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SUMMARY OF COMMERCIAL WASTE WATER TREATMENT PLANTS

James A. Mahonev

TECHNICAL REPORT NO. AFWL-TR-69-121

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FOREWORD

This report was prepared under Program Element 63723F, Subtask 3.1.001. Inclusive dates of research were 1 January 1969 to 15 August 1969. The report was submitted 31 August 1969 by the AFWL Project Engineer, Mr. James A. Mahoney (WLDC-TE).

The assistance of the following personnel are gratefully acknowledged: Donald G. Silva, Major, USAF, Chief Environics Unit, Bertrand F. Ruggles, Captain, USAF, and Dennis I. Hirota, Captain, USAF, all of the Environics Unit of the Civil Engineering Branch.

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ABSTRACT

The purpose of the report is to catalog and review capabilities of commercially available packaged waste water treatment plants for possible Air Force application in limited war and fixed base installations. Definitions of the various methods of treatment are included. Simple curves are furnished for a quick estimate of costs, power requirements, erection time, shipping volume and weight in relation to flow capacity. Included are tables which relate the performances of a number of places tested by the National Sanitation Foundation, by the manufacturer, and by the Florida State Board of Health. A list of firms which manufacture packaged waste water treatment plants is included in an appendix.

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SECTION I

INTRODUCTION

The purpose of this report is to catalog and review capabilities of commercially available packaged waste water treatment plants for possible Air Force application in limited war and fixed-base installations.

A packaged treatment plant is the customized combination of standard components selected by an engineer as a part of an overall waste-water system design. There is a fallacy in using the term "packaged plant" as it refers to a wide variety of fabricated treatment units. There are prefabricated factory-built plants, that handle flows up to 100,000 gallons per day (GPD). Then there are larger plants whose component parts are factory fabricated but field erected and handle flows in the millions of gallons per day.

Although packaged plants are broadly grouped into model or design series, few installations are exactly alike. Customizing of the selected basic plant may then range from the simple selection of accessory items, (comminution or chlorination equipment), to the solution of a complex sanitary engineering design problem resulting in a unique treatment plant composed of standard fabricated parts. Therefore, the design of a "packaged plant" by an engineer is analogous in all essential respects

to his selection and utilization of particular blowers, diffusers, sludge collectors, or pumps in the creation of a conventional basic biological treatment plant. (Ref. 1)

A letter from Hq PACAF dated 22 August 1968 (Appendix I) requested assistance in the area of sanitary treatment of domestic waste water in limited-war operations. The requirements stated were for a packaged treatment plant to serve 2,000 to 3,000 personnel, having the following criteria: transportability, availability, ease of erection, simplicity of operation and a biochemical oxygen demand (BOD) reduction of 90 percent.

From January 1969 to July 1969, the Civil Engineering Branch of the Air Force Weapons Laboratory canvassed manufacturers of packaged waste water treatment plants. A "Product Questionnaire" (Appendix II) was sent to approximately 60 manufacturers. Included in those receiving the questionnaire were 12 manufacturers that had models of their product certified by the National Sanitation Foundation (NSF), Ann Arbor, Michigan. Eight of the firms responded with a copy of the official certification.

The manufacturers response to the questionnaire was very poor. A number of those firms tilling out the form partially answered the different items. A few answered the questionnaire fully. Many firms sent their catalogues and data sheets on their various models without filling out the questionnaire and the data had to be extrapolated.

Information and data used as inputs to this report were obtained from four sources: (1) the "Product Questionnaire," (2) the NSF certification booklets, (3) the Journal of Sanitary Engineering Division, American Society of Civil Engineers, and (4) the manufacturers catalogues. Because the information had to be extracted from so many sources, exact comparisons of the various brands of plants was not made. The information is reported as obtained and presented for the reader to exercise his own judgment.

The National Sanitation Foundation is an independent testing agency, which has the responsibility to develop standard criteria to evaluate the performance of packaged sewage treatment plants. To aid the NSF in their work, the Demonstration Grants Committee (now a part of the Federal Water Pollution Control Administration) of the U.S. Public Health Service approved and gave the Foundation's application a high priority. Subsequently, the Foundation was given a grant, WPD-74, which afforded operating funds over a three year period. (Ref. 2) Fifteen firms sent models of their plants for performance testing in accordance with the Foundations standards. Of these 15 plants, the Foundation has certified 12 plants at the time of this writing. The certification data are the property of the manufacturer of the plant tested and these data can only be released or reproduced by his permission. The manufacturer has an agreement with the Foundation to present

the certification data in its entirety, whenever it is used in advertising, prospectus, bids or other similar uses.

Although most of the plants tested by the Foundation are low flow capacity plants (3,000 to 16,000 GPD), the NSF has determined through its criteria development program that larger plants of a similar design will produce equal or better results and smaller plants will produce somewhat lesser results than any given plant in the series. (Ref. 3)

Before the NSF assumed responsibility for certifying packaged plant performance, various state boards of health became alarmed at the increased use of packaged plants, and developed their own criteria to evaluate plant performance. The Florida State Board of Health was one which established its own criteria. The Board, in the mid-fifties, established conditions for experimental, extended aeration plants. It is interesting to note that one of the conditions was a satisfactory surety bond to be posted by the manufacturer. It was believed that the bond should cover the cost of the waste-water treatment plant so that if satisfactory performance were not obtained, the purchaser would be reimbursed to provide for purchase of another plant. The bonds were written to cover expected efficiency at a given flow and waste strength (usually 200 mg/1 BOD₅ and 200 mg/1 suspended solids). After a year of operation, the Board felt that sufficient data had been collected to lift the surety bond on four manufacturers' plants. (Kef. 4)

The Ohio University, Athens, Ohio, received a U.S. Demonstration Committee Grant to collect data from an oxidation ditch at Sumerset, Ohio. Performance data were obtained over a nine-month period. (Ref. 5)

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SECTION II

DEFINITIONS AND OPERATIONAL CRITERIA

The range of packaged plants, (Tables I and II) is from the size for a single residence (600 GPD) to very large plants of 3 million GPD flow capacity. The methods used for treatment normally are extended aeration and contact stabilization. The extended aeration models are made in sizes up to 100,000 GPD and the contact stabilization plants normally start at 50,000 GPD and up. Most extended aeration plants are preassembled at the factory, but the larger contact stabilization plants are field erected.

There are also other modified forms of the activated sludge process that are classified in the complete mix, longterm aeration group. For the purpose of this report they will be grouped in a class called "Special Devices."

I. Extended Aeration is defined as those total oxidation plants which are designed to maintain about 10 pounds of suspended solids under aeration per pound of average daily biochemical oxygen demand (BOD_5) entering the aeration tanks. At such a high solids to ROD_5 ratio, verv little excess sludge is produced. (Ref. 2)

Table I. Summary of ('ommercial Extended Aerat	ion Package Sewage Treatr	ment Plants.	
MANUFACTURER	MODEL DESIGNATION OR SERIES	TYPE PROCESS	FLOW CAPACITY (GPD)	REMARKS
Aer-()-Flo Corp.	Model "S"	Extended Aeration	1,000-100,000	Factory prefabricated 45 models
Can-Tex Industries	Tex-A-Robic (EA)	Extended Aeration	5,000-50,000	Factory prefabricated
Chicago Pump	Rated Aeration H-Shear	Extended Aeration	1,000-5,000	Factory prefabricated 3 models
	SL-131 Rated Aeration	Extended Aeration	17,500-37,500	Factory prefabricated
	SL-144 Completaire	Mixed Modification of Activated sludge	15,000-75,000	Factory prefabricated duplex up to 150,000 GPD
	SL-138 & SL-139 Rated Aeration- Stepaire	Step Aeration	100,000-500,000	Field erected
13VC0	6DA2-12DA40	Extended Aeration	2,000-40,000	
l'effance Company	1.65EA-40EA	Extended Aeration	1,650-40,000	Factory prefabricated Multiple unit for larger systems
l'orr-01tver	CompleTreater		15,000-240,000	lises Biofilter, Duo Clarigester screening and chlorination.
	CompleTreater- Aeration Plant	Extended Aeration or Contact Stabilization	100,000-500,000	Can be used with either method.

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MANUFACTURER	WODEL DESIGNATION	TYPE PROCESS	FLOW CAPACITY	REMARKS
	UN DENTED			
l)r avo	Aeropack	Extended Aeration or Contact Stabilization	20,000-1,000,000	Sludge Digester used.
Eimco Corp.	Type ADC	Aerobic Digestion	2,000-1,000,000	
	Type ADR	Aerobic Digestion	1,000-15,000	
Infilco Incorporated	Accelo-Biox	Extended Actation	7,500-300,000	Combination of mechani- cal and compressed air aeration
⁴ Likeside Engineering Co.	Spira-Pac	Extended Aeration	Up to 60,000	
	Various Model Numbers	Extended Aeration & Contact Stabilization	2,500-3,000,000	
	Oxidation Ditch	Extended Aeration	10,000-3,000,000	
Lyco Systems Incorpo- rated		Extended Aeration & Contact Stabilization	1,500-1,000,000	Multiple unit hook-up field erected.
Mack Sevage Treatment Systems, Incorporated	Mack	Aerobic Digestion	1,500-40,000	Add-on to increase capacity.
Marolf		Extended Aeration	2,000-21,000	Parallel add-on to increase capacity.
Neptune Microfloc Inc.	Recla-Pak	Extended Aeration	5,000-50,000	Increased capacity by parallel add-on.

WILL FACTUREP	MODEL DESIGNATION OR SERIES	TYPE PROCESS	FLOW CAPACITY (GPD)	REMARKS
Water Pollution Control	Mark I	Extended Aeration	2,000-35,000	Factory assembled
(Sanitaire)	Mark II	Extended Aeration	1,000-10,000	Factory assembled
Sch mie g Sydnor Hydor- dynamics	Centri-Swirl	Extended Aeration	1,000-100,000	30,000 GPD largest single unit
Smith and Lovelace	Oxigest A & B	Extended Aeration	6,000-35,000	Parallel add-on to increase capacity.
	Oxigest C & D	Extended Aeration	რ , ეიი-35, <mark>იი</mark> ი	Parallel add-on to increase capacity.
	Oxigest CY Addifest	Extended Aeration Extended Aeration	2,000-10,000 18,000-90,000	Vield crected
l.inb-kelt	Contac-Pac	Extended Aeration	500-60 , 000	
Yeomans Brothers	Cavitette	Extended Aeration	Single Residence	Modified aeration
	Yeo-Wave	Extended Aeration	3,000-12,000	10 sizes
Permucic	Ameodyne Products	Extended Aeration & Contact Stabilization	1,000-30,000	23 sizes
Komline-Sanderson		Extended Aeration	Up to 200,000	
American Bouser Corp.	0xy-Pak	Extended Aeration	1,000-300,000	W/"Aircomb" diffuse

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WUFACTURER	MODEL DESIGNATION OR SETIES	TYPE PROCESS	FLOW CAPACITY (GPD)	KEMARKS
Merican Padiator 6 tandary Corporation	Bio-Con	Extended Aeration	20,000-80,000	
uburbla System, scorborated	Suburbia	Extended Aeration	50-1500 homes	Predesigned steel components

Table II. Summary of	Corraercial Contact Stabi	llization Package Sewage 1	freatment Plants.	
MANUFACTURER	MODEL DFC GNATION OR SEFIES	TYPE PROCESS	FLOW CAPACITY (GPD)	PEUARKS
her-a-flo Corp.	Model "CS"	Contact Stabilization	იიი, ი ია-იიი, იგ	Field erected, 46 models
Can-Tex Industries	Tex-A-Pohic (CS)	Contact Stabilization	30,000-50,000	Factory prefabricated
	Tex-A-Robic (RC)	Contact Stahilization	50,000-1,250,000	Ficld erected
Chilcago Pump	SL-150 Stahilaire	Confact Stabilization	20,000,0<-000,02	Factory prefabricated, dunlex un to 100,000 GPD.
	SL-144 Completaire	^w ixed Modification of Activated Sludge	15,000-75,000	Factory prefabricated, duplex up to 150,000 GPP.
	SL-138 & 139 Aeratio Stepaire	on Step Aeration	100,002-000,000	Field erected
	111:Ac 20-12DAc 70	Contact Stabilization Contact Stabilization	20,000-70,000 50,000-500,000	Field erected
Befinance Company	20CS-10CS	Contact Stabilization	20,000-100,000	
?wrr-01iver	CompleTreater- Aeration Plant	Contact Stabilization or Extended Aeration	100,000-500,000	
Dravo	Аегораск	Contact Stabilization & Extended Aeration	20,000-1,000,000	Sludge Digestor
F.1.mco	Type CSC	Contact Stabilization	20,000-1,000,000	

Table II. Summary of Com	mercial Contact Stabi	lization Package Sewage 7	Creatment Plants Con	tinued.
MANUFACTURER	MODEL DESIGNATION OF SERIES	TYPE. PROCESS	FLON CAPACITY (GPD)	REMARKS
Lakeside Engineering Co.	Various Models	Contact Stabilization 6 Extended Aeration	2,500-3,000,000	
Lyco Systems Incorporated		Contact Stabilization & Extended Aeration	1,500-1,000,000	Multiple unit add-on, field erected.
Water Pollution Control Corp.	Mark III	4 activated sludge	80,000-2,000,000	Field erected
(Sanitaire)	Mark IV	process Multi process	Up to 100,000	Field erected
Smith and Loveless	Oxigest R	Contact Stabilization	50,000-1,000,000	Field erected
Walker Process Equipment	Sparjair	Contact Stabilization	5,000-100,00	
Hays Process Company	Hays Submerged CA	Contact Stabilization	1,000-40,000	
Fermut 1 t	Ameodyne Products	Contact Stabilization & Extended Aeration	Ι, 000-ζη,000	23 sizes
Water and Sewage, Inc.	Nautilus	Contact Stabilization	Up to 10,000	

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The basic features of all packaged extended aeration sewage plant are the aeration compartment and the final settling or clarification compartments shown in Figures 1 and 2.

Raw sewage flows directly into the aeration compartment without having been subjected to primary sedimentation. The raw sewage is mixed with the return sludge to form the mixed liquor within the aeration chamber and is aerated for approximately 24 hours. When the aeration period is completed, the mixed liquor flows into the sedimentation chamber where the suspended solids and liquids are separated by gravity. The separated solids are returned to the aeration compartment as returned sludge while the liquid flows out of the plant as the effluent.

Accessories that may be used are a bar screen, comminutor, sand filter, or a lagoon (to further stabilize the plant effluent before final disposal), and a chlorinator. Commonly included are foam control sprays and skimmers to remove floating material from the surface of the sedimentation compartment. (Ref. 2)

Table I includes the different sized treatment plants that utilize the extended aeration principle for treating domestic waste waters.

II. Biosorption or contact stabilization is defined as a form of the activated-sludge process where aeration is carried out in two phases in two types of tank: the contact tank where raw sewage solids





are absorbed and adsorbed on the biomasses; and the stabilization tank where the solids which have been removed in a final settling tank are partially stabilized by reaeration before being recombined with the incoming raw sewage. An aerobic digestion tank may be included as a part of the process. (Ref. 6)

The contact stabilization plant process has three compartments under aeration. Kaw sewage flows into the contact aeration compartment and is mixed at the entrance with reaerated or stabilized activated sludge. The mixed liquor is kept under aeration and remains in the contact aeration compartment for one-half to one and one-half hours, depending on the rate of flow of stabilized sludge into the compartment. The mixture then flows into the clarifier for sludge separation. A relatively clear effluent of low organic content flows over a claritier weir. Settled sludge is collected from the bottom of the clarifier and returned to the stabilization compartment by an airlift pump. After four to six hours of aeration, it is again mixed with raw sewage flowing into the contact chamber. Not all the sludge collected in the claritier is returned to the stabilization compartment. A portion of it is diverted to the third aeration compartment, an aerobic digester, where the volume of sludge is reduced by auto-oxidation. When facilities are provided for separating the digested sludge from the supernatant, the latter is returned to the stabilization compartment to be combined with the sludge

returned from the clarifier. Periodically some sludge has to be wasted from the digestor. Figure 3 illustrates the flow through a plant using the contact stabilization method. Table II includes the different sized treatment planes that utilize the contact stabilization process for treating domestic waste.

III. Special Devices.

1. The <u>Oxidation ditch</u> is a modified form of the activated sludge process, and may be classified in the complete mix, longterm aeration group. This is not a mechanically aerated lagoon because the settled solids produced by the process are continually returned to the ditch to provide a high solids concentration in the aeration basin. This process provides a simple, compact, rugged and virtually fool-proof plant that is capable of 90 to 97 percent reductions of BOD₅ and suspended solids.

Normally a primary settling tank is not used in the oxidation ditch process, shown schematically in Figure 4. Raw sewage pases directly through a bar screen to the ditch. The bar screen is necessary for the protection of the mechanical equipment such as rotors and pumps. The oxidation ditch forms the aeration basin where the raw sewage is mixed with active organisms. The rotor is the aeration device that entrains the necessary oxygen into the liquid in the ditch. The velocity of the liquid in the ditch must be kept above a minimum of approximately one



FIGURE 3. CONTACT STABILIZATION PROCESS FLOW DIAGRAM





foot per second to prevent settling of solids. The ends of the ditch must be well rounded to prevent eddying and dead ereas, and the outside edges of the curves must always be given erosion protection.

The oxidation ditch is operated as a closed system and the volatile suspended solids will increase until it will be necessary to periodically remove some sludge from the process.

The compacted earthen ditch may be lined with poured concrete, plastic liners, shotcrete, asphalt, wood or preformed materials. Clay has also been used as a liner. (Ref. 5)

The process, with its longterm aeration basin, is designed to carry mixed liquor suspended solid concentration of 3,000 to 8,000 mg/l. This provides a large organism mass in the system. The food to organism ratio or loading factor is low, ranging from 0.03 to 0.1 lb. BOD₅/day/lb volatile suspended solids. In addition, because of the low food to organism ratio, the growth of volatile sludge is relatively low because of endogenous respiration. The volatile sludge is less reactive and has a lower BOD₅ than equivalent weight of a sludge produced by processes having larger loading factors. Therefore, the solids lost from the system over the effluent weir in the final settling tank will have a lower BOD₅ than the same quantity lost from the system by other processes.

Developed from the oxidation ditch principle is the extended arration aerotor plant. Figure 5, illustrates the flow diagram of this system.



FIGURE 5 LINE DIAGRAM AEROTOR PLANT

The process utilizes a modified form of the activated sludge process and may be classified in the complete mix, longterm aeration group. The plant provides a simple, compact, rugged, odor free and virtually toolproof operation that is capable of 90 to 97 percent reductions of BOD and suspended solids. The largest plant comes with a capacity to handle 1000 capita per day or 100,000 gallons per day. (Ref. 7)

2. <u>Aerated Lagoon</u>. This method is also feasible. A rotor supported by structural members could be used. A second configuration that could be used where electrical power is absent would be the use of an anchored raft powered by an outboard gasoline engine. The raft would be anchored to a pipe column by the use of a swivel arrangement which would be located in the center of the lagoon. Figure 6 illustrates this method of aerating the liquid. The outboard engine and raft would travel in a circular motion inducing a circular motion to the liquor, while the outboard motor propeller adds the needed oxygen to the mixture. A trash trap would be used at the influent entrance to prevent foreign material from damaging the outboard motor or propeller.

J. <u>Rotating Biological Contactor</u>. This device can be considered a combination of the trickling-filter and the activated-sludge process. It exhibits a degree of flexibility and control over the biological process that may prove to be beneficial to the engineer and plant operator. The process is called the rotating biological surface (RBS) waste-water treatment.





Physically the PBS is a series of closely spaced vertical discs, mounted on a horizontally driven shaft. The shaft is supported by bearings and is slowly rotated by power equipment. The rotating shaft alternately dips the disc surfaces into the waste material and then into the air. Waste material continuously flows parallel to the discs. The waste level is slightly less than half the disc diameter. Intimate contact between the waste material and disc surfaces is provided by contoured tank bottoms.

During initial operations, a biomass colony is quickly established on the disc surface. These colonies of bacteria will continue to propagate (in the presence of adequate oxygen and food material) on both wides of the discs. The growth produced is a uniform mass which smoothly covers the entire disc surface. The RBS does not need to be seeded to establish the biological growth, providing the waste material is biodegradeable and has adequate nutrients. The biomass on the discs oxidizes the waste material into metabolic by-products and excess cell material. The treated water containing these solids is directed to a secondary clarifier for separation.

The wet surface on the biomass removes the oxygen from the air after rising from the waste material. The thin waste film on the surface is extremely rich in oxygen and contributes to the high organic and oxygen uptake of the biomass. The oxygen-rich film penetrates the biomass through mixing and diffusion into its innermost colonies. (Ref. 8)

The advantages of this type process are as follows: a) The retention time or volume of waste material retained is very short. Using domestic waste water, the retention time is less than 45 minutes to achieve a 90 percent BOD₅ reduction in the system. b) Bacterial density of the discs is between 18,000 and 30,000 PPM active organisms. c) The components of the RBS are relatively simple and inexpensive. d) Because the PBS discs are molded of low density expandable polystyrene material, they have a bouvancy of 60 pounds per cubic foot. The only power required to rotate the RBS is the power to overcome transmission efficiencies and bearing friction. e) Generally speaking, checking and changing transmission oil, and bearing and chain greasing is all that is required. t) One of the most important benefits of the RBS process is that it requires very little operator attention. g) The physical configurations of the RBS lends itselt extremely well to process staging.

Since manufacturing procedures dictate modules, or packaged fabrication for economies, other advantages may be derived. Units of discs and shafts are shipped assembled. They may be shipped with or without their own tankage. Fiberglass, plastic lined tanks or precast concrete tanks may be supplied. The packaging concept will enable users to add units in increments as the demand requires. (Ref. 8)

IV. Summary.

Table III was produced from information turnished by the manufactures on the "Froduct Questionnaire." and from the NSF certification

booklets. Reference > describes the observations of the Ohio University study and lists 20 random dates of sampling of the oxidation ditch effluent over a nime-month period. Recorded under the oxidation ditch column are the averages of the 20 samplings. The data shown under the columns of the NSF tested plants are averages of tests made every day over a period ranging from 29 days to 39 days.

Table IV was developed from the ASCE <u>Journal</u> concerning tests conducted by the Florida State Board of Health. The sampling was conducted at random dates over a period of one year. The data shown in the table are averages. These data are not comparable since influent loadings and testing conditions differ, sampling methods are unknown, and actual tests performed on the effluent are unknown. Nevertheless, the resultant averages illustrate that the performance of the treatment plants in most cases produces approximately a 90 percent reduction in BOD₅ and suspended solids.

Appendix III is a list of firms that manufacture packaged waste water treatment plauts. It is fairly complete but by no means all inclusive. It was not the author's intention to omit any industrial concern.

Table III.	Package	Treatment	: Plan	t Perf	ormanc	e (Ave	rages)							
	MANUFACTURERS	+Lakeside Engineering Corp. Oxidation Ditch	Aerotor Plant	ζΙ I∋bo≌ oomi∃+	+Neptune Micro Floc	+#Dorr-Oliver, Inc.	*Chicago Pump, hydrodynamics Division	s∋irieubnI x9T-ns)*	*Aer-O-Flo Corp.	•ητού gatrutsetuneM σονω(*	aaslevol & ditm2*	.οnI ,amsjav2 οογι*	*Water Pollution Control Corp. (Sanitaire)	үпвqmo⊃ зіивҮ ≯эв№*
PERFORMANCE BOD ₅ (Mg/l) Influent Effluent %Reduction Suspended So (M	111ds 4/1)	226 7 98	200 10 94	200 20 90	200 3 98	240 5 98	174 14 90	145 6 96	128 14 89	145 6 96	145 8 95	138 9 93	144 16 96	145 13 91
Influent Effluent IReduction Volatile Sol	1ds g/l)	326 5 96		200 30 85	220 5 98	250 0 100	175 20 88	165 10 94	157 15 91	176 13 93	176 14 92	170 12 93	165 13 92	176 16 91

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Note: + Answered the Questionnaire Independent Test According to STM of WW 12th Edition - 6 month test duration NSF Certification - 5 week test duration

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Table III. Pack	age Treatment	t Plant Perf	ormano	ce (Ave	erages)	(Contin	ued)					
Influent	100	100		155	140	127	130	122	137	140	127	137
Effluent	10	10		0	15	7	11	6	10	6	8	11
IReduction	70	70		100	89	95	92	66	93	93	64	93
ин ₃ -и (н _я /1												
Influent	33	10	20		16.4	15.8	13.8	16.3	16.3	14.9	15.8	16.3
Effluent	.1	1	T		1.1	1.1	0.7	0.1	1.1	1.1	0.8	0.9
7.Reduction	ጉጉ	v/	ćγ		٤٩	63	16	96	66	92	65	45 C
P04-R (MR/1)												
Influent		0£	α		51	50	41	52	14	43	51	51
Effluent ZReduction		27 10	7 12		32	36 27	29 30	36 31	33 33	29 32	37 27	35 32
ጀ ርስኮ (ሣደ/1)												
Influent		300		700	306	300	315	276	302	315	300	302
Effluent ZReduction		30 90		54 92	53 83	43 85	38 88	37 87	50 83	44 86	45 85	37 71
рН												
Influent		7.0	د./	7.3	7.4	7.4	7.4	7.3	7.3	7.6	7.4	7.3
Effluent		7.0	7.0	7.2	7.3	7.2	7.4	7.1	7.1	7.8	7.4	7.1
Flow Capacity (1000 GPD)	287	I	e	ļ	6	7.5	Ś	10	7	Q	16	5
Number of Tests	20	1	42	ı	39	31	30	34	34	31	29	34

raves) (Continued) ł ċ

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	siset tedans	ç	9	11	12	21	
(dap o	OLL) VILIAGED WOLA	6	60	20.5	25	.20	
	ansuittä	6.7	1.2	6.9	1.2	7.3	
	Hq Jnsultnl	7.8	6.9	0.1	6.9	7.8	
	uotaonpaav	16	44	80	66	96	
	ansultid	.07	.1	1.5	<u>.</u>	<u>.</u>	
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	nottoube%%	/3	15	£3	T۹	87	
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f Health	sbilo2 bebraqeu2 (mqq) Jaen1lai	114	1/9	569	333	218	
rd of	uoraonpa _{ti} z	6 0	46	11	92	36	
Boal	Jneulil.	18	11	104	15	ウ マ	
la State	(mqq) _C OOB Insulini	181	172	373	345	306	
TableIV. Plants Tested by the Florid	"anufacturer and Test Location	Marolf - Yocam Battery Piant, Tampa	Chicago Pump - South Gate Subdivision Fla.	Yeemans Bros Castoways Motel, Miami Beach Fla.	Waller Process - Anacapri Notel, Fort Lauderdale Fla.	smith & Loveless - DeSoto Lakes Porile Pomes Broward County, Fla.	

SECTION III

DISCUSSION

The manufacturers who completed the "Product Questionnaire" or who furnished catalogues supplied answers to the general questions needed to provide a more complete picture of their packaged treatment plant.

With the exception of the gasoline-powered aerator, all plants require either 110 or 220 volt electrical power. The power furnished by a specific base in a particular locale would be a design problem and requirement of that base. Figure 7 compares plant size in 1,000 gallons par day versus operating power in kilowatt hours.

Shipping volume varies greatly with the type of plant that has been selected. Figure 8 is furnished to indicate the volume required when planning to transport a prefabricated or sectionalized plant. Shipping of a plant has its problems. The Santa Fe Kailroad's local office in Albuquerque was contacted for information concerning clearance widths and heights allowable for transportation of bulky hardware. The maximum allowable width in a boxcar is eight feet and the height allowance is so low that it precludes the boxcar for shipping a packaged treatment unit. A flatcar can handle a height of twelve to thirteen feet and a width of eleven feet. This is ample to transport most sections of a treatment plant. Figure 9 illustrates expected shipping weights per 1,000-gallon per day plants.







One manufacturer states, "Because of shipping limitations of regulatory authorities on overall size of truck and rail shipments, it is necessary to ship several items of equipment or unit assemblies unmounted to avoid exceeding height or width limits."

The rotor and support structure for the exidation ditch and rotating disc are not included in Figure 5. They would occupy a much smaller volume and area than the extended aeration plants. The volume curve flattens out considerably as the larger capacity plants are approached. The larger plants are fabricated in sections for field erection and would take less shipping space or volume ratio to capacity than the smaller extended aeration plants which are prefabricated at the factory.

All the plants require some concrete foundation work. The oxidation ditch and the rotating disc aeration plants require concrete pads to support the rotor or discs and their supporting structure and a foundation pad if a clarifier is used. Both the extended aeration and contact stabilization plants normally require a concrete foundation or pad. Usually the manufacturer will recommend the necessary type of foundation required for a particular plant.

Figure 10 illustrates the estimated cost of an installed treatment plant versus plant size. The oxidation ditch and rotating disc costs are



ESTIMATED INSTALLED COST

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not included in figure 10, but table V illustrates actual contractor bid proposals for construction, over a cross section of the United States.

Table V

	DESIGN	CONTRACTOR'S	\$ COST
	POPULATION	BID * \$	PER CAPITA
Godley, Texas	500	22,500	45.00
Graford, Texas	600	26,500	44.20
Morrison, Colo. **	700	27,000	38.60
Blossom, Texas	1,000	27,000	27.00
West Salem, Ohio	2,000	70,656	35.33
Laurel, Nebraska	2,000	65,61.	32.83
Stayton, Oregon	3.500	74,402	21.20
Glenwood, Minnesota	4,300	69,000	16.00
Fort Stockton, Texas	12,500	167,500	13.40
* Excludes the cost of	land, legal an	d engineering fe	es.
**Bid but not built.	- •		
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Oxidation Ditch Construction Cost Figures. (Ref. 5)

Under normal conditions, the construction costs of the oxidation ditch type plant is 20 to 33 percent below that normally expected for other treatment plants capable of producing an effluent equivalent to the oxidation ditch process.

SECTION IV

APPLICATIONS

All the curves presented in the previous sections are common to each other on the basis of flow capacity. The manufacturers design their "packaged" treatment plants on a factor of 100 gallons per capita per day. Air Force Manual 88-11 lists 85 GPDC for fixed installations and 100 GPCD for permanent installations. In limited-war areas the design factors could be less than that used for fixed installations.

One of the questions asked on the "Product Questionnaire" was whether the mechanics needed any special training to erect a treatment plant in the field. Those that answered the questionnaire stated that specific training in erecting a treatment plant was not required. It is a logical assumption that the artisans are skilled in their trade. Military "Red Horse" or "Prime BEEF" teams should have the skill to erect such a plant. Erection instructions as well as operating instructions are furnished with each plant. Figure 11 is included to filustrate erection time versus plant size.

The operators of the plants do require some training in plant operation. In most cases the company furnishing the plant will send a representative to instruct an operator on the peculiarities of a



specific plant. The operation of a contact stabilization plant is more complex than that of an extended aeration plant.

In planning to use any type of process mentioned in this paper, consideration must be given to the periodic removal of sludge. Each plant in time will require some sludge removal: therefore, an area for sludge drying beds should be included in the design. Because the aeration process produces less overall sludge, the drying beds need not be as large and extensive as those used in a conventional treatment system.

In transporting any of the plants, regardless of capacity, certain portions of the plant are disassembled for crating and handling. Usually these are portions of the superstructure that protrude above the tank structure. In any event, some assembly at the construction site is required. From the review of literature it has been found that a treatment plant can be found to satisfy the needs of almost any population requirement which exists. Plants can be found to serve a population which produce 200,000 to 300,000 gallons per day by using smaller plants in parallel. A splitter box device would be used in conjunction with the smaller plants directing the flow of waste water enually. An effluent collection system would then be used with a single disposal pipe to the final effluent receiving stream.

The "packaged" treatment plant can produce a high quality effluent. The effluent from these plants, no matter what process is used, usually contains less than 10 percent of the influent BOD_5 . Such plants have been successfully used in many small communities and also in many industrial waste applications. They may be used in dense population areas in military cantonments. By dispersing smaller plants to various high density areas of an installation, the need for a large capacity single plant serving the base is reduced.

The Air Force is represented on the National Sanitation Foundation's steering committee by a member of the Civil Engineering Branch of the Air Force Weapons Laboratory. The Air Force's needs and requirements will be presented through this representation at the NSF.

APPENDIX I

Copy of letter from Department of the Air Force Headquarters Pacific Air Forces Director of Civil Engineering

DEPARTMENT OF THE AIR FORCE Headquarters Pacific Air Forces APO SAN FRANCISCO 96553

Reply to Attn Of DCE/SG

Subject: Improved Utilities Systems for Extended Contingency Operations To: HQ USAF (AFOCE/AFMSG)

1. The timely provision of adequate utilities systems is the most unresponsive area we have encountered in supporting extended contingency requirements in SEA over the past four years.

Our reaction time for vertical facilities has continually been 2. improved by the use of pre-engineered structures and other transportable shelters. A quantum jump in this area has been realized through the development of modular dormitories, officers' quarters, crew quarters, dining halls and hospital facilities. Un the other hand, package utilities systems to support such a buildup or for intermediate length deployments have seriously lagged in development. Once initial deployments are supported through the use of Army type field equipment, i.e., field erdalators, burn buckets, etc., no intermediate, quickly erected package systems for water and sewage treatment are available. As a result, conventional brick and mortar plants must be programmed and constructed from "scratch" to satisfy the longer term needs. Meanwhile, the initial field "lashups" are simply expanded by any means possible to support the constant growth that invariably takes place. This has been and is a matter of grave concern to the Medical Service due to high endemicity of enteric disease.

3. The need clearly exists for development of mackaged plants for water and sewage treatment that are similar in concept to the modular/ relocatable approach so successfully used for vertical facilities. This approach is already being taken for power generation to eliminate the need for time-consuming and unresponsive construction of stationary base power plants. Such an approach to water and sewage will eliminate the need to build conventional facilities for theater of operation environments, except where it is known that major bases will continue in service for periods of well over five years. Even in this case, the package units will provide timely interim treatment to meet established medical standards during the long lead time associated with conventional design and construction.

4. Our experience in programming, designing and constructing conventional water and sewage systems in SEA has clearly established that, in spite of the best planning, excessive lead times do exist. This is true because of a variety of reasons such as:

a. Design and construction are far more specialized and complex than for most other base facilities.

b. Many of the components for conventional systems are not readily available off the shelf.

c. The ability to "get by" with temporary field lashups often causes these vital utilities to be slipped in favor of items that are readily attainable and seem more closely associated with the mission.

d. Cost escalation tends to eat up funds reserved for utilities as while these are only in the design stage, other facilities are being completed.

5. As a base is developed and expanded over an extended period, new missions with dramatic impacts on base population become a way of life. This coupled with the increasing numbers of Local National employees causes water and sewage treatment to become more and more critical from a public health point of view - both as concerns the protection of the on-base population and the surrounding communities. Basic field sanitation practices at a fixed installation can only be effective for a limited period of time without dangers of the health hazard rising sharply. To forestall this undersirable situation in future deployments, we propose that research and development be initiated on the following:

a. A package water plant that can treat water from a surface source equivalent to the obtained by conventional treatment means. Both prechlorination and postchlorination provisions are essential. With separate detention, this same plant can be expected to adequately treat iron-bearing ground water which is a common requirement. Each package unit should be designed to support a set number of personnel consistent with transportability to the site and erectability on the site. As

populations increase, a package unit is simply added to the system when appropriate. Our experience in SEA indicates that a single package to support approximately 2000-3000 personnel is highly desirable. This type of a unit would have put us in business for a considerable period of time at all SEA bases. Even today, two to three package units in this order of magnitude would handle most bases. The package plant should be designed to be aggregated from readily available components or as a pre-engineered package developed by industry. The ability to relocate the conjument should be a consideration, but based on our experience in SEA, not an overriding one. In summary, what we need is a product either prepositioned or available in a timely tashion that can be readily transported overseas, erected with a minimum of specialized know-how and operated by personnel with normal training in career field AFSC 563XC.

b. A backage sewage blant that parallels the above is also required. The package sewage plant should be developed to provide approximately 90 percent reduction of the biochemical oxygen demand in airbase sanitary sewage. Unit capacity should if bractical be on the same basis as that provided for water plants. Again, availability, transportability, ease of erection, and simplicity of operation are the key points to be considered in a package sewage treatment unit.

6. Request a formal development program be initialed with industry to achieve a package water and sewage treatment plant capability as outlined above. We believe such a development is a key element to timely, orderly and safe development of bases in the theater of operations. FOR THE COMMANDER IN CHIEF

ARCHIE S. MAYES Brigadier General, USAF DCS Civil Engineering

APPENDIX II

PRODUCT QUESTIONNAIRE

- 1. Trade name and/or models designation.
- 2. Population capacity and gallon per day capacity.
- 3. Cost per unit.
- 4. Weight of plant assembled.
- 5. Shipping floor area and volume.
- 6. Dimensions of assembled unit or units.
- 7. Is plant pre-assembled?
- 8. Does manufacturer assemble plant? If customer assembled, do the mechanics require special training?
- 9. How many man days required for assembly?
- 10. Foundation requirements.
- 11. Power requirements (frequency and wattage 50/60 or 400 cycle).
- 12. Modular or add-on capabilities. Can units be placed in series to increase capacities?
- 13. What treatment process is used?
- 14. Type chemicals used in treatment process or trade name of product. Ouantities required per mgd treated.

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-	Influent	Effluent	% Reduction	Test Procedure and Duration
BOD (mg/liter)				
Suspended Solids (mg/l)				
Volatile Solids (mg/l)				
NH ₃ -N (mg/1)				
PO ₄ -P (mg/1)				
Total Organic (mg/1)				· · · ·
Carbon (mg/l)	1			
COD (mg/l)				
рН				

MANUFACTURER'S PERFORMANCE DATA

APPENDIX III

- 1. +Aer-O-Flo Corporation
 P.O. Box 223
 Florence, Kentucky 41042
- American Schreiber Co. R.D.2 Red Lion, Pa. 17356
- 5. Brennan Chemical Co., Div. of Costello Manufacturing Co. 704 N. 1st St., St. Louis, Mo., 63102
- 7. +Can-Tex Industries, Div of Harsco Corp.
 P.O. Box 340 Mineral Wells, Texas 76067
- +Clow Corp.
 201 N. Talman Ave Chicago, Ill. 60580
- 11. Crape Co. 4100 S. Kedzie Ave., Chicago, Ill. 60632
- 13. +Davco Mfg. Co. 1828 Metcalf Ave. Thomasville, Ga. 31792
- 15. +Defiance Company
 Division of Daveo Industries
 P.O. Drawer 186
 Tallevast, Florida 33588
- 17. Dravo Corp One Oliver Plaza Pittsburg, Pa. 15219
- 19.*Eimco Corporation
 P.O. Box 300
 Salt Lake City, Utah 84110
- 21. Eldib Engineering & Research Inc.
 22. Elgin Softner, Inc.
 170 Blanchard St.,
 Newark, N.J. 07105
 440 So. NcLean Blvd.
 Elgin, Illinois 60120
- 23. FMC Corp., Link-Belt24. Fuller Co.Prudential Plaza, Dept.69-PCD124 Bridge St.,Chicago, Ill.60601Catasauqua, Pa.

- American Bowser Corp 100 N. Broadway Aurora, Illinois 60505
- Beloit Corp., Jones Div. Box A Pittsfield, Mass. 01201
- Brink Equipment Engineering Sales Inorganic Chemicals Div.
 800 N. Lindbergh Blvd., St. Louis, Mo. 63166
- +*Chicago Pump, HYDRODYNAMICS Div. FMC Corporation
 622 W. Diversey Parkway Chicago, Ill. 60614
- Colt Industries, Inc. 1290 Avenue of Americas New York, New York 10019
- 12. Cromar Co., Cromarglass Div P.O. Box 178 Williamsport, Pa. 17701
- 14. Deady Chemical 3155 Fiberglas Rd. Kansas City, Kan. 66115
- 16. De Laval Separator Co. 350 Dutchess Tpke Pough Keepsie, New York 12602
- 18. *Dorr-Oliver, Inc. 77 Havemeyer Lane Stamford, Conn., 06904
- 20. E.I. duPont DeNemours & Co., Inc. 1007 Market St., Wilmington, Del. 19898

- 25. General Filter Co. Arrasmith Trail P.O. Box 350 Ames, Iowa 50010
- 27. Hays Process Box 768 Waco, Texas 76703
- 29. Kisco Boiler & Engineering Co.30. Koch Engineering Co., Inc. P.O. Box 328 St. Louis, Mo. 63116
- 31. Komline-Sanderson Engry Corp 32. Peapack, N. J. 07977
- 33. Litton Systems, Inc., Applied 34.+Lyco Systems, Inc. Sciences Div., 2003 E. Hennepin Ave. Minneapolis, Minn., 55413
- 35. +Mack Vault Company P.O. Box 335 Valley City, Ohio 44280
- 37. National Tank Co., Div of Combustion Engineering, Inc. P.O. Box 1710 Tulsa, 0kla. 74101
- 39. Pacific Flush Tank Co. 4241 Ravenswood Ave. Chicago, Ill. 60613
- 41. Pickands, Mather & Co., Prenco Div. 42. Piqua Machine & Mafg., Co. 420 E. Third St., 2000 Union Commerce Bldg. Cleveland, Ohio 44115
- 44. Resources Control, Inc. 43. +Pollution Control Inc. Frontage Rd Suite 21 Lunken Airport Administration Eldg. West Haven, Conn. 06516 Cincinnati, Ohio 45226
- 45. Rex Chainbelt, Inc. P.O. Box 2022 Milwaukee, Wis. 53201
- 46. *Schmieg Div Sydner Hydordynamics Inc. 1305 Brook Rd. Richmond Va. 23212 50

- 26. Glenfield & Kennedy, Inc. P.O. Box 191 King of Prussia, Pa. 19406
- 28. Hills-McCanna Co. 400 Maple Ave Carpentersville, Ill. 60110
 - Pollution Control Div., 342 Madison Ave., New York, N. Y. 10017
 - *Lakeside Engineering Corp 222 W. Adams St., Chicago, I11. 60606
- P.O. Box 569 Williamsport, Pa. 17701
- +Marolf Hygienci Equipment, Inc. 36. 7337 Slyvania Ave. Toledo, Ohio 43623
- Neptune Micro Floc, Inc. 38. P.O. Box 612 Corvallis, Ore. 97330
- 40. +Pall Corporation 30 Seacliff Ave. Glen Cove, L.I. New York 11542
- Piqua, Ohio 45356
- 47. +Smith & Loveless, Div. of Union Tank Car Co. 96th and Old S. Hwy. Lenepa, Kans. 66215
- 48. Surburbia Systems, Inc., Box 6217 Leawood, Kansas 66206

- 49. Tailor & Co., Inc. 2403 State St., Bettendorf, Iowa 52722
- 51. Vulcan Laboratories, Inc. 408 Auburn Ave. Pontiac, Michigan 48058
- 53. Waste Water Treatment Corp. 415 Lexington Ave. New York, New York 10017
- 55. Welles Products Corp 1600 N. 2nd St., Roscoe, Ill. 61073
- 57. Yeomans Brothers Co. 1999 N. Ruby St., Melrose Park, Ill. 60160

50. Texas Engineering Corp P.O. Box 13161 Houston, Tex. 77019

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- 52. Walker Process Equip. Inc. Div of Chicago Bridge & Tran. Co. Box 266 Aurora, Illinois 60507
- 54. +Water Pollution Control Corp. P.O. Box 744 Milwaukee, Wisconsin 53201
- 56. Western Water Equipment Co. 925 Tanklage Rd. San Carlos, Calif. 94070
- 58. Zurn Industries Inc., (Pollution Control Group) 1801 Pittsburgh Ave. Erie, Pa. 16512
- + National Sanitation Foundation
- * Answered WLDC "Product Questionnaire

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/. E.A. Aerotor Plant, Lakeside Engineering Corporation, Chicago, Illinois.

8. Joost, Robert H., <u>Systemation in Using the Rotating Biological</u> <u>Surface (RBS) Waste Treatmen': Process</u>, Paper presented at The 24th <u>Annual Purdue Industrial Waste Conference</u>, Purdue University, Lafayette, Indiana, 6-8 May 1969.

DOCI	UMENT CONTROL D	ATA - R & D		
(Security classification of title, body of abstra	ast and indexing annotatic	on must be entered when	the overall report is classified)	
ORIGINATING ACTIVITY (Corporate author)		22. REPORT SECURITY, GLASSIFIC		
ir Force Weapons Laboratory (WIDC)		25. GROUE		
(irtland Air Force Base, New Mext	ico 87117			
REPORT THLE			······································	
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1 January 1969-15 August 1969				
AUTHCR(S) (First name, middle initial, last name)				
James A. Mahoney				
REPORT DATE	7 8 . TO	TAL NO. OF PAGES	7b. NO. OF REFS	
September_1969		66	88	
I. CONTRACT OR GRANT NO.	98. OR	IGINATOR'S REPORT N	IUMBER(5)	
PROJECT NO		AFWITR-69-12	1	
			-	
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Extended aeration Contact stabilization Oxidation ditch Waste water treatment Environics						

INCLASSIFIED Security Classification