m height, m/sec	4.21

¹ Billy Edge, Texas A&M University, personal communication, 14 Nov 2003.

Parameter	Value
U_{9mt} = threshold wind speed at 9 m measurement height, m/sec	4.9
K = dimensionless aeolian sand transportation coef., gm/cm-s	0.000116
q_v = volumetric sand transport = $1.026 \times 10^{-10} U_{9m}^3$ in $cm^3/cm\text{-sec}$	1.03E-10
q = transport = $1.631 \times 10^{-10} U_{9m}^3$ in gm/cm-sec	1.63E-10
Θ = jetty azimuth in degrees	9
Length of south jetty crossing bare sand beach (m)	720
Wind velocity and direction station	NOAA Sta.8771510 Galveston Pleasure Pier
Precipitation and evaporation measurement station	Beaumont Research Ctr.
Time zone	GMT

Step g. The spreadsheet was coded so that sand transport was calculated if $U_{z2t} > 4.90$ m/sec and rainfall = N.

Step h. This step was simplified for the Galveston Island project area. During days with precipitation in Galveston, total rainfall was 1 in. or more, which is greater than the evaporation. Therefore, days were either wet or dry, and using Equations 4-16 or 4-18 was an extra calculation that was not necessary.

Step i. Wind direction was rotated 180° to show the direction that the wind was blowing. The angle theta (Θ) for this portion of the jetty was 9° from true north. Therefore, the north-south component of the wind carrying sand across the jetty was $\cos \Theta$.

Step j. The length of bare (nonvegetated) sand crossed by the south jetty was 720 m. This was used as a multiplier factor for hourly transport across jetty q_v (m^3/hr).

Step k. Hourly transport was summed to tabulate monthly values. Then, monthly values were summed to produce annual statistics.

The following figures are annual plots of wind-blown sand transport. Units are in metric (m^3) as per the original computations.

Figure D1. Aeolian sand transport for Galveston Big Reef 1997.

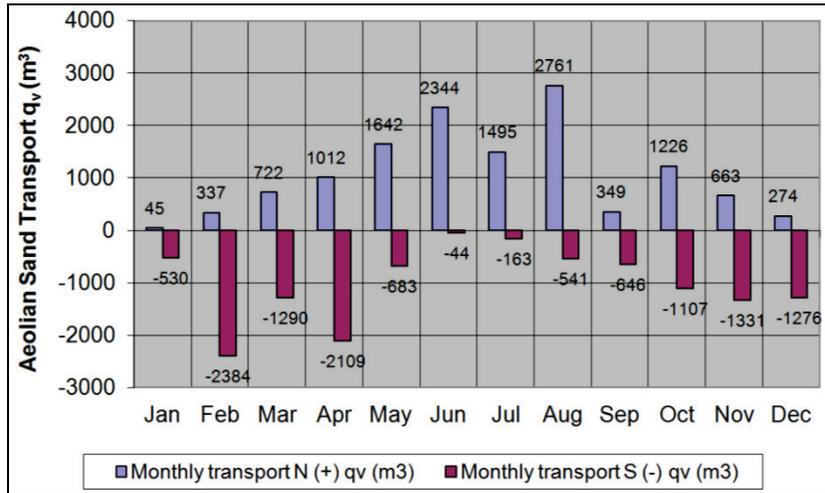


Figure D2. Aeolian sand transport Galveston Big Reef 1998.

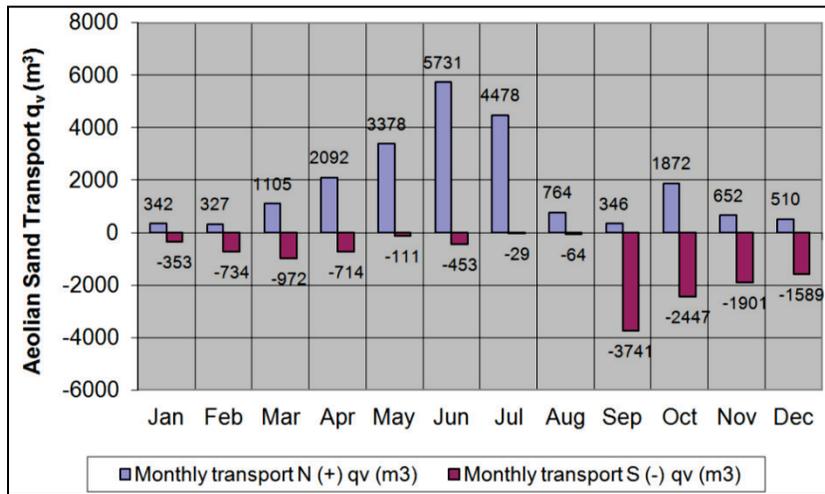


Figure D3. Aeolian sand transport Galveston Big Reef 1999.

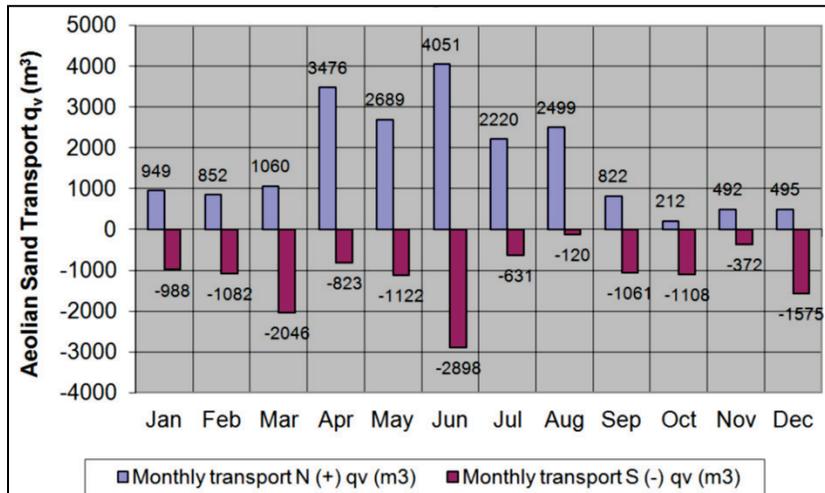


Figure D4. Aeolian sand transport Galveston Big Reef 2000.

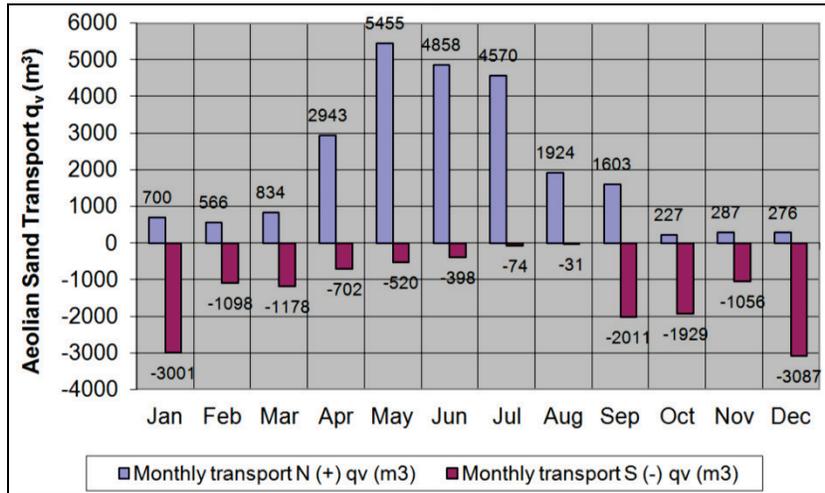


Figure D5. Aeolian sand transport Galveston Big Reef 2001.

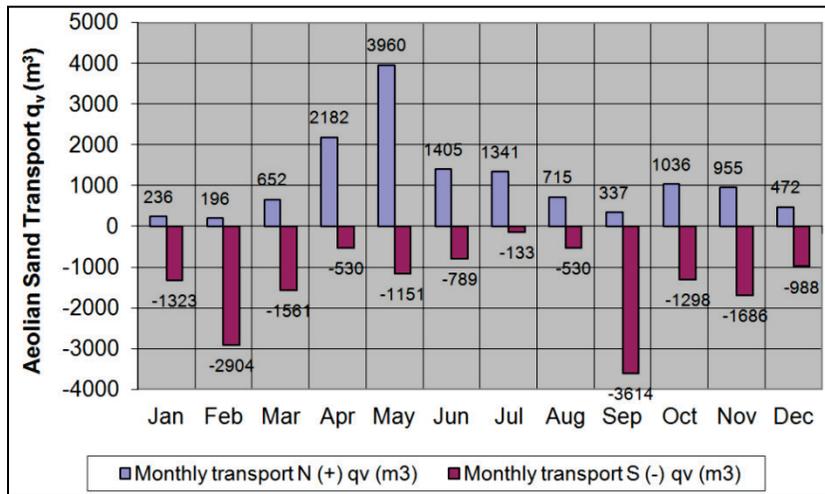


Figure D6. Aeolian sand transport Galveston Big Reef 2002.

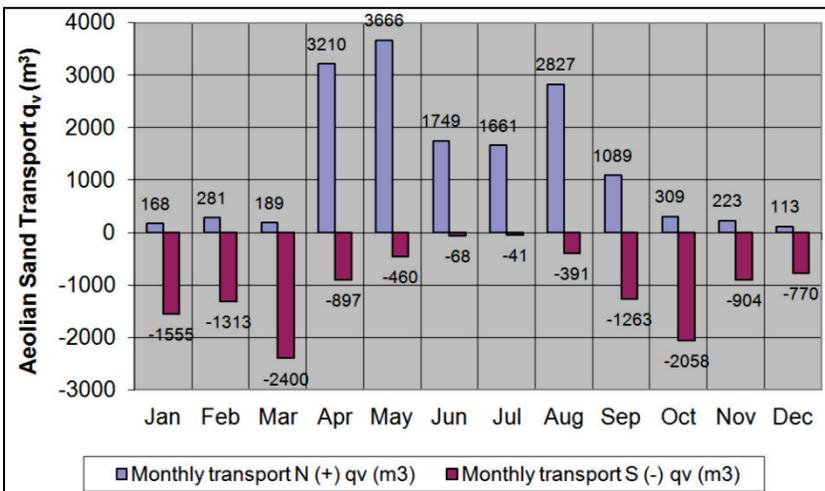


Figure D7. Aeolian sand transport Galveston Big Reef 2003.

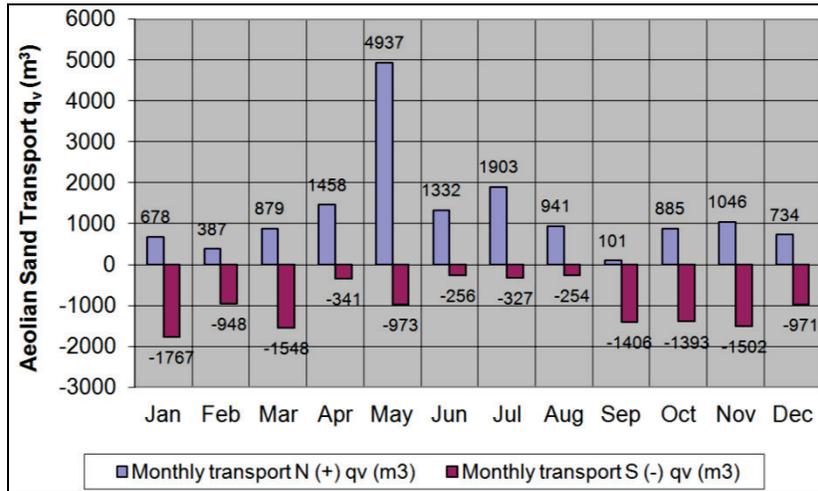


Figure D8. Aeolian sand transport Galveston Big Reef 2004.

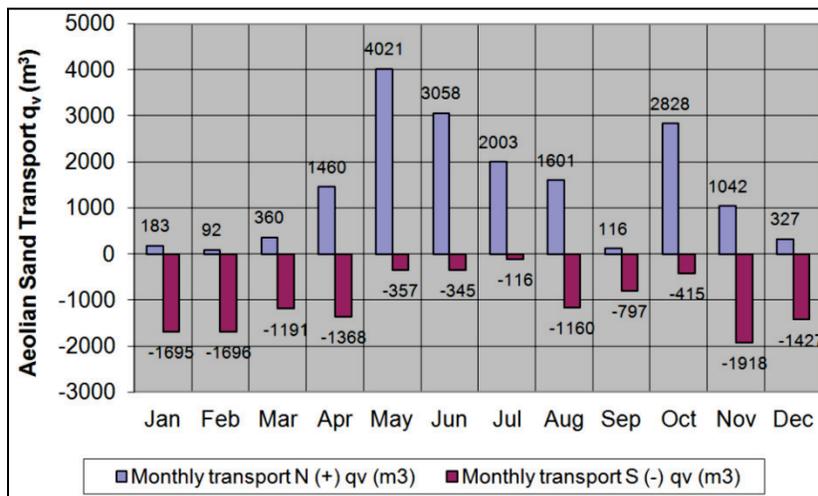


Figure D9. Aeolian sand transport Galveston Big Reef 2005.

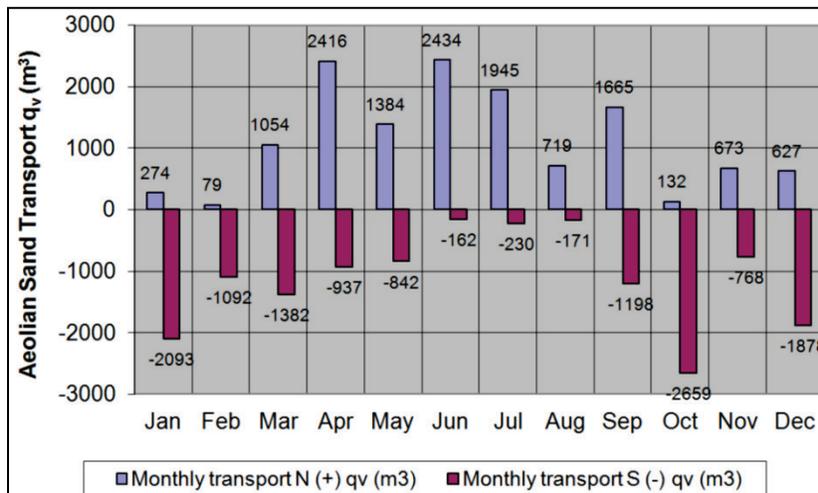


Figure D10. Aeolian sand transport Galveston Big Reef 2006.

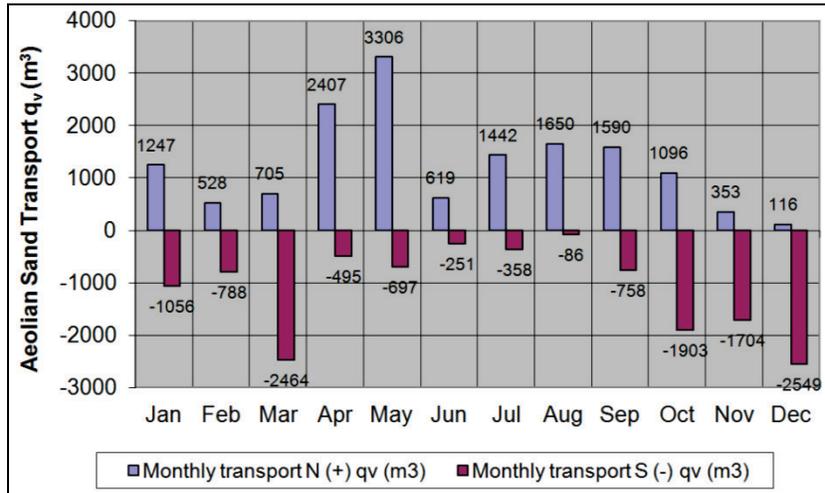


Figure D11. Aeolian sand transport Galveston Big Reef 2007.

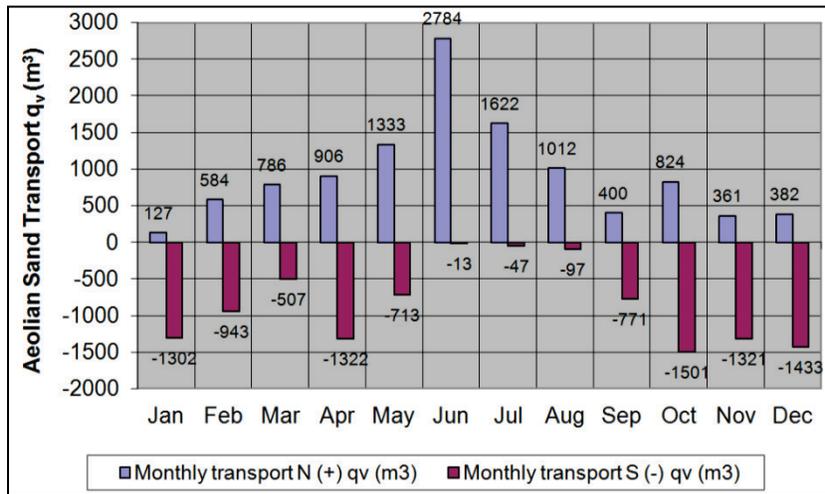


Figure D12. Aeolian sand transport Galveston Big Reef 2008.

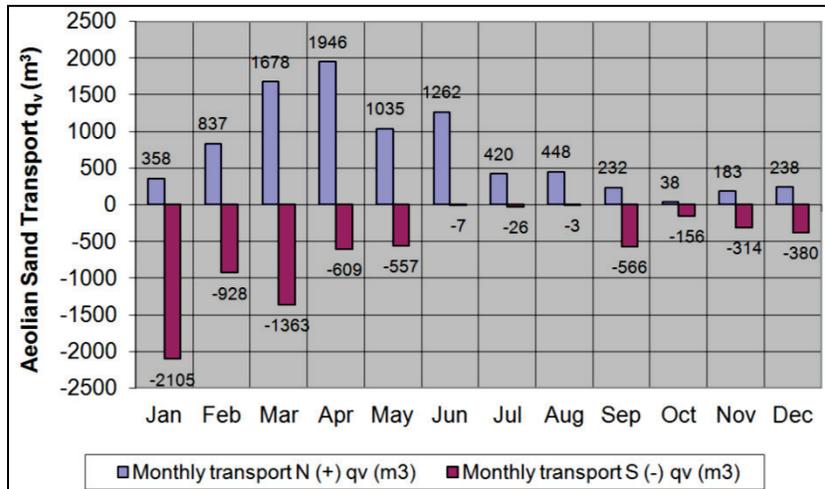


Figure D13. Aeolian sand transport Galveston Big Reef 2009.

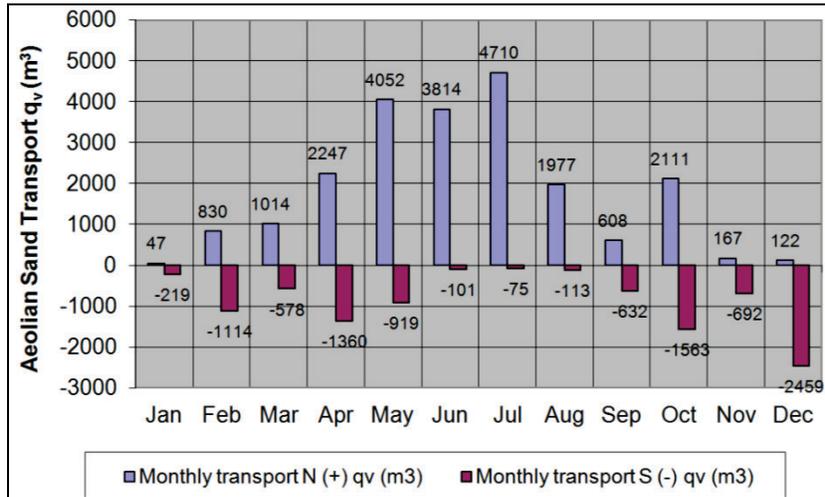
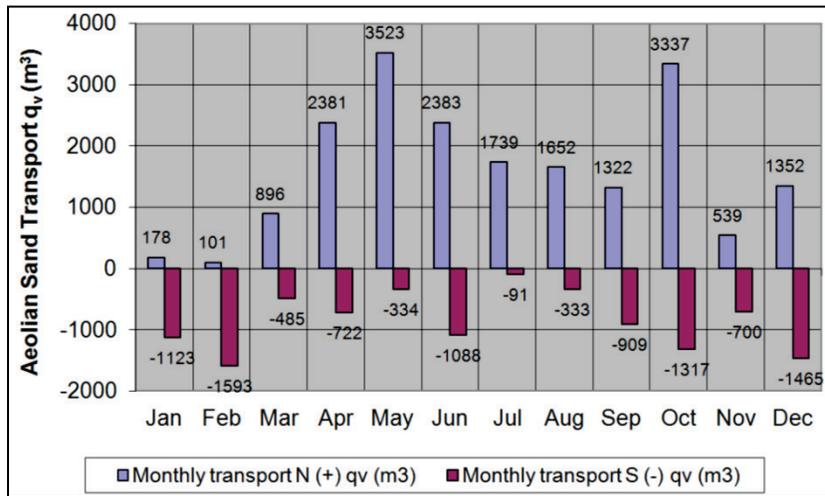


Figure D14. Aeolian sand transport Galveston Big Reef 2010.



Appendix E: Beach Fill Volumes

Table E1 lists calculations used to determine the amount of sand needed to complete a proposed beach fill along the Galveston Seawall. Figure E1 shows profile locations referenced in the table. Table E2 lists volumes for proposed 200 ft wide berm along the entire island with and without a dune feature, divided into Park Board reaches. Advanced fill is estimated at 50%.

Figure E1. Texas A&M 2002 cross-shore profiles.

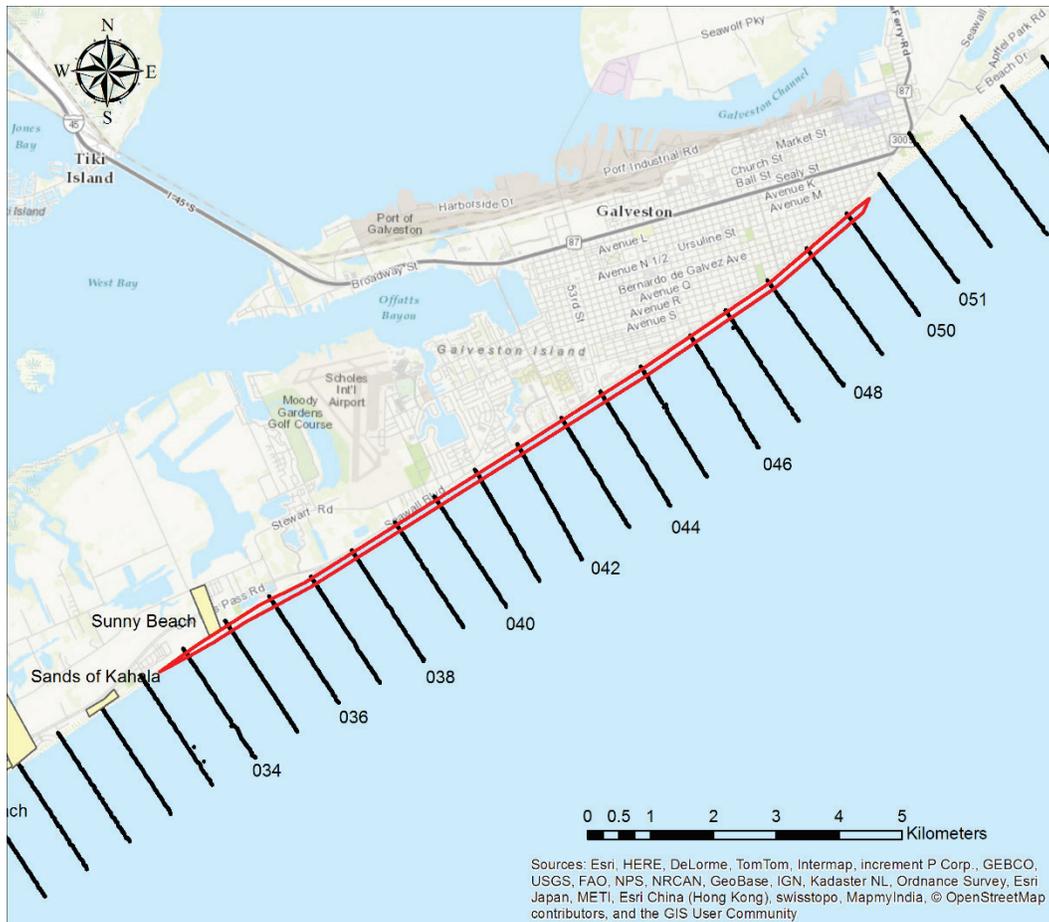


Table E1. Beach fill volumes for seawall (Reaches 1 and 2).

Texas A&M Profile Number	Option 1 (dune and 200 ft berm/beach, 270 ft wide total)			Option 2 (200 ft berm/beach only)		
	Total Volume (yd ³ /ft)	Profile Dist. (ft)	Sand Volume (yd ³)	Total Volume (yd ³ /ft)	Profile Dist. (ft)	Sand Volume (yd ³)
50	219.56	2607	572,400			
49	92.66	2607	241,600	135.84	2607	354,100
48	100.86	2607	262,900	11.86	2607	30,900
47	112.86	2607	294,200	18.57	2607	48,400
46	88.96	2607	231,900	29.30	2607	76,400
45	94.06	2607	245,200	7.42	2607	19,300
44	36.06	2607	94,000	11.85	2607	30,900
43	107.66	2607	280,700	0.00	2607	0
42	222.36	2607	579,700	24.35	2607	63,500
41	229.66	2607	598,700	138.65	2607	361,500
40	214.76	2607	559,900	145.68	2607	379,800
39	227.86	2607	594,000	130.64	2607	340,600
38	223.76	2607	583,300	143.41	2607	373,900
37	175.66	2607	457,900	138.92	2607	362,200
Total			5,596,400			2,691,000

Table E2. Beach fill volumes for comprehensive fill (Reaches 1 to 5).

Profile number	Zero dist. on profile	X for BMAP	Beach fill vol. for translated profile below zero (yd ³ /ft)	Template vol above zero (yd ³ /ft)	2002 pro-file above zero (yd ³ /ft)	Above zero vol. needed (yd ³ /ft)	Tot vol (yd ³ /ft)	Profile dist. (ft)	Sand vol Option 1 (yd ³)	Sand vol (yd ³)
Reach 1										
044-050 (summarized from Table E1)									1,942,200	560,000
Reach 2										
037-043 (Summarized from Table E1)									3,654,200	2,131,600
Reach 3										
026-036 averaged	230	70	54.5	59.259	29.8	29.46	83.96	30000	2,518,800	2,518,800
Reach 4										
007-025 averaged	230	70	52.3	59.259	26.3	32.96	85.26	51700	4,407,900	4,407,900
Reach 5										
002-006 averaged	580		0	59.259	20.9	38.36	38.36	12300	471,800	471,800
Total for all reaches									12,994,900	10,090,100
Advance nourishment (assume 50%)									6,497,500	5,045,100
Total volume including advance									19,492,400	15,135,200

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14. ABSTRACT The Coastal and Hydraulics Laboratory, Engineer Research and Development Center, and the U.S. Army Engineer District, Galveston, conducted a study to support a sand management strategy for the Galveston Park Board of Trustees of Galveston, TX. The long-term management strategy encompasses not only Galveston Park Board of Trustees managed areas, but the entire shoreline of Galveston Island. In the first phase of the project, a sediment budget was recomputed and GenCade, a numerical model, was calibrated for Galveston Island. After discussing potential solutions and actions with the Park Board, engineering analyses and numerical modeling were conducted to quantify the performance of each selected alternative. The long-term solution is a wide beach along Galveston Island that is filled through beach nourishment and backpassing plants on both ends of the island. This solution will require a large volume of sand for the initial construction; therefore, sand management solutions were identified and potential offshore sand sources were identified. Shorter-term and smaller-scale beach nourishment activities were also provided as options within the strategy.					
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