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EVALUATION OF CONCRETE BY ULTRASONIC TESTING, F. E. WARREN AUXILIARY SITES, SQUADRON III

H. T. Thornton, Jr.

Army Engineer Waterways Experiment Station Vicksburg, Mississippi

July 1963

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U. S. Army Engineer Waterways Experiment Station CORPS OF ENGINEERS

Vicksburg, Mississippi

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PREFACE

The soniscope investigation of concrete in the F. E. Warren Auxiliary Sites, Squadron III, was verbally authorized by Mr. J. O. Ackerman, Chief, Engineering Division, U. S. Army Engineer District, Omaha, on 17 August 1960, and confirmed by teletype dated 19 August 1960. The group conducting the soniscope testing consisted of Mr. O. Keifer, Jr., Engineering Division, Omaha District, and Messrs. J. H. Sanderson and Dale Glass, Concrete Division, U. S. Army Engineer Waterways Experiment Station (WES). This party was accompanied by Mr. Ralph Newman, Cheyenne Area, who acted as guide and provided general assistance to the group.

The original report of the investigation, <u>F. E. Warren Auxiliary</u> <u>Sites, Squadron III, Report of Evaluation of Concrete by Ultrasonic Test-</u> <u>ing</u>, dated August 1960, was prepared by Mr. Keifer under the direction of, and with general guidance from, Mr. L. S. Bray, Chief, Materials and Airfield Pavement Design Section, F & M Branch, Engineering Division, Omaha District. This paper, prepared by Mr. H. T. Thornton, Jr., under the supervision of Messrs. T. B. Kennedy, Bryant Mather, and E. E. McCoy, Jr., all of the Concrete Division, WES, is based on the original report, and a considerable amount of the information contained herein was extracted from it verbatim.

Col. Edmund H. Lang, CE, and Col. Alex G. Sutton, Jr., CE, were Directors of the WES during this investigation and the preparation and publication of this report. Mr. J. B. Tiffany was Technical Director.

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SUMMARY

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Preliminary investigations of the concrete construction in various areas of the F. E. Warren Auxiliary Sites, Squadron III, established the fact that some of the structures contained low-strength concrete. On 15 August 1960, a meeting was convened at Air Force Ballistic Missile Division to discuss the problem, identify the scope, and determine the course to be taken for the design of corrective action.

To facilitate further investigation, the Waterways Experiment Station was requested to furnish one of its soniscopes to make velocity tests on concrete at the various sites. On 19, 20, and 21 August 1960, velocity tests were made on the structures where low strength was suspected. During this same time, velocity tests were also made on 6- by 12-in. cast cylinders and on cores taken from the questionable areas. After velocity measurements were obtained on these cylinders and cores, they were subjected to compressive strength tests.

The information on pulse velocity and compressive strength obtained from the test cylinders and cores was used to establish correlation between pulse velocity and compressive strength of the concrete being investigated; this correlation and the pulse velocities obtained from the concrete in question were used to assign compressive strength values to the in-place concrete.

It was concluded that (a) a number of the areas tested had concrete of less than adequate quality, (b) some of the suspected areas contained very uniform concrete of acceptable quality, and (c) ultrasonic testing provides a rapid, economical, and satisfactory means of surveying the quality of the concrete in structures of this and similar types.

EVALUATION OF CONCRETE BY ULTRASONIC TESTING F. E. WARREN AUXILIARY SITES, SQUADRON III

PART I: INTRODUCTION

The Problem

1. This investigation was initiated to evaluate areas of concrete construction at F. E. Warren Auxiliary Eites, Squadron III, near Cheyenne, Wyoming, where the possible existence of low-strength concrete had been indicated by the results of compressive strength tests of cylinders at 28day age and other ages of the concrete. Some of the questionable areas had been investigated by cutting 4-in. cores from the concrete and testing the cores for compressive strength. The low-strength concrete problem was discussed at a meeting at Air Force Ballistic Missile Division (AFBMD) on 15 August 1960 and reported in "Memorandum for the Record," dated 16 August 1960, by Mr. G. L. Otterson of the Construction Division, Omaha District.

Purpose and Scope of Study

2. To facilitate further investigation of the concrete structures suspected of containing questionable concrete, the Waterways Experiment Station (WES) was requested to furnish a soniscope and crew to make a rapid survey of the quality of the concrete by ultrasonic tests. The investigations were to be concentrated in the areas containing concrete of questionable quality which had been designated as most critical from a structural standpoint, and those areas for which the representative test cylinders indicated extremely low strength. Using these criteria, the most important areas were determined to be in various parts of the Launch and Service Buildings and Launch Operations Buildings at sites 2 and 3, and to a lesser extent in these buildings at sites 7 and 9. The soniscope investigation Was confined to these four sites and to the test specimens available in the central laboratory at Cheyenne, Wyoming.

3. Soniscope readings were taken at the four sites on 19, 20, and ²¹ August 1960. On 19 August areas at site 2 were tested; on 20 August

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areas at site 3 were tested; and on 21 August areas at sites 7 and 9 were tested. At each site, areas suspected of containing low-strength concrete were investigated, and in addition, areas of known strength were tested for correlation purposes. Also, during the test period concrete cylinders cast from mixtures used in structures at all the auxiliary sites except site 7, and which were scheduled for compression tests in the central laboratory, were tested with the soniscope. In addition, 4-in.-diameter cores from questionable areas of the in-place concrete were subjected to ultrasonic tests. ÷.

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PART II: TEST EQUIPMENT AND PROCEDURES

Equipment

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4. The soniscope equipment used was similar to that described in Corps of Engineers test method CRD-C 51-57.^{2*} The soniscope is an instrunent that transmits pulses of ultrasomic waves through a material and electronically measures the time of travel from the transmitter to a receiver while each is held against the surface of the material a known distance apart. Knowing the time of travel and the path length, the velocity of the ultrasonic pulses can be computed. This velocity provides an index of the condition or quality of the concrete. In this investigation the pulse velocities were correlated with the known strengths of test cylinders made in the laboratory, and with the strengths of cores from conci de in place in various portions of the structures, in order to provide a basis for evaluating areas of concrete of unknown quality by means of measured pulse velocities.

Procedures

5. Soniscope readings were taken on the 6- by 12-in. test cylinders and on the cores by transmitting ultrasonic pulses through the cylinders or cores from end to end. Soniscope readings were taken on concrete in place either by transmitting pulses through the concrete from a point on one surface to a point on the opposite surface, or by transmitting pulses through the concrete from one point to another point on the same surface of the concrete. The soniscope measured the time of travel of the pulses from one point to the other point, and the lineal distance between the two points was measured with a steel tape. From these two values the pulse velocity was calculated by the following formula:²

Pulse velocity, $fps = \frac{path length, ft}{effective time, sec}$

Raised numbers refer to similarly numbered items in the list of references at the end of this report.

All pulse velocities were computed to the nearest 10 fps.

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6. Soniscope readings were normally taken in sets of two to four readings at locations approximately 1 ft apart. In a few instances the pulse velocity at one point was abnormally high as compared with the other readings in the set. In such cases another reading was taken approximately 6 in. away from the original location, and this reading was compared with the others in the set. In every case the pulse velocity from the extra reading compared favorably with the other velocities of the set, and was recorded in place of the original reading, it being assumed that the original reading had been influenced by reinforcing steel.

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PART III: RESULTS

Summaries of Test Results

7. Soniscope readings and compressive strength test results on the 6. by 12-in. concrete test cylinders in the central laboratory are recorded in table 1. Averages for each set of soniscope readings on the concrete in place at the four sites are recorded in tables 2-6, together with compressive strength test results for comparable test cylinders and 4-in. cores.

Correction for Surface Readings

8. At the start of the investigation an attempt was made to correlate pulse velocity readings taken between points on the same surface of the concrete ("surface readings") with readings taken between points on opposite surfaces of the concrete ("through readings") on an equal basis. However, after the first day's results were computed and studied, it was obvicus that there was a variation in the results of the two types of readings. On the last two days of the tests, surface readings and through readings were taken close together wherever possible to provide a comparison. The average for each of the comparable sets of surface and through readings is listed in table 7, and it is apparent that the variation occurred in all cases. The only explanation for this variation is that the surface readings were normally taken with a path length of 6.0 ft, as compared with a path length of 1.5 to 2.5 ft for the through readings. Studies made by personnel of the Concrete Division, WES, ¹ indicated that there is a definite decrease in pulse velocity with increase in path length. However, no distinction was made in the study discussed in reference 1 between surface and through readings, and no formula was given for computing the difference to be expected.

9. The date summarized in table 7 are plotted in plate 1 to show the relation of surface readings to through readings (the two aggregate types represented in plate 1 are discussed subsequently and are not relevant here). Flate 1 indicates that a factor of 800 fps should be added to the

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surface readings to make them comparable to through readings. This factor has been added to all surface readings listed in tables 2-6.

General Correlation Between Pulse Velocity and Compressive Strength

10. In the evaluation of the data obtained at the F. E. Warren Auxiliary Sites, soniscope readings on concrete of unknown quality were correlated with readings on concrete of known strength without regard to the individual mixture used or the aggregate source. This was done because so many mixtures had been used on the project that it was impractical to get sufficient field data to correlate concrete areas for each mixture. In addition, as may be seen in table 1, most test cylinders available for soniscope testing were from mixtures other than those used in the concrete areas of questionable quality. The concrete tested with the soniscope equipment varied in cement content only from 6-1/2 to 6-3/4 bags per cubic yard except for one mixture which had a cement content of 6 bags per cubic yard. Table 8 lists the mixtures used in the structures and in the test cylinders tested with the soniscope equipment.

11. The fact that so many aggregate sources and combinations of aggregate sources had been used on the project further complicated any comparison of concrete of the same mixture proportions. The aggregates used in all mixtures at all sites were of the same general mineral composition, the main differences being that the aggregates used at sites 1-5 were from dry terrace deposits and those used at sites 6-9 were from river deposits. When the results of the soniscope and compressive strength tests are differentiated on the basis of aggregate source, as shown in plates 1 and 2, it is apparent that the different aggregate sources had an effect on the pulse velocity-compressive strength relation, but did not cause major variation: in the comparative results. The method of obtaining and using the correlation will be apparent in the following section.

Discussion of Results

12. Table 1 and plate 2 show the relation between pulse velocities measured in 6- by 12-in. concrete test cylinders and the compressive

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strength of the cylinders. The cylinders were tested with the soniscope in the central laboratory 6 to 18 hours before they were tested in the standard compression test. The cylinders were of various ages and from all sites except site 7. All cylinders scheduled to be tested in compression while or immediately after the soniscope test team was at Cheyenne were tested with the soniscope; however, only 26 of the 71 concrete cylinders available for test were from the mixtures that had been used in the areas of questionable concrete (sites 2, 3, and 9). The data on the cores were obtained to assist in establishing the correlation between the pulse velocity and compressive strength data.

13. Table 9 and plate 3 are intended to provide a correlation between pulse velocities in concrete of questionable quality and pulse velocities in concrete of known quality. The value used for the compressive strength of the concrete of kncwn quality is based on the results of the compressive strength tests of 4-in. cores from that concrete. The cores were normally cut in sets of three, and there was often wide variation of strength within the sets, as well as between sets cut from the same placement at different times. However, core tests were used as the basis of comparison, since there was less variation in results of tests of cores than in results of tests of cylinders made from the concrete aring placement. The values shown in plate 3 are the averages of each set for each pour in each building tested at each site; table 9 shows these groupings. Plate 3 was then used to obtain the compressive strength value (table 9) assigned to each pour. No attempt was made to compensate for the fact that cores were cut and tested at various ages of the concrete and that soniscope readings were taken at ages different from those represented by the cores. These factors were not considered since they are beyond the degree of accuracy of this investigation.

14. The relation of pulse velocity to core strength shown in plate 3 is similar to the relation of pulse velocity to concrete cylinder strength shown in plate 2. The similarity of the two relations increases the validity of using pulse velocity comparisons to evaluate the quality of concrete of otherwise unknown strength.

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PART IV: CONCLUSIONS

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15. Estimated compressive strengths of areas of concrete of questionable quality at the F. E. Warren Auxiliary Sites, Squadron III, were derived by a comparison of the ultrasonic pulse velocity readings in the concrete of und own strength with the pulse velocity readings in concrete of known quality. Concrete of known quality used for this comparison consisted of (a) concrete in placements where core strengths had been established, and (b) test cylinders which were tested with the soniscope equipment immediately before they were tested in the standard compression test. The relation of pulse velocity to compressive strength is shown in plates 2 and 3, and is an identical relation for in-place concrete and for concrete test cylinders.

16. A range of indicated compressive strength values for each value of pulse velocity would conceivably be more realistic than only one individual value. However, an examination of the ultrasonic readings obtained on concrete of known strength indicates that this range would be narrow, and the limits of such a range have not been determined.

17. Also it appears that a family of curves, one for each mixture, would provide a more accurate representation of the relation between compressive strength and pulse velocity. However, due to the large number of mixtures used on this project and the small number of specimens of each, it was not possible to correlate adequately the compressive strength-pulse velocity relation for each mixture, and the correlation used includes all mixtures.

18. The small amount of variation in the pulse velocity readings at various points within each placement indicated uniformity of concrete within each placement. The columns in the Launch Operations Building at site 2 were a critical area and were tested very thoroughly with the zoniscope equipment (see table 2). Pulse velocity readings showed that the columns contain very uniform concrete of acceptable strength (table 9).

19. The test results indicate that the different aggregate sources had an effect on the pulse velocity-compressive strength relation, but did not cause major variations in the comparative results.

20. From table 9, the following areas have compressive strengths

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Site	Placement	Compressive Strength Indi- cated by Pulse Velocity, psi
2)	LOB pour 2, floor L&S Bldg pour 7, flame pit L&S Bldg pour 21, flame tunnel floor	3600 Below 3000 3400
3	<pre>LOB pour 8, roof L&S Bldg pour 10, flame tunnel L&S Bldg pours 12 and 24, flame tunnel: East wall West wall L&S Bldg pour 17, missile support beam L&S Bldg pour 17-A, flame tunnel roof L&S Bldg pour 21, flame tunnel floor L&S Bldg pour 25, wall L&S Bldg pour 34, vestibule wall L&S Bldg pour 37, LOX tank housing wall</pre>	3700 3800 3600 3300 3800 3800 3900 3700 3500 3500
7	IOB pour 11, vestibule roof I&S Bldg pour 25, wall I&S Bldg pour 30, wall	3700 3700 3800
9	<pre>L&S Bldg pour 24, flame tunnel wall L&S Bldg pour 26, wall L&S Bldg pour 29, flame tunnel roof L&S Bldg pour 30, wall L&S Bldg pour 36, mezzanine wall L&S Bldg pour 42, ramp retaining wall: East wall West wall</pre>	3800 3700 3900 3700 3600 3300 3600

indicated by pulse velocity comparison as being less than 4000 psi:

Note: LOB is Launch Operations Building; L&S is Launch and Service Euilding.

21. The concrete placements listed in paragraph 20 as having compressive strengths lower than 4000 psi, as indicated by ultrasonic tests, have been further evaluated by considering the results of tests on cores cut from a number of the placements. These average core strengths are also listed in table 9. Considering both ultrasonic test results and results of tests on cores, where available, the following are the placements which are definitely indicated as having concrete of strength excessively lower than 4000 psi (3500 psi or lower):

Site	Placement
2	L&S Bldg pour 21, flame tunnel floor
3	LOB pour 8, roof L&S Bldg pours 12 and 24, flame tunnel, west wall
9	L&S Bldg pour 42, ramp, east retaining wall

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22. It appears to be a further conclusion of this investigation that ultrasonic testing is a rapid, economical, and satisfactory means of making a survey of the quality of the concrete in structures of this and similar types.

REFERENCES

- Mather, Bryant, McCoy, E. E., Jr., Roshore, E. C., and Sanderson, J. H., "Use of the soniscope by Concrete Division, U. S. Army Engineer Waterways Experiment Station." <u>Effects of Concrete Characteristics on</u> the Pulse Velocity--A Symposium, Highway Research Board Bulletin 200 (1959), pp 42-45.
- 2. U. S. Army Engineer Waterways Experiment Station, CE, <u>Handbook for</u> <u>Concrete and Cement</u>, with quarterly supplements. Vicksburg, Miss., August 1949.
- 3. Field Soniscope Tests of Concrete; 1953 Tests, by E. C. Roshore. Technical Memorandum No. 6-383, Report 1, Vicksburg, Miss., April 1954.

Site		Cyli	nder Age		Pulse locity	Compressive Strength	
No.	Concrete Placement	No.	days		fps	psi	
	<u>Mix</u>	C-5558B-1	Revised				
l	L&S* Bldg pours 12, 24	1-145 1-146 1-147	28	Avg	13,490 14,110 <u>13,910</u> 13,840	4984 4370 <u>4081</u> 4478	
	Equipment space, flame tunnel	1-161 1-162	28	Avg	14,850 <u>14,850</u> 14,850	4478 <u>4587</u> 4532	
		Mix C-422	26в			•	
8	L&S Bldg pour 7	8-223 8-224 8-225	28	Avg	13,720 13,830 <u>13,560</u> 13,700	4478 4695 <u>4478</u> 4550	
	West curb of ramp	8-272 8-273	7	Avg	12,920 <u>13,090</u> 13,000	3973 <u>4117</u> 4045	
	Mix	<u>C-4236B-R</u>	evised				
9	L&S Bldg pours 29, 42	9-179 9-180	28	Avg	13,680 <u>12,860</u> 13,270	3395 2998 3197	
	1	Mix C-678	2				
ł	L&S Bldg pour 26	4-244 4-245 4-246	28	Avg	14,230 14,190 <u>14,270</u> 14,230	3937 4045 4009 3997	
	L&S Bldg pour 30	4-250 4-251 4-252	28		14,690 14,440 <u>14,230</u> 14,450	3864 3828 <u>3937</u> 3876	
	L&S Bldg pour 26	3-216 3-219 Continued	28)		14,010 <u>13,820</u> 13,920	4045 4442 4243	

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Table 1 Results of Ultrasonic and Compressive Strength Tests on

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		Cylin		Puls		Compressive Strength
ite <u>No.</u>	Concrete Placement	No.	Age days	Veloci fps	•	Street (A
	Mix C-	.6782 (Con	tinued)			*
3	I&S Bldg roof	3-254 3-255 3-256 3-244 3-245 3-247 3-248	7	14 14 14 14 14 14	4,420 4,210 4,010 4,630 4,210 4,210 3,820 4,220	3467 3286 3178 3937 3250 3431 <u>3576</u> 3446
		<u>Mix C-678</u>	<u>31</u>			
l	I&S Bldg pour 15	1-154 1-155 1-156		ןד דן	4,310 4,230 <u>4,920</u> 4,490	5381 4912 <u>5381</u> 5224
2	L&S Bldg pour 18	2-229 2-230 2-231	28	1) 1	13,950 14,470 1 <u>3,910</u> 14,110	4226 4551 <u>4406</u> 4394
		2-235 2-236 2-237	28	1	14,140 14,780 15,240 14,720	3864 4370 <u>4551</u> 4262
	L&S Bldg walls	2-277 2-278 2-279	7	נ <u>ן</u>	13,950 14,000 <u>14,570</u> 14,170	3467 3684 <u>3612</u> 3588
	L&S Bldg pour 25	2-241 2-242 2-243]	14,440 14,030 <u>14,030</u> 14,170	Not deter mined
		Mix C67	784			
4	L&S Bldg pour 35	4-282 4-283	14	Avg	14,420 <u>14,850</u> 14,640	4081 4081 4081
	L&S Bldg pours 32, 38	4-301 4-302 4-303	•		13,450 14,010 <u>13,450</u> 13,640	3467 3431 <u>3395</u>
		(Contin	ued)	****6		2 of 3 sheet

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Table 1 (Continued)

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Site		Cyli	nder Age	Pulse Velocity	Compressiv Strength
No.	Concrete Placement	No.	days	fps	psi
	Mix C.	-6784 (Coi	ntinued)	,	
5	L&S Bldg pour 26A	5-238 5-239 5-240	28	13,990 14,110 <u>14,110</u> Avg 14,070	4587 4370 4406 4454
	L&S Bldg pour 23	5-244 5-245 5-246	28	14,470 14,440 <u>14,610</u> Avg 14,510	4731 4406 <u>4515</u> 4551
	L&S Bldg pour 27	5-295 5-296	7	14,270 <u>14,030</u> Avg 14,150	3973 <u>4153</u> 4063
		5-302 5-303 5-304	7	14,030 13,990 <u>13,950</u> Avg 13,990	4370 4623 <u>4840</u> 4611
		<u>Mix C-678</u>	33		
6	L&S Bldg pour 39	6-260 6-261 6-262	- 28	13,530 13,790 <u>13,830</u> Avg 13,720	4515 4262 <u>4551</u> 4443
	I&S Bldg pours 33, 41	6-266 6-267 6-268	28	13,450 13,450 <u>14,010</u> Avg 13,640	4442 4153 <u>4515</u> 4370
	L&S Bldg pour 32	6-328 6-329	7	13,100 <u>12,960</u> Avg 13,030	3576 <u>3720</u> 3648
	Mix	C-4733B-E	Revised		
5	L&S Bldg pour 42	5-308 5-309	7	13,980 <u>14,110</u> Avg 14,050	3864 <u>3612</u> 3738
		5-315 5-316 5-317	7	13,990 13,910 <u>14,480</u> Avg 14,130	4045 4153 <u>4153</u> 4117

Table 1 (Concluded)

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Concrete Placement	Conc Age days	Soniscope Te Test Location	F Ve	ulse locity fps	Comp. Str	inders ressive ength psi		n. Cores Compressive Strength psi
		Mi	<u>x c-60</u>	<u>34</u>				
LOB pour 7 columns	72	B-2 B-3 C-2 C-3 D-2 D-3 E-2 E-3	Avg	14,410 14,290 14,410 14,440 14,290 14,130 14,130 14,180 <u>14,550</u> 14,340	Avg	3250 3503 <u>3576</u> 3443 ^a	N	×
LOB pour 9 vestibule walls	56	West wall Interior wall	Avg	13,250* <u>14,420</u> 13,840	Avg	3359 <u>3648</u> 3503 ^a 3431 ^b	N	o cores
LOB pour 8 roof	60	Near core 55 Near core 56	Avg	13,920 <u>13,820</u> 13,870	Avg	3460 3220 3400 3575 <u>3460</u> 3423 ^a 3509 ^b	55 56 57	4690 3320 <u>4730</u> Avg 42470
L&S Bldg pour 24 flame tunnel walls	57	East wall		13, 510	Avg	3612 <u>3684</u> 3648ª 3287 ^b	Ņ	o cores
	45	West wall		13, 530	Avg	3395 <u>3395</u> 3395 ^a 3142 ^b	N	o cores
L&S Bldg pour 7 flame pit	88	Floor West wall Near cores 62, 63 Near core 64	Avg	11,265* 11,170* 10,890* <u>11,090</u> * 11,100**	Avg	3431 3395 2467 3214 3214 3359 3180 ^a 3142 ^b	62 63 64	3710 3380 4620 Avg 3903 ^d
		(0	ontim	ned)				

Table 2

Results of Ultrasonic Tests on In-Place Concrete and Compressive Strength Tests

equivalent through readings; all other pulse velocity values taken through

* Soniscope readings taken on one surface of concrete corrected to values of

** Averages using corrected surface readings.

a Cylinders tested at 23-day age.

concrete.

Contraction of the second second

b Cylinders tested at 49-day age.

c Cores cut when concrete was 43 days old.

d Cores cut when concrete was 71 days old.

		Soniscope Te	ests	Cylinders Compressive Strength <u>psi</u>		4.	in. Cores
Concrete Placement	Conc Age days	Test Location	Pulse Velocity fps			Compressiv Strength No. psi	
		Mix C-6	034 (Continued)				
IAS Bldg pour 13 missile sup- port beam	52		13,980	Avg	3214 <u>3251</u> 3233ª 3395°		Ko cores
		M	<u>ix c-6781</u>				
L&S Bldg pour 25 wall	23	West part of north wall	13,720	Avg	3972 3792 <u>3756</u> 3840°		No cores

a Cylinders tested at 28-day age.
b Cylinders tested at 45-day age.
e Cylinders tested at 7-day age.

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Table	3

Results of Ultrasonic	Test: on In-Pla	co Concrete and	l Compressive	Strength Tests

on Test Cylinders and Cores, Sites 2 and 3

		Soniscope Test				inders		in. Cores
A	Conc			ulse		ressive		Compressive
Concrete Placement	Age days	Test Location		locity fps		ength psi	No.	Strength psi
I faceachto	<u>uny</u>			-		<u>poz</u>	101	
		Mix	C-1.7	02				
Site 2	109	Near core 49		13,620*		3007	49	4420 ^d
LOB pour 2	•	Near core 50		12, 510*		3024	50	3700 ^d
floor		Near core 51		12,790*		3305	51	3090d
		Near core 50A		13,200* 13,480*	Avg	3112ª	504 51a	3560 ^e 4120 ^e
				11,710*		3936	52A	<u>3120</u> ^e
		Near cores 51A, 52A		12,850*		3612 4081	1574	Avg 3552
			Avg	12,880**	Avg	3876 ^b		is 1.5 ft m 52A)
						2010		- //
Site 3	110			12,970		3287	35	3060
L&S Bldg				12,930	A	2817 2050c	36 27	2630 2870
pour 21, flame tunnel floor				1½,050* 13,740*	Avg	3052 ⁰ 3467 ^b	37	Avg 2853 ^r
cumer 11001			Avg	13,420**		2401		AV6 2075
	_						~	•
Site 3	123					3070	38	3400
L&S Bldg						<u>2853</u>	39 40	3810 2840
pour 8, flame tunnel floor					Avg	2901° 3214b	40	Avg 3350g
conner 11001						7574	28	3740
		Near core 29		14,210*			29	2850
		Near core 30		13,790*			30	2920
			Avg	14,000**			•	Avg 3170h
								Avg 32461
						~		Avg 3485J
						0	verall	avg 3313
Site 3	109			13,610*		3756		No cores
I&S Bldg						3684		
pour 7, flame						3792		
tunnel floor					Avg	37440		
		(Con	ntim	ed)				
Note: TOP is Tou	nah On	erations Building;		o Iowach (muiaa P		
		gs taken on one sur:						
	throug	h readings; all oth	er pu	lse veloc:	ity va	lues ta	ken th	rough
concrete. ** Averages u	sing o	orrected surface rea	adina	·S.				
		at 23-day age.						
b Cylinders	tested	at 45-day age.						
c Cylinders	tested	at 28-day age.						
		oncrete was 91 days						
		concrete was 64 days						
		concrete was 62 days concrete was 77 days						
		concrete was 17 days concrete was 100 days						
		cores cut at 45-day						
		cores cut at 32-da					(1	of 3 sheets)
							•	-

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ى مەنبە «بالليدەرىي خىچو «بازىيە»، يورى		Soniscope Tes	sts	Cylinders	4-in. Cores		
Concrete Placement	Conc Age days	Test Location	Pulse Velocity fps	Compressive Strength psi	Compressive Strength No. DSi		
		Mix 2-470	02 (Continuea)				
Site 3 IAS Bldg IOX sump	135	South wall	13,740	4623 4550 <u>4659</u> Avg 4611c	No cores		
Site 3 I&S Bldg pour 37, LOX storage tank housing	103	South wall	12,680 <u>12,560*</u> Avg 12,620**	2998 <u>2781</u> Avg 2890c 3214b	32 3850 33 3680 34 <u>3589</u> Avg 3703k		
		Mi	x C-4701B				
Site 3 I&S Bldg pour 17, missile support beam	60		13,290	4220 3684 3790 3431 3960 <u>3756</u> Avg 3807 ^c	No cores		
Site 3 L&S Bldg pour 18-A, floor	64		13,590 <u>13,430*</u> Avg 13,510**	3611 3395 3377 3287 3395 <u>3142</u> Avg 3368 ^c	19 4730 20 3710 21 4170 Avg 4203		
Site 3 I&S Bldg pour 25, wall	54		13, 110	3647 3323 <u>3431</u> Avg <u>3467</u> c 3323 3431 <u>3106</u> Avg <u>3287</u> b	No cores		

Table 3 (Continued)

(Continued)

* Soniscope readings taken on one surface of concrete corrected to values of equivalent through readings; all other pulse velocity values taken through concrete.

- ** Averages using corrected surface readings.
- b Cylinders tested at 45-day age.
- c Cylinders tested at 28-day age.
- k Cores cut when concrete was 57 days old.

(2 of 3 sheets)

		Table 3				
Concrete Placement	Conc Age days	Soniscope Tes Test Location	ts Pulse Velocity fps	Cylinders Compressive Strength psi	<u>4-</u>	in. Cores Compressive Strength
		<u>Mix C-470</u>	DIB (Con. imied)			
Site 3 I&S Bldg pour 15, floor.	66		13,820*	3575 3611 3900 3828 3792 3756 <u>3684</u> Avg <u>3735</u> ° 3647 <u>3685</u> Avg <u>3666</u> °	16 17 18	4450 4450 <u>3100</u> Avg 40001
Site 2 L&S Bldg pour 21, flame tunnel floor	98		13,490 12,990 <u>10,830*</u> Avg 12,440***	3467 3720 3539 3864 3720 4117 4153 3792 <u>3539</u> 0 Avg 3768 ^c	65 66 67 53 54 55 veral:	3830 4890 3930 Avg 4217m 2930 2220 3080 Avg 2743n 2 avg 3480

Table 3 (Concluded)

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* Soniscope readings taken on one surface of concrete corrected to values of equivaleng through readings; all other pulse velocity values taken through concrete.

- ** Averages using corrected surface readings.
 b Cylinders tested at 45-day age.
 c Cylinders tested at 28-day age.
 l Cores cut when concrete was 42 days old.

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- m Cores cut when concrete was 80 days old.
- n Cores cut when concrete was 53 days old.

Table 4

Results of Ultrasonic Tests on In-Place Concrete and Compressive Strength Tests

,	on Test Cylinders and Cores, Site 3 Mix C-4702B-Revised								
		Mix C-4 Soniscope Tes		Revised	<i>(hr)</i>	inders		in. C	0705
Concrete Placement	Conc Age days	Test Location	F	ulse locity fps	Comp Str	ressive ength psi	<u>No.</u>	Comp Str	ressive ength psi
IAS Bldg pours 12, 24, flame tunn/1 walls	72	East wall	Avg	12,850 12,460* <u>13,200*</u> 12,840**	Avg	3756 3720 3395 3900 3684 <u>3612</u> 3678	34 35 36	Avg	3700 3660 <u>3490</u> 361.7°
	81	West wall	Avg	12,650 11,010* <u>12,990</u> * 12,220**	Avg	2889 2889 2709 3250 <u>2708</u> 2889 3040 ^b	37 38 39	Avg	3000 3170 <u>3150</u> 3107 ^d
L&S Bldg pour 10 flame tunnel wall	85	West wall Near cores 25, 26 Near core 27	Avg	12,950* <u>13,590</u> * 13,270**	Avg	3828 3576 3702ª 3720b	25 26 27	Avg	5000 5010 <u>1740</u> 4917e
L&S Bldg pour 10 flame tunnel wall, north part missile support area	77			13, 240	Avg	2456 2456 2384 2311 2492 2456 2426	41 42 43	Avg	3840 3470 <u>3320</u> 3543 ¹

(Continued)

Note: L&S is Launch and Service Building.

* Soniscope readings taken on one surface of concrete corrected to values of equivalent through readings; all other pulse velocity values taken through concrete.

****** Averages using corrected surface readings.

a Cylinders tested at 28-day age.

- b Cylinders tested at 45-day age.
- c Cores cut when concrete was 50 days old.
- d Cores cut when concreie was 59 days old.
- e Cores cut when concrete was 62 days old.
- f Cores cut when concrete was 32 days old.

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Table 4	(Concluded)
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		Soniscope Test	5.3	Cylinders	4-	in. Cores
Concrete Placement	Conc Age days	Test location	Pulse Velocity fps	Compressive Strength psi	No.	Compressiv Strength psi
IAS Bldg pour 34 vestibule wall	71	West wall	12, 540	3973 3684 3503 3539 <u>3684</u> Ave 36778 3901 ^b	13 14 15	3700 4660 <u>4600</u> Avg 43208
LES Bldg pour 17-A, flame tunnel roof	72		13,180*	3467 3431 3359 3250 3359 <u>3540</u> Avg 3401 ^{&}	31 32 33	3220 3640 <u>3520</u> Avg 3460
128 Bldg pour 20 flame tunnel roof	38		14,085*	4804 4659 <u>4298</u> Avg 4587 ^a		No cores
LOB pour 8 rooft	79	Near core 10 Near core 11 Near equip. hatch	13, 150 12, 910 <u>12, 880</u> Avg 12, 980	343. 2744 2817 2889 3178 3142 2889 <u>3142</u> Avg 3029 ^a 3250 ^b	15 11 10	2980 3070 <u>3470</u> Avg 3173

* Soniscope readings were made on 6-in. concrete cores cut from LOB pour 8 at 77day age. These 6-in. cores were cut near (within 2 ft of) the same locations as the 4-in. cores with the same basic number.

	6-in. Cores	
	Pulse	Compressive
No.	Velocity, fps	Strength, pst
10-1	12,890	3928
11-2	12,560	2542
12-4	12,510	2311
	Avg 12,650	Avg 2927
Cylinders tested at	28-day age.	
Cylinders tested at	45-day age.	
Cores cut when concr	ete was 50 days old.	
Cores cut when concr	ete was 47 days old.	
Cores cut when concr	ete was 55 days old	
cores cut when conci	eve was)) uays oru	

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Table 5

Results of Ultrasonic Tests on In-Place Concrete and Compressive

Stre	ength Test	s on Test Cyli: Mix C-4	•	nd Cores,	Site 7	
Concrete Placement	Sonis Conc Age Days	cope Tests Pulse Velocity fps	Cyl: Compi Stre	inders ressive ength osi		n. Cores Compressive Strength psi
I&S Bidg pour 30 wall	45	13,300 <u>13,260*</u> Avg 13,280**	Avg	3756 3684 4081 4009 <u>4009</u> 3908 3792	No	cores
I&S Bldg pour 25 wall	61	12,690 <u>13,270</u> * Avg 12,980**	Avg	4030 3828 4040 3966 ⁸	No	cores
LOB pour ll vestibule roof	79	13,100*	Avg	3683 <u>3250</u> 3467 ^a 3760 ^b	78 79 80	3860 4040 <u>3790</u> Avg <u>3897</u> c

Note: LOB is Launch Operations Building; L&S is Launch and Service Building.

* Soniscope readings taken on one surface of concrete corrected to values of equivalent through readings; all other pulse velocity values taken through concrete.

** Averages using corrected surface readings.

a Cylinders tested at 28-day age.

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b Cylinders tested at 45-day age.

c Cores cut when concrete was 62 days old.

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		Soniscope Te	ests		Cyl	inders	
	Conc Age	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		Pulse Velocity		Compressive Strength	
Concrete Placement	days	Test Location		fps		osi	
	M	ix C-4232B-Revised					
L&S Bldg pour 30, wall	43		Avg	12,990 <u>13,130*</u> 13,060**	Avg	3251 <u>3395</u> 33238	
L&S Bldg pour 36, mezza- nine wall	13			12,790	Avg	1806 <u>1878</u> 1842 b	
L&S Bldg pour 26, wall	45		Avg	12,985 13,175* <u>13,270</u> 13,140**	Avg	4984 4840 4948 4924c	
	M	ix C-4236B-Revised					
I&S Bldg p 24, flame tunnel walls	53	West side		13,330 12,960*		3828 3539	
		East side	Avg	13, 520 <u>13, 120*</u> 13, 080**	Avg	3683° 3612ª	
L&S Bldg pour 29, flame tunnel roof	30	Near top 	Avg	13,310 <u>13,500</u> * 13,400	Avg	3395 <u>2998</u> 3196°	
L&S Bldg pour 42, ramp re- taining walls	30	East wall	Avg	12,390 <u>13,120*</u> 12,750**		place- as 29)	
	19	west wall		12,730 12,560 13.200*		3503 3720 3503	

Results of Ultrasonic Tests on In-Place Concrete and Compressive

Note: L&S is Launch and Service Building.

- No cores cut at any test sites.
- * Soniscope readings taken on one surface of concrete corrected to values of equivalent through readings; all other pulse velocity values taken through concrete.
- ** Averages using corrected surface readings.
- a Cylinders tested at 31-day age.
- b Cylinders tested at 3-day age.
- c Cylinders tested at 28-day age.

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- d Cylinders tested at 45-day age.
- c Cylinders tested at 8-day age.

Site and Placement	Surface Readings Pulse Velocity fps	Through Readings Pulse Velocity fps
M	lix C-6034	
Site 2		14,420
LOB pour 9	12,450	14,420
M	<u>lix C-4702</u>	
Site 3 L&S ,ldg pour 21 L&S Bldg pour 37	13,090 11,760	12,950 12,680
<u>Mix C</u>	-4702B-Revised	
Site 3		
L&S Bldg pours 12, 24 (east) L&S Bldg pours 12, 24 (west)	12,030 11,200	12,850 12,650
<u> </u>	lix C-4701B	
Site 2	10,020	13,240
L&S Bldg pour 21 Site 3	10,030	
L&S Bldg pour 18-A	12,630	13, 590
<u>M</u>	lix C-4226B	
Site 7		10 600
L&S Bldg pour 25 L&S Bldg pour 30	12,470 12,460	12,690 13,300
<u>Mix (</u>	2-4232B-Revised	
Site 9		
L&S Bldg pour 26 L&S Bldg pour 30	12,380 12,330	13,130 12,990
	2-4236B-Revised	
Battary of Battary		
Site 9 I&S Bldg pour 24 (west)	12,160	13,330
L&S Bldg pour 24 (ast)	12, 320	13,520
L&S Bldg pour 29	12,700	13, 310
L&S Bldg pour 42 (east wall)	12,320	12,390
L&S Bldg pour 42 (west wall)	12,400	12,650

Table 7Relation of Surface Readings to Through Readings

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Note: LO3 is Launch Operations Building; L&S is Launch and Service Building.

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anth 1. C-555th-Rewland	L CLARG AAA, PROTEC, 40	00 16	9. FITE 5. C-\$733B-Revised. Class AAA, Bonair, \$000 1b
•	IN PIT ADDRANTES		MCOURRY SAND. LARDEN COARDE ACCINEDATED
Crevent Protex itani Gravel, 3/4 in. mux Dravel, 1-1/2 in. mux Mater:	635 1b 4.7 or 1020 1b 1000 1b 1000 1b 2(5 1b (33 gul)	f⊷y/4 bugu/cu yd	Cement 635 lb 6-3/% bags/cu yd Sani 1130 lb Gravel, 3/% in. max 990 lu Oravel, 1-1/2 in. max 990 lb Water 267 lb (32 gal) Water/cement ratio %.7% gal/bag
Water/cement ratio	4.89 en1/oue		
SITE 2. C-6731. CLASS /	мл, роплоцітн зн, 4000	16	10. BITE 5, C-6784. Class AMA, AdJAREX, 4000 1b
John R.	BROWN PTP AQURESATES		MCGIRNY SAND, TAICUN COARCE AGCREVATUS
Coment Foradith 30 Eard Gravel, 3/4 in. max Gravel, 1-1/2 in. max Water Water/cement ratio	611 15 1.63 15 1.00 15 1.020 15 1.020 15 250 15 (30 gal) 4.62 gal/bag	6-1/2 bags/cu yd	Crownt 611 lb 6-1/2 bags/cu ye Aquarex 310 38 os Sand 1110 lb Gravel, 3/4 in. max 990 lb Gravel, 1-1/2 in. max 990 lb Water 271 lb (32.5 gal) Water/cement ratio 5.0 gal/bag 11. STTP3 6 AND 7, C-6783. Class AAA, P0720LITH 38, 4000 lb
<u>слик 2, с.6034.</u> сцала .	ала, реотех, 4000 15		Ι
JOHN W.	BROWN PIT AGGREGATES		$\frac{COWAN A CORPEGATES}{612 1b} 6-1/2 bags/cu yd$
Crawnt Frotex Garvol, 3/4 in. max Gravol, 1-1/2 in. max Water/coment ratio	635 1b 5.4 oz 970 1b 930 1b 1135 1b 203 1b (33 gal) 4.80 gal/bag	6-3/4 bags/cu yd	Cement 611 1b 6-1/2 bags/cu yd Pozzolith 3R 1.63 1b Eund 1160 1b Gravel, 3/4 in. max 980 1b Gravel, 1-1/2 in. max 980 1b Water 258 1b (31 gal) Water/cement ratio 4.77 gal/bag
. STYPE 2 AND 3, C-HTV2.	Class AAA, PROTEX, 4000	15	12. STTES 6, 7, AND 8, C-42268. Class AAA, PROTEX, 4000 1b
	SEN PIT AGGREGATES		OWAN ADDRYGATES
Coment Protex Soul Gravel, 3/4 in. max Gravel, 1-1/2 in. max	564 1b 4.5 oz 1110 1b 930 1b 275 1b (33 gal)	6 bags/cu yd	Cement 635 ib 6-3/4 bags/cu yd Protex 6.0 oz Sand 1020 lb Gravel, 3/4 in. max 1020 lb Gravel, 1-1/2 in. max 1020 lb Water 258 lb (31 gal)
Witer Nuter/cement ratio	5.50 gal/bag		Water/correct ratio 4.59 mi/bag
. STTE 3. C-6782. Class	AAA, AQUAREX 310 or TOZ	2011TH 3P, 4000 15	13. SITES 6, 7, AND 8, C-4226. Class AAA, PROTEX, 4000 1b
JOHT W. HRC	WH PIT SAND, LARGEN CRA	VEL	
Coment Aquinex 310 Sand Gravel, 3/4 in. max Gravel, 1-1/2 in. max Water Water/coment ratio	611 1b 38 oz 1110 1b 1010 1b 1010 1b 250 1b (31 gal) 4.77 gal/bag	6-1/2 bags/cu yd	Frotax 2.7 os Sand 1040 lb Gravel, 3/8 in. max 1020 lb Gravel, 1-1/2 in. max 1020 lb Water 256 lb (31 gal) Water/cement ratio 4.77 gal/bag
6. SITE 3 AND 4, C-6784.	Class AAA, AQUAREX 310	, 4000 15	14. SITE 9, C-42368-Revined. Class AMA, PROTEX, 4000 15
WASHED I	arsen Sand, Larsen Grave	L	VETTORL AGGREGATES
C-ment Aquarex 310 Gand Gravel, 3/4 in. max Gravel, 1-1/2 in. max Water Water/cement ratio	611 1b 38 os 1110 1b 990 1b 990 1b 271 1b (32.5 gal) 5.0 gal/bag	6-1/2 bags/ cu yd	Cement 635 lb 6-3/4 bags/cu yd Protex 5.1 oz Sand 1030 lb Gravel, 3/4 in. max 980 lb Gravel, 1-1/2 in. max 980 lb Mater 283 lb (3k gal) Water/cement.ratio 5.04 gal/bag
7- HUTPE 3 AND 4, C-HYO2B	-Revised. Class AAA, PR	10TEX, 4000 1b	15. <u>ATTE 9, C-4232B-Revised.</u> Class AAA, Honsir, 4000 lb
<u>ALL</u> Cement Protex	LARSEN PIT AOGRADATES 635 15 5.1 02	6-3/4 bags/cu yd	VEITZEL AGGREGATES Coment 635 lb 6-3/A bags/cu yr Sand 1120 lb
Sand Gravel, 3/4 in. max Gravel, 1-1/2 in. max Water Wa cr/coment ratio	1050 1b 980 1b 980 1b 980 1b 275 1b (33 gal) 4.89 gal/bag		Gravel, 3/4 in. max 980 lb Gravel, 1-1/2 in. max 980 lb Mater 300 lb (36 gal) Mater/cement ratio 5.33 gal/bag
8. CITHE P. 3. AND 4. C-4	701B. Class AAA, Nonai	r, 4000 1b	
ALL	TARGEN PIT AQURHUATES		
Const Date Devel, 3/8 in, max Gravel, 1-1/2 in, max Water Water/coment ratio	635 1b 1140 1b 900 1b 900 1b 792 1b (35 gal) 5.19 gal/bag	6-3/4 bags/cu yd	

Table 9

Evaluation of Concrete of Questionable Quality

by Comparison of Pulse Velocities

Site and Placement	Average Pulse Velocity fps	Strength Indicated by Pulse Velocity, psi	Average Core Strength psi
Site 2			
LOB pour 2, flour	12,880	3600	3552
LOB pour 7, columns	14,340	4350	••••
LOB pour 8, roof	13,870	4100	4247
LOB pour 9, vestibule walls	13,840	4100	
L&S Bldg pour 7, flame pit	11,100	Below 3000	3903
L&S Bldg pour 13, missile support beam	13,980	4200	
L&S Bldg pour 21, flame tunnel floor	12,440	3400	3480
L&S Bldg pour 24, flame tunnel (east wall)	13, 510	4000	
L&S Bldg pour 24, flame tunnel (west wall)	13,530	4000	
L&S Bldg pour 25, wall	13,720	4000	
Site 3			
LOB pour 8, roof	12,980	3700	3173
L&S Bldg pour 7, flame tunnel flcor	13,610	4000	5-15
L&S Bldg pour 8, flame tunnel floor	14,000	4200	3313
L&S Bldg pour 10, flame tunnel (north part)	13,240	3800	3543
L&S Bldg pour 10, flame tunnel (west wall)	13,270	3800	4917
L&S Bldg pours 12, 24, flame tunnel (east wall)	12,840	3600	3617
L&S Bldg pours 12, 24, flame tunnel (west wall)	12,220	3300	3107
L&S Bldg pour 15, floor	13,820	4100	1000
L&S Bldg pour 17, missile support beam	13,290	3800	
L&S Bldg pour 17-A, flame tunnel roof	13,180	3800	3460
L&S Bldg pour 18-A, floor	13, 510	4000	4203
L&S Bldg pour 20, flame tunnel roof	14,085	4200	•
L&S Bldg pour 21, flame tunnel floor	13,420	3900	2850
L&S Bldg pour 25, wall	13,110	3700	•
L&S Bldg pour 34, vestibule wall	12, 540	3500	4320
L&S Bldg pour 37, LOX tank housing wall	12,620	3500	3703
I&S Bldg, IOX sump, south wall	13,740	4100	, ••••
Site 7			
LOB pour 11, vestibule roof	13,100	3700	3897
L&S Bldg pour 25, wall	12,980	3700	
L&S Bldg pour 30, wall	13,280	3800	
		-	
Site 9	12 000	2800	
L&S Bldg pour 24, flame tunnel wall	13,080	3800	
L&S Bldg pour 26, wall L&S Bldg pour 20, flame tunnel most	13,140	3700	
L&S Bldg pour 29, flame tunnel roof	13,400	3900	
L&S Bldg pour 30, wall	13,060	3700	
L&S Bldg pour 36, mezzanine wall (13-day age)	12,790	3600	
L&S Bldg pour 42, ramp, east retaining wall	12,750	3300	
L&S Bldg pour 42, ramp, west retaining wall	12,830	3600	

Note: LOB is Launch Operations Building; L&S is Launch and Service Building.

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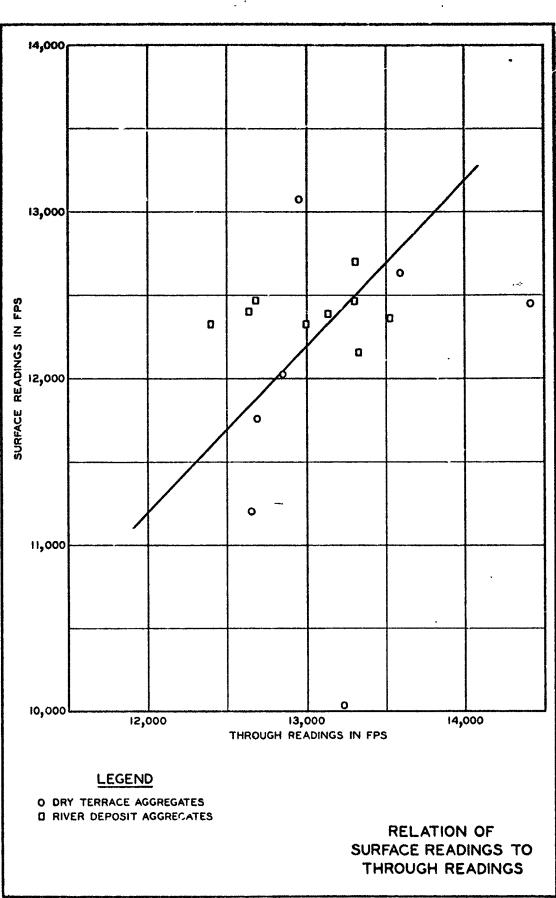


PLATE I

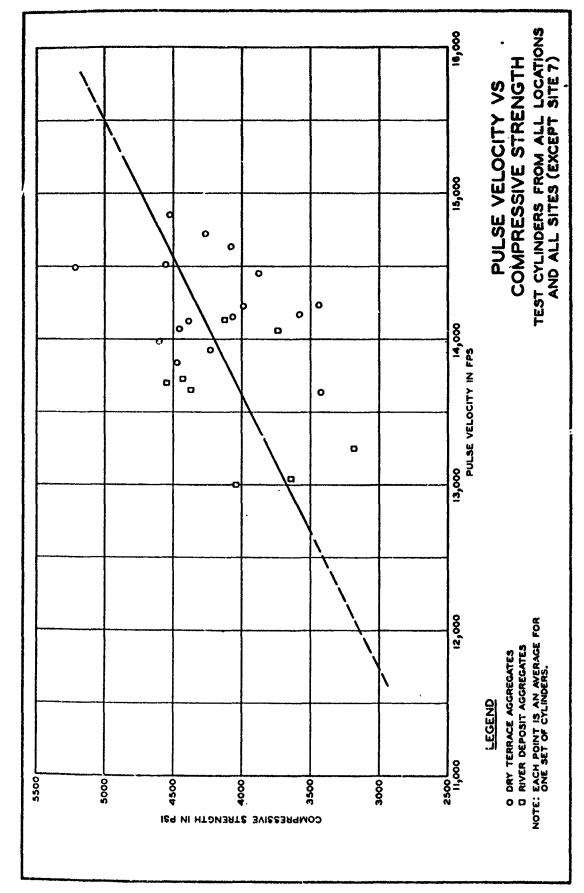


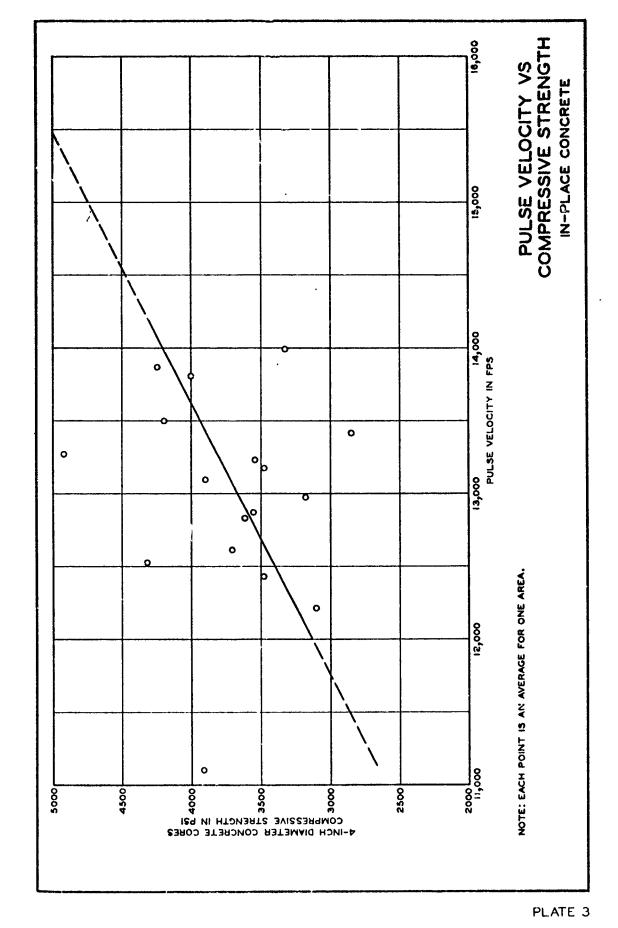
PLATE 2

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