AD-763 811

GEOPHYSICS APPLIED TO GEOTECHNICAL PROBLEMS IN A MARINE ENVIRONMENT: A CASE STUDY - MONTEREY BAY, CALIFORNIA

B. B. Barnes, et al

National Oceanic and Atmospheric Administration

Prepared for:

Advanced Research Projects Agency Bureau of Mines

June 1973

**DISTRIBUTED BY:** 

KUUR

National Technical Information Service U. S. DEPARTMENT OF COMMERCE 5285 Port Royal Road, Springfield Va. 22151

# A UNITED STATES DEPARTMENT OF COMMMERCE PUBLICATION

76381

AD

U.S. DEPARTMENT OF COMMERCE NATIONAL OCEANIC AND ATMOSPHERIC ADMINISTRATION Environmental Research Laboratories

# Geophysics Applied to Geotechnical Problems in A Marine Environment A Case Study: Monterey Bay, California

B. B. Barnes, R. F. Corwin, T. G. Hildenbrand, L. Jackson R. Kessler, W. Takeyama, M. Hornick and R. Jenkins

Annual Report— January 20, 1972 through March 20, 1973 Amount of Contract: \$32,544.00 Principal Investigator and Project Scientist: B. B. Barnes (415) 383-6016 Short title— Marine Geophysics

> Reproduced by NATIONAL TECHNICAL INFORMATION SERVICE U S Deportment of Commerce Springfield VA 22151

> > AUG

DISTRIBUTION STATEMENT A Approved for public release;

Distribution Unlimited

1973

Sponsored by: Advancod Research Projects Agency ARPA Order No. 1579, Amend. 3 Program Code 2F10

This research was supported by the Advanced Research Projects Agency of the Department of Defense and was monitored by the Bureau of Mines under Purchase Order No. H0220019.

June 1973

۰,

## AS DOCUMENT CONTROL DATA FORM

3

Com In Ch. 11	)		Mar 7, 66
Security Classifi	DOCUMENT CON	TROL DATA - R & D	
(Security classifice	tion of title, body of abstract and indexing	annotation must be entered	when the overall report is classified)
Marine Minera	als Technology Center		Unclassified
(U.S. Dept. o	of Commerce)	26. G	ROUP
3150 Paradise	e Drive, Tiburon, Ca.	94920	N/A
Geoph A Case	rsics Applied to Geotechnicai Problems in E Study: Monterey Bay, Caiifornia	n a Marine Environmen🎓	
Annual Report, Januar	pe of reput end inclusive dates) y 19, 1972 through January 19, 1973		
AUTHORIS, (FIRST INNIS, -	•••		
B. B. Barnes L. Jackson M. Hornick	R. F. Corwin T. G. Hildenbrand R. Kessler W. Takeyama R. Jenkins		
REPORT OATE 'ay, 19	73	78. TOTAL NO. OF PAG	ES 70. NO. OF HEFS 127
HO220019	NO.	90. ORIGINATOR'S REP	ORT NUMBER(S)
A PROJECT NO.		N/A	
c.		9b. OTHER RE' ORT NO this reports	(5) (Any other numbers that siev be essign
d.			
This contract U. S. Bureau o	, was monitored by the f Mines	Advanced Rese Dept. of Defe	earch Project Agency
This contract U. S. Bureau o	s Was monitored by the f Mines Geophysical systems consisting of elect were developed and special mapping tech geotechnical data in Montercy Bay, Cali the field service was to try and relati to the mass physical properties and eng floor sediments. Measurements consister leigh Reflection Coefficient continuous	12. SPONSORING MILIT Advanced Rese Dept. of Defe Washington, [ rical and acoustical tools inques applied to obtain fornia. The objective of e qeophysical measurements incering properties of sea dof; (1) mapping the Ray- y by a ship underway, (2)	earch Project Agency ense D. C. 20:01
This contract U. S. Bureau o	s Was monitored by the f Mines Geophysical systems consisting of elect were developed and special mapping tech geotechnical data in Monterey Bay, Cali the field servise was to try and relati to the mass physical properties and eng floor sediments. Heasurements consiste leigh Reflection Coefficient continuous measuring in actu, the propagation velo transverse (shear) waves, and (3) telin measurements of sediment conductivity.	12. SPONSORING MILIT Advanced Rese Dept. of Defe Washington, [ Interim and acoustical tools fornia. The objective of e geophysical measurements intering properties of sea d of; (1) mapping the Ray- littes of compressional and g both underway and in site	and Activity earch Project Agency ense D. C. 20:01
1. SUPPLEMENTARY NOT This contract U. S. Bureau o	s was monitored by the f Mines Geophysical systems consisting of elect were developed and special mapping tech geotechnical data in Monterey Bay. Cali the field exercise was to try and relat to the mass physical properties and eng floor sediments. Measurements consiste leigh Reflection Coefficient continuous transverse (ahear) waves, and (3) tailing measurements of sediment conductivity. P and S wave velocities were measured up hear wave generator. Measurements of a taken with a direct current resistivity face-towed configuration and as a diver- inplanted in the seafloor. These data, flection coefficients (bottom loss) data is of samples taken using a unique dive vibraciber ampler that were specially of ment.	12. SPONSORING MILIT. Advanced Rese Dept. of Defe Washington, [ Vashington, [ Vashington, [ Vashington] fornia. The objective of eqeopysical measurements ineering properties of sea- dof; (1) mapping the Ray- ly by a ship underway, (2) Litles of compressional and g both underway and in eich system defloyed in a sur- operated expandable array along with the computed to analy resperted to core sampler an elegined for the field expe	ABY ACTIVITY earch Project Agency ense <u>D. C. 20:01</u>
1. SUPPLEMENTARY NOT This contract U. S. Bureau o	Beophysical systems consisting of electiver developed and special mapping tech geotechnical data in Monterey Bay, California to the field exercise was to try and related to the mass physical properties and eng filos sediments. Measurements consistent leigh Reflection Coefficient continuous transverse (where) waves, and (3) taking measurenets or sediment conductivity. Pand S wave velocities were measured us shear wave generator. Measurements of a sediment conductivity face-towed configuration and as a diverginglanted in the sefloor. These data, flection coefficients (bottom loss) dats is of samples taken with a direct current resistivity face-towed configuration and as a divery file-grained scale. Grid patterns, for generations, Monterey Bay, California. Surve vibractor, Monterey Bay, California. Surve the field exercise.	12. SPONSORING MILIT Advanced Rese Dept. of Defe Washington, [] rical and acoustical tools indues applied to obtain fornia. The objective of eqeophysical measurements intering properties of sea- intering properties of sea- intering properties of sea- gooth underway, (2) itiles of compressional and of; (1) mapping the Ray- ly by a ship underway, (2) itiles of compressional soft underway and in aits soft application of a ser- operated expandable array along with the computed re- system defloyed in a sur- operated core sampler and seigned for the field expe carried out in Santa Cruz tys were conducted on a profiling and sampling cov- raccuracy precision radio- tem was employed throughout	Any Activity earch Project Agency ense D. C. 20,01
1. SUPPLEMENTARY NOT This contract U. S. Bureau o	Source of the system series of the series was series of the series. The surget of the series of the series of the series. The series and sampling programs were of the series. The series and sampling positioning system field the series of the system series of the series. The system series of the system series of the series of	12. SPONSORING MILIT. Advanced Rese Dept. of Defe Washington, I rical and acoustical tools decopysical measurements inger applied to obtain fornia. The objective of ecopysical measurements intering properties of sea- dof; (1) mapping the Ray- ly by a ship underway, (2) itles of compressional and dof; (1) mapping the Ray- ly by a ship underway, (2) itles of compressional and dof; (1) mapping the Ray- ly by a ship underway, (2) itles of compressional aboth underway and in aitz system defloyed in a sur- operated expandable array along with the computed re- system defloyed in a sur- operated core sampler an elsigned for the field expe arried out in Santa Gruz tys were conducted on a varied out in Santa Gruz tys were conducted on a surgent of the field expe carried out in Santa Gruz tys were conducted on a reacturacy precision radio- tem was employed throughout corrober-stion of data to described. It was con- have ontential applicatio aitu) painmeters of mass a, and thrit only the shear rent of the engineering	ARY ACTIVITY earch Project Agency ense D. C. 20:01

ja



#### NOTICE

The NOAA Environmental Research Laboratories do not approve, recommend or endorse any proprietary product or proprietary material mentioned in this publication. No reference shall be made to the NOAA Environmental Research Laboratories, or to this publication furnished by the NOAA Environmental Research Laboratories, in any advertising or sales promotion which would indicate or imply that the NOAA Environmental Research Laboratories approve, recommend or endorse any proprietary product or proprietary material mentioned herein, or which has as its purpose an intent to cause directly or indirectly the advertised product to be used or purchased because of this NOAA Environmental Research Laboratories publication.

The views and conclusions contained in this document are those of the authors and should not be interpreted as necessarily representing the official policies, either expressed or implied, of the Advanced Research Projects Agency or the U. S. Government.



DI TRIBUTION STATEMENT A Approved for public release; Distribution Unlimited

## TABLE OF CONTENTS

				Page
TEC	CHNICAL REPORT SUMMARY			vi
1.	INTF	RODUCTIO	DN	1
	$\begin{array}{c} 1.1 \\ 1.2 \end{array}$	Discus Operat	ssion of Problems tional Constraints	3 4
2.	PREVIOUS INVESTIGATIONS			
	2.1 2.2	Applic Applic	cation of Acoustics cation of Electrical Resistivity	4 8
3.	THEORY AND TECHNIQUE			9
	3.1	Reflec	tion Coefficient Mapping	9
		3.1.1	Relation of Bottom Loss to Mass Physical Properties	14
	3.2	In Sit	u Shear Wave Generation	15
		3.2.1	Viscoelastic Model	16
	3.3	Direct	: Current Resistivity	19
4.	ELEC	TRONIC	SYSTEMS AND EQUIPMENT	25
	4.1	Reflec	tion Coefficient Mapping Experiment	25
		$\begin{array}{c} 4.1.1 \\ 4.1.2 \\ 4.1.3 \\ 4.1.4 \end{array}$	Acoustic Sources Hydrophone Receivers Transmission Measurements Data Acquisition and Display	25 32 34 39
	4.2	Shear	Wave Experiment	43
		4.2.1 4.2.2 4.2.3	Shear Wave Generator Receiver Array Signal Processing and Display	43 46 47
	4.3	Electr	ical Resistivity System	48
		4.3.1 4.3.2	Expandable Array Surface Towed Array	48 49
	4.4	Sedime	nt Sampling Equipment	50
		4.4.1 4.4.2	Diver Operated Core Sampler Bottom Sitting Vibracorer	52 55
	4.5	Precis	ion Navigation and Positioning System	57
		4.5.1 4.5.2 4.5.3 4.5.4	Automatic Line Follower Recording and Display Indexing Shore Stations	58 58 61

}

iii

-	-			Page
5.	FIE	LD OPER	62	
	5.1	Test	Sites	62
		$5.1.1 \\ 5.1.2$	Monterey Bay Other Test Sites	62 66
	5.2 5.3	Refle Shear	ction Coefficient Mapping Experiment Wave Experiment	67 70
		5.3.1 5.3.2	Field Procedure — Lake Test Site Field Procedure — Monterey Bay Test	70
	E 4	D.I.	Site	71
	5.4	Direc	t Current Resistivity Experiment	73
		5.4.1 5.4.2	In Situ Array Surface Towed Array	73 74
	5.5	Samp1	ing Program	75
6.	GEO	LOGICAL	77	
	6.1	Labora	atory Procedures — Sample Analysis	77
		$6.1.1 \\ 6.1.2 \\ 6.1.3$	Wet Unit Weight, Ywet Specific Gravity of Solids, Gs Water Content, w: Void Ratio, e:	78 78
		614	Porosity, $\phi$	79
		6.1.5	Shear Strength (Penetrometer)	80
		6.1.6	Shear Strength (Vane Shear-Torvane)	81 81
	6.2	Interp	pretation of Sediment Analyses	82
		6.2.1	Interrelationship of Sediment	
		6.2.2	Properties Sediment Dynamics and Deposition Characteristics	82
7.	GEOP	HYSTCAL		82
	7 1	Contin	Porta ANALTSIS AND RESULTS	94
	· • ±	Experi		
		7.1.1	Data Reduction Program	94
		7.1.2	Summary of Processing - Comments	94
		7.1.3	and Findings Correlation of Reflectivity to Sediment Proposition	104
	7.2	Shear 1	Wave Evnoniment	107
		7 0 1		142
		7.2.3	Correlation Between Acoustical	142
			and mass-physical properties	149

19

ŧ,

T

				Page
	7.3	Direc	t Current Resistivity Experiment	154
		7.3.1 7.3.2	Data Interpretation Results	154 159
8.	CONC	LUSION	S AND RECOMMENDATIONS	165
	8.1 8.2 8.3	Refle Shear Direc	ction Coefficient Mapping Experiment Wave Experiment t Current Resistivity Experiment	165 166 168
9.	ACKN	OWLEDG	MENTS	169
10.	REFE	RENCES		169
APP	ENDIX	A.1.	CALCULATIONS FOR UNDERWAY ACOUSTICS EXPERIMENT	179
APP	ENDIX	A.2.	DERIVATION OF BASIC EQUATIONS OF LINEAR VISCOELASTICITY (after Borcherdt, R. D.,	
400			1972)	185
APP	ENDIX	A.3.	SEDIMENT PROPERTY EQUATIONS	191
APP	ENDIX	A.4.	SUMMARY OF SAMPLING PROGRAM	195
APP	ENDIX	A.5.	SUMMARY OF ACOUSTIC, ELECTRICAL RESISTIVITY AND SEDIMENT ANALYSES DATA	199
APP	ENDIX	A.6.	SEDIMENT ANALYSES	203
APP	ENDIX	A.7.	DATA SUMMARY SHEETS REFLECTION COEFFICIENT MAPPING EXPERIMENT	221

v

#### TECHNICAL REPORT SUMMARY

Geophysical systems consisting of electrical and acoustical tools were developed and special mapping techniques applied to obtain geotechnical data in Monterey Bay, California. The objective of the field exercise was to try and relate geophysical measurements to the mass physical properties and engineering properties of seafloor sediments. Measurements consisted of; (1) mapping the Rayleigh Reflection Coefficient continuously by a ship underway, (2) measuring *in situ*, the propagation velocities of compressional and transverse (shear) waves, and (3) taking both underway and *in situ* measurements of sediment conductivity.

P and S wave velocities were measured using a prototype seismic shear wave generator. Measurements of sediment conductivity were taken with a direct current resistivity system deployed in a surface-towed configuration and as a diver-operated expandable array implanted in the seafloor. These data, along with the computed reflection coefficients (bottom loss) data, were correlated to analysis of samples taken using a unique diver-operated core sampler and vibracorer sampler that were specially designed for the field experiment.

The surveys and sampling programs were carried out in Santa Cruz Harbor, Monterey Bay, California. Surveys were conducted on a fine-grained scale. Grid patterns for profiling and sampling covered an area of 30 square miles. A high-accuracy precision radiolocation navigation and positioning system was employed throughout the field exercise.

The experiments, equipment employed, and corroboration of data to previous studies of a similar nature are described. It was concluded that each of the systems employed have potential application for measuring remotely and directly  $(in \ situ)$  parameters of mass physical properties of seafloor sediments, and that only the shear wave system showed potential for measurement of the engineering properties of seafloor sediments.

#### GEOPHYSICS APPLIED TO GEOTECHNICAL PROBLEMS IN A MARINE ENVIRONMENT A CASE STUDY: MONTEREY BAY, CALIFORNIA

#### B. B. Barnes, R. F. Corwin, T. G. Hildenbrand, L. Jackson R. Kessler, W. Takeyama, M. Hornick and R. Jenkins

#### 1. INTRODUCTION

The purpose of this work is to demonstrate the value of applying geophysical remote sensing techniques to the classification of engineering and mass physical properties of seafloor sediments commonly referred to as "geotechnical properties" and to suggest specific studies in which the methods could be further utilized to obtain geotechnical information necessary for seafloor excavation, such as in mining marine mineral deposits of shallow and deep sea origin.

Tools and techniques recently developed in this laboratory for obtaining acoustical and electrical resistivity measurements on target areas in San Francisco Bay (Barnes et al., 1972), Baja California, Mexico (Corwin and Conti, 1973) and more recently in Monterey Bay, California, have greatly enhanced the usefulness of geophysical methods in determining subtle variations observed in a wide variety of sediment types from both estuarine and continental shelf environments. Acoustical and electrical resistivity variations within individual sediment types, detectable by applying fine-grained surveying practices and employing advanced instrumentation, can reflect information useful in depicting geotechnical variations occurring in these two environments. Thus, the methods described herein are powerful tools for use in site selection for undersea foundation emplacement and in predicting a geologic situation before seafloor excavation and mining. To demonstrate this, the present report will give as an example geotechnical variations obtained using geophysical methods which are verified by sediment analyses of core samples in selected test sites. The analyses depict several different mass physical properties and engineering properties, each systematically obtained from diverse kinds of sediments located within a geographical province in the primary

test area located in Monterey Bay, California. An additional advantage of applying advanced sampling equipment to enable more accurate and reliable correlations of accustical and electrical resistivity measurements to sediment analyses is also described in the report.

The distribution of changes in mass physical properties, consisting of porosity, density, mean grain size, and sorting coefficients, and of engineering properties, such as measured by penetrometer and vane shear apparatus, is made complex by a host of oceanographic operators pressure, currents, mineralogy, biology, geochemistry, and structural deformation, to mention a few. The complexity is further enhanced by the interrelation and interdependence of parameters related to the sediment properties. In this study we are mainly concerned with the static situation — the seafloor as it is in one place in space and time.

During the last decade, interest in geotechnical measurements has been steadily growing because of the utilization of our coastal zone and adjacent shelf for engineering purposes, such as, for example, emplacement of oil pipelines, drilling platforms, bridge and dam foundations, and submerged nuclear power plants. More recently the United States mining industry has indicated an interest in developing more rapid and accurate methods to obtain geotechnical data in deep sea areas where ferromanganese deposits are located. Although the feasibility of applying remote sensing instrumentation to geotechnical problems is still widely debated, considerable progress has been made in the development of methods and instrumentation for this purpose. We still have insufficient knowledge of the full potential to reliabily predict sediment property variations and to understand the mechanisms that cause such changes to occur. Knowledge can only be derived from making many empirical measurements in the actual environment, utilizing computed analyses and the best in sediment sampling equipment in conjunction with advanced geophysical tools and methods.

Elements in this study consist of: (1) two separate acoustic experiments based on continuously mapping reflection coefficients by a ship underway and using *in situ* apparatus implanted in the seafloor capable of generating and detecting P and S waves in a saturated medium, (2) an electrical resistivity experiment utilizing especially configured arrays for 100

obtaining *in situ* and surface towed direct current resistivity measurements that indicate sediment conductivity taken in place and by a ship underway and, (3) the use of unique sampling equipment for obtaining cores of the near surface seafloor sediments. The report defines in detail the interrelationship and correlation of data obtained from each of these three sources and relating these data to the overall objective and purpose of the project.

#### 1.1 Discussion of Problems

Application of geophysics to the solution of marine civil engineering problems, or "geotechnics", constitute the subject of this report. The point of view is that of the engineer, and the earth sciences, particularly geophysics, have been brought into the engineering pattern only when they have direct bearing upon a problem. Our goal is to present only those basic geotechnical problems connected with depicting the natural environment of an engineering structure and/or trafficability of seafloor systems, such as mining implements, that may be used for excavation. The case history of Monterey Bay has been used to elucidate the value of applying geophysics to standard practices for determining seafloor properties.

Several cases of differential settlement of oil drilling platforms in the Gulf of Mexico and huge pipelines shifting from their anchorages in the North Sea and Mediterranean Sea have resulted in the loss of production and a considerable amount of revenue. Before these events, not all marine engineers could see clearly that the design of a structure should be preceded by a careful study of its environment, particularly the foundation material on which the structure was to be placed. The problems are more complicated than those encountered on land because of the overlying water column and unusual behavior of marine sediments. The past 15 years have seen great emphasis placed towards diagnosing in advance the state of sediment properties. With the advent of offshore mining on our continental shelves and in the deep sea, undoubtedly more requirements for rapid, but accurate determinations will be necessary; thus,

a new type of technology is needed to assist standard methods of geotechnical measurements.

#### 1.2 Operational Constraints

Certain constraints to applying geophysical methods must be identified before proceeding further. Acoustical measurements taken by a ship underway are particularly affected by topography, acoustic beam width, power, system stability in a hostile environment, and water depth. In situ measurements are likewise affected, but to a lesser degree, their greatest limitation being time and cost factors.

A level set r terrain is most desirable for acoustic measurements. Micro topog whic changes have greater influence on acoustic sources that possess characteristics of narrow beam width and high frequency. Ideally, a flat bottom with little variation of sediment type produces the most reliable data. Water depth becomes an important factor if source and receiver transducers are on the surface. Attenuation of energy by spreading and absorption are neglible in shallower water but can become an appreciable factor with depth. The stability of the systems, in particular the source and receiver orientation to the reflecting surface, are obviously a consideration in obtaining accurate signal ratios between the transmitted and received (reflected) energy. In the case of the towed electrical array, similar constraints apply. In this instance, the sensitivity of the measurement is affected causing the information to be less accurate than *in situ* measurement. These tolerances and the errors induced will be described in this report.

# PREVIOUS INVESTIGATIONS Application of Acoustics

The earliest work in applying reflection acoustical methods began during World War II when the Division of War Research and several Navy research agencies locked into submarine warfare applications. Slant angle measurements at various frequencies and distances provided the first data on the seafloor geological environment. Attention was focused on two aspects relating wave propagation phenomena and sediments. In the period following the war several investigators (Liebermann, 1948; Urick, 1954; Urick and Saling, 1962; Mackenzie, 1960; McKinney and Anderson, 1964; and Jones et al., 1964) directed their efforts toward identifying the characteristics of sound transmission and reflection. Attention was not given to sediment properties until the mid-1950's, when investigations by Hamilton (1956), Shumway (1960); Sarmiento and Kirby (1962), Richards (1962), and Nafe and Drake (1963) produced much of the deductive theory relating sediment properties to acoustics. Direct measurements made by a ship underway did not begin until the mid-1960's (Breslau, 1964, 1967; Taylor-Smith and Li, 1966; Taylor-Smith, 1968).

Recent work by Pawlowicz (1971) indicates that it is possible, with caution, to describe the porosity of seafloor sediments in terms of reflectivity, Furthermore, the strong correlation which exists between reflectivity and porosity can be used to infer qualitatively, the potential load-bearing properties of the substrate. If a more quantitative description is desired, computer data processing, either in the laboratory or aboard ship, could be used. Pawlowicz suggests that it should be feasible to relate porosity to sediment type in general terms, i.e., high porosity relates to silts and clays, and low porosity to sands and gravels; however, for porosities greater than 55 percent, there is a great overlap in sediment type and precise designations may be inconclusive (Taylor-Smith and Li, 1966; Faas, 1969).

The Mine Defense Laboratory has developed a Sea Bottom Classifier (SBC) which determines the softness of a sea bottom by measuring the elongation of reflected acoustic pulses using a standard depth sounder (Stanley and Harris, 1968). If a short narrow-beam acoustic pulse is transmitted into a hard bottom, the received echo is about the same length as the transmitted pulse. On the other hand, in a soft bottom, the acoustic pulse penetrates the bottoms, and volume reverberation within the bottom causes elongation of the echo pulse proportional to pulse penetration.

Li and Taylor-Smith (1969) believed that by profiling with a repeating broad-band acoustic pulse source over areas where a wedge of sediment was underlain by a strong reflector it should be possible, by

comparing a sub-bottom reflection (and in some cases its associated multiples) to the pulse reflected from the sea-floor and/or to its adjacent reflections, that one could give a reasonable analysis of the bulk properties of the sea-floor medium in terms of variations in the frequency spectrum. The values so obtained from laboratory and *in situ* probe measurements, would thus provide data about the mechanical properties of the sediment.

Sub-bottom profiling devices as conventionally used, no matter how complex, can only provide an assessment of the geology of the sediments, not the sediment types or their properties. While it should be possible to obtain a feeling for the hardness of the sea-floor from observed multiples on a sub-bottom record and, to a certain extent, the material sizes making up the surface shown by scattering or lack of penetration observed on a record. In analysing multiples, few attempts have related the energy decay rate of multiples to varying sediment properties.

Other studies by Li and Taylor-Smith (1966) and Breslau (1965), and Shumway (1960), have also pointed out that frequency analysis of seismic energy reflected from the bottom may provide the information needed to absolutely fix acoustic reflection values to sediment types. Experimental studies by Taylor-Smith and Li (1966 and 1969), and Breslau (1965) have used spectrum analysis measurements utilizing the attenuation factors of various sediment types versus grain sizes with the comparison of the various frequency components of an acoustical source.

Results of work done by McCann (1966) and Buchan et al. (1967) show that attenuation plotted against grain size varies for various frequencies within the low-frequency sound source range (fig. 1). According to McCann (1966), two mechanisms are responsible for the acoustic energy loss in saturated sediments: (1) A solid friction loss for sediments with grainto-grain contact, this being characterized by a linear variation of the attenuation coefficient with frequency, and (2) a viscous loss caused by interaction between solids and liquids, this being characterized by an attenuation coefficient which is proportional to the square root of the frequency.



Sediment mean diameter (mm)

Figure 1. Sound attenuation plotted against mean grain diameter. (McCann, 1966).

Reflection coefficients measured continuously by a ship underway have been made Breslau (1965), Li and Taylor-Smith (1965), and Barnes et al. (1972). These consisted of measuring the amplitudes and/or energy in the transmitted and reflected pulses and correlating attenuation to various bottom sediment properties.

Another way of ascertaining sediment properties is by *in situ* measurement, from which one can obtain direct information on the compressional wave velocity, shear wave velocity, attenuation, and Poisson's ratio. Direct measurement of the shear wave velocity in ocean sediments has been restricted to the seismology of high energy sources that produce returns from the deeply buried material and no returns from the soft superficial sediment. This is because shear waves do not propagate at all in a fluid and, consequently, are transmitted poorly in a saturated porous medium. The shear wave velocity for the saturated sediments of the sea floor has been determined indirectly through measuring *in situ* the Stoneley wave velocity (Hamilton et al., 1970; Davies, 1965; Jones, 1958; Cohen, 1968; and Bieda, 1970). It has been demonstrated in laboratory experiments that the generation of torsional waves by a viscoelastometer provides information on the shear wave velocity in saturated sediments.

-mit

mes

The compressional wave, unlike the transverse (shear) wave, is transmitted readily in water and in saturated sediments. Its velocity in marine sediments has been documented in several reports (Nafe and Drake, 1963; Hamilton, 1970a; Lewis, 1971; Horn et al., 1968). If, in oddition to the compressional wave velocity and the shear wave velocity, density is known, one can calculate the dynamic values of rigidity, bulk modulus, Young's modulus, and other elastic constants (Hamilton, 1970a; Nafe and Drake, 1963). Due to the difficulty in measuring the shear wave velocity, the elastic constants have been derived by assuming a value of the sediment bulk modulus from an aggregate theory (Hamilton, 1971; Laughton, 1957; Lewis, 1971).

Other marine sediment properties of interest are porosity, grain size, shear strength, and percent of clay, sand, or silt. Acoustical properties are determined by these physical properties and, thus, have been empirically related to them. The reader is referred to excellent discussions on this subject in Hamilton (1970b, 1971), Horn et al. (1968), Schreiber (1968), and Lewis (1971).

#### 2.2 Application of Electrical Resistivity

Apart from acoustics there has been an appreciable amount of effort devoted to resistivity techniques applied to prediction of porosity in marine sediments.

The earliest marine methods evolved from the application of theory and technology used in land borehole logging to marine investigations, (Bouma et al., 1971; Erchul and Nacci, 1971; Kermabon et al., 1969; Wyllie, 1963). A horizontal array for measuring seafloor resistivity has resulted from this evolution (Corwin and Conti, 1973). For marine use, a horizontal array may be towed over the sea surface or deployed on the seafloor. The

first application provides a fast and convenient method of mapping horizontal changes of average bottom resistivity over large areas. An array deployed in the second manner, however, provides a more detailed picture of the bottom resistivity distribution at a given location.

There has been little previous use of horizontal resistivity arrays on the seafloor. An array of fixed spacing dragged over the seafloor off the coast of Cornwall, England, was used to map lateral changes in seafloor resistivity (Marke, 1965). The fixed electrode spacing used by Marke, however, precluded the acquisition of information about the vertical distribution of bottom resistivity. Schlumberger and Leonardon (1934) used a bottom-deployed array to measure depth to bedrock below seafloor sediments. Dunlap and Johnson (1958) mapped offshore faults and salt domes, also using a fixed array dragged over the seafloor.

The deployment of a surface towed resistivity array in work done by this laboratory is believed to be the first such application using this mode. Several other electrical studies, including self potential, direct current resistivity and electromagnetic sounding have been carried out in this laboratory and are reported in Barnes et al. (1972), Beyer (1972), and Crowin and Conti (1973).

### 3. THEORY AND TECHNIQUE 3.1 Reflection Coefficient Mapping

A simplified approach to a very complex physical process involving wave propagation, attenuation, and absorption has been used in developing the model of the sea floor-water interface. The sea floor is considered a plane interface between two fluids. Energy is focused on a zone of about one square meter surface area. The backscattering strength associated with this surface area depends greatly on the roughness of the sea bottom. A smooth bottom acts as a mirror such that the reflected energy is concentrated in a specular direction. A rough bottom will reflect the energy more evenly in all directions. It is important to note that the terms "smooth" and "rough" must be considered in relation to the acoustical wave length. Where the wave length is only a few centimeters, it is seen that a 100 percent smooth bottom is rare. Even small

stones, holes or ripples will make the bottom rough in the sense considered here. When a sound wave strikes the sea bottom, part of the incident energy will be reflected. The reflected energy is spread in all directions with a typical distribution, as shown in figure 2. The distribution is totally dependent on the roughness of the sea bottom. Figure 3 illustrates how the roughness of the topography can influence the bottom backscattering strength as a function of the grazing angle. The



#### Figure 2. Seafloor scattering - specular energy.



Figure 3. Typical curves for bottom backscattering strength. (After Bodholt, 1969).

angle of incidence is seen to also play an important role in determining the strength of the echo signal. The experiments described in this report were all conducted with the source and receiver set up to record normal incidence. It can be shown that theoretically the most accurate measurements of bottom backscattering strength should be obtained with a narrow beam transducer. If the beam width is 5-10°, the bottom backscattering strength will vary only a few dB or less within the beam, of course depending on the type of bottom. The magnitude of the bottom sampling area will depend on the pulse length of the transmitted sound. The shorter the pulse the smaller the sampling area. Figure 4 illustrates the beam width, pulse length, and area concept.

In the exercise conducted for this investigation, two sources are employed. One is an uncontrolled beam width emitting a .5 millisecond pulse length and the other is a wide beam  $(35^{\circ})$  echo sounder having a pulse length of .25 millisecond.





Details of the mathematical description for acoustic wave propagation, reflection and absorption (bottom loss) is contained in a previous report by Barnes et al. (1972). Several references were consulted and the best mathematical expressions relating to the experiments conducted in this investigation were taken from derivations by Breslau (1967).

From the derivations the most useful equations applicable to data reduction and processing are represented by defining the relationship between the fractional loss of intensity at the sea floor, K, and the Rayleigh Reflection Coefficient, R, i.e.,

$$K = R^2$$
(1)

where K is defined on a peak pressure basis,

$$K = \frac{P_{R^2}}{P_{S^2}} (2D)^2 \frac{1}{e^{-\alpha 2D}}$$
(2)

where

 $P_{R}$  = pressure of the echo pulse,

 $P_{S}$  = pressure of the source pulse,

D = transmission loss due to spherical spreading, and

 $e^{-\alpha 2D}$  = transmission loss due to dissipative attenuation of sound in seawater.

An expression of the fundamental equation for bottom loss, BL, is defined by Breslau (1967) as

$$BL = S_{PL} - E_{PL} - TL_{S} - TL_{A}, \qquad (3)$$

where

BL = bottom loss (in dB) and is equal to  $-10 \log K$ ,

 $S_{PL}$  = pressure level of the source pulse (in dB//1 dyne/cm<sup>2</sup>) and is equal to 20 log  $P_S$ ,

 $E_{PL}$  = pressure level of the echo pulse (in dB//1 dyne/cm<sup>2</sup>) and is equal to 20 log  $P_{R}$ ,

 $TL_{S}$  = spreading loss (in dB) and is equal to 20 log 2D,

 $TL_{A}$  = attenuation loss (in dB) and is equal to 10 2D log e.

A final expression relates bottom loss, BL, to reflectivity, R, expressed in decibels;

$$BL = 20 \log R.$$
 (4)

- 5

ne l'

Other manifestations of these basic expressions that include measurements dealing with the energy versus time waveform of the echo are described in Breslau (1967). Since this investigation was concerned only with peak pressure measurements, the derivations relating to energy in the echo strength are not included in this discussion.

The computational basis for correcting data related to the field configurations (geometry) of acoustic sources and their respective receivers, as used in the Monterey Bay experiment, are contained in Appendix A-1. The mathematical expressions and constants derived were used in computer analyses and processing of the data (see section 7.1.1).

### 3.1.1 Relation of Bottom Loss to Mass Physical Properties

The mass characteristics of the bottom sediment are related to bottom loss through the Rayleigh Reflection coefficient and the acoustic impedance contrast at the interface, and it has been shown that the Rayleigh Reflection Coefficient is related to bottom loss through equation (4) (Breslau, 1967). It is possible to establish a relationship between porosity and bottom loss using the impedance which is dependent upon density and compressional velocity, both factors that relate to the porosity of natural sediments (Barnes et al., 1972; Breslau, 1964, 1967).

In summary, several investigations have ascertained that a linear relationship exists between porosity and density, in particular that bottom loss, as deduced from many measurements made on sediments, bears a definite relationship to the porosity of the sediment. And furthermore, since the porosities of natural sediments are somewhat related to sediment grain size, it is also possible to establish a general relationship between bottom loss and the geological properties of sediments.

For further discussion of the sediment bulk properties and variations within the physical makeup of the grains, interstitial properties, etc., and more specifics on how these factors influence porosity, the reader is directed to the papers by Nafe and Drake (1963), Birch et al. (1942), Hamilton et al. (1956), Shumway (1960), Sutton et al. (1967), Breslau (1964), and Barnes et al. (1972). The information available on this subject is too voluminous to be discussed here.

#### 3.2 In Situ Shear Wave Generation

In an earlier report (Barnes et al., 1972), it was concluded that the best method to generate and directly detect shear waves in saturated marine sediments is by implanting both receivers and a  $S_H$ -source into the sediments and by timing the direct  $S_H$ -wave. Assuring a large response to the direct  $S_H$ -wave, a linear array of horizontal geophones was implanted with the sensitive axis of each geophone parallel to the ground motion of direct  $S_H$ -wave. The  $S_H$ -source was placed at one end of the geophone array. Theoretically, in a homogeneous half space the first arrival detected by each horizontal geophone will be the direct  $S_H$ -wave. The shear wave velocity is, thus, obtained from a time distance graph constructed from the first arrival at each horizontal geophone.

The shear wave source was also employed to generate compressional waves. Little concern was given to the detection of the compressional wave since it is transmitted and detected with little difficulty in saturated sediments. However, it was thought best to implant source and receivers into the sediment. This would avoid the confusion that might exist in differentiations between the direct compressional wave propagating through the sediment and the direct compressional wave propagating through the water.

A linear array of vertical geophones was used to record the arrival time of the direct compressional wave. Theoretically, a vertical geophone will not respond to the direct compressional wave if source and receiver are at the same depth; however, because of the predictable increase in density of natural sediments with depth, the direct P-wave will propagate in an arcuate path and, thus, be detected by a vertically sensitive geophone. The advantage of using a geophone sensitive to vertical displacement instead of a standard omnidirectional geophone or a horizontal geophone is that information on the direct  $S_V$ -wave might be obtained

from the vertical geophone. Barnes et al. (1972) describes in detail successful applications of this technique in moist sand and saturated beach sands.

#### 3.2.1 Viscoelastic Model

There remains the question of what model to employ for the sea floor. Recent reports by Hamilton (1970a, 1972) have clarified this problem. From his data and from an extensive literature search, Hamilton (1972) arrived at the following conclusions for a water saturated porous media: (1) for the small stresses produced by sound waves, marine sediments act essentially as a closed system (interstitial water and solids move together); (2) viscosity of the interstitial water is neglible; (3) dispersion of velocity with frequency is insignificant in the frequency range of interest for geophysical studies; (4) Hookean model and the equations of elasticity that follow may be used in most cases to compute the compressional and shear wave velocity; and (5) a linear viscoelastic model with attenuation proportional to frequency and energy damping independent of frecuency can be used if wave energy losses are to be considered. These conclusions will not be discussed in great detail except the usefulness of the Hookean model to determine wave velocities.

In geophysical and soil mechanic studies of rocks and minerals, the equations of Hookean elasticity have been shown to adequately define the compressional and shear velocity (Anderson and Liebermann, 1968) (Hardin and Richards, 1963). The widespread use of the elastic model is primarily due to the fact that this model does not provide for wave energy losses and that these losses are small in most materials. To investigate whether saturated marine sediments can be considered as a low-loss material, an appropriate model providing for energy absorption was studied and then compared to the elastic model.

An appropriate anelastic model is to consider the seafloor as a homogeneous, isotropic linear viscoelastic medium. Linear viscoelasticity has been discussed in various forms by Borcherdt (1972), Ferry (1961), White (1965), and others. The basic equations of linear viscoelasticity were derived (Appendix A-2) following the approach by Borcherdt (1972).

A linear viscoelastic model accounts for wave energy losses by replacing the Lamé constants of the Hookean equations with complex Lamé constants. In the expressions for rigidity,  $\mu_R + i\mu$ , and bulk modulus,  $K_R + iK_I$ ,  $\mu_R$  and  $K_R$  denote the elastic response; and  $\mu_I$  and  $K_I$  denote the damping of wave energy.  $\mu_I$ ,  $\mu_R$ ,  $K_I$  and  $K_R$  are all real and positive.

Anelasticity can be measured in terms of the dimensionless quantity  $Q^{-1}$  (specific attenuation factor). It is defined as the  $(2\pi)^{-1}$  times the ratio of the energy dissipated per cycle of forced oscillation to the peak energy stored during the cycle (Borcherdt, 1972, p. 12). The specific attenuation factor in terms of velocity (V), attenuation (a), and circular frequency ( $\omega$ ) is found to be

$$Q^{-1} = \frac{2Va/\omega}{1 - (aV/\omega)^2} .$$
 (5)

The specific attenuation factor for dilatation,  $Q_p^{-1}$ , and shear  $Q_s^{-1}$ , are found by substituting the appropriate subscript p or s in equation (5). Note that because velocity is independent of frequency and attenuation is proportional to frequency,  $Q^{-1}$  is independent of frequency.

For a low-loss material (0 <  $Q^{-1}$  <, 1), aV/ $\omega$  << 1; and therefore, (5) may be approximated by

$$Q^{-1} \quad \frac{2Va}{\omega} \quad (6)$$

The expressions for the phase velocity of a homogeneous plane compressional wave  $(V_p)$  and shear wave  $(V_s)$  in a linear viscoelastic medium is given by:

$$V_{p} = q_{p} - \frac{K_{R} + \frac{4}{3}\mu_{R}}{\phi}$$
 (7)

$$V_{s} = q_{s} \frac{\mu_{R}}{\phi}$$
 (8)

where  $\phi$  = sediment density

$$q_{p} = \frac{2(1 + Q_{p}^{-2})}{1 + Q_{p}^{-2} + 1}$$
(9)  
$$q_{s} = \frac{2(1 + Q_{s}^{-2})}{1 + Q_{s}^{-2} + 1} .$$
(10)

As energy losses become small,  $Q_p^{-1}$  (or  $Q_s^{-1}$ )  $\rightarrow 0$  and  $q_p$  (or  $q_s$ ) approach unity. Thus, (7) and (8) can be simplified to the familiar expressions for velocities in an elastic medium:

$$V_{p} = \frac{K_{R} + \frac{4}{3} \mu_{R}}{\phi}$$
(11)

$$V_{\rm S} = \frac{\mu_{\rm R}}{\phi} . \tag{12}$$

-il

The question remains whether ocean bottom sediment can be classified as a low-loss medium. If so, the low-loss approximations, equations (6), (11), and (12) can be employed to describe wave propagation. Figure 5 illustrates the low-loss error percentages as a function of  $Q_s^{-1}$ . Values of  $Q_s^{-1}$  ranging from approximately .5 to 2.0 have been reported by Hamilton (1970a, p. 4048) for ocean bottom sediments off San Diego, California [assumption of no absorption in bulk ( $K_I = 0$ ) was used]. Referring to figure 5 and to the viscoelastic equations discussed above, error percentages ranging from 8 to 43 percent can be introduced in determining the velocity of a homogeneous shear wave, and the error percentage for the corresponding absorption coefficient is between 15 and 184 percent. Because these large errors are not permissible, the linear viscoelastic equations are needed in determining the shear wave velocity or Lamé constants.

Hamilton (1972, p. 635) also reports  $Q_p^{-1}$  values ranging from 0.002-0.044 for sediments off San Diego. The low-loss approximations, equations (6) and (11) can be used without any significant error.



Figure 5. Low-loss error percentages for the velocity and absorption coefficient of a homogeneous shear wave with  $0.05 \le \cdot Q^{-1} \le 1.0$ .

Density,  $V_p$ ,  $V_s$ ,  $a_p$ , and  $a_s$  were measured to calculate  $Q_s^{-1}$ ,  $\mu_R$ ,  $\mu_I$ ,  $K_I$ ,  $K_R$ , Poisson's Ratio, and Young's modulus. Appendix A-3 lists equations that can be employed to determine various sediment properties from the measured properties.

### 3.3 Direct Current Resistivity

The electrical resistivity of a geologic material is related to the porosity of the material (Archie, 1942) with low resistivity implying high porosity. Resistivity ( $\rho$ ) is numerically equal to the electrical resistance between opposite faces of a cube of the material of unit dimensions, and usually is measured in ohm-centimeters (ohm-cm) or ohm-meters (ohm-m) (Grant and West, 1965). As porosity is related to sediment composition

and engineering properties, *in situ* measurement of the vertical variation of the electrical resistivity of marine sediments can provide information about sea-floor structure.

Resistivity techniques are well established on land, and increasing use is being made of them in the offshore environment. Measurements of resistivity and porosity of sea-floor sediments indicate that, except for some highly conductive clays, the formation factor of a marine sediment predicts the porosity of the sediment to within 5 or 10 percent. Formation factor F is defined by the equation:

$$F = \frac{\rho_s}{\rho_{\omega}} , \qquad (13)$$

where  $\rho_s$  is the resistivity of the sediment and  $\rho_{\omega}$  is the resistivity of the overlying water. Figure 6 summarizes some of these measurements. Measurement of the vertical resistivity distribution of the sea floor, therefore, provides a method of determining sub-bottom porosity, and thus geologic and engineering properties.





di.b

Resistivity is measured on land by borehole logging, or by the use of an expanding horizontal surface array. In boreholes, a small probe lowered into the hole provides a continuous record of the resistivity of the material surrounding the probe (Wyllie, 1963). This method has been adapted successfully to marine use by forcing a thin probe into the seafloor sediments (Bouma et al., 1971; Erchul and Nacci, 1971; Kermabon et al., 1969), as shown schematically in figure 7. The use of a logging probe in marine sediments gives a relatively precise determination of resistivity as a function of depth, but does offer some drawbacks. The depth of measurement is limited to the depth to which the probe can be inserted into the sea floor, which may be small where sediment porosity is low. The

\*

t:



Current I is fed into the sediments through current electrodes A, and causes a voltage  $\Delta V$  to appear between potential electrodes B. Sediment resistivity P is then determined from the formula

where K is a factor depending on the geometry of the els."rode arrangement.



volume of bottom material sampled is relatively small for a single insertion of the probe, so interpretation errors may result if the sampled point is not representative of the surrounding area.

Surface-deployed horizontal arrays also are used to measure vertical resistivity profiles on land (Grant and West, 1965; Van Nostrand and Cook, 1966). As shown in figure 8, this method employes two current electrodes to feed low-frequency, commutated direct current into the ground, and two (or more) potential electrodes to measure the potential difference generated at the surface of the earth by the impressed current flow. The potential difference is a function of the electrode configuration and spacing, and of the subsurface resistivity distribution.

For the equispaced Wenner array, shown in figure 8, the "apparent resistivity",  $\rho_a$ , of the earth is given by the formula (Van Nostrand and Cook, 1966)

$$\rho_{a} = 2\pi a \frac{\Delta V}{I} , \qquad (14)$$

where

 $\rho_a$  = apparent resistivity,

a = electrode spacing,

 $\Delta V$  = measured potential difference, and

I = impressed current.

By expanding the array, the current is forced to flow deeply into the earth, and the apparent resistivity at larger electrode spacings is indicative of the material at greater depth. The field data is interpreted by plotting measured apparent resistivity  $\rho_a$  against electrode separation a, and matching these curves with those contained in standard catalogs of published curves (Mooney and Wetzel, 1956), as shown in figure 9.

The use of an expanding horizontal array for sea-floor resistivity measurements (fig. 10) offers a number of advantages over the use of a probe. The depth below the sea floor to which information may be obtained is limited only by the size of the array and the amount of current available, and the measuring process does not disturb the sediments. Also, it is a simpler operation to deploy an electrode array on the bottom than to



A) Small Separation a.

 $V_1$  ond  $V_2$  are potential electrodes  $I_1$  and  $I_2$  are current electrodes



Current I is fed into the ground through the current electrodes  $I_1$  ond  $I_2$ , creating a potential field on the surface of the ground. The voltage difference,  $\Delta V$ , between the potential electrodes  $V_1$  and  $V_2$  is a function of the resistivity of the ground below the electrodes. The apparent resistivity of the ground, Pa, is close to P,

when the separation  $\sigma$  is smoll, and opproaches  $P_2$  os  $\sigma$  becomes very lorge.

Figure 8. Wenner resistivity array.



Figure 9. Standard Wenner resistivity curves (after Mooney and Wetzel, 1956).

drive a probe into the sea floor, and a larger volume of material is sampled at each data point. On the other hand, a probe gives a more precise determination of resistivity and structure at a given point. Also, data interpretation is easier and more accurate with a probe because there is no need to match curves to obtain the resistivity-depth relationship from the field data. The probe and horizontal array methods, then, are complimentary, with the probe offering greater precision, and the horizontal array greater penetration and convenience and less sediment disturbance.

For marine use, a horizontal array may be towed over the sea surface or deployed on the sea floor (fig. 11). A surface-towed array provides a fast and convenient method of mapping horizontal changes of average bottom resistivity over large areas. An array deployed on the sea floor, however, provides a more detailed picture of the bottom resistivity distribution at a given location. Both surface-towed and bottom-deployed arrays were tested for this study.





0

Upper Sediment Loyer

Lower Sediment Loyer

The surface array is easier to taw, but the sea flaar array is closer to the battam, so it can "see" battam resistivities at smaller values of the separation  $\sigma$ .

Figure 10. Surface and seafloor resistivity arrays.

#### 4. ELECTRONIC SYSTEMS AND EQUIPMENT

4.1 Reflection Coefficient Mapping Experiment

4.1.1 Acoustic Sources

The types of sources to be used for predicting the geology of the seafloor and its substructure vary depending on the type of sediments and and the problem being examined. For reflectivity measurements it is desirable to cover a broad spectrum containing varying energy levels, band width, pulse width, and beam concentration.

A) Array at Surface.



1

12



 $\Delta V$  is relatively insensitive to sediment resistivity.

B) Array on Sea Floor.



Measurement of  $\Delta V$  as a function of a could resolve thin sediment layers.

Figure 11. Marine resistivity arrays.

In standard sub-bottom profiling techniques, the aim is to get the highest energy level possible in a pulse of fairly short duration. While these requirements are not entirely divorced from frequency considerations, frequency enters into the argument mainly in terms of both discrimination and penetration. It is a common belief that for discrimination a high frequency is needed, while for penetration the converse is true; in fact some sub-bottom profilers now commercially available are designed to operate with a fixed frequency pattern to achieve a desired combination of penetration and discrimination. In general this is a misapplied conception, particularly if one desires to consider application to solving geotechnical problems. A seismic pulse should have a short time duration for resolution and a large energy content to combat noise. Multi-channel stacking is one way of achieving an effective increase in signal energy relative to uncorrelated noise for a given pulse width. Hutchins (1969), accomplished the same effect by using a large time-bandwidth (TW) product signal. The idea behind this appraoch is to generate a long duration signal having a bandwidth appropriate for the required resolution, to obtain more signal energy, and then to effectively time compress the reflected signals by matched filter detection. Such a signal can be band limited random noise, frequency modulated chirp or any other sufficiently complicated sequence. This concept has been applied in transmitting large TW signals used in radar and sonar to get maximum information from a single transmitted pulse. This is clearly necessary when targets are moving and probably taking evasive action.

When using seismic methods to map subsurface geologic structures buried beneath the ocean floor, it is often desirable to be able to control the energy versus frequency spectrum of the sonic pulse (Caulfield, 1962). Sediments have attenuation factors that are frequency dependent (Shumway, 1960). As greater penetration is required, lower frequencies must be used. Since the ability of a sonic system to resolve range differences depends on the wave lengths being employed, it follows there will be some pulse spectrum that provides the best compromise between penetration and resolution.
In this investigation, a discrete frequency wide-beam transducer operating at 12 kHz was selected to obtain a correlation with the values obtained by Breslau (1964), and a broadband multiple-electrode sparker source was tested as an alternate approach using a low frequency, omnidirectional beam.

Pawlowicz (1971) noted the principal differences between the continuous reflection profiling systems and those systems used for echosounding of water depth are the requirements of high acoustic power output and, generally, lower frequency. Although penetrations of 90 ft into the seafloor have been obtained with frequencies as high as 14 kHz (Murray, 1967), lower frequencies are more commonly used. A lower limit to the frequency used is set by the resolution desired -- layers thinner than one half the wavelength generally cannot be resolved clearly. Pulse duration is another important factor and must be short for good resolution. In general, 2

$$d = v\Delta t, \qquad (15)$$

P

where

d = distance between reflecting layers (m),

v = velocity of sound in water (1500 m/sec), and

 $\Delta t$  = difference between arrival times of the reflected wave fronts.

Thus, when the pulse length becomes comparable with the time of 2d/v sec, the two layers may not be resolved.

Each type of reflection equipment, which differs principally in the type of energy source utilized, has its own uses. For a comprehensive discussion of subbottom systems, see Schlank (1968). Briefly, a transducer system operating at frequencies of 1.5 to 12 kHz can show, under favorable circumstances (shallow water, silt and clay sediments), a subbottom structure to depths of 50 m, although penetration of 20 m is more common. This penetration is more than adequate to satisfy the objectives for determining surface sediment properties as done in this investigation.

Problems with fixed frequencies and various sediment types are as those noted by Moore and Kulm (1970), who describe a particular case:

> "No sub-bottom information was obtained with the 3.5 kHz profiler on the Oregon continental shelf. Cores taken off Oregon indicate that thick sand layers cover

much of the continental shelf and are overlain by only a thin layer of mud (a few cm thick) on the middle and outer shelf. Apparently the signal strength of the 3.5 kHz system is not sufficient to penetrate these sandy deposits."

Our experience with 3.5 kHz, 4.5 kHz, and 12 kHz sources also show little or no penetration in similar sediment even at the shallow depth: of area surveyed.

Discounting the need for subsurface structural data and considering only the surface one can say that for reflectivity measurements involving the sea-floor surface material only, it seems that any sound source may be used as long as the measurement of the pulses is proper for the sound source being used; however, for sub-bottom observations it appears that basically a low-frequency with broad-band characteristics is needed to allow both penetration and resolution of the sub-bottom layers. Regarding the high frequency sources (> 4 kHz), it has been acknowledged that standard echo-sounders can offer simple tools for the geologist to use, and with careful interpretation can provide a wealth of data impossible to obtain as simply by any other means. Any conclusion as to the presence of a certain sediment type should only be made after confirming evidence, such as by cores, grab samples, or photographs examined. The experiments conducted in this study, and in previous work (Barnes et al., 1972) with the 12 kHz and 41 kHz sources tend to corroborate this.

Two sources were used in the reflection coefficient mapping experiment described in this report. They consist of a standard 12 kHz transducer and a Directional Multi-Electrode Sparker Sound Source (DMSS) designed and built in this laboratory, and described in Barnes et al. (1972) and by Poston (1972). The two sources were used simultaneously in tests. The 12 kHz pinger transducer produced a pulse length of 1,625 m/sec, and the multi-electrode sparker sound cource transmitted a nominal pulse length ranging from .3 to .5 m/sec. The higher frequency source was discrete in bandwidth, but the low frequency source was composed of a nominal 2.5 kHz center frequency with a broad-band width with frequencies ranging from 0 to 5 kHz.

Ten of the twenty-five available electrodes on the DMSS sparker were used during the experiment (fig. 12). This number was selected after extensive testing of impulse output versus bubble pulse output for various numbers of grouped electrodes. For this experiment, 300 joules or 800 joules of output could be selected at the capacitor bank. 800 joules was used during the experiment. Several modifications to the previous system, described in Barnes et al. (1972), have been incorporated in this work. The sparker is mounted on a surface towed vehicle fabricated from a metal frame and 2 surfboards (fig. 13). Adjustment of the mounting bracket and DMSS unit makes it possible to orient the electrodes vertically downward into the water or upward toward the air/sea interface. An upward orientation proved better, since there was less bubble pulse and therefore less ambient noise interference. Vertical adjustment





Figure 12. Directional Multiple Electrode Sound Source (DMSS).





Not ta Scole

NOTE: 25 Electrode Holdere, 16 Electrodee Installed, The 4 verticol bare which holde the eporker mounting bar are graduated with 6" markinge.



of the source also enabled the position of the sparker to be varied relative to depth below the water surface. Tests conducted on various other seismic sources (i.e., the Uni Boom, the 3-electrode sparker manufactured by Egerton Germershausen and Grier, Inc., and a 12 kHz transducer manufactured by Ocean Sonics, Inc.) and other non spark-type discharge sources in this geometrical configuration indicated that the quality of the transmitted pulse could be improved if the source was moved closer to the water surface. This modification in geometry differs from the position used in the previous experiment conducted by Barnes et al. (1972). A marked improvement in the quality of seismic subbottom profiles was noted when the position of the electrode tips was set at about 10 inches below the water surface. In most instances, it was discovered that much more information could be obtained from the broadband frequency source. The essential requirements of any acoustic source employed for reflection coefficient mapping is that it should provide a broad frequency band acoustic pulse and be capable of analysing any changes produced in this pulse by the effect of the various bottom and sub-bottom reflectors.

Tests of the broad band multi-electrode sparker revealed much the same results as obtained by Li and Taylor-Smith (1969). Their comments about experimenting with a multi-electrode sparker, were, "as the number of sparks in parallel increased so did the efficiency of producing acoustic energy." In the tests conducted by this laboratory, the energy peaked and then decreased after the number of electrodes was increased at a given power level. Also, the duration of the bubble forming period and pulse length decreased, leading to a greater concentration of energy in the higher frequencies. The pulse length and frequency content was also found to be dependent on the spark gap distance. The DMSS gap was normally held constant, except when the electrodes burned too short and arced to the base of the sparker unit, producing a higher frequency output.

The two sources were used simultaneously and were triggered at a 1 sec rate by the recorder. The time base stability of the recorder is specified to be 10 parts per million. It was intended that the 12 kHz pinger source be triggered at a .25 sec rate and the sparker at a 1 sec rate with the use of an external countdown circuit and relay to trigger the capacitor bank, but the triggering operation was too erratic. The addition of a separate power supply and a one-shot multivibration circuit did not produce good results. After many trials the problem was found in the trigger circuit of the recorder. Random pulses caused the two sources to be triggered out of synchronization. A preliminary study of the repetition rate was done on the first tapes made, and it was discovered that a steady rate of 1 sec repetition was more desirable for computer processing efficiency.

#### 4.1.2 Hydrophone Receivers

There were three hydrophones used in conjunction with the sparker signal. The hydrophones monitored the transmitted and reflected signals.

The sled-mounted hydrophone were used to receive the transmitted impulses from the sparker. The two return impulse hydrophones used during the experiment were the streamer hydrophone and a second sled-mounted hydrophone, not shown in figure 13. The receiver hydrophones were identical in electronics, but of different lengths. The streamer hydrophone was approximately 11 ft long with a fluid filled section of 10 ft,  $8\frac{1}{2}$  in. The sledmounted hydrophone was approximately 6 ft long with a fluid filled section of 62 in. The hydrophones including the impulse unit measured 1-5/8 in in diameter. Six hydrophone elements make up each of the streamers. These are wired in parallel to a preamplifier and analog recording devices. The six hydrophone element sensitivity of 96 dB/u/ubar, yields a basic hydrophone sensitivity of 60.4 dB/u/ubar. The hydrophone arrays were calibrated using a Massa Model 115C calibrated hydrophone and a Navy J-9 sound projector. The measured test values were -60.5 dB/u/ubar.

The sled mounted return impulse hydrophone did not produce useable data during the test due to excessive ringing and noise, so all the processed data was obtained from the streamer hydrophone. Various attempts were made to remedy the problem, including the use of a less sensitive hydrophone, but this resulted in insufficient amplitude to tape the return signal.

All three hydrophones are connected to the same amplifier chassis; however, each hydrophone has its separate amplifier circuit and amplitude control. The streamer and return impulse hydrophone signals were at unity gain, the only amplifier being the preamplifier in the hydrophone array. The average effective dynamic range of the hydrophone is 35.4 dB. The low-end sensitivity is limited by the towing noise observed on the records. The nominal operating dynamic range is 66 dB when the low end sensitivity is limited only by ambient noise generated by the electronics. The streamer hydrophone output was also connected to the Ocean Sonics recorder through the rack mounted amplifier which has a gain of 67 or 37 dB.

The location and position of the monitoring hydrophones was also improved by modification of the tow vehicle. A more sensitive receiver hydrophone was installed in addition to the standard unit used in previous experiments (Barnes et al., 1972). The receiving and impulse hydrophones were placed 27 in and 32 in below the water surface, respectively.

In conjunction with the sparker, an EDO Type 324 (AT/200 A-UQN) transducer array using A.D.P. type crystals was used. The 12 kHz transducer, referred to as the pinger source, was driven by an Ocean Sonics Model GDR-T precision depth recorder. The transmitted pulse is rated at 600 w peak power into the rated 130 ohm load of the transducer, which yields an acoustical power output of 110 dB/v at one yard. The transducer has a beam angle of 35° on the frontal plane. The receiving response is -72 dB/v at one yard (EDO Western Corp.).

The pinger is mounted on the end of a two in diameter pipe which slides up and down in a socket at the end of an A-frame on the port side of the boat. The pipe is locked into position (fig. 14). The depth of the transducer was set at 1.8 ft below the water surface. The distance from the side of the boat to the center of the pinger was 29 in, and the distance from the center of the pinger to the stern was 16½ ft. This location was picked because of the wake characteristics of the boat and convenience of mounting. The A-frame was mounted to the deck with pillow blocks, allowing the transducer to be raised up and inboard, out of the way for portside docking and fast running.

#### 4.1.3 Transmission Measurements

During field tests, the transmission impulses and return impulses were constantly monitored on an oscilloscope. The pulses were visually compared to one another for the reflection measurement and to observe signal/noise ratios. This monitoring is very important since the amplitude of the return pulse is dependent upon: (1) The transmitted amplitude of the acoustic source; (2) the path length of the sound signal to and from the reflecting sea-floor; and (3) the reflecting properties of the sea-floor.

Because the amount of sound energy received is dependent on the amount of energy transmitted, and most echo sounders have no method of displaying the relative energy levels, one cannot assume that the transmitted energy is constant. A variation in the power supply voltage, for



Figure 14. 12 kHz source - tow configuration.

instance, can cause a considerable variation in the electrical power supplied to the transducer. Also, relationships existing between the physical properties of the transducer and those of the surrounding water, such as salinity will affect the output acoustic power even if the electrical power is constant. This is especially true of a sparker device. This has been calculated by Caulfield (1962, p. 342), i.e.,

$$E_{\text{Total}} = R_{\text{sp}} \frac{B^2}{2b} \frac{(k^2)}{(b^2 - k^2)},$$
 (16)

where

 $E_{Total} = Total energy to spark,$   $R_{sp} = spark resistance,$   $B^{2} = V_{0}^{2}/L^{2}K^{2},$  b = R/2L,  $k = (R^{2}/4L^{2} - 1/LC)^{\frac{1}{2}},$  R = Total resistance, L = Total inductance, and C = Total storage capacitance.

Equation (16) describes what occurs in the electrical circuit. For example, the spark resistance  $(R_{sp})$  can be affected by the salinity of the water, which can vary from 3.9 mho/cm for a 2 percent salinity solution to 43.9 mho/cm for a 35 percent salinity solution.

If one desires to know acoustical pressure, the equation approximating the first pressure pulse as a function of time is,

$$p = p_0 e^{-mt}$$
(17)

where

p

= pressure (dyne/cm<sup>2</sup>),

- $p_0$  = peak pressure (dyne/cm<sup>3</sup> at 1 meter),
- m = decay constant (determined by experiment), and
- t = time.

Then the acoustic energy is expressed as

$$E(t) = \frac{1}{pc} \int_{0}^{\infty} 1f(+)1^{2} dt,$$
 (18)

where

 $f(t) = p_0 e^{-mt}$ ,

 $p = density (g/cm^3),$ 

c = velocity of propagation in the medium, and

m = decay constant (determined by experiment).

Caulfield's formula can be correlated to Breslau's (1967, p. 2) formula in terms of peak pressure, i.e.,

$$\int_{0}^{T} P_{S}^{2} dt = P_{S}^{2} (RMS) \tau$$
(19)

where

 $P_s$  = pressure of the source pulse,  $\tau$  = duration of the outgoing pulse, and

T = time.

The formula used to obtain the values of the Rayleigh Reflection Coefficient from the digitized values of  $V_{SR} + V_{SX}$  may be expressed as;

$$R = .008 (4d^2 + 832)^{\frac{1}{2}} \frac{V_{SR}}{V_{SX}}$$
(20)

(see Appendix A-1),

where

R = Rayleigh Reflection Coefficient,

d = water depth,

 $\boldsymbol{V}_{SR}$  = sparker return impulse voltage, and

 $V_{SX}$  = sparker transmitted impulse voltage.

The 12 kHz pinger transducer transmitted output was also constantly monitored during field operations. The voltage applied to the transducer was monitored on an oscilloscope while being recorded or an analog magnetic tape recorder. A fixed resistive attenuator network to reduce the high transmitted voltage to a lower voltage was necessary to make the recording level suitable for input to the tape recorder. The return impulse voltage was amplified by the internal amplifier of the recorder, (section 4.1.2) and then connected to the magnetic tape recorder.

Many problems were encountered in calibrating the pinger transducer due to the small physical size of the tanks available for calibration use in the laboratory, and the large amplitude of the transmitted signal. Proper interrogation of the pulse to be used for the computation of the reflection coefficient was threefore difficult. Problems were encountered in obtaining repetitive measurements using a calibrated hydrophone and a sound source in the laboratory tanks. The calibrating hydrophone and source are linear devices, and the pinger transducer is a tuned, or non-linear device; the ratio of the transmitted to received voltages of the transducer itself could have been used to obtain an absolute calibration value, but physical limitations of calibrating in the tank resulted in relying on manufacturer's specifications. Data for obtaining absolute values of the reflection coefficient were taken from the manufacturer's calibration specifications.

Output variations based on reflectivity measurements have been observed by others employing various kinds of sources. Li and Taylor-Smith (1969, pp. 244, 245) noted intensity fluctuations using a sparker source. In their examination of the intensities of sound returned to the surface, both from bottom and sub-bottom reflections, it was thought that the loss in intensity might be attributed to a large number of factors, of which absorption in the medium is only one. They go on to say that, the effect of the unwanted losses may be reduced from a statistical point of view, by a procedure used in spectral analysis, in so doing, it is essential that each set of pulses analysed is taken from single individual transmissions. However, even with this procedure, a fluctuation in the intensity of the reflection can be caused both by the irregularity of the reflecting

boundaries and by the irregular structural pattern of the medium between the bottom and sub-bottom. The effect is mainly one of random scattering, although interference effects probably play an important part as well. Such fluctuations in intensity may be of diagnostic value.

A statistical study of various reflected signals shows that the observed fluctuations appear to correlate with the state of roughness of the reflector (section 3.1, fig. 3). Li and Taylor-Smith (1969) found that, the standard deviation of the intensity fluctuations, expressed as a percentage of the mean peak to peak amplitude, for the signal transmitted directly through the water from a 3-electrode spark is about 4 percent, whereas the figure obtained for a reflection from a sand-covered floor is about 10 percent and from a clay-covered floor 5 percent. While these are isolated results from which general conclusions cannot be drawn, it is certain that scattering due to boundary roughness is an important factor in signal fluctuations. In fact, this property is employed in the attempt to use the oblique asdic (or sideways-looking sonar) to define the sedimentary state of the sea-floor (Chesterman et al., 1967).

Intensity fluctuations were also noted by Porter and Bell (1972) in their reflection studies with 3.5 kHz and 5 kHz electromechanical transducers. A bottom variation of  $\pm$  0.85 dB was noted in this experiment, which was conducted with the source mounted in a rigid frame on the bottom of an inland body of water. They found that "the magnitude of the observed variations differed significantly and exceeded the values calculated for the component attributable to source-receiver motion," and "concluded that other error sources such as changes in incident angle and undetected spatial variability in sediment composition exist." They did note that a reduced degree of variability was exhibited in the narrow-band data, which they attributed to an inherently higher signalto-noise ratio.

## 4.1.4 Data Acquisition and Display

The shipboard recording equipment consists of a rack-mounted Ampex SP-300, 7-channel magnetic tape recorder. The inputs to the seven

channels are:

Channe1	1:	Pinger transmitted voltage (fig. 15a)
Channel	2:	Sparker received impulse (Streamer Hydro- phone) (fig. 16a)
0	<b>•</b> 1	Discuss used in the IFA 15h

Channel 3: Pinger received impulse (fig. 15b)

Channel 4: Voice

Channel 5: Sparker transmitted impulse (fig. 16b)

Channel 6: Time Code, IRIG B

Channel 7: Sparker received impulse (sled hydrophone).

An Ocean Sonics Series GDR-T precision Sonar recorder and transceiver was used as a visual recorder for the subbottom profile record (fig. 17). The recorder was modified to allow the presentation of either the 12 kHz pinger or the 0.5 kHz sparker seismic returns. The recorder unit also houses the energy source for the 12 kHz pinger transudcer. The output voltage of the transmitter in the recorder to a 35 dB attenuator network to reduce the output voltage to a suitable taping level.

The recorder is also modified so that the amplified output of the received pinger impulse is connected to the tape recorder. The amplifier gain can be changed from a setting of X1, which produced a measured gain of 50, to a setting of X50, which produced a measured gain of 920. Electrical cables from the streamer, and sled-mounted amplifier. The output of the streamer hydrophone was fed from the hydrophone preamplifier to the tape recorder. The streamer hydrophone output was also connected to a Khron-Hite Model 3200 Variable High or Low Pass electronic filter and then to the main hydrophone amplifier. This amplified output was connected to the Ocean Sonics recorder to observe the 2.5 kHz sparker subbottom information. The output from the receiver hydrophone was fed through an attenuator network to the tape recorder. During the survey the attenuation was set at unity. The sparker impulse hydrophone output was fed through an amplifier to the tape recorder. Throughout the survey the amplifier gain was set at unity. The voice channel was used to note the same events noted in the tape roll logs. The time reference for the magnetic tape record was obtained from an IRIG B Time Code generator. The generator and the digital clock used in the navigation system were



a. 12 kHz transmitted signal.



b. 12 kHz received signal.

Figure 15. Pinger (12 kHz) signal wave forms.



a. 0-5 kHz transmitted signal.



b. 0-5 kHz received signal.

Figure 16. Sparker signal wave forms.



Figure 17. Typical subbottom profile record — Monterey Bay test area.

set together in reference to WWV. The timing lines on the Ocean Sonics recorder were also controlled by the same digital clock marker interface so that the locations of the seismic records could be accurately determined.

#### 4.2 Shear Wave Experiment

4.2.1 Shear Wave Generator

The seismic source needed for the shear wave experiment had to fulfill three important requirements. It had to (1) be rich in shear energy, (2) generate waves of low frequency (as attenuation is proportional to frequency), and (3) discharge its energy below the water-sediment interface. Several sources were considered as to their capability of fulfilling the above requirements. A 3 cubic inch air gun was the final choice as a shear-wave generator. Attached to the air gun is a probe, a hollow 1 in diameter pipe with a coned point. The energy from the air gun is transported through the probe and then out through an exit port at the base of the probe (fig. 18). By implanting the 0.5 m long probe into sediment, the source energy is discharged below the water-sediment interface.

The exit port is located on the side of the probe to generate shear waves (in particular,  $S_H$ -waves). With the probe implanted into the sediment, water and a little sediment fill the hollow probe. When the gun is fired, the water-sediment mixture is forced out, producing a horizontal jet stream. Thus, the basic procedure to detect the direct  $S_H$ -wave involves a linear array of transverse geophones with the  $S_H$ -source placed at one end of the detector array. The sensitive axis of the geophones and the water stream from the probe are parallel and both are transverse to the direct  $S_H$ -ray path. This alignment of horizontal geophones and  $S_H$ -source produces maximum response to the direct  $S_H$ -wave (fig. 19).

The 3 cubic inch air gun (Rix Industries, Emeryville, California) is cylindrically shaped with a height of 5 in and diameter of 3 in. The high pressure air escapes from a large diameter single opening which provides for a concentration acoustical source. High pressure is supplied to the air gun from a single Scuba diver's tank. Employing a Scuba tank provides a high pressure range from 300 to 2200 psi; however, a high pressure setting of 600-1000 psi is used for tests. The air gun is remotely triggered by a switch which activates a solenoid, which in turn discharges the high pressure air in the piston chamber of the air gun.

There are two inherent disadvantages concerning this  $S_{\rm H}$ -source. One obvious problem is sediment disturbance. Although the water stream from the probe is directed away from the sediment between the source and receivers, the air discharged into the sediment will eventually produce a cavity after several firings. This reduces the efficiency of producing  $S_{\rm H}$ -waves. The second disadvantage arises when the piston of the air gun returns after discharging the air. This draws excessive amounts of sediment into the probe, causing a blockage. The latter problem was rectified by attaching a fine wire mesh across the exit port, allowing only fines to enter the probe.





ELEVATION



PLAN

Figure 19. Probe for measurement of shear-wave and compressional-wave velocity.

Altogether, the source proved to be successful in producing low frequency shear waves in the frequency ranges from 80-100 Hz.

#### 4.2.2 Receiver Array

Four geophone units are employed to record the arrival of the direct shear wave and direct compressional wave (denoted hereafter as direct  $S_H$ -wave and direct p-wave, respectively). Each unit contains a vertical geophone to respond to the direct p-wave and a horizontal geophone to respond to the direct  $S_H$ -wave (receivers manufactured by Geospace, Houston, Texas; HS-5 Model K, standard frequency of 30 Hz). Each geophone unit was equipped with a 1 in brass stake used to implant units.

#### 4.2.3 Signal Processing and Display

The four horizontal geophone outputs are fed into a four channel voltage gain amplifier. The amplified signals are then displayed on a dual beam oscilloscope which was converted to four independent traces with a Tektronix 1A-4 plug-in. The sweep on the oscilloscope is triggered from the same circuit that triggers the solenoid of the air gun. Unfortunately, the air gun has a 52 m/sec delay time between the activation of source trigger switch and the actual firing of the air gun. Thus, it became necessary to delay the sweep on the oscilloscope in order that the signals could be viewed on the oscilloscope screen. The signals are recorded by means of a single exposure oscilloscope camera equipped with Polaroid type 47 film, 3000 ASA speed. The process is repeated by feeding the four vertical geophone outputs into the amplifier and then recording the signals displayed on the oscilloscope with the camera. A functional block diagram of this simple system is shown in figure 20.





47

h. . . . . . .

## 4.3 Electrical Resistivity System 4.3.1 Expandable Array

The system used to measure bottom resistivity was designed for operation by two Scuba-equipped divers, and consists of separate current and voltage units, a length of line on which electrode separations are marked, and an underwater slate for recording data (fig. 21). The output of the current source is a 10 Hz square wave, with amplitude adjustable between 0 and 500 milliamps in 10 steps. Power is supplied by 16 D-size rechargeable nickel-cadmium batteries, in a series-parallel arrangement which provides a nominal 10 V output. Current electrodes are 1/4 in (0.64 cm) diameter copper rods, 1/2 in (1.27 cm) long.

The voltage unit is a high-impedance (10<sup>6</sup> megohms) rectifying voltmeter, with ranges of 1, 10, 100, 1000, and 10,000 MV full scale, and is powered by mercury batteries. The potentia? electrodes have a nonpolarizing silver-silver chloride element, housed in a 1 in (2.54 cm) outside diameter cylinder of acrylic plastic, and are filled with 2.7 molar potassium chloride saturated with silver chloride. Electrical



Figure 21. Schematic diagram – diver operated direct current resistivity apparatus.

contact to the sea water is made through a rod of porous ceramic material (Corwin, 1973). Each range of the voltmeter is calibrated by measuring a known voltage across a fixed resistor.

Both current and voltage units are housed in clear acrylic plastic cylinders of 6 in (15.2 cm) inside diameter and 1/4 in (0.64) wall thickness, which allows direct reading of the current and voltage meters. All openings are sealed with 0-rings, and the housings are pressurized to a few psi before immersion to check for leaks. The length of the current unit is 12 in (30.5 cm); the voltage unit,  $9\frac{1}{2}$  in (24.1 cm). Sufficient cable is provided for an electrode separation a of 300 cm in the Wenner configuration.

In use, a line marked for Wenner electrode spacings of 10, 30, 100, and 300 cm is laid on the bottom with the ends anchored by diving weights. The current and voltage electrodes then are inserted into the sediment at the locations for a = 10 cm and the value of the voltage,  $\Delta V$ , recorded for several values of the current, I. The electrodes then are moved to the next larger electrode spacing and the procedure repeated. As skin contact resistance is low in salt water, care is taken by the divers not to touch the current electrodes during operation, to avoid the possibility of a dangerous shock.

Electrode spacing, current, voltage, and water depth and temperature are recorded on the slate, and a water sample is taken for later salinity measurement. Although slow and cumbersome to use, especially in water of poor visibility, this prototype system proved reliable in field operation, and yielded data which appears to confirm the feasibility of the sea-floor horizontal array concept.

#### 4.3.2 Surface Towed Array

The equipment used for the towed resistivity array field test is shown schematically in figure 22. A 12 V lead acid automotive storage battery was used as the current source, with current I controlled by the rehostat and monitored on a 0-10 amp ammeter. The switch allows the current to be turned off to monitor the zero level of the potential electrodes, or for polarity reversal. The current electrodes were strips of

zinc-coated pipe hanger strap, 1 in (2.54 cm) wide by about 2 ft (60 cm) long. The potential electrodes were of the silver-silver chloride type described above, and, potential  $\Delta V$  was read on an Esterline-Angus model T171B battery operated strip chart recorder.

In operation, the current electrodes are energized continuously over areas of interest, and are turned off for about 30 sec every 5 min to check the zero level of the potential electrodes. Typical chart recorder output is shown in figure 23. Current polarity is reversed every so often to equalize electrolytic dissolution of the current electrodes. Although continual reversal of current electrode polarity would have, in effect, doubled the signal-to-noise ratio, it was found that this procedure resulted in a degraded current waveform, with several minutes required for the current to reach a stable value. As the current was not recorded continuously but was read from the meter and noted on the potential record, it was decided to hold current polarity constant as described above.

#### 4.4 Sediment Sampling Equipment

To verify reflectivity measurements, it was necessary to have a core sampler which could rapidly sample the first meter of surface sediments and obtain relatively undisturbed samples. This was required for reliable engineering and mass physical property determinations. A core







Figure 23. Towed resistivity - chart recorder output.

length ranging from 1 m to 3 m was decided upon because this depth of penetration was within the wave length of either of the two acoustic sources employed for the reflection coefficient mapping experiment (section 4.1.1). This also made it possible to determine to what degree changes in the near surface lithology or small scale structural variations would influence the reflection coefficient. Several samplers of the gravity-type or piston-type gravity corer variety were tested but found unsatisfactory. It was therefore necessary to design, construct, and test coring devices that would provide the desired capability. It was also decided that the energy source for penetration should not be percussion or rotary since these types tend to induce sediment distortion. Therefore, a steady thrust energy produced by pneumatic power was incorporated in one system for shallow (1 m) penetration, and a vibratory energy system was incorporated in another design to obtain deeper penetration (3 m).

#### 4.4.1 Diver Operated Core Sampler

Simplicity of operation and a minimal number of mechanical-electrical features were other criteria used for design of a shallow penetration corer. A study of figures 24 through 27 gives details of the design and hardware incorporated in the tool. Details covering the laboratory and field test of the diver operated corer are given in a report by Jenkins and Takeyama (1971). The field testing and evaluation of the prototype corer were done in conjunction with the sampling program for the reflectivity experiment conducted in San Francisco Bay (Barnes et al., 1972).



Figure 24. Diver operated sampler (retracted position).



Nat ta scale





Figure 26. Sample retainer - modified Shelby tube.

,h





## 4.4.2 Bottom Sitting Vibracorer

An impact-vibrator hammer and drill pipe mounted in a breakaway tripod (O'Brien and Duley, 1971) was used to sample sediments to depths of 3 m (fig. 28). These samples were tested in the same manner as the diver operated core samples, but their primary purpose was to provide lithologic information beyond the depth capability of the diver core sampler. The energy component of the system was a National Vibrator, Model BH-6. The weight of the vibrator is 350 pounds with an impact force of 240,000-in pounds. It uses 60 cfm of air at 70 psi and has a blow count of 110 blows per min. The exhaust ports are fitted with check valves to prevent water entering the cylinder.



Figure 28. Vibracorer.

The drill pipe is made from round mechanical tubing drawn over a mandrel with a 3.50 in OD, 3.25 in ID, and wall thickness of 0.125 in. The drill bit is made from regular schedule 80 pipe, flush jointed to the pipe, with an OD of 3.50 in, and an ID of 2.875 in. The bit was shaped with a crowd-out relief angle of 15° and is fabricated to permit the installation of polyethylene preloaded cartridge (fig. 29). The cartridge is used as a lining for the drill pipe to reduce wall friction, and because it is collapsible it also serves as a core catcher.

The vibrator and drill pipe are coupled together by a Victaulic coupling. Grooved ends of the coupling are welded to the drill pipe and vibrator.

To provide the support needed for near perpendicular penetration, and to ensure stability, a tripod was designed which supports the drill pipe in the upright position during the initial 1-2 m of penetration. Once penetration has proceded to this point, a coupling on the drill pipe



Figure 29. Plastic core retainer for vibracorer samples.

strikes the top of the tripod, causing the tripod to collapse and allows the drill pipe to continue to penetrate to full length. Sea floor slopes greater than 15° could cause the drill to overbalance and fall on its side. Complete details covering the design and fabrication of the tripod are covered by O'Brien and Duley (1971).

The combination of components used in this core sampler make a tool that is light in weight and simple to construct and operate.

## 4.5 Precision Navigation and Positioning System

A precision radio location system was used in all the field experiments and sampling programs described in this report. The main system, along with a specially designed subsystem, is used in referencing ship's position to geodetic control and for automatic real-time tracking during survey operations. The electronic subsystem enables real-time on-line digital and graphical conversion of lane identification coordinates to x-y grid coordinates. The subsystem was developed at this laboratory (Barnes and Newman, 1972). The ship's position and track are accurately located to within  $\pm$  3 m based on daily repeatability checks of the calibrated value which is fixed by a land survey and represents a geodetic control point at the ship's berth.

Three peripheral input/output units have been added to the system since the work reported in Barnes et al. (1972). These include a digital fathometer, a line follower meter, and a teletype. The programmable calculator/computer allows interface capabilities of these subsystems with the basic navigation-positioning data. A block diagram of the system is shown in figure 30.

## 4.5.1 Automatic Line Follower

The line-follower meter indicates the position of the vessel; whether it is left or right of the prescribed track line and is calibrated to read how far off track the vessel is, from 0 to 100 ft. The incorporation of the line-follower into the navigation subsystem allows the boat operator to follow a more precise course along the track line, and alleviates constantly monitoring the x-y plotter during survey operations.

## 4.5.2 Recording and Display

A U-Tech Model 109 digital depth converter was incorporated into the subsystem, along with a teletypewriter. The "red" and "green" lane counts, x-y coordinates, time of print-out, and digital depth is printed out by a digital printer at 2 min intervals on command from the program in the calculator. At the same time, the teletypewriter prints out the time, digital depth, and x-y coordinates. The format of the teletypewriter printout is conducive to quick observation of time, depth and position data. Greenwich Mean Time was used for all recorded events in the navigation and positioning operation and for all magnetic tape recordings of acoustical data.



.

\*Hewlett - Packord campanents

.

.



The track lines of the survey area were pre-plotted on transparent overlays referenced to conventional maps using the California State Grid Coordinate System (Lambert Coordinates). The scale of the overlays was 1 in = 1,000 ft. Each overlay sheet covered an area 15,000 ft by 10,000 ft, with the size of the sheet being limited by the size of the x-y plotter (fig. 31). The boat operator is guided by observing the line meter or the x-y plotter with the appropriate overlay installed. The vessel is thus directed along a desired track line or to pre-designated core sites.



Figure 31. X-y plot for navigation-positioning data.

#### 4.5.3 Indexing

A marker interface which simultaneously marks the records of the fathometer seismic recorder and Raydist strip chart recorder, and which also controls the Raydist digital printer, is activated by a digital clock contained in the subsystem interface. The digital clock gives selectable time outputs ranging from 1 to 30 min. Records are periodically annotated with time and station numbers, referenced to the vessel's location in lane counts which are printed out on the Raydist digital printer. In addition to automatic operation of the marker interface, the marker can also be manually operated to index events such as those that occur during maneuvering, marking of core sites with buoys, and calibration points.

## 4.5.4 Shore Stations

The two shore station sites required for operating the system were located approximately 15.5 miles apart, southeast of the operations area. The north, or "red", shore station was a 35 ft whip antenna powered by two thermal-electric generators and was located at a site south of Watsonville, California, directly over a USC&GS benchmark designated Holm. This site was at an elevation of 378 ft, and about 6 miles inland from the coast, in line of sight with the calibration point at the ship's berth in Santa Cruz Yacht Harbor. The south or "green" station was also a 35 ft whip antenna but was powered by 110 V ac. This station was located on the Fort Ord military base over a USC&GS benchmark designated Reserve, at an elevation of 485 ft, and approximately 2.6 miles inland from the coast, also in line of sight with the calibration point.

The "red" and "green" lane count — each lane equals 148.2282 ft from each of the respective shore stations to the vessel is automatically connected to x-y grid coordinates on a Hewlett-Packard Model 7100A programmable calculator. These coordinates are then fed to a Hewlett-Packard 9125A digital plotter to provide a real time, automatic plotting, navigation and positioning system. Mis-ties of an average of 0.058 lanes (8.59 ft) from the "red" station, and 0.087 lanes (12.89 ft) from the "green" station were noted at the calibrate point. Since the Raydist is a non line-of-sight system using ground wave transmissions, it was felt these mis-ties must be attributed to one or more of 3 causes, namely, (1) changes in the velocity of wave propagation, (2) insufficient warm-up time before commencing operations, and (3) characteristics of the propagation path from both shore stations, taking into consideration the overland distance, soil conductivity, surrounding vegetation, and topography.

# FIELD OPERATIONS 5.1 Test Sites

## 5.1.1 Monterey Bay

Following the 1971 San Francisco Bay Study (Barnes et al., 1972), three areas in Monterey Bay, California, were selected for continuing investigations (fig. 32). A program was laid out to conduct several surveys including sampling, subbottom profiling, bathymetry, photo reconnaissance, and tasks associated with the acoustical and electrical resistivity research studies. Only one area (Area A) was surveyed due to time restrictions and limitation of funds. The underwater photo reconnaissance was not attempted because of poor visibility. Actual production on the area selected coincides well with the program intended (fig. 33). Sampling with the diver operated corer was limited to water depths less than 140 ft, thus reducing the number of core sites occupied. Possible hazard of decompression sickness was the main reason for not extending core operations into deeper water.

The bottom sediments in the survey area are of several types, consisting of gravels, medium to fine sands, silts and muds. Hard limestone reefs extend seaward from Point Santa Cruz and Point Soquel, and a generally northeast-southwest trending reef outlier parallels the east edge of the Point Santa Cruz extensions. The bottom topography is generally smooth with a gentle southerly gradient except in the reef areas (fig. 34). The wide variability of sediment/rock type, shallow water and even topography were factors influencing the selection of the area.



Figure 32. Monterey Bay test site location and program map.


Figure 33. Acoustic and electrical resistivity coverage - core site index map.



Figure 34. Bathymetric map, Santa Cruz Harbor - Monterey Bay.

<u>.</u>

The program in Area A comprises 30 square miles and was designed to apply fine grained survey practices. A seismic grid pattern of 600 ft line spacing was laid out for the continuous reflection coefficient mapping survey and towed electrical resistivity survey. Core locations were designated in two densities, the shallow water area grid was laid out with hole spacings of 1,200 ft by 1,200 ft, and the deep water area reflects hole spacings of 3,000 ft by 3,000 ft (fig. 34). Two samplers were used, but the data presented in this study was mostly confined to correlations based on analyses from the 1 m core samples taken using the diver operated corer.

### 5.1.2 Other Test Sites

Field tests for the shear wave and electrical resistivity experiments were conducted at two other sites besides Monterey Bay. The first test area was located in lake sediments near Fort Cronkite, San Francisco, California. This body of water, which was once open to the ocean, is now separated by a beach of gravelly coarse sand. Shear wave experiments were carried out on the seaward side of the lake in homogeneous beach material. The test site offered the advantages of an unlayered sediment and no hinderance from water wave action and tides. Electrical resistivity tests were conducted at another test site located in a small bay off the Gulf of California, Mexico (section 5.4.1).

Experiments employing both the shear wave generator and electrical resistivity apparatus were conducted in Monterey Bay, California. Shear-wave measurements and core samples were obtained at ten stations (refer to fig. 33 for location of stations 94, 103, 118, 121, 122, 123, X-1, X-2, X-3, and mooring). These stations were selected on the basis of providing a wide variability of sediment type. Although the method of measuring shear-wave was essentially the same at the Fort Cronkite and Monterey Bay test sites, the field procedures were different and will be treated separately. Figure 33 also shows locations and track lines where the direct current resistivity experiments were conducted.

z

### 5.2 Reflection Coefficient Mapping Experiment

The reflectivity experiment was performed aboard the R/V Doodlebug (fig. 35) with a three man crew, consisting of a navigator, an electronics technician, and a boat operator. The navigator monitored the precision navigation system and attended to operation of subsystems for accurate tracking and position. He also annotated the various location and depth recording printouts, as well as preplotting and selecting the lines :o be traveled as indicated on the x-y plotter and by the line follower (fig. 36).

The electronics technician operates, calibrates, and monitors the subbottom and reflectivity measuring and recording apparatus and annotates the subbottom profile records and magnetic recording tape logs (fig. 37).

The first operation of the day was to calibrate and set all the navigation devices while tied up at the berth, which was also the navigation calibration point. The tape recorder, signal generator and ac voltmeter also were turned on and allowed to warm up before commencing operations. The boat then proceeded to the selected track lines and/or core sites for the day. Buoys marking the core sites were numbered in reference to the site number, and track lines were accordingly annotated (fig. 33). Buoys left overnight at a prescribed core site would be rechecked to insure that they were still in position on the site.



Figure 35. R/V Doc Ilebug.





Figure 36. Navigation system — shipboard setup.

Figure 37. Underway acoustic — shipboard setup.

The seismic and reflectivity operation consisted of loading the tape recorder with a pre-numbered magnetic tape roll. The channels were calibrated and the signal generator set to either 2.5 kHz or 12 kHz and adjusted to a 1 V rms output. The two pinger monitoring channels on the recorder were calibrated. The 12 kHz signal was observed for 60 to 90 sec or more while the tape output level was monitored by the ac voltmeter and oscilloscope. Each tape channel gain control was adjusted for a 1 V rms indication on the voltmeter. The same procedure was used to calibrate the sparker monitoring channels, except that a 2.5 kHz signal was used. The calibration times are noted in a tape log, which was kept constantly for purposes of noting regular gain changes and malfunctions.

Shortly before the line to be profiled was reached, the boat was stopped and the pinger transducer lowered into the water and pinned into position. The sparker sled was lowered from its secured position on the stern with the deck crane and allowed to drift to a position about 50 ft astern of the boat, where it is secured for towing.

The streamer hydrophone was removed from its protective tube and streamed behind the boat until it was about 20 ft astern. An outrigger holding the cable was then extended so the hydrophone was approximately 14 ft from the starboard side of the boat. This aft and side tow positioned the hydrophone outside of the turbulence caused by the boat's wake, thus reducing the noise level.

While the boat headed to the start of the survey line, the precision recorder, power supplies, and capacitor bank are energized. The boat operator then set the throttle to a prescribed rpm for attaining constant boat speed (3-4 knots).

The subbottom profiling record was annotated in selected time measurements of 2-5 min and the record scanned periodically to determine that all was in proper working order. The sound source impulse and receiver waveforms were monitored on the oscilloscope so that all the amplifier gains could be adjusted and logged before the tape recording commences. Upon reaching the starting point of the desired track line, the tape recorder and paper tape punch is started and the time is noted in the tape roll log, as well as on the voice channels of the tape recorder.

During the profiling operation the sparker and pinger transducer receiver-signals were constantly displayed on the dual channel Tektronix Type 422 oscilloscope, except for occasional monitoring of the transmitted impulse waveforms. The monitoring of all these signals was done to insure that the amplitudes were of the proper level for taping and that no distortion was present because of noise or a malfunction of any system. The presence of all signals, including the IRIG B-Time Code and voice, could be observed on the recording level meters on each recording amplifier of the tape recorder.

The tape roll logs and verbal comments on the voice channel included the tape roll number, data, line traveled, direction of travel, calibration times, any change in recording amplitudes, signal source changes, and the start or stopping of the tape recorder or sources. Also noted were sea conditions, and occasionally noting sparker impulse waveforms or any other events the technician thought may influence the waveforms.

## 5.3 Shear Wave Experiment

## 5.3.1 Field Procedure - Lake Test Site

The electronics, powered by a portable generator, were stationed on the beach and the probe placed in sediments that were under about 2 ft of water. Both the source probe and receivers were implanted by hand to a depth of approximately 0.43 m (some experiments were conducted with the receivers at a shallower depth). The four geophone units forming the linear receiver array were inserted at predetermined distances from the source through the use of a 1 ft scaled rod. Various geophone unit spacings were employed, but the end geophone unit of the array was never further than 5 ft from the source. The experiments conducted consisted of recording the signals from the four horizontal geophone outputs, the four vertical geophone outputs, or two horizontal and two vertical outputs. Each experiment required 2 to 4 source shots to obtain the correct amplitude, delay time, and time scale settings. After acquiring data for a particular experiment, the source and receivers were relocated to a nearby station of the same sediment type, and a new test was then conducted.

# 5.3.2 Field Procedure - Monterey Bay Test Site

Because the investigations in Monterey Bay were performed in deeper water, an additional system was required to implant the source probe. The system employed was attached to the diver operated sampler (fig. 38). The system basically includes 3 air operated pistons that displace an inner frame in an up-down vertical motion. The source, attached to the frame moves up or down a track mounted on an outer frame. When on site, the sampler with  $S_{\mu}$  generator is lowered to the bottom with the inner frame in its highest position (i.e., the pistons are retracted). On the bottom, the divers actuate valves located on the outer frame. This high pressure air released from a scuba tank forces the pistons to move, resulting in the inner frame and the attached source with probe to be forced downward into the sediment. To retract the probes, the valves are reversed causing the pistons to raise the inner frame and source probe from the sediment. No difficulty was experienced in employing this source insertion method, even in gravelly coarse sand.

Before lowering the entire system, the four geophone units were correctly aligned and then clamped into four circular rings on a split bar. With the corer resting on the bottom and after inserting the source probe, a diver removed the split bar from the corer assembly. Two holes on one end of the split bar were positioned over two corresponding vertical rods located on the corer baseplate near the source (fig. 38b). The divers pushed the split bar down over the rods with the simultaneous penetration of the stakes on the geophone units into the sediment. The divers then detached the split bar leaving the geophone units implanted. This procedure provided a linear array of evenly spaced and correctly oriented receivers (1 ft spacings with the end unit 4 ft from source). In some cases the divers implanted the geophones without the split bar.

After inserting the geophone units, the divers surfaced, and acoustical measurements were taken. The first two stations went well, but a current drain into the geophone outputs developed at the third station. This caused the traces on the oscilloscope to wander extensively when recording. The current drain was traced to the 30 V battery triggering the air gun. The problem was partially rectified, although



there remained some wandering traces. Several source shots were required to position and focus in on the desired wave arrivals. An average of 24 shots per station were needed to obtain the necessary information on the direct  $S_H$ -wave and direct p-wave; therefore, sediment disturbance could be extensive, even though the water stream plus air from the source probe were directed away from the propagation path of the acoustical waves.

Once the data acquisition was completed, the divers submerged, retracted the source, and removed the geophone units. Core samples were then obtained at the site. The complete cycle of positioning the ship, inserting the probes, acquiring acoustical data, removing the probes, and core sampling took between 3 and 4 hours.

## 5.4 Direct Current Resistivity Experiment

## 5.4.1 In Situ Array

The electrical resistivity system was tested in two locations: a small bay off the Gulf of California at Puerto Escondido, Baja California, Mexico; and Monterey Bay, California. The tests at Puerto Escondido were conducted in warm, clear water over a carbonate sand bottom; those at Santa Cruz were in cold, murky water over a bottom of silty sand. The *in situ* array was tested at both sites.

In actual operation, a line marked for Wenner electrode spacings of 10, 30, 100, and 300 cm, is laid on the bottom, with the ends anchored by diving weights. The current and voltage electrodes then are first inserted into the sediment at the locations for a = 10 cm and the value of the voltage,  $\Delta V$ , recorded for several values of the current, I. The electrodes then are moved to the next larger electrode spacing and the procedure repeated. As skin contact resistance is low in salt water, care is taken by the divers not to touch both current electrodes at the same time, to avoid the possibility of a dangerous shock.

Electrode spacing, current, voltage, and water depth and temperature are recorded on the slate, and a water sample taken for later salinity measurement. Although slow and cumbersome to use, especially in water of poor visibility, this prototype system proved reliable in field operation, and yielded data which appears to confirm the feasibility of the sea-floor horizontal array concept (fig. 39).

### 5.4.2 Surface Towed Array

The simple towed resistivity system described in section 4.3.2 was tested only in Monterey Bay. The purpose of the test was to determine whether data consistent with bottom properties which were measured directly or were determined by other geophysical methods could be obtained by a towed resistivity system. A Wenner array was used, with electrode spacing (a) of 20 m. While it would have been desirable to run several array spacings, extending the array spacing to greater distances proved impractical.

The array was towed from the R/V Doodlebug at about 3.5 knots, using the Raydist navigation system described in section 4.5. The track covered is shown in figure 33.



Figure 39. Formation factor vs a-spacing: diver operated Wenner array.

#### 5.5 Sampling Program

The objective of the sampling program was to determine by core analysis, the mass physical and engineering properties of the sediments at the same locations where acoustic reflectivity measurements were made.

The sampling program was conducted aboard the R/V NOAA's Ark, a converted LCM6, 56 ft LOA and 14 ft beam (fig. 40). An 8-ton telescoping crane mounted on the bow of the vessel was used to raise and lower equipment over the side. Air for the pneumatic-hydraulic diver-operated sampler and the vibratory sampler was supplied by a compressor delivering 85 cfm of air at 100 psi.

Since the sampling ship did not have an accurate positioning system, the R/V Doodlebug would drop marker buoys at each drill site for the projected day's work program. The choice of the mooring used at each location (one or two point anchoring), was dependent upon the water currents, wave action and the velocity and direction of the wind. After anchoring, the sampler to be used at a particular site was prepared and put over the side of the ship.

A vibratory bottom-sitting drive sampler (open type) was used on several occasions. This unit was capable of obtaining a sample 300 cm long and 7.0 cm in diameter. Sampling with this equipment was limited to nine core sites due to operating difficulties (section 4.4.2, fig. 28).



Figure 40. R/V NOAA's Ark.

The procedure for obtaining a core with the vibracorer was to lower the vibracorer and tripod slowly over the side of the vessel until the tripod reached the bottom.

This was indicated by a float attached to the tripod with a length of line the same depth as the water. When the float sank into the water the tripod was on the bottom. At this point, the air was turned on to the impact-vibrator and the winch operator played out the wire rope attached to the upper part of the vibrator. When there was slack in the wire rope it was determined by experience that the drill pipe had penetrated to its maximum depth. The winch operator would then raise the vibrator and tripod slowly onto the deck.

When the diver operated sampler was used, divers were sent down to activate the valves for coring, remove and seal the sample tube, and replace another tube into the sampler. A cycle of operation including anchoring, sampling, retrieving the anchor, and moving to the next sample site would take approximately 1-2 hrs. When wind and water conditions were favorable and malfunctions or equipment breakdown occurred, five sample sites could be cored in one working day. Depth of water and the number of divers available also affected the number of samples taken per day. All dives were made within the U.S. Navy, no decompression limits. At 125 ft a sample could be taken within a total, surface-bottom-surface, time of 6 min.

Most of the samples were obtained with the diver-operated, bottomsitting drive sample" (piston type) described in section 4.4. The sampler was capable of obtaining a sample 70 cm long and 7.9 cm in diameter. The length of samples taken ranged from a maximum of 65.6 cm to a minimum of 4.5 cm. When a sample was taken, the upper piston was sealed and a plastic cap forced over the bit end under water — thus insuring *in situ* water contents. The tube, with sample, was then handed aboard the research vessel *NOAA's Ark* and placed in racks in a vertical position, oriented with top of sediments up. When the ship returned to its mooring at Santa Cruz, the sample was taken ashore and shipped by truck to Moss Landing, California, where a soils laboratory had been set up.

## 6. GEOLOGICAL DATA ANALYSIS AND RESULTS

### 6.1 Laboratory Procedures - Sample Analysis

Generally, there was a 4 to 6 hr interim between actual taking of the sample and the processing in the laboratory. The sample tube was cleaned, and weighed to 0.1 g. The seals were then removed and the excess water was siphoned off, weighed and recorded. Preweighing of all sample tubes facilitated the calculation of the total weight of the sample. The volume of the sample was calculated knowing the cross sectional area of the sample tube and the length of sample retained in the tube. The sample tube was then clamped into a horizontal core-ejector and extrusion of the sample commenced.

Eighty-two samples were collected from the upper Monterey Bay Area with 71 engineering samples obtained by the bottom-sitting drive sampler, piston type, and 9 lithologic samples with the bottom-sitting vibratory sampler, open type. Both types of samplers were designed and developed at MMTC (Jenkins and Takeyama, 1971; O'Brien and Duley, 1971).

Table A-4.1 in Appendix A-4, summarizes the equipment used, corrected depth and other information related to the samples. Mathematical equation used to calculate sample volumes were:

$$C_{a} = \frac{D_{w}^{2} - D_{e}^{2}}{D_{e}^{2}}$$
, (21)

where  $C_a$  is the area ratio,  $D_w$  is the maximum outside diameter of the sampler cutter, and  $D_e$  is the inside diameter of the sample cutter;

$$C_{o} = \frac{D_{w} - D_{t}}{D_{t}}, \qquad (22)$$

where  $C_0$  is the outside clearance ratio controlling outside friction and  $D_+$  is the outside diameter of the sample tube;

$$C_{i} = \frac{D_{s} - D_{e}}{D_{e}},$$
 (23)

where  $C_i$  is the inside wall friction and  $D_s$  is the inside diameter of the sample tube. The gross recovery ratio,

$$R_{g} = \frac{L_{e}}{H} , \qquad (24)$$

where  $L_e$  is the length of the sample obtained and H is the depth of penetration of the sampler. This factor was not determined in the sampling because the sampling equipment was not monitored to determine penetration depth. Table 1 summarizes the parameters and ratios for the two types of samplers used in this project.

# 6.1.1 Wet Unit Weight, Ywet

Wet unit weight was determined by direct measurements:

$$\gamma_{wet} = \frac{W_{total} - W_{tare}}{V_{total}}$$
(25)

where  $W_{total}$  is the weight of the sample plus the sample tube and all seals;  $W_{tare}$  is the weight of the sample tube and all seals alone, and  $V_{total}$  is the volume of the sediments in the sample tube.  $\gamma_{wet}$  represents the in-place unit weight or bulk density.

Wet unit values ranged from a maximum of 2.87 g/cm<sup>3</sup> at core site No. 50, to a minimum of 1.85 g/cm<sup>3</sup> at core site No. 103. Higher values of  $\gamma_{wet}$  were noted in the use of the vibratory drive sampler as compared to the diver operated drive sampler at the same location. This is probably caused by plugging of the sample tube and subsequent compacting of the sediment within the tube.

## 6.1.2 Specific Gravity of Solids, G

A representative sample of approximately 100 g was retained and overdried at  $110^{\circ}$ C for 10-12 hrs or until a constant weight was observed. This sample was then allowed to cool in a desiccator, then it was split and weighed. The volume of the solids (V<sub>s</sub>) was determined directly by a

Type of Sampler	D <sub>e</sub> (in)	D <sub>s</sub> (in)	D <sub>t</sub> (in)	D <sub>w</sub> (in)	Area Ratio (C <sub>a</sub> )	Clearance Ratio	
						Outside (C <sub>0</sub> )	Inside (C <sub>i</sub> )
Piston-type Drive Sampler (Diver operated)	3.34	3.37	3.44	3.44	5.19	0.0	0.72
Open-type Drive Sampler (Vibracorer)	2.75	3.69	3.75	3.75	86.5	1.7	23.5

Table 1. Sampling Equipment Specifications

Beckman Model 930 Air Comparison pycnometer. The weight of solids  $(W_s)$  was determined with a Sartorius balance to 0.01 g.

Specific gravity of the solids,  $G_s$ , was determined from

$$G_{s} = \frac{W_{s}}{V_{s}} .$$
 (26)

Corrections for salt content in the specific gravities were not made. Specific gravities of the solids for over 100 samples ranged from 2.57 to 2.75. The majority of samples were between 2.62 - 2.70 and an average of 2.66.

### 6.1.3 Water Content, $\omega$ ; Void Ratio, e; Porosity, $\phi$

Water content is a ratio, in percent, of the weight of water in a sediment sample,  $W_W$ , to the weight of the over-dried solids,  $W_S$ , and is defined as

$$\omega = \frac{W_{W}}{W_{S}} \times 100 . \qquad (27)$$

In this analysis, determination of water content was found by the calcium carbide gas pressure method and follow AASHO Designation (see T217-67 I, A2-2, ASTM 1970, p. 456-459). Using local soils from Monterey Bay and distilled water, a conversion curve was calculated by regression analysis with a correlation coefficient of .99 and standard error of estimate equal to  $\pm$  1.93. Equation 28 was used to calculate water content,  $\omega$ ;

 $\omega = 1.33$  (calcium carbide method) - 3.34. (28)

Another measure for water-saturated sediments is the void ratio, e, which was the ratio of the volume of the void spaces,  $V_v$ , to the volume of the solid sediments,  $V_s$ , or

$$e = \frac{V_v}{V_s} .$$
 (29)

Void ratio was calculated in the laboratory from,

$$e = \frac{G_s \gamma_w V}{W_s} - 1 , \qquad (30)$$

where  $\gamma_{w}$  is the unit weight of sea water.

Porosity is the relation expressed in percent of the volume of the voids,  $V_{i}$ , to the total volume, V, which is defined by,

$$\phi = \frac{V_{\mathbf{v}}}{\mathbf{V}} \times 100. \tag{31}$$

In this report perosity was computed from other measured parameters and expressed as

$$\phi = \frac{e}{1+e} \times 100$$
 (32)

### 6.1.4 Grain Size Analysis

A representative sample from any one lithologic zone observed in the core sample was weighed, put into a mixing bowl of water, stirred and allowed to stand for one hour. The sample was wet sieved in a U.S. Standard No. 200 sieve and all material retained was dried overnight at 110°C, or until a constant weight was observed. The wash water was evaporated in the drying oven at 110°C and both fractions were cooled in desiccators. The coarse fraction was sieved using a nest of sieves (openings 12.5, 4.69, 1.981, 1.397, .991, .701, .495 mm) for 7 min and the fraction passing No. 35 (.495 mm) was sieved in a nest of sieves (openings .351, .246, .175, .124, .088, .074 mm) for 10 min in a gyrotating sifting machine. The size fraction finer than No. 200 (.074 mm) was split, and a representative sample of approximately 35-40 g was used for the determination of the grain size distribution of the fine-grained soil by the hydrometer analysis method. The sample was placed in a beaker with 250 cc of distilled water and 5 cc of a 10 percent by weight solution of sodium hexmetaphosphate (calgon) and hand-mixed. The suspension was transferred to a dispersion cup (ASTM D422-63), and mechanically mixed for 10 min. The dispensed soil suspension was then transferred to a 1000 cc graduated cylinder, distilled water added to increase the volume to 1000 cc, and placed in a constant temperature bath. Approximately  $1\frac{1}{2}$ -2 hrs in the bath were needed to bring the solid suspension up to test temperature at 25°C. All hydrometer tests were run 4 hrs to find the percentage break-off point of the silt to clay fraction at 5 microns (U.S. Bureau of Soils classification).

Results of sieve and hydrometer analysis were graphed on a cumulative frequency curve by weight from which the mean diameter was selected from average diameter of the 16, 50, and 84 percentiles (Folk and Ward, 1957).

## 6.1.5 Shear Strength (Penetrometer)

A standard Proctor penetrometer with interchangeable needles was first used by the divers to obtain *in situ* measurements but was discarded as being unmanageable and not enough bottom time. Penetration tests were conducted in the laboratory using a pocket-size penetrometer which was calibrated in T/ft<sup>2</sup> and Kg/cm<sup>2</sup>. All measurements were taken as the sample was slowly ejected, but still within the core barrel.

### 6.1.6 Shear Strength (Vane Shear-Torvane)

Attempts were made to use a hand vane shear tester operated by divers to obtain *in situ* measurements of shear strengths, but this was discarded because of low bottom visibility and consumption of bottom time. Vane shear measurements were made in the laboratory with a hand Torvane device calibrated at 1 rev =  $1.0 \text{ Kg/cm}^2$ . Here again measurements were taken within the sample tube as the sample was ejected. Most of the measurements were taken when a visual change in lithology was evident.

### 6.2 Interpretation of Sediment Analyses

Table A-4.3 in Appendix A-4 represents a tabulation in sediment analyses made on approximately 85 core sites contained in Santa Cruz harbor, Monterey Bay (fig. 33). The datums for parameters of mass physical properties, such as, porosity, density, mean grain diameter and sorting cuefficient, as well as engineering properties of shear strength, were posted at respective core sites, and contoured interpretations prepared. Figures 41 through 46 show the results of the interpretations. Summary of the parameters datums at each core site and the acoustic values are contained in table A-4.4 in Appendix A-4.

#### 6.2.1 Interrelationship of Sediment Properties

Computer analyses of the parameters representing the mass physical and engineering properties were done using a regression analysis program. Cross-correlations of the parameters resulted in the correlation coefficients given in table 2. The degree of correlation is reflected in percent with a value of 1.000 representing a perfect correlation. The relative interdependence or interrelationship of one parameter to another is reflected in the correlation coefficient. As predicted in other studies cited previously (section 2.1), the correlation of porosity and density (.952) is very good. Also, the correlation of sorting coefficient to mean grain diameter is within reasonable expectations (.830). Evidence of correlation between penetrometer and vane shear measurements (.317) is poor, and may be attributed to the predominance of sand, which has always been an inherent problem in obtaining reliable shear strengths. Other correlations appear to be negligible, thus their significance in diagnosing the interdependence or interrelationship of these parameters appears to be minor.

### 6.2.2 Sediment Dynamics and Deposition Characteristics

The correlation of acoustical properties to sediment properties is given in section 7.1.3. The value of relating the two measurements in a positive correlation is obvious considering the economics and time involved





Figure 42. Density distribution - Santa Cruz harbor.







Figure 44. Sorting coefficient distribution - Santa Cruz harbor.









Mass Physical and Engineering Properties	Correlation Coefficient	Standard Error of Estimate
Porosity vs:		
Density Shear Strength (Penetrometer) Shear Strength (Torvane) Grain Size (mean dia)	0.952 0.075 0.110 0.022	±0.075
Sorting Coefficient	0.026	
Bulk Density (wet) vs: Shear Strength (Penetrometer) Shear Strength (Torvane) Grain Size (mean dia) Sorting Coefficient	0.117 0.091 0.069 0.030	
Shear Strength (Penetrometer) vs: Shear Strength (Torvane) Grain Size (mean dia) Sorting Coefficient	0.317 0.082 0.005	±0.197
Shear Strength (Torvane) vs: Grain Size (mean dia) Sorting Coefficient	0.086	
Grain Size (mean diameter) vs: Sorting Coefficient	0.830	±0.301

### Table 2. Correlation Matrix - Sediment Data (Linear Regression Analysis)

in sampling by core drill from a surface platform at sea. The Monterey Bay study would be incomplete if the sediment dynamics and depositional characteristics were not mentioned. Figure 47 is a map showing the distribution of sediment types in the test area. As can be seen, considerable variation occurs. Figure 48 represents 3-component diagrams of the distribution of sediment types. The distribution was quite different from that found in the San Francisco Bay study (Barnes et al., 1972, p. 125), in which a marked propensity toward finer grained size fractions was noted. Obviously, the forces affecting sediment transport and deposition are different in the two areas. Wolfe (1970), described the effects of the coastal current and swell induced forces on the distribution of sediments in Monterey Bay. The results of his study are shown by vector diagrams that give the direction of major sediment transport in the northern radius. The dominant wave refraction patterns have also been added to provide a complete concept of the dynamics, as shown in figure 49. The deflection of flow by the promontory at Santa Cruz headland



Figure 47. Bottom sediment composition - Santa Cruz harbor.





Figure 48. Three-component diagrams sediment types - Santa Cruz harbor.







Figure 49. Sediment transport pattern and wave refraction pattern Monterey Bay (after Wolfe, 1970).

coupled with swell-induced bottom currents over submarine topographic obstructions (rock outcrops) have resulted in an erratic distribution of many sediment types in the near shore regions of Santa Cruz harbor. Very poor sorting of sediment grain size combine with a high variability of sediment type. This has resulted in many of the problems of sand deposition in the vicinity of the Yacht Harbor basin entrance (fig. 33), as well as the absence of deposition further south in the beach areas of the city of Capitola.

The area within the 30 ft contour on the bathymetric map (fig. 34) and inward to the shore, could be defined as a secondary circulation celi. It is a type of occurrence more commonly found in estuaries and in closed bays than the offshore areas of the coast. It is only because at the unique set of topographic circumstances, both the marine environment and the shore line that cause such a situation in Santa Cruz harbor. Except for the storm season, very little material is deposited from the rivers that enter the harbor. Nearly all the sediment load is borne in by the longshore current and swell-induced bottom currents. The forces found in the so-called secondary cell existing in the shallow water region, are created out of components of the dominant forces. It is the activity of this subordinate current within the secondary cell that plays the important role in deposition of the sediment load or ultimate transportation to other localized areas along the harbor coastline.

### 7. GEOPHYSICAL DATA ANALYSIS AND RESULTS

7.1 Continuous Reflection Coefficient Mapping Experiment

### 7.1.1 Data Reduction Program

Program tasks under a subcontract to the Raytheon Company, Portsmouth, Rhode Island, specified that automatic data processing and reduction be performed on a series of analog acoustic signals acquired in the reflection coefficient mapping experiment. Original specifications of data volume consisted of 222 track miles of acoustic traverse recorded on 7-channel magnetic tape (fig. 32 - Area A). Processing of this data consisted of analyzing sequential sets of 15 acoustic pulses, spaced along a track line at 600 ft intervals for each of 4 data channels (2 channels of 0-5 kHz signals and 2 channels of 12 kHz signals). Each 4-channel set included therefore, 60 discrete pulses resulting in 2200 sets entailing examination of 132,000 individual pulses (see figs. 50-54).

Statistical analysis also was included on each separate set of data values for the 0-5 kHz and the 12 kHz signals with supporting data involving primarily calculation of water depth, recorder gain variations (acquisition and digital processing gains) and time code indices.

The acoustic data was further processed to yield printout listings and digital tapes delineating the following parameters for each data set:

- 1. Beginning and ending time code indices.
- Water depth 15 individual values with the average range and standard deviation.
- 3. Computer spherical spreading loss used to compute bottom loss values with the average, range and standard deviation.
- 4. Sound speed in water column used for depth and spherical spreading loss computations.





POSITION TAPES
INITIALIZE INPUT PARAMETERS
NOTIFY OPERATOR OF NEXT TIME CODE
IF DESIRED, PAUSE
REPEAT IF CHANGED
AUTOMATICALLY SEEK NEXT TIME CODE
SAMPLE DATA (FIG. 3)
PROCESS DATA (FIG. 4.5)
REPEAT FOR SET OF TIMES

Figure 51. Detail program flow: initialization.

COMPLETE

- 5. Acquisition system gain.
- 6. Digitizing system gain.
- Amplitude of individual pulses with the average, range and standard deviation.
- 8. Bottom loss or reflection coefficient with the average, range and standard deviation.

Initially, processing entailed locating in the seismic wave train, the peak pressure pulse transmitted by the acoustic source and the first seismic reflection event received from the seafloor. The events sampled were located by determining the time interval from transmission to reception of the water path (ray path travel time based on analysis of the 12 kHz wave train) from time domain analysis.

Regarding the quantities, volume, values to be found, and the presentation format, it was mutually agreed with the contractee that due to the experimental nature of the project some flexibility was required. As originally conceived, the analog data acquisition phase was designed to



Figure 52. Detail program flow: sample data and save on tape.

acquire simultaneous wave trains (0-5 kHz signals at one per second and 12 kHz signals at 4 per second) with ships speed adjusted in such a manner that a nominal 600 ft track line interval would equal a 1 min processing cycle. Accordingly, machine processing was set to first treat 0-5 kHz signals and then the 12 kHz signal, generating 15 pulse-sets for each, every minute. The actual data acquisition program resulted in both the 0-5 kHz and 12 kHz signals transmitted simultaneously on a one per second basis (section 4.1.3). A slower ships speed also resulted in the 600 ft track line interval being equivalent to a 2 min processing cyc'e, thus nearly trebling the number of data points acquired.

Further, because of the high variability encountered in the acoustic wave trains and unforeseen problems which resulted from variaticn in the data acquisition system operation, a preliminary editing cycle on all



Figure 53. Detail program flow: process data.

data was required. This preliminary editing cycle was necessary to insure that all time intervals specified could in fact be computer processed. Original program scheduling was predicted upon this preliminary editing cycle, having been performed prior to initiation of the processing, but this turned out to be impractical during field operations. The impact of these data acquisition changes resulted in higher cost per processed unit. It was necessary, because of budget limitations, to selectively reduce the volume of data to be processed. Only 40 percent of the total data volume was processed as a result.

An example of the processed data format is presented in table 3. As the volume of processed data in this format is extremely extensive, a data summary was prepared and is included in Appendix A-5.



Figure 54. Detail program flow: maxima location.

Digital Processing Objective. Digital processing objectives involved determination for each set of pulse pairs (i.e., i = 1, 2, --15, see table 3) of:

- (a) V<sub>pxi</sub> = peak value of Pinger Transmit,
- (b) V<sub>pri</sub> = peak value of Pinger Receive,
- (c)  $t_i \equiv time difference between Pinger,$

transmit and Pinger Receive

(d) V<sub>sxi</sub> = peak value of Sparker Transmit,

(e) V<sub>sri</sub> = peak value of Sparker Receive,

and calculate

(f) D<sub>i</sub> = water depth.
1.23 × 25. 1.11 ------81.0 81.5 .30 V 321 82.3 -15 15 81.7 23 1.23 ...... .07 •• PINGER DEONE MAX = PINGER DEONE MAX = --82.5 81.5 30.00 .31 1.24 ~ -1.07 13 VEL = 4800 FT/SEC VEL = 4800 FT/SEC P.NGER GAIN = 20X PINGER GAIN = 201 81.2 1.10 82.0 82.3 1.1 .715 2.3 12 12 81.7 .131 .28 .12 .34 -1-1 11 11 81.4 82.3 1.12 .14 .18 .31 -. 590 10 10 1.02 81.4 1.11 1.28 1.28 . 68 6.18. • . NUMBER PULSE NUMBER 81.0 1.11 1.15 .870 .639 81.2 1.10 .28 ... PINGER REFLECTION COEFFICIENT = . 690 ( .137) SPARKER REFLECTION COEFFICIENT = . 596 ( .091) SPARKER REFLECTION COEFFICIENT = .619 ( .076) PINGER REFLECTION COEFFICIENT = .880 ( .101) PULSE 81.8 1.10 81.5 .67 -. 32 . 32 . 32 .09 • WATER DEPTH = 81.4 ( .5) WATER DEPTH = 81.9 ( .5) 83.0 81.0 .67 554 5.1 1.13 .81 642 3.8 .75 11.4 82.3 .34 . 594 1.10 ~ 82.0 ..... 90.7 1.08 .24 1.7 1.09 81.0 81.5 .27 . 30 . 4 . 4 .10 .16 1.1 81.2 82.0 . 35 . 35 742 742 1.23 . .23 .. 81.4 81.8 .30 .30 30 4.0 1.10 •. --1.06 . 93 . 503 . 1.1 -57 -24 -2453 80.7 .50 .24 2.6 81.0 HIN. win. 1.14 1.29 3.8 1.12 . 35 . 78 83. h ROLL NUMBER 24 82.5 ROLL NUMBER 24 DATE 07/25/72 SIGHA MAX. DATE 07/25/72 TIME 17.09.53 17.10.53 WAX. .000 w. .03 SIGMA 1 .137 13 JHI L -1.08 41.4 61.9 1.10 101 AVE. AVE. 2 . 2 PING WHIT SPKS XMIT PING REC PING REC PING REC SPKO XMIT SPKO REC SPAP REC SPAP REC ATER DPTH HEAD DOTE

0

Data Format for Computer Processed Acoustic Data - Reflectivity Experiment Table 3.

....

## Using these values, perform

(1) Pinger reflection coefficient:

$$G_p(D_i-1.83)(V_{pri}/V_{pxc}), and$$
 (33)

(2) Sparker reflection coefficient, where Gp and Gs are defined as respective system acquisition and digitizing gains:

$$G_{s}(4D_{i}^{2}+832)^{\frac{1}{2}}(V_{sri}/V_{sxi}).$$
 (34)

In addition calculate:

(3) Pinger Bottom Loss:

$$-20 \text{ Log}_{10} \text{ R}_{pi}$$
, and (35)

(4) Sparker Bottom Loss:

$$-20 \log_{10} R_{si}$$
. (36)

Having completed these determinations, statistically treat each of the resulting nine arrays as follows:

Defining each array as  $A_{j}$  (j array),

 $A_j \quad j = 1, 2, \dots 9$ 

and each element within these arrays as  $a_{ij}$  (i<sup>th</sup> element), calculate

$$\bar{A}_{j} = \frac{1}{15} \sum_{i=1}^{15} a_{ij}$$
 (average), (37)

Maximum and Minimum (range), (38)

$$\sigma A_{j} = \left(\frac{1}{14} \sum_{i=1}^{15} (a_{ij} - \bar{A}_{j})^{2}\right)^{\frac{1}{2}} \text{ (standard deviation).}$$
(39)

For these analyses, a processing cycle time of 1 min was used where the first pulse pair (i=1) occurred 7.0 sec before the listed time code. The results of these determinations and calculations were output in the format presented in table 3.

Digital Processing Techniques. As outlined above, there were 5 basic wave form parameters to be determined for each of the 15 pulse pairs within each pulse set  $(V_{pxi}, V_{pri}, V_{sxi}, V_{sri}, t_i)$ . Having determined these parameters, a series of subsequent data reduction steps were performed to obtain the remaining arrays of required output information (i.e.,  $D_i$ ,  $\bar{D}$ ,  $\sigma_0$ ,  $R_{pi}$ ,  $\bar{R}_p$ ,  $\bar{R}_s$ ,...etc.)

The general program structure was constructed to process a basic block of 15 pulse pairs per set; each pulse pair was comprised of a transmit pulse and a received pulse from two distinct acoustic systems. For each of the two acoustic systems, (a 0-5 kHz system and a 12 kHz system) we ascertained the maximum value of the transmitted wave forms envelope, the maximum value of the received waveforms envelope, and, in the case of the 12 kHz system, the time difference between these transmit and receive maximums. Having determined these values, the reflection coefficients and bottom losses as defined in equations 33 through 36, were computed.

Data Format. The original analog signal format suggested two acoustic systems transmitting at a repetition rate of 1:4 with the pinger system having the higher repetition rate. Employing this format, the maximum available analog-to-digital conversion rate of 30 kHz could be applied to each of the four waveforms of interest on a non-interfering basis. Specifically, the bandwidth of the 0-5 kHz; is nominally 5 kHz; the 12 kHz approximately 2 kHz. A 30 kHz sampling rate was thus sufficiently above the Nyquist rate to insure individual maxima to be determined with little or no error.

The suggested analog signal acquisition format was not achieved as system design constraints necessitated both systems transmitting synchronously at one per sec repetition rate. As a consequence, the 30 kHz sampling rate had to be alternated between the Pinger and Sparker channels, which resulted in an equivalent 15 kHz sampling rate per channel. For the 12 kHz system that had a 2 kHz bandwidth, the 15 kHz sampling rate still allowed the maximum to be determined directly. For the 5 kHz system, however, it was necessary to use an FFT interpolation technique to obtain the required degree of time resolution.

As a further complication, the signal of the analog data was difficult to decipher because of either transient noise or multiple reflections. The simple routine of finding the maximum within a given observation window could not be routinely applied. To insure data quality, a modified receive-search procedure was developed in which the received data was scanned in time until a preset threshold value was exceeded, indicating the leading edge of the returning pulse. The procedure then selected the largest value occurring in the next  $\tau$  sec ( $\tau$  approximately equal to the transmit pulse deviation), storing this value as the received maximum.

Further, provisions were made to add a small sample of the pinger return during the time it should have negligible amplitude (i.e., during the "off" period). The maximum of these samples, essentially system background noise, also is presented. Large values suggest periods of spurious data.

Processing Parameters. To maintain processing flexibility, attempts were made to make all key parameters detailed selectable at runtime.

- Sampling Data The sample rate, sample size, and onset threshold level are selectable.
- Check Data The check sample size and the channel examined (of the four) can be specified.

Block Size — The number of pings examined can be varied.

- Receive Parameters The onset threshold (within the data acquisition window) and the subsequent time to find a maximum are selectable. This permitted observation of the waveforms on the scope and established a threshold which lies above the noise level yet assures detection of the return.
- Anticipated Depth The receive data observation window must be wide enough to accomodate any depth variations from one block of pulse sets to the next. This window is automatically positioned for the next block of pulse sets by using the previously calculated depth. Its location is held constant for a given block of pulses.

For the initial block, the program accepts the operator's estimate of this initial depth, requiring for input the observed return time as seen on the scope.

Miscellaneous Constants — Also accessible at run-time are several other constants. Included are: the speed of sound; a pinger gain factor related to attenuator settings during record, and a scale constant for each receive channel.

The following parameter values (one exception discussed below) were used during processing:

sample rate	=	30	kHz	
sample size	=	256	samples	
check sample size	=	15	samples	
number of pings	=	15		
observe block every	=	1	minute	
receive peak sample size	=	10	samples	
speed of sound	=	4800	ft/sec.	

Occasionally, shallow water required a smaller sample size (128 instead of 256). This was necessary since the transmit observe windows would be of length 1/30,000 (sample rate) by 2 (alternate sampling) by 256 (number of samples) = 17.7 m sec or a depth of 41.5 ft. Thus, one could not begin observing the receive channels until after the primary returns had arrived. By decreasing the sample size to 128, depths as small as 20.8 ft cculd be processed. Under this condition, however, the observation window is only  $\pm$  10.4 ft (as opposed to  $\pm$  20.8 ft) so that the depth variations had to be closely monitored.

Detail Program Flow Diagrams. Figures 40 through 44 present a detailed breakdown of the overall program flow presented in figure 40.

## 7.1.2 Summary of Processing — Comments and Findings

The tables 4 and 5 outline in summary form first the requested times to be processed along with the actual times processed, and second, a copy of the log made out at the time of processing. As a general comment, the actual time required to process any given block of required times varied from 1:1 to 2:1 with a general average of 1.5:1 necessitated by the required preprocessing editing.

Roll No.	Requested Times	Total	Actual Times Processed	'Tota <b>i</b>	Comments
001	175553-190353	69	175552-190353	69	
002	164753-173553	49			Possibły 10 mins processable
003	173953-180953	31	173953-174453	6	
	183153-190353	33	175253-180953	18	
			183253-184153	10	
			184453-190353	20	
004	153353-162953	55	153353-162953	55	
006	164553-173953	55	165753-173953	-43	
	175153-182353	33	175153-182353	33	
017	143753-153653	60	143753-153653	60	
018	162553 - 164353	19	162953-164353	15	
	174153-175753	17	174153-175753	17	
019	180953-181953	11	180953-181953	11	
020	151753-160153	45	151853-155353	36	
	160753-164553	39	160753-164553	39	
021	152353-162553	63	152353-162553	63	
022	165653-181953	84	165653-181953	84	
023	190353-195753	55	190353-195753	55	
024	162553-170553	41	162553-170553	41	Long Form only
	170953-174753	39	170953-174753	39	
025	180053-181353	14	180053-181353	14	
	160953-165153	43	160953-165153	43	
026	172353-184453	82	172353-184453	82	
027	184753-190753	21	185653-190753	12	
	155753-164153	45	155853-164153	-44	
028	165753-174753	51			Inverted Tape
	180753-182153	15			Bad Tape
029	182553-193353	69	183353-193353	61	
030	163153-174153	71	163153-174153	71	
031	174553-181753	33	175153-181753	27	
	192153-201753	57	192153-201753	57	

Table 4. Processing Specifications

F.

Roll No.	Requested Times	Total	Aetual Times Processed	Total	Comments
032	171753-182153	65	171753-182153	65	
033	182453-184553	22	183153-184553	15	
	185153-200053	70	185153-200053	70	
034	165553-170953	15	165553-170953	15	
035	171153-180353	53	172153-180353	43	
	185153-193153	41	185153-193153	41	
036	193553-195953	25	194053-195953	20	
	174253-175953	18	174253-175953	18	
037	192553-201353	49	192553-201353	49	
038	201753-202753	11	202353-202753	5	
	203553-204153	7	203553-204153	7	
	204953-214353	55	205053-214353	54	
039	171453-180353	50	171453-180353	50	
041	160753-165953	53		0	Sparker channel
042	170953-180353	55		0	Not processable
046	154453-162353	40	1	0	11
047	163553-170353	29		0	
	170953-180653	58		0	**
048	181953-185153	33			
	185953-191753	19		0	"
	192353-195753	31			
049	154553-164153	57		0	
	164653-165553	10			
050	170753-180753	61		0	2 11
	181553-183553	21			
051	184753-193853	52		0	11
	194753-201553	19			
052	202553-211153	47		0	11
054	182753-183953	13		0	"
055	185153-190553	15			Bad Sparker
	190753-194053	34			
	194353-201753	35			

Table 4. Processing Specifications (Con't)

Roll No.	Requested Times	Total	Actual Times Processed	Total	Comments
058	182153-185353 185753-193553	33 39	182153-183553 185753-193553 183753-185353	15 39 17	Long Form only
059 060	194753-202153 170953-173753	35 29	194753-202153	35	Bad Sparker

Table 4. Processing Specifications (	(Con't)
--------------------------------------	---------

Data Summary, Short Form. A summary form of all the processed data generated per table 3 is contained in the Appendix A-5. The summary sheets present the numeric values of the average reflection coefficient and one standard deviation for both Pinger and Sparker signal systems, together with the average bottom loss values as a function of data and time code index.

## 7.1.3 Correlation of Reflectivity to Sediment Properties

Regression analyses were done by computer processing to determine the correlation coefficients for the 12 kHz and 0-5 kHz acoustical data to the sediment data. The Rayleigh Reflection Coefficients and bottom loss values taken over areas circumscribed about each core site were, respectively, averaged for a representative value that was applied to each of the core site locations. The core sites each were thus referenced by respective acoustical and sediment property values or datums. These data were then cross-correlated and correlation coefficients obtained through the regression analyses. The objective was to relate the degree

Roll No.	Findings	Comments
001	Cal's ok. Good data. Few drop-outs. Added -10 dB to pinger channels	Good run with very few few drop-outs.
003	Calibration signals on during time required for processing. Added -10 dB to Pinger channels. Reasonable data.	Segmented Runs: a/. 173953 174433 Cal's 1745 - 1751 b/. 175253 - 180953 c/. 183253 - 184153 Sparker bad 1842,43 d/. 184453 190353
004	Cal's ok. Deep water, multiple drop-outs. Pinger transmit low. Added -10 dB to Pinger channels	Processed complete run drop-outs could not be picked up due to poor S/N.
006	Calibration signals on during time requested for processing. Not possible under present program configuration. Added -10 dB to Pinger channels. Large interference spi- ker in Pinger Rec. Sparker Rec.drop-outs.	Segmented Runs: a/. 165753 173953 Cal's 1646 1656 Inter. 1710 1712 b/. 175153 182353 Many data drop- outs.
017	Cal's ok. Reasonable data. Few drop-outs. 10 dB in Pinger channels.	Processed complete run
018	Cal's on during requested times. Time-code drop-outs. Multiple Pinger Rec. drop-outs. Added -4.0 dB to Pinger channels	Segmented Runs: a/. 162953 164353 Cal's on to 1629 b/. 174153 175753 Multiple drop-outs
019	Cal's ok. Reasonable data added -4.0 dB to Pinger channels.	180953 181953
020	Cal's ok. Data starts 150805 Time-Code skip Sparker Transmit funny 155853. End on 155353 Problems after with Sparker. Addec -4.0 dB to Pinger channels	151953 155353 Reasonable data. Will review problems at later date.
020	155433-160153 Sparker very erratic10 dB in Pinger channel	Unable to auto process
020	160753-164653 Sparker problems. Time Code drop-outs. Very difficult to process -10 dB in Pinger channel	Segmented Runs: a/. 160753 161253 ok b/. Missed 161353-162953 Sparker Transmit and Time Code problems c/. 163053-164653 ok Sparker acting funny
021	Cal's ok. Time Code out 161253, 13. Funnies on tape -10 dB Pinger channels.	Processed complete run

## Table 5. Shipboard Data Analysis and Commentary Log

Roll N	o. Findings	Comments
022	Cal's ok. Pinger transmit loss Sparker low. Added -10 dB Pinger channels.	Processed complete run
023	Cal's ok. Reasonable data. Few drop-outs. -10 dB Pinger channels.	Processed complete run
024	Cal's ok. Reasonable data. Few drop-outs. -10 dB in Pinger channels.	Processed complete run
025	Cal's checked. Pinger Xmit Low (3.0 Vp) Sparker Xmit high (5.0 Vp) 800j source. -10 dB in Pinger channels. Sparker data erratic and multiple drop-outs. Time Code drop-outs.	Segmented Runs: a/. 180053 181353 high sparker, low Pinger b/. 160953 - 165153. Levels ok, but returns erratic. Questionable sparker data run a/.
026	Cal's checked. Both Pinger and Sparker Xmit's low at beginning of run10 dB in Pinger ehannels. Pinger Xmit about nor- mal at end of run.	Processed complete run
027	Cal's cheeked. Cal's on during time required to be processed10 dB Pinger channels. Highly variable sparker data Time Code skips, Good levels.	Segmented Runs: a/. 185653 - 190753 Cal's on in beginning b/. 155853 164153 Tape on 155803 variable data, missed 162053, and 162553
028	Tape from net re-wound. Rewound tape. Findings; channels 1. Noise 2. Time Code 3. Noise 5. Pulsed data, but not of the normal form.	Tape to be re-examined later.
029	Cal's eheeked ok. On until 183300 Pinger Xmit low10 dB in Pinger ehannels. Time Code Skips 190453.	Segmented Runs: a/. 183353 190353 b/. 190553 193353
030	Cal's ok. Pinger low -10 dB Pinger ehannels	Processed complete run
031	Cal's eheeked but Pinger still low10 dB in Pinger ehannels. Cal's on during data. 175153 variable returns	Segmented Runs: a/. 175153 - 181753 b/. 192153 - 201753
032	Cal's cheeked, Pinger low10 dB in Pinger ehannels. Very erratic sparker.	Segmented Runs: a/. 171753 - 175253 b/. 175353 - 182153
033	Cal's ehecked. Pinger low. Cal's on during data 183153. Very poor sparker data? Highly variable. Pulse Rep. rate erratic. Time Code skips. Difficult tape.	Segmented Runs: a/. 183153 - 184553 b/. 185153 - 200053 Difficult Tape!

Table 5. Shipboard Data Analysis and Commentary Log (Con't)

Roll No.	Findings	Comments
034	Cal's checked, Pinger low, Pulse Rep. er- ratic10 dB in Pinger channels.	Completed
035	Cal's checked. Levels wavered in time Ch #1 and #5. Rep. rate erratic. Cal's on during times requested10 dB in Pinger.	Completed. Segmented Runs.
036	Cal's checked10 dB in Pinger, Cal's on during time requested. Pinger drop-outs	Completed, Segmented runs a/. 194053 - 195953 b/. 174253 - 175953
037	Cal's checked. Erratic Rep. Rate -10 dB Pingers	Processed complete run.
038	Cal's checked. On during time requested. -10 dB Pinger very low Pinger Xmit.	Difficult tape. Segmented runs.
039	Cal's checked. Pinger low. Pinger Xmit 2.0 Vp off tape, Big change 174853. Time Code skips.	Difficult tape. Completed Run.
041	041 052 Sparker Xmit Missing	
054	No Sparker Transmit Total Tape	Pinger channels ok. could process with additional software development.
055	No Sparker Transmit Total Tape	Same as 054
058	Calibration ok. Reasonable data same pinger Rec. drop-outs ≈184000. Pinger Rec. channel too high. Added -10 dB to both Transmit and Receiver data Pinger Gain change at 183700.	Segmented Runs: a/. 185800 →193600 Wrong gain factor used, should have been X1. Requires sealing. b/. 182153→183553 Time Zero of processing 000053. c/. 183653→185353 Some Pinger Rec. drop-outs.
059	All channels present with Cal's	Water depth became less than 40'. Required Segmented runs.
060	No Sparker Transmit Total Tape	Same as 054

Table 5. Shipboard Data Analysis and Commentary Log (Con't)

of correlation between acoustical properties (bottom loss and reflection coefficient) calculated from the peak amplitudes generated in the inputresponse function of the two sources to parameters measured in the sediment analyses (i.e., porosity, density, mean diameter, sorting coefficient, bearing capacity, and shear strength). No edit of the data was accorded as to number of samples taken, other than to restrict the area of influence about a core site. This area was defined by drawing circles about each core site to a radius that would produce tangency to circles scribed about adjacent core sites. Since the core hole spacing was uniform in pattern within the high density area and low density area, the areas of influence for the statistical sample were uniformly the same within the two densities. Corrections were made on the acoustic data for obvious wild point values because of errors incurred in the analog to digital conversion. In such cases, the reflection coefficient and corresponding bottom loss values were recomputed.

The correlations obtained from the first analysis, which incorporated the entire area surveyed, are reflected in the correlation matrix given in table 6. Plots of the data represented in the mathematical correlation are shown in figures 55 through 66. One can immediately see the

100000									E		_
VARIABLE NUMBER	1	2	3	4	5	6	7	8	9	10	A
1 2 3 4 5 6 7 8 9 1,0	1.000	963 1.000	.111 173, 1.000	085 .039 .266 1.000	.120 041 114 083 1.000	.014 .066 074 047 217 1.000	006 .030 294 051 195 .141 1.000	009 .004 088 .142 .093 .129 .841 1.000	133 .080 .138 .046 .097 278 657 642 1.000	131 .087 .145 .067 262 625 625 620 .989	

Table 6. Correlation Matrix - Acoustic and Sediment Properties

BMDO2R - STEPWISE REGRESSION - VERSION OF APRIL 13, 1965 HEALTH SCIENCES COMPUTING FACILITY, UCLA

PROBLEM CODE	ARPA
NUMBER OF CASES	42
NUMBER OF ORIGINAL VARIABLES	10
NUMBER OF VARIABLES ADDED	0
TOTAL NUMBER OF VARIABLES	10
NUMBER OF SUB-PROBLEMS	24

VADIAT	DI C	MCAN C	TANDARD DEVIATION
VARADA		MEAN 3	TANDARD DEVIATION
PROSTY	1	44.03357	4.80310
DENSTY	2	2.08262	.1,931.9
PNITMR	3	.27602	.20130
T-VANE	4	.09076	.10524
M-DIA	5	.20355	.32998
SORT C	6	1.45900	.32749
SKR BL	7	5.42912	.73977
SKR WO	8	5.67864	.85959
PGRRFL	9	1.13074	.22659
PGR WO	10	1.11633	.23224

results show very poor correlations exist. Another representation of the data is given in the contoured maps of bottom loss versus sediment properties in figures 67 and 68. Comparison of the surface distribution of acoustical properties derived from the two sources to any one of the mass physical properties and/or engineering properties again gives little indication of positive correlations. Greater resolution of the acoustical distribution over the entire survey area is shown in figures 69 and 70 for selective comparison to figures 41 - 46.

The results of the correlation represent a marked contrast to the previous results obtained in the San Francisco Bay study (Barnes et al., 1972) and other studies (Breslau, 1964, 1967). Correlation coefficients derived from other studies of acoustics using a pinger acoustic source (12 kHz) and the broad band sparker (0-5 kHz) source are shown in table 7. The difference in the results of previous studies and the Monterey Bay study may be attributed to several causes, each of which bears more data processing of existing data than could be accomplished in the time span of this study. Some of these probable causes can be enumerated as; (1) too large an area was taken for the representative statistical sample; (2) sediment velocity variability in the near shore region may be more severe than assumed; (3) the changing water depth coupled with topography changes over short distances may have altered normal incidence path between source and reflector; (4) stability of the source-receiver systems in a surface towed configuration.

An attempt was made to delve into each of these probable causes. The best explanation of the analysis can be made by drawing an analogy between the San Francisco Bay study and this study. The first of which can be drawn on the basis of size of area surveyed. The previous study encompassed a relatively small area containing approximately 1.5 sq miles (1 mile by 1.5 miles), while this study encompassed nearly 30 sq miles (5 miles by 6 miles). Although the control for navigation-positioning and line density and core site patterns was constant in both stations, variability of sediment type was much greater in Monterey Bay as well as the variability of water depth and the range of water depth. The previous study contained sediments that consisted mainly of fine grained silt and











Figure 57. Plot of bottom loss (0-5 kHz) vs density.









Plot of bottom loss (12 kHz) vs penetrometer. Figure 60.



Figure 61. Plot of bottom loss (0-5 kHz) vs vane shear.



Figure 62. Plot of bottom loss (12 kHz) vs vane shear.



Figure 63. Plot of bottom loss (0-5 kHz) vs mean grain diameter.







Figure 65. Plot of bottom loss (0-5 kHz) vs sorting coefficient.



Figure 66. Plot of bottom loss (12 kHz) vs sorting coefficient.







Figure 69. Acoustic distribution of bottom loss (0-5 kHz) Santa Cruz harbor.



Figure 70. Acoustic distribution of bottom loss (12 kHz) Santa Cruz harbor.

	Correlat Coeffici	ion ient	Standard Error** dB/Unit		
	Barnes et al.	Breslau	Barnes et al.	Breslau	
Porosity	.73	.70	±.455	±.039	
Grain Size	92	.646	±.227	±.292	
Sorting	.92	. 343	±.252	±.660	
Fine Materiai (Sil: & Clay)	.89	. 786	±.305	±.015	

Table 7. Comparison of Correlative Values of Barnes et al. (1972) vs Breslau (1967)\*

<sup>®</sup>Bottom loss calculations are based on peak pressure amplitude measurements and Rayleigh Reflection Coefficient determinations.

<sup>\*\*\*</sup> The standard error of estimate (S) is the standard deviation about a line of average relationship. Assuming a normal distribution, 68 percent of all cases will lie within a range of ±S, 95 percent will fall within ±2S, and 99.7 percent will fall within ±3S. In this instance, the relationship of bottom loss to the other variable is of interest. The values of S are therefore in terms of decibels (dB) per unit of paired variable.

clay fractions with very little material, the Santa Cruz harbor sediments are highly variable ranging in grain size from gravel to silt with admixtures that are poorly sorted and show great diversity in physical properties and engineering properties. According to D. L. Bell (Raytheon Company, Portsmouth, Rhode Island, personal communication), it was learned that such high variability of sediment type can have decided effects on the range of velocity of sound propagation through the sediments. Bell's recent studies of nearshore sediments of the continental shelf off New England, show that sediment velocity changes greatly over short spacial distances and that this has an appreciable influence on corrections applied to acoustic attenuation. Using advanced state-of-the art tools and techniques developed by Raytheon Company, he has been able to accurately measure marine sediment velocities to within 2 percent of the standard velocity for sea water. This accuracy has resulted in obtaining new information on the variability of marine sediment velocities. According to Bell, 40 percent of marine sediments in the nearshore environment range

in velocity within 5 percent below to 2 percent above the standard velocity of sea water. He further believes that sediments can also be categorized into 8 or 9 types through velocity analyses.

Water depths in the San Francisco Bay study area ranged from 25 to 40 ft, as compared to depths ranging from 18 to 250 ft in Monterey Bay. Tests for correlations as a function of water depth were made on the results obtained from the data taken for this study. The results are shown in the correlation matrix given in table 8. Sediment analyses and acoustical data were correlated from core sites along line numbers 16 (sites 38-44); 26 (sites 57-66); 36 (sites 81-88); 43 (sites 100-105); and 45 (sites 109-115) (figs. 33, 34). Figures 71-90 show the cross-sections

Mass Physical and Engineering Properties	Correlation Coefficient		Standard Error of Estimate (±)	
	Sparker BL	Pinger BL	Sparker BL	Pinger BL
	Line No. 16	(sites 38-44)		
Porosity	0.393	0.576	0,3472	0.7072
Oensity	0.602	0.627	0.3771	0.7233
Shear Strength (Penetrometer)	0.554	0.343	0.3694	0.6479
Shear Strength (Torvane)	0.€10	0.861	0.3785	0.3117
Mean Grain Diameter	0.582	0.6217	0.3740	0.7217
Sorting Coefficient	0.0618	0.5789	0.3225	0.7082
	Line No. 26	(sites 57-66)		
Porosity	0.506	0.275	0,4814	0.7601
Oensity	0.3961	0.195	0.5125	0.8061
Shear Strenoth (Penetrometer)	0.4998	0.394	0.6235	0.8506
Shear Strength (Tervane)	0.304	0.421	0,5834	0.8665
Mean Grain Diameter	0.406	0.8665	0.5102	0.674
Sorting Coefficient	C.271	0.7032	0.5935	0.07882
	Line No. 36	(sites 81-88)		
Porosity	0.485	0.508	0.6219	0.4170
Oensity	0.570	0.404	0.639	0.4011
Shear Strength (Penetrometer)	0.577	0.545	0.6388	0.4235
Shear Strength (Torvane)	0.559	0.524	0.6339	0.4198
Mean Grain Diameter	0.382	0.217	0.5113	0.3805
Sorting Coefficient	0.571	0.425	0.6370	0.4040
	Line No. 43 (	sites 100-105)		
Porosity	0.744	0.534	0,3849	0.5128
Density	0.611	0.489	0.4561	0.5035
Shear Strength (Penetrometer)	0.328	0,779	0.6064	0.5733
Shear Strength (Torvane)	0.925	0.696	0.743	0.696
Mean Grain Diameter	0.288	0.673	0.5518	0.5450
Sorting Coefficient	0.778	0.551	0.7299	0.5163
	Line No. 45 (	sites 109-115)		
Porosity	0.837	0.080	0.6270	0.9056
Oensity	0.914	0.226	0.4643	0.9377
Shear Strength (Penetrometer)	0.289	0.600	0.6759	1,1112
Shear Strength (Torvane)	0.797	0.713	0.8018	1,2802
Mean Grain Diameter	0.618	0.618	1.3458	1.0751
Sorting Coefficient	0.615	0.485	1 3440	1 0163

Table 8. Correlation of Mass Physical and Engineering Propertiesvs Acoustics (Linear Regression Analysis).











.

coefficient vs acoustics - Line 26.












.







×,

















obtained of the sediment properties and acoustical data, as well as geologic cross-sections along each line. Analyses show no definite trends, except that correlation coefficients seem to run much higher as the statistical sampling number is reduced from the overall area surveyed, to line-by-line, point-to-point sample areas. Geologic structure, such as noted in the exposed bedrock outcrops and thinly covered subcrop are indicated by the seismic subbottom profiles. These profiles and the crosssections show the influence on the correlations obtained (figs. 17, 41-47, 67 and 68). Such structural variation over short distances may have direct bearing on the results obtained using the high frequency (12 kHz) source as compared to the lower broad band frequency (0-5 kHz) source. It was previously noted in table 6 that the correlation of the input-response functions of the two sources was relatively high (.657) despite their inherently different power and frequency characteristics. As mentioned previously (section 4.1.1), the depth of penetration and resolution of acoustical signal reflected off the bottom and subbottom horizons vary according to the source employed, consequently near surface structural as well as sediment property variations (within the wave length of the signal) can have a decided influence on the reflection coefficient obtained.

Stability of the surface-towed source and receiver systems was not considered a problem. Obviously, sea conditions are a limitation, and should be considered an important factor in obtaining reliable data. The experiments in both the Monterey Bay study and the San Francisco Bay study were conducted under the best prevailing weather conditions with due regard to the adverse circumstances inherent in sea state, swell, wave height and currents.

If the trend analyses, which shows that higher correlation coefficients in shallower water exist as compared to those noted in deeper water, has any validity, it may be concluded that a submerged tow vehicle, which would house the source and receiver systems, could operate with even greater effectiveness. This concept ought to be tested.

# 7.2 Shear Wave Experiment

From the records, the arrival time of the direct wave was taken to be the point determined from the intersection of the base line and the slope of the waveform. To obtain the shear wave velocity or compressional wave velocity, the time of the first arrival from each of the four traces (produced by either the four vertical geophone outputs or four horizontal geophone outputs) were plotted on a time distance graph.

Before discussing the results, the sources of error inherent in the seismic method should be mentioned. One source of error is in the distance between geophone units. Implanting the receivers by hand could result in an inch of misplacement. For a compressional wave velocity of 1500 m/sec, an error of approximately 8 percent in velocity can result assuming 1 in separation between two receivers, when in fact one receiver is displaced an inch.

Another source of error is in the time scale on the oscilloscope. An appropriate time scale for a wave velocity of 1500 m/sec is .1 m sec/cm; however, 0.5 m sec/cm was the lowest time scale achieved. The possible error that can be introduced using this time scale is 9 percent for a 1500 m/sec wave velocity. On account of the large errors introduced, the error for a particular measurement is not discussed. For the same reason, regression equations were not determined for the acoustical-mass physical property relationship.

It should be mentioned that the primary concern thus far has been to develop an inexpensive tool to investigate the feasibility of detecting  $S_{\rm H}$ -wave in saturated marine sediments. Of secondary importance was construction of a high precision monitoring system. It was felt that if the detection of shear waves was found feasible, a refined system could then be designed.

# 7.2.1 Results from Lake Sediments

The determined acoustical and mass physical properties are listed in table 9. Figure 91 is an example of the signals received by a geophone implanted 2 in from the source (trace 1, horizontal geophone output; trace

	Mass Physical Properties								
Sediment Type	Porosity (%)	Grain S Median	ize (mm) Mean	Gravel (%)	Sand (%)	Clay and Silt (%)			
gravelly coarse sand	46.2	0.800	0.833	33.7	66	.3			
Two is a second second	Aco	ustic Prope	erties (In a	itu Measur	ements)				
Vp (m/sec)	V (m/s	ec)	e (gm/cm <sup>3</sup>	)	a <sub>s</sub> (m <sup>-1</sup> )	ω (Hz)			
1690	12	5	1.68		.921	604			
		Comp	uted Proper	ties*					
Q <sub>s</sub> <sup>-1</sup>	μ <sub>R</sub>	μI	K <sub>R</sub>	λ <sub>R</sub>	σ	E			
0.40	.02	.09	4.77	4.75	5.49	8.04			

Table 9. Acoustic and Mass Physical Properties at Beach Test Site

 $\mu,~K,~\lambda,~and~E$  have units of dynes/cm  $^2$  X  $10^{10}.$ 

2 vertical geophone output) and by another geophone unit implanted 4 in from source (trace 3, horizontal geophone output; trace 4, vertical geophone output). The receivers were inserted 1 in into the sediment, whereas the exit port on the source probe was at a depth of  $1\frac{1}{2}$  in. Although the voltage gain was high, the signal quality was poor. This is attributed to the high attenuation of the shear wave and to the low response of the vertical geophone to the direct p-wave.

In figure 91, the first arrival detected by each horizontal geophone is assumed to be the direct  $S_H$ -wave. The polarity reversal between the first arrival detected by the two horizontal geophones is caused by reversing the sensitive axis of the horizontal geophone, whose output is trace 1,  $180^\circ$  with respect to the other horizontal geophone. The polarity reversal is a necessary condition of an  $S_H$ -wave and was used here to aide in the identification of the direct  $S_H$ -wave. Presence of the polarity reversal eliminates the first arrival as being a compressional wave, a vertically polarized shear wave, or a Stoneley wave; however, a Love wave will also produce a polarity reversal with the employed geophone orientation, since the Love wave is a low frequency surface wave of the  $S_H$  type, formed from the superposition of internal reflections within a surface layer.



Horizontal Scale :  $5 \frac{\text{msec}}{\text{cm}}$ Vertical Scale :  $10 \frac{\text{mv}}{\text{cm}}$ 

Figure 91. Signal wave forms, trace 1 and 3: signals from horizontal geophones, 2 in and 4 in from source, trace 2 and 4: signals from vertical geophones, 2 in and 4 in from source ( $S_H$ : direct  $S_H$ -wave; p direct p-wave;  $S_V$ : direct  $S_V$ -wave).

A layer over a half space or a velocity gradient produced from vertical inhomogeneities is a necessary condition for the existence of the Love wave. For a source and receiver located in the distance between the source and boundary of the half space, the direct  $S_H$  wave will be the first arrival. This is attributed to insufficient time (or distance) for the superposition of reflections to form the Love wave. Beyond the critical distance (the distance at which the direct wave and head wave arrive at the same time), the Love wave will be the first arrival due to the superposition of the head wave with other waves. For the lake sediments, no visible change in the sediment type was observed within the top 1.1 m, with the probes implanted 0.45 m, the next interface below the water-sediment interface is situated at a minimum depth of 0.65 m beneath the probes. The minimum critical distance is thus 1.3 m; therefore, it is reasonable to assume that the direct  $S_H$  wave is the first arrival on the traces of horizontal hydrophones shown in figure 92.



Horizontal Scale : 5 msec cm Vertical Scale : 10 mv cm

Figure 92. Signals from four horizontal geophones (2', 3', 4', 5' in from source).

In addition to velocity, the attenuation of the direct  $S_H$ -wave was measured from records shown in figure 92. Four horizontal geophones at 2, 3, 4, and 5 in from the source were implanted at the same depth as the exit port on the source probe,  $1\frac{1}{2}$  in. Note the polarity reversal of trace 1 due to reversing the sensitive axis  $180^\circ$  with respect to the other three geophones. From the measured attenuation, the specific attenuation factor  $(Q_s^{-1})$  was calculated to be 0.40. The error in using the low loss approximations (i.e., Hookean equations of elasticity) for this sediment is approximately 5 percent for the shear velocity.

The arrival of the direct p-wave was difficult to measure. The problem stemmed primarily from the low response of the vertical geophone to the direct p-wave. For a vertical geophone implanted further than 2 in from the source, the actual beginning of the compressional signal was absent. Receivers within 2 in of the source detected the beginning of the compressional signal until an appropriate oscilloscope time scale was set to measure the arrival time differences. This problem was rectified somewhat by implanting the receivers and source at different depths.

# 7.2.2 Results from Monterey Bay Sediments

The *in situ* measurements and the calculated properties are listed in table 10. The mass-physical properties of the stations listed are indicated in Appendices A-5 and A-6. The attenuation of the shear wave was not measured because of the previously mentioned difficulties with the geophone probes. The source and receivers were at different depths. As the radiation pattern varies with the angle from the source, attenuation could not be measured, therefore, the low-loss approximation for velocity had to be used in order to determine the calculated values listed in table 10. As discussed earlier, low-loss approximation for the shear velocity could result in errors as high as 43 percent for a sediment with  $Q_s^{-1}$  of 2.

Examples of the better quality records obtained are shown in figure 93. Figure 94 illustrates a record of poorer signal quality. The depth of the source and the receivers varied. Generally, the source probe and the receivers were inserted  $1\frac{1}{2}$  in and 1 in, respectively. The four geophone units were spaced 1 in apart with the nearest geophone unit 1 in from the source. Distances between the exit port on the source probe and the receivers are noted on the records. Usually, only the two nearest geophone units to the source provided information on the direct waves. The first arrival at the other two geophone units was generally a head wave.

Core	V <sub>s</sub>	Vp	φ	dynes/cm <sup>2</sup> x 10 <sup>10</sup>			A	
Site	(m/sec)	(m/sec)	(g/cm <sup>3</sup> )	μ	K	λ	E	σ
Mooring	567	1622	2 06	66	1 52	4 00		
¥1	504	2020	2.00	.00	4.55	4.09	1.91	.431
N1	504	2030	2.24	.57	8.46	8.09	1.66	.467
X2	521	1512	2.08	.57	3 97	3 50	1 62	122
X3		1530	2 23		0.57	5.55	1.02	.433
94	120	1405	1.00					
100	430	1400	1.99	.38	3.37	3.61	1.11	452
103	308	1400	1.85	.18	3 39	3 27	52	475
118	524		2 22	64	0.05	3.27	. 52	.4/5
121		1440	2.35	.04				
100		1440	2.03					
122	475		2.03	46				
123	371	14.*0	2 00	21	4 14	2.05		
	0.1	14.0	2.09	.31	4.14	3.95	.90	.465

Table 10. In Situ Measured and Calculated Sediment Properties



a. Signals from four horizontal geophones.



b. Signals from four vertical geophones.

Figure 93. Two records obtained at the mooring site. Distance between exit port on source probe and receiver is provided on the trace.



Horizontal Scale : 1 msec Vertical Scale : 1 mv cm

Figure 94. Signals from four horizontal geophones at station 122 (distance between exit port on source probe and receiver is provided on trace).

Difficulty in obtaining the arrival of the direct p-wave also occurred at this test area. The problem stemmed from the absence of a clearly defined first arrival and from information loss when setting the oscilloscope time scale to an appropriate level. A 1 m sec/cm or 0.5 m sec/cm time scale were used in recording the direct p-wave; therefore, significant errors in arrival time differences could have been introduced.

With no knowledge of the attenuation of the first arrival at the horizontal geophone, it is difficult to state whether this first arrival is the direct  $S_{\text{H}}$ -wave or the Love wave. The feasibility of being the Love wave was investigated for the sediment at the mooring station. At this station a vibracore was obtained which provided information on the sediment properties in the top 1.7 m. Extending to 0.8 m was a very fine sand density 2.14 g/cm<sup>3</sup> overlaying a dense clayey silt of density 2.4 g/cm<sup>3</sup>. As the source probe was implanted 0.45 m, the minimum critical distance is 0.70 m. It is therefore unlikely the first arrival at the nearest two hydrophones (0.3 m and 0.61 m from the source) is the Love wave. At the other stations a core length of less than 0.6 m was taken.

As arrival time measurements were generally made only on the nearest receivers, 1 in and 2 in from the source, the time for the direct  $S_H$ wave to propagate to these receivers is very short. Therefore, if the layer thickness is large, the direct  $S_H$  wave can arrive before the Love wave. If the layer thickness is small or if there exists a velocity gradient in the surface sediment, the Love wave can arrive first; but the time required to form the Love wave must be short. Consequently, the energy forming the Love wave cannot penetrate further than 2 in from the source. For this case, the Love wave velocity would be comparable to the shear wave velocity. Therefore, it will be assumed that the first arrival is the direct  $S_H$ -wave (bearing in mind that further mathematical and experimental investigations are needed to clearly establish the extent to which this can be assumed).

# 7.2.3 Correlation Between Acoustical and Mass-Physical Properties

Previous investigations by Hamilton (1970b), Horn et al. (1968), Schreiber (1968), and others have found predictable relationships for compressional velocity vs porosity and compressional velocity. Porosity is the ratio of the volume of voids occupied by the water to the total volume. As the rigidity is of low magnitude in saturated sediments, the compressibility of the water largely determines the compressional wave velocity rather than the compressibility of the grains. Therefore, a strong correlation is expected to exist between the compressional wave velocity and porosity. Figure 95 illustrates the relationship obtained between the compressional velocity and porosity.

Also, the compressional velocity is strongly related to the sediment's grain size. This affect is due to the fact that density, porosity, and other sediment porperties are a function of grain size. In marine sediments, a decrease in grain size produces an increase in porosity, and consequently a decrease in compressional velocity. Velocity-mean grain size relationship is shown in figure 96.

In discussing the relationship of the shear wave data with the massphysical properties, it was more convenient to use rigidity as the shear



Figure 95. Compressional velocity vs mean grain size and porosity.



Figure 96. Dynamic rigidity vs mean grain size.

parameter. Hamilton (1971) and Hardin and Richart (1963) give excellent discussions on the correlation of rigidity with other sediment properties such as porosity and grain size. From these reports, the following conclusions were derived for sands:

- dynamic rigidity should be a minimum in the coarse sands as they are rounder and possess fewer intergrain contacts;
- (2) an increase in porosity will cause a decrease in rigidity for sands of the same grain size and angularity;
- (3) although porosity increases as the grain sizes decrease, rigidity may increase as the grains become finer because finer grains have more intergrain contacts and higher angularity;
- (4) at some fine sand-silt sizes, the intergrain friction and interlocking will be maximum, and, therefore, rigidity will be maximum for natural uncemented sediments; and
- (5) with an increase in amounts of silt and clay, the sand grains are no longer in contact; the intergrain friction and interlocking are no longer effective, and rigidity becomes dependent on the cohesion between finer particles.

As mentioned previously, the determined empirical relationships between rigidity and porosity and between rigidity and mean grain size are illustrated in figures 95 and 96. These relationships generally follow the conclusions of Hamilton (1971) and Hardin and Richart (1963) discussed above. Maximum rigidity occurs for a fine sand of grain size 2.5 phi units (or .24 mm). For the larger grains sizes, the rigidity is less due to fewer intergrain contacts and rounder grains. Moreover, the rigidity decreases rapidly when silt is introduced into the sediment. This phenomenum is better illustrated in figure 97. Included in figure 98 is the variations of rigidity with percent of sand and gravel. As shown, the increase in the amount of silt reduces the rigidity. This is caused by an increase in porosity and a decrease of density.



Figure 97. Dynamic rigidity vs porosity.

An additional relationship of rigidity vs Poisson's ratio is shown in figure 99. Poisson's ratio is the ratio of the transverse strain to the longitudinal strain. For a liquid, rigidity is zero, and Poisson's ratio reaches its maximum value, 0.5. Consequently, as rigidity increases, Poisson's ratio will decrease.







Figure 99. Dynamic rigidity vs Poisson's ratio.

#### 7.3 Direct Current Resistivity Experiment

### 7.3.1 Data Interpretation

For maximum flexibility in the interpretation of field data, it would be desirable to have available sets of interpretation curves for a seafloor array, similar to those published for land use. Such curves are computed by calculating the potential field generated by a single current electrode placed over a layered medium, with each layer having arbitrary resistivity and thickness. A geometrical factor, depending on the array configuration and spacing, then is used to obtain the voltage difference,  $\Delta V$ , appearing between the potential electrodes for a given current I<sup>13</sup>.

Terekhin (1962a and b) and Van'yan (1956) have examined the theoretical potential generated by a current electrode on a layered sea floor, but their results are not in a form suitable for immediate computation. Other workers in the Soviet Union apparently have published interpretation curves for seafloor arrays, but we have not yet been able to obtain copies of their work. Equations in a form suitable for computer solution are given in Barnes et al. (1972). A simpler, although slightly more timeconsuming method of interpreting sea-floor resistivity data has been used for all of our work to date.

For analytical purposes, an array on the sea floor differs from one on land only in that a layer of conductive sea water is present above the array. If the depth and resistivity of the water layer (which are easily measured) are known, the apparent resistivity  $\rho_{\omega}$  of the water layer may be obtained from the standard interpretation curves. The water layer then may be considered as a known resistance in parallel with the sea floor, and the true apparent resistivity of the sea floor,  $\rho_{a}$ , obtained from the total measured resistivity  $\rho_{t}$  by using the formula

$$\frac{1}{\rho_a} = \frac{1}{\rho_t} - \frac{1}{\rho_\omega} , \qquad (40)$$

where

$$\rho_t = 2\pi a \frac{\Delta V}{I} . \qquad (41)$$

Once the value of  $\rho_a$  has been calculated, published standard interpretation curves may be used to obtain the true bottom resistivity profile.

The curves of apparent bottom resistivity  $\rho_a$  vs electrode separation a obtained in Puerto Escondido are shown in figure 100. Locations B-5a and B-5b were about 30 m apart, in water 12 m deep, and location B-3 was in an area about one km away in water 6 m deep. Bottom water salinity was 32 0/00 in both locations, and temperature ranged between 21 and 23°C.

Figure 101 shows the geologic interpretation obtained by matching the curves of figure 100 against the Wenner interpretation curves published by Mooney and Wetzel (1956a). The resistivity profiles of the two nearby locations, B-5a and B-5b, show no evidence of bedrock. The bottom sediment layers do not, of course, extend to infinity, but the bedrock in this area apparently is too deep to be "seen" with a maximum value of electrode spacing of 300 cm. The presence of bedrock is evident at a depth of 4.8 m at location B-3, which was in shallower water, close to shore. It is significant that even this low-powered array, with relatively small electrode spacings, was able to provide information to a depth of almost 5 m into the sub-bottom.



Figure 100. Data from Puerto Escondido, Mexico.



Figure 101. Data interpretation — seafloor array Puerto Escondido test site.

The tests in Monterey Bay using the in situ expandable survey were run in water 6 m deep, at a temperature of 15°C and salinity of 34 0/00. The formation factor vs electrode separation curve of figure 39 shows an initial decrease in apparent resistivity with depth. A core (vibracorer) of 1.7 m taken at this location showed compact silty sand overlying a loosely packed layer of sand and gravel beginning 30 cm below the surface. The sand and gravel graded into well compacted sandy silt over the next meter. Unfortunately, the sediment was disturbed considerably during the the process of coring and handling, so no resistivity measurements on the core itself were possible. It is reasonable to assume, however, that the resistivity of the loosely packed sand and gravel was less than that of the fairly compact overlying silty sand, and that of the well-compacted sandy silt beneath. This interpretation agrees with that obtained by curve matching, shown on figure 9 (section 3.3) and figure 102. The high-resistivity layer at a depth of 1.7 m may have been the cause of the core tube stopping at that depth.

Surface Towed Array. As mentioned in section 3.3, a surface-towed horizontal resistivity array could provide a rapid method of measuring bottom resistivity values over large areas of the seafloor. A surface-towed array cannot offer the resolution of bottom structure available from a bottom-deployed array, but if electrode spacings are large enough to rorce a sufficient amount of current into the bottom, and if the impressed current is large enough to produce signal levels well above the background noise, considerable information about bottom properties may be obtained.

This is apparent from figures 102 and 103. The ratio  $\Delta V/I$  (the voltage appearing between the potential electrodes divided by the impressed current is plotted against the formation factor of the bottom, with the sea floor considered as a single homogeneous layer and the ratio of electrode separation a to water depth d is equal to 4/3. For an impressed current I of 3 amps, a change in sea-floor formation factor from 2 to 3 will result in a change of about 0.7 millivolts in  $\Delta V$ , and a formation factor change from 9 to 10 gives a change of about 0.2 millivolts in  $\Delta V$ . A 0.2 millivolt change is detectable in practice, so unit changes in sea-floor formation factor detectable in sea-floor formation factor detectable arrangement.

The water was calm near shore, but waves increased to about 5 ft (1.5 cm) in deeper water, resulting in considerable ship motion (which did not impede the profiling operation). Water surface temperature was  $15^{\circ}$ C, and salinity 34 0/00, over the entire area, giving resistivity  $\rho_{\omega}$  of 0.24 ohm-meters. Time, core site, current I, and water depth were noted on the chart recorder output of  $\Delta V$  (fig. 23).

Previous measurements of resistivity over water-covered areas have been by Schlumberger and Leonardon (1934); Volker and Dijkstra (1955); and Bonlos (1972), but these were done with stationary rather than towed arrays. Unz (1959) discusses some of the theory of water-surface resistivity measurements.



Figure 102. Data interpretation — seafloor array Monterey Bay test site.



Figure 103.  $\Delta V/I$  vs formation factor of seafloor sediments.

In contrast to the bottom-deployed array, data obtained from a surfac∈-towed array is handled the same as data obtained from an identical array on land. Differences between the land and marine cases arise mainly during actual application:

- (1) Compared to most land situations, the resistivity of the first (salt water) layer in the marine case is very low, so a large fraction of the impressed current will flow through the water layer. Therefore, higher current levels must be used in offshore work to assure that sufficient current will penetrate the seafloor to give accurate readings.
- (2) The contact resistance of sea water is much lower than that of most earth materials, so high voltages are not needed to force a large current flow into sea water. While several hundred volts may be required to force one amp into the earth, twelve volts are sufficient to force 10 amps into sea water, using electrodes of the same size.
- (3) Noise sources for the land and marine cases are somewhat different in magnitude and origin. While the use of commutated direct current is helpful in removing the effect of dc drift of the potential electrodes on land, excessive changes of dc level often are difficult to compensate. At sea, the drift of the silver-silver chloride potential electrodes is very small, but the effects of waves, currents, and other marine phenomena may be troublesome. Typically, the noise level of a towed electrode pair is less than a few tenths of a millivolt.
- (4) In contrast to the onshore situation, the offshore resistivity profile is continuous with every point along the ship's course sampled. Whereas one or more electrodes (usually two or all four) must be dug up and re-implanted for each resistivity measurement on shore, all electrodes are in continuous contact with the water in the offshore case. This allows much faster (and thus more economical) coverage offshore, as a towing speed of at least 3 knots may be maintained in most sea states, with speeds of up to 10 knots feasible in some situations.

#### 7.3.2 Results

The towed resistivity data was analyzed by the following procedure:

(1) The apparent resistivity  $\rho_a$  is calculated using the equation

 $\rho_a = 2\pi a \frac{\Delta V}{I}$ ,

where a is the electrode separation (20 m),  $\Delta V$  is the measured potential difference at the core site, and I is the measured current.

- (2) Using the calculated value of  $\rho_a$  and the known water depth, the sediment resistivity  $\rho_s$  may be found by reference to a standard set of two-layer resistivity curves, such as those given by Mooney and Wetzel (1956a) or Van Nostrand and Cook (1966, p. 91).
- (3) The formation factor (F) is calculated by dividing the sediment resistivity  $\rho_{\rm S}$  by the water resistivity  $\rho_{\omega}$  (0.24 ohm-m):

 $F = \frac{\rho_s}{\rho_\omega} .$ 

The data is summarized in Appendix A-5. The data was recorded continuously (except for short periods when the current was turned off to check the zero level of the potential electrodes). For analysis, however, the data has been tabulated only at individual core sites. The values of formation factor F are considered accurate to  $\pm$  10 percent, except at the deepest core sites (71, 71, 83, and 84), where the amount of current penetrating the bottom may have been insufficient for accurate readings.

The surface towed resistivity data shown in table 11 covers the survey area shown in figure 104. The data is shown graphically in figures 105 through 107. Figure 105 shows the formation factor (F) and the porosity ( $\phi$ ) measured on core samples for the three east-west traverse lines shown in figure 106. The expected inverse correlation between F and  $\phi$  is readily apparent in lines 124-117 and 107-99, but does not appear on line 116-108. The overall pattern of sediment porosity distribution along the three east-west survey lines is evident in figure 106: soft (high-porosity) sediments toward the center of the area, grading into harder material at the eastern and western boundaries, with a probable bedrock outcrop in the northwestern corner. This pattern also may be seen in the formation factor contour map (fig. 107), which is quite similar in general outline to the porosity contour map (fig. 108).

Core Site	Depth (m)	I	۷۵	Formation Factor (F)
	Y	Line 1		
107	15.2	3.70	10.2	4.00
106	16.8	3.65	10.0	5.67
105	16.5	. 3.60	9.7	4.41
104	15.8	3.60	9.5	3.65
103	15.8	3.60	9.5	3.65
102	15.5	3.60	9.7	3.76
101	14.9	3.60	9.9	3.76
100	14.3	3.60	10.5	4.71
99	14.0	3.60	10.6	4.41
		Line 2		
108	12.5	3.80	12.2	5.06
109	12.8	3.75	11.4	4.00
110	13.7	3.72	11.1	4.41
111	13.7	3.70	10.8	4.13
112	13.7	3.87	10.4	3.44
113	14.3	3.70	10.5	4.00
114	14.6	3.68	10.4	4.13
115	15.2	3.70	10.7	5.45
116	13.7	3.70	11.3	5.45
		Line 3		
123	12.8	3.67	12.7	8.52
122	12.2	3.70	11.6	5.90
121	11.3	3.65	12.0	4.13
120	11.6	3.70	11.8	3.88
119	11.3	3.70	12.6	4.88
118	11.3	3.70	12.3	4.26
	(1.0	3.6/	12.8	5.25
118	11.2	Line 4		
100	11.5	3.50	12.4	6.14
109	12.2	3.60	11.2	3.76
03	14.0	3.70	10.5	9 13
95 X-1	19.0	3.70	9.8	3.76
84	10.3	3.70	9.0	3.35
71	21.0	3.70	8.4	3.00
	20,0	3.02	1.0	2.33
72	2.7 1	Line 5	7 1.	
82	21.4	3.0	/.4 0.c	1.60
¥-2	19.5	2.80	0.5	2.70
94	17.1	2 65	9.0	3.20
103	15 2	2.65	9.1	3.08
112	12.4	2.65	9.5	3.08
121	10 4	2.65	11.7	3.44
161	10.7	2.02	11.7	3.44

Table 11. Towed Resistivity Data



Figure 104. Index map of towed resistivity survey.



Figure 105. Direct current resistivity test – east-west profil of F vs  $\phi$ .



Figure 106. East-west formation factor profiles surface-towed resistivity.

÷



Figure 107. Formation factor contour map.



Vigure 108. North-south formation factor and porosity profiles surface-towed resistivity.

Formation factor and porosity along the two north-south traverse lines (118-71 and 121-72) are shown in figure 108. The inverse correlation between F and  $\phi$  again is apparent on these profiles, although, as mentioned above, the deep-water values may be somewhat suspect. The low porosity at core site X-2 is reflected as a peak in the formation factor curve at that point. As the depth at X-2 was 64 ft (19.5 m) roughly equal to the electrode separation a, it appears that valid data may be obtained at a water depth at least equal to the electrode separation.

A statistical analysis of formation factor and porosity over the entire survey area showed no mathematical correlation between the two parameters, although reasonable correlation along individual survey lines is evident in figures 105 through 108. There are two possible explanations for the lack of mathematical correlation:

- It is quite possible that the porosity measured on some of the cores was not representative of true *in situ* porosity. The sediments are disturbed to some extent when the core is taken, and the degree of disturbance probably varied from core to core. A few "wild points", due to erroneous porosities, could have a strong negative effect on the mathematical correlation coefficient.
- (2) The diver-operated coring device has a maximum penetration of 70 cm, and most cores in the survey area did not achieve maximum penetration. The electric current penetrates the bottom to a depth considerably greater than one meter, and so measures sediment properties at depths greater than those sampled by the cores. If sediment porosity changes significantly with depth at a given location, or if bedrock is within a few meters of the sediment surface, this information will be reflected in the towed resistivity values, but may not be evident in the cores.

# 8. CONCLUSIONS AND RECOMMENDATIONS

8.1 Reflection Coefficient Mapping Experiment

The correlation matrices and other results of this experiment indicate that there is some potential in using remote sensing acoustical apparatus by a ship underway for mapping, at least qualitatively, the mass physical properties of seafloor sediments. Correlations obtained in this study were inconclusive, but in-depth evaluation of the data would provide

information on alternate approaches to the problems involved in data acquisition and analysis. It was obvious from the preliminary analysis of the results that there are many problems in applying surface towed systems to map sediment properties, even with the utmost navigation control and advanced acquisition instrumentation. Improved automatic data processing, using real-time, on-line displays is absolutely essential. It was learned that at least four problems exist which may affect the reliability of the data:

- (1)Determining what is a representative statistical sample.
- (2) Obtaining accurate near-surface sediment velocity data.
  (3) Control of the distance between the source-receiver system and the seafloor to maintain the same differential area that the energy impinges upon.
- (4) Maintaining the stability of the sensor package in a horizontal orientation, for attaining normal incidence.

It is recommended that further research be conducted in data processing and analysis procedures, using the data derived from this study. Investigation of all the permutations of data analysis in this study was beyond the capability and funds allotted. Development should be pursued in the engineering design of self-contained submerged tow systems capable of powering and accurately navigating at any depth and at a fixed elevation in a horizontal orientation. This development is particularly desirable in view of the requirements for more information on deep sea sediment properties and the need to obtain these measurements in as rapid and economical manner as possible.

# 8.2 Shear Wave Experiment

In the past, the detection of the shear wave in saturated marine sediments was accomplished indirectly by measuring the Stoneley wave velocity or employing a viscoelastometer. The high energy losses of the shear wave and the low sediment rigidity prevented direct measurements of the shear wave. It is felt that, on the basis of the data and discussions presented, the described seismic method successfully detected the direct  $S_{\mu}$ wave in gravelly coarse sand at a lake test site. For most of the stations in the Monterey Bay test area, it could not be clearly established that the detected wave was the shear wave or the Love wave, which has been attributed to insufficient core lengths to assertain sediment stratigraphy. As drastic changes in the sediment properties and, thus, shear velocities are not generally anticipated, and because the separation distances between source and receivers is small the Love wave, if detected, would have comparable velocity to the shear wave. Therefore it was assumed that the velocity obtained is the shear velocity.

A refined seismic method is planned for future *in situ* tests. A multi-channel magnetic tape recorder will be used instead of the oscilloscope. This will increase the accuracy in time measurements and reduce the number of source shots. Hydrophones will be employed to detect the compressional wave. Moreover, both receivers probes and source probe will be inserted mechanically to obtain accurate distances between probes. A sampler capable of taking longer cores is also a necessary addition.

The source proved adequate in generating shear waves; however, the air gun will introduce air into the sediment. Some air might enter the sediment region between the source and receivers although the source energy is directed away from this sediment. This will reduce the value of compressional wave velocity largely through the reduction in the bulk modulus. Therefore, consideration has been given to develop a new source such as a bi-directional electro-magnetic hammer. Being bi-directional, the new source can produce stacked records to magnify the S<sub>H</sub> waves or records showing the polarity reversal of the S<sub>H</sub> waves through reversing the direction of the source.

As discussed, the dynamic values of rigidity, bulk modulus, Young's modulus, and Poisson's ratio can be computed from known values of the compressional velocity, shear wave velocity, and density. As the time durations of acoustical stresses last only for a thousandth of a second, the dynamic constants will vary considerably with those determined by conventional static tests in the laboratory. Clearly, *in situ* acoustical measurements can provide useful information about these constants, particularly if acoustical properties can be related with those resulting from static laboratory tests.

Empirical relationships do exist between acoustical and mass physical properties. These relationships can be used in two ways. If porosity, textural properties, shear strength, bearing capacity, or settlement can be related to the acoustical properties for a particular marine environment (bay, shelf, abyssal plains/hills), one need only secure the acoustical properties and a disturbed core sample to ascertain the sediment type to correlate these relationships and obtain the approximate mass physical and engineering properties of the site. Reversing the procedures, one could obtain the acoustical properties by consulting these relationships if the mass physical properties are measured from an undisturbed core sample. Future work should entail securing these relationships and determining the accuracy that can be expected in using these relationships.

In conclusion, knowledge of the acoustical properties of seafloor sediments, as derived from shear wave analysis, can provide useful information in obtaining the geotechnical variations within the marine environment. This information would be extremely helpful in solving engineering problems related to excavation, tunneling, or for construction of platforms, dams, and bridges in the marine environment.

#### 8.3 Direct Current Resistivity Experiment

From the above discussion of results, it is evident that a towed offshore resistivity array can give information about seafloor properties, even with the use of unsophisticated equipment. Several improvements in equipment and data handling could result in even more rapid, more accurate, and less expensive resistivity data acquisition:

- (1) Other electrode arrays, such as the Schlumberger and Anipole, should be investigated theoretically in the field. It should be possible to optimize array configuration and electrode spacing for expected water depths and bottom properties.
- (2) The use of a multiple electrode array would enable multiple-layer interpretations of bottom geology to be made. Electrodes could be switched sequentially to vary their spacing, or arrays could be devised in which a single pair current electrodes is continuously energized and the signal between multiple potential electrode pairs recorded.
- (3) Improved accuracy and speed would result from electronic processing of the data, with the current signal divided electronically into the filtered potential signal. This information could be stored on tape, along with water depth, temperature, and salinity, for automatic processing of the data.

In summary, the results of this study indicate that a towed offshore resistivity system can, especially when used in conjunction with other geophysical techniques, provide valuable data about seafloor properties. System improvements, such as those suggested above, could make the technique even more accurate and economical.

# 9. ACKNOWLEDGMENTS

This research was supported in part by the Department of Defense, Advanced Research Projects Agency and was monitored by the Bureau of Mines, Department of Interior under contract H0220019. The authors are grateful to the supporting services of the Marine Minerals Technology Center and the advisory services of the Department of Engineering Geoscience of the University of California, Berkeley, particularly the technical assistance of Professors Thomas McEvilly, Patrick Wilde and consultant Dr. Peter Rogers.

The credits for the fine line drawings and art work go to Mr. Yok S. Gee, Cartographer for the Center.

Our appreciation also goes to the operators of the R/V Doodlebug and R/V NOAA's Ark, Captain Peter Sheppard and John Brown and engineer Herb Price.

# 10. REFERENCES

Albers, Vernon M. (1969), Underwater acoustics instrumentation, Instrument Soc. of America, 72-75.

- Anderson, O. L., and P. C. Liebermann (1968), Sound velocities in rocks and minerals: Experimental methods, extrapolations to very high pressures and results in physical acoustics, principals and methods, ed. W. P. Mason (Academic Press, New York, N.Y.).
- Archie, G. E. (1942), The electrical resistivity log as an aid in determining some reservoir characteristics, *AIME* 146:54-62.
- Barnes, B. B., R. F. Corwin, J. H. Beyer, Jr., and T. G. Hildenbrand (1972), Geologic Prediction: Developing tools and techniques for the geophysical identification and classification of sea-floor sediments, NOAA Tech. Rept. ERL 224-MMTC 2, 163 pp.

- Barnes, B. B., and R. Newman (1971), A subsystem for electronic positioning and navigation at sea, *Proc. of the IEEE Conf. on Engineering* in the Ocean Environment, 121-146.
- Bieda, G. E. (1970), Measurement of the visco elastic and related mass physical properties of some continental terrace sediments, M.S. Thesis, U.S. Naval Postgraduate School, Monterey, Calif.
- Biot, M. A. (1962), Mechanics of deformation and acoustic propagation in porous media, J. Appl. Phys., 33:1482-1498.

Birch, F., V. F. Schairer and H. C. Spicer (1942), Handbook of physical constants, *Geol. Soc. America Special Paper* 36, 325 pp.

Bobber, Robert J. (1970), Underwater electroacoustic measurements, Naval Res. Labs., pp. 11-15, 17-45 and 193-273.

Bodholt, H. (1969), Scientific echo sounding systems — measuring target strength and backscattering strength, SIMRAD Tech. Bull. No. 5, SIMRAD, Inc., Kew Gardens, N.Y., 11 pp.

Borcherdt, Roger D. (1971), Inhomogeneous body and surface plane waves in a generalized viscoelastic half space, Ph.D. Thesis (Nov).

Boulas, F. K. (1972), Electrical sounding on the water surface at Khor Kundi El-Bahari in Egypt, *Geophysical Prospecting* 20(2):304-316.

Bouma, A. H., W. E. Sweet, F. B. Chmelik and G. L. Huebmer (1971), Shipboard and in situ electrical resistivity logging of unconsolidated marine sediments, Proc. Third Annual Offshore Technology Conf., 1(1351):253-368, Houston, Texas.

Boyce, R. E. (1968), Electrical resistivity of modern marine sediments from the Bering Sea, J. Geophys. Res. 73(14):4759-4766.

Boyce, R. E. (1967), Electrical resistivity logging of modern marine sediments from the Bering Sea, M.S. Thesis, San Diego State College, Calif., 172 pp.

Breslau, L. R. (1967), The normally incident reflectivity of the sea floor at 12 KC and its correlation with physical and geological properties of naturally occurring sediments, Woods Hole Oceanographic Institution Reference No. 67-16, 131 pp.

Breslau, L. R. (1964), Classification of sea floor sediments with a shipborne acoustic system, Woods Hole Oceanographic Contribution No. 1678 in Le Petrole et Mer, Sec. 1, No. 132 (printed in French-English transl.), 9 pp. (Also in La Revue Petroliere, 1965 reprint).

Buchan, S., F. C. D. Dewes, D. M. McCann and D. Taylor-Smith (1967), Measurements of the acoustic and geotechnical properties of marine sediment cores, in Marine Geotechnique, ed. Richards (Univ. of Illinois Press, Urbana, Ill.), pp. 65-92.

Bucker, H. P. (1964), Normal mode sound propagation in shallow water, J. Acoust. Soc. Am. 36:251-258.

Caulfield, David D. (1962), Predicting sonic pulse shapes of underwater spook discharges, Woods Hole Oceanographic Institution Rept. No. 1270 (ONR-sponsored research).

Chesterman, W. D., J. M. P. St. Quinton, Y. Chan and H. R. Matthews (1967), Acoustic surveys of the sea floor near Hong Kong, *Internatl. Hydro*. *Rev.* 44(I):35-54.

Coggon, J. H., and H. F. Morrison (1970), Electromagnetic investigation of the sea floor, *Geophys.* 35(3):476-489.

Cohen, S. R. (1968), Measurement of the visoelastic properties of watersaturated clay sediments, M.S. Thesis, U.S. Naval Postgraduate School, Monterey, Calif.

Connors, D. N., and K. Park (1967), The partial equivalent conductances of electrolytes in seawater, revised, *Deep-Sea Res.* 14:481-484.

Cooley, J., P. Lewis and P. Welch (1967), Historical notes on the FFT, IEEE Trans. on Audio and Electroacoustics AV-15(2):76-79.

Corwin, R. F. (1973), Application of self-potential to offshore prospecting, Ph.D. Thesis, Univ. of California, Berkeley, Calif.

Corwin and Conty, A rugged silver-silver chloride electrode for field use, *Rev. Sci. Instru.* (to be published).

Dakhnov, V. N. (1959), Geophysical well logging, Colo. School of Mines Quarterly 57(2):445 pp.

Davies, D. (1965), Dispersed Stoneley waves on the ocean bottom, Bull. Seismol. Soc. Am. 55:903-918.

Dunlop, H. F., and C. H. Johnson (1958), Research and progress in exploration, *Geophysics* 23(2):267-284.

Erchul, R. A., and V. A. Nacci (1971), The use of electrical resistivity measurements to predict porosity of marine sediments, Proc. Interntl. Symp. on Engineering Properties of Sea-Floor Soils and their Geophysical Identification, Seattle Wash., pp. 296-308.

- Ewing, W. M., W. S. Jardetzky and F. Press (1957), Elastic Waves in Layered Media (McGraw-Hill, New York, N.Y.) 380 pp.
- Faas, R. W. (1969), Analysis of the relationship between acoustic reflectivity and sediment porosity, *Geophysics* 34(4):549-553.

Flügge, W. (1967), Viscoelasticity (Blaisdell Press, ).

Folk, R., and W. Ward (1957), Soils analysis, J. Sedim. Pet. 27:3-26.

Fung, Y. C. (1965), Foundations of Solid Mechanics (Prentice-Hall, Inc., New York, N.Y.).

Garrels, R. M., and M. E. Thompson (1962), A chemical model for sea water at 25°C and one atmosphere total pressure, Am. J. Sci. 260:57-66.
- Grant, F. S., and G. F. West (1965), Interpretation Theory in Applied Geophysics (McGraw-Hill, New York, N.Y.), 584 pp.
- Griffiths, D. H., and R. King (1965), Applied Geophysics for Engineers and Geologists (Pergamon Press, Oxford), 78-82.
- Hamilton, E. L. (1972), Compressional wave attenuation in marine sediments, *Geophys.* 37(4):p. 620.
- Hamilton, E. L. (1971), Elastic properties of marine sediments, J. Geophys. Res. 76:579-603.
- Hamilton, E. L. (1970), Sound velocity and related properties of marine sediments, North Pacific, J. Geophys. Res. 75(23):4423-4444.
- Hamilton, E. L. (1970), Reflection coefficients and bottom losses at normal incidence computed from Pacific sediment properties, *Geophys*. 35:995-1004.
- Hamilton, E. L. (1969), Sound velocity, elasticity and related properties of marine sediments, North Pacific, Part I: Sediment Properties, Environmental Control and Empirical Relationships, Naval Undersea Res. Dev. Center Tech. Publ. 143.
- Hamilton, E. L., and H. W. Menard (1956), Density and porosity of seafloor surface sediments off San Diego, Bull. Am. Assoc. Petrol. Geol. 40:754-761.
- Hamilton, E. L., H. P. Bucker and D. L. Kein (1970), Velocities of compressional and shear waves in marine sediments determined in situ from a research submersible, J. Geophys. Res. 75:4039-4049.
- Hardin, B. O., and L. E. Richart, Jr. (1963), Elastic wave velocities in granular soils, J. Soil. Mech. and Foundations Div., Chem. Soc. Civil Engr. SM 1, pp. 33-65; and discussions, SM 5, pp. 103-118.
- Harvey, H. W. (1960), The Chemistry and Fertility of Sea Waters (Cambridge Univ. Press, Cambridge, Mass), p. 4.
- Hirowaka, M. C. (1966), Engineering properties of marine sediments near San Miguel Island, USN Civ. Engrg. Lab. Tech. Rept. R-503, Port Hueneme, Calif.
- Horn, D. R., B. M. Horn and M. N. Delacy (1968), Correlation between acoustical and other physical properties of deep sea coves, J. Geophys. Res. 73:1939-1957.

Humphrey, Burton, and William J. Gunman (1967), Study of a pulsed underwater sound projector, PC 088 R000Z, FHR 2229-5, PCD-TR-67-16, pp. 3, 9-11 (Dec.).

- Hutchins, Roger W. (1969), Broadband electro-acoustic sources for high resolution sub-bottom profiling, presentation at Internat1. Oceanology Conf.
- I. C. Logic Handbook (1972), Published by Data Scan, pp. 79-80.

Internatl. Oceanog. Tables (1966), Natl. Inst. Oceanography of Great Britain and UNESCO.

- Jenkins, R., and W. Takeyama (1971), Bottom-sitting diver-operated core sampler, NOAA Rept. ERL-MMTC Tech. Memo., 13 pp.
- Jones, J. L., C. B. Leslie and L. E. Burton (1964), Acoustic characteristics of underwater bottoms, J. Acos. Soc. of Am. 36:154-163.
- Jones, R. (1958), In situ measurement of the dynamic properties of soil by vibration methods, Geotech. 8:1-21.
- Keller, G. H. (1965), Deep sea nuclear sediment density probe, Deep Sea Res. 12:373-376.
- Keiler, G. H. (1964), Investigation of the application of standard soil mechanic techniques and principles to buy sediments, Proc. First Symp. Military Oceanography, p. 329.
- Kesser, G. H., and F. C. Frischknecht (1966), Electrical Methods in Geophysical Prospecting (Pergamon Press, Oxford), 519 pp.
- Kermabon, A. C., C. Gehin and P. Blavier (1969), A deep-sea electrical resistivity probe for measuring porosity and density of unconsolidated sediments, *Geophys.* 34(4):554-571.
- Knepoff, L., and G. J. F. MacDonald (1960), Models for acoustic loss in solids, J. Geophys. Res. 65:2191-2197.
- Knepoff, L., and G. J. F. MacDonald (1958), Attenuation of small amplitude stress waves in solids, *Rev. Modern Phys.* 30:1178-1192.
- Laughton, A. S. (1957), Sound propagation in compacted ocean sediments, Geophys. 22:233-260.
- Levy, J. D. (1961), Viscoelastic Properties of Polymers (John Wiley & Sons, New York, N.Y.).
- Lewis, L. (1971), An investigation of ocean sediments using the deep ocean sediment probe, Ph.D. Thesis, Dept. of Ocean Engrg., Univ. of Rhode Island, 76 pp.
- Lewis, L., V. A. Nacci and J. Gallagher (1970), In situ investigations of ocean sediments, Offshore Technol. Conf. Paper No. 1290.
- Li, W. N., and D. Taylor-Smith (1968), Identification of seafloor sediments using underway acoustics, *Geophys. Prospecting* 17(3):pp 234, 239-249.
- Li, W. N., and D. Taylor-Smith (1966), Identification of seafloor sediments by a ship underway, *Geophys. Prospecting* 19(1):45-47.
- Lieberman, L. N. (1948), Reflection of sound from coastal sea bottoms, J. Acous. Soc. of Am. 20:305-309.
- Longsworth, L. G. (1932), Transference numbers of aqueous solutions of potassium chloride, sodium chloride, lithiam chloride and hydrochloric acid at 25°C by the moving boundary method, J. Am. Chem. Soc. 54:2751-2758.

- Lyman, J., and R. H. Fleming (1940), Composition of sea water, J. Marine Res., 3:134-146.
- MacKenzie, K. V. (1960), Reflection of sound from coastal bottoms, J. Acous. Soc. Am. 32:221-231.
- Marke, P. A. B. (1965), The development and use of offshore mineral exploration techniques, Ph.D. Thesis, Geophys. Dept., Imperial College, London, S.W. 7, 166-278.
- McCann, C., Theory of propagation of compressional waves in marine sediments, Mar. Sci. Lab. Geol. Rept. No. 66-5 (unpublished).
- McKinney, C. M., and C. D. Anderson (1964), Measurements of backscattering of sound from the ocean bottom, J. Acous. Soc. Am. 36:158-163.
- Mooney, H. M., and W. W. Wetzel (1956), The Potential About a Point Electrode Resistivity Curves for a Two- Three- and Four-Layered Earth (Univ. of Minn. Press, St. Paul, Minn.). (Available from University Microfilms, A Xerox Company, Ann Arbor, Mich.).
- Moore, D., and G. Shumway (1959), Sediment thickness and physical properties, Pigen Pt., Calif., J. Geophys. Res. 64(3):p. 367.
- Moore, T. C., and L. D. Kulm (1970), A high-resolution sub-bottom profiling system for use in ocean basins, Reprint from *Marine Res.* 28(2): pp. 271, 272, 279.
- Murray, H. W. (1967), Topography of the Gulf of Maine field season of 1940, Geol. Soc. Am. Bull. 22(3):523-552.
- Nafe, J. E., and C. L. Drake (1963), Physical properties of marine sediments, in The Sea (John Wiley & Sons, New York, N.Y.), M. N. Hill, ed.

Nafe, J. E., and C. L. Drake (1957), Variation with depth in shallow and deep water marine sediments of porosity, density, and velocities of compressional and shear waves, *Geophys.* 22:523-552.

- Noorany, I. (1972), Underwater soil sampling and testing, a state-of-theart review, ASTM-STP 501.
- Officer, C. B. (1958), Introduction to the Theory of Sound Transmission (McGraw-Hill, New York, N.Y.), 284 pp.
- Parasnis, D. S. (1962), Principles of Applied Geophysics (Methuen, London; John Wiley & Sons, New York, N.Y.).
- Park, K. (1964), Parial equivalent conductance of electrolytes in seawater, Deep Sea Res. 11:729-736.
- Pawlowicz, E. F. (1971), Ocean engineering significance of marine seismic reflection profiling technology, Tech. Note N-1157, Port Hueneme, Calif., pp. 1, 7, 8, 22 (May).
- Physics of Sound in the Sea, Part I (1946), Transmission, Summary Tech. Rept. of the Natl. Defense Res. Committee, section on sound transmission studies.

Porter, W. J., and D. L. Bell, Variability of bottom reflected sound and its influence on sediment prediction models, (unpublished).

Poston, A. (1973), Patent application, Serial No. 033453, for invention of a directional multiple electrode acoustic source.

Preiss, K. (1968), In situ measurement of sediment density by gamma radiation, Deep Sea Res. 15:637-647.

Preiss, K. (1967), Nondestructive measurement of the water content and density of sediment using radiosotopes, in Marine Geotechnique (Univ. Illinois Press, Urbana, Ill.), 307-318.

Rayleigh, J. W. S. (1945), The Theory of Sound, <u>2</u> (Dover Publ., New York, N.Y.).

Rechtin, E. L., J. Steele, R. Scales (1957), Engineering problems related to design of offshore mobile platforms, Paper No. 10, Annual meeting Soc. Nava<sup>1</sup> Architects and Marine Engineers.

Richards, A. F. (19c7), Marine Geotechniques (Univ. of Ill. Press, Urbana, Ill.).

Richards, A. F. (1962), Investigations of deep sea sediment cores, Part II, Mass physical properties, U.S. Navy Hydrographic Office, Tech. Rept. TR-106, 146 pp.

- Rijkswaterstaat, The Netherlands (1969), Standard graphs of resistivity prospecting, European Assoc. of Exploration Geophysicists.
- Rose, V. C., and J. R. Ronsy (1971), A nuclear gage for in-place measurement of sediment density, Offshore Technology Conf. Paper No. 1290.

Sarmiento, R., and R. A. Kirby (1962), Recent sediments of Lake Maracaibo, J. Sed. Petrol. 32:698-724.

Schlank, J. J. (1968), Sub-bottom Sounding: A survey for naval applications, M.S. Thesis, Naval Postgraduate School, Monterey, Calif.

- Schlumberger, C. M., and E. G. Leonardon (1934), Electrical exploration of water — covered areas, Trave. AIME Geophysical Prospecting 110.
- Schopper, J. (1966), A theoretical investigation of formation factor/permeability/porosity relationship using a network model, Geophys. Prospect. 14:301-341.

Schreiber, B. D. (1968), Sound velocity in deep sea sediments, J. Geophys. Res. 73:1259-1268.

Shumway, G. (1960), Sound speed and absorption studies of marine sediments by a resonance method, Parts I and II, *Geophys.* 25:451-467, 659-682.

- Smith, R. J. (1962), Engineering properties of ocean floor soils, ASTM Spec. Tech. Publ. No. 322, 280-302.
- Stoneley, W. C., and W. G. Harris (1968), Acoustic sea bottom classifier, Navy Mines Defense Lab. Rept. NSRDC 2739, Panama City, Fla. (July-AD 835600).

Strick, E., and A. S. Ginzberg (1956), Stoneley-wave velocities for a fluid solid interface, *Bull. Seis. Soc. Am.* 46:281-292.

Sutton, G. H., H. Berkheimer and J. E. Nafe (1957), Physical analysis of deep sea sediments, *Geophys.* 22:779-812.

Taylor-Smith, D. (1968), Physics and sea floor minerals, *Contemp. Phys.* 9(6):565-585.

Taylor-Smith, D., and W. N. Li (1966), Echo sounding and sea floor sediments, *Marine Geol.* (Elsevier Publ. Co., Amsterdam), 4:353-364.

Taylor-Smith, D., and W. N. Li (1966), Identification of sea-bottom sediments by ship underway, Reprint from Geophys. Prospect. 24(1):45-47.

Terekhin, E. I. (1962), Theoretical basis of electrical probing with an apparatus immersed in water, in Applied Geophysics in U.S.S.R., ed. N. Rast (Pergamon Press, New York, N.Y.), 169-195.

Unz, M. (1959), Interpretation methods for geophysical exploration of resources, Geophys. 24(1):109-141.

Urick, R. J. (1954), The backscattering of sound from a harbour bottom, J. Acous. Soc. Am. 26:231-235.

Urick, R. J., and D. S. Saling (1962), Backscattering of explosive sound from the deep sea bed, J. Acous. Soc. Am. 34:1721-1724.

Van Nostrand, R. G., and K. L. Cook (1966), Interpretation of resistivity data, Geol. Survey Professional Paper 499, U.S. Government Printing Office, Washington, D.C., 310 pp.

Van'yan, L. L. (1956), Theoretical curves for electrical sea probing with a sea bottom apparatus, *Appl. Geophys.* No. 50, Gostoptekhizdat.

Vees, E., and H. F. Winterkorn (1967), Engineering properties of several pure clays as functions of mineral type, exchange ions, and phase composition, Highway Res. Board No. 198, 68-70.

Volker, A., and J. Dijkatra (1955), "Determination des salinites des eaux dans le sous sol du Zuiderzee par Prospection Geophysique, Geophys. Prospect. 3(2):111-125.

Von Arx, W. X. (1962), Introduction to Physical Oceanography (Addison-Wesley, Reading, Mass.), 422 pp.

White, J. E. (1965), Seismic Waves; Radiation, Transmission and Attenuation (McGraw-Hill, New York, N.Y.), 302 pp.

Wilde, P. (1969), Tables of formalities and p ion in sea water of total Cl, Na, K, Mg, Ca, Sr, Br, and So<sub>4</sub> for the salinity range of 28 to 40 percent assuming dittmarian conservancy, Univ. of Calif., Berkeley, Calif., 62-63.

Wilson, E., and E. L. Hamilton (1970), Sound velocities and related properties of marine sediment, North Pacific, J. Geophys. Res. 75:4423-4446. Wolfe, S. C. (1970), Coastal currents and mass transport of surface sediments over the shelf of Monterey Bay, Calif, *Marine Geol.* 8:321-336.

Wood, A. B. (1941), A Textbook of Sound (G. Bell, London), 578 pp.

- Wyllie, M. R. J. (1963), The Fundamentals of Well Log Interpretation (Academic Press, New York, N.Y.), 238 pp.
- Wyllie, M. P. J., and A. R. Gregory (1953), Formation factors of unsolidated porous media: Influence of particle shape and effect of cementation, *Petrol. Trans.* 198:103-110.

APPENDIX A.1. CALCULATIONS FOR UNDERWAY ACOUSTICS EXPERIMENT

n

#### APPENDIX A.1. CALCULATIONS FOR UNDERWAY ACOUSTICS EXPERIMENT

A.1.1 Calculation of Reflection Coefficient (R) for Sparker (G-5 kHz Source)



Streamer hydro signal

e-m coupling pulse time bottom Rfl.

Assuming a point source:

$$u_{monitor} = u_{source} \cdot \frac{const.}{d_m - d_s}$$
 (A-2)

Assume  $d_s$  and  $d_r << D$  so source to receiver reflector path length is approximate

$$\sqrt{(D-d_s)^2 + (\ell/2)^2} + \sqrt{(D-d_r)^2 + (\ell/2)^2} = reflection path.$$

So almost exactly

reflector path = 
$$2\sqrt{(D-d_s)^2} = (\ell/2)^2$$

$$\sqrt{4(D-d_{S})^{2} + \ell^{2}}$$

Note: In deeper water  $\ell << D$  so reflector path  $\simeq 2D - (d_s + d_r)$ .

Now

$$u_{rcvr} = u_{source} \cdot \frac{Const.}{\sqrt{4(D-d_s)^2 + \ell^2}} \cdot R.$$
 (A-3)

Eliminating  $u_{source}$  between (1) and (2)

$$\frac{u_{\text{monitor}}}{u_{\text{revr}}} = \frac{1}{R} \cdot \frac{\sqrt{4(D-d_{s})^{2} + \ell^{2}}}{d_{m} - d_{s}} \quad . \tag{A-4}$$

Solving for R,

$$R = \frac{\sqrt{4(D-d_s)^2 + \ell^2}}{d_m - d_s} \cdot \frac{u_{rcvr}}{u_{monitor}}$$
(A-5)

<sup>e</sup>monitor <sup>= u</sup>monitor <sup>• G</sup>impulse hydro <sup>•</sup>  
<sup>e</sup>rcvr <sup>= u</sup>rcvr <sup>• G</sup>streamer hydrophone  
<sup>•</sup> 
$$\frac{^{u}rcvr}{^{u}monitor} = \frac{^{e}rcvr}{^{e}monitor} \cdot \frac{^{G}imp. hyd}{^{G}sta hyd}$$

Eliminating  $u_{rcvr}/u_{monitor}$  between (A-5) and (A-6) we have for R for the sparker system:

$$R = \frac{\sqrt{4(D-d_s)^2 + \ell^2}}{d_m - d_s} \cdot \frac{G_{imp. hydro.}}{G_{streamer hydro}} \cdot \frac{e_{rcvr}}{e_{monitor}} \cdot (A-7)$$

$$geometrical hydrophone gain ratio f VoHages recorded on Mag. tape, i.e., S_r/S_x$$

(A-6)

Substituting the numerical data from below we have:

$$R = .532 \sqrt{4D^{2} + 832} \cdot (.0233) \cdot \frac{e_{str. rcv}}{e_{imp. hyd}}$$
(A-8)  
$$R = .0124 \sqrt{4D^{2} + 832} \cdot \frac{e_{str. rcv}}{e_{imp. hyd}} .$$
(A-9)

Numerical Data:

Sparker:

$$d_{s} = 10" = .832'$$
$$d_{m} = 2' 8^{1}{}_{2}" = 2.71'$$
$$d_{r} = 15".$$

Pinger:

d = 22"

Constants: V = 4.66 ft/m-sec t = 6.2 m-sec

 $\frac{G_{imp. hyd.}}{G_{streamer hyd.}} = .0233$ 

Geometrical Factor = 
$$\frac{\sqrt{4(D-.832)^2 + (28.8)^2}}{2.71 - .832}$$
 (A-10)

$$= .532 \sqrt{4D^2 + 832} . \tag{A-11}$$

Streamer Calibration Flow Chart:



 $\frac{G_{imp. hydro.}}{G_{streamer hyd.}} = \frac{.7 \text{ mv}}{30 \text{ mv}} = .0233 .$ 

(A-12)

A.1.2 Calculation of Reflection Coefficient for Pinger (12 kHz Source)

u = particle amplitude or pressure

In the pinger, unlike the sparker, the source and receiver are the same transducer; however, to aid analysis assume they are two separate transducers.



Also assume that the calibrated hydrophone, MC-115, is located at the reference point 3 feet below the pinger. From previous tank calibrations, when  $e_x = 183 v$ , the MC-115 develops lv. The pinger receiver in the same location would develop 4.65.

Assuming a point source,

 $u_{x} = c \frac{u_{s}}{3}$  (A-13)  $u_{r} = c \frac{u_{s}}{2(D-d)} R$ . (A-14)

Eliminating c u<sub>s</sub>

$$u_r = u_x \frac{3}{2(D-d)} \cdot R$$
 (A-15)

Let  $u_r = e_r/4.65$  and  $u_x = e_x/183$ .

Then

$$e_r = \begin{bmatrix} e_x \\ 183 \end{bmatrix} \begin{bmatrix} 3 \\ 2(D-d) \end{bmatrix} 4.65 R.$$
 (A-16)

APPENDIX A.2 DERIVATION OF BASIC EQUATIONS OF LINEAR VISCOELASTICITY (after Borcherdt, R. D., 1972)

Preceding page blank

### APPENDIX A.2 DERIVATION OF BASIC EQUATIONS OF LINEAR VISCOELASTICITY (after Borcherdt, R. D., 1972)

#### A.2.1 Body Waves in a Linear Viscoelastic Medium

This section is included for completeness, although a detailed study will not be given. The development of the basic equations will follow the approach given by Borcherdt (1972), and the reader is referred to that thesis for a more comprehensive study.

Viscoelasticity is a property of solids and liquids which exhibit both viscous and elastic behavior under deformation. The dissipation and storage of mechanical energy is observed from this behavior. In addition, the constant that links stress and strain in the theory of elasticity becomes a time dependent function in the constitutive equation of viscoelastic theory. For sufficiently small strains, the behavior of a viscoelastic material can be described by the linear theory of elasticity. Employing the superposition principal, the stress strain relation can be written as follows:

$$P_{ij}(t) = \int_{-\infty}^{\infty} r_{ijk\ell}(t-\tau) de_{k\ell}(\tau) \qquad (A-19)$$

where  $P_{ij}(t) \equiv components$  of the 2<sup>nd</sup> order time dependent stress tensor,  $e_{ij}(t) \equiv components$  of the 2<sup>nd</sup> order time dependent strain tensor, and  $r_{iik\ell} \equiv components$  of the 4<sup>th</sup> order time dependent relaxation function.

This equation can be written in a more condensed form as a convolution:

$$P_{ij} = r_{ijk\ell} \quad de_{k\ell} \quad (A-20)$$

If the material is isotropic, a further simplification can be made:

$$P_{ij} = r_s \quad de_{ij} \quad i \neq j \qquad (A-21)$$

$$P_{kk} = r_k \quad de_{kk} \tag{A-22}$$

where  $\mathbf{r}_{s} \equiv$  relaxation function describing shear behavior and

 $r_k \equiv$  relaxation function describing bulk behavior. The primes denote the deviator components of stress and strain:

$$P'_{ij} = P_{ij} - \frac{1}{3} \delta_{ij} P_{kk}$$
 (A-23)

 $e'_{ij} = e_{ij} - \frac{1}{3} \delta_{ij} e_{kk}$  (A-24)

It would be convenient to simplify these stress strain relationships by considering the steady state problem. Moreover, it will be assumed sufficient time has elapsed so initial conditions may be neglected. Let  $P_{ij} = P_{ij}e^{i\omega t}$  and  $e_{ij} = E_{ij}e^{i\omega t}$  where  $P_{ij}$  and  $E_{ij}$ are complex constants independent of time. Equations (A-21) and (A-22) can now be written as

$$P_{ij} = i\omega R_{s}(i\omega) E_{ij} \qquad i \neq j \qquad (A-25)$$

$$P_{kk} = i\omega R_k(i\omega) E_{kk}$$
(A-26)

where  $R_s$  and  $R_k$  represent the Fourier transforms of the shear and bulk relaxation functions, respectively. These quantities,  $R_s$  and  $R_k$ , can be formulated into a more familiar form:

$$\mu \equiv \frac{1\omega R_s}{2}$$
 (A-27)

$$\pm \equiv \frac{i\omega R_k}{3}$$
 (A-28)

$$\lambda \equiv \frac{i\omega \left(R_k - R_s\right)}{3} . \qquad (A-29)$$

It follows

$$P_{-ij} = 2 \mu^3 E_{ij} \quad i \neq j$$
 (A-30a)

$$P_{kk} = 3 \kappa E_{kk} . \tag{A-30b}$$

Other useful relationships are

$$P_{ij} = \delta_{ij} \lambda E_{kk} + 2 \mu E_{ij}$$
(A-31)

and

$$\underline{P}_{ij} = \delta_{ij} \left( \kappa - \frac{2}{3} \mu \right) E_{kk} + 2 \mu E_{ij} . \qquad (A-32)$$

Attention will now be focused on the equation of motion and strain tensor. From the conservation of linear momentum, the equation of motion is:

$$P_{ij,j} + f_i = \phi \frac{\partial^2 u_i}{\partial t^2}$$
 (A-33)

where

 $f_i \equiv body$  force per unit volume,

 $\varphi$   $\equiv$  mass density, and

 $u_i \equiv \text{component of the displacement vector.}$ 

The strain tensor, assuming infinitesimal displacement is given by

$$e_{ij} = \frac{1}{2} \left( u_{i,j} + u_{j,i} \right)$$
 (A-34)

Steady state simplification will be employed again. Let  $u_i = U_i e^{i\omega t}$  where  $U_i$  is a complex constant independent of time. Therefore, the equation of motion and strain tensor simplify to

$$P_{-i,j,i} + \phi \omega U_{j} = 0 \tag{A-35}$$

and

$$E_{ij} = \frac{1}{2} \left( U_{i,j} + U_{j,i} \right)$$
 (A-36)

(where body forces have been neglected). Using (A-36) and (A-32) the equation of motion, (A-35), can be written in terms of  $u_i$  (in vector form):

$$\left( \left( \left( x + \frac{4}{3} \mu \right) \nabla \theta - \mu \nabla x (\nabla x u) + \Phi \omega^2 u = 0 \right) \right)$$
(A-37)

where

and

 $\theta \equiv \nabla \mathbf{u}$ .

Note that this equation is equivalent to the equation of motion for an elastic medium except for the Lamé constants which may now be complex (i.e.,  $\mu = \mu_R + i\mu_I$  and  $\kappa = \kappa_R + i\kappa_I$ ). Also, the expression for the time rate of energy and 2nd law at thermodynamics reveal that  $\mu_I$  and  $\kappa_I$  are non-negative (Borcherdt, 1972, p. 43).

By taking the divergence and the curl of the equation of motion, (A-37), one can obtain, respectively, the equations for dilatational and rotational waves:

 $\nabla^2 \theta + \frac{\omega^2}{\alpha^2} \theta = 0 \tag{A-38}$ 

$$r^{2} \left(\frac{\nabla xu}{2}\right) + \frac{\omega^{2}}{\beta^{2}} \left(\frac{\nabla xu}{2}\right) = 0, \qquad (A-39)$$

where the complex compressional velocity ( $\alpha$ ) and the complex shear velocity ( $\beta$ ) are given by

$$\alpha = P \cdot V \cdot \left(\frac{\kappa + \frac{4}{3}\mu}{\phi}\right)^{\frac{1}{2}}, \qquad (A-40)$$

$$\beta \equiv P.V. \left(\frac{\mu}{\Phi}\right)^{\frac{1}{2}} . \tag{A-41}$$

(P.V. represents the principal value). The respective velocities of the shear and compressional wave in homogeneous isotropic linear viscoelastic medium are

$$V_{p} = \left[ \operatorname{Re}\left(\frac{1}{\alpha}\right) \right]^{-1}, \qquad (A-42)$$

$$V_{s} = \left[ \operatorname{Re}\left(\frac{1}{\beta}\right) \right]^{-1}.$$
 (A-43)

The absorption coefficient of these two waves are

$$a_{p} = -\omega I_{m}\left(\frac{1}{\alpha}\right), \qquad (A-44)$$

$$a_{s} = -\omega I_{m}\left(\frac{1}{\beta}\right). \tag{A-45}$$

Little will be said concerning the quantity  $Q^{-1}$  except to define  $Q_{\kappa}^{-1}$ ,  $Q_{S}^{-1}$ , and  $Q_{p}^{-1}$ . The specific attenuation factor in bulk will be denoted as  $Q_{\kappa}^{-1}$  and is expressed as

$$Q_{\kappa}^{-1} = \frac{\kappa_{\rm I}}{\kappa_{\rm R}} \quad . \tag{A-46}$$

Similarly, the attenuation factor for general dilatation and shear is given respectively as

$$Q_{p}^{-1} = \frac{\kappa_{I} + \frac{4}{3} \mu_{I}}{\kappa_{R} + \frac{4}{3} \mu_{R}}$$
(A-47)

and

$$Q_{\rm S}^{-1} = \frac{\mu_{\rm I}}{\mu_{\rm R}} \quad . \tag{A-48}$$

It is convenient to express the compressional and shear velocity in terms of  $Q_p^{-1}$  or  $Q_s^{-1}$ . From (A-43)

$$\frac{1}{V_{s}} = \operatorname{Re} \sqrt{\frac{\phi}{\mu}} = \frac{\sqrt{\phi}}{|\mu|} \operatorname{Re}(\sqrt{\mu})$$
(A-49)

or

$$\frac{1}{V_{s}} = \frac{\sqrt{\Phi}}{|\mu|} \operatorname{Re} \sqrt{\frac{|\mu| + \operatorname{Re}(\mu)}{2}} + i\sqrt{\frac{|\mu| - \operatorname{Re}(\mu)}{2}} . \qquad (i -50)$$

Further reduction using (A-48) yields

$$V_{s} = \sqrt[4]{\frac{\mu_{R}}{\phi}} \sqrt{\frac{2(1+Q_{s}^{-2})}{\sqrt{1+Q_{s}^{-2}+1}}} = q_{s} \sqrt{\frac{\mu_{R}}{\phi}}$$
(A-51)

similarly,

$$v_{p} = \sqrt{\frac{\kappa_{R} + \frac{4}{3} \mu_{R}}{\phi}} \sqrt{\frac{2(1 + Q_{p}^{-2})}{\sqrt{1 + Q_{p}^{-2} + 1}}} = q_{p} \sqrt{\frac{\kappa_{R} + \frac{4}{3} \mu_{R}}{\phi}} .$$
 (A-52)

This concludes the discussion on the basic equations of body waves in linear viscoelastic medium. For further discussions, refer to Borcherdt (1972).

We can check the correctness of the formula from the following consideration. The term 2(D-d) is the distance from source to receiver. If we move the receiver to the reference point as shown this term equals 3' and there is no reflection (R=1). Then (A-16) becomes:

$$e_r = \begin{bmatrix} e_x \\ 183 \end{bmatrix} \begin{bmatrix} 3 \\ 3 \end{bmatrix} 4.65,$$
 (A-17)

for  $e_x = 183v$ . This yields  $e_r = 4.65$  volts which is the value from tank measurements. Solving for R with d = 22" = 1.83'

$$R = 26.2 (D' - 1.83') \frac{e_{rcvr}}{c_{xmtr}} .$$
 (A-18)

Tank Calibrations of Pinger - Block Diagram:







APPENDIX A.3 SEDIMENT PROPERTY EQUATIONS

Property	Symbol	Method of Calculation
Shear-wave dissipation function	Q_5^1	$Q_{S}^{-1} = \frac{2 V_{S}^{a} S_{S}}{\omega} / \left[ 1 - \frac{V_{S}^{a} S_{S}}{\omega} \right]^{2}$
Rigidity representing elastic response	<sup>μ</sup> R	$\mu_{\rm R} = \phi \left( \frac{V_{\rm s}}{q_{\rm s}} \right)^2$
Rigidity representing wave energy damping	۲	$\mu_{I} = \mu_{R} Q_{S}^{-1}$
Bulk modulus representing elastic response	ĸ <sub>R</sub>	$K_{R} = \phi V_{p}^{2} - \frac{4}{3} \mu_{R}$
Lamé constant representing elastic response	<sup>λ</sup> R	$\lambda_{\rm R} = K_{\rm R} - \frac{2}{3} \mu_{\rm R}$
Poisson's ratio	σ	$\sigma = \left[ \frac{(V_p q_s / V_s)^2 - 2}{2(V_p q_s / V_s)^2 - 2} \right]$
Young's modulus	E	$E = \mu_p(1+\sigma)$

APPENDIX A.3 SEDIMENT PROPERTY EQUATIONS

## A-3.2 Notation Summary

Symbol_	Description
a <sub>s</sub>	Shear-wave attenuation coefficient
a	Compressional-wave attenuation coefficient
E	Young's modulus
κ <sub>I</sub>	Bulk modulus representing wave energy damping
KR	Bulk modulus representing elastic response
q <sub>p</sub> =	$\sqrt{2(1+Q_p^{-2})}/\sqrt{1+Q_p^{-2}} + 1$
q <sub>s</sub> =	$\sqrt{2(1+Q_{s}^{-2})}/\sqrt{1+Q_{s}^{-2}} + 1$
Q_{S}^{-1}	Shear-wave dissipation function
$Q_p^{-1}$	Compressional-wave dissipation function
Vs	Snear-wave velocity
۷ <sub>p</sub>	Compressional-wave velocity
φ	Bulk density
λI	Lamé constant representing wave energy damping
۸R	Lamé constant representing elastic response
μI	Rigidity representing wave energy damping
<sup>μ</sup> R	Rigidity representing elastic response
σ	Poisson's ratio
ω	Angular frequency

APPENDIX A.4 SUMMARY OF SAMPLING PROGRAM

# Preceding page blank

Site No	Corrected Water Depth (ft)	Core Length (cm)	Predominant Sediment Type	Corer Type Driver Sampler: Piston type-P Open type-C
23 24 25 26 38	102 106 106 113 124	60.4 43.8 63.2 65.1 61.7	silty-sand silty-sand sandy-silt silty-sand silty-sand	Р Р Р Р
39 40 41 42 43	117 111 105 99 100	62.2 65.6 60.9 65.0 62.9	silty-sand silty-sand silty-sand silty-sand silty-sand silty-sand	Р Р Р Р
44 45 46 47 48	97 92 92 95 95	63.6 64.0 65.4 63.3 61.7	silty-sand silty-sand silty-sand silty-sand silty-sand	P P P P
49 50 51 52 57	1C2 105 114 121 123	62.0 27.9 54.5 47.9 42.2	silty-sand silty-sand silty-sand sandy-silt silty-sand	Р Р Р Р
58 59 60 61 62	118 111 105 96 93	55.6 62.3 64.4 65.2 62.9	silty-sand silty-sand silty-sand/sandy-silt silty-sand silty-sand	Р Р Р Р
63 64 65 66 68	91 89 87 85 83	62.3 40.2 60.9 51.1 13.8	silty-sand silty-sand silty-sand silty-sand silty-sand silty-sand	Р Р Р Р
69 69 70 71 72	83 83 83 84 85	45.2 90.2 62.6 63.8 63.2	silty-sand silty-sand silty-sand silty-sand silty-sand silty-sand	Р О Р Р
73 73 81 82 83	90 90 73 70 68	54.5 104.0 6.3 32.6 4.5	silty-sand silty-gravelly sand silty-sand silty-sand sandy-silt	Р О Р Р
84 85 86 87 88	55 69 73 75 73	54.2 63.7 52.2 10.4 11.0	silty-sand silty-sand silty-sand silty-sand silty-sand silty-sand	Р Р Р Р
92 93 94 95A 95B	45 48 53 56 56	9.0 63.9 59.7 38.7 5.1	silty-sand silty-sand sandy-silt sandy-silt sandy-silt	Р Р Р Р

Site No	Corrected Water Depth (ft)	Core Length (cm)	Predominant Sediment Type	Corer Type Driver Sampler: Piston type-P Open type-O
 100 102 103	43 47 48	34.1 39.9 51.0	gravelly-sand silty-sand/sandy-silt sandy-silt	P P P
103 104	48 49	38.1 63.0	sandy-silt silty-gravelly sand	0 P
105 109 109 110 111	49 38 38 40 42	41.2 54.3 36.3 51.2 23.7	silty-gravelly sand silty-sand silty-sand silty-sand sandy-silt	P 0 P P
112 113 114 115 118(1) 118(2)	42 43 45 46 32 32	41.7 59.7 51.7 6.0 47.2 56.8	silty-sand silty-gravelly sand silty-sandy gravel silty-sand silty-sand silty-sand silty-sand	P P P P
119 120 121 121 122	33 34 34 34 34 36	14.9 22.4 54.9 31.4 55.8	silty-sand silty-sand silty-sand silty-sand silty-sand	P P 0 P
123 123 124 1X 2X	39 39 38 56 60	52.7 34.3 24.1 15.2 31.2	silty-sand silty-sand silty-sand silty-sand silty-sand	P 0 0 P P
3X T.H.1 T.H.2 S.C. Buo	63 17 17 17 17	6.0 64.6 62.8 62.0	silty-sand silty-sand silty-sand gravelly-silty sand and sandy-silt	р Р Р Р
S.C. 8uo 7/10	y 17	259.0	gravelly-silty sand	0
S.C. 8uo 8/28	y 17	170.2	silty-gravelly sand	0

APPENDIX A.5 SUMMARY OF ACOUSTIC, ELECTRICAL RESISTIVITY AND SEDIMENT ANALYSES DATA

Preceding page blank

rosity Dens	ity	Penetrometer	Torvane	Diameter	Sorting Coefficient	Resistivity	Corrected	Observed	Corrected	Observed
0.030 0	0.030 0	0	.080	0.090	1.1408		1.353		5.522	
14 63 2.09 0.030 0.0	0.030 0.0	0.0	080	0.086	1.154/		100-1		4 956	
15.41 2.07 0.560 0.1	0.560 0.1	0.1	10	0.066	1.3964		1.410		4.833	
12.28 2.04 0.500 0.0	0.500		24	0.081	1.3771		1.402	1.385	4.582	
	0.043 0.1	0.1	00	0.092	1.2030		1.496	1.428	4.250	4.468
16 27 2.05 0.280 0.0	0.280 0.0	0.0	960	0.089	1.1180		1.558	1361	5. 364	5.520
12.10 2.11 0.320 0.	0.320 0.	0.	076	0.074	1.1734		105	700 1	4.638	5.259
16.49 2.01 0.600 0.	0.600 0.	o o	080	0.069	1 2910		1.252		4.481	5.282
44.66 2.08 U.300 U	0.300	C	con .		01011		1 198		4.971	6.263
<b>13.16 2.12 0.430 0</b>	0.410	0	.046	0.095	1.1180		121 1		5.850	6.753
44.32 2.08 0.460 0.	0.460 0.	0.	113	0.086	24/1-1		1.121	1.217	6.694	6.945
47.80 1.96 0.050 0.	0.050 0.		501	0.000	440C 1		1.142	1.187	6.318	5.500
44.66 2.05 0.420 U.	0.420		110	200.0	1.3000		1.245		5.257	5.709
44.13 C.U3 C.U3				000 0	1 3284		1.379	1.372	5.704	5.459
42.30 2.13 0.110 0.	0.110		120	0.090	1 4384		1.361	1.363	5.832	5.935
24.64 2.87 0.220 0.	0.220	5 c	020	0.080	1.4015		- 1.369	1.355	5.145	5.300
	0.140	0	040	0.065	1.5359		1.341	1.315	1 003	
AK SE 1.97 0.180 0.	0.180 0.	0	056	1.120	1.6366		1.462			
7 40 0 400 0.	0.400 0.	0	103	1.110	1.7423		1.303		3.850	4.093
46.81 2.07 0.200 0.	0.200 0.	0	060	1.680	2.3774		1 152		4.768	5.144
47.17 1.91 0.700 0.	0.700	00	120	0.075	1 4142		1.081		5.308	
37.69 2.32 U.U5U 0 2 74 2 02 0.420 0	0.420	00	020.	0.123	1.3093		1.012		5.018	5.675
		c	011	0 088	1 2813		0.980		5.383	5.685
	0 400	00	060	0.102	1.1744		0.979		5.310	A.8/5
42.36 2.08 0.330 0	0.330	0	.700	0.085	1.4261		0.989		207 S	5.604
42.23 2.12 0.400 0	0.400 0	0	.100	0.085	1.4686		0.907	0.824		
	000 0	C	000	0 118	1.8451		0.937		5.535	
49.97 I.U. 0.000		; c	990	0.001	1 3801		0.929	0.863	5.341	0.040
46.64 Z.Ub U.ZUU U.ZUU U.ZUU	0.200	ic	100	0.083	1.1978		0.941	0.858	5.219	5.485
42.24 2.0C 0 200		ic	000	0.097	1.4142	2.23	0.969	0.865	6.083	
48.32 1.96 0.200	0.230	5c	076	0.076	2.0701	1.50	1.007	0.943	5.590	
2 22 22 22 22 22 22 22 22 22 22 22 22 2			010	1 367	1 1972		1.062	066.0	5.293	5.357
41.72 2.19 0.100	0.100		010.0	100.0	2 0622		0.834	0.801	7.188	
47.59 1.94 0.000	0.000		010 0	C. 540	1.2566		0.719	0.683	7.233	8.333
		, _	000	0.067	1.5683	2.77	0.862	001 0	0.6/ .6	106. C
40 2F 2.20 0.660	0.660		0.180	0.217	2.2856	3.08	0.823	0.793	0.200	1.160

20<u>0</u>

e te	Porosity	Oensity	Penetrometer	Torvane	Mean Diameter	Sorting Coefficient	Resistivity	Fing Corrected	ler Observed	Corrected	rker Observed
5	46.38	1.97	0.5:0	0.100	0.081	1.1704		0.840		6.520	
9	49.57	1.87	0.200	0.100	0.096	1.1244		0.815	0.799	5.933	
~	33.33	2.61	0.000	0.000	0.330	1.8150		0.881	0.866	5.965	
-	35.77	2.49	0.000	0.000	0.154	1.9570		0.835		5.818	
~	39.87	2.22	0.000	0.000	0.133	1.2759			•		
-	43.08	2.17	0.330	0.086	0.233	2.0054	3.44				
-	47.17	1.99	0.250	0.040	0.061	1.7321	3.00				
5	50.52	1.86	0.000	0.020	0.357	1.8257					
5								2.378		5.500	
0	21.14	2.93	1.000	QN	0.620	1.3693	4.41	2.423		5.557	
1								2.331		5.640	5.714
~	46.89	1.94	0.650	0.100	0.353	1.5811	3.76	23.94		6.800	6.086
-	50.69	1.85	0.200	0.060	0.123	1.9695	3.88	2.188	2.064	6.814	
-	45.80	1.91	0.420	0.050	0.713	4.2517	3.88	2.587		6.200	
	34.85	2.31	0.550	0.100	0.499	3.1192	4.26	2.634		5.483	
								2.321		4.975 8 775	
	45.83	1.97	0.000	0.000	0.160	1.3744	4.88	1.900		7.350	
-	50.15	1.96	0.200	0.050	0.130	1.3693	4.13	2.409		7.433	
_	51.43	1.78	0.000	0.000	0.043	2.4121	4.00	2.631		8.950	8.267
~	46.78	1.91	0.500	0.100	0.311	3.0064	3.65	2.417		7.975	
-	49.88	1.88	0.500	0.030	0.871	3.4473	4.00	2.424		7.275	
-	46.98	1.91	0.650	QN	4.870	6.6771	4.00	2.360		6.975	
	36.99	2.31	QN	QN	0.178	1.2247	5.45	2.156		4.875	
-	33.82	2.33	0.100	0.020	0.285	2.0736	4.41				
-	47.06	2.01	CN	ND	0.093	1.2813	4.88				
~	49.62	1.90	0.500	0.050	0.490	2.4403	4.00				
_	46.09	2.03	0.500	0.050	0.056	1.8081	4.13				
~	44.81	1.96	0.100	0.030	0.340	1.5652	3.76				
~	46.89	1.93	0.100	0.020	0.145	1.6514	(00.6)				
_	41.49	2.24	0.100	0.050	3.730	8.0623	3.08				
~	42.06	2.08	0.000	0.000	0.717	2.5641	2.64				
~	37.12	1.75	IN	NN	0.1/4	10007-7					

# APPENDIX A.6 SEDIMENT ANALYSES

Preceding page blank

	Sorting Coefficient (So)	1.1408 Q09 0.1317 Ln	1.547 00g .1438 Ln	1.3964 00g .3339 Ln	1.3031 Qbg .2647 Ln	1.3771 009 .3200 Ln	1.2030 709 .1849 Ln	
	Grain Size Distribution (mm)	0.09 md 0.095 0.45 0.073 0.5 0.011 00a	0.086 md 0.096 075 .072 025 .012 008	0.066 md .078 075 .040 025 .019 008	0.072 md .090 075 .053 025 .018 00a	0.081 md .11 075 .058 025 .026 00a	C.092 md 10 075 .016 025 .017 008	
	Engineering Properties	0.080 T= 0.30 P <sup>+</sup>	0.080 T 0.30 P	- 110 T - 56 P	- 076 T	.086 T .46 P	.043 P	Clay
	Coef. of Iniform. (Cu)	2.25	2.81	3.30	3.07	10.00	3.57	9
-	Size Analysis ( (*)	0.03 6 <sup>1</sup> 0.04 CS <sup>2</sup> 0.36 MS <sup>3</sup> 71.81 FS <sup>6</sup> 24.05 S <sup>5</sup> 3.7: C <sup>6</sup>	0.02 C5 19 MS 70.47 FS 24.50 S			5.84 C 7.32 G .18 CS .48 MS 61.72 FS 28.01 S	7.29 C .09 G .07 CS .07 CS .19.06 S 19.06 S	<sup>5</sup> Silt
	Porosity ¢(%)	42.53	44.63	45.41	42.28	46.78	37.89	Sand
	Void Patio (e)	0.751	.806	.832	.795	879	.610	Fine
	Water Content W(3)	36.0	38.2	37.7	38. 38	37.5	1.16	4
	Spec. Grav. (G <sub>s</sub> )	2.68	2.66	2.68	2.59	2.62	2.66	um Sanc
	Ave Wet							Mediu
(Wet)	mterval Wet gm/cm)	2.15	2.09	2.07	2.04	1.97	2.15	
Density	Sample Interval (cm)	0-60.4	0-43.8	0-63.2	0-65.1	0-61.7	0.62.2	se Sand
	Vol. (cc)	3337.7	2420.4	3492.4	3597.4	3409.5	3437.2	<sup>2</sup> Coar
	Wt. (gm)	7168.8	5049.1	7237.8	734.3	6706.9	7384.4	
	Site No	23	24	25	26	R.	6£	Grave

<sup>†</sup>Penetrometer (kg/cm<sup>2</sup>) \*Torvane (kg/cm<sup>2</sup>)

\*\* mean diameter

equals NOTE: T-M M-B T-B

Top-Middle Middle-Bottom Top-Bottom

Sorting	Coefficient (So)	1.1180 Q09 .1116 Ln	1.1734 Qbg .1599 Ln	1.3838 Q0g .3248 Ln	1.2910 QDg .2554 Ln	1.1180 009 .1116 Ln	1.1748 Q09 .1611 Ln
Grain Size	Distribution (mm)	0.089 md .10 075 .080 025 .010 00a	.074 md .095 075 .069 025 .013 00a	0.069 md .090 075 .047 025 .021 00a	.083 md .10 075 .060 025 .020 00a	.095 md .10 0,5 .060 0,5 .010 00a	.086 md .098 07 5 .071 02 5 .013 00a
Engineering	Properties	.28 P	.076 T .32 P	- 09 - 09 - 09	. 30 P	.046 T 41 P	.113 T .46 P
Coef. of	Uniform. (c <sub>u</sub> )	3.59	3.33	2.86	3.41	1.90	2.32
Size	Analysis (%)	.46 G .18 CS .45 MS 75.89 FS 17 47 S 5.55 C	20 6 .06 CS .08 CS .03 MS 691 FS 23.11 S 6.39 C	.04 6 .03 CS .03 CS .29 MS 51.02 FS 43.68 S	.01 6 .01 6 .01 CS .01	.01 G .00 CS .24 MS 78.70 FS 18.06 S 2.99 C	15 6 15 CS 15 CS 99 MS 71.98 FS 22.16 S 4.57 C
Porosity	¢(2)	45.24	42.10	46.49	<b>41</b> .66	43.18 8	44.32
Void	Ratio (e)	.826	121.	. 869	.807	.760	.796
Water	Content W(%)	36.4	33.5	37.8	37.7	37.2	36.2
Spec.	Grav.	2.67	2.66	2.66	2.66	2.66	2.68
Ave	Het						
(Net)	Wet (gm/cm)	2.05	2.11	2.01	2.08	2.12	2.08
Sample	Interval (cm)	0.65.6	0-60.9	0-65.0	0-62.9	0-63.6	0-64.0
alo	Vol. (cc)	3625.1	3365.3	3591.9	3475.9	3514.5	3536.6
C.am	Wt. (9m)	7436.2	111.4	7228.0	7213.7	7442.2	7352.0
	Site	40	41	42	43	44	45

Sorting Coefficient (So)	1.5664 009 .4475 Ln	1.3608 QDg .3081 Ln	1.3000 009 .2624 Ln	1.3284 00g .2840 Ln	1.4384 QDg .3635 Ln	1.4015 QDg .3376 Ln
Grain Size Distribution (mm)	.066 md .093 075 .038 025 .027 00a	.078 md .10 075 .054 025	.092 mJ .12 075 .071 025 .024 00a	.090 md .12 075 .068 025 .026 00a	.084 md .11 075 .058 025 .051 00a	.080 md .11 075 .056 025 .027 QCa
Engineering Properties	.50 P	1 II. 4 2 P.	- 10 - 35 - 35	- d [[].	- 13 - 22.	. 33 P
Coef. of Uniform. (C_)	00°5	4.75	3.55	4.35	3.48	5.55
Size Analysis (%)	.14 G .34 CS .72 MS 47.47 FS 42.63 S	8.70 C .41 G .11 CS .83 MS 62.48 FS	6.16 C 1.08 G .32 CS .50 MS 69.41 FS	5.65 C .29 G .37 CS .37 CS .51 MS 69.67 FS	5.88 C 3.22 5 .17 CS .43 MS 62.35 FS	5.77 C 147 G .14 CS .55 MS 61.42 FS 29.89 S 7.53 C
Porosity ¢(%)	47.80	44.66	44.13	42.30	24.64	47.70
Void Ratio (e)	.916	.807	. 064.	. /33	.327	512.
Water Content W(T)	36.3	35.4	33.1	35.1	38° 19	38.9
Spec. Grav. (G, )	2.69	2.66	2.67	2.67	2.68	2.61
Ave						
Interval Wet (gm/cm)	1.96	2.05	2.03	2.13	2.87	1.94
Density Sample Interval (cm)	0-65.4	0-63.3	0-61.7	0-62.0	0-27.9	0-54.5
ple Vol. (cc)	3614.0	2.2666	3409.5	3426.1	1541.7	7.110E
Mt. (gm)	7096.5	6835.1	6935.7	7285.6	4425.0	5848.5
No	46	47	48	-	20	- 15

Sorting Coefficient (50)	1.5359 QD9 .4291 Ln	1.6366 QDg .4926 Ln	1.7423 QD9 .5552 Ln	2.3774 QD9 .8660 Ln		1.8787 009 .6306 Ln	1.6330 QDg .4904 Ln	
Grain Size Distribution (mm)	.065 md .092 0.5 .039 0.2 .026 00a	1.12 md .15 075 .056 025 .047 00a	1.11 md .17 075 .056 025 .057 00a	1.68 md .26 075 .046 025 .107 00a		.074 md .12 075 .034 025 .043 0Da	.093 md .12 0,5 .045 0,5 .037 0Da	
Engineering Prooerties	.14 PT	.056 T .18 P	- 103 T 40 P	- 09 T - 20 P		.70 P	.06 T .32 P	
Coef. of Uniform. (C_)	15 40	5.80	30.00	29.17		45.0	20.00	
Size Analysis (%)	24 6 .16 CS .16 CS .64 MS 41.43 FS 47.73 S	1.74 MS 1.74 MS 64.30 FS 26.56 S		506 5.10 C5 20.91 MS 39.21 F5 25.28 S 25.28 S		4.22 MS 41.95 FS 41.06 S 12.27 C	4.31 G 1.01 CS 7.23 MS 43.89 F5 33.89 S 9.89 C	
Porosity \$(%)	47.17	46.55	47.48	46.81	45.50	47.17	48.85	
Void Ratio (e)	. 893	.871	. 904	.880	.835	.893	.955	
Water Content K(%)	37.9	38.9	3.81	38.1	37.4	34.0	36.1	
Spec. Grav. (Gs)	2.64	2.59	2.71	2.75	2.63	2.64	2.63	-
Ave Net								1 90
(Wet) Interval Wet (gm/cm)	1.97	1.97	2.01	2.07		1.91	1.88	
Density Sample Interval (cm)	0-47.9	0-42.2	0-55.6	0-62.3	0-64.4	0-37.4	37.4-64.4	
nple Vol. (cc)	2646.7	2331.9	3072.5	3442.7	3558.7	2066.7	1492.0	
5an Wt. (9m)	5214.5	4602.7	6178.3	7137.8	7068.9	3947.4	2805.0	
Site	52	21	58	59	60	dol	Bottom	-9

Sorting Coefficient (So)	1.4142 009 .3466 Ln	1.3093 QDg .2695 Ln	1.2813 009 .2479 Ln	1.1744 00g .1609 Ln	1.4261 QD9 .3550 Ln	1.4686 QDg .3843 Ln
Grain Size Distribution (mm)	.075 md .096 075 .048 025 .024 QDa	.123 md .12 075 .070 025 .025 00a	.088 md .11 0,5 .067 0,25 .022 00a	.102 md .12 075 .087 025 .017 00a	.085 md .12 075 .059 025 .031 QDa	.085 md .11 075 .051 025 .030 00a
Engineering Properties	.50 P	-03 + 42 P	.55 P	- 09 - d 07	- 70 - 1 	00 - 0
Coef. of Uniform. (C,)	18.40	7.33	11.29	2.16	6.71	3.96
5ize Analysis		73.17 FS 18.02 5		95 6 95 6 38 CS 2.02 MS 79.72 F5 13.25 5		2.73 MS 2.73 MS 2.73 MS 57.53 FS 31.64 5 6.48 C
Pcrosity ¢(%)	37.69	45.74	42.95	42.92	43.76	42.23
Void Ratic (e)	.605	. 843	.753	.752	.178	.731
Water Content W(%)	36.4	36.7	35.9	37.7	37.4	35.1
Spec. Grav. (G,)	2.66	2.66	2.65	2.63	2.67	2.65
Ave						
(Wet) Interval Wet (gm/cm)	2.32	2.02	2.11	2.12	2.08	2.12
Density Sample Interval (cm)	0-65.2	0-62.9	0-62.3	0-40.2	0-60.9	0-51.1
vol. (cc)	3029.9	3475.8	3442.7	2221.4	3365.3	2823.8
San Wt. (9m)	7033.4	7039.4	7258.6	4714.8	6989.1	1.5993.1
No	61	62	3	64	65	99

Sorting Coefficient (So)	1.8451 Q09 .6125 Ln	1.3601 QDg .3222 Ln		1.2247 Q09 .2027 Ln		1.1255 Q09 .1182 Ln	1010 000	.1805 Ln
Grain Size Distribution (mm)	.118 md .16 075 .047 025 .056 00a	.0 <b>9</b> 1 md .12 9, 5 .063 0 <sub>2 5</sub> .028 00 <b>a</b>		.096 md .12 075 .080 025 .020 00a	.096 md .12 Q., .080 Q25	.163 md .19 0,5 .15 0,25 .020 QDa		.083 md .099 075 .015 00a .015 00a
Engineering Properties	.00 ₽ 000.	.20 P		.10 T .35 P	1.00 T	.30 T 2.50 P		- d - 30 
Coef. of Uniform. (C_)	26.19	7.14		3.44	33.02	4.09		5.47
Size alysis (%)	9.01 G 3.73 CS 5.47 MS 45.79 FS 25.67 S	49 6 .88 CS .88 CS .2.57 MS 73.15 FS 15.57 S 7.34 C		.16 6 .41 CS 3.04 MS 72.96 FS 172.96 FS 5.59 C	43.84 6 3.10 CS 7.45 MS 31.37 FS 10.30 S 5.59 C	1.43 G .84 CS 3.96 MS 81.80 FS 6.36 S 5.61 C		54 6. .36 CS .36 CS 1.90 MS 690 FS 6.90 C
Porosity ¢(%)	49.97	46.64		47.40	44.69	39.09		45.24
Void Ratio (e)	666 .	.874		106.	808.	.642		. 826
Water Content W(%)	34.3	0.65		35.7	30.3	24.0		34.8
Spec. Grav. (G,)	2.67	2.65	2.66	2.63	2.68	2.69		2.67
Ave							1.94 2.03 1.99	
(Wet) Net (gm/cm)	1.91	2.02		1.89	1.99	2.08		2.02
Density Sample Interval (cm)	0-13.8	0-45.2	0-92.6	0-9.2	9.7-79.7	79.7-90.2		0-62.6
ple Vol. (cc)	762.6	2497.7	3779.4	406.4	2933.0	439.9		3459.3
Sam Wt. (gm)	1454.3	5042.0	8222.0	768.1	5807.3	915.0		6983.0
Site	68	69	169	dol	<b>SIDDIM</b>	mottoß	N CO C	70

Sorting Coefficient (So)	1.4142 Q09 3466 Ln	2.0701 00g .7276 Ln	1.1972 QD9 1.0942 Ln	2.0191 009 .7027 Ln	3.4157 0Dg 1.2284 Ln	2.0622 QD9 .7238 Ln
Grain Size Distribution (mm)	.027 00a .11 075 .055 025 .027 00a	.076 md .12 075 .028 025 .046 00a	.357 md .43 075 .30 025 .065 00a	.438 md 53 075 .13 025	. 736 md . 98 0,5 . 084 0,5 . 448 00a	.227 md .37 075 .087 025 .141 00a
Engineering Properties	.20 P	.23 P	.01 T .10 P	10 T	.10 T 1.00 P	- a 0.0
Coef. of Uniform. (C_)	2.58	21.13	2.66	20.00	10.30	6.75
Size Analysis (±)	12 6 70.86 FS 22.55 5	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1.78 C 1.	16.44 G 10.06 CS 38.63 MS 15.49 FS 14.28 S	26.90 G 25.37 FS 25.37 FS 25.37 FS 25.37 FS 25.37 FS 25.37 FS 25.37 FS	.67 6 9.05 CS 33.66 MS 35.49 FS 16.49 5 4.64 C
Porosity e(X)	48.32	46.12	47.72	48.34	19 <i>.1</i> E	£7.59
Votd Ratio (e)	SEQ.	.856	Jtr.	986.	909	906,
Water Content W(T)	39.1	33.6	4.16	34.8	1.65	35.6
Spec. Grav.	2.66	2.66	2.66	2.66 2.68	2.65	2.66
a ti						2.00
(det) Interval Wet (on/on)	1.96	1.94	2.19	1.92	2.08	1.94
Sample Interval (cm)	0-63.8	0-63.2	0-54.5	0-104.0	87.0-104.0	0-06.3
Ter Vol.	625.6	492.4	5011.7	4357.6 3645.3	712.3	348.1
Samp Samp	6925.6	6765.3	6598.0	0.999.0	1481.6	674.2
Site	и	24	8	730 Jan	aottoa	81

-	-		Annual and a second second						
Sortino	Coefficient (So)	1.2566 Q09 .2284 Ln	1.5683 QD9 .4450 Ln	2.2856 (09 .8266 Ln	1.1704 Q09 .1574 Ln		1.1244 QG9 .1173 Ln	2.9580 QDg 1.0845 Ln	
Grain Size	Distribution (mm)	.54 md .60 0,5 .38 0,25 .110 00a	.067 md .091 Q <sub>75</sub> .037 Q <sub>25</sub> .027 Q0a	.217 md .35 Q <sub>75</sub> .067 Q <sub>25</sub> .141 QOa	.081 md .10 075 .073 025 .014 00a		.096 md .11 075 .087 025 .011 QDa	.530 md .35 0,5 .04 0,2 .155 00a	
Fnaineerina	Properties	.01 T .10 P	- d 00.	. 18 . 66 P	- 10 - 50 - 7 - 7 - 7		.20 P	.10 T .60 P	
Coef. of	Uniform. (c_)	1.88	33.33	7.10	14.00		1.77	31.58	
Size	Analysis (%)	11.60 6 32.18 CS 47.88 MS 4.65 FS 2.56 S		2116 7.50 CS 33.21 MS 27.39 FS 23.88 S 23.88 S	22.60 S 22.58 MS 67.07 FS 22.60 S 8.47 C		.13 MS 1.37 MS 81.56 FS 13.47 S 3.45 C	17.56 6 5.06 CS 4.06 MS 25.40 FS 36.38 S 36.38 S	
Porosity	\$(%)	46.29	52.17	40.26	46.38		49.57	47.94	
Void	Ratio (e)	. 862	160.1	.674	.865		.983	126.	
Mater	Content W(*)	29.8	39.4	32.2	38.1		37.2	34.8	-
Spec.	Grav.	2.67	2.65	2.71	2.60	2.65	2.63	2.66	
Ave	Wet								1.89
y (Wet)	Wet (om/cm)	1.91	1.81	2.20	1.97		1.87	1.91	
Densit Samole	Interval (cm)	0-32.6	0-04.5	0-54.2	0-63.7	0-52.2	0-36.7	36.7-52.2	
ole	Vol. (cc)	1801.5	193.41	2995.1	3520.1	2884.6	2028.0	856.5	
Sam	Wt. (9m)	3446.5	450.7	6594.7	6934.0	5792.8	3972.4	1635.9	
	No No	82	83	<b>7</b> 0	82	86	top	Bottom	-8

	ž	80	8	26	6	3.	954	doj	Bottom	2
3	<b>3</b>	1432.5	1511.3	1107.3	7674.0	6557.7	4131.8	2230.3	1747.3	
mple	(cc)	547.7	607.9	4973.4	1.1636	3299.0	2138.6	1.9911	939.4	-
Sample	Interval (cm)	0-10.4	0-11-0	0-60-0	0-63.9	0-59.7	0-38.7	0-21.7	21.7-38.7	
ty (Met) Interval	Wet (gm/cm)	2.61	2.49	2.22	11.2	1.99		1.86	1.86	
Ave	ž									1.86
Spec.	Grav. (6,)	2.72	2.70	2.66	2.69	2.64	2.63	2.64	2.63	
Mater	Content W(1)	s. V	39.5	35.4	38.3	0.95		38.9	38.2	
Votd	Ratio (e)	.500	195.	-663	.757.	. 893		1.021	366 .	
Paras ( + u	\$(x)	33.33	35.77	39.87	43.08	11.12		50.52	49.95	
	Analysis (T)	4.47 G 6.12 CS 6.12 CS 89.10 FS 14.14 S	1.20 CS 1.20 CS 6.11 MS 6.11 MS 24.13 CS 24.13 CS 25.13 C	64.96 MS		6.28 MS 57.22 S	9.45 C	2.40 G 4.36 CS 11.55 M5 25.68 FS 43.74 S 12.27 C	22.48 G 111.38 CS 111.49 MS 13.27 FS 32.71 S 32.71 S	1
Case of	Uniform.	18.33	34.15	1.89	5.60	15.00		62.14	50.66	t
Continue of the	Properties	8.8	4 00.	800	. 33 P	.25 P		T 20. 100 P	.07 T .25 P	
	Distribution (mm)	.330 md 28 011 .085 011 .097 00a	.154 md .18 04. .047 53. .066 00a	.133 md .14 0.1 .086 0.1 .027 000	531 M EE2.	061 md 1000 051 1000 050 1000 050		.357 md .11 075 .033 025 .038 008	.548 md .80 0,5 .045 0,5 .377 00a	
	Sorting Coefficient (So)	1.8150 Q09 .5961 Ln	1.9570 00g .6714 Ln	1.2759 00g .2436 Ln	2.0054 00g .6959 Ln	1.7321 009 .5493 Ln		1.8257 00g .6020 Ln	4.2154 QDg 1.4390 Ln	T
	sorting Coefficient (So)	1.8752 QDg .6292 Ln	1.3693 (D9 .3143 Ln		1.5811 Q09 .4581 Ln	1.7014 Q09 .5314 Ln		1.9695 009 .6768 Ln	2.0182 QD9 .7458 Ln	
-----------	------------------------------------	---	---	--------	---	--	-------	--	---	
	orain Size Distribution (mm)	.058 md .088 0,5 .025 0,5 .031 00a	.62 md .75 0.75 .40 0.25 .175 0.08		.353 md .40 075 .16 025 .120 00a	.085 md .11 0,5 .038 025 .036 00a		.123 md .12 075 .031 025 .044 00a	.055 md .080 Q <sub>75</sub> .018 Q <sub>25</sub> .031 QDa	
	Properties		1.0 P		.10 T .65 P	.05 T .30 P		.20 P	.75 P	
	Uniform. (c,)	46.25	1.93		9.43	3.39		23.16		
64-5	Analysis (%)	.11 G .60 CS 7.22 MS 25.58 FS 51.99 S	17.67 G 37.89 CS 39.65 MS 3.93 FS .86 S		14.19 6 3.67 CS 44.12 MS 21.12 FS 13.03 S 3.87 C	.52 G 1.21 CS 7.71 MS 33.68 FS 50.04 S 6.84 C		3.12 G 4.32 C 9.49 MS 27.86 FS 44.51 S	27.92 S 21.78 FS 57.92 S 15.34 C	
Demositer	¢(\$)	45.71	21.14		46.89	44.54		50.69	59.15	
P.P.C.N	Ratio (e)	.842	. 268		. 883	.803		1.028	1.448	
un tell	Content W(%)	39.1	33.3		32.6	29.9		39.3	40.0	
Case	Grav.	2.64	2.72	2.68	2.68	2.69	-	2.63	2.65	
	Wet						1.97			
/ (Het)	Wet (gm/cm)	2.05	2.93		1.94	1.99		1.85	1.55	
Density	Interval (cm)	0-05.1	0-34.1	0-39.9	0-16.0	16.0-39.9		0-51.2	0-38.1	
010	Vol. (cc)	281.8	1884.4	204.9	884.2	.320.7		829.3	.596.4	
C ame	Wt. (gm)	576.9	5524.8	4580.0	1715.3	2628.2 1		5220.8	2990.0	
	Site	958	100	102	qoT	mosto8	1-8-1	103	VEOI	

Contino	Coefficient (So)		4.2517 GD9 1.4473 Ln	2.3570 009 .8574 Ln		3.1192 Qbg 1.1376 Ln		1.3744 QDg .3180 Ln	2.5820 QDg .9486 Ln	
Grain Size	Distribution (mm)		.713 md .94 075 .052 025 .444 002	.115 md .15 0.5 .027 925 .061 008		.499 md .72 075 .074 025 .233 00a		.160 md .17 075 .090 025	.603 md .80 075 .120 025 .34D 00a	
Fnoineering	Properties		. 05 . 42 P	.05 T .25 P		. 10 - 55 - P		F d 000.	⊢d 000.	
Coef. of	Uniform. (c_u)		51.06	34.78		28.66		1.86	5.93	
Size	Analysis (%)		25.58 G 13.83 CS 14.33 MS 13.30 FS 25.16 S 8.70 C	1.75 G 5.54 CS 11.41 MS 26.42 FS 42.59 S 12.29 C		18.65 G 16.31 C5 24.19 MS 14.55 F5 19.37 5 6.93 C		3.45 G 5.03 C 115.94 MS 58.63 FS 14.41 S 2.50 C	21.89 G 21.25 CS 21.79 MS 27.21 F5 7.14 5	
Porosity	\$(%)		45.80	44.54		34.85		45.83	44.75	
Void	Ratio (e)		. 845	. 803		.535	-	.846	.810	-
Water	Content W(%)		32.3	30.6		7.1E		31.6	29.9	
Spec.	Grav. (G <sub>S</sub> )	2.61	2.59	2.62		2.62	2.70	2.70	2.70	
Ave	Het				1.93				-	1.98
(Het) Interval	Wet (gm/cm)		1.91	1.95		2.31		1.97	1.99	
Sample	Interval (cm)	0-63.0	0-35.0	35.0-63.0		0-41.2	0-54.3	ſ−39.3	39.3-54.3	
ple	Vol. (cc)	3481.4	1934.1	1547.3		2276.7	3.0005	2171.7	828.9	
Sam	Ht. (9m)	7056.1	3694.1	3017.2		5272.5	6221.1	4278.3	1649.5	
	NO	104	dol	Bottom	-8-1	105	601	dol	motto8	8

 Sa	mple	Sample	y (tet) Interval	Ave	Spec.	Water	Void	Porosity	Size Analveis	Coef. of Uniform.	Engineering Properties	Grain Size Distribution	Sorting Coefficient
.tw (me)	(cc)	Interval (cm)	(m/cm)	2	(6 <sub>S</sub> )	H(2)	(e)	\$(%)	(%)	(c <sup>n</sup> )		( um )	(So)
3346.0 1786.2	1520.9 879.9	0-36.3 0-21.0	2.03		2.72	32.5 28.0	. 754	42.99	9.22 G 9.22 G 23.62 MS 39.36 FS 39.36 FS 39.56 S	4.33	F 900.	.268 md .40 075 .090 025 .155 00a	2.1082 Q'9 .7458 Ln
1352.7	641.1	21.0-36.3	2.11		2.71	23.0	.616	38.12	.61 6 1.62 CS 8.05 MS 64.50 FS 21.72 S 3.50 C	1.81	T 01.	.103 md .12 0,5 .018 025 .051 008	2.5820 009 .9486 Ln
5550.0	2829.3	J-51.2	1.96	2.07	2.64	45.4	1.006	50.15	1.32 G 2.45 CS 12.60 MS 60.81 FS 19.02 S	2.90	.26 P	.130 md .15 075 .080 025 .0350 Q0a	1.3693 (09 .3143 Ln
2335.1	1309.7	0-23.7	1.78		2.57	39.4	1.059	51.43	1.16 CS 1.16 CS 4.33 MS 65.38 S 65.38 S		- d 00.	.043 md .064 Q.5 .011 Q25 .026 QDa	2.4121 QD9 .8805 Ln
4686.7	2304.3 1420.2	0-41.7	16.1		2.62 2.62	33.7	.879	46.78	12.39 6 12.30 CS 15.65 MS 23.47 FS 28.92 S 7.27 C	23.33	.10 50 P	.311 md .47 075 .052 025 .209 00a	3.0064 Q09 1.1007 Ln
	884.2	25.7-41.7	1.92		2.62	32.6	.853	46.03	2.64 G 9.43 CS 14.72 MS 33.10 FS 33.19 S 6.92 C	6.25	.03 T	.187 md .25 0,5 .050 025 .100 00a	2.2361 Q09 .8047 Ln
				1.91									

	Sorting Coefficient (So)		2.4473 009 1.2376 Ln	2.4852 QDg .9104 Ln		6.6771 QDg 1.8987 Ln	2.0272 Q09 .7067 Ln	1.2247 00g .2027 Ln
	Grain Size Distribution (mm)		.871 md .82 075 .069 025 .376 008	.554 md .42 075 .068 025 .176 008		4.87 md 10.7 Q75 5.23 QD8	.218 md .30 07 s .073 02 s .113 00a	.178 md .18 0,5 .120 025 .0300 00a
	Properties		.03 05 05	.03 T		. 10 P	.05 T .10 P	
	Lutform. (c_)		30.00	11.67		105.45	8.25	1.73
	Analysis (%)		24.67 6 6.85 CS 6.85 CS 12.84 MS 27.87 FS 20.53 S 7.24 C	17.35 G 4.41 CS 13.14 MS 31.71 FS 26.05 S 7.34 C		58.99 G 6.98 CS 7.02 MS 14.70 FS 8.60 S 3.71 C	4.29 G 11.19 CS 16.97 MS 41.88 FS 19.76 S 5.91 C	5.25 G 3.55 CS 9.95 MS 72.69 FS 6.45 S 2.11 C
	¢(%)		49.88	46.98		46.98	45.15	36.99
	Ratio (e)		566.	. 886		. 886	.823	.587
	Content V(X)		35.9	33.1		33.7	31.3	36.0
	Grav.	2.63	2.63	2.63	2.63	2.63	2.63	2.63
-	Net				1.90			1.93
(liet)	Wet (gm/cm)		1.88	16.1		1.91	1.94	2.31
Density	Interval (cm)	0-59.7	0-30.0	30.0-59.7	0-51.7	0-23.0	23.0-51.7	0-06.0
-1-	Vol. (cc)	3299.0	1657.8	1641.2	2856.9	1271.0	1585.9	331.56
	Wt. (gm)	6399.4	3116.7	3134.7	5850.0	2427.6	3076.6	765.9
	No	13	tob	W03309	14	da		8 <b>1</b>

	Sorting Coefficient (So)	1.9493 009 .6675 Ln	2.0736 Q09 .7293 Ln	1.2813 QD9 .2479 Ln		2.4403 009 .8921 Ln	.8906 009 1159 Ln	1.8081 Q09 .5923 Ln
-	Grain Size Oistribution (mm)	.234 md .23 0,5 .100 025 .1400 00a	.285 md .43 , 0, s .1 , 0, s .1 , 0, s .165 ,00a	.093 md .11 0,5 .067 2,25 .021 00a		.490 md .530 075 .089 025 .220 00a	.090 md .115 Q75 .089 Q25 .035 QDa	.056 md .085 075 .026 025 .029 00a
	Engineering Properties	► d 000.	.02 T 0.1 P			.50 P	F d 000.	- 02 0.5 0.5
	Coef. of Uniform. (C_)	3.57	4.37	2.83		4.42	15.00	22.66
	Size Analysis (%)	3.56 6 10.32 CS 27.11 MS 46.46 FS 10.53 S 2.02 C	1.43 G 13.04 CS 40.17 NS 37.12 FS 7.41 S			18.79 6 7.00 CS 11.55 MS 48.18 FS 9.48 S 5.00 C	8.29 G 1.52 CS 2.41 MS 41.95 FS 37.26 S 8.57 C	.16 6 .12 CS 1.08 MS 30.00 FS 53.73 S 14.91 C
	Porosity ¢(%)	39.61	33.82	47.06		49.62	46.00	46.09
	Void Ratio (e)	.656		688.		.985	.852	. 855
	Water Content W(%)	37.7	26.3	9.98 9.		36.7	31.7	38.6
	Spec. Grav. (G <sub>s</sub> )	2.58	2.71	2.67	2.69	2.69	2.69	2.64
	Ave Wet							1.93
Thet)	interval Wet (gm/cm)	2.28	2.33	2.01		1.90	1.96	2.03
Density	Sample Interval (cm)	0-47.2	0-56.8	0-14.9	0-22.4	0-15.0	15.0-22.4	0-54.9
-	Ple Vol. (cc)	2608.3	3138.8	823.37	1237.82	828.92	408.9	3033.8
	Wt. (9m)	5941.7	7306.3	1653.6	2500.4	1574.9	801.4	6147.3
1	No	1)	2)	61	20	dol	mostoa	21 21

	Sorting Coefficient (So)	1.7002 009 .5307 Ln		1.5652 009 .4480 Ln	2.7386 009 1.0074 Ln			1.6514 Q09 .5016 Ln	6.2634 QD9 1.8347 Ln	3.2489 009	1.1/63 LM
	Grain Size Distribution (mm.)	.144 md .185 075 .064 025 .061 0Da		.340 md .49 075 .2 025 .145 00a	.132 md 1.5 075 .20 625 .650 00a			.145 md .18 075 .066 025 .057 QDa	3.9 md 5.1 075 .13 025 2.485 00a	.133 md	.19 075 .018 025 .086 008
	Engineering Properties	.036 T .18 P		r a BO Cri	. 25 P			.02 T .10 P	.01 T .75 P		
	Coef. of Uniform. (c <sub>u</sub> )	5.60		<b>a</b> 89.	7.38			3.53	38.00		
	Size Analysis (%)	10.23 G 1.55 CS S.20 MS 49.72 FS 26.60 S		4.02 G 21.72 CS 45.14 MS 20.51 FS 7.20 S 1.41 C	31.68 6 15.89 CS 23.46 MS 18.26 FS 8.47 S 2.24 C			2.80 G 6.08 CS 10.05 MS 49.75 FS 26.45 S 4.87 C	50.29 6 8.51 CS 8.51 CS 8.08 MS 19.16 FS 11.04 S 2.92 C	6.60 6	47.32 FS 18.25 S 15.86 C
	porosity \$(%)	40.83		44.81	41.79			46.89	40.08	46.95	
	Void Ratio (e)	169.		.812	.718	-0		. 883	.669	.885	- an advanta
	Water Content W(T)	39.7		30.8	27.0			32.0	25.6	37.0	
	Spec. Grav. (G <sub>S</sub> )	2.72	2.63	2.65	2.63		2.62	2.61	2.62	27.2	
	Ave					1.98				1.98	
y (Het)	Interval Wet (gm/cm)	2.31		1.96	2.00			1.93	2.02	2.03	
Densit	Jampie Interval (cm)	0-31.4	0-55.8	0-40.8	40.8-55.8		0-52.7	0-39.7	39.7-52.7	0-34.3	
	ple Vol. (cc)	1315.7	3083.5	2254.6	828.9		2312.2	2193.8	718.4	437.2	
	Wt. (9m)	3050.0	6251.1	4419.0	1660.5		5905.6	4224.3	1451.0	2924.6	
	Site	121V	122	dol	motto8		123	Lop	mostoß	NE2I	

Sorting Coefficient (So)	1.2089 Q09 .1897 Ln	8.0623 (09 2.0872 Ln	2.5641 QDg .9416 Ln	2.2650 QDg .8608 Ln	1.2961 QD9 .2594 Ln	1.3009 QD9 .2630 Ln	1.3266 009 .2827 Ln
Grain 5ize Distribution (mm)	.173 md .19 0,5 .13 0,2 .0300 0,00	3.73 md 6.50 075 .10 025 3.20 00a	.717 md 1.00 075 .39 025 .305 00a	.174 md .33 075 .059 025 .135 00a	.167 md .21 075 .125 025 .042 00a	.176 md .22 075 .13 025 .045 0Da	.213 md .22 0.5 .125 02 . .047 00a
Engineering Properties	.25 P	.10 P	н а 00.		- 40 - 40	.70 P	.05 140 140
Coef. of Uniform. (Cu)	1.94	7.24	2.53	9.61	2.19	2.02	1.99
Size Analysis (%)	5.98 G 2.92 CS 7.49 MS 80.53 FS 2.47 S	31.54 6 5.78 CS 5.78 CS 19.01 MS 28.79 FS 11.94 5	2.94 C 26.99 G 34.66 CS 32.10 MS 4.99 FS		8.12 C .44 G .74 CS 7.76 MS 81.84 F5 7.27 S	1.95 C 59 G 1.30 CS 12.14 MS 79.28 FS 79.28 FS	2) 11.14 G 1.78 CS 8.02 MS 71.90 FS 5.80 S 1.36 C
Porosity \$(%)	49.70	41.49	42.06	39.72	42.66	42.69	ers (7/10/7 43.75
Void Ratio (e)	886	607.	.726	. 659	.744	.745	840y div .778
Water Content W(%)	37.2	38.3	29.1	37.9	28.2	34.3	36.8 36.8
5pec. Grav. (G,)	2.67	2.70	2.71	2.62	2.66	2.67	2.67
Ave							
(Wet) Interval Wet (gm/cm)	1.88	2.24	2.08	2.23	2.01	2.06	2.11
Density Sample Interval (cm)	0-24.1	0-15.2	0-31.2	0-06.0	0-64.6	0-62.8	0-62.0
ple Vol. (cc)	1009.8	839.9	1724.1	331.6	3569.8	3470.3	3426.1
Nt. (om)	1905.0	1877.7	3583.4	739.9	7160.0	7136.0	7341.8
Site	124V	1-X	X-2	£-X	Len T	Z#H1	

Sorting	(So)		4.0551 QDg 1.4000 Ln	4.7140 009 1.5505 Ln	4.0277 009 1.3932 Ln			3009 009 2630 Ln		.7500 00g .3218 Ln	
Grain Size			.040 md .074 075 .0045 025 .035 008	.035 md .0660 07 5 .0027 025	.035 md .073 075 .0045 025 .034 00a			.175 md .22 075 .13 025 .045 00a	.29 075 .09 075 .00 0085	.048 md .29 0.75 .0064 0.25 .042 0.03	
Engineering	Li oper ti es		F 05.	.25 T .20 P	.15 T .60 P			15 T 10 P	F 4 500.	.75 P	
Coef. of	(c <sup>n</sup> )							1.90	5.50		
Size Analucic			.51 G .51 G .20 CS 22.09 FS 4884 5 27.21 C	.41 G .40 C5 .47 MS 18.32 FS 51.32 5 29.05 C	.19 G .16 CS .91 MS 22.34 FS 49.82 S			1.25 6 10.93 MS 82.48 FS 3.90 5 .87 C	19.82 G 1.76 CS 5.77 MS 53.10 FS 14.59 5 4.96 C	.18 G .15 C5 1.79 MS 23.62 FS 46.57 S 21.29 C	
Perosity	0(2)	(7/10/72) 42.43	36.22	58.57	43.18		8/28/72)	48.87	48.42	48.32	
Void	(e)	C. Buoy .768	. 568	1.414	.760		C. Buoy	. 956	666.	.935	
Mater	H(2)	36.7	38.1	32.6	39.4		ŝ	36.0	35.3	35.0	
Spec.	(6 <sub>S</sub> )	2.67	2.66	2.66	2.66			2.67	2.67	2.67	
Ave						2.08 1.78 1.83					1.90 1.91 1.90
(Het) Interval	(gm/cm)		2.60	1.50	2.16			1.90	1.91	1.91	
Sample Interval	(Cm)	0-80.0	80.0-148.0	148-151	151-259		0-170.2	0-29.7	29.7-75.2	75.2-170.2	
ple	(cc)	3352.0	2849.2	125.7	4525.2		7131.4	1244.3	1906.4	3980.5	
Sam U+	( m6 )	7173.3	5869.4	188.0	9774.4		14293.1	2364.2	3641.0	7602.8	
Site	No		Top	9[bbiM	Bottom	x 00 00		dol	e[bbiM	Bottom	<b>3</b> , 00 00

APPENDIX A.7 DATA SUMMARY SHEETS - REFLECTION COEFFICIENT MAPPING EXPERIMENT



# Best Available Copy



DATE 6/28/70 FOLL'O 1

TIME	PTNG:	- CHA-1	EL.	SPAR	KER CHA	NNEL	WATER DE	РТН
	AME	STGMA	B-L055	AVE	SIGMA	B-LUSS	AVE	SIGMA
18 06.00	145	064	7.1	.185	.098	15.6	73.7	5 1
18.07.00	5.7	155	5.5	408	.150	8.5	84.3	-1.1
18.08.00	FAS	079	5.0	. 486	.222	7 3	93.0	1.5
18.09.01	507	185	4.6	.573	270	5.7	94.5	
18 10 00	5. 2	244	4 9	615	101	4 6	07 5	5
18 11 00	641	021	7.5	440	101	7 4	01 0	.,
18 12 00	51.9	105	4.8	. 408	082	6.2	01 3	.5
18 13 00	FLE	102	5.2	.527	128	5.0	91.1	. 7
18 14 00	6.1	11-7	4 2	.566	070	5 0	80 6	. / 7
19 15 00	F.(7)	0.04	4 7	502	066	6.1	20 0	
18 16 00	626		A 1	5116	060	6.0	20.3	. 7
19 17 00	6.7.1	157	3 6	415	0.82			• 2
18 19 10			4.2	428	103	7.6	30.0	• • • •
19 19 00	617	012	4.2	417	.103	7.0	39.9	.4
19 20 00	617	0.2.9	4.3	426	097	7.5	20 4	.0
19 01 00			4.2	452	077	1.5	37 7	• 5
19 22		• • 1 1	4.5	. 45/		0.9	3/./	• 5
10.22.04			4.4	.43/	.101	7.4	87.3	• 3
18.23.00			4./	.439	.102	1.4	86.9	. 3
18.24.00		.1.5.	5.0	.426	.198	1.1	86.4	. 4
18.25.00		. 1.41	4.0	. 447	.083	1.2	86.3	. 4
18.26.0"			5.2	.441	. 1160	1.2	86.0	. 4
18.27.00	• • •		4.6	. 464	. 160	6.8	35.6	.5
18.28.01	- F / - 1	.103	5.0	.475	.060	6.5	35.3	.3
18.29.01	• 5 3 3	.141	5.5	.45?	.072	7.0	85.6	. 4
18.30.00		. 0.45	5.3	.421	.106	7.9	85.3	.4
18:31.00	• E 4 0	. 073	5.3	.453	.002	7.0	55.3	•5
18.32.00	. 54.	. 071	5.4	.420	.087	7.5	85.2	• 5
18.33.00	. 5 . 1	. 177	5.6	.445	.117	7.4	85.1	.6
18.34.00	.555	.051	5.2	.450	.092	7.1	84.9	. 3
18.35.00	.5.55	• 900	5.5	.465	.071	6.8	84.7	. 4
18.36.00	.550	. 101	5.2	. 428	.081	7.5	34.6	.3
18.37.00	. 55, 4	. 130	5.1	.476	.046	6.5	84.5	.3
18.39.01		. "> "	5.6	.461	. 1146	6.5	64.3	.5
18.39.01	.551	. 101	5.1	. 429	.110	7.7	84.2	.3
18.40.00	.5.4.2	. 71	4.5	. 462	.068	6.8	34.1	.3
18.41.00		.071	5.3	.482	.051	6.4	84.1	. 4
18.42.00	/	. 1.37	5.0	.474	.047	6.5	53.9	. 3
18.43.Ur	.50.5	. 1148	5.2	.471	.975	6.7	83.9	.5
18.44.00	. 5. 7. 6	. 137	5.1	.481	.041	6.4	83.9	.6
18.45.00	. = ; 4	.1.57	5.1	.451	.075	7.1	37.8	.5
18.46.00	. 5 7 ')	. 722	5.2	.448	.057	7.0	83.8	.3
18.47.00	. 613	.1.17	6.1	.462	. 164	6.8	84.0	.7
18.48.00	. 5 . 7	. 155	5.1	.429	.053	7.4	83.9	.3
18.49.00	.142	.002	6.3	.446	.041	7.2	33.8	.3
18.50.00	. 5 27	. 47	5.7	. 437	.077	7.3	83.9	. 3
18.51.00	540	. 1126	5.3	. 445	.068	7.1	34.0	. 3
18.52.00	F 4 0	. 11	5.3	. 422	.038	7.5	63.8	.4
18 53 00	E 1 13	.0.1	6.0	.493	. 094	6.3	83.9	.5
		+				~ · · ·		
18.54.00	F 17	. 0 37	5.5	. 410	. 062	7.8	83.6	.5

DATE 6/2: // . . WATER DEPTH PI CHA CHA MEL TIME SPARKEP CHANNEL STUMA B-LOSS SIGMA H-LOSS SIGMA AV-AVE AVE 18.56.00 .519 .500 . 154 . 167 6.1 5.9 52.7 .2 . 5.17 .5 5.7 .511 32.3 .r.2n . 156 . 27 18.58.00 5.5 . 117 . 477 .4 . 065 81.7 6.5 18.59.00 .100 .037 . 15? 6.1 31.5 6.9 . 1 15 . 1.9 2 dr.5 19.00.00 .433 . 072 7.4 1.5 6.4 . 510 19.01.00 . 10? . 154 6.0 6.0 . 3 . 4 . . 4 .461 . 059 19.02.00 .130 80.0 .4 6.3 0.0 19.03.00 FHI .136 6.1 6.0 79.7 .4 6.5 . 411 7.4 19.04.00 . 4.15 .174 .143 6.9 77.2 . Reproduced from best available copy 0 . 223

TTHE	DI CL	C CHANN	<b>E1</b>	COAD	KED CHA	NALET	WATER DE	PTH
11nc	AVE	SIGMA	B-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA
7.16.53	P78	.108	1.2	.530	.286	6.4	102.3	.6
7.17.53	. 694	.043	3.2	.798	.342	2.6	101.6	.4
7.18.53	.717	.081	3.0	.997	.715	1.3	101.0	.4
17.19.53	.76.3	. 197	2.8	.841	.641	3.3	100.0	.5
17.20.57	.771	.188	2.3	.572	.180	2.3	99.1	.0
17.21.53		.124	1.8	+ 7/4	.207	5.0	97.4	.0
7.22.53	479	020	3.4	541	182	5.8	97.0	
	•••	• 17 2 *						
								~
		_						
		_		224				

•

ſ

1100	PINCE	P CHANE	IEL	SPAR	KER CH	AP MEL	WATER DE	EPTH
	AVE	STGMA	B-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGM
17.52.57	.584	.036	4.7	.511	.068	5.9	88.7	.5
17.53.53	.565	.065	5.0	.453	.114	7.3	88.6	.5
17.54.53	.551	. 151	5.2	.518	. 039	.5.7	88.4	.3
17.55.57	.500	.032	4.9	.503	.038	6.0	88.3	.4
17.56.53	.5:3	.038	5.0	.475	. 159	6.5	88.1	.4
17.57.53	.538	. 970	5.5	.479	.086	6.5	87.8	.6
17.58.57	545	.1.33	5.0	. 455	.089	7.0	87.5	.8
17.59.54	. 750	.044	5.1	.4/0	.0/5	0./	87.4	
18.00.5		. 197	5.0	. 440	.000	1.2	24 0	
18 02 57	. 5 10	140	5.5	.472	. 044	6 6	86.8	. 2
18.03.53	5 (4	. 677	5.5	511	.067		86.4	.5
18.04.57	.532	.030	5.5	.483	.086	6.5	86.2	
18.05.53	5.5	.060	5.5	.488	.065	6.3	85.9	.6
18.06.53	.554	.046	5.1	.481	.061	6.4	85.6	.5
18.07.53	.575	.130	5.4	.463	.126	7.3	85.2	.5
18.08.53	.514	. 162	5.8	.520	.100	5.9	84.8	.6
18.09.53	.537	.134	5.4	.457	.113	7.3	84.7	.3
								•
					·			

## DATE 06779/79 PHL10 3

#### DATE 06/29//5 301210 7

TIME	PING	IT CHAS	Hel.	SPAR	KEP CH	ANNEL	HATER DI	EPTH
	A 1/ 12	SIG"4	H-LOSS	AVE	SIGMA	8-LOSS	AVF	SIGM
18.44.53	. 2 . 3	.074	1.1	. 429	.104	7.9	07.0	7
18.45.53	.622	.232	3.4	.483	.050	6.4	93.0	
18.46.5?	. 450	.141	4.0	.485	.055	6.3	93 2	2.4
18.47.53	. 6 %	. 148	1.6	.437	048	7.2	93.2	• • •
18.48.53	95,9.	.104	1.6	. 448	.118	7.6	07 7	
18.49.53	. 6 3 3	.120	1.7	.508	.041	5.9	07.5	
18.50.53	.784	.167	2.4	. 494	.053	6.2	07 7	
18.51.53	•75 <sup>A</sup>	. 228	2.1	. 445	.072	7.2	92.5	5 5
18.52.57	.748	. 276	2.5	. 448	.079	7.1	92.5	5.2
18.53.53	.774	.197	2.6	.458	. 067	6.9	94.0	2+0
18.54.53	.1.95	.301	3.4	.510	.147	6.5	07.9	- 7 5
18.55.53	.730	. 227	2.4	.477	. 152	6.5	02 6	
18.56.53	.736	.269	2.6	. 422	109	8 0	02.0	2.4
18.57.53	RUG	.132	1.4	. 468	.051	6.6	92.0	2.0
18.58.53	.Fd1	.334	3.1	. 499	.068	6 1	-0.0	• /
18.59.53	. 416	.334	4.0	.454	.125	7 5	01.7	9.0
19.00.53	. 6 6	.357	3.4	. 408	.118	8 4	20.2	9.4
19.01.53	.971	.087	1.2	.445	160	8.2	94.9	y.5
19.02.53	.743	.211	3.0	.524	. 059	5 7	74.0	
19.03.53	. 643	.076	3.2	518	102	5.7	94.9	.4

Reproduced from best available copy.

226

.. .

8

TIME	PINGE	P CHANN	EL	SPAR	KER CHA	NNEL	WATER DEPTH	
	AVE	SIGMA	B-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGM
17.39.53	. 436	. 0.43	3.9	.639	. 327	5.1	91.1	.4
17.40.53	.630	.033	4.0	.786	.382	3.3	90.9	.4
17.41.53	.619	.037	4.2	.565	.271	6.0	90.7	.5
17.42.53	. 538	. 030	3.9	.542	.356	7.3	90.6	.3
17.43.95		• 11 2 1	4.3	•51/	• 305	5.5	90.4	.4
						••		
				······································				
				-				
· · · · · · · · · · · · · · · · · · ·			1 10-			è	an inaga anu sin sing 1 ann a	

## DATE 6/29/72 POLLTO 7

TIME	e tuca	CHANN	E1	SPAR	KER CHA	MMEL	WATER DE	PTH
	AVE	SIGMA	B-LOSS	AVE	SIGMA	6-LCSS	AVE	SIGMA
18.32.53	.573	.023	4.8	.485	.153	5.4	91.3	.3
18.33.53	.581	.045	4.7	.464	.198	4.8	91.9	.4
18.34.5	- 262	. 107	2.0	- 29.1		2.4	20.6	- 0 A
10.32.21		0.4	4.0	.607	. 423	2.7	93.1	.3
18 40 57	577	.064	4.8	.582	.482	2.3	92.9	.4
18.41.57	.54?	.187	5.4	.522	.551	2.5	93.1	.6
		•			·····			· · · · · · · · · · · · · · · · · · ·
							-	
		_						
*********								
								-
				28				
								agamentagani vin ukamarka

TIME	PT.CE AVE	STUMA	HLUSS	AVE	SIGMA	NNEL 9-LOSS	WATER DE	EPTH SIGM
16 23 57	0.0.4	170	1.0					
16 24 53	640	.134	1.9	.4/4	.107	0.0	115.4	1.0
16 25 53	663	3.15	3.0	-205		0.0	114.7	5.0
16.26.53	Euc	401	3.7	570	101	4.0	113.7	8.1
16.27.53	.7.1	.200	3.0	516	114	4.7	116 3	12.1
16.28.53		296	2.5	.524	. 096	5 7	116.0	0 4
16.29.53	.77>	.249	2.0	.490	.074	ó. 1	118.0	6.4
			-					

#### DATE 06/74//9 50100 4

.

TIME	PTUC	CHAN.	EI	SPAR	KER CHA	NNEL	HATER D	POH
	AVE	\$15"A	3-L055	AVE	SIGMA	B-LOSS	AVE	SIGHA
5 74 57	<u> </u>	1 4 7		557	0 77	2.0	01.0	4.1
5.35.53	.724	. 0.6.9	2.8	.540	.062	7.4	91.0	0.1
5.36.57	. 641	1.97	3.4	.564	. 064	5.0	42.4	5.5
5. 37.53	. = ; 7	. 302	3.8	.520	.115	5.9	88.3	10.5
5.38.53	.72"	.186	2.9	.591	.091	4.7	94.4	.4
5.39.53	.601	.150	3.2	. 455	.086	3.7	93.3	5.4
5.40.51	.74-	.167	2.6	. 636	.039	3.9	94.7	.5
5.41.57	.745	. "61	2.6	. 550	.110	5.4	95.0	.8
5.42.5		.196	2.9	. 595	.105	4.6	93.7	5.4
5.43.5			2.3	.0/5	. 1198	3.5	95.5	
5 45 57	710	204	2.6	566	094	9 • 1 5 1	97.0	.7
5.46.27	. F 1F	266	3.0	.573	. 196	4.9	93.2	8.0
5.47.57	. 71 .2	. 294	2.6	.549	.059	5.3	94.9	6.3
5.48.57	.7	.225	2.8	.527	.104	5.7	95.3	5.9
5.49.57	. 7	.214	2.1	. 549	. 078	5.3	95.7	5.9
5.50.57	. 4	.25F	3.1	.549	. 087	2.3	96.1	6.0
5.51.57	. 11	. 0.54	1.9	.558	.076	4.7	99.0	.5
5.52.5	.7	.067	2.0	.564	.093	5.1	99.3	• 8
5.53.51			2.5	.569	83	5.0	95.7	7.5
5.54.5	4		1.9	- 72.1	. 1.90	2./	99.1	.7
5 56 53	613	7/ 5	2.7	5.71	026	51	25.0	10.0
5.57.57	797	.7	1.8	.552	.092	5.1	95.0	1.1.8
5.58.51	754	.244	1.7	527	.094	5.0	97.9	7.6
5.59.53	. 0	. 657	.5	.518	. 196	5.4	101.0	. 4
6.00.57	.041	. 151	.5	. 512	.077	5.9	131.5	.7
6.01.53	. 3.97	. 714	1.1	. 540	.116	5.6	100.6	5.5
6. 02.53	.035	.265	. 8	.511	.075	5.9	102.9	1.0
6.03.57		. 340	1.1	.561	74	5.1	100.3	7.5
6.04.57	.7: -	.411	1.2	.516	.125	6.0	yR.7	10.1
6.05.5			<u><u><u></u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u></u>	4.8	.107	5.5	100.5	9.3
6.07.57	015	260	. 1.2	.557	.075	5.2	1 17 0	0.7
6. 08.57	. 6.4	771	2.8	-5.56	.005	2.5	101.5	9.6
6.19.51	.7.7	. 7 27	2.5	. 529	.085	5.6	103.3	9.2
6.10.57	.7,0	2.4	2.8	.535	.103	5.6	105.5	6.5
6.11.57	. 925	.1 97	1.9	. 578	. 072	4.8	1.07.6	.9
6.12.57	.417	. 7 5 7	3.1	. 529	. 190	5.7	103.8	5.9
6.13.57	.P.A.T	. 174	1.5	.581	.077	4.5	108.7	.7
6.14.57	.7.4	.241	2.3	. 595	. 171	4.6	107.8	5.6
0.15.57	6	.196	1.4	.592	.075	4.6	110.3	1.0
0.10.5			2.8	.584	.116	4.6	199.1	9.3
6 19 57			2.0		. 004	2.0	136.9	9.7
6 10 57	-1-		1.9	· 144	• : 91	5.9	117.9	.8
6 20 57		170	3.1	600	.107	4.0	117.4	4./
6.21 57	- / 6	2.25	1 4	. 404	007	4.7	112 4	.0
	•				• 7 '	1.0	11.0	9.0

.

#### DATE 07/06/72 . 6

.

TIME	PT Cit	tet_	SPAR	KER CH	Authel	ATTR D	PTH
	AV1: 57,44	U-LASS	AVE	SIGMA	E-LOSS	AVE	SIGMA
	······································			20, 270			
17.52.57	. 1 24 . 1 () 5.	3.3	.635	.077	4. Ú	100.7	.5
17.53.57	.757	2.5	. 548	.083	5.3	1.01.4	. 3
17.54.57	7.0 .00%	2.9	. 563	. 176	5.1	101.5	.7
17.55.57	.717 .057	3.0	.530	081.	5.6	102.1	. 4
17.56.53	.620 .1ye	2.7	. 457	.114	7.1	101.2	5.5
17.57.57	. 6.55 . 262	3.1	. 570	. 077	5.0	100.2	8.2
17.58.57	.6: .193	3.ú	.471	.091	6.7	1.01.9	6.3
17.59.53	.7.,7 .047	2.3	.535	. 075	5.5	104.0	. 6
18.00.53	. 4 . 5 . 217	3.2	.525	.089	5.7	102.8	5.5
18.01.57	.752 .030	2.5	. 542	. 192	5.4	104.6	. 4
18. 2. 23		3.1	. 528	.165	5.6	102.0	7.5
,18.03.53	.5 .4 . 24"	3.6	.507	.058	5.1	101.7	8.5
18.04.53	.772 .0.30	2.3	.535	.266	5.2	105.5	.5
18.05.57	.743 .746	2.4	. 51) 0	.082	4.5	105.5	. 4
18.06.57	.721 .155	2.8	. 400	.108	5.4	105.6	.6
18.07.53	.915 .036	1.9	.571	. 1.30	4.8	1.05.3	. 4
18.08.57	.71	2.5	. 490	.117	6.3	103.9	5.5
18.09.57	4.1 .76	3.3	.557	.117	5.3	102.2	7.9
18.10.57	.7.1 .112	3.2	.574	.007	4.9	104.5	.6
18.11.57	.7.7 .124	2.8	.547	.166	5.4	103.9	.5
18.12.27	.72 .157	2.5	. 641	350.	3.9	104.3	.5
18.13.57	.500 .200	3.6	.531	.108	5.7	102.2	7.7
18.14.57	.745 .035	2.0	. 656	. 58	3.7	135.4	.5
18.15.57	. 263 .051	1.2	. 554	.122	5.3	106.3	. 6
18.16.57	.737 .079	2.7	.628	.063	4.1	100.9	.5
18.17.57	.743 .1.22	2.5	. 553	. 071	5.2	107.5	.5
18.18.57	.764 .036	2.4	.546	.104	5.4	109.0	.4
18.19.57	744 .002	2.6	.581	. 088	4.0	109.7	.4
18.20.53	752 151	2.7	.651	. 167	3.8	100.2	. 6
18.21.53	. 4.1 . 210	2.9	.512	.130	6. ú	109.8	5.7
18.22.57	.744 .771	2.5	.651	. 978	3.8	110.9	.5
18.23.57	4+7 .7.10	6.1	.555	. 428	0.1	107.3	9.9



231

. . .

. . .

#### DATE 07/14/17 PULL'C 17

TIME	P1-3-	CHAN	VEL	SPAR	KER CH	ANNEL	WATER DEPTH	
	A V -	SIGMA	H-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA
14.37.53	. 416	.155	8.0	.509	.246	7.4	68.8	. 8
14.38.53	.415	.195	8.0	.476	.236	8.2	69.5	2.2
14.39.57	. 47 :	. 947	6.6	.590	.150	5.1	68.3	. 4
14.40.57	. 120	. 167	7.6	.579	.104	4.9	68.0	.8
14.41.57	.449	. 1146	7.0	.459	.238	8.5	67.8	1.0
14.42.57	. 4 24	. 153	7.5	.490	.217	7.7	67.9	. 4
14.43.57	.441	. 935	7.1	.560	.133	5.4	67.8	.6
14.44.57	.332	.1.03	8.4	.553	. 194	5.3	64.8	7.4
14.45.57	.412	. 743	7.6	. 358	.215	10.9	67.1	.5
14.46.57	. 4 31;	. 145	7.3	.507	.221	6.9	67.4	.8
14.47.57	. 2 . 4	.174	8.6	.371	.190	9.8	63.3	8.6
14.48.57	. 423	. 147	7.5	.434	198	8.6	67.8	.5
14.49.57	. 4 3 4	. 172	7.4	.485	193	7.1	67.9	. 6
14.50.53	.420	150	7.6	.541	.084	5.4	67.9	.7
14.51.57	.419	. 172	7.9	.313	180	11.8	67.5	.8
14.52.57	. 411	056	7.8	.560	.191	5.9	67.7	
14.53.53	460	.098	6.9	.359	197	10.5	67.9	.7
14.54.57	.429	214	7 4	377	206	10.0	68 7	
14.55.57	451	018	6 9	488	166	7 0	60.7	1 0
14 56 53	435	050	7 5	468	117	7.0	40 0	1.0
14 57 53	410	0.47	7.5	.400	• 11/	1.0	69.0	1.4
14 59 57	470	047	7.0	.40/	•10/	0.8	04.0	1.1
14 50 57	. 4 . 4	0.75	7.2	547	.120	0.1	09.0	1.2
19.09.0	431	210	/.5	.74/	.144	2.0	70.0	
15.04.57		. 141	0.9	. 400	.12/	1.5	70.5	. /
15.01.5	. 4 4 1)	. 1144	/.0	.715	•14.5	0.3	70.8	.8
12.02.5	. 4.56	• 737	0.8	.517	.200	6.6	70.9	1.2
15.13.5	. 41/	. 05 5	7.7	.454	.102	7.1	71.0	.7
15.04.5	.4.50	.107	7.4	.520	.178	6.6	71.0	.7
15.05.5	. 45, 3	. 148	6.9	.616	.081	4.3	71.5	.6
15.06.57	. 425	.060	7.5	.444	.133	7.5	71.7	. 6
15.07.54	.449	. 179	7.1	.524	.146	6.1	72.1	.7
15.08.57	. 437	.149	7.2	.527	.097	5.7	72.2	.5
15.09.53	. 44.	.1154	7.2	.482	.124	6.7	72.7	.9
15.10.5*	. 4.50	.034	7.5	.508	.137	6.3	72.7	.5
15.11.57	. 411	. 164	7.8	.581	.083	4.8	72.6	.5
15.12.57	. 45.4	.139	6.9	.546	.084	5.4	73.0	.5
15.13.57	.3.4	.135	9.1	.56?	.123	5.3	71.9	5.5
15.14.57	.293	.176	9.7	.483	.211	7.5	70.8	8.0
15.15.57	. 755	.141	9.9	.520	.102	5.8	74.1	1.3
15.16.57	. 450	.024	6.8	.511	.108	6.1	73.9	.7
15.17.53	.464	.020	6.6	.507	.169	6.1	73.9	.7
15.18.53	. 447	. 150	7.1	.602	.174	4.9	74.1	.7
15.19.57	.457	.129	6.8	.510	.151	6.3	74.2	1.0
15.20.57	. 4.5	. 156	7.2	.548	.154	5.5	75.1	. 8
15.21.53	.47%	. 022	6.4	.500	.083	6.1	74.2	. 8
15.22.57	. 7 20	.163	8.7	.508	. 192	6.6	71.6	7.4
15.23.57	. 450	. 733	6.6	.572	.084	4.9	74.4	.5
15.24.57	.440	.065	7.1	. 497	.172	6.9	74.2	
45 05 53	474					0.,	1 2	• *
12./2.5	. 4 / 1		6.5	.540	. 162	6 0	74 0	A.

				0.40			WATER DE	DTL
TIME	PINGE	P CHANN	EL	SPAR	STOMA	ANNEL GALOCE	AVE	SIGH
	AVE	SIGMA	8-L055	AVE	SIGHA	0-1/155		
16.57.53	.785	.056	2.1	.590	.081	4.7	113.5	.4
16.58.53	.755	.048	2.5	.525	.175	4.8	112.6	.6
16.59.53	.783	.043	2.1	.540	.117	5.6	111.9	.6
17.00.53	.700	. 161	2.4	.592	.065	4.6	111.3	.5
17.01.53	.761	. 054	2.2	.545	.086	5.4	110.5	.3
17.02.53	.743	. 147	2.6	.502	.238	4.3	109.6	.4
17.03.53	.751	77	2.5	.523	.101	5.8	108.7	.4
17.04.53	.738	.021	2.6	.561	.081	5.1	108.5	.4
17.05.53	.762	. 060	2.4	.593	.108	4.7	108.1	.5
17.06.53	.727	. 140	2.8	.527	.088	5.7	107.3	. 3
17.07.53	.73H	. 143	2.7	.577	.102	4.9	106.7	.3
17.08.53	. 726	.038	2.8	.495	.110	6.3	106.4	.7
17.09.53	.732	.037	2.7	.604	.176	4.7	105.7	.5
17.10.53	. 41?	.400	3.8	.548	.112	5.4	95.5	10.4
17.13.53	.751	.037	2.5	.535	.078	5.5	103.2	. 4
17.14.53	.740	.137	2.6	.541	.119	5.6	102.6	.5
17.15.53	.766	. 152	2.3	.564	.103	5.2	102.0	.3
17.16.53	.735	.053	2.7	.537	.114	5.6	101.5	.3
17.17.53	.794	. 193	2.1	.556	.114	5.3	100.8	.5
17.18.53	.753	.041	2.5	.570	.128	5.1	100.4	.5
17.19.53	.747	.023	2.5	.586	.087	4.7	100.1	.4
17.20.53	.731	. 047	2.7	.557	.126	5.3	99.5	. 4
17.21.53	.744	.040	2.6	.552	.118	5.4	98.9	.2
17.22.53	.734	. 139	2.7	.559	.128	5.3	99.4	.2
17.23.53	.727	. 1127	2.8	.564	.080	5.1	98.3	.5
17.24.53	.728	.063	2.8	.571	.116	5.1	98.0	. 3
17.25.53	.743	.131	2.6	.617	.073	4.3	98.1	.7
17.26.53	.744	.140	2.6	.562	.106	5.2	99.6	.3
17.27.57	.737	.1156	2.7	.551	.099	5.3	100.0	.4
17.28.53	.720	. 1127	2.9	.499	.111	6.3	99.7	.6
17.29.53	.72?	. 024	2.8	.537	. 191	5.5	99.6	.4
17.30.53	.679	. 166	3.4	.506	.092	ó.1	99.4	.4
17.31.57	.723	.048	2.8	.484	. 167	6.4	99.6	.3
17.32.53	. 696	.171	3.2	.540	.085	5.5	99.4	.5
17.33.53	. 648	. 148	3.1	.504	.105	6.1	99.0	.4
17.34.53	.69f	.050	3.2	.538	. 169	5.5	99.7	.2
17.35.57	.693	.037	3.2	.517	.084	5.8	98.4	.2
17.36.53	.667	. 72	3.6	.542	.079	5.4	97.9	.2
17.37.53	.711	.030	3.0	.548	. 166	5.3	97.8	.3
	71.4	AE 4	7 4	407	078	4 5	97.7	. 5

#### DATE 07/14/72 BOLL 10 17

TIME	PINCH AVE	P CHARM SIGMA	B-LOSS	AVE	SIGMA	HNEL B-LOSS	AVE	SIGMA
15.27.5"	. 41.0	.135	6.7	.556	.070	5.2	73.7	.7
15.28.57	. 4	. 023	7.0	. 481	.121	6.7	77.4	.6
15.29.57	. 1. 2	. 920	6.7	.454	.086	7. U	73.5	. 4
15.70.57	. 1 7	150.	6.8	.544	.099	5.4	73.2	.6
15.31.57	. 6/ 1	. 075	7.2	.472	.119	6.8	73.1	. 8
15.32.5"	. 171	.129	6.5	.560	. 075	5.1	72.4	.4
15.33.5'	1 : 4	. 258	6.9	.481	.164	7.3	72.3	. 8
15.34.57		1.9	6.9	.635	.084	4.0	72.3	.7
15.75.27	7 6	. 1.1.0	6.9	. 495	.139	0.5	70.4	4.1
15.36.57	.4.3	. 1:17	7.6	.462	.160	7.0	71.4	. ,
						•=		
							Sector Sector	
							the second second	
				100			me	
					1.00			
			- the second second			-		
						TT TO ALL		
							to the server manage	
	$\sim$							
				24				
			2	34				

	PT: C-	C CHANN	<b>E</b> 1	SPAR	KER CHA	NNEL	WATER DE	РТН
	AVE	SIGHA	B-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGH
17.41.57	.45:	.101	6.8	.651	.182	4.1	83.7	.9
17.42.53	.451	. 974	6.5	.459	.083	6.9	84.0	1.3
17.43.57	.404	.155	6.4	.398	.083	8.2	81.9	
17.44.57	.457	• 1.0 *	6.5	. 499	.105	6.3	80.8	.9
17.46.57	.472	. 150	6.6	.445	.103	7.3	79.7	.7
17.47.53	. 427	.197	7.7	.403	.142	6.9	78.5	2.7
17.48.57	.431	. 773	7.5	.403	.072	8.0	79.3	.6
17.49.57	. 4 4 4	.031	6.1	. 474	.086	6.6	78.9	1.4
17.50.53	.417	. 98?	0.0	.476	.088	5.4	79.0	.0
17.51.51	.454	144	75	406	.112	6.3	76.6	4.3
17.53.53	479	.121	6.4	.415	.056	7.7	78.3	.8
17.54.57	.4.52	. 354	7.4	.462	. 167	6.8	79.6	.5
17.55.57	. 4 2 0	.105	7.8	.485	.102	6.5	78.3	1.5
17.56.57	. 4 . 4	.127	6.3	. 471	.080	6.7	79.8	1.0
17.57.57	.470	.150	6.5	.445	. 164	/ 1	/9.1	. 9
				•				

#### DATE 07/14/7 21. 21.

				**			·
TIMF	PT CLT CHANA	EL.	SPAR	KEP CHA	NNEL	WATER DE	PTH
	AV: SIGMA	3-L055	AVE	SIGMA	H-LUSS	AVE	SIGMA
				1			
16.29.57	.77 .117	7.9	. 327	.201	11.0	68.7	2.0
16.30.57	.3107	8.7	.489	.258	7.0	67.0	7.9
16.31.57	.3 7 .131	9.9	.579	.166	5.3	69.6	6.6
16.32.57	. 4 . 1 . 1 . 5	7.5	.396	. 200	9.7	73.9	
16.33.57	. 7. 7 . 7. 7	7.0	.425	.124	8.0	57.0	10.2
16.34.57	. d 4 . to 7	7.3	.422	.170	3.4	73.3	7.7
16.35.57	.4'1. 3	6.8	.450	. 096	7.1	73.5	5.9
16.36.5	. 7.7 .1.77	7.7	. 458	.068	5.4	71.3	7.6
16.37.57	.41 4	7.0	.477	.120	5.8	74.2	1.6
16.38.57	• 4 · · • • • • • • •	0.8	.432	. 974	7.4	74.5	1.3
16.39.57		8.4	.410	.164	5.6	71.8	7.4
16.40.57	• 7 • 7 • 1 , 3	6.7	.404	.115	3.2	70.4	10.0
16.41.5	.41 17	7.0	.414	.102	8.6	74.0	6.8
16.42.5*	. 1 . 7	7.7	. 445	.109	7.4	74.5	6.1
16.43.5		6.6	.437	.070	7.3	72.8	7.8
				100	1.1.1.1.1.1		
-							
		- F 1 1 1 1 1					
	•						
					-		
	and a second descent of the second se	· · · ·		0.200	10 de administration		
		••					
	and the second sec						ante 🖝 antito Ministerio en estatute a depute de a d
· · · · · · · · · · · · · · · · · · ·							
					Allowed in some or our statements		
And a support and the support and the support							
		-					
			226				
			230			Setting and	
							1 0
•							

TIME	- 19	n Critery	EL	SPAR	KEP CHA	NEL	WATER DE	PTH
	AV.	SIGAA	o-LOSS	AVE	STGMA	8-LOSS	AVE	SIGMA
18.09.57	. 4 - 0	.1 1	7.1	.519	.080	5.8	76.3	1.1
18.10.5%	.4.0	. 665	6.3	.532	.107	5.6	75.7	.6
18.12.53	. 4 / '		6.4	- 528	.109	5.7	75.7	
18.13.57	. 470	071	6.6	.551	.118	5.4	75.8	.7
18.14.57	.474	• 0 n 1	6.6	.583	. 150	4.7	75.4	.7
18.15.57	. 4 ~	.000	7.0	.488	. 075	6.3	74.8	.8
18.10.57	. 4. 7	.057	6.9	.468	. 197	6.8	74.3	• 2
18.18.57	.472	.070	7.4	-494	.083	0.9	74.2	
18.19.57	. 4 -, 0	. 030	6.7	. 467	.062	6.7	73.6	.8
7								
	_				_			
					_			

.

TIME	PI 1-1 1.4. 35	1	SPAR	KEP CHA	NNEL	WATER DE	PTH
	A'/ ST DA	-1 055	AVE	STGMA	8-1055	AVE	SIGMA
				510 4			-
15.18.57	. 4. ^ . 1 . 5	8.3	. 473	. 072	5.6	75.0	.4
15.19.5"	. 474 . 71.9	6.6	.460	. 054	6.8	75.3	.7
15.20.57	. 4 4.7 ? 77	7.1	. 472	.031	5.5	75.1	1.1
5.21.5	.4.6 .045	6.9	. 501	.087	6.1	75.6	.5
5.22.57	. 4: " . " . " . "	7.1	.467	. 977	6.7	76.0	.6
5.23.57	4.27 .124	7.5	.495	. 053	6.3	76.3	. 8
5.24.57	. 417 . 173	7.8	.519	. 179	5.8	76.0	.4
5.25.57	.1.1 .164	7.2	. 481	. 145	6.4	76.8	. 8
5.26.57	.4 ? .121	6.4	.471	. 057	6.6	76.2	1.4
5.27.57	.4 34 .071	7.3	. 479	. 055	5.4	76.4	1.2
5.28.5"	. 4 14 . 0 . 2	7.2	. 485	. 138	6.3	76.9	.7
5.29.57	.4	7.0	. 449	.009	7.1	77.1	.5
5.30.57	.4:3 .1.14	6.9	.503	. 060	6.9	77.2	. 8
5.31.57	. 4 1.17	6.9	. 459	. 149	0.8	77.6	.7
5. 72.57	1-1 1 1 2	7.0	. 495	. 070	6.2	77.2	. 8
5.33.5"	.4.5 .1.53	7.2	. 499	. 067	5.1	77.5	.5
5.34.5"		7.8	. 496	.076	6.2	77.7	.5
5.35.57	. 4.7 .143	8.5	. 478	. 079	6.5	74.9	7.2
5.36.5"	.44" . 757	7.2	. 547	. 047	5.3	78.0	. 4
5. 37.57	. 4.2.6 . 7.7	7.6	. 478	. 164	5.5	78.1	1.0
5.38.57	. 4 = 7 . 1 9	7.7	. 490	. 051	6.1	77.9	2.1
5.39.57	4.1 157	9.1	.515	.054	5.8	77.5	2.8
5.40.57	.4: 3 .141	8.5	. 483	.041	6.3	78.2	.6
5.41.51	.4 14 .000	6.2	.500	. 050	5.9	78.4	.6
5.42.57	. 4.74 .114	7.8	. 550	. 174	5.1	78.2	.9
5.13.57	.4.5 .1.4	7.2	. 473	. 153	6.5	74.5	. 6
5.44.57	4	6.7	.556	. 067	5.2	78.5	. 4
5.45.5	.4.2 .141	8.2	. 494	.090	0.2	78.1	1.4
5.46.57	. 47= . 777	6.6	.584	.121	4.8	79.0	.5
5.47.57	. 4'. ' . ^ /7	7.0	. 492	.072	6.2	78.5	.8
5.48.57	= · · · · · · · ·	6.0	.550	.124	5.3	78.9	. 4
5.49.57	.4.2 1.47	8.3	.537	. 685	5.6	77.9	1.7
5.50.57	. 474 . "5"	6.5	. 497	. 084	6.3	78.6	. 4
5.51.57	iarr inge	6.0	.510	.120	6.0	77.9	. 7
5.52.57	7 . 925	6.3	.507	.056	5.9	78.0	. 3
5.55.57	A.C . 094	7.3	. 597	.823	2	76.3	3.2
5.55.5	. 4 7 . 1, ?	7.2	. 497	. 057	0.1	76.3	3.2
5.56.57	.476 .710	6.5	.502	. 166	5.1	74.9	.3
5.58.5	.4:7 .0.3	7.7	. 535	.967	1	16.5	.7
5.59.5	. 4 ?	7.9	.773	. 820	2	75.9	1.5
6.00.5'	4 - 5 - 5-50	6.9	207	586	- 4	75.8	.7
			- 0		• •	1.10	• •

.

-

4

....



....

238

---

.

....

. .

----

-----

#### DATE 07/26/72 " 1110 2" . ....

......

.

z

.

8

.

TIME	PT C 1/	ct	SPAR	KER CH	ANNEL	HATER DEPTH		
	697 S1.3"A	3-LOSS	AVE	SIGMA	J-LOSS	AVE	SIGMA	
	•• •							
16.07.53	.374 .107	8.3	.545	.109	5.4	75.9	6.1	
16.08.53	.417 . 55	7.4	.610	. 190	4.4	75.9	. 9	
16.09.57	.274 .114	10.6	. 444	.077	7.2	76.2	5.8	
16.10.57	.414 .037	8.1	.533	. 076	5.5	77.2	1.2	
16.11.53	.745 .167	8.0	. 465	. 156	ó.7	74.6	7.5	
16.12.57	. 41,8 .165	7.1	. 530	.100	5.5	76.1	6.2	
16.13.53	.745 .177	7.1	.567	.141	5.2	72.5	9.9	
16.15.53	.76174	7.5	. 685	. 636	.4	74.8	9.5	
16.17.53	.497 .181	6.7	.1.67	.209	4.5	75.5	8.8	
16.19.57	.452 .752	6.8	. 153	.111	2.3	79.4	. 6	
16.21.53		5.2	. 470	. 423	3.9	79.9	.9	
16.22.53	.3 / .135	7.5	.319	. 575	. ô	79.7	5.5	
16.24.53	.517 .027	5.8	. 097	.302	. 6	80.0	. 6	
16.27.53	. 424 . 177	6.1	. 479	.742	0	77.1	7.4	
16.26.57	.000 . 147	6.2	. 154	.020	25.1	79.9	. 6	
16.27.57	. 424 .175	6.1	. 186	. 929	21.7	76.9	7.5	
16.28.53	.437 .121	6.5	.101	.159	23.1	77.9	6.1	
16.30.53	. 4 1, 10	7.8	. 426	.103	7.9	77.6	5.7	
16.31.53	. 434 . 0 42	7.5	. 448	.154	7.9	79.3	. 6	
16.32.53	.787 .191	6.5	. 456	.134	7.5	75.3	8.9	
16.33.57	.41.7 .145	8.2	. 457	.111	7.2	79.4	. 6	
16.34.57	.765 .163	8.6	.208	.166	16.0	74.9	7.5	
16.35.53	.795 .134	7.5	.125	. 028	18.3	79.0	6.1	
16.36.53	.421 .172	7.6	.490	.057	6.3	79.5	1.6	
16.37.53	.411 .091	8.0	. 452	. 157	7.0	70.9	. 7	
16.38.57	.458 .127	7.2	.134	. 194	18.8	78.2	5.4	
16.39.53	.75" .177	7.8	. 372	.157	9.8	75.6	8.0	
16.40.53	.416 .117	6.1	. 478	. 161	6.5	79.4		
16.41.53	.425 .102	7.8	. 448	.124	7.5	79.0	2.2	
16.42.53	.381 .200	6.3	.480	.071	6.5	75.1	8.9	
16.43.53	.443 .179	7.2	.466	.122	7.2	72.3	. 8	
16.44.53	.365 .141	7.7	. 466	.179	7.7	79.1	5.5	
16.45.53	.791 .130	7.7	. 408	.129	8.5	77.7	5.8	



239 ----

.. .

# DATE 07/21/77 111 6 21

TIME	PILLE CHANNEL			SPAR	RKER CH	WATER DEPTH		
	AVE S	ICMA E	-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA
	420	044	7 7	447	1 075	-1 A	70 9	5
6 44 57	. 4 .1 · ·	057	6 1	•01/	1 410	-1.4	79.0	
0.10.5	. 49" .	051	7 1	- 13A	1.419	5.9	70 9	. 5
0.14.5	· · · · · ·	1.01	- 6 1	.714			70.9	
0.17.5		01.0	6.0	- 730	075	5.0	79.0	1.0
6 17 57	4.7	073	- 6.4	.558	.060	5.1	70.0	
6 19 57	4.5 5	112	7.9	.540	.070	5.4	79.7	1.5
6 19 57	. 45.1	055	6.2	.552	.087	5.3	80.0	.6
6.20.53	Sec.	106	5.0	.591	.088	4.7	79.8	1.0
6.21.53	.462	699	6.9	.559	.066	5.1	80.0	.8
6.22.57	.467	~5n	6.6	.545	. 068	5.3	80.0	.7
6.23.57	. 4 nº .	174	6.4	.545	.031	5.3	0.76	.4
6.24.57	. 4 . 2 .	Fe6	6.4	.544	.045	5.3	79.7	.4
6.25.53	. 470 .	061	6.5	.515	.034	5.8	79.7	.9
			2	40				
			2	40				
			2	40				

TIME	PTUCHP CHAUSEL			SPAR	KEP CHA	HATER DEPTH		
	AV 2	SIJMA	B-LOSS	AVE	SIGMA	A-LOSS	AVE	SIGMA
15.23.53	. E ÷ į.	. 240	4.7	.531	. 061	5.6	88.0	. 7
15.24.57	571	.066	4.9	.575	. 129	4.8	37.6	.8
15.25.57	545	. 000	4.6	.499	. 068	0.1	07.3	. 8
15.26.57	. 5:27	.1.0	5.8	.661	. 161	3.6	87.6	. 8
15.27.57		. 170	6.4	.644	.076	3.9	86.1	.5
15.28.57	. 510	.035	5.9	.670	.055	3.5	85.1	.5
15.29.57	.5,1	.1179	6.1	.647	580.	3.8	84.0	. 6
15.30.53	.5,21	. 75	5.7	.519	. 154	5.7	82.3	.5
15.31.5%	. 4 , "	. 11-4	6.2	.545	.066	5.3	82.7	.9
15. 72.57	· 6	. 7 15 .	6.4	.661	. 074	3.6	82.5	.6
15.33.57		. 075	6.1	.711	. 1/3	3.0	82.1	• 8
15.34.57	• • • •	• 15	5.7	./15	. 1.61	2.9	37.0	. 0
15.35.57		. 11.31	5.1	20	074	4.1	37.0	.4
15.30.51	4.75	.141	2.1	+029 EAE		4.1	9.2.0	
15. 5/.5	A .: C	070	0.0	6.47	180	4 2	82 6	.0
12.0.2	4 - 7	0.04	6.4	650	230	4 2	32.3	1.0
15 40 57	4.7.1	001	6 7	.574	231	6.3	82.8	
15 41 53	En O	. 003	5.8	.597	.108	4.6	33.1	. 4
15.42.57	622	101	5.7	.573	.084	4.9	83.0	.6
15.43.57	445	. 063	6.9	.663	. 199	3.7	82.7	.9
15.44.57	4.4.4	081	7.1	.645	. 069	3.9	81.8	.7
15.45.53	535	632	5.4	.605	.081	4.4	81.9	. 7
15.46.53	. 4 : 5	.064	6.7	.564	.072	5.0	30.7	.6
15.47.53	. A :- A	. 0 . 7	6.5	.595	.076	4.6	80.5	. 8
15.48.57	. 490	. 0.53	6.1	.571	.066	4.9	80.9	.3
15.49.57	. 475.	. 100	6.5	.548	. 0.90	5.3	81.0	. 4
15.50.53	.427	.137	8.0	.558	.143	5.3	86.6	.9
15.51.53	.412	.087	6.9	.533	. 164	5.5	79.9	.9
15.52.53	. 167	. 160	6.7	.562	.091	5.1	80.1	.8
15.53.53	.497	.162	6.1	.523	. 161	5.7	/9./	./
15.54.53	.481	.887	0.5	. 700	. 182	5.0	79.0	.0
15.55.55		. 190	1.2	• 241	. 1140	2.4	77 0	2.0
17.70.7	. 46.7	. 145	7.0		040	4.0	70 2	
15 59 57	475	.050	6.5	.560	.065	5.1	79.2	.7
15 50 57	44.5	.061	6.7	.569	.055	4.0	78.8	.3
16.00.53	. 477	.054	6.5	.551	.041	5.2	78.7	. 4
16.01.53	.401	.071	6.8	505	.068	6.0	78.5	.5
16.02.57	429	nes	7.6	.517	.063	5.8	78.5	.5
16.03.53	.460	.067	6.7	.534	.044	5.5	79.1	. 4
16.04.53	.451	.154	6.8	.528	.082	5.6	79.1	.6
16.05.53	. 46.3	. 159	6.8	.525	. 154	5.6	79.2	.7
16.06.53	. 46.6	. 171	6.7	.536	.062	5.5	79.3	.4
16.07.57	. 47'	.155	6.5	.570	.073	5.0	79.7	.5
16.08.53	. 4 . 7	.108	6.9	.531	.055	5.5	79.7	.5
16.09.53	. 474	. 1.74	6.6	.551	.043	5.2	79.6	.3
16.10.5?	. 454	.054	6.9	.54?	. 1169	5.4	79.5	.8
16.11.53	. 150	.084	6.9	. 490	. 039	6.2	79.8	.5

TIME	PINCHE CHANNEL			SPAR	KER CHA	WATER DEPTH		
	AVE	SIGHA	B-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA
16.56.53	.761	. 957	9.0	.613	.251	4.8	77.8	.5
16.57.53	.350	. 093	9.5	. 472	.127	6.8	77.0	5.0
16.58.53	.346	. 695	9.6	. 494	.121	6.4	78.3	1.4
6.59.57	• 355	.122	9.3	.514	.099	5.9	77.2	5.7
7.00.53	.350	.101	9.4	.525	. 199	5.7	78.4	2.3
7.01.53	. 797	. 057	8.1	.558	.162	5.4	79.2	. 6
7.12.51	.370	.060	8.5	.468	.132	6.9	79.4	. 4
1.03.53		.072	9.7	.572	.201	5.3	79.9	.6
7.04.54		. 6.54	9.5	.574	.147	5.1	30.3	. 8
7 . 112 . 5.5		.060	9.0	.542	-143	5.6	80.8	. 6
7.110.5		. 06.9	9.7	.509	.160	6.2	81.6	.5
7 09 57	7.24	. 1 31	11.2	.515	.170	6.2	77.1	7.9
7 00 57	770	055	0.8	.500	.105	0.2	81.1	.5
7.10.57	7'.0	057	0.7	17/4	.200	2.2	. 81.0	./
7 11 53	945	065	0.5	.570	. 232	4.1	01.4	• 7
7.12 53	725	076	0.3	. 729	-140	2.0	01.7	.0
7.13.57	7.1	0.47	8.5	642	146	4.2	81 4	1.5
7.14.53	3-1	.062	9.2	.560	135	5 3	81 3	.0
7.15.57	7.45	060	9.0	.557	151	5.4	81 2	
7.16.53	. 3711	. OFF	9.9	. 493	.100	6.3	81.2	. 7
7.17.53	25.6	675	9.3	.556	123	5.3	81.3	
7.18.53	.332	. 984	9.9	.541	.142	5.7	80.7	2.7
7.19.57	. 300	. 173	9.3	.531	.129	5.7	81.3	.5
7.20.53	.340	.084	9.7	.662	.134	3.8	21.3	.5
7.21.53	.318	.185	10.3	.518	.087	5.8	81.3	.9
7.22.53	.344	. 076	9.5	.613	.175	4.6	81.4	.5
7.23.53	.722	.067	10.0	.557	.092	5.2	81.4	.6
7.24.53	.323	. 75	10.0	.566	.159	5.3	81.6	.5
7.25.53	.254	.161	11.0	.551	.117	5.4	80.6	3.3
7.26.53	.325	.102	10.1	.612	.119	4.4	81.7	. 6
7.27.53	.367	.091	9.1	.604	.110	4.5	82.3	2.6
7.28.53	.747	<u>. nen</u>	9.5	.533	.125	5.7	81.2	.3
7.29.53	. 335	. 79	9.8	.593	.103	4.7	81.8	.8
7 74 57		. 159	9.3	.566	.106	5.1	81.7	.6
7.01.20		•16r	9.8	.580	.122	4.9	80.7	5.4
7 77 57	770		9.1	.597	. 182	4.6	82.2	.7
7 34 53	767	.0/0	9.0	. 795	.124	4.7	82.0	.5
7 35 57	246	100	9.3	.4/2	.083	0.7	82.0	./
7.36 53	266	040	9.0	+ 229	.093	5.2	81.5	1.4
7.37.53	3.4	080	0.9	-4/3	.112	5.7	01./	./
7.38.53	. 767	. 072	1.5	- 520	127	5.7	01.2	2.1
7.39.53	36.9	.087	8.9	.551	144	5.5	82.1	
7.40.53	.379	.054	8.5	.527	128	5.0	82.1	. /
7.41.53	35.6	160	9.2	561	170	5 4	82 7	
7.42.53	.367	.065	8.9	. 472	.101	6.7	82 1	. 7
7.43.53	.367	.082	9.0	.503	.085	6 1	82.5	• 0
7.44.53	.359	.075	9.1	543	.095	5.4	83.0	. 4
							0 7 8 0	

DATE 0	7/21/72	FOLLEO	53

TIME	PINCEP CHANNEL			SPAR	SPARKER CHANNEL			WATER DEPTH		
	AVE	AVE SIGMA B-LOSS		AVE	AVE SIGMA B-LOSS		AVE SIGN			
17.46.53	. 787	. 160	8.4	.581	.153	5.1	83.5	.6		
17.47.53	.354	. 194	9.4	.563	.140	5.2	83.4	1.9		
17.48.57	. 353	.097	9.4	.508	.121	6.2	83.4	1.9		
17.49.53	.782	.171	8.5	.649	.062	3.8	84.0	.6		
17.50.53	. 422	. 186	7.8	.591	.125	4.8	83.4	3.2		
17.51.53	. 148	.052	7.0	.591	.136	4.8	84.4	.4		
17.52.53	. 415	.183	7.8	.628	.075	4.1	84.6	.5		
17.53.53	.363	. 194	9.1	.541	.116	5.5	84.2	3.9		
17.54.53	. 446	. 154	7.2	.606	. 066	4.4	85.9	. 8		
17.55.53	• 41) 0	.109	8.2	.608	.157	4.0	84.2	4.9		
17.56.53	.41n	. 182	7.9	.578	.140	5.0	85.2	.7		
17.57.53	. 424	.114	7.8	.603	.117	4.5	83.9	3.1		
17.58.53	. 41?	.135	7.2	.540	.122	5.5	83.5	5.6		
17.59.53	.424	.101	7.7	.60R	.139	4.5	86.8	.7		
18.00.57	. 441	.110	7.4	.491	. 178	6.3	87.7	.9		
18.01.53	. 4 7 9	.104	8.1	.507	.219	4.9	88.6	. 8		
18.01.57	.361	.009	8.6	.776	.261	2.8	89.8	1.1		
18.03.53	.269	.039	11.2	.563	.205	5.5	87.5	4.5		
18.04.53	. 392	. 096	8.4	.633	.403	5.6	85.0	1.0		
18.05.53	.751	. 185	9.1	.576	.196	5.3	82.3	. 8		
18.06.53	. 368	.093	9.0	.580	.115	4.9	86.1	2.6		
18.07.53	.712	.164	10.7	.433	.134	7.6	81.1	1.2		
18.08.53	. 372	.121	7.8	.538	.100	5.5	84.0	6.6		
18.09.53	.418	.070	7.7	.494	.114	6.3	84.9	.7		
18.10.53	.744	.119	9.9	.597	.173	4.9	84.3	4.3		
18.11.53	. 356	.097	9.3	.660	.169	3.9	57.3	.7		
18.12.5	.367	.031	8.9	.786	.217	2.6	87.9	. 8		
18.13.53	. 397	.061	8.1	.478	.175	6.9	88.3	.7		
18.14.53	.796	.070	8.2	.573	.107	5.0	80.9	1.1		
18.15.53	.378	.097	8.7	.635	.162	4.2	91.7	.7		
18.16.53	. 423	. 190	7.9	.494	.110	6.4	90.9	3.9		
18.17.53	. 391	.092	8.4	.636	.186	4.4	92.3	1.0		
18.18.53	. 444	. 165	7.2	.639	.141	4.1	92.1	.6		
18.19.53	.442	. 157	7.2	.518	.164	6.2	92.7	. 4		

			đ
DATE	07/21/72	ROLLNO 23	ļ

TIME	PINGE	P CHANN	IEL	SPAR	KER CHA	NNEL	WATER DEPTH		
	AVE STOMA 6-LOSS			AVE	SIGMA	B-LUSS	AVE	SIGMA	
19.03.53	.530	. 095	5.6	.652	.153	3.9	108.1	.8	
19.04.53	.=25	. 074	4.7	.620	.098	4.3	107.4	.8	
19.05.57	.560	.198	5.2	.597	.167	4.8	107.1	.7	
19.06.53	. 537	.122	5.6	.683	.178	3.6	106.6	. 4	
19.07.53	9	. 1 36	4.7	. 682	.229	3.7	106.7	. 8	
19.08.57	. 567	. 1.94	5.1	.676	.108	3.5	106.0	1.1	
19.09.57	.623	. 161	4.1	.638	.158	4.2	105.1	.8	
19.10.57	. 570	.073	4.8	.626	.145	4.3	104.0	.4	
19.11.57	.519	. 694	5.8	.711	.203	3.3	103.8		
19.12.53		.146	6.0	.614	.160	4.5	102.4	1.5	
19.13.53	.513	.079	5.4	.582	.140	5.0	102.4	./	
19.16.53	.556	.105	5.3	.602	.236	5.8	101.6	.5	
19.17.53	.506	.687	5.1	.671	.197	4.3	99.7	.9	
19.18.53	. 5 2 1	.128	6.0	.569	.331	6.9	98.5	.8	
19.19.57	.531	.114	5.7	.598	.074	4.5	95.3	.8	
19.20.57	. 664	. 1169	5.0	.424	.505	8.7	94.4	.5	
19.21.53	.740	.110	9.7	.295	.252	13.6	5.4	2.1	
19.22.53	.356	.122	9.2	.528	.280	7.1	99.7	1.0	
19.23.57	. 466	. 059	7.1	.297	.168	11.9	18.5	.8	
19.24.53	.760	.170	10.2	.281	.242	13.8	81.5	2.3	
19.25.53	. 7 (1 )	.1.39	8.8	.177	.101	16.4	86.7	. 9	
19.26.53	. = 4 p	7 9	5.3	.518	.493	7.5	93.7	./	
19.27.53	. 570	.081	5.0	1.038	.540	2.3	96.8	.8	
19.28.57	.532	.135	6.0	.634	.236	5.3	97.0	4.2	
19.29.57	.531	.112	5.7	.564	.199	6.1	98.4	.4	
19.30.53	. 612	.047	4.3	.442	.182	8.5	97.8	.0	
19.31.57	.601	. 140	4.3	.569	.047	4.9	96.5	./	
19.32.53	. 617	.J7P	4.3	.518	.060	5.8	95.9	.0	
19.33.57	.554	.174	5.0	.510	.121	6.4	96.1	.4	
19.34.57	. 4 1. 0	.125	4.3	.555	.138	5.8	95.8	.4	
19.35.57	.507	.129	6.2	.512	.177	7.0	95.3	2.1	
19.36.53	. 5 3 1	. 170	4.8	.57?	.148	5.5	94.8	.0	
19.37.53	. 5.23	.139	6.1	.544	.111	5.0	9.1.0	5.7	
19.38.53	. 4 6 2	.110	6.3	.617	.155	4.8	94.3	.4	
19.39.53	. 547	.1.40	5.7	.48.3	.256	8.8	92.5	2.9	
19.40.57		• 159	4.7	.644	.074	3.9	95.0		
19.41.57	.577	.126	5.2	.545	.069	2.5	72.1	5.2	
19.42.57	.= 9 9	.131	4.9	.506	.129	0.5	91.3	2.2	
19.43.53	• • • •	. 747	4.7	.575	.11/6	4.9	91./	./	
19.44.57		.144	4.6	.582	.064	4./	91.4	• 7	
19.45.57	.532	. 161	4.8	.552	.1.82	0.0	91.0		
19.46.53	. 501	. 135	4.7	.586	.100	2.4	91.8	. 4	
19.47.53	.573	.157	4.9	.464	.200	0.5	90.1	.0	
19.48.53	. 72	. 771	4.9	.572	.143	2.5	39.0	• 7	
19.49.57	34	. 1/7	5.5	.501	.150	4.2	34.4		
19.50.57	. 4 3	.104	/ . 1	.586	. 1151	4.1	30.0	0.4	
19.51.53	.515	.146	0.2	.548	.145	0.0	37.3	3.1	
19.52.57	.522	.079	5.7	.619	.072	4.2	05./	• 0	
19.53.57	.530	.050	5.7	.616	.089	4.3	08.5	.8	
19.54.57	.542	. 1.38	5.4	.533	.142	6.1	8 <sup>H</sup> • 1	.4	

TIME	PT.C.C.C.AMMEL AV- SIJMA B-LOSS		SPARKER CHANNEL AVE SIGMA B-LOSS		NIEL B-LOSS	AVE SIGM		
19.55.5 <sup>7</sup> 19.56.5 <sup>3</sup> 19.57.5 <sup>7</sup>	. 5 1 7 . 54 7 . 34 7	. 975 . 949 . 236	6.1 5.2 11.5	.506 .539 .503	.069 .202 .199	4.4 6.7 7.4	37.8 88.1 79.5	.5 .5 9.1
	-							
								and dant ingenderied
			······································					
								an an an an and
						·····		
				245				

TIME	PINGE	P CHANN	IEL	SPAR	KER CH	WATER DEPTH		
	AVE	SIGMA	B-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA
7.09.53	. 476	.073	6.5	.576	.069	4.8	81.2	.5
7.10.53	. 4717	.053	6.6	.588	.174	5.2	81.8	.5
7.11.53	.450	. 194	7.2	.574	.082	4.9	82.1	. 4
7.12.53	.502	.037	6.0	.569	.129	5.2	32.2	. 4
7.13.53	.453	.087	7.1	.602	.182	5.0	82.7	.6
7.14.53	. 497	.067	6.1	.640	.152	4.3	83.3	.5
7.15.53	. 477	.045	6.5	.700	.106	3.2	83.5	. 4
7.16.53	. 449	.143	6.4	. 629	. 095	4.1	82.2	5.5
7.17.53	.485	. 128	6.3	.566	. 087	5.0	83.5	. 4
7.18.53	.431	.114	7.7	.605	.074	4.4	84.7	4.1
7.19.53	.445	.144	6.5	.571	.125	5.1	83.1	5.4
7.20.53	.415	.101	7.3	.541	.152	5.7	83.5	5.9
7.21.53	. 455	.097	7.1	.539	.170	5.9	84 - 8	.5
7.22.53	.485	.093	6.5	.583	.131	4.9	84.7	.5
7.23.53	.465	. 196	6.9	.576	.137	5.1	85.2	.5
7.24.53	.511	. 152	5.9	.584	.058	4.7	35.3	.5
7.25.53	.478	.103	6.7	.619	.161	4.6	85.1	2.0
7.26.53	.469	.095	6.8	. 492	.134	6.5	84.7	3.5
7.27.53	.470	.093	6.8	.594	.098	4.6	85.1	.7
7.28.53	. 474	.097	6.7	.555	.089	5.2	85.0	3.3
7.29.53	.440	.161	6.7	.56?	.085	5.1	84.3	5.7
7.30.53	.498	. 164	6.1	. 606	.076	4.4	85.8	.6
7.31.53	.409	.184	6.5	.563	.105	5.1	83.2	7.5
7.32.55	.470	.033	6.7	.611	.110	4.4	36.0	.6
7.33.54	.48/	•1U <sup>R</sup>	6.4	.564	.125	5.3	85.5	.3
7.34.5	. 448	.081	7.1	.582	.114	4.9	85.0	.5
1.35.55	.485	. 070	6.4	.559	.095	5.2	86.0	.7
1.30.55	. 494	. 169	6.2	. 590	.070	4.6	85.0	.5
7 70 57	.4/4	. 184	5.6	.541	.060	5.4	86.4	.3
7.30.20	.455	•11 4	7.2	.511	.139	6.1	85.5	2.5
7 40 57	.40/	.076	6./	.633	.104	4.1	86.2	.6
7 40 . 21	.420	.144	0.9	.590	.170	5.0	84.5	5.5
7 42 57	.454	.093	7.0	.644	.283	3.4	86.8	.7
7 43 53	. 435	. 101)	1.0	.596	.135	4.8	86.7	.5
7 44 57	.4/?	.081	0.1	.638	.128	4.1	87.2	.5
7 45 57	. 4 3 3	.109	1.2	.703	.122	3.2	38.6	3.3
7 46 57	.407	.00/	0.8	.539	.113	4.0	87.2	.5
7 47 57	. 4(15	• 20-	0.9	. 79.5	.139	4.8	83.6	7.3
1.11.21	. "01	•	0.5	.011	.064	4.3	36.6	.5

TIME	PTHE	CHAN	'EL	SPAR	SPARKER CHANNEL			WATER DEPTH		
	AVE	SIJMA	B-LOSS	AVE	STGMA	3-LUSS	AVF	SIGM		
16.09.53	,533	. ^57	5.5	.707	.128	3.1	88.9	.5		
16.10.57	. 499	.144	5.5	.585	.09?	4.8	87.9	5.5		
16.11.53	.516	. 147	5.8	.585	.116	4.9	89.6	.6		
16.12.53	. 525	.1.10	5.9	.635	.105	4.1	88.6	5.0		
16.13.57	. 524	.104	5.9	.587	.133	4.9	91.2	.7		
16.14.53	.55%	. 132	5.1	. 583	.126	4.9	90.9	.4		
16.15.53	. 505	.035	5.0	.546	.117	5.5	91.8	.5		
16.16.53	.519	.154	5.2	.575	.107	4.9	91.0	5.6		
16.17.53	.578	.122	4.8	.581	.103	4.8	93.2	.6		
16.18.53	.507	.074	5.0	. 495	.162	6.6	94.2	.3		
16.19.53	.575	. 140	4.8	.636	.178	4.3	94.8	.5		
16.20.53	. 534	.11 <sup>n</sup>	5.7	.581	.124	4.9	95.2	. 6		
16.21.53	. 544	. 154	5.3	.689	.186	3.7	95.5	.6		
16.22.53	. 586	. 927	4.7	.542	.081	5.4	97.0	.6		
16.23.53	.527	. 971	5.6	.606	.198	4.9	98.1	.6		
16.24.53	.539	.110	5.3	.601	.159	4.8	97.7	.5		
16.25.53	. = 8 R	.034	4.6	.548	.108	5.4	97.9	.4		
16.26.53	.571	.071	4.9	.592	.123	4.8	98.1	.2		
16.27.53	.575	.163	4.3	.620	.235	4.7	97.4	5.6		
16.28.53	.584	. 197	4.8	.560	.112	5.2	99.5	.6		
16.29.53	. 574	.107	5.0	.597	.141	4.7	100.6	.6		
16.30.53	.535	.164	5.0	.529	.213	6.3	100.8	5.9		
16.31.57	. 641	. 145	3.9	. 678	.112	3.5	103.3	.6		
16.32.53	.622	.091	4.2	.544	.161	5.8	102.5	.6		
16.33.53	.526	.143	6.0	.710	.207	3.4	102.9	.3		
16.34.53	.598	.051	4.5	.678	.166	3.7	101.6	.5		
16.35.53	.566	.081	5.0	.706	.179	3.4	100.8	.5		
16.36.57	.544	.171	4.8	.720	.270	3.6	99.3	5.4		
16.37.53	. 465	.232	5.8	.755	.380	3.8	98.8	9.2		
16.38.57	.409	.206	9.1	. 434	.301	8.6	91.6	11.1		
16.39.57	.467	.107	6.8	. 421	.140	7.9	98.7	1.7		
16.40.53	.552	.093	5.3	.541	.332	6.7	102.0	1.0		
16.41.53	.467	.140	6.6	.849	.307	2.1	100.6	1.1		
16.42.57	. 445	. 167	3.9	.698	.309	4.1	106.0	. 4		
16.43.53	.562	.070	3.6	.610	.152	4.6	104.2	.5		
16.44.57	. 411	.275	6.5	.582	.137	5.0	99.0	8.5		
16.45.53	• <u>• • • • • •</u>	•11 <sup>n</sup>	4.6	.755	.165	2.6	105.3	.8		
16.46.57	. 461	.951	3.6	.673	.185	3.7	105.8	.7		
16.47.53	.644	.060	3.9	.702	.078	3.1	106.4	.5		
16.48.53	.636	. 1163	4.0	.677	.122	3.5	106.0	.7		
16.49.53	.450	.100	3.7	.787	.317	1.8	109.1	.6		
16.50.57	. 41, 2	. 194	4.4	. 559	.143	3.8	108.4	.5		
16.51.53	.556	.1.74	4.2	.643	.179	4.2	106.8	5.4		

DATE 07/25/72 301250 24

8

.

8
### DATE 07/2 /7" \_ ?"

. . . . . . . . . .

----

----

-----

----

----

.. . ...... ....

----

. .

- -

- --

----

. . . .

. . . . . .

. . . .

	-							
TIME	prove.	. Conste	=L	SPAR	KER CH	ALMEL.	WATER DE	PTH
	4;	ST = 1 0	S-LOSS	AVE	STGMA	H-LUSS	4VF	SIGMA
18.00.53	.6.9	7 .	6.0	. 152	.006	22.8	.30.7	. 4
18.01.57	1.0	.11	6.5	. 259	.012	24.8	97.1	.3
18.02.57	. 447	.140	0.5	. 143	. 112	27.6	09.8	5.4
18.03.53	. 4 . 7	.132	7.6	. 754	.010	25.5	39.2	5.3
18.04.57	. 1. 5	. 737	7.5	. 150	.007	26.2	37.4	10.9
18.05.53	. 6 : "	.17=	6.2	. 1:52	.016	26.1	\$7.3	10.0
18.06.53	. 417	.150	6.2	. 954	.011	25.2	d8.7	6.5
18.07.57	. 4 . 4	.137	6.7	. 15A	. 011	24.9	59.1	4.9
18.08.57	. 6 17 63	· " 55	5.5	. 164	.016	24.3	91.7	.2
18.09.57	.=14	. 1.75	5.9	. 950	. 917	23.9	91.9	.7
18.10.57	. 167	.197	6.5	. 152	.013	24.3	91.1	7.8
18.11.57	. 7. 0	.791	7.0	. 156	.012	25.2	57.6	3.8
18.13.53	. 7 15 6		4.7	. 741	. 044	12.2	94.0	. 4

1

1

• - ----

-----

-----

1.4

17.23.53	AVE							
17.23.53		SIGMA	a-Loss	AVE	SIGMA	B-LOSS	AVE	SIGH
	. P( A	.057	1.9	. 630	.198	4.4	114.7	.4
17.24.53	.774	.089	2.3	.688	.237	3.9	114.5	.7
17.25.53	.770	. 177	2.3	.629	.111	4.2	113.9	.6
17.26.53	.705	.170	2.4	.663	.195	4.1	113.0	.5
17.27.53	.762	.000	2.4	·64N	.227	4.5	112.4	
17.28.53	.730	. 197	2.7	.689	.145	3.4	112.0	.0
17.29.53	. 681	.110	3.5	.600	.252	4.1	111.1	.2
17.30.53	.720	. 181	2.9	.677	.187	3.8	111.0	
17.71.57	.7(1	.069	3.1	./11	.204	- 3.5	110.4	.7
17.32.53	. 726	.040	2.8	. /2!!	.120	3.0	109.9	.0
17.33.5*	. 491	<u>. 181</u>	3.3	./02	.154	3.3	109.5	
17.34.53	. 6/4	.109	3.0	.002	.172	3.0	108 3	
17.35.53	. 565	. 0/1	3.3	.073	122	3./	100.5	.2
17.30.53	.000	+112	3.7	- 00-	218	2.8	110.8	.7
17.37.23	701	062		. 802	176	2.2	111.1	
17.10.2	7.0	084	3.1	.916	.215	1.2	110.6	. 4
17 40 53	F.F.A	0.47		.642	.161	4.2	108.5	3.9
17 41 53	638	084	3.3	.557	.194	5.7	109.7	. 7
17 42 53	71.6	.097	3.1	.827	.165	1.8	102.4	.5
17 43 53	681	107	3.4	.637	.212	4.6	108.1	. 4
17 44 53	460	080	3.7	.650	.113	3.9	108.1	.5
17.45.53	. 412	.104	4.4	.643	.213	4.5	107.5	. 4
17.46.53	.640	. 552	3.4	.620	.178	4.6	106.8	.6
17.47.57	. 6	.069	3.7	.519	.104	5.9	196.7	.4
17.48.57	. 6.6.2	. 0.69	3.9	.560	.135	5.3	106.0	. 3
17.49.53	. 61.6	.164	4.9	.546	.165	5.7	104.2	2.9
17.50.53	. 641	. 163	3.9	.649	.130	4.0	1.04.7	.5
17.51.53	. 427	.ngn	4.2	.614	.178	4.8	103.7	.6
17.52.53	.677	. 197	4.1	. 554	.183	5.8	103.8	. 4
17.53.57	. 61.4	.154	4.3	. 623	.140	4.4	102.9	
17.54.53	. 4.31	.037	4.0	. 596	.138	4.8	101.9	
17.55.53	. 61 !!	.r7=	4.4	.581	.143	2.1	101.0	
17.56.53		.645	3.9	.704	.249	4./	101.0	
17.57.54			4.9	. 4//	.215	D.U 5 2	100 1	1 6
17.58.55		.114	2.1		150	4 7	100.4	1.0
17.59.54		· 37	4.0	570	176	5.5	100.2	
10.00.23	510	4 2 4	4.5	670	180	37	97.6	6.5
10.01.20	554	601	5 2	872	128	1.3	38.7	.5
10.02.51	5.1	134	6.0	.733	.207	3.2	98.2	.7
18 04 57	546		5.0	.552	.230	6.2	97.6	
18.05.53	552	. 6.41	5.2	.605	.141	4.7	96.3	. 4
18,06.53	547	0.50	5.3	.653	.116	3.8	95.9	.5
18.07.53	541	.034	5.0	.560	.158	5.5	95.4	. 4
18.08.53	1.6	.175	7.2	. 498	.184	6.7	91.8	8.2
18.09.57	.450	.195	8.2	.562	.180	5.6	88.6	10.2
18.10.57	F.47	.075	5.3	.569	.162	5.4	94.0	.4
18.11.53	. 4 4 4	.140	6.7	.56?	. 198	5.1	91.2	6.1

# DATE 07/1 //1 111.0 24

TIME	ot an	C - 1 1	1-1	SPAR	KEE CHA	SHEL	NATER DE	PTH
	1.V-	51.5MA	3-LOSS	AVE	SIGMA	e-Loss	AVE	SIGMA
18.13.27		• 4	5.0		.1/4	3.0	97.0	
18.14.57		.157	5.8	.014	.125	4.4	92.0	
18.15.53		. 47	5.9	. 7011	.164	2.2	97.2	
18.16.57			9.0	. 184	.122	4.5	87.4	1.5
18.17.57	• •	130	6.7	. 492	. 198	3.0	91.9	3.7
18.18.57	• * • •	.100	9.4	.507	.185	0./	36.1	8.3
18.19.57		<u>. 11</u>	. 6.5	. 417	.123	4.4		4.2
18.20.57	• 7.57	.161	1.4 . 1	.576	.139	5.4	01.5	6.2
18.21.5.	•	· * 3 *	9.7	.490	.295	3.0	54.9	8.4
18.22.53	• * . *	.107	0.5	. 546	.214	6.3	39.0	5.5
18.23.53	• * . *	.1 = 1.	8.1	.65=	.163	4.1	37.5	5.0
18.24.57	. 1 7	· 1 + 1	6.5	.400	.213	7.1	30.0	2.2
18.25.57	4.7.5	· · · · · · · · · · · · · · · · · · ·	6.7	.549	.138	5.0	09.0	.2
18.26.57	. 427	.151	8.3	.511	.176	6.5	86.7	6.7
18.27.53	. 7	.135	11.0	.557	.119	5.3	31.0	10.1
18.28.24	. 77 .	. 1	9.0	. 194	.149	0.7	84.4	7.8
18.29.57		. " 51	6.4	.480	.187	7.2	80.4	.3
18.30.57		. 1	9.5	. 537	.153	0.2	35.5	5.9
18. 1.51	. 1 . 7	. 1 , 7	7.2	. = 1 °	.168	5.4	37.8	4.1
18.72.57	. 4 7	. 1 10	7.0	. 411	.120	4.4	87.5	5.0
19.33.57	. 2 ?	. 0 .1	6.9	. 597	.146	4.9	88.2	2.1
18.34.53			7.8	.520	.163	6.2	37.2	4.7
18.35.57	. 7.7	.141	9.4	. 533	.164	5.9	82.5	9.6
18.36.57		.1/"	8.1	.572	.133	5.1	d8.1	1.5
18.37.51		. 1.20	8.4	. 534	.167	0.1	84.0	6.0
18. 78.57		.165	9.8	.= 37	.100	5.7	53.5	5.0
18. 19. 51	11:1		0.8	.564	.174	5.5	87.9	.5
18.40.53		. 7	7.6	. 494	.21P	7.3	56.6	3.8
18.41.53	. 4 . 1	. 9 - 12	7.2	.538	.172	6.0	\$5.5	5.6
18.42.51	A . 7		7.5	. 568	.228	0.1	37.8	.5
18.43.53	7.1	. 4 3 2	8.9	.547	.168	5.9	56.4	3.5
18 44.53		4 37	10.7	.911	.57C	2.1	90.6	4.7

ħ.

.

4

-----

-- -

. . . . . . .

....

TIME	PINGEP	CHAN	EL	SPAR	KEP CH	ANNEL	WATER DEPTH		
	AV =	STGMA	H-LOSS	AVE	SIGMA	B-LUSS	AVE	SIGM	
15.58.53	. 493	.147	6.7	.541	.218	6.5	81.9	2.1	
15.59.53	.530	. 101	5.6	.484	.247	8.1	81.6	2.0	
16.00.53	. = 37	.12?	5.9	.579	.141	5.2	82.9	.5	
16.01.53	.581	. 135	4.7	.645	.071	3.9	81.4	.3	
16.02.53	. 5.3.5	.125	5.9	.602	. 159	4.4	82.8	3.4	
16.04.57	.566	. 647	5.0	.250	.186	10.4	84.4	.3	
16.05.53	. 475	.175	6.1	.261	.105	10.6	93.3	5.5	
16.06.57	. 47.5	175	7.4	.149	.086	11.9	82.8	5.3	
16.07.53	.581	.134	4.8	.207	.178	3.6	85.0	.6	
16.08.57	.532	.110	5.7	.340	.213	9.5	85.3	.4	
16.09.53	.4.2 %	.143	6.7	.230	.113	11.9	85.5	.3	
16.11.57	.517	.121	6.1	.217	.160	11.4	85.7	.3	
16.12.57	. F19	.131	6.1	.299	.200	8.8	86.0	.3	
16.13.57	. 539	.126	5.5	.283	+194	10.8	86.3	• 4	
16.14.57	.525	.100	5.7	.218	.208	7.1	86.5	.6	
16.15.53	.437	.164	5.8	.958	.742	4.0	85.2	5.5	
16.16.53	.566	. 050	5.0	.609	.868	4.6	86.7	.5	
16.17.53	.503	. 179	5.3	.680	1.304	2.7	87.1	. 6	
16.18.53	. 425	.181	7.2	1.273	1.923	3.2	85.8	5.5	
16.19.53	. 4 4 4.	.191	7.0	.295	.319	4.4	85.0	6.2	
16.21.53	.505	.114	6.2	.583	.606	2.9	87.4	.5	
16.22.53	.567	. 166	5.0	.728	.804	. 8	87.6	.3	
16.23.53	.541	.187	5.5	.325	.268	5.8	87.9		
16.24.53	. 514	. 175	5.4	.484	.565	2.3	86.5	5.7	
16.26.57	.572	. 174	4.9	.594	.551	5.7	87.9	.5	
16.27.57	. 591	. 150	4.6	.491	.248	7.2	88.0	.0	
16.28.57	. 500	.1117	5.6	.349	.240	7.7	88.5	.5	
16.29.53	. 471	. 202	6.5	.698	.557	3.1	88.2	6.9	
16.30.57	.55?	.184	5.3	.250	.201	8.9	88.3		
16.31.53	.539	•127	5.6	.194	.181	6.1	89.6	4.1	
16.32.57	.= 30	.107	5.5	.565	. 433	7.3	38.9	.0	
16.33.57	.57.7	. 170	4.9	.394	.500	3.5	88.9	.4	
16.34.53	.517	.126	6.1	.414	.280	6.5	69.2		
16.35.5*	.550	.151	5.3	.294	.173	8.7	87.6	5.4	
16.36.57	. 44	•12 <sup>n</sup>	2.7	.394	. 255	8.1	89.2	2.4	
16.37.5*		. 070	5.1	. 313	.250	1.4	89.9	. 4	
16.38.57		.124	2.9	.40?	.336	5.9	90.2		
10.39.53	.507	.152	5.8	. / 93	.803	3.0	90.0	7.0	
10.41.54		.101	2.9	.502	.2.39	1.1	¥0+0	1.1	

DATE 07/77/7 " CLL"0 2?

ŕ

.

.

#### DATE 07/27/7 Telt"0 27

TIME	PT 17	C 144.15	'EL	SPAR	KEP CHA	INNEL	WATER DE	PTH	
	h.M.	STAMA	S-LOSS	AVE	SIGMA	P-LASS -	AVE	SIGMA	-
18.56.53	./:4	.173	7.1	.590	.125	4.9	84.8	3.1	
18.57.53	1 3	.192	9.2	.565	.145	5.4	82.0	6.1	-
18.58.53	17	.102	5.9	.487	.151	6.5	35.9	.4	
18.59.57	. 4 .2 **	.1 32	8.0	. 465	.178	7.4	85.1	1.5	
19.00.57	. 51 /	.135	6.2	.562	.134	5.3	84.9	.9	
19.01.57	. 51	.122	6.1	.57?	.207	5.6	83.2	4.6	
19.02.53	. 4 17	.157	7.7	.579	.166	5.3	82.1	5.4	
19.03.53	.F14	.125	6.2	.607	.056	4.4	83.6	2.7	
19.04.53	.567	.124	6.4	.539	.178	0.1	83.1	3.9	
19,05.53	. 5 4 1	. 164	5.4	. 497	.135	6.5	33.8	.4	
19.06.53	. 5. 4. 7	. Col.	5.3	.546	.141	5.6	83.6	.4	
19.07.53	. 0 3 2	.136	7.3	.587	.155	5.1	80.7	4.9	

.

1.2

. . . .

....

1

.

.

,

.

Reproduced from best available copy. 0

\_ 252

.

DATE 07/07/10	T CE	2	°	Reproduced best availab	from le copy.	C		
TIME	PT -0-	- CHAN	"EL	SPAR	KER CH	ALNET	HATER D	EPTH
	AV-	STAMA	B-LOSS	AVE	SIGMA	9-1059	AVE	SIGMA
18.33.57	. 6 2 .	. "07	4.1	.590	.155	4.9	107.5	. 7
18.34.53	. 4	. 123	3.7	.577	. 197	4.9	1.16.8	
18.35.57	. 6 20	. 157	4.1	.550	.164	4.3	1115.8	1.9
18.36.53	.157	. Oat	3.9	.579	.143	2.2	1.15.6	5
18.37.57	. 6 . 1		4.0	. 561	.167	5.6	111.7	. 6
18.38.57		. 1 1	5.3	.582	.074	1.8	1.13 4	1 6
18.39.57	1.1		4.1	. 626	155	4.7	103.1	
18.40.57		3,6	3.9	.518	.159	0.4	1.12.4	.0
18.41.57	1	157	4.2	. 5.15	155	1.0	1.1 8	.0
18.42.57	7	1 70	4.8	557	1 3 8	5.6	100.8	7 0
18.43.53	4 :	- , c	4.4	5.87	058	4 7	100 5	3.9
18.44.27	.1 .	1.1	4.4	. 670	run	4 4	100 0	• 0
18.45.57	. 6	0.4	4 5	51.4	217	7.2	12.0	• /
18.46.57			4 5	555	+ C L C	5.2	10.7	
18 47 57	= 7	. 77	4.7	6.27	4 3 7	7.7	4.1	
18 48 53			4.7	- O.	+ 1 (1 (*	3.7	3×.0	• 0
18 49 57		5.5.4	4.5		-19/	0.2	94.0	• ?
18 50 57			4.7	• • • • • • • • • • • • • • • • • • • •	• • • • 1	2.2	97.4	• 2
10.70.21		• • •	4.5		.125	4./	97.3	.5
10.21.2			2.2	. 567	.144	٤.4	95.5	5.2
10.52.5	•	• • •	4./	• 6.9 1	.130	3.4	96.3	.5
18.53.54	- 12, cl -	11	5.7	.544	.145	4.ú	95.6	1.6
18.54.54	. 74	• • • •	4.8	.755	.126	3.2	95.6	. 8
18.55.5	• = 2.7	.117	5.9	. 503	. 243	4.3	94.2	4.3
19.56.57	• /		5.2	• ~ 15	.120	4.4	95.0	.7
18.57.57	7		0.1	. 625	.201	3.4	97.6	2.9
18.58.57	. = . (	. 847	5.7	. 529	.161	4.0	94.5	. 6
18.59.57		. 1 . 5	5.7	. 759	. 280	0.6	97.5	2.7
19.00.53	• <sup>-</sup> • • •	. 1 11	5.3	. 471	.156	7.2	93.9	.5
19.01.57		. that	5.0	. 430	.107	4.1	93.6	.6
19.02.57	.F20	. 17	5.6	. 4 3 1	.213	4.7	93.5	. 6
19.03.57	100 2	. " 0 "	5.7	. 653	.125	3.8	47.0	. 6
19.05.57	. 5 1 4	· 5.54	5.8	. 224	.142	10.7	92.5	1.0
19.06.53	. 5	. 040	6.1	.143	. 047	17.3	92.5	. 9
19.07.57	. F . /.	.11"	6.2	.133	. 136	17.8	92.1	1.9
19.08.57	• E 1 3	. ^ 7'	6.0	.131	. 165	14.4	22.5	1.1
19.09.57	. 7 / 7	. " . "	5.6	.149	.061	15.2	92.7	1.4
19.10.52		. 75:	5.4	.157	. 053	10.5	22.4	1.1
19.11.57		.1.2	6.5	.151	.054	13.5	27.0	1.7
19.12.57	. 0 - =	.13=	6.5	.159	.058	16.4	32.0	.5
19.13.53	. 4 12	. 117	6.4	.145	.085	15.7	30 B	4 0
19.14.57		1000	6.1	.121	. 950	16.9	91 7	5
19.15.53	. F		5.7	.123	.028	18 4	21 2	
19.16.57	. 4	. 1	0.6	.135	. 0.30	17 5	1.0	
19.17.53			5.9	.150	1.06	15 7	20 0	• D.
19.05.53	IT S P		5 3	107	1.21	11 3		.4
19.06.53			6 2	1.7	101	1.2	3	1.0
19.07.57	A .	115	6.4	• 1 3 \	.100	17.9	7.7	1.9
19.08.53	F	1.70	6 4			17.0	92.1	1.8
			0.1		11/1	1 4 6		1 0
10 00 53	E . 7		6.1			10.0	90	1.0

£

TIME	PINCE	P CHAN	NEL.	SPAR	KER CH	ANNEL	WATER DE	PTH
	AVE	SIGMA	B-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGM
19.05.53	17	.069	5.8	.586	.244	4.1	92.2	1.0
19.06.53	.490	.192	6.2	.715	.092	3.0	92.2	1.0
19.07.53	.Eu?	.114	6.3	.574	.260	6.4	91.8	1.9
19.08.53	.512	. 75	5.9	.555	.305	6.8	92.2	1.0
19.09.53	. 523	.070	5.7	.720	.221	3.5	92.4	1.4
19.10.53	. 537	.001	5.5	.600	.237	5.5	92.1	1.1
19.11.53	.464	.107	6.6	.777	.224	2.9	91.7	1.7
19.12.53	.483	.107	6.5	.654	. 199	3.8	91.9	.5
19.13.57	.091	.117	6.6	.588	.175	5.3	90.5	4.0
19.14.53	.492	.116	6.7	. 666	.219	4.4	91.1	1.3
19.15.57	.523	. 010	5.6	. 623	.251	5.5	91.0	.5
19.16.53	. 4 . 3	.1.04	6.6	.572	.252	6.4	91.0	.6
19.17.53	.515	. 069	5.8	.587	.339	6.8	90.5	.4
19.18.53	.492	. 172	6.3	. 668	.232	4.3	90.7	.5
19.19.53	.521	. 162	5.7	.717	.143	3.1	90.4	.7
19.20.53	.470	.137	7.1	.627	.216	4.8	89.7	1.7
19.21.53	.401	.117	6.6	.442	.278	8.2	88.8	4.8
19.22.53	. 4 - 7	. 770	6.3	.626	.161	4.7	89.8	.7
19.23.53	. 491	. 1161	6.2	.531	.190	6.1	89.2	.4
19.24.53	. 519	.037	5.7	.720	.210	3.2	89.0	.5
19.25.57	. 4 ,=	. 194	6.3	.540	.201	6.2	8.86	.4
19.26.53		.162	6.1	.695	.206	3.5	88.7	.6
19.27.53	.5,31	.027	5.5	. 558	.205	4.4	88.5	.4
19.28.53	.459	. 057	6.3	. 615	.273	5.7	88.4	.5
19.29.53	.5ù?	. 144	5.9	.627	.324	5.6	88.1	.5
19.30.53		. 948	5.9	.720	. 205	3.5	87.9	.4
19.31.57	.556	. 142	5.9	.761	.280	1.9	87.4	.4
19.32.57	. 495	. 161	6.2	. 497	.241	4.1	87.3	.4
19.33.57	.512	. 124	5.8	.117	. 144	17.0	87.0	. 4

-----

.

~

3

3

ι¢.

DATE 07/27/72 POLL+0 22

. .

.

TIME	PLACE	P CHANN	EL	SPAR	KER CHAI	NNEL	WATER DE	PTH
- The	AVE	STGMA	a-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA
16.31.53	.707	.21?	2.7	.572	.164	5.4	123.4	3.4
16.32.57	.742	.191	2.7	.610	.162	4.9	124.1	. 6
16.33.53	.751	. 166	2.4	.703	.100	3.2	123.5	1.0
16.34.53	.727	.1.63	2.9	.642	.134	4.1	122.7	.5
16.35.53	.736	.673	3.1	.618	.104	4.3	122.2	.5
16.36.53	.727	.060	2.8	.650	.075	3.8	121.3	. 4
16.37.53	. 54.7	. 193	3.7	.505	.236	7.5	120.8	. 9
16.38.57	. 492	.1187	3.3	.568	.186	5.7	120.4	.8
16.39.53	· 711 *	. 150	3.1	.574	.173	5.5	119.2	
16.40.53	. 676	. 168	3.4	.469	.188	7.7	119.0	.0
16.41.57	. 679	.1190	3.5	. 66?	.085	3.7	118.3	.0
16.42.53	. 652	.111	3.7	.721	.061	2.9	115.1	•0
16.43.57	.695	.052	3.2	.630	.101	3.5	11/.5	.4
16.44.57	.651	. 167	3.5	.589	+110	4.8	11/.2	•0
16.45.57	.678	. 154	3.4	.663	.162	4.2	110.0	
16.46.53	.665	.111	3.4	.501	.182	7.1	110.0	. 7
16.47.53	.706	. 167	3.1	.513	.204	/.0	117+1	
16.48.57	. 648	. 059	3.3	. 4/2	.222	8.0	117.0	.4
16.49.57	. 694	.084	3.2	27	.101	4.1	117.5	.0
16.50.53	. 642	. 196	3.9	.570	.215	0.1	113.0	.0
16.51.57	.573	.086	3.5	. 5/1	.123	5.0	112 2	1 0
16.52.53		.112	4./	.012	.100	1 3	111.5	
16.53.5.	.537	.100	4.1	670	105	4.0	111.0	.5
16.54.53	. 590	+JUA	4.7	- 538	4 2 9	4.5	110.4	. 4
16.55.53		.123	4.3	.017	220	7.9	109.4	. 4
10.70.24	.04/		3.9	580	143	5.3	109.0	.5
10.7/.53	.023	247	7.2	588	.122	4.8	109.5	.7
10.20.20	. 640	070	4.4		.209	6.4	107.9	.5
10.39.33	610	057	3.0	. 652		3.8	107.4	. 8
17.00.01	.009	009	4 4	.545	.178	6.1	136.9	.7
17.02.57	633	0.51	4.0	.575	.214	6.2	106.7	. 8
17.02.93	633	. 1.32	3.7	. 417	.262	9.6	106.0	. 4
17 04 53	612	. 047	4.3	.667	.085	3.6	105.4	. 6
17 05.53	.612	.037	4.4	.635	.064	4.0	104.8	1.0
17.06.53	.615	.019	4.2	.465	.222	8.1	104.4	. 9
17.07.53	E Q C	. 1146	4.5	.558	.136	5.7	103.9	. 6
17.08.53	593	. 123	4.5	.592	.115	4.7	103.3	.7
17.09.53	.590	.014	4.6	. 633	.117	4.2	103.1	. 4
17.10.53	.608	. 125	4.3	.675	.093	3.5	102.5	1.0
17.11.53	.585	. 0.43	4.7	.597	.165	5.3	102.1	. 9
17.12.53	.590	.918	4.6	.519	.175	6.3	101.6	.5
17.13.53	.589	.014	4.6	.587	.226	6.0	101.2	.7
17.14.53	.570	. 024	4.8	.606	.145	4.8	100.5	.6
17.15.53	.537	. 13.7	4.7	.593	.215	6.0	100.1	.5
17.16.57	.517	. 124	4.8	.580	.221	6.2	100.0	. 5
17.17.53	.571	. 021	4.9	.59?	.195	4.7	99.7	• 5
17.18.53	.583	. 021	4.7	.530	.186	4.9	99.4	. 8
	E 10	000	A (	554	075	5 2	99.0	. 6

¢

.

L

						-		
TIME	PT De		el	SPAR	KER CHA	NNEL	WATER DE	тн
	AV-	\$13" 4	A-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA
17.21.53		. 052	5.3	. 575	. n 9 3	3.5	99.1	.8
17.22.53	5.55	.027	5.0	.485	.220	5.2	99.0	. 4
17.23.53	F.O.T	.197	6.0	. 505	.226	5.9	96.5	3.5
17.24.53	E . 1	. 0 ; 0	5.2	. 558	.210	0.1	97.3	.8
17.25.57	= 7 1	. 0.4.4	4.9	. 525	.198	5.0	97	.7
17.26.53	F 7	.025	5.1	. 332	.207	5.8	95.4	. 8
17.27.53	544	. 0:27	5.3	.561	.221	6.5	94.2	.5
17.28.57	= 47	. n.1=	5.3	. 536	.253	7.5	96.0	.8
17.29.53	= 2 2	. * 54	5.4	. 50	. 263	7.2	95.4	. 8
17.30.57	547	26	5.3	. 528	.202	7.5	95.5	.5
17.31.57	.5.1.5	.1.0	6.1	. 516	. 768	4.3	94.8	1.2
17.32.57	F ( 1	. 0 5 -	5.5	.560	.200	6.1	94.7	1.0
17.33.57		.051	5.7	. 615	.158	5.0	94.5	
17.34.57	5.53	• " > c	6.1	. 428	.267	13.5	94.6	.7
17.35.57	. 254	. 944	5.6	.556	.204	6.2	8. FQ	.8
17.36.53	.F_4	. 1 4 1	5.8	.236	.275	17.3	97.7	.6
17.37.57	. 6 95	.137	7.4	. 534	.190	0.5	94.5	3.5
17.38.53	.4.7	75	6.2	.524	.205	6.9	93.6	.7
17.39.53	. 4 2 4	.117	6.5	. 686	.051	3.3	92.3	2.6
17.40.53	. 61 3	. 175	6.1	. 46ª	.208	8.1	93.3	.7
17.41.53	. 191	. 197	6.4	.563	.218	6.6	91.7	5.0

)

TIME	P1 0-1	CHVA.	EL	SPAR	KEP CH	ANNEL	WATER DEPTH		
	AVē	SIGMA	8-LOSS	AVĚ	SIGMA	8-LOSS	AVF	SIGMA	
19.21.53	. 497	.167	3.5	.426	.211	8.9	111.3	.6	
19.22.53	. 6 , 2	.713	2.9	.46C	.14?	7.0	109.7	5.4	
19.23.57	.735	.125	2.8	.539	.141	5.9	110.5	.7	
19.24.53	. 4 47	.156	3.5	.554	.074	5.2	110.9	3.9	
19.25.53	.543	. 711	4.4	.504	.171	6.7	105.8	7.1	
19.26.53	.762	· P51	2.4	. 596	.118	4.7	1.08.5	.5	
19.27.53	.717	.165	3.3	.510	.086	6.0	136.9	3.3	
19.28.53	.729	.117	2.9	.516	.115	4.4	107.5	1.2	
19.29.53	. 6.54	.160	3.0	. 507	.075	4.4	105.4	<u>1./</u>	
19.30.5	. / 4 .	. 16 /	2.0	. 728	.191	0.3	100.2		
19.51.53		- 211	2.9	+ 10/		3.0	104.3	5.2	
19.32.5			5.0	. 701	.100	5.5	104.5	0./	
19.3.5		0.70		+707	.131	<u> </u>	101.7	/.0	
19.34.51	• / ] *	• 17 / 1	2.9	+ 707 507	+140	4./	107.0	. 7	
19.00.00		- 100	2.9		474	· · · ·	100.0	• • • • • • • • • • • • • • • • • • • •	
19.30.21	0 8	0.45	3.2	619	145	4 9	102.7	. / 7	
19.07.53	734	047		.622	10	4.2	102.02		
10 30 57	7.52	044	2.5	. 667	080	3.6	101.6	.5	
10 40 53	675	207	3.1	.643	184	4.6	99.7	5.2	
10 41 57	674	.213	3.7	. 566	.004	3.6	101.4	5.2	
19.42.57	AU	.0.50	3.2	.574	.142	5.1	100.0	.5	
19.43.53	. 630	197	4.7	.539	.186	0.1	97.9	4.6	
19.44.57	.674	131	3.7	.665	. 197	3.0	98.9	.7	
19.45.57	. 6 - 1	.057	3.4	.500	.210	5.8	99.6	.6	
19.46.57	. +7=	. 056	3.4	.662	.091	3.7	98.4	.5	
19.47.53	.6:2	.169	4.5	.543	.149	5.7	97.0	4.6	
19.48.57	. 420	.190	3.8	.541	.211	0.7	96.2	5.7	
19.49.57	. 475	. 61	3.4	. 591	.177	5.4	97.3	.9	
19.50.53	. 627	.180	3.7	. 427	.264	10.4	95.7	5.4	
19.51.57	. 465	. 149	3.6	. 462	.109	3.7	96.8	.4	
19.52.57	.631	.13=	4.3	. 564	.227	6.4	96.7	.7	
19.53.57	.51.6	. 242	4.9	. 430	.279	9.4	97.2	7.1	
19.54.57	• <b>5 5 1</b>	.199	5.9	. 385	.257	11.2	97.9	5.0	
19.55.57		.14?	5.0	.651	.182	4.5	95.9	.5	
19.56.57	. 7 7	.170	4.4	.657	.104	3.8	94.3	5.3	
19.57.53		140	4.8	. 582	.231	10.4	93.9	5.5	
19.58.54		• • • • •	4./	.512	.725	1.3	95.3	.9	
19.59.55			4.8	- 491	.101	0.8	97.2	.8	
20.00.5		•102	5.2	.740	./11	0.0	97.0	1.0	
20.01.5		•1/1	0.2	.771	121	2.1	90.2	4.2	
20 07 57	547	140	4./	· C 4 C	155	4.7	94.5	.0	
20.04 57	- 4 F 4 C	127	5.0	565	227	5 0	94.2	5.4	
20.05.53	FOR	.116	4.6	. 620	.177	4.8	24.2		
20.06.53	5.07	148	5.0	.624	.137	4.3	32.6	5.1	
20.07.53	550	.111	5.2	.503	.199	7.1	93.6	.8	
20.08.53	.641	.062	3.9	.614	.082	4.3	93.6	.7	
20.09.53	F41	045	3.8	. 523	.167	6.4	93.2	1.2	
00 40 57	P . P				0.04	4 7	00.0		

DATE 07/28/73 2.11 L 31

.

Ł

2

257

.

	standard in and ages - and unders	Contract of the local division of the local		10 mm m
DATE	07/2-11		1	11

TIME	PT TO PART	nt	SPAR	KER CH	ANNEL	WATER DE	PTH
	AM- 513"4	3-LOSS	AVE	SIGMA	H-LOSS	AVE	SIGMA
			· ··· ·· ····				
17.51.53	.F 44 .136	5.8	.570	.153	5.3	90.6	4.4
17.52.53	. 417 .107	4.4	.580	.058	4.6	97.5	5.2
17.53.57	. 514 .131	5.4	.527	.201	7.0	91.4	1.3
17.54.57	· · · · · · · · · · · · · · · · · · ·	4.5	. 591	.164	5.3	91.5	.6
17.55.57	• · · · • · · · · · ·	5.2	.57?	. 088	5.0	91.5	.9
17.56.53	. 110 .142	6.6	.532	.146	6.2	80.0	5.9
17.57.53	• · · · · · · · · · · · · · · · · · · ·	5.0	.575	.149	5.6	91.1	.6
17.50.54	. /	4.9	.561	.151	5.7	90.6	. 6
17.79.2	<u>• • / • • • • • • • • • • • • • • • • •</u>		•174	•177	18.0	89.1	3.5
10.00.0	• 20 • 19 °	5.8	. 743	.110	5.5	90.6	. 4
10.01.5		4.9	. 734	.105	5.0	91.6	. 4
19 03 67	• 1 • 1 ) •	0.1	5/	. "62	2.0	80.8	1.4
18 04 57		0.0		- 199	2.8	05.5	7.2
18 05 57	F/1 440	5.0	.493		9.4	89.9	.5
18.06.57	E.Z. 120	5 7	. 745	·152	0.0	84.5	5.4
18.07.57	F.7 1.9	5.1	.027	1154	4.1	84.4	1.2
18.08.53		6.7	525	+112	2.0	1 1 1	1.7
18.09.57	510 113	5 4	602	049	4.5	20 5	5.0
18.10.57	.4.7 .140		605	006	4.4	07.5	./
18.11.53	617 154	5.6	547	154	4.5	88 2	1.2
18.12.53	= 77 - 075	4.9	.567	047	5.0	88 7	3.0
18.13.53	EA7 122	5.6	.500	192	7 3	37 4	7 4
18.14.53	547 547	5.5	.551	.143	6.0		
18.15.57	.178 .151	7.1	.552	153	5.0	85 4	A 3
18.16.53	E 10 1157	4.5	591	.043	4.6	89.1	4.5
18.17.53	.44.5 .104	7.7	.507	198	7.4	85.7	5.3
An annual controller in a linear state provide the second							5.5
	and the second	545 S 10					
		** ** * ·*·	-				
					and a second		
		-					
				· · · · · · · · · · · · · · · · · · ·			
				·			
	· · · · · · · · · · · · · · · · · · ·						
			·				
			1.1				
manage Bloom							
					•		and the second second
		· · · · · · · · · · · · · · · · · · ·	0.10 MBB 80 18 80 00 41	a	•		
				••			
		25	8				

)

TIME	PINCE	P CHANN	EL	SPAR	KER CHA	NNEL		PTH
	AVE	SIGMA	B-LOSS	AVE	SIGHA	B-LOSS	AVE	SIGHT
20.11.53	. 609	.131	4,6	.545	.183	5.9	92.6	1.0
20.12.53	.534	.127	5.7	.515	.214	6.9	91.3	5.3
20.13.53		<u>.10n</u>	5.1	.547	.179	6.0	92.4	.7
20.15.53	540	.130	5.5	. 78.3	.107	4.8	92.4	.3
20.16.53	549	.126	5.5	.471	.231	8.6	90.5	3.0
20.17.53	.564	.168	4.5	. 489	.165	7.1	90.0	5.7
	,							

DATE 08/01/70	CILL	MO 32						
TIME	PIACE	D CHANN	EL	SPAR	KER CHA	INNEL	HATER DE	РТН
	AVE	51644	3-LOSS	AVE	SIGMA	B-LASS	AVE	SIGMA
17.17.53	.761	. 169	3.1	.606	. 094	4.4	124.7	.4
17.18.53	. 629	.230	3.9	.470	.181	7.5	122.6	5.2
17.19.53	. 6.94	.070	3.3	.584	.157	5.3	123.2	.6
17.20.53	. 477	.105	3.5	.649	.112	3.9	122.5	1.6
17.21.57	. 6 4 7	.105	3.7	.466	.184	7.6	122.0	.9
17.22.53	.747	.121	3.2	.623	.173	4.9	121.4	1.1
17.23.53	.FAP	.143	3.8	. 662	. 171	3.6	120.0	2.1
17.24.53	.634	.139	3.8	.581	.13?	4.9	119.8	.0
17.25.53	.640	.100	3.4	. 557	.189	3.7	110.1	
17.26.53	. 673	. 059	3.5	.60A	.171	5.2	118.4	.8
17.27.53	. 676	.157	3.4	.015		4.3	11/./	.0
17.28.57	.641	-115	4.0	.585	.168	2.4	110.0	. 4
17.29.57	7	. 140	3.4	.5/9	.138	2.0	115 5	
17.30.5*	/ .	.135	3.4	30	.104	4.0	110.0	1.4
17.31.54		.101	4.0	.79.	.100	4./	113.0	1 0
17.32.54		. 1, / 4	4.0	.405	-210	6.0	113 2	1.0
17.33.5	. 0.4 /	057	3.9	20	248	7 5	112.5	
17.34.2		477	3.7	482	160	7 1	111.4	1.7
17.02.2	4 12 13	073	4.1	626	165	4 9	111.1	.7
17 77 57	645	075	3 0	.591	. 973	4.6	110.4	. 8
17 38 53	635	771	4.0	.555	139	5.5	109.9	.8
17.39.53	. 616	.121	4.5	.549	.153	6.0	109.0	2.2
17.40.53	. 624	.080	4.2	.600	.153	5.2	109.6	1.1
17.41.57	.6.7	.140	4.8	.541	.201	6.8	106.8	4.5
17.42.53	.6.3	. 071	4.7	.577	.170	5.6	107.7	1.4
17.43.53	. 672	.1343	3.5	.601	.061	4.5	107.0	.7
17.44.53	.622	. 120	4.1	.589	. 146	4.6	105.9	. 6
17.45.53	.65=	. 157	3.7	.519	.156	6.5	105.8	.9
17.46.53	. 417	.134	4.6	.502	.135	6.7	104.1	4.7
17.47.57	. 6.1.0	.156	4.0	.509	.142	6.6	103.2	4./
17.48.57	. 470	.024	3.4	. 525	.19/	/.1	104.2	1.2
17.49.5	.6.51		4.1	• 51 /	. 173	4.2	103.0	
17.50.51	• • • • •		3.8	. 491	.]8/	1.4	103.0	.0
17.51.5			4.2	. 704	.10/	0.9	102.4	
17 55 57	6.00		3.0	. 401	- 620	0.9	1.10.6	1.1
17 56 57	6.15	177	3.9	. 285	.214	13.0	100.1	
17 58 57	6 7	120	4 7	195	.162	16.1	V8.5	2.2
17 53 53		1 27	4.1	148		17.8	100.0	5.9
17.53.53	672	.621	3.4	.543	.054	5.3	101.6	.3
17.54.57	. 675	. 142	3.5	.526	.117	5.8	101.2	.5
17.55.57	. 475	.021	3.4	.531	.140	5.3	100.6	.7
17.56.53	ALU	. 132	3.6	.484	.189	7.6	100.1	.5
17.57.57	. 6: "	027	3.6	.521	.202	7.2	99.8	1.0
17.58.53	. 1.11	.137	4.3	.500	.204	7.4	9A.5	2.2
17.59.57		. 227	3.6	.479	.131	7.1	99.7	. 4
18.00.57	. 445	. 93=	3.9	.561	.121	5.3	95.4	.4
18.01.57		.125	4.6	.531	.154	6.1	97.5	2.6
19 02 67	1	127	4 .1	. 408	102	0 -	97.6	.7

	PLICE	CHANN	FI	SPAR	KER CHA	NNEL	WATER DE	PTH
	AVE	SIGMA	H-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGM
18.03.53	. 617	. 142	4.2	.542	.174	6.3	97.3	.5
18.04.53	.6.3	.043	4.4	.549	.116	5.4	97.0	5.4
18.05.53	.60°	. 116	4.0	.503	.177	7.0	95.5	2.7
18.07.53	.557	.062	5.1	.520	.189	7.0	95.8	.9
18.08.57	. 5 h. F	.029	5.1	.603	.080	4.5	95.7	1.1
18.09.53	.51.0	.030	5.2	.468	•221	8.5	95.2	1.2
18.10.7	. E 5.4	.071	5.2	.537	.134	5.9	94.7	.5
18.12.57	51,8	.16-	6.0	.573	.057	4.9	94.9	1.5
18.13.57	. 520	.113	5.9	.502	.188	7.5	93.5	5.0
18.14.57	.519	.127	6.1	. 570	.204	6.7	93.4	2.3
18.16.57	.541	.107	5.4	.522	.178	6.8	94.0	. 9
18.17.53	.526	.080	5.7	.288	.221	14.1	93.9	1.3
18.18.57	.516	.110	6.3	.442	.165	8.3	93.8	. 8
18.19.57	. 445	.141	7.6	.500	182	7.5	93.3	1
18.20.21	. 735	.110	6.9	.509	.148	6.7	93.0	1.1

TIME	PLEE	E CHA'	EL	SPAR	KER CH	ANNEL	WATER D	FPTH
	AVr	SIGMA	B-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA
18.51.57	.537	.180	6.2	. 458	.122	7.3	89.9	5.7
18.52.5*	. 437	.215	8.5	.475	.229	8.8	87.8	8.4
18.53.57	.5t <sup>r</sup>	.174	4.5	.303	.239	13.7	91.8	7.0
18.55.53	.500	•21ª	4.8	.135	. 088	18.6	91.2	7.0
18.56.53	. 600	.130	4.8	.105	.027	19.8	94.2	.8
10.5/.53	.550	.181	5.9	.108	.035	19.7	91.4	6.4
10.77.7		.216	4.8	. 367	.188	11.0	91.4	6.2
10.70.7		+12°	4.7	.544	.146	6.2	94.2	.8
18 58 57		.150	5.6	.520	.136	6.3	91.4	6.4
		.192	0.9	. 542	.25?	13.1	93.2	4.0
9.00.53	4,3	•15"	2.9	. 185	.186	10.3	92.7	5.7
9.01.57	F 11	210	2.0	.414	+195	9.7	90.3	9.5
9.02.53	554	r G 1	4.9	•	•/21 245	11.5	91.0	8.7
9.03.57	515	193	5.5	1421	.247	10.5	93.3	5.1
9.04.53	652	.171	4.7	. 324	242	12.0	97.0	5./
19.05.53	. 460	.186	7.5	.257	.197	14 7	22.8	7.0
9.06.57	.492	.211	6.1	252	.214	14.9	92.0	6.0
9.07.53	. 44+	.226	6.1	.241	.155	14.4	91.4	8.3
9.08.53	.463	.263	7.1	.212	.191	17.1	91.1	7.7
9.09.57	.401	.231	7.4	.353	.225	12.0	87.4	10.9
9.10.53	. 484	.226	5.4	. 322	.248	13.8	90.6	9.9
9.11.53	.4111	.292	5.8	.436	.129	8.1	0.86	12.0
9.12.53	.FeR	. 192	5.0	.302	.238	14.1	95.5	.5
9.13.53	.382	.282	6.2	. 361	.235	12.0	3P.7	10.0
9.14.5		•176	6.2	.306	.20	13.2	94.6	4.6
9 16 53	450	.125	4.8	.246	.197	15.1	96.3	.8
9.17.53	.452	. 204	2.2	.301	.07	13.4	92.3	8.7
9.18.53	410	244	2.1	415	.193	9.3	94.6	6.7
9.19.53	529	177	5 1		.700	10.5	93.2	8.3
9.20.57	535	,217	5.4	362	212	11.9	97.4	6.4
9.21.53	. 572	.142	5.3	.288	214	13 0	95.0	5.7
9.22.53	. 472	.265	4.8	.180	155	17 5	03.2	2.1
9.23.53	.740	. 3 (14	5.1	.198	.184	17.5	87.5	12 8
9.24.53	.505	.215	5.9	.166	.151	18.3	95.1	8.8
9.25.57	.531	.190	5.3	.260	.219	15.0	96.0	6.2
9.26.53	. 341	.719	5.5	.379	.180	10.5	38.8	11.4
9.29.57	.471	.267	5.1	.221	.184	13.7	23.3	13.1
9.30.53	.537	.234	4.5	.192	.208	18.1	96.2	9.7
9.31.5*	.378	.292	6.5	.103	.059	20.8	90.2	12.4
9.27.53	. 61.6	.192	4.0	.226	.175	15.3	97.4	6.2
0 20 57	.634	.141	4.2	.352	.231	12.3	99.4	. 9
0 30 57	· - 2 h		4.3	.339	.241	12.1	95.4	8.7
0 14 57		. 753	3.8	. 464	.178	8.1	96.5	8.9
9.32.53	.402	• 175	4.6	. 425	.189	9.2	93.5	11.1
9.33.53	574	160	4.0	.158	.128	18.5	98.7	7.2
9.34.53	547	174	4.4	.318	• 192	12.7	99.3	6.0
9.35.53	4.1	244	7 9	.785	.231	14.4	99.5	5.9
	•		1.0	. 704	. 102	1.1	95.5	8.7

- 2

TIME	ET. 7-	L CHANN	-C)	SPAR	KEP CH	ANDEL	HATER DE	PTH
	AV-	SIGMA	H-LOSS	AVE	SIGMA	3-LOSS	1VE	SIGMA
19.36.57	. F. A. F	.172	5.9	. 476	.148	7.1	101.6	5.0
19.37.57	. 4 / 1	.285	6.0	. 378	.224	11.2	96.0	9.7
19.38.53	.444	.755	8.6	.247	.209	15.6	97.3	12.1
19.39.53		.711	5.1	.331	.735	13.0	19.2	10.9
19.40.53	/	-23.			221	10.5	1.12.7	10.1
10 42 57	F-10	. 217	4.8	. 411	.238	10.4	101.6	8.1
19.43.57	FUT.	.192	4.7	.460	.170	8.2	104.9	.7
19.44.57		.167	5.6	. 352	.206	11.5	104.3	4.8
19.45.57	.312	262.	5.3	. 378	.249	12.0	95.8	12.0
19.46.57	.427	.754	6.0	.49?	.140	7.0	109.1	12.9
19.47.57	. 47	.249	5.7	. 468	.175	8.2	102.9	9.8
19.48.57			4.8	.459	1/5	8.5	104./	11 7
19.49.5	• 4 ;	054	5.2	125	.190	12 5	100.5	1.2
10 51 53		151	5.1	. 420	.241	10.2	108.9	1.6
19.52.57		.310	4.5	. 465	.175	7.8	105.5	9.1
19.53.57	.5.71	.752	4.3	.416	. 267	11.0	107.1	9.0
19.54.57	. 5 ; 4	. 207	5.0	.470	.171	7.7	107.8	8.7
19.55.57	.4.0	· 24K	7.1	.448	.236	9.4	111.5	.7
19.56.57	· ^ · /	. 251	5.5	.438	.201	8.8	109.4	7.1
19.58.5	. 47	.2/5	6.0	-104	-135	15 4	105.7	11.0
20.00.57	7 . 1	75.0	10.9	2.118	5.407	4.5	94.5	18.7
				· · · · · · · · ·				

I I ME	PI Ge	C CHANA	EL	SPAR	KER CH	ANNEL	WATED DE	DTH
	AME	STGMA	B-LOSS	AVE	SIGMA	3-L035	AVE	SIGMA
8.31.53	. 470	. 1.46	4.1	. 490	.171	7.0	01 0	1.0
8.32.57	.550	.192	5.2	.344	.245	12.7	01 3	1.0
8.33.57	. Kid	.035	4.4	.410	.257	11.0	91.7	.7
8.74.57	.=74	. 159	4.3	.441	.227	9.6	91.4	.4
8.35.53	.577	. 000	4.8	.514	.181	7.0	91.3	.7
8.30.54	.530	.100	5.7	.384	.220	10.7	91.2	.6
8 79 57	.547	•116	5.5	.390	.224	10.7	69.8	5.1
8 30 57	• 7 4 F	.124	5.6	.489	.192	7.8	89.9	5.5
8.40.53	5:7	1.10	4.5	•440	.180	8.3	91.3	1.0
8.41.53	. 4 + -	. 204	2.0	.404	.212	8.9	92.2	3.3
8.42.57	545	.051	4.5	380	243	10.1	86.4	8.2
8.43.53	. 4	.15=	6.7	.204	.170	16.7	39 3	.7
8.44.57	. 570	. 193	4.9	.466	.224	8.7	90.7	0.3
8.45.57	. 5 + ?	.051	5.0	.541	.184	6.4	90.3	.9
								*

AVE         SIGWA         B-LOSS         AVE         SIGMA         G-LOSS         AVE         SIGMA           16.55.57         .977         .156         1.3         .649         .073         3.6         122.0         .9           16.56.57         .91         .120         1.2         .650         .095         3.8         121.3         .9           16.57.53         .617         .070         3.4         .626         .105         4.2         120.3         .8           16.59.57         .641         .709         3.3         .592         .112         3.8         110.3         .7           15.59.57         .641         .137         1.6         .502         .294         3.2         114.4         .6           17.01.57         .724         .119         2.7         .602         .094         3.7         117.5         1.0           17.02.53         .727         .777         .76         .116         .51         .774         .131         14.5         .9           17.04.57         .754         .142         .22         .651         .115         .4.5         114.0         11           17.05.57         .744         .097	1 1 1 1 1	PINE	P CHALN	EL	SPAR	KER CHA	NNEL	WATER DE	PTH
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		AVE	SIGMA	B-LOSS	AVE	SIGMA	ů-LOSS	AVE	SIGM
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	16.55.57	. 27.7	.156	1.3	.649	.073	3.8	122.0	.9
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	16.56.57		.120	1.2	.650	.095	3.8	121.3	.9
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	10.57.53		. 170	3.4	.020	.105	4.2	120.3	.8
10       10 <td< td=""><td>16 59 57</td><td>P 6</td><td>137</td><td>3.3</td><td>568</td><td>-102</td><td>3.0</td><td>119.3</td><td>. /</td></td<>	16 59 57	P 6	137	3.3	568	-102	3.0	119.3	. /
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	17.00.57	730	.110	2.7	. 662	. 194	3.7	117.5	1.0
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	17.01.53	961	.101	1.4	. 619	.205	3.4	116.4	.5
17. n3.53       .747       2.8       .555       .178       4.3       114.5       .9         17. n4.57       .749       .144       2.2       .611       .115       4.5       114.0       1.1         17. n5.53       .729       .144       2.2       .611       .115       4.5       114.0       1.1         17. n5.53       .729       .142       2.5       .592       .111       4.7       113.2       .7         17. 06.53       .729       .098       2.9       .626       .073       4.1       112.6       .5         17. 07.57       .774       .097       2.4       .623       .090       4.2       111.9       .8         17. 08.57       .764       .092       2.2       .641       .066       3.9       110.9       .7         17. 09.53       .777       .060       2.3       .624       .980       4.2       110.1       .9	17.02.53	.7.17	. 777	2.7	.629	. 176	4.1	115.7	.9
17.04.57 .705 .144 2.2 .611 .115 4.5 114.0 1.1 17.05.53 .751 .62 2.5 .599 .111 4.7 113.2 .7 17.06.53 .729 .008 2.9 .626 .073 4.1 112.6 .5 17.07.57 .74 .097 2.4 .623 .090 4.2 111.9 .8 17.08.57 .7.4 .052 2.2 .641 .066 3.9 110.9 .7 17.09.53 .770 .060 2.3 .624 .088 4.2 110.1 .9 	17.03.53	.745	.147	2.8	.555	.178	4.3	114.5	.9
17.05.53       .751       .462       2.5       .592       .111       4.7       113.2       .7         17.06.53       .722       .498       2.9       .626       .073       4.1       112.6       .5         17.05.53       .727       .498       2.9       .626       .073       4.1       112.6       .5         17.05.57       .744       .097       2.4       .623       .090       4.2       111.9       .8         17.08.57       .784       .092       2.2       .641       .066       3.9       110.9       .7         17.09.53       .772       .560       2.3       .624       .088       4.2       .10.1       .9	17.04.5*	.792	.140	2.2	.611	.115	4.5	114.0	1.1
17.06.53       .722       .998       2.9       .626       .073       4.1       112.6       .5         17.07.57       .744       .097       2.4       .623       .090       4.2       111.9       .8         17.08.57       .744       .097       2.4       .623       .090       4.2       111.9       .8         17.08.57       .744       .097       2.4       .623       .090       4.2       111.9       .8         17.08.57       .764       .92       2.2       .641       .066       3.9       110.9       .7         17.09.53       .772       .060       2.3       .624       .088       4.2       110.1       .9	17.05.53	.751	.1.82	2.5	.592	.111	4.7	113.2	.7
17.07.52       .774       .097       2.4       .725       .190       4.2       111.9       .8         17.08.57       .774       .097       2.2       .641       .066       3.9       110.9       .7         17.08.57       .777       .061       2.3       .624       .088       4.2       .10.1       .9         17.09.57       .777       .061       2.3       .624       .088       4.2       .10.1       .9	17.06.53	.727		2.9	. 626	.073	4.1	112.6	.5
17.00.5     .764     .797     2.2     .741     .005     3.9     110.9     .7       17.09.53     .775     .661     2.3     .624     .088     4.2     110.1     .9	17.07.54	. / + 4	. 197	2.4	. 625	.190	4.2	111.9	.8
	17 00 53	.764	060	2.2	+741	.000	0.9	110.9	./
		• • • •							

"

TIME	PINGE	P CHEWN	IEL	SPAR	KER CHA	NNEL	WATER DE	PTH
	AVE	SIGMA	B-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGM.
17.21.53	.961	.199	1.1	.713	. 093	3.0	103.2	.2
17.22.53	.91?	.164	.9	.704	.130	3.2	102.5	. 4
17.23.53	.749	.188	2.8	.702	.103	3.2	102.1	.9
17.24.53	.745	.081	2.6	.695	.085	3.2	101.5	. 6
17.25.53	.0(3	.185	2.3	.624	.131	4.3	100.1	5.4
17.26.53	.216	.149	1.8	.757	.129	2.5	101.1	.7
17.27.53	.792	.062	2.2	.711	.103	3.1	109.3	. 4
17.28.53	. 953	.200	1.7	.740	.243	2.9	100.3	.5
17.29.53	. 152	.116	1.5	.654	.214	3.0	100.0	.5
17.30.57	.748	.071	2.6	.660	.095	3.7	99.6	. 4
17.31.53	.779	.062	2.2	.719	.076	2.9	99.6	. 8
17.32.53	. 832	1650	1.6	.668	.178	3.9	99.2	. 9
17.33.53	.760	.156	2.7	.640	. 058	3.9	. 98.4	2.7
17.34.53	.876	.036	1.2	.673	.137	3.7	98.7	.5
17.35.53	. 0 / 7	.050	1.5	.629	. 193	4.1	98.4	. 6
17.36.53	. 7	.16 <sup>m</sup>	2.4	.740	. 193	2.7	97.8	1.7
17.37.53	. 17	.055	1.8	.687	. 090	3.3	97.9	.5
17.38.53	. 715	.226	2.6	.532	.182	6.3	96.0	5.2
17.39.57	.750	.157	2.8	.688	.143	3.5	96.6	1.2
17.40.33	.724	. 744	2.1	.686	.103	3.4	96.9	. 8
17.41.53	.7.52	.082	2.8	.701	.102	3.2	96.9	. 8
17.42.53	.660	.137	4.0	.713	.140	3.1	94.0	5.4
17.43.53	. 4 4 0	.100	3.2	.682	. 194	3.4	95.9	.5
17.44.53	.750	. 170	2.5	.744	. 191	2.6	95.8	. 7
17.45.53	. 4.85	.147	3.5	.690	. 193	3.3	95.4	. 6
17.46.57	.744	.12?	2.7	.672	.090	3.5	95.3	.6
17.47.53	. 6 . 7	.141	3.7	.627	.074	4.1	93.7	4.7
17.48.53	.769	.091	3.1	.663	.135	3.8	95.1	. 3
17.49.57	.7(~	-08r	3.1	.675	.115	3.5	94.8	.9
17.50.53	.740	.070	2.6	.592	.146	4.8	94.7	.5
17.51.53	.724	. 264	2.8	.708	. 0.91	3.1	94.7	.5
17.52.53	.74?	. 67	2.6	.645	.128	4.0	94.9	1.0
17.53.53	.739	.101	2.7	.642	.102	4.0	95.4	1.0
17.54.53	. 641	.160	3.6	.647	.118	3.9	94.2	4.6
17.55.57	. 404	.154	3.6	.652	.078	3.8	94.3	4.9
17.56.57	.714	.084	2.9	·67R	.092	3.4	95.5	. 8
17.57.57	.Frf	.215	4.1	.601	.194	4.5	93.3	4.9
17.58.57	.747	. 191	2.6	.695	.065	3.2	95.0	. 6
17.59.5*	. 715	.141	3.2	.718	.173	3.1	94.7	1.1
18.00.57	.720	.131	2.8	.657	.090	3.7	94.7	1.0
18.01.57		. 21 C	5.3	.684	.068	3.3	91.4	6.2
18.02.57	• • • • •	.141	4.3	.722	.073	2.9	93.6	. 8
18,03,57	. 7	~ 17	3.1	7 1 1	0.87	2.7	93.5	1 1

1

4

1

DATE 08/12/72 20110 35

266

•

i.

-

TIME	PTIC	C CHAN	EL	SPAR	KER CHA	NMEL	WATER DE	PTH
	AVL	SIGMA	B-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA
18.51.57	.77~	.109	2.3	.741	.104	2.7	95.7	1.2
18.52.53	.719	.241	2.9	.665	.130	3.7	93.1	7.2
18.53.53	.771	. 091	2.3	.650	.089	3.8	95.9	. 5
18.54.53	.652	.167	4.2	. 688	.122	3.4	96.5	1.2
18.55.53	.715	.161	3.3	.677	.083	3.4	95.1	5.2
18.56.57	.633	.145	3.5	.655	.111	3.8	97.0	.5
18.57.53	.685	.184	3.7	.578	. 093	4.9	97.3	.8
18.58.53	.569	.300	4.6	.566	.096	5.1	94.4	7.4
18.59.53	.765	.190	2.4	.650	.122	3.9	97.2	1.1
19.00.53	. 426	.237	4.0	.633	. 188	4.1	96.0	5.5
19.01.53	. 6 50	. 222	4.6	.654	.141	3.9	96.5	5.0
19.02.57	.611	. 3.2.6	5.4	.619	.102	4.3	93.8	7.9
19.03.57	.672	55	3.2	.705	. 086	3.1	95.7	6.3
19.04.57	. K. F	.104	4.2	.734	-113	2.8	97.4	.7
19.05.53	. 4 5 7	• 25 •	4.5	. 689	. 093	3.3	97.2	1.1
19.06.57	.641	.150	4.2	.739	. 092	2.7	96.4	1.7
19.07.53	.557	.270	4.5	.681	.151	3.6	93.7	7.4
19.08.53	. 1.22	.277	3.3	.636	. 093	4.0	92.8	8.1
19.09.53	.607	.246	4.3	. 590	.106	3.3	95.1	6.6
19.10.57	.551	. 303	4.6	.549	.125	5.4	92.0	8.6
19.11.53	.632	.262	4.7	.658	.098	3.7	96.2	1.4
19.12.53	. 450	.277	3.4	.645	.104	3.9	93.9	7.0
19.13.53	.516	•31 <sup>P</sup>	4.6	.557	.085	5.2	91.2	10.3
19.14.53	. 694	.201	2.8	.661	.157	3.9	95.5	7.0
19.15.57	.511	. 352	4.1	.632	•114	4.1	91.1	10.3
19.16.57	. 490	.153	3.5	.624	.104	4.2	97.7	. /
19.17.53	.376	. 759	5.1	.599	.216	3.8	88.3	11.1
19.18.53	.525	•358	4.6	•58P	.162	4.9	90.1	14.1
19.19.53	.724	.103	2.9	.783	.120	2.2	98.5	. 9
19.20.53	.527	.227	5.5	./46	. 057	2.0	99.0	1.0
19.21.53	.586	.032	4.8	.646	. 166	3.8	99.2	.0
19.22.53	.483	.215	5.3	.544	.086	3.9	90.8	1.2
19.23.53	.445	.224	6.3	.0.9	.122	4.0	97.1	9.4
19.24.53	- 51	. 121	4./	.//6	. 059	2.2	100.2	1.2
19.25.5.	.44/	.743	0.4	. 633	.128	4.1	100 (	8.4
19.26.57	.581	. 1.3 *	4.7	.627	. 1191	4.1	100.0	/
19.27.57	.421	.231	5.6	.694	.000	3.2	404 7	/.0
19.28.54		.023	4.4	• 0/1	.159	3./	101.3	1.0
19.29.53	.532	.137	5.1	.6/7	.168	3.5	100.8	3.0
19.30.54		.155	4.1	.70%	.103	7.0	100.0	2.0
19.31.53	• " u f	. :04	4.2	.052	• 1.40	3.9	72.05	0.4

DATE 08/02/72 FULL'O

¢

1

4

TIME	PINGS	P CHARM	EL	SPAR	KER CHA	NNEL	WATER DE	PTH
	AV-	SIGMA	8-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA
17.42.53	.738	.119	2.8	.719	.117	3.0	109.0	. 8
17.43.53	.733	.132	2.9	.645	. 179	3.9	109.7	.7
17.44.53	.743	.171	2.9	.619	.113	4.3	111.6	4.6
17.45.53	.621	.205	3.8	.633	.084	4.1	110.0	5.4
17.47 53	./1/	.160	3.4	.631	.120	4.1	1.12.4	1.1
17.48.53	.614	268	4.5	.545	123	5 5	112.0	
17.49.57	.7(.7	.127	3.2	.636	. 196	4.0	114.1	.7
17.50.53	.645	.185	3.4	.616	.108	4.4	113.3	5.7
17.51.53	.721	. 192	2.9	.645	. 176	3.9	115.5	.7
17.52.53	.751	. GoA	2.5	.620	.101	4.3	116.2	. 7
17 54 57	.631	.290	3.3	.599	.091	4.5	114.1	7.7
17.55.57	600	2:24	3.5	.622	. 181	4.2	117.6	.6
17.56.53	644	. 320	3 3	.595	110	4.0	110.0	5.0
17.57.53	.751	.114	2.6	.651	. 0.92	3.8	120.2	.7
17.58.53	.165	.247	3.2	.634	.101	4.1	119.6	5.7
17.59.53	.776	.112	2.3	.534	.175	4.6	122.0	. 8

TIME	PINGe	D CHANN	ιέι.	SPAR	KER CHA	NNEL	WATER DE	EPTH
	AVE	STG*A	5-L055	AVE	SIGMA	B-LOSS	AVE	SIGM
19.40.53	. 777	.247	9.0	.589	.144	4.8	92.3	10.0
19.41.53	.557	.273	4.5	1.644	.537	-4.0	101.7	12.3
19.42.5	. 5111	261	4.1	.002	286	2.9	100.2	11.7
19.44.53	.577	.237	4.8	.610	.122	4.4	106.5	7.1
19.45.53	.721	.145	3.0	.660	.130	3.8	109.7	4.9
19.46.57	. K (a R	.181	3.9	.714	.078	3.0	109.2	5.4
19.47.5%	. 411	. 3114	5.1	.646	.120	4.0	106.0	9.8
19.48.5	473	313	5 7	.770	127	2.3	106.7	10.6
19.50.53	. 4+4	.091	3.4	.655	. 096	3.8	111.7	.7
19.51.57	263	.276	6.6	.575	.112	5.0	107.3	9.1
19.52.53	. 543	.301	3.9	.656	.118	3.6	107.8	11.0
19.53.57	. 659	.152	4.0	.719	.091	3.0	113.5	. 9
19.54.53		.306	3.9	.688	.122	3.4	100.9	8.9
19.55.54	624	230	5.5	. 707	.105	3.1	113.8	6.4
19.57.53	AL S	267	3.7	.743	.106	2.7	114.5	6.1
19.58.53	F67	-32F	4.7	.789	. 190	2.1	113.8	8.2
19.59.53	.734	.191	2.8	.634	. 176	4.0	117.5	. 7
				• • • • • • • • • • • •				

TIME	PILO	P CHAN	NEL	SPA	RKER CH	ANNEL	WATER DEPTH		
	AVE	SIGMA	8-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA	
10 05 57									
19.25.53	772	.115	2.3	.503	.111	0.2	122.7		
19.20.55	.742	. 778	2.6	.556	.115	5.3	121.8	.0	
19.27.5	.P54	.155	1.5	.604	.127	4.6	120.8		
19.20.53	. 745.	.124	2.7	.565	.138	5.3	120.0		
19.79.53	. 774		2.2	.613	.100	4.4	119.0		
10 74 57	.719	. 175	2.9	.570	.104	5.0	118.2		
10 70 67		.124	3.1	.605	.066	4.4	117.4	.4	
10 37 57	. /	. 1.91	2.8	.588	.090	4.7	116.9		
10 74 57	.6.01	.150	3.7	.608	.142	4.5	114.7	4.8	
10 75 57		.175	4.2	.580	.121	4.9	114.1	4.1	
19.32.24	.67	.164	3.9	.558	.120	5.3	113.2	1.1	
10 77 57	.763	.107	2.4	.581	.145	5.0	113.7	4.5	
10 70 57	.721	.174	3.2	.555	.076	5.2	112.5	2.6	
19.30.53	. 720	. 947	2.5	7.323	10.527	2.2	112.3	2.0	
19.39.51		. 770	1.8	.540	.182	4.6	111.7		
10 44 57		.247	1.5	.567	.142	5.2	109.9	1.7	
19.41.5	.042	.247	1.1	.541	.176	4.5	148.8	5.0	
19.42.54	.770	. 971	2.3	.560	.119	5.2	109.7	5.0	
19.43.5	. 65.0	.197	.4	.f i6	.289	3.5	108.2	.,	
19.44.23		. 97	.5	.619	.186	4.5	108.1		
19.45.54	. 473	.154	3.8	.565	.113	5.2	106.4		
19.40.5	.01.4	. 986	.9	.703	.131	3.2	107.2	4.0	
19.47.5.	.654	.121	3.7	.573	.133	5.1	105.3	4.2	
19.40.5.	.070	. 197	.3	.635	.134	4.1	106.0	4.6	
10 50 57	. 9117	.002	.9	.589	.129	4.6	104.2	4.0	
10 54 17	. * 6.7	.102	1.4	.605	.174	4.7	105.2		
10 53 57		.207	1.8	.576	.135	5.0	104.4	.8	
1.7+32+31 90 ET ET		. 151	3.2	. 596	.145	4.8	104.3	1.1	
10 54 57	.4/2	.124	.3	.635	.275	3.5	103.8	4	
10 55 57		. 779	1.6	.57P	.107	4.9	103.1		
10 64 67	./31	.208	3.2	.579	.085	4.8	102.3	1.5	
10 57 57	.41"	. 186	.9	.668	.122	3.6	102.5	5	
10 59 57	. 420	.195	1.8	.603	.195	3.7	101.8		
19 60 57	. 46.5	.130	1.4	.569	.165	5.3	100.6	4.2	
20.00 57	. 4/4	.202	1.5	. 693	.131	3.3	100.6	3.1	
	.00.1		1.3	.602	.185	4.8	100.9	.0	
20.02.53	2.7	.213	2.9	.574	.092	4.9	98.7	4.4	
20.03 53	. / C	. 262	3.0	.632	.193	4.4	97.9	5.9	
20.04.53		• nac	1.7	.671	.138	3.7	99.9	.7	
0.05.51	.777	.124	2.4	.579	.149	5.0	99.4	.5	
0.06.51		.107	2.3	.617	.164	4.5	98.7	.6	
0.07.53	.021	.060	4.2	.596	.072	4.6	98.3	.5	
0.08.51	670	.11"	1.9	.509	.117	6.1	98.2	.5	
0.09.57		.054	4.0	.543	107	5.5	98.0	.5	
0.10.57	.467	. 138	4.4	.617	.094	4.3	97.3	.3	
0.11.53		.150	5.5	.602	.089	4.5	94.4	7.2	
0.12 51			4.1	. 648	.133	3.9	95.4	5.7	
0.13 57	.042	.117	4.0	.612	.057	4.3	96.8	5	
	.725	198	6.0	.574	.187	4.1	94.8	1 1	

DATE 08/04/72 PCLLMU

DATE 08/04/72	POLLNO 38						
TIME	DINGHR CHANN	FI	SPAR	KER CHA	NNEL	WATER DEF	РТН
TIME	AVE SIGHA	B-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA
20.23.53	.644 .979	3.9	.596	.235	5.9	97.4	1.1
20.24.53	.495 .144	6.6	.502	.330	9.4	94.4	6./
20.25.53	.555 .060	5.2	. 336	.292	12.9	96.4	.0
20.26.53	.553 .050	5.2	.585	.273	6.8	96.2	
20.27.53	.510 .134	6.3	.592	.273	6.3	94.2	5.5
20.35.53	.547 .121	5.7	.633	.285	5.9	96.0	4.7
20.36.57	.495 .188	5.9	.550	.242	6.8	94.4	7.4
20.37.53	.499 .182	7.0	.530	.286	1.8	97.2	4.8
20.38.53	.497 .184	7.0	10515	8.1198	10 6	07.9	8.1
20.39.53	.474 .202	7.7	.434	. 307	10.0	07 4	6 1
20.40.53	.550 .120	5.5	. 385	. 342	4 7	96.9	6.3
20.41.57	.544 .100	5.5	.491	. 290	8.7	97.1	6.8
20.50.53	.717 .124	3.1	.727	115	4.2	92.9	3.1
20.51.53	.541 .122	2.1	• 730	•110	7 1	100.2	.5
20.52.53	.580 .068	4.0	525	286	8.0	98.7	4.6
20.53.53	. 561 . 1 111	2.2	512	710	8.7	98.1	6.1
20.54.53	·-41 ·[34	4 3	505	.278	8.4	99.8	2.3
20.55.5	700 004	2 9	.606	.232	5.5	100.8	. 6
20.70.20	749 022	2.5	. 499	.350	9.7	100.6	. 4
20.57.50	661 222	3.4	.549	.301	7.5	100.2	7.1
20.50.50	647 155	3.5	. 449	.311	10.0	98.3	4.6
20.09.00	712 .124	3.2	.792	.105	2.1	98.7	. 8
21.00.53	.651 .222	3.5	.643	.303	0.0	98.2	7.2
21.01.57	769 .157	3.4	.612	.291	6.4	96.4	7.0
20.56.53	.660 .113	3.8	.515	.275	7.9	100.7	.6
20.57.53	.629 .024	4.0	.635	.293	6.1	100.6	. 4
20.58.53	.594 .217	4.4	.525	.287	8 1	100.0	6.8
20.59.53	. 629 . 142	4.4	.526	.284	5.0	99.4	.5
21.00.53	.455 .127	3.4	.668	.233	4.7	97.5	4.5
21.01.53	.528 .179	5.2	.636	.251	5.4	98.1	7.5
21.02.53	.652 .159	4.2	.720	.205	3.6	97.5	5.7
21.03.57	.924 .180	.9	.840	.618	4.9	100.5	4.4
21.04.53	.597 .184	5.2	.169	.121	16.6	99.3	1.5
21.05.53	.777 .064	2.2	.156	.065	16.6	100.0	.0
21.06.53	.74 .041	2.5	.155	.040	10.4	100.4	.0
21.07.53	.744 .041	2.6	.219	.141	14.0	100.8	.0
21.08.53	.747 .C5P	2.6	.196	.218	14.4	101.2	5.6
21.09.53	.572 .190	3.0	.7.23	-2311	12.0	100.0	7.0
21.02.53	.661 .151	4.0	. 7/1	.310	5 4	90.5	
21.03.57		1.4	.047	267	6.4	100.0	.6
21.05.53	./3/ .110	2.0	.000	270	6.3	100.4	. 8
21.06.54	./41 .1140	2.0	402	268	8.5	99.7	4.2
21.07.55	. 107 . 107	4.7	407	. 260	8.5	98.7	6.2
21.08.57		7.0	. 47/ 574	267	6.7	100.0	6.3
21.09.53	654 079	3.9	.595	,282	6.4	102.3	.6
21.10.55	627 046	4 1	.606	.272	6.4	102.7	.6
21.11.2.	235 046	1.5	. 668	.170	4.2	103.4	.7
21 17 57	705 060	2.0	. 497	.309	9.3	103.7	. 8
<b>CT</b> • 13 • 20	• / • • • • • • •	2.0					

## DATE 08/04/20 011210 34

TIME	PI . 1	- CHA-10	121	SPAR	KEP CH	ANNEL	WATER DE	РТН
	AVE	SIGMA	3-L055	AVE	SIGMA	B-LOSS	AVE	SIGMA
21.14.53	. 9: 4	.125	2.0	.540	.333	8.2	104.3	. 4
21.15.57	. Aid	.030	1.9	.646	.296	5.9	104.7	.6
21.16.53	.754	. 090	2.5	. 351	.283	12.5	105.3	.5
21.17.57	.77*	.117	2.3	. + 62	.241	4.8	105.6	.8
21.18.57	. 65=	.100	4.1	. 621	.285	0.1	105.5	2.5
21.19.57	. 64:	.170	4.3	.654	.236	5.0	104.9	5.3
21.20.57	.690	.101	3.6	.660	.234	4.8	104.4	4.5
21.21.53	. 895	.165	1.8	.590	.260	0.1	108.0	.6
21.22.53	.700	·1114	2.1	.489	.328	9.5	104.2	.7
21.23.53	.799	. 161	2.0	.568	.296	7.0	108.8	.7
21.24.53	.817	.050	1.8	.604	.265	6.2	109.5	.8
21.25.53	. 9.41	.155	1.5	.547	.329	8.2	117.0	.9
21.26.53	.76.1	. 714	2.0	.660	.251	5.0	109.2	5.6
21.27.57	.8;1	. 963	1.9	.506	.296	5.5	110.9	.7
21.28.53	. 7 4 1	.162	3.0	. 659	.237	4.9	110.8	3.2
21.29.57	.7:7	.145	2.3	.584	.314	7.2	112.1	.7
21.30.53	.817	.037	1.8	.629	.273	5.8	112.9	. 6
21.31.57	.7.00	.124	2.1	.667	.298	5.5	113.4	. 4
21.32.57	. 91 4	.112	2.0	.633	.283	5.9	114.2	. 6
21.33.53	.705	.1 35	2.5	. 661.	.231	4.7	115.6	4.2
21.34.53	.7++	.15=	2.6	.609	.322	6.7	115.4	1.3
21.35.57	.817	. 0 . 7	1.3	.607	.318	6.7	116.6	.9
21.36.57	.857	.11%	1.4	.724	.119	2.9	117.6	.5
21.37.53	. 833	.06=	1.4	.641	.238	5.1	117.5	.8
21.38.53	.837	.167	1.4	.667	.307	5.6	1.17.2	.7
21.39.53	.9.1	.16°	2.2	.687	.241	4.5	118.3	1.3
21.40.53	.791	.274	2.5	. 593	.166	3.7	116.5	6.6
21.41.53	.9.40	.114	1.6	.607	.319	6.7	119.8	.7
21.42.57	.774	.255	2.0	.704	.260	4.5	119.1	5.6
21.43.53	. 944	.160	1.7	.536	.278	7.6	121.4	.7

DATE 08/03/72	ROLL	10 30						19 July
TIME	PTUCE	ChANE	۲I	SPAR	KEP CHA	INNEL	WATER DE	РТН
1 1 1 1 1	AVL	STGTA	B-LOSS	AVE	SIGMA	8-LOSS	AVE	SIGMA
17.14.5?	.695	.979	3.2	.616	.158	4.9	99.7	.6
17.15.53	.760	. 189	2.4	.606	.179	5.2	100.1	. 4
17.16.53	.724	. 196	2.9	.673	.175	4.2	100.3	.7
17.17.57	. 677	. 174	3.5	. 540	.246	7.2	100.7	. 4
17.18.53	.602	.157	3.5	. 467	.193	3.6	100.0	4.6
17.19.53	.753	.124	2.6	.688	. 158	3.3	101.4	. 4
17.20.53	. 441	. 1179	3.3	.717	. 971	2.9	1.01.3	. 4
17.21.53	.770	. 107	2.8	.677	.196	4.3	101.8	.5
17.22.53	. 556	. 047	3.7	.646	.054	3.8	102.1	. 4
17.23.53	. 6 11 7	. 0.3 2	3.3	. 463	.077	3.6	102.4	. 3
17.24.53	. 6 10	. 166	3.2	. 655	.071	3.7	102.6	. 4
17.25.53	. 677	.043	3.4	.675	.083	3.5	102.5	.3
17,26.53	.69-	. 027	3.2	.627	. 182	4.1	102.5	.6
17.27.53	. 477	.032	3.4	.660	.044	3.6	102.3	.5
17.28.53	. 61.7	. 0.5 0	3.5	. 048	.276	10.2	102.7	. 4
17.29.57	. 67 .	. 1.25	3.4	. 650	.102	3.9	102.0	. 4
17.30.57	. hr .	. 614	3.6	. 664	.084	3.6	100.8	. 4
17.31.53	. 6 . 7	. 9,43	3.5	.622	.162	4.8	100.5	.5
17.32.57	. 6.7	.065	3.6	.670	.179	3.5	100.9	. 4
17.33.57	. 6.12	. 1.46	3.9	. 525	.161	4.9	101.4	.5
17.34.57	. 657	. 105	3.7	.615	.159	5.0	101.7	.3
17.35.57	. ft "	.0.20	3.6	. 645	. 070	3.9	102.3	. 4
17.36.53	. F 1.7	. 643	3.6	.475	.256	9.0	102.8	.4
17.37.53	. 4 : 1	. 615	3.3	.491	.263	8.7	107.3	.5
17.38.53	. 7 % *	. 661	3.0	.513	.229	7.8	104.0	.3
17.39.57	.51 6	• 1,10	3.6	.312	. 254	13.8	104.5	.5
17.40.57	. 471	.150	3.4	.525	.242	7.7	105.2	.5
17.41.57	ofre F	.0.54	3.3	.509	. 226	7.7	105.6	. 6
17.42.53	.6 7	.154	3.6	. 451	.277	9.9	104.0	.3
17.43.53	.715	.130	3.1	.402	.262	10.7	106.7	.5
17.44.53	.715	.070	2.9	. 439	. 156	3.9	107.3	.6
17.45.57	. 741	040.	2.7	. 632	.166	4.7	107.9	.5
17.46.53	.757	.000	2.3	. 47?	.180	4.2	108.3	.5
17.47.53	.737	. 175	2.7	.624	.232	5.6	100.8	.5
17.49.57	.947	.010	.9	.530	.235	7.1	110.3	.3
17.50.53	.051	. 1150	. 4	.585	.151	5.3	111.0	. 4
17.51.53	,010	.051	.5	.561	.207	0.3	111.4	.6
17.52.53	.974	.057	.2	.705	.061	3.1	112.0	.6
17.53.53	. 96 1	. 942	. 4	.598	.151	5.1	112.6	.6
17.54.57	.0,5	.103	.9	.675	.053	3.4	113.4	.6
17.55.57	.941	.180	.8	.684	.185	3.9	114.1	•>
17.56.57	.007	.257	.7	· 62P	.169	4.7	113.3	5.6
17.57.57	.050	.052	.2.	.694	.103	3.3	115.3	.6
17.58.57	.041	•u3u	. 6	.655	.078	3.7	116.1	.5
17.59.57	.077	. 185	• 2	.712	.148	3.1	117.0	• 6
18.00.57	. 010	.122	.9	.684	. 067	3.3	117.9	. 6
18.01.57	. 0., 4	.111	.5	. 461	.104	3.7	118.6	• 6
18.02.57	. 020	.120	.7	.681	.095	3.4	119.3	.7
18.03.57	.977	.070	.2	.645	.103	3.9	120.4	.7

1

\$

•

1 40

Reproduced from best available copy.

## DATE 8/29/72 21140 50

TIME	PI	O CHANT	EL	SPAR	KER CH.	ANNEL	MATER DE	PTH
	AV-	SIGHA	8-LOSS	AVE	SIGMA	5-LOSS	AVE	SIGMA
20.00.00	.7	. 314	2.8	.477	.098	6.6	44.7	.7
20.11.00	.72.7	.105	3.1	.473	. 192	6.7	44.7	.6
20.02.01	.772	. 251	2.6	.500	.075	6.1	44.6	.6
20.03.00	. 6 . 7	.123	3.7	. 421	.027	7.5	43.8	.3
20.14.03	. 6.5 3	.1+1	3.9	.378	. 173	0.6	43.7	.6
20.15.01	. 674	.241	4.0	.332	. 056	9.7	43.7	.9
20.76.0"	.0.,5	.346	1.7	.402	.063	8.0	43.3	.7
20.17.00	. 6 4 1	. 255	4.5	.355	.164	6.9	43.8	.5
20.18.01	.7.4	.145	2.9	.329	. 078	9.9	42.9	. 4
20.10.07	.6.1	. 2 . 4	4.4	.435	.091	7.4	42.7	.9
20.11.00	. 650	.225	3.8	.431	.068	7.4	42.1	.6
20.12.07	.7.20	. 240	3.3	. 428	. 083	7.5	42.4	1.0
20.13.01	.7.1	.242	3.6	.477	. 057	6.5	41.5	.5
20.14.00	.6:1	. 7.6	4.1	.415	.177	8.4	42.2	. 8
20.15.0	.4 27	.139	8.1	.385	.066	3.4	41.1	.5
20.16.00	. 7 . 1	.117	8.7	.501	. 059	6.1	40.9	. 3
20.17.00	.217	. 6 37	8.6	.484	.072	6.4	42.3	5.6
20.18.00	.295	.129	8.8	:479	.099	0.0	42.5	6.3
20.19.01	.13:	.134	2	.551	.268	5.9	43.8	8.1
20.17.09	. 6 57	. 313	4.8	. 429	.100	7.6	39.1	.5
20.18.07	. 614	.21?	4.8	.275	.091	11.6	39.4	. í
20.19.01	?	. 274	6.2	. 363	.151	9.5	39.3	.7
20.20.00	.535	. 262	5.2	.285	.110	11.4	38.0	. 8
20.21.07	. 643	. 217	4.3	.297	. 178	1.0.9	37.3	. 8
20.22.00	. 6 14	.191	4.8	.220	.069	13.1	36.9	.7
19.49.07	.550	.252	6.0	.602	.158	4.6	49.4	.6
19.50.00	. 507	.179	5.6	.620	.107	4.3	48.7	.6
19.51.00	. 6 .	.291	3.9	.659	.103	3.7	48.1	.6
19.52.07	.611	.257	5.0	.606	.104	4.5	48.0	.6
19.53.00	.629.	.200	4.6	. 451	. 149	7.0	47.3	. 4
19.54.00	. 4 . 4	.191	3.6	.460	. 058	ò.8	46.7	. 4
19.55.01	. 453	.195	4.1	.422	. 064	7.6	46.3	.5
19.56.60	. 446	.1.35	4.0	.457	.076	6.9	45.8	.5
19.57.00	. 7 . 1	.173	3.1	. 474		6.6	45.3	.5
19.58.00	.537	.171	5.0	.431	. 150	7.4	45.3	. 8
19.59.00	. 4.133	.199	3.7	.383	. 062	8.4	44.8	.3

274

2

'4

TIME	PINCER CHANNEL			SPARKER CHANNEL			WATER DEPTH		
	AVE	SIGMA	8-LOSS	AVE	SIGMA	B-LOSS	AVE	SIGMA	
19.01.00	. + 77	.194	3.7	.537	.071	5.5	42.9	.3	
19.02.00	. 636	.247	4.7	. 443	.121	7.4	44.1	.9	
19.03.00	. 410	.248	5.1	. 513	.116	4.4	44.8	. 9	
19.04.00	.595	·218	5.0	.538	.104	5.5	45.4	. 8	
19.05.00	.542	.191	4.9	.592	.169	4.9	46.5	.5	
19.06.00	.782	.369	2.9	.538	.123	5.6	46.3	.5	
19.07.00	.660	.313	4.6	.557	.077	5.2	47.3	.9	
19.08.00	.628	.235	4.6	.543	.266	6.2	48.0	. 8	
19.09.00	.534	.199	6.0	.556	.184	5.6	47.1	1.1	
19.10.00	. 81.0	.163	2.1	. 485	.078	6.4	47.9	.7	
19.11.00	.676	.1.47	3.6	.509	.195	ó, 0	47.8	.7	
19.12.00	. 624	.134	4.3	.454	. 073	7.0	48.3	. 8	
19.13.00	.720	.?7?	3.4	.463	.073	6.8	48.7	. 4	
19.14.00	. 661	.165	3.9	.552	. 161	5.2	49.2	.6	
19.15.00	.665	.269	4.4	. 441	.146	7.5	49.3	. 4	
19.16.00	. 5.15	.132	6.3	.428	.066	7.5	49.4	.5	
19.17.00	.537	.200	6.0	.487	.077	5.4	50.1	.5	
19.18.00	.7(16	.215	3.4	.452	. 196	7.1	59.3	.5	
19.19.00	.68.0	.195	3.7	.467	.080	6.7	50.3	.3	
19.20.00	.726	.164	3.0	.480	.123	6.7	50.5	.6	
19.21.00	.747	.173	2.7	. 474	.074	6.6	50.4	.7	
19.22.00	.743	.164	2.8	.494	.077	6.2	51.1	.8	
19.23.00	.697	.131	3.3	.487	.077	6.4	51.2	.9	
19.24.00	.738	.121	2.8	.465	.047	6.7	51.6	. 8	
19.25.00	.796	.151	2.1	.518	.079	5.8	51.7	.3	
19.26.00	.734	.155	2.9	.497	.097	6.2	51.5	.8	
19.27.00	.7:5	.144	3.1	.515	.080	5.9	51.6	.5	
19.28.00	. 825	.177	1.9	.530	.099	5.6	51.8	. 4	
19.29.00	.730	.194	3.0	.513	.068	5.9	51.9	.4	
19.30.00	.741	.199	2.4	.575	.073	4.9	52.4	.5	
19.31.00	. 434	.227	4.5	.537	. 193	5.5	52.1	.8	
19.32.00	• 6 • • •	.211	4.8	.481	.065	0.4	5/./	. 5	
19.33.00	. 4 3 4	. 96	3.5	. 503	. 167	2.1	23.1	. 4	
		1 45 11	64	.549	.112	7.4	21./	.0	

.

275

-

		NO 58						
TIME	PINGE		EL	SPAR	KER CHA	NNEL	WATER DE	PTH
	AVE	SIGMA	B-LOSS	AVE	SIGMA	8-1022	AVE	SIGHA
18.21.53	.364	.018	8.8	.651	.110	3.8	56.1	.3
18.22.53	.362	.026	8.8	.678	- 1.41	3.6	55.9	.0
18.23.53	. 329	.062	9.9	+ 71	114	5.0	54.7	2.2
18.25.53	.357	.014	9.0	.615	.114	4.4	54.4	.5
18.26.53	.347	.027	9.2	.560	.096	5.2	54.0	.7
18.27.53	.325	.03?	9.8	.528	.090	5.7	54.7	.4
18.28.53	.308	.087	10.7	.624	.155	4.4	52.3	3.1
18 30 53	.332	.03/	9.0	.023	.104	4.2	53.9	.0
18.31.53	.342	.019	9.3	.599	.084	4.5	52.9	.6
18.32.53	.341	.012	9.4	.538	.093	5.5	52.8	.3
18.33.53	.345	.036	9.3	.600	.073	4.5	52.6	.5
18.34.53	.351	.022	9.1	.539	.107	5.5	52.5	.6
18.35.53	.340	.013	9.4	.577	.132	5.1	52.2	.5