NUSC Technical Report 4331

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The Response of Cable-Moored Axisymmetric Buoys To Ocean Wave Excitation

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KIRK T. PATTON Ocean Science Department





15 June 1972

NAVAL UNDERWATER SYSTEMS CENTER

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This study was prepared originally as a dissertation in partial fulfillment of the requirements for the degree of Doctor of Philosophy in Mechanical Engineering at the University of Rhode Island. The work was accomplished under NUSC Project No. A-071-50-00, "Buoy Dynamics," Principal Investigator, K. T. Patton (Code TA12), and Navy Subproject and Task No. ZR-001-01-611-1N, Program Manager, Dr. J. H. Huth (DLP/MAT 03L4).

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investigated using numerical methods on metric buoys are developed for six degree and hydrodynamic forces. Equations of m simultaneous axial and transverse wave p numerical technique is shown for digital of tions of motion for the buoy and its moori currents, and a quasi-random wind wave characteristics are compared with respon graphic buoy system moorings. This com forces and configurations can be predicted	a digital computer. He es of freedom includi otion for the mooring ropagation. A unique computer simulation ng cable are coupled model. Simulated bu use characteristics of parison indicates that d within 5 percent an	quation ng cros cable methoc of cable and are oy syste bserved t steady d that dy lation.	s of motion for axisyr s-coupled hydrostatic are developed to allow d of characteristics dynamics. The equa- e excited by winds, em parameter response from actual oceanc- y state buoy system ynamic motions can
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ABSTRACT

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Motions and dynamic forces imposed upon a moored buoy system by the oceanic environment are of vital interest to the user of the system. If instrumentation for monitoring the environment is motion sensitive, it is of little value if its response to platform motion is greater than its response to the changing environment. The buoy system designer is also concerned with motions and forces in buoy systems in order to design for the highest probability of system survival under extreme conditions.

In response to these needs, this study investigates digital computer simulation of buoy system dynamics for simple buoy systems, i.e., a surface buoy moored on a single mooring line. The buoy system can be excited by winds, waves, and currents. Winds can act from any compass direction, and currents can vary in strength and direction as a function of depth in the water column. Wind waves are simulated by first computing their properties with the Sverdrup-Munk-Bretschneider method and then by using Borgman's energy partitioning scheme on a two-parameter Bretschneider spectrum to compute component sine wave amplitudes and frequencies. Since the component Stokesian waves are linear, the principle of superposition can be used to sum component magnitudes in order to compute vator particle motions.

Equations of motion for the buoy are developed for six degrees of freedomthree translational and three rotational. Hydrostatic and hydrodynamic forces

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and moments acting on an oblate spheroid moving on the free surface of an infinite body of water are investigated in detail. The set of integro-differential equations for buoy motions are reduced to a set of nonlinear, ordinary differential equations with nonconstant coefficients by using the Haskind hypothesis to evaluate the hydrodynamic force and moment integrals and to represent them as frequency dependent coefficients. Buoy motions are coupled through hydrostatic, hydrodynamic, and mooring line forces.

N.S

Cable dynamics are also investigated. A set of coupled, hyperbolic, partial differential equations for cable motions are developed, and characteristic equations are derived to effect a method of characteristics solution. A unique numerical method of characteristics technique, based upon Hartree's method, is developed for the solution of the cable equations in the time-space domain. Buoy motions, which are dependent upon the cable tensions, serve as the upper boundary conditions. 'Lower boundary conditions are prescribed at the anchor, where there can be no motion.

For certain buoy systems, where many mass discontinuities exist along the cable, or for shallow water moorings, where slack cable conditions can exist, a lumped-mass method of computing cable dynamics is developed as opposed to the finite-difference method just described. In general, for cable dynamics the lumped-mass numerical method is an order of magnitude faster in computation time than the finite difference method.

The equations of motion developed for the buoy were solved numerically in the time domain using a fourth-order, Runge-Kutta integration method. Cable

ii

equations can be solved either by finite-difference methods or by integrating with the Runge-Kutta algorithm for the lumped-mass model.

In order to validate the numerical models developed, two buoy systems were instrumented and deployed in Block Island Sound. The motion data from these experiments, along with data published in the literature, are compared with simulated buoy motion data. This comparison indicates that steady-state buoy system forces and configurations can be predicted within approximately 5 percent and that buoy system dynamics can be predicted within approximately 50 percent. There are some indications that the surge and sway hydrodynamic forces acting on the buoy are being underestimated by the computer model.

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The buoys were installed and removed through the efforts of Lt. Robert B. Doughty, Master, and the crew of the U.S. Coast Guard buoy tender, REDWOOD. The many trips required to inspect and service the buoys were made possible by Dr. Andrew J. Nalwalk, of the University of Connecticut, and Oram A. Campbell, Master of the R/V UCONN and his crew.

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LIST OF SYMBOLS AND NOTATIONS

Buoy Dynamics

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a	Spheroid minor half-diameter
a	Frequency parameter
Α	Transform matrix from inertial to cable coordinates
A _H	Plan area of immersed buoy volume
A s	Profile area of immersed buoy volume
A 1	Buoy windage – profile area above free surface
A_2	Plan area of buoy
b	Spheroid major half-diameter
В	Hydrostatic force vector
с _р	Buoy hull drag coefficient
cv	Current component in y direction
CW	Current component in z direction
C _L	Buoy hull lift coefficient
d	Spheroid buoyant force moment arm
d	Viscous drag force component
D	Viscous drug force vector
D	Wind duration
$\overline{\mathtt{D}}$	Dimensionless duration parameter
e	2. 7182

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Buoy Dynamics (Cont'd)

F	Fetch
F _D	Force vector on an immersed body B
F	Dimensionless fetch parameter
F ₁	Dimensionless wave height parameter for Bretschneider spectra
F ₂	Dimensionless wave period parameter for Bretschneider spectra
g	Gravitation constant
G	Gravity vector
G _D	Moments on an immersed body B
н	Hydrodynamic force vector
H _D	Buoy draft
Н	Angular wave damping coefficient
H _{CB}	Height of center of buoyancy below free surface
H _{CG}	Height of center of gravity
н _{СР}	Height of center of pressure
H _{ML}	Height of the center of gravity of the buoy above mooring line termination
H W	Height of wind center of pressure from center of gravity
^H 1/3	Significant wave height
Ħ	Dimensionless significant wave height parameter
i	$\sqrt{-1}$
I	Structural mass moment of inertia
I	Hydrodynamic mass moment of inertia coefficient
IA	Area of the intersection of a body B with the free surface

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Buoy Dynamics (Cont'd)



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Buoy Dynamics (Cont'd)

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- u_j **Pressure function** Velocity components of the buoy center of gravity u, v, w v Wind velocity vector. W Wind force vector WD Wind drag force on buoy W_L Wind lift force on buoy WV Wind velocity components Spatial coordinates x, y, z Yo Bessel function of second kind of order 0 Yaw angle α
 - β Pitch angle
 - β Dimensionless frequency
 - Y Roll angle
 - ε Wave component phase angle
 - η Instantaneous free surface height below mean free surface
 - θ Cable tilt angle
 - θ Rotation vector
- $\theta', \theta'', \theta'''$ Angular displacements of body B
 - λ Wavelength
 - *μ* Absolute viscosity
 - v Kinematic viscosity
 - ξ Instantaneous wave surface slope
 - π 3.1416...

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Buoy Dynamics (Cont'd)

- ρ Fluid mass density
- P_A Air mass density
- σ Anguiar frequency of buoy motion
- "1/3 Significant wave period
 - **7** Dimensionless significant period parameter

- **φ** Cable transverse tilt angle
- $\Phi(s, y, z; t)$ Velocity potential
 - ω Angular frequency of wind waves
 - **Ω** Buoy rotation matrix

Subscripts

d	Viscous	drag
---	---------	------

- D Drag
- h Hydrodynamic
- i Coordinate system
- L Lift
- max. Maximum
- min. Minimum
 - o Static position
 - og Static position of center of gravity
 - S Surface
 - w Wave
- x, y, z Coordinate directions
- α, β, γ Buoy rotations
 - 1/3 Statistically significant

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Cable Dynamics

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a	Cable section radius
Α	Transform matrix from inertial to cable coordinates
C _{DN}	Normal drag coefficient
C _{DT}	Tangential drag overficient
Ch	Characteristic velocity
С _в	Current velocity vector
đ	Cable diameter
D	Viscous drag
F _H	Hydrodynamic inertia force
g	Gravitational constant
G	Cable loading function
h	Node separation in grid
Н	Cable loading function
î, j, k	Direction indices
I	Cable loading function
k	Time step size in grid
К	Linearized tension - strain derivative
m _h	Hydrodynamic mass
ฉี	Force vector on a cable segment
ŕ	Vector from origin to point on cable
Re	Reynolds number
S	Stokes number
t	Time
Ŧ	Tension vector

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Tension vector

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Cable Dynamics (Cont'd)

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Velocity components
Cable in-water weight per unit length
Spatial coordinates
Force components on cable
Strain
Cable tilt angle
Characteristic velocities
Cable structural mass per unit length
Kinematic viscosity
3.1416
Fluid mass density
Cable transverse tilt angle
Angular frequency of oscillation
Point (i, j) in time-space grid
Point (i-1, j) in time-space grid
Buoy
Point (i-1, j-1) in time-space grid
Point (i+1, j) in time-space grid
Point (i, j-1) in time-space grid
Point i+1, j-1) in time-space grid
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Subscripts (Cont'd)

- j Time
- L Low
- N Normal
- P_1 Point between B-A in grid for tensile characteristic

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- P_2 Point between A-D in grid for tensile characteristic
- Q1 Point between B-A in grid for transverse characteristic
- Q2 Point between A-D in grid for transverse characteristic
- R Point (i, j+1) in time-space grid
- S Current velocity
- T Tangential

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I. INTRODUCTION

Clean, green, windy billows notching out the sky, Grey clouds tattered into rags, sea-winds blowing high, And the ships under topsails, beating, thrashing by, And the mewing of the herring gulls.

Dancing, flashing green seas shaking white locks, Boiling in blind eddies over hidden rocks, And the wind in the rigging, the creaking of the blocks, And the straining of the timber hulls.

> John Masefield "Cardigan Bay"

This study is concerned with the analysis and simulation of the dynamics of simple oceanic buoy systems. The analysis must include the effect of the significant forces that act on the buoy system and are imposed by the ocean environment — wind, waves, and currents. Because of the highly nonlinear nature of the problem, numerical methods are favored in order to provide a realistic simulation.

Buoys have been employed by mariners for centuries as aids to navigation and to support mooring chains. In this country, navigational buoys were in service in the Delaware River in 1767 and in Boston Harbor by 1808. At present, the United States Coast Guard maintains over 24,000 buoys in the navigable waters of the United States and its possessions. Navigational buoy system design is largely a matter of employing "rules of thumb" evolved over decades of experience with these buoys. The vast majority of navigational

buoys are moored in waters less than 100 ft deep, and the most common cause of failure is collision with a vessel. Thus, these buoy systems are characterized by massive steel buoys moored with heavy chain to large concrete clumps. They are serviced on a regular basis and are recovered and overhauled annually.

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In recent years, oceanographers have used buoy systems to support current meters, thermistor chains, and other oceanographic instrumentation. The state of the art in oceanography has advanced to the point where oceanographers are no longer satisfied with data taken at a single point over a rather short time duration. Multiple measurements to be made simultaneously over wide areas of the ocean or long-duration measurements are made most economically with a buoy system equipped with self-recording or telemetering instrumentation. This economy can be realized only if the buoy system is designed to have a life on station greater than the desired measurement time.

The basic design philosophy of deep-sea oceanographic buoy systems is quite different from that of the navigational buoys: Oceanographic vessels are usually small and are not equipped for handling heavy objects over the side at sea; thus, the buoys and mooring line components must be of relatively light weight. The mooring lines are miles long and thus preclude the use of heavy chains (except at the bottom) and tend to be made up of light, high-strength wire ropes or of synthetic fiber ropes. Designers of oceanographic buoy systems are faced with the near-impossible task of designing a lightweight, highly compliant structure to survive for periods of a year or more in one of the harshest environments known to man.

Isaacs¹ describes a mean time between failure (MTBF) of 121 days for the taut-moored Scripps Institution of Oceanography "Catamaran" buoys. The observed system failures were due to parting of the nylon mooring line near the surface. The writers hypothesize that high tensile loads in the nylon line are caused by tensile waves propagating up and down the cable. Richardson² of theWoods Hole Oceanographic Institution (WHOI), anchored 106 buoy systems between Cape Cod and Bermuda. The MTBF for these systems was found to be about 90 days. The WHOI buoy system failures were attributed to mooring line failures, fish bite of synthetic mooring lines, and theft. In 1967, WHOI³ set nine long term buoy moorings of which only one was recovered on station after 60 days. Three of these were found adrift. WHOI was more successful in 1968, ⁴ when only 3 of 14 long-term buoy moorings failed.

All the oceanographic buoy systems described above were taut-moored systems, the majority of which utilized synthetic rope in their mooring lines. Their short life on station and their low recovery rate indicate a need for an accurate engineering method of computing the dynamic response of the buoy system to the ocean environment.

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Slack moored buoy systems have a much greater MTBF and are more reliable. The catenary of the mooring line provides the necessary compliance; thus, dynamic tensions in the mooring line are reduced. Navigational buoy systems are slack moored and are very reliable. Smith⁵ cites a long history of successful moorings in the Gulf of Mexico for the NOMAD buoy system. Over a 5-year period, a number of the NOMAD buoys were kept on station for periods of a year or more. Smith also describes fifteen, 25-ton barges that

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were slack moored for 8 months in water depths over 12,000 ft with no failures.

Oceanographers are also concerned with the effects of buoy system motions on their instrumentation. For example, instrumentation fastened to the mooring line of a slack-moored system will undergo depth excursions that are dependent on the current structure. Webster^{6, 7} discusses errors in selfrecording current meter data due to buoy system motion. Webster shows a current energy spectrum developed from a current meter that is attached to the mooring line of a buoy system (figure 1). The energy introduced into the current data by the buoy system motions is far greater than the energy of the currents themselves.

Marcus⁸ compared anemometer data taken from a NOMAD buoy in the Gulf of Mexico with other meteorological observations in the area over a 6-month period. The mean error of the wind speed data was 0.2 knot with a standard deviation of the error of ± 4.67 knots. Huff⁹ shows a power spectrum of anemometer data taken from another NOMAD buoy system moored off Fermuda. This spectrum (figure 2) indicates that a large amount of energy was introduced into the spectrum by the motion of the buoy. Huff also shows an increase in the average wind speed deviation from the mean with increasing mean wind speed that levels out at high wind speeds. This variation is characteristic of sea surface slopes, which have an upper limit due to gravity. This upper limit implies that the wind speed error is due to the pitch and roll motions of the buoy. Day¹⁰ found that wind data sampled every 10 min from a



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Figure 1. A Current Meter Energy Spectrum (From Webster, reference 7.)



Figure 2. An Anemometer Power Spectrum (From Webster, reference 9.)

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buoy-mounted anemometer had to be time averaged over a 2-hr period to remove errors due to buoy motions.

The two-dimensional, steady-state configurations of buoy cable systems have been investigated by Wilson.^{11, 12} Wilson constrained the upper end of the cable to be at the mean ocean surface and did not consider cable elasticity. Patton, ¹³ as part of this dissertation research, developed a numerical method to determine the equilibrium configuration of buoy cable systems. The threedimensional configuration for any current structure (currents may vary in strength and direction as a function of depth), as well as the buoy draft, is computed. The elasticity of the cable is considered and the stretch is also computed. Martin¹⁴ developed a numerical method similar to Patton's, but it is restricted to two dimensions. Martin also experimentally investigated the elastic properties of nylon rope and the drag of buoy models. Smith⁵ presented a graphical method for two-dimensional buoy system configurations but neglected tangential drag and elasticity.

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The study of the motions of bodies floating on the ocean surface and being excited by waves originated in 1749 with Euler¹⁵ in his classic work, <u>Scientia</u> <u>Navalis</u>. Froude¹⁶ was concerned with the rolling and roll stability of ships. Froude recognized the nonlinearity of the problem and included viscous resistance in the equations of motion. Kriloff¹⁷ investigated ship motions and wrote coupled equations of motion. Both Frouder and Kriloff assumed that the ship did not influence the waves, which allowed them to treat the hydrodynamic properties of the ship as a body oscillating on a free surface. By far, the

single greatest problem in ship dynamics is the description of the hydrodynamic forces acting on the ship. Lewis¹⁸ introduced the "strip theory" in 1929,wt . . allowed computation of three-dimensional hydrodynamic characteristics from two-dimensional theory. Haskind^{19,21} assumed that the hydrodynamic equations could be linearized in such a manner that velocity potentials could be superimposed. This method allowed the use of three velocity potentials: (1) incident wave potential, i.e., the velocity potential of the waves alone; (2) diffracted wave potential, i.e., the velocity potential of the body fixed on a free surface exposed to waves; and (3) forced heave potential, i.e., the velocity potential of the body oscillating in still water. Naval architects currently favor the Haskind hypothesis as opposed to the earlier Froude-Kriloff hypothesis, which assumes that body dimensions are small compared with the waves.

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John^{22, 23} wrote complete sets of coupled equations of motion for floating bodies in harmonic waves and considered the influence of the body on the waves (after Haskind). John also included an external force term that could be used to describe a mooring cable. Heave and surge motions of a sphere were computed for various wave frequencies.

St. Denis and Pierson²⁴ linearized the decoupled equations of motion for a ship and investigated ship motions in confused seas by summing the ship's responses to sine wave seas of different frequencies. Korvin-Kroukovsky²⁵⁻²⁷ used the "strip" method to compute the hydrodynamic characteristics of the ship and included cross-coupled hydrodynamic forces. A complete discussion of the state of the art in the prediction of ship motions is presented in the proceedings of the fifth symposium on naval hydrodynamics.²⁸ Current

research indicates that cross-coupled hydrodynamic forces are the same order of magnitude as other hydrodynamic forces and can not be neglected.

A large oceanographic buoy was built in 1965 by General Dynamics/ Convair Division. As part of the design process, ²⁹ model tests were conducted in a towing tank for various buoy hull shapes, and buoy motions were simulated on an analog computer. The analog computer simulation considered the dynamics of the planar motions of the buoy alone; the mooring line was treated as an elastic spring. The following quote is from reference 29:

• The simulation was not fully successful; some results are considered inconclusive. Due to the difficulty in obtaining reasonable agreement with the model data, the analog computer study was terminated short of its goal.

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The 40-ft-diameter "MONSTER" buoy described in reference 29 has proven to be a successful ocean data station. At sea motions of this buoy are described by Devereux³⁰ and Uyeda.³¹ Gaul and Brown³² correlated buoy heave acceleration power spectra from the "MONSTER" buoy and from a small wave sensing buoy.

Paquette³³ developed a two-dimensional, lumped-mass analog computer model for buoy system dynamics. The buoy was assumed to follow an elliptical orbit (major axis vertical and equal to the wave height), and its motions were not integrated as part of the system dynamics. The lumped-mass cable elements were acted upon by tensions on adjacent elements, weight and buoyancy forces, and velocity-squared drag forces. Cable hydrodynamic masses and linear

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damping forces were neglected. Paquette concluded that at least ten mass elements are needed for a deep-sea mooring line to adequately describe the system dynamics in the band of ocean wave frequencies that were considered (0 to 0.5 Hz). The coupling of tensile waves into transverse waves due to the steady-state curvature of the cable was also noted.

Bivens and Swann³⁴ also developed a two-dimensional. lumped-mass simulation of buoy system dynamic but included the buoy dynamics. However, hydrodynamic cross-coupled terms were neglected. Rudnick³⁵ measured motions of the "FLIP" spar buoy at sea and compared motion power spectra with the power spectra predicted from a linear, decoupled buoy motion model. Blumberg and Osborn³⁶ developed a digital computer simulation for submerged buoy motions. The mooring line was considered to be a rigid, massless, and dragless link. Hydrodynamic forces acting on the buoy included no crosscoupled terms, and the equations of motion were linearized. Millard³⁷ describes tension measurements made at sea as part of the Woods Hole Oceanographic Institution buoy reliability program. Tension amplitudes were correlated with recorded currents and wind speeds. Millard's data indicate that the tension amplitudes are attenuated with length down the mooring cable. A very comprehensive study of buoy system dynamics has been conducted by Prof. Nath, of Oregon State University.³⁸ The two-dimensional motions of a buoy and its mooring cable were considered. Buoy motions were solved by use of recurrence formulas and served as boundary conditions for the cable. Cable dynamics were solved by using a numerical method of characteristics solution.

Hydrodynamic forces acting on the buoy and cable were included, and nonlinear stress-strain properties of the cable were used. Transfer functions between wave spectra and line tension spectra along the cable were developed and compared with MONSTER buoy data.

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Hsu and Blenkarn³⁹ utilized momentum flux equations to compute the hydrodynamic forces acting on a moored ship. Equations of motion were solved numerically, and the mooring lines were considered as elastic springs. Each wave was assumed to impart an impulse; thus, the forcing function was composed of a series of impulses acting on the ship. Burke⁴⁰ assumed that a set of linear response functions for the vessel were known and developed sets of statistical relations for vessel motions in a random sea. This technique was applied to predict drilling vessel motions, and the results were compared with drilling vessel motions recorded at sea.

The present investigation has produced a three-dimensional, numerical model for buoy system dynamics. The model includes cross-coupled hydrodynamic forces and can be excited by wind, current, and wave forces. Cable dynamics are investigated with both finite-element (lumped masses) and finitedifference (distributed mass) methods. In this study, finite-element methods were found to be attractive because of their relative economy with regard to numerical computational time. Finite-difference methods, while more rigorous, are more involved numerically and require relatively large amounts of computer time.

The deterministic model is excited by a numerical wave model having the same spectral characteristics as the ocean waves. The computed buoy system

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response is then sampled to provide motion spectra by using Fast Fourier Transform (FFT) techniques.

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To validate the model, two oceanographic buoys were equipped with motion sensing instrumentation and installed in Block Island Sound. Buoy motions were monitored and recorded for various wind, current, and wave conditions. These data are compared with buoy motions computed with the numerical model for the same environmental conditions.

II. PROCEDURE

In order to predict the response of the buoy system (figure 3) to the ocean environment, a deterministic model of the system dynamics must be constructed and excited by a random model of the oceanic conditions. The two major structural components of the system (the buoy and the mooring line) are treated separately and then are combined to form the deterministic buoy system model.

2.1 System Dynamics

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Buoy motions can be described by the equations of motion for a body with six degrees of freedom floating on the free surface of a fluid. ²⁸ The major problem encountered in the solution of the set of six, coupled, elliptical, differ-. ential equations of motion is the description of the hydrodynamic forces acting on the buoy. In their most rigorous form, the buoy equations of motion would be integro-differential equations since the dynamic pressures must be integrated over the immersed surface of the buoy. Analytical solution of these equations of motion for an arbitrary body in a random sea state has not been accomplished up to this time. If the hydrodynamic forces can be expressed as variable coefficients in the equations of motion, the equations can be written as a set of six, ordinary differential equations that can be solved by using the approximate methods of numerical techniques. The problem now is to define the variable hydrodynamic coefficients. If the fluid is assumed to be incompressible and



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Figure 3. A Simple Buoy System

irrotational, the hydrodynamic properties of certain simple two- and threedimensional bodies can be computed. Also, if the waves on the free surface of the fluid are deterministic and linear, the velocity potentials for various body motion modes can be superimposed to construct the case of a body floating on a free surface and responding to waves propagating on that free surface.

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This study employs the technique described above. Equations of motion are written as a set of six, coupled, ordinary differential equations with variable coefficients; hydrodynamic coefficients are computed by assuming that the fluid is incompressible and irrotational and that velocity potentials can be superimposed; aerodynamic and hydrodynamic viscous forces are assumed to follow a velocity-squared law; and the body is assumed to be axisymmetric about a vertical axis (as are most oceanographic and navigational buoys), which simplifies the computation of the hydrodynamic coefficients.

Dynamics of cables are investigated and simulated. The most direct approach, i.e., solution of the cable equations of motion by a finite-difference method, is developed first. Since the cable equations are a set of nonlinear, hyperbolic, partial differential equations, analytical solutions are intractable, and a numerical method of solution is devised. Although more accurate, the finite-difference method can be very expensive with regard to digital computer time. A lumped-mass simulation of cable dynamics is also investigated and developed. Lumped-mass inethods offer significant savings in computational time at the expense of truncation of the higher frequency cable dynamics.

The buoy equations of motion and the two sets of cable equations (finitedifference and lumped-mass) are then coupled and solved numerically on a

UNIVAC 1108 digital computer. A numerical model of ocean waves is used to excite the buoy system dynamics model. Steady-state buoy system configurations are solved as the zeroth-order case of buoy system dynamics.

2.2 Experimental Validation

In order to validate the computer model, two oceanographic buoys were equipped with motion sensing instrumentation and were monitored. The recorded buoy motion data have been reduced in statistical form and will be correlated with buoy motions predicted from the computer models. Also, buoy motion data reported in the literature have been used to validate the computer models.

III. ANALYTICAL DEVELOPMENT AND DISCUSSION

3.1. Buoy Dynamics

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Consider an axisymmetric buoy having six degrees of freedom, floating on the free surface of a fluid, constrained by a mooring line, and exposed to wind, waves, and currents (figure 4). The buoy is being acted on by the following:

Inertial forces and moments

Hydrostatic forces and moments

Hydrodynamic forces and moments

Wind forces

Mooring line tensions.

The inertial forces can be separated into those due to gravity (weight) and those due to the motion of the buoy. Hydrostatic forces can be obtained by integration of the hydrostatic pressure acting on the submerged surface of the buoy. Likewise, hydrodynamic forces can be obtained by integration of the hydrodynamic pressures acting on the submerged surface of the buoy. Hydrodynamic forces are classified as inertial or dissipative. Energy is being dissipated through viscous effects, radiation of pressure waves, and radiation of surface waves generated by the motion of the buoy. Energy is introduced to the system through hydrostatic and hydrodynamic forces due to currents and surface waves.



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Figure 4. Buoy Coordinate Systems

Wind forces acting on the exposed surface of the buoy also introduce energy into the system. Mooring line tensions indicate a path of energy removal from the system. If the buoy were treated as a "black box" that transforms

From figure 4, it is seen that four sets of coordinates must be considered. It is desired to solve for the coordinates of the center of gravity of the buoy in inertial coordinates, (x_{og}, y_{og}, z_{og}) , but the hydrostatic and hydrodynamic forces and moments are due to fluid motions relative to the buoy. Any point in space has coordinates x_i , y_i , z_i relative to R_i (i = 0, 1, 2, 3); therefore,

energy from one form to another, we can draw a schematic as shown in figure 5.

$$x_{o} = x_{og} + x_{i}$$
$$y_{o} = y_{og} + y_{i}$$
$$z_{o} = z_{og} + z_{i}$$

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The only difference between R_1 and R_2 is a space rotation about the axes of the buoy. Thus, we have

$$\begin{bmatrix} X_2 \\ Y_2 \\ Z_2 \end{bmatrix} = \Omega \cdot \begin{bmatrix} X_1 \\ Y_1 \\ Z_1 \end{bmatrix}$$

where Ω is a 3-by-3 orthogonal rotation matrix,

	Cos V cos B	-cos y sinß sina +sin y cosa	cos t sin b cosa + sin t sin a	2 2 2 2
Ω	-SIN & cos,B	SIN T SINB SINA +COST COS A	-SIN FSINBCOSA +COSTSINA	
	-SIN B	- cos B sina	cos,8 cos ∝	• (2

(1)



MATRIX EQUATIONS OF MOTION

 $\dot{MQ} = MG - B - H + W - T$

WHERE:

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- M MASS MATRIX
- **Q** ACCELERATION VECTOR
- **G** GRAVITY VECTOR
- **B HYDROSTATIC FORCE & MOMENT VECTOR**
- H HYDRODYNAMIC FORCE & MOMENT VECTOR
- W WIND FORCE VECTOR
- T CABLE TENSION VECTOR

Figure 5. Energy Flow in Buoy System

Since Ω is orthogonal, we see that $\Omega' = \Omega'$; thus,

$$\Pi^{-1} = \begin{bmatrix}
\cos x \cos \beta & -\sin x \cos \beta & -\sin \beta \\
-\cos x \sin \beta \sin \alpha & \sin \beta \sin \alpha & -\cos \beta \sin \alpha \\
+\sin x \cos \alpha & +\cos x \cos \alpha \\
\cos x \sin \beta \cos \alpha & -\sin x \sin \beta \cos \alpha \\
+\sin x \sin \alpha & +\cos x \sin \alpha
\end{bmatrix}$$
(3)

The spatial coordinates become

$$X_{o} = X_{og} + X_{z} \cos y \cos \beta - Y_{z} \sin y \cos \beta - Z_{z} \sin \beta, \qquad (4A)$$

$$Y_{0} = Y_{0g} + X_{2} (-\cos \sin \beta \sin \alpha + \sin \gamma \cos \alpha) + Y_{2} (\sin \sin \beta \cos \alpha + \cos \gamma \sin \alpha) + Z_{2} (-\cos \beta \sin \alpha) ,$$
^(4B)

And

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$$Z_{0} = Z_{0g} + X_{2} (\cos \gamma \sin \beta \cos \alpha + \sin \gamma \sin \alpha) + Y_{2} (-\sin \gamma \sin \beta \cos \alpha + \cos \gamma \sin \alpha) + Z_{2} (\cos \beta \cos \alpha). \quad (4C)$$

The center of the coordinate system, which is aligned with the waterplane area, is located directly above the center of gravity of the buoy. Motions of fluid particles due to waves are described relative to this coordinate system. The dynamical equations of motion will be written in buoy coordinates, but displacements will be transformed to the R_0 coordinate system in order to solve for cable tensions.

Using the free body of the buoy (figure 4) and applying Newton's Second Law, we can develop the equations of motion for the buoy. In matrix form, the

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equations of motion for the buoy are

$$M\ddot{Q} = MG - B - H - W - T, \qquad (5)$$

where

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- M is the structural mass matrix
- \dot{Q} is the acceleration vector
- G is the gravitational acceleration vector
- B is the hydrostatic force vector
- H is the hydrodynamic force vector
- \boldsymbol{W} is the wind force vector
- T is the mooring line tension vector.

Each of these forces will be considered in turn.

Using a coordinate system with the origin located at the center of gravity of the buoy and including moments and products of inertial, we can write the structural mass matrix as

$$M = \begin{bmatrix} m & 0 & 0 & 0 & 0 & 0 \\ 0 & m & 0 & 0 & 0 & 0 \\ 0 & 0 & m & 0 & 0 & 0 \\ 0 & 0 & 0 & I_{\alpha\alpha} & I_{\alpha\beta} & I_{\alpha\beta} \\ 0 & 0 & 0 & I_{\beta\alpha} & I_{\beta\beta} & I_{\beta\beta} \\ 0 & 0 & 0 & I_{\gamma\alpha} & I_{\gamma\beta} & I_{\gamma\gamma} \end{bmatrix}$$

(6)

If the coordinate system is aligned with the principal axes of the buoy,

the structural mass matrix is

$$\mathbf{M} = \begin{bmatrix} \mathbf{m} & \mathbf{0} & \mathbf{0} & \mathbf{0} & \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \mathbf{m} & \mathbf{0} & \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \mathbf{0} & \mathbf{m} & \mathbf{0} & \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \mathbf{0} & \mathbf{0} & \mathbf{I}_{\alpha\alpha\alpha} & \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \mathbf{0} & \mathbf{0} & \mathbf{0} & \mathbf{I}_{\boldsymbol{\beta},\boldsymbol{\beta}} & \mathbf{0} \\ \mathbf{0} & \mathbf{0} & \mathbf{0} & \mathbf{0} & \mathbf{0} & \mathbf{I}_{\boldsymbol{\gamma},\boldsymbol{\gamma}} \end{bmatrix}, \qquad (7)$$

The acceleration vectors are

$$\vec{Q} = \begin{bmatrix} \vec{x} \\ \vec{y} \\ \vec{z} \\ \vec{x} \\ \vec{z} \end{bmatrix} \text{ and } \vec{G} = \begin{bmatrix} \vec{g} \cos \vec{y} \cos \vec{x} \\ -\vec{g} \sin \vec{x} \end{bmatrix} -\vec{g} \vec{x} \end{bmatrix}$$

3.1.1 Wind Forces

The wind force vector is now considered. For an axisymmetric buoy about the x_2 exis and assuming a velocity-squared viscous drag, the drag and lift forces acting on the body are given by

$$D = \frac{1}{2} \mathcal{P}_{p} G_{p} A_{i} WV^{2}$$

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and

$$L = \frac{1}{2} \int_{R} G_{L} A_{2} WV^{2}, \qquad (10)$$

where

 C_{p}

 G_{L}

 ${f D}$ is the drag force

L is the lift force

 \mathcal{P} is the air density

is the drag coefficient (subcritical, $\frac{WV H P_{A}}{M_{B}} < 5 \times 10^{5}$) is the lift coefficient (subcritical, $\frac{WV H P_{A}}{M_{B}} < 5 \times 10^{5}$)

 A_1 is the vertical projected area

 A_2 is the horizontal projected area

WV is the wind velocity

 \mathcal{M}_{n} is the absolute viscosity of the air

A is the buoy width .

Because of the axial symmetry, the drag and lift coefficients of the buoy are the same in the y_2 and z_2 directions. Given wind velocity components WV_{yo} and WV_{zo} in the R_o coordinate system and assuming that the wind velocities are an order of magnitude greater than the displacement velocities of the buoy, we can transform to the R_2 coordinate system and can compute the wind forces and moments. For forces acting on a point that lies on the axis of symmetry of the buoy, the transform from inertial coordinates to buoy coordinates is independent of α rotations.

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The wind components in the R_2 coordinate system are

$$\begin{bmatrix} WV_{x2} \\ WV_{y2} \\ WV_{z2} \end{bmatrix} = \Omega \cdot \begin{bmatrix} O \\ WV_{y0} \\ WV_{z0} \end{bmatrix}$$
(11)

Neglecting the small wind velocity component acting along the axis of symmetry, the magnitude of the wind velocity in the R_2 coordinate system is

$$WV_{R_2} = \sqrt{WV_{Y_2}^2 + WV_{I_2}^2}, \qquad (12)$$

and the wind forces become

 $W_{D_2} = \frac{i}{2} \mathcal{P}_{R} G_{D} A_{I} W_{R_2} | W_{R_2} | \qquad (13)$

and

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$$W_{L_2} = \frac{1}{2} \mathcal{P}_{R_1} \mathcal{C}_L \mathcal{A}_2 \mathcal{W}_{R_2} \mathcal{W}_{R_2} \mathcal{W}_{R_2}$$
(14)

(Note that velocities squared is written as the product of the velocity and its absolute value in order to maintain the sign convention.) Resolving the wind drag force into y_2 and z_2 components, we find that

$$W_{y_2} = \frac{WV_{y_2}}{WV_{R_2}} \cdot W_{\rho_2}$$
(15)

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and

$$W_{z_2} = \frac{WV_{z_2}}{WV_{R_2}} \cdot W_{p_2} \cdot$$
(16)

The wind moments are computed by usiving the wind forces and the moment arm:

$$W_{\alpha_2} = O \tag{17A}$$

$$W_{\mathcal{B}_{\mathcal{Z}}} = -W_{\mathcal{Z}_{\mathcal{Z}}} \cdot H_{w} \tag{17B}$$

and

$$W_{\gamma_2} = -W_{\gamma_2} \cdot H_w \tag{17C}$$

where H_w is the height from C. of G. to the wind force center of pressure.

In the $\,{\rm R}_{2}^{}\,$ coordinate system, the wind forces and moments are

$$W_{R_2} = \begin{bmatrix} -W_{X_2} \\ W_{Y_2} \\ W_{Z_2} \\ 0 \\ W_{B_2} \\ W_{Y_2} \end{bmatrix}$$

(18)

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3.1.2 Mooring Line Forces

Mooring line tensions are acting on the buoy at the mooring line termination point. This point is taken to be below the center of gravity and along the axis of symmetry a distance H_{ML} from the center of gravity. If the space orientation of the cable is described by angles Θ and ϕ relative to the R_0 coordinate system (figure 4), we can develop a 3-by-3 orthogonal rotation matrix to transform from inertial to cable coordinates. The inverse of the matrix can be used to compute the tension components acting on the buoy. This rotation matrix, (A), is developed in the next section on cable dynamics but is used here.

The tension components at the buoy end of the cable are

$$T_{B_{o}} = A^{-\prime} \begin{bmatrix} T \\ 0 \\ 0 \end{bmatrix} = \begin{bmatrix} T_{x_{o}} \\ T_{y_{o}} \\ T_{z_{o}} \end{bmatrix}$$
(19)

In the R_2 coordinate system, they become

$$T_{B_2} = \Omega \cdot T_{B_0} = \begin{bmatrix} T_{x_2} \\ T_{y_2} \\ T_{z_2} \end{bmatrix}.$$
(20)

The moments due to the mooring line tension are

$$T_{\alpha_2} = 0 \tag{21A}$$

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$$T_{\beta_2} = T_{z_2} \cdot H_{ML} \tag{21B}$$

$$T_{\chi_2} = T_{\chi_2} \cdot H_{ML}$$
 (21C)

The forces and moments due to cable tensions are then

$$T = \begin{bmatrix} T_{x_2} \\ T_{y_2} \\ T_{\overline{z}_2} \\ O \\ T_{\beta_2} \\ T_{\gamma_2} \end{bmatrix}$$
(22)

3.1.3 Hydrostatic Forces

The hydrostatic forces are considered next. If the displacements of an elemental volume of fluid just below the free surface and in the immediate vicinity of the buoy are given relative to the inertial coordinates (η) , and if the slope of the free surface above this particle is also given (ξ) , the buoyant forces and moments can be computed. The assumptions that the buoy diameter is small relative to the wavelength and that the presence of the buoy does not influence the shape of the free surface as a plane intersecting with the body volume.

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Most oceanographic buoys have axial symmetry, and their shape can be approximated by an oblate spheroid. Consider an ellipsoid of revolution (figure 6) with a major diameter of 2b and a minor diameter of 2a. The equation of the surface is

$$\frac{\chi_b^2}{H^2} + \frac{\gamma_b^2}{b^2} + \frac{\Xi_b^2}{b^2} = 1.$$
 (23)

Let a plane intersect the oblate spheroid at a height H_d from the geometric center at an angle \mathcal{A}' . The intercepts of the plane are

$$\begin{array}{l} x = -H_{p} \\ y = \infty \\ z = -H_{p} / t + n \mathcal{B}' \end{array}$$

The equation of the plane becomes

$$\frac{X_{b}}{-H_{p}} + \frac{Z_{b}}{-H_{p}/\tan\beta'} = 1$$
⁽²⁴⁾

or

$$X_b + Z_b \tan \beta' + H_p = 0 \qquad (24A)$$

The intersection of the body and the plane is therefore given by

$$\frac{X_{b}^{2}}{H^{2}} + \frac{Y_{b}^{2}}{b^{2}} + \frac{Z_{b}^{2}}{b^{2}} - 1 = X_{b} + Z_{b} \tan \beta' + H_{b}$$
(25)

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Figure 6. Immersed Volume of an Oblate Spheroid

The survey of

$$\frac{\chi_{b}^{2}}{H^{2}} + \frac{\chi_{b}^{2}}{b^{2}} + \frac{z_{b}^{2}}{b^{2}} - \chi_{b} - z_{b} t_{An} B' - H_{b} - 1 = 0, \quad (25A)$$

which is an ellipse. In order to compute the volume of the "cut" oblate spheroid, the area of any section parallel to the cutting plane must be defined. In the $x_b - z_b$ plane, we find that on the ellipse

$$X_{b}^{2} = A^{2} - \frac{A^{2}}{b^{2}} \cdot Z_{b}^{2}$$
(26)

and on the line

$$X_{b}^{2} = Z_{b}^{2} t_{n}^{2} B' + 2 H_{p} Z_{b} t_{n} B' + H_{p}^{2} . \qquad (27)$$

Equating (26) and (27), we can compute the z coordinates of intersecting points P_1 and P_2 by

$$(\tan^2 \beta + \frac{\mu^2}{b^2})Z_b^2 + 2Z_bH_b \tan\beta' + (H_b^2 - \mu^2) = 0$$
(28)

Solving for $\mathbf{z}_{\mathbf{b}}$, we find

$$Z_{b} = \frac{-H_{b} \tan \beta' \pm \frac{H_{b}}{b} \sqrt{b^{2} \tan^{2} \beta' + A^{2} - H_{b}^{2}}}{(\tan^{2} \beta' + \frac{H^{2}}{b^{2}})}$$
(29)

For P_1 , the coordinates are

$$Z_{b_{1}} = \frac{-H_{0} \tan \beta' + H_{b} \sqrt{b^{2} \tan^{2} \beta' + H^{2} - H_{0}^{2}}}{(\tan^{2} \beta' + H^{2} / b^{2})}$$
(30)

or

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and

$$X_{b_1} = - Z_{b_1} \tan \beta' - H_{p}; \qquad (31)$$

for P_2 , the coordinates are

$$Z_{b_{2}} = \frac{-H_{p} \tan \beta' - \frac{\eta_{b}}{5} \sqrt{b^{2} \tan^{2} \beta' + H^{2} - H_{p}^{2}}}{(\tan^{2} \beta' + \frac{\eta^{2}}{5^{2}})}$$
(32)

and

$$X_{b_2} = - Z_{b_2} \tan \beta' - H_{b_j}$$
⁽³³⁾

thus,

$$Z_{b_1} - Z_{b_2} = \frac{2 \frac{\mu_b}{b} \sqrt{b^2 t_{nn^2} \beta' + \mu^2 - H_p^2}}{(t_{nn^2} \beta' + \mu^2_{b^2})}, \qquad (34)$$

The b' axis of the intersecting ellipse is

$$b' = \frac{1}{2} \frac{(Z_{b_1} - Z_{b_2})}{\cos \beta'}$$

From equation (34), we substitute and find

$$b' = \frac{H_{/b} / b^2 t_{Hn}^2 \beta' + H^2 - H_b^2}{\cos \beta' (t_{Hn}^2 \beta' + H^2 / b^2)}$$
(35)

The z coordinate of the center of the elliptical section is

$$Z_{c} = Z_{b_{1}} - b' \cos B' = \frac{-H_{b} \tan B'}{(\tan^{2} B' + H^{2}/b^{2})}$$
(36)

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The x coordinate of the center of the elliptical section is

$$X_{c} = \frac{H_{b} \tan^{2} \beta'}{(\tan^{2} \beta' + H^{2} / b^{2})} - H_{b} \qquad (37)$$

The dimension of the a^{\dagger} axis of the intersecting ellipse is one-half the y dimension at the center. The y dimension at the center is

$$Y_{b} = \pm \sqrt{b^{2} - \frac{H_{b}^{2} t nn^{2} B'}{(t nn^{2} B' + H^{2} b^{2})^{2}}} - \frac{b^{2}}{H^{2}} \left(\frac{H_{b} t nn^{2} B'}{(t nn^{2} B' + H^{2} b^{2})} - H_{b}\right)^{2}$$
(38)

and the axis length is

$$H' = \sqrt{b^2 - \frac{H_b^2}{(t_{Hn}^2 B' + H^2/b^2)}},$$
(39)

The area of the intersecting ellipse is

$$A = \pi H'b' = \pi \int_{a}^{b} -\frac{H_{p}^{2}}{(tnn^{2}B' + H_{p}^{2})^{2}} \cdot \frac{H_{b}}{b} \int_{cos,B'(tnn^{2}B' + H_{p}^{2})}^{b'} \frac{H_{b}^{2}}{cos,B'(tnn^{2}B' + H_{b}^{2})}$$

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$$A = \frac{\pi H b}{\cos B'} \left(\frac{1}{(\tan^2 B' + H^3_{b^2})^{\frac{1}{2}}} - \frac{H_b^2}{b^2 (\tan^2 B' + H^3_{b^2})^{\frac{3}{2}}} \right)$$
(40)

The volume of the "cut" oblate spheroid is given by

$$V = \int_{-H_{10}}^{H_{D}} A dH'$$
(41)

The lower limit of the integral can be found by setting the area of the cutting plane equal to zero (A = 0). The deepest draft is

$$H_{L0} = \sqrt{b^2 t A n^2 B' + A^2} ; \qquad (42)$$

thus, with this lower limit, the volume is

$$V = \int \frac{TT H b}{\cos \beta'} \left(\frac{1}{(t a n^2 \beta' + A^2 b^2)^2} - \frac{H'^2 (\cos^2 \beta')}{b^2 (t a n^2 \beta' + A^2 b^2)^2} \right) dH',$$

- $\cos \beta' (b^2 t a n^2 \beta' + B^2)^2$

or, when integrating, it becomes

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$$V = TT A b^{2} \left[\frac{2}{3} + \frac{H_{D}}{(b^{2} t A n^{2} B' + A^{2})^{\frac{1}{2}}} - \frac{H_{D}^{9}}{3(b^{2} t A n^{2} B' + A^{2})^{\frac{3}{2}}} \right]$$
(43)

The location of the centroid is given by

$$H_{cB}' = \frac{\int H' \, dV}{V}$$

Expanding, we find

$$H_{cB}' = \frac{1}{V} \int_{-\cos\beta'}^{H_{b} \cos\beta} \left(\frac{1}{(t_{Rn}^{2}\beta' + n_{b}^{2})^{2}} - \frac{H'^{2}/\cos^{2}\beta'}{b^{2}(t_{Rn}^{2}\beta' + n_{b}^{2})^{3}k} \right) H' dH'$$

Thus, carrying out the integration, we see that the centroid location becomes

$$H_{cg}^{'} = \frac{\frac{\cos \beta'}{2} \left[H_{p}^{2} - \frac{(b^{2} t_{Hn}^{2} \beta' + H^{2})}{2} - \frac{H_{p}^{4}}{2(b^{2} t_{An}^{2} \beta' + H^{2})} \right]}{\left[\frac{2}{3} \sqrt{b^{2} t_{An}^{2} \beta' + H^{2}} + H_{p} - \frac{H_{p}^{3}}{3(b^{3} t_{An}^{2} \beta' + H^{2})} \right]}, \quad (44)$$

The H dimension of the centroidal plane is

$$H_{c8} = \frac{\frac{1}{2} \left[\frac{H_{b}^{4}}{2 (b^{2} t nn^{2} B' + H^{2})} - H_{b}^{2} + \frac{(b^{2} t nn^{2} B' + H^{2})}{2} \right]}{\left[\frac{2}{3} \sqrt{b^{2} t nn^{2} B' + H^{2}} + H_{b} - \frac{H_{b}^{3}}{3 (b^{2} t nn^{2} B' + H^{2})} \right]},$$
(45)

The x coordinate of the centroid is

$$X_{cB} = \left(\frac{\tan^2 \beta'}{(\tan^2 \beta' + \mu_{b}^2)} - 1\right) \cdot \mathcal{H}_{cB}$$
(46)

The z coordinate of the centroid is

$$Z_{cB} = \left(\frac{-\tan\beta'}{(\tan^2\beta' + H^2/b^2)}\right) \cdot H_{cB}$$
(47)

The moment arm for the buoyant force is

 $d = d_1 + d_2,$

where the distances d_1 and d_2 are given by

$$d_{I} = \sqrt{X_{cB}^{2} + \Xi_{cB}^{2}} \cdot \cos(\alpha' + \beta'),$$

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$$d_1 = \mathbb{Z}_{CB} \cos \beta' + X_{CB} \sin \beta',$$

and

$$d_2 = H_{cq} \sin \beta'$$

The angle ∞' is defined as

$$\infty' = tan^{-1} \frac{X_{CO}}{Z_{CB}},$$

34

The moment arm is

$$d = Z_{co} \cos \beta' + (H_{cg} - X_{cs}) \sin \beta', \qquad (48)$$

For the special case of a sphere of radius A, the immersed volume is

$$V = \pi H^{3} \left[\frac{2}{3} + \frac{H_{p}}{H} \cos \beta' - \frac{H_{p}^{3}}{3H^{3}} \cos^{3} \beta' \right], \quad (49)$$

The height of the plane intersecting the center of buoyancy is

$$H_{cg} = \frac{3\left[\frac{H_{b}^{4}}{H^{2}}\cos^{2}\beta' - 2H_{b}^{2} + H^{2}sec^{2}\beta'\right]}{4\left[2H \cdot sec\beta' + 3H_{b} - \frac{H_{b}^{3}}{H^{2}}\cos^{2}\beta'\right]},$$
(50)

The coordinates of the center of buoyancy are

$$X_{C3} = -\cos^2 \beta' \cdot H_{CB}$$
(51)

and

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$$\mathcal{I}_{CB} = -SIN \mathcal{B}' \cos \mathcal{B}' \cdot H_{CB} \qquad (E2)$$

The buoyancy moment arm is then

$$d = \mathbb{Z}_{cs} \cos \beta' + (H_{cs} - X_{cs}) \sin \beta'. \tag{53}$$

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If the buoy is pitched and rolled with angles \mathcal{B} and \mathcal{F} and the sea surface slope at the buoy is \mathcal{B}_{w} and \mathcal{F}_{w} , the slope of the sea surface relative to buoy coordinates is $\mathcal{B}_{s} = \mathcal{B} - \mathcal{B}_{w}$ and $\mathcal{F}_{s} = \mathcal{F} - \mathcal{F}_{w}$. The angle between the sea surface plane and the buoy vertical axis is

 $\mathcal{B}' = \cos^{-1}(\cos \gamma_5 \cos \beta_5)$. With the buoy tilt angle defined, along with the location of the buoy center of gravity relative to the sea surface, the buoyant force and moment can be computed for an oblate spheroid. The buoyant force B acts normal to the plane of the sea surface through the center of buoyancy. In buoy coordinates, the buoyant force vector is

$$B_{R_2} = \begin{bmatrix} -B \cos \gamma_s \cos \beta_s \\ -B \sin \gamma_s \\ -B \cos \gamma_s \sin \beta_s \end{bmatrix}.$$
(54)

The β buoyant moment becomes

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$$\mathcal{M}_{\beta} = (B\cos \delta_{s}\cos \beta_{s})(\mathbb{Z}_{CB}\cos \alpha) - (\mathcal{H}_{cc} - \mathcal{X}_{cs})(B\cos \delta_{s}\sin \beta_{s}), (55)$$

and the δ buoyant moment becomes

$$M_{s} = (B\cos \delta_{s}\cos \beta_{s})(z_{CB}\sin \alpha) - (H_{CG} - X_{CB})(B\sin \delta_{s})$$
(56)

The same transformations as used above can be applied to any axisymmetric buoy if the buoyancy can be defined as a function of the draft and tilt angle of the buoy. The angular stability of axisymmetric buoy hulls, as defined by the locations of the centers of buoyancy and gravity, is discussed in appendix A.

3.1.4 Hydrodynamic Forces

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The hydrodynamic forces acting on the buoy that are due to the waves incident on the buoy and the motion of the buoy in the fluid must be included in the equations of motion. Ideally, the computation of these forces should be made for a buoy moving in a viscous fluid exposed to a random sea state. However, the solution to this general problem is not tractable and the forces acting on a buoy moving sinusoidally in an ideal fluid is considered in this study. These forces are considered as being composed of two components — inertial and dissipative. Energy is dissipated from the buoy, which is moving in an ideal fluid, by the generation of surface waves that radiate cylindrically to infinity. Dissipative forces due to viscosity will be included in the equations of motion as separate force components.

The separation of dissipative forces into those due to surface wave generation and those due to viscous drag is supported by Havelock, ⁴¹ who concluded through dimensional analysis of the decay of oscillations of a prism on a free surface that the viscous damping is an order of magnitude less than the damping due to surface wave generation. Ogilvie²⁸ cites other model experiments which support Havelock's conclusions. Ogilvie also lists generalized equations of motion for ships in a seaway in which the viscous damping is either neglected or included as a separate force term, which is the current practice among naval architects.

The analysis of the motion of floating bodies conducted by John^{22, 23} illustrates the computation of the hydrodynamic forces. Consider a mechanical system consisting of a liquid and a partly immersed body B. The liquid is assumed to be incompressible and to have irrotational motion. The free surface extends to infinity in all directions (figure 7). The body B is assumed to be rigid and to describe a forced motion under the influence of external forces. The state of the liquid is described by the velocity potential $\phi(x, y, z; t)$, which satisfies LaPlace's equation. The boundary condition that the normal velocity of the particles along with the pressure is continuous across the surface must be satisfied. In addition, the pressure on the free surface is assumed constant and equal to the atmospheric pressure. Under these conditions, energy is gained or lost by the system only through waves arriving or departing at infinity or through the external forces.

The difficulties arising from the fact that the velocity potential, ϕ , is a solution of the potential equation determined by nonlinear boundary conditions on a variable boundary force linearization of the problem in order to make it tractable. Restricting the analysis to infinitesimal motions, note that the boundary conditions become linear conditions for the potential function ϕ on fixed surfaces corresponding to the rest or equilibrium position. The average free surface lies in a horizontal plane, and the average immersed surface S^o for the body B is for a position of equilibrium for B. On the average free surface, y = 0, the wave equation is

$$\phi_{tt} + g \phi_{y} = 0 \tag{57}$$

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Figure 7. An Object on a Free Surface

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On the average immersed surface, we find

$$\frac{\partial \phi}{\partial n} = X_t p_i + Y_t p_2 + Z_t p_3 + \theta_t' g_1 + \theta_t'' g_2 + \theta_t''' g_3, \quad (58)$$

where

1 B

X, Y, Z are the coordinates of the center of gravity of B $\theta', \theta'', \theta'''$ are the angular displacements of B

N is the unit normal of S°

 $p_{\mathbf{k}}$ are the components of n

 $\mathbf{g}_{\mathbf{k}}$ are the components of the moment of n about the center of gravity.

Six differential equations of motion for B must also be written. For small perturbations, they are linear, second-order differential equations in x, y, z, $\theta', \Theta'', \Theta'''$ with constant coefficients and integrals of ϕ in the inhomogeneous part. They are of the form

$$\frac{M}{P}Y_{tt} = -\iint_{S}\phi_{t} P_{2} dS - g \left[I^{A}(Y-Y_{o}) + I_{x}^{A} \Theta''' - I_{z}^{A} \Theta'' \right]_{S}^{(59)}$$

where

M is the mass of B

P is the fluid density

 I^{A} is the area of A, the intersection of the body and free surface $I_{x}^{A}, I_{\underline{x}}^{A}$ are the moments of A about vortical planes

G is the acceleration of gravity.

The integral expresses the hydrodynamic forces, whereas the right-hand term represents the hydrostatic forces.

For a buoy undergoing simple harmonic oscillations, ϕ can be defined as

$$\phi(\bar{x},\bar{y},\bar{z};t) = \operatorname{Re}\left[V(\bar{x},\bar{y},\bar{z})e^{-i\sigma t}\right], \qquad (60)$$

where

 σ is the angular frequency of the oscillation.

For an incompressible fluid, V is a solution to

$$\nabla^2 V(\bar{x}, \bar{y}, \bar{z}) = 0$$
 in $\bar{y} < 0$,

where V is complex valued. For small oscillations of the buoy, the amplitude of induced wave motion will be small compared with the wavelength. Thus, the linearized dynamic condition for ϕ on the free surface is

$$g\bar{\eta}(\bar{x},\bar{z};t) + \phi_t(\bar{x},0,\bar{z};t) = 0, \qquad (61)$$

where

 $ar{\eta}$ is the free surface elevation.

Equation (61) with the linearized kinematic condition,

$$\phi_{\overline{y}}(\overline{x}, 0, \overline{z}; t) = \overline{\eta}_{t}(\overline{x}, \overline{z}; t) , \text{ yields}$$

$$\frac{\partial}{\partial y} \nabla(\overline{x}, 0, \overline{z}) - k \nabla(\overline{x}, 0, \overline{z}) = 0 \quad \text{on } \overline{y} = 0 , \quad (62)$$

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where

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is the wave number,
$$k = \sigma^2 g = 2\pi/\bar{\lambda}$$
,

 $ar{\lambda}$ is the free wavelength.

The normal velocity across the immersed surface of the buoy is continuous; thus, we find

$$\frac{\partial}{\partial \bar{n}} \phi(\bar{x}, \bar{y}, \bar{z}; t) = (\dot{\bar{Q}} + \dot{\Theta} \times \bar{r}) \cdot n, \qquad (63)$$

which is equation (58) restated in vector form where

 \overline{Q} is the position vector of the body C. of G.; $\overline{Q} = X p_1 + Y p_2 + \overline{Z} p_3$, Θ is the rotation vector of body B; $\Theta = \Theta' q_1 + \Theta'' q_2 + \Theta''' q_3$, \overline{r} is the position vector of some point on the immersed surface of the body.

The kinematic condition is to be satisfied on the immersed surface in the undisturbed position, i.e.,

$$\frac{\partial}{\partial \bar{n}} \nabla(\bar{x}, \bar{y}, \bar{z}) = \sum_{j=1}^{6} \frac{\partial}{\partial \bar{n}} V_j(\bar{x}, \bar{y}, \bar{z}) = -i\sigma \left[\bar{Q} \cdot n + \Theta \cdot (\bar{r} \times n)\right]_{(64)}$$

Applying a Sommerfeld radiation condition at infinity, we find that a disturbance in the finite region should only produce an outgoing wave at a loge distance:

$$V(\bar{d}, \Psi, \bar{\gamma}) - A(\Psi) \bar{d}^{-\frac{1}{2}} e^{k\bar{\gamma}+ik\bar{d}} \longrightarrow O \text{ as } \bar{d} \rightarrow \infty$$
, (65)

where

$$d = (\bar{x}^2 + \bar{z}^2)^{\frac{1}{2}}$$

$$\psi = t_{\text{Hn}}^{-1} (\bar{z}/\bar{x})$$

To put the equations in dimensionless form, let

where a is a typical buoy dimension. Introduce the pressure function u_j by

$$i \sigma V_j(\bar{x}, \bar{y}, \bar{z})/g \bar{n} Q_j^\circ = n U_j(\bar{x}, \bar{y}, \bar{z}) \quad j = 1, 2, 3 \quad (66A)$$

and

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$$i \sigma V_j(\bar{x}, \bar{y}, \bar{z}) / g \bar{\mu} \Theta_j^\circ = \mu U_{ij}(x, y, \bar{z}) \quad j = 4, 5, \delta \quad (66B)$$

The boundary value problem is to find a potential $U_j(X, Y, Z)$, j=1,2,...,6that is continuous in the fluid space in such a manner that

$$\nabla^{2} \mathcal{U}_{j}(X, Y, \Xi) = 0 \quad \text{in } Y < 0 \quad ,$$

$$\frac{\partial}{\partial Y} \mathcal{U}_{j}(X, 0, \Xi) - \mathcal{H}\mathcal{U}_{j}(X, 0, \Xi) = 0 \quad \text{outside } S^{\circ},$$

$$\frac{\partial}{\partial n} \mathcal{U}_{j}(X, Y, \Xi) = h_{j}(X, Y, \Xi) \quad \text{on } S^{\circ},$$

$$\mathcal{U}_{j}(\mathcal{A}, \mathcal{Y}, Y) - A_{j}(\mathcal{Y}) \mathcal{A}^{-\frac{1}{2}} e^{\frac{ny+ind}{2}} = 0 \quad \text{as } \mathcal{A} \rightarrow \infty,$$

$$h_{j} \text{ represents the prescribed function that depends on the mode of}$$

where h_j represents the prescribed function that depends on the mode of oscillation. The h_j are given as

$h_1(X_3Y,\mathbb{Z}) = h_X$	$h_{y}(x, y, z) = \gamma n_{z} - z n_{y}$
$n_{z}(x,y,z)=n_{y}$	$r_{s}(x,y,z) = Zn_{x} - Xn_{z}$
$h_{s}(x,y,z)=n_{z}$	$h_{6}(x, y, z) = x n_{y} - y n_{x}$

The source potentials G of unit strength in the lower half-space which satisfy the sets of boundary conditions are

$$G(x,y,z;\xi,\eta,\xi) = R^{-1} + R^{(-1)} - \pi R e^{R(y+\eta)} \left[S(N\bar{\omega}) + Y_0(N\bar{\omega}) - i2 J_0(N\bar{\omega}) \right] -2R e^{R(y+\eta)} \int_{y+\eta}^{0} e^{-\eta \mu} (\mu^2 + \bar{\omega}^2)^{-\frac{1}{2}} d\mu_{y}^{(67)}$$

where

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$$\overline{\omega} = \left[(x - \xi)^{2} + (y - \eta)^{2} \right]^{\frac{1}{2}}, \quad \overline{\omega}' = \left[(x - \xi)^{2} + (y + \eta)^{2} \right]^{\frac{1}{2}}, \\ R = \left[\overline{\omega}^{2} + (z - \xi)^{2} \right]^{\frac{1}{2}}, \quad R' = \left[\overline{\omega}^{\prime 2} + (z - \xi)^{2} \right]^{\frac{1}{2}},$$

 $S_o(A\bar{\omega})$ is the Struve function of order 0

 $J_o(\mathbf{n}\,\overline{\boldsymbol{\omega}}), Y_o(\mathbf{n}\,\overline{\boldsymbol{\omega}})$ is the Bessel functions of first and second kind of order 0. The solution to the boundary value problem is now in the form

$$\mathcal{U}_{j}(x,y,z) = \frac{1}{4\pi} \iint f_{j}(\xi,\eta,\zeta) G(x,y,z;\xi,\eta,\zeta) d\mathcal{S}, \quad (68)$$

where f is the strength of the distributed sources over the immersed surface and is a continuous complex function. If the forces and moments are written as components in phase with the acceleration and velocity, we find that

$$F_{a} = -\rho \bar{n}^{3} M \ddot{\bar{Q}}_{j} - \rho \sigma \bar{n}^{3} N \dot{\bar{Q}}_{j} \qquad j=1,2,3$$
 (69)

For simple harmonic motions, the body vectors become

$$Q_j(t) = \operatorname{Re}\left[\overline{Q}_j^{\circ}(t)e^{-i\sigma t}\right] \qquad j=1,2,3$$

and

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$$\Theta_{j}(t) = \operatorname{Re}\left[\Theta_{j}^{\circ}(t) e^{-i\sigma t}\right] \qquad j = 4, 5, 6 \quad (70)$$

The moments are

$$G_d = -\rho \bar{n}^4 I \dot{\theta}_j - \rho \sigma \bar{n}^4 H \dot{\theta}_j \quad j=4,5,6$$

Then, M and N are the hydrodynamic mass and the linear damping coefficients, and I and H are the hydrodynamic mass moment of inertia and rotational damping coefficients. The dimensionless coefficients are

$$M = \frac{\overline{M}}{\rho \overline{n}^{3}} = Re \left[\iint_{S} u_{j}(x, y, z) n \, dS \right] \, j = 1, 2, 3 \, (71)$$

$$N = \frac{\overline{N}}{\rho \sigma \overline{H^3}} = \operatorname{Im} \left[\iint_{S} \mathcal{U}_{j}(x, y, z) \, n \, dS \right] \, j = 1, 2, 3 \, (72)$$

$$I = \frac{\overline{I}}{\rho \overline{A}^{4}} = Re \left[\iint_{S} \mathcal{U}_{j}(X,Y,Z) \cdot (r \times n) dS \right] j = 4,5,6, (73)$$

And

$$H = \frac{\overline{H}}{\rho \sigma \overline{R}^{4}} = Im \left[\iint_{S} u_{j}(x, y, z) \cdot (r \times n) dS \right] j = 4,5,6$$
(74)

Kim⁴² has evaluated these integrals numerically for spheroids of various aspect ratios and has plotted the dimensionless hydrodynamic coefficients versus the frequency parameter. Barakat⁴³ evaluated the Fredholm integrals with an approximate analytical solution for the case of a sphere on the free surface. Kim's data are shown in figures 8, 9, and 10. A curve-fitting program (CURFIT) built into the Government Services Administration remote terminal



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Figure 8. Hydrodynamic Mass and Wave Damping for Swaying or Surging Oblate Spheroids (From Kim, reference 42.)


Figure 9. Hydrodynamic Mass and Wave Damping for Heaving Oblate Spheroids (From Kim, reference 42.)

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(From Kim, reference 42.)

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computer system was used to develop approximate equations for Kim's data for a sphere. Program CURFIT fits six curves to the data by a least-squares fit of the candidate curve's linear transform. The six curves are of the following types:

$$Y = A + B \cdot X$$

$$Y = A e^{BX}$$

$$Y = A X^{B}$$

$$Y = A + B/X$$

$$Y = 1 / (A + B \cdot X)$$

$$Y = X / (A + B \cdot X)$$

Coefficients for each curve and an index of determination (best fit) are computed. Kim's data for the sphere are approximated by the following functions:

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HYDRODYNAMIC MASS

$$M_{z} = M_{x} = 1.089 + 0.529 \,\text{m}' \qquad 0 < \text{M}' < 0.74 \qquad (75\text{A})$$
$$= 1 / (-0.0318 + 0.954 \,\text{m}') \qquad 0.74 < \text{m}' < 3.4 \qquad (75\text{B})$$

DAMPING

$$N_x = N_x = 0$$
 $0 < n' < 0.1$ (76A)

$$= -0.069 + 0.71 \text{ m}'$$
 $0.1 < \text{m}' < 1.37 (76B)$

$$= 1.595 e^{-0.415 H'}$$
 1.37 < H' < 3.4 (76C)

Heave

HYDRODYNAMIC MASS

$$M_{y} = 1.85 \qquad 0 < A' < 0.1 \quad (77A)$$

= 1.02 · A' - 0.256
$$0.1 < A' < 3.4 \quad (77B)$$

DAMPING

$$N_{y} = 0.126 + 1.7 A' \qquad 0 < A' < 0.4 \quad (78A)$$

= 1.18 e^{-0.83A'} 0.4 < A' < 3.4 \quad (78B)

Dimensionless hydrodynamic mass and wave damping coefficients for a sphere based upon Kim's study along with the above approximate curves are shown in figure 11. The set of approximate functions will be used in the computer simulation of buoy dynamics for a spherical buoy.

Birkoff⁴⁴ has investigated the influence of body symmetry on the hydrodynamic mass dyadic. An oblate spheroid possessing an axis of symmetry has five hydrodynamic mass values along the main diagonal: heave, surge, sway (same as surge), pitch, and roll (same as pitch). For an ideal fluid, the yaw hydrodynamic mass is zero. However, Lamb⁴⁵ studied the rotational motion of a sphere in a viscous fluid and identifies a force proportional to angular acceleration that can be considered as a yaw hydrodynamic mass. For a fully immersed oblate spheroid in an ideal fluid with the centers of gravity and pressure coincident, all the off-diagonal terms would be zero. However, for a half-immersed oblate spheroid, the following hydrodynamic forces are coupled:

Surge-Pitch

Sway-Roll

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Pitch-Surge

Roll-Sway.

Thus far, hydrodynamic mass and wave damping have been computed for heave, sway, surge, pitch, and roll. Using Lamb's analysis for a rotating sphere, we can approximate the yaw hydrodynamic mass for an oblate spheroid

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Figure 11. Least-Squares Fits to Kim's Parameters

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in a viscous fluid:

$$M_{aa} = \frac{4}{3} \pi \rho h^{5} \cdot \frac{1 + \beta h}{1 + 2\beta h + 2\beta^{2} h^{2}}$$
(79)

The viscous damping in yaw is

$$N_{\alpha\alpha} = \frac{4}{3} \pi \mu A^{3} \frac{3+6BA+6B^{2}A^{2}+2B^{3}A^{3}}{1+2BA+2B^{2}A^{2}}, \qquad (80)$$

where

- A is the major diameter
- is the fluid density
- \mathcal{M} is the viscosity
- ${oldsymbol {\mathcal V}}\,$ is the kinematic viscosity

$$\beta$$
 is defined as $\sqrt{\sigma/2\nu}$

 σ is the angular frequency of oscillatory motion.

Since the center of pressure and center of gravity of the buoy do not necessarily coincide, force components due to hydrodynamic mass or damping will induce moments about the center of gravity. The projected area of an oblate spheroid in the y-x or z-x body planes is a semiellipse. For $H_D < 0$, i.e., less than half immersion, the center of pressure is located a distance

$$H_{cp} = \frac{2}{3} \frac{b}{H} \sqrt{\left(H^2 - H_p^2\right)^3}$$
(81)

below the centroid of the oblate spheroid. For $H_D > 0$, i.e., more than half immersion, the center of pressure is located a distance

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$$H_{CP} = \frac{\left(\frac{\pi}{2}Hb - \frac{b}{4}H_{p}\sqrt{H^{2} - H_{p}^{2}} - H_{p}^{2} - H_{p}^{2}\right)}{\frac{\pi}{2}Hb + \frac{b}{4}H_{p}\sqrt{H^{2} - H_{p}^{2}} + H_{p}^{2}Sin^{-1}(H_{p}^{2}-H_{p}^{2})}$$
(82)

below the centroid.

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In the coordinate system shown, a positive y acceleration will induce a negative \mathcal{F} moment and, conversely, a positive \mathcal{F} acceleration will induce a negative y force if the center of gravity is below the center of pressure. Thus, the coupled roll-sway hydrodynamic mass moment of inertia is

$$M_{h_{y}} = M_{h_{y}} = -M_{h_{y}} (H_{cg} - H_{cP})$$
(83)

Also, a positive z acceleration will induce a positive β moment and vice versa. Thus, the coupled pitch-surge moment is

$$M_{h_{BZ}} = M_{h_{ZB}} = M_{h_{ZB}} (H_{CG} - H_{CP})$$
 (84)

Summarizing the inertial hydrodynamic force coefficients, i.e., the elements of the hydrodynamic mass dyadic for a half-immersed oblate spheroid, we find

$$\mathcal{M}_{h} = \begin{bmatrix} m_{h_{XX}} & 0 & 0 & 0 & 0 & 0 \\ 0 & m_{h_{YY}} & 0 & 0 & 0 & m_{h_{YY}} \\ 0 & 0 & m_{h_{22}} & 0 & m_{h_{23}} & 0 \\ 0 & 0 & 0 & I_{h_{\alpha\alpha}} & 0 & 0 \\ 0 & 0 & m_{h_{\beta2}} & 0 & I_{h_{\beta\beta}} & 0 \\ 0 & m_{h_{\gamma\gamma}} & 0 & 0 & 0 & I_{h_{\gamma\gamma}} \end{bmatrix}$$
(85)

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For the special case of a sphere, the elements of the dyadic are

$$\begin{split} m_{h_{XX}} &= 1.85 \ \rho \, n^3 & 0 < n' < 0.1 \\ &= (1.02 \ n'^{-0.255}) \ \rho \, n^3 & 0.1 < n' < 3.4 \end{split}$$

$$\begin{split} m_{hyy} &= m_{h_{ZZ}} = (1.089 + 0.529 \text{ A}') \text{ DA}^{3} & 0 < \text{A}' < 0.79 \\ &= (1.0 / (-0.0318 + 0.959 \text{ A}')) \text{ DA}^{3} & 0.79 < \text{A}' < 3.9 \\ I_{hax} &= I_{hgg} = I_{hyy} = \frac{4}{3} \pi \text{ PA}^{5} \cdot \frac{1 + B \text{ A}}{1 + 2 B \text{ A} + 2 B^{2} \text{ A}^{2}} \\ m_{hyy} &= m_{hyy} = -m_{hyy} (H_{cg} - H_{cp}) \\ m_{h_{ZB}} &= m_{h_{BZ}} = m_{h_{ZZ}} (H_{cg} - H_{cp}) \end{split}$$

where

- \mathbf{A}' is defined as $\frac{\mathbf{A}\sigma^2}{\mathbf{S}}$
- A is the sphere radius

 σ is the angular frequency

 \mathcal{P} is the fluid density

$$\beta$$
 is defined as $\sqrt{\sigma/2\nu}$

 ${m V}$ is the kinematic viscosity

 $H_{c\sigma}$, $H_{c\rho}$ are locations of centers of gravity and pressure.

In a similar fashion, the dissipative force coefficients due to surface wave generation are

$$N = \begin{bmatrix} n_{xx} & 0 & 0 & 0 & 0 & 0 \\ 0 & n_{yy} & 0 & 0 & 0 & n_{yy} \\ 0 & 0 & n_{yzz} & 0 & n_{z,B} & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & n_{Bz} & 0 & n_{B,B} & 0 \\ 0 & n_{yy} & 0 & 0 & 0 & n_{yy} \end{bmatrix}$$
(86)

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Again, for the special case of a sphere. the elements of the dyadic are

$$N_{AX} = (0.126 + 1.7 \cdot h') \rho \sigma A^{3} \qquad 0 < h' < 0.4$$

= (1.18 e^{-0.13A'}) $\rho \sigma A^{3} \qquad 0.4 < h' < 3.4$

$$N_{yy} = N_{zz} = 0 / \qquad 0 < A' < 0.1$$

= (-0.069+0.71A') $\rho \sigma A^{3}$ 0.1< A'< 1.37
= (1.595 e^{-a4/15A'}) $\rho \sigma A^{3}$ 1.37< A'< 3.4

$$\begin{split} n_{BB} &= n_{YY} = 0 \\ n_{YT} &= n_{YY} = -n_{YY} \left(H_{CG} - H_{CP} \right) \\ n_{ZB} &= n_{BZ} = n_{ZZ} \left(H_{CG} - H_{CP} \right) \end{split}$$

Dissipative forces due to viscosity are assumed to follow a velocitysquared drag law. There is some question as to the validity of representing an unsteady force with a coefficient based upon steady flow experimental measurements. Martin⁴⁶ has found that the mean drag coefficient for a plate started impulsively from rest is an order of magnitude greater than for steady flow. However, there is no general, analytical method available to compute the viscous forces for a fully turbulent, oscillatory flow. Schlicting⁴⁷ cites use of a method of successive approximations for unsteady laminar flow. Since viscous forces, $f(a^2)$, are an order of magnitude less than hydrodynamic inertia and wave damping forces, $f(a^3)$, and because steady currents are acting on the buoy, the viscous forces are assumed to follow a velocity-squared drag law for subcritical Reynold's numbers.

Analagous to the hydrodynamic mass dyadic, the viscous force matrix will contain ten elements. The viscous force coefficient matrix is

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$$\mathbf{D} = \begin{bmatrix} d_{XX} & 0 & 0 & 0 & 0 & 0 \\ 0 & d_{YY} & 0 & 0 & 0 & d_{YS} \\ 0 & 0 & d_{ZZ} & 0 & d_{ZB} & 0 \\ 0 & 0 & 0 & d_{XX} & 0 & 0 \\ 0 & 0 & d_{BZ} & 0 & d_{BB} & 0 \\ 0 & d_{XY} & 0 & 0 & 0 & d_{XY} \end{bmatrix},$$
(87)

where

$$d_{XX} = \frac{p}{2} G_{DR} A_{H}$$

$$d_{YY} = d_{ZZ} = \frac{p}{2} G_{DS} A_{S}$$

$$d_{\alpha\alpha} = d_{BB} = d_{YY} = \frac{H}{3} T M R^{3} \frac{3 + 6BR + 6B^{2}R^{2} + 2B^{3}R^{3}}{1 + 2BR + 2B^{2}R^{2}}$$

$$d_{YS} = d_{YY} = -d_{YY} (H_{CG} - H_{CP})$$

$$d_{ZB} = d_{BZ} = -d_{ZZ} (H_{CG} - H_{CP})$$

The projected area in heave is

$$A_{\mu} \cong \pi b^2 \tag{88}$$

and in surge or sway is

$$A_{s} = \frac{\pi}{2} A b + \frac{b}{H} H_{0} \sqrt{A^{2} - H_{0}^{2}} + A b \sin^{-1} \frac{H_{0}}{|A|} .$$
(89)

For half-immersed spheroids. Hoerner⁴⁹ shows plots of drag coefficient (surge or sway) versus the ratio of vertical to horizontal axis dimensions.

Hoemer's plot for a subcritical drag coefficient is fitted by the function

$$G_{DS} = 0.354 \text{ A/b}$$
 Re < 10⁵; subcritical

for various spheroids. The drag coefficient in heave is approximated with onehalf the value for a sphere, $C_{DH} = 0.3$.

3.1.5 Ocean Waves

In order to compute the magnitude of the hydrodynamic forces with the coefficients just derived, the relative motion of the fluid surrounding the buoy, relative to the buoy, must be computed. A mathematical model of the sea state must be developed.

The following hypothesis offered by St. Denis and Pierson²⁴ in 1953 has been verified by Dalzell, 49,50 Gerritsma, 51 and others for the motions of ships in a random seaway:

1. Assume that the sea can be represented as the linear sum of elementary waves, each traveling in the manner described by the classical Airy formulas of linearized water wave theory. Each component wave train will have random phase.

2. Weight the component waves to have the same spectral characteristics as the observed sea state.

3. Assume that the body response to a random sea is the sum of its responses to the various frequency components.

Within the constraints imposed by the assumptions made in the derivation of the hydrodynamic coefficients, i.e., body dimensions are small compared with

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a wavelength, the St. Denis-Pierson hypothesis should be better suited to the case of a buoy, with dimensions on the order of 10 ft, than to ships, with dimensions on the order of 100 ft. (For example, as a worst case, a 10-ft buoy in waves with 100-ft wavelengths would only cause a peak error of 3.3 percent in the computed elevation of the mean waterplane. This error will decrease as the wavelengths become longer.)

From the Airy formulas, water particle motions in deep water for waves traveling along the z axis are described as follows:

Wave Height

$$X_{w} = A_{w} \sin(kz - \sigma t) \tag{90A}$$

Vertical Velocity Component

$$\dot{X}_{w} = -\frac{\mu_{w} S k}{\sigma} \cos(kz - \sigma t)$$
(90B)

Horizontal Velocity Component

$$\dot{z}_{w} = \frac{A_{w}gk}{\sigma} SIN(kz - \sigma t)$$
(90C)

Vertical Acceleration Component

$$\ddot{\mathbf{x}}_{w} = -\mathbf{n}_{w} \mathbf{g} \mathbf{k} \, SIN \left(\mathbf{k} \mathbf{z} - \boldsymbol{\sigma} \mathbf{t} \right) \tag{90 D}$$

Horizontal Acceleration Component

$$\ddot{z}_{w} = -A_{w} g k \cos(kz - \sigma t) \qquad (90E)$$

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Wave Slope

$$B_{w} = -TAN^{-1} (n_{w} k \cos(kz - \sigma t))$$

$$\cong -n_{w} k \cos(kz - \sigma t)$$
^(90 F)

Angular Velocity of Free Surface

$$\dot{\beta}_{w} \cong - H_{w} \, k \, \sigma \, \sin \left(k \Xi - \sigma t \right) \tag{90G}$$

Angular Acceleration of Free Surface

$$\dot{\mathcal{B}}_{w} \cong \mathcal{A}_{w} k \sigma^{2} \cos(kz - \sigma t)$$
^(90 H)

where

k is the wave number
$$(k = 2\pi/L_o)$$

 L_o is the wavelength

 σ is the angular frequency

t is the time.

The assumption that body dimensions are small compared with wavelengths implies that body displacements are small compared with wavelengths. Thus, we find that

$$X_{w} \cong -H_{w} SIN \ \sigma t , \qquad (91A)$$

$$\dot{X}_{W} \cong -A_{W}\sigma \cos \sigma t , \qquad (91B)$$

$$\dot{z}_{w} \cong -A_{w}\sigma SIN \sigma t$$
, (91C)

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$$\ddot{X}_{w} \cong A_{w} \sigma^{2} SIN \sigma t , \qquad (91D)$$

$$\ddot{z}_{w} \cong -A_{w} \sigma^{2} \cos \sigma t , \qquad (91E)$$

$$\beta_{W} \cong A_{W} K \cos \sigma t , \qquad (91F)$$

$$\beta_{w} \cong -n_{w} k \sigma S N \sigma t, \qquad (91G)$$

and

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$$A_{\rm W}^3 \cong -A_{\rm W} \, \mathrm{K} \, \sigma^2 \, \mathrm{cos} \, \sigma t$$
 (91H)

Transforming the water mass velocities and accelerations to body coordinates, we see that the velocities are

$$\begin{bmatrix} \dot{x}_{w_2} \\ \dot{y}_{w_2} \\ \dot{z}_{w_2} \end{bmatrix} = \Omega \cdot \begin{bmatrix} \dot{x}_w \\ O \\ \dot{z}_w \end{bmatrix} , \qquad (92A)$$

and that the accelerations are

$$\begin{bmatrix} \ddot{X}_{w_2} \\ \ddot{Y}_{w_2} \\ \ddot{z}_{w_2} \end{bmatrix} = \bigcap \cdot \begin{bmatrix} \ddot{X}_w \\ O \\ \vdots \\ \ddot{z}_w \end{bmatrix}$$
(92B)

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The location of the intersection of the free surface and the "vertical" axis of the buoy is

$$\mathcal{H}_{p} = X_{of} \cos\beta\cos\gamma + x_{2} - X_{w}\cos\beta\cos\gamma .$$
(93)

In order to develop a random wave model, the statistical properties of the sea state must be described. The development of wind waves on a body of water is either limited by the distance to land in the direction from which the wind blows (fetch limited) or by the length of time during which the wind acts on the water surface (duration limited). For the fetch limited case, Bretschneider⁵² normalized the original wind wave forecasting relations of Sverdrup and Munk⁵³ and included much additional data. Bretschneider's dimensionless curves have been approximated by piecewise linear functions by Patton⁵⁴ The approximate functions are as follows:

Significant Period

1

$$\overline{T_{l_g}} = 2.47 \times 10^{-2} \qquad \overline{F} < 2.0 \times 10^{-2} \quad (94A)$$

$$LOG \ \overline{T_{l_g}} = -1.136 + 0.283 \ LOG \ \overline{F} \qquad 2 \times 10^{-2} < \overline{F} < 1.2 \times 10^{5} \quad (94B)$$

$$\overline{T_{l_g}} = 2.0 \qquad \overline{F} > 1.2 \times 10^{5} \quad (94C)$$
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Significant Height

$$\overline{H}_{y_3} = 5.74 \times 10^{-4}$$
 $\overline{F} < 1.6 \times 10^{-2}$ (95A)

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LOG
$$\widehat{H}_{g} = -2.5 + 0.415 \text{ LOG } \overline{F}$$
 $1.6 \times 10^{-2} < \overline{F} < 5.0 \times 10^{4}$ (95B)

$$\overline{H}_{y} = 2.82 \times 10^{-1}$$
 $\overline{F} > 5.0 \times 10^{4}$, (95C)

Weigel⁵⁵ extends Bretschneider's dimensionless curves to the case of duration limited wind waves. These curves are approximated by

Significant Period

$$LOG \ \overline{T}_{y_{g}} = -0.2 + 0.224 \ LOG \ \overline{D}$$
 (96)

Significant Height

$$\log \overline{H}_{i_3} = -2.272 + 0.3282 \log \overline{D}$$
, (97)

where

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$$\overline{T}_{3} = \frac{9}{2\pi} \frac{7\nu_{3}}{WV}$$
, the dimensionless significant period

$$\overline{H}_{y} = \frac{\mathcal{G} H_{y}}{WV^{2}}$$
, the dimensionless significant height

$$\overline{F} = \frac{GF}{WV^2}$$
, the fetch parameter

 $\overline{D} = \frac{g D}{WV}$, the duration parameter

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 $\mathcal{T}_{\mathcal{V}}$ is the significant period

 $H_{\mathcal{X}}$ is the significant wave height

F is the fetch

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 \mathcal{D} is the duration

 \boldsymbol{G} is the gravitational constant

WV is the wind velocity

In order to determine if the waves are fetch or duration limited, the minimum duration for a given fetch must be determined. From Bretschneider's curves, the minimum duration is given by

$$\log\left(\frac{D_{\min} \cdot WV}{F}\right) = 1.477 - 0.255 \text{ LOG }\overline{F}$$
(98)

If the duration (D) is less than the minimum duration (D_{\min}) the waves are duration limited; if greater, the waves are fetch limited.

For a given wind speed, fetch, and duration, the significant wave height and period can be computed by using the above equations. Longuet-Higgins⁵⁶ and Bretschneider⁵² have shown that the distributions of wave heights and squared periods can be represented by Rayleigh distributions. Thus, the wave height

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distribution is

$$p(H) = \frac{\pi H}{2 H^2} e^{-\frac{\pi}{4} \frac{H^2}{H^2}}, \qquad (99)$$

and the wave period distribution is

$$p(T) = 2.7 - \frac{T^3}{\overline{T^4}} e^{-0.675 \left(\frac{T}{\overline{T}}\right)^4}$$
(100)

The mean wave height and periods are

$$\overline{H} = 0.625 H_{1/3}$$
(101)

and

$$\overline{T} = 0.758 T_{y_3}$$
 (102)

Ocean wind wave amplitude spectra are of the form

$$S_{H^2}(\omega) = A \,\omega^m e^{-Bn} \tag{103}$$

Pierson and Moskowitz⁵⁷ analyzed 54 data sets and evaluated A, B, m, and n. The exponent m was set equal to -5 and n was found to vary between 2 and 4 depending on the wind speed. Kettler⁵⁸ optimized the parameters by using a least-squares fit of each data set used by Pierson-Moskowitz and proposed the form

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$$S_{H^2}(\omega) = 10 \cdot \omega^{-5.5} e^{(-b\omega^{-4.5})} + G$$
, (104)

where

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$$b = (1.75 \times 10^5) W^{-4,305}$$
$$G = 0.03054 e^{(0.154.WV)}$$

The Bretschneider spectrum, given by

$$S_{H^{2}}(\omega) = \propto g^{2} \omega^{-3} e^{-0.675 \left(\frac{g}{W\omega F_{2}}\right)^{4}}$$
(105)

where

$$F_{1} = \frac{gH}{WV^{2}}$$

$$F_{2} = \frac{g\overline{T}}{2\pi WV}$$

$$\propto = 3.437 \frac{F_{1}^{2}}{F_{2}^{4}}$$

was chosen for this study since it closely resembles the optimized spectrum and is integrable.

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the form

$$X_{w}(t) = \int_{0}^{\infty} \cos\left(\omega t + \epsilon(\omega)\right) \cdot \sqrt{S_{H^{2}}(\omega) d\omega} , \qquad (106)$$

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where $\in (\omega)$ is a random variable whose values are equally probable for any value between 0 and 277. The integral is not an integral in the Riemann sense since the function is discontinuous because of the random variable. The expression indicates that the random wave heights can be represented as a finite number of cosine waves of different frequency, each having random phase. Let the spectral density $S_{\mu^{2}}(\omega)$ be partitioned into N frequency bands (figure 12) in such a manner that

$$0 < \omega_0 < \omega_1 < \omega_2 \cdots < \omega_N = W,$$

where the spectral density is essentially zero if $\,\omega\,$ is greater than $\,\mathcal{W}\,$. The width of the nth band is

$$\Delta \omega_n = \omega_n - \omega_{n-1}$$
⁽¹⁰⁷⁾

and the mean angular frequency of the nth band is

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$$\widetilde{\omega}_{n} = \frac{1}{2} \left(\omega_{n} + \omega_{n-1} \right). \tag{108}$$

The random wave height model is now defined as

$$X_{w}(t) = \sum_{n=1}^{N} \sqrt{S_{H^{2}}(\overline{\omega_{n}})} \Delta \omega_{n} \quad \cos(\overline{\omega_{n}t} + \epsilon_{n}),$$

where ϵ_{n} , $n = 1, 2, 3, \dots, N$ are independent random
variables distributed uniformly over the interval 0 to 2π . Borgman⁵⁹
indicates that an equally spaced subdivision $\Delta \omega_{n} = \overline{\omega}/N$ will
result in $X_{w}(t)$ repeating itself with period $2\pi/\overline{\omega_{n}}$. Borgman

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partitions the spectrum on an energy basis using the cumulative spectrum given

by

$$S_{H^2}(\omega) = 2 \int_0^{\omega} S_{H^2}(\omega) d\omega \qquad ; \qquad (109)$$

thus,

$$S_{H^{2}}(\overline{\omega}) \Delta \omega_{n} \cong \frac{1}{2} \left(\overline{S}_{H^{2}}(\omega_{n}) - \overline{S}_{H^{2}}(\omega_{n-1}) \right); \qquad (110)$$

hence,

$$x_{w}(t) = \frac{1}{\sqrt{2}} \sum_{n=1}^{N} \sqrt{\overline{S}_{H^{2}}(\omega_{n}) - \overline{S}_{H^{2}}(\omega_{n-1})} \quad COS(\overline{\omega}_{n}t + \epsilon_{n}). \quad (111)$$

The periodicity is avoided if the set of (\mathcal{W}_n) values are chosen to make

 $\overline{S}_{H^2}(\omega_n) - \overline{S}_{H^2}(\omega_{n-1})$ constant for all n. Let

$$\overline{S}_{H^{2}}(\omega_{n}) - \overline{S}_{H^{2}}(\omega_{n-1}) = h^{2}; \qquad (112)$$

then, the instantaneous wave height is

$$X_{w}(t) = \frac{h}{\sqrt{2}} \sum_{n=1}^{H} \cos\left(\overline{\omega}_{n} t + \epsilon_{n}\right), \qquad (113)$$

The frequencies, $\overline{\omega}_n$, are given by

$$S_{H^2}(\overline{\omega}_n) = \frac{n}{N} \overline{S}_{H^2}(\infty) \qquad n = 1, 2, \cdots, N \quad ,$$
⁽¹¹⁴⁾

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which corresponds to an equal subdivision of the energy coordinate axis for

$$S_{H^2}(\omega)$$
.

Using the Bretschneider-Pierson spectral density of the form

$$S_{H^2}(\omega) = \frac{AB}{\omega^5} e^{-\frac{B}{\omega^4}}$$
(115)

which is directly integrable, we find that

$$\overline{S}_{H^{2}}(\omega) = 2 \int_{0}^{\omega} \frac{AB}{S^{5}} e^{-B/S^{4}} dS = \frac{A}{2} e^{-B/\omega^{4}} .$$
(116)

Integrating out to infinity, we find that

$$\overline{\widetilde{S}}_{H^2}(\infty) = \frac{A}{2}$$

and

$$\omega_n = \left[\frac{B}{LOG_e(N/n)}\right]^{\frac{1}{4}},\tag{117}$$

where the coefficient is given by

$$B = -0.675 \left(\frac{g}{WV F_2}\right)^4$$
(118)

Next, we define the various elements of the random sea state model:

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Wave Height

$$X_{w}(t) = \frac{1}{2} \sum_{n=1}^{N} -\sqrt{S_{H^{2}}(\overline{\omega_{n}}) \Delta \omega_{n}} \quad SIN\left(\overline{\omega_{n}} t + \epsilon_{n}\right)$$
(119)

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Vertical Velocity Component

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$$\dot{X}_{W}(t) = \frac{1}{2} \sum_{n=1}^{N} -\overline{\omega}_{n} \sqrt{\hat{S}_{H^{2}}(\bar{\omega}_{n})} \Delta \omega_{n} \quad COS(\bar{\omega}_{n} t + \epsilon_{n})$$
(120)

Horizontal Velocity Component

$$\dot{\boldsymbol{z}}_{w}(t) = \frac{1}{2} \sum_{n=1}^{N} -\overline{\omega}_{n} \sqrt{S_{H^{2}}(\overline{\omega}_{n})} \Delta \omega_{n} \quad SIN(\overline{\omega}_{n} t + \epsilon_{n})$$
(121)

Vertical Acceleration Component

$$\ddot{X}_{w}(t) = \frac{1}{2} \sum_{n=1}^{N} \overline{\omega}_{n}^{2} \sqrt{S_{H^{2}}(\overline{\omega}_{n})} \Delta \omega_{n} SIN(\overline{\omega}_{n}t + \epsilon_{n})$$
(122)

Horizontal Acceleration Component

$$\ddot{z}_{w}(t) = \frac{1}{2} \sum_{h=1}^{N} - \tilde{\omega}_{n}^{2} \sqrt{S_{H^{2}}(\bar{\omega}_{n})} \Delta \omega_{n} COS(\bar{\omega}_{n}t + \epsilon_{n})$$
(123)

Wave Slope

$$\mathcal{B}_{w}(t) = \frac{1}{2} \sum_{n=1}^{N} k_{n} \sqrt{S_{H^{2}}(\overline{\omega}_{n}) \Delta \omega_{n}} COS(\overline{\omega}_{n} t + \varepsilon_{n})$$
(124)

Angular Velocity of Free Surface

$$\dot{\mathcal{B}}_{w}(t) = \frac{1}{2} \sum_{n=1}^{N} -k_{n} \,\overline{\omega}_{n} \sqrt{S_{\mu^{R}}(\overline{\omega}_{n}) \Delta \omega_{n}} \, SIN\left(\overline{\omega}_{n}t + \epsilon_{n}\right) \quad (125)$$

Angular Acceleration of Free Surface

$$\ddot{\mathcal{B}}_{w}(t) = \frac{1}{2} \sum_{n=1}^{N} -k_{n} \, \bar{\omega}_{n}^{2} \sqrt{S_{\mu^{2}}(\bar{\omega}_{n}) \Delta \omega_{n}} \, \mathcal{C}OS(\bar{\omega}_{n}t + \epsilon_{n}) \quad (126)$$

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Again, the wave height, velocity components, and acceleration components must be transformed to body coordinates according to equations (92A), (92B), and (93).

A ten-component, random sea state model was programmed in FORTRAN for use with the UNIVAC 1108 digital computer at the Naval Underwater Systems Center, New London Laboratory. This program is shown in appendix B as subroutine "RWAVE." The ten-component model was found to truncate the low and high ends of the spectrum (the first and ninth frequency components). Since the low-frequency end of the spectrum is important in the computation of the response of mechanical systems, Borgman's frequency partition method was modified to include the low-frequency energy by the following numerical scheme:

1. Use a trial and error method to compute the frequency at which the value of the spectral density rises above some threshold value, for example, $S_{\mu^2}(\omega) > 0.01 \ ft^2$. Let this frequency be denoted ω_0 .

2. Thus,

$$\widetilde{\omega}_{i} = \frac{1}{2}(\omega_{i} - \omega_{o})$$

and

S.

$$\Delta \omega_i = \omega_i - \omega_o ,$$

3. Use the energy computed for the second partition $\overline{S}_{H^2}(\omega_2) - \overline{S}_{H^2}(\omega_1)$ to compute the component amplitude.

This method accounts for the energy in the low-frequency end of the spectrum.

3.1.6 Numerical Solution of the Equations of Motion

3 B

Having computed the wind wave displacements, velocities, and accelerations, we can now compute the hydrostatic and hydrodynamic forces acting on the buoy (terms B and H in equation (5)). The hydrostatic forces and moments depend on the position and orientation of the waterplane relative to the buoy. The wave height, given by equation (91A) for a sinusoidal wave model or by equation (119) for a random wave model, must be transformed to buoy coordinates. The transformation is

$$X_{w_2} = X_w \cos\beta\cos\gamma - X_{o_6}\cos\beta\cos\gamma, \qquad (127)$$

where X_{06} is the vertical height of the center of gravity of the buoy below the mean waterplane with no buoy pitch or roll. The transformed wave height must be subtracted from the heave motion of the buoy in order to apply equation (93), the location of the intersection of the waterplane and the "vertical" axis of the buoy. Restricting the study to waves traveling along the x axis of the coordinate system, the wave slope \mathcal{B}_{W} in the inertial coordinate system is given by equation (91F), for sinusoidal waves and by equation (124) for the random wave model. The attitude of the waterplane, relative to the buoy coordinates, is described by the angles $\mathcal{B}_{S} = \mathcal{B} - \mathcal{B}_{W}$ and $\mathcal{J}_{S} = \mathcal{J}$. The slope of the sea surface is

$$\beta' = \cos'(\cos \gamma_s \cos \beta_s). \tag{128}$$

With the location (equation (93)) and slope (equation (128)), the buoyant forces and moments for a sphere are given by equations (49), (55), and (56).

In a similar manner, the hydrodynamic forces are computed by considering water mass movements relative to the buoy. With the assumption that the buoy dimensions are small relative to the wavelength and applying the Froude-Kryloff hypothesis* for the viscous forces, we find that water mass velocities and accelerations are given by equations (91B), (91C), (91D), (91E), (51G), and (91H) for a sinusoidal wave model and by equations (120), (121), (122), (123), (125), and (126) for a random wave model. Water mass accelerations are simply transformed to buoy coordinates (equation (100B)) and subtracted from buoy accelerations to compute the motion of the buoy relative to the water mass. The relative acceleration is

$$\ddot{Q}' = \ddot{Q} - \Omega \cdot \ddot{Q}_{w} , \qquad (129)$$

where

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Q is the acceleration vector relative to water mass \ddot{Q} is the buoy acceleration \ddot{Q}_{u} is the water mass acceleration.

Water mass velocities are more complex because ocean currents exist and must be included. There is a difference in reference frame that must be resolved since buoy velocities cause a reactive force (i.e., a positive buoy velocity

^{*}The presence of the buoy does not appreciably change water mass movements in the vicinity of the buoy.

causes a negative force) while waves and currents cause active forces (positive velocities cause positive forces). The velocity vector of the buoy, relative to the water mass, is given by

$$\dot{\mathbf{Q}}'' = \dot{\mathbf{Q}} - \boldsymbol{\Omega} \cdot \dot{\mathbf{Q}}_{w} - \boldsymbol{\Omega} \cdot \dot{\mathbf{Q}}_{c} , \qquad (130)$$

where

 \hat{Q} is the buoy velocity vector \hat{Q}_{w} is the particle velocity vector due to waves \hat{Q}_{c} is the current velocity vector.

For current velocity components C^{*+} and CW acting in the positive y_0 and z_0 directions, respectively, the current velocity vector is

$$\dot{Q}_{c} = \begin{bmatrix} 0 \\ CV \\ CW \end{bmatrix}$$
(131)

For viscous dissipative forces, which are functions of the velocity squared, the velocities are

$$\dot{Q}'^2 = \dot{Q}' | \dot{Q}' | \qquad (132)$$

The absolute value is used in order to preserve the sign convention.

If the buoy were free floating, not moored, the equations of motion for the buoy could now be integrated for a given set of environmental conditions to solve for the buoy response. The equations of motion for a spherical buoy are summarized. Equation (5) is

$$M\ddot{Q} = MG - B - H - V. - T.$$

In the R_2 coordinate system, the heave acceleration is given by

$$\ddot{x} = \frac{1}{m} \Big[mg \cos \vartheta \cos \vartheta - B \cos \vartheta \cos \vartheta \Big] \\ - m_{h_{XX}} \cdot \ddot{x}' - n_{XX} \cdot \dot{x}' - d_{XX} \cdot \dot{x}'' \Big| \dot{x}'' \Big| - W_{X_{R}} \\ + T_{X} \cos \vartheta \cos \vartheta + T_{Y} \sin \vartheta + T_{Z} \cos \vartheta \sin \vartheta \Big] .$$

The sway acceleration is given by

$$\begin{split} \ddot{y} &= \frac{1}{m} \Big[mg \sin x \cos \beta - B \sin x \cos \beta - m_{hyy} \cdot \ddot{y}' \\ &- m_{hyy} \cdot \ddot{y}' - n_{yy} \cdot \dot{y}' - n_{yy} \cdot \dot{y}' - d_{yy} \cdot \dot{y}'' | \dot{y}' | \\ &- d_{yy} \cdot \dot{y}'' | \dot{y}'' | - W_{y_2} - T_x \sin x \cos \beta \\ &+ T_y \cos x - T_x \sin x \sin \beta \Big]. \end{split}$$

The surge acceleration is given by

$$\begin{split} \vec{\Xi} &= \frac{1}{m} \left[-mg \sin\beta + B \sin\beta - m_{h_{\XiE}} \cdot \vec{\Xi}' - m_{h_{\XiB}} \cdot \vec{B}' \right] \\ &- n_{\XiE} \cdot \vec{z}' - n_{\XiE} \cdot \vec{B}' - d_{\XiE} \cdot \vec{z}'' |\vec{z}''| - d_{\XiE} \cdot \vec{B}'' |\vec{B}''| \\ &- W_{\XiE} - T_{X} \sin\beta + T_{\Xi} \cos\beta \right] . \end{split}$$

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The yaw acceleration is given by

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$$\ddot{\alpha} = \frac{1}{I_{\mu\alpha}} \left[-I_{h_{\alpha\alpha}} \cdot \ddot{\alpha} - d_{\alpha\alpha} \cdot \dot{\alpha} \cdot |\dot{\alpha}| \right].$$

The pitch acceleration is given by

$$\ddot{\mathcal{B}} = \frac{1}{I_{BB}} \left[-M_{B} - I_{h_{BB}} \cdot \ddot{\mathcal{B}}' - M_{h_{BE}} \cdot \ddot{\mathcal{Z}}' - N_{BE} \cdot \dot{\mathcal{Z}}' \right]$$
$$-d_{BB} \cdot \dot{\mathcal{B}}'' \left| \dot{\mathcal{B}}'' \right| - d_{BE} \cdot \dot{\mathcal{Z}}'' \left| \ddot{\mathcal{Z}}'' \right| - W_{BE}$$
$$-H_{ML} \left(-T_{x}' \sin \mathcal{B} + T_{E} \cos \mathcal{B} \right) \right].$$

The roll acceleration is given by

$$\ddot{\delta} = \frac{1}{I_{Mix}} \left[-M_{\delta} - I_{h_{\gamma\gamma}} \cdot \ddot{\delta}' - m_{h_{\gamma\gamma}} \cdot \ddot{\gamma}' - n_{\gamma\gamma} \cdot \dot{\gamma}' \right] - d_{\gamma\gamma} \cdot \dot{\delta}'' |\dot{\delta}'| - d_{\gamma\gamma} \cdot \dot{\gamma}'' |\dot{\gamma}''| - W_{\gamma_2} - H_{ML} \left(-T_{\chi} \sin \delta \cos \beta + T_{\gamma} \cos \gamma - T_{\chi} \sin \delta \sin \beta \right) \right]$$

Hydrodynamic inertias and wave damping are computed for motions of the buoy relative to the water mass. Thus, for waves traveling along the z axis, we see that

$$\begin{split} \ddot{X}' &= \ddot{X} - \ddot{X}_{w_{2}} & \dot{X}' = \dot{X} - X_{w_{2}} & \dot{X}' = X - X_{w_{2}} \\ \ddot{Y}' &= \ddot{Y} & \dot{Y}' = \dot{Y} & \dot{Y}' = \dot{Y} \\ \ddot{Z}' &= \ddot{Z} - Z_{w_{2}} & \dot{Z}' = \dot{Z} - Z_{w_{2}} \\ \ddot{Z}' &= \ddot{C} & \dot{Z}' = Z - Z_{w_{2}} \\ \ddot{Z}' &= \ddot{C} & \dot{Z}' = Z - Z_{w_{2}} \\ \ddot{Z}' &= \ddot{C} & \dot{Z}' = Z - Z_{w_{2}} \\ \ddot{Z}' &= \ddot{C} & \dot{Z}' = Z - Z_{w_{2}} \\ \ddot{Z}' &= \ddot{C} & \dot{Z}' = Z \\ \ddot{Z}' &= \ddot{C} & \dot{Z}' = Z \\ \dot{Z}' &= \ddot{Z} & \dot{Z}' = Z \\ \ddot{Z}' &= \ddot{Z} & \dot{Z}' = Z \\ \ddot{Z}' &= \ddot{Z} & \dot{Z}' = Z \\ \dot{Z}' &= \dot{Z} &= \dot{Z}' & \dot{Z}' = Z \\ \dot{Z}' &= \dot{Z}' &= \dot{Z}' & \dot{Z}' = Z \\ \dot{Z}' &= \dot{Z}' & \dot{Z}' = Z \\ \dot{Z}' &= \dot{Z}' &= \dot{Z}' & \dot{Z}' = Z \\ \dot{Z}' &= \dot{Z}' &= \dot{Z}' & \dot{Z}' = Z \\ \dot{Z}' &= \dot{Z}' &= \dot{Z}' & \dot{Z}' = Z \\ \dot{Z}' &= \dot{Z}' &= \dot{Z}' & \dot{Z}' = Z \\ \dot{Z}' &= \dot{Z}' &= \dot{Z}' & \dot{Z}' = Z \\ \dot{Z}' &= \dot{Z}' &= \dot{Z}' &= \dot{Z}' & \dot{Z}' \\ \dot{Z}' &= \dot{Z}' &= \dot{Z}' &= \dot{Z}' &= \dot{Z}' \\ \dot{Z}' &= \dot{Z}' &= \dot{Z}' &= \dot{Z}' \\ \dot{Z}' &= \dot{Z}' &= \dot{Z}' &= \dot{Z}' &= \dot{Z}' \\ \dot{Z}' &= \dot{Z}' &= \dot{Z}' &= \dot{Z}' \\ \dot{Z}' &= \dot{Z}$$

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where the subscript W_Z indicates that the computed wave induced water particle motions are transformed to the buoy coordinates. The viscous drag forces and moments must include a contribution due to the steady-state currents; thus,

$$\dot{X}'' = \dot{X}'$$

$$\dot{Y}'' = \dot{Y}' - GV$$

$$\dot{z}'' = \dot{z}' - GW$$

$$\dot{x}'' = \dot{x}'$$

$$\dot{\beta}'' = \dot{\beta}'$$

$$\dot{y}'' = \dot{y}'$$

The draft of the buoy is $H_0 = H_{ce} + X'$ where H_{ce} is the height of the center of gravity of the buoy from the bottom of the buoy hull. The buoyant force is

$$B = \rho G \pi A^{3} \left[\frac{2}{3} + \frac{H_{0}}{H} \cos \beta' - \frac{H_{0}^{3}}{3} \cos^{3} \beta' \right].$$

The pitch and roll buoyant moments are given by

$$M_{\beta} = B \cdot \left(\mathbb{Z}_{c_{\beta}} \cos \beta' + (\mathcal{H}_{c_{\beta}} - X_{c_{\beta}}) \sin \beta' \right)$$

and

$$\mathcal{M}_{\gamma} = \mathbb{B} \cdot \left(Y_{cB} \quad \cos \gamma' + (\mathcal{H}_{cc} - X_{cB}) \sin \gamma' \right).$$

The hydrodynamic inertia coefficients are summarized in equation (85). Wave damping coefficients are summarized in equation (86). Viscous drag force coefficients are summarized in equation (87). Finally, wind forces are shown in equation (18).

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With all the coefficients in the equations of motion defined and with the sea state model available as a forcing function, the dynamics of a free-floating buoy* (no mooring line) can be investigated by numerical solution of equation (5). The matrix equations of motion can be rewritten to yield six, coupled, secondorder, ordinary differential equations with nonconstant coefficients — three force equations and three moment equations. They can be rewritten as twelve first-order equations.

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These equations can be solved numerically in the time domain for the six buoy motions using a standard higher order numerical integration algorithm (usually Milne Predictor-Corrector methods or Runge-Kutta methods). In this study, a fourth-order, Runge-Kutta method was used to integrate the buoy equations of motion on a UNIVAC 1108 digital computer. Although a bit slower than Predictor-Corrector methods, the Gill's Mechod Runge-Kutta subroutine is available on the UNIVAC 1108 and was found to be easier to implement for large numbers of coupled, second-order differential equations.

Errors in the fourth-order, Runge-Kutta method are on the order of h^5 , where h is the integration step size. For the smaller oceanographic buoys, it was found that a step size on the order of 10^{-2} sec was required for numerical stability. If a step size of 5×10^{-3} were used, the error would be on the order of 3.125×10^{-12} .

The computer programs used are described in detail in appendix B.

^{*}The mooring line tension force will be developed in the next section on cable dynamics and the equations of motion for the buoy will be coupled with those for the cable.

3.2 Cable Dynamics

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A study of the dynamics of cables in the ocean environment must include hydrodynamic forces acting on the cable and be capable of including nonlinear stress-strain properties for the cable. Since oceanic currents may flow in various directions at different depths and the excitation at the ends of the cable may be described by a three-dimensional force vector, the analysis must be conducted in three dimensions.

3.2.1 The Cable Equations

Consider a free body of a cable segment of length dS_0 (figure 13) being acted upon by an external force per unit length, \vec{Q} , and assume perfect flexibility. The geometric center of the cable segment lies at a point (x_0, y_0, z_0) in a cartesian coordinate system. The external force $\vec{Q} dS_0$ acting on the cable segment can be resolved into components; they are

$$\vec{Q} dS_0 = X dS_0 \cdot \hat{i} + Y dS_0 \cdot \hat{j} + Z dS_0 \cdot \hat{k}$$
 (133)

Writing Newton's Second Law, we see that in the x direction

$$\mu(S_{o}) dS_{o} \frac{\partial^{2} x}{\partial t^{2}} = X dS_{o} + \left[\hat{T}(S_{o} + dS_{o}) - \hat{T}(S_{o}) \right] \cdot \hat{i} ;$$
^(134A)

in the y direction

$$\mu(S_{o}) dS_{o} \frac{\partial^{2} y}{\partial t^{2}} = Y dS_{o} + \left[\tilde{T}(S_{o} + dS_{o}) - \tilde{T}(S_{o})\right] \cdot j \quad ; \quad (134B)$$

and in the z direction,



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$$\mu(S_{o}) dS_{o} \frac{\partial^{2} z}{\partial t^{2}} = Z_{o} dS_{o} + \left[\overline{T}(S_{o} + dS_{o}) - \overline{T}(S_{o})\right] \cdot \hat{k} , \quad (134C)$$

where $\mathcal{M}(S_{o})$ is the mass per unit length of the cable. Divide through by dS_{o} and let $dS_{o} \rightarrow 0$. Taking the limit, we can write equation (134A) as

$$\mathcal{M}(S_0)_{\partial t^2}^{\partial^2 X} = \lim_{dS_0 \to 0} \left(X + \frac{1}{dS_0} \left[\overline{T}(S_0 + dS_0) - \overline{T}(S_0) \right] \cdot \hat{i} \right),$$

or

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$$\mu(S_{o})\frac{\partial^{2}x}{\partial t^{2}} = X + \frac{\partial\overline{T}}{\partial S_{o}}\cdot\hat{1} \qquad (135A)$$

and, in a similar manner, equations (134B) and (134C) become

$$\mu(S_o)\frac{\partial^2 y}{\partial t^2} = Y + \frac{\partial \overline{T}}{\partial S_o} \cdot \hat{j}$$
(135B)

and

$$\mu(S_{k})\frac{\partial^{2}z}{\partial t^{2}} = Z_{k} + \frac{\partial\overline{T}}{\partial S_{k}}\cdot\hat{k}$$
(135C)

Define the vector \vec{r} from the origin to a point S₀ on the cable segment. The vector is defined as

$$\vec{r} = x \cdot \hat{i} + y \cdot \hat{j} + z \cdot \hat{k}$$

A vector tangent to the cable is

$$\frac{\partial \vec{r}}{\partial S_{o}} = \frac{\partial X}{\partial S_{o}} \cdot \hat{i} + \frac{\partial Y}{\partial S_{o}} \cdot \hat{j} + \frac{\partial Z}{\partial S_{o}} \cdot \hat{k}$$

The unit vector tangent to the cable is

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$$\frac{\frac{\partial \vec{r}}{\partial S_o}}{\left|\frac{\partial \vec{r}}{\partial S_o}\right|} = \frac{\frac{\partial \vec{r}}{\partial S_o}}{\sqrt{\left(\frac{\partial x}{\partial S_o}\right)^2 + \left(\frac{\partial y}{\partial S_o}\right)^2 + \left(\frac{\partial z}{\partial S_o}\right)^2}}$$

At point S_0 , the tension \vec{T} is tangential to the cable if the cable is assumed to be perfectly flexible (i.e., the cable can not support bending moments). The tension is

$$\overline{T} = T \frac{\left(\frac{\partial \overline{r}}{\partial S_{o}}\right)}{\sqrt{\left(\frac{\partial x}{\partial S_{o}}\right)^{2} + \left(\frac{\partial y}{\partial S_{o}}\right)^{2} + \left(\frac{\partial z}{\partial S_{o}}\right)^{2}}},$$

where T is the magnitude of the tension. The x component of the change of tension along the cable is

$$\frac{\partial T}{\partial S_{o}} \cdot \hat{i} = \frac{\partial}{\partial S_{o}} \left(T \cdot \hat{i} \right) = \frac{\partial}{\partial S_{o}} \left(\frac{T}{|\frac{\partial F}{\partial S_{o}}|} \cdot \frac{\partial X}{\partial S_{o}} \right)$$
(136)

Substituting equation (136) into equation (135A), we find that

$$\mu(S_{t})\frac{\partial^{2}x}{\partial t^{2}} = X + \frac{\partial}{\partial S_{t}} \left(\frac{T}{\left| \frac{\partial}{\partial S_{t}} \right|} \cdot \frac{\partial X}{\partial S_{t}} \right), \qquad (137A)$$

and in a similar manner for the y and z forces, we find that

$$\mu(S_{\bullet})\frac{\partial^{2} y}{\partial t^{2}} = \Upsilon + \frac{\partial}{\partial S_{\bullet}} \left(\frac{\Upsilon}{\left| \frac{\partial F}{\partial S_{\bullet}} \right|} \cdot \frac{\partial Y}{\partial S_{\bullet}} \right)$$
(137B)
and

$$\mathcal{M}(S_{\bullet})\stackrel{\partial^{2}}{\partial \mathcal{E}} = \mathcal{Z} + \frac{\partial}{\partial \mathcal{Z}_{\bullet}} \left(\frac{\mathcal{T}}{|\mathcal{C}_{\bullet}|} \cdot \frac{\partial \mathcal{Z}}{\partial \mathcal{S}_{\bullet}} \right). \tag{137C}$$

The strain, $(\boldsymbol{\epsilon})$, is defined as

$$\epsilon \equiv \frac{dS - dS_o}{dS_o} = \left| \frac{\partial \tilde{r}}{\partial S_o} \right| - 1 , \qquad (138)$$

where S is the strained length of the cable. Substituting equation (138) into equations (137A). (137B). and (137C), we can write the equations of motion as follows:

$$\mathcal{M}(S_{\bullet})\frac{\partial^{2} x}{\partial t^{2}} - \frac{\partial}{\partial S_{\bullet}}\left(\frac{T}{1+\epsilon} \cdot \frac{\partial x}{\partial S_{\bullet}}\right) - \chi = 0 \quad , \qquad (139A)$$

$$\mathcal{M}(\mathcal{S}_{o})\frac{\partial^{2} Y}{\partial t^{2}} - \frac{\partial}{\partial \mathcal{S}_{o}}\left(\frac{T}{1+\epsilon} \cdot \frac{\partial Y}{\partial \mathcal{S}_{o}}\right) - Y = 0 , \qquad (139B)$$

and

$$\mu(S_{o})\frac{\partial^{2}z}{\partial t^{2}} - \frac{\partial}{\partial S_{o}}\left(\frac{T}{1+\epsilon}\cdot\frac{\partial z}{\partial S_{o}}\right) - \frac{b}{Z_{o}} = 0 \qquad (139C)$$

In addition, the auxiliary equation (138) becomes

$$\varepsilon = \frac{dS}{dS_o} - 1$$

A constitutive relation,

$$T = T(\epsilon) , \qquad (140)$$

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must also be defined.

This set of equations is standard for cable systems and has been developed by Cristecu, 60 Schram, 61 Whicker, 62 Lindsay, 63 and others.

It is desirable to transform the equations of motion to a "natural" coordinate system (i.e.. a coordinate system aligned normal and tangential to the cable), because hydrodynamic forces are usually defined as normal and tangential to the cable. The coordinate systems are shown in figure 14. Rotating about the y axis, the transform is

$$A1 = \begin{bmatrix} \cos \theta & 0 & \sin \theta \\ 0 & 1 & 0 \\ -\sin \theta & 0 & \cos \theta \end{bmatrix}$$

Rotating about the z axis, the transform is

$$A2 = \begin{bmatrix} \cos\phi & \sin\phi & 0 \\ -\sin\phi & \cos\phi & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

for both rotations, we that

$$A = A2 \cdot A1 ;$$

thus,

$$A = \begin{bmatrix} \cos \phi \cos \theta & \sin \phi & \cos \phi \sin \theta \\ -5 \ln \phi \cos \theta & \cos \phi & -\sin \phi \sin \theta \\ -5 \ln \theta & 0 & \cos \theta \end{bmatrix}.$$
(141)

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Figure 14. Cable Coordinates

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A is the transform matrix from the x, y, z coordinate system to the x", y", z" system. Since A is orthogonal, we see that $A^{-1} = A^{t}$; thus,

$$A^{-1} = \begin{bmatrix} \cos\phi\cos\theta & -\sin\phi\cos\theta & -\sin\theta\\ \sin\phi\\ \cos\phi\sin\theta & -\sin\phi\sin\theta & \cos\theta \end{bmatrix}, \quad (142)$$

In the double-primed system, let x" be along the cable and z" and y" normal to each other and the cable. Since $\frac{1}{1+\epsilon} \left(\frac{\partial \vec{r}}{\partial S_{\bullet}} \right)$ is a unit vector tangent to the cable, $\frac{1}{1+\epsilon} \left(\frac{\partial \vec{r}}{\partial S_{\bullet}} \right)$ transforms to $\begin{bmatrix} 1\\0\\0 \end{bmatrix}$ in the double-primed coordinate system. This transformation is

$$\frac{1}{1+\epsilon} \left(\frac{\partial \vec{r}}{\partial S_o} \right) = A^{-1} \begin{bmatrix} 1 \\ 0 \\ 0 \end{bmatrix} ,$$

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$$\begin{pmatrix} \frac{1}{1+\epsilon} \end{pmatrix} \begin{bmatrix} \frac{\partial x}{\partial s_{o}} \\ \frac{\partial y}{\partial s_{o}} \\ \frac{\partial z}{\partial s_{o}} \end{bmatrix} = \begin{bmatrix} \cos \phi \cos \theta \\ \sin \phi \\ \cos \phi \sin \theta \end{bmatrix}.$$
(143)

The components of equation (143) are

$$\frac{\partial x}{\partial S_{e}} = (1 + \epsilon) \cos \phi \cos \theta , \qquad (144A)$$

$$\frac{\partial Y}{\partial S_0} = (1+\epsilon) \sin \phi , \qquad (144B)$$

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and

$$\frac{\partial z}{\partial S_o} = (1 + \epsilon) \cos \phi \sin \theta \qquad (144C)$$

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Let H, G, and I be forces per unit length acting on the cable along the x'', y'', and z'' axes, respectively. In earth coordinates, the cable forces per unit length are

$$\begin{bmatrix} X \\ Y \\ Z \end{bmatrix} = A^{-1} \begin{bmatrix} H \\ G \\ I \end{bmatrix}$$
(145)

Writing equations (139A), (139B), and (139C) in vector form (in the unprimed coordinate system), we see that

$$\begin{bmatrix} \frac{\partial}{\partial S_{o}} (T \cos \phi \cos \theta) \\ \frac{\partial}{\partial S_{o}} (T \sin \phi) \\ \frac{\partial}{\partial S_{o}} (T \cos \phi \sin \theta) \end{bmatrix} + A^{-1} \begin{bmatrix} H \\ G \\ I \end{bmatrix} = \mathcal{M}(S_{o}) \begin{bmatrix} \frac{\partial \mathcal{U}}{\partial t} \\ \frac{\partial \mathcal{V}}{\partial t} \\ \frac{\partial \mathcal{W}}{\partial t} \end{bmatrix},$$
(146)

where u, v, and w are velocities in the x. y, and z directions, respectively. Transforming to the double-primed coordinate system, equation (146) becomes

$$A\begin{bmatrix}\frac{\partial}{\partial S_{o}}(T\cos\phi\cos\theta)\\ \frac{\partial}{\partial S_{o}}(T\sin\phi)\\ \frac{\partial}{\partial S_{o}}(T\cos\phi\sin\theta)\end{bmatrix} + \begin{bmatrix}H\\G\\I\end{bmatrix} = \mathcal{M}(S_{o})A\begin{bmatrix}\frac{\partial U}{\partial U}\\ \frac{\partial V}{\partial U}\\ \frac{\partial V}{\partial U}\\ \frac{\partial W}{\partial U}\end{bmatrix}. (147)$$

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$$\begin{aligned} \frac{\partial T}{\partial S_{o}} \left(\cos^{2}\phi\cos^{2}\theta + \sin^{2}\phi + \cos^{2}\phi\sin^{2}\theta \right) \\ + T \frac{\partial \Phi}{\partial S_{o}} \left(-\cos^{2}\theta\cos\phi\sin\phi + \sin\phi\cos\phi - \sin^{2}\theta\sin\phi\cos\phi \right) \\ + T \frac{\partial \theta}{\partial S_{o}} \left(-\cos^{2}\phi\sin\phi\cos\phi + \cos^{2}\phi\sin\phi\cos\phi \right) \\ + T \frac{\partial \theta}{\partial S_{o}} \left(-\cos^{2}\theta\sin\phi\cos\phi + \cos\phi\sin\phi - \sin^{2}\theta\sin\phi\cos\phi \right) \\ + T \frac{\partial \phi}{\partial S_{o}} \left(\cos^{2}\theta\sin^{2}\phi + \cos^{2}\phi + \sin^{2}\theta\sin^{2}\phi \right) \\ + T \frac{\partial \theta}{\partial S_{o}} \left(\sin\phi\cos\phi\sin\phi - \sin\phi\cos\phi - \sin\phi\cos\phi\sin\theta\cos\phi \right) \\ + T \frac{\partial \theta}{\partial S_{o}} \left(\sin\phi\cos\phi\sin\phi\cos\phi - \sin\phi\cos\phi\sin\theta\cos\phi \right) \\ + T \frac{\partial \phi}{\partial S_{o}} \left(\cos\phi\sin\phi\sin\phi - \cos\phi\cos\phi \right) \\ + T \frac{\partial \phi}{\partial S_{o}} \left(\cos\phi\sin\phi\sin\phi - \cos\phi\cos\phi \right) \\ + T \frac{\partial \phi}{\partial S_{o}} \left(\cos\phi\sin\phi\sin\phi - \cos\phi\cos\phi \right) \\ \end{aligned}$$

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which can be reduced to

$$\begin{bmatrix} \frac{\partial T}{\partial S} \\ T \frac{\partial \phi}{\partial S} \\ T \frac{\partial \phi}{\partial S} \end{bmatrix}$$

(148)

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Now consider the term on the right side of equation (147),

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If U.V, and W are the velocity components along the x", y", and z" axes, respectively, the velocity components in inertial coordinates are

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$$\begin{bmatrix} u \\ v \\ w \end{bmatrix} = \mathcal{A}^{-1} \begin{bmatrix} U \\ V \\ W \end{bmatrix}$$

then, the acceleration components are

$$\begin{bmatrix} \frac{3}{2} \\ \frac{$$

Multiply through by the mass density and the transformation matrix (A),

$$\mu(S,)A\begin{bmatrix}\frac{\partial U}{\partial t}\\\frac{\partial V}{\partial t}\\\frac{\partial V}{\partial t}\end{bmatrix} = \mu(S,)A\frac{\partial}{\partial t}(A^{-1})\begin{bmatrix}V\\V\\W\end{bmatrix} + \mu(S,)AA^{-1}\begin{bmatrix}\frac{\partial U}{\partial t}\\\frac{\partial V}{\partial t}\\\frac{\partial V}{\partial t}\\\frac{\partial W}{\partial t}\end{bmatrix}.$$
(149)

Considering the first term on the right of equation (149), we see that

$$\frac{\partial}{\partial t}(A^{-1}) = \frac{\partial}{\partial t}\begin{bmatrix} \cos\phi\cos\theta & -\sin\phi\cos\theta & -\sin\theta\\ \sin\phi & \cos\theta & 0\\ \cos\phi\sin\theta & -\sin\phi\sin\theta & \cos\theta \end{bmatrix}$$

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$$\begin{cases} -\cos\theta \sin\theta \frac{1}{2\theta} \\ -\cos\theta \sin\theta \frac{1}{2\theta} \\ -\cos\theta \sin\theta \frac{1}{2\theta} \\ (\cos\theta \frac{1}{2\theta}) \\ (\cos\theta$$

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The above matrix can be reduced, and we find that

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$$A \frac{\partial}{\partial t} (A^{-1}) = \begin{bmatrix} 0 & -\frac{\partial \phi}{\partial t} & -\cos\phi \frac{\partial \theta}{\partial t} \\ \frac{\partial \phi}{\partial t} & 0 & \sin\phi \frac{\partial \theta}{\partial t} \\ \cos\phi \frac{\partial \theta}{\partial t} & -\sin\phi \frac{\partial \theta}{\partial t} & 0 \end{bmatrix}, \quad (150)$$

Substituting into equation (149), the accelerations become

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s.

$$\mathcal{M}(S_{o})A \begin{bmatrix} \frac{3}{2} \\ \frac{3}{2} \\ \frac{3}{2} \\ \frac{3}{2} \end{bmatrix} = \mathcal{M}(S_{o}) \begin{bmatrix} \frac{3}{2} \\ -\sqrt{\frac{3}{2}} \\ -\sqrt{\frac{3}{2}} \\ -\sqrt{\frac{3}{2}} \\ \frac{3}{2} \\ +U\cos\phi \frac{3}{2} \\ -\sqrt{\sin\phi} \frac{3}{2} \\ \frac{3}{2} \\ \frac{3}{2} \\ +U\cos\phi \frac{3}{2} \\ -\sqrt{\sin\phi} \frac{3}{2} \\ \frac{3}{$$

(151)

Substituting equations (148) and (151) into equation (147) and writing the component equations, we obtain the equations of motion:

$$\frac{\partial T}{\partial S_{o}} + H = \mathcal{M}(S_{o}) \left(\frac{\partial U}{\partial t} - V \frac{\partial \phi}{\partial t} - W \cos \phi \frac{\partial \theta}{\partial t} \right)$$
(152)
$$T \frac{\partial \phi}{\partial S_{o}} + G = \mathcal{M}(S_{o}) \left(\frac{\partial V}{\partial t} + U \frac{\partial \phi}{\partial t} + W \sin \phi \frac{\partial \theta}{\partial t} \right)$$
(153)

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$$T\cos\phi\frac{\partial\theta}{\partial S_{c}} + I = \mu(S_{c})\left(\frac{\partial W}{\partial t} + U\cos\phi\frac{\partial\theta}{\partial t} - V\sin\phi\frac{\partial\theta}{\partial t}\right)$$
(154)

In addition, we have the strain definition,

$$\frac{\partial S}{\partial S_{\mu}} \approx 1 + \epsilon \qquad (155)$$

and the constitutive relation,

$$T = T(\epsilon) \tag{156}$$

Equations (152) through (156) give five equations in eight unknowns $(T, \phi, \theta, U, V, W, S \& \epsilon)$, Three more geometrical equations can be developed by considering the velocities. The velocities are

$$\begin{bmatrix} u \\ v \\ w \end{bmatrix} = A^{-1} \begin{bmatrix} v \\ v \\ W \end{bmatrix}$$

and the space derivative of the velocity vector is

$$\frac{\partial}{\partial S_{o}}\begin{bmatrix} u \\ v \\ w \end{bmatrix} = \frac{\partial}{\partial S_{o}} (A^{-'}) \begin{bmatrix} U \\ V \\ W \end{bmatrix} + A^{-'} \begin{bmatrix} \frac{\partial U}{\partial S_{o}} \\ \frac{\partial W}{\partial S_{o}} \\ \frac{\partial W}{\partial S_{o}} \end{bmatrix}$$
(157)

Consider the left-hand term of equation (157). The space derivative of the

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$$\frac{\partial}{\partial S_{v}} \begin{bmatrix} u \\ v \\ w \end{bmatrix} = \frac{\partial}{\partial S_{v}} \begin{bmatrix} \frac{\partial X}{\partial t} \\ \frac{\partial Y}{\partial t} \\ \frac{\partial Y}{\partial S_{v}} \end{bmatrix} = \frac{\partial}{\partial t} \begin{bmatrix} \frac{\partial X}{\partial S_{v}} \\ \frac{\partial Y}{\partial S_{v}} \\ \frac{\partial Y}{\partial S_{v}} \end{bmatrix}$$

From equations (144A), (144B), and (144C), we see that

$$\frac{\partial}{\partial S_{v}}\begin{bmatrix} u \\ v \\ w \end{bmatrix} = \frac{\partial}{\partial t} \begin{bmatrix} (1+\epsilon)\cos\phi\cos\phi \\ (1+\epsilon)\sin\phi \\ (1+\epsilon)\cos\phi\sin\phi \end{bmatrix},$$

(158)

Rewrite equation (157) and transform with A; thus

$$AA^{-1}\begin{bmatrix}\frac{2\overline{V}}{\overline{\partial}S_{v}}\\\frac{2\overline{V}}{\overline{\partial}S_{v}}\\\frac{2\overline{W}}{\overline{\partial}S_{v}}\end{bmatrix} = A\frac{2}{\overline{\partial}S_{v}}\begin{bmatrix}u\\v\\w\end{bmatrix} - A\frac{2}{\overline{\partial}S_{v}}(A^{-1})\begin{bmatrix}V\\V\\W\end{bmatrix}$$

and

$$\begin{bmatrix} \frac{\partial U}{\partial S_{\bullet}} \\ \frac{\partial V}{\partial S_{\bullet}} \\ \frac{\partial W}{\partial S_{\bullet}} \end{bmatrix} = A \frac{\partial}{\partial t} \begin{bmatrix} (1+\epsilon) \cos \phi \cos \theta \\ (1+\epsilon) \sin \phi \\ (1+\epsilon) \cos \phi \sin \theta \end{bmatrix} - A \frac{\partial}{\partial S_{\bullet}} (A^{-1}) \begin{bmatrix} U \\ V \\ W \end{bmatrix}.$$
(159)

By analogy with the development of equation (148), the first term on the righthand side of the above equation is

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$$\frac{\partial \epsilon}{\partial t}$$

$$(1+\epsilon) \frac{\partial \phi}{\partial t}$$

$$(1+\epsilon) \cos \phi \frac{\partial \theta}{\partial t}$$

By analogy with the development of equation (150), the second term on the righthand of equation (139) is



Substituting into equation (159), and writing the component equations gives

$$\frac{\partial V}{\partial S_{o}} = \frac{\partial \epsilon}{\partial t} - \left(-V \frac{\partial \phi}{\partial S_{o}} - W \cos \phi \frac{\partial \phi}{\partial S_{o}} \right), \qquad (160A)$$

$$\frac{\partial V}{\partial S_{o}} = (1+\epsilon)\frac{\partial \phi}{\partial t} - \left(U\frac{\partial \phi}{\partial S_{o}} + WSIN\phi\frac{\partial \phi}{\partial S_{o}}\right), \qquad (160B)$$

and

$$\frac{\partial W}{\partial S_{o}} = (1+\epsilon)\cos\phi\frac{\partial\theta}{\partial t} - (U\cos\phi\frac{\partial\theta}{\partial S_{o}} - V\sin\phi\frac{\partial\theta}{\partial S_{o}}) \quad (1500)$$

Rewriting the constitutive relation, we obtain the time derivative of tension:

$$\frac{\partial T}{\partial t} = \frac{dT}{d\epsilon} \frac{\partial \epsilon}{\partial t}$$

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$$\frac{\partial T}{\partial S} - \mathcal{M}(S) \left(\frac{\partial U}{\partial t} - V \frac{\partial \phi}{\partial t} - \tilde{W} \cos \phi \frac{\partial \phi}{\partial t} \right) + H = 0 \quad (161)$$

$$\Gamma \frac{\partial \phi}{\partial S_{o}} - \mathcal{M}(S_{o}) \left(\frac{\partial V}{\partial t} + V \frac{\partial \phi}{\partial t} + W \sin \phi \frac{\partial \phi}{\partial t} \right) + G = 0 \quad , \quad (162)$$

$$T\cos\phi \frac{\partial\theta}{\partial S_{o}} - \mathcal{M}(S_{o})\left(\frac{\partial W}{\partial t} + U\cos\phi \frac{\partial\theta}{\partial t} - V\sin\phi \frac{\partial\theta}{\partial t}\right) + I = 0, \quad (163)$$

$$\frac{\partial V}{\partial S_{o}} - \frac{\partial \epsilon}{\partial t} + \left(-V \frac{\partial \phi}{\partial S_{o}} - W \cos \phi \frac{\partial \phi}{\partial S_{o}} \right) = 0, \qquad (164)$$

$$\frac{\partial V}{\partial S_o} - (1+\epsilon)\frac{\partial \phi}{\partial t} + \left(U\frac{\partial \phi}{\partial S_o} + W \sin \phi \frac{\partial \theta}{\partial S_o}\right) = 0 , \qquad (165)$$

$$\frac{\partial W}{\partial S_{\bullet}} - (1+\epsilon) \cos \phi \frac{\partial \Theta}{\partial t} + (U \cos \phi \frac{\partial \Theta}{\partial S_{\bullet}} - V \sin \phi \frac{\partial \Theta}{\partial S_{\bullet}}) = 0 \quad (166)$$

and

$$\frac{\partial T}{\partial t} - \frac{dT'}{d\epsilon} \frac{\partial \epsilon}{\partial t} = 0$$
(167)

We have seven, nonlinear, partial differential equations in seven unknowns. Using the constitutive relation equation (167), these reduce to six equations with six unknowns. Jeffrey and Taniuti^{6,1} show that an analytic solution in the form of a power series for a single, nonlinear, hyperbolic, partial differential equation can only be obtained locally, or "in the small" for a point P on a noncharacteristic curve \mathcal{T} in the time-space plane. Physically, it is known that the flexible cable can propagate both tensile stress waves and transverse flexural waves. It is also known that the characteristic velocity of the transverse waves is a function of the state of stress of the cable. Thus, the usual method of characteristics approach to solve nonlinear wave equations is not tractable because the characteristics diverge in the time-space domain. Solutions must be obtained simultaneously at the same locations in the time-space domain in order to continually update the transverse wave characteristic. A numerical method, which is an extension of Hartree's "hybrid" method, ⁶⁵⁻⁷⁰ will be employed to conduct simultaneous integrations at nodal points of a rectangular grid in the time-space domain. Because this method utilizes integration along characteristics in the immediate vicinity of the time-space grid nodal points, it is necessary to rewrite the cable equations in their "normal" form, i.e., characteristic equations.

First, the characteristics must be found. Assume a linear variation of tension with strain. Equation (167) reduces to

$$T = \frac{dT}{d\epsilon} \epsilon = K \epsilon \, .$$

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Rewriting equations (161) through (166) in the form $AU_t + BU_s + G = O$ (168) gives

0	μV	μW cos φ	-,M	0	0	∂€∕H
0	-JUU	-MW sind	0	-µ	0	30/St
0	0	-u(Ucoso-Vsino)	0	0	-µ	30 St
-1	0	0	0	0	0	oust
0	-(1+E)	0	0	0	0	arst
0	0	$-(1+\epsilon)\cos\phi$	0	0	0	JW St

Rewriting equation (168), we see that

$$A^{'}AU_t + A^{'}BU_s + A^{'}C = 0 ,$$

or

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$$U_t + A^{-1}B U_s + A^{-1}C = 0$$
⁽¹⁶⁹⁾

The six characteristics are derived from equation (169). This derivation is shown in detail in appendix C. From appendix C the six equations (equations (161) through (166)) yield six characteristics:

$$h_{i} = \pm \sqrt{\frac{1}{\mathcal{U}(S_{o})}} \frac{dT}{d\varepsilon} , \qquad (170)$$

$$\lambda_2 = -\sqrt{\frac{1}{\mu(S_i)}} \frac{dT}{d\epsilon} , \qquad (171)$$

$$\lambda_{3} = + \sqrt{\frac{1}{\mathcal{U}(S_{o})}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon} , \qquad (172)$$

$$\lambda_{4} = -\sqrt{\frac{1}{\mu(S_{e})}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon} , \qquad (173)$$

$$\lambda_{5} = + \sqrt{\frac{1}{\mathcal{U}(S_{o})}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon}, \qquad (174)$$

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$$\lambda_{\epsilon} = -\sqrt{\frac{1}{\mu(s_{\epsilon})}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon}$$
(175)

Rewrite the cable equations in their normal form using the characteristics. To find the characteristic equations, we can write equation (161) in terms of ϵ using the constitutive relation (equation (167)) multiplied by $\frac{1}{2}$ and subtract from equation (164) multiplied by dS. This operation is

$$\begin{bmatrix} \frac{1}{2t} & \frac{dT}{de} & \frac{\partial e}{\partial s} & dt & -\left(\frac{\partial V}{\partial t} & dt & -V & \frac{\partial \phi}{\partial t} & dt & -W & \cos\phi & \frac{\partial \theta}{\partial t} & dt\right) \\ & +\frac{1}{2t} & H & dt \end{bmatrix} \\ + \begin{bmatrix} \frac{\partial V}{\partial s} & ds & -\frac{\partial e}{\partial t} & ds & +\left(-V & \frac{\partial \phi}{\partial s} & ds & -W & \cos\phi & \frac{\partial \theta}{\partial s} & ds\right) \end{bmatrix} = 0$$
(176)

Since there are only two independent variables, s and t , the total differential of any dependent variable, for example U, is

$$dU = \frac{\partial V}{\partial s} \, ds + \frac{\partial V}{\partial t} \, dt \, . \tag{177}$$

The variables in equation (176) are expanded as follows:

$$-\frac{1}{2t}\frac{dT}{d\epsilon}\stackrel{2\epsilon}{\Rightarrow} dt - \frac{2\epsilon}{3t}ds + \frac{2V}{3t}dt + \frac{2V}{3s}ds - V\stackrel{2}{\Rightarrow} \frac{dt}{dt}$$
$$-V\stackrel{2\theta}{\Rightarrow} ds - W\cos\phi\stackrel{2\theta}{\Rightarrow} dt - W\cos\phi\stackrel{2\theta}{\Rightarrow} ds - \frac{1}{2t}Hdt = 0, (178)$$

or

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$$-\frac{dT}{d\epsilon} \frac{\partial \epsilon}{\partial s} dt - \frac{\partial \epsilon}{\partial t} ds + dU - V d\phi - W \cos \phi d\theta - \frac{1}{2} H dt = 0.$$
(179)

In terms of the characteristic values, λ , we see that

$$\left(-\lambda_{i,z}^{2}\frac{\partial\epsilon}{\partial s} - \frac{1}{\omega}H\right)dt - \frac{\partial\epsilon}{\partial t}ds + dV - Vd\phi - W\cos\phi d\theta = 0, (180)$$
where $\lambda_{i,z} = \pm \sqrt{\frac{1}{\omega}\frac{dT}{d\epsilon}}$ are the axial wave characteristics
in the t-s plane; hence,
$$\frac{ds}{dt} = \pm \sqrt{\frac{1}{\omega}\frac{dT}{d\epsilon}}$$

The characteristic equation becomes

$$-\sqrt{\frac{1}{2}} \frac{dT}{d\epsilon} \cdot d\epsilon + dU - V d\phi - W \cos\phi d\theta - \frac{1}{2} H dt = O_{(181)}$$

This equation represents the motion of the cable in the axial direction (stretching). If we let the \propto characteristic be associated with $+\sqrt{\frac{1}{\mathcal{U}}\frac{dT}{d6}}$ and represent disturbances traveling down the cable, we see that

$$\frac{dU}{d\alpha} - \sqrt{\frac{1}{\mu}} \frac{dT}{d\epsilon} \frac{d\epsilon}{d\alpha} - V \frac{d\phi}{d\alpha} - W \cos\phi \frac{d\theta}{d\alpha} - \frac{1}{\mu} H \frac{dt}{d\alpha} = O_{(162)}$$

Similarly, let \mathcal{A} be associated with disturbances traveling up the cable,

$$-\sqrt{\frac{1}{24}} \frac{dT}{d\epsilon} ; \text{ then,}$$

$$\frac{dU}{d\beta} + \sqrt{\frac{1}{24}} \frac{dT}{d\epsilon} \frac{d\epsilon}{d\beta} - \sqrt{\frac{d\theta}{d\beta}} - W\cos\phi \frac{d\theta}{d\beta} - \frac{1}{24}H \frac{dt}{d\beta} = 0 . \quad (183)$$

Again. equation (162) multiplied by (//) dt is subtracted from equation (165) multiplied by ds to give

$$\frac{\partial V}{\partial s} ds + \frac{\partial V}{\partial t} dt + U \frac{\partial \phi}{\partial s} ds + U \frac{\partial \phi}{\partial t} dt + W \sin \phi \frac{\partial g}{\partial s} ds$$
$$+ W \sin \phi \frac{\partial g}{\partial t} dt - (1+\epsilon) \frac{\partial g}{\partial t} ds - \frac{d}{dt} \frac{\partial f}{\partial s} \frac{\partial f}{\partial t} - \frac{1}{dt} G dt = 0.$$

Dividing through by ($1+\pmb{\epsilon}$), we find that

$$-\left(\frac{1}{\mu}\frac{\epsilon}{(1+\epsilon)}\frac{dT}{d\epsilon}\frac{\partial\phi}{\partial s}dt + \frac{\partial\phi}{\partial t}ds\right) + \frac{1}{(1+\epsilon)}Ud\phi + \frac{1}{(1+\epsilon)}dV$$
$$+ \frac{1}{(1+\epsilon)}W\sin\phi d\theta - \frac{1}{\mu(1+\epsilon)}Gdt = 0$$

The transverse wave characteristics in the t-s plane are

$$\lambda_{3,4} = \pm \sqrt{\frac{1}{\mathcal{U}}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon} ;$$

hence,

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$$\frac{ds}{dt} = \pm \sqrt{\frac{1}{\mu} \frac{\epsilon}{(1+\epsilon)}} \frac{dT}{d\epsilon},$$

and

$$dV - (1+\epsilon) \int \frac{i}{\pi} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon} d\phi + U d\phi + W \sin \phi d\theta - \frac{i}{\pi} G dt = 0$$

Let the \propto characteristic be associated with $+\sqrt{\frac{1}{2}\frac{\epsilon}{(1+\epsilon)}\frac{dT}{d\epsilon}}$ and represent disturbances traveling <u>down</u> the cable in the y'' - x'' plane:

$$\frac{dV}{d\alpha} - \left((1+\epsilon) \int_{\mathcal{U}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon} + U \right) \frac{d\phi}{d\alpha} + W \sin\phi \frac{d\theta}{d\alpha} - \frac{1}{\mathcal{U}} G \frac{dt}{d\alpha} = 0 \quad (184)$$

Let the \mathcal{B} characteristic be associated with $-\sqrt{\frac{1}{\mathcal{A}}\frac{\varepsilon}{(1+\varepsilon)}}\frac{dT}{d\varepsilon}$ and represent disturbances traveling up the cable in the y'' - x'' plane:

$$\frac{dV}{dB} + \left((1+\epsilon) \int \frac{1}{\mu} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon} - U \right) \frac{d\phi}{dB} + W \sin\phi \frac{d\theta}{dB} - \frac{1}{\mu} G \frac{dt}{dB} = 0.$$
(185)

Finally, equation (163) multiplied by $(\frac{1}{24})$ dt is subtracted from equation (166) multiplied by ds to give

$$-(1+\epsilon)\cos\phi\left(\frac{\partial\theta}{\partial t}ds + \frac{1}{24}\frac{\epsilon}{(1+\epsilon)}\frac{dT}{d\epsilon}\frac{\partial\theta}{\partial s}dt\right) + dW$$
$$+(U\cos\phi - V\sin\phi)d\theta - \frac{1}{24}Idt = 0.$$

The transverse wave characteristic in the t-s plane is

$$\lambda_{s,e} = \pm \sqrt{\frac{1}{\mathcal{M}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon}}$$

hence

$$\frac{ds}{dt} = \pm \sqrt{\frac{1}{M} \frac{\epsilon}{(1+\epsilon)}} \frac{dT}{d\epsilon}$$

and

Let

$$dW - (1+\epsilon)\cos\phi \sqrt{\frac{1}{4}} \frac{\epsilon}{d\epsilon} \frac{dT}{d\epsilon} \cdot d\theta + (U\cos\phi - V\sin\phi)d\theta - \frac{1}{4}Idt = 0$$

the \propto characteristic be associated with $+\sqrt{\frac{1}{4}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon}$ and

represent disturbances traveling down the cable in the z'' - x'' plane:

$$\frac{dW}{d\alpha} - \left((1+\epsilon)\cos\phi/\underline{L}\frac{\epsilon}{d\alpha}\frac{dT}{d\epsilon} - (U\cos\phi-V\sin\phi)\right)\frac{d\theta}{d\alpha} - \underline{L}I\frac{dt}{d\alpha} = O_{(186)}$$

Let the **B** characteristic be associated with

 $-\sqrt{\frac{1}{\mathcal{L}}\frac{\epsilon}{(l+\epsilon)}}\frac{dT}{d\epsilon}$ and

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represent disturbances traveling up the cable in the z'' - x'' plane:

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$$\frac{dW}{dB} + \left((1+\epsilon)\cos\phi \int_{\mathcal{U}}^{1} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon} + (U\cos\phi - V\sin\phi) \frac{d\theta}{dB} - \frac{1}{\mathcal{U}}I\frac{dt}{dB} = O_{(187)}$$

The characteristic equations are now summarized. For tensile waves traveling down the cable with velocity $+\sqrt{\frac{1}{\mathcal{U}(S_{*})}} \frac{dT}{dE}$, we have

$$\frac{dV}{d\alpha} - \sqrt{\mu_{x}} \frac{dT}{d\epsilon} \frac{d\epsilon}{d\alpha} - V \frac{d\theta}{d\alpha} - W \cos \theta \frac{d\theta}{d\alpha} - \frac{1}{\mu_{x}} H \frac{dt}{d\alpha} = 0 \quad (188)$$

For tensile waves traveling up the cable with velocity $-\sqrt{\frac{dT}{dC}}$, we have

$$\frac{dU}{dB} + \int \frac{dT}{dE} \frac{dE}{dE} - V \frac{d\theta}{dB} - W \cos \phi \frac{d\theta}{dB} - \frac{d}{dE} H \frac{dE}{dE} = 0 \qquad (189)$$

For transverse waves in the y'' - x'' plane traveling down the cable with velocity $+ \sqrt{\frac{i}{\mathcal{U}(S_{*})}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon}$, we have

$$\frac{dV}{d\alpha} - \left((1+\epsilon) \int_{\mathcal{M}(S_{0})}^{1} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon} + U \right) \frac{d\phi}{d\alpha} + W \sin\phi \frac{d\theta}{d\alpha} - \frac{1}{\mathcal{M}(S_{0})} G \frac{dt}{d\alpha} = 0 \quad (190)$$

For transverse waves in the y'' - x'' plane traveling up the cable with velocity

$$-\sqrt{\mu(s_{0})}\frac{\epsilon}{(1+\epsilon)}\frac{dT}{d\epsilon}, \text{ we have}$$

$$\frac{dV}{dB} + \left((1+\epsilon)\sqrt{\frac{1}{\mu(s_{0})}}\frac{\epsilon}{(1+\epsilon)}\frac{dT}{d\epsilon} - U\right)\frac{d\theta}{dB} + W \sin\phi \frac{A\theta}{dB} - \frac{1}{\mu(s_{0})}G\frac{dt}{dB} = 0. (191)$$

For transverse waves in the z'' - x'' plane traveling down the cable with

velocity
$$+\sqrt{\frac{1}{14(S_{1})(1+\epsilon)}} \frac{\epsilon}{d\epsilon} dT$$
, we have

$$\frac{dW}{d\alpha} - \left((1+\epsilon)\cos\phi \int \frac{1}{\mu(s_{i})(1+\epsilon)} \frac{\epsilon}{d\epsilon} \frac{dT}{d\epsilon} - \left(U\cos\phi - V\sin\phi \right) \frac{d\theta}{d\alpha} - \frac{1}{\mu(s_{i})} I \frac{dt}{d\alpha} = O_{(192)}$$

For transverse waves in the z'' - x'' plane traveling up the cable with velocity

$$-\sqrt{\frac{1}{\mu(S_{i})}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon}, \text{ we have}$$

$$\frac{dW}{dS} + \left((1+\epsilon)\cos\phi\sqrt{\frac{1}{\mu(S_{i})}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon} + \left(U\cos\phi-V\sin\phi\right)\frac{d\theta}{dS} - \frac{1}{\mu(S_{i})}\frac{I}{dE} = 0 \quad (193)$$

Equations (188) through (193) form the basis for a numerical solution of the three-dimensional equations of motion for a buoy mooring cable. The original set of six partial differential equations with six unknowns has been transformed to a set of six ordinary differential equations with six unknowns with the restriction that integration operations must be carried out along characteristic curves.

The solution of sets of coupled, nonlinear, partial differential equations with different characteristic velocities is usually accomplished by assuming that the equations can be decoupled. At best, they can then be linearized and a separation of variables method can be used to obtain an analytical solution. At worst, the normal forms can be integrated numerically along characteristics.

Examination of the functional form of the transverse wave characteristics show that they are dependent on the state of stress in the cable. Thus, the cable equations can not be decoupled if they are subject to time or space varying forces. Also, the particular cable system studied here is a combined initial value boundary value system:

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1. All the dependent parameters are known at time = 0.

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2. The displacements at the lower bound (anchor) are constants for all time and their time derivations are zero.

3. The displacements at the upper bound (buoy motions) must be solved for simultaneously as are cable motions.

Buoy system cable motions, as posed here, can best be handled numerically with a modification of Hartree's method, i.e., solving for the values of the dependent variables at rectangular grid nodes in the time-space domain. Hartree's method, as described by Ames, ⁶⁶ was developed for a single hyperbolic equation whose coefficients may be time or space dependent. In this study, the basic method will be extended for a set of coupled hyperbolic equations with variable coefficients.

3.2.2 Finite-Difference Methods

Consider a rectangular grid in the time-space plane (figure 15). Assume that the values of the six independent parameters are known at the nodal points on the jth time line. It is desired to advance the solution to the nodal points on the (j + 1)th time line; specifically, for the ith point R. If the characteristics are known at point R, characteristic lines can be drawn back from R to intersect the jth time line. The six parameter values at points E, A, and D can be linearly interpolated to find the values at each intersection point, P_1 , P_2 , Q_1 , and Q_2 . Now, the characteristic equations, (188) through (193), can be used to advance the solution to point R. For tensile waves, the length between point A and point P_1 in the grid is



Figure 15. Grid for Adaptation of Hartree's Method

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$$S_{R} - S_{P_{1}} = \frac{1}{2} (Ch_{1R} + Ch_{1P_{1}}) \cdot k$$

If constant cable mass and modulus is assumed, the tensile characteristic is independent of the tension; thus,

 $Ch_{1R} = Ch_{1P_1} = Ch_1 = \sqrt{\frac{1}{2k}} \frac{dT}{d\epsilon}$. The location of point P_1 is given by $S_R - S_{P_1} = Ch_1 \cdot k$; in a similar manner, point Q_1 (associated with tensile waves propagating up the cable) is given by $S_R - S_{Q_1} = -Ch_1 \cdot k$. For tensile waves traveling down the cable, the finite-difference equation is

$$U_{R} - U_{P_{I}} - \frac{1}{2} (Ch_{1_{R}} + Ch_{1_{P_{I}}}) (\epsilon_{R} - \epsilon_{P_{I}}) - \frac{1}{2} (V_{R} + V_{P_{I}}) (\phi_{R} - \phi_{P_{I}}) - \frac{1}{2} (W_{R} + W_{P_{I}}) [Cos(\frac{1}{2} (\phi_{R} + \phi_{P_{I}}))] (\theta_{R} - \theta_{P_{I}}) - \frac{1}{\sqrt{2}} \cdot \frac{1}{2} (H_{R} + H_{P_{I}}) \cdot \kappa = 0$$
⁽¹⁹⁴⁾

For tensile waves traveling up the cable, the finite-difference equation is

$$U_{R} - U_{Q_{I}} + \frac{1}{2} (Ch_{1_{R}} + Ch_{1_{Q_{I}}}) (\epsilon_{R} - \epsilon_{q_{I}}) - \frac{1}{2} (V_{R} + V_{Q_{I}}) (\phi_{R} - \phi_{q_{I}})$$

$$- \frac{1}{2} (W_{R} + W_{Q_{I}}) [cos(\frac{1}{2}(\phi_{R} + \phi_{Q_{I}}))] (\Theta_{R} - \Theta_{Q_{I}})$$

$$- \frac{1}{2} (H_{R} + H_{Q_{I}}) \cdot k = 0$$
(195)

For transverse waves in the y'' - x'' plane traveling down the cable, point P_2 is located by

$$S_{R} - S_{P_{2}} = \frac{1}{2} (Ch_{2R} + Ch_{2P_{3}}) \cdot k$$

where

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$$Ch_{2R} = \sqrt{\frac{1}{M} \frac{\epsilon}{(1+\epsilon)}} \frac{dT}{d\epsilon}$$

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The finite-difference equation is

$$V_{R} - V_{P_{2}} - \frac{1}{2} \left(Ch_{2R} + Ch_{2P_{2}} \right) (\phi_{R} - \phi_{P_{2}}) + \frac{1}{2} \left(V_{R} + W_{P_{2}} \right) \left[SIN \left(\frac{1}{2} (\phi_{R} + \phi_{P_{2}}) \right) \right] (\Theta_{R} - \Theta_{P_{2}}) - \frac{1}{2} \cdot \frac{1}{2} \left(G_{R} + G_{P_{2}} \right) \cdot K = 0$$
(196)

For transverse waves in the y'' - x'' plane traveling up the cable, point Q_2 is located by

$$S_{R} - S_{Q_{2}} = -\frac{1}{2} (Ch_{2R} + Ch_{2Q_{2}}) \cdot k$$
.

The finite-difference equation is

$$V_{R} - V_{Q_{2}} + \frac{1}{2} (Ch_{2_{R}} + Ch_{2_{Q_{2}}}) (\phi_{R} - \phi_{Q_{2}}) + \frac{1}{2} (W_{R} + W_{Q_{2}}) \left[SIN (\frac{1}{2} (\phi_{R} + \phi_{Q_{2}})) \right] (\theta_{R} - \theta_{Q_{2}}) - \frac{1}{2} \cdot \frac{1}{2} (G_{R} + G_{Q_{2}}) \cdot \kappa = 0$$
(197)

For transverse waves in the z'' - x'' plane traveling down the cable (note that the characteristics for the transverse waves are the same), the finite-difference equation is

$$W_{R} - W_{P_{2}} - \left\{ \frac{1}{2} \left[(1 + \epsilon_{R}) Ch_{2R} + (1 + \epsilon_{P_{2}}) Ch_{2P_{2}} \right] COS \left(\frac{1}{2} (\phi_{R} + \phi_{P_{2}}) \right) - \frac{1}{2} (U_{R} + U_{P_{2}}) COS \left(\frac{1}{2} (\phi_{R} + \phi_{P_{2}}) \right) + \frac{1}{2} (V_{R} + V_{P_{2}}) SIN \left(\frac{1}{2} (\phi_{R} + \phi_{P_{2}}) \right) \right\} (\Theta_{R} - \Theta_{P_{2}}) - \frac{1}{2} (I_{R} + I_{P_{2}}) \cdot k = 0 .$$
(198)

For transverse waves in the z'' - x'' plane traveling up the cable, the finite-

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difference equation is

$$W_{R} - W_{Q_{2}} + \left\{ \frac{1}{2} \left[(1 + \epsilon_{R}) Ch_{2R} + (1 + \epsilon_{Q_{2}}) Ch_{2Q_{2}} \right] COS \left(\frac{1}{2} (\phi_{R} + \phi_{Q_{2}}) \right) + \frac{1}{2} \left(U_{R} + U_{Q_{2}} \right) COS \left(\frac{1}{2} (\phi_{R} + \phi_{Q_{2}}) \right) - \frac{1}{2} \left(V_{R} + V_{Q_{2}}^{r} \right) SIN \left(\frac{1}{2} (\phi_{R} + \phi_{Q_{2}}) \right) \right\} \left(\theta_{R} - \theta_{Q_{2}} \right) - \frac{1}{2} \left(V_{R} + V_{Q_{2}}^{r} \right) SIN \left(\frac{1}{2} (\phi_{R} + \phi_{Q_{2}}) \right) \right\} \left(\theta_{R} - \theta_{Q_{2}} \right) - \frac{1}{2} \left(V_{R} + V_{Q_{2}}^{r} \right) SIN \left(\frac{1}{2} (\phi_{R} + \phi_{Q_{2}}) \right) \right\} \left(\theta_{R} - \theta_{Q_{2}} \right)$$

$$(199)$$

Rewriting the six finite-difference equations, we see that

$$U_{R} - \frac{1}{2} (Ch_{1R} + Ch_{1P_{i}}) \in_{R} - \frac{1}{2} (V_{R} + V_{P_{i}}) \phi_{R} - \frac{1}{2} (W_{R} + W_{P_{i}}) [\cos(\frac{1}{2}(\phi_{R} + \phi_{P_{i}}))] \Theta_{R}$$

$$+ \left\{ -U_{P_{i}} + \frac{1}{2} (Ch_{iR} + Ch_{iP_{i}}) \in_{P_{i}} + \frac{1}{2} (V_{R} + V_{P_{i}}) \phi_{P_{i}} + \frac{1}{2} (W_{R} + W_{P_{i}}) [\cos(\frac{1}{2}(\phi_{R} + \phi_{P_{i}}))] \Theta_{P_{i}} - \frac{1}{2} \cdot \frac{1}{2} (H_{R} + H_{P_{i}}) \cdot k \right\} = 0 , \qquad (200)$$

$$\begin{aligned} U_{R} + \frac{1}{2} (Ch_{1R} + Ch_{1Q_{1}}) &\in_{R} - \frac{1}{2} (V_{R} + V_{Q_{1}}) \phi_{R} - \frac{1}{2} (W_{R} + W_{Q_{1}}) \Big[COS(\frac{1}{2}(\phi_{R} + \phi_{Q_{1}})) \Big] \Theta_{R} \\ + \Big\{ -U_{Q_{1}} - \frac{1}{2} (Ch_{1R} + Ch_{1Q_{1}}) &\in_{Q_{1}} + \frac{1}{2} (V_{R} + V_{Q_{1}}) \phi_{Q_{1}} + \frac{1}{2} (W_{R} + W_{Q_{1}}) \Big[COS(\frac{1}{2}(\phi_{R} + \phi_{Q_{1}})) \Big] \Theta_{Q_{1}} \\ - \frac{1}{2} \cdot \frac{1}{2} (H_{R} + H_{Q_{1}}) \cdot K \Big\} = 0 \end{aligned}$$
(201)

$$V_{R} - \frac{1}{2} \left((1 + \epsilon_{R}) Ch_{2R} + (1 + \epsilon_{P_{2}}) Ch_{2P_{2}} \right) \phi_{R} + \frac{1}{2} (W_{R} + W_{P_{2}}) \left[SIN(\frac{1}{2}(\phi_{R} + \phi_{P_{2}})) \right] \Theta_{R} + \left\{ -V_{P_{2}} + \frac{1}{2} ((1 + \epsilon_{R}) Ch_{2R} + (1 + \epsilon_{P_{2}}) Ch_{2P_{2}}) \phi_{P_{2}} - \frac{1}{2} (W_{R} + W_{P_{2}}) \left[SIN(\frac{1}{2}(\phi_{R} + \phi_{P_{2}})) \right] \Theta_{P_{2}} - \frac{1}{2} (W_{R} + W_{P_{2}}) \left[SIN(\frac{1}{2}(\phi_{R} + \phi_{P_{2}})) \right] \Theta_{P_{2}} - \frac{1}{2} (G_{R} + G_{P_{2}}) \cdot K \right\} = O_{q_{1}}^{(202)}$$

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$$V_{R} + \frac{1}{2} \left((1+\epsilon_{R}) Ch_{2R} + (1+\epsilon_{q_{2}}) Ch_{2Q_{2}} \right) \phi_{R} + \frac{1}{2} (W_{R} + \tilde{W}_{q_{2}}) \left[SIN(\frac{1}{2}(\phi_{R} + \phi_{q_{2}})) \right] \Theta_{R} + \left\{ -V_{Q_{2}} - \frac{1}{2} ((1+\epsilon_{R}) Ch_{2R} + (1+\epsilon_{q_{2}}) Ch_{2R_{R}}) \phi_{Q_{2}} - \frac{1}{2} (W_{R} + W_{q_{2}}) \left[SIN(\frac{1}{2}(\phi_{R} + \phi_{q_{2}})) \right] \Theta_{\tilde{x}_{2}} - \frac{1}{2} (W_{R} + W_{q_{2}}) \left[SIN(\frac{1}{2}(\phi_{R} + \phi_{q_{2}})) \right] \Theta_{\tilde{x}_{2}} - \frac{1}{2} \left[(G_{R} + G_{Q_{2}}) \cdot K \right] = 0,$$
(203)

$$\begin{split} W_{R} - \left\{ \frac{1}{2} \left((1+\epsilon_{R}) C_{1+2R}^{\prime} + (1+\epsilon_{P_{R}}) Ch_{2P_{R}} \right) COS \left(\frac{1}{2} (\phi_{R}+\phi_{P_{R}}) \right) \\ - \frac{1}{2} (U_{R}+U_{P_{R}}) COS \left(\frac{1}{2} (\phi_{R}+\phi_{P_{R}}) \right) + \frac{1}{2} (V_{R}+V_{P_{R}}) SIN \left(\frac{1}{2} (\phi_{R}+\phi_{P_{R}}) \right) \right\} \Theta_{R} \\ + \left[\left\{ \frac{SAME}{COEFFICIENT} \right\} \Theta_{P_{2}} - \frac{1}{2} \cdot \frac{1}{2} \left(I_{R}+I_{P_{R}} \right) \cdot k \right] = O_{1} (204) \end{split}$$

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$$W_{R}^{i} + \left\{ \frac{1}{2} \left((1+\epsilon_{R}) Ch_{2R}^{i} + (1+\epsilon_{q_{2}}) Ch_{2q_{2}}^{i} \right) COS\left(\frac{1}{2} (\phi_{R} + \phi_{q_{2}}) \right) + \frac{1}{2} (U_{R}^{i} + U_{q_{2}}^{i}) COS\left(\frac{1}{2} (\phi_{R}^{i} + \phi_{q_{2}}) \right) - \frac{1}{2} (V_{R}^{i} + V_{q_{2}}^{i}) SIN\left(\frac{1}{2} (\phi_{R}^{i} + \phi_{q_{2}}) \right) \right\} \Theta_{R}^{i} + \left[- \left\{ SAME \atop COEFFICIENT \right\} \Theta_{q_{2}}^{i} - \frac{1}{2} \cdot \frac{1}{2} (I_{R}^{i} + I_{q_{2}}^{i}) \cdot K \right] = O_{\bullet}^{(205)}$$

In determinant form, they become

$$\begin{aligned} U_{R} + O + O + A_{I4} & \epsilon_{R} + A_{I5} & \phi_{R} + A_{I6} & \Theta_{R} + A_{I7} &= O \\ U_{R} + O + O + A_{24} & \epsilon_{R} + A_{25} & \phi_{R} + A_{26} & \Theta_{R} + A_{27} &= O \\ O + V_{R} + O + O + A_{35} & \phi_{R} + A_{36} & \Theta_{R} + A_{38} &= O \\ O + V_{R} + O + O + A_{45} & \phi_{R} + A_{46} & \Theta_{R} + A_{47} &= O \\ O + O + W_{R} + O + O + A_{56} & \Theta_{R} + A_{57} &= O \\ O + O + W_{R} + O + O + A_{66} & \Theta_{R} + A_{67} &= O \end{aligned}$$

where the A_{ij} are the coefficients in the preceding set of equations, (200) through (205). The six simultaneous equations are solved to give

$$W_{R} + A_{56} \Theta_{R} = -A_{57}$$

$$-(W_{R} + A_{66} \Theta_{R} = -A_{67})$$

$$(A_{56} - A_{66}) \Theta_{R} = A_{67} - A_{57}$$

$$\therefore \Theta_{R} = \frac{(A_{67} - A_{57})}{(A_{56} - A_{66})}$$

$$W_{R} + A_{56} \Theta_{R} = -A_{57}$$

$$\therefore W_{R} = -A_{57} - A_{55} \Theta_{R}$$

$$V_{R} + A_{35} \phi_{R} = -A_{36} \theta_{R} - A_{37} \qquad \therefore V_{R} = -A_{35} \phi_{R} - A_{36} \theta_{R} - A_{37}$$

$$U_{R} + A_{14} \in_{R} = -A_{15} \phi_{R} - A_{16} \Theta_{R} - A_{17}$$

$$- (U_{R} + A_{24} \in_{R} = -A_{25} \phi_{R} - A_{26} \Theta_{R} - A_{27})$$

$$(A_{14} - A_{24}) \in_{R} = (A_{25} - A_{15}) \phi_{R} + (A_{26} - A_{16}) \Theta_{R} + A_{27} - A_{17}$$

$$\therefore \in_{R} = \frac{(A_{25} - A_{15})\phi_{R} + (A_{26} - A_{16})\theta_{R} + A_{27} - A_{17}}{(A_{14} - A_{24})}$$

and

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$$U_{R} = -A_{14} \epsilon_{R} - A_{15} \phi_{R} - A_{16} \theta_{R} - A_{17} .$$

The numerical procedure within the time-space plane grid is as follows:

Step 1

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Compute the coefficients of the six simultaneous equations using the values previously computed along the two preceding iso-time lines.

Parameters between grid points are estimated by linear interpolation along an iso-time line; for example,

$$U_{P_1} = U_A - (h_{P_1} / h)(U_A - U_B),$$

where

$$h_{Pi} = S_A - S_{Pi}$$

Accelerations are needed to compute the hydrodynamic mass terms in the loading functions. Accelerations are estimated by using an Euler numerical approximation along an iso-space line, for example,

$$\dot{U}_{\rm R} = (U_{\rm R} - U_{\rm A})/k$$

In order to begin the iteration, assume that the values of the para-

meters at point R are equal to the values at point A.

Step 2

Solve the simultaneous equations for the six parameters at point R.

Step 3

Go back to step 1 using the new values at R in the coefficients and recompute values at R.

Step 4

Repeat this procedure three times. *

^{*}A more efficient method would be to specify an error and iterate until the computed parameter values converge within the error band. However, three iterations were found to give good convergence.

From the above scheme, it is obvious that solutions along two iso-time lines are needed to start the iteration. Use the initial conditions along the cable at t = 0 to fill in the parameter values at points C. E. and F. Assign these same values to points along the cable at t = k to obtain parameter values at points B, A, and D. Begin iteration at grid point (1, 2); S = h, t = 2k, and iterate down the cable. At the lower end of the cable, set S = hand t = 3k and repeat.

Velocities along the upper boundary are described by the motion of the cable end. However, at the beginning of each new iso-time line, strain and angles must be computed at S = 0 for the previous time increment. Hartree's method can not be used here because parameter values at times less than 0 are not available. A linear extrapolation along the previous iso-time line is used to obtain the required values; for example,

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The same problem exists along the lower boundary; only here the velocities are set to 0. Once again the values of strain and angles are determined by linear extrapolation along the provious iso-time line.

This method fails to converge if points P_1 or P_2 fall outside the space interval A-D. Since the locations of these points are determined by the characteristics, the minimum relative size of the space-to-time increment is equal to the value of the greatest characteristic. The tensile wave characteristic is always greater than the transverse wave characteristic; thus,

$$\left(\frac{h}{k}\right)_{min.} = Ch_1$$

The maximum space increment is determined by the size of the smallest wavelength in the cable. The transverse wave characteristic will determine the smallest wavelength. If the transverse wave characteristic is known to be 250 ft/sec and the cable is being excited by a sinusoid with a 2-sec period, the wavelength will be 500 ft in length. At least ten points are needed to describe a sinusoid; thus, the maximum size of the space increment is 50 ft. The time increment for a 3,000-ft/sec tensile wave characteristic is

$$k_{mnx.} = \frac{h}{Ch_1} = \frac{50 \text{ ft.}}{3000 \text{ ft./sec}} = 0.01667 \text{ sec}$$

Note that the characteristics must always be real, finite values. If the characteristics are zero or imaginary, the equations become ultrahyperbolic with multiple solutions for a given set of initial conditions. In this system, this is possible if the tensions are less than zero. Transverse wave characteristics are then imaginary and the problem is indeterminate. This study is further constrained by the requirement that the tension must be greater than zero at all times over the whole length of cable.

3.2.3 Cable Lor ing Functions

The loading functions H, G, and I (figure 16) in the cable equations are composed of the weight components in the double-primed coordinate system, the steady-state and dynamic drag components (viscous forces), and hydrodynamic inertia components (acceleration proportional forces).

The weight components are found by simply transforming the weight per unit length from the inertial coordinate system to the cable coordinates. The weight components are $W_c'' = A \cdot W_c$, or



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Figure 16. Cable Loading Functions

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$$W_{c}^{"} = \begin{bmatrix} w_{c} \cos \phi \cos \theta \\ -w_{c} \sin \phi \cos \theta \\ -w_{c} \sin \theta \end{bmatrix}$$
(206)

Drag forces are assumed to follow a velocity-squared drag law of the form:

$$D = \frac{1}{2} \mathcal{P} C_p A V^2. \tag{207}$$

On a unit length basis, they become

$$D = \frac{1}{2} \rho C_p d \nabla |\nabla|, \qquad (208)$$

where d is the cable diameter. In order to maintain a sign convention, equation (208) is rewritten as

$$D = \frac{1}{2} \rho C_p d V |V|. \qquad (209)$$

Casarella and Parsons⁷¹ have reviewed the state of the art for analysis of hydrodynamic forces on cable systems. Two approaches are described: the use of loading functions* to compute the normal and tangential drag force components, and the direct computation of normal and tangential drag using normal and tangential drag coefficients. The latter method, used by Wilson^{11, 12} for mooring problems, is employed in this study because it lends itself to threedimensional cable problems and only requires a simple transformation of the relative velocity components to cable coordinates. The first method requires

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^{*}Loading functions are defined as the ratio of the hydrodynamic force component of interest to the drag force when the cable is oriented normal to the flow.

multiple transformations to define the normal and tangential forces and to resolve them into three force components.

If the current velocity components are given by

$$G_{s} = \begin{bmatrix} 0 \\ V_{s} \\ W_{s} \end{bmatrix}$$

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(no vertical currents) and the velocity components of the cable element are



then the velocity components of the water relative to the cable are

$$\begin{bmatrix} U_R \\ V_R \\ W_R \end{bmatrix} = \begin{bmatrix} 0 - \dot{X} \\ V_s - \dot{Y} \\ W_s - \dot{z} \end{bmatrix}$$

Transforming to cable coordinates gives

$$\begin{bmatrix} U_{R}^{"} \\ V_{R}^{"} \\ W_{R}^{"} \end{bmatrix} = \begin{bmatrix} -\dot{x}\cos\phi\cos\theta + (V_{s}-\dot{y})\sin\phi + (W_{s}-\dot{z})\cos\phi\sin\theta \\ \dot{x}\sin\phi\cos\theta + (V_{s}-\dot{y})\cos\phi - (W_{s}-\dot{z})\sin\phi\sin\theta \\ \dot{x}\sin\phi + (W_{s}-\dot{z})\cos\theta \end{bmatrix}$$

The drag force components are now written:

$$D_{X''} = \frac{1}{2} \rho G_{DT} d U_{R}'' | U_{R}'' | , \qquad (210A)$$

$$D_{y''} = \frac{1}{2} \rho G_{pN} d V_{R}'' |V_{R}''|, \qquad (210B)$$

and

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$$D_{z''} = \frac{1}{2} \rho C_{DN} o W_{R}'' |W_{R}''| .$$
(210C)

Hoerner, ⁴⁸ Wilson, ¹¹ and others show plots of normal and tangential drag coefficients versus Reynolds number for stranded cables. Wilson's data were used in the G.S.A. program "CURFIT," and the following expressions for normal and tangential drag coefficients were developed:

$$C_{DN} = 1.32027 + 10.6962 / Re \qquad 0 < Re < 100 ,$$

= 1.4
$$100 < Re < 5 \times 10^{5} , (211A)$$
$$C_{DT} = 0.60546 \cdot (Re^{-0.475?}) \qquad 0 < Re < 5 \times 10^{5} , (211B)$$

where $R_e = \frac{Vd}{\nu}$; ν is the kinematic viscosity. These expressions are valid only in the subcritical Reynolds number region (Re 5 X 10⁵). However, since this Reynolds number would apply to a 2-in.-diameter cable moving at 35 knots, it is felt that the range of Reynolds numbers covered is adequate.

Hydrodynamic inertia forces are defined normal and tangential to the cable. Transforming the accelerations to cable coordinates and assuming steady ocean currents, we find that the accelerations become

$$\begin{bmatrix} \ddot{x}'' \\ \ddot{y}'' \\ \vdots \\ \ddot{z}'' \end{bmatrix} = \begin{bmatrix} \ddot{x} \cos\phi \cos\theta + \ddot{y} \sin\phi + \ddot{z} \cos\phi \sin\theta \\ -\ddot{x} \sin\phi \cos\theta + \ddot{y} \cos\phi - \ddot{z} \sin\phi \sin\theta \\ -\ddot{x} \sin\theta + \ddot{z} \cos\theta \end{bmatrix}$$

The hydrodynamic inertia force vector is

$$\begin{bmatrix} F_{h_{mx}} \\ F_{h_{my}} \\ F_{h_{mz}} \end{bmatrix} = \begin{bmatrix} m_{h_{xx''}} & 0 & 0 \\ 0 & m_{h_{yy''}} & 0 \\ 0 & 0 & m_{h_{zz''}} \end{bmatrix} \begin{bmatrix} \ddot{x}'' \\ \ddot{y}'' \\ \ddot{z}'' \\ \ddot{z}'' \end{bmatrix}$$

The hydrodynamic mass dyadic is a diagonal matrix due to the axisymmetry of the cable. Also, for a cylindrical object of infinite length, the tangential hydrodynamic mass ($M_{h_{XX}}$) is zero. For a smooth, constant diameter cable, the normal hydrodynamic masses ($M_{h_{YY}}$, $M_{h_{ZZ}}$) are equal.

Lamb, 45 Basset, 72 and others have used potential flow theory to compute the normal hydrodynamic mass of a circular cylinder:

$$M_{h_N} = \pi \rho d^2 / 4$$
 (212)

Miller⁷³ and Miller and Hagist⁷⁴ have investigated the frequency dependence of hydrodynamic mass for various bodies. Their data show a linear decrease in hydrodynamic mass with increasing frequency in the Stokes number region $0 < \text{St.} < 3 \times 10^5$. By using Miller's data for a 5:1 cylinder,⁷⁴ we can see that the slope is -1.62×10^{-6} . Equation (212) is modified to include the frequency dependence as follows:

$$M_{h_N} = \left(1.0 - (1.62 \times 10^{-6}) \cdot \frac{\omega d^2}{v}\right) \pi \rho d^2 / 4 ; 0 < \frac{\omega d^2}{v} < \frac{3 \times 10^5}{(213)}.$$

The components of the hydrodynamic inertia force are

 $F_{h_{m\chi}} = 0 , \qquad (214A)$
$$F_{h_{my}} = \mathcal{M}_{h_N} \cdot \ddot{\mathcal{Y}}'' , \qquad (214B)$$

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$$F_{h_{M2}} = \mathcal{M}_{h_N} \cdot \ddot{z}'' \,. \tag{214C}$$

The loading functions are summarized by using equations (206), (210), and (214):

$$H = w_c \cos \phi \cos \theta + (0.60546 \cdot (Re_{u''}^{-0.4758})) \stackrel{f}{=} d U_R'' | U_R'' |_{(215A)}$$

$$G = -w_{c} \sin \phi \cos \theta + G_{DNV''} \stackrel{P}{\geq} d V_{R}'' |V_{R}''| + (1.0 - (1.62 \times 10^{-6}) \cdot \frac{\omega d^{2}}{2}) \pi \rho \frac{d^{2}}{4} \cdot \ddot{\gamma}'' , \qquad (215B)$$

and

$$I = -w_{c} SIN\theta + C_{DN_{w''}} \stackrel{P}{\geq} dW_{R}'' |W_{R}''| + (1.0 - (1.62 \times 10^{-6}), \frac{\omega d^{2}}{2}) \pi \rho \frac{d^{2}}{4} \cdot \ddot{z}'' , \qquad (215C)$$

where

$$Re_{v''} = \frac{V''d}{v}$$
$$Re_{v''} = \frac{V''d}{v}$$
$$Re_{w''} = \frac{W''d}{v}$$

$$G_{DN_{v''}} = 1.32027 + 10.6962 / Re_{v''} \qquad 0 < Re_{v''} < 100$$

= 1.4
$$100 < Re_{v''} < 5 \times 10^{5}$$

$$G_{DN_{w''}} = 1.32027 + 10.6962 / Re_{w''} \qquad 0 < R_{2w''} < 100$$

= 1.4
$$100 < Re_{w''} < 5 \times 10^{5}$$

3.2.4 Lumped-Mass Model

The finite-difference analysis for cable dynamics can become very expensive in computer time and can require large amounts of computer "storage" because of the large arrays. A lumped-mass analysis can offer significant savings in computational time at the expense of simulation accuracy. In general, the lumped-mass analysis will truncate the high-frequency response of the system. However, for many engineering applications, the high-frequency, low-amplitude response is not of interest. and the cable can be represented as a small number of lumped masses.

Assume that a uniform cable of length L can be broken up into N segments of equal length (figure 17). The length of each segment is

$$\Delta L = L/N$$
.

Assume that the properties (weight, mass, hydrodynamic forces, etc.) of the cable can be concentrated at points 2, 3, ..., N, which are located ΔL ,

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 $2\Delta L, \ldots, (N-1)\Delta L$ from the upper end of the cable. All the forces acting on the cable span from $(N-1)\Delta L \pm \frac{\Delta L}{2}$ are concentrated at the nth mass point. Cristecu's cable equations, (135A), (135B), and (135C), are written for each mass point:

$$\mathcal{M}(S_0) \Delta L \frac{d^2 X_n}{dt^2} = X_n \Delta L + \left(\overline{T_n} \cdot \hat{i} - \overline{T_{n-1}} \cdot \hat{i}\right), \qquad (216A)$$

$$\mathcal{M}(S_{o}) \Delta L \frac{d^{2} Y_{n}}{dt^{2}} = Y_{n} \Delta L + \left(\overline{T}_{n} \cdot \hat{j} - \overline{T}_{n-1} \cdot \hat{j}\right), \qquad (216B)$$

and

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$$\mu(S_{o}) \Delta L \frac{d^{2} Z_{n}}{dt^{2}} = Z_{n} \Delta L + \left(\overline{T}_{n} \cdot \hat{k} - \overline{T}_{n} \cdot \hat{k}\right). \qquad (216C)$$

This is equivalent to modeling the cable with a system of spring-mass elements as shown in figure 18. In order to compute the forces acting on each mass element, the cable angles (\oint and θ) for each cable element must be defined. From the geometry between the nth and (N + 1)th mass element, we see that

$$\Theta_{n} = SIN^{-1} \left(\frac{(\mathbb{Z}_{n+1} - \mathbb{Z}_{n})}{\sqrt{(X_{n+1} - X_{n})^{2} + (\mathbb{Z}_{n+1} - \mathbb{Z}_{n})^{2}}} \right)$$
(217)

and

$$\phi_{n} = SIN^{-1} \left(\frac{(\gamma_{n+1} - \gamma_{n})}{\sqrt{(\chi_{n+1} - \chi_{n})^{2} + (\gamma_{n+1} - \gamma_{n})^{2} + (\Xi_{n+1} - \Xi_{n})^{2}}} \right)$$
(218)

Tensions between the lumped masses are computed by using the elastic properties of the cable and the deformation of the cable. If the effective cable



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modulus is E_c and the deformation of the cable of length ΔL is δ , the spring constant along the cable is

$$K_{x''} = \frac{F}{S} = \frac{\pi \frac{d^2}{4}\sigma}{S} = \pi \frac{d^2}{4} \frac{E_c}{\Delta L}, \qquad (219)$$

The stretched length between the nth and (N + 1)th mass element is

$$\sqrt{(X_{n+1} - X_n)^2 + (Y_{n+1} - Y_n)^2 + (Z_{n+1} - Z_n)^2}.$$

If the difference between the stretched length of the cable and the unstretched length (Δ L) is less than zero, the tension is zero since the cable can not support compression. (This is analogous to the ultrahyperbolic equations that occur in the finite-difference analysis if tensions go to zero.) Otherwise, the tension in the nth cable segment is defined as

$$T_{n_{\chi''}} = K_{\chi^{n}} \left(\sqrt{(X_{n+1} - X_{n})^{2} + (Y_{n+1} - Y_{n})^{2} + (\Xi_{n+1} - \Xi_{n})^{2}} - \Delta L \right).$$
(220)

Transforming to inertial coordinates, the tension becomes

$$T_{n} = A^{-1} \cdot \begin{bmatrix} T_{n_{X''}} \\ O \\ O \end{bmatrix} = \begin{bmatrix} T_{n_{X''}} \cos \phi_{n} \cos \phi_{n} \\ T_{n_{X''}} \sin \phi_{n} \\ T_{n_{X''}} \cos \phi_{n} \sin \phi_{n} \end{bmatrix}$$
(221)

These tension components are used in equations (216) to compute the tension difference across the mass element.

As before, the forces $X_n \triangle L$, $Y_n \triangle L$ and $\Xi_n \triangle L$ acting on each mass element consist of weight, viscous drag, and hydrodynamic inertia

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forces. The weight force vector per unit length is

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$$W_c = \begin{bmatrix} w_c \\ 0 \\ 0 \end{bmatrix}$$

where W_c is the weight per unit length in water of the cable. Again, if the current velocity components at the nth element are

$$C_{s} = \begin{bmatrix} 0 \\ V_{s_{n}} \\ W_{s_{n}} \end{bmatrix}$$

and the velocity components of the nth element are



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then the relative velocity components are

$$\begin{bmatrix} U_{R_n} \\ V_{R_n} \\ W_{R_n} \end{bmatrix} = \begin{bmatrix} 0 - \dot{x_n} \\ V_{S_{ii}} - \dot{y_n} \\ W_{S_n} - \dot{z}_n \end{bmatrix}, \qquad (222)$$

If the mean cable angles at the nth mass element are computed, we find

$$\overline{\phi}_n = \frac{1}{2} \left(\phi_n + \phi_{n-1} \right) \tag{223A}$$

and

$$\overline{\Theta}_{n} = \frac{1}{2} \left(\Theta_{n} + \Theta_{n-1} \right)$$
(223B)

The relative velocity components are transformed to cable coordinates as follows:

$$U_{R_{n}}^{"} = -\dot{X}_{n} \cos \overline{\phi}_{n} \cos \overline{\theta}_{n} + (V_{s_{n}} - \dot{Y}_{n}) \sin \overline{\phi}_{n} + (W_{s_{n}} - \dot{z}_{n}) \cos \overline{\phi}_{n} \sin \overline{\theta}_{n},$$

$$V_{R_{n}}^{"} = \dot{X}_{n} \sin \overline{\phi}_{n} \cos \overline{\theta}_{n} + (V_{s_{n}} - \dot{Y}_{n}) \cos \overline{\phi}_{n} - (W_{s_{n}} - \dot{z}_{n}) \sin \overline{\phi}_{n} \sin \overline{\theta}_{n},$$

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$$W_{R_n}'' = \dot{X}_n \sin \overline{\theta}_n + (W_{s_n} - \dot{z}_n) \cos \overline{\theta}_n$$

The drag forces per unit length are

$$D_{x_{n}}'' = \frac{1}{2} \rho C_{pT} d U_{R_{n}}'' |U_{R_{n}}''|,$$

$$D_{y_{n}}'' = \frac{1}{2} \rho C_{pN} d V_{R_{n}}'' |V_{R_{n}}''|,$$

and

$$D_{z_n}^{"} = \frac{1}{2} \rho C_{DN} d W_{R_n}^{"} |W_{R_n}^{"}|,$$

where the drag coefficients are computed from equations (211A) and (211B).

The accelerations of the nth mass element in cable coordinates are

$$\begin{split} \ddot{X}_{n}^{"} &= \ddot{X}_{n} \cos \overline{\phi}_{n} \cos \overline{\theta}_{n} + \ddot{Y}_{n} \sin \overline{\phi}_{n} + \ddot{\Xi}_{n} \cos \overline{\phi}_{n} \sin \overline{\theta}_{n} , \\ \ddot{Y}_{n}^{"} &= -\ddot{X}_{n} \sin \overline{\phi}_{n} \cos \overline{\theta}_{n} + \ddot{Y}_{n} \cos \overline{\phi}_{n} - \ddot{\Xi}_{n} \sin \overline{\phi}_{n} \sin \overline{\theta}_{n} , \end{split}$$

and

$$\ddot{z}_n'' = -\ddot{x}_n \sin \theta_n + \ddot{z}_n \cos \theta_n$$
.

The hydrodynamic inertia force components per unit length are

$$F_{hm_{X_n}}^{"} = 0 , \qquad (224A)$$

$$F_{hm_{y_n}}^{"} = -M_{h_n} \cdot \dot{y}_n^{"}, \qquad (224B)$$

and

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$$F_{hm_{z_n}}^{"} = -M_{h_n} \cdot \tilde{Z}_n^{"}, \qquad (224C)$$

where \mathcal{M}_{h_n} is computed from equation (213). The drag and hydrodynamic inertia forces per unit length are summed and transformed back to inertial coordinates and added to the weight force; thus,

$$\begin{bmatrix} X_{n} \\ Y_{n} \\ Z_{in} \end{bmatrix} = \begin{bmatrix} w_{c} \\ 0 \\ 0 \end{bmatrix} + A^{-i} \cdot \begin{bmatrix} D_{x_{n}}^{"} + 0 \\ D_{y_{n}}^{"} + F_{hm_{y_{n}}}^{"} \\ D_{z_{n}}^{"} + F_{hm_{z_{n}}}^{"} \end{bmatrix}$$
(225)

All terms have been defined in the equations of motion for the cable elements. The three equations, (216A), (216B), and (216C), must be integrated simultaneously for each mass element. Thus, if the cable is broken up into ten segments, there will be nine mass elements, each having three degrees of freedom. Therefore, there will be $9 \times 3 \times 2 = 54$ simultaneous first-order equations. Note that the upper half of the first cable length and the lower half of the last cable length are not included in these equations of motion. These cable segments are assumed to be moving with the upper and lower boundaries and their properties should be lumped in with the properties of the buoy and anchor. The computer program developed for the lumped-mass simulation of cable dynamics will be discussed in a later section.

3.3 Steady-State Buoy System Configurations

As an introduction to the coupling of the buoy and cable equations of motion and the resulting computer programs, the steady-state buoy system configuration (zeroth-order case of buoy system dynamics) will be investigated. The specification of the proper mooring line length for a moored buoy system is critical in the design of the system in order to avoid tow-under of the buoy, minimize the "watch circle" of the buoy, reduce steady-state tensions in the moor, etc. This analysis offers a method to select the proper mooring line length for a given buoy, water depth, and current.

If the time dependent terms in the cable equations (equations (161) through (167)) are allowed to go to zero, we find that

$$\frac{\partial T}{\partial S_{0}} + H = \gamma , \qquad (226)$$

$$T \frac{\partial \Phi}{\partial S_{o}} + G = 0 , \qquad (227)$$

$$T\cos\phi \frac{\partial\Theta}{\partial S_{o}} + I = 0, \qquad (228)$$

and

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$$\epsilon = \frac{1}{K} T . \tag{229}$$

Letting cable velocities go to zero in the loading functions, we find that

$$H = w_{c} \cos \phi \cos \phi + (0.60546 \cdot (R_{ev_{R}}^{-0.475\epsilon})) \frac{\rho}{2} d U_{R}^{"} |U_{R}^{"}| ,$$

$$G = -w_{c} \sin \phi \cos \phi + G_{p_{N'V''}} \frac{\rho}{2} d V_{R}^{"} |V_{R}^{"}| ,$$

and

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$$I = -w_c \sin \Theta + C_{DN_{W''}} \frac{\rho}{2} dW_R'' |W_R''|,$$

where

$$U_{R}'' = V_{s} \sin \phi + W_{s} \cos \phi \sin \theta$$
$$V_{R}'' = V_{s} \cos \phi - W_{s} \sin \phi \sin \theta$$
$$W_{R}'' = W_{s} \cos \theta$$

$$C_{DN_{V''}} = 1.32027 + 10.6962 / Re_{V''} \qquad 0 < Re_{V''} < 100$$

= 1.4
$$100 < Re_{V''} < 5 \times 10^{5}$$

$$G_{DN_{W''}} = 1.32027 + 10.6962 / Re_{W''} \qquad 0 < Re_{W''} < 100$$

= 1.4
$$100 < Re_{W''} < 5 \times 10^{5}$$

$$R_{e_{v''}} = \frac{V''d}{\nu}$$

$$R_{e_{v''}} = \frac{V''d}{\nu}$$

$$R_{e_{w''}} = \frac{W''d}{\nu}$$

Expanding equations (226), (227), and (228) to include the loading functions and taking total derivatives, we find that the steady-state cable equations become

$$\frac{dT}{dS_{o}} = -w_{c}\cos\phi\cos\phi - (0.60546(Re_{v_{R}}^{-0.4758}))\frac{\rho}{2}dU_{R}^{"}|U_{R}^{"}|,$$
⁽²³⁰⁾

$$\frac{d\Phi}{dS_o} = \frac{1}{T} \left(w_c \sin\phi \cos\phi - G_{DNV''} \stackrel{\rho}{\geq} dV_{R}'' |V_{R}''| \right), \qquad (231)$$

$$\frac{d\theta}{dS_{o}} = \frac{1}{T\cos\phi} \left(W_{c} \sin\theta - C_{DN_{W''}} \frac{\rho}{2} dW_{R}^{"} |W_{R}| \right), \qquad (232)$$

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$$\frac{dS}{dS} = 1 + \epsilon = 1 + \frac{1}{K}T \qquad (233)$$

Before integrating these equations, the upper boundary conditions (at the buoy) must be determined. The tension at the buoy is

$$T_{\rm B} = \sqrt{\left(D_{\rm isp} - W_{\rm B}\right)^2 + D_{\rm rag}^2}, \qquad (234)$$

where

Disp is the buoy displacement

Drag is the buoy drag force due to surface currents

 W_{B} is the buoy weight.

For convenience, let the z axis of the inertial coordinate system be aligned with the surface current. Then, the initial buoy moor angle is

$$\Theta_{o} = TAN^{-1} \left(\frac{Drag}{(Disp - W_{0})} \right), \qquad (235)$$

and the initial moor transverse angle is

$$\phi_{o} = 0 \qquad (236)$$

Unfortunately, the displacement and drag of the buoy are functions of the cable tension at the buoy. Thus, a trial and error solution must be used. For a given current profile, $V_s = f_1(x)$; $W_s = f_2(x)$, the depth of water is the controlling parameter. When the vertical projection of the cable is equal to the depth of water, the correct solution has been obtained. The differential equations defining the cable shape (equations (230) to (233)) are controlled by the unstressed cable length S_0 . Each iteration must take into account the stretch in the segment before the x component of segment is computed. Thus, if the integration step size is dS_0 and the tension and angles for that segment are T, θ , and ϕ , the stretched length is

$$dS = (1+\epsilon) \, dS_{o} \, . \tag{237}$$

From the cable properties, we see that

$$\varepsilon = \frac{\sigma}{E_c} = \frac{4T}{\pi d^2 E_c} .$$

Rewriting equation (237), we see that the stretched length increment is

$$dS = \left(1 + \frac{4T}{\pi d^2 E_c}\right) dS_o \qquad (238)$$

The components of the cable incremental length dS are

$$dx = dS \cos\phi \cos\theta$$
.

$$dy = dS \sin \phi$$
,

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$$dz = dS \cos\phi \sin\theta$$

Since both the displacement and drag of the buoy are functions of the draft of the buoy (equations (52) and (95) developed for an oblate spheroid - similar equations can be developed for buoys of different shape), the draft H is incremented upward from its "free-floating" value, i.e., as if the buoy were floating on a calm surface with no mooring. The configuration of the cable is then computed using a fourth-order, Runge-Kutta numerical integration algorithm for the given current structure. which may vary in magnitude and direction as a function of depth. The components of the incremental length dS are computed and summed. The x component of the end of the cable is tested logically to see if it falls within a specified error band about the water depth. If the computed vertical projection is less than the water depth, the buoy draft is incremented upward and the process is repeated. If the computed vertical projection falls within the error band, the solution can be accepted or the width of the error band can be reduced and the process repeated until the solution achieves the desired accuracy. This process is shown schematically in figure 19.

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This method has been programmed in FORTRAN IV and is shown in appendix B. Subroutines are included for the displacement and drag of oblate spheroids. spherical buoys, cylindrical buoys (cans or spars), torroidal buoys ("donut" buoys), and discus buoys ("monster" buoys). The user must input the buoy characteristics (weight, dimensions, etc.) the cable characteristics (diameter. length. cable modulus, and weight per foot in water - negative if buoyant), and the environmental characteristics (water depth, current structure, etc.). The output includes unstretched length, stretched length, tensions,



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Figure 19. Buoy System Configuration Computation Procedures

the cable angles, and the x, y, z coordinates of each integration step. The step size of the buoy draft is incremented upward as some percentage of the total buoy draft (as if it were fully submerged). Downward buoy draft increments are taken as half the prior upward increments to avoid a lock-step situation where the solution swings between two values and never converges. Subsequent draft increments are halved to avoid the same situation. If the initial error band specified is too narrow and the solution overshoots the error band, the solution may converge too slowly. A limiter is built in to stop the program after 15 configurations are computed. The user should then open up the initial error band. If the buoy draft is increased to its maximum value, the program prints out a statement that the buoy sinks and the computation stops. This indicates that the buoy tows under either because it has insufficient excess buoyancy or the system drag is too high.

If the mooring line is made up of more than one type of cable, rope, or chain, logical "IF" statements are used to change the cable properties at the proper cable lengths. The writer has found that at the transition of cable to chain, the integration step size must be reduced to ensure numerical stability.

Also, if the mooring line has objects (instruments, buoys, weights, etc.) attached to it, the change in tension and angles across the discontinuity must be computed from the free body of the object (figure 20). If the cable parameters just above the discontinuity are T_H , ϕ_H and θ_H , the tension components at that point are

$$T_{H_{\chi}} = T_{H} \cos \phi_{H} \cos \theta_{H} , \qquad (239A)$$



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Figure 20. Free Body of an Object on the Mooring Line

$$T_{Hy} = T_{H} SIN \phi_{H} , \qquad (239B)$$

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$$T_{H_{z}} = T_{H} \cos \phi_{H} \sin \Theta_{H} \qquad (239C)$$

By summing the forces on the free body, we can determine the tension components below the discontinuity. The tension components are

$$T_{L_x} = T_{H_x} - W_o ,$$

$$T_{L_y} = T_{H_y} + D_{oy} ,$$

and

$$T_{L_{\mathbf{Z}}} = T_{H_{\mathbf{Z}}} + D_{o_{\mathbf{Z}}},$$

where

 W_o is the in-water weight of the object D_{oy} is the drug of the object in the minus y direction D_{o_z} is the drag of the object in the minus z direction .

The cable tension just below the discontinuity is

$$T_{\rm L} = \sqrt{T_{\rm L_X}^2 + T_{\rm L_Y}^2 + T_{\rm L_Z}^2} , \qquad (240)$$

and the cable angles become

$$\Theta_{L} = TAN^{-1} (T_{L_{2}} / T_{L_{X}}), \qquad (241)$$

and

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$$\Phi_{\rm L} = T A N^{-1} \left(T_{\rm Ly}^{*} / \sqrt{T_{\rm Lx}^{*2} + T_{\rm Ly}^{*2}} \right)$$
(242)

The integration down the cable can resume using the new tension and angles. The process can be repeated for other objects along the mooring line.

Computer programs developed for buoy system statics and dynamics are shown and discussed in appendix B. The experimental data taken and the validation of the analytical models will now be discussed.

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IV. EXPERIMENTAL MEASUREMENTS

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AND COMPARISON WITH MODELS

4.1 <u>Steady-State Buoy System</u> Configurations

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The steady-state analysis of the buoy system configuration is important since it serves as the set of initial conditions for the dynamic analysis. Also, the steady-state analysis is the first step for the buoy system designer to ensure that his system will not tow-under or have other undesirable static characteristics. The analysis for both shallow and deep water oceanographic buoy systems are compared with data taken at sea to validate the steady-state analysis.

4.1.1 Torroid and Current Meter Array at Station BRAVO

On 22 August 1967, the writer installed a buoy-supported, current meter array at station BRAVO (41° 51.15'N, 71° 46.50'W) at the Block Island — Fishers Island (BIFI) Oceanographic and Acoustic Range in Block Island Sound. This array was recovered on 19 September 1967, and the current meter data were analyzed. The components of the array are shown in figure 21. The three Braincon type 316 current meters were suspended below the buoy at cable lengths of 15, 60, and 105 ft. The buoy and array were moored with a 70-lb Danforth anchor and a 100-lb lead weight to keep the line pull horizontal. The buoy used in this experiment is shown in figure 22.



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Figure 21. Current Meter Array at Station BRAVO

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Figure 22. Torroidal Buoy Used for the Three-Current-Meter Array

Twenty-eight samples of the current meter data were selected and are shown in table 1. Data from the bottom current meter were highly variable and erratic and were not regarded as reliable. The data shown are 10-min time averages due to the photographic method used to record data. The instruction manual for the type 316 current meter⁷⁵ gives the following ranges and accuracies:

	Range	Accuracy		
Current speed	0.5 – 5 knots		<u>+</u> 0.15 knot	
Current direction	0 - 360 deg		<u>+</u> 5 deg	
Current meter tilt	0 40 deg	,	<u>+</u> 1 deg	
Timing mechanism	5 months		<u>+</u> 10 sec/day.	

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The weight in sea water of the current meter is given in the instruction manual as 67 lb. The cylindrical body of the current meter is $8\frac{1}{2}$ in. in diameter by $38\frac{3}{4}$ in. long with a 36-in. vertical vane attached to al'gn it with the flow. Sunblad⁷⁶ gives a normal drag coefficient of 0.59 based on frontal area derived from tow tank data for the type 316 current meter.

The torroidal buoy has an 8-ft outer diameter and a 3-ft hole through the center. The buoy weighed 1200 lb in air. The mooring line was 5/8-in.-diameter polypropylene rope having 0.02 lb/ft buoyancy. The 3/8-in.-diameter wire rope at the anchor weighed 0.2 lb/ft. The cable modulus of elasticity for the polypropylene rope was taken as 1.67×10^5 lb/in.² (50° F) and for the steel cable as 12.0×10^6 lb/in.².

Williams⁷⁷ and Nalwalk <u>et al.</u>⁷⁸ have made current measurements at station BRAVO and have found a two-layer current structure. On an ebb tide

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Time	s	peed (knots)		D	irection (d	eg)	Obse	rved Ti	lt Angle
(hr)	15	60	105	15.	60 60	105	15	60	105
333	1 177	1 27	701	278	270	301	10	22	40
666	1 205	1 16	1.04	276	276	310	10	20	40
1 000	1,200	1 08	1 11	281	277	300	7	18	40
1 9 9 9	1 000	1.00	1.84	284	283	257	6	14	40
1.000	945	026	1.04	201	282	217	6	11	18
2 000	. 343	713	777	202	276	206	5	7	41
2.000	. 000	. 710	1 021	230	210	119	4	7	40
2.333	. (1)	. 210	1.031	200	210	115	-1	5	40
2.000	. 538	. 497	4.000	290	204	10	7		10
3.009	. 509	.341	3.92	316	302	171	3	С	10
3.333	.378	. 214	.110	335	316	160	2	4	17
3.666	. 420	. 159	.069	27	188	151	3	1	7
49.0	1.253	1.086	.764	290	280	215	15	25	18
50.0	1.169	1.088	1.386	298	275	202	15	25	40
51.0	.940	. 943	.697	291	272	246	10	20	40
52.0	.748	.516	. 523	308	289	110	6	11	40
53.0	.617	. 265	1.472	21	91	109	5	5	9
54.0	. 790	.578	.687	83	89	17	5	5	0
55.0	.908	.666	. 901	93	86	56	4	10	40
56.0	. 824	. 814	1.964	90	169	1	5	5	19
20.0	. 632	. 699	. 756	71	96	1	5	5	40
21.0	. 578	. 345	. 859	41	104	5	5	5	5
22.0	.630	. 363	1.394	283	242	312	6	15	40
23.0	. 910	. 845	4.539	<u> </u>	267	305	10	23	40
24.0	1.178	1.108	2.141	278	268	309	16	30	41
25.0	1.219	. 985	· . 545	283	260	306	14	30	40
26.0	1.158	1 050	1.283	288	257	306	15	26	18
27.0	. 893	** /*	1.894	298	257	201	9	20	18
28.0	.691	.42	. 832	347	254	111	6	6	40

TABLE 1. CURRENT METER DATA - STATION BRAVO

(current setting to the east) the bottom layer appears to set to the northeast. On a flood tide (current setting to the west) the bottom layer sets to the northwest. Williams and Nalwalk never observed bottom currents greater than 1.5 knots, which make the data from the bottom current meter questionable. Also, the high tilt angles of the bottom current meter preclude proper response of the Savonius rotor to the ambient currents. In the following study, which was made to compare predicted current meter tilt angles with observed tilt angles, the current is modeled as an upper layer having a thickness of 70 ft and a bottom layer with a thickness of 50 ft. The current speed and directions from the two upper current meters. The speed and direction of the lower layer is assumed to be equal to the speed and direction from the bottom current meter. Bottom currents greater than 1.5 knots were set equal to the value of the upper layer current.

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The computer program for steady-state buoy configurations shown in appendix B was modified to include the effects of the current meters and lead weight by solving the free body at each object (equations (239A) through (242)). The torroidal buoy subroutine was used and cable configurations were integrated with a fourth-order, Runge-Kutta algorithm using a 1-ft step size. Current meter tilt angles were computed by balancing moments on each current meter. The program was run on the GSA-360 time sharing computer and results for the first ten cases are shown in table 2 and in figure 23. Average tilt angle errors for the ten cases are as follows:

TABLE 2. COMPARISON OF OBSERVED CURRENT METER TILT ANGLES

Time (hr)	Obse	srved Ti (deg 4	ilt Angle 1 deg)	Compu	ted Tilt An _l	gle (deg)	Tilt An	gle Error ((deg)
	15	60	105	15	60	105	15	60	105
. 333	10	22	40	10.59	20.62	49.86	.59	-1.38	1
. 666	10	20	40	9.91	19.39	48.82	09	61	1
1.000	7	18	40	8.38	16.69	44.87	1.38	-1.31	ł
1.333	9	14	40	8.31	16.33	49.00	2.31	2.33	I
1.866	9	11	18	6.46	13.27	34.32	.46	2.27	16.32
2.000	ល	2	41	4.45	9.25	25.82	55	2.25	ļ
2.333	4	2	40	3.11	6.50	13.50	89	50	I
2.666	4	ວ	40	1.99	4.17	10.92	-2.01	83	I
3.000	en	က	10	1.34	2.81	8.49	-1.66	19	-1.51
3.333	5	4	17	. 65	1.37	5.38	-1.35	-2.63	-11.62
					A	verage error	0.22	0.20	1.06

WITH DATA COMPUTED BY INTEGRATION OF CABLE EQUATIONS

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Overall average error -0.49





CURRENT METER REFERENCE TIME (hr)

Figure 23. Current Meter Tilt Angles Computed by Integrating Down the Cable Compared With Observed Data

Upper current meter	+0.22 deg
Middle current meter	+ 0.2 deg
Lower current meter*	+1.06 deg
Overall average error	+ 0.49 deg.

With the exception of the lower current meter, these errors fall within the + 1 deg accuracy of the tilt indicator indicating good agreement of the computer model with observed data. The computer study indicated that the 100-lb weight was never picked up off the bottom by the strongest currents and that the weight and drag of the current meters control the buoy system configuration to a greater degree than the weight and drag of the polypropylene rope. With this in mind, a simple statics model of the buoy system was developed; it was assumed that the rope was not deflected between current meters and that half of the drag force acting on each rope span could be assumed to be concentrated at the end of the current meter to which it was attached. Since the tensions in the integrated cable configurations were observed to be very small, the buoy draft was computed for the buoy weight and the weight of the current meters only. Thus, the vertical force component is equal to the weight of the current meters. The simplified model is shown on figure 24. Lateral deflections computed by the integrated configurations were very small; thus, the simplified model was restricted to two dimensions. Drag forces on the rope spans and on the current meters were computed as if the currents were acting normal to the rope or current meter. The small buoyancy and the stretch of the rope were neglected.

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^{*}The errors of the lowest current meter were not computed for data where the observed tilt angle was 40 deg.



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Figure 24. A Simple Statics Model of the Current Meter Array

Buoy system configurations for the case at current meter reference time of 0.333 hr are shown in figure 25 and are quite similar. Computed current meter tilt angles were compared with the observed tilt angles. The angles were computed by using a uniform current equal to the mean value of the current speeds from the upper two current meters. The angles are shown in table 3 and on figure 26. The average errors for the first ten data sets are as follows:

Upper current meter	- 2.15 deg
Middle current meter	- 0.26 deg
Lower current meter	+ 7.64 deg
Overall average error	+ 1.74 deg.

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These errors indicate that the simple statics model is about three times less accurate than integration down the cable but may be adequate for engineering applications. The average errors for 28 data sets are as follows:

Upper current meter	- 3.77 deg
Middle current meter	- 2.69 deg
Lower current meter	16.66 deg
Overall average error	3.4 deg.

The preceding study indicates that the steady-state buoy system configuration model can predict current meter inclination angles to within $\frac{1}{2}$ deg on the average. No tension data were recorded; thus, the steady-state tension errors were not computed. The shallow water buoy system described above is most heavily influenced by the weight and drag of the current meters. In most deep water buoy systems, the weight and drag of the mooring cables are the predominant forces.

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Time	Speed ((knots)	Obse	rved ?	filt Angle	Comp	uted Tilt An	gle (deg)	Tilt An	gle Error	(deg)
(hr)	15	60	15	$\log + 1$	deg)	15	60	105	15	60	105
		00	10	. <u> </u>	105	10		105			103
. 33	1.177	1.27	10	22	40	6.90	20.77	62.53	-3.10	-1.12	-
.66	1.205	1.16	10	20	40	6.45	19.50	60.90	-3.55	50	-
1.00	1.079	1.08	7	18	40	5.38	16.45	56.27	-1.62	-1.55	-
1.33	1.099	1.07	6	14	40	5.43	16.59	56.51	57	2.59	-
1.66	. 945	. 928	6	11	18	4.06	12.53	48.42	-1.94	1.53	30.42
2.00	. 836	.713	5	7	40	2.78	8.64	37.62	-2.22	1.64	-
2.33	.717	.578	4	7	40	1.94	6.06	28.31	-2.06	94	-
2.66	• 538	.497	4	5	40	1.24	3.88	18.99	-2.76	-1.12	-
3.00	.509	.341	3	3	10	. 84	2.62	13.07	-2.16	38	3.07
3.33	.378	. 214	2	4	17	.41	1.27	6.42	-1.59	-2.73	10.58
3.66	. 420	. 159	3	1	7	.39	1.22	6.15	-2.61	. 22	85
49.0	1.253	1.086	15	25	18	6.31	19.11	60.36	-8.69	-5.89	42.36
50.0	1.169	1.088	15	25	40	5.88	17.88	58.57	-9.12	-7.12	-
51.0	.940	.943	10	20	40	4.10	12.66	48.72	-5.90	-7.34	-
52.0	.748	.516	6	11	40	1.85	5.78	27.17	-4.15	-5.22	-
53.0	.617	.265	5	5	9	. 90	2,82	14.03	-4.10	-2.18	5.03
54.0	.790	.578	5	5	10	2.17	6.76	31.01	-2.83	1.76	21.01
55.0	. 908	.666	4	10	40	2.87	8.92	38.51	-1.13	-1.08	~
56.0	.824	. 814	5	5	19	3.10	9.64	40.76	-1.90	4.64	21.76
20.0	.632	.699	5	5	40	2,05	6.40	29.64	-2.95	1.40	-
21.0	.578	.345	5	5	5	. 99	3.09	15.31	-4.01	-1.91	10.31
22.0	. 630	.363	6	15	40	1.14	3.57	17.58	-4.86	- 11.43	-
23.0	. 910	. 845	10	23	40	3.56	11.04	44.70	-6.44	-11.96	-
24.0	1.178	1.108	16	30	41	6.03	18.31	59.22	-9.97	-11.69	-
• 0	1.219	.985	14	30	40	5.61	17.10	57.35	-8.39	-12.90	-
ير. 0	1.158	1.056	15	26	18	5.66	17.25	57.58	-9.34	-8.75	39.58
27.0	. 893	699	9	20	18	2.93	9,12	39.15	~6.07	-10.88	21.15
28.0	. 691	.342	6	6	40	1.24	3.87	18.92	-4.76	-2.13	
							28 Set aver	age error	-3.77	-2.69	16.66

TABLE 3. COMPARISON OF OBSERVED CURRENT METER TILT ANGLES WITH DATA COMPUTED BY ASSUMING NO CABLE CURVATURE

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Overall average error 3.4



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Figure 26. Current Meter Tilt Angles Computed by Simple Statistics Model Compared With Observed Data

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4.1.2 WHOI Mooring No. 279

Millard³⁷ describes tension measurements made on a taut-moored buoy system. The buoy system (figure 27) was installed in water 2685 m deep at Woods Hole Oceanographic Institution Site D. Tensions were recorded at four locations along the mooring line, and currents were recorded at a depth of 12 m for the $2\frac{1}{2}$ months that the buoy system was on station. Berteaux and Walden⁷⁹ describe the properties of the wire rope and plaited nylon rope used in this buoy system as follows:

	Diameter (in.)	Weight/ft in Sea Water (lb/ft)
1/4-in. 1 × 50 wire rope	0.25	0.090
5/8-in. plaited nylon	0.625	0.01047.

The cable modulus for the wire rope was taken as 1.682×10^7 lb/in.². Nylon rope is subject to both elastic and inelastic deformation when loaded. New rope, when first loaded. will acquire a permanent deformation, the amount of which depends on the initial load. Furthermore, if the load is left on the rope, the rope is subject to creep and the permanent deformation increases with time. Martin¹⁴ discusses the various mechanisms for the deformation of nylon rope. Using Martin's curve for the percent stretch versus load for the 5/8-in.-diameter plaited nylon rope, the rope modulus is computed as follows:

$$E = 3.52 \times 10^{5} \text{ lb/in.}^{2}; 0 < T < 1000 \text{ lb}$$
$$E = 6.79 \times 10^{5} \text{ lb/in.}^{2}: 1000 < T < 2000 \text{ lb}$$
$$E = 1.041 \times 10^{6} \text{ lb/in.}^{2}: 2000 < T.$$



Figure 27. Woods Hole Oceanographic Institution Mooring No. 279

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Webster⁸⁰ measured currents for a 2-month period at site D. Webster's data were curve-fitted using the program CURFIT on the GSA time-sharing computer, and the following function for the current strength as a function of depth was developed:

$$C = 2.6$$
, Cs , $D^{-0.418}$

where

C is the current (ft/sec)

Cs is the surface current (ft/sec)

D is the depth (meters).

The steady-state buoy system configuration computer program was modified to include Webster's current profile, Martin's elastic properties for the nylon rope, and the cable properties given by Berteaux and Walden. Since information on the variations of current direction with depth was not available, the currents were assumed to be acting in the same direction at all depths. Furthermore, the initial inelastic stretch due to the emplantment and the dynamic wave loads is not known. The no-current elastic stretch of the nylon rope was assumed, and the tensions in the system were computed while currents acted on the system. Tensions at the junction of the wire rope and nylon rope are shown in figure 28 as functions of the surface current and of the no-current clastic stretch. Data taken by Millard at the same location on the mooring line are also shown on figure 28. Because of the creep properties of the nylon rope, the inelastic stretch will increase and the no-current elastic stretch will decrease as time increases. The shape of the tension curve will remain roughly the same, but the tension bias will decrease with time. Since the


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Figure 28. Comparisons of WHOI Data With Computed Mean Mooring Line Tensions

creep properties of the nylon rope are not known, computation of errors between observed and computed tensions in this case is of little value.

Millard analyzed the tension versus surface current data shown on figure 28 and least-squares fitted the linear function:

 $T = 12.3 \cdot Cs + 369$.

Least-squares linear fits to the computed curves A, B, and C are

 $T = 12.59 \cdot Cx + 608 ,$ $T = 12.25 \cdot Cs + 381 ,$

and

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$$T = 12.10 \cdot Cs + 203$$
.

Slopes of the computed linearized functions are in very close agreement, which indicates that the functional form of the computed tensions is accurate.

From this comparison, it is obvious that more experimentation on the elastoplastic properties of nylon rope is needed in order to predict the steadystate configurations of deep ocean, taut-moored bucy systems. Also, a deep ocean buoy system should be installed with both recording tensiometers and inclinometers to better validate the steady-state computer model.

4.2 <u>Experimental Measurements of</u> Buoy System Dynamics

In order to validate the computer simulation of buoy system dynamics, buoy motions as measured at sea will be correlated with computer simulated buoy system response to the same environmental conditions.

Two oceanographic buoys were equipped with motion sensing instrumentation and installed in Block Island Sound. The smaller of the two buoys, a $3\frac{1}{2}$ -ft-diameter sphere, was installed in March 1970 off Great Salt Pond entrance, Block Island, Rhode Island. The larger buoy, an 8-ft-diameter torroid was installed at station BRAVO during May 1970. A description of the instrumentation and a discussion of the measurements taken are presented in reference 81.

4.2.1 Spherical Buoy at Station D

The $3\frac{1}{2}$ -ft-diameter spherical buoy is shown in figure 29. This buoy was loaned to the writer by Dr. A. Nalwalk, of the Marine Sciences Institute of the University of Connecticut. The buoy was equipped with the following instrumentation:

Heave motion statistical accelerometers

Current meter

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Heave accelerometer

Surge Accelerometer

Sway accelerometer

Pitch pendulous potentiometor

Roll pendulous potentiometer

Cable pitch pendulous potentiometer

Cable roll pendulous potentiometer

Cable tension gage

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The buoy system was installed in 62 ft of water, west of Great Salt Pond entrance at Block Island by the Research Vessel, UCONN, on 2 March 1970.

The $1\frac{1}{2}$ -in.-diameter, 14-conductor armored cable was laid along the bottom to



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Figure 29. Spherical Buoy at Block Island Station D

the beach and was carried over the beach to the BIFI field station: A Snodgrass wave sensor was also installed off the beach in 25 ft of water. The arrangement of cables and instruments is shown schematically in figure 30.

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Two weeks of statistical heave accelerometer data were collected. Some buoy motion data were also recorded. The electrical conductors in the cable began to fail after a month of use and a number of attempts were made to repair the cable in order to continue collecting data. The buoy broke loose during a storm in early November 1970 and is missing.

The statistical accelerometers were designed to count at 0.35-, 0.50-, 0.65-, 0.80-, 1.20-, 1.35-, 1.50-, and 1.65-g heave acceleration levels. The pulses from the accelerometers actuated counters in the van at the BIFI Field Station, and the total count was recorded daily. Sea state data were based on estimates by the resident engineer (Carl T. Milner) and by Coast Guard observations reported by the ESSA Marine Weather Service. Data were recorded during the period 4 March to 12 March 1970.

Data from the 22 observations of positive acceleration counts were used to generate figure 31, a plot of the counts per hour versus acceleration level for various sea states. The negative acceleration counters did not work, because of the failure of leads in the cable. Figure 31 indicates that for any sea state greater than sea state 0, the buoy will always undergo 1.2-g accelerations at a rate of 1,000/hr. The number of cycles per hour for higher acceleration levels will increase with increasing sea state. Figure 32 is the conditional probability of the buoy exceeding various positive acceleration levels given that the buoy exceeds 1.2-g level accelerations. Figure 33 shows the buoy heave acceleration



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Figure 32. Conditional Probabilities for Heave Motions



amplitude histogram for various sea state conditions. Rayleigh distributions are plotted over the histograms and were computed by using the mean of the histograms. Longuet-Higgins 55 and Bretschneider 51 have shown that the distribution of wave heights is given by the Rayleigh distribution.

Analog data of the spherical buoy motions were also recorded. On 16 March 1970, the following records were obtained on a two-channel strip chart recorder:

Wave height (20 min)

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Buoy heave and buoy surge (15 min)

Buoy heave and buoy pitch (10 min)

Buoy heave and buoy roll (10 min)

Buoy heave and cable pitch (10 min)

Buoy heave and cable roll (10 min).

Winds were 15 to 20 knots, northeast, with an estimated sea state 3 at the buoy. Results of a simple "quick look" analysis are shown in figures 34 through 37.

One-hundred samples of each record were digitized and analyzed on the GSA-440 time sharing computer. Figure 34 shows means, variances, and standard deviations for each parameter. In addition, the correlation matrix for simple product-moment correlations is shown. Parameters that should be coupled appear to be coupled, and parameters that should be decoupled appear to be decoupled. For example, heave-surge, heave-pitch, and surge-pitch are coupled, and heave-roll, surge-roll, and pitch-roll are decoupled. Also, cable angles are mildly coupled to buoy displacements but decoupled from buoy angles.

VARIABLE		MEAN		VARIANO	<u>)E</u>	STANDARD DEVIATION
HEAVE ACCELERATION		0 g		0.0242 g	2	0.1555 g
SURGE ACCELERATION		0 g		0.00978 (2. g	0.0991 g
PITCH ANGLE	1.1 deg	BOW DO	WN	104.2 deg	2 5	10.20 deg
ROLL ANGLE	14.3 deg	PORT		55.3 deg	2 5	7.58 deg
CABLE PITCH	-27.1 deg	5		24.0 deg	2 g	4.90 deg
CABLE ROĻL	-7.3 deg	5		16.2 deg	2 5	4.01 deg
THE CORRELATIO	ON MATR	IX				
	<u>HEAVE</u>	SURGE	PITCH	ROLL	CABLI <u>PITCH</u>	CABLE <u>ROLL</u>
HEAVE	1.0	0.4808	0.6050	-0.0706	0.2435	0.0454
SURGE		1.0	-0.4199	0.0054	-0.1605	0.1975
PITCH			1.0	-0.1021	0.0063	-0.0946
ROLL				1.0	0.2121	-0.3813
CABLE PITCH					1.0	-0.2282
CABLE ROLL	L					1.0

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Figure 34. Spherical Buoy Motion Parameter Statistics

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Figure 35. Spherical Buoy Motion Amplitude Histograms -Wave Height, Heave, and Surge

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Figure 36. Spherical Buoy Motion Amplitude Histograms - Pitch and Roll



Figure 37. Spherical Buoy Motion Amplitude Histograms --Cable Pitch and Roll

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One hundred amplitudes from each parameter record were digitized and were used to generate parameter amplitude histograms. These probability distributions are shown in figures 35 and 37. The median amplitudes were computed and were used to compute Rayleigh distributions, which are plotted over the histograms. The Rayleigh distributions of the form

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$$p(H) = \frac{\pi}{2} \frac{H}{H^2} e^{-\frac{\pi}{4} \frac{H^2}{H^2}}$$

where

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p(H) is the probability of parameter H

- H is the parameter
- \overline{H} is the mean value of the parameter,

were found to match the histograms quite well. The fact that the amplitude probability distributions all appear to fit a Rayleigh distribution indicates a linear transform from wave height to buoy response.

If the functions relating the mean buoy motion amplitude parameters to sea state were known, the probability distribution for any parameter in any sea state can be computed from the Rayleigh distribution.

From the statistical accelerometer data, the mean heave acceleration amplitude can be plotted versus mean wave height (using Vine and Volkman's 30 relations for sea state and mean wave height). This curve, shown in figure 38, can be approximated in the region H = 1 to 10 ft with the linear function:

$$\overline{H_{V}} = 0.11 + 0.0086 \,\overline{H}$$

Assuming a linear transform for the other motion parameters and averaging pitch and roll amplitude means, we can write a set of linear equations for the



mean motion amplitudes. In addition, these $\epsilon_{i,1}$ actions can be substituted into the Rayleigh equation to find the amplitude probability distribution for any parameter. The linear equations and their amplitude distributions are as follows:

For 1 < H < 10 ft, the mean heave acceleration amplitude is

$$\overline{H_V} = 0.11 + 0.0086 \overline{H}$$
.

The amplitude probability distribution is

$$p(Hv) = \frac{T}{2} \frac{Hv}{Hv^2} e^{-\frac{T}{4}\frac{Hv}{Hv^2}}$$

The mean surge acceleration amplitude is

$$\overline{S_{u}} = 0.0688 + 0.00537 \,\overline{H},$$

and the probability distribution is

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$$p(S_u) = \frac{\pi}{2} \frac{S_u}{S_u} e^{-\frac{\pi}{4} \frac{S_u}{S_u}}$$

The mean sway acceleration amplitude is

$$\overline{S_w} = 0.0688 + 0.00537 \,\overline{H}$$

and the probability distribution is

$$p(S_w) = \frac{\pi}{2} \frac{S_w}{S_w^2} e^{-\frac{\pi}{4} \frac{S_w}{S_w^2}}$$

The mean pitch angle amplitude is

$$\overline{Pt} = 5.59 + 0.436 \overline{H}$$
,

and the probability distribution is

$$p(Pt) = \frac{\pi}{2} \frac{Pt}{Pt^2} e^{-\frac{\pi}{4}} \frac{Pt}{Pt^2}.$$

The mean roll angle amplitude is

$$\overline{RI} = 5.59 + 0.436 \,\overline{H}$$

and the probability distribution is

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$$p(RL) = \frac{\pi}{2} \frac{RL}{RL^2} e^{-\frac{\pi}{4} \frac{RL}{RL^2}}$$

The mean cable pitch angle amplitude is

$$\overline{\mathsf{GPt}} = 3.41 + 0.294 \,\overline{\mathsf{H}} \,,$$

and the probability distribution is

S)

$$p(CPt.) = \frac{\pi}{2} \frac{CPt}{CPt} e^{-\frac{\pi}{4} \frac{CPt}{CPt}}$$

The mean cable roll angle amplitude is

$$\overline{CRL} = 3.41 + 0.294 \overline{H}$$

and the probability distribution is

$$p(GRL) = \frac{T}{2} \frac{CRL}{CRL^2} e^{-\frac{T}{4} \frac{CRL^2}{CR^2}}$$

The preceding en pirical equations will serve as a first-order approximation to the buoy motion parameters for the spherical buoy and can be used for design purposes.

4.2.2 Torroidal Buoy at Station BRAVO

An 8-ft-diameter, torroidal oceanographic buoy was outfitted with buoy motion sensing instrumentation and telemetry and was installed in Block Island Sound during May-June 1970. The buoy (figure 39) was installed by the USCGC MARIPOSA on 30 April 1970 at station BRAVO in 120 ft of water. A telemetry receiving station was established in the generator building at the Watch Hill Lighthouse, Watch Hill, Rhode Island (figures 40 and 41). A schematic of the instrumentation arrangement is shown in figures 42 and 43. A detailed description of the instrumentation, circuitry, and calibrations is described in reference 81.



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Figure 39. Torroidal Buoy at Station BRAVO



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Figure 40. The Shore Station at Watch Hill Lighthouse



Figure 41. Telemetry Receiver and Recording Equipment



Figure 42. Torroidal Buoy Motion Experiment Setup

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The buoy contained ise following motion sensing instrumentation:

Heave accelerometer

Surge accelerometer

Sway accelerometer

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Pitch angle potentiometer

Roll angle potentiometer.

In addition, a self-recording current meter was attached to the mooring cable a' a depth of 60 ft. Wind speeds were recorded on a paper tape recorder at the Watch Hill Lighthouse. Wind directions were logged every 4 hr by the duty personnel at the Lighthouse. Also, the ESSA weather reports on 163.5 kHz were monitored during each data run, and reported conditions at Coast Guard Stations bordering Block Island Sound were logged on the data sheets.

The buoy transmitted data every 12 hr for a $\frac{1}{2}$ -hr time period. Each data transmission was preceded by a calibration sequence consisting of two voltage levels from the potentiometer sensors. The FM signals from the buoy containing the five mixed frequencies were demodulated and recorded on an FM tape recorder. The composite signal and a 12.5-kHz phase-lock signal were also recorded on two AM channels.

During the 51 days the buoy was on station, 55 data transmissions were recorded. The only major problem encountered was the failure of the operational amplifier that mixed the five frequencies from the voltage controlled oscillators in the buoy. This component was replaced and tests continued. Structurally, the only failure was the loss of a cotter pin on a shackle holding one of the three chain bridle legs under the buoy. The loss of this pin allowed the shackle to come undone in such a manner that the buoy bridle had only two active legs. The buoy was recovered on 19 June 1970 by the USCGC REDWOOD.

Because of the large amount of recorded data, a simple "quick look" analysis of the data was performed by playing back the recorded data for each of the five motion parameters on a "memoscope." A 2-min sample of each parameter for each run was traced out. The mean width of the band trace-out was measured, and the value of the parameter double amplitude (in volts) was logged. This value was assumed to be the "significant" amplitude, i.e., the mean of the hig' it one-third of the amplitudes. It was found that the data on the pitch channel were too noisy to be used in this fashion; thus, pitch was not included in this analysis. The logged values of buoy heave, surge, and sway acceleration amplitudes along with buoy roll angle amplitude are shown in table 4. The environmental conditions are also shown for each run.

The statistics of the measured parameters are shown in figure 44. The matrix of simple product-moment correlation coefficients is also shown. These statistics were computed on the GSA-440 time sharing computer. Inspection of the correlation matrix indicates that all the significant buoy motions are well correlated. In addition, they correlate well with wind speed and, to a lesser extent, with computed wind wave height. A similar analysis was conducted with current speed. The elements of that correlation matrix are as follows:

	Heave	Sway	Surge	Roll
Current	0.1537	-0.00572	0.1040	-0.0237.

Thus, it appears that buoy motion amplitudes are not correlated with current speed. Buoy motions are not significantly affected by current speed.

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TABLE 4. SUMMARY OF OBSERVED ENVIRONMENTAL AND BUOY RESPONSE DATA FOR THE TORROUAL BLOW BRAND

					40.4	1 34 1	J YYN	חאטונ	D D R C	HA X	D V V	i	;					
			pul.M		Comput	ed Depth & C	urrent	Compute	puiw ba	Observe	d Waves	Par	ameter I	buble An	nplitude			
Date (1967)	Time (hr)	Speed (hanots)	Direction (deg)	Duration (hr)	Water Depth (ft)	Surface Current (knots)	Set (deg)	H1/3 (t)	T _{1/3} (sec)	Swell (ft)	(1) Wind	Heave	Sway	Surge	Pitch	Roll	Mean Pitch (V)	Mean Roll (V)
4/30	2230	.]	•	120.0	1.00	065].	1].	,].	.	0.3].].	.	.
5/1	1030	8.0	135.0	2.0	120.1	0.66	065	1.597	5.270	1.0	ł	0.5	0.45	0 5	r		•	-0.55
5/1	2230	•	ı	1	120.3	0.615	065	ı		1	ı	,	·	•		1		,
5/2	1030	6.3	130.0	2.0	120.4	0.234	065	1.719	5.467	1.5	I	0.5	0.55	0.65	•	0.25	ı	-0.5
5/3	2230	•	ı	•	123.2	0.456	265	,	•	2.0	0	0.25	0.15	0.2	ı	0.18	1	-0.5
5/4	2230	3.5	22.5	2.0	121.7	1.04	265	0.306	2.31	1.5	•	0.25	0.2	0.15	0.15	0.25	-0.45	-0.35
5/2	1032	4.3	202.5	1.0	122.1	1.26	265	0.472	2.87	1.0	•	0.18	0.15	0.14		0.2	1	-0.55
315	0017	6.0	180.6	3.0	123.6	1.30	265	1.600	5.27	3.0	1.0	0.5	0.55	0.55	,	0.2		-0.5
4/9	0365	14.8	252.5	5.0	122.6	0.95	265	1.717	3.459	2.5	1.5	0.5	0.5	0.45	ı	0.2	ı	-0.5
5 6	2100	34.5	292.5	10.0	123.5	0.93	265	4.670	5.01	2.0	1.5	0.85	0.6	0.65	ı	0.2	4	-0.5
3/7	0060	16.9	292.5	12.0	122.1	0.50	265	2.006	3.66	1.0	1.75	0.55	0.45	0.32	,	0.28	,	-0.52
5/7	2100	17.0	292.5	12.0	122.8	0.36	265	2.019	3.67	0.5	•	0.56	0.58	0.52	ı	0.32	,	-0.52
5/8	0060	10.4	261.0	1.0	121.8	0		1.136	2.97	0.75	0.5	0.52	0.75	0.65	1	0.28	,	-0.5
5,78	2100	10.0	223.0	3.5	122.1	0	ı	2.169	4.68	1.5	•	0.5	0.55	9.4		0.25	ı	-0.55
6 /5	0060	0	225.0		121.3	0 41	065	ı	ı	1.0	0	0.2	0.25	0.1	ł	0.15	,	-0.5
5/10	0060	0.87	225.N	ı	121.0	0.68	065	,	•	1.5	•	0.45	0.35	0.32	ı	0.32	,	-0.52
2/10	2100	19.0	225.0	1.0	120.6	0.58	065	2.495	6.59	1.0	1.0	0.65	0.5	0.55	ł	0.40	,	-0.60
5/11	0060	3.9	292.5	1.0	121.1	0.73	065	0.380	2.57	2.0	•	0.40	0.37	0.45	ı	0.40	,	-0.55
5/11	2100	0	ł		120.7	0.66	065	ı	1	1.25	•	ı	ı	ı	ı			ı
5/12	0060	0	270.0	•	120.7	0.75	065	•	,	1.0	•	0.6	0.5	0.5	ı	0.45		-0.52
5/12	2100	12.2	90.0	1.5	120.9	0.66	065	3.714	8.04	1.0	1.0	0.3	0.35	0.36	•	0.40		~0.62
5/14	2100	8.7	45.0	4.0	120.9	0.39	065	0.698	2.27	,	1.5	0.65	0.7	0.65		0.45	,	-0.6
5/15	0060	7.8	45.0	1.0	120.7	0.23	065	1.519	4.96	1.5	0.5	0.65	0.5	6.4	,	0.20	ł	-0.5
3/16	0060	11.3	90.06	3.0	120.8	0	ı	3.186	7.44	2.5	1.0	0.25	0.25	0.21	ı	0.11	,	-0.26
5/17	0060	20.9	135.0	10.0	121.2	0.46	265	9.097	9.66	ı	4.5	0.8	0.65	0.75	ı	6.50	,	-0.6
5/18	0060	20.9	67.5	10.0	120.7	0.78	265	4.562	5.94	2.5	4.0	ı	ı	ı	ı	,	ı	
5/25	0830	18.2	90.0	3.0	120.9	0.88	065	8.265	11.989	,	2.5	0.8	0.65	0.65	ı	0.5		-0.75
5/25	2030	17.3	135.0	12.0	121.1	0.87	065	6.41	8.022		1.75	0.4	0.55	9.4	,	0.25	,	-0.65
5/26	0830	•	ı	ł	120.5	1.08	065	·	1	1.0	0	0.45	9.4	0.3	•	0.2		-0.65
5/26	2020	21.7	202.5	3.0	120.6	0.99	065	3.73	5.106	2.25	0.5	0.8	0.5	0.7	0.25	0.25	9.4	-0.5

TABLE 4. (Cont'd) SUMMARY OF OBSERVED ENVIRONMENTAL AND BUOY RESPONSE DATA E n n n n n

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Time Speed Direction Duration Water Surface Serie Wile Serie Serie <td></td> <td></td> <td></td> <td>Wind</td> <td></td> <td>Compu</td> <td>ted Depth & C</td> <td>Current</td> <td>Comput</td> <td>ed Wind</td> <td>Observe</td> <td>d Waves</td> <td>đ</td> <td>rameter 1</td> <td>Doulule A</td> <td>mplitude</td> <td></td> <td></td> <td></td>				Wind		Compu	ted Depth & C	Current	Comput	ed Wind	Observe	d Waves	đ	rameter 1	Doulule A	mplitude			
5/71 660 4.8 315.0 6.0 120.1 1.05 1.05 1.0 2.0 0.3 <th0.3< th=""> 0.3 0.3 0</th0.3<>	Date (1967)	Hine Fri	Speed (knota)	Direction (deg)	Duration (hr)	Water Depth (ft)	Surface Current (knote)	Set (deg)	H1/3 (ft)	T _{1/3} (sec)	Swell (ft)	Wind (ft)	Heavo	Sway	Surge	Pitch	Roll	Mean Pitch V)	Mean Roll (V)
$3\sqrt{77}$ 200 11.0 11.0 11.0 120.1 0.22 0.6	5/27	0830	4.8	315.0	6.0	120.2	1.05	065	0.348	1.754	2.5	0.3			.		.		
5/28 630 4,4 315,0 0.5 120,1 0.74 216,0 10 0.25 0	5/27	2030	13.0	315.0	11.0	120.3	0.92	065	1.116	2.703	2.0	0.5	0.8	0.6	0.5	ı	0.2		-0.55
5/28 2000 1.4 25.0 6.0 120.3 0.65 0.645 1.5 0.3 0.45 0	5/28	0830	4.4	315.0	0.5	120.1	0.74	065	0.483	2.899	2.0	0	0.25	0.3	0.35	,	0.2		-0.55
5/79 2030 3.5 15.0 5.0 17.0 17.0 2.00 2.00 2.00 2.00 0.0 </td <td>5/28</td> <td>2030</td> <td>4.4</td> <td>225.0</td> <td>6.0</td> <td>120.3</td> <td>0.65</td> <td>065</td> <td>0.463</td> <td>2.814</td> <td>1.5</td> <td>0.3</td> <td>0.45</td> <td>0.5</td> <td>0.24</td> <td>,</td> <td>0.2</td> <td></td> <td>-0.6</td>	5/28	2030	4.4	225.0	6.0	120.3	0.65	065	0.463	2.814	1.5	0.3	0.45	0.5	0.24	,	0.2		-0.6
j j< j j j j j j j j j j j j	5/29	2030	3.5	135.0	5.0	120.7	0.23	065	0.306	2.306	1.0	0.5	0.25	0.2	0.25	ı	0.15		-0.6
5/11 200 10.4 270.0 4.0 122.1 0.82 2.56 1.136 2.56 0.6 0.5 0.55	5/30	2030	8.7	135.0	6.A	121.9	090	265	1.888	5.731	ı	1.5	0.45	9 .4	0.35	t	0.2		-0.6
0 1.4 270.0 0.5 12.7 1.15 265 0.461 2.83 0.5 0.55 0.55 6/1 2000 4.3 225.0 2.15 122.7 123 123 123 123 0.55 0.461 2.85 0.461 2.95 0.461 0.55 0.465 0.461 0.55 0.461 0.55 0.465 0.451 0.55 0.451 0.55 0.455 0.55 0.455 0.55 0.451 0.55 0.455 0.55 0.455 0.55 0.55 0.55 0.55	5/31	2030	10.4	270.0	4.0	122.1	0.82	265	1.136	2.968	,	2.0	0.6	0.7	0.75	1	0.4		-0.7
6/1 2030 4.3 225.0 2.5 123.1 1.23 1.23 1.23 1.23 2.0 0.461 2.83 - 2.0 0.46	6/1	0830	7.4	270.0	0.5	122.7	1.15	265	1.366	4.874	ı	1.5	0.55	0.55	0.55	1	0.25		-0.52
6/2 3890 2.2 225.0 2.0 122.2 1.28 355 0.121 1.440 1.0 0.45 0.45 6/2 2030 7.8 255.0 2.0 123.1 1.12 255 0.461 2.53 0.475 0.45 0.45 6/1 0580 4.3 255.0 3.0 123.5 1.02 255 0.461 2.53 0.475 0.45 6/4 0580 4.3 315.0 7.0 123.3 0.72 255 0.461 2.56 0.451 1.57 0.55 0.45 6/4 0500 0 18.0 7.25 1.22.3 0.72 255 0.45 0.55 0.45 0.55 6/4 2030 0 157.0 2.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55 <td< td=""><td>6/1</td><td>2030</td><td>4.3</td><td>225.0</td><td>2.5</td><td>123.2</td><td>1.29</td><td>265</td><td>0.461</td><td>2.83</td><td>1</td><td>2.0</td><td>0.48</td><td>0.5</td><td>0.55</td><td>ı</td><td>0.2</td><td></td><td>-0.65</td></td<>	6/1	2030	4.3	225.0	2.5	123.2	1.29	265	0.461	2.83	1	2.0	0.48	0.5	0.55	ı	0.2		-0.65
6/2 2030 7.8 225.0 2.0 123.4 1.32 255 0.461 2.872 1.0 1.5 0.535 0.445 6/3 0530 4.3 225.0 3.0 123.4 1.12 255 0.461 2.862 1.0 1.5 0.535 0.445 6/4 0830 4.3 315.0 7.0 123.3 0.72 255 0.461 2.862 1.15 0.535 0.43 6/4 0830 8.7 315.0 7.0 123.3 0.72 255 0.461 2.862 0.45 0.35 0.43 6/4 0830 8.7 7.0 123.3 0.70 255 255 0.46 2.863 0.45 0.35 0.43 6/5 0330 8.7 70.0 2.32.3 0.70 2.55 0.46 0.75 0.55 0.45 0.45 0.45 0.45 0.45 0.45 0.45 0.45 0.45 0.45 0.45	6/2	0640	2.2	225.0	2.0	122.2	1.28	265	0.121	1.449	1.0	1.0	0.45	0.5	0.45	1	0.25		-0.65
6/3 0830 4.3 225.0 3.0 12.4 1.12 265 0.461 2.632 1.0 1.5 0.55 0.43 6/3 2030 4.8 225.0 8.0 123.5 1.02 265 0.430 2.055 -1 15 0.50 0.43 6/4 0830 4.1 315.0 7.0 123.3 0.70 265 -1 15 0.50 0.44 0.35 6/4 0830 8.7 9.0 2.53 0.70 255 2.65 0.36 1.75 0.35 0.35 6/5 0830 0 135.0 0.70 225.0 0.71 2030 1.57 0.75 0.35 0.35 6/7 2030 0.7 2050 0.73 2050 0.741 1.59 0.75 0.35 6/1 2030 2.5 2030 0.75 265 2.731 1.5 0.35 0.35 6/1 2030	6/2	2030	7.8	225.0	2.0	123.4	1.32	265	1.518	5.138	2.0	0	0.75	0.6	0.55	ı	0.25		-0.64
6/3 2030 4.8 225.0 8.0 123.5 1.02 255 0.430 2.15 0.54 0.54 0.53 6/4 0630 4.3 315.0 7.0 123.3 0.72 255 0.53 1.67 - 0.55 0.35 0.35 6/4 0630 6.1 135.0 7.0 123.3 0.70 255 2 - - 1.0 0 0.35 0.35 6/5 0830 8.7 9.0 135.0 - 122.2 0.22 255 - - 1.0 0 0.35 0.35 6/7 2030 2.6 3.20 0.5 122.2 0.47 265 - 1.0 1.0 0.35 0.35 6/7 2030 2.6 3.21 0.45 2.73 1.00 1.5 0.35 0.35 6/10 2030 2.6 2.5 0.5 0.5 0.5 0.5 0.5 0.5 </td <td>6/3</td> <td>0530</td> <td>4.3</td> <td>225.0</td> <td>3.0</td> <td>122.4</td> <td>1.12</td> <td>265</td> <td>0.461</td> <td>2.832</td> <td>1.0</td> <td>1.5</td> <td>0.55</td> <td>0.44</td> <td>0.45</td> <td></td> <td>0.24</td> <td>,</td> <td>-0.6</td>	6/3	0530	4.3	225.0	3.0	122.4	1.12	265	0.461	2.832	1.0	1.5	0.55	0.44	0.45		0.24	,	-0.6
6/4 0830 4.3 315.0 7.0 123.3 0.72 265 0.306 1.672 - 0.55 0.3 0.3 0.3 6/4 2030 0 180.0 - 123.3 0.70 255 - 1 0 0 0.35 0.35 6/5 0330 8.7 9.0 2.5 122.2 0.72 255 - 1 0 0 0.35 0.3 6/5 2030 0 135.0 - 1 255 0.45 255 - 1 1<0 0 0 23 0.3 6/6 0830 0 2 2 0.45 0.55 2 0.45 0.55 0 0.55 0 0.55 0 0.55 0 0.55 0 0.55 0 0.55 0 0.55 0 0.55 0 0.55 0 0.55 0 0.55 0 0 0 0<	6/3	2030	4.8	225.0	8.0	123.5	1.02	265	0.430	2.026	ı	1.5	0.54	0.58	0.6	ı	0.25		-0.55
6/4 2030 0 180.0 - 121.3 0.70 265 - - 1.0 0 0.35 0.33 0.35 6/5 0330 8.7 130.0 0 1.80.0 2.5 122.2 0.22 265 - 1 0 0 0.35 0.33 6/5 2030 0 1.31.0 - 1.22.2 0.22 265 - 1 0 0 0 235 0.33 6/7 2030 2.6 2.5 0.30 0.7 265 0.41 135 0.2 0.25 0.2 0.25 0.2 <th0< th=""> 0.2 0.2</th0<>	6/4	0830	4.3	315.0	7.0	123.3	0.72	265	0.306	1.672	ı	0.5	0.3	0.35	0.3	1	0.25	,	-0.65
6/5 0330 8.7 50.0 2.5 122.2 0.22 265 .888 5.731 1.5 0 0.28 <th0< th=""> <th0< th=""> 0.28</th0<></th0<>	6/4	2030	•	160.0	•	123.3	0.70	265	ı		1.0	•	0.35	0.35	0.3	•	0.32		-0.65
6/5 2030 0 135.0 - 122.9 0.22 265 - - 1.0 0 0.25 0.25 6/7 2030 2.6 225.0 0.5 122.2 0.47 265 0.166 1.713 3.0 0 2.5 0 6/7 2030 2.6 225.0 0.5 127.2 0.47 265 0.166 1.713 3.0 0 2.5 0 6/8 2030 0 272.0 9.0 121.0 0.375 065 - - 1.0 0 0.25 0 6/9 0830 0 225.0 9.0 121.0 0.755 065 1.159 3.210 0 0 2.5 0 6/10 2330 0 - - 1.00 0 0 0 0 0 1.6 0 0 0 1.5 0 1.5 0 1.5 0 0 1.5	6/5	0830	8.7	60.0	2.5	122.2	0.22	265	.888	5.731	1.5	0	0.28	0.2	0.25	,	0.18	,	-0.6
6/7 2030 2.6 225.0 0.5 122.2 0.47 265 0.168 1.713 3.0 0 -	6/5	2030	0	135.0		122.9	0.22	265	ı	•	1.0	0	0.25	0.2	0.25	,	0.15	,	-0.6
6/8 0830 0 270.0 - 1,~5 0.45 665 - 2.55 0 -	6/7	2030	2.6	225.0	0.5	122.2	0.47	265	0.168	1.713	3.0	0		ı	ı	ı			ı
6/8 2030 4.3 225.0 9.0 121.3 0.375 065 0.341 1.800 1.5 0 0.4 0.55 6/9 0830 0 225.0 - 120.8 0.756 065 - - 1.0 0 0.3 0.45 6/10 8.0 1.21.0 0.72 065 - - 1.0 0 0.3 0.45 6/10 0830 0 - - 120.6 0.72 065 - - 1.0 0 0.3 0.45 6/10 0830 0 - - 120.6 0.72 065 - 1.0 0 0 3 1.47 2 6/11 1430 0 - 120.6 0.72 065 1.68 5.732 1.0 0 0 -	6/8	0830	•	270.0	,	۰. ۲	0.45	065	•	ı	5.5	0	1	,	•	ı	ı		•
6/9 0830 0 225.0 - 120.8 0.756 065 - - 1.0 0 0.3 0.45 6/9 203t 5.7 325.0 8.0 121.0 0.72 065 1.159 3.210 0 - 0.5 0.5 0.5 -	6/8	2030	4.3	225.0	9.0	121.3	0.375	065	0.341	1.800	1.5	0	• ••	0.55	••	ı	3.25	•	-0.6
6/9 2034 8.7 3.25.0 8.0 121.0 0.72 065 1.159 3.210 0 -	6/3	0830	¢	225.0	ı	120.8	0.756	065	,	,	1.0	0	0.3	0.45	0.3	ı	0.25		-0.6
6/10 0530 0 - - 120.6 0.78 065 - - 1.0 0 -	6/9	2036	8.7	0.222	8.0	121.0	0.72	065	1.159	3.210	•	ı	ı	ı	ı	ł		,	,
6/10 2030 5.0 180.0 - 120.9 0.72 065 0.624 3.294 1.0 0.33 1.47 - 6/11 1300 0 - - 120.6 0.72 065 - - 1.0 0 - <td>6/10</td> <td>0830</td> <td>c</td> <td>·</td> <td>ı</td> <td>120.6</td> <td>0.78</td> <td>065</td> <td>ı</td> <td>ı</td> <td>1.0</td> <td>0</td> <td>ı</td> <td>ı</td> <td>•</td> <td>ı</td> <td></td> <td></td> <td>,</td>	6/10	0830	c	·	ı	120.6	0.78	065	ı	ı	1.0	0	ı	ı	•	ı			,
6/11 0 - - - 120.6 0.72 065 - - 1.0 0 - - - 6/11 2030 8.7 225.0 1.0 120.9 0.636 065 1.888 5.732 1.0 1.0 - - - 6/12 0830 0 225.3 - 120.5 0.483 065 - - 1.0 0 - - 6/12 0830 0 225.3 - 120.5 0.483 065 - - 1.0 0 - -	6/10	2030	5.0	180.0	•	120.9	0.72	065	0.624	3.294	1.0	0.33	1.47	ı	ı	ı		1	
6/11 2030 8.7 225.0 1.0 120.9 0.636 065 1.888 5.732 1.0 1.0 6/12 0830 0 225.0 - 120.5 0.483 065 1.0 0 1.0 0 1.0 0 1.0 0 1.0 0	6/11	1430	c	ı	•	120.6	0.72	J65	۱	•	1.0	•	,	ı	t	ı	1		•
6/12 0830 () 225.0 - 120.5 0.483 065 1.0 0 Mean- : 1.44 120.0 0.70138 0.455	6/11	2030	8.7	225.0	1.0	120.9	0.636	065	1.888	5.732	1.0	1.0	ı	1	ł	,			,
Means - 2 744 0.70138 0.4573 0.455	6/12	0830	e	225.0		120.5	0.483	065	1		1.0	•	•	•		•			
	Means		2 344			120.0	0.70138						0.4773	0.45386	1 0.43431		0.2646		0.5625

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VARIABLE MEAN VARIANCE STANDA DEVIAT WIND 8. 198 (knots) 52. 72 (knots ²) 7. 261 (knots) SIGNIFICANT J. 62 ft 4. 44 ft ² 2. 10 ft WAVE HEIGHT J. 62 ft 4. 44 ft ² 2. 10 ft SIGNIFICANT HEAVE 0. 197 g 0.00574 g ² 0.0758 g ACCELERATION AMPLITUDE 0. 1397 g 0.0024 g ² 0.049 g SIGNIFICANT SWAY ACCELERATION AMPLITUDE 0. 135 g 0.00284 g ² 0.0532 g SIGNIFICANT SURGE ACCELERATION AMPLITUDE 0. 135 g 0.00284 g ² 0.0532 g SIGNIFICANT ROLL AMPLITUDE 14 deg 25. 9 deg ² 5.09 deg CURRENT 0. 7022 (knots) 0.1435 (knots ²) 0.3787 (kd THE CORRELATION MATRIX WIND I.0 0.4989 0.3765 0.4553 0.413 WIND I.0 0.4989 0.3765 0.4553 0.413 WAVE 1.0 0.8058 0.8448 0.467							
WIND 8.198 (knots) 52.72 (knots ²) 7.261 (knots) SIGNIFICANT 1.62 ft 4.44 ft ² 2.10 ft SIGNIFICANT HEAVE 0.197 g 0.00574 g ² 0.0758 g ACCELERATION AMPLITUDE 0.1397 g 0.0024 g ² 0.049 g SIGNIFICANT SWAY 0.1397 g 0.0024 g ² 0.049 g ACCELERATION AMPLITUDE 0.135 g 0.00284 g ² 0.0532 g SIGNIFICANT SURGE 0.135 g 0.00284 g ² 0.0532 g ACCELERATION AMPLITUDE 0.00284 g ² 0.0532 g SIGNIFICANT ROLL 14 deg 25.9 deg ² 5.09 deg CURRENT 0.7022 (knots) 0.1435 (knots ²) 0.3787 (ka THE CORRELATION MATRIX WIND 1.0 0.4989 0.55/2 0.199 WAVE 1.0 0.4989 0.3765 0.4553 0.413 HEAVE 1.0 0.8058 0.8448 0.467 SWAY 1.0 0.8642 0.488 SURGE 1.0 0.555 0.4553 0.413	VARIABLE		MEAN		VARIAN	CE	STANDARD DEVIATION
SIGNIFICANT WAVE HEIGHT 1.62 ft 4.44 ft^2 2.10 ft SIGNIFICANT HEAVE ACCELERATION AMPLITUDE 0.197 g 0.00574 g^2 0.0758 g SIGNIFICANT HEAVE ACCELERATION AMPLITUDE 0.197 g 0.00574 g^2 0.0758 g SIGNIFICANT SWAY ACCELERATION AMPLITUDE 0.1397 g 0.0024 g^2 0.049 g SIGNIFICANT SURGE ACCELERATION AMPLITUDE 0.135 g 0.00284 g^2 0.0532 g SIGNIFICANT ROLL AMPLITUDE 14 deg 25.9 deg^2 5.09 deg SIGNIFICANT ROLL AMPLITUDE 14 deg 25.9 deg^2 0.3787 (ka CURRENT $0.7022 \text{ (knots)$ $0.1435 \text{ (knots}^2)$ 0.3787 (ka THE CORRELATION MATRIX MIND WIND 1.0 0.7699 0.6418 0.4839 0.5922 0.199 WAVE 1.0 0.4989 0.3765 0.4553 0.413 HEAVE 1.0 0.8058 0.8448 0.467 SWAY 1.0 0.8642 0.4882	WIND		8.198 (kno)	ts)	52.72 (kn	ots ²)	7.261 (knots)
SIGNIFICANT HEAVE ACCELERATION AMPLITUDE 0.197 g 0.00574 g^2 0.0758 g SIGNIFICANT SWAY ACCELERATION AMPLITUDE 0.1397 g 0.0024 g^2 0.049 g SIGNIFICANT SURGE ACCELERATION AMPLITUDE 0.135 g 0.00284 g^2 0.0532 g SIGNIFICANT SURGE ACCELERATION AMPLITUDE 0.135 g 0.00284 g^2 0.0532 g SIGNIFICANT ROLL AMPLITUDE 14 deg 25.9 deg^2 5.09 deg SIGNIFICANT ROLL AMPLITUDE 14 deg 25.9 deg^2 0.3787 (ka CURRENT 0.7022 (knots) $0.1435 \text{ (knots}^2)$ 0.3787 (ka THE CORRELATION MATRIX WIND WAVE HEAVE SWAY SURGE RO WIND I.0 0.4989 $0.3765 \text{ o}.4553$ 0.413 0.413 HEAVE I.0 $0.8058 \text{ o}.8448$ 0.467 0.4862 0.486	SIGNIFICANT WAVE HEIGHT		1.62 ft		4.44 ft ²		2.10 ft
SIGNIFICANT SWAY ACCELERATION AMPLITUDE 0.1397 g 0.0024 g^2 0.049 g SIGNIFICANT SURGE ACCELERATION 	SIGNIFICANT HEAVI ACCELERATION AMPLITUDE	Ξ	0.197 g		0.00574	g ²	0.0758 g
SIGNIFICANT SURGE ACCELERATION AMPLITUDE 0.135 g 0.00284 g ² 0.0532 g SIGNIFICANT ROLL AMPLITUDE 14 deg AMPLITUDE 25.9 deg ² 5.09 deg CURRENT 0.7022 (knots) 0.1435 (knots ²) 0.3787 (knots ²) THE CORRELATION MATRIX WIND WAVE HEAVE SWAY WIND 1.0 0.4989 0.3765 0.4553 WAVE 1.0 0.8058 0.8448 0.467 SWAY 1.0 0.8642 0.488	SIGNIFICANT SWAY ACCELERATION AMPLITUDE		0.1397 g		0.0024 g	2	0.049 g
SIGNIFICANT ROLL 14 deg 25.9 deg ² 5.09 deg CURRENT 0.7022 (knots) 0.1435 (knots ²) 0.3787 (knots ²) THE CORRELATION MATRIX WIND WIND WAVE HEAVE SWAY SURGE RO WIND I.0 0.7699 0.6418 0.4839 0.5852 0.199 WAVE I.0 0.4989 0.3765 0.4553 0.413 HEAVE I.0 0.8058 0.8448 0.467 SWAY I.0 0.8058 0.8448 0.467	SIGNIFICANT SURGE ACCELERATION AMPLITUDE	E	0.135 g		0.00284	g ²	0.0532 g
CURRENT 0.7022 (knots) 0.1435 (knots ²) 0.3787 (knots ²) THE CORRELATION MATRIX WIND WAVE HEAVE SWAY SURGE ROY WIND 1.0 0.7699 0.6418 0.4839 0.5852 0.199 WAVE 1.0 0.4989 0.3765 0.4553 0.413 HEAVE 1.0 0.8058 0.8448 0.467 SWAY 1.0 0.8642 0.488	SIGNIFICANT ROLL AMPLITUDE		14 deg		25.9 deg ²	2	5.09 deg
THE CORRELATION MATRIX WIND WAVE HEAVE SWAY SURGE RO WIND 1.0 0.7699 0.6418 0.4839 0.5852 0.199 WAVE 1.0 0.4989 0.3765 0.4553 0.413 HEAVE 1.0 0.8058 0.8448 0.467 SWAY 1.0 0.8642 0.488 SURGE 1.0 0.557	CURRENT		0.7022 (kn	ots)	0.1435 (knots ²)	0.3787 (knots)
WIND WAVE HEAVE SWAY SURGE RO WIND 1.0 0.7699 0.6418 0.4839 0.5852 0.199 WAVE 1.0 0.4989 0.3765 0.4553 0.413 HEAVE 1.0 0.8058 0.8448 0.467 SWAY 1.0 0.8642 0.488 SURGE 1.0 0.557	THE CORRELATION	MATR	IX				
WIND 1.0 0.7699 0.6418 0.4839 0.5852 0.199 WAVE 1.0 0.4989 0.3765 0.4553 0.413 HEAVE 1.0 0.8058 0.8448 0.467 SWAY 1.0 0.8642 0.488 SURGE 1.0 0.557		WIND	<u>WAVE</u>	HEAVE	<u>SWA Y</u>	SUR G	<u>E ROLL</u>
WAVE 1.0 0.4989 0.3765 0.4553 0.413 HEAVE 1.0 0.8058 0.8448 0.467 SWAY 1.0 0.8642 0.488 SURGE 1.0 0.557	WIND	[1.0	0.7699	0.6418	0.4839	0.5892	0.1997
HEAVE 1.0 0.8058 0.8448 0.467 SWAY 1.0 0.8642 0.488 SURGE 1.0 0.557	WAVE		1.0	0.4989	0.3765	0.4553	0.4134
SWAY 1.0 0.8642 0.488	HEAVE			1.0	0.8058	0.8448	0.4671
SURGE 1.0 0.557	SWA Y				1.0	0.8642	0.4885
	SURGE					1.0	0.5571
ROLL 1.0	ROLL						1.0

Figure 44. Torroidal Buoy Motion Statistics for 6-week Period

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The GSA-440 computer was also used to fit curves to the observed parameter significant amplitudes. Six curves were fitted using least-squares methods.

In general, the linear plot had the best index of determination when fitted to the data. Figures 45 through 50 show the observed buoy motion significant amplitudes plotted versus wind speed, computed significant wave height, and buoy heave acceleration. The least-mean-squares linear curve is shown on each plot. The least-mean-squares linear functions were transformed to engineering units and are summarized in figure 51. If the buoy motion amplitudes are assumed to be distributed by the Rayleigh distribution, the mean amplitude is $62\frac{1}{2}$ percent of the significant amplitude. The empirical equations shown in figure 51 were again transformed in such a manner that they are in j'

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The cumulative probability distributions for wind, current, observed and computed wave height, and buoy motion amplitudes are shown in figures 53 through 55 for the time period that the buoy was on station.

A complete spectral analysis was performed on two runs of buoy motion data taken on 10 and 11 June 1970. A Fast Fourier Transform method that is programmed and is available on the NUSC UNIVAC 1108 computer was used to compute power spectra and cross correlations. The data were digitized at a rate of 64 samples per second on automatic data processing equipment by the Data Analysis Branch at NUSC. A set of buoy motion spectra for the second data run, which was taken at 2030 EDST, on 11 June 1970, is shown in figures 56 through 61. The wind was at 10 knots from the southwest and had been blowing



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Figure 47. Buoy Motions versus Wave Height - Heave and Sway

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Figure 48. Buoy Motions versus Wave Height - Surge and Roll

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Figure 50. Roll Motion versus Heave Motion

ON WIND (W in knots) $Hv_{1/3} = 0.0064 + 0.0163 \bullet W (g's)$ HEAVE ACCELERATION SWAY-SURGE ACCELER- $Su_{1/3} = Sw_{1/3} = 0.1056 + 0.003825 \bullet W$ (g's) ATION. $Pi_{1/3} = Rl_{1/3} = 12.83^{\circ} + 0.1373 \bullet W$ (deg) PITCH-ROLL ANGLE ON SIGNIFICANT WAVE HEIGHT ($H_{1/3}$ in ft) $Hv_{1/3} = 0.168 + 0.01792 \bullet H_{1/3}$ (g's) HEAVE ACCELERATION $Su_{1/3} = Sw_{1/3} = 0.120 + 0.01015 \cdot H_{1/3}$ (g's) SWAY-SUR GE ACCELER-ATION PITCH-ROLL ANGLE $Pi_{1/3} = Rl_{1/3} = 12.36 + 1.0 \cdot H_{1/3}$ (deg) ON HEAVE ACCELERATION AMPLITUDE ($Hv_{1/3}$ in g's) $Su_{1/3} = Sw_{1/3} = 0.0268 + 0.557 \bullet Hv_{1/3}$ (g's) SWAY-SURGE ACCELER-ATION

Figure 51. Empirical Equations for Significant Buoy Motion Amplitudes (Based on Linear, Least-Mean-Squares Equations)

 $Pi_{1/3} = Rl_{1/3} = 7.82 + 31.35 \bullet Hv_{1/3}$ (deg)

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PITCH-ROLL ANGLE


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Figure 52. Empirical Equations for Mean Buoy Motion Amplitudes



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Figure 53. Wind and Current Cumulative Distributions

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Figure 55. Significant Buoy Motion Cumulative Distributions

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Figure 56. Computed Wind Wave Spectral Levels - 2030 hr, 11 June 1970



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Figure 57. Observed Heave Spectral Levels



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Figure 58. Observed Surge Spectral Levels



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Figure 59. Observed Sway Spectral Levels



Figure 60. Observed Pitch Spectral Levels



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Figure 61. Observed Roll Spectral Levels

steady at that strength for 1 hr. The wind wave spectrum was computed by using a two-parameter Bretschneider spectrum. Figure 56 shows the computed wind wave power spectrum. Figures 57 through 61 show the buoy motion power spectra. Figure 57, the buoy heave acceleration power spectra, shows a peak in the energy around 0.5 rad/sec (T = 12.5 sec). There is also a peak around 1.5 rad/sec, which corresponds to the peak of the computed wind wave power spectrum.

The peak at the lower frequency may be caused by swell; a 1-ft swell from the south was observed visually during the data run. The same type of energy distribution is seen in the other spectra. Buoy roll angle and sway acceleration spectra indicate relatively more energy at wind wave frequencies; thus, it is concluded that the buoy was oriented in such a manner that the swayheave plane was close to a southwest-northeast orientation.

Buoy motion parameters were cross-correlated with buoy heave acceleration and are shown in figures 62 through 65. These plots indicate the existence of two modes with rather strong coupling between motion parameters. If a linear system is assumed, the input and output spectra can be related by the transfer function:

$$S_{A^{2}}(\omega) = H(\omega)^{2} \cdot S_{H^{2}}(\omega) ,$$

where

 $S_{A}^{(\omega)}$ is the buoy motion power spectrum $S_{A}^{(\omega)}$ is the wave height power spectrum $H(\omega)$ is the transfer function.



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Figure 62. Cross-Spectral Levels of Surge and Heave







Figure 64. Cross-Spectral Levels of Pitch and Heave



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Figure 65. Cross-Spectral Levels of Roll and Heave

Wave spectra determined by Williams⁷⁷ for similar conditions in Block Island Sound indicate that swell appears in the wave spectrum as a horizontal line to the left of the wind wave peak and is about 5 dB down from the peak. Figure 56 was modified to include swell. The buoy motion power spectra were smoothed and the difference in decibels between them and the wave spectrum (including swell) were plotted as the square of the absolute value of the transfer function in decibels. Figures 66 through 70 show the transfer functions and indicate the existence of two modes. The relative magnitudes of the peaks indicate that the modes are a heave mode (low frequency) and a roll mode (high frequency). Observations made by the writer while servicing the buoy at sea indicate that the sway-surge mode has the lowest frequency, the heave mode has the next highest frequency, and the pitch-roll mode has the highest frequency.

These spectra indicate that the heave, surge, and sway motions of the buoy are primarily excited by the sea swell, whereas buoy pitch and roll are excited by the higher frequency wind waves.

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Analysis of measured buoy motion data for the $3\frac{1}{2}$ -ft spherical buoy and for the 8-ft torroidal buoy have yielded sets of empirical equations that can be used to predict mean buoy motions and amplitude distributions for various sea states. Buoy motion amplitude distributions appear to follow a Rayleigh distribution, which indicates that the dynamic system is linear or near-linear.

Spectra for the torroidal buoy indicate the existence of a heave mode and a pitch-roll mode within the range of wave frequencies. In general, both buoys are hard-coupled to the sea surface in heave. The torroidal buoy appears to respond in pitch-roll motions to a greater extent than does the spherical buoy.



Figure 66. Gain Function for Heave and Wave Height



Figure 67. Gain Function for Sway and Wave Height



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FREQUENCY (Hz)

Figure 68. Gain Function for Surge and Wave Height



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Figure 69. Gain Function for Pitch and Wave Height



Figure 70. Gain Function for Roll and Wave Height

4.3 <u>Simulation and Comparison of</u> <u>Buoy System Dynamics</u>

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4.3.1 Spherical Buoy at Station D

Steady-state configurations of the spherical buoy system, installed at Station D, are shown in figure 71 for various uniform currents. Very little of the 3/4-in. DiLock chain is picked up off the bottom even under the worst current conditions — 1 knot. Steady-state tensions never go over 100 lb, and the watch circle radius can vary from 38 ft at 0 knot to 62 ft at 1 knot. These configurations were computed on the GSA time-sharing computer using the steady-state buoy system configuration program shown in appendix B. The Savonius rotor current meter used in the spherical buoy system failed 3 days after emplantment, thus; cable angles can not be correlated with current strength for this buoy system. However, the computed configurations are used as initial conditions for the buoy system dynamics simulation when current strength and direction are computed from the Coast and Geodetic Survey current tables.⁸³

The lumped-mass dynamic equations of motion for the cable and the equations of motion for the spherical buoy were programmed and solved numerically in the time domain using a fourth-order, Runge-Kutta numerical integration scheme. Five mooring line mass elements were used — three for the cable and two for the chain. The buoy was allowed six degrees of freedom (heave, surge, sway, yaw, pitch, and roll), and each mooring line mass element was allowed three degrees of freedom (x_i , y, and z). The program shown in appendix B can accept wind and current vectors coming from any <. V



Figure 71. Computed Steady-State Configurations of the Spherical Buoy System

direction; however, the waves are assumed to be two-dimensional and are constrained to come in on the z axis of the inertial coordinate system. A complete listing of the input data needed to describe the spherical buoy system is shown in appendix D. Mean wave heights and periods were computed from reference 82 for various sea state conditions. The spherical buoy system dynamics model was excited with the ten-component random wave model based upon a two-parameter Bretschneider spectrum having the mean wave height and period for each sea state. The average wind strength and direction and the average current strength over the tame period in which the statistical accelerometers were in operation were also used to force the model. The wave amplitudes for each component were allowed to build linearly over two component wave periods. This procedure minimized transient motions. The solution was allowed to proceed in time as transients decayed. Finally, buoy heave accelerations were sampled over a time period, and the mean heave acceleration amplitude was computed.

These computed amplitudes are shown in figure 72 and are compared with the amplitudes derived from the statistical accelerometer data. The computer model overestimates the heave acceleration amplitudes at the lower sea states and agrees quite well at the higher sea states. Since the observed sea states are based upon the Block Island resident engineer's visual observations, a plus or minus one sea state error band is shown in figure 72 for the observed data. It is concluded that the computer model provides a conservative estimate of buoy accelerations for design purposes.

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Figure 72. Observed and Simulated Heave Motions of the Spherical Buoy

On 16 March 1970. strip chart recordings of the buoy motion instrumentation were made. The computed Bretschneider wind wave spectrum for that location, date, and time based upon the observed wind speed, direction, and duration is shown in figure 73. This spectrum was used to determine component amplitudes for the random sea forcing function. The observed wind and the current as computed from the tidal current tables were also used to force the model on the UNIVAC 1108. Computed buoy heave, sway, and surge accelerations. pitch, roll, cable pitch, and cable roll were sampled in the same manner as the observed data and were analyzed on the GSA time-sharing computer.

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The reduced spherical buoy motion parameters based upon computed motions and observed motions are shown on figures 74 and 75, respectively. Since the spherical buoy was not equipped with a yaw sensing device, the motions designated surge and sway and pitch and roll are not known relative to the z axis, along which the waves are traveling. Visual observations made from the beach while the data were being recorded indicated that the buoy was aligned in one direction with little or no yaw motion. A comparison of figures 34 and 75 indicates that the observed sway may really have been surge and that pitch and roll should be interchanged. Figure 75 reflects these changes.

A comparison of figures 74 and 75 indicates that the computer model predicts buoy heave accelerations with good engineering accuracy (-9.5 percent error for heave acceleration standard deviation), underestimates buoy sway accelerations (-71 percent error for sway acceleration standard deviation), and overestimates buoy pitch and roll motion (+54 percent error for pitch angle and



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Figure 73. Bretschneider Spectrum - 1200 hr; 16 March 1970 Station D

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VARIA	VARIABLE			MEAN		VARIANCE		STANDARD DEVIATION			
HEAVE ACCELERATION			0 g		0.0198 g ²		0.1408 g				
SWAY ACCELERATION			0 g		$0.0009~\mathrm{g}^2$		0.0288 g				
SURGE ACCELERATION			0 g		0.00	$15 g^2$	0.0390 g				
PITCH			-1.37 d	leg	137.	00 deg 2	11.70 deg				
ROLL			-1.01 d	leg	315.	07 deg^2	17.75 deg				
CABLE PITCH			-19.37 deg		13.38 deg^2		3.66 deg				
CABLE ROLL		38.29 deg		15.72 \deg^2		3.97 deg					
THE CORRELATION MATRIX											
1	HEAVE	<u>SWA Y</u>	SURGE	PITCH	ROLL	CABLE <u>PITCH</u>	CABLE <u>ROLL</u>				
HEAVE	1.0	0.4774	-0.2632	-0.1251	-0.0410	0.2660	-0.3602				
SWAY		1.0	-0.6762	-0.6023	-0.5559	0.1582	-0.2172				
SURGE			1.0	0.1980	0.3751	0.1502	0.1979				
PITCH				1.0	0.7363	-0.3377	0.2582				
ROLL					1.0	-0.1546	0.3268				
CABLE PITCH						1.0	0.0388				
CABLE ROLL							1.0				



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VARIABLE			MEAN		VARIANCE		STANDARD DEVIATION	
HEAVE ACCELERATION		0 g		$0.0242~\mathrm{g}^2$		0.1555 g		
SWAY ACCELERATION		0 g		$0.0098~\mathrm{g}^2$		0.0991 g		
SURGE ACCE LERATION		_		_				
PITCH		14.3 deg		55.3 \deg^2		7.58 deg	5	
ROLL		1.1 deg		104.2 \deg^2		10.20 deg		
CABLE PITCH		-7.3 deg		16.2 \deg^2		4.01 deg		
CABLE ROLL		-27.1 deg		24.0 \deg^2		4.90 deg	3	
THE COR	RELAT	ION MAT	TRIX					
	HEAVE SWAY			<u>PITCH</u>	ROLL	CABLE <u>PITCH</u>	CABLE <u>ROLL</u>	
HEAVE	1.0	0.4806		-0.0706	0.6050	0.0454	0.2435	
SWAY		1.0		0.0054	-0.4199	0.1975	-0.1605	
SURGE			1.0	-	-	-		
PITCH				1.0	-0.1021	-0.3813	-0.2121	
ROLL					1.0	-0.0946	0.0063	
CABLE PITCH						1.0	-0.2282	
CABLE ROLL							1.0	

Figure 75. Observed Spherical Buoy Motion Parameter Statistics

74 percent error for roll angle standard deviations). Mooring line pitch and roll angle standard deviations are underestimated by -33.6 percent and
-19.0 percent, respectively.

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The simple product-moment correlation matrix for the observed data indicates moderate coupling for heave-sway, heave-roll, sway-roll, and pitchcable pitch. Because of the axial symmetry of the buoy, it is expected that heave-pitch and surge-pitch would also be coupled. The simulated buoy motion correlation matrix indicates coupling between heave-sway, sway-surge, swaypitch, swav-roll, and pitch-roll. The modal coupling indicated by the two correlation matrices do not agree. This poor agreement is probably due to the rather short sample time used to compute the product moment correlations. An error estimate based upon an assumed bandpass white noise sea spectrum with a bandwidth of 1 flz indicates normalized errors of about 22 percent in the standard deviations and about 48 percent in the product-moment correlations. The comparison of simulated buoy motions with observed buoy motions for this case indicates reasonable agreement for parameter standard deviations but poorer agreement for product moment correlations. In view of the limited amount of data and its relatively poor quality, this particular comparison will not be extended.

1.3.2 Torroidal Buoy at Station BRAVO

The buoy motion computer model with lumped-mass cable elements was modified and input values were changed in order to simulate motions of the torroidal buoy BRAVO. A subroutine to compute the buoyant forces and moments

by polar integration around the torroid as a function of its draft and tilt angle was incorporated in the program (appendix E). A subroutine to compute simple statistical properties (mean, variance, and standard deviation) of the input wave height and output buoy motions (heave, surge, and sway accelerations and pitch and roll angles) was also incorporated in the program.

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Winds were assumed to act from the southwest (the predominant wind direction during May and June), and an average ebb current of 0.7 knot was assumed (uniform in depth, setting to the east). Thus, the buoy system coordinate system has the z axis pointing southwest and the y axis pointing southeast (figure 76). The S-M-B method was used to compute the mean wave height and period for winds of 5, 10, 15, 20, and 25 knots for the southwest winds with an assumed duration of 4 hr. The computed mean wind wave heights and periods for station BRAVO are shown in figure 77. Resulting Bretschneider spectra for these conditions are shown in figure 78. These spectra were used to compute random wave component amplitudes and frequencies which, in turn, forced the buoy motion computer model.

Initial runs with a five-element cable model were found to be very timeconsuming in machine time since the relatively low mass and high elastic modulus of the cable elements required that numerical integration step sizes on the order of 0.001 sec be used for numerical stability. A step size of 0.0005 sec was used for accuracy. The program was rewritten, and inputs were recomputed for a three-element mooring line model. Cable masses were concentrated at the current meter in the middle of the cable, and the lengths of heavy anchor chain were broken up to form the other two mooring line lumped masses. This





Figure 77. Computed Mean Wave Heights and Periods at Station BRAVO



Figure 78. Computed Wind Wave Spectra

procedure greatly reduced the highest natural frequency of the system and the step size could be increased to 0.01 sec before numerical instability occurred. A step size of 0.005 sec, which allowed an order of magnitude increase in computational speed, was used. No significant difference was seen in computed buoy motions when using the three-lump or five-lump cable model.

The run procedure is as follows. First, the model is acted upon by the mean wind and current components (no waves) and allowed to converge to its steady-state configuration. Then, the random wave components are introduced and buoy motions sampled at every integration for 60 sec. Each run required about 20 min of computer time on the UNIVAC 1108. The ratio of computer time to solution time was 9.15:1.

The results of four runs with mean wind speeds of 5, 10, 15, and 20 knots are shown in figures 79, 80, and 81. The least-mean-squares plots of the observed data, as shown in figures 45 through 50 and as summarized in figure 51, are also shown in figures 79, 80, and 81. Since the yaw orientation of the buoy was not known during the at-sea measurements, the coefficients of the surge and sway empirical functions and of the pitch and roll empirical functions were averaged. Also, since the winds and waves in the computer model are acting along the z axis, only heave, surge, and pitch motions are compared with observed data. The : verage error indicated in the simulation over the range of wind speeds considered are as follows:

1. On Mean Wind Speed

Mean heave acceleration amplitude+14.45 percentMean surge acceleration amplitude-63.6 percent

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Figure 79. Simulated and Observed Buoy Motions versus Wind Speed

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Figure 81. Simulated and Observed Surge and Pitch versus Heave

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	Mean pitch angle amplitude	-42.4 percent
<u>2.</u>	On Mean Wave Height*	
	Mean heave acceleration amplitude	+2.22 percent
	Mean surge acceleration amplitude	-60.15 percent
	Mean pitch angle amplitude	-41.43 percent
<u>3.</u>	On Mean Heave Acceleration Amplitude	

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Mean	surge	acceleration	amplitude	-60.4	percent
Mean	pitch a	angle amplitu	de	-42.3	percent.

Again, assuming bandpass-limited white noise spectra of about 1-Hz bandwidth, note that the error in the standard deviations of the simulated buoy motion is about \pm 13 percent. Also, known and estimated errors in the sensors and instrumentation (including possible observer error in reading the memoscope) indicate an overall error of \pm 20.13 percent in the observed data.

The differences in observed and computed mean heave acceleration amplitudes fall within these error bands and thus indicate that the computer model offers reasonable accuracy for this motion. However, even if these error bands are taken into account, the model is systematically underestimating surge and pitch motions. Since surge and sway motions are underestimated with both the spherical buoy and with the torroidal buoy, it is suspected that the transverse hydrodynamic mass and damping used in the model may be in error. Recent

^{*}Note that wave heights for the observed data were based upon visual observations while wave heights for the simulated buoy motions were computed by the S-M-B method.

communications with E. Geller and R. Canada, who are affiliated with the National Data Buoy Project, have indicated that the results of their model buoy tests in towing basins do not agree with present theory because surge and sway hydrodynamic forces are less than predicted analytically. This communication confirms the above suspicion, but judgment is reserved until more test tank data are published.

The environmental conditions as measured during the data run of 11 June 1970 at 2030 EDST were used as input to the computer model, and buoy motions were computed. Power spectra for wave height, buoy heave accelerations, buoy sway acceleration, buoy surge acceleration, buoy pitch motions, and buoy roll motions were computed by a Fast Fourier Transform method.⁸⁴⁻⁸⁶ A total of 1,024 samples, sampled at 0.06-sec intervals for each parameter, were transformed. The samples were smoothed by averaging over a 0.3-sec interval to prevent aliasing in the spectra. Eight ensemble averages were used, and they resulted in 64 statistical degrees of freedom and a standard error of 17.66 percent. The frequency resolution is 0.0163 Hz. Williams⁸⁷ states that a resolution on the order of 0.02 Hz is adequate to define ocean swell spectral peaks in his study of ocean wave spectra in Block Island Sound.

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The input wave spectrum is shown in figure 82 along with the frequencies of the components that form the "random" wave forcing function. Figure 83 shows the computed spectrum level of the "random" wave model compared with the theoretical spectrum level. As expected, the "random" wave model exhibits characteristics of narrow-band white noise, which is reasonable since it is made up of a finite number of sine wave components close to one another in







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Figure 83. Simulated Wave Height Spectral Levels

the frequency domain.

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The standard deviations for the input wave height and the output buoy motions are as follows:

Wave height	0.4807 ft
Heave acceleration	1.9321 ft/sec ²
Sway acceleration	0.4217 ft/sec^2
Surge acceleration	0.4974 ft/sec^2
Pitch angle	2.98 deg
Roll angle	4.437 deg.

Output spectral levels are shown in figures 84 through 88. In general, the spectra exhibit more deterministic properties than the buoy motion spectra from the at-sea data. More smoothing was done on the at-sea spectra than on the simulated spectra, but the at-sea spectra do not appear to have the narrow-band characteristics of the simulated spectra, especially in heave, surge, and pitch. This indicates that the model may be inadequately damped. The natural frequency of the buoy in heave when at middraft is computed to be 0.36 Hz. The heave, surge, and pitch spectra exhibit peaks at about this frequency. Since surge and pitch are decoupled from heave hydrodynamically, the coupling must be effected through the cable tensions acting on the buoy.

The second peak in the heave spectrum is located at about the same frequency as the peak of the water particle acceleration spectra. The forcing function component due to the heave hydrodynamic inertia force caused by water particle acceleration is driving the buoy in this frequency range. The surge spectrum also indicates that this forcing mechanism is active in that mode.



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Figure 84. Simulated Heave Spectral Levels



Figure 35. Simulated Sway Spectral Levels

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Figure 87. Simulated Pitch Spectral Levels



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Figure 88. Simulated Roll Spectral Levels

Both pitch and roll spectra show peaks at this frequency. Pitch and roll motions have computed natural frequencies at about 0.7 Hz and would tend to respond to this forcing mechanism through coupling from surge and sway motions. Since the water particle motions are constrained to the x - z plane, one would not expect a response out of plane. However, the current vector has an out-of-plane component; thus, the mooring line tension force will have an outof-plane component. Heave and surge motions will couple into sway and roll motions because of the mooring line tension. This coupling also illustrates another unusual feature of this dynamic system in that the response of the system is strongly dependent on the mean values of the system element spatial locations — a strong argument against linearized, decoupled models of buoy systems.

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Both heave and surge show peaks at higher frequencies (1.05 and 1.4 Hz). These peaks are probably due to natural frequencies for the lumped-mass cable model. Sway and roll spectra have nearly the same shape and thus indicate a strong coupling in these modes. Surge and sway (also, pitch and roll through coupling) indicate some response at very low frequencies. This is due to the lowest natural frequency of the buoy system in a horizontal mode. If the analogy is made with a pendulum having a length order of magnitude with the water depth, the natural frequency of this motion would be quite low.

The heave spectrum computed from the experimental data (figure 57) indicates a peak at 0.4 Hz, which agrees well with the peak in the simulated heave spectrum due to the buoys natural frequency in heave. Surge, sway, pitch, and roll spectra all have spikes at 0.6 Hz. This is in agreement with

the peaks in the simulated sway, surge, pitch, and roll spectra. All the spectra, especially in roll, indicate energy at frequencies out to 2 Hz. This energy may be due to vibration in the mooring line coupling into the buoy.

This comparison of simulated buoy motions with observed buoy motions for the torroid buoy at station BRAVO on 11 June 1970 indicates that the computer model is distributing energy in its response in about the same way as the actual buoy system. However, the model is apparently underdamped and is filtering out some energy between natural frequencies. Again, more test tank data are needed for buoy hulls to determine their hydrodynamic characteristics.

The simulations of buoy system dynamics for the spherical and torroidal buoys moored in Block Island Sound have used the lumped-mass model of cable dynamics. In the course of this research, it was found that if mean tensions in the cable are very low or if a number of force discontinuities are present along the cable (both conditions common to shallow water moorings), the finitedifference method is usually not suitable. With very low tensions, the cable equations can go ultrahyperbolic, which could cause the numerical method to break down. When many force discontinuities are in the line, a large number of nodes are needed in the cable segments between the discontinuities in addition to equation of motion for each discontinuity. The resulting computational time becomes prohibitive.

4.3.3 WHOI Mooring No. 238

In order to validate the buoy dynamics simulatation using the finitedifference cable model, mooring line tension data taken with WHOI mooring No. 238 (reference 37) is compared with simulated tension data for the same

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buoy system. WHOI mooring No. 238 is essentially the same as WHOI mooring No. 279 (figure 27) and was moored at the same location. Tensions were measured just below the torroidal surface buoy and were telemetered ashore. Computed inputs for the simulation of WHOI m~oring No. 279 are shown in appendix D, and the buoy system dynamics program incorporating the finite-difference cable model is shown in appendix B. Since the WHOI data are shown against wind speed, the primary input is wind speed. The fetch was assumed to be 100 miles for this location in the North Atlantic, and wind durations were taken to be 24 hr. The Webster current profile, with a surface current of 1.5 ft/sec, was assumed to act in the direction of the wind for all cases.

Strains and cable angles at each node must be read in as initial conditions. The steady-state buoy system configuration program was used to compute these parameters for each wind speed and the given current profile. The initial cable angles are shown in figures 89 and 90. The standard deviation for the cable tension just below the buoy was computed for each run. Millard³⁷ presents a "scattergram" of "dynamic tension amplitude" versus 2-hr mean wind speed. Tensions were recorded on a Rustrak recorder, which has a very slow chart speed — on the order of tentimeters per day. Thus, this "amplitude" was read by measuring the breadth of a very thick line. This actually represents the tension difference between the highest and lowest tension that occurred in a 2-hr period. It is assumed that these "amplitudes" correspond to the highest 1/10th wave heights found in wind-wave height distributions. The WHOI data of "dynamic tension amplitude" versus wind speed is shown in figure 91.

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Figure 89. Initial Strains for WHOI Mooring No. 238



Figure 90. Initial Angles for WHOI Mooring No. 238



Figure 91. Computed and Observed Tension Amplitudes at 12 m for WHOI Mooring No. 238

Tension standard deviations for each run were first converted to mean amplitudes by assuming sinusoidal variations, converted to highest 1/10th amplitudes by assuming a Rayleigh distribution, and finally converted to double amplitudes. The resulting expression is

$$T_{\rm WHOT} = (1.57)(2.03)(2.0) \cdot T_{\rm SD}$$

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The simulated "dynamic tension amplitudes" are plotted in figure 91. Errors for the simulation were not computed in this case since the meaning of the WHOI "dynamic tension amplitude" is not clear. However, the simulation computes dynamic tensions that are order of magnitude and that increase with increasing wind speed. A better set of buoy motion and tension measurements are needed for deep sea buoy systems in order to fully validate the buoy motion dynamic simulation with the finite-difference cable model.

V. SUMMARY

5.1 Restatement of the Problem

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The object of this investigation was to evaluate the forces acting on the components of a simple buoy system exposed to the oceanic environment and to develop a numerical model of buoy system dynamics to simulate buoy system response. Axisymmetric buoy hull shapes were considered in general, and hydrostatic and hydrodynamic forces and moments on oblate spheroids were studied. The set of integro-differential equations of motion for the buoy were reduced to a set of ordinary differential equations with nonconstant coefficients by using frequency dependent hydrodynamic force coefficients published in the literature.

A "quasi-random" wind wave model was developed to simulate the motions of the water masses in the immediate vicinity of the buoy. Wind wave properties were computed with the S-M-B method from the mean wind speed, fetch, and duration. Borgman's energy partitioning method was applied to a two-component Bretschneider spectrum, and sinusoidal wave component amplitudes and frequencies were computed. The random phase components were summed to compute instantaneous water particle motions.

Major assumptions made in the investigation of buoy dynamics include the following:

1. Dissipative forces can be separated into those due to surface wave generation and those due to viscous drag.

2. Infinitesimal buoy motions exist; this assumption was made by $John^{22,23}$ in the derivation of the hydrodynamic force and moment integrals and subsequently by Kim, ⁴² who evaluated the integrals for oblate spheroids.

3. The Haskind hypothesis is valid; i.e., it is assumed that the general problem of an object moving on a free surface in response to gravity waves on that free surface can be linearized to the extent that the velocity potential for that motion is the sum of

a. the linearized velocity potential of the gravity waves alone,

b. the velocity potential of the object moving on the free surface with no waves, and

c. the velocity potential due to the waves generated by the motion of the body.

The Haskind hypothesis was used in order to apply Kim's coefficients for oblate spheroids, which were derived for objects oscillating on a free surface, to the case of objects oscillating on a free surface with gravity waves.

4. The St. Denis-Pierson hypothesis is valid, i.e., it was assumed that the sea can be represented as the linear sum of elementary waves of random phase.

5. Body dimensions are small compared with a wavelength. This assumption was made in orde, to use the computed water particle motions as the motions of the mass of water in the immediate vicinity of the buoy hull.

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Equations of motion for an elastic cable capable of supporting both axial and transverse waves simultaneously were written. A unique finite-difference numerical technique, an extension of Hartree's method for hyperbolic partial differential equations, was developed for the solution of sets of coupled hyperbolic equations. A lumped-mass cable model was also developed. Major assumptions made in the development of the cable dynamics model are that the cable is homogeneous and perfectly flexible and that hysterisis damping is negligible compared with viscous damping. The equations of motion for the buoy and cable were programmed and solved numerically on a UNIVAC 1108 digital computer in the time domain. Two types of buoy hulls were considered — a spherical buoy and a torroidal buoy. Both shallow water and deep water moorings were simulated using the lumped-mass and finite-difference cable models. Simulated data were compared with observed data in two steady-state cases and in three dynamics cases.

5.2 Conclusions

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Steady-state buoy system configurations were simulated using the method described in chapter III for a shallow water buoy system and for a deep water buoy system. Comparison of simulated to observed configuration parameters (current meter tilt angles for the shallow water mooring and mean tensions for the deep water mooring) indicate good agreement between the computer model configurations and the configurations of the actual buoy systems. Although this comparison of a few bits of data from two buoy systems does not constitute a full validation of the steady-state buoy system configuration model (angles and tensions all along the mooring line for many current profiles and winds and for many different buoy systems should be simulated and correlated for a full validation), it does indicate that the simulation errors are on the order of 5 percent, which is adequate for engineering analysis.

Buoy system dynamics were simulated for three cases (two shallow water moorings where the lumped-mass cat.. nodel was used and one deep water mooring where the finite-difference cable model was used), and simulated buoy motion parameters were compare.. with observed parameters. In general, the lumpedmass cable model is more adaptable to shallow water buoy systems, which, if slack-moored, tend to have very low tensions at the bottom and cable angles that approach $\overline{W}/2$ and, if taut-moored, tend to have very high dynamic strain levels, which can cause slack cable. The lumped-mass cable model can handle these cases easily (if tensions on an element go to zero, the element simply free falls through the water), whereas the finite-difference cable model breaks down with slack cable conditions since the equations of motion are ultrahyperbolic and possess an infinite number of equally valid solutions. Also, the lumped-mass model is more adaptable when the mooring line contains a number of mass or force discontinuities, i.e., instruments, subsurface buoys, sentinels, etc.

The comparison for both shallow water cases, one a spherical buoy and the other a Richardson torroid, indicate that the surge and sway hydrodynamic forces were underestimated in the simulation. Also, the comparison of observed to sin flated shallow water torroidal buoy motion spectral levels indic; tes that the simulation is underdamped in all modes. There is good agreement in simulated and observed heave motions, the most important motion parameter for the buoy system designer. In general, the simulation appears to predict heave motions within ±15 percent and the other motions within ±50 percent. Since the environmental conditions were not monitored at either buoy during the motion measurements but were inferred from wind speeds measured ashore, from computed tidal currents based upon previous current measurements, and from visual observations of sea conditions, it is impossible to draw conclusions on the validity of the model, except that it computes buoy system motions that are order of Liagnitude with observed motions.

The comparison of simulated deep sea buoy system dynamics using the finite-difference model is inconclusive because of uncertainties in monitoring the environment and in the statistical meaning of the tension data collected.

5.2 Suggestions for Further Study

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Future research in the area of buoy system dynamics should involve the emplantment and the fitting of motion sensing instrumentation to a wide spectrum of oceanic buoy system types — both shallow and deep water. The environment at each site should be adequately monitored (winds, waves, and currents), and a complete set of buoy system motions should be recorded. The measurements should include angles and tensions along the mooring line as well.

Another key area that should be investigated involves the hydrodynamic forces acting on a body on the free surface of a fluid when the free surface is subjected to random gravity waves. John's analysis assumed infinitesimal body motions in order to linearize the free surface boundary conditions. Kim's analysis assumed sinusoidal body motions in order to evaluate the hydrodynamic force and moment integrals. An investigation of the validity of these assumptions should be made in order to better understand the nature of the body-fluid interaction. Also, test tank data are needed to validate theoretical force and moment coefficients.

Extension of the analysis presented in this disaertation to nonaxisymmetric buoy hulls to include cross-coupled hydrodynamic forces and moments in the other modes would be of significance as a more general study of buoy system dynamics.

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Finally. more research on the behavior of wire ropes and synthetic lines that are used in the ocean environment is needed for the prediction of buoy system performance.

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Appendix A

*5.

ANGULAR STABILITY OF AXISYMMETRIC BUOYS

It is known from visual observation and from collected motion data that buoys do not become unstable in the sense of Liapunov; i.e., buoy motions will not build to infinity. However, buoys will undergo large excursions and can be upset. For example, a buoy that undergoes gross heaving motions in the sea so that it is alternately awash and then rises out of water to fall over on its side would be deemed unstable by the casual observer; however, the buoy would actually be stable since its motions are bounded and do not tend to infinity with time. Of course, if this buoy housed meteorologic or oceanographic instrumentation, it would be of little value because of its wild motions.

The "stability" of this type of motion is best described in the pitch (or roll) phase plane, i.e., a plot of pitch angular velocity versus pitch. However, pitching (and rolling) motions are heavily influenced by the draft of the buoy, i.e., the location of the center of buoyancy relative to the center of gravity. Thus, the heave motions and resulting buoy draft must be considered. Because the righting moment depends on the location of the center of buoyancy relative to the center of gravity, the pitch (or roll) equations of motion are similar to the nonlinear equations of motion for a pendulum.

A wide spectrum of axisymmetric buoy shapes will be considered, ranging from a spar buoy with a high draft-to-beam ratio to a discus buoy with a low draft-to-beam ratio (figure A-1). First, consider the spar buoy. In the absence of external forces (mooring line tensions, etc.), static stability is maintained only if the center of gravity is below the center of buoyancy:

$$L_{CG} < H/2$$
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This is obvious from the undamped pitch equation of motion for the spar buoy:

$$I_{\mathcal{B}}\ddot{\mathcal{B}} + \rho g \frac{\pi}{4} B^2 H \left(\frac{H}{2} - L_{cg}\right) \sin \beta = 0$$

Also, if the buoy is tilted at extreme angles, we see that

The spar buoy will not return to its initial equilibrium position but will undergo a complete re-olution and seek equilibrium at either 2π or -2π . A typical pitch phase plane for the spar buoy is shown on figure A-2 for two possible conditions:

1.
$$L_{cg} < H/2$$

2. $L_{cg} > H/2$

Note that if the buoy is initially unstable, L_{CG} > H/2, it is stable at $\pm \pi$; i.e., it is stable upside down.

A cable attached to the bottom of the spar buoy will tend to stabilize the buoy. Adding a restoring moment due to the cable tension in the equation of motion yields

$$I_{\mathcal{B}}\ddot{\mathcal{B}} + \mathcal{P} \subseteq \prod B^{2} H \left(\frac{H}{2} - L_{c_{\mathcal{G}}} \right) SIN \mathcal{B} + K L_{c_{\mathcal{G}}} \left(1 - \cos \mathcal{B} \right) L_{c_{\mathcal{G}}} SIN \mathcal{B} = O,$$

where K is a cable spring constant. For a stable system, the sum of the two
restoring moment terms must be positive. Solving for $L_{c_{\mathcal{G}}}$, we see that



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Figure A-1. Axisymmetric Buoy Shapes

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Figure A-2. Spar Buoy Phase Planes

$$L_{cg} = \frac{G}{2} + \frac{1}{2}\sqrt{G} \cdot \sqrt{G-2H} ,$$

where

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$$G = \frac{\rho G \pi B^2 H}{4K(1 - \cos \beta)}$$

in general, a spar buoy is designed to minimize heave response. Thus, the shallowest buoy draft would be the static draft of the buoy minus one-half the largest wave height:

$$H_{MIN} = H_o - \frac{1}{2} \mathcal{N}_{MAX}$$

For a stable buoy, we see that

$$L_{CG} < \frac{H_{MIN}}{2} = \frac{H_o - \frac{1}{2} \mathcal{N}_{MAX}}{2}.$$

The spherical bucy shape is subject to the same types of moments, but the study is complicated by the fact that the center of bucyancy will deviate from the vertical axis of the buoy as the buoy pitches. If the center of gravity lies below the center of bucyancy (5/8 H from the bottom), the buoy is positively stable and will have a phase plane representation similar to that shown in figure A-2 for the spar buoy. The upper phase plane applies if $L_{CG} < 5/8$ H, and the lower phase plane applies if $L_{CG} > H$. However, the buoy is neutrally stable if 5/8 H $< L_{CG} < H$, since the center of bucyancy is always directly below the center of gravity. In this situation there is no definite stable position, because there is no restoring moment. Again, the tension of a mooring line attached below the center of bucyancy will stabilize the buoy.

Any buoy with a draft-to-beam ratio less than 0.5 and a hull height-tobeam ratio less than 1 can be bistable; i.e., it can be stable right side up or upside down. Torroid and discus buoys are of this class. If the center of gravity is located far kelow the buoy hull (achieved by means of ballast weights on a boom or tripod), the phase plane is similar to the upper curve for the spar buoy. If the center of gravity is far above the buoy hull (due to heavy instruments or equipment), the lower curve for the spar buoy phase plane would arply — the buoy is stable when upside down. If the center of gravity is near the geometric center of the buoy hull, the buoy is equally stable right side up or upside down. The phase plane for this situation is shown on figure A-3. Inspection of figure A-3 indicates that the width of the stable (in the sense that the buoy is right-side-up) region in the phase plane can vary from 2**TT** to 0 depending on the vertical location of the center of gravity.

This simple discussion of buoy stability did not consider other modes of possible unstable motion since they have never been observed to offer serious problems. Cross-coupled moments due to the hydrodynamic forces acting on the buoy and the horizontal tension components were neglected in this discussion. Even with these restrictions, a few design guides are apparent. The buoy should be designed with the center of gravity below the center of buoyancy if at all possible. Also, the mooring line attachment point should be as low as possible to offer the greatest righting moment if the buoy does capsize.

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Figure A-3. Torroid or Discus Buoy Phase Planes

Appendix B

COMPUTER PROGRAMS FOR THE MODELS

Steady-State Buoy System Configurations

This program computes the three-dimensional spatial configuration, tensions, and strains of either an elastic or inelastic buoy system mooring line. Winds from any compass direction can act on the buoy, and currents that vary in strength and direction as a function of depth can act on the bucy and mooring line. The mooring line can be composed of segments having different properties (weight in water, mass, drag, elasticity, etc.). Point mass discontinuities (to simulate current meters, hydrophones, etc.) can also be accounted for.

The basic cable equations (equations (230) to (233)) and a discussion of the development of this program are included in chapter III of the main text. The logic employed in this program is shown in figure B-1, which generally illustrates the computational operations.

The input data are as follows:

- 3. Buoy major diameter (ft)
- 2. Buoy minor diameter (ft)
 - a. Vertical diameter for an oblate spheroid
 - b. Hole diameter for a torroid





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- c. 0 for a cylinder
- d. 0 for a sphere
- 3. Buoy weight (lb)
- 4. Maximum draft of hull (ft)
- 5. Free draft with no mooring line (ft)
- 6. Buoy windage (ft²)
- 7. Wind drag coefficient
- 8. Cable diameter (in.)
- 9. Cable weight in water per unit length (lb/ft)
- 10. Effective cable modulus of elasticity $(lb/in.^2)$
- 11. Unstretched cable length (ft)
- 12. Surface current (knots)
- 13. Water depth (ft)
- 14. Wind speed component in y direction (ft/sec)
- 15. Wind speed components in z direction (ft/sec).

The integration step size "B" is normally set at 1/100th of the total cable length. However, if the cable properties are changed from a lightweight line to a very heavy line (for example, anchor chain), the step size should be changed in inverse proportion to the in-water weights. Also, the normal and tangeninai drag coefficients ("DRGON" and "DRGCT") should be changed accordingly if the mooring line section is not circular, for example, hair-faired cable and chain. Occasionally, the solution will not converge into the depth error band. This occurs if the first draft increment is too large and the computed x dimension of the mooring line "overshoots" the depth. With each overshoot, the buoy draft increment is halved; thus, convergence may be very slow. The initial buoy draft increment should be halved if this occurs.

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	THIS PROGRAM COMPUTES STEADY STATE BUOY SYSTEM CONFIGURATIONS. THE DRAFT OF THE BUOY CONTROLS THE BUOY'S DISPLACEMENT AND DRAS WHICH DETERNIME THE UPPER BUONDARY CONDITIONS ON THE CABLE. THE CABLE CONTSTITUTION AND TS COMPUTED AND TS YEARLOAD AND TS YEARLOAD AND TS CONDITIONS ON THE CABLE. IS TREEED AND TT THE VALENT TS DECREASED. IS TREEESED AND THE BUOY DRAFT IS DECREASED. UNITS ARE IN FEET, POUNDS, SECONDS COONDINATE SYSTEM	Z SURFACE CURRENT O AUOY TOWARD BUOY ALONG Z AXIS / 1 TOWARD BUOY ALONG Z AXIS / 1	ANCHOR Cases	BEGIN TERATION AT THE FREE FLOATING DAAFT OF THE BUOV. Y.E AS THERE SER NO MOORING CALLE. DIMENSION TENILODIARING CALLE. DIMENSION TENILODIARING CALLE. MEAD 14 THE BUOY PARAMETERS - MAJOR DIAMETER. MIMOR DIAMETER. FEAD 14 THE BUOY WIMDAGE AND WIND DAAG SOEFFICIENT READ 100-DDJ.0D2-WD4-WHAFRE READ 101-DDJ.0D2-WD4-WHAFRE READ 101-DDJ.0D2-WD4-WHAFRE READ 101-DIAMETERS - DIAMETERIA READ 101-DIAMETERS - DIAMETERIAN WEIGHT PER FONT READ 101-DIAMETECISM READ 101-DIAMETECISM
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1 i 1 l ŧ 1 1 1 i : 1 i 1 2 260518 9262 1 í í 1 1 SINGORD RIJJESGRT(((DISP-M9)=#2)+((DKB+MFZ)=#2)+YEY##2) RIJJESGRT((DRB+WFZ)=#2)+((DISP-HB)#2)) RIJJESTAN(DRB+WFZ)/(DISP-MB)) CONTINE THE CURRENT PROFILE - D+RECTION AND MAGNITUDE - AS A CONDUTE THE CURRENT PROFILE - D+RECTION AND MAGNITUDE - AS A FUNCTION OF DEPTH: RELATIVE TO THE SURFACE CURRENT. EXPRESS AF Y AND Z COMPONENTS GOING IN THE MEGATIVE DIRECTIONS. Ìu ¢ IF WORE THAN ONE TIPE OF CABLE IS USED IN THE WADRA CHANGE CAB. Characteristics Here, use a logical if at the proper length Referent to change them back before the next configuration is computed DAYE DEG. ł READ IN THE ENVRIONMENTAL PARAMETERS - SURFACE CURRENTIMIS.) WATER DEPTH AND WIND COMPONENTS READ 102.CUR.DEEP READ 102.WINDZ ÷ 10 1 AT A 100 FORWAT (SF10,3) 101 FORWAT (SF10,3) 102 FORWAT (SF15,4) 104 FORWAT (SF2,2) 104 FORWAT (SF2,2) 104 FORWAT (SF2,2) 104 FORWAT (SF2,2) 105 FORWAT (SF2,2) ł KINEMATIC VISCOSITY AND DENSITY FOR SALT WATER 1 ; ۱ 1 . ٠ i. CALL ONLBU(BD1,802,H,HM,RHO,C,DISP,038) INITIAL ERROR BAND WIDTH = SENVIVED DOPLOEEP/20.0 THE INITIAL BLOY DRAFT INCREMENT DHAMA/20. Hambree-dh Continue Lei C=1.65+CUR WIND FORCES ON THE BUOY WFY20.0034+VCO=WAREA+VINDYAABS(WINDY) WFZ50.0034+VCO=WAREA+VINDZ=ABS(WINC2) UCEVeSTWRRZ) J+WeCOS(R(2))+SIMRR3)) VCEVeCOS(R(2))-WeSIN(R(2))=SIA(R(3)) KCEWeCOS(R(3))-WeSIN(R(2))=SIA(R(3)) 1 1 . 1 FOR A UNIFORM CURRENT V=0_0 VISEL.45E=5 M=3 H=0 X=0 J=1 E=144.eEC DC=DIA/12. RH0#1.992 B=.e1+SH 7=0.0 2=0.0 S=0.0 S7R=0.0 PATTON THE U U U . ı. ~ ю υu J JUDUUUU υυ U 00000 υU U ; -----119a 120a 121a 30.05 **9**0 -06 10 *86 86 103+ 040 12 02+ r, 050 å 16* 18 ğ ğ 00175 00177 00200 00200 00200 00200 00200 00200 00200 00200 00200 00200 00200 00200 00117 00117 00125 00131 00131 00131 00135 00140 00145 00145 00145 00145 00145 00145 00145 00145 00145 00145 82100 87100 87100 87100 00160 00161 00162 00163 00164 00170 00172 00172 00173 00173 00166 **10100**

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1 1 1 0 1 0 0 0 1 1 1 0 1 0 0 1 0 0 1 1 1 0 1 0 1 0 0 0 0 1 1 1 1 0 1 0 1 0 0 0 1 1 1 1 1 0 0 1 0 <th>ATTON</th> <th>IF INSTRUMENTS/SINKERS/CR OTHER CONCENTRATED FORCES ARE ACTING ON The monathg i the change the tensions and and to by incing a</th> <th>LOSICAL IF AT THE JROPER LENGTHIS) AND THEN SOLVING FOR THE STATIC FORCES ON THE REEBODY OF THE ITEM.</th> <th>C#XC=V*C=C05(8(2))=C05(8(3))</th> <th>CWYC==WYC+SIN(R(2)) +COS(R(3)) CWZC==WYC+SIN(R(3))</th> <th>CRENT=UCeOC/VIS DRGcT=_6055e(CRENTee=_4759)</th> <th></th> <th></th> <th></th> <th>D(2)=(=CWYC+0。5eRHOeDR6CNeDCeVCeABS(VC))/R(1) DTHE /DS</th> <th>D(3)=(=Cw2C+D。SaRHOeDR4CN+DC+NC+ABS(wC))/(R(1)=COS(R(2))) CALL RUNGE(N,R,D,S,B,M,K)</th> <th>€0_70(5)4);K</th> <th>DX=DS=COS(R(Z))=COS(R(3))</th> <th>X=X+UX EXIL]=R(1) BH(1)=R(2)=R(2)=R(2)=R(2)=R(2)=R(2)=R(2)=R(2</th> <th>[545-54]5.616</th> <th>TEST THE X COORDINATE AGAINST THE MATER DEPTH JF(Y=(DEEP=DDP))8+9+9</th> <th></th> <th>IF(J=15)17,16,13</th> <th>60 TU 7 IF(X=(DEEP+DDP))10,11,11</th> <th></th> <th>60 T0 Y</th> <th>0H20H25</th> <th>ERROR BAND HALF WIDTH KUST BE GREATER THAN THE STAP SIZE IF (DOP-B/2,112,7,7</th> <th>VRITE (4,106) Errovit freu nor Frivitere lizes</th> <th></th> <th>FUKHAT(KIH UKAFT UISFLAGEMENT) Vrite(4.102)H.Disp</th> <th>X50,0 V-0 A</th> <th>720,0 </th> <th>DO 82 [=1,100 CER.T</th> <th>D5#9+(1,0+1,272+TEN(L)/(E+(DC++2)))</th> <th>DXZ/S4CO5(PHI(L))+CO5(THE(L)) DY4DS+SIN(PHI(L))</th> <th>02=b54E05(PHT(L))T55H(THE(L)) ETR=ETD+D5</th> <th></th>	ATTON	IF INSTRUMENTS/SINKERS/CR OTHER CONCENTRATED FORCES ARE ACTING ON The monathg i the change the tensions and and to by incing a	LOSICAL IF AT THE JROPER LENGTHIS) AND THEN SOLVING FOR THE STATIC FORCES ON THE REEBODY OF THE ITEM.	C#XC=V*C=C05(8(2))=C05(8(3))	CWYC==WYC+SIN(R(2)) +COS(R(3)) CWZC==WYC+SIN(R(3))	CRENT=UCeOC/VIS DRGcT=_6055e(CRENTee=_4759)				D(2)=(=CWYC+0。5eRHOeDR6CNeDCeVCeABS(VC))/R(1) DTHE /DS	D(3)=(=Cw2C+D。SaRHOeDR4CN+DC+NC+ABS(wC))/(R(1)=COS(R(2))) CALL RUNGE(N,R,D,S,B,M,K)	€0_70(5)4);K	DX=DS=COS(R(Z))=COS(R(3))	X=X+UX EXIL]=R(1) BH(1)=R(2)=R(2)=R(2)=R(2)=R(2)=R(2)=R(2)=R(2	[545-54]5.616	TEST THE X COORDINATE AGAINST THE MATER DEPTH JF(Y=(DEEP=DDP))8+9+9		IF(J=15)17,16,13	60 TU 7 IF(X=(DEEP+DDP))10,11,11		60 T0 Y	0H20H25	ERROR BAND HALF WIDTH KUST BE GREATER THAN THE STAP SIZE IF (DOP-B/2,112,7,7	VRITE (4,106) Errovit freu nor Frivitere lizes		FUKHAT(KIH UKAFT UISFLAGEMENT) Vrite(4.102)H.Disp	X50,0 V-0 A	720,0 	DO 82 [=1,100 CER.T	D5#9+(1,0+1,272+TEN(L)/(E+(DC++2)))	DXZ/S4CO5(PHI(L))+CO5(THE(L)) DY4DS+SIN(PHI(L))	02=b54E05(PHT(L))T55H(THE(L)) ETR=ETD+D5	
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CATE 060572 PAGE	נונו) פנונו הקונו האז ננו ידאפ ננו יגיץ גַב	<pre>/ COMPILATION. 0 *DIAGNOSTICe MECSAGE(S)</pre>							
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DATE 060372 PAGE 8 10112156.591			TELATIVE LOCATION, MAME) 0060 R 000001 B 0000 R 06005 CD 0000 R 000032 HD	RH0,C.DISP,DRB) BLATE SPHEROIDAL BUOY 0=.35351(H/A19=3); 2=H0=*2)+A*B*ASIN(HD/ABS(A))	0 •DIAGNOSTICE WESSAGE(S)	
PATTON GI FOR OBLEU-OBLBU UNIVAC 1168 FORTRAN V LEVEL 2206 0023 This compilation #AS Dome on 06 Mar 72 at 1011215	SUBROUTINE OBLEU ENTRY POINT 000126 Storage USED (Alock, NAME, LENGTH) 0001 *Code no0142 0002 *bata no0034 0062 *6lank 000000	EXTERNAL REFERENCES (BLOCK, NAME) 0003 SGRT 0004 ASIN 0005 NERR35	STORAGE ASSIGNMENT FOR VARIABLES (BLOCK, TYPE, DDGD R DCCUDD A	D0101 1* SUEROUTINE OBLOURDS19527H7H3 00101 2* C DISPLACEWENT AND ORAG FOR AN 00103 3* A=BD2/2 AN 00104 4* B=BD1/2 AN 00105 5* HD=A A 00106 6* IF(H0-6E A) HD=A A 00106 6* IF(H0-6E A) HD=A A 00110 7* VUL=5:5164543 (E3A2) #15.56664 (H/ 00111 8* AD=1.57144845 (E3A2) #15.56664 (H/ 00111 8* AD=1.57144845 (E3A2) #15.56664 (H/ 001112 9* 01.48232.584H0940L 001112 9* 01.592.484H0940L 001112 9* 01.592.484H0940L 001112 9* 01.592.484H0940L 001112 10* 02* 354.484060 001112 10* 02* 354.4840 001112 10* 01* 00112 10* 01* 00113 10* 01* 00115 12* FU0N<	END OF UNIVAC 1108 FGRTRAN V COMPILATION.	

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DATE 060372 PAGE

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GI FOR TCHAUTTORBU Univac 1104 Fortran V Lével 2206 0023 This compilation #AS Dong on 06 MAR 72 AT 10112:59

PATTON

ENTRY POINT 000166 SUBROUTINE TORE

STORAGE USED (HLOCK, NAME, LENGTH)

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EXTERNAL REFERENCES (BLOCK, NAME)

SURT ATAN NERR35 0003 0004 0005

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1 ţ STORAGE ASSIGNMENT FOR VARIABLES (BLOCK, TYPE, RELATIVE LOCATION, NAME)

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1 í XI=5.1416+ [R*2] = [R*2] = [1,572-ATAN[AR675221(I.05AR57]] + [H=R] = \$2871 ([R*2] = ([R=H] *22]) SUBROUTINE TORAU(R.E.F.H.C.DRBIDISP) Diselacevent and drag for a torroidal buoy with cans e is torroid dia. (OD) — F is can xsect area If(H—R)4,445 AIT_RF#2) = (1.572-ATAN(ARG/SGRT(1.0-ARG)) - (R=H) = SGRT((R+e2) = ((R=H) +=2)) 8= (E-2,0+8)/2,0 DAB# (A1+2,0+8+1)+0,6+ (C++2) G3(R-(2.0+R-H))/R (H-2.04R)2,3,3 =3.1416s(R**2) RGs(R-H)/R CONTINUE CONTINUE CONTINU 10 00 0 υυ 10300 10100 00103 00106 001112 00112 00.16 00121 00122 00123 11100 00120 00124 10100 01100

DR8#2.0*DR8 DISPE(6.2832*5*41+F*H)*64.0 *ETURN* 22.5 00125 00125 00127 00120

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0 BUAGNOSTICE HEGSARE(S) END OF ANYAC' LIDA FORTHAN V COMPILATION.

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DATE ASAT73 BLAF 10						E: RELATIVE LOCATION: NAME)	LL'RHO.C.DISP.DRB) CYLINDRICAL BUOY (CAN OR SPAR) S(C)	0 =DIAGNOSTICE MESSAGE(S)			
	J.CVLBU Forthan V LEVEL 2206-0025 Atton vas done on 06 mar 72 at 10:1	IE CTLBU ENTRY POINT 000041	ISED TOLOCK WARE, LENGTH)	1 = =CODE 000054 0 = DATA 000014 2 ==BLANK 000000	REFERENCES (BLOCK, NAME) 3 MERRSS	SSIGNMENT FOR V/°IÅBLES (BLOCK, TYP Dood4 injps	C SUBROUTIME CYLBU(BD1,8D2,H DISPLACENT ANO DRAG OF A TF(M.66.L)H=L DISP=25.3*(BD1+*2)*RH0+H DISP=25.3*(H0+1.+*8D1+H+C*AB RETURN ENU	UNIVAC 1108 FORTRAN V COMPILATION			
	JI FOR CYLAU UNITAC 1108 THIS COMPILA	SUBROUTIN	STORAGE U	000	EXTERNAL 1 000:	STORAGE A: 0000 00	00101 00101 00103 00105 00105 00100 55 00100 54 00100 54 00100 54	END OF		,	

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· · · · · · · · · · · · · · · · · · ·	101131 1.936				 RELATIVE LOCATION. NAME.) 0001 000124 9L 0000 A 00000 A1 0000 000007 INJUS 	CiONBIDISE)	•RADe(R=F)+((R=F)==2))	te (R-F) + ((R-F) == 2)] +3.1416=(R=2) = (H=E)	0 #DIAQNOSTICe NESSAGE(S)	
PATTON	GI FOR DISBUDISBU UNIVACTIOS FORTRAN V LEVEL 2205 0025 This compliation as dome on 06 MAR 72 AT 1011	SUBROUTINE DISBU ENTRY POINT 000134 	0001 *CODE 000171 0000 *DATA 000026 0002 *BLANK 000000	EXTERNAL REFERENCES (ALOCK, NAME) 0003 NERRIS	STORAGE ASSIGNMENT FOR VARIABLES (BLOČK, TYP 0001 000096 3L 0001 000057 4L 0000 r 000001 rad	00101 14 SUBRQUTINE DISBU(R+E+L+H 00101 2* C DISPLACEMENT AND DRAG FOR 00101 2* C DISPLACEMENT AND DRAG FOR 00101 3* C E HEIGHTOF BEVEL *FILH 00101 3* C E HEIGHTOF BEVEL *FILH 001003 4* I E MELENT OF SEVEL * FILH 001005 5* 2 AILEC.GRRAH-(H+02) F/F 001005 5* 2 AILEC.GRRAH-(H+02) F/F	00110 76 RADER-F-HE/E 00111 88 3159-54.081.0472-He((RADes) 00113 108 3 1F(H-1)4.5.5 00115 118 5.4	00120 136 0150 136 0161/2612(06(H=2)=R 00121 136 07555415(C422) 00122 139 015554.001.0472425((R=2)+ 00123 154 9 RETURN 00123 164 ENO	END OF UNIVAC 1108 FORTRAN & COMPILATION.	

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101131 3.106 ł 1 i ł l ļ i DATE 040572 PAGE 12 1 ì i 1 į 1 ċ , ı ENTRY POINT 000075 STORAGE USED (BLOCK, WAME, LENGTH) 000024 000024 0000024 5 +CODE +UATA +BLANK SUBROUTINE SPHBU 0000 0000 0000

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Î ļ Í. 1 ł į . 1 1 ; 200011 IN-000 ļ į ļ 0000 STORÃGE ASSIGNMENT FOR VARIABLES (BLOCK, TYPE, RELATIVE LOCATION, NAME) İ TA 100000 A 0000 \$ SUGROUTINE SPHBU(R.H.C.DRB.DISP) DISPLACEMENT AND DRAG FOR A SPHERICAL BUOY If (H-2,000,2,3,3) Ì 1 ł . ; ODGO R GOOGO ARG ł EXTERNAL REFERENCES (BLOCK, NAME) , , , ; . ŧ ļ Seat ATAN NERRISE **P** Q : 000010 2L 000 -83 - Topp ł ł : į İ

H=2.04 ARG4(R-H)/R A12(H482)4(1.572-ATAN(ARG/59RT(I.0-ARE)) A12(H482)4(1.572-ATAN(ARG/59RT(I.0-ARE)) (H420.48A1F(C42) (H420.48A1F(C42)) (H420.48A1F(C42)) (H420.48A1 A12(H4) ł END OF UNIVAC 1104 FORTRAN V CONFILATION. ł 3 558 00101 CdIU1 00103 00107 00107 00110 00110 00112 00113 00113

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ţ 10:13: 4.230 000102 1306 000117 9L . ī 1 ţ ٤ 1000 ٠, 1 2 060372 PAGE__ 1 000054 1206 1 000074 7L 0 = 000090 0 I 1 í i ł SUBADUTINE NUMBE(N'Y'F'X'N'N'K) THIS ROUTINE PERFORMS RUMBE KUTTA CALCULATIONS BY GILLIS METHOD DIMENSION Y(16),F(16),G(16) DATE 1000 . ł ļ i. ł 1 STORAGE ASSIGNMENT FOR VARIABLES (BLOCK, TYPE, RELATIVE LOCATION, NAME)
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 121.N

 Y(1)=Y(1)+A+(F(1)+H+0(1))
 Y(1)=2.05AH+F(1)+(1.0-5.05A)+8(1)

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 4(1)=2.05AH+F(1)+(1.0-5.05A)+8(1)

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 100.29259532

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 17< Y00 NEED MONE ACCUMACY USE A=0.2928932</td>

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 000023 1070 000036 5L 000033 1NJPS 1 1 ł ł ł 1 1 I 1 I 1000 GI FOR RUNGE, RUNGF, UNIVAC'1108 FORTRAW V LEVEL 2206 0023 THIS COMPILATION WAS DONE ON 06 MAR 72 AT 10:13:04 ł 0001 000121 10L 0001 000032 4L ENTRY POINT 000134 HEN41 60 TO (1.4.553.7).H 1 DO 2 121.N 2 G(1130.0 STORAGE USED (BLOCK, NAME, LENGTH) EXTERNAL REFERENCES (BLOCK, NAME) 000000 000053 000000 I ł PATTON ŧ *CODE *DATA *ULANK NERR25 NERR35 I 0001 000021 1L 0001 000030 3L 0000 R 000021 A SUBRCUTINE RUNGE b U υ 000 ļ 0001 ۱ 174 Ĉ ŧ •

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Buoy System Dynamics for the Spherical Buoy

This program simulates the motions of a spherical buoy exposed to winds, currents, and a random sea. The mooring line is simulated as five, elastically connected, lumped masses. Cable weights, drag forces, and inertia forces are concentrated at each mass. The buoy is allowed six degrees of freedom, and each mooring line mass element is allowed three translational degrees of freedom. With a total of 21 degrees of freedom, 42 first-order differential equations are integrated simultaneously in the time domain.

The equations of motion for the buoy and the development of the forces acting on the buoy are presented in chapter III of the main text. The lumpedmass cable equations are also shown in that chapter.

Major computational procedures in this program are shown in figure B-2. The input data are as follows:

1. Buoy hull radius (ft)

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2. Height of the center of gravity of the buoy above the mooring line connection point on the buoy (ft)

3. Height of the mooring line connection point below the buoy hull (ft)

4. Buoy weight (lb)

- 5. Buoy structural and floodwater mass (lb-sec $^2/ft$)
- 6. Yaw mass moment of inertia (lb-sec $^2/ft$)
- 7. Pitch mass moment of inertia (lb-sec $^2/ft$)
- 8. Roll mass moment of inertia (lb-sec $^2/ft$)
- 9. Effective buoy wind drag coefficient (dimensionless)



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- 10. Effective buoy wind lift coefficient (dimensionless)
- 11. Buoy windage (profile area) (ft^2)
- 12. Buoy plan area (ft^2)

13. Height of the wind center of pressure above the center of gravity of the buoy (ft)

- 14. Mean wave height (ft)
- 15. Mean wave period (sec)

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16. The unstretched cable lengths between mass elements (ft) (5 required)

17. Upper mooring line segment diameter (ft)

18. Upper mooring line segment weight in water per unit length (lb/ft)

19. Upper mooring line segment mass per unit length (lb-sec $^2/$ ft)

20. Lower mooring line segment diameter (ft)

- 21. Lower mooring line segment weight in water per unit length (lb/ft)
- 22. Lower mooring line segment mass per unit length (lb-sec $^2/ft$)

23. Surface current y component (ft/sec)

24. Surface current z component (ft/sec)

25. Wind speed y component (ft/sec)

26. Wind speed z component (ft/sec)

27. Initial buoy displacements x, y, and z (ft)

28. Initial cable element displacements (including the anchor clump)

x, y, and z for each of 6 sets (ft).

The user of this program should first estimate the highest natural frequency in the system. In general, the upper mooring line segment will be lightest and the highest natural frequencies are in the axial mode along the cable. Using the same variable names as those in the program we can estimate the highest natural frequency:

$$f_{\rm HI} = \frac{1}{2\pi} \sqrt{\frac{2 \, \text{EA}}{\text{DCSM1} \cdot \text{CLO(2)}}},$$

where

EA is the cable modulus* (lb)

- DCSM1 is the mass per unit length of the upper mooring line segment $(lb-sec^2/ft^2)$
- CLO(2) is the unstretched cable length between the first and second cable mass elements (ft).

It the Jable lengths were very short or the cable modulus were very high in the lower mooring line segment, an estimate should be made of its highest natural frequency also.

For numerical stability, the integration step size should be about 1/20th the shortest period present. Thus, the step size is computed:

$$R = 0.05 (1/f_{HI})$$

In this program, the wave component amplitudes, frequencies, and phase angles are computed externally and are listed in the body of the program (MOMEG, AMP, and PHS). The water depth (DEEP) is also listed, and the x coordinate of the mooring anchor slump should be set equal to the water depth. The total time of the simulation is controlled by a logical "IF" statement (statement

^{*}The cable modulus is the product of the cable material elastic modulus and the actual cable cross-sectional area.

no. 491), which shifts to the "STOP" control if the maximum simulated time is exceeded.

In this program, buoy motion output data are not printed for the first 6 sec of simulation as initial transients decay. After this time, the following buoy system outputs are printed every 800 time steps:

Simulation time (sec)

4.5

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Water particle vertical acceleration (ft/sec²) Buoy heave acceleration (ft/sec²) Buoy sway acceleration (ft/sec²) Buoy surge acceleration (ft/sec²) Buoy pitch angle (deg) Buoy roll angle (deg).



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BUOT DYNAMICS	PHS(9)=0,306	C THE CABLE PARAMETERS	READ 105,CLO(1),CLO(2),CLO(5),CLO(4),CLO(5),FLO(6)	C THE UPPER CABLE PROPERTIES	C THE LOWEN CABLE DODOFOTTES	KEAD 109.01A4.0%C2.0CSM2	EA1=2.16E5 Fares	MC1=UMC1sCL0(1)	MC2=DWC2+CL0(5)	UGT1=0,05+DIA1+CL0(1) UGT2=0,2+D142+CL0(1)	UGNIE1.2001A10CLU(J) UGNIE1.2001A10CLU(J)	D6N2=1.2*DIA2+CL0(5)	CSM1#CCSM1#CC(1)	CSMKEULSMZ#CLU(5) CHMY1±0.0	CHMT2=0,753+(UIA2+02)=CL0(5)	CHMV1=1-56+(UIA1++2)+CLO(1)	C THE WATEL DEDIN C THE WATEL DEDIN	DEEP=52.0		G THE CURRENTS IN THE WATER COLUMN C READ THE Y AND Y COMPONENTE OF THE STIREACE AND SENT	READ 108.CYS.CZS	C IF THE CURRENT VARI'S WITH DEPTH, INSERT THE FUNCTION CONTRACTION TION CONTRACTION CONTRACTICACTICACTICACTICACTICACTICACTICACT	C SINGRGIM AND ULKECILYN	C FOR A UNIFORM CURRENT	CTECTS CTECTS		C THE WIND ACTING ON THE BUOY	L READ INE Y ANU Z COMPONENTS OF THE MEAN WIND Read 108. windy. wind?	#FX==0.0034+MCL=WAREL+IWINDY++2+WINDZ++2)	WFT=0.0034=MC0=WAREA=WINDY=ABS{WINDY} WFZ=0.003w=WC0=w=0F4=windy=aBc/windy;		MFGM#=MFY#WCPHT	411 Mit42	R=0.0005	X=0.0	011	C THE INITIAL CONDITIONS	DO 58 121,41,2	D(1)#0.0 58 Y(1)=0.0	C READ THE INITIAL BUOY DISPLACEMENTS - X,Y,Z	C THE INITIAL BUOY ANGLES ARE ZERO	
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÷ 1 DATE 100172 PAGE 1 i ! SINGLE TRAIN OF MAVES, ASSUME THEY ARE COMING IN ON THE 008T#=D08T#=#K=(MUNE6(I)==2)=AMP(I)=C05(ANE) THE BUOY MOTIONS RELATIVE TO THE WATER MASS DISPLACEMENTS Y(8)=0.0 Y(10)=0.0 Y(12)=0.0 Read The Initial Cable Displacements Do SJ 1=14,44,6 Head 109,Y(1),Y(1+2),Y(1+4) Contiaue S(ANG WHTTAWHIM/(2,0=PER) IF(A.GE.(2,0=PER))WHTTAHTM VELTBET(1) VELTBET(2) VEL2BET(2) VEL6BET(2) VEL6BET(1) VEL6BET(1) VEL6BET(1) ANG MONEG(I) SAMP **21/32.2 (1) +SIV(ANG) DXWILLXN-XOMEG(I) +AND I) *COS (ANG I JONSON + #XOUII #XOO (- CMOMEG .) JALBTV+KK+AMF DZ1=7 (5)-02W BT1=Y(10)-BTW GM1=Y(12)-GMK 087#=087#=6K# BUOY DYNAMICS =Y:2)=XH CHA-BXEN ZH-AMP 006MW=0. OXW=0.0 DZW=0.0 AKE (MUME)X1=Y(1 ELOCI 20=#20 DY1=Y AX1 011 2001 щe 6 ŝ 3 ų υυυ 140# 140# 424 162* 1655* 1655* 1655* 85. 87* 66# 61***** 5 62.9 <u>.</u> 7 74 ŝ 862 000 25 ě Ż

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ATE 100172 PAGE 8			.NTS																			T							ŗ		
blo ² DriAMICS D	0641=7(11)-D644 0811=7(9)-081=	C UGHIEF(11)	ר "אובא שאבא אבעסכווונא אבעאדער זט דאב פעסד ויכעטואט כטאַאבי טאבטיגו	UY2=CY1-CY 0/2=u/1-C2	UALSEUALI		C THE CUEFFICIENTS FOR A SPHERICAL BUOY	C THE BUDYANCY FORCES AND MOMENTS	M=XC0+X1-XML If M-, F _ O _ O1M=O _ O	HBTEH+(EK+XML-+XCG) +(1,0+CCS(BT1)) MGM-H-100-XML-+XCG) +(1,0+CCS(BT1))	C HEAVE BUOYANT FORCE, BXX	IF(M.GE.BR)60 T0 21 BXX=67.1e(Hee2)e(3.0eBR=H)	60 TO 22 21 TE/H GE /2 044B1160 TO 20	BXX2263。4s(BKse3)=57。1s(HPse2)s(3。0\$BR=HP) Cû To 22	24 BXX=268.4*(BR**3) WRITE(4,103)		22 IF(HBT.GE.BR)G0 TO 25	V88567.1s(H87**2)*(3.0*884=MBT) G0 T0 26	25 IF(H6T_GE_(2.0+8R))G0 T0 30 H8TP=2.0+8R=H4T	VBB=268.44(BR==3)=67.1=(HBTP==2)=(3.0=BR=HBTP) 26 BAB=44R=2461=2461=42614.4941	C ROLL BUOYANT MOMENT	V66#61.14 (H6M4e2) = 15.048ReHGM)	27 KGK91		C HEAVE-PITCH BUOYANT FORCE "BXB dxb=vb6s=bxx	C HEAVE-ROLL BUOYANT FORCE, BXG Bx3=vgg=axx	C PITCH-HEAVE BUOYANT MOMENT.BBX	BBKEBXXBX BR+XWL=XCG)=SIN(BT1) C Roll-HEAVE BUOYANT NOMENT-BGX	BGXEBX6018R+XML+XCG)+SIN(GM1) 30 continul	C THE HYDRODYMAMIC MASSES AND MOMENTS OF INERTIA	C THE DIMENSIONLESS FREQUENCY
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i 1 ļ DAYE 100172 PAGE ١ ł 1 ļ ì ļ ì ł PITCH AND ROLL NATEH 195sMUs(BR**3)s;3,0+6,0sBX+6,0*(BAs*2)+2,0*(BAs*3)) 1 /11.0+2,08BA+2,04:BAs*2)) NBB-NAA+NZZ*(XMM**2) NGGENUB HHAAL16.65*(BK**5)*(1.0+8A)/(1.0+2.0*8A+2.0*(BA*2)) Pitch and Roll HHBE:Hhize(XHM**2) Cross-Coupled Hydromasses Cross-Coupled Hydromasses THE DAMPING DUE TO WAVE GENERATION FR=((6,2852/PER)++2)+BR/32,2 BLamu=Sort(6,25/(PER+MU)) Batelderr Hydrotynamic Force Moment Arm Xhm=XLG-XmL-0,625+H CROSS COUPLED WAVE DAMPING 60 T0 43 44 CNYYE1.5954EXP(+0.4154FR) 45 NXXE1.9884FR4(BR+5)+CNXX NYYE1.9884FR4(BR+5)+CNXX CNXXII.184EXP(-0.834FR) IF(FR.LE.0.1)CNYT20.0 IF(FR.6E.1.37)G0 T0 44 CNYT2-0.05940.714FR IF(FK.6E.0.1)60 T0 31 CMX1.85 G0 T0 33 L1 F(FK.6E.3.4)60 T0 32 CMX1.02*(FR**(-.256)) 60 T0 33 MHXX=1.986+(8R+=3)+CMX MHYY=1.946+(8R+=3)+CMY MHZZ=1.988+(8K+=3)+CMZ IF(FR.GE.0.4)60 TO 41 CNXX=0.126+0.17#FR GO TO 42 MAX850.0 Max850.0 Max650.0 Max650.0 Max8520.0 Max8520.0 Max820.0 M BUOT DYMAMICS NH6X=0.0 32 CHX=1.0 HEAVE HEAVE 31 49 U U J υ u U υU JU U 265. 272. 275* 295. 269* 290* 291* 292* 315+ 200 1000 06+ 08+ .09 112 2994 05 9 ğ

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-1 i , PAGE 1 i i 1 i 1 i 100172 1 ! ī 73 CARLED CONTONT STATE C į i ; 1 ł ŧ DATE FAL=0.0 EM1=-xCG+r=F2+COS(Y(10))=COS(Y(12))+FX*SIN(Y(10))=COS(Y(12)) FGM=-xCG+rY*COS(Y(10))=COS(Y(12))+FX*SIN(Y(12))=COS(Y(10))) CPIT=57.44ATAN((Y(18)-ZMPT)/Y(Y(14)=XMPT)) CROLL=57.44ATAN((Y(16)-ZMPT)/SQRT((Y(14)=XMPT))=*2+{Y(18)-ZMPT}) ţ HEAVE =X MOTION D(1)=(#B=BXX=bX8=BXG+MHXX=DDX==MHXB=DDBT1=MHXG=DDGM1=NXX=DX1 ÷ 1 **2)) 00 71 I=13.37.6 NT=(1-1)/2)-K NT=(1-1)/6) CL=SGwf(((Y(1+1)-Y(1+7)).*2)+((Y(1+3)-Y(1+9)).*2)+ 1 ((Y(1+5)-Y(1+11)).*2)) 1 ((Y(1+5)-Y(1+11)).*2)) 1 ((Y(1+5)-Y(1+11)).*2)) 1 ((Y(1+5)-Y(1+11)).*2)) 1 ((Y(1+5)-Y(1+11)).*2)) 1 ((Y(1+5)-Y(1+11)).*2)) 1 ((Y(1+5)-Y(1+11)).*2)) 2 (CL=SC-CHMT=CHMT2 CHMT=CHMT2 CHMT2 UATIONS OF MOTION FOR THE BUDY 0011=(T(1)-VELXB)/R 0011=(T(3)-VELYB)/R 0021=(T(5)-VELXB)/R 00211=(T(9)-VELBB)/R 006M1=(T(11)-V'LGB)/R BUOY DYNAMICS F2=T(o) 72 J U v υυυυ *767 302**.** 303**.** 2029 2029 2029 2409 2409 345e 346e 397e 4124 440+ **** 376. 304 • 405+ 406+ 406+ 408. 609. 010. 411. 414 1102 418+ *19* 426+ 427+ 428+ +6×3 +073 +073 +1773 +1773 9/9-9/9 996 0010 421+ 00032 00032 00032 000555 000555 0000555 0000555 0000555 0000555 0000555 0000555 0005550 0005550 0005550 0005550 000550 000550 0000550 0000550 0000550

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 uill)=c-BGX-u6G=WH6X=0DX1-WHGY=DC*1+MH6G=DDGX#=NGX=DX1-NGY=DY1

 uill)=c-BGX-u6G=WH6X=0DX1-WHGY=DC*1+MH6G=DDGX#=NGX=DX1-NGY=DY1

 1 - WGG=DGH1-DGGY=U72=ABS(JT2)-DGGG=DGKZ=ABS(DGH2)+WFGH+FGM)

 2 :GW1(H=HGG)

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BUOT CTHAMICS

218 IF(Y(1),GE_DEEP)Y(1)=DEEP
IF(A,GE_28,0)60 T0 88
60 T0 5
92 WRITE(4,106)
88 STOP
88 END *757 *757 *757 00772 00777 00100 01002 01002

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Buoy System Dynamics for the Torroidal Buoy at Station BRAVO

- 12

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The previous program, developed for the spherical buoy, was modified to simulate the torroidal buoy dynamics. As indicated in figure B-3, the basic computational procedures remain the same as for the spherical buoy. However, the wave component amplitudes, frequencies, and phase angles are computed in a subroutine (RWAVE) in the program and do not have to be listed in the program. The buoyant forces and moments for the torroidal buoy are also computed in a subroutine (TORBU) using an integration method developed in appendix E. Finally, since the output motions were to be displayed as spectrum, subroutines using Fast Fourier Transform (FFT) methods were employed to compute the spectra.

Buoy hull hydrodynamic force functions in the program were modified according to Kim's data for a half-beam to draft ratio of 3.2:1. Terms are included to account for the hydrodynamic mass and drag of the three-leg chain bridle under the buoy, and the effective hydrodynamic centers are modified to account for the bridle. The computed inputs for the torroidal buoy are shown in appendix D.

A three-element, lumped-mass cable model was used in this simulation since the actual masses in the system were concentrated at three places along the mooring line (the current meter, the sentinel, and halfway down the 3/4-in. chain). Initial runs using a five-lump cable model were compared with runs using a three-lump cable model, and no significant difference in buoy motions was noted. However, there was an order of magnitude increase in computational speed since the two, high-natural-frequency cable lumps were summed into the



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Input data for this program are as follows:

- 1. Buoy hull radius (ft)
- 2. Height of the center of gravity of the buoy above the mooring line

connection point on the buoy (ft)

- 3. Height of the mooring line connection point below the buoy hull (ft)
- 4. Buoy weight (lb)
- 5. Buoy structural and floodwater mass (lb-sec $^2/ft$)
- 6. Yaw mass moment of inertia (lb-sec $^2/ft$)
- 7. Pitch mass moment of inertia (lb-sec $^2/ft$)
- 8. Roll mass moment of inertia $(lb-sec^2/ft)$

9. Effective buoy wind drag coefficient (dimensionless)

10. Effective buoy wind lift coefficient (dimensionless)

11. Buoy windage (profile area) (ft^2)

12. Buoy plan area (ft^2)

13. Height of the wind center of pressure above the center of gravity of the

buoy (ft)

- 14. Mean wind speed causing the wind waves (ft/sec)
- 15. Wind duration (hr)
- 16. Surface current y component (ft/sec)
- 17. Surface current z component (ft/sec)
- 18. Wind y component (ft/sec)
- 19. Wind z component (ft/sec)
- 20. Initial displacements of the buoy x, y, and z (ft)

21. Initial cable displacements (including the anchor clump) x, y, and z for each of four sets (ft)

Again, the user should estimate the highest natural frequency in the system and adjust the integration step size as required. Computed buoy motions are sampled every 12th time step, and 1024, or 2¹⁰, samples are stored. Simple statistical estimates (mean, variance, and standard deviation) of the output motions are computed. Each data point is smoothed to reduce aliasing by averaging with the four data points closest to it in time. Spectra are computed using FFT subroutines and are smoothed for plotting. Statements 557 through 595 plot the spectral levels in dB. A Stromberg Datagraphics, Inc. integrated graphics system peripheral to the Naval Underwater Systems Center UNIVAC 1108 computer was used for this procedure.

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DATE 230272 PAGE MHAAI6.65*(BR**5)*(1.0+8A)/(1.0+2.0*BA+2.0*(BA**2)) Pitch and Roll MHBB=MH22*(XHM**2) MHBB=MH22*(XHM**2) Cross-coupled Htdromasses THE HYDRODYNAMIC MASSES AND MOMENTS OF INERTIA
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 MHXX=1,908+(8R**3)+CMX MHYY=1,988+((8R**3)+CMY+U,8045) MHZZ=MHYY THE DIVENSIONLESS FREQUENCY FR=((6.2832/FER)+*2)+0R/32.2 ULANB=S0RT(6.25/(PER+NU)) Baelang=0R Hydra90R Hydra91C Fokce Homent Arm Hydra910 HEAVE FF(FR.6E.0.2)GO TO 31 CMX22.31 GO TO 33 GO TO 32 CMX21.35725*(FR**(-.330597)) CMX21.35725*(FR**(-.330597)) BUOY UYNAMICS S#AY-SURGE DAL2=DAL1 DBT2=DBT1 DGM2=DGM1 CMY=0.215 MHXB=0.0 60 10 33 4HBX=0.0 4HGX=0.0 CHX=1.0 CMZ=CHI ŝ 31 З2 **5**55

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42HM=28HM MHGY=MHYG HEAVE IF(FR.GE.0.8)60 TO 41 CHXX=FR/(0.132617+0.873644•FR) Go TO 42

CNXX=1,2476+EXP(-0,37376+FR) CNYT=0,14 NXX=1,988+FR+(BR++3)+CNXX NYX=1,988+FR+(BR++3)+CNYY

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BUOY DYNAMICS DATE 23	1 AN=22N	PITCH AND ROLL	MAAI4.195.7U6(BR*63)a(3,0+6.0+BA+6.0+(BA**2)+2.0*(BA**3)) \ /// 0+3 A/2A+3 A+42/A+4+3/	- //	NGG=NBB	CROSS COUPLED WAVE DAMPING	NX8=0.0			N (GL=NY + (1, 282+0, 544)		-8Z=28V	NGY=NYG		AREAS	ARXX=3,1416*(BR**2)	06XX=1.2=ARXX	DGYY=15,5	044510-100 0644510-0	D688=0.0	D666=0,0	XCPRS=3,25	CR055-COUPLED VISCOUS DRAG		0682=0628	0667=0646		TENSIONS AND FORCES FOR THE 3 LUMP MASS CABLE MODEL	THE FIRST ELEMENT IS CABLE & THE LAST TWO ARE CHAIN	CABK-EA/CLO(1)	XMPT=Y(2)+XC6+COS(Y(10))+COS(Y(12))	YMPT=Y(4)-XCG+COS(Y(10))+SIN(Y(12))	Even Li (2) + 20(4):11(1,10) = 4COS(1(12))	VE-3001()(VAT ((VAT)/**2/*)(NT)((VO)/**2/*((VMT)(VO))**2)) TEN(1)=CABK*(CL-CLO(1))	IF(TEN(1),LE.0.0)TEN(1)=0.0	T(4) =TEN(1) = (Y(14) -XHPT)/CL	T (5)=TEN(1)=(Y(18)=7HPT)/CL T (6)=TEN(1)=(Y(18)=2HPT)/CL	ADD 165 LBS. TO ACCOUNT FOR THE CHAINS UNDER THE BUOY	Fx=T(4)+165.0		F2=T(6)	「カトール・シー FRT=★ググル・トレンチアのスイン・スコントキグのミンション・トロントロインシンション・スコン	FGM=+XCG+(FY=COS(Y(10))=COS(Y(12))+FX=SIN(Y(12))=COS(Y(10)))	00 71 I=13,25,6	K1=((1-1)/2)-2	N=1/1=1/0) NP2NT+1	CL=SURT(((T+1)-Y(1+7))**2)+((Y(I+3)-Y(I+9))**2)+	1 ((Y(1+5)-Y(1+11))**2))
		U				U								u	υ	,							υ				U I	υ	ر									U										
	294.0	\$657	- 102	2980	\$652	240	-100	202		100	506	307+	-90C	-000	110	312*	515		316.	317-	318*	119.	321-	322	323.	324+	325+	1020	120	329.	330+	331		3748	135e	* 955	955	.600	5404	-1+0	242		3454	3464	0110 1110	-0+0	350+	351e
	20200	00000	*****	00-05	00200	00 JU6	20507	01000	11200	1400	00514	00515	00516	01200	00516	1400	00520	12500	00523	00524	00525	00526	02500	05200	00531	00532	00532	25500	2000	00534	00535	00536	00540	0.1541	00542	90244	00546	99500	00547	005500	12200	00553	00554	00555	00560	00562	00563	00563

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: PAC 272023 73 CABK=6/CLO(NT) FEN(NT)=CCABK*(CL-CLO(NT)) FEN(NT)=CCABK*(CL-CLO(NT)=0.0 FFEN(NT)=CCABK*(CL-CLO(NT)=0.0 FFEN(NT)=FEN(NT)=(T(1+2))-T(1+1))/CL T(K1+3)=FEN(NT)=(T(1+2))-T(1+1))/CL T(K1+3)=FEN(NT)=(T(1+2))-T(1+1))/CL T(K1+3)=FEN(NT)=(T(1+1))-T(1+3))/CL T(K1+3)=FEN(NT)=(T(1+1))-T(1+3))/CL T(K1+3)=FEN(NT)=(T(1+1))-T(1+3))/CL T(FE)=(T(1+1))-T(1+1))/CL T(FE)=(T(1+1))-T(1+1))/CL T(FE)=(T(1+1))-T(1+1))/CL T(T(1+5)-T(1+11))/CL T(T(1+5))-T(1+1))/CL T(T(1+5))-T(1+1))/CL T(T(1+5))-T(1+1))/CL T(T(1+5))-T(1+1))/CL CTHE=(T(1+1))-T(1+1)-C2)=CPH1=(T(1+4))-C2)=CPH1=STHE CTHE=CT(1)=STHE+(T(1+2)-C2)=CPH1=(T(T+4)-C2)=SPH1=STHE CREET(1)=STHE+(T(1+2)-C2)=CPH1=(T(T(1+4)-C2))=SPH1=STHE CREET(1)=STHE+(T(1+2)-C2)=CPH1=(T(1+4)-C2))=SPH1=STHE CREET(1)=STHE+(T(1+4)-C2)=CTHE CREET(1)=STHE+(T(1+4)-C2)=CTHE CREET(1)=STHE+(T(1+4)-C2)=CTHE CREET(1)=STHE+(T(1+4)-C2)=SPH1=STHE CREET(1)=STHE+(T(1+4)-C2)=SPH1=STHE CREET(1)=STHE+(T(1+4)-C2)=SPH1=STHE CREET(1)=STHE+(T(1+4)-C2)=SPH1=STHE CREET(1)=STHE+(T(1+4)-C2)=SPH1=STHE CREET(1)=STHE+(T(1+4)-C2)=SPH1=STHE CREET(1)=SSH1=STHE=CURABSICURD)-DGN(NP)=SPH1=STHE CREET(1)=STHE=CURABSICURD)-DGN(NP)=SPH1=STHE CREET(1)=STHE=CURABSICURD)-DGN(NP)=SPH1=STHE CREET(1)=STHE=CURABSICURD)-DGN(NP)=SPH1=STHE CREET(1)=STHE=CURABSICURD)-DGN(NP)=SPH1=STHE CREET(1)=STHE=CURABSICURD)-DGN(NP)=SPH1=STHE CREET(1)=STHE=CURABSICURD)-DGN(NP)=SFH1=STHE CREET(1)=STHE=CURABSICURD)-DGN(NP)=STHE CREET(1)=STHE=CURABSICURD)-DGN(NP)=STHE CREET(1)=STHE=CU DATE YAW – ALFA ROTION D(7)=(MHAA®DDALW-NAA®DAL1-DGAA®DAL2®ABS(DAL2)+FAL)/(ALIN+MHAA) SURGE – Z MOTION D (5)=(HHZZ*0D2%-HHZ8*0DBT1-NZ2*0Z1-NZ8*0BT1-DGZZ*0Z2*ABS(DZ2) 1 -0622=0bT2*ABS(DBT2)+WFZ+FZ)/(M8+M12Z) 0(6)=T(5) HEAVE —X MOTION D(1)=(#B-BXX-BXB-BXG+MHXX+DDXM-MHXB+DDBT1-MHXG+DDGM1-NXX+DX1 . —hXB+DBT1-NXG+DGM1-DGXX+DX2+ABS(DX2)+WFXX+FX)/(MB+MHXX) THE EQUATIONS OF MOTION FOR THE BUDY DDX1=(Y(1)-VELXB)/R UDX1=(Y(3)-VELYB)/R UDZ1=(Y(3)-VELYB)/R DDBT1=(Y(3)-VEL4B)/R DDGT1=(Y(11)-VEL6B)/R DDX1=0.0 DDX1=0.0 DDZ1=0.0 DDBT1=0.0 DDBT1=0.0 SWAY - Y NOTION BUOY DYNAMICS 0(2)=Y(1) υ υ o υ 0000 υu υu υu *06r *06r 192* 194 194 195 396* +00+ ***** 401+ 408+ 409+ *R65 •661 ÷90+

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			D	UT UTWALCS DATE 23	3027
00633	410+			D(6) = Y (7)	
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00643	4254			K1=((1+1)/2)-2	
C4000	420+	U		ELEMENT X MOTION	
**900	4270			U(I)=(+1(KI)+I(KI+3)+CLF(KI))/CM(KI)	
20045	+ 5d+	·			
14000	****	ر		CLEMENT T MOLION	
				D(1+4/)=(-F(4)+1/+1/+(K1+4)-CCF(K1+1)/CE(K1+1) D(1+4/)-(-/)/	
2000		U		ULIT 2 4011101	
0650	1111	•		U(1+4)=(-T(K1+2)+T(K1+5)+CLF(K1+2))/CM(K1+2)	
00651	キゴウゴ		81	D([+5)=Y([+4])	
00651	4354	U			
00653	436 4			CALL RUNGE (N.Y.D.A.R.M.K)	
00654	+ CO +		9		
00000			3	ALFA537,467(8) Drff-f3 //////	
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10657	4110	U			
00000	1420	,		IF(A.LE.5.0)60 TC 52	
00662	キワナナ			IF(J,EQ,12)60 TU 51	
90064					
0665	4004			60 TO 52	
00666	446*		51	CONTINUE	
00667					
0/900	* 0 * *				
10672				ACA(L)=D(1) ACY(L)=D(1)	
0673					
10674	452			BPIT(L)=Y(10)	
0675	4534			BROL(L) = Y (12)	
0676	4244			L=L+2	
0677	455.	4	22	CONTINUE	
0677	- 00 	0			
11500	# / C #	ر		LNSIABILITT LIMITER Do 60 t-1 to	
0103	#65#		60	IF(Y(1).6T.10000.0)60 TO 92	
0703	460+	J	•	THE CABLE ELEMENTS CANT SINK THRU THE BOTTOM	
0706	461.		1	D0 218 I=14,26,6	
11200	462*		218	IF(Y(I),GE,DEEP)Y(I)=DEEP	
91201	4634			IF(L.EQ.2049)60 TO 88	
0112			AA		
0717	4994	U	;		
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CALL DSTAT(X#V.#VMN.#VVR.#VSD) CALL DSTAT(ACX.AXMN.AXVR.#VSD) CALL DSTAT(ACX.AXMN.AXVR.AXSD) CALL DSTAT(ACL.AZMN.AZVR.AXSD) CALL DSTAT(BPL.BPM.JBVR.ABSD) CALL DSTAT(BPL.BPM.JBVR.BPSD) CALL DSTAT(BPL.BPM.JBVR.BPSD) ATTE(4.127)#VMN.AXMN.ATMN.AZMN.4DFM.BFMN ARITE(4.127)#VMS.AXSD.ATSD.AZSD.HPSD.PASD ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPSTN ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPSTN ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.ATSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AXSD.AZSD.HPTAR ARITE(4.127)#VYSD.AXSD.AXSD.AXSD.AXSD.AXSD.AXSD.AXSD.AX	<pre>Xwv(L)=TXwv(L-u)+Xwv(L)+Xwv(L+2)+Xwv(L+2))/5,0 ACX(L)=TACX(L-u)+ACX(L)+ACX(L+2)+ACX(L+2))/5,0 ACX(L)=TACX(L-u)+ACY(L-2)+ACY(L)+ACY(L+2)+ACY(L+4))/5,0 ACX(L)=TACZ(L-u)+ACZ(L-2)+ACZ(L)+ACY(L+2)+ACY(L+4))/5,0 ACZ(L)=TACZ(L-u)+ACZ(L-2)+ACZ(L)+ACY(L+2)+BPIT(L+4))/5,0 C 433 H0L(L)=TBPIT(L-4)+BR0L(L-2)+BR0L(L-2)+BR0L(L+2)+BR0L(L+4))/5,0 C C0HPUTF POWER SPECTRA 443 H0L(L)=TBPIT(L-4)+BR0L(L-2)+BR0L(L)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTF POWER SPECTRA 443 H0L(L)=TBR0L(L-2)+BR0L(L)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTF POWER SPECTRA 443 H0L(L)=TBR0L(L-2)+BR0L(L)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTF POWER SPECTRA 443 H0L(L)=TBR0L(L-2)+BR0L(L)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTE POWER SPECTRA 445 H0R0L(L-2)+BR0L(L)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTE POWER SPECTRA 445 H0R0L(L)+BR0L(L+2)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTE POWER SPECTRA 445 H0R0L(L-2)+BR0L(L)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTE POWER SPECTRA 445 H0R0L(L-2)+BR0L(L-2)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTE POWER SPECTRA 445 H0R0L(L-2)+BR0L(L-2)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTE POWER SPECTRA 445 H0R0L(L-2)+BR0L(L+2)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTE POWER SPECTRA 445 H0R0L(L-2)+BR0L(L+2)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTE POWER SPECTRA 445 H0R0L(L-2)+BR0L(L+2)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTE POWER SPECTRA 445 H0R0L(L+2)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTE POWER SPECTRA 445 H0R0L(L+2)+BR0L(L+2)+BR0L(L+4))/5,0 C 20HPUTE POWER SPECTRA 445 H0R0L(L+2)+BR0L(L+2)+BR0L(L+4))/5,0 C 20H0TR 445 H0R0L(L+2)+BR0L(L+2)+BR0L(L+2)+BR0L(L+4))/5,0 C 20H0TR 445 H0R0L(L+2)+BR0L(L+2)+BR0L(L+2)+BR0L(L+4))/5,0 C 20H0TR 445 H0R0L(L+2)+BR0L(L+2)+BR0L(L+2)+BR0L(L+4))/</pre>	C MAVE HEIGHT CALL FFOUR(DATA.COSI.ISCH.0.BIN1) CALL FFOUR(DATA.COSI.ISCH.0.BIN1) CALL FFOUR(DATC.COSI.ISCR.0.BIN1) CALL FFOUR(DATC.COSI.ISCR.0.BIN1) CALL FFOUR(DATC.COSI.ISCR.0.BIN1) CALL FFOUR(DATF.COSI.ISCR.0.BIN1) CALL FFOUR(DATF.COSI.ISCR.0.BIN1) CALL FFOUR(DATF.COSI.ISCR.0.BIN1) CALL FFOUR(DATF.COSI.ISCR.0.BIN1) CALL FFOUR(DATF.COSI.ISCR.0.BIN1) CALL FFOUR(DATF.COSI.ISCR.0.BIN1) CALL FFOUR(DATF.COSI.ISCR.0.BIN1) CALL FFOUR(DATF.COSI.ISCR.0.BIN1) CALL FFOUR(DATF.COSI.ISCR.0.BIN1) CALL FFOUR(DATF.COSI.ISCR.0.BIN1)	UATS(I)=DATS(I)+DATA(I) UATSS(I)=DATS(I)+DATA(I) UATSS(I)=DATSS(I)+DATC(I) UATSS(I)=DATSS(I)+DATC(I) UATSS(I)=DATSS(I)+DATE(I) UATSS(I)=DATES(I)+DATE(I) UATSS(I)=DATES(I)+DATE(I) HAUN-SQUAL NRUNARUNAI PHS(I)=PHS(I) PHS(I)=PHS(I) PHS(I0)=PHS(I) PHS(IN)=PHS(IL) CO 400 NN=IPHS(IL) PHS(IN)=PHS(IL) CO 400 NN=IPHS(IL) CO 400 NN=IPHS(IL) PHS(IN)=PHS(IL) CO 10 SOO	450 D0 411 [1=1-306 DAT(1)=DATAS(1)/8.0 UAT(1)=DATS(1)/8.0 DATC(1)=DATS(1)/8.0 DATC(1)=DATS(1)/8.0 UATE(1)=DATS(1)/8.0 411 DATF(1)=DATS(1)/8.0 UDATF(1)=DATS(1)/8.0 1P=1/2 FP=1/2 FP=1/2 FP=1/2 A +DATA(1+1)+DATA(1-2)+DATA(1-1)+DATA(1) 1 +DATA(1+1)+DATA(1-2)+DATA(1-1)+DATA(1)
00000000000000000000000000000000000000	1 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4		00000000000000000000000000000000000000	22222222222222222222222222222222222222
00720 00721 00722 00724 00724 00725 00725 00725 00745 00745 00745	00761 00762 00765 00764 00766 00766 00770 00773 00773		01005	01027 01035 01035 01035 01035 01035 01045 01045 01045 01045 01045 01045 01045 01045

* 230272 PAGE acxC=(0ATB(I=J)+0ATB(I=Z)+0ATB(I=J)+0ATB(I)
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DATE 230272 PAGE CALL LINESG(2,153,FR0,PA2) Gall PAGE6(2,0,11,1) 60 T 130 Call TILEG(2,9,9HFREQUENCY,3,8HUB LEVEL,14,14HPITCH SPECTRUM) Call LINESG(2,153,FR0,PPT) Call LINESG(2,0,1,1,1) Call TITLEG(2,9,9HFREQUENCY,8,8HUB LEVEL,13,13HROLL SPECTRUM) Call LINESG(2,153,FR0,PRL) Call Extre(2,153,FR0,PRL) Call Extre(2,153,FR0,PRL) Call Extre(2,0,1,1,1 0 *DIAGNOSTIC* MESSAGE(S) END UF UNIVAC 1108 FORTRAN V COMPILATION. 01165 01165 01165 01165 01165 01170 01177 01173 01175 01177 01177 01177 01201 01201

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0001 +CUDE 0000 +UATA 0002 +BLANK	000452 200000 000000					
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0003 AL0610 0004 NEXF65 0005 AL06 0006 EXP 0007 Sunt 0007 Sunt						
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0001 000235 402L 0000 R 000255 402L 0000 R 000055 6 0000 R 000055 6 0000 R 000055 0M
0001 000061 304L 0000 R 000042 DURMIN 0000 R 000051 FP2 0000 R 000054 IS 0000 R 000024 SPEC
0001 000321 1676 0000 R 000012 DDVEG 0000 R 000650 FP1 0000 000134 INJP S 0000 R 000044 P3PARL
0001 00033 1616 0000 R 000053 8PAR 0090 R 000045 8PAR 0006 R 000046 H3PARL 0000 R 000045 P3PAR 0000 R 000045 P3PAR
0001 000207 1436 0000 k 000053 ffar 0000 k 000037 ffar 0000 k 000047 43par 0000 k 000047 45par 0000 k 000041 Tfar

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+1 FOR TORBU.TCRBU Cuivac 1108 Fortrain V Level 2206 U023 Timis cumpliantum #AS Jone on 23 Feb 72 at 12:44:06

ENTRY POINT 000340 SUBRUUTINE TORBU

STOMAGE USED (BLOCK, RAME, LENUTH)

EXTEMMAL REFERENCES (BLOCK, NAML)

SIN CUS Surt Atan Nearas 0003 0004 0005 0005 0005

STOMAGE ASSIGNMENT FOR VARIABLES (BLOCK, TYPE, RELATIVE LOCATION, NAME)

0001 000245 19L 0000 R 000012 ALF 0000 R 000002 06AM 0000 R 000004 RS 0000 R 000004 RS
0001 000231 18L 0001 000016 9L 0000 R 000016 BXT1L 0000 I 000000 J 0000 R 000015 X
0001 000220 16L 0001 000132 8L 0000 R 000014 ARTY 0000 000041 INJP \$ 0000 R 00007 VOLX
0001 000225 11 0001 000235 31 0000 - 000011 24 0000 000013 48 0000 000013 48
0041 000145 10L 0041 040275 20L 0404 0400013 Am 0044 0400001 AAm 0344 4000001 AAm 0344 4000001 AAm

SubkOUTINE TUNGUTH.TL.BXX.BTT.BXT.BTT.BIX) C FUR THE 8 FOUI DIA. HICHARDSON BUOY USED IN THE BUOY WOTION C FOR THE 8 FOUI DIA. HICHARDSON BUOY USED IN THE BUOY WOTION C EXPENTMENT C L=1.570B 06AH=0.1 HIE2.75 C6H=1.202 VOL=0.0 9 COLLEO.0 9 COLLEO.0 HETH-(ZCBH-H)*(1.0-COS(T1L))+RT*SIN(T1L)*SIN(9AW) If(J.EC.1)HETH If(HB.LE.0,0)HETO.0 If(HB.LE.0,0)HETO.0 If(HB.GE.(1,99*RS))GO TO B If(HB.GE.(1,99*RS))GO TO 1 ARTSGRTI(RS*22)/(RS-HB)**2))-1.0) ARTSGRTI(RS*22)/(RS-HB)**2))-1.0) ALF=2.0*ATAN(ART) ALF=2.0*ATAN(ART) 000

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AK=0.5*(KS**2)*(ALF=SIN(ALF))	60 TU 18 A CONTINIE	IF(H8.6T.(1.01*RS))60 T0 10	AKEL.5708+KS+KS	GO TU 18	10 ARTY=((RS**2)/((RS-(2.0*KS-HB))**2))-1.0	IF(ARTY.LE.0.0)ARTY=0.0	ART=SURT(ARTY)	IF(Aht.GE.25.0)60 TO 16	ALFEZ.OCATAN(ART)	AK=3.1416*(R5**2)=0.5*(R5**2)*(ALF=SIN(ALF))	60 TU 18	16 AH=1.5708+(R5+*2)	G0 T0 18	11 AR=3.1416*(K5**2)	18 [F(J.EQ.1)60 TO 3	60 TU 19	3 BXX:402.124+KT+AR	J=2	60 T0 9	19 VOL=VUL+UGAMekT*AR	X=RT=SIN(GAM)	VOLX#VOLX+X#UGAM#KT#AR	GAMEGAM+UGAM	IF(GAM.GE.1.57)GO TO 20	40 T0 9	20 BXT1L=128.0*VOL	UXT=BXTIL-bXX	C THE RIGHTING ARM	XM=(VOLX/VUL)+COS(TIL)-(ZGBH-H)+SIN(TIL)	bTT=dXTILeXM	UTX=EXT*AM	RETURN	END	
*				:	•	:	•	<u>.</u>	*		•		*		40	*	•	<u>.</u>	*	•	•	•	•	•2	•		•		•		*	*		!
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ERU OF UNIVAC 1108 FORTRAN V COMPILATION. 0 «DIAGNOSTIC« MESSAGE(S)

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12:44: 8.128

L FON MUNUE AUME UNIVAC 1104 FORTHAN Y LEVEL 2206 U023 THIS COMPLATION #AS DONE ON 23 FEB 72 AT 12:44:08

ENTRY POINT 000134 SUBROUTINE RUNGE

STORAGE USED (BLOCK, NAME, LENGTH)

EXTERNAL REFERENCES (BLOCK, NAME)

0003 NEHR25 0004 NEHR35

STORAGE ASSIGNMENT FOR VARIABLES (BLOCK, TYPE, RELATIVE LOCATION, NAME)

000117 9L 000117 9L
1000
0001 000054 1206 0001 000074 7L 0000 R 000000 G
0001 000023 1076 0001 000036 5L 0000 000075 INJP S
0001 000121 10L 0001 000032 4L 0000 1 000052 1
0001 000021 1L 00u1 000030 3L 000c r 000063 a

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12:44: 9.365 2000 R 000001 S 22 UATE 230272 PAGE 0000 I 00000 N 0 *DIAGNOSTIC+ MESSAGE(S) STORAGE ASSIGNMENT FOR VARIABLES (BLOCK, TYPE, RELATIVE LOCATION, NAME) 0000 I 00003 7 SUBROUTINE DSTAT(PAR.PRMN.PRVR.PRSD) DIFENSION PAR(2048) N=1024 S=0.0 S=0.0 D0 3 _____2047.2 S=S+PAR(J) PAR(J) *PAR(J) PRVR=(S1-PRMNeS)/N PRVR=(S1-PRMNeS)/N PRVR=(S1-PRMNeS)/N RETURN RETURN 41 FOR USTAT+DSTAT Unival 1100 Furtran V Level 2206 0023 This cuapilation #as uone on 23 Feb 72 at 12:44:09 000002 INJPS END OF UNIVAC 1108 FORTRAN V COMPILATION. ENTRY POINT 000045 STOMAGE USED (BLOCK, NAME, LENGTH) EXTERNAL REFERENCES (BLOCK, NAME) 0000 **BUOY LYNAXICS** 000062 000017 000000 +CODE +JATA +BLANK SURT NERR35 0001 000012 1106 0000 R 000002 51 SUBROUTINE USTAT n 5000 4000 0001 0000 0002 - NM 75 0 A 0 0 1 1 1 1

00101 00105 00105 00107 00112 00113 00115 00115 00115 00115 001210

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GA FUM FFOUR,FFOUR Unl.ac 1106 Fontran V LEVEL 2206 0023 This cumpleation was Jone on 23 FEB 72 at 12:44:10

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SUBHUUTINE FFOUR ENTRY POINT 000322

STURAGE USED (BLOCK, NAME, LENGTH)

EXTERMAL REFERENCES (BLOCK, NAME)

0003 UNSCR 0004 NENR33 STOHAGE ASSIGNMENT FOR VARIABLES (BLOCK, TYPE, RELATIVE LOCATION, NAME)

0001 000143 30L 0001 000153 91L 0000 000015 E 0000 1 000005 LINC 0000 R 000010 VINC
001 000257 1706 001 000024 80L 000 R 000013 CSE 000 I 000013 12 000 I 000003 TINC
000175 1536 00 000041 60L 00 000001 A 00 000021 INJP\$ 00 0000021 INJP\$ 00
0000 00000 00000 00000 00000
001 000021 194 001 000023 494 001 000225 944 000 1 000006 1140Ex 000 1 000006 1140Ex

SUBROUTINE FFOUR(X,C,ISCR,T,BIN,J) Complex F,x(1),A	INTEGER T, TINC, THTA, BIN	DIMEMSION CSE(2) (1) ISCR(1)	EQUIVALENCE (E.CSE)	2 TINC=BIN/T	LINC=T/2	0 FOC=FINC	O THTASO	0 CSE(1)=C(THTA+1)	INDEX=BIN/4+1-THTA	CSE(2)=-C(1NDEX)	0 LOC=LOC+1	LOC1=LOC-LINC	E#E+(X(LOC1)-X(LOC))	x(roc1)=x(roc1)+x(roc)	X(LQC)=E	THTA=THTA+TINC	IF(THT3-61N/2) 20,30,30	0 IF(THTA-BIN/4) 80.50.50	0 INCEX=BIN/2+1-THTA	CSE(1)=+C(INDEX)	INDEX=THTA-(BIN/4-1)	CSE(2)=-Č((ĨŇŬĔX)	G3 T0 60
*		*7	*3	4-5 7	7.0	4 U	37 4 (5)	10+ 8	31.0	12*	k3+ 6	144	15*	16*	17.	18.	19=	20*	21+	220	23*	240	25e
10100	10100	00105	00106	00107	00110	TITOC	00112	0113	41100	00115	00116	00117	00120	12100	00122	00123	00124	00127	00132	20133	90134	00135	00136

FF9 FF10 FF10 FF15 FF15 FF16 FF16 FF16 FF16 FF16 322

FF23

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FAGE FF20 FF20 FF700 FF FF36 230272 DATE 0 *DIAGNOSTIC* MESSAGE(S) 30 If.LOC-T) 90.91.91 90 LOC=LOC+LINC 60 T0 40 91 FF(2=LINC) 92.93.94 72 LINC=LINC, 92.93.94 92 FF(2=LINC) 92.93.94 93 D0 100 LUC=2.1.2 100 T0 LUC=2.1.2 100 LUC=2.1.2 100 LUC=2.1.2 100 LUC=2.1.2 100 LUC=10 100 LUC=10 100 LUC=2.1.3 100 LUC=10 100 3 I=2.12 00 3 I=2.1 END OF UNIVAC 1108 FORTRAN V COMPILATION. BUOY DYNAMICS

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0ATE 230272 PAGE 25 12:44:12, 95			L 20000 1 0000 2 24.MI _00000 0000		
BUOY DYNAMICS Ga for Gnscr,gnscr Univac 1106 foathan V level 2206 0023 This compilation #as done on 23 feb 72 at 12:44:12	SUBHOUTINE GNSCR ENTRY POINT 000062 Sto446E USED (dlock, na4E, length) 2001 *Code 000075 0000 *UATA 000024 0002 \$dlank 000000	EXTERNAL REFERENCES (BLOCK, NAME) 0003 Nerrjs	STORAGE ASSIGNMENT FOR VARIABLES (BLOCK, TYPE, RELATIVE LOCATION, NAME) 0001 000013 11L 0001 000020 1116 0001 000046 BL 0000 1 000001 M 0000 1 000002 N 0000 1 000000 TM	00101 1. SUBROUTINE GNSCRISCR.T) 00103 2. SUBROUTINE GNSCRISCR.T) 00105 4. SCRI1)=1 00105 5. N=1 00105 5. N=1 00116 7. N=7/2 00116 7. 11 00 9 4=1,M 00116 10. 9 9 5CR(N)=TM+SCR(U) 00112 10. 10 4=1,M 00112 11. 10 4=4,M 00112 12. 10 4=4,M 00112 12. 60 70 11 00122 12. 60 70 11 00122 12. 60 70 11 00122 12. 60 70 11	CND OF UNIVAC 1108 FORTRAN V COMPILATION. 0 «DIAGNOSTIC« MESSAGE(S

. . . 1 1 1 ;;;; ; ; : i 12144113.208 , ł SANI 100000 ; : ; i 1 . i • 0000 ; i ļ ; 35 ļ ; i • ł ł ł 1 ī ŗ DATE 230272 PAGE ļ : ;;;; . ļ . 1 20000 1 0000 i ; ŧ FF62 FF63 FF65 1911 1911 1911 1 1 t I ļ . : ł 0 *DIAGNOSTIC* MESSAGE(S) 1 STORAGE ASSIGNMENT FOR VARIABLES (BLOCK, TYPE, RELATIVE LOCATION, NAME) 0000 C 000000 E ł i GA FUR UNSCR,UNSCR Univac 11cg Fortran V Level 2206 0023 This compilation #AS Uyne on 23 Feb 72 at 12:44:13 SUBROUTINE UNSCR(X,SCR,T) COMPLEX E,X(1) INTEGER T,SCR(1) D0 9 J=1.T I=SCR(J) I=SCR(J) I=SCR(J) I=SCR(J) X(1)=E X(1)=E V(1)=E V(1)=E FCUNN FETUNN END END OF UNIVAC 1108 FORTRAN V COMPILATION. 0001 000030 9L ENTRY POINT 000047 • : STORAGE USED (BLOCK, NAME, LENGTH) EXTERNAL REFERENCES (BLOCK, NAME) BUOY DYNAMICS • 000061 000023 000000 ł I į ı ÷ +CODE +DATA +6LANK 0001 000012 1066 0000 1 000002 J NERRJS SUBHJUTINE UNSCR 0003 00.01 0000 0002 *** ****** 1 00101 00103 00105 00110 001114 001114 001114 001114 001117 001127 , .

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SUBROUTINE COSINE ENTRY POINT 000126

STORAGE USED (BLOCK, NAME, LENGTH)

CATERNAL REFERENCES (BLOCK, NAME)

0003 RND 0004 UCOS 0005 DSIN 0006 NERR3\$

STORAGE ASSIGNMENT FOR VARIABLES ABLOCK, TYPE, RELATIVE LOCATION, NAME)

0000 0 000045 1MDEX 0000 0 000004 SN
0000 D 000006 CSJ 0003 R 000000 HND
0000 D 000002 CS 0000 I 000014 J
0000 D C00000 ANG 0000 I C00012 INTB
001 000060 1206 000 000030 INJP 5 000 D 000010 SNJ

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4 • • • •	SINE ### IO/14/70 ### Å.H.NUTTALL Sing(C-BIN)	ION ANG.CS.SN.CSJ.SNJ			0717958648D0/BIN		1								U=SN	JeCS	* * * * * * * * * * * * * * * * * * *	()	D(SNJ)		F
	C *** SUBROUTINE CO SUBROUTINE CO	UQUBLE PRECIS	INTEGER BIN	DIMENSION C(1	ANG=6.2831853	INTB=BIN/B	INDEX=2+INTB+	CS=DCOS (ANS)	SN=DSIN(ANG)	CSJ=1.000	SNJ=0.000	C(1)=1.0	C (INDEX)=0.0	D0 9 J=1.INT8	ANG#CSU+CS-SN	Skulicsuesness	CSUEANG	C ()+1)=RND (CS	ATHUT ANDEX-LUNAR	RETURN	END
ł	48	•	* 7	5 *	•	**	8	•6	10+	11+	12+	13*		15+	10.	170	180	19*	20+	21+	-22
	00100	00103	00104	00100	001/00	20100	00110	C0111	00112	00113	00114	00115	00116	00117	00122	00123	00124	00125	00126	00130	15100

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END OF UNITAR 1100 FORTRAN V COMPLEATION. T DEDIAGNOSTICE MESSAGE (S)

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; ! ļ 12:44:15.517 REGIS WITH MNEWIS FOR X=RND (DX 1 L.C. SKIP NI IF RUMD UP No ROUND UP, EXIT A2 C== 0 SKIP NI IF H = D'C GENERATE DATA UNDER STHULE PRECISION DX C C++ LINGREMENT C C++ C+1 M C++ 40000000000 A4 C++ C+M JUMP IF DX < 0 SKIP NI IF ROUND INCREMEN 0007777 ŝ POINT Xers Trail XOdS ----SPOX SOCIATE No Dan ₹. ŧ, JUST NUS Ų 4 ş X I Ì 1 ī. i. A1.(004000.0) Fin A4. (0400000.0) A1, (004000,0) FIN ł A0++0.X11 X042.44 44,1 44,40 2,X11 44,573X 44,573X 44,40 2,X11 AO , NEG 12, 44 A2,A4 0.5 8419 A3+9 FIN3 FIN2 Z A3+1 0 REGNAM SA C าวรัฐรา + 2 **4**4 \$(1) RND+ FIN FIN2 FINI FINS \$(0) \$0X NEG 000014 00000 0 040000 000000 400000 000000 81 7777777000 88 8 88 8 n # n o # # ð 300 8 33 8 32 23288220002 20 1000000 000000 í 8 000035 000037 000037 000037 000035 000035 000035 000035 000035

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Buoy System Dynamics for the Torroidal Buoy Used in WHOI Mooring No. 238

This program (figure B-4) is basically the same as the previous program except that the mooring line forces and dynamics are simulated with the finitediffer ence method described in chapter III of the main text. Subroutine "MOOR" (figure B-5) takes the spatial values of the six variables (strain, two angles, and the velocity components) describing the cable motions and updates them for the next time step. The tension and angles at the top of the cable are then used to compute the mooring line forces acting on the buoy.

For numerical stability, the value of the tensile wave characteristic should never exceed the quotient of the nodal spacing, H, and the time step, K, in the subroutine:

$Ch_1 < H/K$.

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The list of input values is the same as that for the previous program. In this particular program, the mooring line is composed of two segments, and the cable weights, masses, etc. are changed at a cable length of 4800 ft. There are 20 nodes spaced 400 ft apart to simulate dynamics of an 8,000-ft mooring line.



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F₁, ure B-4. Torroidal Buoy WHOI Mooring No. 238 Dynamics Simulation Flow Chart



Figure B-5. Finite-Difference Cable Dynamics Simulation Flow Chart

į I0:50:52.667 i : 001 AMP AMP AMP AMP AMP AMP AMP AMP CMZ CMZ CMZ CMZ 1085 1225 2116 3716 1 010527 00020 ļ ł ŧ i I ł ł n ł ı. PAGE ł 010572 i S 1 ł . UATE LOCATION, NAME. ł 1 ł ł : I ļ RELATIVE G FOR SIZ19,51279 Univac 1108 Fuktran V Level 2206 0023 This compilation #as done on 01 may 72 af 10:50**:5**2 VARIABLES (BLOCK, TYPE, NAME, LENGTH) (BLUCK, NAML) BUOT CTAANICS 0000000 010637 000000 FOK (dLOCK, •CODE •UATA •BLANK HEFERENCES REAVE TORBU TORBU DSTAT DSTAT DSTAT SURCS SURT SURT SIR SIR SIR STOPS STOPS ASSIGNMENT 010515 010531 010543 000164 STORAGE USED MAIN PHCukAM 6001 6000 0002 EXTERNAL STORAGE

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i ł I i ł I I i ÷ ŝ DATE 010572 PAGE FOGWATIGFE,5) Read in the Buoy Parameters Buoy Radius, MT. Of C.Of G. From Moor Point and the Length of the Moor Point Belom the Bottom of the Buoy READ THE WEAN WIND CAUSING THE SEA STATE (WHICH MAY BE DIFFERENT Than the wind acting during the simulation) and its duration Read 100.winds.adur Call Rwave(Winds.adure THE CURRENTS IN THE WATER COLUMN Real the Y and 2 components of the surface current Read 108.CTS.C2S If the current varies with depth. Insert the functions for Strength and direction READ 109, BR & XGG, XML INEATIAS INEATIAS READ 107, BUM BU ALLIN, BTIN, GMIN READ 107, WOD BASG COEFFICIENTS AND AREAS REAG 107, WCD, WCLAREA, WAREL, WCPHT THE WATER AT 40 DEG F ł FORMAT (11H BUCY SINKS) FOR A UNIFORM CURRENT CT=CTS CZ=C2S (6F10.2) (9H UNSTABLE) (3F10.4) (8F8.2) THE VISCOSITY MUE2,735E-5 AMAT(SF10.4) FORMAT(1F40.4) FORMAT (1F50.4) FORMAT (3F10.5) (2F10.4) (1F20.4) (1F30.4) BUOT CYNAMICS 1 RMAT (9H 1 FORMAT ł RHAT FORMAT FORMAT CRWAT υυυ U 000 υu υ 0000 84* 85* 85* 86 å -19 •?9 53. å 10 2 3~ **** 878 878 205 205

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DATE 010572	XX.688.8X8788X)	BY,VBY,NuY,STRN,PH1,THE) 0664) 1)) +COS(THE(1)) 1)) SIN(THE(1)) 1)) +SIN(THE(1)) 1)) +COS(T(12)) +FX+SIN(T(10)) +COS(T(12))) 10) +COS(T(12)) +FX+SIN(T(10)) +COS(T(12)))			OTION FO ^R THE BUOY K6+MHXX*DDX _N =MHX8*DDB11=MHXG*DDGM1=NXX*DXX 1=DGXX*DXX23 <u>*A</u> B5(DX2) <u>+MFX+FX)/(M</u> B+MHXX)	YG+DDGM1_NY +DY1_HYG+DGM1-DGYY+DY2+ÅBS(DY2) 42) +#FY+FY) / (48+MHYY)	20+DDBT1"NZ2+DZ1-NZ8+DBT1-DGZ2+DZ2+ÅB5(D22) f2)+#f2+ ^t 2)/(M4+MHZ2)	14+DAL1-UGAA+DAL2+ABS(DAL2)+FAL)/(ALIN+MHAA)	v <#DDX1-#HBZ*D0Z1+MHBB*D0BT#-MBX*DX1-MBB*DBT1 *ABS[DZ2]-D68B9#D8T2=AB5[DBT2]+WFBT+FBT]/
UOT DYNAMICS	CALL TORBU(H, b11) CALL TORBU(H, 6M1) CALL TORBU(H, 6M1) MOORING LINE TENS MOORING LINE TENS VELOCITY COMPONEN VECABES:0+R	VBY=Y(3)-XCG*Y(1 WBY=Y(5)-XCG*Y(9) Call Mook(a.kCA5, OTDE=6.87E5 D0 83 I=1.21 IF(1.6E.14)DTDE=5.3	3 TEU(I)=D10E+STRN(FXTIN(I)+COS(PHI FXTEN(I)+COS(PHI FZTEN(I)+SIN(PHI) FZTEN(I)+SIN(PHI) FZTEN(I)+SIN(PHI) FAL=0+0 FGM=-XCG+(FYECOS)	0 CONTINUE 00X1=(Y(1)-VELXB), 00X1=(Y(3)-VELXB), 00X1=(Y(5)-VEL2B), 00B11=(Y(9)-VELBB),	UDGMI=(Y(11)-VELG DDX1=0.0 DDX1=0.0 DDX1=0.0 DDR1=0.0 DDR1=0.0 DDGM1=0.0	THE EGUATIONS OF P Heave	SWAY - Y MOTION D(3)=(MHYY#DDYW=MH D(3)=(G40GM2#ABS(D6 D(4)=Y(3)	SURGE - Z MOTION D(S)=(MHZ2+DU2W-MH D(S)=(MHZ2+DU2W-MH D(S)=Y(S) D(6)=Y(S)	YAH <u>- ALFA HOTION</u> U(T)=(MHAA+DDALW-h D(8)=Y(7)	PITCH - BETA MOTIC 0(9)=(=BBX=BBb=MHB 1 - NGZ=DZ1=DG5L=DZ2 2 (BTIN+MHBB) 0(10)=7(9)

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BUOY DYNAMICS	Roll- Gamma motion D(11)=(-6Amma motion) D(11)=(-86X-866-mmGX*0DX1-mm6f*0DY1+mm66*0D6MNGX*0X1-NGT*0PY1 1 -N66*0GM1-U66Y*0DY2*ABS(UY ²)-D666*0GM2*ABS(D6M2)+MF6M+F6M)/ 2 (6M1N+Mm66) 2 (6M1N+Mm66)	CALL RUNGE (M.Y.C.A.R.M.K) 60 T0 (50.40).K 40 Alfas57.4*7(10) 6AMMAE57.4*7(12) 12-41 15(J.E0.20160 T0 72	72 TENB(N)=TEN(1)/1000.0 AWY(1N)=DDXM AWY(1N)=DDXM AWY(N)=DDXM ACX(NN)=D(1) ACX(NN)=D(1) ACX(NN)=D(2) BP11(NN)=T(10) BP11(NN)=T(10) BP11(NN)=T(12) ACX(NN)=D(2) ACX(NN)=D(2) ACX(NN)=D(2) ACX(NN)=D(2) ACX(NN)=D(2) ACX(NN)=D(2) ACX(NN)=T(12) ACX(N	73 CONTINUE INSTABILITY LIMITER D0 60 1=1,M 60 1F(TN).6E.10000.0)60 TO 92 1F(NN.EG.20000.0)60 TO 92 61 CALL DSTAT(AMV.AMVR.AVB) CALL DSTAT(AMV.AMVR.AVSD) CALL DSTAT(AMV.AVVR.AVSD) CALL DSTAT(ACY.AVVN.AVYR.AYSD) CALL DSTAT(ACY.AVVN.AVYR.AYSD) CALL DSTAT(ACY.AVVN.AVYR.AYSD)	CALL DSTATRACTACKTASTON MATTE (4.100) TEMN, BRVR, BSD) CALL DSTATRACTARC, BRMN, BRVR, BSD) MATTE (4.100) AWNN, AWVR, WSD) MATTE (4.100) AWNN, AWVR, WSD) MATTE (4.100) AWNN, AWVR, ANSD MATTE (4.100) AWNN, AWVR, ANSD MATTE (4.100) BPNN, BRVR, BSD MATTE (4.100) BPNN, BRVR, BSD MATTE (4.100) BPNN, BRVR, BSD MATTE (4.100) AWN, BRVR, BSD MATTE (4.100) AWNN, BRVR, BSD MATTE (4.100) AWNN, BAVR, BSD MATTE (4.100) AWNN, BAVR, BSD MATTE (4.100) AWNN, BAVR, BSD MATTE (4.100) AWNN, BAVR, BSD MATTE (4.100) AWNN, BAVR, BRSD MATTE (4.100) AWNN, BAVR, BSD MATTE (4.100) AWNN, AWNN
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72 PAGE 11	•	:						0061 304L 0042 DURMIN 0051 FP2 0054 IS 0024 SPEC	, , ,			1 1 1 1							
0ATE 0105	:						1 1 1 1	0000 R 00 0000 R 00 0000 R 00 0000 R 00 0000 R 00 0000 R 00		4 WAVE		7) "AHP(10)							
								000321 1676 000012 DOMEG 000050 FP1 000134 INJP5 000134 P3PARL	,	4EG.AMP.PHS) Compute the Mean	JOYS	SPEC(10), PHS(10)							
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	3 72 AT 10:50 [:] 57	000417		ł		·	-	000303 1616 000653 8PAR 000043 FPARL 000046 H3PARL 000046 H3PARL 000045 P3PAR 000040 TPARL		AVEIDER WAVE ME	D E FETCH FOR DE	101 , MOMEG (10) , D		KSC ∕(WINDS++2)	25.ALOG10(FPAR)	CH/ (WINDS+3600.) 35,304,304) CASE 3600.0/FETC ^H 541.06.0/TETC ^H		,283+AL0610 (FPA) \RL P3PAR=0,0247
GYNANICS	EVEL 2206 002. Ne un 01 may '	ENTRY POINT	PAME, LENGTH	000452 000173 00000	(BLOCK. NAME)		- Or variables	××××× 10000000 000000000000000000000000		BROUTINE RWAVE E THE BRETTSCH	JIGHT AND PERIC	MENSION OMEG() AI MOMEG	TCH=6.08E5	ARE32.24FETCH	ARL=1.477-0.2	RMIN=TPAR+FET((DUR-DURMIN) 3(RATION LIMITEC Aredurewinuse: Abi - 5 - 5 - 5 - 5	ARELO OFFFAR	PARL=+1.136+0. PAR=10.0**P3P/ (FPAR.LT.0.02)
RU01	VE RAN V L #AS DC	AVĊ	(BLOCK,	-COUE -DATE -BLANK	RENCES	ALOGIO NEXP65 ALOG SGRT NER35	WHENT F	7 1436 7 544 7 5948 7 6948 7 6948 7 6948 7 6948 1 1948		ns S	Ψ¥	101	2		44	34	203 1P	L'01. 1.11.	304 PJ
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<pre>compress to an entropy and the second of Pan i</pre>	c1=	IF(FPAR,GT,1.45)P3PAR=2.0		
<pre>c</pre>		H3PARL=-2.5+0.415+AL0610 (FPAR)		
<pre>c</pre>	***~	M3PAR=10.0++H3PARL		
<pre>c refract straights will fulles For A 9 conportent WAVE MODEL refract straights will straights refract straights will straight straights refract straights will straight straights refract straights will straight straights refract straights will straight straights refract straights will straight straight straights refract straights will straight straight straight straights refract straights will straight str</pre>	25 *	IF (FPAR.LT.0.016)H3PAR=0.00057		
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1 FF2:22:25:275:FF2:40:105:45:10 1 FF2:22:25:275:FF2:40:105:45:10 1 FF2:22:25:FF2:40:105:45:10 1 FF2:22:25:FF2:40:105:45:10 1 FF2:22:25:FF2:40:105:40:40:10 1 FF2:22:25:FF2:40:105:40:40:10 1 FF2:25:FF2:FF2:FF2:FF2:FF2:FF2:FF2:FF2:F		COMPLETE EDEGLIENCIES AND AMPLITUDES	S FOR A 9 COMPONENT WAVE MODEL	
0.0 FF2=3.5.50=FU(0.5.50=41005) 0.0 0.0 15:1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1	-9-	FP1=32.2**HTM/(FINDS*2)		
1.1 PARE=10.6754(132.22/(41005.6P2)).0004) 1.2 PARE=10.6754(132.22/(41005.6P2)).0004) 1.2 CELO.0755 1.3 CELO.0755 1.4 CELO.0755 1.5 CELO.0115 1.5<	30*	FP2=32.2*PER/(6.2832*#INUS)		
Do 401 0.401 0.401 0.401 Do 401 0.401 0.401 0.401 Do 401 0.401 0.401 0.401 District 0.401 0.411 0.401 District 0.401 0.412 0.401 District 0.401 0.401 0.401 District 0.401 0.401 0.401 Distrin	•le	APAK=3.437+(FP1++2)/(FP2++ ⁴)		
0.0 0.0.1 15-10 0.0 0.0.1 15-10 0.0 0.066(1)5-0PAR/(ALO6(6)))*0.25 0.0 0.066(1)5-10-0A 0.0 0.000	32*	BPAK=-0.675*((32.2/(#INDS#FP2))**"		•
0: 0=10.0715 0: 0=10.0715 0: 0=10.0715 0: 0=10.0715 0: 0=10.0715 0: 0=10.0715 0: 0=10.0715 0: 0=10.0715 0: 0=10.0715 0: 0=10.0715 0: 0=10.0715 0: 0=10.0715 0: 0=10.0715 0: 0=11.2046 0: 0=15.12046 0: 0=055 0: 0=055 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: 0: <td>JJ*</td> <td>D0 401 IS=1+9</td> <td></td> <td></td>	JJ*	D0 401 IS=1+9		
0.0 0.00000000000000000000000000000000000	キオの	G=10.0/1S		
00 00 <td< td=""><td>35+ 40.</td><td>11 OMEG(15)=(-BPAR/(ALOG(G)))*+0.25</td><td></td><td></td></td<>	35+ 40.	11 OMEG(15)=(-BPAR/(ALOG(G)))*+0.25		
 402 ORECHOLOG 402 ORECHOLOG 404 ORECHICLE.0.001160 TO 402 414 EXERTILE.0.001160 TO 402 414 EXERCITIONEGE(11)-OMI 414 EXERCITIONEGE(11)-OMI 414 EXERCITIONEGE(11)-OMI 414 EXERCITIONEGE(11)-OMI 414 EXERCITIONEGE(15)-10MI 415 EXERCITION 415 EXERCITION 415 EXERCITION 416 EXERCITION 416 EXERCITION 416 EXERCITION 417 EXERCITION 418 EXERCITION 418 EXERCITION 418 EXERCITION 418 EXERCITION 419 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 410 EXERCITION 	36 *	OMEG(10)=530.0		
30 402 402 5FE(T=L037,0=APAR/(044=5))=EXP(EPAR/(044=4)) 401 5FE(T=L037,0=APAR/(044=5))=EXP(EPAR/(044=4)) 414 5FE(T=L037,0=APAR/(044=5)) 414 004E6(11)=C04 415 004E6(11)=C04 415 004E6(11)=C04 415 004E6(11)=C04 415 004E6(15)=104K6(15))=45) 415 00404 415 00404 416 15=10 417 15=10 418 15=10 419 404 415 15=10 410 415=10 411 415 412 416 413 416 414 416 415 410 414 410 415 410 410 410 410 410 410 410 410 410 410 410 410 410 410 410 410 410 410 410	s7*	0*0=0°0		
39. FFET:11037:000160 TO 402 41. FESPET:11037:000150 TO 402 42. DO 403 15:22:9 43. MOREG(15)=00406 (154.1) / 2.0 44. MOREG(15)=00466(154.1) / 2.0 45. MOREG(15)=00466(154.1) / 2.0 46. MOREG(15)=00466(154.1) / 2.0 40. MOREG(15)=00466(155.1) / 2.0 40. MOREG(15)=00466(155.1) / 2.0 40. MOREG(15)=00466(155.1) / 2.0 40. MOREG(15)=00466(155.1) / 2.0 40. MOREG(15)=0.055 40. MOREG(15)=0.055 40.	38+ 40	12 QM=OM+0.05		•
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41. XXHEG(11)=OM6(5:1)=OM5 42. XXHEG(11)=OM6(5:1)=OM5 42. XXHEG(11)=OM6(5:1)=OM5 42. XXHEG(11)=OM6(5:1)=OM5 42. XXHEG(15)=ZXHEG(15:1)=OM5 42. XXHEG(15)=ZXHEG(15:1)=OM5 42. XXHEG(15)=ZXHEG(15:1)=OM5 42. XXHEG(15)=ZXHEG(15:1)=OM5 42. XXHEG(15)=ZXHEG(15:1)=OM5 43. XXHEG(15)=ZXHEG(15:1)=OM5 44. XXHEG(15)=ZXHEG(15:1)=OM5 44. XXHEG(15)=ZXHEG(15:1)=OM5 44. XXHEG(15)=ZXHEG(15:1)=OM5 44. XXHEG(15)=ZXHEG(15:1)=OM5 44. XXHEG(15)=ZXHEG(15:1)=ZXHEG(15:1) 44. XXHEG(15)=ZXHEG(15:1)=ZXHEG(15:1) 44. XXHEG(15)=ZXHEG(15:1)=ZXHEG(15:1) 55. XXHEG(15)=ZXHEG(15:1)=ZXHEG(15:1) 55. YXHEG(15)=ZXHEG(15:1)=ZXHEG(15:1) 55. YXHEG(15)=ZXHEG(15:1)=ZXHEG(15:1) 55. YXHEG(15)=ZXHEG(15:1)=ZXHEG(15:1) 55. YXHEG(15)=ZXHEG(15:1) 55. YXHEG(15)=ZXHEG(15:1) 55. YXHEG(15)=ZXHEG(15:1) 55. YXHEG(15)=ZXHEG(15:1) 55.	40+	IF(SPECT.LE.0.001)60 TO 402		
42. DOMEG(1)=OMEG(15)+UMEG(15+1)/2.0 43. 403 15=2.0 45. 403 15=2.0 45. 403 15=1.0 45. 403 15=1.0 45. 403 00 441 15=1.0 45. 403 00 441 15=1.0 45. 403 00 441 15=1.0 45. 403 00 441 15=1.0 45. 58EC(15)=00MEG(15))=451) 45. 48P(15)=0.5 45. 48P(15)=0.5 45. 48P(15)=0.5 45. 48P(15)=0.5 46. 48P(15)=0.5 46. 48P(15)=0.5 55. P14(15)=0.455 55. P14(15)=0.455 55. P14(15)=0.455 56. P14(15)=0.455 57. P14(15)=0.455 56. P145(15)=0.455 57. P145(15)=0.455 58. P145(15)=0.455 59. P145(15)=0.455 59. P145(15)=0.455 50. P15(15)=0.455 51.0 P145(15)=0.455 52. P145(15)=0.455 53. P145(15)=0.455	41=	XOHEG(1)=0H+0.5+(ONEG(1)-0M)		
43. 00 403 IS=2+9 45. 403 Bole(1(5)=004E6(1(5)).04E6(1(5)) 45. 403 Bole(1(5)=004E6(1(5)) 45. 403 Bole(1(5)=004E6(1(5)) 40. 404 IS=10.8 40. 404 IS=10.9 50. 404 MPC(15)=0.5 50. 405(1)=0.5 50. 405(1)=0.5 50. 405(1)=0.5 50. 405(1)=0.5 50. 405(1)=0.5 50. 405(1)=0.5 50. 405(1)=0.5 50. 405(1)=0.5	42+	DOMEG(1)=OMEG(1)-OM		
<pre>444 444 444 447 447 447 444 447 444 444</pre>	-01	00 403 IS=2+9		
45e 403 DOWEG(IS+1)=OMEG(IS) 404 APP(IS)=0.451.00 404 APP(IS)=0.53*.00APP((HOMEG(IS)) 50 APP(1S)=0.53*.00APP(SC(IS)=DOWEG(IS)) 50 APP(1S)=0.53*.00APP(SC(IS)=DOWEG(IS)) 50 APP(1S)=0.53*.00APP(SC(IS)=DOWEG(IS)) 51 C USE A RANDOM NUMBER TABLE TO SELECT RANDOM PHASE ANGLES FR 52 C RANDOM NUMBER TABLE TO SELECT RANDOM PHASE ANGLES FR 53 PHS(1)=0.255 54 PHS(1)=0.555 55 PHS(1)=0.555 56 PHS(1)=0.555 56 PHS(1)=0.555 57 PHS(1)=0.555 58 PHS(1)=0.555 59 PHS(1)=0.555 50 PHS(1)=0.557 50 PHS(1)=0.557 51 PHS(1)=0.557 52 PHS(1)=0.557 53 PHS(1)=0.557 54 PHS(1)=0.557 55 PHS(1)=0.557 56 PHS(1)=0.557 56 PHS(1)=0.557 57 PHS(1)=0.557 58 PHS(1)=0.557 58 PHS(1)=0.557 59 PHS(1)=0.557 50 PHS(1)=0.557 50 PHS(1)=0.557 50 PHS(1)=0.557 50 PHS(1)=0.557 50 PHS(1)=0.557 51 PHS(1)=0.557 52 PHS(1)=0.557 53 PHS(1)=0.557 54 PHS(1)=0.557 55 PHS(1)=0.557 56 PHS(1)=0.557 56 PHS(1)=0.557 56 PHS(1)=0.557 56 PHS(1)=0.557 57 PHS(1)=0.557 58 PHS(1)=0.557 58 PHS(1)=0.557 59 PHS(1)=0.557 50 PHS(1)=0.557 50 PHS(1)=0.557 50 PHS(1)=0.557 50 PHS(1)=0.557 51 PHS(1)=0.557 52 PHS(1)=0.557 52 PHS(1)=0.557 53 PHS(1)=0.557 54 PHS(1)=0.557 55 PHS(1)=0.557 56 PHS(1)=0.557 56 PHS(1)=0.557 56 PHS(1)=0.557 57 PHS(1)=0.557 58 PHS(1)=0.557 58 PHS(1)=0.557 59 PHS(1)=0.557 50 PHS(tte	MOMEG(1S)=(OMEG(1S)+OMEG(1S+1))/2		
<pre>Up for total</pre>	45+ 40	33 DOMEG(IS)=OMEG(IS+1)=OMEG(4S)		
<pre>4 *** 1 *EFC(15)=1100/EVEXTANTOCE (15))**4) 4 *** ********************************</pre>	* 1 • 1	DQ 404 [S=1.8		
404 ANP(15)=9,3=50RT(SPEC(15)=00WEG(1S)) 500 AMP(9)=0,0 522 C EVEC A RANDOM NUMBER TABLE TO SELECT RANDOW PHASE ANGLES FR 525 PHS(1)=0.305 525 PHS(1)=0.535 525 PHS(1)=0.535 526 PHS(1)=0.535 526 PHS(1)=0.535 526 PHS(1)=0.535 526 PHS(1)=0.535 526 PHS(1)=0.535 527 PHS(1)=0.535 528 PHS(1)=0.535 528 PHS(1)=0.535 528 PHS(1)=0.535 528 PHS(1)=0.535 528 PHS(1)=0.535 529 PHS(1)=0.535 529 PHS(1)=0.535 529 PHS(1)=0.535 520 PHS(1)=0.537 520 PHS(1)=0.537 520 PHS(1)=0.537 520 PHS(1)=0.537 520 PHS(1)=0.537 520 PHS(1)=0.537 520 PHS(1)=0.537 520 PHS(1)=0.537 520 PHS(1)=0.535 520 PHS(1)=0.537 520 PHS(1)=0.535 520 PHS(2)=0.535 520 PHS(2)=0.		SPEC([5)=(105/.00APAK/(MOTEG(15)		
50AMP (9)=0.051CUSE A RANDOM NUMBER TABLE TO SELECT RANDOM PHASE ANGLES FR52CUSE A RANDOM NUMBER TABLE TO SELECT RANDOM PHASE ANGLES FR53PHS(1)=0.405554PHS(2)=6.25555PHS(1)=5.25056PHS(5)=0.57559PHS(6)=5.55559PHS(6)=5.55559PHS(6)=0.57559PHS(6)=0.57559PHS(6)=0.57559PHS(6)=0.57550PHS(6)=0.57550PHS(6)=0.57550PHS(6)=0.57551PHS(6)=0.57552PHS(6)=0.57553PHS(6)=0.57554PHS(6)=0.57555PHS(6)=0.57556PHS(6)=0.57557PHS(6)=0.57558PHS(6)=0.57559PHS(7)=0.55750PHS(7)=0.55751PHS(7)=0.65752PHS(7)=0.65754PHS(7)=0.65755PHS(7)=0.65756PHS(7)=0.65757PHS(7)=0.65758PHS(7)=0.65759PHS(7)=0.65750PHS(7)=0.65751PHS(7)=0.65752PHS(7)=0.65754PHS(7)=0.65755PHS(7)=0.65756PHS(7)=0.65757PHS(7)=0.65758PHS(7)=0.65759PHS(7)=0.65759PHS(7)=0.65750PHS(7)=0.65750PHS(7)=0.65750PHS(7)=0.657 <td></td> <td><pre>4 #EAP(BFAK/(NUMED(13)/****) 14 AUD/101=0 #**COD1/COF(/15)*DOMER/1</pre></td> <td></td> <td></td>		<pre>4 #EAP(BFAK/(NUMED(13)/****) 14 AUD/101=0 #**COD1/COF(/15)*DOMER/1</pre>		
51. C USE A RANDOM NUMBER TABLE TO SELECT RANDOM PHASE ANGLES FR 52. C EACH COMPONENT 53. PHS(1)=0.306 54. PHS(1)=4.655 55. PHS(1)=4.655 56. PHS(1)=4.655 57. PHS(1)=5.20 57. PHS(1)=5.20 59. PHS(1)=5.20 59. PHS(1)=5.20 59. PHS(1)=5.20 59. PHS(1)=0.55 50. PHS(1)=0.55 51. PHS(1)=0.55 52. PHS(1)=0.55 53. PHS(1)=0.55 54. PHS(1)=0.55 54. PHS(1)=0.55 54. PHS(1)=0.55 54. PHS(1)=0.55 55. PHS(1)=0.55 56. PHS(1)=0.55 57.				
52. C EACH COMPONENT 53. PHS(1)=0.306 54. PHS(1)=0.306 55. PHS(1)=4.655 56. PHS(1)=4.655 56. PHS(1)=4.655 56. PHS(1)=5.207 57. PHS(1)=5.207 56. PHS(1)=5.207 56. PHS(1)=5.207 56. PHS(1)=5.207 56. PHS(1)=5.207 56. PHS(1)=5.207 56. PHS(1)=5.207 56. PHS(1)=5.207 57. PHS(1)=5.207 56. PHS(1)=5.207 57. PHS(1)=7.207 57.		ARP (31-U.U. LICE A RANDOM NUMBED TARE F TO RELEU	CT RANDOM PHACE ANGLES ER	
55 PHS(1)=0.306 54 PHS(1)=0.455 55 PHS(1)=14.655 56 PHS(1)=14.655 57 PHS(1)=14.655 58 PHS(1)=2.4555 59 PHS(1)=5.20 59 PHS(1)=5.25 51 PHS(1)=5.455 58 PHS(1)=5.455 59 PHS(1)=5.455 61 PHS(1)=5.455 62 PHS(1)=5.455 61 PHS(1)=5.455 62 PHS(1)=5.455 63 END 61 PHS(1) 62 PHS(1) 63 PHS(1) 64 PHS(1)<		EALL COUDANENT		
54. PHS(1)=4.555 55. PHS(1)=4.655 56. PHS(1)=5.20 57. PHS(5)=5.20 59. PHS(6)=3.220 59. PHS(6)=5.445 61. PHS(6)=5.445 61. PHS(6)=0.657 61. PHS(7)=5.445 61. PHS(7)=5.445 61. PHS(7)=5.445 62. END 62. END 64. PHS(7)=0.657 64. PHS(7)=0.657 65. PHS(7)=0.657 65. PHS(7)=0.657 66. PHS(7)=0.657 66. PHS(7)=0.657 66. PHS(7)=0.657 66. PHS(7)=0.657 66. PHS(7)=0.657 66. PHS(7)=0.657 66. PHS(7)=0.657 66. PHS(7)=0.657 67. PHS(7)=0.657 66. PHS(7)=0.657 67. PHS(7)=0.657 67. PHS(7)=0.657 67. PHS(7)=0.657 67. PHS(7)=0.657 67. PHS(7)=0.657 67. PHS(7)=0.657 67. PHS(7)=0.657 67. PHS(7)=0.657 67. PHS(7)=0.657 67. PHS(7)=0.657 77. PHS(7		CACH CURVEN		
55. PHS(1):4.655 56. PHS(1):4.655 57. PHS(1):5.20 57. PHS(5):5.225 58. PHS(6):5.225 59. PHS(6):5.225 60. PHS(6):5.245 61. PHS(7):5.545 61. PHS(7) 61. PHS(7):5.545 61. PHS(7) 61. PHS(7) 61. PHS(7) 61. PHS(7) 61.				
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56• PHS(4)=5.20 57• PHS(4)=5.20 59• PHS(5)=2.2545 60• PHS(6)=2.226 61• PHS(9)=2.435 61• PHS(9)=2.435 61• PHS(9)=2.435 62• Return 63• END 62• END 62• END 64• IJOB FORTRAN V COMPILATION• 0 *DIAGNOSTIC• MESSAGE(S)	554	PHS(3)=4.655		
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61. PHS(5)=0.657 62. RETURN 63. END 64. OF UNIVAC 1106 FORTRAN V COMPILATION• 0 *DIAGNOSTIC• MESSAGE(S)	60*	PHS (6) 20, 435		
62* RETURN 63* END 40 OF UNIVAC 1106 FORTRAN V COMPILATION• 0 *DIAGNOSTIC* MESSAGE(S)	-1-	Duc (c) = 0 - 6 - 7		
62* REIVER 63* ROD 40 OF UNIVAC 1106 FORTRAM V COMPILATION• 0 *DIAGNOSTIC+ MESSAGE(S)				
63* END 40 of Univac 1106 Fortrak v compilation• 0 =diagnostic= message(s)		RETUKR		1
4. OF UNIVAC 1106 FORTRAN V COMPILATION• 0 *DIAGNOSTIC* MESSAGE(S)	63*	END		
	IL OF UNIVAL	10 1106 CODTEAN V COMPTLATION.	*DI##NOSTIC= WESSAGE(S)	
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BUOT DTNAMICS	24 AH=0.54(R5##2)#(ALF-SIN(ALF)) 34 GO TU 18 34 CONTINUE 34 CONTINUE 35 DIF(Hb.61,(1,01#K5))GO TO 10 36 DIF(Hb.61,(1,01#K5))GO TO 10 37 DIF(Hb.62C) 38 DIF(Hb.62C) 39 DIF(Hb.62C) 30	60 TO TO TO TO TO TO TO TO TO TO TO TO TO	<pre>be Ak=3.2416=(RSee2)-0.5e(RSe²2)e(ALF-SIN(ALF)) be 06 T0 15 be 16 AR=1.5708e(RSee2) be 11 AR=1.816*(RSee2) be 18 Ar=1.816*(RSee2) be 18 IF(J.E0,1160 T0 3</pre>	00 00 10 19 10 3 8XX=402,124eR1eAR 10 3 2 0 10 10 10 9 10 10 10 9 10 10 10 10 10 10 10 10 10 10 10 10 10 1	GAMEGAMOGAM GAMEGAMOGAM 1F(GAMGGL,1.57)60 T0 20 60 T0 9 1XTL=128.0+VUL 20 BXTL=128.0+VUL 1XTL=43.4 1XTL	OF UNIVAC 1108 FORTRAN V COMPILATION. D #DIAGNOSTIC# MESSAGE
	8 8 8 9 9 9 9 8 8 8 9 9 9 8 8 8 9 9 9 9	100 100 100 100 100 100 100 100 100 100	000000 000000 000000	* * * * * * * * 60 - N M J M 7 J J J J J J J J	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	O ENC
	00132 00132 00133 00133	00141 00141 00141 00141 00141 00141 00141	00147 00150 00152 00152 00152	00156 00160 00161 00162 00162 00162	00165 00166 00170 00172 00172 00172 00173 00173 00173 00173 00173 00173 00173 00173 00173 00173 00173 00173 00173 00170 000170 00000000	

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ļ 10:51: 0.349 411 ACUA ACCUA ACCUA ACCUA ACCUA CCAP1 CCAP1 CCAP1 CCAP1 CCAP1 CCAP2 CCAP1 CCAP2 CCC 000074 3L 000374 A 000514 ACVP2 000550 ACVP2 000550 ACVP2 000456 CHMCT 000456 CHMCT 000456 CHMCT 000456 CHMCT 000530 CTR 000453 L 000463 L 000551 FHEB 000551 FHEB 000552 SP2 000552 SP2 000552 SP2 000552 SP2 000552 W 000552 W 000552 W 000552 W DATE 000577 2L 002506 4716 000555 4716 000455 ACVD 000455 ACVD 000455 ACVD 000455 ACVD 000455 ACVD 000455 ACVD 000552 CHHCN 000552 CHCN 000552 CHHCN 000552 CHHCN 000552 CHHCN 000552 CHHCN 000552 CHHCN 000552 CHHCN 000552 CHCN 000552 CHCN 000552 CHCN 000552 CHCN 000555 CHCN 0000555 CHCN 0 RELATIVE LOCATION, NAME) GI FOR MOOR, MOOR Univac 1108 Fortran V Level 2206 0023 This compilation was done on 01 may 72 at 10:51:00 (BLOCK, TYPE, 000606 ENTRY POINT 002352 STORAGE USED (BLOCK, NAME, LENGTH) EXTERNAL REFERENCES (BLOCK. NAME) VARIABLES BUUT CTHAMICS 002412 000760 000000 I. STORAGE ASSIGNMENT FOR 0000451 42L 000542 47UB 000542 47UB 000552 47UB 000552 47UB 000552 47UB 000552 47UB 000552 76742 000552 76742 000551 742 000551 742 000552 592 000552 40 000552 40 000552 40 000553 40 000555 40 000 +CODE +DATA +BLANK SIN SIN NEXP65 SORT SORT SUBROUTINE MOOK 1000 00003 0003 0005 0005 0005

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í i 1 1 4 i 1 ! t ł i i ï ı ł . 1 ł ł Ł ÷ 1 1 t ł 16 1 ı 1 ł ł i DATE 010572 PAGE ī ì. ļ ı i ļ , SUBROUTINE MOORIT.K.UBY.VBT.WBY.EP.PHIP.THEP) THIS SUBROUTINE COWPUTES THE RESPONSE OF A MOORING LINE IN THE THIS SUBROUTINE COWPUTES THE RESPONSE OF A MOORING LINE IN THE ELONGIONS ARE CONSILTERO. TO THE MOTIONS OF BUOY. THREE DIMENSIONAL MOTIONS ARE CONSILTERO. OF THE MOORING LINE, THE MOORING CLUMP TS ASSUMED TO BE OF INTINITE MOOSING LINE, THE MOORING CLUMP RELECTION OF MANDE, THE MOORING LINE, THE MOORING CLUMP DIFFERENTIAL EQUATIONS IS EFFECTED WITH A METHOD OF CHARACTERISTIC DIFFERENTIAL EQUATIONS IS EFFECTED WITH A METHOD OF CHARACTERISTIC THE MOOR.'G LINE MASS ABO. OF 5/6 NYLON MOPE. ţ 1 ì I. DIMENSION UN(21), VN(21), MN(21), PHIN(21), THEN(21), EN(21) DIMENSION UP(21), VP(21), MP(21), PHIP(21), THEP(21), EP(21) DIMENSION UO(21), VO(21), MO(21) DIMENSION A(6,7) THE CABLE PROMERTIES, INITIAL CONDITION⁷ AND COEFFICIENTS VSM=0.0 WSWE-1.5 SMAXEGOO0.0 The MIRE ROPE PFOPERTIES UIAE-0.312 #TC=0.125 #TC=0.125 CABLE MASS PEH UNIT LENGTH X 100 CABLE MASS PEH UNIT LENGTH X 100 DISTRUTE THE WASS OF THE INSTPUMENTS OVER THE CABLE CMU=0.672 TRANSFORM THE BUOT VELOCITIES TO CABLE COORDINATES UNIL)=UBY*COS(PHIP(1))*COS(THEP(1)) 1 *#8**COS(PHIP(1))*COS(THEP(1)) 1 *#8**SCOS(PHIP(1))*COS(THEP(1)) 1 *#8**SIN(PHIP(1))*COS(THEP(1))*UBY*COS(PHIP(1)) UNIL)=-UBY*SIN(PHIP(1))**BY*COS(THEP(1)) 1 *#1)=-UBY*SIN(THEP(1))**BY*COS(THEP(1)) USCUP(1) UACUP(1) UCCUP(1-1) UCCUP(1-1) ļ AMES CRITERIA - H/K GREATER THAY ANY CHARACTERISTIC i. IF (T.6T.0.0)60 TO 3 UP(1)=0.0 VP(1)=0.0 VP(1)=0.0 VP(1)=0.0 VP(1)=0.0 VO(1)=0.0 VO(1)=0.0 VO(1)=0.0 VO(1)=0.0 CHMCN=0,00153 DTDE=6,87E5 RH0=1,987 H=0.05+SMAX BUOT DYNAMICS CDN=1.4 CHMC1=0.6 CDT=0.05 ** 0000000000 U J υυ 000 υu *** 10000 ÷.9, 50°* 96.96 28* 31. *07 *** *** 50+ ÷... د/ ۲ :0; \$ •9, ********* *** 8 å

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DATE 010572 PAGE						:												JUS BETHEEN THO DISCREAT CABLES	FIUN TO ESTIMATE THE STRAINS ACROSS THE	5) + FP(1))	1) - E P (] 5)						11FS AT 4800 ET			ES							UNUTAIUNE IN THE T-S PLANE USING HARTREE'S	N GRAD END DIGTOPPETE WYANT												
BUOY DYMAHICS	UD=UP(I+1)	UE=UO(I)	UF=U0(1+1)	VA=VP(I)	VB=VF(I~1)	VC=V0(I-1)	VUTV711)		VF = VU(1+1) MA-LD(1)						EALEP(1)	EBEEP(I-1)	ED=EP(1+1)	C STRAINS ARE DISCONTINUC	C USE AN EULER APPROXIMAT	IF(1.59,13)EU=2.0+EP(13	IF(1.t0.14)EB=2.0.EP(14	PHIA=PHIP(I)	(J-I)dIHd=9IHd	PHIORPHIP (1+1)	THEATTHEP(I)		C CHANGE THE CABLE PROPEN	IF(I.6E.14)60 TO 42	G0 T0 41	· THE NTLUN ROPE PROPERTI	44 44410,006 #17-3,0166	CMU=0.455	CHMCN=0.00425	DTDE=3.06E4	SUNTINCE 14	Tute offers with the termination of the termination of the termination of the termination of the termination of the termination of the termination of the termination of the termination of the termination of the termination of the termination of the termination of the termination of the termination of the termination of the termination of terminatio of termination	METHOD IA HYBETO RETHON	ARANGEMENT OF POINTS I		2+7 R	•	•	ل==A==D				T+I I T-I	0=N.	CH1=11300.0	IF(I.6E.14)CH1=2609.0 Srem*(I-1)
	5å .	59.	•0•	c	* 29		• • • • •		67 •	•R•	•6ª	10.	71.	72+	-2-	74.	75	• • •	78+	-64	80*			10.0				-96			10	3.	**				+6	0 #0	• •	2 *	ب م						**	,	3 •	* *
	00153 -	00154	00155	00156	20157	12100	00162	00163	00154	00165	00166	00167	00770	17100	C0172	C4100	00174	74100	74.100	00175	22100	10200	20200	00,00	00205	00700	90700	00207		00212	00213	00214 5	00415 5	00216		00217	90417 5	00217 fo	00217 Ju	00217 10	DT 21700	01 /1200		00217 10	00217 10	00217 10	00217 11 00217 11	0220 - 11	00221 11	00222 11 U0224 11

DATE 010572 PAGE 18				SIN(THEA).		
BUOT DYNAMICS	SP1=SK+CH1€K SQ1=SR+CH1€K HP1=SR-SF1 HQ1=HP1 EREA EREA EP1=EA+(H01/H)⊕(EA-Ed) EP2=EP1 E92=E01	UR=UA VR=VA MR=VA MR=THEA PHIR=THEA ACUA=(UB-UC)/K ACUD=(UD-UE)/K ACUD=(UD-UE)/K ACUD=(VD-VE)/K	CVOCIVD-VE)/K ACVACIVD-VE)/K ACUACIVD-VE)/K ACUCIKD-VE)/K ACUCIKD-VE)/K ACUCIKO-VE)/K ACUCIKOA ACVACIACIVA ACVACIACIVA ACVACIACIVA ACVACIACIVA CURRENT PKOFILE C	WSM=2,9/([SR/3,2802)**0.418) WSM=2WSM USSEVSM*5/4(PHIA)-WSM*COS(PHIA) USSEVSM*COS(PHIA)-WSM*COS(PHIA) VSS=VSM*COS(PHIA)-WSM*COS(PHIA) VSS=VSM*COS(PHIA)-WSM*COS(PHIA) CONTINUE CONT	HP2=58-592 HP2=58-592 HP2=58-582 EP2=E4-(HP2/H) = (EA-EB) E02=E4+(H02/H) = (EA-EB) UP1=UA-(H01/H) = (UA-UB) UQ1=UA+(H02/H) = (UA-UB) UQ2=UA+(H02/H) = (UA-UB) VP1=VA-(HP2/H) = (VA-VB)	V01=VA+(H01/M)+(V0-VA) V02=VA+(H02/M)+(V0-VA) V02=VA+(H02/M)+(V0-VA) W01=MA+(H01/M)+(WA-WB) W02=MA+(H01/M)+(WA-WB) W12=MA+(H02/M)+(WD-WA) M102=PH1A+(H02/M)+(PH1A-PH1B) PH102=PH1A+(H02/M)+(PH1A-PH1B) PH102=PH1A+(H02/M)+(PH1A-PH1B) PH102=PH1A+(H02/M)+(PH1A-PH1B) PH102=PH1A+(H02/M)+(PH1A-PH1A)
	1100 1120 1220 1220 1220 1220 1220 1220	1234 1234 1234 1234 1234 1234 1234 1234		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1654 1664 1664 1714 1714 1714
	00235 00225 00225 00225 00232 00232 00232 00235 00235	00244 00244 00244 00244 00244 00244 00244 00244	00255 00255 00255 00255 00255 00255	00265 00265 00265 00265 00265 00265 00265	00272 00272 00275 00276 00276 00200 00200 00200	002020 00206 00206 002112 002112 002112 002112 002112 002112 002112 002112 002112 002112 002112 002112 002112 00206 002006 00200 000000

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E 01057	i I					
DAT			ēl ŧ (USS+UR] ≢ABS (USS+UR))RAGT è (USS+UP1) ≢ABS (USS+UP1))RAGT ŧ (USS+UP1) ≢ABS (USS+UQ1))RAGT ŧ (USS+UR) ∉ABS (VSS+VR)	-DRAGN# (VSS+VP2) #ABS (VSS+VP2) -URAGN# (VSS+VQ2) #ABS (VSS+VQ2) -URAGN# (VSS+VQ2) #ABS (VSS+VQ2) -1 #ABS (MSS+MP2) =ACMP2#CHMCN -#Q2) #ABS (MSS+MQ2) =ACMQ2#CHMCN -#Q2) #ABS (MSS+MQ2) =ACMQ2#CHMCN	HUL TANEOUS EQUATIONS R+PHIP1)) PHIP1=A(1,6)+THEP1	R+РНI@1)) РНI@1-А(2,6) *ТНЕ@1
BUOT UTHAMICS	THEP1=THEA-(HP1/H) • (THEA-1728) THE01=T4EA+(H02/H) • (THED-THEA) THE02=ThEA-(HP2/H) • (THEA-THEB) THE02=THLA+(H02/H) • (THED-THEA) ACUR=(UR-UA)/K	ACUFICIRE-MAIN ACUFICACHPI/H) * (ACUA-ACUB) ACUPICACUA-(HPI/H) * (ACUD-ACUB) ACUDIACUA-(HF2/H) * (ACUD-ACUB) ACV2ZACVA+(HE2/H) * (ACUD-ACVA) ACVPZEACWA-(HP2/H) * (ACUD-ACVA) ACWPZEACWA-(HP2/H) * (ACUD-ACWB) ACWPZEACWA-(HP2/H) * (ACUD-ACWB) ACWPZEACWA-(HP2/H) * (ACUD-ACWB)	THE CABLE LOAUIXG FUNCTIONS DRAG5=0.5+HH0+CON+DIA DRAG4=0.5+HH0+CON+DIA DRAG4=0.5+HH0+CON+DIA DRAG4=0.5+HH181.4-COS(IHER)-DRAG CHR2=WTC=COS(PH1P1)+COS(IHEP1)+D L-ACUP1+CHHC1 L-ACUP1+CHUC1 L-ACUP1+CHUC1 L-ACUP1+CHUC1 L-ACUP1+CHUC1	1	COEFFICIENTS IN THE MATRIX OF SI A(1,1)=1.0 A(1,2)=0.0 A(1,2)=0.0 A(1,5)=-0.5e(NR+VP1) A(1,5)=-0.5e(NR+VP	A(2,1)=1,0 A(2,2)=0,0 A(2,4)=0,0 A(2,4)=CA1 A(2,4)=CA1 A(2,5)=-0,5*(R+WQ1) A(2,5)=-0,5*(R+WQ1) A(2,5)=-0,0 A(2,1)=-0,0 A(2,1)=0,0 A(2,1)=0,0 A(3,2)=1,0 A(3,2)=1,0
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1/201	<	A (3, 3) #0.0						
2250	233	A(3,4)=0.0						
0373		A(3,5)==0.5*(((1.0+ER)*CH2R)+((1.0+EP2)*CH2P2))						
10374	235*	A(3.6)#0.5+(#R+#P2)&SIN(0.54(PHIR+PHIP2))						
10.475	2 16.	A 1 4 7 4 7 4 7 4 7 4 7 4 7 4 7 4 7 4 7 4	•					
1.10	- 1 1 0	1						
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0376	239 +	A(4,1)=0.0						
0377	9741	A(4,2]=1.0						
0400	2414							
20405	2434	A{\\5)=0.54{((1.0+ER)=CH2R)+{(1.0+EQ2)+CH2G2)}			ţ	•		
0403	344	A(4.6)=0.50(084402)#SIN(0.56(PHIR+PHIO2))						
0404	1.510	A/4.77-202-0.54(//).04F8)\$CH28)4(//).04F02)\$CH202)}						
			,					
*								
0404	247*	0						
0405	248=	A(5,1)=C.D						
0405	2494							
1010	-030		1		1111	1		
0110	-102	A (5,4)=0.0						
0411	252*	A(5+5)=C.0						
0412	253*	A(5.6)=(0.5)+((1.0+ER)+CH2R)+(1.0+EP2)+CH2P2))+COS(0.5)	*(PHIR					
	1110							
2140								
0412	255*	2 =0.5=(VR+VP2)=5IN(0.5=(PHIR+PHIP2))))				1	1	
0413	2564	<u>& (5.7)</u>		;	1	•		
	- 2 3 0							
0740								
1110	258 •	A(6,1)=0.0						
19415	259*	A(6r2)=0.0						
0416	260.							
1417	* 197	A(6:4)=0.0		•				
0420	202*	A (6,5)=0.0						
0421	20.54	A(6.6)2(0.54)2(0.52)2(0.54)2(0	CPHIR					
4.95								
1240	\$65\$	<pre>2 = 0.554 (Y+VQ2) #51N(0.554 (PH R+PHIG2)))</pre>						
0422	266*	A(6.7)==#02=A(6.6)#THEQ2=1.00.0+0.5+(CIR+CIQ2)+K/CMU						
			1	-		1		
	×007	C SOLUTION OF THE SIX SIMULIANEOUS EQUATIONS						
10422	2094							
10423	270*	THER=(A(6,2)~A(5,7))/(A(5,6)~A(6,6))						
0424	271+	#8==4(5.7)=4(5.6)#THER						
		0.2007.100.000.000.000.000.000.000.000.0000000						
0240	*C/>	VKII4A(0,7)+A(0,0)#INEKIA(0,5)#PN4K						
6427	2744	FP=(2,2,2,-1,1,7)+(4(2,5)=4(1,5))#PHTP+(4(2,6)=4(1,6))#T		1	•			
10.00								
0640	276+	UR#=A(1,7)=A(1,5)*PHIR=A(1,6)#IHER=A(1,4)*ER						
04.30	2774	C THE CARLE CAN'T SUPPORT COMPRESSION THUS STRAIN CAN NEVEL	R BE NE					
イウナフ	20/2	IF (EK.E.U.U.U.U.U.U.U.U.U.U.U.U.U.U.U.U.U.U						
0433	279*	78 CONTINUE						
1110	280.	C REDEAT TRE DARK 'SS						
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0.40	289*	50 CONTINUE						

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Appendix C

DERIVATION OF THE CABLE CHARACTERISTICS

In order to rewrite the cable equations in their "normal" form, the characteristic roots of the cable equations must be determined. From equation (168) of the main text, we see that $AU_t + BU_s + C = 0$,

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Multiplying through by the inverse of the A matrix yields

$$A^{-1}A U_t + A^{-1}B U_s + A^{-1}C = 0 ,$$

or

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$$U_t + A^{-1}B U_s + A^{-1}C = 0.$$

To find the inverse of the A matrix, partition the A matrix into four 3-by-3 matrices:

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$$A = \begin{bmatrix} D & -\mu I \\ E & 0 \end{bmatrix}$$
$$D = \begin{bmatrix} 0 & \mu V & \mu W \cos \phi \\ 0 & -\mu U & -\mu V \sin \phi \\ 0 & 0 & -\mu (U\cos \phi - V\sin \phi) \end{bmatrix}$$

$$E = \begin{bmatrix} -1 & 0 & 0 \\ 0 & -(1+\epsilon) & 0 \\ 0 & 0 & -(1+\epsilon)\cos\phi \end{bmatrix}$$

Let A^{-1} be partitioned in the same way,

$$A^{-1} = \begin{bmatrix} K_1 & K_2 \\ K_3 & K_4 \end{bmatrix}$$

then

$$\begin{bmatrix} D & -\mu I \\ E & O \end{bmatrix} \begin{bmatrix} K_1 & K_2 \\ K_3 & K_4 \end{bmatrix} = \begin{bmatrix} I & O \\ O & I \end{bmatrix},$$

$$DK_{1} - \mu K_{3} = I$$

$$DK_{2} - \mu K_{4} = 0$$

$$EK_{1} + 0 = 0 \qquad E \text{ nonsingular } \therefore K_{1} = 0$$

$$EK_{2} + 0 = I \qquad \therefore K_{2} = E^{-1}$$

Substituting into the first equation, we see that

$$K_3 = \frac{1}{2}I$$
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F. om the second equation, we see that

$$K_{\mu} = \pm DK_{z} = \pm DE^{-1}.$$

$$K_{1} = 0$$

$$K_{2} = \begin{bmatrix} -1 & 0 & 0 \\ 0 & -\frac{1}{(1+\epsilon)} & 0 \\ 0 & 0 & -\frac{1}{(1+\epsilon)\cos\phi} \end{bmatrix}$$

$$K_{3} = \begin{bmatrix} -\frac{1}{12} & 0 & 0\\ 0 & -\frac{1}{24} & 0\\ 0 & 0 & -\frac{1}{24} \end{bmatrix}$$
$$K_{4} = \begin{bmatrix} 0 & -\frac{V}{(1+6)} & -\frac{W}{(1+6)} \\ 0 & \frac{U}{(1+6)} & \frac{W \sin \phi}{(1+6)\cos \phi} \\ 0 & 0 & \frac{(U\cos \phi - V\sin \phi)}{(1+6)\cos \phi} \end{bmatrix}$$

The A^{-1} matrix is written

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$$A^{-1} = \begin{bmatrix} 0 & 0 & 0 & -1 & 0 & 0 \\ 0 & 0 & 0 & 0 & -\frac{1}{(1+\epsilon)} & 0 \\ 0 & 0 & 0 & 0 & 0 & -\frac{1}{(1+\epsilon)\cos\phi} \\ -\frac{1}{44} & 0 & 0 & 0 & -\frac{V}{(1+\epsilon)} & -\frac{W}{(1+\epsilon)} \\ 0 & -\frac{1}{44} & 0 & 0 & \frac{U}{(1+\epsilon)} & \frac{W\sin\phi}{(1+\epsilon)\cos\phi} \\ 0 & 0 & -\frac{1}{44} & 0 & 0 & \frac{U}{(1+\epsilon)\cos\phi} \end{bmatrix}$$

$$\underline{A^{-1}B} = \begin{bmatrix} \frac{dT}{d\epsilon} & 0 & 0 & 0 & 0 & 0 \\ 0 & \epsilon \frac{dT}{d\epsilon} & 0 & 0 & 0 & 0 \\ 0 & 0 & \epsilon \frac{dT}{d\epsilon} & 0 & 0 & 0 & 0 \\ 0 & 0 & \epsilon \frac{dT}{d\epsilon} \cos \phi &$$

$$\begin{bmatrix} 0 & V & W\cos\phi & -1 & 0 & 0 \\ 0 & -\frac{U}{(1+\epsilon)} & -\frac{W\sin\phi}{(1+\epsilon)} & 0 & -\frac{1}{(1+\epsilon)} & 0 \\ 0 & 0 & -\frac{(U\cos\phi - V\sin\phi)}{(1+\epsilon)\cos\phi} & 0 & 0 & -\frac{1}{(1+\epsilon)\cos\phi} \\ -\frac{1}{4}\frac{dT}{d\epsilon} & -\frac{UV}{(1+\epsilon)} & -\frac{UW\cos\phi}{(1+\epsilon)} & 0 & -\frac{V}{(1+\epsilon)} \\ 0 & \left(-\frac{\epsilon}{4}\frac{dT}{d\epsilon} + \frac{U^2}{(1+\epsilon)}\right) \left(\frac{2UW\sin\phi}{(1+\epsilon)} - \frac{VW\sin^2\phi}{(1+\epsilon)\cos\phi}\right) & 0 & \frac{U}{(1+\epsilon)} & \frac{W\sin\phi}{(1+\epsilon)\cos\phi} \\ 0 & 0 & \left(-\frac{\epsilon}{4}\frac{dT}{d\epsilon}\cos\phi + \frac{(U\cos\phi - V\sin\phi)^2}{(1+\epsilon)\cos\phi}\right) & 0 & \frac{(U\cos\phi - V\sin\phi)}{(1+\epsilon)\cos\phi} \end{bmatrix}$$

To find the characteristics. λ , use $|A^{-1}B - \lambda I| = 0$:

$$\begin{bmatrix} 0-\lambda & V & W\cos\phi & -1 & 0 & 0 \\ 0 & \frac{U}{(1+\epsilon)}-\lambda & -\frac{W\sin\phi}{(1+\epsilon)} & 0 & -\frac{1}{(1+\epsilon)} & 0 \\ 0 & 0 & -M-\lambda & 0 & 0 & -\frac{1}{(1+\epsilon)} & 0 \\ -\frac{1}{4}\frac{dT}{d\epsilon} & -\frac{UV}{(1+\epsilon)} & -\frac{UW\cos\phi}{(1+\epsilon)} & -\lambda & -\frac{V}{(1+\epsilon)} & -\frac{W}{(1+\epsilon)} \\ 0 & (-\frac{c}{4}\frac{dT}{cle}+\frac{U^{2}}{(1+\epsilon)}) & N & 0 & (\frac{U}{(1+\epsilon)}-\lambda) & \frac{W\sin\phi}{(1+\epsilon)\cos\phi} \\ 0 & 0 & (-\frac{c}{4}\frac{dT}{d\epsilon}\cos\phi+M(U\cos\phi-V\sin\phi)) & 0 & M-\lambda \\ \end{bmatrix} = 0$$

where

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 $\underline{A^{-1}G}$

$$M = \frac{V\cos\phi - V\sin\phi}{(1+\epsilon)\cos\phi}$$
$$N = \frac{2VW\sin\phi}{(1+\epsilon)} - \frac{VW\sin^{2}\phi}{(1+\epsilon)\cos\phi}$$

The first reduction is

$$-\lambda \begin{bmatrix} -\frac{U}{(1+\epsilon)} - \lambda & -\frac{W}{V} \frac{SiN\Phi}{(1+\epsilon)} & 0 & -\frac{1}{(1+\epsilon)} & 0 \\ 0 & -M - \lambda & 0 & 0 & -\frac{1}{(1+\epsilon)} \cos \phi \\ -\frac{UV}{(1+\epsilon)} & -\frac{UV\cos\phi}{(1+\epsilon)} & -\lambda & -\frac{V}{(1+\epsilon)} & -\frac{W}{(1+\epsilon)} \\ \begin{pmatrix} -\frac{E}{4t} \frac{dV}{d\epsilon} + \frac{U^2}{(1+\epsilon)} \end{pmatrix} & N & 0 & \begin{pmatrix} U\\(1+\epsilon) - \lambda \end{pmatrix} & \frac{WSiN\Phi}{(1+\epsilon)\cos\phi} \\ 0 & \begin{pmatrix} -\frac{E}{4t} \frac{dT}{d\epsilon}\cos\phi + M(U\cos\phi - V\sin\phi) \end{pmatrix} & 0 & 0 & M - \lambda \end{bmatrix}$$

$$+ \frac{1}{U} \frac{dT}{d\epsilon} \begin{bmatrix} V & W \cos \phi & -1 & 0 & 0 \\ -\frac{U}{(1+\epsilon)} - \lambda & -\frac{W}{(1+\epsilon)} & 0 & -\frac{1}{(1+\epsilon)} & 0 \\ 0 & -M - \lambda & 0 & 0 & -\frac{1}{(1+\epsilon)} & 0 \\ -\frac{c}{U} \frac{dT}{d\epsilon} + \frac{U^{2}}{(1+\epsilon)} & N & 0 & (\frac{U}{(1+\epsilon)} - \lambda) & \frac{V \sin \phi}{(1+\epsilon)\cos \phi} \\ 0 & (-\frac{c}{U} \frac{dT}{d\epsilon} \cos \phi + M(U\cos \phi - V\sin \phi)) & 0 & M - \lambda \end{bmatrix} = 0$$

The second reduction is

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$$\lambda^{2} \begin{bmatrix} -\frac{U}{(1+\epsilon)} - \lambda & -\frac{W \sin \phi}{(1+\epsilon)} & -\frac{1}{(1+\epsilon)} & 0 \\ 0 & -M - \lambda & 0 & -\frac{1}{(1+\epsilon)\cos \phi} \\ -\frac{\epsilon}{U}\frac{dT}{d\epsilon} + \frac{U^{2}}{(1+\epsilon)} & N & \frac{V}{(1+\epsilon)} - \lambda & \frac{W \sin \phi}{(1+\epsilon)\cos \phi} \\ 0 & -\frac{\epsilon}{U}\frac{dT}{d\epsilon}\cos\phi + M (U\cos\phi - V\sin\phi) & 0 & M - \lambda \end{bmatrix}$$

$$-\frac{1}{\sqrt{d\epsilon}} \frac{dT}{d\epsilon} - \frac{W}{(1+\epsilon)} - \frac{W}{(1+\epsilon)} - \frac{W}{(1+\epsilon)} - \frac{1}{(1+\epsilon)} - \frac{1}{(1+\epsilon)} - \frac{1}{(1+\epsilon)} - \frac{1}{(1+\epsilon)\cos\phi} = 0$$

$$-\frac{1}{\sqrt{d\epsilon}} \frac{dT}{d\epsilon} + \frac{U^2}{(1+\epsilon)} - \frac{W}{1+\epsilon} - \frac{$$

Note that these two matrices are the same. Since they must be nonsingular, we see that

$$\lambda^2 - \frac{1}{\mathcal{U}} \frac{dT}{d\varepsilon} = 0 \quad ; \quad$$

thus,

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$$\lambda = \pm \sqrt{\frac{1}{\mu} \frac{dT}{d\varepsilon}}$$

The third reduction is

$$-\frac{1}{(1+\epsilon)}\begin{bmatrix} 0 & -M-\lambda & -\frac{1}{(1+\epsilon)\cos\phi} \\ -\frac{\epsilon}{M}\frac{dT}{d\epsilon} + \frac{U^2}{(1+\epsilon)} & N & \frac{W\sin\phi}{(1+\epsilon)\cos\phi} \\ 0 & -\frac{\epsilon}{M}\frac{dT}{d\epsilon}\cos\phi + M(U\cos\phi - V\sin\phi) & M-\lambda \end{bmatrix}$$

$$+\left(\frac{U}{(l+\epsilon)}-\lambda\right)\left[\begin{array}{c} -\frac{U}{(l+\epsilon)}-\lambda & -\frac{W\sin\phi}{(l+\epsilon)} & 0\\ 0 & -M-\lambda & -\frac{1}{(l+\epsilon)\cos\phi}\\ 0 & -\frac{\epsilon}{\mu}\frac{dT}{d\epsilon}\cos\phi+M(U\cos\phi-V\sin\phi) & M-\lambda\end{array}\right] = 0$$

The fourth reduction is

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$$-\frac{1}{(1+\epsilon)}\left(\frac{\epsilon}{\mu}\frac{dT}{d\epsilon}-\frac{U^{2}}{(1+\epsilon)}\right)\left[-\frac{\epsilon}{\mu}\frac{dT}{d\epsilon}\cos\phi+M(U\cos\phi-V\sin\phi) \quad M-\lambda\right]$$

$$+\left(\frac{U}{(1+\epsilon)}-\lambda\right)\left(-\frac{U}{(1+\epsilon)}-\lambda\right)\left[-\frac{e}{4\pi}\frac{dT}{d\epsilon}\cos\phi+M(U\cos\phi-V\sin\phi) - M-\lambda\right] = 0$$

Again. the matrices are the same; since they must be nonsingular, we see i. at

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$$-\frac{1}{(1+\epsilon)}\left(\frac{\epsilon}{\mu}\frac{dT}{d\epsilon}-\frac{U^{2}}{(1+\epsilon)}\right)+\left(\frac{U}{(1+\epsilon)}-\lambda\right)\left(-\frac{U}{(1+\epsilon)}-\lambda\right)=0$$

or

$$\lambda = \pm \sqrt{\frac{1}{\mathcal{U}} \frac{\epsilon}{(1+\epsilon)}} \frac{dT}{d\epsilon}$$

The fifth reduction is

$$(-M-\lambda)(M-\lambda) - \left(\frac{\epsilon}{\omega} \frac{dT}{d\epsilon} \cos\phi + M(U\cos\phi - V\sin\phi)\right)\left(\frac{-i}{(i+\epsilon)\cos\phi}\right) = 0$$

or

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$$\lambda = \pm \sqrt{\frac{1}{\mu}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon} .$$

Summarizing, the six characteristics are

$$\lambda_{1} = \pm \sqrt{\frac{1}{\mathcal{U}(S_{0})}} \frac{dT}{d\epsilon}$$

$$\lambda_{2} = -\sqrt{\frac{1}{\mathcal{U}(S_{0})}} \frac{dT}{d\epsilon}$$

$$\lambda_{3} = \pm \sqrt{\frac{1}{\mathcal{U}(S_{0})}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon}$$

$$\lambda_{4} = -\sqrt{\frac{1}{\mathcal{U}(S_{0})}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon}$$

$$\lambda_{5} = \pm \sqrt{\frac{1}{\mathcal{U}(S_{0})}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon}$$

$$\lambda_{6} = -\sqrt{\frac{1}{\mathcal{U}(S_{0})}} \frac{\epsilon}{(1+\epsilon)} \frac{dT}{d\epsilon}$$

Appendix D

COMPUTED INPUT DATA FOR THE SIMULATIONS

This appendix provides computed input data for the following:

1. Steady-state configurations of the 8-ft torroidal buoy and current

meter array at Block Island Sound station BRAVO.

2. Steady-state configurations of the 8-ft torroidal buoy and cable used for WHOI Mooring No. 279.

3. Dynamics of the $3\frac{1}{2}$ -ft spherical buoy and cable at station DELTA off

New Harbor, Block Island.

4. Dynamics of the 8-ft torroidal buoy and cable at station BRAVO in

Block Island Sound.

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5. Dynamics of the 8-ft torroidal buoy and cable used for WHOI Mooring No. 238.

Inputs for the Simulation of Steady-State Configurations of the 8-ft Torroid and Current Meter Array at BRAVO

The components used in the buoy system are shown in figure 21 and are described in chapter IV of the main text. The input data are as follows:

1.	Buoy diameter BD1	8.0 ft
2.	Torroid section diameter BD2	2.5 ft
3.	Buoy weight WB	1200.0 lb
4.	Maximum bull draft HM	2.5 ft

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	weight) HFREE	0.7 22
6.	Buoy windage WAREA	12.84 ft ²
7.	Wind drag coefficient WCD	9.971
8.	Cable diameter DIA:	
	Upper line - 5/8-in. polypropylene	0.625 in.
	Lower line - 3/8-in. wire rope	0.375 in.
9.	Cable weight in water per unit length	
	WTC:	
	Upper line -	-0.02 lb/ft
	Lower line -	0.2 lb/ft
10.	Cable modulus of elasticity EC:	
	Upper line -	1.67×10^5 lb/in. ²
	Lower line -	1.20×10^7 lb/in. ²
11.	Mooring line length SM	235.0 ft
12.	Current speeds at three depths:	
	CUR1, CUR2, and CUR3	1.0 to 1.77 knots
13.	Current directions at three depths:	
	DIR 1, DIR 2, and DIR 3	0-360 deg
14.	Water depth DEEP	120:0 ft
15.	Wind speed v component	0.0 ft/sec
16.	Wind speed x component	0.0 ft/sec.

The cable properties DIA, WTC, and EC are changed at a cable length of 230.0 ft from the values for the 5/8-in. polypropylene to the values for 3/8-in.

wire rope. Changes in tension and angles across the current meters were found by inserting the following procedures in the program:

1. Use and IF statement to locate the current meters along the mooring line.

2. Use the computed tension and angles to compute the force components acting on the top of the current meter.

3. Solve the statics equations for the current meter by using the current meter in-water weight and computed drag force components to find the force components acting on the bottom of the current meter.

4. Compute the current meter tilt angle.

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5. Transform back to cable coordinates to find the new cable tension and angles.

6. Increase the cable length by 3 ft, i.e., the length of the current meter.

7. Continue integration down the cable.

The following data were required to accomplish the above:

Current meter weight in water WCM	67 lb
Current moter drag coefficient CDCM	0.59
Current meter frontal area ARCM	2.285 ft ² .

Current strengths and directions as functions of depth were computed as follows:

1. Average strengths and directions from current meters 1 and 2.

2. For depths of 0 to 70 ft, set the upper layer strength and $\omega = c^{4} \omega^{2}$ equal to these values.

Test current meter 3 strength against mean strength of meters 1 and
 If greater, set the lower layer current equal to the mean strength in the upper layer.

4. Current strength and direction in the lower layer (70 to 120 ft) set equal to current meter 3 strength and direction.

Integration step size (B) was set equal to 1.0 ft, and the limiting depth error bandwidth DDP was set equal to 1 ft.

Inputs for the Simulation of Steady-State Configurations of the 8-ft Torroid and Cable Used for WHOI Mooring No. 279

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The components used in this mooring are shown in figure 27 and are described in chapter IV of the main text. Buoy dimensions are the same but the buoy weight is increased to account for the instruments in the buoy, the chain bridle, and instruments directly beneath the buoy. The input data are listed as follows:

1.	Buoy diameter BD1	8.0 ft
2.	Torroid section diameter BD2	2.5 ft
3.	Buoy weight WB	2100.0 lb
4.	Maximum hull draft HM	2.5 ft
5.	Free draft HFREE	1.04 ft
6.	Buoy windage WAREA	19.84 ft ²
7.	Buoy wind drag coefficient	0.971
8.	Cable diameter DIA:	
	Upper cable - 1/4-in. GAC	0.25 in.

	Lower cable - 5/8-in. plaited	nylon 0.625 in.
9). Cable weight in sea water per un	it length WTC:
	Upper cable ~	0.090 lb/ft
	Lower cable -	0.0105 lb/ft
01	Cable modulus of elasticity EC:	
	Upper cable -	1.682×10^7 lb/in. ²
	Lower cable -	3.52×10^5 lb/in. ² 0 < T < 1000 lb
		6.79 x 10^5 lb/in. ² 1000 lb < T < 2000 lb
		$1.041 \ge 10^6$ lb/in. ² 2000 lb < T
11	. Mooring line length SM	8000.0 ft
12	2. Surface current CUR	0-1.46 knots
13	. Water depth DEEP	8800.0 ft
14	. Wind speed y component	0.0 ft/sec
15	. Wind speed z component	0.0 ft/sec.

Cable properties were changed at a length of 4800 ft from the buoy and the changes in tension and angles were computed across the instruments in the line as before. The input data for the current meter and tensiometers are listed as follows:

Current meter:

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Weight in water	120.0 lb
Drag coefficient	1.4
Frontal area	2.92 ft ²

Tensiometer:

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Weight in water	50.0 lb
Drag coefficient	1.4
Frontal area	0.875 ft ²

The drag force acting on the buoy was increased to account for the chain bridle and the instruments directly below the buoy. An effective area-drag coefficient product of 22.18 ft² was added to the area-drag coefficient product of the buoy to account for the instruments and chains.

Inputs for the Simulation of Buoy System For the Spherical Buoy

The spherical buoy dimensions were mea.ured and the buoy and its instrumentation were weighed. The center of gravity and mass moments of inertia were calculated from the known weights and dimensions. The cable and chain were weighed in air and the mass and weight in sea water of each were computed. Mean wind wave heights and periods and the frequencies, amplitudes, and phases were computed on the GSA computer by using program RWAVE (see subroutine RWAVE in appendix A) for a given wind speed, a 10-hr duration, and a 15-inile fetch. Input data are listed as follows:

6.	Yaw mass moment of inertia ALIN	11.967 lb-sec 2 /ft
5.	Buoy mass MB	13.66 lb-sec 2 /ft
4.	Buoy weight WB	440.0 lb
3.	Mooring line connection height XML	0.208 ft
2.	Center of gravity height XML	0.208 ft
1.	Buoy hull radius BR	1.75 ft

7.	Pitch mass moment of inertia BTIN	17.46 lb-sec ² /ft
8.	Roll mass moment of inertia GMIN	19.875 lb-sec ² /ft
9.	Wind drag coefficient WCD	0.5
10.	Windage (profile area) WAREA	5.435 ft ²
11.	Wind lift coefficient WCL	0.25
12.	Plan area WAREL	4.81 ft ²
13.	Wind center of pressure height	1.068 ft
14.	Mean wave height WHTM	0.5 to 4.05 ft
15.	Mean wave period PER	2.2 to 6.0 sec
16.	Unstretched cable lengths CLO(I)	18.75 ft
		12.5 ft
		12.5 ft
17.	Upper cable diameter DIA1	0.0416 ft
18.	Upper cable weight in sea water DWC1	0.35 lb/ft
19.	Upper cable mass per unit length DCSM1	$0.0124 \text{ lb-sec}^2/\text{ft}^2$
20.	Lower cable diameter DIA2	0.125 ft
21.	Lower cable weight in sea water DWC2	7.3 lb/ft
22.	Lower cable mass per unit length DCSM2	$0.233 \text{ lb-sec}^2/\text{ft}^2$
23.	Surface current y component CYS	0.0 ft/sec
24.	Surface current z component CZS	0.845 ft/sec
25.	Wind y component	-30.0 ft/sec

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26.	Wind z component		-8.0 ft/sec
27.	Initial buoy displacements	y(2)	-0.21 ft
		y(4)	-41.1 ft
		y(6)	62.35 ft
28.	Initial cable element displa	acements y(I)	15.04 ft
			27.54 ft
			38.76 ft
			49.46 ft
			55.95 ft
			62.00 ft
		y(f + 2)	-29.79 ft
			-20.54 ft
			-13.06 ft
			-6.86 ft
			-3.31 ft
			0.00 ft
		y(1 + 4)	55.86 ft
			46.06 ft
			34.47 ft
			20.48 ft
			10.43 ft
			0,00 ft.

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The highest natural frequency of the system was estimated by using the method described in appendix A, and a time step size (B) of 5 x 10^{-4} see was used. The currents were uniform over the 62-ft water depth. A 30-lb force was added to the vertical cable force component acting on the buoy to account for the instrument package just below the buoy.

Inputs for the Simulation of Buoy System Dynamics for Torroidal Buoy BRAVO

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Again, buoy dimensions and weights were measured or taken from the manufacturers' drawings, and the properties of the buoy were calculated. The chain bridle below the buoy was assumed to be rigid, and its mass and drag were included in the computations. In this case, each cable element weight, mass, and drag were listed directly in the program since the mooring line was composed of many elements (cable chains, current meter, sentinel, etc.). Input data are listed as follows:

1.	Buoy hull radius RR	4.0 ft
2.	Center of gravity height XCG	13.064 ft
3.	Mooring line connection height XML	11.782 ft
4.	Buoy weight WB	2100.0 lb
5.	Buoy mass MB	65.25 lb
6.	Yaw mass moment of inertia ALIN	283.44 lbsec ² /ft
7.	Pitch mass moment of inertia BTIN	445.12 lb-sec 2 /ft
8.	Roll mass moment of inertia GMIN	445.12 lb-sec ² /ft
9.	Wind ag coefficient	0.971
10.	Wind lift coefficient	0.25
11.	Windage (profile area)	19.84 ft ²
12.	Plan area	50.3 ft ²

13.	Wind center of pressure he	eight	6.19 ft
14.	Wind speed causing the way	ves	
	(11 June 1970)		16.9 ft/sec
15.	Wind duration		
16.	Surface current y compor	ent	
	(11 June 1970)		-0.21 ft/sec
17.	Surface current z compor	ent	
	(11 June 1970)		-0.597 ft/sec
18.	Wind y component		0.0 ft/sec
19.	Wind z component		
	(11 June 1970)		-16.9 ft/sec
20.	Initial buoy displacements	y(2)	-0.25 ft
		y (4)	-0.7 ft
		y(6)	208.6 ft
21.	Initial cable element displa	cements y(I)	64 18 ft
			110.45 ft
			120.91 ft
			120.9 ft
		y(I + 2)	-0.8358 ft
			-0.7416 ft
			-0.6817 ft
			0.0 ít
		y(I + 4)	200.84 ft
			193.35 ft

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0.0 ft.

In this case, the time step size was taken to be 5×10^{-3} sec. Because of the chain bridle, the hydrodynamic force moment arm was computed to be 4.16 ft below the center of gravely. The surge and sway area-drag coefficient products include the effects 6, the steel bracing and chain bridle under the buoy.

Inputs for the Simulation of Buoy System Dynamics for the Torroidal Buoy and Cables Used in WHOI Mooring No. 238

Buoy dimensions, weights, masses, etc. are the same as those for the torroidal buoy at station BRAVO. The wind fetch length in subroutine RWAVE was changed to 100 miles to better simulate deep-sea wind wave conditions. Cable dynamics were simulated with subroutine MOOR, and the six dependent cable properties (tension, two angles, and three velocity components) were calculated at 20 points along the cable. Total unstretched mooring line length was 8000 ft: 4800 ft of 1/4-in. galvanized steel aircraft cable (polyo!efin jacketed to 3/8-in. diameter) and 3200 ft of 5/8-in. plaited nylon.

The upper cable properties are listed as follows:

1.	Diameter	0.0312 ft
2.	Weight per unit length in sea water	0.125 lb/ft
3.	Mass per unit length	$0.0054 \text{ lb-sec}^2/\text{ft}^2$
4.	Characteristic velocity	11, 300.0 ft/sec
5.	Elastic modulus-cross section product	6.87×10^5 lb

The lower cable properties are listed as follows:

1.	Diameter	0.052 ft
2.	Weight per unit length in sea water	0.0105 lb/ft
3.	Mass per unit length	0.00455 lb-sec ² /ft
4.	Characteristic velocity	2609.0 ft/seç
5.	Elnetic modulus-cross section product	3.06×10^4 lb.

Cable properties were changed from wire rope to nylon rope at 4800 ft (I = 14). The Webster current profile was used to compute steady drag forces on the cable. Numerical stability was maintained by using a time step (d) of 0.02 sec. The H/K quotient is equal to 20,000 ft/sec, which is larger than the tensile wave speed in the upper cable (11,300 ft/sec). Initial strains and cable angles were computed with the steady-state, buoy system configuration program with the same current profile and a surface current of 1.5 knots. Winds of 10, 20, and 30 knots were used. and the computed steady-state strains and cable angles served as input (initial conditions) ' e dynamics model.

Appendix E BUOYANT FORCES AND MOMENTS FOR A TORROIDAL BUOY

In order to simulate the dynamics of the torroidal buoy, a method to compute the buoyant force and righting moment for a given buoy draft and tilt angle was developed. This computation was included in the program as subroutine TORBU, and it updated the buoyant force and moment for each integration time step.

Consider a torroid with major radius R and minor radius r, partially immersed in a fluid with a mean draft H and a tilt angle θ (figure E-1. The area of an immersed circular segment with draft h_s is given by

$$A_s = \frac{1}{2} r^2 (\alpha - SIN\alpha), \qquad (E-1)$$

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$$\propto = 2 \text{ TAN}^{-1} \sqrt{\frac{r^2}{(r-H_s)^2} - 1}$$

If $H_{s} > r$, we can redefine the draft

$$H_{s}' = 2r - H_{s}, \qquad (E-2)$$

and



$$x' = 2 \text{ TAN}^{-1} \sqrt{\frac{r^2}{(r-H_s')^2} - 1}$$

Thus, the area becomes

$$A_{s} = \pi r^{2} - \frac{1}{2}r^{2}(\alpha - SIN\alpha') \qquad (E-3)$$

Define the section draft, H_s , as a function of ϕ , the radial angle about the torroid axis. The maximum draft in the direction of the tilt angle θ ($\phi = \pi/2$) is

and the minimum draft is

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 $H_{S_{MIN}} = H - R S IN \theta$. For any angle ϕ around the torroid,

$$H_{s} = H + R \sin\phi \sin\theta + (\Xi_{q} - H)(1 - \sin\theta), \qquad (E-4)$$

The last term represents the small change in the mean draft H due to the fact that the tilt is about the center of gravity and not about the waterplane center.

By using the draft for any section as defined above, we can compute the immersed area of any section from equation (E-1) or (E-3). The immersed volume is found by integrating around the torroid,

$$V = 2 \cdot \int_{-\frac{\pi}{2}}^{\frac{\pi}{2}} A_{s} R d\phi , \qquad (E-5)$$

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$$B = 2 \cdot \vartheta \cdot R \int_{-\pi_2}^{\pi_2} A_s \, d\phi \,, \qquad (E-6)$$

where \mathcal{J} is the weight density of the fluid. The centroid of the submerged volume V is computed in order to find the righting moment:

$$\overline{X} = \frac{2 \cdot \int_{-\frac{\pi}{2}}^{\frac{\pi}{2}} R \sin \phi A_{s} R d\phi}{2 \int_{-\frac{\pi}{2}}^{\frac{\pi}{2}} A_{s} R d\phi}$$
(E-7)

The righting arm is

$$X_{RA} = \overline{X} \cos\theta - (Z_{G} - H) \sin\theta , \qquad (E-8)$$

and the righting moment is

$$M = X_{\rm RA} \cdot B \tag{E-9}$$

The above equations were programmed for subroutine TORBU, and the differential volumes were summed in 2-deg steps around the torroid. Puoy draft and tilt angle are the inputs, and the heave buoyant force, tilt righting moment, cross-coupled tilt-heave buoyant force, and cross-coupled heave-tilt righting moment are the outputs. Plots of the displacement, righting moment, and the cross-coupled force and moment for the Richardson torroid used at station BRAVO are shown in figures (E-2) through (E-5).



Figure E-2. Torroid (8-ft) Displacement versus Draft



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Figure E-3. Torroid (8-ft) Righting Moment versus Tilt Angle



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Figure E-4. Torroid (8-ft) Coupled Moment versus Tilt Angle



