

NEW ENGLAND DIVISION
CORPS OF ENGINEERS, U. S. ARMY
BOSTON, MASSACHUSETTS



DATA REPORT OF FROST INVESTIGATIONS FISCAL YEARS 1943 - 1949



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DATA REPORT OF FROST INVESTIGATIONS FISCAL YEARS 1943 - 1949

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DATA REPORT OF FROST INVESTIGATIONS FISCAL YEARS 1943 - 1949

SYNOPSIS

This report presents, in three volumes, a summary of the frost investigations which were conducted under the supervision of the Frost Effects Laboratory from 1943 through 1949, together with all data obtained from these investigations.

Frost studies were first initiated at Dow Field, Bangor,
Maine, in the winter and spring of 1944 to determine the influence of frost action in the subgrade soils beneath both rigid
and flexible pavements upon the load carrying capacity of the
pavements during the frost melting period. The need for such
studies became apparent during the pavement evaluation program
in 1943 when it became obvious that, during the spring thaws,
evaluations of many airfields in the northern United States
were controlled by conditions resulting from frost action in
the subgrade soils. Since limited data were available at that
time on the effects of frost action under airfield pavements, an
extensive program of investigations were authorized by the Chief
of Engineers for the purpose of developing and establishing design
and evaluation criteria for such pavements.

Frost investigations were conducted in the New England
Division, the Missouri River Division, and the Great Lakes
Division under the direction of the Frost Effects Laboratory,
New England Division. The observations and testing of the

effects of frost action were studied during the fiscal years
1943 - 1946 at 17 airfields located in the northern United States.

Several laboratory studies were made on different phases of the frost-soil problem in addition to investigations conducted at various airfields to study the field effects. Theoretical studies to determine rates and depth of frost penetration were made. Thermal and physical constants necessary for mathematical analysis were obtained from tests performed under controlled temperature conditions to determine the thermal conductivity of frozen and unfrozen cohesionless materials. Work of previous investigators in this field was reviewed. A study was made of methods of making frost susceptible soils nonfrost susceptible by the use of various admixtures and also of methods of preventing the leaching out of salt admixtures.

The data in the Data Report were presented in 36 separate reports that were prepared during the various stages of the frost investigational program.

I. INTRODUCTION

- 1-01. Authorization. The preparation of this Data Report as part of the frost investigation program for fiscal year 1948-1949 was authorized by letter dated 26 October 1949 from the Office, Chief of Engineers to Division Engineer, New England Division, subject: "Authorization New England Division FY 1949 Airfields Investigational Program".
- 1-02. Purpose. The overall purpose of the frost investigational program is to develop and establish design and evaluation criteria for concrete and flexible pavements placed on subgrade or base soils subject to seasonal frost action. The specific purpose of this report is to present in unified form all data and results of tests obtained during the four years of frost investigations conducted at 17 different sites in northern United States together with results of the supplemental laboratory and theoretical studies.
- 1-03. Scope. This report presents all data obtained from frost investigations for both field and laboratory tests conducted under the supervision of the Frost Effects Laboratory.

 No analyses of data or conclusions are presented.
- 1-04. Locations of Field Investigations. The field investigations were conducted at the following Army Air Force installations:

NEW ENGLAND DIVISION

SITE	NORTH LAT.	WEST LONG.	ELEV. ABOVE MSL	PHYSI OGRAPHY
Presque Isle Airfield Presque Isle, Maine	47°	68°	500	Glaciated region of rolling hills.
Houlton Airfield Houlton, Maine	460	68°	470	Narrow valley flank- ed by high hills
Dow Field Bangor, Maine	45°	69°	170	Glaciated region of rolling hills
Bedford Airfield Bedford, Massachusetts	42°	62°	130	Rolling terrain of low relief
Otis Field Sandwich, Massachusetts	42°	70°	120	Flat outwash plain
GREAT 1	LAKES D	IVISION		
Truax Field Madison, Wisconsin	43°	89°	860	Low level marsh
Selfridge Field Mt. Clemens, Michigan	43°	83°	580	Level lake plain
MISSOUR	I RIVER	DIVISIO	<u>N</u>	
Pierre Airfield Mt. Clemens, Michigan	朴。	100°	1720	Ravines to pre- dominating flat plateau
Fargo Municipal Airfield Fargo, North Dakota	47°	97°	900	Bed of ancient lake - very flat.
Bismarck Municipal Airfield	47°	101°	1650	Ascending and descending benches
Bismarck, North Dakota				
Watertown Airfield Watertown, South Dakota	45°	97°	1730	Flat to rolling
Casper Airfield Casper, Wyoming	43°	107°	5320	Gullies to rolling hills, mountains to south

SITE	NORTH LAT.	WEST LONG.	ELEV. ABOVE MSL	PHYSI OGRAPHY
Sioux Falls Airfield Sioux Falls, South Dak	Щ ^о ota	96°	1420	Flat flood plain
Fairmont Airfield Fairmont, Nebraska	41°	98°	1630	Flat plain
Great Bend Airfield Great Bend, Kansas	39°	98°	1890	Wide flat valley
Garden City Airfield Garden City, Kansas	38°	101°	2880	Flat to slightly undulating prairie land
Pratt Airfield Pratt, Kansas	38°	99°	1950	Gently rolling prairie land inter- spersed with low knolls and occa- sional shallow ponds or "buffalo wallows"

1-05. Frost Investigation Reports. The data presented originally appeared in 36 separate volumes covering various phases of the frost investigational program, from 1943 to 1949 and have been combined and assembled in this Data Report. The complete list of reports on frost investigations prepared by the Frost Effects Laboratory is as follows:

TESTS IN FISCAL YEAR	TITLE OF REPORT	REPO!	
1943-1944	Frost Investigations and Pavement Behavior Tests at Dow Field	Jan.	1946
1944-1945	Frost Investigation 1944-1945 (prepared by Missouri River Division - contains reports on Sioux Falls Airfield, Fairmont Airfield, Great Bend Airfield, Garden City Airfield, and Pratt Airfield)	Jul.	1945

TESTS IN FISCAL YEAR	TITL	OF REPORT	REPOR DATEI	
1944-1945	Comprehensive Re	port, Frost Investigation	Feb.	1947
	Appendix 1	Dow Field		
		Presque Isle Airfield		
	Appendi x 3			
	Appendix 4	Houlton Airfield		
	Appendix 5	Truax Field		
	Appendix 6	Pierre Airfield		
		Watertown Airfield		
	Appendix 8	Casper Airfield		
	Appendix 9	Fargo Municipal Airfield		
•	Appendix 10	Bismarck Municipal Airfield	d	
1944-1945	Appendix 11	Subsurface Temperature Investigations at Fierre and		
		Watertown Airfields		
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		Lakes Division and Missour		
		River Division		
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1944-1945		Investigations 1944-1945		
	(Published)		Apr.	1947
1945-1946		eport, Frost Investigations		
	1945-1946		June	1947
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		Sioux Falls Airfield		
		Watertown Airfield		
		Fargo Municipal Airfield		
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1945-1946		Investigations and Traffic		
	Tests - Selfrid	ge Field	June	1946

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TESTS IN FISCAL YEAR	TITLE OF REPORT	REPOR DATEI	-
1945-1946	Report on Studies of Base Course Treatment to Prevent Frost Action	June	1946
1946-1947	Comprehensive Report, Frost Investi- gations, 1946-1947 Appendix 1 NED Investigations (Dow and Bedford) Appendix 2 GLD Investigations (Selfridge)	Apr.	1948
	Appendix 3 MRD Investigations (Sioux Falls and Fargo)		
1946-1947	Report on Studies of Base Course Treatment to Prevent Frost Action	Aug.	1947
194 7- 1948	Summary Tabulation of Airfield Pavements (Draft)	June	1948
1947-1948	Addendum No. 1 to Report on Frost Investigation, 194:-1945 (in draft form - to be published)	June	1948

- 1-06. Presentation of Data Report. The Data Report is presented in three volumes as follows:
- a. Volume I presents a summary of the frost investigation program together with a list and description of airfields investigated and the tests performed. Reports on special studies are presented as appendices in Volume I as follows:
 - Appendix A Report on Studies of Base Course Treatment to Prevent Frost Action.
 - Appendix B Report on Laboratory Tests on Frost
 Penetration and Thermal Conductivity
 of Cohesionless Soils
 - Appendix C Report on Mathematical Studies of Thermal Changes in a Soil Mass.
 - Appendix D Report on Special Test Section at Dow Field, Bangor, Maine.

Appendix E Laboratory and Field Test Procedures Appendix F Bibliography b. Volume II presents the data obtained from all fields investigated in the New England Division as follows: Presque Isle Airfield Houlton Airfield (3) Dow Field (4) Bedford Airfield Otis Field c. Volume III presents the data obtained from all frost investigations conducted in the Great Lakes Division and the Missouri River Division. The following fields are located in the Great Lakes Division: (1) Truax Field(2) Selfridge Field The fields investigated in the dissouri River Division are: Pierre Airfield Fargo Municipal Airfield Bismarck Municipal Airfield Watertown Airfield Casper Airfield Sioux Falls Airfield Fairmont Airfield (8)Great Bend Airfield Garden City Airfield (9) (10) Pratt Airfield 1-07. Description of Frost Action. Frost action is defined as the physical phenomenon by which layers or lenses of ice are built up within a soil mass. Three conditions must occur simultaneously for these ice layers to form. These are as follows: a. SOIL. Frost Action within a soil is a function of its void size which may be conveniently expressed as a - 8 -

function of grain size. In this investigation, any soil which contains three per cent or more by weight of grains smaller than 0.02 mm. is considered frost susceptible and a soil in which frost action is possible.

b. WATER. Frost Action depends upon the availability of water either by virtue of an adjacent ground water table, a capillary supply, or water within the soil voids.

c. TEMPERATURE. Frost action within soils requires the maintenance of freezing temperature slightly below the surface of ice lens formation. The greatest accumulation of ice will occur when the penetration of the freezing temperature is slow; a rapid penetration may result in few or no ice lenses.

The process of frost action may be described as follows: The water in the void spaces becomes cooled below the normal freezing temperature of water. This supercooled water has a high molecular attraction to ice crystals. Thus, the supercooled water travels to ice crystals, which form in the larger voids, solidifying upon contact. This process repeated forms an ice lens. A single lens will continue to grow in thickness, always against the direction of heat transfer, until the formation of a lens at a lower elevation cuts off the source of water, or until the temperature rises above freezing.

Frost heaving is directly associated with frost action and is the visible evidence on the surface that ice lenses have formed in the soil mass. The frost boils, as referred to by highway engineers, are caused by a rapid thawing

of an area of severe frost action beneath a flexible pavement. Such thawing occurs largely from the surface down and the excess water liberated from the thawed area is prevented from draining downward by the still frozen underlying soil and ice layers. The excess water causes the thawed soil to become exceedingly soft. Likewise the pumping of water from joints in concrete slabs during the spring may be the result of excess water liberated from thawed ice layers in the subgrade.

1-08. Definitions. In this report certain terms and words are used with specialized meaning. They are defined as follows:

- (1) TEST AREA. The test area is the portion of the airfield selected for observations and investigations.
- (2) TRAFFIC TEST AREA. The traffic test area is the portion of the test area subjected to traffic tests.
- (3) TEST LANE. A test lane is the portion of the traffic test area subjected to a specific number of repeated wheel loads per day.
- (4) TURNARCUND. A turnaround is the portion of the traffic test area used for turning traffic equipment.
- (5) PASS. A pass is one movement of the traffic test equipment over a test lane.

- (6) TRAFFIC. Traffic is the operation of making passes of the testing equipment over the traffic test areas.
- (7) COVERAGE. One coverage is one application of a definite wheel load over each point in a given test lane.
- (8) CYCLE. One cycle of coverages equals the coverages applied during one day.
- (9) PAVEMENT. The term pavement is defined as a covering of a prepared or manufactured product superimposed upon a subgrade or base to serve as an abrasive and weather resisting structura medium.
- (10) BASE. The term base applies to the course of specially selected soils, minerals, aggregates or treated soils placed and compacted on the natural or compacted subgrade.
- (11) SUBGRADE. The term subgrade applies to the natural soil in place or to fill material upon which a pavement or base is constructed.
- (12) FLEXING. Flexing is the visible spring or vertical elastic movement of the pavement under a moving wheel load.
- (13) MAP CRACKING. Map cracking is the developmen of a definite crack pattern in the pavement surface under the action of repeated loadings

Map cracking is distinguished by the formation of continuous connected cracks enclosing polygonal pavement segments.

- (14) CONSOLIDATION. Consolidation is the increase in unit weight per unit volume, or decrease in volume of a given weight of a material due to the action of applied loadings. Consolidation is considered to be synonymous with compaction in this report.
- (15) PERMANENT OR VERTICAL DEFORMATION. Permanent or vertical deformation is the accumulative non-elastic part of the total vertical movement of the surface of the pavement which remains after the load is removed.
- (16) FROZEN SOIL. Frozen soil is referred to in this report as follows:
 - ly frozen soil is a soil in which water in the soil is frozen within the natural voids existing in the soil, without observable accumulation of ice lenses of frost forms exceeding in volume such natural void spaces.
 - (b) Stratified Frozen Soil. A stratified frozen soil is a soil in which a part of the water in the soil is frozen in

in the form of observable ice lenses, occupying space in excess of the original soil voids.

- (17) ICE CRYSTALS. The formation of ice particles found in the pores of homogeneous frozen soil is referred to as ice crystals.
- (18) ICE LENSES. Ice lenses are the ice formations in stratified frozen soil occurring in repeated layers essentially parallel to each other and normal to the direction of heat loss.
- (19) FROZEN ZONE. The limits of depth within which the soil is frozen is referred to as the frozen zone.
- (20) FORST PENETRATION. The maximum depth from the surface to the bottom of the frozen soil.
- (21) DEPTH OF FREEZING TEMPERATURE PENETRATION. The depth of freezing temperature penetration is the maximum depth below the surface of freezing temperature.
- (22) FROST ACTION. Frost action is the accumulation of water in the form of ice lenses in the soil under natural freezing conditions.
- (23) FROST HEAVE. Frost heave is the raising of the pavement surface due to the accumulation of ice lenses. The amount of heave in most soils is approximately equal to the cumulative

thickness of ice lenses.

- (24) FROST SUSCEPTIBLE SOIL. Frost susceptible soil is a soil in which frost action is possible. Any soil which contains three per cent or more by weight of grains smaller than 0102 mm. diameter shall be considered susceptible to frost action.
- (25) NON-FROST SUSCEPTIBLE MATERIALS. Non-frost susceptible materials are crushed rock, clean sand and gravel, gravel, slag, cinders, or any other cohesionless material in which frost action is not possible.
- the mean daily temperature varies from 32°F is called a degree day. The difference between the daily mean temperature and 32°F equals the degree-days for that day. The degree-days are plus when the daily mean temperature is below 32°F and minus when above. A cumulative degree days-time curve is obtained by plotting cumulative degree-days against time.
- (27) DEGREE HOUR. A degree hour is the cumulative total of degrees per hour below 32°F.
- (23) FREEZING PERIOD. The freezing period is the time during which the frost is in the ground and there is no reduction in strength of foundation materials due to frost action.

- (29) FROST MELTING PERIOD. The frost melting period is the time of the year during which the frost in the foundation materials is returning to a liquid state.
- (30) NORMAL PERIOD. The normal period is the time of the year, summer and fall, when there is no reduction in strength of foundation materials due to frost action.
- of the combined duration and magnitude of below-freezing air temperatures occurring during any given winter and is the maximum ordinate of the degree days time curve.
- (32) NORMAL FREEZING INDEX. Normal freezing index is computed for normal air temperatures based upon a long period of record, usually 10 years or more.
- (33) GROUND WATER TABLE. The ground water table is the free water surface nearest to the ground surface.
- (34) DENSITY. Density is the unit dry weight in pounds per cubic foot.
- (35) WATER CONTENT. Water content is the ratio, expressed as a percentage, of the weight of water in a given soil mass to the weight of solid particles.

(36) DEGREE OF SATURATION. The ratio, expressed as a percentage, of the wolume of water in a given soil mass to the total volume of intergranular space. Pre cent saturation is synonymous with degree of saturation in this report.

II. INVESTIGATIONS

- 2-01. General. The effects of frost action were studied at 17 airfields located in the northern United States as shown on map, Plate

 1. A total of 44 test areas were investigated at these airfields.

 Twenty-four test areas had flexible pavements and sixteen test areas had rigid pavements. Four turfed areas adjacent to paved areas were investigated. The individual test areas were selected to encompass as closely as possible the full range of the following variables influencing frost action.
 - a. Air temperatures ranging from mild to extreme in severity.
 - b. Ground water table varying from an elevation near the surface of the pavement to an elevation greater than 90 feet below the pavement surface.
 - c. Precipitation prior to freezing period varying from light to relatively moderate.
 - d. Base and subgrede materials varying in water content from relatively dry to saturated.
 - o. Subgrades varying from plastic fat clay to non-plastic silty gravelly sand.
 - f. Base materials varying from plastic sand-clay-gravel to crushed rock.
 - g. Rigid and floxible pavements.
 - h. Pavement designs which would support light to heavy aircraft.
- 2-02. Tests Conducted. Conditions beneath pavements were observed by means of test pits and auger borings during the various seasons of the year, particularly during and shortly following the frost melting period. A great many variables were involved and to encompass them the following observations and measurements were made:

- a. <u>Comprehensive Investigations</u>. At some sites more extensive or comprehensive investigations were conducted than at others. Comprehensive investigations consisted generally of observing and recording the following data and performing the indicated tests in selected test areas as follows:
 - (1) Continuous air temperature during the freezing and frost melting period.
 - (2) Subsurface temperatures under pavements during freezing and frost melting period.
 - (3) Precipitation prior to and during freezing period.
 - (4) Measurement of pavement heave.
 - (5) Location of ground water table in base and subgrade.
 - (6) Ice lens formation in base and subgrade during the freezing and frost melting period.
 - (7) Water content and density variation with depth in base and subgrade during summer, winter and spring.
 - (8) Conditions under turfed areas with and without snow cover.
 - (9) Pavement bearing tests.
 - (10) California Bearing Ratio tests on subgrades during normal and frost melting periods.
 - (11) Laboratory tests on soil samples obtained from test areas.
- b. <u>Limited Investigations</u>. Some airfields were selected for obtaining the minimum data believed to be basic for understanding the effect of frost action at the site. These less comprehensive studies, referred to hereinafter as limited investigations, consisted generally of items (1) through (7), as listed above.

Subsurface temperatures were measured by means of thermocouples and/or thermometers instalked at various depths below the pavement (and turf) in selected areas.

The reduction in bearing capacity of the runways was measured by pavement bearing tests conducted on the surface of the pavement and also on top the base during the frost melting period for comparison with similar tests conducted in the summer and fall. Tests were also performed with a 24-inch diameter steel plate placed at corners of rigid slabs to determine loads required to cause rupture during normal and frost melting periods. Field CBR tests were performed at some of the sites.

Traffic tests with light and heavy wheel loads were conducted at four of the sites to obtain quantitative results for immediate application in evaluating pavements at airfields where a reduced strength during the frost melting period would limit the evaluation. Traffic tests were conducted at Dow Field, Truax Field, Selfridge Field, and Pierre Airfield.

The types of investigations conducted at each airfield during the various fiscal years are shown in the following tabulation:

PRESQUE ISLE AIRFIELD

TYPE OF INVESTIGATION

1943-44

1944-45

1945-46

1946-47

COMPREHENSIVE

X X

LIMITED

TRIFFIC TESTS

HOULTON AIRFIELD

TYPE OF INVESTIGATION

1943-44

1944-45

1945-46

1946-47

COMPREHENSIVE

X

TR/FFIC TESTS

DOW FIELD

TYPE OF INVESTIGATION	1943-44	FISC.	1946-47	
COMPREHENSIVE		Х	X	x
LIMITED	x			
TRAFFIC TESTS	x	x		

REMIRKS: Special Test Section constructed for study during 1946-47.

Results of observations and details of construction presented in Appendix D.

BEDFORD AIRFIELD

TYPE OF INVESTIGATION				
	1943-44	1944-45	1945-46	1946-47
COMPREHENSIVE				
LIMITED			x	x
TR/FFIC TESTS				

OTIS FIELD

TYPE OF INVESTIGATION	1943-44	FISC. 1944-45	1945-46	1946-47
COMPREHENSIVE				
LIMITED		x		
TR/FFIC TESTS				

TRUIX FIELD

TYPE OF INVESTIGATION	1943-44	FISC. 1944-45	1946-47	
	29-7	2,44-4/	1945-46	1940-4
COMPREHENSIVE		x	x	
LIMITED				
TR/FFIC TESTS		x		

SELFRIDGE FIELD

TYPE OF INVESTIGATION

1943-44

1944-45

1945-46

1946-47

X

TR/FFIC TESTS

X

PIERRE JRFIELD

TYPE OF INVESTIGATION

1943-44

1944-45

1945-46

1946-47

COMPREHENSIVE

X

LIMITED

TRAFFIC TESTS

X

REMARKS: Surface water infiltration studies conducted during 1944-45

F/RGO MUNICIPIL / IRFIELD

TYPE OF INVESTIG.TION	1943-44	FISC. 1944-45	1946-47	
COMPREHENSIVE		x		
LIMITED			X	x
TR/FFIC TESTS				

BISMARCK MUNICIPAL AIRFIELD

TYPE OF INVESTIGATION 1944-45 1946-47 1943-44

COMPREHENSIVE

X

LIMITED

TRAFFIC TESTS

WATERTOWN AIRFIELD

FISCAL YEAR 1944-45 1945-TYPE OF INVESTIGATION 1946-47 1943-44

COMPREHENSIVE

X

LIMITED

X

TRAFFIC TESTS

PEMARKS: Surface water infiltration studies conducted during

1944-45

CASPER AIRFIELD

TYPE OF INVESTIGATION 1946-47 1943-44

COMPREHENSIVE

X

LIMITED

TRAFFIC TESTS

REMARKS: Surface water infiltration studies conducted during

1944-45

SIOUX FALLS AIRFIELD

TYPE OF INVESTIGATION

1943-44

1944-45

1946-47

COMPREHENSIVE

X

X

TRAFFIC TESTS

REMARKS: Surface water infiltration studies conducted during 1944-45

FAIRMONT AIRFIELD

TYPE OF INVESTIGATION

1943-44

1944-45

1945-46

1946-47

COMPREHENSIVE

LIMITED X

TRAFFIC TESTS

REMARKS: Surface water infiltration studies conducted during 1944-45

GREAT BEND AIRFIELD

TYPE OF INVESTIGATION

1943-44

1944-45

1945-46

1946-47

COMPREHENSIVE

LIMITED X X

TRAFFIC TESTS

REM/RKS: Surface water infiltration studies conducted during 1944-45

GARDEN CITY AIRFIELD

TYPE OF INVESTIGATION FISCAL YEAR

1945-44 1944-45 1945-46 1946-47

COMPREHENSIVE

X

LIMITED

TRAFFIC TESTS

PRATT AIRFIELD

TYPE OF INVESTIGATION FISCAL YEAR
1943-44 1944-45 1945-46 1946-47

COMPREHENSIVE

LIMITED

X

TR/FFIC TESTS

REMIRKS: Surface water infiltration studies conducted during 1944-45

2-03. Methods and Tests Procedures. The methods and test procedures that have been used in connection with the frost investigations in each of the three Divisions are presented in detail in Appendix E, Volume I.

III. DATA FOR AIRFIELD INVESTIGATED

3-01. General. A description of each airfield at which tests were conducted is presented in the following paragraphs. Weather data, grain size curves, classification of base and subgrade materials, typical logs and other pertinent data for each airfield are presented on plates in Volumes II and III. The selection of airfields was based upon particular characteristics such as weather, ground water, soil type, and other conditions which would influence frost action. These conditions are noted for each airfield. A geographic location map showing the location of all airfields is presented on Plate 1, Volume I. Temperature data, precipitation data, and normal freezing data are shown in Fig. 2, Plate 2, Volume I.

3-02. Descriptive Airfield Data.

mental frost action experienced during previous winters. The airfield is located in the northeastern part of Maine in the city of Presque Isle. The region is hilly and glaciated. The normal freezing index, based on a 31-year record, is 2061. The normal rainfall for the three month period of September, October and Movember is 10 inches. Three test areas representing portland cement concrete, bituminous concrete pavements, and turfed surfaces were selected for investigation. The rigid pavement, seven inches thick, is constructed on 30 to 36 inches of sand and gravel base; the flexible pavement, four inches thick is constructed on 24 to 30 inches of sand and gravel base. The subgrade is a frost susceptible clayer silt, sand and gravel mixture (GC) with 10 to 35 per cent by weight firer than 0.02 mm. grain size. The ground water table is slightly below six feet in depth, except during

the frost melting period when it rises to about two feet below the pavement surface. During the winter of 1942-1943 approximately 500 square yards of runway pavement heaved.

Comprehensive frost investigations were conducted for a period of two consecutive years at this site, during 1944-1945 and 1945-1946. The results of tests and observations obtained during these periods are presented in Volume II on Plates 6 to 43 inclusive.

b. Houlton Airfield. This site is in Arcostock County, Houlton, Maine. The terrain and weather are similar to Presque Isle Airfield. Houlton Airfield is located in a narrow valley flanked on the sides by relatively high hills. The normal freezing index is 1780 based on a 41-year record. The normal rainfall during the three months period preceding freezing is 9 inches. Two test areas were selected. One test area is located on the parking apron and has a seil coment base with 1-1/2 inches of bituminous concrete wearing course. The other test area, located on the N-S runway, has six inches of sand and gravel base underlying four inches of bituminous concrete wearing course. The subgrade is a frest susceptible silty sand and gravel (GF) with 6 to 15 per cent by weight finer than 0.02 mm. grain size. The ground water table was generally below the explored depth of six feet. No serious heaving or pavement failures due to frest action have been netted during the operation of the airfield.

Limited frost investigations were conducted at Houlton Airfield during 1944-1945. All data are shown on Plates 44 to 49 inclusive, Volume II.

c. <u>Dow Field</u>. This site was selected because of detrimental frost action during previous winters and the availability of data obtained from previous frost study at this airfield. Dow Field is located two miles

west of the city of Bangor, Maine. The region consists of rolling terrain with hills composed of a thin mantle of slightly gravelly silt (glacial till) overlying bedrock. In the low areas the glacial till is overlain by a layer of silty clay. The normal freezing index is 1275 on the basis of a ten-year record. The normal rainfall during the three month period preceding freezing is 11 inches. Fourteen test areas were investigated which included six with portland cement concrete pavement, seven with bituminous concrete pavement and one with turfed surface. Generally the rigid pavement is seven inches thick, overlying approximately 15 inches of sand and gravel base. flexible pavement in the test areas is generally 3.5 inches thick overlying a sand and gravel base varying from 24 to 63 inches in thickness. The subgrade underlying pavements and turfed areas is generally a silty clay (CL) with 40 to 97 per cent by weight finer than 0.02 mm. grain size. The ground water table is from four to six feet below the surface and rises to a depth of one to four feet during the frost melting period. Frost action was studied at Dow Field in three test areas as a part of the Pavement Evaluation Program during the summer and fall of 1943 and winter of 1943-1944. During this previous investigation, a glacial till subgrade (GC) was encountered in addition to the silty clay (CL) subgrade. Comprehensive frost investigations were conducted at Dow Field for a period of four years, beginning in the fall of 1943. Eighteen test areas were investigated at this site including runway shoulders and a turfed area. Locations of the test areas investigated are shown on Plate 50. Complete data from investigations at Dow Field are presented on Plates 50 to Plates 151, inclusive. Traffic tests using various whool loads were conducted during the spring of 1944 and 1945 on both

^{*} Traffic tests conducted in the Spring of 1944 are referred to on the plates as Pavement Behavior Tests.

bituminous and rigid pavements to determine the load carrying capacity of the pavements over a weakened subgrade caused by frost action. Photographs of equipment used during these tests and data obtained are shown on Plates 117 to 151, inclusive.

Bedford Airfield. This site which is located approximately 14 miles northwest of the city of Boston in the towns of Bedford, Concord and Lexington, Massachusetts, was selected chiefly to determine the extent of frost development in non-frost susceptible soils and to augment the tests made and data obtained in the frost investigations at other airfields. The airfield site is an area of rolling terrain of low relief. The hills surrounding the field are composed of glacial till with bedrock generally at shallow depths or exposed. The low area at the site generally consists of a glacio-fluvial deposit of sand with gravelly phases varying in thickness from 3 to 10 fect overlying silt and fine sand of depths in excess of 15 foot. The normal freezing index based on 56-year record is 680. The normal rainfall for the three month period prior to freezing is 10.2 inches. Two test areas were selected. One test area is located in a portland cement parking apron. The other test area is located on a portion of bituminous paved runway. The free draining sand and gravel base in both areas consists of a GW material with three per cent by weight finer than 0.02 mm. The subgrade consists of a well graded sand (SW) with from zero to ten per cent by weight finer than 0.02 mm. The ground water table ranges from 4 to 8 feet below the surface.

The limited investigations at this site were conducted for two years, 1945-1946 and 1946-1947. Results are shown on Flates 152 to 161, inclusive.

Otis Field. Otis Field, selected because of previous occurrences of frost action, is located within the limits of Camp Edwards, Massachusetts. The site is of glacio-fluvial origin and is part of an extensive outwash plain. The area is generally flat consisting of extensive deposits of variable sands and gravels with occasional boulders. Winter temperatures at Otis Field are fairly mild, with a normal freezing index of 202 based on a 21-year record. The normal rainfall for three months period preceding freezing is 13 inches. The test area is located in a cut section on the NE-SW runway. The flexible pavement, five to seven inches thick, overlies a non-uniform subgrade generally consisting of intermixed pockets of sands, silts, and gravels resulting in several gradations of frost susceptible soils in the upper portion of the subgrade. The subgrade at greater depths consists of fine to medium sand with occasional gravel and small quantities of silt. Ground water was not encountered at an explored depth of 15 feet. Frost action was observed in January 1943 when pavement heaves had developed at several locations on the paved runways. Differential heaves of three to six inches occurred in unsealed portions of the bituminous concrete pavement.

Limited frost investigations were conducted during one year only, 1944-1945. Results of observations, including some data obtained in 1943 are presented on Plates 162 to 167, inclusive.

area forming one of the upper reaches of the marshes along the Yahara River and Lakes Mendo to and Morona at Madison, Visconsin. The winter temperatures are severe with a normal freezing index of 1227 based on a 43-year record. The normal rainfall for the three month period preceding freezing is 7 inches.

Three test areas were selected. Two test areas consisted of bituminous concrete and one consisted of portland cement concrete pavements. The bituminous concrete pavement, 2.5 inches thick, is constructed on a crushed rock base, and a sandy clay and gravel (GF) sub-base. The two flexible pavement test areas, located on a runway and taxiway, differ in the thickness of base and sub-base. The taxiway test area has eight inches of base and 15 to 17 inches of sub-base. The runway test area has eight inches of base and 21 to 31 inches of sub-base. The portland cement concrete test area, located on the parking apron, consists of a six-inch slab which was constructed on a base of sand-clay-gravel (GF) varying from about three to five feet in thickness. The original subgrade is a silty clay (CL) with lenticular deposits of fine sand occurring at varying elevations. The ground water table is fairly uniform throughout the field, normally varying from six to eight feet below the pavement surfaces.

In addition to comprehensive investigations in both the rigid and flexible pavement test areas for two years, 1944-1945 and 1945-1946, a traffic test was conducted during the first year of investigations. The test results and observations are presented on Plates 168 to 207 inclusive, in Volume III of this report.

east of Mt. Clemens and 30 miles northeast of Detroit, Michigan. The field is located on a level lake plain adjacent to Lake St. Clair. The normal freezing index based on a 46 year record is 521. The normal precipitation for the three month period, September to November inclusive, is 7.7 inches. A test area on the portland cement concrete parking apron was selected for observation and study. The 10-inch concrete slab thickened to 12 inches on

the edge is placed on approximately 12 inches of sand and gravel (GF). The subgrade immediately beneath the base consists of a sandy silt (ML) overlying a sand with a high percentage of fines (SF) underlain by lean clay (CL). The base contains from two to five per cent by weight finer than 0.02 mm. All the subgrade soils are frost susceptible. The ground water table varies from approximately 3.5 feet to 6.0 feet below the surface.

Comprehensive frost investigations were made during 1945-1946, including a traffic test. The specific purpose of the traffic test at this site was to determine the supporting capacity of the portland cement pavement by means of a 60,000-pound load on a B-29 dual wheel assembly during and directly after the frost melting period. Limited frost investigations were conducted during 1946-1947. The results of these investigations are presented on Plates 208 to 232 inclusive.

h. <u>Fiorro Airfield</u>. Pierre Airfield, approximately three miles northeast of the city of Pierre, South Dakota, is located on a relatively level plateau about two miles north of the Missouri River. The normal freezing index is 1294 on basis of 46-year record and the ground water table is at a depth greater than 25 feet from the surface. The normal rainfall during the three months preceding freezing is 2 to 4 inches. This airfield was selected to determine the effects of frost in an area having a low annual precipitation, low water table, and naturally dry subsurface soil conditions. Three test areas consisting of portland cement concrete, bituminous concrete, and turfed surfaces were selected for investigations. The rigid pavement, seven inches thick, was constructed on 7 to 14.5 inches of sand and gravel base. The flexible pavement, 5.5 inches thick, overlies

a sand and gravel base of 6 to 15.5 inches thickness. The subgrade is a mixture of clay, silt, and sand (CL) susceptible to frost action. No serious heaving of pavements or pavement distress due to frost action has been noted during the period of operation of the airfield.

Comprehensive frost investigations were conducted for a period of two years, 1944-1945 and 1945-1946. Results of tests and observations are shown on Plates 233 to 268, inclusive in Volume III.

Fargo Municipal Airfield. Fargo Municipal Airfield is located approximately 1.5 miles northwest of the city limits of Fargo, North Dakota. The airfield is located on a generally smooth, flat plain, originally the bed of an ancient glacial lake. Winter temperatures at Fargo Airfield are the most severe of all the 17 airfields investigated. The normal freezing index, based on a 63-year record, is 2646. The normal rainfall for the three month period preceding freezing is 3.7 inches. One test area was investigated which consisted of 1.5 inches of bituminous concrete wearing surface constructed over a soil cement base course having a thickness of approximately 6.5 inches. A sub-base of sand and clay material (CL-SF), approximately 15 inches in thickness, overlies about eight inches of black clay with sand gravel and cinders (OH-CH). The subgrade is a medium fat to fat clay (CL). The ground water table during the freezing period varies from five to seven feet in depth below the pavement. During the frost melting period it rises to a depth of three fect. The sub-base and subgrade materials are considered susceptible to frost action. A moderate amount of frost action occurred during this investigation but is not considered detrimental to the pavement.

Comprehensive tests and observations were conducted during 1944-1945 and limited investigations were made during the following two years in the same bituminous paved area. The results of tests and observations are presented on Plates 269 to 277, inclusive, Volume III.

Bismarck Municipal Airfield. Bismarck Municipal Airfield j. is located south of the southeast limits of the city of Bismarck adjacent to Fort Lincoln, North Dakota. The airfield site is on a relatively flat, elevated bench about two miles east of the Missouri River. Winter temperatures are extreme with a normal freezing index of 2552 based on a 69-year record. The normal rainfall for the three month period preceding freezing is 2.6 inches. One test area of bituminous concrete pavement located on a runway was selected for investigation. The pavement is 4.5 inches thick and was constructed on a six-inch sand and gravel base course and approximately three feet of frost susceptible silt and fine sand (CL-ML) subgrade overlying sand and gravelly materials to a depth of approximately 12 feet where a very compact clay is encountered. This results in the formation of a perched water table at a depth of about 12 feet below the surface. The normal elevation of natural ground water is approximately 40 feet below the surface. Prior to the period of frost investigation, no indications of frost heaving or other major present changes due to frost action have been noted.

Comprehensive frost investigations were conducted at Bismarck Municipal Airfield during one year only 19/4-19/5 in a flexible pavement area. Result of tests and observations are presented on Plates 278 to 282, inclusive.

k. Watertown Airfield. Watertown Airfield is located adjacent to the northwest city limits of Watertown, South Dakota. The general terrain

of the airfield site varies from flat to rolling. Winter temperatures are generally severe with a normal freezing index of 1742 based on a 40-year record. The normal rainfall for three month period preceding freezing is 4.4 inches. Three test areas were selected. A portland coment concrete area consists of an eight-inch slab constructed directly on the subgrade which consists principally of frost susceptible silty, clayey sand (SF-OL). A bituminous concrete test area, consists of five inches of asphaltic concrete everlying eight inches of sand and gravel base on the silty, clayey sand subgrade. A turfed area with subgrade conditions similar to those encountered under the paved areas. A well defined water table at approximately 12 feet below the surface exists in more gravelly materials which is found in the deeper subgrade. No serious heaving of the pavement or pavement failures due to frost action have been observed.

Comprehensive tests were conducted during the first year, 1944-1945, and limited to tests and observations the following year under both rigid and flexible pavements. Test results are presented on Plates 263 to 297, inclusive.

eight miles northwest of the city of Casper, Wyoming. This site was selected to determine the effects of frost in an area having a comparatively low annual precipitation, an extremely low water table, and dry subsurface conditions. The general terrain of the airfield site is substantially flat. The weather conditions in the airfield region are moderate with the normal freezing index of 532 based on a five year average. The normal rainfall for the three month period preceding freezing is 4.4 inches. Two test areas were selected. One test area on the apron consists of portland coment concrete

pavement, seven inches thick, placed directly on a compacted frost susceptible sand and sandy clay subgrade. The other test area consists of a flexible pavement taxiway section constructed of five inches of asphaltic concrete on 7 to 13 inches of sand and gravel base. The ground water table is in excess of 90 feet below the surface of the airfield. Pavement heave or distress due to frost action has not been serious at this airfield.

Comprehensive frost investigations were conducted during one year only, 1944-1945. Results of tests and observations are presented on Plates 298 to 302, inclusive.

Sioux Falls Airfield. Sioux Falls Airfield is located northwest of the city of Sioux Falls, South Dakota. The airfield is located in a flat flood plain just above the Big Sioux River. Levee construction along the north and northwest side of the airfield protect the airfield from flood waters of the Big Sioux River. Severe winter weather conditions are indicated by a normal freezing index of approximately 1100 based on a 46-year record. The normal rainfall for three month period preceding freezing is 5.5 inches. Two test areas were selected. One is on a taxiway pavement of two inches of bituminous concrete with approximately 9.5 inches of gravel, sand, and clay base overlying 12 inches of select silty clay sub-base (CL). The subgrade soils consist of a mixture of clay, silt and sand (CL-CH). The second test area is located on the portland cement concrete apron. The concrete pavement was placed directly upon the frost susceptible compacted subgrade. The normal elevation of ground water is approximately nine feet below the surface. During flood stage the level of the Big Sioux River is above the surface elevation of the airfield. However, no appreciable back drainage through subterranean water courses has been recorded. No severe pavement distress due to frost action has been observed. A previous investigation of frost

conditions existing under a taxiway pavement at this airfield was made in March 1944. Excavations made at that time indicated the presence of appreciable ice lenses extending from the top of the subgrade to a depth of approximately three feet.

Studies were conducted at the airfield for a period of three years in the same test areas. Comprehensive tests were made during 1944-1945 and 1945-1946 and limited investigations the following year, 1946-1947.

Results of these tests are presented on Plates 303 to 323, inclusive.

n. Fairmont Airfield. Fairmont Airfield is located on a level plain approximately two miles south of the town of Fairmont, Nebraska. Moderate winter weather conditions are indicated by a normal freezing index of 581 based on a 46-year record. The normal rainfall for three months preceding freezing is 6.5 inches. The test area consisted of an eight-inch portland cement concrete pavement constructed directly on a silty clay (CL-CH) subgrade. The ground water is located approximately 90 feet below the pavement surface.

Only limited investigations were conducted at this site during 1944-1945 in a rigid pavement area. Results are presented on Plates 324 to 326, inclusive.

p. Great Bend Airfield. Great Bend Airfield, approximately three miles west of the city of Great Bend, is located in the wide, flat valley of the Arkansas River. Winter temperature conditions in the airfield region are extremely variable, with extremes of very mild to occasionally severe winters. The 46-year normal freezing index is only 28. The normal rainfall for the three month period preceding freezing is 5.3 inches. One test area consists of a seven-inch portland coment concrete pavement constructed on a six-inch sandy gravel base. The subgrade consists principally

of a silty clay (CL) and sandy silt (CL-SF). The water table ranged from 12 to 15 feet below the surface during the period of this investigation.

No pavement failure due to frost action has been observed.

Limited frost investigations were conducted for two consecutive years in a rigid pavement test area. Results are presented on Plates 327 to 331, inclusive.

q. Garden City Airfield. Garden City Airfield, approximately nine miles southeast of Garden City, Kansas, is located on flat to slightly rolling prairie land with the Arkansas River approximately one mile south and west of the airfield. The H4-year normal freezing index is 56. The normal rainfall for the three month period preceding freezing is 4.1 inches. One test area was selected and is located on a runway pavement consisting of bituminous concrete having a thickness of 1.5 inches constructed on a sand, gravel, and clay (SC) base course with a thickness of approximately 10.5 inches, overlying a silty clay (CL) subgrade. Ground water elevation is more than 90 feet below the surface. Pavement distress due to frost action has not been previously recorded at this arifield. In a previous investigation made on the airfield pavement in January 1944, the presence of ice lenses and frost formations were observed in the subgrade. The freezing index for the 1943-1944 season was approximately 244.

year only, 1944-1945. Results are presented on Plates 332 to 335, inclusive.

r. Pratt Airfield. Pratt Airfield is located approximately three miles north of the city of Pratt, Kansas, on a gently rolling prairie land interspersed with low knolls and occasional shallow ponds. Mild winter weather conditions are indicated by the 46-year normal freezing index of 28.

The normal rainfall for the three month period preceding freezing is 6.3 inches. One test area was selected and is located on a taxiway pavement consisting of a seven-inch thickness of portland cement concrete, overlying a silty sand cushion (SF-CL) of average thickness of three inches, but ranging from zero to 12 inches. The subgrade consists of a silty clay (CH-CL). The ground water elevation in the airfield region is approximately 90 feet below the surface. The sand cushion tends to pond water after periods of precipitation. The source of the water in the sand cushion is believed to be surface water infiltrating through cracks and joints in the pavement, and also entering at the juncture of the pavement and turf shoulder.

Limited frost investigations were conducted in a rigid pavement test area during one year only, 1944-1945. Test results are presented on Plates 336 to 338, inclusive.

3-03. <u>Drainage Data</u>. The surface and subsurface drainage facilities at the several test areas are summarized in the following tabulation:

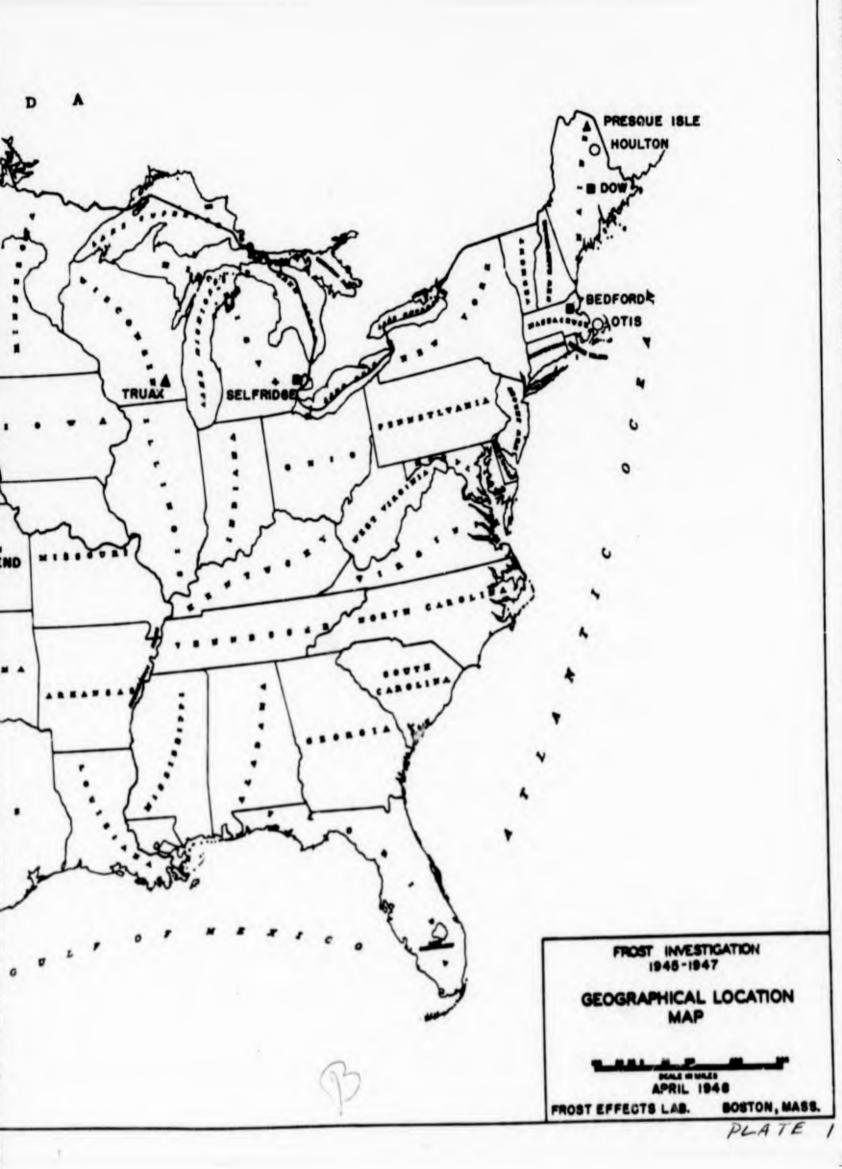
AIRFIELD	TEST AREA	SURFACE DRAINAGE	SUBSURFACE DRAINAGE	
Presque Isle	A	Surface runoff from pavement collected by catch basins in valley in apron area.	Base course continued through shoulder to edge of fill on one edge.	
	В	Surface runoff from pavement and shoulder collected by shallow turf or rock gutters which drain to a catch basin at end of taxiway.	6-inch open joint pipe, 4-foot depth backfilled with sand and gravel at outside edge of surface treated gravel shoulders.	
Houlton	A	Surface runoff from apron collected in ditch at pave-ment edges	Open joint pipe, 5-foot depth, to intercept sidehill seepage at east edge. Backfilled with sand and gravel.	
	В	Surface runoff from pave- ment collected by combina- tion drains and catch basins at runway edges and ditches along outside edge of land- ing strip.	Open joint pipe, 5-foot depth, at edges of bit. conc. runway. Backfilled with sand and gravel.	
Dow	A	Surface runoff from pavement collected by catch basins located 75 feet from and spaced 225 feet longitudinally.	oper. joint pipe, 4-foot depth backfilled with bank-run sand	
	B and C	Surface runoff from pave- ment collected by catch basins located at edge of pavement spaced 225 feet and catch basins at edge of bit. treated shoulders and at 250 feet from in turfed area.	Open joint pipe at bit. conc. pvt. edges and skip pipe at 175 feet from runway at bit. surf. treated shoulder edges.	

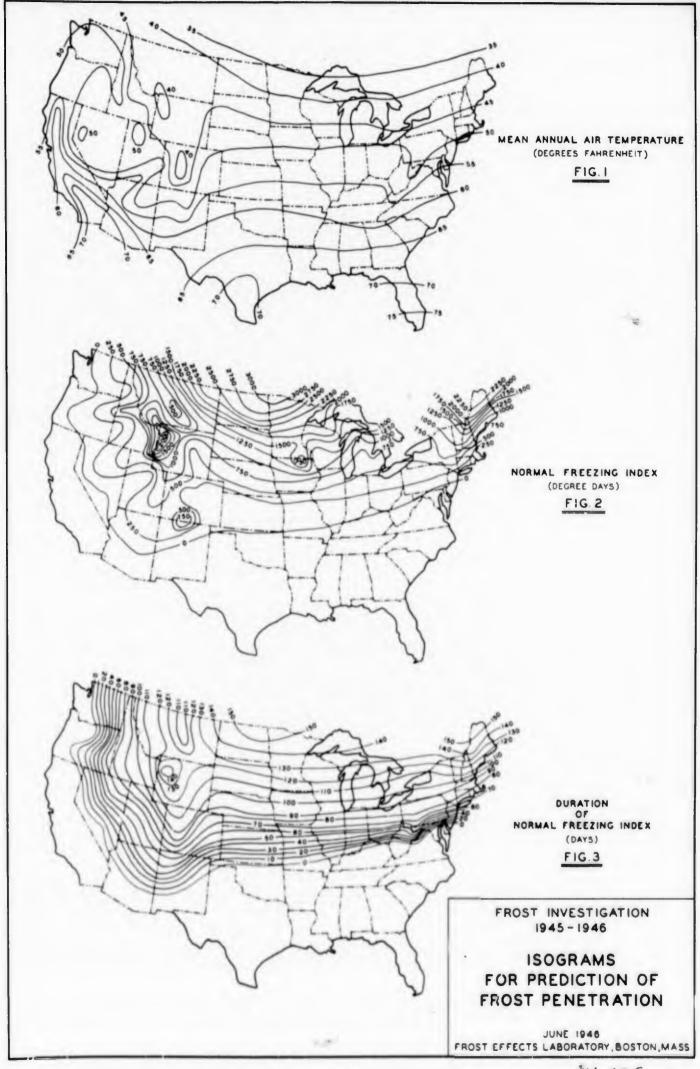
	AIRFIELD	TEST AREA	SURFACE DRAINAGE	SUBSURFACE DRAINAGE
1	Bedford	A	Surface runoff collected by catchbasins and carried away by closed joint drains to large open ditches and natural drainage outlets.	None
		В	Surface runoff collected by catchbasins and closed joint pipe generally located 250 feet from centerline of runway, also by surface inlets located 75 feet on each side of centerline of the runways and spaced 250 feet apart.	Open joint pipe located 75 feet on each side of the centerline in trenches backfilled with bank run sand and gravel. Top of trenches paved.
	Otis	A	Surface runoff collected by longitudinal turf swales located 150 feet from £ runway with catch basins to closed joint pipe.	é-inch non-reinforced open joint pipe laid in 2-foot wide trenches at edge of pavement, backfilled with well-graded sand and gravel. Pipe inverts are about 4 feet below pavement edge.
	Truax	Å	Surface runoff from of of pavement to edge of shoulder collected by catch basins in shallow gutter at shoulder edge.	None
		В	Surface runoff from f of pavement to edge of shoulder collected by catch basins in shallow gutter at shoulder edge.	Perforated tile pipe in trenches filled with coarse sand at edges of pavement. Top 2 inches is clay top soil.
		С	Surface runoff from pavement and adjoining turfed area collected by catch basins in turfed area at low points.	Trench filled with sand and gravel and containing a V. C. pipe with open joints along south edge. None at north edge.
	Selfridge	A	Surface runoff from pavement collected by shallow gutters at edges and into catch-basins, thence to one of three pumping stations.	6-inch perforated bell and spiget tile pipe around periphery of apron at 5-foot depth in trench backfilled with clean sand and gravel. Top 12 inches compected top soil.

AIRFIELD	TEST AREA	SURFACE DRA INA GE	SUBSURFACE DRAINAGE
Pierre	A and B	Surface runoff collected by shallow swale at edge of shoulders.	None
Fargo	A	Surface runoff from pavement collected by combination drains at pavement edges.	Combination drains backfilled with coarse aggregate located in shoulder with open joint pipe in trench.
Bismarck	A	Surface runoff collected by shallow swale at edge of shoulders.	None
Watertown	A and B	Surface runoff drains to open shallow swale at edge of pavement.	None
Casper	A	Surface runoff collected by catch basins local in shallow swale in prement area.	None
	В	Surface runoff collected by shallow swale at edge of pavement.	None
Sioux Falls	A and B	Orainage of airfield principally by surface runoff. Temporary pending relieved by seepage into subsurface permeable strata.	None
Cirmont	Λ	Drainage provided by surface drainage and by comprehensive storm sewer system.	None
Great Bend	A	Drainage secured by drainage ditches and by drainage into sump ponds.	None

AIRFIELD	TEST	SURFACE DRAINAGE	SUBSURFACE DRAINAGE
Garden City	Α.	Drainage secured by surface drainage and storm sewer system. Interceptor drainage ditch protects the paved areas from water draining from higher area.	None
Pratt	A	Drainage secured by surface drainage and a storm sewer system.	None







NEW ENGLAND DIVISION CORPS OF ENGINEERS, U. S. ARMY BOSTON, MASSACHUSETTS

DATA REPORT OF FROST INVESTIGATIONS Fiscal Years 1943 - 1949

APPENDIX A

REPORT ON STUDIES OF BASE COURSE TREATMENT TO PREVENT FROST ACTION

1945 - 1947

FROST EFFECTS LABORATORY

June 19L9

Report of STUDIES OF BASE COURSE TREATMENT TO PREVENT FROST ACTION 1945 - 1947

SYNOPSIS

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REPORT ON STUDIES OF BASE COURSE TREATMENT TO PREVENT FROST ACTION

SYNOPSIS

This report presents studies and results of tests, using admixtures, on frost susceptible base course and subgrade soils. These studies and tests were conducted by the Frost Effects Laboratory under controlled temperature conditions for the purpose of developing methods of preventing or reducing frost action in frost susceptible soils. Frost action is defined as an accumulation of water in the form of ice lenses in the soil under natural freezing conditions.

The design of airfield pavements at locations where frost penetrates into the base is founded upon the assumption that the base is not weakened or adversely affected by frost action. However, in some geographical areas, such non-frost susceptible bases are not economically available and methods to make frost susceptible bases non-frost susceptible may prove to be economically feasible and allow for speedier construction. The success of these methods depends first upon the permanency of the treatment and second upon the economy of the treatment in contrast to the importation of non-frost susceptible materials.

The laboratory studies were conducted over a period of two years. During the fiscal year 1945-1946 tests were performed to determine the suitability of various economically available admixtures in preventing frost action. Tests were also made to determine whether leaching of salt admixtures could be retarded or prevented by the use of bituminous materials. The works of other investigators was reviewed and are

summarized in this report. Admixture studies were continued during the fiscal year 1946-1947 and, in addition, investigations were made (a) to verify the hypothesis that the amount of salt required to prevent frost action is a function of the void ratio and (b) to determine the influence of rock content in a frost susceptible soil on type and quantity of admixture required to prevent frost action.

The admixtures selected for use in these studies were flake calcium chloride, Bunker "C" oil, Tarmac T-2 (RT-2) and "Darex A.E.A." The materials selected for testing consisted of a glacial till, a silt, a frost susceptible sand and gravel and mixtures of the latter two materials.

ment with those obtained by Winn and Rutledge (summarized in this report). Results of tests indicate that (1) Bunker "C" oil and RT-2 may be used to prevent frost action singly or in combination with calcium chloride, (2) "Darex A.E.A." is not effective in preventing frost action in silt when used in quantities up to two per cent of the dry weight of the soil, (3) addition of bituminous material with salt retards leaching only slightly, (4) that the air temperature and void ratio may be used to determine the approximate quantity of salt required to prevent frost action, (5) no relationship between the rock content of the soil and the amount of admixture required to prevent frost action is indicated for the three soil mixes tested.

I INTRODUCTION

- 1-01. Authorization. The 1945-1946 frost investigation program was authorized by the Chief of Engineers by letter to the Division Engineer, New England Division, dated 4 August 1945, subject "Frost Investigation, During Fiscal Year 1945-1946" and subsequent indersements, File SPEER. The program for the fiscal year 1946-1947 was authorized by the Chief of Engineers by two letters to the Division Engineer, New England Division, dated 25 July 1946 and 12 August 1946, subject "Funds for Investigational Program for Fiscal Year 1947."

 The investigations reported herein constitute a part of the authorized program.
- 1-02. Furpose. The purpose of these investigations has been to study methods and perform laboratory tests to develop treatments to prevent frost action in soils susceptible to frost action.
- 1-03. Scope. This report presents (1) a summary in the form of excerpts from the conclusions of reports of investigations performed by others on the effect of admixtures on frost action, (2) a sutdy of previous investigations to determine the relationship between void ratio and the amount of salt required to prevent frost action, (3) the results of laboratory tests performed to determine the suitability of various admixtures and combinations of admixtures to prevent frost action in frost susceptible materials, (4) the results of laboratory tests to determine whether leaching of salts could be retarded or prevented by the addition of bituminous materials, (5) the results of laboratory tests to determine the effect of rock content of soils on

the amount of admixture required to make them non-frost susceptible.

Representative data are presented herein. A complete record of test data is on file at the Soils Laboratory of the New England Division.

No field tests were performed for these investigations.

1-Oh. Acknowledgments. Frost action tests were conducted in the Soil Mechanics Laboratory, Harvard Graduate School of Engineering. Dr. A. Casagrande of Harvard University and Dr. P. C. Rutledge of Northwestern University were employed as consultants on these investigations. The investigations reported herein were conducted along the lines established by previous investigations at Purdue University.

1-05. Definitions. The description of the tests and analyses of results involve a specialized use of certain terms and words. The words and terms used in this report are defined in the "Glossary" at the end of this report.

II STUDIES OF BASE COURSE TREATMENT TO PREVENT FROST ACTION, FISCAL YEAR 1945 - 1946

2-01. Purpose. The purpose of this investigation has been to study methods and perform laboratory tests to develop treatments to prevent frost action in base materials susceptible to frost action.

2-02. Scope. This report presents (a) a summary of previous investigations performed by others, to study the effect of admixtures on frost action, in the form of excerpts from the stated conclusions of these investigations, (b) the results of laboratory tests performed to determine the suitability of various admixtures and combinations of admixtures to prevent frost action in materials susceptible to frost action, and (c) the results of laboratory tests to determine whether leaching of salts could be retarded or prevented by the addition of bituminous materials. Representative data are presented herein. A complete record of test data is on file at the Frost Effects Laboratory. No field tests were performed during this investigation.

- 2-03. Review of Previous Investigations. The following three studies, of the treatment of base courses to prevent frost action all conducted by personnel of Purdue University, were reviewed:
- a. "Frost Action in Highway Bases and Subgrades" by H. F. Winn and P. C. Rutledge, May 1940.
- b. "Use of Calcium Chloride in Subgrade Soils for Frost Prevention" by F. L. Slate, December 1942.
- c. "The Migration and Effect on Frost Heave of Calcium Chloride and Sodium Chloride in Soil," by Charles Slosser, July 1943.

The studies reported in paragraph 2-03a above were made "to determine the resistance to frost action of various types of treated soils and soil mixtures now in common use as road bases and subgrades." Three basic soils were selected for study, a sandy clay, a pit run gravel, and a fairly uniform, washed, concrete sand. The results of classification tests on these soils are summarized on Plate A-1. These three soils were combined in the following percentages to form seven different soil mixtures: 10, 20, 40 and 60 per cent sandy clay with 90, 80, 60 and 40 per cent sand respectively, and 15, 16.5 and 20 per cent sandy clay with 85, 83.5 and 80 per cent pit run gravel, respectively.

These soils and soil mixtures were tested in a remolded compacted state at varying per cent saturation, with and without the following admixtures: calcium oxide, sodium chloride, calcium chloride, portland cement, tar, cutback asphalt, road oil, emulsified asphalt and vinsol. The method of testing was briefly as follows:

- a. The remolded moist soil or soil mixture with or without admixture was compacted in a cylindrical form three inches inside diameter and seven inches high.
- b. Water under a pressure of 30 pounds per square inch was then forced to flow through a selected number of the specimens for about 24 hours.
- c. The specimens were then placed in a freezing cabinet and the air temperature at the top of the sample was progressively reduced over approximately 21 days from about 30°F. to minus 10°F. or minus 15°F. During this period, the air temperature immediately below the bottom of the specimen was maintained at about 40°F. Some specimens were

provided with a source of water at the base of the specimen.

daily measurements of the elongation or heave of the specimens were made. At end of test the specimens were frozen either to or nearly to the bottom. The specimens were examined for ice lenses and tested for water content variation with depth.

The conclusions arrived at as a result of this investigation are quoted as follows:

"1. Estimates as to the extent to which frost action may be expected to occur in natural soil, treated soil, or stabilized soil can be made only when the limiting conditions of initial and attainable moisture content are known."

"2. In general, the natural fine-grained sandy clay started to heave sooner, heaved at a greater rate, reached a greater total heave, reached capillary saturation more readily, and had less resistance to moisture content fluctuation than did treated and stabilized sandy clay exposed to the same condition."

"3. The available dataindicate that the frost line penetrates a graded-soil mixture at a greater rate than it does a natural fine-grained sandy clay. Rapid freezing results in less ice segregation and less total heave for the same depth of frost penetration."

"4. Percentage-of-heave data from individual tests should not be used as criteria for rigid comparisons of the frost action resistance of natural soils, treated soils, or stabilized soils, but may be used as a basis for general classifications of the materials into heaving and non-heaving groups."

- "5. Once capillary saturation is reached and ice segregation begins in a treated sandy clay, the rate of heaving is only slightly less than for the same soil untreated."
- M6. The available data indicate that there is a critical density for sandy clay at which frost action occurs most readily, when material is saturated. Below the critical density, frost action is directly proportional to density above the critical density, frost action is inversely proportional to density. Increasing the density above the critical density increases the period of inactivity before heaving starts and decreases the rate of heaving and total heave in a manner similar to the addition of admixtures."
- "7. Groups of specimens of natural or treated sandy clay included in this investigation, which had no variables except density and moisture content at the time of compaction, approached the same ultimate dry density during air drying."
- "8. Any of the types of soils, treated soils, or stabilized soils included in this investigation can be saturated by water under a pressure of 30 pounds per square inch applied for 24 hours or less."
- "9. The available data on field soil temperatures indicate that periodic fluctuations of short duration in air temperature do not cause corresponding fluctuations in the temperature of the subgrade soil; soil temperatures are a function of cumulative air temperatures."

"REGARDING ADMIXTURES."

"10. All the admixtures tested are much more effective in reducing frost action when used with well-graded soil mixtures than when used with natural sandy clay."

"11. Calcium oxide (2, 6, 10 per cent in sandy clay; 4 per cent in graded soil mixture)* does not increase the mixture's resistance to frost action or moisture content fluctuation sufficiently to warrant its use for these purposes. The mixtures take on water readily, and, provided water is available for capillary rise, the degree of saturation at the beginning of the freezing period is of little consequence."

"12. Sodium chloride (natural sandy clay plus 1, 2, 3, 6
per cent; graded soil mixture plus $\frac{1}{2}$, 1, 2, 3 and 4 per cent) and
calcium chloride (natural sandy clay plus 2, 4 per cent) provide good
resistance to frost action primarily because of the lowering effect
of the admixture on the freezing point. The data indicate that as
long as the soil retains the chemical in its full concentration,
2 per cent or less chemical prevents freezing at -10° to -15°F. and
thereby prevents frost damage."

"13. The resistance to frost action of a soil cement mixture (natural sandy clay plus 4, 6, 8, 10, 12 per cent; graded soil mixture plus 4, 6, 8, 10 per cent) is inversely proportional to the degree of saturation of the mixture at the beginning of the freezing period."

"14. In general, the resistance to frost action of bituminous mixtures is inversely proportional to the degree of saturation at the beginning of the freezing period."

fied asphalt, and vinsol**add stability to a sandy clay by inhibiting

Percentages investigated. All percentages are given in terms of dry
weight.

^{**} A by-product of turpontine distillation.

capillary motion of the water to various degrees, the amount being closely related to the percentage of admixture and moisture content of the mixture at the time it is exposed to the water."

"16. Vinsol is effective as a waterproofing agent and frost action preventive when the moisture content of the sandy clay-vinsol mixture is between 4 and 10 per cent."

"17. On the basis of the data presented in this paper, the following group classifications can be made:

Group No. 1, damaged by frost action at all percentages of initial moisture content. Sandy clay (natural); graded mixtures of clay plus gravel and clay plus sand; sandy clay plus 2, 6, 10 per cent CaO; graded scil mixture plus 4, 6 per cent CaO; sandy clay plus 1 per cent NaCl; sandy clay plus 1 per cent CaCl₂; sandy clay plus 4 per cent portland cement; sandy clay plus 2, 4, 6 per cent TC; sandy clay plus 2, 4 per cent AES-1; sandy clay plus 2, 4 per cent MC-1.

Group No. 2, damaged only when initial moisture content was approximately 100 per cent saturation. Sandy clay plus 6, 8, 10, 12 per cent Portland cement; sandy clay plus 4, 6, 8 per cent TM-2; sandy clay plus 4, 6, 8 per cent AES-1; sandy clay plus 4 per cent SC-3; graded soil mixture plus 2 per cent AES-1; graded soil mixture plus 2, 4, 6 per cent RC-3; sandy clay and graded soil mixtures plus 1, 2, 3, 5 per cent vinsol (also when moisture content is below 4 per cent).

Group No. 3, no frost damage at all degrees of initial moisture content. Sandy clay plus 3, 4, 6 per cent NaCl; graded soil mixture plus $\frac{1}{2}$, 1, 2, 4, 6 per cent NaCl; sandy clay plus 3, 4, 6

per cent CaCl₂; graded soil mixture plus 1, 2, 4 per cent CaCl₂; graded soil mixture plus 4, 6, 8 per cent portland cement; graded soil mixture plus 4, 6 per cent TM-2; graded soil mixture plus 4, 6 per cent AES-1; graded soil mixture plus 4.4 per cent Bitumuls Stabilizer; sandy clay plus 6, 8 per cent SC-3; graded soil mixture plus 2, 4, 6 per cent SC-3.**

The studies reported in paragraph 2-03b were performed to determine the percentage of calcium chloride necessary to prevent frost action. One soil, a silt, was selected for study. The results of the classification tests are shown on Plate A-1.

The test procedure was similar to that reported by Winn and Rutledge as briefly described on Page A-6. The principal conclusions from his investigations are quoted as follows from his report, page 440, "(1) The presence of a small percentage of calcium chloride in silt will usually protect that soil from damaging frost heave. (2) Small quantities, as low as one-half of one per cent, of calcium chloride in silt will reduce the frost heave appreciably. (3) A soil that has heaved because of frost contains a moisture content greater than normal. This water makes up the ice lenses causing the heave, and it is drawn up from the ground water. (4) The water rising to form ice lenses carries calcium chloride upward with it. (5) As a general average, it can be said that protection from frost heave in silt is afforded by 2 per cent calcium chloride, in clay by 1 per cent calcium chloride, and in graded mixes by $\frac{1}{2}$ per cent calcium chloride." These tests indicate that under the conditions tested the silt required at least 4 per cent calcium chloride to prevent frost action at

9°F., with a gradually decreasing temperature.

The studies reported by Charles Slesser were made to "trace the movement of calcium chloride and sodium chloride in various soils and to evaluate the important variables governing this movement" and "to determine the practicability of treating subgrades with those chemicals in order to reduce or eliminate frost heave."

The soil tested consisted of a silt, called LaPorte silt, for which classification data are summarized on Plate A-1.

The principal conclusions from his investigations are quoted as follows from Fage 14 of his report:

migrated differently under similar conditions of exposure. Under the influence of soil capillarity and natural evaporation, sodium chloride tended to form a white crust on the surface of the unpaved road, and, hence, was more susceptible to lateral surface-washing during rain periods than was calcium chloride. On the other hand, calcium chloride did not tend to accumulate on the unpaved road surface to the same extent as sodium chloride, under the influence of soil capillarity and natural evaporation, because of its greater moisture-attracting power and its higher solubility. With exposed fine-grained soils, lateral movement proceeded primarily by surface waching from the top of the road proper to the side ditches, rather than by lateral movement below the surface."

"Important variables affecting the movement of water-soluble chemicals in soil and hence their permanence included: (1) evaporation, (2) soil texture, (3) percolating water, (4) soil cover, and (5) temperature, when high enough or low enough to effect a change of phase of the water. As regards base-exchange phenomena, the calcium and sodium A-12

cations were more persistent in fine-grained soil than the chloride anion."

"In general, increased effectiveness in reducing heaving in soil resulted from increases in the amount of calcium chloride or sodium chloride added -- up to a certain percentage of chemical, above which no heaving took place. In a coarse-textured soil, heaving was greatly reduced by an admixture of only 0.33 per cent of either chemical. One or two per cent of either chemical was effective in reducing heaving in a silt which had, in the untreated state, heaved badly both in the field and in the laboratory."

Station in connection with the water repellent investigation performed slow-freeze tests* on five soils with and without several water repellents. Information regarding tests performed was furnished this office be letter. The method of testing was similar to that described in paragraph 2-03, page A-6. Grain size distribution curves and the Atterberg limits are shown on Flate A-2. A summary of the data for these tests is included as Flate A-3. Five photographs with the degree hour curves and rate of heave curves as prepared by U. S. Waterways Experiment Station, Vicksburg, Mississippi are included as Plates A-4 to A-8 inclusive. The principal conclusions from these tests are summarized as follows:
All soils in the untreated state when tested were subject to severe frost action. Two per cent Stabinol*effectively reduced the heave in the clayey silt (sample 6), sandy silt (sample 7), and the gravelly clay sand (sample 8) but was ineffective in the silty clay (sample 3)

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^{*} The term slow-freeze tests used in the report by the Waterways Experiment Station is synonymous with the term frost action as used in this report.

^{**} Stabinol as used in these experiments consisted of 3 parts of portland cement and 1 part of a complex salt consisting of unneutralized abjetic acid, sodium resinate and calcium resinate.

and the clayey silt (sample 5). One per cent 321* will not materially reduce the heave. The heave was reduced by the addition of one per cent of 321 plus 0.4 per cent of FeSO1 in the silty clay (sample 3) and the gravelly clay sand (sample 8) but not in the other three soils.

Investigations by others indicate that the addition of calcium chloride or sodium chloride to water will result in lowering the freezing temperature of water to a minimum after which the freezing point will be raised by the continued addition of salt. Commercial producers of these salts report that the percentage of salts which will produce the minimum freezing temperature of water are as follows:

SALT	PER CENT	SALT BY	WEIGHT	OF WATER	FREEZING FOINT
Calcium Chloride (I	Pure)		48		-59.8°F.
(7	77-80% Flake	CaCl ₂)	61		-59.8°F.
Sodium Chloride (Pu	ıre)		30.4		-6°F

The addition of salt to soil for the purpose of making it non-frost susceptible is based upon the fact that the salt lowers the freezing point of the water thus lowering the temperature at which frost action will occur. The maximum benefit from the salt in reducing frost action appears to occur when the water in the voids contains the percentage of salt tabulated in the preceding paragraph. Hence, based upon these percentages for calcium chloride and sodium chloride, the percentage of pure calcium chloride by weight of dry soil which will give lowest freezing temperature varies from about 5 to 18 per cent for

^{* 21} is a finely powdered, white resinous substance formed by reacting sidium hydroxide with rosin in such proportions that one-fourth of the abietic acid in the rosin is neutralized, thus resulting in a complex salt of three parts abictic acid and one part sodium abietate.

soils with void ratios from 0.27 to 1.0 respectively and the percentage of sodium chloride by weight of dry soil which will give lowest freezing temperature varies from about 3 to 11 per cent for soils with void ratios from 0.27 to 1.00 respectively when the soils are 100 per cent saturated.

- 2.04. Description of Laboratory Cold Room and Equip t. The investigations were carried out in the cold room laboratory at Harvard University Graduate School of Engineering. General layout of the cold room and equipment is shown in Plate A-9.
- a. Cold Room. The cold room is a walk-in refrigerator with inside dimensions six feet nine and one-half inches long by six feet nine and one-half inches wide. It is insulated on all six sides with four inches of cork. A pressure controlled unit cold blower and externally located freen compressor coels the room to 40°F. to an accuracy of 1.0°F.
- cabinet located as shown on Plate A-9. This cabinet consists of an air space at the top cooled by longitudinal coils hung from the top of the cabinet using a second compressor with sulphur dioxide refrigerant. Beneath this air space are four drawers arranged side by side. The temperature within the top of the cabinet may be fixed at any desired temperature with an accuracy of plus or minus 0.5°F. The temperature may be made equal to or less than that of the cold room by adjustment of a bimetallic DeKhotinsky type temperature control located in the air space above the drawers. With the cold room at approximately 40°F., the freezing cabinet can be lowered to a temperature of

minus 5°F. A small fan in the air space at the top aids in maintaining a uniform temperature throughout the air space. The drawers are effectively sealed in place by slightly inflating an inner tube installed around the lower side of each drawer.

2-05. Frost Action Tests.

a. Materials Tested. Three types of soil with selected admixtures were tested for frost action. The soils consisted of (1) that portion of a glacist till passing thru 1-inch sieve, designated East Boston Till, (2) a silt designated New Hampshire silt, and (3) a send and gravel designated frost heaving gravel with 100 per cent passing the g-inch sieve which was prepared in the laboratory so that it would have frost heaving characteristics. The East Boston till was a grey, well-graded boulder clay (GC) composed mainly of sub-angular particles. It was obtained from Breed's Hill, Winthrop, Massachusetts, a glacial drumlin deposit. The New Hampshire silt (ML) was a brown uniform silt with a small percentage of sand sizes obtained from a varved deposit located south of Manchester, N. H. The frost heaving gravel (GF) was a combination of a washed pea gravel, a bank run gravelly send and the New Hampshire silt. The pea gravel was brown with subangular particles, 100 per cent passing the 2-inch sieve and 98 per cent retained on the No. 8 sieve. The bank run gravelly sund was a clean brown uniform gravelly sand with subangular particles. The frost heaving gravel was prepared using 25 per cent gravel, 45 per cent gravelly sand and 30 per cent silt. The grain size distribution curves with the specific gravity, Atterberg limits and classification of these materials are shown on Plate A-10.

b. Admixtures. The selection of admixtures for testing was made to add information on new and combinations of admixtures to the existing data. Flake calcium chloride (77-80 per cent pure) was selected as a salt. Studies by Slesser indicate that there is less lateral migration under a pavement of calcium chloride than sodium chloride. Calcium chloride in solution with water as the solvent gives a much lower freezing point than does sodium chloride. However, based upon the Purdue freezing tests either calcium chloride or sodium chloride appear to be equally effective in preventing frost action. Bunker "C" oil which conformed to specifications in Bureau of Standards Bulletin No. CS 12-40 for No. 6 fuel oil was selected as one of the admixtures. This material was selected because it is one of the least expensive bituminous materials and had not been tested previously. Tarmac T-2 (Federal Specification RT-143 Grade RT-2 as amended 30 August 1914) was chosen for a comparison of the results of the previous tests with the tests using the Bunker "C" oil. Following is an analysis of the RT-2:

Engler Specific Viscosity 40°C	8.8
Specific Gravity at 25°C/25°C	1.119
Water, % by volume	1.7
Total Bitumen, % by weight	94.3
Distillation, % by weight	
To 170°C 200 235 270 300	1.4 1.9 7.5 22.4 32.8

Softening point of Distillation Residue	
(R & B)	37.8°C
Sulfonation Index (total distillate to	
300°C)	4.4
Sulfonation Index (total distillate 300	

0.43

The following admixtures and combinations of admixtures on basis of percentage of the dry weight of the soil were used:

to 355°C)

PERCENT BUNKER "C" OIL	PERCENT RT-2	PERCENT CALCIUM CHLORIDE
0, 2, 4 and 6	-	0.5
	•	0.5
	•	1
0, 2, 4 and 6		2
- 0,	2. 4 and 6	
•		0.5
		1
	•	-
•		3, 6, 8 and 10
Dense) -		0, 1, 2 and 3
•	•	2
Loose) -		2, 3.5, 4.5 and 5.5
	BUNKER "C" OIL 0, 2, 4 and 6 0, 1, 2 and 4 0, 2, 4 and 6 0, 2, 4 and 6 0, 0, 0, 0, 1, 2, 4 and 6	BUNKER "C" OIL O, 2, 4 and 6 O, 1, 2 and 4 O, 2, 4 and 6 O, 2, 4 and 6 O, 1, 2 and 4 O, 1, 2 and 4 O, 2, 4 and 6 In 2, 3 and 4 Loose) RT-2 RT-2

c. <u>Preparation of Samples</u>. Each of the three soils was air dried, thoroughly mixed and lumps broken down. All sizes retained on a No. 4 sieve were removed from the East Boston till.

Two types of test specimens were prepared; those without admixtures and those with admixtures. Specimens without admixtures were prepared by compacting soil at a predetermined water content into a split container 3.3 inches in diameter and 6.5 inches high, (see Plate A-11) to a selected unit dry weight. There salt only was used as an admixture, it was first dissolved in water and then the solution added to the soil. Where a bituminous material only was used as an

admixture, the required quantity of water was first added and mixed then the bituminous materialwas added and mixed. Where both salt and bituminous material were added, the salt was dissolved in water and thoroughly mixed with the soil followed by the addition of the bituminous material.

Most specimens were compacted to 95 per cent Modified A.A.S.H.O. density at the optimum water content for that density. Where admixtures were used the Modified A.A.S.H.O. density was determined for each different admixture percentage and combinations of admixtures. Some specimens of New Hampshire silt were compacted to a relatively low unit dry weight to investigate the effect of compaction.

The following table summarizes the average molding data for the specimens tested:

Material	% Dry Soil mix	Content Weight and Ad-	Lbs.	Dry Weight of Soil per cubic ft.	Satu Per	ee of ration cent	Ro	oid atio
Maccitat	Avg.	Range	Avg.	Range	AVg.	Range	Avg.	Range
East Boston Till 28 semples	8.1	5.6-9.7	123	117-127	59	36-77	0.40	0.35-0.47
New Hampshire Sil- 4 samples (molded at 95+% mod. AASHO density		13.0-14.9	102	101-103	57	53 - 65	0.66	0.64-0.66
New Hampshire Silt	19.0	18.1-20.3	85	84-87	56	53-60	0.97	0.93-1.00
Frost Heaving Graves & samples	rel 6.	3 5.7-6.7	130	127-132	61	57 - 65		0.27-0.32
Frost Heaving Grav	rel 8.2	2 7.9-8.6	115	114-117	50	48-52	0.46	0.44-0.47

All specimens were numbered consecutively and all numbers skipped represent samples which were not tested.

After the samples were molded, they were photographed, dipped in paraffin so that they were covered with two thin coverings about 1/32 of an inch thick and then placed in a greased cardboard tube. Just prior to placing in the freezing cabinet the paraffin was removed from one end of the sample and the open end was placed on a piece of filter paper on a porous stone. The cardboard tube was sealed to the drawer pan by the use of a rubber membrane and a clean dry sand placed around the samples for insulation. Prior to placing the sample on the porous stone the water level was adjusted to the elevation of the top of the porous stone so that water was available at the bottom of the sample. A schematic diagram showing the samples ready for freezing is shown on Plate A-9. All samples in series B and C were weighed prior to placing on the porous stone.

The capacity of the freezing cubinet was 16 samples and a total of 48 samples were tested in three series. These series have been differentiated by letters A, B, and C before the sample number. Twenty-eight samples were prepared using the East Boston till, eight samples using the New Hampshire silt and 12 samples using the frost heaving gravel.

d. Test Procedure. All samples were allowed to absorb water by capillarity for approximately three days prior to freezing while they were being brought to temperature equilibrium. The samples were frozen by a gradual lowering of the cabinet temperature at the top of the sample while maintaining a constant temperature (40°F) at the bottom of the sample. The following table shows the temperatures applied during the tests. Zero time for each test

is designated as the date when the temperature of the cabinet was reduced to 32°F. The last date of the test is the day on which the samples were taken from the cabinet to the cold room where the temperature was approximately 40°F.

		Series A		Series B		Series C
Days	Temp.	Accumulated Degree hrs.	Temp.	Accumulated Degree hrs.	Temp.	Accumulated Degree hrs.
-2	34	0	34	0	40	0
-1	35	0	34	0	35	0
0	32	0	32	7	32	O
1	32	ST	31	31	31	डी
1 2 3 4 5 6	31	48	31	58	31	48
3	30	96	30	106	55	71+
4	29	168	29	181	30	70
5	29	SHO	29	255	27	190
	28	336	28	354	27	312
7 8	27	456	27	474	26	456
	26	600	26	618	26	600
9	25	768	25	789	25	772
10	514	960	24	982	514	966
11	23	1176	23	1206	23	1185
12	22	1416	22	7بلبل	22	1426
13	20	1704	20	1734	20	1719
14	18	50/10	18	2070	18	2057
15	16	517517	16	2454	14	2486
16	14	2856	14	2886	14	2920
17	12	3336	12	3368	13	3374
18	10	3864	10	3893	11	3879
19	5	4512	5	4530	5	4526
20	5	5160	5 5 5	5193	5 5 5	5164
21	5	5808	5	5854	5	5802
55	5	6456	-	-	5	6393
23	55555	7104	•	•	•	-

The air temperatures in the cold room and cabinet were determined by means of mercury thermometers, thermocouples and recording thermographs. One recording thermograph was placed in the cold room. The second recording thermograph was placed in the freezing cabinet above the drawers. These thermographs were used to determine

^{*} Temperature of cabinet rose to 55°F. between 2nd and 3rd day cancelling degree hours accumulated.

the range of temperatures during the tests. Two thermometers reading to 1/5°F. were placed in the cold room and two in the freezing cabinet. Two copper constantan thermocouples were placed in antifreeze in the cabinet near the top of the drawers, and two were placed in water in the cold room. Two additional thermocouples were placed in the cabinet drawers beneath the porous stone to determine the temperature of the water at the base of the specimen during the test.

The temperature control of the freezing cabinet was erratic during series C and adjustments in the applied temperatures were made in an attempt to make the degree hour curve agree with that of series A and B without extending the period of testing. The degree hour curves for each series are shown on Plate A-12. Water was available at the bottom of all samples during the freezing period. It was maintained level near the top of the porous stone by means of a discharge pipe adjusted to the level of the stones as shown on Plates A-9 and A-13. Water was admitted to the system at a rate that would allow a very slow runoff from the overflow discharge pipe. This was adjusted to the rate at which the sample took on water and was checked frequently each day. Heave measurements were taken three times a week during the test. At the start of series A a scale was used to measure from the top of the sample to a straight edge laid across the freezing cabinet drawer. This method was replaced by one using a 0.001 inch Ames dial extensometer permanently mounted on a steel bar that would span the drawer. Two shims were provided which were machined to .001 inch and three extensions were provided for

the extensometer. This arrangement permitted the measurement of approximately four inches of heave as shown on Plate 14. Those samples which heaved more than four inches were measured with a scale. At the completion of the tests the actual length of each sample was measured as it was removed from the drawers to check the heave measurements.

At the conclusion of the freezing period the samples were removed from the drawers, weighed, the cardboard tube and paraffin removed and photographs taken. The samples were then broken up for the ice lens observation and increment water content determination.

Water contents were taken at approximately every 1.5 inches of height or closer to obtain a water content profile after frost action or freezing had occurred.

e. Summary of Test Results. The three soils tested were affected by frost action in the untreated state. For the percentage of admixtures tested the initial minimum percentages which prevented frost action are presented in the tabulation below. These minimum percentages were reduced an amount which is difficult to evaluate due to the capillary action which took place prior to freezing.

INITIAL PERCENTAGE OF ADMIXTURE

Admixture	East Boston Till	New Hampshire Silt	Frost Heaving Gravel
Calcium Chloride	1%	3% (loose)	1% (dense) 2% (loose)
Bunker "C" Oil	6%	6%	Not Tested
RT-2	6%	Not Tested	Not Tested
Bunker "C" Oil and Calcium Chloride	1% 0.5%	Not Tested	1% 2%
RT-2 and Calcium Chloride	1% 0.5%	Not Tested	Not Tested

A summary of the data from the frost action tests is shown on Plate A-15. On Plates A-16 to A-20 inclusive, the water content data is plotted with the photographs taken before and after freezing for the samples with no admixture, for those with a maximum of admixtures which heaved, and those with the minimum of admixtures which did not heave. As shown on Plate A-15 the dry weight in pounds per cubic foot before testing was computed for the soil alone from the wet weight, the water content, and the percentage of admixture. The void ratio and degree of saturation shown on this table were computed from the dry weights computed as shown above, the specific gravity of the soil and the water content of the soil before and after testing.

All samples tested in the freezing cabinet absorbed water. The increase in water content is shown on Plate A-15 in the columns "Water Content" and "Water Content of the Bottom Inch."

The samples which were not frozen or only partially frozen and which

did not show heave as a result of frost action might have heaved if the temperature conditions had been such that the additional water had been frozen.

2-06. Leaching Tests.

- was the East Boston till as described in paragraph 2-05a.
- b. Admixtures. The admixtures consisted of flake calcium chloride alone and in combination with Bunker "C" oil. The following admixtures and combinations of admixtures on basis of percentage of dry weight of the soil were used:

Series		Soil		Compaction % Mod. AASHO Density	Percent Bunker "C" Oil	_	rcent Chloride
1	East	Boston	Till	95	0, 0.5, 1, 1.5 2 and 4		2
5	East	Boston	Till	95	0	0.5, 1,	1.5 and 2
5	East	Boston	Till	86	0	0.5, 1,	1.5 and 2
5	East	Boston	Till	76	0	0.5, 1,	1.5 and 2

determine the rate of leaching of calcium chloride. For all tests the soil was prepared as described in paragraph 2-05c and compacted in either a consolidation ring or a lucite cylinder at density indicated in above table. In those samples tested in the consolidometer, water was forced upward through the test specimens, 1.25 inches thick, under a pressure of six pounds per square inch equivalent to a hydraulic gradient of 133. Larger pressures then this resulted in the

washing of fines from the specimen. The tops of the specimens were loaded to a unit pressure slightly greater than six pound per square inch. The thickness of the samples in the lucite cylinder varied. In the samples tested in the lucite cylinder, water was forced down through the test specimens by a vacuum of approximately nine inches of mercury equivalent to a hydraulic gradient of from 87 to 145. The water which passed through the sample was collected.

oil and calcium chloride and in the second series calcium chloride alone. The percentages of calcium chloride and the void ratios were varied in the second series to determine the effect of void ratio on the rate of leaching of the salt. Tests with the lucite cylinder were observed to detect any change in particle arrangement due to the flow of water thru the sample under the head used.

The water passing through the specimens was periodically tested to determine the presence of calcium chloride in solution.

This was accomplished by noting the presence of a white precipitate with the addition of silver nitrate and when no precipitate was formed, it was assumed that all calcium chloride was washed from the specimens. The number of changes of water, computed from the volume of voids in the sample and the total amount of water passing through the sample, required to wash the salt completely out of the sample was determine.

In conjunction with the first series of leaching tests, permeability tests were performed on duplicate samples using a falling head permeameter.

d. Summary of Test Results. A summary of the leaching test data is shown on Plate A-21. Several samples were discarded as the pressures used in washing were high enough to cause piping through the samples. Data from these tests have been omitted from this report as they were not completed.

Test results are generally not consistent. It is believed that the inconsistencies are due to stratification accompanying compaction of remolded soil of the type used for these tests. The stratification is believed to have caused large variations in the average coefficient of permeability, thus affecting directly the time for leaching.

III STUDIES OF BASE COURSE TREATMENT TO PREVENT FROST ACTION, FISCAL YEAR 1946 - 1947

3-01. Purpose. The purpose of the investigation was to continue the study of methods and to perform additional laboratory tests to develop treatments for preventing frost action in base materials susceptible to frost action by investigation of (a) the hypothesis that the amount of salt required to prevent frost action is a function of the void ratio, (b) the influence of rock content of frost susceptible base materials on the type and percentage of admixtures required to render the materials non-frost susceptible, (c) methods of permanently retaining salt within the base materials.

3-02. Scope. This report presents the results of the investigations made since those reported in "Report on Studies of Base Course

Treatment to Prevent Frost Action" - Part 1. It presents (a) a study of previous investigations to determine the relationship between void ratio and the amount of salt required to prevent frost action, (b) the results of laboratory tests to determine the effect of rock content of soils on the amount of admixture required to make them non-frost susceptible, and (c) the results of laboratory tests to determine the effectiveness of "Darex AEA" as an admixture for preventing frost action. Representative data are presented herein. A complete record of test data is on file at the Soils Laboratory of the New England Division. No field tests were performed during this investigation.

3-03. Review of Previous Investigations.

- a. Review of Admixtures Tested. Numerous laboratory tests have been performed using various admixtures in soils to determine their effectiveness in preventing frost action (1), (2), (3), (6), (7)*. Brief mention of these admixtures is made below. For a more detailed account, reference has been made to the reports of the original investigations.
- (1) Calcium Oxide. Three different sails were used with varying amounts of admixture and with a wide range of water content at the start of the test. In each test the frost action was severe. Each specimen gained considerable in water content during the test. Calcium oxide did not increase the sails resistance to frost action.
- (2) Water Repellents. The water repellents, Stabinol, 321, and a combination of 321 and ferrous sulphate were tested in five different soils. None of the admixtures prevented frost action.

 * Figures shown thus (1), (2) refer to bibliography.

In three specimens treated with Stabinol and two specimens treated with 321 plus ferrous sulphate, frost heave was reduced to minor proportions. Additional tests using varying quantities of these admixtures in specimens tested with varying water contents are required before any definite conclusions can be drawn.

- (3) Sodium Chloride and Calcium Chloride. These salts have been tested in a wide variety of soils. They have been found to be effective in preventing frost action. A smaller percentage by weight is required to prevent frost action than with other admixtures tried. They have one serious defect, namely, a marked tendency to migrate. Treatment with sodium or calcium chloride will give temporary protection from frost action.
- performed using portland cement as an admixture. In these tests there appears to be a definite relationship between density or woid ratio and percentage of frost heave. As the density decreased or the woid ratio increased, the percentage of heave increased. More test data are required before any definite conclusions can be made concerning this admixture.
- (5) Vinsol Resin. Tests performed using vinsol resin in sandy clay and sandy clay plus sand revealed one significant relationship. The amount of heaving which occurred, varied with the water content of the specimens at the start of the test. Specimens with a high or low degree of saturation heaved considerably those in the middle range showed a marked decrease in heaving. Vinsol as

an admixture to prevent frost action is of doubtful value due to its apparent dependence on the water content of the soil, a factor obviously difficult if not impossible to control.

- (6) <u>Bituminous Materials</u>. Asphalt emulsions, asphalt cutbacks, tars and oils have been tested in a variety of soils. Each of these materials used as an admixture has been effective in preventing frost action. The amount of admixture required to prevent frost action varies for different soils and the void ratio or density to which the soil is compacted. Bituminous admixtures are the only ones which show any promise of providing effective permanent treatment to prevent frost action.
- b. Analysis of Previous Investigations. A further study of all tests which have been performed using admixtures to prevent frost action was made. As a result of this study, certain significant relationships, not previously developed, are set forth concerning sodium chloride, calcium chloride and bituminous materials.
- sodium chloride in the brine for the verious specimens testedwas computed and the freezing point of the brine taken from tables compiled by the International Salt Co. The freezing temperature of the brine was plotted against the air temperature at which frost action started and a curve developed showing the probable start of frost action in soils treated with sodium chloride. This curve is shown on Plate A-22. The air temperature at which frost action starts was found to be considerably lower than the freezing point of the brine. This can be explained in part by the fact, as

determined by laboratory tests (3), that the sodium chloride migrates towards the top of the sample resulting in a concentration of salt at the top of the specimens greater than the computed average. It is also partly due to the fact that during freezing the soil temperature remains higher than the air temperature.

Laboratory tests have been performed in which the magnitude of the difference between air and soil temperatures during cooling for certain cohesionless soils has been determined (4). Another factor which probably accounts for some of this difference is the phenomenon, determined by laboratory tests (5), which permits soils to be cooled below the freezing temperature of water without freezing.

Using the aforementioned brine tables, calculations have been made to determine the quantity of sodium chloride required per cubic foot of dry soil to lower the freezing point of the brine to various temperatures in soils with 100 per cent saturation of brine for varying void ratios of soil. The results of these calculations are shown graphically on Plate A-23. Using the difference between the air temperature at which frost action started and the freezing point of the brine, as determined from Plate A-22, it is possible to prevent frost action at that predetermined air temperature. Since Plate A-22 is based on a small number of laboratory tests and no field test data are available to check the results of these tests, the method of designing the admixture should be expected to yield only approximate results. It should also be borne in mind that treatment with sodium chloride will give only temporary protection,

sodium chloride having a marked tendency to migrate or leach out of the soil (3). To date, attempts to find some method which will prevent or retard leaching have been unsuccessful.

- (2) Calcium Chloride. The average concentration of calcium chloride in the brine for the various specimens tested was computed and the freezing point of the brine was taken from tables published by the Solvay Technical and Engineering Service. As was done for sodium chloride, the freezing temperature of the brine has been plotted against the air temperature at which frost action started and a curve developed showing the probable start of frost action in soils treated with calcium chloride. This curve is shown on Plate A-24. Plate A-25 has been prepared showing the weight of flake calcium chloride required per cubic foot of dry soil to lower the freezing point of the brine to various temperatures for various void ratios of soil. The soil is assumed to be 100 per cent saturated with brine. These two plates furnish sufficient information to design a mixture of soil and calcium chloride which will not be frost susceptible for the range of temperatures commonly encountered in soils. The above method of design is based on laboratory test data. No field tests have been performed to check the results of these laboratory tests. Treatment with calcium chloride will give only temporary protection from frost action due to the fact that this salt will eventually leach out of the soil.
- (3) Bituminous Materials. There are two factors which possibly could lead to the development of a quantitative method of

design for bituminous admixtures. The first is the percentage of fine soil particles in the soil of such a size (0.02 mm. in diameter) as to make the soil frost susceptible. The susceptibility of a soil to frost action appears to be proportional to the quantity of fine soil particles. The second factor is the waterproofing property of bituminous meterials. If this waterproofing is effected by filling the voids in the soil with bituminous meterial up to a point where capillary water is shut off, then the void ratic with admixture of the soil gives a measure of this waterproofing. For any soil, therefore, there could be a critical void ratio with admixture at which frost action would be eliminated. Such a critical void ratio is well defined in the tests performed on Bunker "C" cil and tar (RT-2) at the Soils Laboratory of the New England Division.

A study of the test data using asphalt cutbacks and asphalt emulsions indicates that a similar critical void ratio with admixture exists. In this latter case the critical void ratio with admixture is not too clearly defined but by observing whether the sample gains any water during the test and considering that frost action has been stopped in those samples showing very little or no gain in water even though a slight heave has been recorded, it is possible to arrive at an approximate void ratio with admixture at which frost action is prevented. In the tests using tars, definite critical void ratios with admixture are indicated. The admixture, TC, exhibits a characteristic not observed in other tests. Frost heave was prevented in some of the samples tested even though considerable gain in water content occurred during the test.

The void ratio with admixture at which no frost action occurred has been plotted against the percentage of soil particles finer by weight than 0.02 mm. in diameter and the results are shown on Plate A-26.Before any definite conclusions can be arrived at, it appears necessary to test each admixture with a number of soils with various percentages of soil particles finer by weight than 0.02 mm. in diameter. It may even be necessary to run tests on soils of varying gradations of soil sizes with a view to determining the void ratio with admixture at which frost action is prevented in each soil in a family of grain size curves. From the tests already performed it is evident that bituminous admixtures if used in sufficient quantity can prevent frost action. All bituminous admixtures are not equally effective. Those admixtures which prevent frost action at the highest void ratios with admixtures (that is filling the least volume of void space) can be considered at the most effective frost action preventives. From Plate A-26 it is, therefore, apparent that asphalt emulsion, "AES-1," gives the best results, followed by tar and asphalt cutbacks and lastly by Bunker "C" oil. The percentage of admixture used is meaningless unless accompanied by the density and grain size characteristics of the soil tested.

3-04. Description of Laboratory Cold Room and Equipment. The laboratory tests were carried out in the Soil Mechanics Laboratory, darvard Graduate School of Engineering. General layout of the cold room and equipment is shown on PlateA-9. (For description of Cold Room and equipment used, see paragraph 2-04, Part 1).

3-05. Tests for Frost Action.

- a. Soils Tested. Three gradations of soil were tested with Bunker "C" oil as an admixture. They have been designated as Soil Mix No. 1, Soil Mix No. 2, and Soil Mix No. 3. A silt, designated "New Hampshire Silt" was tested with "Darex A.E.A." The three soil mixes consisted of varying percentages of silt, washed and and washed gravel passing a ½-inch sieve. The percentage of each soil mix according to the Bureau of Soils Classification was as follows:

 Soil Mix No. 1, 17% silt, 58% sand and 25% gravel; Soil Mix No. 2,

 17% silt, 33% sand and 50% gravel; Soil Mix No. 3, 17% silt, 15% sand and 68% gravel. The silt in each mix was that designated as "New Hampshire Silt" (ML), a brown uniform silt obtained from a varved deposit located south of Manchester, N. H. The grain size distribution curves with the specific gravity and properties of these materials are shown on Plate A-27.
- b. Admixtures Tested. Two admixtures were used for testing, namely, Bunker "C" oil, which conformed to specifications in Bureau of Standards Bulletin No. CS 12-40 for No. 6 fuel oil and "Darex A.E.A.," a product of the Dewey and Almy Chemical Company consisting substantially of a triethanolamine salt of a sulfonated hydrocarbon. Bunker "C" oil had been tested previously and proved to be effective in preventing frost action. It was desired to obtain further information concerning this material when used in soils having a varying gravel content. "Darex A.E.A." was selected for testing because it had not been tried before and because if it did prove successful in preventing frost action, judging from the

small quantities usually added to concrete for air entraining purposes it might provide an economical means for treating soils. The following percentages of admixtures by dry weight of soil were used in the samples tested:

Soil	Per Cent Bunker "C" Oil	Per Cent Darex A.E.A.
Soil Mix No. 1 Soil Mix No. 2	0,2,4,5,6 0,2,4,5,6	
Soil Mix No. 3	0,2,4,5,6	0.05, 0.10, 0.60, 0.90
		1.20, 2.00

- c. Preparation of Samples. Each of the soils was air dried, thoroughly mixed and lumps broken down.
- (1) Soil Mix No. 1, Soil Mix No. 2, and Soil Mix No. 3 were tested with and without admixture. The silt was tested with admixture only having been previously tested without admixture. All specimens were prepared by compacting with a ten-pound hammer with area of face equal to 3.14 sq. inches dropped eighteen inches for a varying number of blows in seven layers. They were prepared with a predetermined water content in a split container 3.3 inches in diameter and 6.5 inches high (See Plate A-11) to a selected unit dry weight. The predetermined water content used was the optimum water content required for Modified A.A.S.H.O. density. The unit dry weight used was approximately 95 per cent of Modified A.A.S.H.O. density.
- "Darex A.E.A.," tests were performed to determine the effect of mixing time on density under Modified A.A.S.H.O. compaction. The "Darex A.E.A." was added to the water and then mixed into the soil

with a mechanical mixer for a time ranging from one to ten minutes.

A summary of these tests are shown in tabular form below. No significant effect on density was caused by varying the mixing time when the soil was mixed for two or more minutes.

Per Cent Darex A. E. A.	Water Content*	Dry Density Lbs/Cu.Ft.**	Mixing Time Minutes
0.25	14.4	103.6	1
0.25	14.2	106.7	2
0.25	14.1	106.5	3
0.25	16.1	107.5	4
0.25	15.9	107.4	4 5 6
0.25	15.3	107.7	6
0.25	15.9	109.0	7
0.25	13.8	106.2	7 8
0.25	14.1	106.7	9
0.25	14.0	106.8	10
0.70	14.8	106.6	2
0.70	14.4	107.0	
0.70	14.4	106.7	5
0.70	14.3	106.8	6
0.70	14.3	106.6	4 5 6 8
0.70	14.2	107.2	10

^{*}Determined from wet weight and oven dry weight. The latter included "Darex A.E.A." residue

**Oven dry weight. No correction for "Darex A.E.A." in sample.

- (3) In the preparation of the test specimens using Bunker "C" oil as an admixture, the required water was first added and mixed, then the Bunker "C" oil, preheated to a temperature of 140°F, was added and mixed.
- (4) Photographs were taken of samples for each percentage of admixture used in each soil immediately after molding, with the exception of those samples which were room dried, and the samples were then dipped in paraffin. The samples were then slipped into greased cardboard tubes. Samples of silt molded with 0.10 and 0.90 per cent

"Darex A.E.A." were tested as molded and also air dried from one to four days in order to determine the effect of initial water content of the sample on frost action. Those samples which were air dried were weighed as molded and weighed again at the end of the drying period to permit calculation of the water content just before dipping into the paraffin. Prior to placing in the freezing cabinet, the paraffin was removed from one end of the specimen and the open end was placed on a piece of filter paper on a porous stone. The cardboard tube was sealed to the drawer pan by use of a rubber membrane and clean dry sand was placed around and level with the top of the samples in the drawer for insulation. The water level was adjusted to the top of the porous stone so that water was available at the bottom of the sample throughout the tests. A schematic diagram showing the samples in the freezing cabinet is shown on Plate A-9.

(5) The capacity of the freezing cabinet was sixteen samples. Fifteen samples were tested together in Series D. This series consisted of three sets of five samples, each set molded with Soil Mixes 1, 2 and 3. One sample of each set contained no admixture and four samples contained varying percentages of Bunker "C" oil. Sixteen samples were tested together in Series E. All of this series was run using "New Hampshire Silt" with "Darex A.E.A." as an admixture.

d. Test Procedure.

(1) The test procedures followed in these tests are described in paragraph 2-05d. with the exception that cabinet air temperatures were measured separately for each drawer. Two mercury thermometers were suspended directly over the samples in two alternate

drawers. Two copper constantan thermocouples were placed in an antifreeze liquid near the tops of the samples in the other two drawers.

The thermometers and thermocouples were used to obtain daily temperatures in each drawer during the test. The degree hour curves for each test were computed and they are shown on Plates A-28 and A-29.

e. Summary of Test Results.

(1) All the soils tested were affected by frost action in the untreated state. In series D, frost action was reduced to a very slight amount in Soil Mix No. 1 by the addition of six per cent Bunker "C" oil, frost action was provented in Soil Mix No. 2 by the addition of five per cept Bunker "C" oil and frost action was prevented in Soil Mix No. 3 by the addition of six per cent Bunker "C" oil. Soil Mixes No. 2 and No. 3 with four per cent or more Bunker "C" oil showed no gain in water content during the test. In the untreated samples, water migrated to the top. Samples treated with Bunker "C" oil generally had a higher water content at the bottom. The addition of this admixture apparently retarded the tendency of the water to pass through the sample and accumulate in the upper soil layers. The addition of the larger percentages of admixture waterproofed the soil and thereby prevented the samples from taking on additional water through capillary action. Soil samples which compacted to higher densities showed no gains in water with the addition of smaller percentages of Bunker "C" oil. Gains in water content stopped at approximately the same void ratio with admixture in each soil mix. Water content profiles for Series D with photographs taken before and after

freezing for the samples with no admixture, for those with a minimum of admixture which heaved and those with a minimum of admixture which did not heave are shown on Plates A-30 through A-35. A summary of frost action test data for Series D is contained in Plate A-39.

affected by frost action. No relationship between the amount of heave which occurred and the amount of "Darex A.E.A." used, or the initial water content or any other predetermined characteristic of the sample was apparent as a result of these tests. Water content profiles for selected samples in Series E and a sample in Series B (tested in a previous investigation) without admixture, together with photographs taken before and after freezing are shown on Plate A-19 and on Plates A-37 through A-39. A summary of frost action test data for Series E is contained in Plate A-40.

GLOSSARY

Certain terms and words which have a specialized use in this report are defined below.

Admixture is a material which is added to a soil to prevent frost action.

Degree Hour is the cumulative total of the algebraic difference between 32 degrees Fahrenheit and the hourly mean temperature below 32 degrees Fahrenheit.

Density is the unit weight in pounds per cubic foot.

Dry Density is the unit weight in pounds per cubic foot obtained from the wet density, by deducting the weight of water and admixture, i. c. the unit weight of the soil particles.

Frost Action is the accumulation of water in the form of ice lenses in soil or base materials under natural freezing conditions.

Frost Heave is the raising of the surface due to the accumulation of ice lenses. The amount of heave in most soils is approximately equal to the cumulative thickness of the ice lenses. Expressed as a percentage, it is the ratio (multiplied by 100), of the increase in height of the soil sample divided by its original height.

Frost Susceptible Base consists of a soil which contains more than three per cent of grains smaller than 0.02 mm. in diameter, placed and compacted on a subbase or subgrade.

Ice Lenses are the ice formations in frozen soil occurring in repeated layers essentially parallel to each other and normal to the direction of heat loss.

Non-Frost Susceptible Base consists of a soil which contains less than three per cent of grains smaller than 0.02 mm. in diameter placed and compacted on a subbase or subgrade.

Degree of Saturation is the ratio, expressed as a percentage, of the volume of the water to the volume of the voids.

Rock Content in this report is the portion of a soil retained on a No. 10 sieve.

Void Ratio is the ratio of the volume of the voids to the volume of the soil particles.

Void Ratio with Admixture is the ratio of the volume of the soil not filled by the seil particles and admixture to the volume of the soil particles and admixtures.

Void Ratio without Admixture is the ratio defined as "Void Ratio."

Volume of the Voids is the volume of the soil not occupied by the soil particles.

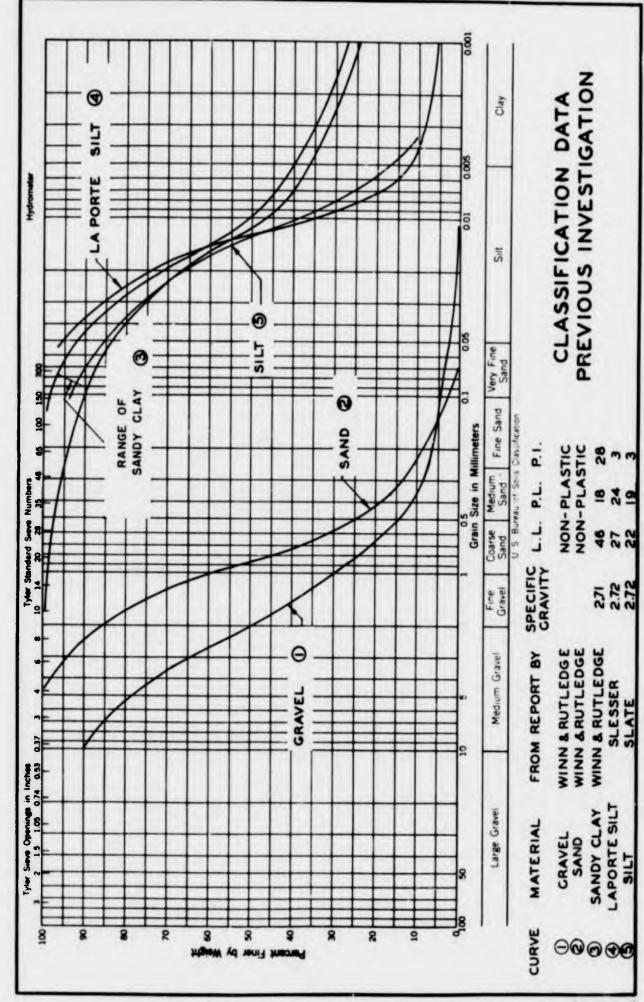
Water Content is the ratio, expressed as a percentage, of the weight of water in a given soil mass to the weight of soil particles.

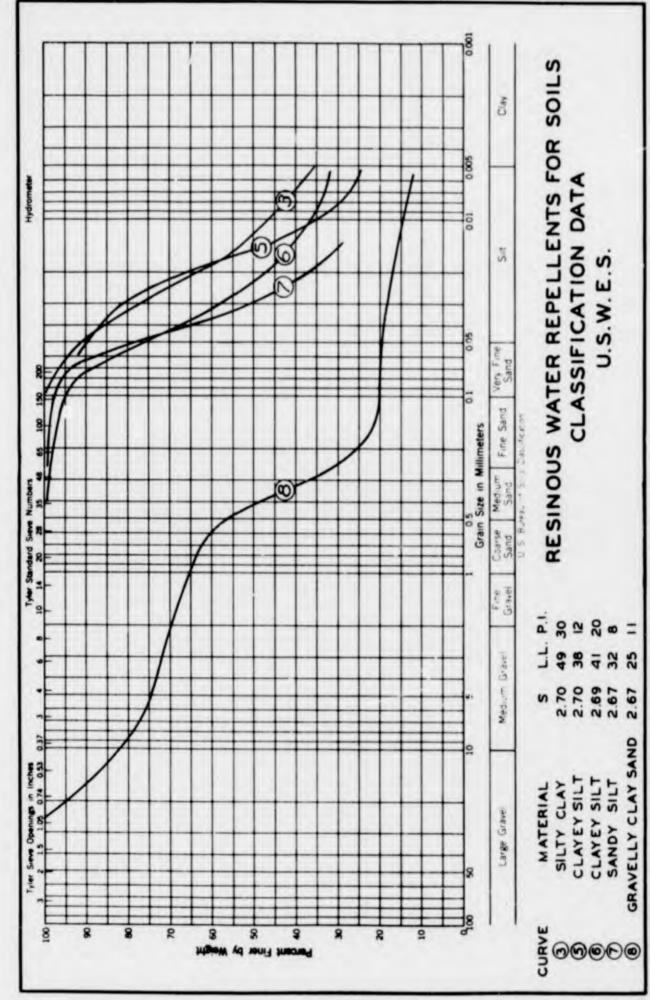
Wet Density is the unit weight in pounds per cubic foot of the soil, including the weight of water and any admixture.

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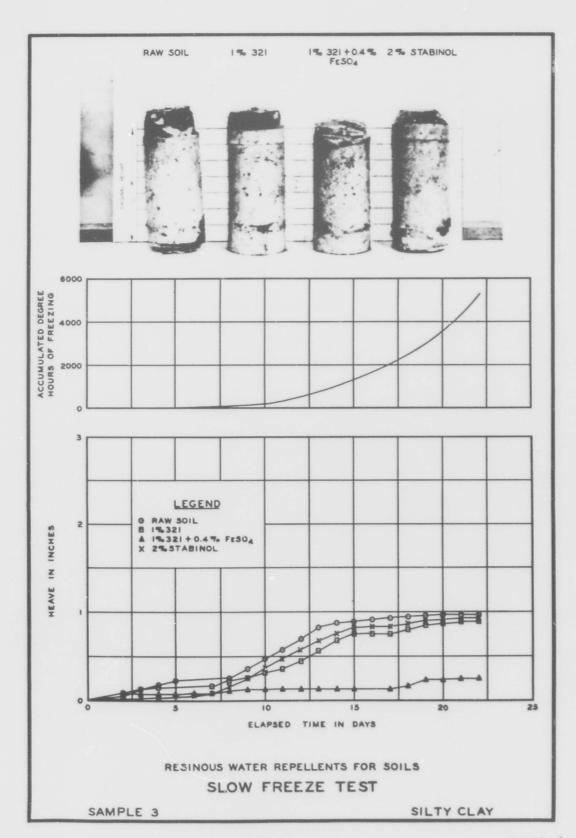


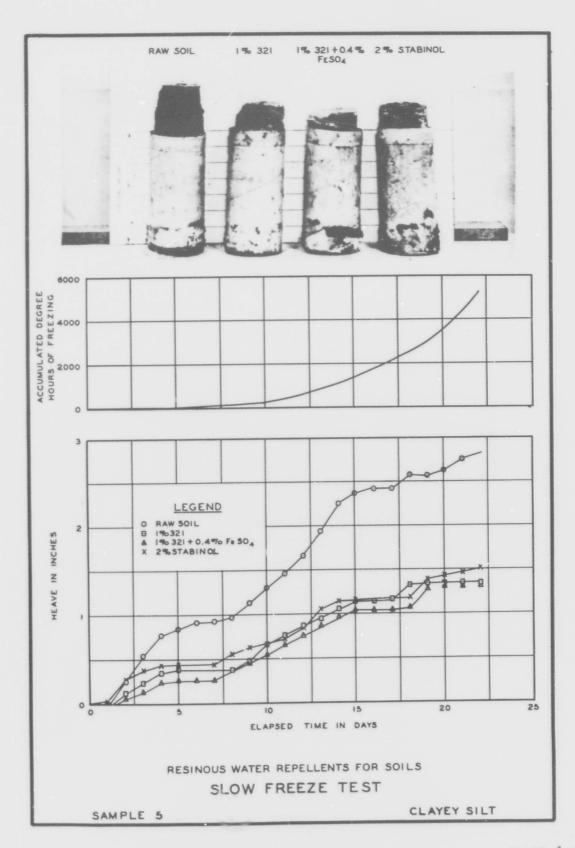
SUMMARY OF DATA OF SLOW FREEZE TESTS

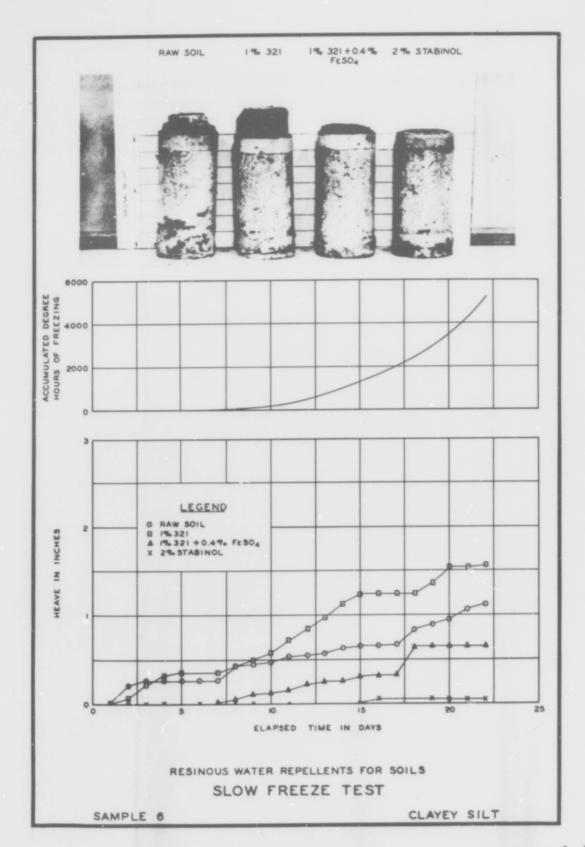
RESINOUS WATER REPELLENTS INVESTIGATION (1)

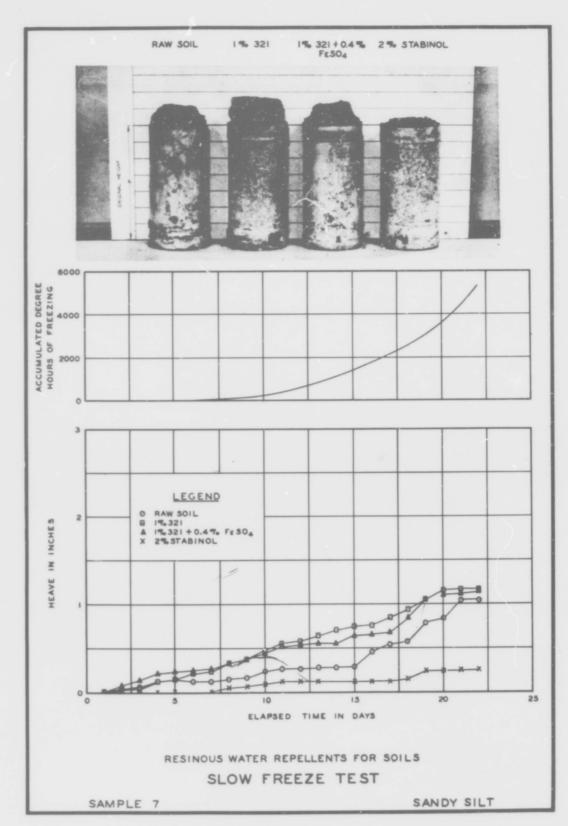
200		Specific	Wat	Mater Content	nt	Dry Weight	Per Cent	Per Cent
7100		Gravity	As Molded	After	After	Jod	Saturation	Heave
3	15, 721	2.70	20.1	0.01	27.7	98.7	Ek	14.5
Clay	1% 321 + 0 JU. FeSOL		19.1	8	25.5	95.7	281	3.6
1	Z/0 2 CMDTTOT		C-64	Y	6.55	6.1.6	2	15.5
Clayey	1% 321	2.2	19.6	-	2,4 8,9 8,9 8,9	97.0	67.R	19.3
Silt	15 321 7 0 J& Poso ₄ 25 stabinol		19.8	8.6	23.5	95.4 98.1	2%	19.3
6 Clayey	1,521	2.69	18.2		20.7		878	24.3
Silt	15 321 7 0 15 Fe SO ₄		18.2	8.9	19.8	101.5	42	2.01
7 Sandy	16 30	2.57			25.4		12	
Silt	1% 321 7 0 16% Fe SO ₁ , 2% Stabinol		565	2 C B	24.5 13.9	98.6	228	17.5
89		2.67	7.5		15.9		·	
Clay	1% 321 4 0.1% Fe SO ₁₄		0, 00 a	n v u	2, 7, 1, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2,	121.1	& 84 <u>4</u>	200
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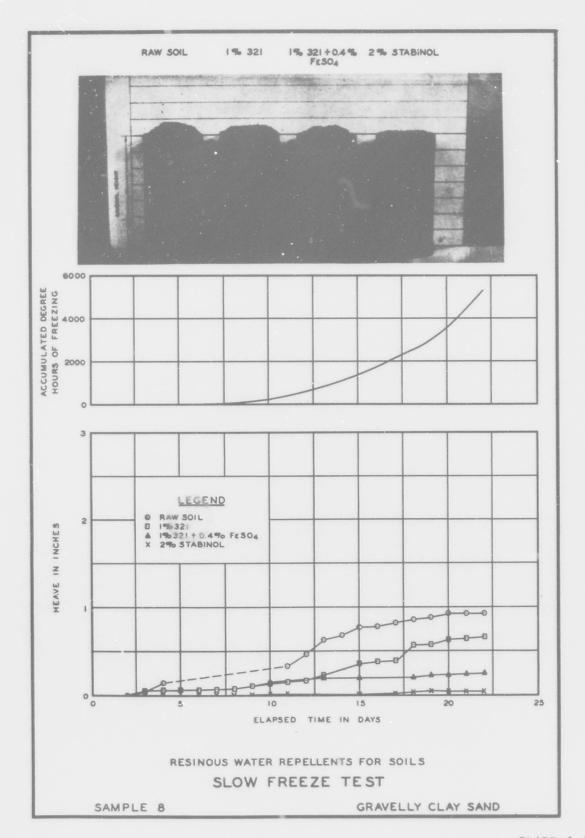
(1) Mississippi River Commission, U. S. Waterways Experiment Station, Draft of Interim Report of Water Repellents Investigation and pertinent data supplied by letter.

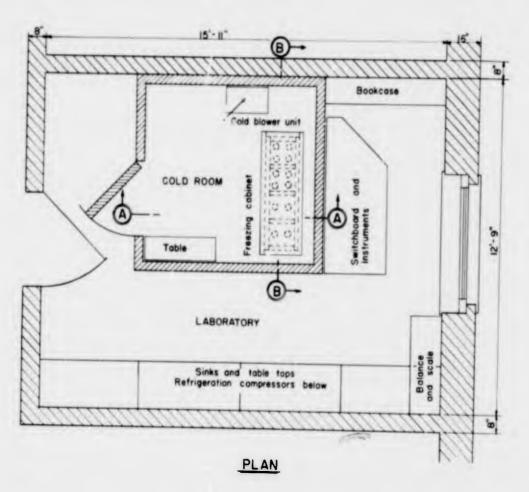






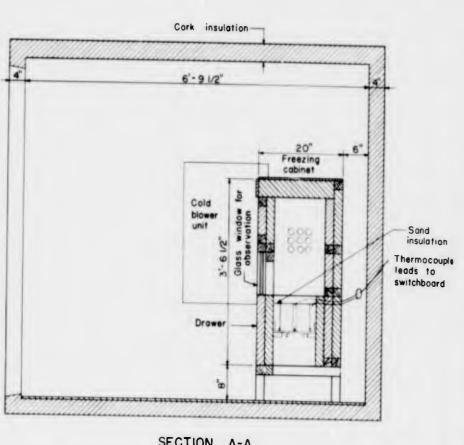


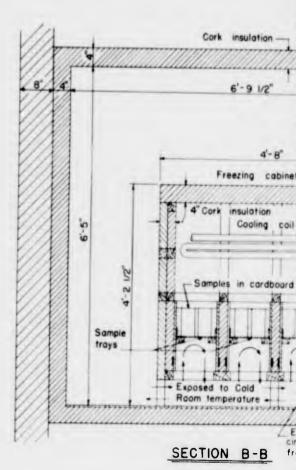




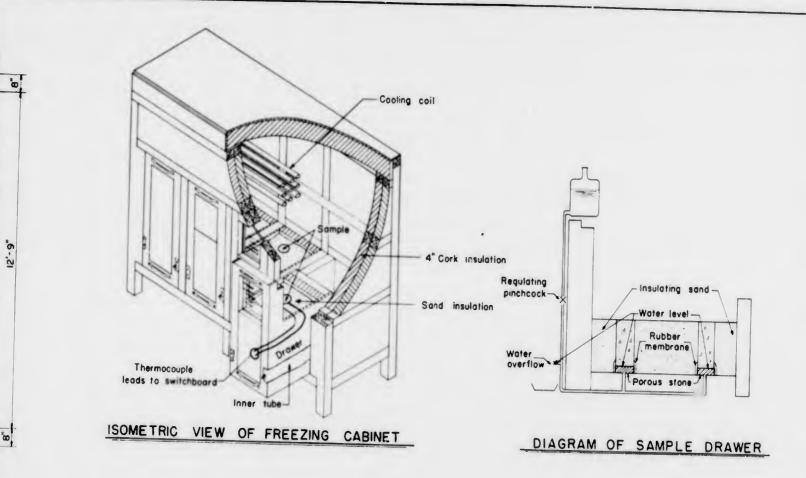


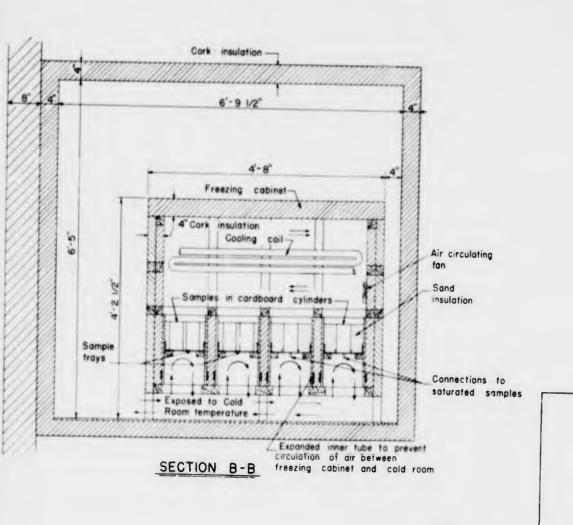
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SECTION A-A





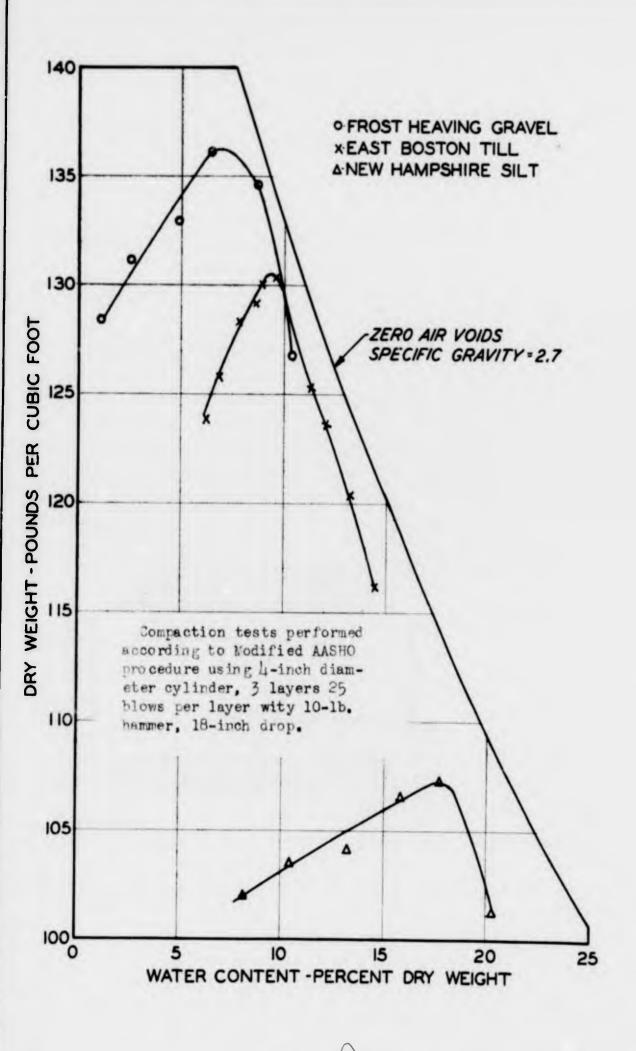
FROST INVESTIGATION

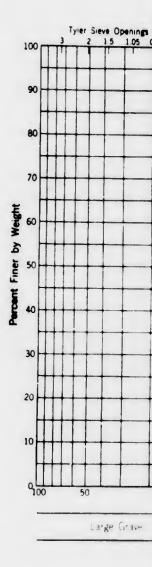
DETAILS OF COLD ROOM AND TEST APPARATUS

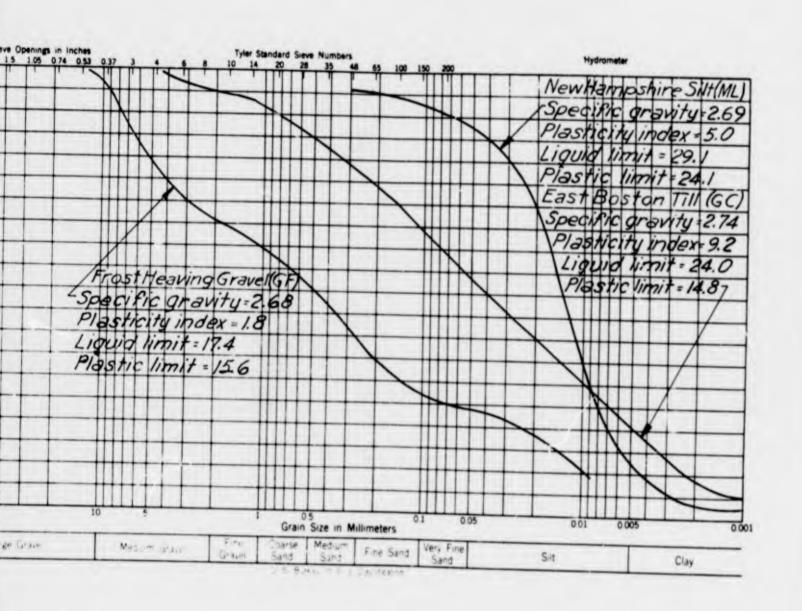
SOILS LABORATORY
NEW ENGLAND DIVISION

JUNE, 1947 BOSTON, MASS

PLATE A-9







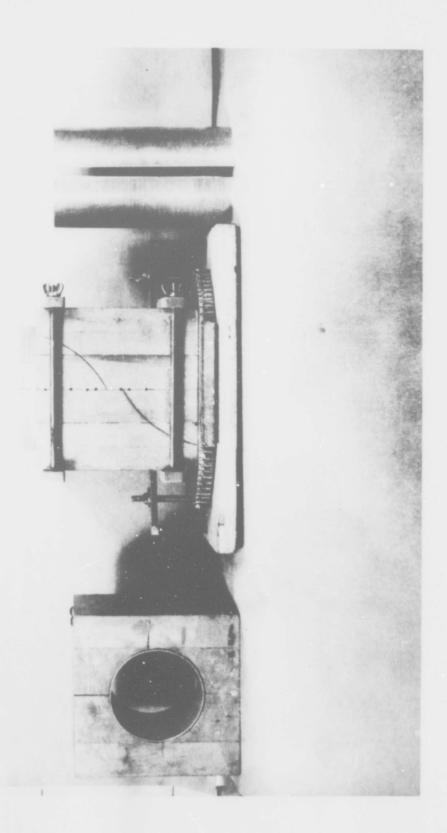
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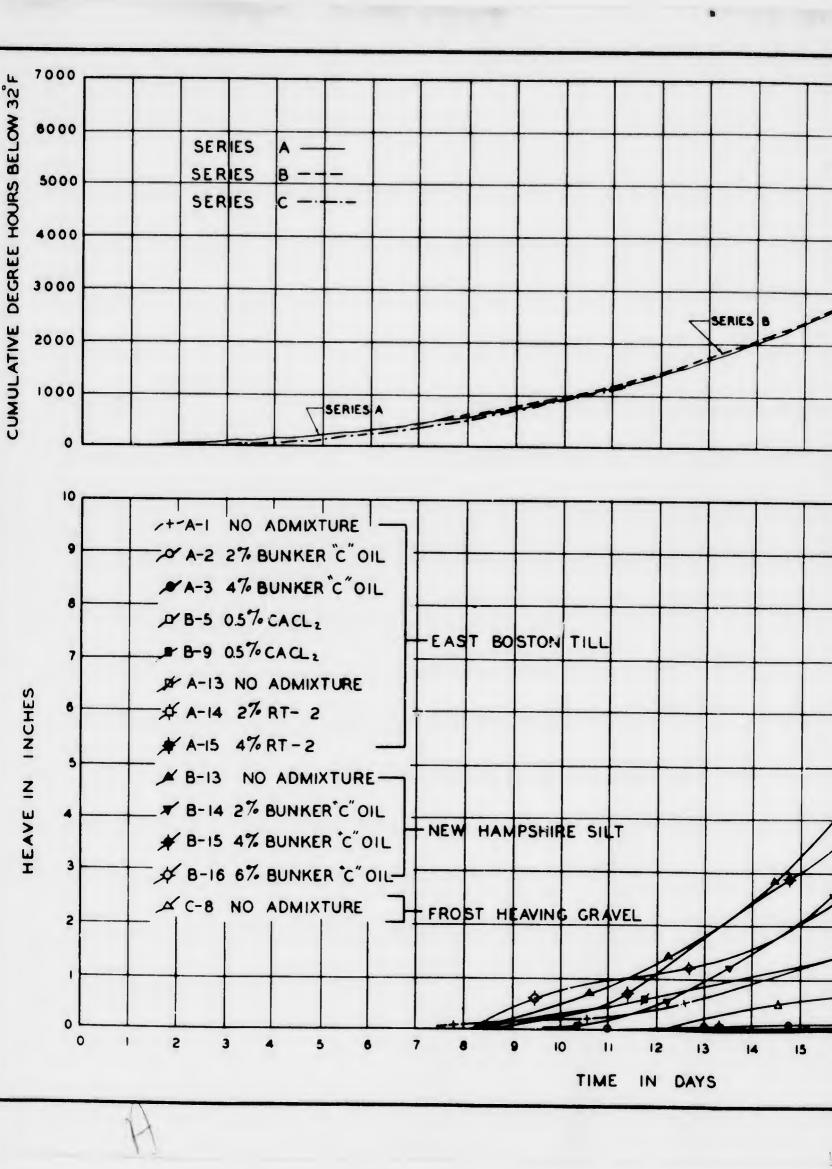
FROST INVESTIGATION
BASE COURSE TREATMENT
TO PREVENT FROST ACTION

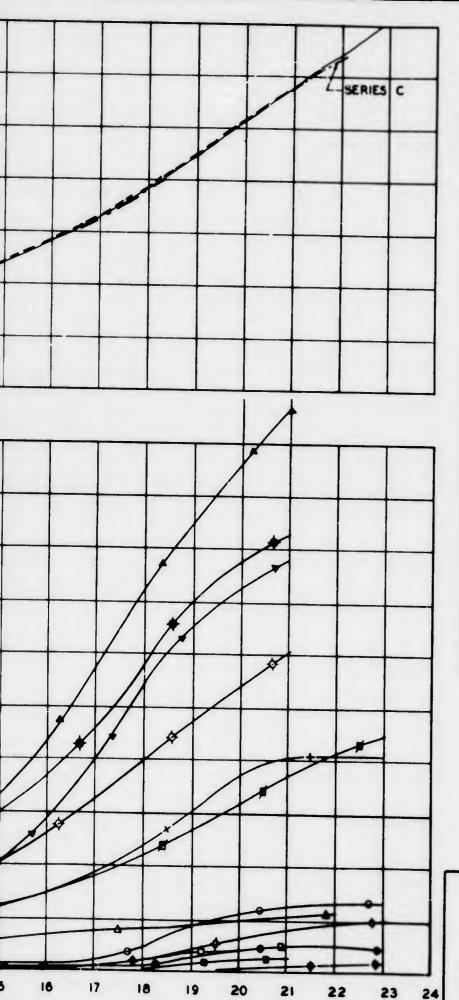
SUMMARY OF SOIL TEST DATA

JUNE 1946 FROST EFFECT'S LABORTORY, BOSTON, MASS

PLATE A-10







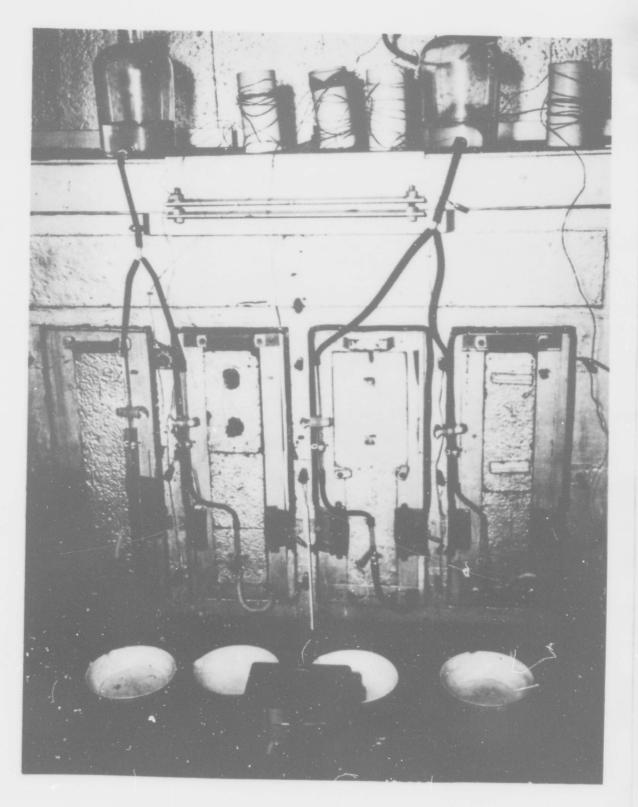
APPLIE) TEM	PERAT	URES					
ELAPSED	TEST SERIES							
(DAYS)	A (°F)	B (°F)	C (°F)					
-2 -1 0 12 3 4 5 6 7 8 9 10 11 12 13 14 15 16 7 18 19 20 21 22 23	352210998765432086420 5555**	34 34 32 31 30 99 28 72 20 20 81 10 10 55 55 7	40 35 32 31 35 37 27 26 22 22 22 22 22 22 21 35 55 55 55 55 55 55 55 55 56 56 57 57 57 57 57 57 57 57 57 57 57 57 57					

* FINAL DAY OF TEST

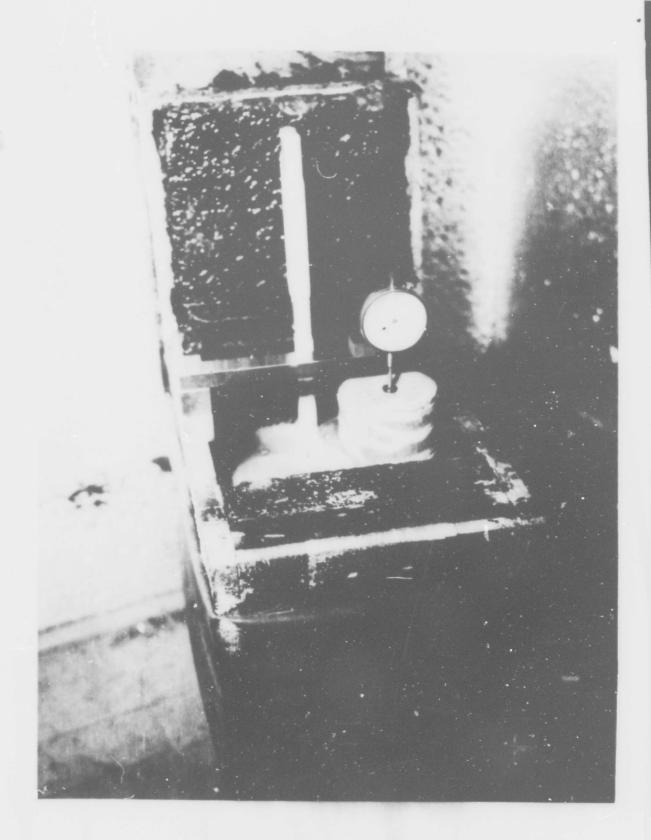
FROST INVESTIGATION
BASE COURSE TREATMENT
TO PREVENT FROST ACTION
1945 - 1946

HEAVE AND TEMPERATURE DATA

JUNE 1946
FROST EFFECTS LABORATORY , BOSTON, MASS.



Freezing cabinet showing Method used to supply water to the samples



Measuring Heave with 0.001 inch Extensometer

SERIES		PER CENT		VEE	DEN	SITY	LENGTH	TA	TER CCI	TENT	PATERN	TION	WALK			
AND	SOIL	OF ADA	IXTURE		p.c.f.	DRY WT.	TEST IN	END OF	START	GAIN	START	EN'D OF	RATIC AT		CONTENT	1
NO. BUNKER "C" R	RT-2	CaCl2	BEFORE TEST	BEFORE	DAY3	TEST	TEST	TEST	4	TEST	START	BEFORF TEST	AFTER TEST			
A-1	East				139.1	127	26	1.6.4	0.7	26.7	77	100/	+35	10	110	
A-5	Boston	2	-	-	138.4	124		20.6	9.2	11.4	65	100/	.37	9	79	П
A-3	7111	4	-		133.8	186		12.9	7.1	5.5	46	87	.42	7	51	1
A-4		6	-	-	133.€	119		8.7	6.2	5.5	1,1	58	-44	6	13	L
A-5		-		1	140.0	127		13.7	9.0	4.3	75	100/	.35	9	13	ı
A-6		2		1	:1,0.9	126	1 1	10.6	9.0	1.6	70	83	.36	9	12	ı
A-7		L	-	1	135.0	130		9.5	6.5	2.6	1.7	65	.42	7	11	L
4-8		6		1	132.2	117		6.1	5.5	0.5	36	39	.1.6	Ü.	9	ı
A-9			-	2	135 .5	125		13.1	1.9	1.0	68	100	.37	5	12	ı
A-10		5	-	2	1:7.5	123		11.7	3.6	2.5	63	60	.40	9	14	L
A-11		6		2	132.8	116		94	6.5	2.9	142	60	.1,5	6	11	ı
A-12	U X	5	-	3	133 4	117		7.5	3.6	1.7	37	48	.46	6	9	1
A-13					136.6	125		1.6.F	9.0	37.2	71	100	•37	10	21	
A-14		-	5		133.4	121		21.6	8.0	13.6	54	100/	.41	8	5/1	ı
A-15	×		4		133.4	119		11.7	7.7	4.0	51	78	.43	8	15	ı
A-16			6		133 -1	117	1	9.3	7.5	1.8	47	58	-1.7	7	11	
8-1			-	1	137.L	125	514	14.0	6.6	54	65	100/	.37	9	15	
B-2			1	1	139.0	125		12.4	2.0	3.4.	68	100+	•37	9	214	
B-3	7 3		5	1	138.2	124		10.8	8.5	2.3	62	79	.35	3	15	ı
8-4			14	1	140.0	122		10.0	9.4	0.6	67	72	.40	9	11	
P-5			*	0.5	138.5	126		17.7	5.5	8.5	71	100/	•36	9	18	
P=6			1	0.5	138.4	125		13.6	9.0	4.6	68	1004	•37	9	16	1
F-7			5	0.5	136.7	155		12.4	6.4	4.0	60	89	.39	8	114	
B-8			4	0.5	137.2	127		9.1	7.7	14	E/E	64	.41	8	12	
B-9		-	-	0.5	139.6	127		16.L	9.2	7.2	72	100	•35	9	19	
E-10		1	-	0.5	138.7	126		12.1	6.8	3.5	66	93	.36	9	15	ı
B-11 B-12		5	-	0.5	138.0	121		11.4	B.1	3.3	61	86	.58	8.	13	1
B-13	New	-	:	0.5	114.6	101		8.1	13.4	89.1	55	100/	.66	7	29	⊢
B-14	Hamp-	2	:		117.6	101		82.5	13.9	68.6	53	100/	.66	114	26	ı
B-15	shire	4			122.3	103		82.7	14.9	67.8	65	100	.64	15	21	
F-16	Silt	6	•	-	121.0	101	1	63.1	13.0	50.1	56	100/	.66	13	50	
c-20				3	107.6	87	25	26.2	20.3	5.9	60	75	.93	50	31	
C-19			-	6	107.2	85	Ť	23.2	19.4	3.8	56	67	.98	19	27	1
C-21			-	9	108.1	85	1	23.8	15.1	5.7	54	71	.9€	18	32	
C-22	-		•	10	106.9	814		22.E	18.1	4.7	53	67	1.00	18	26	
c-8	Frost		-		139.4	132		11.2	5.7	5.5	57	100+	•27	6	11	
C-17	Heaving		-	1	140.6	131		10.0	6.0	4.0	59	99	•27	6	10	
C-6	Grevel			2	11,2.9	132		9.1	6.4	3.7	65	92	.27	6	9	
C-7			-	3	175.9	130		€.1	6.7	1.1.	64	77	.29	7	8	
0-9			-	5	128.8	117		9.7	8.3	1.4	52	61	-lala	8	12	
C-10 C-11			-	3.5	128.2	114		9.0	8.6	04	51	54	.17	9	10	
C-15				4.5	128.6	114		8.9	8.0	0.9	48	53	-47	8	10	
		;		5.5	130.8	115		6.2	7.9	0.6	1.9	53	.46	8	10	
C-13		1		2	143.7	131		6.0	6.4	1.6	64	80	•5€	6		
C-11.	11	2 7		2	143.3	129		7.6	6.5	1.1	62	73	.29	7	9 9	
C-15 C-16		*	-	2	143.3	128		7.4	6.4	1.0	59	69	.30	6		
6-16		L	-	2	11.3.0	127	1	7.1	6.6	0.5	58	63	•32	7	10	

⁽a) Length of test computed from time samples were placed in drawer until samples were removed.

TEA	TER CCI	TENT	SATURA	TION					DEGREE			
end of	START	GAIN DUFING	START	END	RATIC AT	OF FOTT	CONTENT CU INCK	HOURS	HOURS TC START	HRAVE IN PER	FROST	FRCCEN ZONE
7237	TEST *	TEST 4	4	7597	START	BEH CRF TEST	AFTER TEST	TRST	OF HEAVE	CENT		
1.6.L 20.6 10.9 8.7 13.7 10.6 9.5 6.1 13.1 11.7 9.5	70.10.700* 00.50 00.700* 00.50	30.7 12.5 1.6 1.6 1.6 1.6 1.6 1.6 1.6 1.6 1.6 1.6	77 60 41 75 70 47 36 63 42 37	100/ 67 56 100/ 63 65 39 100 60 40	35 35 36 42 46 37 40 45	10 97-6 0 97-0 9 0 0 6	110 79 41 13 12 11 9 12 11	7155	793 1205 3931 3931	#30.00000000000000000000000000000000000	Severe Severe Voderate None Slight Tone None None None None None None	Entire Pottom 1.3" Bottom 1" Fore None None None None None None None
11.7 9.5	8.0 7.7 7.5	37.2 13.6 4.0 1.8	71 54 51 47	100/ 100/ 78 58	.37 .41 .43 .47	10 8 8 7	21 24 15 11		639 2912 5871	69.9 16.3 3.5 0.0	Severe Severe Slight No: e	None Bottom 0.5" not frozen Bottom 1.75" not frozen Bottom 1" not frozen None
14.0 12.4 10.6 10.0 17.7 13.6 12.4 4.1 16.4 12.1 11.4 5.1	8.6.0 8.4.2 9.4.2 9.4.7 9.6.6 10.6	5.4 2.3 0.5 6.5 4.6 1.4 7.2 3.3 1.5	65 68 67 71 68 60 55 72 66 61 45	100/ 100/ 79 72 100/ 100/ 89 61 100/ 93 86	.37 .37 .36 .40 .36 .37 .39 .41 .35 .36 .98	99899988998	15 11 16 16 14 12 19 13 14	54.21	1735 3693 2455	0.00	None None None None None Moderate None Moderate None None None	Top 3" None None None None Pottom C.5" not frozen Frozen 1"-3.5" from bottom Nore None Pottom 0.5" not frozen None None None
102.5 82.5 82.7 63.1	13.4 13.9 14.9 15.0	89.1 60.6 67.8 50.1	55 53 65 56	100/ 100/ 100/ 100/	.66 .64 .66	13 14 15 13	29 26 21 20		600	161.5 115.5 126.0 91.0	Severe Severe Severe	Bottom 2.6" not frozen Bottom 1.9" not frozen Pottom 1.9" not frozen Pottom 1.6" not frozen
26.2 23.8 23.8 22.8	19.4 19.1 18.1	5.9 3.8 5.7 4.7	60 56 54 53	7P 67 71 67	.93 .98 .98 1.00	20 19 18 18	31 27 32 26	6393	:	0.0	None None None	None None None
11.2 10.0 9.1 8.1 9.7 9.0 8.9 8.5 8.0 7.6 7.4 7.1	5.7 6.0 6.14 6.7 8.6 6.0 7.0 6.5 6.6	3.7 1.4 0.4 0.9 0.6 1.6 1.1	57 59 65 64 52 51 88 19 64 62 59 58	100,7 99 92 77 61 54 53 60 73 69 63	27 27 29 44 17 46 26 29 30	666789686767	11 10 9 8 12 10 10 10 8 9		916 -	16.5 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Severe Fone None None None None None None Hone	Entire None None None None None None None Non

les were

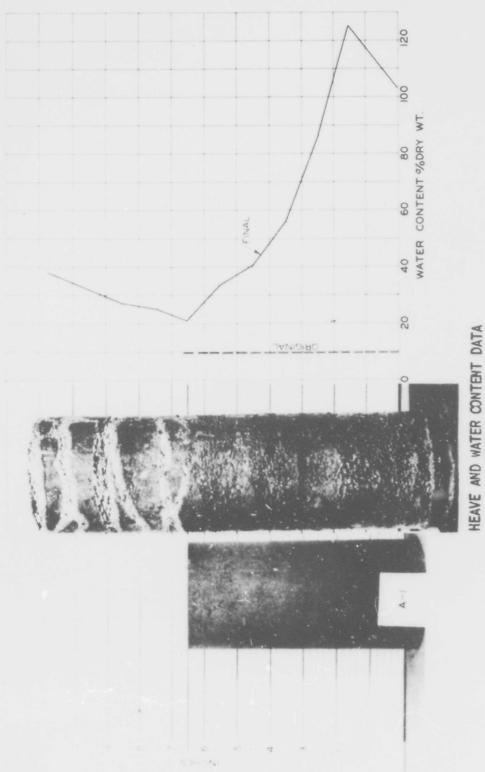
FROST INVESTIGATION 1945 - 1946

BASE COURSE TREATMENT TO PREVENT FROST ACTION

SUMMARY OF FROST ACTION TEST DATA

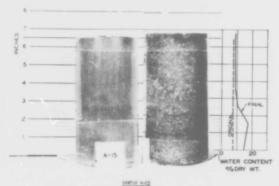
JUNE 1948 FROST EFFECTS LABORATORY, BOSTON, MASS.



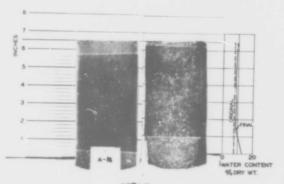


HEAVE AND WATER CONTENT DATA SAMPLE A-1

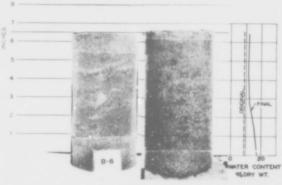
EAST BOSTON TILL WITH NO ADMIXTURE. ORIGINAL DRY WEIGHT 127 PCF, WATER FINAL WATER CONTENT CONTENT 9.7 PER CENT AND 76 PER CENT SATURATION. 46.4 PER CENT.



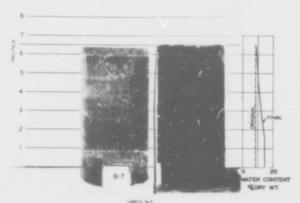
BAST MOSTOR TILL WITH L PER CONT NT-2 ADMIXTORS, ORIGINAL DRY MEIGHT 12L per, MATER CONTENT 7.7 PER CONT AND 96 PER CONT SATURATION,

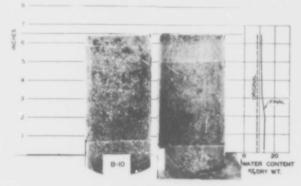


EAST MOTOR TILL MITS 6 FOR COST MT-2 ADMINISTRATION ONIGINAL DRY MEIGHT LIS. por, MATER CONTONT 7.5 FOR COST AND SS. FOR COMM MATERIATION,



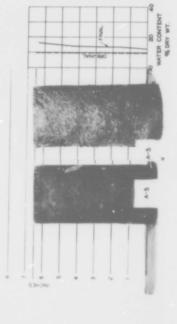
SAFTA NATION TILLERS I PER CRES NT-0 AND 0.5 FEE CENT CALCTIN COUNTRY ANNIHOUSE. ON DUTHAL DRY MADDE 127 per, NATER CRETISH 30 FEE CRES AND 127 FEE CRES ANNIHOUSE.



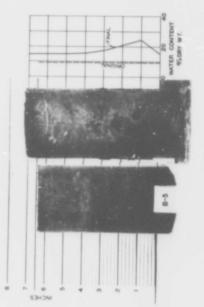


EAST MOTOR TILL HITM I PRE COST MENNEN TO THE ONLY CALCING ONLY AND DAS FOR CONT CALCING ONLY AND EAST OF THE COST AND GO OF TATES OF THE COST AND GO OFF TATES OF THE COST AND GO FOR COST TATES OF THE COST AND GO FOR COST TATES OF THE COST AND GO FOR COST TATES OF THE COST TATES OF

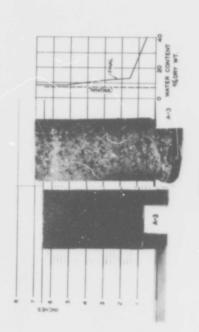
HEAVE AND WATER CONTENT DATA



saft when the part i per calety such es arighes, gridle determined by when the part is set, when cover $\varphi_{i,k}$ for one are in the such areasens,



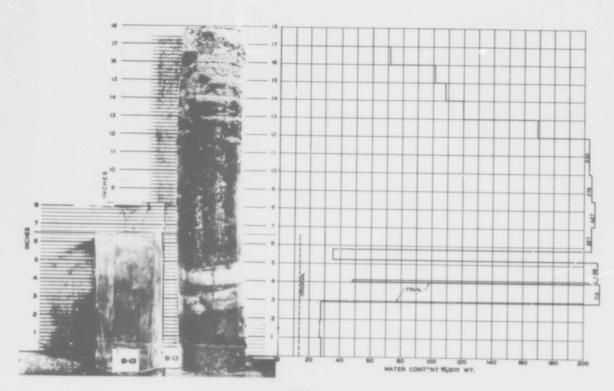
MATERITY TILL WITH G.5 FIR CHICK CLICINS CHICKES CHICKED CHICKED STITUS. BUT WEIGHT 127 pet, Matericanners, 5,5 for chicked and 75 for chickensy.



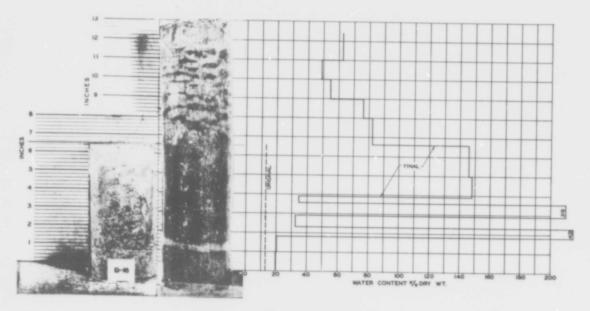
SAR' NOTES TILL NOT A SE SONDE "O" CIL ANTENNA. GEERIAL NO NODRE 105 set, auto covere "1,1 to Com", ac 55 to core derivation.

that work of the state of the s

WATER CONTENT

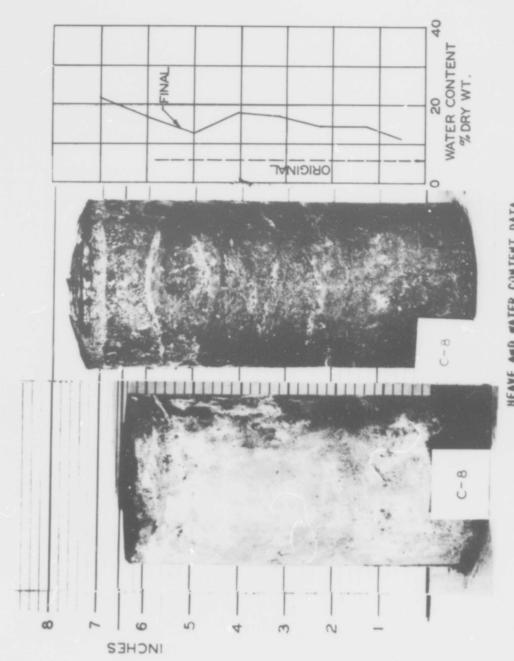


SAMPLE 9-13
NEW GAMPSHIRE SILT WITH NO ADMIXTURE. ORIGINAL DRI WEIGHT 101 per.
WATER CONTENT 15.L PER CERT AND 55 PER CERT SATURATION.



SAMPLY 3-16
NEW NAMEWOFIES SILT WITH 66 HONESS No. OIL ADMIXTHES, ONIGINAL DET WEIGET
107 per, NATHE CUSTEKT 13.0 PER CHST AND 62 PER CHST SATURATUR.

HEAVE AND WATER CONTENT DATA



HEAVE AMD MATER CONTENT DATA
SAMPLE C-3
PROST HEAVING GRAVEL WITH NO ADMIXTURE. ORIGINAL
DRY WEIGHT 132 pof, WATER CONTENT 5.7 PER GENT AND
58 PER CENT SATURATION.

FROST INVESTIGATION SUNMARY OF LEACHING THAT DATA

PAGE COURSE TREATMENT TO PREVENT FROME A

Sample Fercent No. Celcium Chloride		Percent	Unit dry	Water	Content	V01. of				
	Bunker "C" 011	weight, pcf	Refore Testing	After Testing	Voids cc	Ratio	Before Testing	After Testing		
1 2 2 2 2 4 2 2 4 4 2 2 5 CA 2 - 1 2 5 CA 2 5 CA 2 - 1 2 5 CA 2		0 2 2 2 1 1 1 5 5 5 5 5 5 5 5 5 5 5 5 5 5	123 124 124 123 123 126 126 125 125 127 125 127 127 123 113 113 114 102 103 78	8 7 2 2 3 6 7 5 6 7 7 7 5 3 8 4 2 8 9 3 5 4 5	14.6 13.1 13.3 11.5 12.9 13.7 13.7 13.7 13.7 13.7 13.7 14.6 16.3 17.6 16.3 17.6 17.6 17.6 17.6	71. 75.2.2.2.2.2.2.2.2.3.2.3.2.3.2.3.2.3.2.3.	0.3780 0.3780 0.3780 0.3780 0.37788 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.37788 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.37788 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.37788 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.37788 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.37788 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.37788 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.3778 0.37788 0.377	62.66.2.2.0.0.3.1.9.3.0.0.1.6.5.0.6.6.2.2.0.0.3.1.9.3.0.0.1.6.5.0.6.6.2.2.0.0.3.1.9.3.0.0.1.6.5.0.6.6.5.0.6.6.5.0.2.0.0.1.6.5.0.0.1.6.5.0.6.6.5.0.2.0.0.1.6.5.0.0.1.6.5.0.6.6.5.0.2.0.0.1.6.5.0.0.1.6.5.0.6.6.5.0.2.0.0.1.6.5.0.0.1.6.5.0.0.1.6.5.0.0.1.6.5.0.0.0.1.6.5.0.0.1.6.5.0.0.1.6.5.0.0.1.6.5.0.0.1.6.5.0.0.1.6.5.0.0.0.0.1.6.5.0.0.1.6.5.0.0.1.6.5.0.0.1.6.5.0.0.1.6.5.0.0.1.6.5.0.0.0.0.1.6.5.0.0.0.1.6.5.0.0.0.1.6.5.0.0.0.1.6.5.0.0.0.1.6.5.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0	100 + 1.00 + 100 +	

[•] Material and mix same as Sc. Test performed in Lucite Sylinder. Note: Jamule No. 1 same as sample No. 4 - 2.0CA

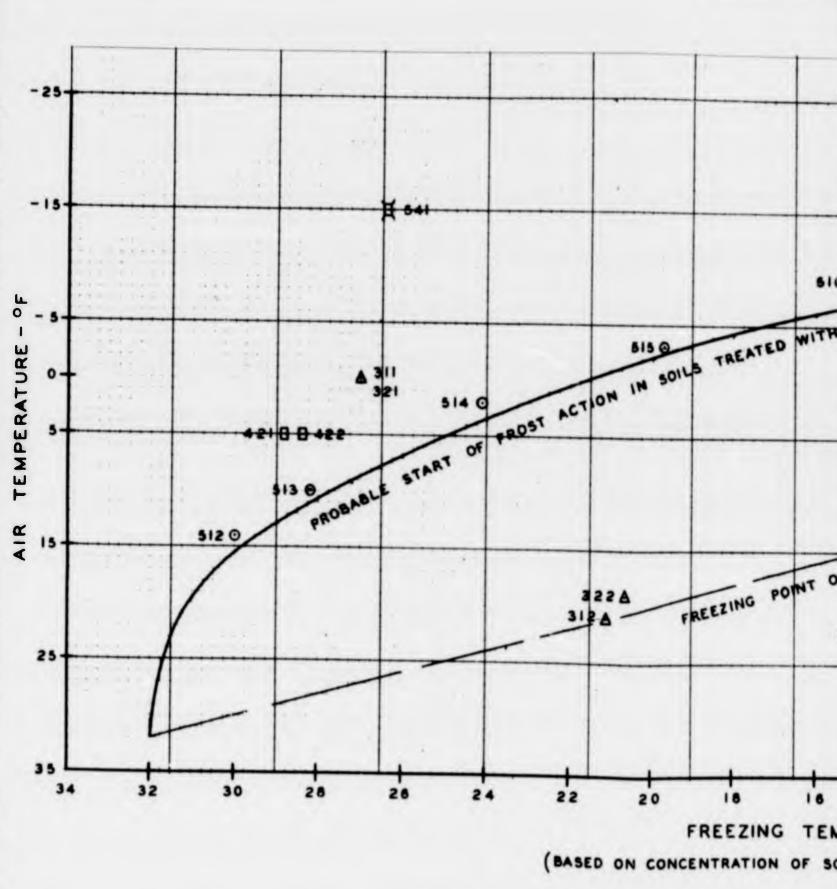


T INVESTIGATION

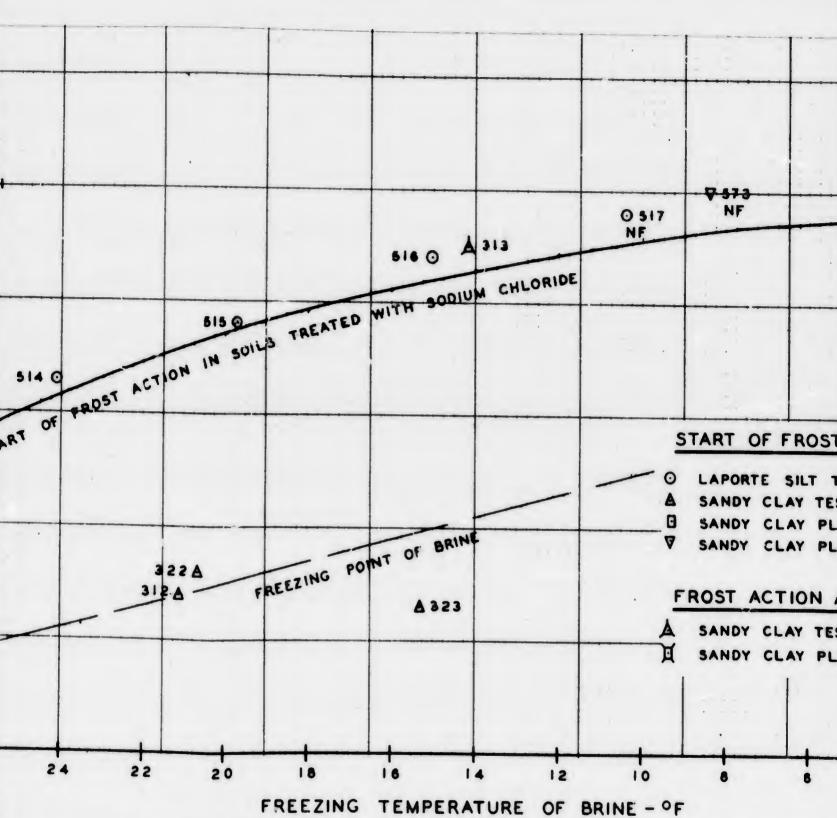
LEACHING THAT DATA

ENT TO PREVENT FROM: ACTION

65.7	Coefficient of Permeability 10-4 0.006 0.007 0.007 0.00045 0.000 0.230 0.230	Hydraulic Gradient 133 133 133 145 133 123 123 123 123 123 123 123 123 123	No. of Changes of Water in Sample to Leach Calt 21 52 28 14 12 11 48 20 13 9 54 21 16 25 14 21 16 25 14 21	360 21,7 230 21,5 197 365 177 365 176 176 176 176 176 176 176 176	
	•	107	1 ⁰	126 159 7!i	Sucite Cyl.

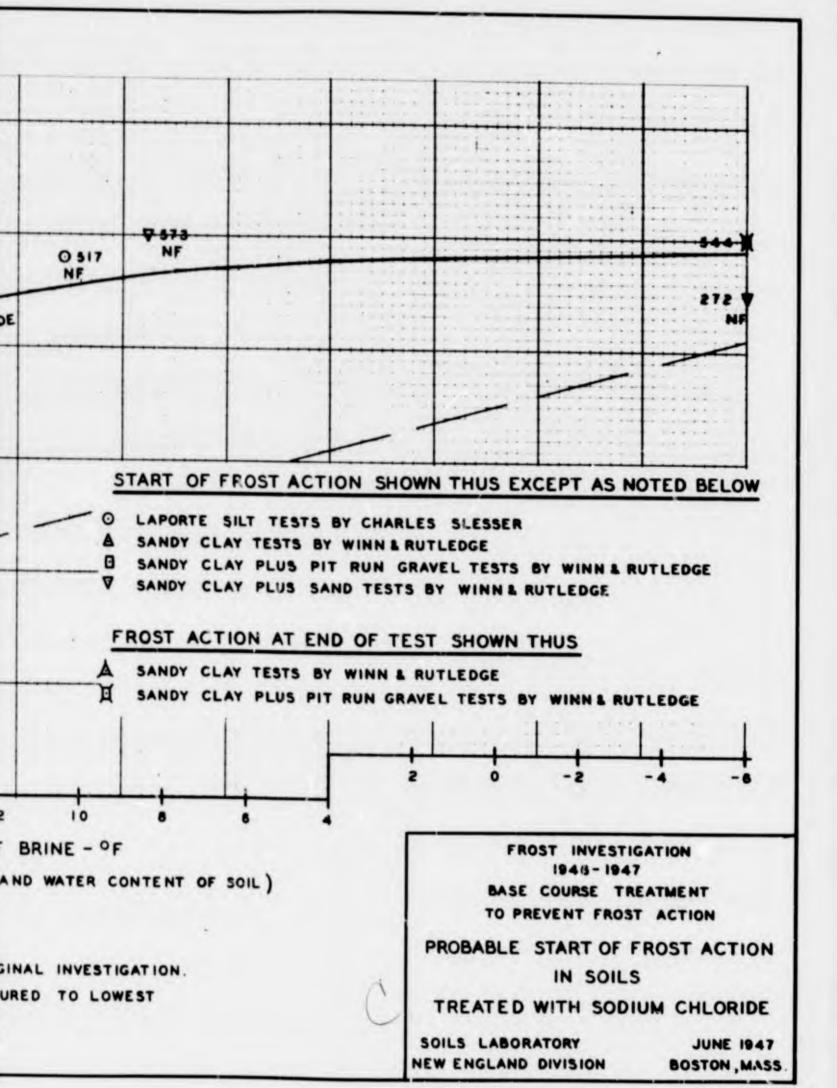


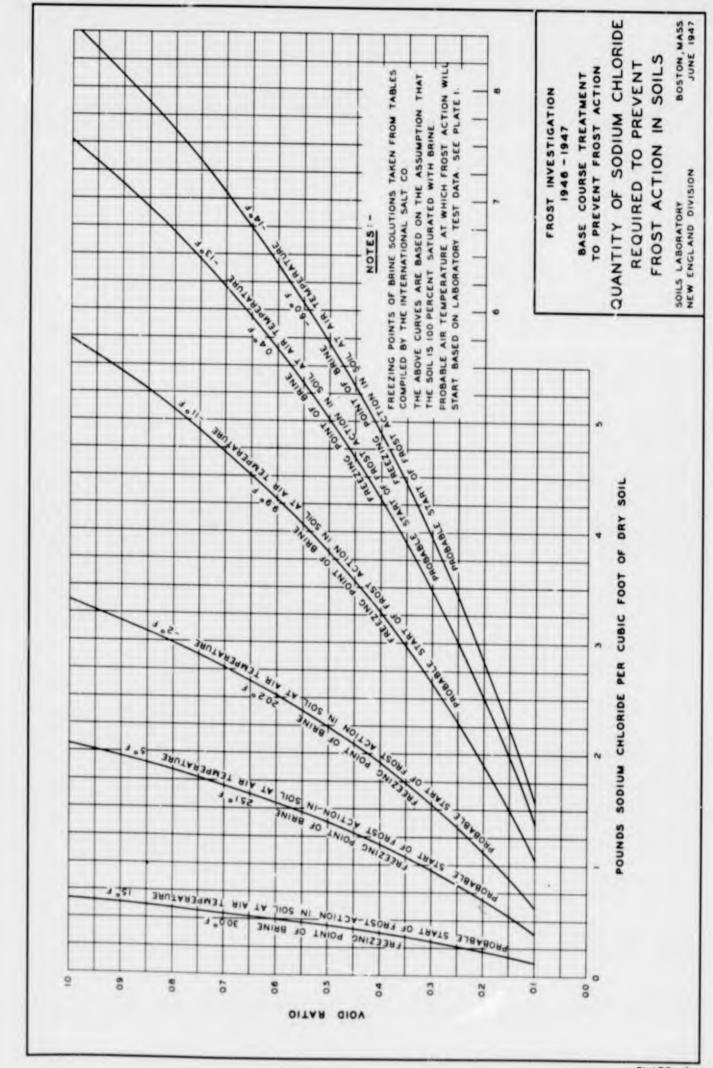
NOTES:- FIGURES BESIDE PLOTTED POINTS ARE SPECIMEN NUMBERS ASSIGNATED THE LETTERS "NF"UNDER PLOTTED POINTS INDICATE THAT NO FITTEMPERATURE TESTED.

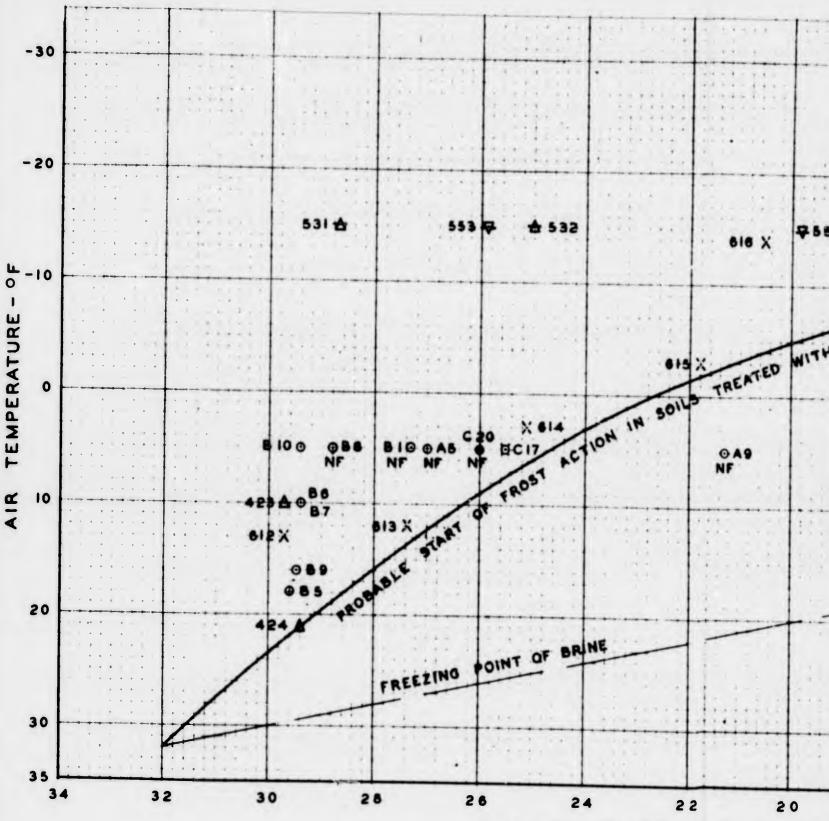


(BASED ON CONCENTRATION OF SODIUM CHLORIDE AND WATER CONTENT OF SOIL)

LOTTED POINTS ARE SPECIMEN NUMBERS ASSIGNED IN THE ORIGINAL INVESTIGATION. "UNDER PLOTTED POINTS INDICATE THAT NO FROST HEAVE OCCURED TO LOWEST STED.

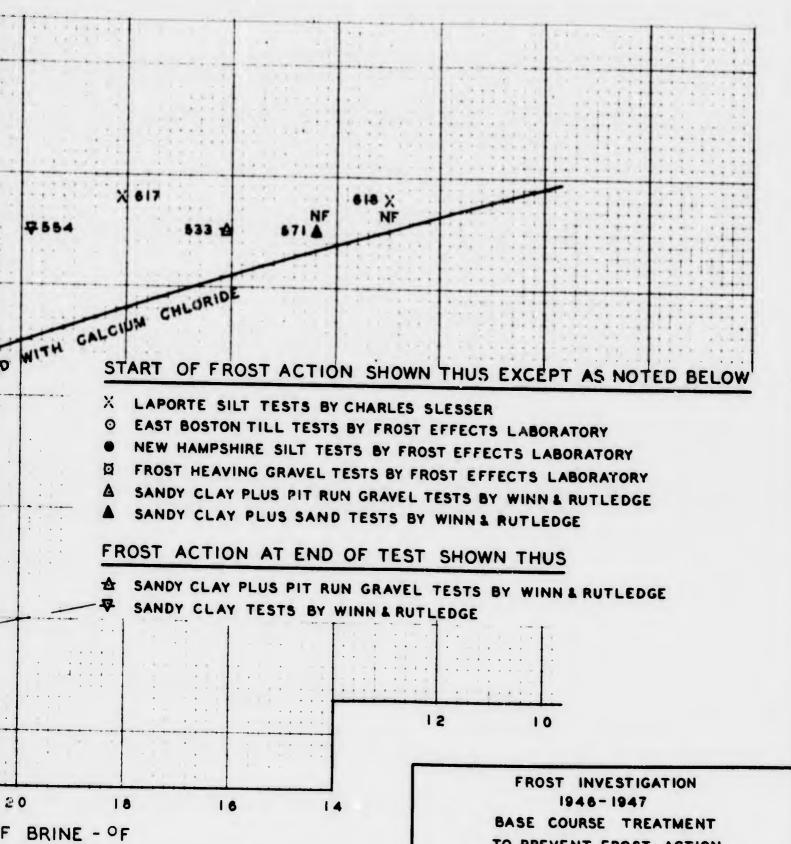






FREEZING TEMPERATURE OF BRI
(BASED ON CONCENTRATION OF CALCIUM CHLORIDE AND

NOTES: - FIGURES BESIDE PLOTTED POINTS ARE SPECIMEN NUMBERS ASSIGTHE LETTERS "NF"UNDER PLOTTED POINTS INDICATE THAT NO FITTEMPERATURE TESTED.



TO PREVENT FROST ACTION

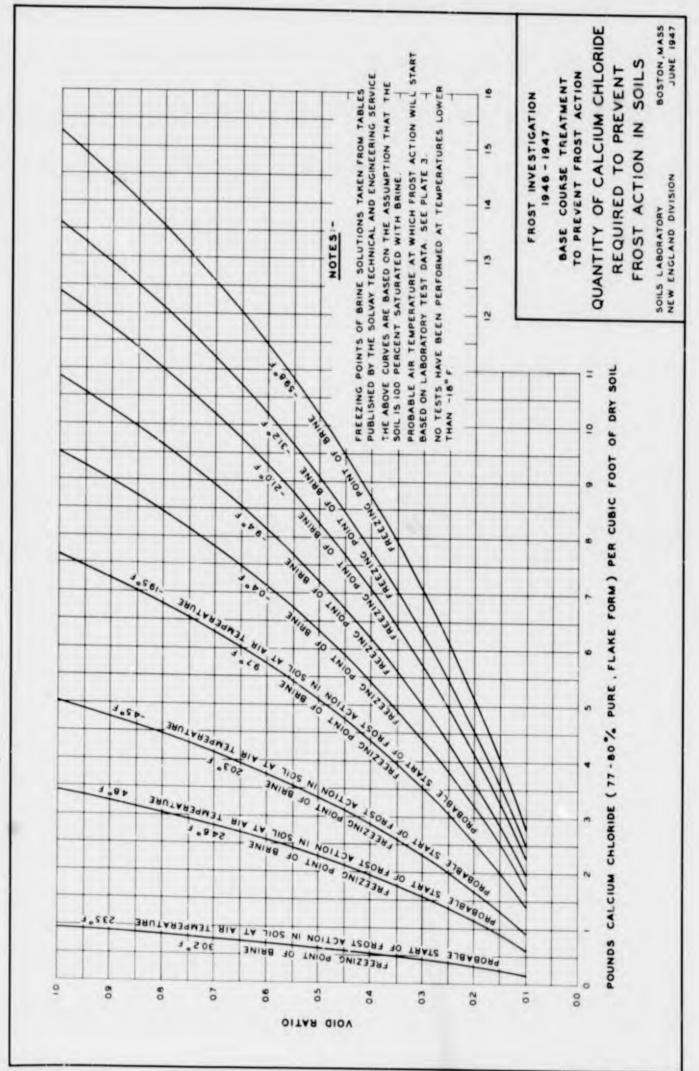
PROBABLE START OF FROST ACTION IN SOILS

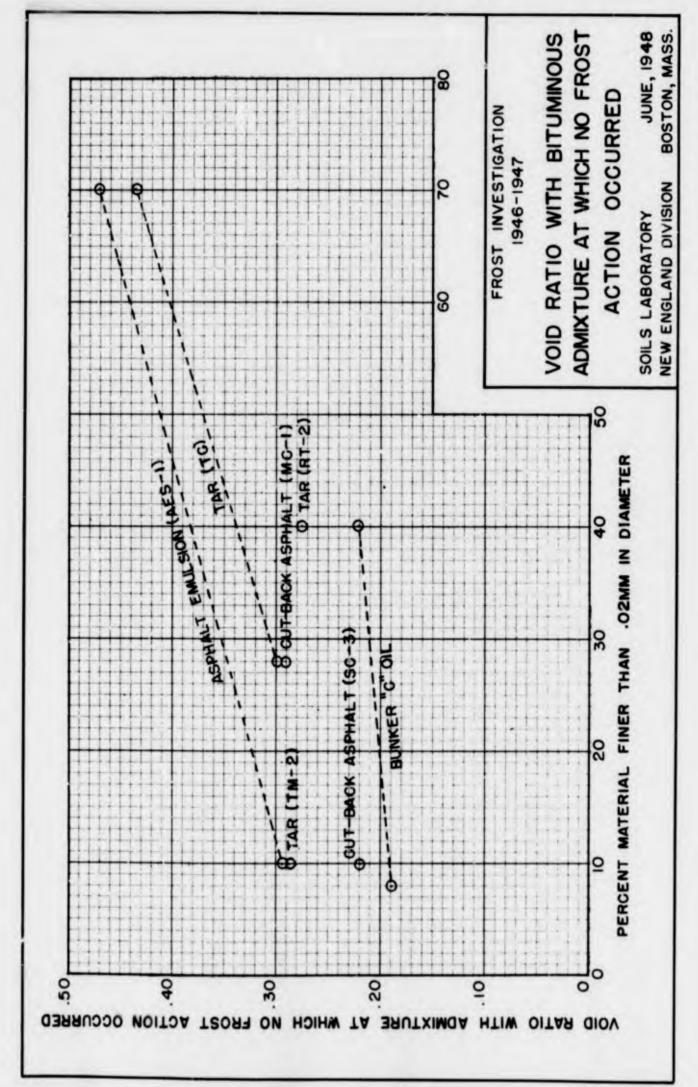
TREATED WITH CALCIUM CHLORIDE

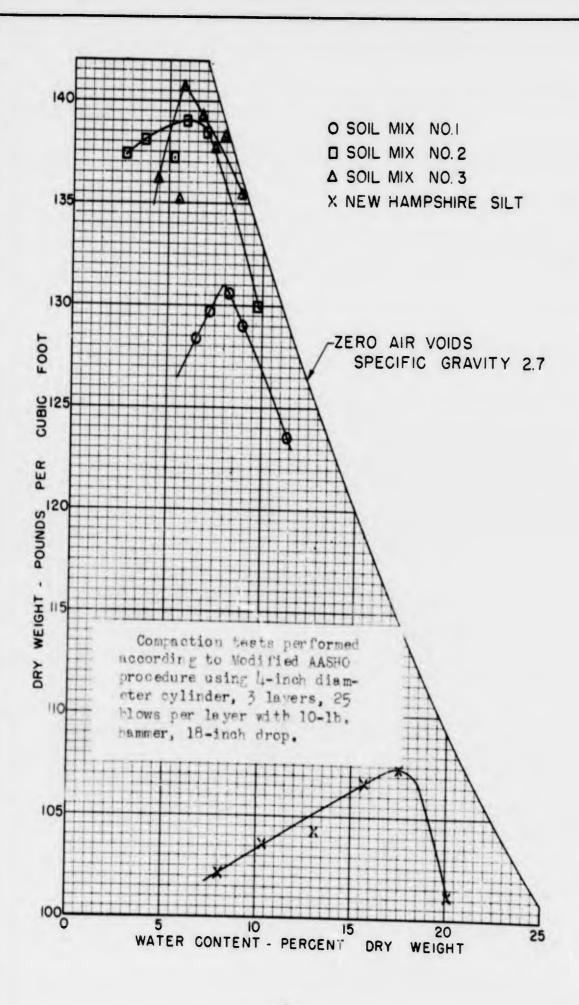
SOILS LABORATORY NEW ENGLAND DIVISION

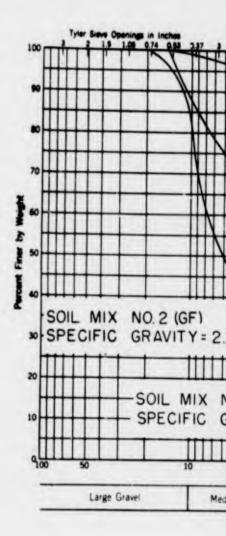
JUNE 1947 BOSTON, MASS

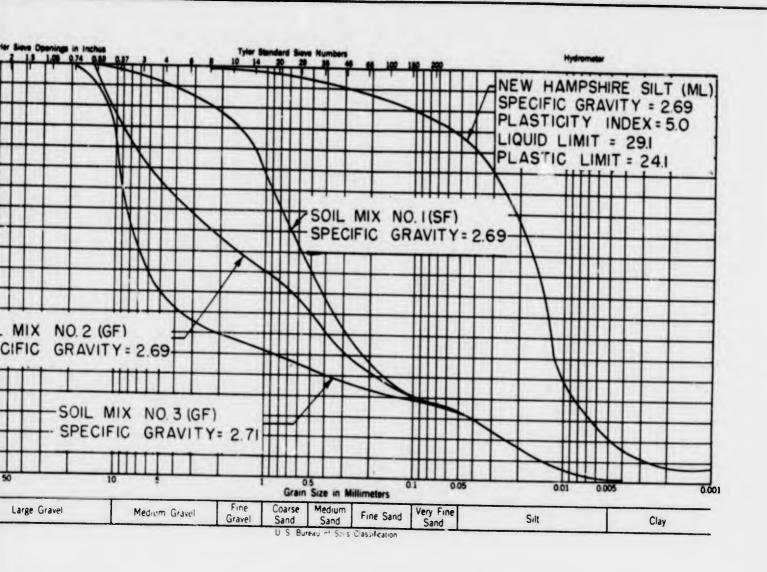
AND WATER CONTENT OF SOIL)











FROST INVESTIGATION

1946-1947

BASE COURSE TREATMENT
TO PREVENT FROST ACTION

SUMMARY OF SOIL TEST DATA

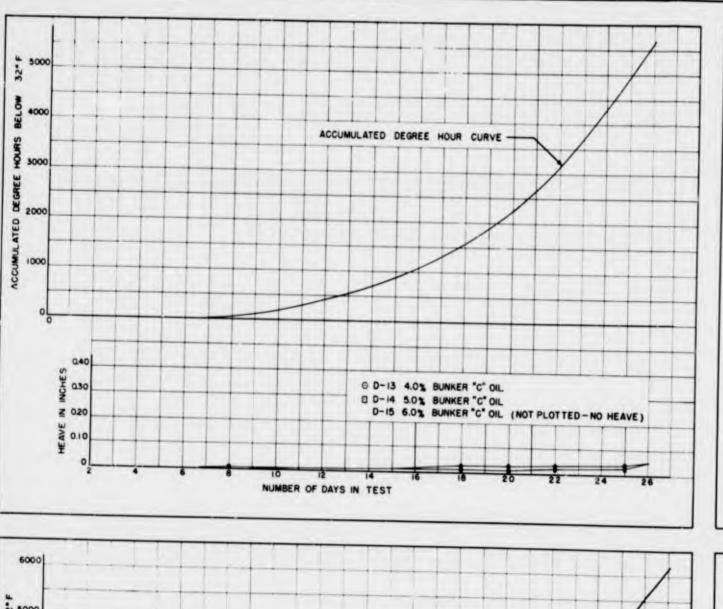
SOILS LABORATORY

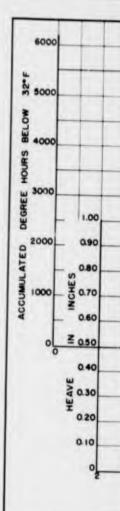
JUNE 1947

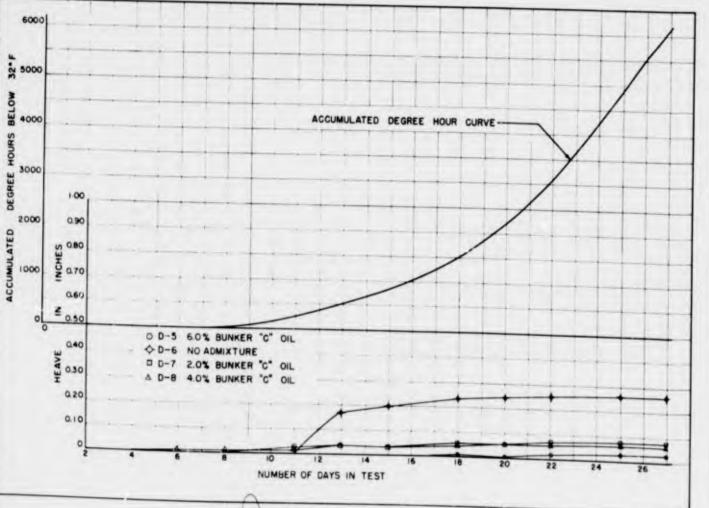
NEW ENGLAND DIVISION

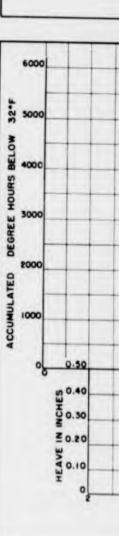
BOSTON , MASS

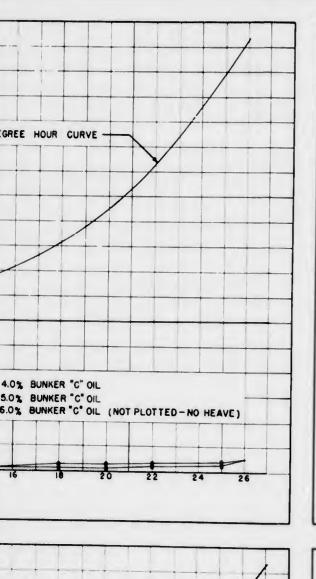


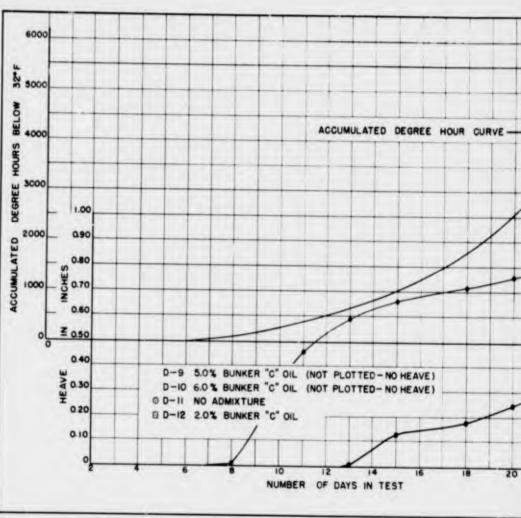


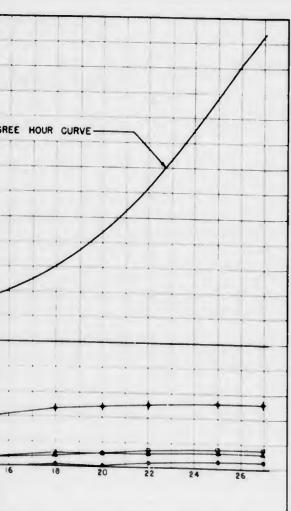


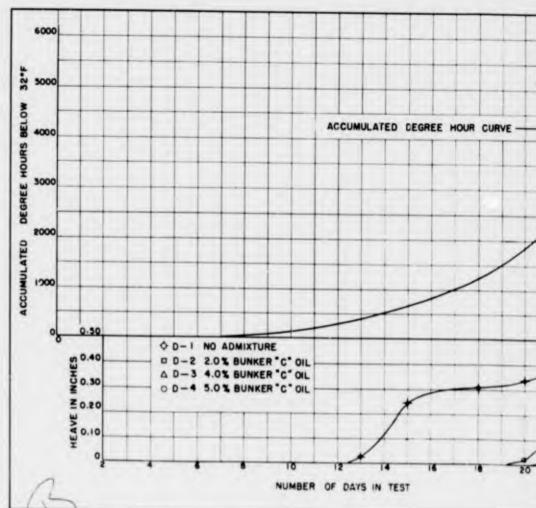


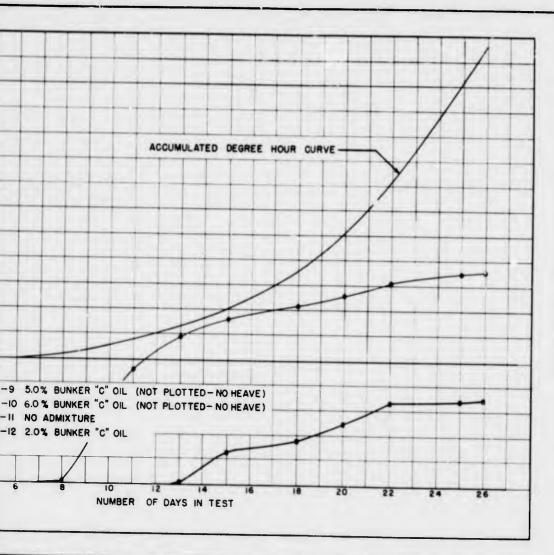


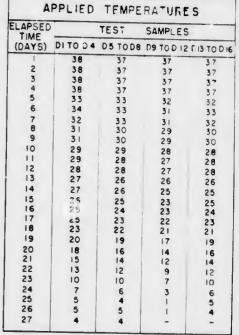




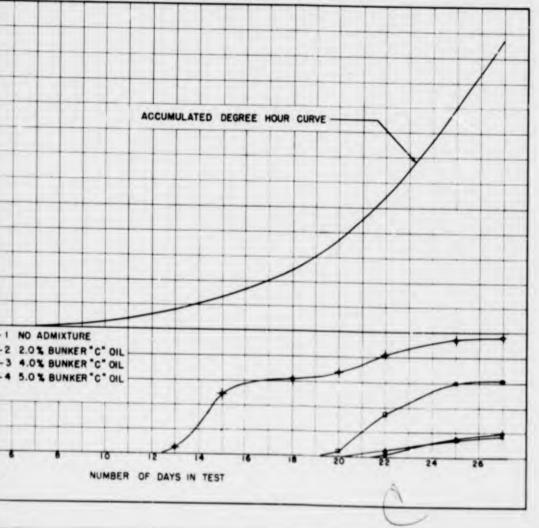








Specimens D-1 through D-5 prepared with Soil Mix No. I. Specimens D-6 through D-10 prepared with Soil Mix No. 2. Specimens D-11 through D-15 prepared with Soil Mix No. 3.



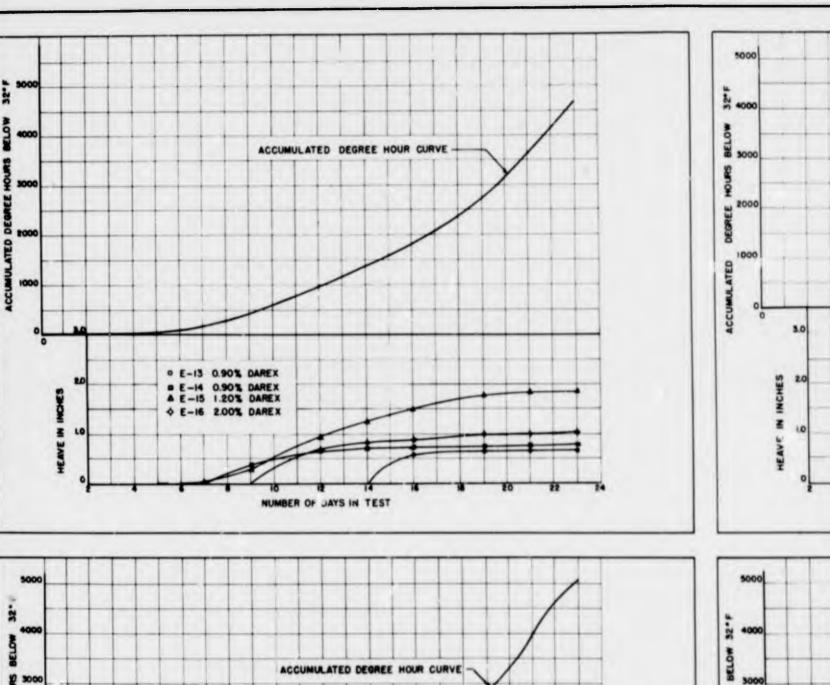
FROST INVESTIGATION 1946 - 1947

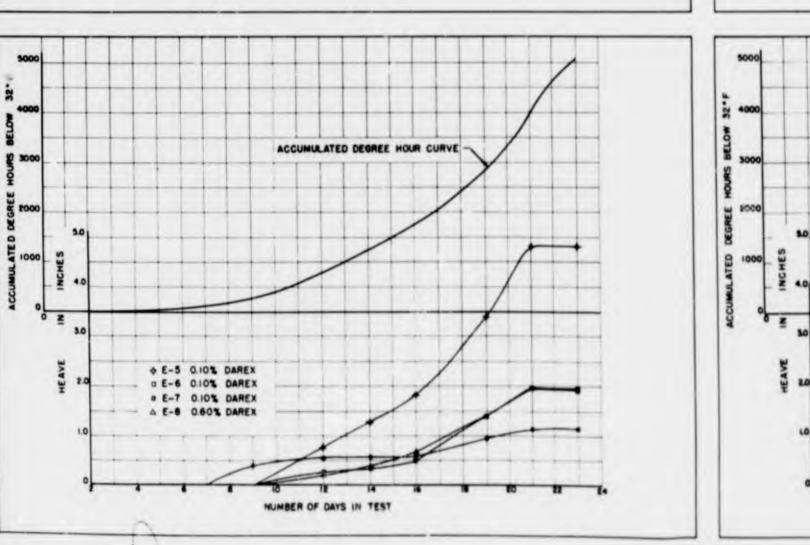
BASE COURSE TREATMENT TO PREVENT FROST ACTION

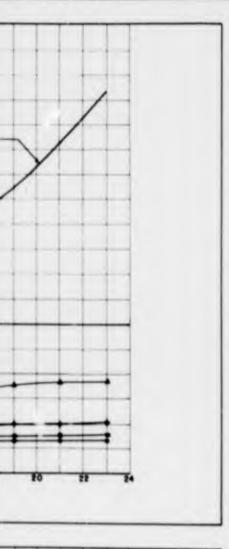
RATE OF HEAVE AND CUMULATIVE TEMPERATURE DIAGRAM SERIES D

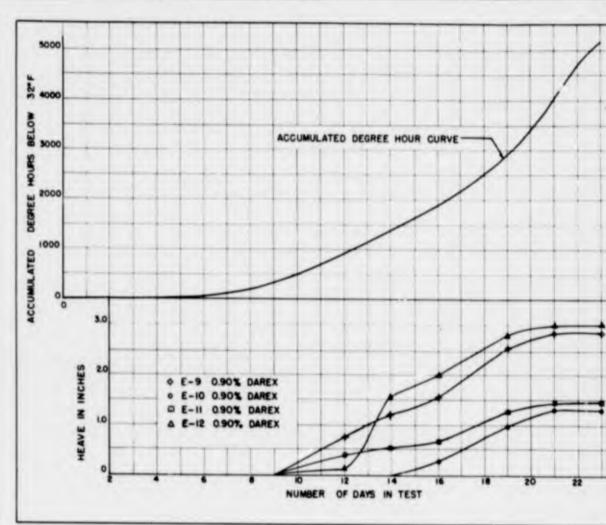
SOILS LABORATORY NEW ENGLAND DIVISION

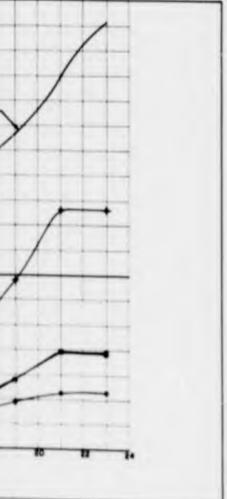
JUNE 1947 BOSTON, MASS.

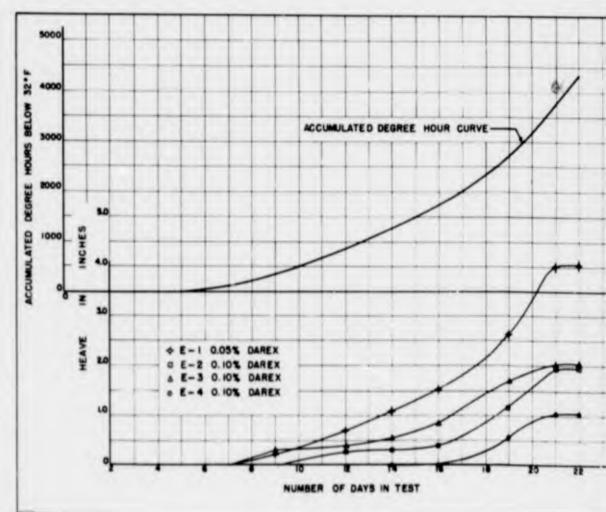


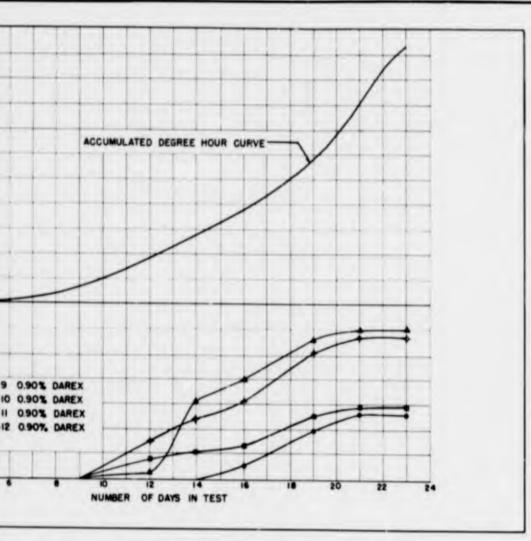






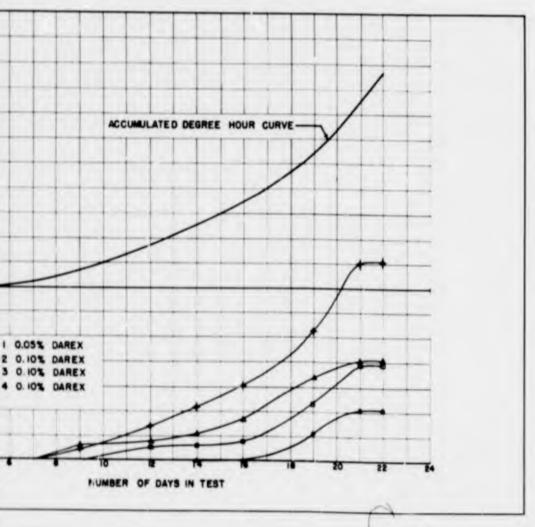






AF	PPLIED	TEMPE	RATURE	S			
ELAPSED		TEST	SAMPLES				
(DAYS)	EI TO E4	E5 TO E8	E9 TO E12	EIS TO EIE			
1	39	39	38	39			
5	34	34	34	34			
3	34	33	34	34			
4	33	32	33	33			
5	31	31	3)	31			
6	30	31	30	30			
7	28	30	29	26			
8	28	29	28	28			
9	27	28	27	26			
10	25	26	25	24			
11	25	24	24	25			
12	24	24	24	25			
13	23	23	22	23			
14	23	23	23	23			
15	23	22	22	23			
16	22	22	22	23			
17	21	20	21	22			
18	18	17	17	19			
19	16	14	15	18			
20	12	11	11	15			
21	6	6	6	11			
22	6	5	5	10			
23	-	10	7	6			

All specimens were prepared with New Hampshire Sitt.

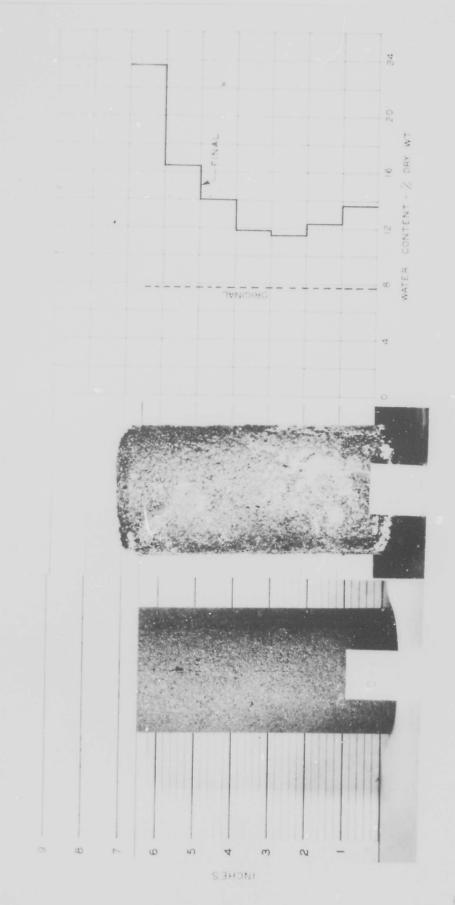


FROST INVESTIGATION 1946-1947

BASE COURSE TREATMENT TO PREVENT FROST ACTION

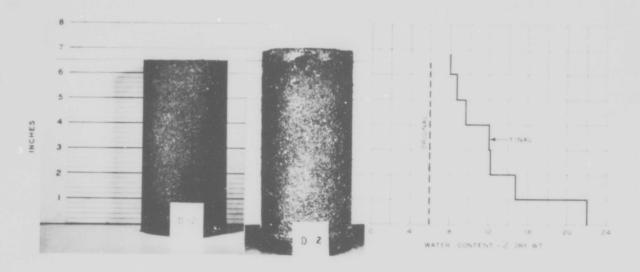
RATE OF HEAVE AND CUMULATIVE TEMPERATURE DIAGRAM SERIES E

SOILS LABORATORY NEW ENGLAND DIVISION JUNE 1947 BOSTON, MASS.



HEAVE AND WATER CONTENT DATA

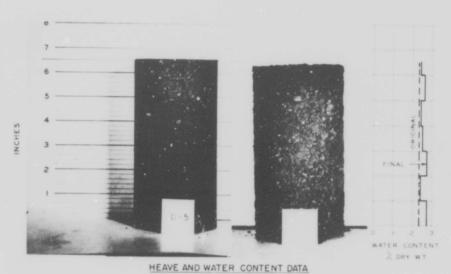
SOIL MIX NO.1 WITH NO ADMIXTURE ORIGINAL DRY WEIGHT 122.9 P.C.F. WATER CONTENT START OF TEST 7.8 PER CENT AND SATURATION 57 PER CENT



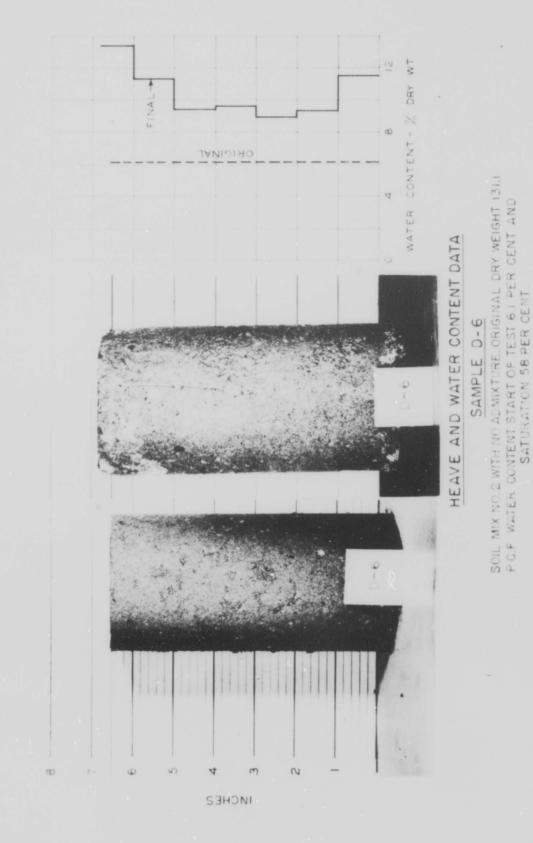
HEAVE AND WATER CONTENT DATA

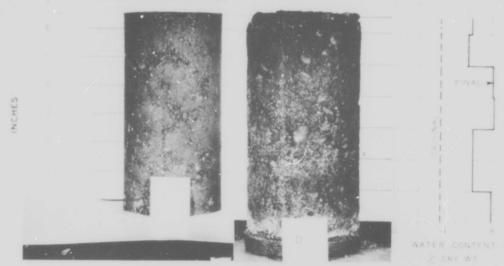
SAMPLE D-2

SOIL MIX NO.1 WITH 2.0 PER CENT BUNKER "C" OIL ORIGINAL DRY
WEIGHT 121.3 P.C.F. WATER CONTENT START OF TEST 6.0 PER CENT
ACCUSATORATION 125-2



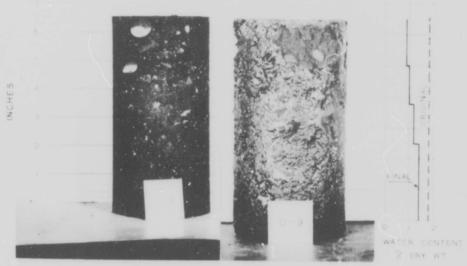
SAMPLE D-5
SOIL MIX NO.1 WITH 6.0 PER CENT BUNKER "C"OIL.ORIGINAL DRY
WEIGHT 120.8 PC F. WATER CONTENT START OF TEST 2 3 PER CENT
AND SATURATION 17 PER CENT.





HEAVE AND WATER CONTENT DATA SAMPLE D-7

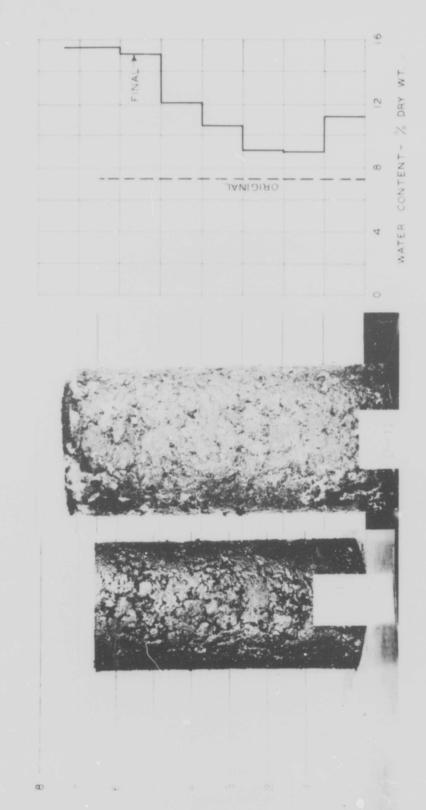
SOIL MIX NO 2 WITH 2 0 PER CENT BUNKER"C"OIL ORIGINAL DRY WEIGHT 126.7 P.C.F. WATER CONTENT START OF TEST 3.9 PER CENT AND SATURATION 3.3 PER CENT



HEAVE AND WATER CONTENT DATA

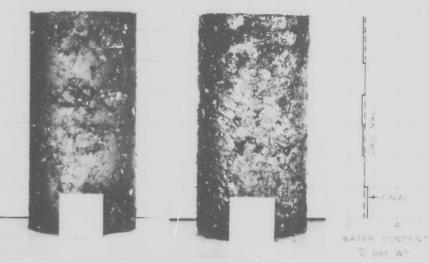
SAMPLE D-9

SOIL MIX NO 2 WITH 5 0 PER CENT BUNKER "C" OIL ORIGINAL DRY
WEIGHT 126.6 P.C.F. WATER CONTENT START OF TEST 18 PER CENT AND SATURATION IS PER CENT.



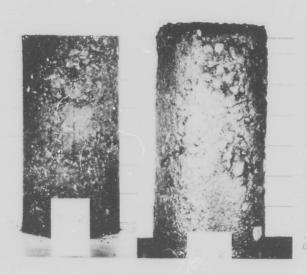
HEAVE AND WATER CONTENT DATA

SOIL MIX NO.3 WITH NO ADMIXTURE, ORIGINAL DRY WEIGHT 134.8 P.C.F. WATER CONTENT START OF TEST 7.3 PER CENT AND SATURATION 77 PER CENT.



HEAVE AND WATER CONTENT DATA SAMPLE D-15

SOIL MIX NO.3 WITH 6 OPER CENT BUNKER "C" OIL ORIGINAL DRY WEIGHT 133.5 P.C.F. WATER CONTENT START OF TEST 1.3 PER CENT AND SATURATION 14 PER CENT

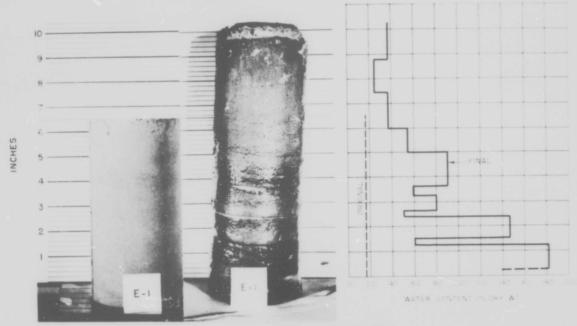


WATER CONTENT - % DRY WT

HEAVE AND WATER CONTENT DATA

SAMPLE D-12

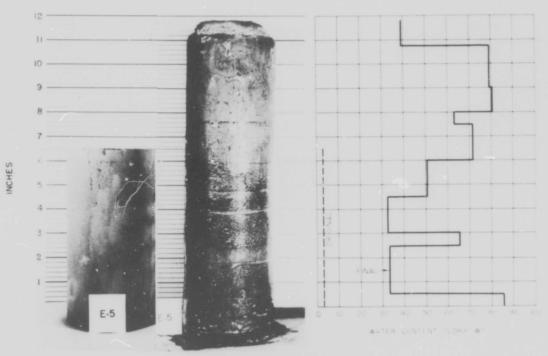
SOIL MIX NO. 3 WITH 2 O PER CENT BUNKER "C" OIL ORIGINAL DRY WEIGHT 126.4 P.C. F. WATER CONTENT START OF TEST 4 O PER CENT AND SATURATION 32 PER CENT.



HEAVE AND WATER CONTENT DATA

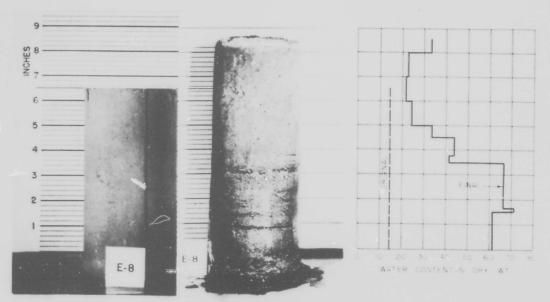
SAMPLE E-I

NEW HAMPSHIRE SILT WITH 0.05 PER CENT DAREX A E A. ORIGINAL DRY
WEIGHT 104-3 PCF WATER CONTENT START OF TEST 16.1 PER CENT
AND SATURATION 71 PER CENT.



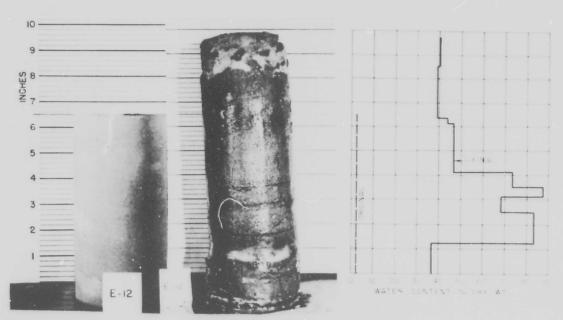
HEAVE AND WATER CONTENT DATA SAMPLE E-5

NEW HAMPSHIRE SILT WITH 0.10 PER CENT DAREX AE A. ORIGINAL DRY WEIGHT 103.0 PC F. WATER CONTENT START OF TEST 3.0 PER CENT AND SATURATION 13 PER CENT.



HEAVE AND WATER CONTENT DATA SAMPLE E-8

NEW HAMPSHIRE SILT WITH 0.60 PER CENT DAREX AE A ORIGINAL DRY WEIGHT 102 5 P.C.F. WATER CONTENT START OF TEST 14.6 PER CENT AND SATURATION 62 PER CENT.



HEAVE AND WATER CONTENT DATA SAMPLE E-12

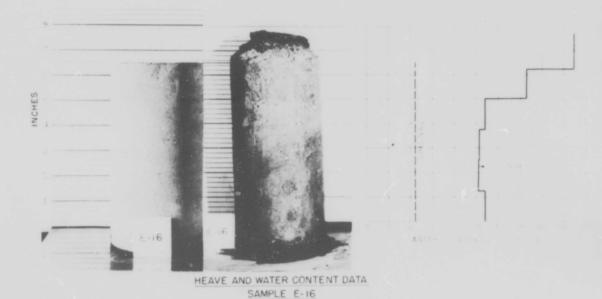
NEW HAMPSHIRE SILT WITH 0.90 PER CENT DAREX AEA. ORIGINAL DRY WEIGHT IOLOPE, F WATER CONTENT START OF TEST 2.4 PER CENT AND SATURATION 10 PER CENT.



HEAVE AND WATER CONTENT DATA

SAMPLE E-15

NEWHAMPSHIRE SILT WITH 120 FEH CENT LIAHEX AEA, ORIGINAL, DRY
WEIGHT IOLZ P.G.F. WATER CONTENT START OF TEST II. 6 FER CENT AND
SATURATION 48 FER CENT



NEW HAMPSHIRE SILT WITH 2 OF PER CENT DAREX A E A ORIGINAL DRY WEIGHT 100 4 P.C.F. WATER CONTENT START OF TEST ILLI PER CENT AND SATURATION 45 PER CENT

2NO2		Entire	Ettire	Betire		None	Beir	Brtire	BALLY			Entire	Estire	Top 2	None	None
FROST		Moderate	Slight	None	None	Serere	-derate	Sheht	\$11,6ht	Some .						
HEAVE	MOCENT	7.4	in.e	14	1.5	70	0**	1.1	6*0	0.0	0*0	13.2	7.	2.0	6.0	0.0
DEGREE HOURS TO START	(AFFROX)	900	1250	1250	1250	0022	1,000	907	907			175	700	095	98	
DEGREE	118*	628	0782	9870	2870	0619	0619	0619	9619	9969	9969	9969	6560	6730	5710	5710
CONTENT N INCH	A1753 TEST \$	15.7	22.0	11.7	6.1	2.7	11.5	8.0	5.1	77	1.0	11.2	13.3	4.1	1.9	3.6
BOTTON	SEFORE TEST \$	7.6	0.0	9.4	0.7	2.3	6.1	3.9	2.7	1,6	9.0	7.5	0.4	3.0	2.0	1.1
	ADMIXTORE		-31c	426	-235	-156		.25%	-165	-162	920		*365	-105	277.	-062
START TEST	ADMIXTURE	3960	386.	786.	907	066	-261	*385	916.	*35c	652	*255	.336	.380	305	2967
	TEST F	1007	06	25	36	310	8:	3	12	30		1007	2	zi.	36	*1
SATURATION	STAKE OF TEST S	25	1.5	333	*	21	9.	33	त्ते	370	5	11	24	×	36	.46.
	GALE DURING TEST	7.0	9.0	7.5	1.0	2*0	5.4	3.3	5.0	9*0-	0*0	1.4	6.0	-0.2	200	00
MATER CONTEST	START OF TEST	7.6	0.0	9.7	0.4	2.3	3	3.6	2.7	2	*0	7.3	la.c	3.0	2.0	
RA:	A TEST	34.41	12.¢	0.0	5.6	5.5	10.4	7.2	2.0	1.2	9.0	12.0	10.0	2	3.6	
LENGTS OF PEST	E Na	22	12	ži.	2	22	12	2.2	12	n	92	8	92	*	8	1
	ERFORE TEST TEST	122.6	121.3	121.0	1354	120.5	131-1	126.7	127.5	128.46	155.3	135.0	128.4	128.3	129.4	
THESE	SETORE TREE LESACE	130.5	15151	131.6	130.3	131.0	139.0	18.2	136.4	195.5	40%	Tillaco.	136.1	157.2	139.7	
FECURE CO.	ŧ	0	**	4		*	0		4	6			Eu.	4	*	
1108		MIR 1.	1 177	1 XX	1 XIN	1 104	M1 2.	2 118	2 178	M27 2	2 228	*C 10*	E 278	E EL	. 10	,
SANTE		X	X	Z	1	x	X	Z	X	x	200	7	7	7	7	-

(Bureau of Seile elessification). (a) Langth of test computed from time sample was placed in drawer until it was removed.

• Max 1 securities of approximately 175 silt, 565 and, 256 gravel (bursan of Soils oless Max 2 consists of approximately 175 silt, 355 and, 566 gravel (* Mrs 1 sectiate of approximately 175 silt, 585 Mrs 2 consists of approximately 175 silt, 595 Mrs 3 coosists of approximately 175 silt, 195

PLATE A-39

SCHOOLS OF FACTOR ACTION TEST DATA. SERIES E.

	FROZEN		Sptire																Bottom 2"
	FROST	ACTION																	
	HEAVE	PIRCES!	624	30.5	31.7	16.0	74.1	30.5	17.6	8	457	50.9	5.23	16.2	13.5	1.00	0.98	200	161.5
	DEGREE DEGREE NOUNS SECONS TO STAFF IN (APPROX.)		150	950	150	17.0	28.0	62	153	280	350	1360	350	350	1570	350	150	163	t23
			2177	1775	W.12	2777	206	300	200	705	217	51.77	2117	2177	4,54	1644	distr.	Less.	75.75
	CONTENT	ATTER TEST		135.6	35.2	15	65.0	65.3	6.50	61.5	4.00	41.5	307	30.5	26.1	27.	25.25	27.0	8;
	NATES CONTENT SOTTON LAYER		16.1	24.6	M.5	5.9	3.0	1.6	0.5	1	12.9	15.01	1.0	2.4	1.1	5.0	11.6	11.1	13
smr.	108	END CF FEST S	1004	100/	1cox	feet	1004	1004	130	1334	¥601	1004	1304	120	130	130	1004	1304	* cot
3518548	SATURATION	START OF TEST S	r.	25	22	38	139	2	ć.	50	34	53	8	30	4	0	77	11.5	55
TESTS UNITED TAKEN AND IN NEW SOMESHIPE SILLY	PUID BATIC	START TEST	0,63	69.0	12.0	64.0	69*0	69*0	0.0	60.0	99*0	O.c.s.	5963	3405	94.0	62.0	99*0	0.65	0*60
CREAT DATES	NO. DAYS		None	None	None		14	16	7	None	Kene	None	1	60	W.	7	Your	None	None
12575		CATS DURING TENT	57.3	33.4	59.1	25.00	2.2	707	28.00	33.7	20.0	22.03	75.	43.44	30.00	30.00	27.0	20.6	69.1
	RATES COLTEST	P CEL	452	0.37	27.6	31.3	57.2	0.37	30.1	6.47	54	37.0	28.5	15.51	27.73	31.1	**	31.6	100.5
	*****	STAFE OF	14.1	The c	24.5	0.00	3.0	3.6	5.0	N. A.	12.9	12.7	0.1	7.2	7	6.0	11.05	11.11	13-4
		ST N	16.1	3Pmg	24.5	24.5	The.7	5.07	24.0	The contract	12.9	12.1	13.2	13.5	12.44	13.2	11.6	11.11	13-4
	LEASTER OF	ng(S)	82	8	8	8	12	23	20	10	23	12	10	52	23	53	2	23	23
		TEST TEST TEST	104.3	100.0	0.35	101.9	105.0	100.0	28	13.5	100.7	101.7	120.7	101.5	1014	9F.0	101.2	1004	101
	7112430	BEFORE TEST	121.1	310.0	1124	116.7	118.3	118.0	9.611	118.5	317.05	115.5	124.09	113.5	311.5	34140	114.2	113.6	3115.06
	PECENT.	-	80.	-10	970	370	•10	cr.	.10	034	66.	-50	05*	96*	650	950	1.20	2.00	None
	M		1-4	Z	1	i	8	ĭ	7	Z	I	8-10	11-4	7	8-13	F-1.	8-15	8-16	8-13
	_		_	_	_	_	_	_											

(A) Length of test computed from time sample was placed in drawer until it was removed.

- As leternined from oven dry weight. No correction applied for evaporation of Darex AEA.

NEW ENGLAND DIVISION CORFS OF ENGINEERS, U. S. ARMY BOSTON, MASSACHUSETTS

DATA REPORT OF FROST INVESTIGATIONS FISCAL YEARS 1943 - 1949

APPENDIX B

REPORT ON LABORATORY TESTS ON FROST PENETRATION AND THERMAL CONDUCTIVITY OF COHESIONLESS SOILS
1944 - 1946

FROST EFFECTS LABORATORY

JUNE 1949

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Studies of the Temperature Required to Freeze Scil Moisture

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2-06

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APPENDIX B

REPORT ON

LABORATORY TESTS ON FROST PENETRATION AND THERMAL CONDUCTIVITY OF COHESIONLESS SOILS

SYNOFSIS

and subgrade materials beneath pavements and the rate and depth of frost penetration must consider the thermal properties of frozen and unfrozen soils. The principal soil thermal properties influencing the rate of freezing are thermal conductivity, volumetric heat, latent heat of soil moisture, and freezing temperature of soil moisture. To supplement the extensive program of field investigations and theoretical and mathematical studies the laboratory study reported herein was initiated. Laboratory tests for frost penetration and thermal conductivity were conducted and previous investigations of thermal properties of soils were reviewed and summarized.

REPORT ON

LABORATORY TESTS OF FROST PENETRATION AND THERMAL CONDUCTIVITY OF COHESIONLESS SOILS

I. INTRODUCTION

- laboratory frost penetration and thermal conductivity tests performed by the Frost Effects Laboratory and a review of tests conducted by other investigators dealing with the thermal properties of soils. These studies were part of the authorized Frost Investigation program for Fiscal Year 1944-1945 and Fiscal Year 1945-1946.
- 1-02. Purpose. The specific purposes of this investigation are as follows:
- a. Determine the amount and rate of temperature changes within cylinderical laboratory specimens of sand due to a suddenly applied freezing air temperature at the top of the specimens.
- b. Determine the thermal conductivity of several representative base course materials and one bituminous concrete mixture.
- c. Review and summarize the results of previous investigations of the thornal properties of soil.
- 1-03. Scope. This report presents a summary of all the data obtained in the tests and investigations to accomplish the stated purpose. The laboratory tests were in a specially

constructed cold room with controlled temperature conditions.

The review of previous investigations consisted of library research.

1-04. Definitions.

- a. Frost Action is the accumulation of water in the form of ice lenses in the soil or base materials under natural freezing conditions.
- b. Non-Frost Susceptible Materials are crushed rock, sand, sand and gravel, gravel, slag, cinders or any other cohesionless material in which frost action is not possible.
- c. Density is the unit dry weight in pounds per cubic foot.
- d. Water Content is the ratio expressed as a percentage of the weight of water in a given soil to the weight of solid particles.
- e. Specific Heat, c, of any substance is defined as the Btu's of heat required to raise one pound one degree Fahrenheit.
- f. Volumetric Heat Capacity, C, of a soil is defined as the heat in Btu's required to raise one cubic foot of soil and moisture contained therein one degree Fahrenheit.
- g. British Thermal Unit, Btu, is the heat required to raise the temperature of one pound of water one degree Fahrenheit at its maximum density. It is equal to 252 calories.
- h. Latent Heat of Fusion, L, is the quantity of heat required to change a unit mass of ice to water with no temperature change.

- i. Thermal Conductivity, k, is the quantity of heat that will pass through a unit area of unit thickness in unit time under a unit temperature gradient.
- j. Degree of Saturation, G, is the ratio, expressed as a percentage, of the volume of water in a given soil mass to the total volume of voids.
- k. Temperature Gradient, S, is the rate of change of temperature between two points.
- conducted in the Soils Mechanics Laboratory, Harvard University.

 The use of the facilities of the Soil Mechanics Laboratory including the cold room was made available by Harvard University through the cooperation of Dr. Arthur Casagrande. Dr. Casagrande made frequent suggestions and assisted greatly in these tests by giving freely of his own time for consultation. In addition, members of the Staff of the Graduate School of Engineering furnished the temperature measuring equipment and valuable advice on its use. Personnel of the U. S. Engineer Office, Providence, R.I., assomplished, through a cooperative agreement, the theoretical mathematical studies utilized in the analysis of results and furnished valuable advice on test apparatus for determining the coefficient of thermal conductivity.

II. INVESTIGATION AND STUDIES

2-01. Description of Laboratory Cold Room and Equipment.

The thermal conductivity and frost penetration investigations

were carried out in the cold room laboratory, at Harvard
University Graduate School of Engineering. General layout of
the cold room and equipment is shown on Flate B-1.

erator with inside dimensions 6 feet 9-1/2 inches long by 6 feet 9-1/2 inches wide and 6 feet 5 inches high. It is insulated on all 6 sides with 4 inches of cork. A pressure controlled unit cold blower and an externally located freon compressor cool the the room to any desired air temperature within a range of 30° to 50°F and to an accuracy of 1.0°F.

b. Freezing Cabinet. Within the cold room is a freezing cabinet located as shown on Plate B-1. A photograph of the freezing cabinet is shown on Plate B-2. This cabinet contains an air space at the top cooled by longitudinal coils, hung from the top of the cabinet, using a second compressor with sulphur dioxide refrigerant. Beneath this air space are 4 drawers arranged side by side as shown on photograph, Plate B-2. The temperature at the top of each drawer may be fixed at any desired temperature with an accuracy of 0.8°F and equal to or less than that in the cold room by adjustment of a bimetallic De-Khotinsky type temperature control located in the air space above the drawers. With the cold room at approximately 42°F. the top of the freezing cabinet can be lowered to a temperature of -10°F. A small fan in the air space at the top aids in maintaining a uniform temperature throughout the air space. The bottom of the drawers consist of a 24 gage galvanized iron sample tray and is set 15 inches above the iloor of the cold room.

Detail of the trays is shown on Plate B-1. The temperature of the bottom of the drawers is determined by the cold room temperature. Thus, test specimens may be placed in the drawers and subjected to any desired temperatures at the top and bottom within the temperature ranges of the two pieces of cooling equipment. Circulation of air between the air spaces at the top and bottom of drawers was prevented by inflating a tire inner tube installed in the air space between drawers until this space was sealed.

Two of the drawers are equipped in such a manner that water may be supplied to the bottom of the test specimens.

- ments in test specimens were made using 72 copper-constantan thermocouples and a Leeds and Northrop Potentiometer, type K. The thermocouples were arranged in groups of twelve. A switching arrangement permitted the rapid measurement of the temperature at individual thermocouples successively as desired. The constant temperature junction consisted of an ice water bath in a thermos bottle. The accuracy of a temperature determination was $\not= 0.1^{\circ}$ F. Flate B-3 illustrates the switches and temperature indicating apparatus.
 - 2-02. Experiments to Measure Temperature Changes in Test Specimens.
- a. General. A series of tests were conducted, each series consisting of tests on 12 test specimens compacted at various densities and water contents.

- b. Material Tested. The material tested consisted of a cohesionless, siliceous, medium sand obtained from a glacial outwash deposit at South Lowell, Mass. The grain size and specific gravity of the material (Lowell sand) as used for tests, after thorough mixing and removal of sizes larger than 2 mm and smaller than 0.07 mm, are shown on Figure 5, Flate 2-4. This material is considered not susceptible to frost action and contains particle sizes that were very small in proportion to the size of test specimen.
- c. Test Specimen. Each test specimen was a cylinder 3.36 inches in diameter and 6.5 inches high contained in a quart cardboard container. To measure temperature changes, several thermocouples were placed along the axis of the cylinder. Details of sample cylinder and location of thermocouples are shown on Plate B-1. From 2 to 4 specimens were placed in each drawer of the cold cabinet. The top of each specimen, except those which were saturated, was sealed with paraffin about 2 mm in thickness to prevent evaporation from the surface. The space between specimens and the sides of the drawer was filled from the bottom of the drawer to top of specimens with the same sand as the specimens, placed dry.
- d. Test Conditions. Each test consisted in permitting the 12 specimens to reach cold room temperature of approximately 40°F and then suddenly applying at the top of the specimen a predetermined freezing temperature while maintaining the bottom of the specimen at cold room temperature and recording the temperature

changes in the specimen vs. time until equilibrium was established.

e. Sample Proparation. The quart cardboard container, with top and bottom removed, forming the sides of a test specimen, was placed into a split forming jacket, illustrated on Plate B-5. The split forming jacket had holes through which thermocouples could be inserted into the center of the specimen during coapaction. Placement and compaction of the material in a dense state was accomplished by using a device (shown on the left, Flate B-5) and procedure developed by Dr. Yen Chan in connection with his work on a thesis for the degree of Doctor of Engineering at Harvard University. For the dense state, material having the desired water content was placed in ten equal layers and each layer compacted by an increasing number of blows in a sequence which resulted in a uniform degree of compaction throughout the specimen. The compacting device consisted of an anvil and a falling hammer guided by a rod running through a hole in the center of the weight. The anvil was placed on the soil to be compacted and the hamser dropped on it from a predetermined height. Thermocouples were inserted at regular intervals along the longitudinal axis of the samples as the placement progressed. After removal from forming jacket the thermocouple lead wires were cemented with paraffin to the cardboard container. Loose samples were constructed by first comenting the thermocouples at the desired positions, then carefully placing material having the desired water content around the thermocouples. At the same time the side of the container was lightly tapped to obtain a

small amount of compaction to minimize consolidation during handling and testing. To obtain saturated samples in either loose or dense state, the material was placed dry in containers which were equipped with screen bottoms. These specimens were then placed in the drawers, fitted with pans for water supply, and saturated by forcing water upward through the specimens until free water appeared on the surface.

f. Test Procedure. Four different tests were conducted with the same specimens, that is, the same specimens were tested under four different freezing temperatures, namely, approximately 30°F, 25°F, 20°F, and 10°F. In the application of the different freezing temperatures to the top of the test specimens, a time lag to reach the desired temperature was evident. The time lag for the 30°F, 25°F, and the 20°F tests was small. For the 30°F test the loaded drawers were left open until the compressor had cooled the coils of the freezing cabinet. The drawers were then closed and the time of closing was considered as the starting time of the test. The loaded drawers were closed and seuled in the 20°F and the 25°F tests and the freezing comprossor was started and time vs. cabinet air temperature was observed until the test temperature was reached. For these tests the starting time was assumed to be the time at which the cabinet temperature had reached the average between the test temperature and the cold room temperature. Cork insulation was placed over the top of the samples in the 10°F test, the drawers closed and the cabinet temperature lowered to 10°F. The drawers were opened

momentarily, the cork sheet was then removed, the drawers closed and the cabinet sealed. The starting time for the test was taken as the time of the final closing of the drawers.

g. Data Obtained. The data obtained from the four series of tests are summarized on Plate R-4. Table A summarizes the principal test conditions for each test performed. Figure 1 is a typical set of time vs. temperature curves for each of the thermoccuples in a selected test. Figure 2 is a typical set of temperature gradients at selected times after suddenly applying a surface temperature. Figure 3 presents representative data showing the penetration of the 32°F temperature vs. time into test specimens at different water contents and unit dry weights and with two different, suddenly impressed, surface temperatures. For 4 tests, conditions were such that equilibrium was reached with the 32°F temperature approximately at the midpoint of the specimen, and equilibrium temperature gradients for both the frozen and the unfrozen portion of the samples for these 4 tests are shown in Figure 4, Plate B-1.

h. Analysis of Test Results. From the tests described it is possible to investigate the effect of the surface boundary upon temperature conditions in the test specimens. The temperature gradients at equilibrium were plotted for all tests, similar to the typical equilibrium temperature gradients shown on Figure 4, Plate B-4. These temperature gradients were then extrapolated to the top and bottom surface of the samples. The specimen temperatures at the top and bottom were then determined

and are recorded on Table A, Plate B-4. The difference between the temperature of the specimen at the top or bottom and the air temperature at the top or bottom respectively is termed the "boundary temperature difference". The equilibrium temperature gradients of each specimen, expressed in degrees F per foot, have been computed and are recorded on Table A, Plate B-4. Plates B-6, B-7, and B-8 are plots of equilibrium temperature gradients vs. boundary temperature differences for the 3 different water contents tested. The scattering of the test data is rather wide, however, it will be noted that small increases in the equilibrium temperature gradient produce substantial increases in the boundary temperature difference for all water contents and in general, the greater the water content the greater the boundary temperature difference for a given temperature gradient. Field observations indicate that equilibrium temperature gradients at one foot depth below the ground surface as large as 6°F. per foot may be expected at Bedford, Massachusetts, and 10°F. per foot at Fresque Isle, Maine. For gradients of these magnitudes, the boundary temperature difference is approximately 1°F. and 2°F. respectively as indicated by the test results on specimens at 2.8 per cent water content (Plate B-7). It is interesting to note on Plates B-6, B-7, and B-8 that the boundary temperature difference at the bottom of the specimens is approximately the same as the boundary temperature difference at top of the specimens even though the water content and the condition of the top and bottom of the specimens varied.

The importance of the evaluation of the boundary temperature difference lies in its relation to the prediction of frost penetration. Knowledge of the magnitude of the boundary temperature difference may permit refinements in theoretical methods for frost prediction and a more exact analysis of observed frost penetrations.

A study of Table A, Plate B-4 indicates that the time for temperature equilibrium to be reached within a given test specimen is dependent upon the magnitude of the temperature differential between top and bottom of the specimen and the density and water content of the specimen. This is as expected for the density and water content of the specimens influence the thermal properties of the material. Specimens at very low water content reached equilibrium temperature quickly because of the small latent heat of fusion and volumetric heat capacity. Saturated specimens reached equilibrium more slowly because of the greater latent heat of fusion and volumetric heat capacity. The same results are illustrated by Figure 3, Plate B-4 in which the rate of penetration of the 32°F temperature is a function of the magnitude of temperature difference between top and bottom of a specimen, and its density and water content.

From Figure 4, Plate 3-4 theratios of the thermal conductivity in the frozen to the unfrozen state can be determined. It may be shown that this ratio is equal to the ratio of the equilibrium temperature gradient in the unfrozen zone

to that in the frozen zone since the quantity of heat passing through the sample at equilibrium is equal to $k_1 \times s_1$ and also equal to $k_2 \times s_2$. These ratios have been determined for the four tests plotted and at the density and water contents of the materials tested the coefficient of thermal conductivity in the unfrozen state varied from 52 to 85 per cent of that in the frozen state.

- 2-03. Experiments to Measure Thermal Conductivity in Unfrozen and Frozen State.
- a. General. Five different base course materials were tested for thermal conductivity in the unfrozen state. Each material was tested at maximum density except one which was tested at several densities between maximum and minimum. Tests were performed on specimens at water contents ranging from almost dry to a saturated condition. The same materials were also tested for thermal conductivity in a frozen condition.
- b. Materials Tested. Thermal conductivity tests were performed on the following materials:
 - (1) Sand (Lowell Sand) consisted of a cohesion-less, siliceous sand from a glacial outwash deposit at South Lowell, Mass.
 - (2) <u>Crushed Rock</u> (Winchester Crushed Rock)

 consisted of a fine grained quartz dicrite

 obtained from quarry at Winchester, Mass.
 - (3) Slag (Mystic Slag) consisted of basic residue from blast furnace located at Everett, Mass.

- (li) Cinders (Somerville Cinders) consisted of commercial grade cinders obtained locally as a result of burning bituminous coal.
- (5) Sand and Gravel (Bangor Sand and Gravel)
 consisted of a well graded sand and gravel
 of glacial origin obtained from Bangor, Maine.
- (6) Blended Bituminous Concrete Aggregates consisted of locally processed aggregates of sand and partially crushed gravel obtained from bins of commercial plant at Cambridge,
- (7) Asphaltic Bituminous Concrete consisted of the blended bituminous concrete aggregate and 4.5% bitumen.
- c. Test Specimen. Each test specimen was of cylindrical shape, 5.36 inches in diameter and 10.67 inches in height, and was contained in a bruss cylinder of 1/16 inch wall thickness. Brass was used because of its high thermal conductivity in comparison with soil. A thermocouple was placed at the exact midpoint of the cylinder as shown on Plate 3-1. Cylinder ends were 1/16 inch brass and were sealed to prevent leakage and changes in water content of specimen during tests. Plate 3-9 shows photograph of brass cylinder and cover.
- d. Sample Preparation. The selected test material was placed in approximately five equal layers and compacted using the device and procedure described in paragraph 2-02 e. Care

was taken not to injure or displace the thermocouple during compaction. A template was used to center the thermocouple accurately in the cylinder. After the cylinder was full the cover was put on and sealed first with glyptal, then paraffin.

e. Test Conditions.

- of subjecting the cylindrical test specimen immersed in a bath, located outside the cold room, to a constant temperature of approximately 75°F. The specimen was then suddenly immersed in a second bath, located in the cold room, to a constant temperature of approximately 40°F. The resulting temperature changes were measured at the midpoint of the specimen. Baths consisted of circulating water maintained at constant temperature by addition of hot or cold water or ice as required. Plate B-1 shows details of constant temperature bath and Plate B-10 shows photograph of bath with test specimens immersed.
- subjecting the test cylinder to a constant freezing temperature of approximately minus 4 degrees F. inside the freezing cabinet until temperature equilibrium was reached and then immersing it into a brine both in the cold room at a constant temperature of approximately 27 degrees F. The bath consisted of circulating brine maintained at constant temperature by the addition of either hot water or dry ice as required. The resulting temperature change was measured at the midpoint of the specimen until temperature equilibrium was reached.

- f. Method of Computing Thermal Conductivity. The equation used for computing the thermal conductivity from the data obtained together with the nomenclature are contained on Plate B-11. The equation, designated (b) was derived for the following assumptions:
 - (1) The temperature of the exterior boundary of the soil is equal to the temperature of the liquid bath into which the container is immersed.
 - (2) The range of temperature change during any specific test was either above or below the freezing point of water, hence latent heat of fusion was not a factor.
- test data the thermal conductivity, k, may be computed if the volumetric heat capacity is known. It may be assumed that the volumetric heat capacity can be computed for a given soil using equation (a), Plate 3-11. This equation is based upon the assumption that the volumetric heat capacity is equal to the sum of the volumetric heat capacities of the dry soil and of the water present in the soil. The value for the specific heat, c, of dry soil is a variable depending upon the mineral and chemical constitutents of the soil. Reference to the tabulations of specific heat for various minerals, rocks, and dry soils based upon tests by various investigators will show that the specific heat of dry soil, minerals, and rocks varies within narrow

limits and that a value of approximately 0.2 Btu/(1b) (degF) is a good average value. The value of the specific heat of water is 1 Btu/(1b)(degF) and of ice is 0.5 Btu/(1b)(degF). Hence, using equation (a), Plate B-11, the volumetric heat capacity of each test may be computed using the assumed value for specific heat of dry soil shown on Plate B-11 and the determined water content and density. By substituting the computed value for volumetric heat capacity, the thermal conductivity, k, may be computed using equation (b). An example for the determination the thermal conductivity, k, is presented on Plate B-12.

h. Test Data and Results. The data obtained from the thermal conductivity tests are summarized in tabular form on Plate B-12. Grain size curves of the materials tested are shown in Figures 4 and 5, Plate B-11. Figures 2 and 3 show typical curves of temperature changes at the thermocouple located at the midpoint of the specimen vs. time. Curves for determining the thermal conductivity from the measurements obtained, together with an example, are also contained on Plate B-11, Fig. 1. Flate B-13 contains a plot of the test results to illustrate, in general, the greater thermal conductivity of frozen material as the water content is increased. The results of tests on the blended bituminous concrete aggregate and asphalt bituminous concrete are tabulated on Plate B-11 but have not been plotted.

i. Tests by Other Investigators. A comprehensive investigation of the thermal conductivity of ten different soils

was performed by Harrison E. Patten (1). The results of his investigations upon eight soils are presented on Plate B-14 in a form similar to tests conducted by Boston District as summarized on Plate B-12. Tests on the remaining two soils were incomplete and are not included. On Plate B-14, Table A, is a summary of test data, Figure 2 contains reported grain size curves, and Figure 1 is a summary plot of thermal conductivity vs. water content for the densities tested. It will be noted that densities of test specimen were very low and generally much less than that at which the soils would normally be found in their natural state.

The results of additional tests made by others are summarized on Plate 3-15. It will be noted that the values of the unit dry weight of soil as tested were not determined by some of the investigators Shanklin states that in his tests, the soil was well tamped. Also, it is not certain whether the water content is expressed as a percentage of the dry or wet weight in these tests.

j. Analysis of Test Results. The tests performed by the Frost Effects Laboratory and summarized on Plate B-12 may be compared with those performed by H. E. Fetten as summarized on Plate B-14. It will be noted that the sample of coarse quartz tested by Patten approaches in grain size distribution that of Lowell Sand, and further that the thermal conductivities of these two soils are approximately the same at equal water

⁽¹⁾ Patten, Harrison E. "Heat Transference in Soils", U. S. Dept. of Agriculture, Bulletin No. 59, September 1909.

contents. The test results on the Hudson River Sand are quite different from all other materials tested. The finer grained soils, Podunk fine sandy loam, Leonardtown silt loam, Hagerstown loam, Galveston clay and fine quartz flour all have approximately the same thermal conductivity at equal water contents. The thermal conductivity of the muck soil is much less than that of the Somerville cinders and Mystic slag tested.

The tests upon the Lowell sand indicate that the influence of water content upon thermal conductivity is much greater than density.

There is no apparent relation between grain size or other characteristics of the various soils tested which may be correlated with thermal conductivity. To satisfactorily determine the thermal conductivity of a given soil, tests are required at several different moisture contents. The test procedure herein described is considered a satisfactory method for the rapid determination of the coefficient of thermal conductivity.

Analyses of investigations made by others and of the controlled laboratory tests indicate that the thermal conductivity of frozen cohesionless soils is greater than unfrozen at high water contents and is approximately equal at 1. we water contents and the thermal conductivity of most types of soils increases with increasing water content and increasing unit dry weight. A reasonable range of the thermal conductivity of most cohesionless soils frozen or unfrozen is from 0.6 to 1.5

Btu/ft. F. Mr. This range does not include the organic soils such as peat, soils of volcanic origin, or cohesive soils which may be expected to differ in thermal properties.

2-04. Review of Studies of Latent Heat of Soil Moisture by Others.

The latent heat of soil moisture is directly determined from the quantity of water in the soil which freezes. Investigations reported by others* of the percentage of water which freezes were reviewed and are summarized in the following paragraphs.

Public Roads tests were performed to determine the percentage of water in the soil which freezes, this percentage determining the latent heat. In performing the tests, the entire soils was subjected to below freezing temperature and water content of the soil maintained constant during the test. The materials tested were clean quarts sand (standard Ottawe sand), and soils containing silt and clay.

These tests indicated that, for the conditions tested, all water in a clean quarts sand froze at 0 degrees C (32 degrees F) and from 32 to 85 per cent of the water in the soils containing silt and clay froze at a temperature of minus 1.5 degrees C (29.3 degrees F), the percentage of water freezing

^{* &}quot;Forcentage of Water Freezable in Soils" Bureau of Public Roads, Public Roads February 1925.

[&]quot;Frogress Report on an Investigation of Frost Action in Soils" by fackintosh, Proceedings of International Conference on Soils Mechanics and Foundation Engineering, 1936.

depending approximately on the amount of fines.

Mackintosh were performed at Harvard University on a plastic clay, and indicated that for the conditions tested, about 33 per cent of the original water in the clay froze at a temperature between minus 1.0 deg. C and minus 2.0 deg. C (30.2 deg. F and 28.4 deg. F).

The latent heat of soil moisture is a direct function of the percentage of its water content that freezes. The percentage of water which will freeze in a soil is primarily dependent on the void size, the degree of saturation, the presence of soluble salts, the temperature of the soil, and the molecular attraction between ice and water. For all practicable purposes, all the water in clean cohesionless soils or the GW, GP, SW, and 37 classifications will from to at 32°F. In silt soils of the ML classification, most of the water may be expected to freeze at 32°F. In the remaining soils of the GC, GF, SC, SF, CL, OL, MH, CH, and OH classifications, a leaser percentage of the water contained in the voids will freeze end will depend on the factors listed above. The range of values of the percentage of water frozen, for the usual conditions encountered in plastic soils, may vary from 35 to 80. Figure 1, Plate 3-10 shows the relationship between density in pounds per cubic foot and latent heat of fusion in Hew pur cubic foot for various water contents assuming that all the water freezes. This figure is a nomographic prosentation of the graph shown on Figure 3. The average latent host, where there are several soil layers at different water

contents, may be determined using the following equation:

$$L = \frac{L_1 d_1 + L_2 d_2 + L_3 d_3 + \dots + L_n d_n}{d_1 + d_2 + d_3 + \dots + d_n}$$

where:

L is average latent heat of soil moisture

L₁, L₂, L₃, are latent heats of soil moisture in layers 1, 2, 3, etc.

 d_1 , d_2 , d_3 , etc. are the thicknesses of layers 1, 2, 3, etc. (note that $d_1 \neq d_2 \neq d_3 \neq \cdots d_n$ equal depth of freezing)

2-05. Review of Studies of Volumetric Heat of Soil by Others.

Studies by various investigators, as summarized by H. E. Patten
in "Heat Transference in Soils," demonstrated that the total

volumetric heat of a given volume of soil is the sum of the

volumetric heats of the individual components of the soil, i.e.,
dry soil and water or ice. Figure 2, Plate B-16 shows the relationship between unit dry weight, water content, and volumetric heat
for average values of the specific heats of soil, water and ice.

There are tabulated below a number of values for specific heats for various soils, rocks, and minerals. These values were obtained from "Handbook of Chemistry and Physics" 1945 Edition, and "Machanical Engineer's Handbook" by Marks, 1941. Most determinations were made at about room temperature.

MATERIAL	SPECIFIC HEAT BTU/(Lb)(°F)	MATERIAL	SPECIFIC HEAT BTU/(Lb)(°F)
Asbestos	0.195	Humus	०•मिन
Basalt	0.20	Kaolin	0.224
Calcspar	0.20	Marble	0.21
Cement	0.20	Mica	0.206
Chalk	0.214	Quartz	0.188
Clay, dry	0.22	Salt, Rock	0.21
Cinders	0.18	Sand	0.195
Dolomite	0.222	Sandstone	0.22
Gneiss	0.18	Serpentine	0.25
Granite	0.192	Talc	0.209
Hornblende	0.195		

Based on the specific heats of the various soils, rocks, and mineral an average value of 0.2 Btu/(lb)(°F) has been used for the specific heat of soil and values at 1.0 and 0.5 Btu/(lb)(°F) for water and ice, in all calculations involving the prediction of the depth of frost penetration. Figure 2, Plate B-Kshows the relationship between density in pounds per cubic foot and volumetric heat in BTU per cubic foot per degree F for various water contents for both frozen and nonfrozen states. Where the soil is fully saturated, the volumetric heat of the nonfrozen soil is nearly constant, varying from 40 to 45 Btu/(cu.ft.)(°F) within reasonable limits of unit dry weight and for frozen soil it may be considered constant at approximately 32 Btu/(cu.ft.)(°F). Where there are several layers at different unit dry weights and water contents, the following equation may

be used to determine an average value for volumetric heat:

$$c = \frac{c_1 d_1 + c_2 d_2 + c_3 d_3 + \cdots + c_n d_n}{d_1 + d_2 + d_3 + d_n}$$

where:

C is the average volumetric heat

C₁, C₂, C₃, etc. are volumetric heats in frozen or unfrozen states for layers 1, 2, 3, etc.

 d_1 , d_2 , d_3 , etc. are thickness of layer 1, 2, 3, etc.

From the data as presented on Plate E-17 the freezing temperature of soil moisture is considered as 32°F.

2-06. Studies of the Temperature Required to Freeze Soil Moisture.

The determination of the temperature at which soil moisture freezes was made from field investigations, and from a study of investigations performed by others* and are summarized below.

a. Supercooling. Studies by Bouyoucos involved tests in cooling cohesionless and cohesive soils with and without agitation. The tests indicate that cohesionless soils could be supercooled without agitation to a temperature of 24.4 deg. F. and

^{* &}quot;Degree of Temperature to which Soils can be Cooled Without Freezing," by G. Bouyouccs - Journal of Agricultural Research, November 1920.

[&]quot;Ice Pressure Determination in Clay Soils," Engineering News Record, 25 July 1935.

[&]quot;A Progress Report on an Investigation of Frost Action in Soils," by Mackintosh - Proceedings of the International Conference on Soil Mechanics and Foundation Engineering, 1936.

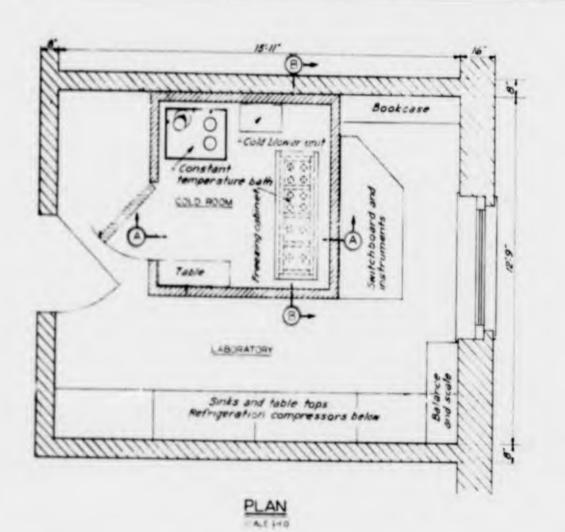
[&]quot;Studies of Frost Penetration," by H. U. Fuller - Journal N.E. Water Works Association, September 1940.

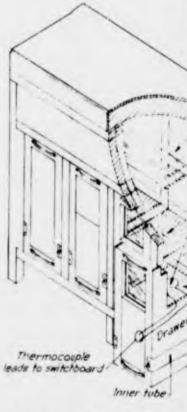
can be supercooled to 21.2 deg. F before they froze. Distilled water can be supercooled to 21.2 deg. F before it freezes. With constant agitation, it can be cooled to about 30.2 degrees F before it freezes.

- b. Bureau of Public Roads Tests. Tests performed by the Bureau of Public Roads on clean quartz and soils containing silt and clay, indicated that the freezing temperature was below 32°F, in some cases being as low as 29.3°F.
- performed independently at Harvard University, both on soft clay. In the first series of tests, the temperature at the bottom of the zone of ice lenses ranged from minus 1.0°C to minus 2.0°C (30.2°F to 28.4°F) and in the second series of tests, the boundary temperature between ice lenses and the unfrozen clay ranged from minus 0.5°C to minus 0.7°C (31.1°F to 30.7°F).
- d. Observations by Fuller. Observations were made by Fuller on a gravel soil and on a clay soil at Portland, Maine, by reading thermometers installed at 6-inch intervals of depth and observing the frost penetration. These observations indicated that the temperature at the bottom of the frozen layer was 32.5°F, for both types of soil.
- e. Field Observations by Frost Effects Laboratory.

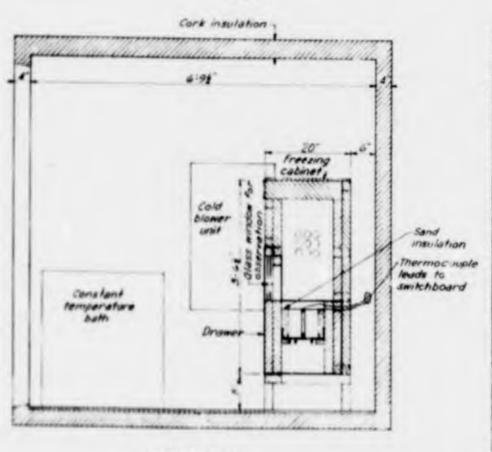
 The depth of frost penetration as observed in test pits compared with the temperature of the soil at that depth in adjacent subsurface temperature installations are shown on Plate 3-17 for 145 locations during 1944-1947 indicated a temperature range of

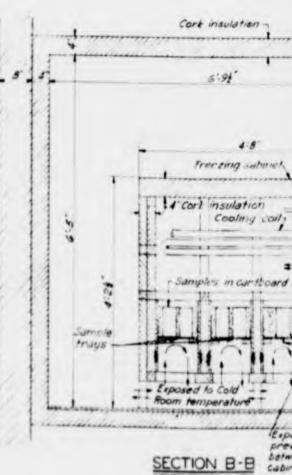
25°F to 40°F. Discounting the three values below 30°F and approximately 35 values above 34°F the average temperature at the maximum depth of frost penetration was 32°F.





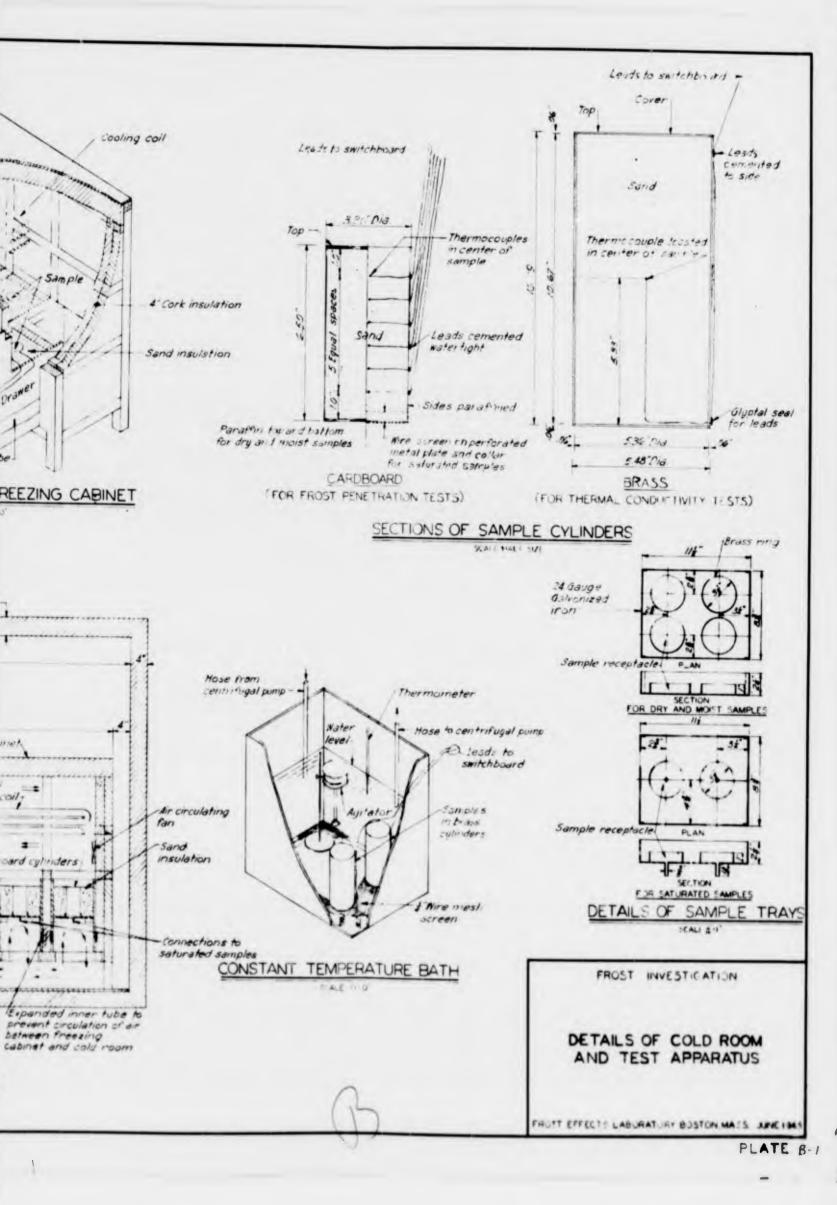
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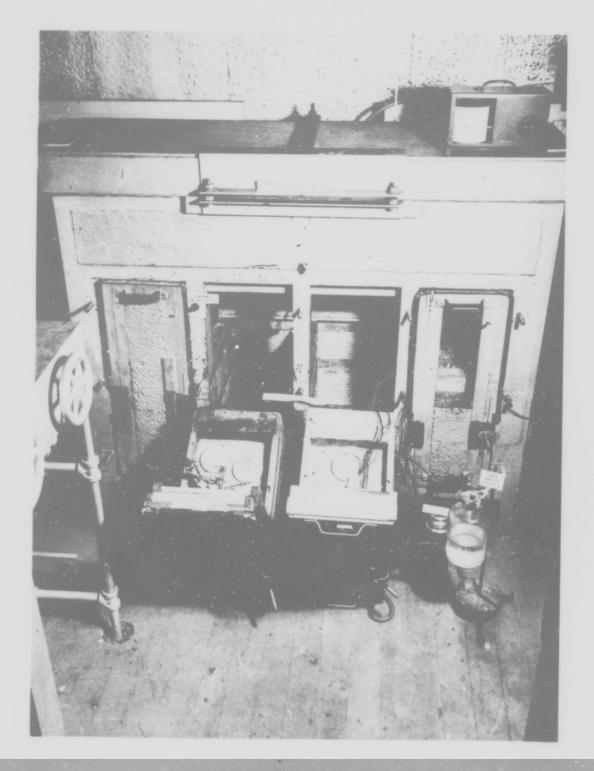




SECTION A-A

SECTION B-B





View of Freezing Cabinet

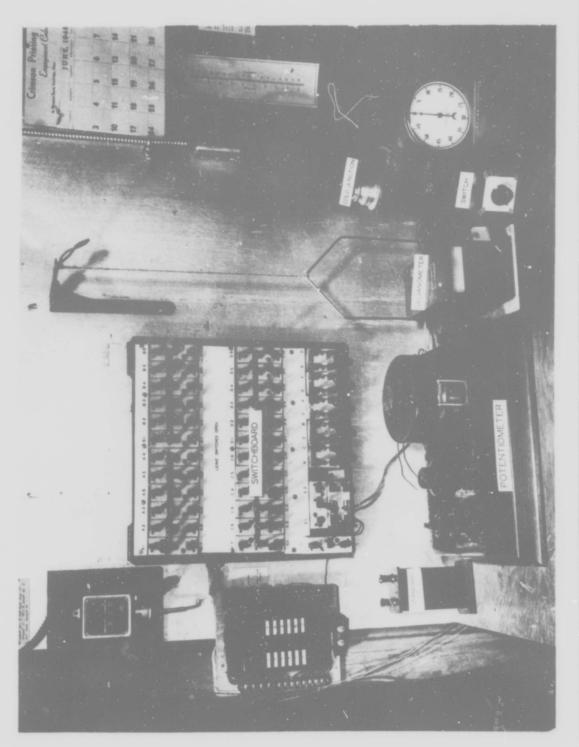
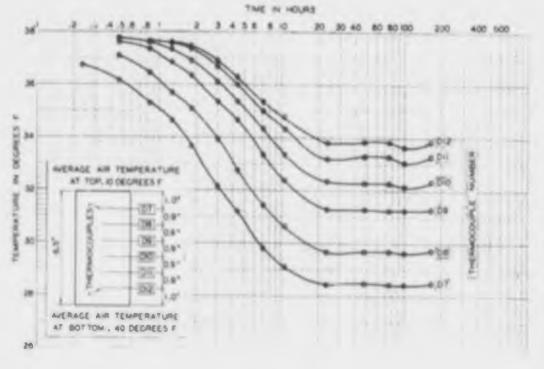
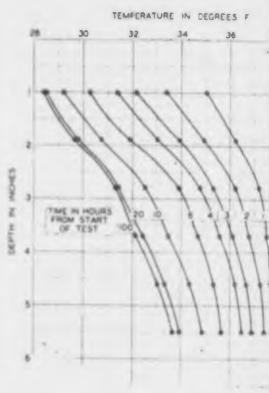


PLATE B-3





SAMPLE C-8

SAMPLE C-8

TYPICAL SET OF TIME TEMPERATURE CURVES FIG. I

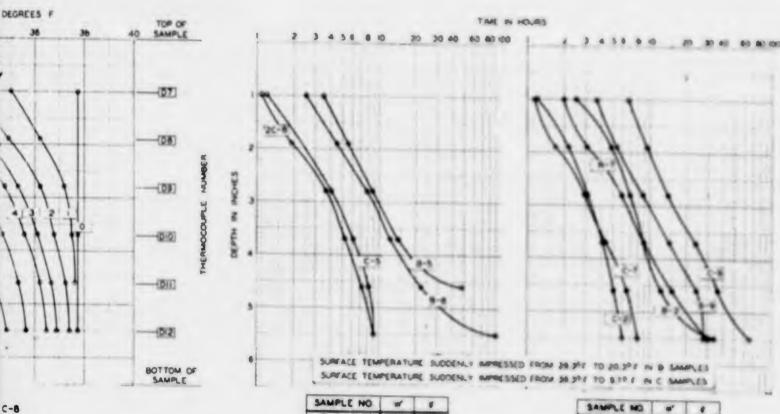
TYPICAL SET OF TEMPERATURE G

FIG. 2

			-			RUN	A							RUN	8									
MAPU	1917	Saran Contract	-	- CONTRACT	THE PERSON NAMED IN	AND DESCRIPTIONS			ATR TROP	SUPPLE .	TIME PIG SUTTLEMENT CONTESTED		PEATE STATE	SECTION OF THE PERSON OF THE P	ot-		APRIADE AND POSTERNING		ALME AND	ETHANILATES SAFEE TRESSATING		MOTE THE THE	148	
	Lis, Ny.						TOT IS	DISTRIBUTE OF	PE IS ASSESSED.	10	sorese of	TRESTAND TRESTAND TREAT	1.0	11-4	the or crecipes	SUPPLIES OF	SURFACE STATISTICS OF THE PERSONS SERVICES OF THE PERS	900 C	NOTICE OF	INACTOR IN OPECUARS OF/PT.	5-0	b-0		
Manage of the same	95.1 95.1 15.2 15.4 15.4 15.4 15.4 15.4 15.4 15.4 15.4	120222222222	101111111111111111111111111111111111111	H.3 H.3 H.3 H.3 H.3 H.3 H.3 H.3 H.3 H.3	SERESTERE	No.1 10.4 10.6 10.6 10.6 10.6 10.6 10.6 10.6 10.6	11.3 11.3 11.3 11.3 11.3 11.3 11.3 11.3	TATATATATATATATATATATATATATATATATATATA	8-3 1-5 1-6 1-6 1-5 1-1 1-5 1-5 1-1 1-5 1-1 1-5 1-1	50 50 50 50 50 50 50 50 50 50 50 50 50 5	20.3 20.3 20.3 20.3 20.3 20.3 20.3 20.3	9.1 9.1 9.1 9.1 9.1 9.1 9.1	50 50 50 50 50 50 50 50 50 50 50 50 50 5	以よう 対点の 対点は を表示 がある をある がある がある がある がある がある がある がある がある がある が	15.3 13.3 15.5 15.5 15.5 15.7 16.7 16.7 16.3 16.3	13.0 16.3 16.3 16.3 16.4 16.4 16.4 16.4 16.4 16.4 16.4 16.4	222222222222222222222222222222222222222	5.0 5.0 6.0 6.0 6.0 6.0 6.0 8.0 7.0						
						RUN	c					Alexandra de		RUN	D	-	340	1 40						
5 0 5 0 5 1 5 5 5	01.9 0a.0 bm.a 101.a 1 1.9 1 1.9 1 1.9	8.0 8.0 8.0 81.0 81.0 81.0	122212222222	NAME OF PERSONS ASSESSED.	CETT FESSOSSI	95-8 95-7 94-6 15-6 95-8 17-8 17-8 17-8 18-1 18-1 18-3 18-3	23.43.84 23.43.84 23.43.84 23.43.84 24.43	113 123 124 124 124 124 124 124 124 124 124 124	255422551411	222242222233	22222222222222222222222222222222222222	######################################	80 50 50 50 50 50 50 50 50 50 50 50 50 50	27.8 27.5 27.5 27.5 27.5 27.5 27.5 27.5 27.5	35.1 37.4 8.3 8.4 8.4 8.4 8.4 8.4 8.4 8.4 8.4 8.4 8.4	10,0 11,0 15,0 15,0 10,7 11,0 15,1 12,0 8,5 8,5 8,7	2.0 2.0 1.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2	5.7 3.8 9.7 9.6 1.8 5.6 9.6 1.8 5.7 5.6 6.7 5.7						

SUMMARY OF TEST CONDITIONS

TABLE A



RATURE GRADIENTS

5.00 4.4 5.00 4.4 6.0 8.4 7.4

3.7 3.2 2.6 2.6 3.0 5.7 5.6 5.6

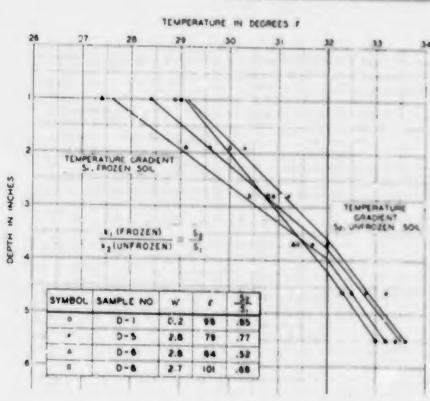
SAMPLE NO		
8-5 C-5	2.6	81,6
6-6. C-6	2.7	103.4

SAMPLE NO	46"	
8-5 C-5	0.2	96.3
8-7, C-7	2.0	102.4
8-9 C-9	23.8	10.0

FIG. 3a

FIG. 36

PENETRATION OF 32 DEGREES F TEMPERATURE VS TIME

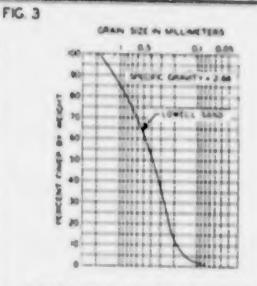


SAMPLES 0-1, 0-5, 0-6, 0-8

EQUILIBRIUM TEMPERATURE GRADIENTS

FIG 4

REVISED FEBRUARY, 1947



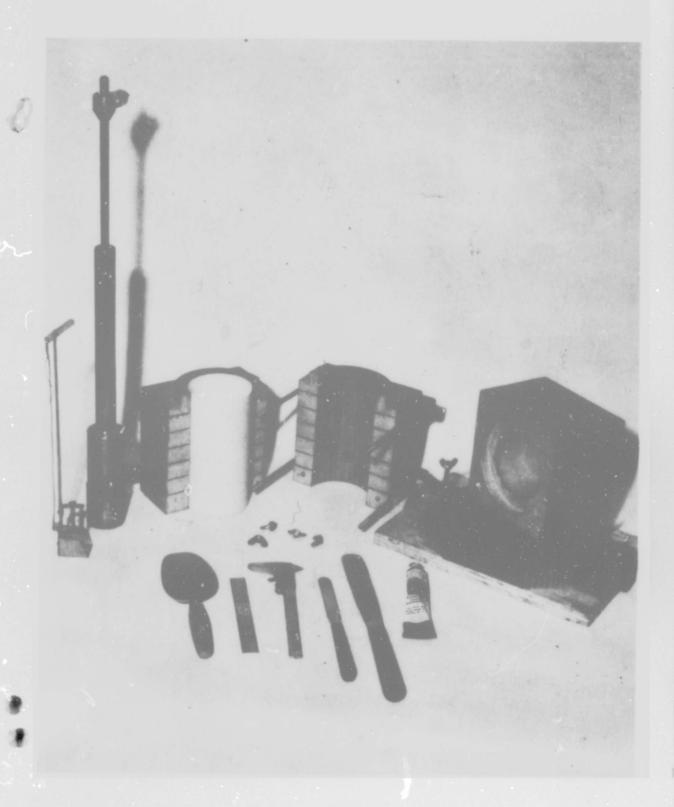
GRAIN SIZE GRADATION CURVE

FIG. 5

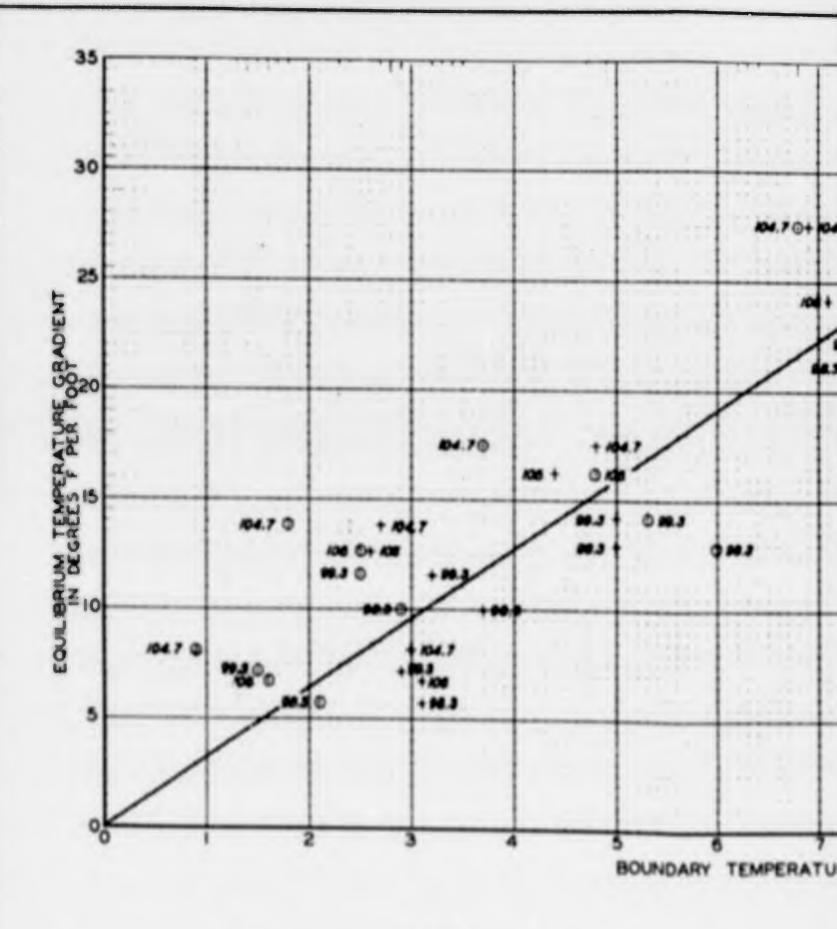
FROST INVESTIGATION

INVESTIGATION OF TEMPERATURE CONDITIONS IN LABORATORY SPECIMENS

FROST EFFECTS LABORATORY BOSTON MASS JUNE 945

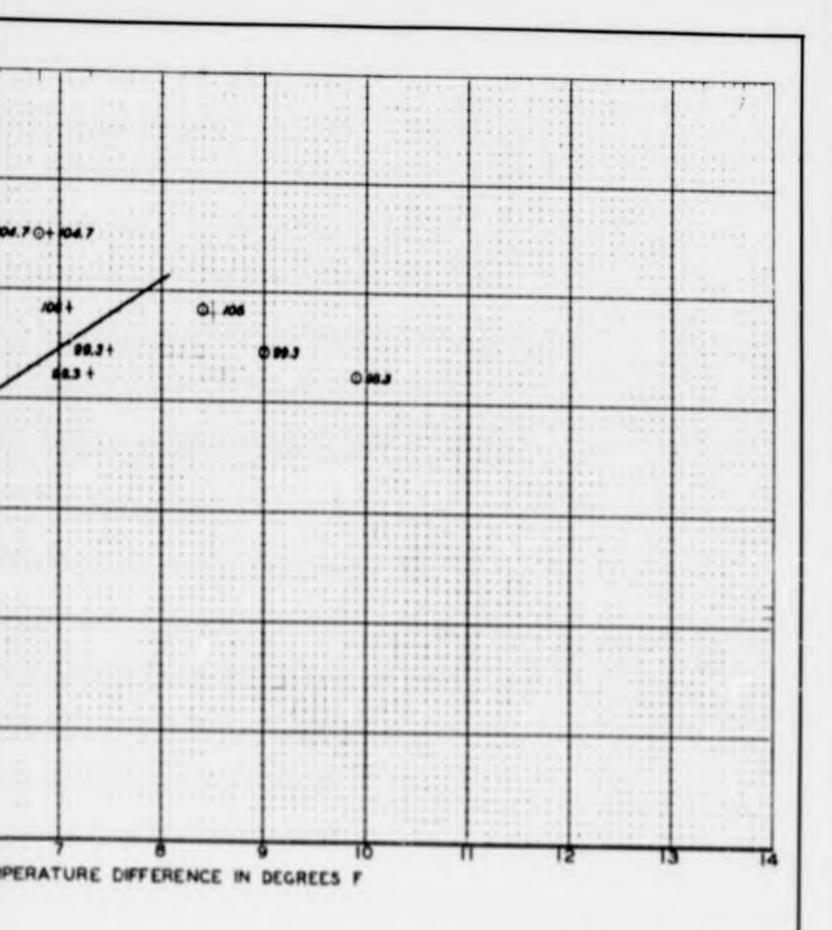


Laboratory equipment for compacting samples in cardboard cylinders



LEGENO

- O TOP SPECIMEN WITH THIN PARAFFIN LAYER
- + BOTTOM SPECIMEN IN CONTACT WITH SHEET METAL
- UNIT DRY WEIGHT IN LOS. PER CU. FT.

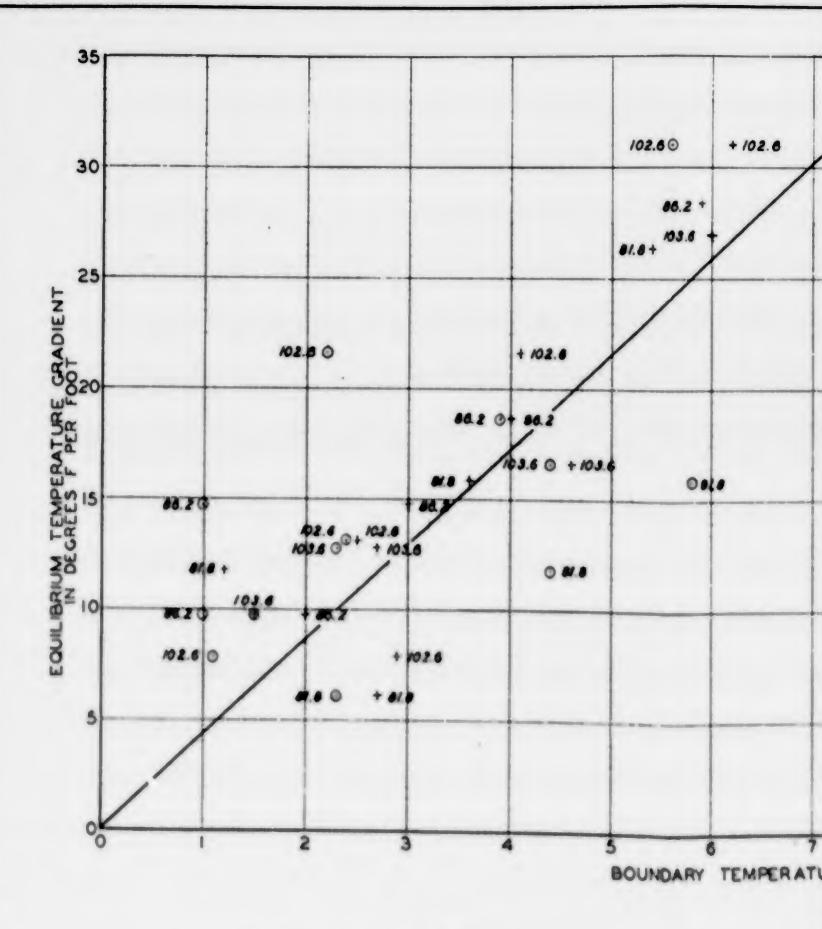


FROST INVESTIGATION

0.2 PERCENT WATER CONTENT

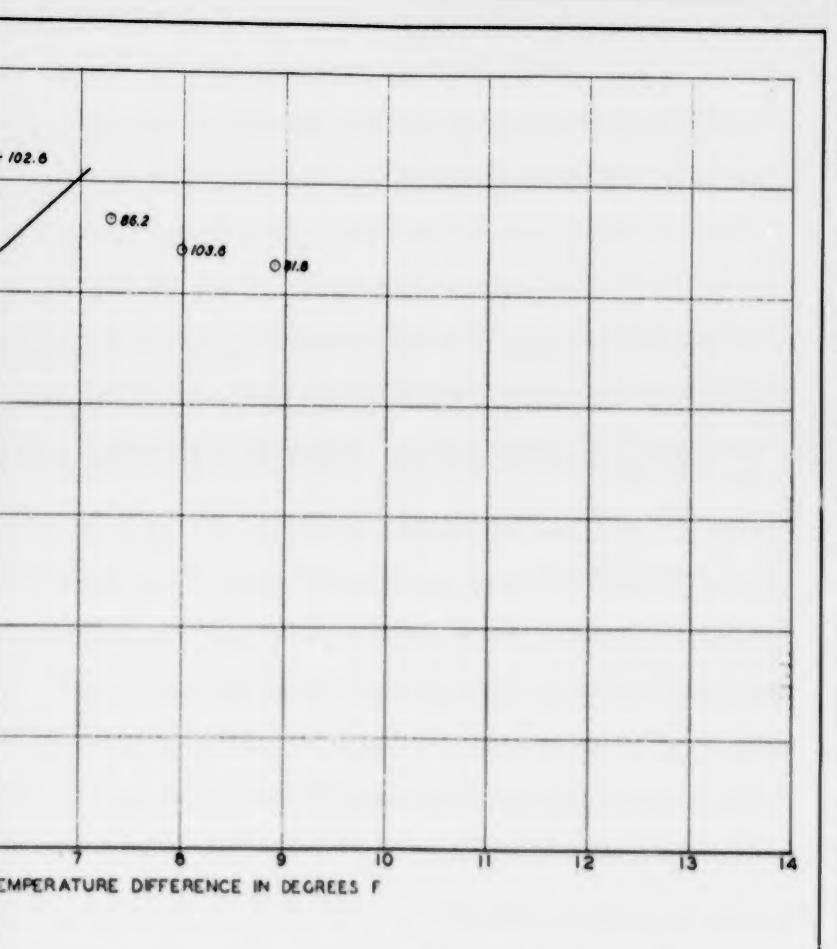
FROST EFFECTS LABORATORY

BOSTON, MASS



LEGENO

- O TOP-SPECIMEN WITH THIN PARAFFIN LAYER
- + BOTTOM SPECIMEN IN CONTACT WITH SHEET METAL
- 101 NUMBERS BESIDE PLOTTED POINTS INDICATE UNIT DRY WEIGHT IN LBS. PER CU. FT.

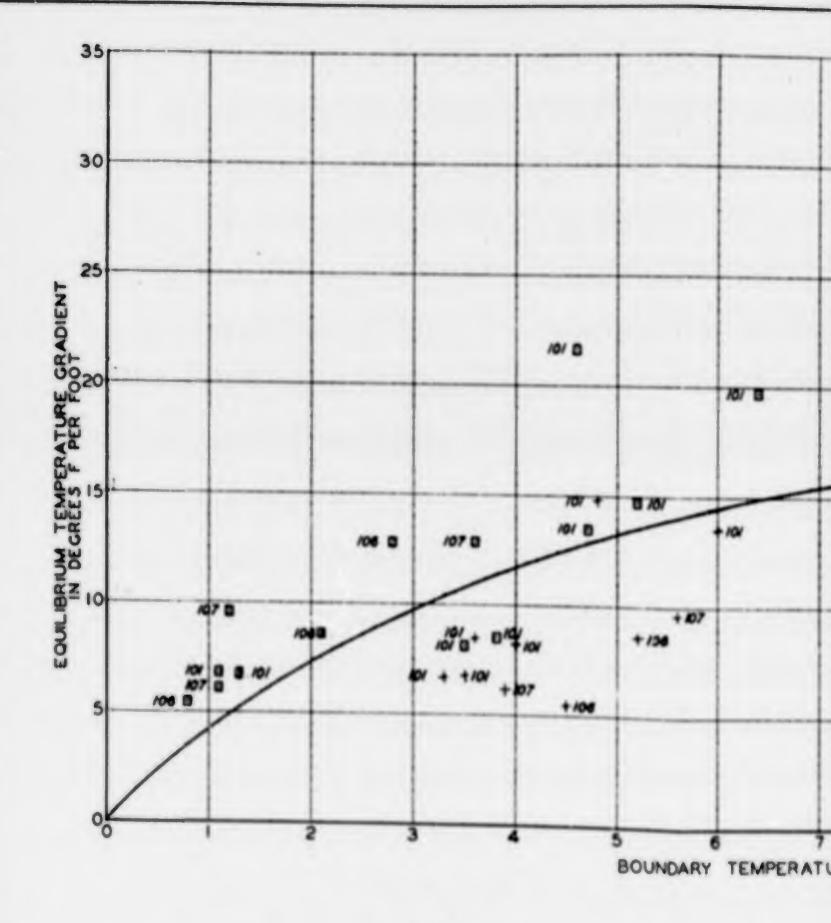


FROST INVESTIGATION

BOUNDARY TEMPERATURE DIFFERENCE 2.8 PERCENT WATER CONTENT

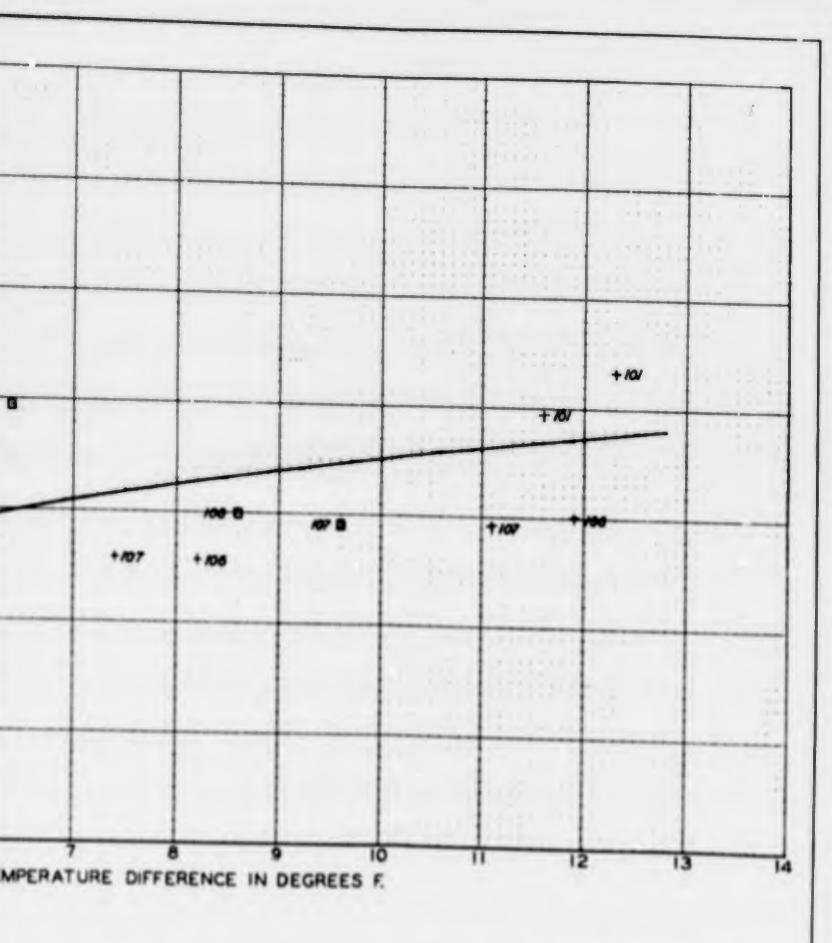
FROST EFFECTS LABORATORY

BOSTON , MASS.



LEGENO

- TOP SPECIMEN WITH NO PROTECTION
- + BOTTOM SPECIMEN IN CONTACT WITH SHEET METAL
- 101 NUMBERS BESIDE PLOTTED POINTS INDICATE UNIT DRY WEIGHT IN LBS PER CU FT.

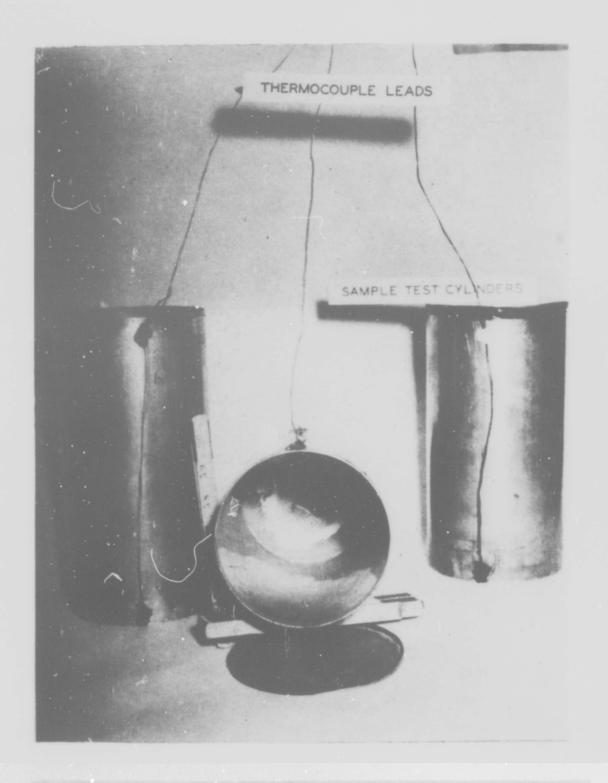


FROST INVESTIGATION

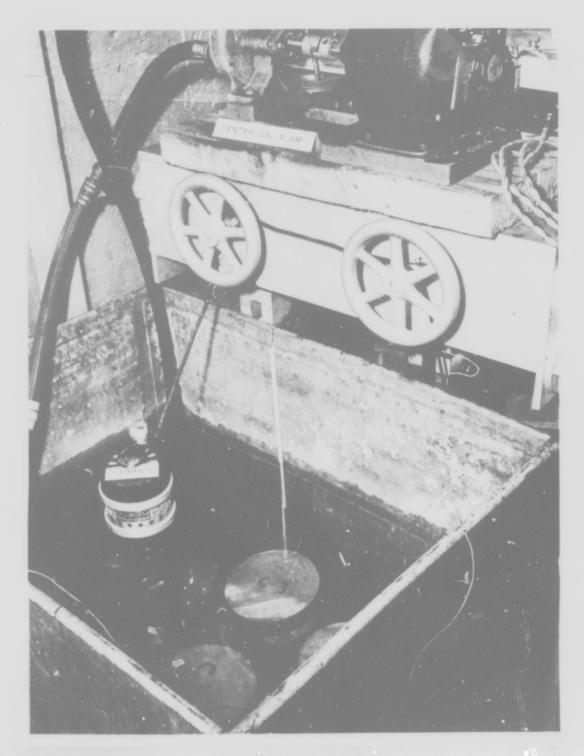
BOUNDARY TEMPERATURE DIFFERENCE 20 TO 23 PERCENT WATER CONTENT

FROST EFFECTS LABORATORY

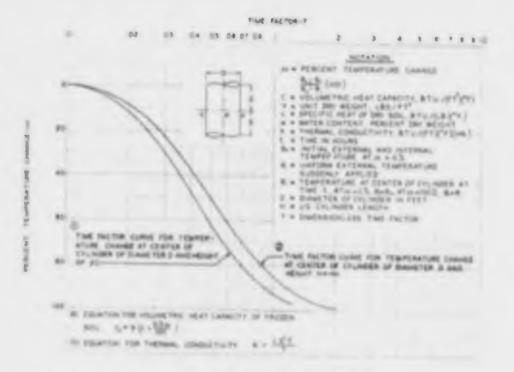
BOSTON HASS



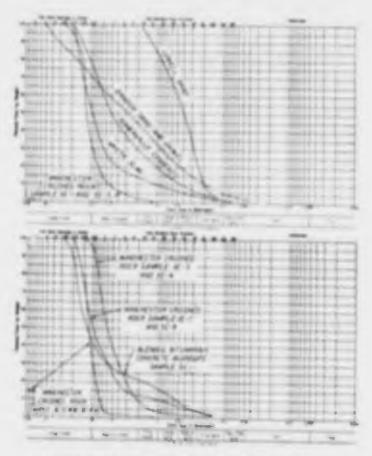
Cylinders used for thermo-conductivity tests



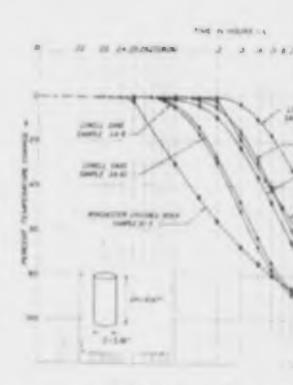
Constant Temperature Bath with Test Specimens Immersed



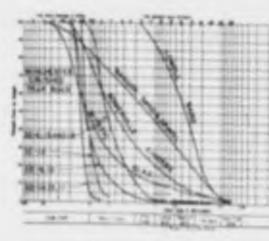
TIME FACTOR CURVES FOR TEMPERATURE CHANGE AT CENTER OF CYLINDER FIG.I



GRADATION OF MATERIALS TESTED IN UNFROZEN STATE FIG.4



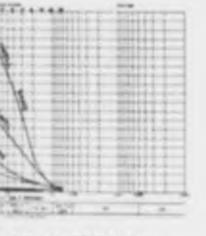
THERMAL CONDUCTIVITY DET UNFROZEN MATERI FIG. 2



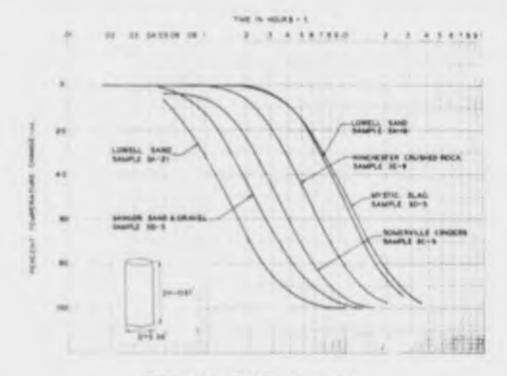
GRADATION OF MATER TESTED IN FROZEN S



TIME CURVES IVITY DETERMINATIONS IN MATERIAL FIG.2



OF MATERIALS FROZEN STATE



THERMAL CONDUCTIVITY DETERMINATIONS FROZEN MATERIAL

FIG.3

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EAMON, 700 DETERMINED OF THERMAL CONDUCTIVES

SINCE MASS.

FOR DATA FOR BANKET AND DO-S, MARY BALLES FOLLOWS

Y HIS BY A FOLKY BY

X A COT LES /FT 

W A SAN.

EACHTOMS

(A 1 5.4 * (4 + 6.6 ))

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(A) 5 * A 10 (2 + 6.6 )

(A) 5 * A 10 (2 + 6.6 )

(A) 6 * A 10 (2 + 6.6 )

(A) 7 * A 10 (2 + 6.6 )

(A) 8 * A 10 (2 + 6.6 )

(A) 9 * A 10 (2 + 6.6 )

(A) 10 * A 10 (2 + 6.6 )

(A) 10 * A 10 (2 + 6.6 )

(A) 10 * A 10 (2 + 6.6 )

(A) 10 * A 10 (2 + 6.6 )

(A) 10 * A 10 (2 + 6.6 )

(B) 10 * A 10 (2 + 6.6 )

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(B) 10 * A 10 (2 + 6.6 )

(B) 10 * A 10 (2 + 6.6
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FROST INVESTIGATION

THERMAL CONDUCTIVITY DETERMINATIONS FROZEN AND UNFROZEN BASE MATERIALS

FROST EFFECTS LABORATORY, BOSTON, MASS.

SERIES	MATERIAL	SPECIFIC	SPECIFIC HEAT (1)		NONFROZEN					
NO.		CHAVITY	DRY SOIL BTU./(LB)(DEG.F)	LABORATORY SAMPLE NO.	UNIT DRY WEISHT LBS./CU.FT.	WATER CONTENT PER CENT DRY WEIGHT W	VOLUMETRIC HEAT CAPACITY TOTAL SAMPLE BEU/(FT ³)(DEG.F			
3 A	LOWELL SAND	2.66	0.20	3A-4	105.0	0,2	21.2			
	(Well graded	2.66	0.20	SA-4a	105.0	0.2	21.2			
	medium to	2.66	0.20	3A -5(3)	101.0	0.2	20.4			
	coarse sand (2)	2.66	0.20	3A-8c	106.5	16.4	38.8			
		2.66	0.20	8A-7	101.0	20.9	41.3			
l		2.88	0.20	3A-8	103.0	4.5	25.3			
		2.66	0.20	34•	83.5	4.9	20.8			
i		2.66	0.20	3A-10(3)	84.5	2.3	18.8			
}		2.66	0.20	3A-11(3)	91.1	1.9	19.9			
1		2.66	0.20	3A 12	109.0	2.2	24.3			
		2.66	0.20	3A-13	103.0	2.0	22.7			
		2.66	0.20	3A-15	89.3	2.1	19.7			
		2.68	0.20	3A-16	105.0	5.1	26.4			
		2.66	0.20	3A-17	90.8	2.1	20.1			
3B	BANGOR SAND &	2.70	0.20	3B-1	127.0	3.4	29.8			
	GRAVEL! Well	2.70	0.20	38-2	131.5	1.1	27.7			
	graded - 14"	2.70	0.20	3 B-3	127.0	9.3	36.3			
	raximum)	2.73	0.20	38-11	133.3	0.32	27.1			
		2.70	0.20							
		2.70	0.20				•			
		2.00	0.20							
0 C	SOMERVILLE	2.27	0.18	3C-1	60.9	20.7(5)	23.6			
	CINDERS (Well	2.27	0.18	3C-2	60.0	36.6	32.8			
	graded - 1"	2.27	0.18	3C-3	60.8	21.2(8)	23.9			
	maximum)	2.27	0.18	3C-1	61.7	11.3	18.1			
		2.27	0.18	3C-8	61.9	1.1	11.9			
30	MYSTIC' SLAG	2.45	0.17	3D-1	79.1	9.1	17.5			
	(14 "maximum)	2.45	0.17	3D-2(7)	81.2	33.5	40.9			
		2.45	0.17	3D-6	92.3	0.6	16.3			
3E	WINCHESTER	2.91	0.20	3E-1	99.2	1.9	21.7			
	CRUSHED TRAP	2.91	0.20	3E-2	100.0	2.1	22.1			
	ROCK(# "raximum)	2.91	0.20	3E_3	98.5	4.4	23.6			
		2.91	0.20	3E-4	98.5	27.2	46.5			
		2.91	0.20	3E-5(7)	99.3	28.4	48.0			
		2.91	0.20	3E-6a(7)	100.0	27.7	47.7			
		2.91	0.20	3E_7	102.0	2.5	23.0			
		2.91	0.20	35-8	102.0	26.7	47.7			
		2.91	0.20	3E-21	112.4	0.21	22.7			

Results of test on a Sample of bituminous concrete and on the aggregate will be added to this table.

(1) Assumed

(2) Minimum dry density 92.9 lbs/cu.ft.
Maximum dry density 110.9 lbs/cu.ft.

(8) Sample not properly sealed; some water leaked into sample during test.

(4) Slight leaking into cylinder during test.

(5) Average - w = w =

(6) Non-uniform wa

(7) Test results a results of

(8) Cover lifted of heaving.

specimen (9) Average neave

	FROZEN											
THERMAL CONDUCTIVITY U/(FT)(DE3.F)(HR) k	LABORATORY SAMPLE NO.	UNIT DRY WEIGHT LBS/CD.PT.	VOID RATIO	WATER CONTENT PER CENT DRY WEIGHT	PER CENT SATURATION G	VOLUMETRIC HEAT CAPACITY TOTAL SAMPLE BTU/(FT3)(DEG.F)	THERMAL CONDUCTIVITY BTU/(FT)(DEG F)(HR) k					
0.188	3A-18	108.2	0.563	0.165	0.780							
0.188	3A-19	102.9	0.812	0.185	0.760	21.3	0.185					
0.169	3A-20	102.5	0.820	5.4	23.2	20.7	0.164					
1.025	3A-21	106.2	0.563	16.5	77.7	23.3 30.0	0.885					
1.000	3A-22	102.6	0.618	18.5			1.755					
0.718	3A-23	102.5	0.620	20.5	79.6 88.2	30.0	1.540					
0.469	3A-24	105.0	0.581	2.2	10.1	31.0	1.610					
0.335	3A-25	100.0	0.565	4.2		22.2	0.460					
0.352	3A-26(4)	111.8	0.475	0.66	19.8	23.4	0.912					
0.582	3A-27(4)	111.1-	0.496	0.98	3.7 5.25	22.4	0.265					
0.476	2. (3/		0.400	0.86	0.20	22.8	0.314					
0.463												
0.777												
0.437												
0.890	3B-4(4)	130.8	0.289	2.1	19.8	27.5	0.725					
0.673	3B_5	127.1	0.324	3.6	30.2	27.7						
1. 125	3B-ri	130.8	0.289	9.9	90.0	32.8	1.038					
0.472	3B-17	127.1	0.328	10.6	87.8	32.2	1.528					
	38-8	130.2	0.295	1.8	16.5	27.2	1.489					
	3B_9	130.2	0.295	10.3	94.3	32.8	0.885					
	38-10	132.9	0.267	0.23	2.3	26.7	1.475 0.465					
0.353	3C-5	60.8	1.327	11.7	20.0	14.5	0.372					
0.462	3C-8	60.8	1.327	35.5	60.7	21.7	0.700					
0.354	3C-7	63.4	1.233	0.09	0.03	11.4	0.152					
0.297							0.100					
0.173												
0.188	30-3	87.2	0.750	5.5	18.9	17.2	0.245					
0.553	3D-4	87.2	0.750	27.7	90.6	26.9	0.673					
0.146	3D-5	89.3	0.710	0.21	0.73	15.3	0.122					
0.350	3E-9	102.8	0.767	1.5	5.7	21.3	0.328					
0.371	3E-10(F)	102.8	0.767	25.8	97.9	33.8	1.189					
0.403	3E-11	106.5	0.708	2.2	9.1	22.5	0.417					
0.849	3E-12	103.6	0.754	1.2	4.8	21.3	0.334					
2.320	3E-13(9)	106.5	0.70€	28.1	91.1	33.1	0.999					
1.850	35-14(9)	103.5	0.754	25.0	96.5	33.6	1.080					
0.371	3E-15	104.7	0.734	2.0	7.9	22.0	0.375					
1.479	3E-17	102.5	0.772	0.21	0.784	20.6	0.157					
0.196	3E-18	111.3	0.631	0.12	0.553	22.3	0.198					

HANGE OF STONE	SIZES - SERIES SE
3E-1 & 3E-9	(1/2" - 3/4")
3E-2 & 3E-10	$(1/2^n - 3/4^n)$
3E-3 4 3E-11	(1/8" - 3/8")
3E-4 & 3E-12	(3/8" - 1/2")
3E-5 & 3E-13	(1/8" - 3/8")
3E-8a 4 3E-14	(3/8" - 1/2")
3E-7 & 3E- 15	50%(1/2" - 3/4")
	254(3/8" - 1/2")
	25%(1/8" - 3/8")
3E-8 & 3E 17	(1/2" - 3/4")
3E-21 & 3E-18	(1/8" - 3/8")
	3E-1 & 3E-9 3E-2 & 3E-10 3E-3 & 3E-11 3E-4 & 3E-12 3E-5 & 3E-13 3E-6a & 3E-14 3E-7 & 3E-15

FROST INVESTIGATION 1945-1946

THERMAL PROPERTIES OF SOILS
SUMMARY OF TEST DATA

FROST EFFECTS LABORATORY BOSTON, MASS. JUNE 1946

PLATE B-12

WATER CONTENT - PERCENT DRY WEIGHT

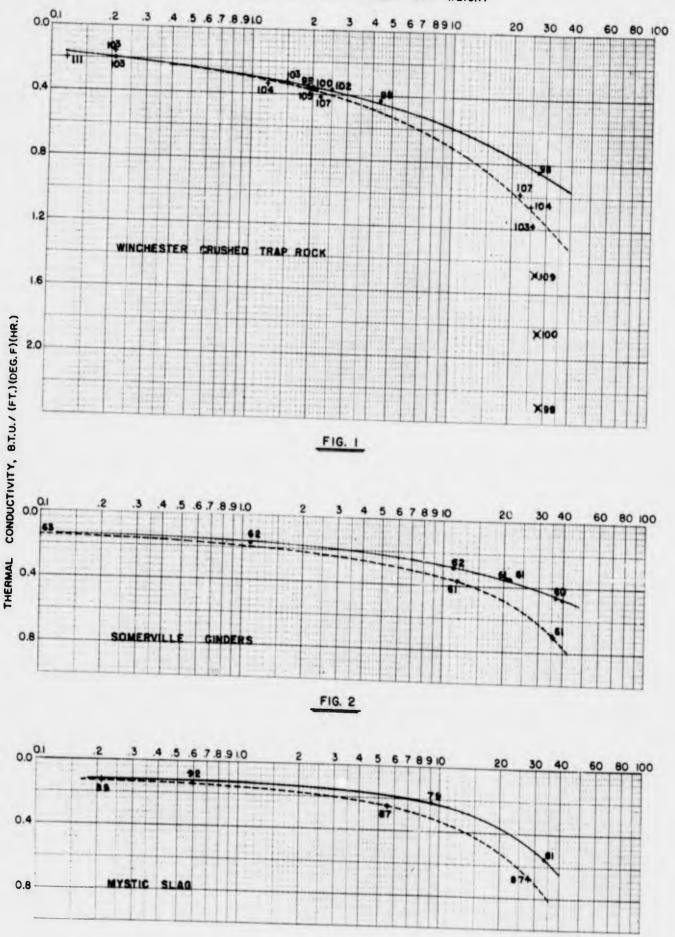


FIG. 3

0.0 0.8 1.2

1.6

THERMAL CONDUCTIVITY, B.T.U./(FT.)(DEG.F)(HR.) 0.8

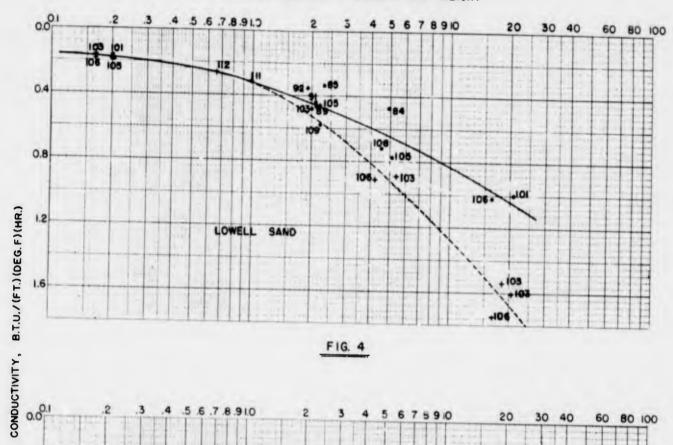
> 1.2 1.6

NOTES UNIT C FOOT.

- UNI --+-- FR

X OBS





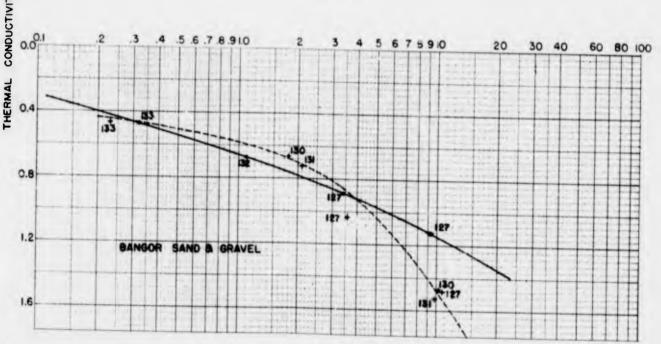


FIG. 5

NOTES

00

100

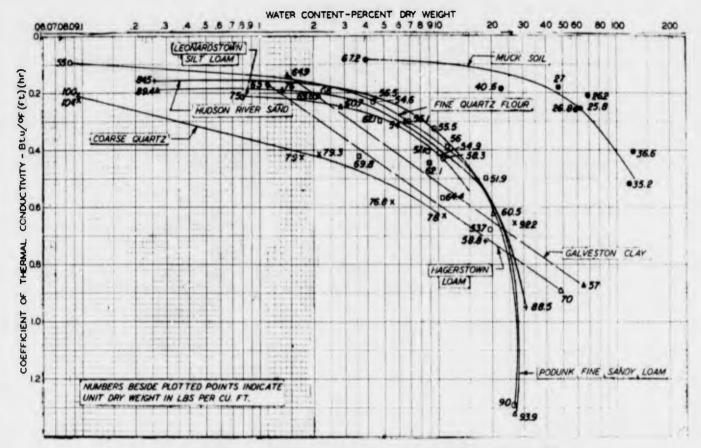
NUMBERS BESIDE PLOTTED POINTS INDICATE UNIT DRY WEIGHT OF SAMPLE IN POUNDS PER CUBIC FOOT.

- UNFROZEN MATERIAL
- --+-- FROZEN MATERIAL
- X OBSERVATION IN ERROR

FROST INVESTIGATION 1945-1946

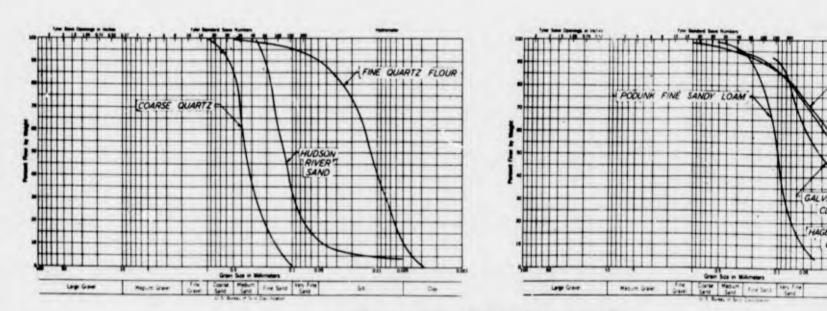
THERMAL CONDUCTIVITY VS.
WATER CONTENT
OF BASE MATERIALS

FROST EFFECTS LABORATORY, BOSTON, MASS. JUNE 1946



THERMAL CONDUCTIVITY VS WATER CONTENT

FIG. I



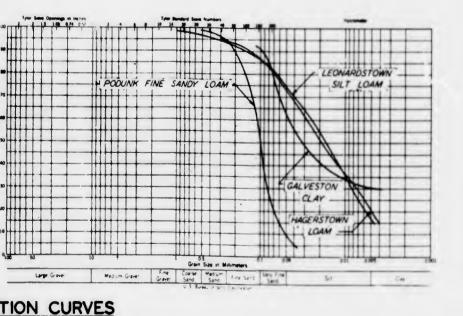
GRAIN SIZE GRADATION CURVES



Material	Unit Dry Laterial Weight		Specific Heat	Volumetric Heat Capacity	Thermal Conductivity	
	lbe/o.f.	% Dry Wt.		Btu/(97)(cf)	Btu/oF (ft)(hr	
COARSE QUARTZ	104 100 79 79.3 76.8 78.0 92.2	0.095 0.095 1.710 2.160 5.520 10.910 26.700	0.1900 0.1900 0.1900 0.1900 0.1900 0.1900 0.1900	17.3 17.1 16.3 16.7 18.0 23.3 42.1	0.221 0.206 0.123 0.115 0.561 0.630 0.653	
FINE QUARTZ FLOUR	55 54.6 56.1 57.1 55.5 58.3 53.7 90.0	0.0833 4.280 6.930 8.720 9.530 10.920 19.670 26.650	0.1900 0.1900 0.1900 0.1900 0.1900 0.1900 0.1900	10.2 12.7 11.5 12.1 15.2 17.4 20.7 11.5	0.0981 0.232 500 0.403 0.323 0.427 0.680 1.290	
HUDSON RIVER SAND	84.5 56.5 58.8 88.5	0.257 4.500 13.120 30.760	0.1900 0.1900 0.1900 0.1900	14.2 13.3 21. 13.	0.1575 0.210 0.720 0.953	
PODUNK FINE SANDY LOAM	89.4 76.0 66.0 64.7 54.0 54.9 60.5	0.268 1.330 2.140 2.830 6.601 10.080 20.250 26.930	0.1900 0.1900 0.1900 0.1900 0.1900 0.1900 0.1900	17.2 15.5 13.7 13.2 13.6 16.0 23.7 13.0	0.191 0.191 0.209 0.214 0.302 0.418 0.623 1.32	
LEONARDSTOWN SILT LOAM	75.0 69.6 69.8 62.1 64.4 56.0 51.9	0.806 2.127 3.580 4.690 8.980 10.650 11.570 18.350	0.1900 0.1900 0.1900 0.1900 0.1900 0.1900 0.1900 0.1900	118 117 15.7 147 17.3 1 .0 17.1	0.214 0.210 0.122 0.239 0.143 0.562 0.388 0.500	
HAGERSTOWN LOAM	65.0 70.0	1.12 18.96	0.1914 0.1914	13.2	0.1686 0.993	
GALVESTON CLAY	64.9 57.0	1.41 67.55	0.2097 0.2097	₩:•5 57•1;	0.139 0.868	
MUCK SOIL	67.2 40.6 27.0 26.8 25.5 26.2 36.2 36.6	3.93 22.95 47.06 58.98 62.93 69.42 119.20 123.00	0.1900 0.1900 0.1900 0.1900 0.1900 0.1900 0.1900 0.1900	19.4 17. 17. 23.6 21.1 23.1 49.5 51.9	0.0842 0.184 0.190 0.260 0.257 0.208 0.519 0.402	

DATA SUMMARY TABULATION

TABLE A



(I) H.E. PATTEN "HEAT TRANSFERENCE IN SOILS" U.S. DEPARTMENT OF AGRICULTURE BULLETIN NO. 59 SEPTEMBER 1909.

NOTE

FROST INVESTIGATION

SUMMARY OF THERMAL CONDUCTIVITY TESTS BY H. E. PATTEN (1)

FROST EFFECTS LABORATORY, BOSTON, MASS. JUNE 1945

REVISED JULY, 1947

PLATE B-14

	SOIL	WATER	THERMAL CO	ONDUCTIVITY	UNIT DRY
INVESTIGATOR	TESTED	CONTENT %	WATTS CM/°C	BTU FT/°F/HR	WEIGHT LBS/CU.FT.
Shanklin	Clean yellow builders sand	0.15 1.6 4.2 9.0	0.003 0.0035 0.0048 0.013	0.174 0.202 0.278 0.752	
	Yellow sandy clay	0.89 3.87 8.5 25.0	0.0025 0.0029 0.0032 0.014	0.145 0.168 0.185 0.810	
Kennelly (a)	Fine white quartz sand (b)	0.2 7.0 13.8	0.0025 0.0044 0.0062	0.45 0.255 0.359	
	Fine sandy soil	0.2 4.9 8.0 15.9	0.0021 0.0023 0.0024 0.0028	0.121 0.133 0.139 0.162	
Teichmuller	Clean yellow sand	0.2 4.1 9.8	0.0031 0.0125 0.0161	0.179 0.729(c) 0.932	
	Average sandy soil	12.0	0.0085	0 - ₹435	
Ingersoll and	Quartz Medium fine	0.0		0.442	103
Koepp	sand Sandy clay Calcareous	15.0		0.94 1.47	109 111
	earth	43.0		1.14	104
Berggren	Dry soil Moist soil (frozen)	0.0		0.19	
	Moist soil (unfrozen)			0.48	
	Wet soil (frozen)			1.21	
	Wet soil (unfrozen)			0.97	

(a) Test equipment considered unsatisfactory by Shanklin.

(b) Passing 0.25 mm mesh.

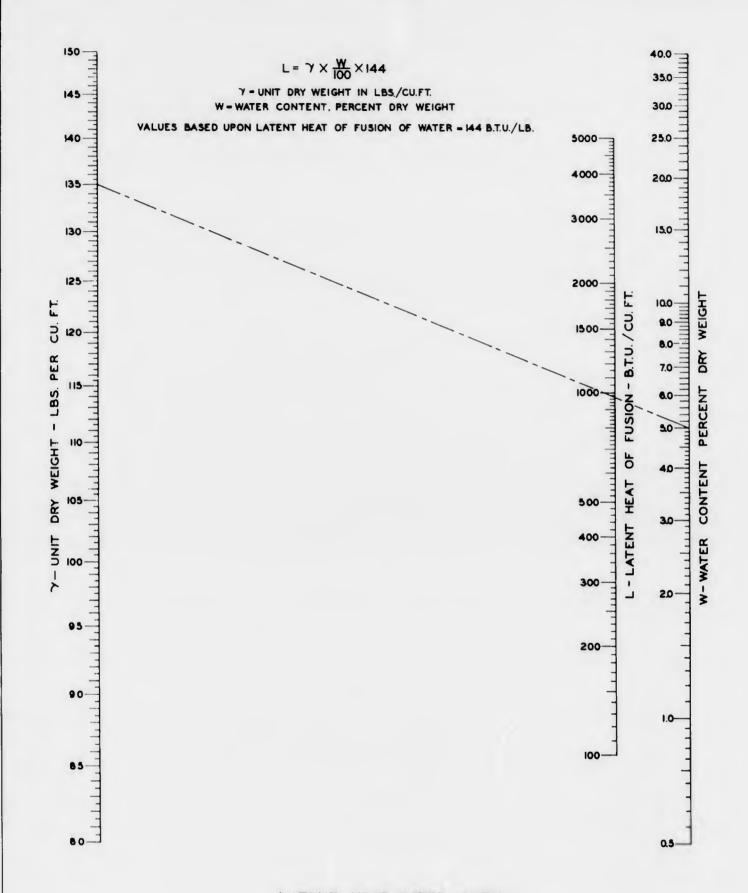
(c) Shanklin believes water content for this value should be about nine per cent.

FROST INVESTIGATION 1945 - 1946

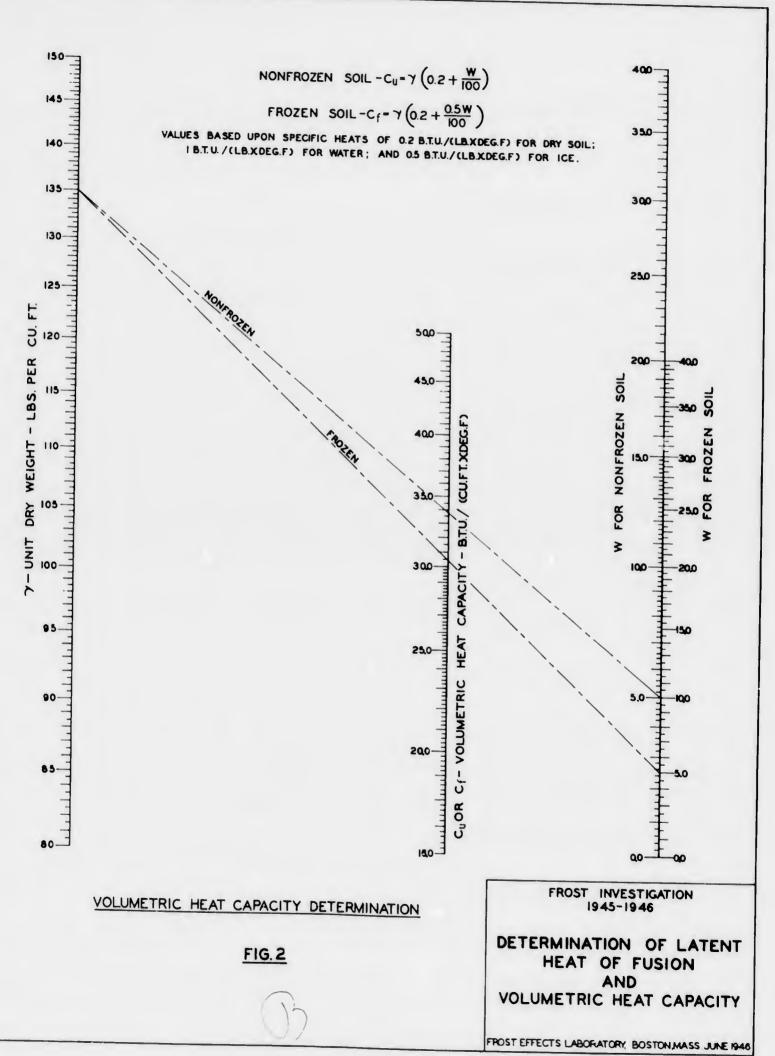
THERMAL CONDUCTIVITY
TESTS

BY OTHER INVESTIGATORS

BOSTON, MASS. FROST EFFECTS LABORATORY , JUNE 1946



LATENT HEAT DETERMINATION



OBSERVED TEMPERATURES AT DEPTHS OF FRO

AIRFIELD	YEAR	TEST	TYPE OF TEMPERATURE	THICKNESS PAVEMENT AN (INCHES	D BASE	CLASS OF	MEASURED DEPTH OF	TEMPERATURE AT DEPTH	DISTANCE BETWEEK NEAREST	
		ARBA	MEASURING INSTALLATION	AT TEMPERATURE INSTALLATION	AT TEST PIT	FROZEN SUBGRADE SOIL	FROST PENETRATION (1NCHES)	OF PROST PENETRATION (°F)	TEMPERATURE INSTALLATION AND TEST PIT (FEET)	AIRPIELD
Presque Isle	1944-45	В	Thermocouple Thermocouple	31 30	37 40 141 31 30 31 30 24 31	GC GC GC GC GC GC	64, 56 70 65 68 48 64, 21, 71 68	32.1 33.0 34.2 33.5 33.0 31.6 32.5 32.1	47 51 33 320 40 45 552 1118 37	cr
	1945-46	A	Thermocouple	31	31 31 31 42 42 41 31 31	60 60 60 60 60 60 60 60 60 60 60 60 60 6	50 70 148 578 588 140 142 50 68 518 65 186 52 50	32.6 32.1 33.4 32.2 34.4 32.7 33.2	277 655 1062 25 15 30	
		c	Thermocouple	30	38 31 35 46 30	88888888888888888888888888888888888888	68 68 142 50 146	37.2 35.0 32.9 31.5 31.6 31.7	8 12 15 20 35 48	
		furf	Thermocouple	-	29 5(TS) 2(TS) 4(TS) 2(TS)	60 60 60 60	18 36 32 50	31.9 32.1 30.7 29.8 31.6	цв 25 10 50 173	Closet Fn11s
Dow	1944-45	В	Thermocouple	2 0	248404844843	CT CT CT CT CT CT CT CT	15 17 17 17 17 17 17 17 17 17 17 17 17 17	35.1 34.9 35.6 35.8 34.9 31.0 35.6 35.6 34.3 33.2	39 182 163 114 15 124 56 148 50 92 166	Herro
					24 40 37 38 37 26 29 26 32	CT CT CT CT CT CT	45年6年7月17年38年6年6年5年30年3月36年19年3月38年3月38年19年2日19年19月3日19年3月38年3月38日19年3日19年3月38日19年3月38日19年3月3日19年3月3日	34.7 34.6 34.7 35.2 35.5 34.9 34.2 35.4	198 80 112 122 140 72 56 80	Treat Bend Farje
		С	Thermocouple	41	78 73 99 73	CT CM CT CT CT	65 178 25	35.5 33.0 35.2 35.8 34.1 34.9	156 38 280 294 62 218	Trace Tellor1
		Turf	Thermocouple	-	30 2(TS) 0	CL CL	48 13 20	35.6 31.8 32.6	-	
	1945-46	D B	Thermometer Thermocouple	142 38	7(TS) 41 37 36 42 37	CL CM CM CL CL	24 50 31 36 14 19	32.5 32.5 32.4 33.1 34.0	124 146 14 26 38 20 32 22	3 lfridge
		P	Thermocouple	20	37 28 28 29 26 26	CL CL CL CL CL	148 36 54 53 26 59	32.0 32.7 35.6 33.1 35.1 35.3	22 117 131 116 100 100	
		Turf	Thermocouple	•	2(TS) 2(TS) 2(TS) 1(TS)	CT CT CT	15 24 24 24 26	33.0 33.3 32.9 33.4	70 14 14 130	NOTE (TS) Ind

URES AT DEPTHS OF FROST PENETRATION

CE E T URB	AIRFIELD	YEAR	TEST	TYPE OF TEMPERATURE	THICKNESS PAVEMENT AN (INCHES	D BASE	CLASS OF PROZEN	MEASURED DEPTH OF	TEMPERATURE AT DEPTH	DISTANCE BETWEEN MEAREST
PIT			AREA	MEASURING INSTALLATION	AT TEMPERATURE INSTALLATION	AT TEST PIT	SUBGRADE SOIL	PROST PENETRATION (INCHES)	OF FROST PERETRATION (°F)	TEMPERATURE INSTALLATION AND TEST PIT (PEST)
	ं दिल्ल	251 1-1.7	1/	The promotor	110	111	°L	11	71.0	1
				man or some or the gapes	3^	100 mm	71 	19 14-17 20 11 16	71.77.77.77.77.77.77.77.77.77.77.77.77.7	7 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -
			*1	in here. O deck	Or.	2 C C C C C C C C C C C C C C C C C C C	- 1 2L	2-11	70. 70. 70. 70.	1.000
			I. ht	and had to place	-	29 19 19 10 10 10 10 10 10 10 10 10 10 10 10 10			70.00	2/15 1-7 1-7 1-7 1-7 1-7 1-7 1-7 1-7 1-7 1-7
	Choice Palls	10/14/5	:	מיתי ביותר ביותר	n	12	719 - du	77	77.	110
		101646		טן יייטטט זי טיניי	 	10 10 10 12	71 71,711 71,711	76 17 17 31	32.5 32.5 32.2 33.0	210 /2 /2 7/ 2/
			2	T'. Imom tor	4	12		1 <u>.</u> 1.	32.1 32.0	2 '
		19/1-17	3	The manter	12	10	, , , , , , , , , , , , , , , , , , , ,	1.0	77 ° 77 ° 70 °	7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7
	larre	104,46 104,46		יין בייסטטריין אין אין אין אין אין אין אין אין אין	1' 1'	17. 17. 14.	71-77	1 o or 1 i	30°°° 20°°° 33°1	2:
ı	Court De 1	2011.15				11,	CL, Co	11, 12	33.0° 3.0°	55
1	Treat Bend Far to	19h 47	A .	Turranter	12	13	/* d.	17	37.0	37
	"a' rtown	1944-445	 	Lighten duttedin	20	7	77, 4-13, 14	1/	31.7	15
		, , , ,	Turf	The recouple	10	10 10 -(TS)	or-ci,or or-ci,ci or-ci,ci	31 32 112	31 • 1 31 • 0	100 60
ı	Trace	1 (7	ملامارا دفسل ملاماري	12	l _{i2}	n a		3. •' 31.5	13 16
			D	בין פי פשמט בין דיי	ي م	35 30	-11	10	32.0 31.0	η. 30
	79 ⁴ fo r 1	10/4/1/2	î.	רבול ומה השמת מות מות מות מות מות מות מות מות מות מ	27	21: 21 ₁	7 : 7 :	21;	32.1 32.2	21
			ņ	"larracinator	j.	25 10 21 21 21	20 20 20 20 20 20 20 20 20 20 20 20 20 2	5. 50 50 54 54 54	20.6 30.0 31.0 31.0 30.1	17 33 33 0 15 37 51,
	3 ltridge	101, 40	Å	Thansomter	24	21 21: 2: 2: 2: 2: 2: 2: 2: 2: 2: 2: 2: 2: 2:	:5, ::12-5: ::2 ::2 ::3	2!; 3!; 35	30 ° 3 32 ° 5 30 ° 7	210 250 220 230
		171 -17	4.	Ting to to hope	26	5.5		31 50	72. 70.77	777

NOTE

(TS) Indicates Topsoil.

NEW ENGLAND DIVISION CORPS OF ENGINEERS, U. S. ARMY BOSTON, MASSACHUSETTS

DATA REPORT OF FROST INVESTIGATIONS FISCAL YEARS 1943 - 1949

APPENDIX C

REPORT ON MATHEMATICAL STUDIES OF THERMAL CHANGES
IN A SOIL MASS
1945 - 1946

FROST EFFECTS LABORATORY
JUNE 1949

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dinni pie.	Nm A . Mathematical Chidica of Themes	
SOPPLEME	NT A - Mathematical Studies of Thermal Properties A-1 thre	u A-3

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LIST OF FLATES

PLATE	TITLE
C-1	Prediction of Frost Penetration
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A-2	Curves for Determination of erf (B) and B

REPORT ON MATHEMATICAL STUDIES OF THERMAL CHANGES IN A SOIL MASS.

SYNCPSIS

A comprehensive, mathematical study of the thermal conditions induced in a semi-infinite, homogeneous, isotropic, soil mass by variations of the surface air temperature is presented in this report. The studies are presented in the form of 17 problems. The problems deal with the determination of the thermal diffusivity, depths of frost and melt penetration, effect of radiation and surface film and the effect of an insulating layer. A series of formulae were developed to predict the depth of frost penetration and several of these were selected to predict the depth of frost penetration for comparison with actual field measurements. The theoretical depths of frost penetration obtained using the developed formulae compare reasonable well with observed depths from field investigations.

I. INTRODUCTION

- 1-01. Authorization. The frost investigation program was authorized by the Chief of Engineers by letter to the Division Engineer, New England Division, dated 4 August 1945, Subject:

 Frost Investigation, Fiscal Year 1945 1946. The mathematical studies reported herein are a part of this program.
- 1.-02. Purpose. The purpose of these studies was to develop mathematical solutions for problems of heat transfer through soils so that these solutions could be used to supplement and analyze data from field and laboratory frost investigations.
- 1-03. Scope. This report presents the solution of 17 problems in a condensed form with examples. A detailed development of each equation is on file in the New England Division Office.
- 1-04. Acknowledgement. These studies were principally performed by Dr. L. A. Pipes, Harvard University.

II. MATHEMATICAL STUDIES

2-01. General. The depth to which a pavement, base, and subgrade will be frozen during a winter will depend principally upon the magnitude and duration of freezing air temperature, the thermal properties of the materials and the subsurface temperature conditions at the start of freezing. All these factors have been used in this study. Methods of predicting the depth

of frost penetration are presented which give results reasonably close to the measured values at the various test areas.

Observations of depths of freezing have been made over a period of one to three years at 17 airfields. The results of these observations together with pertinent data influencing freezing are summarized on Plate C-1. In addition, there are also tabulated the values for predicted depth of freezing based upon equations (83), (93), (154), (158), developed in Supplement A and from the design curve which is based on the correlation of the Freezing Index and measured depth of frost penetration at several airfields. This correlation is shown on Plate C-2 for both portland cement and bituminous concrete pavements. The equations selected from the mathematical problems presented in Supplement A of this appendix were considered to represent the principal variations of assumptions in the most usable form.

- 2-02. Values of Thermal Properties Used in Equations. The thermal properties used in calculating the depth of frost penetration were determined in the following manner:
- a. Thermal Conductivity, k. The thermal conductivity used for computations of depth of freezing is for all cases 1.3 Btu/(ft)(Hr.)(OF). Based upon tested values for thermal conductivity, as discussed in paragraph 2-03 j Appendix B this value may be somewhat high as an average value for all soils to a depth of 16 feet; however, it is believed to be a reasonable value for the pavement, based, and subgrade soils which are frozen or unfrozen.
 - b. Mean Soil Temperature, Vo. The mean soil

particular location. At a depth of about 16 feet below ground surface the amplitude of soil temperature change approaches zero. Values for the mean air temperature in the United States are given in Figure 1, Plate 3, of Volume I.

- water in the voids will be frozen and thus the latent heat may be determined from the water content using graph shown in Fig. 1, Plate B-16, Appendix B. For cohesive soils, there will be a portion of the measured water content (in percentage of dry weight of soil) which will not freeze at 32°F as discussed in paragraph 2-04, Appendix B. Thus, for some soils, not all the water content contributes latent heat. On Plate C-1 it is considered that all of the water freezes, the value of latent heat as tabulated being determined from Figure 1, Plate B-16, Appendix B.
- d. Volumetric Heat, C. The average volumetric heat at each location was determined using the equation given in paragraph 2-05, Appendix B. Values for the volumetric heat for each different soil were determined using the average water content and unit dry weight as tabulated on Plate C-1 and Figure 2, Plate B-16 of Appendix B. Soils within the depth of freezing were considered to be totally frozen in determining the average volumetric heat.
- 2-03. Analyses of Problems. Problems 1 to 5 inclusive, Supplement A to this appendix, are presented in a form

thermal diffusivity and apply only to cases where temperatures are above freezing or where no latent heat is involved. The accuracy of determinations of the values of thermal diffusivity "a", in the manner outlined is dependent on the accuracy of temperature measurements at various depths, at the same or different times. With the exception of Problem 3, the results involve the determination of the slope of the temperature curve at any given point which introduces additional errors

The remaining problems, with the exception of Problem 15, are concerned with the depth of freezing, "x". Problem 15, deals with the effect of ground film and radiation. The effect of neglecting ground film and radiation in the prediction of frost penetration, gives values which are too large, but the percentage of error decreases with the increase in penetration. Problems 5 and 7 neglect the latent heat of fusion; both problems assuming that the air temperature is periodic over a sufficiently long period so that the interior soil temperature is also periodic. Problem 7 is further complicated by assuming that the soil is composed of two layers, the solution being obtained only by cut-and-try method. Problems 8 and 9 consider latent heat but neglect volumetric heat. Problems 10 through 14 consider both latent heat and volumetric heat. Problem 16 assumes that the temperature of the soil varies uniformly with the depth. Problem 17 considers the effect on the depth of freezing of an insulation layer placed over the soil.

a. Discussion of Equations. Equations (83), (93), (154), and (158) from Problems 9, 10, 16 and 17, respectively were selected for study and comparison with observed depths of frost penetration. The results of computations for these four formulae for all airfields are contained on Plate C-1 together with all pertinent data necessary for the computations. Equation (83) $(x = \sqrt{48kF/L})$ gives values which are consistently too high. Equation (93) (x = $\sqrt{\frac{48 \text{ k F}}{L \neq C \text{ (v_0 - 32 } \neq \text{F/2t})}}$ gives values of "x" which, though lower than those for equation (83), are still consistently too high. Equation (154) $(x = \sqrt{\frac{24 \text{ k F}}{L \neq C (v_0 - 32 \neq F/2t}})$ gives values of "x" which range from higher to lower than the observed values. Assuming that the temperature at the surface of the pavement is essentially the same as the temperature at the bottom of the pavement, a column is contained in Plate C-11 in which the thickness of pavement is added to the predicted depth as determined by equation (154). Equation (158) $(x_R = -\frac{d}{2} / \sqrt{(\frac{d}{2})^2 / \frac{24 k F}{L / C (v_0 - 32 / F/2t)}})$ has been used in areas observations were made with a turf cover. The results of these calculations bracket the observed depths but in general the dispersion is great, with the average very high. Also contained in Plate C-1 are predicted depths using the design curve (Figure 2, E.M. Part XII, Chapter 4, Mar. 1946). The values of "x" thus obtained bracket the observed depths with the greatest percentage within 6 inches of the observed depths. Comparison of the relative merits of each formula is given below:

		s of Pre served D		Fer Cent of Observations
Equation	Avg.	Max.	Min.	Within 6 ins.
83	1.60	3.39	1.09	6
93	1.32	2.69	.83	29
154	.94	1.89	•58	42
154 / pave.	1.10	2.31	.67	54 63
Dosign Curve	1.08	1.93	•67	63
158 (Turf)	1.95	5.67	.92	50

In all calculations, the average value of 1.3 BTU/(ft)(°F)(hr) was used for the thermal conductivity and the full value of the latent heat as derived from the nomogram in Figure 1, Plate B-16. Because of the inability to determine for each separate layer of soil, the exact quantity of water which froze, it was assumed that all the water to the maximum depth of freezing was frozen, which is contrary to the data presented and analyzed in paragraph 2-04.

4.55	TEST		MEAN	FREEZING	FREEZING		MENT			BASE (ļ	51	BGRADE (1)
SITE	AREA	YEAR	AIR TEMP	OF DAYS	DURATION DAYS	TYPE	THICK.	CLASS.	THICK.	CONTENT	DENSITY LBS /FT.3	(III) CLASS.	THICK.	CONTENT	DENSITY LBS./FT
	• • • •		(A ⁰)	(F)	(t)		INCHES		INCHES	% (w)	(7)		INCHES	% (w)	(7)
	п	1943 - 1944	42.5	1815	108	B. C.	4.0	G W	17	9.9 4.2 p	135 135 M	CL	-	305	81
	XX 10 XX			1745	130	P.C.C.	7 0	G W	15	9.2	133 W	le r	3.8	10.3 H	107 ¥
	A	1944 - 1945	ĺ	1445	104	P.C.C	7. 0	GW	15	11.1	11.9	Cr	42	28.4 (4)	9.2
	•			1445	104	B.C.	3.5	GW	31	0.9	131	[cc	-	25.4 NF	103 4
DOW	c			13.45		B.C.	3.5	GW	42	9.2	121	CL		24.2 NF	92 N
	TURF			1445	104	B.C. T.S	3.5	G W	42	9 2	121	CL	-	10.3	109
	0	1945 - 1946		1420	••	B.C.	3.5	GW					_	[23.4 15.2 NF	112 N
	E C			1420	••	B.C.	3.5	GW	36	13.5 *	136 4	CL	-	17.7 % 22.4 #	101
	F			10 60	75 74	P.C.C	3.5	G W	20	10.1 # 8.1 #	138 4	CL		22.4 # 22.5 #	101
	A	1944 - 1945	390	2060	11.5	7 S.	7.0	GW	33	6.3	134	C L	-	22.4 (+)	101
				2000	11.5	a c	4 0	C.A.	4		- 1	GC	_	18.2 NF	113 N
PRESQUE	TURF			140	25	T. S.	5.0	[cw	30	0.5	134]	GC		14.2 NF	11 4 N
ISLE	A	1945-1946		2240	126	PCC	7.0	GW	32	10.4 #	133 #	GC		[15.9 NF	IIO N
	С			5530	120	B C	3 5	[C.R.	3.5	6.3 #	- 1	6 6	-	17.0 K	117 1
	0			5570	12 4	B, C.	3 5	C.R.	3.5	10.5 **	4 13 2	GC	-	18.4 ##	120
	TURF	1945 - 1946	46.5	2240 925	12.6	P.C.C	5.0	GW		4.6 %	110 #	GC	-	13.6 (4)	115
BEDFORD				345	67	B.C.	5.0	GW GW		4 8 (4)	119 (4)	3 W	-	5.3 #	103
OTIS	À	1944-1945	4 0.7	300	60	3.6	6.0	-	-	4 8 (4)	- (+)	SFOR	30	9.5 # #	4 12 4
0113												S.P.ML	2 4	23.5 K #	## 87 II 9
HOULTON	A	1944 - 1945	405	1605	107	8.C.	1.5	S.CEM.	•	14,3	113	GP	30	0.6	126
	A	1944 - 1945	46.0	1210	• •	B.C.		1				GC	-	10.1 NF	13.4 NF 11.6
				.2.0	•	B.C.	2.5	C.R.	15	5.3 ×	# 141	SF	3 6	21.1 ≱ 0.1 †	108 pt
	•			1245	• •	6 C	2.5	C.R.	1.]	L CL	-	21.1 4	108 %
TRUAX	C	1945-1940		1245	9.7	P.C.C.	6.0	GF	2.5	7.7 12.7	112	CL	-	3 0.0	8 2
INUAL				1020	• 3	B. C.	2.5	[C.R	1	6.0 K	* 140]	SF	3 6	27.5 A	95 W
	С			1060	100	P.C.C.	7.0	GF	30	10.1 #	# 129	LCL	3.5	27.5 % 35.5 %	95 a.
	D			1055	••	B.C.	2.5	C.R.	20	-	- 1	LSF	:	20.4 +	113 %
SELFRIDGE	•	1945-1946	47.6	045	0.1	P.C.C.	10.0	GF	3 0	5.6 *	# 13 0 J	ML	13	18.0	112
- LEI KIUGE	A	1944 - 1945	47.5	960	104	P.C.C.	7.0					L C L	3 0	22.3 # 36.2 #	100 %
				•••	71	B.C.		G F	7	6.7 ×	135 %	CH	3.2	18.1 M	1081
PIERRE	TURF			•••	••	T. S.	5.5 4.0 (I)	GF -	!	6.4 *	140 #	L C L		14.1 # 12.1 #	97
LIENNE	A	1945-1946		1023	••	P.C.C.	7.0	[G!	,	7.4 %	#1427	LCH	27	18.7 % 21.6 %	
	с			1025	••	B.C.	• 0	[GF		16.0 M	4150]	CH	-	21.7 KK	97, 97,
	A	1944-1945	46.2	915	7.6	B.C.	2.0	Lec_	10	13.6 4	# 115	CL OR	-	23.0 %	95 %
SIOUX	A	1945-1946		1220	0.2	B.C.	2.0	Le c Fe r	10	7.0 美 16.7 典 7.1 抗	# 11.2 # 13.71	CLORCH		30.9 #	44
FALLS	•			1310	100	P.C.C	6.0	Ler	12	214 #	1 103	SF-CL	-	290 K	94 9
												CL	11	33 0 % 27.5 % %	8 5 ¥
	A	1944-1945	42.5	860	6.2	P.C.C.	8.0	•	-	•	-	SF-OL	32	130 (H)	97 (112 9 97 #
				.40	5.9	B.C.						OL-CL	20	23.8 # 3.8 #	1200
WATER-					, ,		5 0	GF	•	4.9 #	130 #	SF-OL OL-CL	2.2	14.1 # 22.1 #	103 M
TOWN	TURF				0.4	T. S.	9.0	-	-	-	-	SF-OL	5 2	4.4 4	1204
	A	19 25-1946		1715	• •	P.C.C.	0.0	-	-		-	GP SF-OL	2.4	4 4 4	1364
												OL-CL	2.2	23.6 (4)	112 G 87 G
		1.		1715	• •	B.C	5 0	GF	1 2	4 9 (44)	139 (4)	ST-OL	30	14 1 (+)	103 4
FARGO	A	1944 - 1945	39.2	1305	7.0	B C	1,5	S.CEM CL-SF	0.5	11 1 K	# 122 # 120	OH-CH	•	26.7 A	1204
TANGO	^	1945-1946		2465	125	8 C	1,5	SCEM	8.5	11.1 (4)	12241	OH-CH	12	31.1 ± 26.7 (+)	994
	^	1944-1945	550	3.0	3	PCC	7.0	SWORSF	- 11	10 8 (4) 2 0 H	120 #	SF-CL	35	311 (+)	100 #
GREAT												CL-SF		131 #	
BEND	A	1945-1946		130	11	PCC	7 0	5 F	5	2 0 (+4	120 (44	SF-CL	35	17.0 #	102 m
												CL-SF		131 (4)	1024
BISMARCK	A	1944-1945	390	12 8 0		B C	4.5	3 C	•	4,7 #	130 #	CL-ML	49	17 0 43 16 8 #	1004
	X	1944-1945	47.4	355	5.5	PCC	7.0	-	-	-	•	SF-CL	2.0	11.4 8	87 A
CASPER												37	12	0.0 H	120 4
	•			335	5 0	0 C	5.0	GW	7	3 8 #	131 #	SF-CL	1.2	53 6	111 A
EA1811611	Α	1944 - 1945	9 2 0	3 6 0	7 0	PCC						37-CL	3 2	3 5 4	109 A
FAIRMONT	A						0.0	•	-	•	•	CH	2.5	28 0 p	80 +
	•	1944-1945	5 5 0	2.5	7	8 C	1.5	S C	10	5 0 A	12 0 M	CL	3.0	150 4	107
GARDEN														1 0.1 A	95 A

- 7			BASE (III)		9,11	BGRADE (II	1)			(iv) [
ĸ.	(III)	THICK.	WATER	DENSITY	(111)	THICK.	WATER	DENSITY	AVG. VOL.	AVG. LATENT HEAT	OBSERVED DEPTH OF	P	REDICTED	DEPTH	OF FR
ES	GW	IN INCHES	(w)	(7)	CLASS.	INCHES	(W)	LBS./FT.	B.T.U / (FT, 3X°F) (C)	(L)	FREEZING IN INCHES	EQ. 83	EQ.93	EQ.154	EQ.154 PLUS PAVE.
	e w	36	4.2 H	135 A	CL	3.0	10.3 H	107 #	3 9.5 4 2 .6	2020 1300 3000	48 50 48	106	8.7	444	>484 65
	G W	15	11.1	11.0	C C C C C	4.2	28.4 fb)	92 (L)	44.1	3035	54	• • •	50 4	41	444
	G W	31	0.0	131	Cr	-	25.4 NF 31.1 24.2 NF	103 NF	3 9.2	2310	52	7.5	••	47 4	304
	G W	42	9.2 9.2	121	CL	-	19.3	100	4 0.0 4 0.0	1960	0 0 0 2	7.0	► 6 7 I► 6 9	4.7	50
	GW	40	- 13.5 W	- 128 ×	C L		23.4 15.2 NF	112 NF	30.0	3300	2.4			41	5.2
	GW GW	3 6	10.1 A 10.1 #	136 a	CL	-	22.4 # 22.4 #	115 # 101 #: 101 #	42.1 41.5 41.6	2530 2205 2110	5 0 4 8 4 4	71	0.2	444 • 47 4	► 474 504
4	G W	33	8.1 #r 	136 #	C L G C	-	22.5 #	110 *	44.6	2700 3210	3.4	50 4	► 5 2 4	42 4 37	44
	[C.R.	4		- 1	G C	-	17. 0 16.2 NF	10 8 11 3 NF	36.7 37.4	1945	70	**	0.5	60	▶ 174
	[e.w	30	0.5	13.4	GC	-	14.2 NF	114 NF	39.1	2790	13	''	• 7	4 2	▶70◀
	GW [C.R. [GW	3.5	10.4 A	133 4	G C	-	15.6 NF 17.0 M 13.6 #	110 NF] 117 # 115 #	411	2450	7.	 • i	► 80 4	57	64
	C.R.	2.6 3.5 2.5	6.3 # 10.5 ##	# 132 ## 142	GC	-	16.4 ##	120 ##	440	2740		65	7.0	64	72 d
H	G W	10	4.8 (6)	110 (4)	G C SW SW		13.6 (H 5.3 #	115 (4)	37.5 26.0	2230	8 O 4 O	10	7.	5.4	
+	G W	14	4 . 8 (4)	119 (4)	SFOR	3.0	5 3 (43 5 3 (43 9 5 # #	103 (4) 103 (U) 4 % 12 4	26.0 26.0 27.4	805 805	2.6		7 0	35	9 0 9 4 4 9
					ML 3 P G P	2.4	23.5 4.4	11.9	57.4	2205	.2 6	3.4	31 4	554	204
	S.CEM.	•	16.3	113	GF GC	30	0.8	126	3 4.6	2100	49	• 1	72	₱ 31 4	254
	C A	15	5.3 A	H 141]	CL 3F	23	21.1 MF	NF II 6	30.0	5362	4.0	• •		41	314
	CR GF	18	7.7	[22]	r çr		21.1 to 27.0	100 %	4.3.6	1005		7.6		45	
	GF GF	4.0	12.7	115	L Cr	2.6	300 275 #	****	3 6 6	2070	5.5	7.4	0.2	4.4	₽504
	GF	3.0	6.0 M	# 140] # 129	SF CL CL	3 6	19.8 % 27.5 %	11 8 #		2430	4.	6 2	5.3	3 7	▶474
	C.R.	2 C	-	- 1	2 F	32	35.5 # 20.4 # 25.9 #	1137 1	43.0	2550	4.0	1 7 1	534	37	444
T	GF	3 0	11.5	<u> </u>	ML SF	13	10.0 22.3 ×	112	44.6	2 2 0 0	3.5	3 0	43	304	404
H	GF	,	8.7 д	135 #	CL	32	36.2 g	100 H 85 (1)	37.0	2150					
(I)	G F	:	6.4 #	140 #	CCF	-	23.5 Å	90 mJ	32.9	1040	42	0.2	12	394	484
	[67	7	7-4 #		CH	27	12.1 x 16.7 % 21.6 %	00 # 07 #	33.6	2025	•				
	[ct_	7	19.9 m 0.7 m	* [6	CL		21.7 共长 12.7 共	9 7 A A	30.0	1070	44	74	124	37 > 45 4	444
	Cor	10	7 0 %	p 132	CL OR	-	23.6 A	::::]	400	2040	40	3.6	4.9	354	> 37 €
	ود	12	7.1 H 21.4 H	# 137 # 103	CLORCH SF-CL CH	4.0	200 H	*; #]	36.5	3020	4.7	4 0	334	•	40
1	-	-	-		C L S C	11	27.5 M K	9.5 th 9.7 th	3 6.0	3 0 0 0	4.2	3.4	30	3.5	>414
					OL-CL GP	3.5	14.6 K 23.6 H	112 B 07 A 120 (H)	3 0.7	2325		3 0	5 2	P 37 4	4.5
	GF	•	4.9 g		SF-OL OL-CL	22	22 1 4	103 M	20.0	1970	42	. 2	5.5	314	444
		-	-		SF-OL	5 2	4.4 a	12 0 th)	50.6	1240	42				
		-			SF-OL OL-CL	24	23 8 60	11 2 445 97 441	29.7	2010	75 TH	••	▶77 ◀	• •	• •
	GF 3.CEM	12	4 9 644		GP-OL	30	3.8 (+) 14.1 (+) 4.4 (+)	120 (+1	20.6	1330	79 TH	100	• 0		
	S CEM CL-SF S CEM	14	11.1 K	# 122 # 124]	OH-CH	•	26.7 A	::::	4 3.2	2905	4.6	0.5	5 0	414	P434
	CL-SF	11	10.6 (4)	122 (H) 124 (H)	SF-CL	12	26.7 (+) 31.1 (+)	9 9 (4)	42.3	2395	72 TH	6 F	→ 73 ◆	52	11
					CL-BF		131 4	102 4	36.4	325	13	3 7	18 4	P 13 4	20
	3 F	5	2 0 (+4	120 14	SF-CL OR	33	15.1 (+)	1000	36.2	1000	2 4 TH	274	21 4	13	P 22 4
1	3 C	•	47 #	130 #	CL-SF CL-ML		13 ((+)	1024)							
+	-	-			SF-CL	29	10 6 p	14	267	1005	**	1	7 0	• 10 d	
	GW	7	3 0 #		37	12	9 9 W	11 4 A	20.2	1030	2.0	4.2	364	104	>114
1					37-CL	3.5	6.2 # 3.5 # 6.2 #	117 #	29.7	070	10	• 1	4.7	3 4	3.0
1	3 C	-	3.0 5	•	CH	2.5	230 p	117 6	3 0 1	3550	1.5	3 i	2.0	▶ 20 •	2.6
		10	3 0 M	128 %	C L	3.0	15.0 A	107	3 6.2	•••	12	15 4	▶ II ◀	• •	14
1					CH-CL		163 #	.,,							
						()									
						1	1								
						1		1							

	AVG. LATENT HEAT	OBSERVED DEPTH OF	P	REDICTED	DEPTH	OF FR	EZING	IN
,	BTU./FT.3	FREEZING			IIICITES	EQ.154		2501011
	(L)	IN INCHES	EQ. 83	EQ.93	EQ.154	PLUS PAVE.	EQ.158	DESIGN
	1300	50	108	62 67 56	444 82 41	0 4 8 4 0 5		5 4 ◀ ▶ 5 7 5 6
	3035	54	6.5	504	41	414		▶53 ◀
	2310	52	7.5	**	474	50 ◀		▶83 ◀
l	1995	60 62 24	70	F 6 7	47	50	37	51
ı	2530 2205 2110	5 0 4 8 4 4	71 76	9.5	444	► 474 504		>53 ◀
	2700	54	50 4	> 254	37	44	37	47 4
	1945	70	**	8.5	0.0	▶674	3,	6.3
	2790	13	••	6.7	6.2	▶70◀	12.4	6.3
	2450	7.	9 i	▶ 80 4	3.7	0.4		6.5
	2740	••	0.5	7.6	64	724		P 65 4
+	2230	40	9.6	7.6	34		P564	
1	805 805 2285	2.6	9.2	70	35	54		D 4 1 ◀ D 3 8 D 2 7 ◀
		-2 6	34	31 4	554	2 4 4		P264
+	2160	49	81	78	P 31 4	524		3 6
1	2365	4.6	• •	3.0	41	314		F414
	1805	5 •	7.6	• •	4.5	• 5		▶50 ◀
	2070	4 6	7.4	9 2 5 3	3 7	P504		▶50 4 ▶45 4
ı	2550	4.0	6.1	114	3.7	444		>444
ļ	2200	3.5	7.6	P 634	44	• •		4.6
l			3 0	43	304	404		▶36◀
	1840	42	6.2	9.4	394 > 37	464		P114
	16.60	•				42	34	3 0
	1870	44	7.4	624	37 > 45 4	444 51		P484
t	2640	40	5.6	4.9	354	▶ 374		P414
l	3020	4.7	. 0	334	3.0	40		P414
	3 9 6 0	4 2	5 4	5 0	3 5	▶414		6.1
1	2325	3.6	5 6	9 2	P 37 4	45		42 4
	1970	4.2	• •	5 5	394	444		P414
	1240	4.2					43-6	
	2010	75 TH	••	▶77 ◀	5 5	4.4		5.0
	1330	79 TH	108	• •	0.4			
1	2005	4.6	0.5	5 0	424	P414		514
-	3305	72 TH	•1	▶ 73 4	5.2	13		114
		13	37	10 4	→ 13 4	30		11 4
	1 6 0 0	2 4 TH	274	£1 4	15	→ 22 ◀		17
	1905	4.6	7.6	7 0	P 104	14		▶80 •
	0 5 0	30	4.8	304	204	P 114		▶ 87 ◀
		1 •	• 1	47	34	36		▶ 2 7
	3550	1.5	a (2.6	> 20 4	2.0		11
	••0	12	15 4	▶ II ◀	• •	14		14
							1	

EQUATIONS:

93
$$x = \sqrt{\frac{46 \text{ k f}}{1 + c \left(v_0 - 32 + \frac{f}{2t}\right)}}$$

154 x =
$$\sqrt{\frac{24 \text{ k F}}{\text{L} + \text{C} \left(v_0 - 32 + \frac{\text{F}}{2\text{L}}\right)}}$$

158
$$x = -\frac{d}{2} + \sqrt{\left(\frac{d}{2}\right)^2 + \frac{24 \text{ k F}}{L + C\left(v_0 - 32 + \frac{F}{2t}\right)}}$$

DESIGN CURVE - FIG 2, E.M. PART XII, CHAPT. 4, MARCH 1946

X - DEPTH OF FROST PENETRATION IN FEET.

K = THERMAL CONDUCTIVITY IN STU /(FT)(OF)(HR)

F = FREEZING INDEX IN DEGREE-DAYS

L = AVERAGE LATENT HEAT IN STU /(FT)

C = AVERAGE VOLUMETRIC HEAT IN STU /(FT)(OF)

V₀ = MEAN ANNUAL AIR TEMPERATURE IN OF

t = DURATION OF FREEZING INDEX IN DAYS

d = THICKNESS OF INSULATION LAYER IN FEET

AN AVERAGE VALUE FOR THERMAL CONDUCTIVITY (R) = 1.3 B.T.U /(FT)(OF)(HR.), 15 USED THROUGHOUT THESE CALCULATIONS

VALUE FOR "d" USED IN EQUATION IS 8 IS THICKNESS OF TOPSOIL IN FEET.

NOTES:

- (I) PAVEMENT TYPES ARE AS FOLLOWS: B.C. BITUMINOUS CONCRETE. P.C.C. PORTLAND CEMENT CONCRETE. T.S. TOPSOIL (TURFED AREAS ONLY).
- (II) VALUES USED FOR WATER CONTENT AND DENSITY ARE FOR FREEZING PERIOD WHEN AVAILABLE. EXCEPTIONS ARE NOTED AS FOLLOWS:

 H VALUES FOR NORMAL PERIOD.

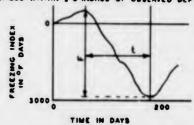
 H# VALUES FOR FROST MELTING PERIOD.

 (4) ASSUMED VALUES.

 NF VALUES FOR NON-FROZEN BUSGRADE SOIL (FREEZING PERIOD.).
- (III) BOIL CLASSIFICATION FOR AIRFIELDS EXCEPT AS FOLLOWS:

 C.R. CRUSHED ROCK,
 S.CEM. SOIL CEMENT,
- (IV) DEPTHS OF FREEZING OSTAINED FROM TEST PIT OBSERVATIONS EXCEPT AS FOLLOWS: TH DEPTHS OF FREEZING OSTAINED FROM THERMOMETER AND THERMOCOUPLE OSSERVATIONS.
- CLOSEST VALUE TO OBSERVED DEPTH.

 VALUE WITHIN & SINCHES OF OBSERVED DEPTH.

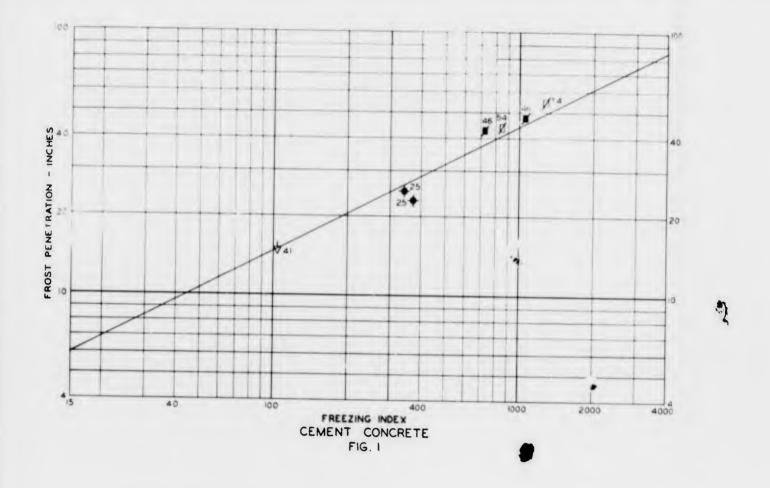


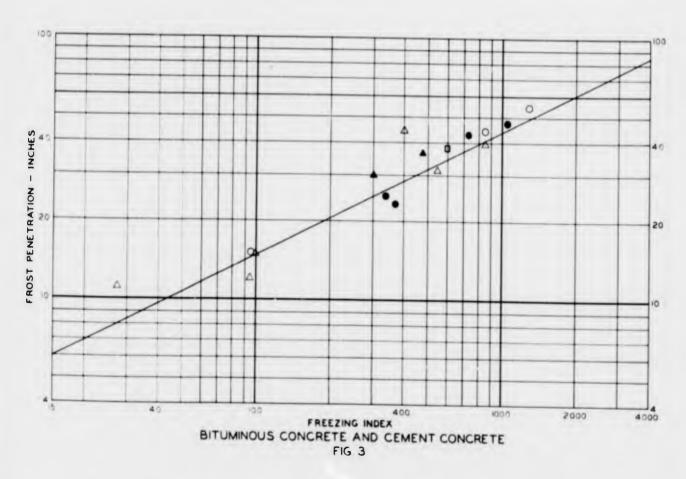
RELATIONSHIP SETWEEN F& &

FROST INVESTIGATION 1945-1946

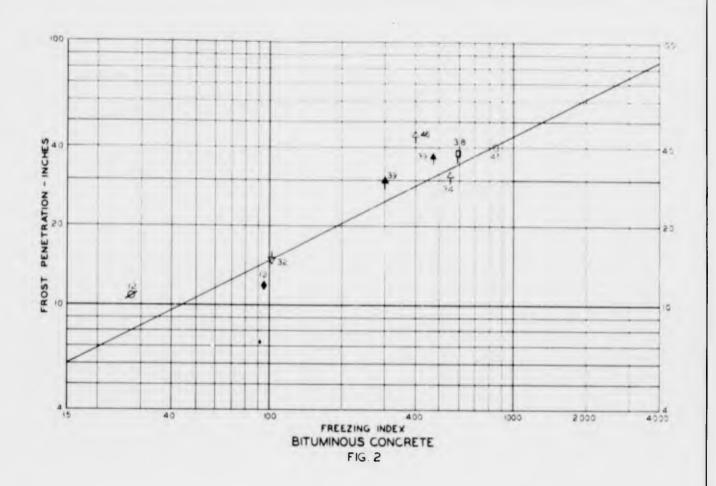
PREDICTION OF FROST PENETRATION

FROST EFFECTS LABORATORY, BOSTON, MASS. JUNE 1946









LEGEND ,

1944-1945	1945-1946	1946-1947	
$\dot{\uparrow}$	+	_	PRESQUE ISLE
4		P	DOW
	_	<u>-</u>	WATERTOWN
Ø	#	-	TRUAX
-	•	марыю	SIOUX FALLS
-	+		BEDFORD
Ø	-		GARDEN CITY
Δ	A	٥	BITUMINOUS CONCRETE
0	•	•	CEMENT CONCRETE

NOTES -

NUMBERS NEXT TO PLOTTED VALUES SHOW COMBINED THICKNESS OF PAVEMENT PLUS BASE.

--- DESIGN CURVE FROM ENGINEERING MANUAL PART XII CHAPTER 4, JULY 1946

FROST PENETRATION IS IN NON-FROST SUSCEPTIBLE MATERIALS ONLY

FROST INVESTIGATION 1946-1947

CORRELATION BETWEEN FROST PENETRATION AND FREEZING INDEX

FRIST EFFECTS LABORATORY BISTON MASS JAN 948

PLATE C-2



APPENDIX A MATHEMATICAL STUDIES OF THERMAL PROPERTIES

Symbols and Definitions

The following symbols are used in the mathematical studies:

Symbol	Definition	Unit
A	Temperature difference between annual mean air temperature (v _o) and freezing temperature (32°)	o _F
a	Thermal Diffusivity = k/C	ft ² /hr.
В	Amplitude of air temperature change for yearly cycle = 1/2 range	o _F
С	Volumetric heat	BTU/(ft ³)(^o F)
c ₁ ,c ₂ ,c _n	Volumetric heat of layers 1,2,n respec- tively	BTU/(ft ³)(^O F)
c _f	Volumetric heat in frozen state	BTU/(ft ³)(^o F)
Cu	Volumetric heat in non-frozen state	BTU/(ft ³)(^o F)
С	Specific heat	BTU/(Ib)(^O F)
d ₁ ,d ₂ ,d _n	Thickness of insulation layers 1, 2,n, respectively	ft.
E	Surface coefficient = k\psi	
е	Base of natural (Napierian) logarithms = 2.718 +	
F	Freezing index	^O F days
Н	Total heat given up by soil = Qt	BTU/ft ²
h	Depth below ground surface	ft.
i	Thermal gradient	^O F/ft
ĸ	Constant of integration	
k	Thermal conductivity	BTU /(ft)(^O F)(hr)

Symbol	Definition	Unit
k ₁ ,k ₂ ,k _n	Thermal conductivity of layers 1,2,n respectively	BTU/(ft)(^O F)(hr)
k _f	Thermal conductivity in frozen state	BTU/(ft)(^O F)(hr)
k _u	Thermal conductivity in non-frozen state	BTU/(ft)(^O F)(hr)
L	Latent heat of fusion of water in soil	BTU/ft ³
Q	Rate of heat flow from ground surface = ki	BTU/(ft ²)(hr)
R	Thermal resistance = $\frac{d_1}{k_1} + \frac{d_2}{k_2} + \dots + \frac{d_n}{k_n}$	BTU/(^O F)(hr)
s	Thickness of upper soil layer	ft
T	Time period of temperature change for I year	365 days
t	Time increment = duration of freezing index	day
V	Temperature amplitude in soil at depth "h"	o _F
^V f	Average air temperature during period of freezing	o _F
^v o	Average soil temperature = mean annual air temperature	o _F
v _p	Constant suddenly impressed air tem- perature	o _F
^v s	Variable air temperature during period	o _F
x	Depth of freezing = depth of melting for rising soil temperatures	ft
×R	Depth of freezing, when soil is overlain by an insulation layer	ft

Symbol	Definition	Unit
z	Elevation of a point from the boundary layer - measured in opposition to "h"	ft
β	Growth coefficient of melted layer = $\frac{h}{2\sqrt{24at}}$	
ω	Parameter = $2\pi/T$	
N,Ζ,θ	Dimensionless parameters for simplifi- cation of equations	
P,G,m,y, δ , ψ	Parameters for simplification of equa- tions	
φ	Mean temperature gradient in period Δt	^O F/ft
In	log to the base "e"	

PROBLEM NO. I

Given a homogeneous, isotropic soil mass of semi-infinite extent, with its initial temperature at temperature " v_0 ". Its surface temperature is suddenly changed to temperature " v_p ".

(a) Find the thermal diffusivity "a", by measuring the temperature gradients at different times, neglecting latent heat.

The temperature at any depth "h" at time "t" is

$$v_{(h,t)} = v_0 + (v_p - v_0) [1 - erf(\beta)] \dots [1]$$

where the erf (β) is the probability-integral, also known as the Gauss "error-function" of β , and can be expressed as

erf
$$(\beta) = \frac{2}{\sqrt{\pi}} \int_{0}^{\beta} e^{-u^{2}} du \dots [2]$$

At any time "t", the temperature gradient "i" can be expressed as the slope of the temperature

At time "t₁",
$$i_1 = -(v_0 - v_p) \frac{e^{-\frac{h^2}{96 \text{ at}_1}}}{\sqrt{24\pi at_1}}$$
....[7]

and at time "t₂",
$$i_2 = -(v_0 - v_p) \frac{e^{-\frac{h^2}{96at_2}}}{\sqrt{24\pi at_2}}$$
.....[8]

$$\frac{i_1}{i_2} = \frac{e^{-\frac{h^2}{96at_1}}}{e^{-\frac{h^2}{96at_2}}} \sqrt{\frac{t_2}{t_1}} = \sqrt{\frac{t_2}{t_1}} \cdot e^{\frac{h^2}{96a}} (\frac{1}{t_2} - \frac{1}{t_1})$$
 [9]

Let
$$\frac{1}{t_2} - \frac{1}{t_1} = \delta$$

$$\therefore a = \frac{h^2 \delta}{96 \ln \left[\frac{i_1}{i_2} \sqrt{\frac{t_1}{t_2}}\right]} = \frac{k}{C} \qquad (13)$$

Values of "i₁" and "i₂" / be obtained by plotting the temperature profiles for times "t₁" and "t₂" and then drawing tangents to curves at depth "h".

This problem is generally one confined to the laboratory. In nature, the soil temperature is not uniform at any time and the temperature over a given period never changes from one constant value to another.

(b) Find the thermal diffusivity "a" by noting the time required for a point at depth "h" to change its temperature by $(v_0 + v_p)$.

Substituting $\frac{v_0 + v_p}{2}$ for $v_{h,t}$, equation [1] becomes

$$\frac{v_0 + v_p}{2} = v_0 + (v_p - v_0) [1 - erf(\beta)]$$

Then erf
$$(\beta) = 1/2 \dots [14]$$

From tables of error functions or Fig. 5, Plate A-2 when erf

$$(\beta) = 1/2, \beta = 0.477.$$

From definition $\beta = \frac{h}{2\sqrt{24at}}$

Then h = $2\sqrt{5}\sqrt{24at}$ = 2 x 0.477 $\sqrt{24at}$

Thus, with a soil mass of very great depth at a uniform temperature "v $_0$ ", and the surface temperature is suddenly changed to temperature "v $_p$ ", then with a thermometer or thermocouple placed in the soil at depth "h" and the time noted when the soil temperature reaches a value halfway between "v $_0$ " and "v $_p$ ", a value of "a" can be obtained. This problem is confined to the laboratory.

PROBLEM NO. 2

Given a homogeneous, isotropic soil mass of semi-infinite extent, exposed to periodic temperature changes over a sufficiently long period so that the interior temperatures are also periodic.

Find the thermal diffusivity "a", by measuring the tempera-

ture gradients at different times, one quarter year apart, neglecting latent heat.

The surface temperature can be expressed as

The temperature at any depth "h" at any time "t" is

where
$$m = \sqrt{\omega/48a} = \sqrt{\frac{\pi}{24aT}}$$

At any time "t", the temperature gradient "i" can be expressed as the slope of the temperature versus depth curve,

Then
$$i = \frac{dv}{dh} = -mBe^{-mh}cos(\omega t - mh) + mBe^{-mh}sin(\omega t - mh)$$
. [18]

At time "t_|"

and at time "t₂"

Now let f(h) equal the sum of the squares of the thermal gradients at depth "h",

$$f(h) = i_1^2 + i_2^2 \dots [21]$$

Since by hypothesis $t_2 = t_1 + T/4$; substitution in equation [23] gives

$$f(h) = 2m^2B^2e^{-2mh}$$
 [24]

At depth h_1 , $f(h_1) = 2m^2B^2e^{-2mh_1}$, and at depth h_2 $f(h_2) = 2m^2B^2e^{-2mh_2}$

Then
$$\frac{f(h_1)}{f(h_2)} = \frac{2m^2B^2e^{-2mh_1}}{2m^2B^2e^{-2mh_2}} = e^{2m(h_2-h_1)}$$
 [25]

$$\ln \frac{f(h_1)}{f(h_2)} = 2m(h_2 - h_1) \qquad \qquad [26]$$

from which the value of "a" can be computed.

Example

Temperature profiles are shown in Figure 1 on Plate A-1. Using equation [27] and drawing tangents to the temperature profiles for the months of June and September, the results of "a" are as follows:

Depth in feet	^h 2 ^h 1	Ju i	ne i _I 2	Se i ₂	ot.	f(h)	f(h ₁) f(h ₂)	$\ln \frac{f(h_1)}{f(h_2)}$	m	m ²	a
10.0		.38	.144	.60	.360	.504					
	4.3						2.431	.8883	. 103	.0107	. 0335
5.7		. 96	.922	.55	.303	1.225					
	2.2						1.345	.2964	.067	.0045	.0797
3.5		1.3	1.64	.10	.010	1.648					
	2.0						2.495	.8998	.225	.0506	.0071
1.5		2.0	4.00	.23	. 053	4.053	0.760	1 0150	500	0501	0014
ا م	1.0				7.07		2.762	1.0159	.508	.2581	.0014
0.5		3.3	10.89	.55	.303	11.193					

The results of the example indicate the difficulty of determining "a" from field observations.

PROBLEM NO. 3

Given a homogeneous, isotropic soil mass of semi-infinite extent, exposed to periodic temperature changes over a sufficiently long period so that the interior temperatures are also periodic.

Find the thermal diffusivity "a" from the temperature amplitudes at various depths, neglecting latent heat.

From equations [16] and [17],

$$v_s = 32 + A + B \cos \omega t$$
, and

$$v_{(h,t)} = 32 + A + Be^{-mh} cos(\omega t - mh)$$

At depths "h_|" and "h₂",

$$v_{h_1} = 32 + A + Be^{-mh} \cos(\omega t - mh_1)$$
, and [28]

$$v_{h_2} = 32 + A + Be^{-mh} \cos(\omega t - mh_2)$$
 respectively [29]

For maximum values,

$$v_{h_1} = 32 + A + Be^{-mh_1}$$
, and $v_{h_2} = 32 + A + Be^{-mh_2}$ [30] & [31]

$$\frac{v_{h_1}}{v_{h_2}} = \frac{32 + A + Be^{-mh_1}}{32 + A + Be^{-mh_2}} \text{ or } \frac{v_{h_1} - 32 - A}{v_{h_2} - 32 - A} = e^{m(h_2 - h_1)}$$
 (32)

Since
$$A = v_0 - 32$$
, $v_h - 32 - A = V$

$$m = \sqrt{\frac{\pi}{24aT}} = \frac{1}{h_2 - h_1} \ln \left[\frac{v_1}{v_2}\right] \dots$$
 [34]

Example

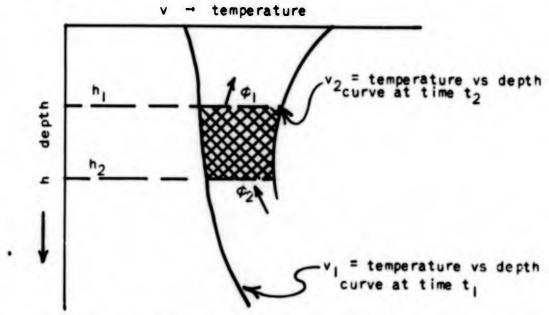
From the curves given in Figure 2, Plate A-I the following values of "a" are derived

Depth in feet	h ₂ - h ₁	v o _F	$\frac{v_1}{v_2}$	$\ln \frac{v_1}{v_2}$	m	m ²	a (ft ² /hr
10.0	4.3	4.7	1.660	.5068	.1179	.01390	.0258
5.7	2.2	7.8	1.256	.2278	.1035	.01071	. 0335
3.5 1.5	2.0	9.8	1.224	.2021	.1011	.01022	.0351
0.5	1.0	13.7	1.142	.1328	.1328	.01764	.0203

PROBLEM NO. 4

Given a homogeneous, isotropic soil mass of semi-infinite extent, subjected to a change of temperature over a period of time.

Find the thermal diffusivity "a" from temperature variation with depth, at two or more different times, neglecting latent heat.



Let "Q" = total quantity of heat absorbed by a layer of soil of depth (h_2-h_1) and of unit cross sectional area in the period " Δt " $(\Delta t=t_2-t_1)$.

Then
$$Q = C \int_{h_1}^{h_2} (v_2 - v_1) (dh)$$
 [35]

= C \times Area between temperature curves and depths "h2" and "h1"

Let "Q₁" and "Q₂" equal quantities of heat per unit area of surface, transmitted out of and into the layer through planes "h₁" and "h₂", respectively, in time " Δ t".

$$Q_2 = 24 \,\mathrm{k} \phi_2 \Delta t \quad ... \qquad [37]$$

Now the increase in heat in the layer during the time interval " Δt " is the difference between heat input and heat output.

$$Q = Q_2 - Q_1 = 24k(\phi_2 - \phi_1) (t_2 - t_1) = C \times Area$$
 [38]

or
$$\frac{k}{C} = \frac{Area}{24(\phi_2 - \phi_1)(t_2 - t_1)} = a$$
 [39]

In general terms

$$\mathbf{a} = \frac{(\mathbf{v}_2 - \mathbf{v}_1) (\mathbf{h}_2 - \mathbf{h}_1)}{(\phi_2 - \phi_1) (\mathbf{t}_2 - \mathbf{t}_1)} = \frac{\Delta \mathbf{v} \Delta \mathbf{h}}{\Delta \mathbf{i} \Delta \mathbf{t}}$$

$$= \frac{d\mathbf{v}}{d\mathbf{t}} \times \frac{d\mathbf{h}}{d\mathbf{i}} = \frac{d\mathbf{v}}{d\mathbf{t}} \times \frac{\mathbf{i}}{d\mathbf{i}/d\mathbf{h}} \qquad (40)$$

where $\frac{"dv"}{dt}$ is the tangent to the time-temperature curve at a given depth and time and $\frac{"di"}{dh}$ is the tangent to the temperature-gradient curve. Time interval " Δt " must be expressed in hours.

Example

Using equation [39], and values or thermal gradient " ϕ " from Figure 1, Plate A-I for the months of May and June (typical example is plotted in Figure 4, Plate A-I) the following values of "a" are obtained.

For May to June

 $\Delta t = 31 \times 24 = 744 \text{ hrs.}$

Depth in feet	h2-h1	(^O F)	$\frac{v_2 + v_1}{2}$	φ	$\phi_2 - \phi_1$ $(\Delta\phi)$	Δt times Δφ	Area	a (ft ² /hr)
10.0		2.18		-0.23				
	4.3		3.23		.52	386.9	13.889	. 0359
5.7		4.27		-0.75				
	2.2	1-3-6	4.81		.33	245.5	10.582	.0431
3.5		5.35		-1.08				
	2.0		5.68		.75	558.0	11.360	. 02 04
1.5		6.00		-1.83				
	1.0		6.11		1.72	1279.7	6.110	.0048
0.5		6.22		-3.55				

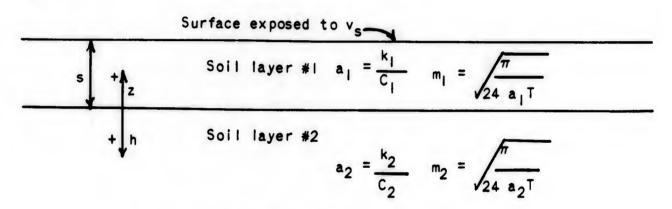
Using equation [40] and values of $\frac{"dv"}{dt}$ obtained from Figure 2, Plate A-I, and values of thermal gradient slope $\frac{"dt"}{dt}$ obtained from Figure 3, Plate A-I. Values of $\frac{"dt"}{dt}$ were obtained from tangents drawn to the gradient curves.

Depth in feet	$\frac{dv}{dt} =$	OF/mo	onth	di =	°F/ft²		а	= ft ² /	hr
	Apr.	June	Nov.	Apr.	June	Nov.	April	June	Nov.
10.0 5.7 3.5 1.5 0.5	0.7 2.5 3.6 5.3 6.5	3.9 4.7 4.0	-2.1 -4.3 -5.8 -5.0 -6.2	.035 .085 .170 .620	.135	070 150 245 355 380	.0278 .0408 .0294 .0119 .0057	.0257 .0402 .0344 .0097 .0027	.0417 .0398 .0328 .0125 .0226

PROBLEM NO. 5

Given two layers of homogeneous, isotropic soils, possessing different soil properties, the uppermost of which is exposed to a periodic temperature change.

Find the thermal diffusivity "a" of both layers, neglecting latent heat.



Layer #2 of infinite extent
Arrows indicate direction of measurements for "z"
and "h"

It is assumed that the impressed periodic surface temperature can be expressed by equation [16]

It can then be shown that the temperature at any point "z" in layer #1 at time "t" is,

$$V_{||(z,t)|} = 32 + A + \frac{B}{Z} \left[e^{m_1 z} \cos(\omega t + m_1 z - \theta) + N e^{-m_1 z} \cos(\omega t - m_1 z - \theta) \right]$$
 [41]

and the temperature at any point "h" in layer #2 at time "t" is,

$$v_{2(h,t)} = 32 + A + \frac{B}{Z}(1 + N)e^{-m_2h}\cos(\omega t - m_2h - \theta)$$
 [42]

where "N", "Z" and " θ " are parameters expressed as follows:

$$N = \frac{\sqrt{k_1C_1} - \sqrt{k_2C_2}}{\sqrt{k_1C_1} + \sqrt{k_2C_2}}. \qquad (43)$$

Now, differentiating equations [41] and [42] and following the procedure outlined in Problem 2, equations [18] to [27] inclusive, it can be found that

$$m_1 = \sqrt{\frac{\pi}{24a_1}} = \frac{1}{2(z_1 - z_2)} \ln \frac{f(z_1)}{f(z_2)} \dots$$
 [46]

and __

$$m_2 = \sqrt{\frac{\pi}{24a_2T}} = \frac{1}{2(h_2 - h_1)} \ln \frac{f(h_1)}{f(h_2)} = \text{equation [27]}$$

However, values of "a" may be as much as 20 per cent in error.

If the data were available as to the exact location of layer boundaries and for soil temperature profiles, then, at the soil interface, we have

$$k_1 i_1 = k_2 i_2 \dots \dots [47]$$

This ratio of thermal conductivities may be of some help.

PROBLEM NO. 6

Given a semi-infinite, homogeneous soil mass, the surface of which is exposed to periodic temperature changes over a sufficiently long period so that the interior temperatures are also periodic.

Find the depth of frost penetration "x", neglecting latent heat.

$$v_s = 32 + A - B \sin \omega t \dots$$
 [48]

and that B>A, so that the temperature drops below freezing.

The temperature at depth "h" at any time "t" is

$$v_{(h,t)} = 32 + A - Be^{-mh} \sin(\omega t - mh) \dots [49]$$

To find the trace of freezing temperature (32°F) surface "x",

Then

$$32 = 32 + A - Be^{-mx} \sin(\omega t - mx)$$
 [50]

or A = Be
$$\sin(\omega t - mx)$$
 [51]

and
$$x = \frac{1}{m} \ln \frac{B}{A} \sin(\omega t - mx) = \sqrt{\frac{24aT}{\pi}} \ln \frac{B}{A} \sin(\omega t - mx)$$
 [53]

Now "x" is a maximum when $\sin (\omega t - mx) = 1$

Example

Given: $v_0 = 42^\circ$; $B = 20^\circ$; $a = 0.03 \text{ ft}^2/\text{hr}$

Find "x".

$$x = \sqrt{\frac{24 \times 0.03 \times 365}{3.1416}} \ln \frac{20}{10} = \sqrt{83.60} \ln 2$$

$$= 9.14 \times 0.693 = 6.34 \text{ ft.}$$

PROBLEM NO. 7

Given two layers of homogeneous, isotropic soil possessing different soil properties, the uppermost of which is exposed to periodic surface temperature changes over a sufficiently long period so that the interior temperatures are also periodic.

Find the depth of frost penetration "x", neglecting latent heat.

Let the surface temperature be expressed by equation [48] $v_s = 32 + A - B \sin \omega t$

and let B>A so that the temperature drops below freezing.

As indicated by equations [41] and [42], the temperature at any point "z" at time "t" in layer #1 can be expressed as

$$V_{1}(z,t) = 32 + A - \frac{B}{Z} \left[e^{m_1 z} \sin(\omega t + m_1 z - \theta) + Ne^{-m_1 z} \sin(\omega t - m_1 z - \theta) \right]$$
[55]

and at any point "h" at time "t" in layer #2 by

$$v_{2(h,t)} = 32 + A - \frac{B}{Z}(1 + N)e^{-m_2h} \sin(\omega t - m_2h - \theta) \dots$$
 [56]

where "N", "Z" and " θ " are parameters given by equations [43] [44] and [45] respectively.

Proceeding in the manner indicated in Problem #6, the trace of freezing temperature (32°F) surface "x" in layer #1 is

$$\frac{AZ}{B} = e^{m_{\parallel}Z} \sin(\omega t + m_{\parallel}z - \theta) + Ne^{-m_{\parallel}Z} \sin(\omega t - m_{\parallel}z - \theta) \dots [57]$$

and in layer #2

$$\frac{AZ}{B(1+N)} = e^{-m_2x} \sin(\omega t - m_2x - \theta)$$
 [58]

Solution for "x" in equations [57] and [58] is by cut and try methods only.

PROBLEM NO. 8

is

Given a homogeneous, isotropic soil mass of semi-infinite extent, the surface of which is exposed to periodic temperature changes over a sufficiently long period so that the interior temperatures are also periodic.

Find the depth of freezing "x", neglecting volumetric heat but considering latent heat of fusion "L".

It is assumed that the periodic temperature change can be expressed by equation [48]

The temperature gradient through a frozen layer of thickness "x"

$$i = \frac{32 - A - 32 + B \sin \omega t}{x} = \frac{B \sin \omega t - A}{x}$$
 [59]

Now, the heat liberated in freezing a layer of thickness "x" is

The heat conducted out in time "dt" is

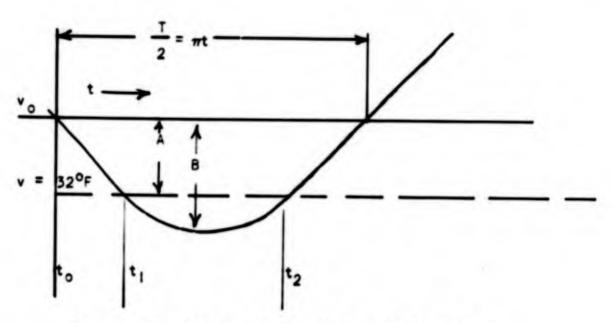
Equating [60] and [61], and substituting [59] for "i"

$$Ldx = 24kidt = \frac{24k}{x} (Bs in\omega t - A) dt \dots$$
 [62]

or
$$\frac{Lx dx}{24k}$$
 = (B sin ωt - A)dt [63]

Integrating

$$\frac{1x^2}{24k} = \frac{-8}{\omega} \cos \omega t - At + \overline{K} \qquad (64)$$



From sketch, when t = t , x = 0 and A = B sinut

and cos t =
$$\frac{1}{8}\sqrt{8^2 - A^2}$$

$$0 = \frac{-B}{\omega} \cdot \frac{1}{B} \sqrt{B^2 - A^2} - \frac{A}{\omega} \arcsin \frac{A}{B} + \overline{K}$$

and
$$\overline{K} = \frac{\sqrt{B^2 - A^2}}{\omega} + \frac{A}{\omega} \arcsin \frac{A}{B}$$

and
$$\frac{Lx^2}{24k} = -\frac{B}{\omega}\cos\omega t - At + \frac{1}{\omega}\sqrt{B^2 - A^2} + \frac{A}{\omega}\arcsin\frac{A}{B}$$
 [66]

$$x = \sqrt{\frac{24k}{L}} \left[-\frac{B}{\omega} \cos \omega t - At + \frac{1}{\omega} \sqrt{B^2 - A^2} - \frac{A}{\omega} \arcsin \frac{A}{B} \right] [67]$$

By rewriting equation [63] we have

$$\frac{dx}{dt} = \frac{24k(B \sin \omega t - A)}{Lx} \qquad \qquad [68]$$

Equating $\frac{\text{"dx"}}{\text{dt}}$ to 0, the maximum and minimum penetration is reached

Two values of "t" satisfy equation [69] as can be seen from foregoing sketch. At time "t_|", penetration is zero and at time "t₂", melting begins and penetration is maximum.

Now
$$t_1 - t_0 = \frac{T}{2} - t_2$$

$$\therefore t_2 = t_{\text{max}} = \frac{T}{2} - t_1 \quad ... \quad .$$

when

Integrating equation [63]
$$\frac{L}{24 \text{ k}} \int x dx = \int K(B \sin \omega t - A) dt$$

$$\frac{L \times \frac{2}{\text{max}}}{48 \text{k}} = \left[-\frac{B}{\omega} \cos \omega t - At \right] \frac{t_2}{t_1} \qquad . \qquad . \qquad [71]$$

$$= \frac{-B}{\omega} (\cos \omega t_2 + \cos \omega t_1) - A(t_2 - t_1)$$

Now
$$\cos \omega t_1 = \frac{\sqrt{B^2 - A^2}}{B}$$
 and $\cos \omega t_2 = -\frac{\sqrt{B^2 - A^2}}{B}$

Substituting these values in equation [72]
$$\frac{Lx_{max}^2}{48 \text{ k}} = \frac{B}{\omega} \sqrt{\frac{B^2 - A^2}{B} + \frac{B}{\omega} \sqrt{\frac{B^2 - A^2}{B}} - A(\frac{T}{2} - 2t_1)}$$

$$= \frac{2}{\omega} \sqrt{B^2 - A^2} - A(\frac{T}{2} - 2t_1) = \frac{2}{\omega} \sqrt{B^2 - A^2} + A(2t_1 - \frac{T}{2})$$
 [73]

Substituting $\frac{2\pi}{T}$ for " ω " and $\frac{1}{\omega}$ arc sin $\frac{A}{B}$ for "t₁"

Then
$$x^2_{\text{max}} = \frac{2k}{L} \left[\frac{24T}{\pi} \sqrt{B^2 - A^2} + A \left(\frac{24T}{\pi} \arcsin \frac{A}{B} - \frac{24T}{2} \frac{\pi}{\pi} \right) \right]$$

$$= \frac{2kT}{\pi L} \left[24\sqrt{B^2 - A^2} + A(24 \text{ arc sin } \frac{A}{B} - 12\pi) \right]$$

$$\times_{\text{max}} = \sqrt{\frac{48kT}{\pi L}} \left[\sqrt{8^2 - A^2} + A(\arcsin \frac{A}{B} - \frac{\pi}{2}) \right] \dots$$
 [74]

If the portion of the curve below $32^{\circ}F$ is assumed to be parabolic, the equation for the surface temperature for that portion can be expressed by

$$v_s = (B - A) \left(\frac{4t^2}{(t_2 - t_1)^2} - 1 \right) + 32 \dots$$
 [75]

or =
$$(B - A) \left[\left(\frac{2t - t_2 - t_1}{t_2 - t_1} \right)^2 - 1 \right] + 32 \dots$$
 [76]

Proceeding in the manner outlined by equations [59] through [74] inclusive, then

Example

Using the same data as was used for example for Problem No. 6,

$$v_0 = 42^{\circ}$$
; B = 20°; a = 0.03 ft²/hr and further

assuming C = 30 and L = 2880, then for use in equation [74]

$$k = aC = 0.03 \times 30 = 0.9$$

Then
$$x_{\text{max}} = \sqrt{\frac{48 \times 0.9 \times 365}{3.1416 \times 2880}} \left[\sqrt{400 - 100} + 10 (\arcsin \frac{10}{20} - 1.5708) \right]$$

$$=\sqrt{1.74 \times (17.32 - 10.47)} = \sqrt{11.9} = 3.45 \text{ ft.}$$

for use in equation [77]

$$t_1 = \frac{12T}{\pi} \cdot \frac{\pi}{6} = 2T$$
 hours $= \frac{T}{12}$ days

$$t_2 = \frac{T}{2} - t_1 = \frac{5T}{12} = 10T$$
 hours

Then

$$x_{\text{max}} = \sqrt{\frac{4 \times 0.9}{3 \times 2880}} (20 - 10)(10T - 2T)$$

=
$$\sqrt{.000417 \times 10 \times 8 \times 365}$$
 = $\sqrt{12.15}$ = 3.49 ft.

PROBLEM NO. 9

Given a homogeneous, isotropic soil mass of semi-infinite extent, the surface of which is exposed to a variable surface temperature which is a general function of time.

Find the depth of freezing "x", neglecting volumetric heat but considering latent heat of fusion "L".

The surface temperature can be expressed as

Proceeding in the manner indicated by equations [59] to [62] inclusive, then

$$i = \frac{32 - f(t)}{x}$$
 [79]

dH = Ldx = 24 kidt =
$$24k\left[\frac{32 - f(t)}{x}\right]$$
dt [80]

Then
$$xdx = \frac{24k}{L} [32 - f(t)]dt$$
 [81]

$$\frac{x^2}{2} = \frac{24k}{L} \int_0^t [32-f(t)] dt \dots [82]$$

Now
$$\int_{0}^{t} 32 - f(t)dt = F = freezing index in degree days$$

$$\therefore \times_{\text{max}} = \sqrt{\frac{48\text{kF}}{L}} \qquad [83]$$

Example

Using data from examples for Problems No. 6 and No. 8 $v_0 = 42^{\circ}$; $a = 0.03 \text{ ft}^2/\text{hr}$; C = 30 and L = 2880.

$$F = (32-v_s) t$$
 $v_s = v_0 - 8 = 42 - 20 = 22^0 F.$
 $t = t_2 - t_1 = \frac{T}{3} = 121.7 \text{ days}$

$$x = \sqrt{\frac{48 \times 0.9 \times 1217}{2880}} = 4.26 \text{ ft.}$$

PROBLEM NO. 10

Given a homogeneous, isotropic soil mass of semi-infinite extent, the surface of which is exposed to a uniform temperature above freezing " v_0 " which is suddenly reduced to below freezing " v_f ".

Find the depth of freezing "x", assuming that the latent heat "L" is greatly in excess of the volumetric heat "C".

The heat liberated in freezing a small depth "dx" is "L dx", which must be conducted upwards through a distance "x".

Hence the rate of upward flow

$$= \frac{24k}{x} (32 - v_f) dt \dots (85)$$

Now, if the unfrozen soil has a heat capacity of "C_u" per unit volume and is originally at temperature " v_0 ", then the heat liberated in lowering the temperature of the layer of thickness "dx" from " v_0 " to the freezing point (32°F), is

$$dH = C_u(v_0 - 32)dx$$
 [86]

Then,
$$Ldx + C_u(v_0 - 32)dx = \frac{24k}{x} (32 - v_f)dt$$
. [87]

or x dx =
$$\frac{24k(32 - v_f)}{L + C_{ij}(v_0 - 32)}$$
 dt [88]

Integrating,

$$\frac{x^2}{2} = \frac{24k(32 - v_f) t}{L + C_u(v_o - 32)} + \overline{K}$$

When t = 0, x = 0, therefore $\overline{K} = 0$, and

If the volumetric heat of frozen soil, " C_{ϕ} ", is considered, then equation [87] becomes

$$Ldx + C_u(v_0 - 32)dx + \frac{1}{2}C_f(32 - v_f)dx = \frac{24k}{x}(32 - v_f) dt. . . [90]$$

and equation [89] therefore becomes

But by definition $(32 - v_g)t = F$ (in degree days)

Therefore equation [90] becomes

$$x_{\text{max}} = \sqrt{\frac{\frac{3 \text{ k F}}{1 + C_u (v_o - 321 + C_f F)}}{2t}}$$
 (92)

Substituting a weighted average value "C" for "C" and "Cf", equation [92] becomes

$$x_{\text{max}} = \sqrt{\frac{48 \text{ k F}}{L + C(v_0 - 32 + \frac{F}{2t})}}$$
 [93]

Example

Using same data as for Example for Problem No. 9, v_0 =42°; B = 20°; a = 0.03 ft²/hr; C = 30; and L = 2880 and further C_u = 32 and C_f = 28.

Using equation [92]

$$x = \sqrt{\frac{48 \times 0.03 \times 30 \times 1217}{2880 + 32[(10) + 28 \times 1217]}} = \sqrt{\frac{52,600}{2880 + 320 + 140}}$$

$$= \sqrt{\frac{15.75}{2880 + 320 + 140}}$$

Using equation [93]

$$x = \sqrt{\frac{48 \times 0.03 \times 30 \times 1217}{2880 + 30(10 + 1217)}} = \sqrt{\frac{52,600}{2880 + (30 \times 15)}}$$

$$=\sqrt{15.80}=3.98$$
 ft.

PROBLEM NO. II

is

Given a homogeneous, isotropic soil mass of semi-infinite extent, the surface of which is exposed to periodic temperature changes over a sufficiently long period so that the interior temperatures are also periodic.

Find the depth of freezing "x", assuming that the latent heat "L" is greatly in excess of the volumetric heat "C".

The surface temperature is assumed to follow the form expressed by equation [48]

The thermal gradient "i" through a frozen layer of thickness "x",

Proceeding in the same manner as in Problem #10, equations [85] to [87] inclusive,

$$Ldx + C_u(v_0 - 32) dx = \frac{24k}{v} (8 \sin \omega t) dt$$
 [96]

Transposing and integrating

$$\frac{x^2}{2} = \frac{24kB}{L + C_{11}(v_0 - 32)} \left(-\frac{1}{\omega} \cos \omega t \right) + \overline{K}$$

Now when t = 0, x = 0

$$\overline{K} = \frac{24 \text{ k B}}{L + C_{u}(v_{o} - 32)} \cdot \frac{1}{\omega}$$

$$x^{2} = \frac{48 \text{ k B}}{\omega[L + C_{U}(v_{0} - 32)]} (1 - \cos\omega t) \dots$$
 [97]

But I - cos $\omega t = 2 \sin^2 \frac{\omega t}{2}$

$$\therefore x^{2} = \frac{96kB \sin^{2} \frac{\omega t}{2}}{\omega [L + C_{u}(v_{0} - 32)]}$$

or x = 2 sin
$$\frac{\omega t}{2} \sqrt{\frac{24kB}{\omega[L + C_{\nu}(v_{0} - 321)]}}$$
 [98]

When
$$\omega t = \pi$$
, $\sin \frac{\omega t}{2} = 1$, "x" is max. and $t = \frac{T}{2}$

$$\therefore \times_{\text{max}} = 2 \sqrt{\frac{24 \text{ kBT}}{2\pi [L + C_{11}(v_0 - 32)]}} = \sqrt{\frac{48 \text{ kBT}}{\pi [L + C_{11}(v_0 - 32)]}} \dots [9]$$

Example

Using values from examples for preceding Problems $v_0 = 42^\circ$; $B = 20^\circ$; $a = 0.03 \text{ ft}^2/\text{hr}$; C = 30; $C_u = 32$; $C_f = 28$; and L = 2880

$$x = \sqrt{\frac{40 \times 0.03 \times 30 \times 20 \times 365}{3.141612880 + 52 \times 101}} = \sqrt{\frac{315,000}{3.1416 \times 3200}}$$
$$= \sqrt{31.35} = 5.60 \text{ ft.}$$

PROBLEM NO. 12

Given a homogeneous, isotropic soil mass at the freezing point, but with the soil moisture unfrozen. The surface temperature varies as a general function of time but always below freezing.

Find the depth of freezing "x".

The surface temperature " v_f " can be expressed by equation [78] $v_f = f(t)$

Proceeding in the manner indicated by equations [79] and [80] in Problem #9, then

$$i = \frac{32 - f(t)}{x}$$

and dH = Ldx = 24 kidt = 24k
$$\frac{32 - f(t)}{x}$$
 dt [100]

Integrating

$$\frac{x^2L}{48k} = \int_0^t [32 - f(t)]dt \dots$$
 [102]

or
$$x = \frac{48 \text{ k}}{L} \int_{0}^{t} [32 - f(t)]dt$$
 [103]

Now considering the volumetric heats " C_u " and " C_f " and proceeding in the manner indicated in Problem #10, equations [87] to [90] inclusive, equation [100] becomes

$$Ldx + C_u[32 - f(t)]dx + \frac{C_f}{2}[32 - f(t)]dx = \frac{24k}{x} [32 - f(t)]dt . [104]$$

and equation [103] becomes

$$x = \frac{48 \text{ k} \int_{0}^{t} [32 - f(t)]dt}{1 + C_{u}[32 - f(t)] + \frac{C_{f}[32 - f(t)]}{2}} \dots [105]$$

Substituting the weighted average value "C" for "C $_{\rm U}$ " and "C $_{\rm f}$ " equation [105] becomes

Now since "v_f" is always below freezing,

$$\int_{0}^{t} [32 - f(t)]dt = F$$

Therefore equation [106] can be written

Example

(

Using values from examples for previous Problems

$$v_f = 22^\circ$$
; a = 0.03; C = 30; and L = 2880

$$x = \sqrt{\frac{48 \times 0.03 \times 30 \times 1217}{2880 + 45 \times 10}} = \sqrt{\frac{52,600}{3330}}$$
$$= \sqrt{15.80} = 3.98 \text{ ft.}$$

PROBLEM NO. 13

Given a homogeneous, isotropic, semi-infinite mass of frozen soil at freezing temperature, the surface of which is exposed to a constant temperature above freezing " v_0 ".

Find the depth of melting "x".

The temperature at any point "x" at any time "t" can be expressed by the equation

$$v_{(x,t)} = f(x,t) = 32 + A \int_{0}^{\beta} e^{-u^2} du \dots$$
 [108]

From equation [62]

$$-L\frac{dh}{dt} = 24 k \frac{dv}{dx} \dots \text{(minus sign denotes melting)} \dots$$
 [109]

Now, differentiating equation [108]

$$v = f(x,t)$$

$$dv = \frac{dv}{dx} dx + \frac{dv}{dt} dt \qquad \dots \qquad [110]$$

When x=h, dv=0

$$\therefore 0 = (\frac{dv}{dx}) \cdot (\frac{dh}{dt}) + (\frac{dv}{dt}) \cdot \dots \cdot \dots \cdot \dots \cdot (111)$$

Combining equations [109] and [111]

$$0 = (\frac{dv}{dx}) \cdot \frac{24k}{x=h} (\frac{dv}{dx}) + (\frac{dv}{dt})_{x=h} \text{ or } \frac{dv}{dt} = \frac{24k}{L} (\frac{dv}{dx})_{x=h}^{2}. \quad [112]$$

From equation [108] by differentiation,

and

Substituting "h" for "x" and substituting equations [113] and [114] in equation [112],

$$Ae^{-\frac{h^2}{96at}} \cdot \frac{h}{4t\sqrt{24 \ at}} = \frac{24k \ A^2 \ e^{-\frac{2h^2}{96at}}}{96 \ L \ at} \quad . \quad . \quad [115]$$

Now from equation [3]

$$\beta = \frac{h}{2\sqrt{24at}}$$

Equation [115] becomes

or
$$\beta = \frac{k A e^{-\beta^2}}{2aL}$$
 [117]

Hence
$$\frac{\beta_e \beta^2}{A} = \frac{k}{2aL}$$
 [118]

Now from equation [108]

$$\frac{1}{A} = \frac{1}{v_p - 32} \cdot \int_0^6 e^{-u^2} du \cdot \dots$$
 [119]

Substituting equation [119] in equation [118]

$$\beta e^{\beta^2} \int_{e^{-u^2} du}^{\beta} = \frac{k(v_p - 32)}{2aL}$$
 [120]

Now from equation [2] $\int_{0}^{\beta-u^2} du = \frac{\text{erf } (\beta)\sqrt{\pi}}{2}$

$$\therefore \beta e^{\beta^2} \operatorname{erf}(\beta) = \frac{k(v_p - 32)}{\sqrt{\pi} \operatorname{al}} = \frac{C(v_p - 32)}{l\sqrt{\pi}} \dots \dots [121]$$

If β is small, then

$$\beta e^{\beta^2} \int_{e^{-u}}^{\beta - u^2} du \approx \beta^2 \cdots$$
 [122]

Substituting in equation [120]

Since from equation [3] $\beta = \frac{h}{2\sqrt{24at}}$

$$\beta^2 = \frac{h^2}{96at} = \frac{x^2}{96at}$$
 [124]

Now, consider the series expansion

$$e^{\beta^2} = 1 + \beta^2 + \frac{\beta^4}{2!} + \frac{\beta^6}{3!} \dots$$

Then

$$\beta \cdot \beta^2 = \beta + \beta^3 + \frac{\beta^5}{2!} + \frac{\beta^7}{3!} \dots$$

and
$$\int_{e}^{\beta} -u^{2} du = \beta - \frac{\beta^{3}}{3 \cdot 1!} + \frac{\beta^{5}}{5 \cdot 2!} - \frac{\beta^{7}}{7 \cdot 3!} + \dots$$

Hence

$$\beta e^{\beta^2} \int_0^{\beta - u^2} du = (\beta + \beta^3 + \frac{\beta^5}{2} + \frac{\beta^7}{6})(\beta - \frac{\beta^3}{2} + \frac{\beta^5}{10} - \frac{\beta^7}{42} \dots)$$

$$= (\beta^2 + \frac{2\beta^4}{3} + \dots) \quad \dots \quad [126]$$

Then equation [120] becomes

$$\beta^2 + \frac{2\beta^4}{3} = \frac{C(v_p - 32)}{2L} \qquad (127)$$

and

$$x = \sqrt{72at(\sqrt{1 + \frac{4(v_p - 32)C}{3L}} - 1)}$$
 [128]

Example

Using data from examples for previous Problems.

$$a = 0.03$$
; C = 30; and L = 2880. Also assuming $v_p = 42^{\circ}$

Using equation [125]

$$x = \sqrt{\frac{48 \times 0.03 \times 30 \times 121.7 \times 10}{2880}} = \sqrt{\frac{52.600}{2880}}$$

$$= \sqrt{18.25} = 4.27.11$$

Using equation [128]

$$x = \sqrt{72 \times 0.03 \times 121.7} \left[\sqrt{1 + \frac{4 \times 10 \times 30}{3 \times 2880}} - 1 \right]$$

$$= \sqrt{263 (\sqrt{1.139} - 1)}$$

$$= \sqrt{263 \times .065}$$

$$= \sqrt{17.09} = 4.14 \text{ ft}$$

Using equation [120] and Figure 6, Plate A-2

$$\frac{C(v_p - 32)}{L \times 2} = \frac{30 \times 10}{2880 \times 2} = 0.0529 = \lambda \beta$$

From Figure 6, Plate A-2, $\beta = 0.225$

Since
$$x = 2\beta\sqrt{24}$$
 at $= 2 \times 0.225\sqrt{24} \times 0.03 \times 121.7$
= $0.45\sqrt{87.62} = 0.45 \times 9.36$
= 4.22 ft.

The solutions by these three methods are in very close agreement, but the method using equation [120] and Figure 6, Plate A-2, gives the exact answer and is the easiest to use.

PROBLEM NO. 14

Given a homogeneous, isotropic, semi-infinite soil mass just above freezing temperature which is exposed to a constant temperature below freezing " v_p ".

Find the depth of freezing "x", assuming that the thermal gradient varies uniformly from the surface to the depth of freezing.

The surface temperature can be expressed by equation [78]

Then, as indicated in equation [100]

$$Ldx = 24k \frac{[32 - f(t)]}{x} dt$$

The volumetric heat " C_f " liberated in the frozen zone of thickness "dx" in time "t", is

$$dH = \frac{C_{\uparrow}[32 - f(t)]}{2} dx \qquad ... \qquad [129]$$

$$\therefore \frac{24k}{x} [32-f(t)]dt = Ldx + \frac{C_f[32 - f(t)]}{2} dx$$

$$= L + \frac{C_f}{2} [32 - f(t)] dx \dots [130]$$

or

Integrating

Now since "vp" is always below freezing

$$\int_{1}^{t_2} [32 - f(t)] dt = F \text{ and equation [132] becomes}$$

and

If the surface temperature is expressed by the form

Then

$$x^{2} = \frac{48k}{\omega} \{ (A - \frac{DB}{G})(\frac{2}{\sqrt{D^{2} - G^{2}}}) \text{ [arc tan(}\frac{G + Dtan(\frac{\omega t_{2}}{2})) - G^{2}) \}}$$

arc tan
$$(\sqrt{D^2 - G^2})$$
] + $\frac{B}{G}\omega(t_2 - t_1)$ }. [136]

where D = L +
$$\frac{C_f A}{2}$$
 and G = $\frac{C_f B}{2}$

Examples

Using data from examples for preceding Problems

$$v_0 = 42^\circ$$
; B = 20°; a = 0.03 ft²/hr; C = 30; C_e = 28;

$$C_u = 32$$
; L = 2890; and $v_p = 22^\circ$

Using equation [134]

$$x = \sqrt{\frac{48 \times 0.03 \times 30 \times 1217}{2880 \times \frac{28}{2}(32 - 22)}} = \sqrt{\frac{52,600}{3020}}$$
$$= \sqrt{17.40} = 4.17 \text{ ft.}$$

Using equation [136]

$$D = 2880 + \frac{28 \times 10}{2} = 3020$$

$$G = \frac{28 \times 20}{2} = 280$$

$$\omega = \frac{2\pi}{1} = \frac{2 \times 3.1416}{365} = 0.01724$$

$$t_1 = \frac{T}{12}$$
 and $t_2 = \frac{5T}{12}$ (from example Problem #8)

Now
$$\frac{\omega t_1}{2} = \frac{2\pi}{T} \times \frac{T}{24} = \frac{\pi}{12}$$

$$\frac{\omega t_2}{2} = \frac{2\pi}{T} \times \frac{5T}{24} = \frac{5\pi}{12}$$

$$(A - \frac{OB}{G}) = 10 - \frac{3020 \times 20}{280} = 10 - 215.7 = -205.7$$

$$\sqrt{D^2 - G^2} = \sqrt{3020^2 - 280^2} = \sqrt{9.042.000} = 3007$$

G + D
$$\tan \frac{\omega t_1}{2}$$
 = 280 + 3020 + $\tan \frac{\pi}{12}$ = 280 + 3020 $\tan 15^{\circ}$
= 280 + 3020 \times 0.26795
= 1089

G + D
$$\tan \frac{\omega t_2}{2}$$
 = 280 + 3020 $\tan \frac{5\pi}{12}$ = 280 + 3020 $\tan 75^{\circ}$
= 280 + 3020 x 3.84103
= 11.550

arc tan
$$(\frac{G + D \tan \frac{\omega t_1}{2}}{\sqrt{D^2 - G^2}}) = arc \tan \frac{1089}{3007} = arc \tan 0.36215$$

= 19.91°

arc tan
$$(\frac{G + D \tan \frac{\omega t_2}{2}}{\sqrt{D^2 - G^2}}) = arc \tan \frac{11550}{3007} = arc \tan 3.84105$$

= 75.41°

$$\frac{B}{G}\omega(t_2-t_1)=\frac{20\times 2\pi}{280\times 7}\cdot\frac{1}{3}=\frac{40\pi}{840}=0.14960$$

Therefore

$$x^{2} = \frac{48 \times 0.03 \times 30}{0.01724} [(-205.7)(\frac{2}{3007}(75.4)^{\circ} - 19.9)^{\circ}] + 0.14960]$$

$$= 2505.8 [(-0.13681)(55.50^{\circ}) + 0.14960]$$

$$= 2505.8 [(-0.13681)(0.96866 \text{ radians}) + 0.14960]$$

$$= 2505.8 \times 0.0171 = 42.85$$

$$= \sqrt{42.85} = 6.55 \text{ ft.}$$

PROBLEM NO. 15

Given a homogeneous, isotropic mass of frozen soil of semi-infinite

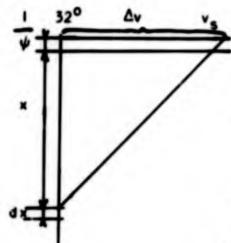
extent, exposed to a surface temperature "vs" which is below freezing.

Find the effect of radiation and ground film.

In accordance with Newton's Law of cooling

The rate at which heat passes out through a unit area of surface

$$\frac{dQ}{dt} = 24ki_{x=0} = 24k\psi(v_{x=0} - v_s) = \frac{24k}{l}(v_{x=0} - v_s) = 24E(v_{x=0} - v_s)[139]$$



is

$$\frac{dQ}{dt} = Ldx + \frac{C_f \Delta v}{2} dx = \frac{24k\Delta v}{x + \frac{1}{4k}} \dots [139]$$

or
$$(x + \frac{1}{\psi})dx = \frac{24k\Delta v}{L + \frac{C_f}{2}\Delta v}$$
 (dt) . . . [140]

Now let
$$y = x + \frac{1}{\psi}$$

Then
$$ydy = \frac{24k\Delta v}{dt}$$
 dt [141]

or
$$y^2 = \frac{48k\Delta vt}{L + \frac{C_f}{2}\Delta v} + \overline{K} = (x + \frac{1}{\psi})^2 \dots$$
 [142]

When t = 0, x = 0.
$$\therefore \overline{x} = \left(\frac{1}{\psi}\right)^2$$

$$\therefore (x + \frac{1}{\psi})^2 = \frac{48k\Delta vt}{L + \frac{C_{\gamma}}{2}\Delta v} + (\frac{1}{\psi})^2 \qquad (143)$$

$$\times = -\frac{1}{\psi} + \sqrt{\frac{48k\Delta vt}{L + \frac{C_f}{2}\Delta v} + (\frac{1}{\psi})^2} \qquad . \qquad [144]$$

From figure above $\Delta v = 32 - v_s$

$$\therefore x = -\frac{1}{\psi} + \sqrt{\frac{48kt(32 - v_s)}{L + \frac{C_f}{2}(32 - v_s)} + \frac{1}{\psi})^2} \qquad . \qquad . \qquad [145]$$

The value $\frac{\|\cdot\|^{\prime\prime}}{\psi}$ may be regarded as an extra layer of soil having the same thermal conductivity $\|\cdot k\|$ as the base soil, but having no volumetric heat capacity. The value $\frac{\|\cdot\|^{\prime\prime}}{\psi}$ is also a function of the velocity of air over the surface. For large values of $\|\cdot E\|$ (5 or 6), the value of $\frac{\|\cdot\|^{\prime\prime}}{\psi}$ is small, but for small values of $\|\cdot E\|$ (1 or 2), the value of $\frac{\|\cdot\|^{\prime\prime}}{\psi}$ becomes appreciable. The following table indicates the effect of neglecting the value $\frac{\|\cdot\|^{\prime\prime}}{\psi}$ in equation [145] using the following values:

L = 800; t = 180; and $C_f = 30$.

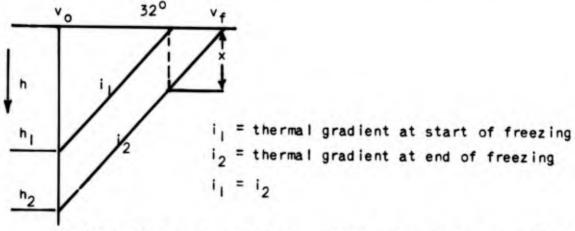
v _s	Per Cent Error in Omitting 1/\psi from Equation [145]											
(°F)	E = 6			E = 5		E = 2		E = 1				
	k=0.5	k=1.0	k=1.5	k=0.5	k=1.0	k=1.5	k=0.5	k=1.0	k=1.5	k=0.5	k=1.0	k=1.5
31 27 22 17 12 7 2	3.6 1.7 1.2 1.0 .9	5.1 2.4 1.8 1.5 1.3 1.2	6.2 2.8 2.1 1.8 1.6 1.5	4.4 2.0 1.5 1.3 1.1 1.0	6.1 2.9 2.1 1.8 1.6 1.5	7.5 3.4 2.6 2.2 2.0 1.8	10.8 5.0 3.7 3.1 2.8 2.6 2.5	15.2 7.1 5.2 4.4 4.0 3.7 3.5		10.0 7.4 6.3 5.7	29.4 14.1 10.4 8.9 7.8 7.4 6.9	36.2 16.7 12.8 10.8 9.7 9.0 8.5

The per cent error in omitting the value $\frac{\|\cdot\|^{n}}{\psi}$ from equation [145] for airport runways and highways where the snow is plowed off and the surface exposed to the wind, will be small, since the value of "E" will be about 5 or 6 and the temperatures involved will be the temperatures indicated in the middle and lower portions of the above table.

PROBLEM NO. 16

Given a homogeneous, isotropic soil mass at a temperature " $_{\rm O}$ " suddenly exposed to a surface temperature below freezing " $_{\rm F}$ ".

Find the depth of freezing "x", assuming that the temperature varies uniformly with the depth.



The heat conducted out of the soil is given by equation [61]

$$dH = 24k i dt$$

when t = 0 H = 0
$$\therefore \overline{K}$$
 = 0 and H = 24kit [147]

The total heat given up the soil as indicated in sketch is

$$H = \frac{h_2 - h_1}{2} (v_0 - 32 + v_0 - v_f) C + Lx$$

But $h_2 - h_1 = x$

$$\therefore H = \dot{x} \left[\frac{C}{2} (2v_0 - 32 - v_f) + L \right] = 24kit \dots$$
 [148]

Now

Substituting equation [150] in equation [148]

$$\times \left[\frac{C}{2} (2v_0 - 32 - v_f) + L \right] = \frac{24 \, \text{kF}}{x}$$
and
$$\times = \sqrt{\frac{C}{2} (2v_0 - 32 - v_f) + L}$$

$$(151)$$

From equation [149]

$$-v_{f} = \frac{F}{t} - 32$$

Hence equation [152] becomes

$$x = \sqrt{\frac{24 \text{ k F}}{L + \frac{C}{2}(2v_0 - 64 + \frac{F}{t})}}$$
 (153)

or
$$x = \sqrt{\frac{24 \text{ k F}}{L + C(v_0 - 32 + \frac{F}{2t})}}$$
 [154]

Example

Using data from examples for previous Problems

$$v_0 = 42^{\circ}$$
; $C = 30$; $a = 0.03$; and $L = 2880$

Then

$$x = \sqrt{\frac{24 \times 0.03 \times 30 \times 1217}{2880 + 30(42 - 32 + \frac{1217}{2 \times 121.7})}} = \sqrt{\frac{26,300}{3330}}$$
$$= \sqrt{7.88} = 2.81 \text{ ft.}$$

PROBLEM NO. 17

Given a homogeneous, isotropic soil mass of semi-infinite extent, overlain by an insulation layer of thickness "d", all at temperature "vo" and suddenly exposed to a surface temperature below freezing " v_f ".

Find the depth of freezing "xR", assuming that the temperature varies uniformly with the depth, that there is no significant change in temperature gradients due to the insulation layer, and neglecting latent heat "LR" and volumetric heat "CR" of the insulation layer.

Now equation [151] can be written as

or
$$x_R(d + x_R) = \frac{24 \text{ k F}}{L + \frac{C}{2}(2v_0 - 32 - v_f)}$$
 [156]

Solving for xp

$$x_R = -\frac{d}{2} + \sqrt{(\frac{d}{2})^2 + \frac{24 \text{ k F}}{\frac{C}{2}(2v_0 - 32 - v_f) + L}}$$
 [157]

Substituting equation [149] for v_{ϕ}

$$x_R = -\frac{d}{2} + \sqrt{(\frac{d}{2})^2 + \frac{24 \text{ k F}}{L + C(v_0 - 32 + \frac{F}{2t})}}$$
 [158]

Example

Using same data as for examples for previous Problems $v_0 = 42^{\circ}$; C = 30; a = 0.03 and L = 2880; also d = 1.0 ft.

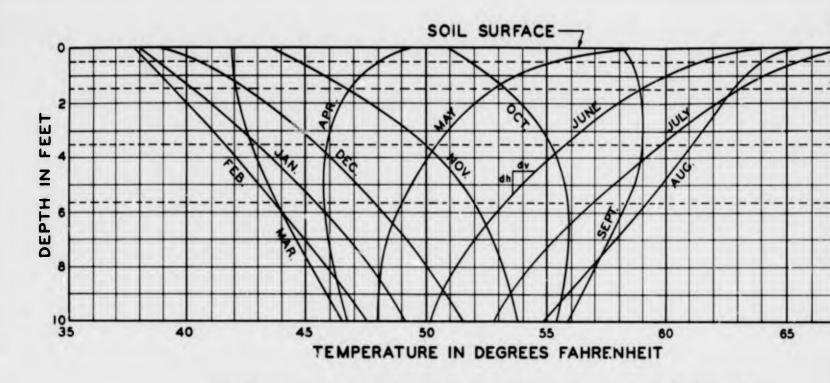
$$x_R = -0.5 + \sqrt{(0.5)^2 + 24 \times 0.03 \times 30 \times 1217}$$

$$2880 + 30(42 - 32 + \frac{1217}{2 \times 121.7})$$

$$\times_{R} = -0.50 + \sqrt{0.25 + \frac{26300}{3330}} = -0.50 + \sqrt{8.13}$$

$$= -0.50 + 2.85$$

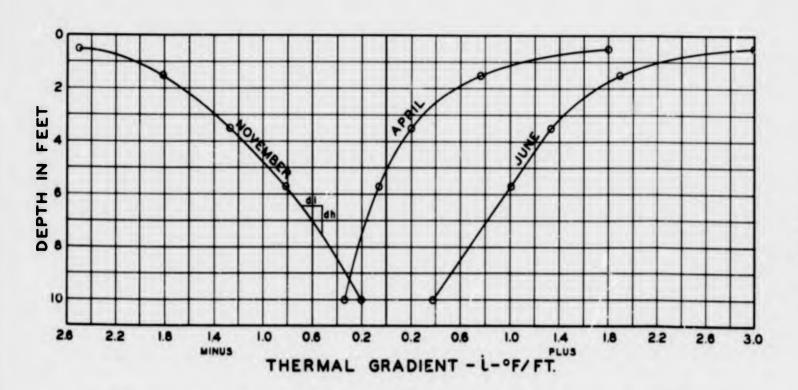
$$= 2.35 \text{ ft.}$$



MONTHLY TEMPERATURE PROFILES FOR A TURFED SOIL

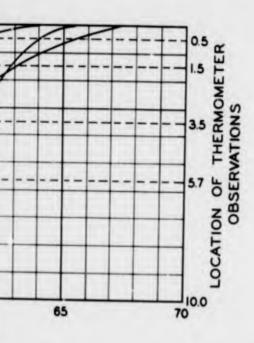
(ADAPTED FROM "RADCLIFFE OBSERVATIONS, RADCLIFFE OBSERVATORY, ENGLAND, 1915

FIG. 1

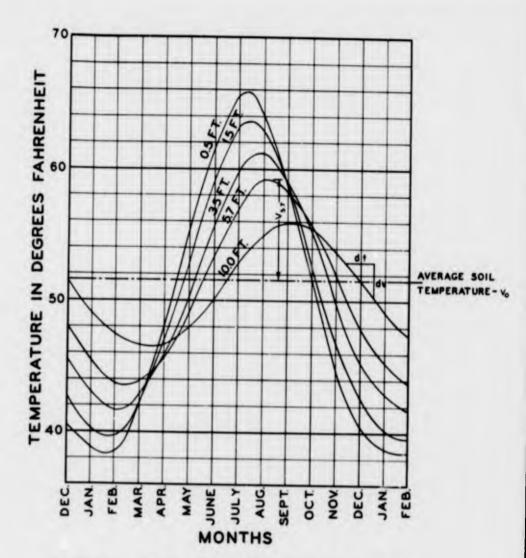


THERMAL GRADIENTS FOR MONTHS OF APRIL, JUNE AND NOVEMBER FIG. 3

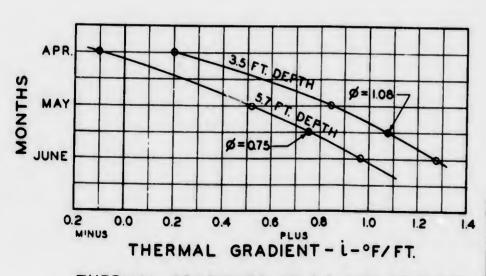




SOIL NGLAND, 1915")



ANNUAL SOIL TEMPERATURE CURVES
AT DEPTHS OF OBSERVATION
FIG. 2

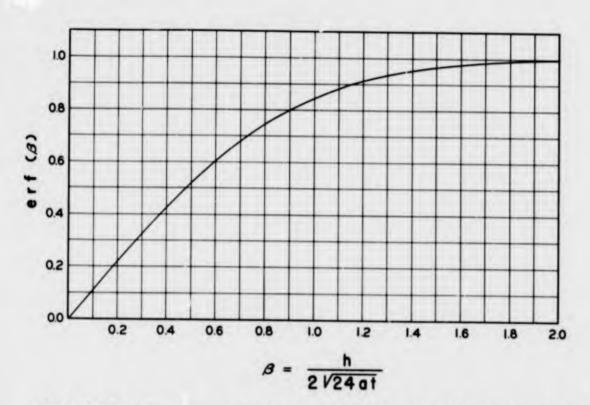


THERMAL GRADIENTS AT 3.5 AND 5.7 FOOT DEPTHS OF OBSERVATION FIG. 4

FROST INVESTIGATION 1945-1946

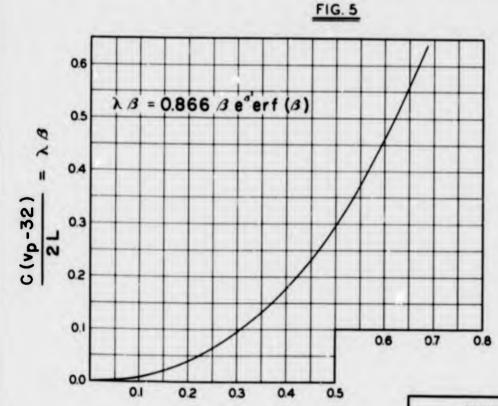
THERMAL CURVES
FOR A TURFED SOIL

FROST EFFECTS LAB, BOSTON, MASS JUNE 1946



1

THE PROBABILITY INTEGRAL (GAUSS "ERROR FUNCTION")



AND TEMPERATURE DATA

B

FIG. 6

FROST INVESTIGATION 1945-1946

FOR DETERMINATION OF erf (3) AND 8

FROST EFFECTS LAB, BOSTON, MASS. JUNE 1948

NEW ENGLAND DIVISION CORPS OF ENGINEERS, U. S. ARMY BOSTON, MASSACHUSETTS

DATA REPORT OF FROST INVESTIGATIONS FISCAL YEARS 1943 - 1949

APPENDIX D

REPORT ON SPECIAL TEST SECTION AT DOW FIELD 1946 - 1947

FROST EFFECTS LABORATORY

June 1949

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FOREWORD

The special test section at Dow Field, Bangor, Maine, was constructed for correlation with controlled laboratory tests and to determine the rate of frost penetration in non-frost susceptible base materials covered with rigid and flexible pavements, and to study the effect of density, degree of saturation and water supply on frost action in frost susceptible soils for a period of several years while exposed to natural weather conditions. The test section was made up of 14 portland cement concrete cylinders, eight feet high and five feet inside diameter containing either cinders, sand and gravel, crushed rock, or silty clay.

This report presents the construction details and results of observations. The data presented contain complete records showing changes in soil temperature with variations of air temperature pavement heave and settlement. Unexpected difficulty was experienced during the first season of observations due to the generation of heat in the fresh, moist sawdust used for insulation between the cylinders (elements). Corrective measures were applied with some success.

The test section was abandoned after the first season and later dismantled due to curtailment of investigational program and lack of funds.

REPORT ON TEST SECTION DOW FIELD, BANGOR, MAINE

I. INTRODUCTION

- 1-01. Authorization. The frost investigation program was continued by authorization contained in two letters from the Chief of Engineers to the Division Engineer, New England Division, dated 25 July 1946 and 12 August 1946, Subject: "Funds for Investigational Program for Fiscal Year 1947". The Frost Effects Laboratory, established at the New England Division by direction of the Chief of Engineers, as stated in Circular Letter No. 3221, dated 11 August 1944, was continued in its function. Addendum No. 1 to the original instructions and outline dated August 1946 from the Chief of Engineers, covers the details of the Test Section constructed at Dow Field, Bangor, Maine.
- 1-02. Purpose. A test section was constructed and observed (a) to determine the rate of frost penetration into non-frost susceptible base materials with rigid and flexible pavements and (b) to study the effect of density, degree of saturation and water supply on frost action in frost susceptible soils for a period of several years.
- 1-03. Scope. This report contains the construction details and results of observations made in a test section containing 14 combinations of pavement, base, and subgrade materials exposed to freezing temperature during the first year of observations. The test section includes 14 portland cement concrete cylinders each eight feet high and five feet inside diameter, referred to hereinafter as elements, containing specimen

material of either cinders, sand and gravel, crushed rock or silty clay. The flexible type pavement was simulated by a four-inch cover of bituminous concrete placed over the specimen material, and the rigid pavement by a six-inch cover of portland cement concrete. The effect of shallow ground water was obtained in four of the elements by connections to a controlled water supply. The rate of frost penetration in each test element was determined by temperatures recorded by thermocouple installations. Observations were made of degree of saturation, water content, and density at time of placing, and pavement heave and subsidence during freezing and frost melting periods. Observations of the water content and density were made after the frost melting period in the four elements in which the subgrade was placed at a low density.

1-04. Definitions. The description of the tests and analysis of results involve a specialized use of certain terms and words. These words and terms are defined for use in paragraph 1-, main body of this report.

II. DESCRIPTION OF TEST SECTION

2-01. General. An area, "Reclamation Yard", located at the southerly end of Dow Field, was made available for the construction of a test section. The work was performed by hired labor during the period from August to October 1946. The test section consisted of 14 elements, A to H inclusive, J to N inclusive and P, five feet in diameter and eight feet in depth. Each element was enclosed in a reinforced cement concrete pipe of standard tongue and groove construction eight feet long and 5 feet 10 inches outside diameter set upright in a reinforced cement concrete base of 12-inch thickness placed one foot below the original ground surface. The elements were placed in two rows 7.75 feet from center to center. The embankment

surrounding the elements was constructed of sand and gravel to a height approximately five feet below the top of the elements. Sawdust was placed on the gravel fill to six inches below the finished grade and a blanket of sand and gravel was placed to bring the embankment up to the top of the elements. Details of the test section are shown on Plates D-2 and D-2. Photographs taken during construction are shown on Plates D-16 and D-17. The sawdust was used for insulation to minimize heat transfer between elements and adjoining fill. Additional insulation was provided along the top of the pipe, as shown in Figure 4, Flate D=2 to reduce heat transfer down through the concrete pipe.

The top of the test section, 60.5 feet in length by 21.75 feet in width, was sloped for surface drainage. The side slopes were two feet horizontal to one foot vertical.

2-02. Pavements. To simulate the effect of airfield pavements, the test elements were capped with portland cement concrete or bituminous concrete. Three elements B, D, and F had six inches of portland cement concrete pavements. The remaining 11 elements were designed for two inches of bituminous concrete pavement. Replacement of bituminous concrete at the end of the year indicated that the pavement averaged four inches thick. The pavements are similar in quality to those used in airfield construction. The cement concrete was a ready mixed, 1:2:4 proportion, with six bags of cement per cubic yard and with aggregate sizes shown by grain size curve in Figure 8, Plate D-3 Compression test results on six molded 12-inch cylindrical samples from the portland cement concrete batches were as follows:

⁷ day strength = 1,894 pounds per square inch

¹⁴ day strength = 2,770 pounds per square inch

²⁸ day strength = 3,646 pounds per square inch

The bituminous concrete contained five per cent of 85 to 100 penetration asphalt cement. The gradation of the aggregate in the mix is shown by the grain size curve in Figure 8, Plate D-3 The pavement was sealed with an application of RC-2 at a rate of 0.25 gallon per square yard. Clean well-graded cover sand was applied to the seal coat at a rate of 20 pounds per square yard.

2-03. Non-Frost Susceptible Base Materials.

- a. Cinders (Elements A and B). The behavior of cinders as a base course and insulating material when subject to freezing was investigated in elements A and B. The cinders used were retained on a 1-inch screen to remove excessive fines and were hand tamped into the portland cement concrete cylinders to awoid excessive crushing of the particles. The dry densities obtained ranged from 53 to 68 pounds per cubic foot with the water contents between one and two per cent as shown in Figures 1 and 2 on Plate D-3. Four inches of bituminous concrete and six inches of cement concrete were placed directly on the cinders in elements A and B respectively.
- b. Sand and Gravel (Elements C and D). Sand and gravel was compacted in elements C and D at dry densities ranging from 127 to 131 pounds per cubic foot and at water contents ranging from six to eight per cent as shown in Figures 3 and 4 on Plate D-3. The grain size gradation is shown by curve 2 of Figure 7 on Plate D-3. The material is well graded, and of GW classification. Four inches of bituminous concrete and six inches of cement concrete were placed directly on the sand and gravel in elements C and D respectively.
- c. Crushed Rock (Elements E and F). Crushed rock was placed in elements E and F at dry densities ranging from 107 to 112 pounds per

cubic foot and one per cent water content as shown in Figures 5 and 6 on Plate D-3. The grain size gradation is shown by curve 1 of Figure 7 on Plate D-3. The original placing of material in element E did not result in the desired 110 pounds per cubic foot density. The material was removed and, in the second attempt, each foot layer was compacted to take a pre-weighed quantity necessary for the required density. The crushed rock was placed by dumping from a high-body dump truck and compacted with a power tamper. Four inches of bituminous concrete and six inches of cement concrete were placed directly on the crushed rock in elements E and F respectively.

2-04. Frest Susceptible Materials (Elements G, H, J, K, L, M, N and P). A frost susceptible silty clay (CL) representative of the existing top five feet of the subgrade soil in place under the airfield pavement at Dow Field was placed as a subgrade at controlled densities and water contents in eight elements. The grain size gradation curve is shown on Figure 7 on Plate D-3. Four of the elements, G, H, J, and K, had a controlled water surface seven feet below the pavement and four elements, L, M, N, and P, were not in contact with a water surface to simulate a water table at infinity. The silty clay was placed dense at a high water content, loose at a high water content, dense at a low water content, and loose at a low water content in elements G, H, J, and K, respectively. These conditions were duplicated in elements L, M, N, and P, respectively. Table of element variables is shown on Plate D-2. The silty clay was difficult to handle but the densities and water contents at which it was placed were approximately those desired. Plate D-13 shows the material placed in element M at 28 per cent water content after the silty clay had been run through a mixer to bring the

water content up ten per cent and element P with the material placed at 22 per cent water content. During the frost melting period the pavement in those elements, H, K, M, and P, in which the subgrade was placed in a loose condition settled so that the pavement and base had to be removed, the subgrade brought up to grade by the addition of silty clay and new base and pavement placed. The condition of pavement in elements H and M after settlement and prior to reconstruction is shown on Plate D-19. At the time of reconstruction density and water content determinations were made by driving a 2.75-inch diameter thin-walled, seamless steel tubing into the subgrade. These density and water content tent values showed considerable variation from those determined during the construction of the test elements. All values of density and water content determinations are shown on Plates D-3 and D-1.

The silty clay subgrade was covered with a six-inch layer of sand and gravel which was treated with RT-5 sprayed at the rate of one gallon per square yard. A four-inch layer of bituminous concrete pavement was placed over the sand and gravel.

2-05. Insulation.

a. The top edges and top 14 inches of the sides of each concrete cylinder were insulated with sawdust as shown in Figure 4, Plate D-2. It was originally planned to use cork board and granulated cork for the insulating material. Failure to get delivery of the material necessitated the use of readily available sawdust. The sawdust was held in place by a permanent plywood form. After the sawdust was placed in the form, a plywood cover was nailed on. For water-proofing against surface seepage the cover was treated with double-ply

15-1b. felt thoroughly coated on all surfaces with three coats of roofing pitch. The felt overlapped the pavement surface by two inches and extended three inches outside the perimeter of the plywood form, as shown by Plate D-19.

b. The insulating fill between cylinders and the bulk of the embankment consisted of sawdust as shown in section in Figure 3, Plate D=1. Because of the scarcity of cinders in this vicinity, it was necessary to use sawdust. The sawdust was hauled by truck and shovelled into place by hand. Where possible, the trucks were driven over successive layers of the material in an effort to obtain as great an initial settlement as possible. Attempts to compact the sawdust by hand in the constructed areas did not appear to be successful. Sawdust insulation was also used in backfilling the trench excevated for the water supply lines.

c. The use of sawdust introduced a heating effect not originally anticipated. The decomposition of the sawdust used in the embankment generated sufficient heat to raise its temperature to over 100 degrees Fahrenheit. This condition was observed at the start of measuring temperatures in the test section. Two corrective measures were attempted. On November 14, the surface of the test section was soaked with water. This resulted in reducing the temperature of the sawdust from approximately 100 degrees to 50 degrees Fahrenheit.

Soaking was continued until the beginning of freezing weather on 28 November. The cooling effect of the water was only temporary as subsequent temperature readings again indicated a rise in the sawdust temperature. A second corrective measure was attempted from the 3rd

to the 10th of December 1946. A total of 120 auger holes on two-foot centers surrounding the test elements were dug in order to reduce the volume of sawdust in the embankment. The holes, nine inches in diameter, were dug through the sawdust to gravel and backfilled with gravel, as the gravel was intended to serve as a ventilation tube for releasing the heat within the sawdust. Temperatures decreased upon completion of this work, but it is possible that colder weather had a greater influence than the replacement of a small portion of sawdust.

2-06. Source of "ater. Elements G, H, J, and K, were connected at the base to a controlled water supply as shown in Figure 5 on Plate D-2. Atwo-layer filter 12 inches thick consisting of sand with gradation of grain stes shown by curves 5 and 6 in Figure 7 of Plate D-3. designed to prevent the washing out of the fines contained in the silty clay was placed between the subgrade and the base of the element. A reservoir was installed in the instrument room by excavating a pit of 2 x 4 foot area to a depth of five feet. The excavation was lined with cement concrete taken from the same batches used for the test element bases. A galvanized metal supply tank with ball and cock valve to control the water level was installed in the pit as shown on Plate D-20. The water level was maintained at the top surface of the filter material. Copper tubing was laid from the tank to the cylinders as shown in Figure 5, Plate D-2.

2-07. Ground Water Observation Wells. Observation wells for measuring the actual depth to the controlled source of water was installed in test elements G, H, J, and K. The observation wells consisted of 3/4-inch pipe perforated at the lower end and capped at the top with a coupling and plug. A short iron rod was welded to the coupling to facilitate removal of the plug. Figure 5 of Flate D-2 shows the

installation detail. Observation wells also were installed in the north and south edge of the test section embankment. The north and south wells were used to detect a possible rise of the natural ground water capable of seeping into the cylinders. This precaution was made since the controlled supply of water from the reservoir was intended to serve as the only source of ground water.

2-08. Thermocouples. The temperature of the pavement, base, and subgrade materials in each test element was measured by thermocouples set in the positions shown on Plates D-3 and D-4. The thermocouple details are the same as used in the previous investigations and are shown on Plates 3 and 4, Volume II. Nine thermocouples were installed in each of the 14 test clements. A vertical board in the center of the empty cylinder, as shown in Figure 1, Plate D-2 was used to hold the thermocouple at the desired position. As the material was filled over a thermocouple, it was detached from the board and the lower part of the board was sawed off and removed. Precaution against the wire snapping because of frost action was made by allowing sufficient slack and covering the cables with a heavy coating of grease. The outlet sleeve of the cylinders shown in Figure 3, Plate D-2 was made watertight by sealing with battery compound. The entire thermocouple line leading to the entrance of the instrument room was buried in a trench backfilled with cinders and gravel to a depth of one foot above the top bank of ducts. Figure 2, Plate D-21 and Figurel, Plate 3-22 show several thermocouple cables in position before backfilling.

Thermocouples R and S in Figure 1, Plate D-1 were installed at two locations in the embankment outside the test elements. These

installations each consist of seven units spaced in the gravel and sawdust fill as shown in Figure 5, Plate D-1. Temperatures within the embankment were observed for comparison with those obtained within the test elements.

- 2-09. Temperature Measuring Equipment. The temperatures of the thermocouples were measured during the period 11 October to 23 December 1946 on a Leeds and Northrup direct reading portable type potenticmeter by connecting each of the thermocouple leads with the instrument separately. On 23 December 1946 a Leeds and Northrup Micromax Temperature Indicator was installed. Each of the thermocouple leads was connected permanently to the instrument and the temperature at any thermocouple was obtained by switching the proper switch. The switch panels and the temperature indicating scale are shown on Plate D-23.
- 2-10. Instrument Room. A section of Building T162 in the Reclamation Yard at Dow Field was partitioned off for use as an instrument room to contain the temperature measuring instruments and water supply controls. The interior walls were constructed of 2-inch insulation board and the exterior wall was sheathed with matched boards with the space between the study packed with sawdust. Two 3-Kilowatt electric heaters, with a thermostatic control, were installed to maintain a constant room temperature.
- 2-11. Thermograph. A thermograph was installed in a shelter located 25 feet north of the test section as shown in Figure 2, Plate D-1. to obtain continuous records of air temperature.
- 2-12. Bench Marks. Two bench marks were established for reference points to measure changes in elevation of the element pavements

throughout the freezing and frost melting periods. One bench mark consisted of a bolt set in the concrete foundation of the northwest corner of the nearest warehouse building in the Reclamation Area. The second bench mark consisted of a la-inch pipe set inside a 22-inch pipe casing. The casing was approximately five feet long to protect the bench mark pipe through the frost zone and the bench mark pipe was set firmly in the clay subgrade. This bench mark was located approximately 40 feet south of the elements.

III. OBSERVATIONS AND MEASUREMENTS

3-01. General. Observations of conditions pertinent to frost action studies such as rate of frost penetration, subsurface temperatures, ice lens formation, pavement heave, and changes in density and water content were proposed for the investigations. No suitable means for measuring rate of frost penetrattion, ice lens formation, and changes in density and water content were developed consequently only weather conditions, subsurface temperature, and pavement heave measurements were mado. A summery of the test data is presented on Table D-1.

3-02. Subsurface Temperature Measurements. Subsurface temperature measurements were made at intervals ranging from two to nine days, commencing 11 October 1946 and ending 26 December 1946. Two sets of readings were made daily, except Sunday, beginning 27 December 1946. The first set of readings was made between 7 a.m. and 10 a.m.; the second was made between 1:30 p.m. and 4:30 p.m.. The morning readings were made to measure the effect of the more constant, colder night temperature. Typical subsurface temperature gradients for the elements are shown on Plate D-5. The afternoon readings were made to determine

the time lag between the varying air temperature and the soil temperature within the clements.

To further investigate the time effect of air temperature fluctuations, measurements were made at two-hour intervals on the following days:

4 April 1947; 5 a.m. to 7 p.m., (Clear)

5 April 1947; 5 a.m. to 1 -.m., (Clear until 11 a.m.)

12 April 1947; 5 a.m. to 3 p.m., (Cloudy until 1:30 p.m.)

15 April 1947; 5 a.m. to 7 p.m., (Clear)

21 April 1947; 5 a.m. to 1 p.m., (Cloudy)

26 April 1947; 5:30 and 7:30 a.m., (Cloudy until 7:30 a.m.)

The results of all thermocouple temperature measurements are tabulated in

Tables D-2 tc D-17. All temperatures are shown in degrees Fahrenheit.

Prior to the installation on 23 December 1946 of the Micromax temperature measuring equipment, the hand operation of the potentiometer required two hours to obtain a set of temperature readings. The automatic equipment required 15 minutes. For this reason, a greater range in air temperature occurred during the time required to obtain the sets of temperature readings made from 11 October to 23 December 1946.

3-03. Air Temperature. Continuous records of air temperature were obtained from the thermograph. The minimum and maximum daily temperatures from the thermograph records shown on Plate D-6 were used to calculate the 1946-1947 freezing index plotted on Plate D-7. The air temperatures at the time of reading the thermoccuples were obtained from the thermograph recordings.

3-O4. Surface Temperature. The temperature at the surface of each element was measured by a mercury thermometer. While reading the thermoccuples for a given test element, the thermometer was placed on the pavement surface of the test section and shielded from the sun by a wooden board. These readings are tabulated for comparison with other temperature readings in Tables D-2 to D-17 inclusive.

3-05. Weather Conditions. The direction and intensity of the wind as indicated by the Beaufort scale is tabulated with the temperature data in Tables D-2 to D-17. Cloud and sun conditions prevailing during the time of temperature measurements also are shown. The test section was kept clear of snow cover to climinate consideration of its insulation effect during this investigation.

3-06. Water Table. The observation wells in elements G, H, J, and K were examined each time temperature measurements were made. The measured depth from surface of element to water varied from 7.1 to 7.3 feet.

The north and south observation wells were dry except on four separate occasions during the latter part of November and December when the water table appeared in the wells at depths ranging from 7.6 to 7.9 feet.

3-07. Pavement Heave. The heave of the pavement on each of the test elements was determined by level surveys as an indication of the frost action which occurred. The original elevation of five points on each test cylinder pavement was referenced to a bench mark not subject to frost action. The five level points on top of each test element consisted of the center of pavement and the quarter points along the

pavement circumference. Changes in elevation were measured to the nearest hundredth foot on the following dates:

Changes in elevation of the pavement at the center of the elements are plotted on Plates D-8 to D-11 for comparison with the penetration of the 32°F, temperature.

IV. ANALYSIS OF RESULTS

4-01. Weather. The winter during this investigation was milder than usual for Banger, Maine. The 1946-1947 freezing index shown on Plate D-7was 965 as compared with a 13-year normal of 1275. Rainfall and snowfall are not pertinent to this study because of controlled ground water conditions and because the top of the test section was maintained clear of snow. The direction and intensity of the wind may have an effect on the rate of drying the pavement surfaces, but no measurable i influence is evident from the pavement temperature data.

4-02. Test Element Temperatures. At the time of reading, morning and afternoon, the pavement surface temperatures were usually warmer than the air temperatures. The bituminous concrete surface was warmer

than the cement concrete during a majority of the observations. The continuous air temperatures recorded on the thermograph show that the daily maximum and minimum air temperatures usually occurred at the time of measuring the afternoon and morning test element temperatures. The hourly change in air temperature is reflected in the elements to a depth of approximately one foot. Below this depth the temperature gradient in all the test elements was approximately the same. The warming effect of the sawdust embankment surrounding each test element may have limited the depth to which the daily fluctuations of air temperature were transferred within the test elements.

The effect of fluctuations in air temperature measured at two-hour intervals during clear and cloudy days is shown in Plates D-12 and D-13. The same data is tabulated in Tables D-2 to D-17, but the plotted data are arranged to facilitate a comparison of the temperature changes of the various materials as a function of hir temperature changes. The clear day selected for analysis is 4 April 1947 when temperature measurements were made every two hours from 5 a.m. to 7 p.m. The cloudy day selected for analysis is 12 April 1947 when an unusually warm spell occurred. Temperature measurements on this day were made from 5 a.m. to 3 p.m.; at 1:30 p.m. on 12 April the weather cleared and the sun appeared at 2 p.m. However, the rain until 11 a.m. and cloudy weather until 2 p.m. offcred a good centrast with the readings of the clear day. The thermograph records of continuous air temperature are reproduced in Figure 1 of Plates D-12 and D-13, for consideration of air temperature conditions for three days prior to the time of making the two-hour interval rendings. The curves

in Fig.2, Plates D-12 and D-13 indicate the surface temperature reaction of each test element. The surface temperature of all elements was warmer than the air during a clear day. The rain on 12 April during the 5 a.m., 7 a.m., and 9 a.m. readings kept the surface temperature of all test elements approximately equal to the air temperature. However, the surface temperature of all test elements became warmer than the air after the rain ended despite continued cloudiness.

Figures 3, 4, and 5 on Plates D-12 and D-13 show the temperature changes within the elements and embankment during the same two-hour intervals. The thermal characteristics of the material is indicated by comparing the temperature versus time curves. During the daylight hours the surface temperature is higher than the air temperature and reaches its peak one to three hours ahead of the air temperature. Approximately two hours were required for changes in surface temperature to reach the midpoint of either four inches of bituminous concrete or six inches of cement concrete. However, consideration must be given to the magnitude and rate of change in air temperature. For the test period on h april there was a 20 degree Fahrenheit drop in air temperature during the 12 hous prior to the measurements, as shown on Plate D-12. For the test period on 12 April the air temporature was more constant and dropped nine degrees Fahrenheit prior to the measurements, as shown on Plate D-13. The peaks of temperature changes in relation to time are thus more pronounced for the 4 April measurements. The results on Plates D-12 and D-13 show a consistently lower temperature on top of the base course for the elements capped with cement concrete. During a clear day the base course surface

beneath the cement concrete pavement is from 4 to 20 degrees

Fahrenheit colder than the base course surface beneath the bituminous

concrete. Although there is no distinction between the various

materials for temperatures measured below approximately one foot,

consideration must be given to the effect of the sawdust embankment.

It is believed that the insulation value of the sawdust embankment,

in addition to its heat producing characteristics had a greater

effect through the side walls of the test element than the air temperature acting vertically through the element surface.

The continued effect of the decomposition of the sawdust insulation is indicated by the temperature gradients of the four elements adjacent to the thermocouple installation (S) located in the center of the test embankment. Typical temperature gradients are shown on Plate D-14. The temperature lag in the sawdust is shown and a comparison of the temperature measurements in the test embankment and those in the test areas indicates that temperatures were consistently higher in the test elements than in the runway test areas. The average depth of freezing temperature penetration was 17 inches in the test elements and 47 inches in the runway test areas.

4-03. Depth of Freezing Temperature. The frozen zones of the materials in each test element is best analyzed by comparing the depth of freezing temperature versus time. The graphs on Plates D-8 thru D-II show the freezing temperature zone determined from thermocouple readings made in the morning about one hour after daybreak. The boundary of the frozen zone is considered to be the depth of penetration of the 32-degree Fahrenheit temperature. The plotted depth of the 32-degree temperature was interpolated by assuming a straight line temperature

gradient between the temperature of any two adjacent thermccouples which bracket 32 degrees Fahrenheit.

An additional summary of the depth of the 32-degree Fahrenheit penetration is shown on Plate D-15. The maximum depth during the test period is shown in Figure 1 by a bar graph representative of each test element. The mean depth of the 32-degree temperature for the month of January is correspondingly shown in Figure 2, Plate D-15.

In the elements containing the non-frost susceptible base materials the maximum depth of freezing temperature penetration was consistently greater under the portland coment concrete than under the bituminous concrete. The freezing temperature penetration under the portland cement concrete pavement was greatest in the crushed rock and least in the sand and gravel. Under the bituminous concrete pavement there was very little difference between the three types of base materials with the penetration in the crushed rock slightly greater than in the sand and gravel and into the cinders slightly less than in the sand and gravel. The actual depths of freezing temperature penetration is shown below.

<u>Material</u>	Depth of Freezing Tempe Bitumineus Concrete	cature Penetration Coment Concrete	
Cinders	19 inches	26 inches	
Crushed Rock	22 inches	30 inches	
Send and Gravel	20 inches	23 inches	

Based up in the depth of freezing temperature penetration these tests do not indicate the insulating value of cinders compared with the other materials tested.

In the elements containing the frost susceptible subgrade materials, the frost penetration was least in the elements with ground

water with the exception of elements K and M as shown in the following table:

ELEMENT	DENS ITY	WATER CONT ENT	DEGREE OF SATURATION	WATER TABLE	FREEZING TEMPERATURE PENETRATION (inches)
G	dense	high	high	yes	12
H	loose	high	high	yes	14
J	dense	low	low	yes	16
K	loose	low	low	yes	18
L	dense	high	high	no	22
M	loose	high	high	no	17
N	dense	low	low	no	21
P	loose	low	low	no	19

A similar study of the thermocouple readings obtained during the afternoon show the same bottom depth of the frozen zone. The distinction between the morning and afternoon readings is the greater number of periods during the afternoon when the temperature near the surface was above 32 degrees Fahrenheit. These values measured during the afternoon appear as thawing periods.

4-04. Pavement Heave and Settlement. The only evidence of frost action available in this investigation is indicated by the amount of pavement heave. The pavement heave, and in some cases the pavement settlement, are shown related to time and depth of freezing temperature on Plates D-8 to D-11.

Pavement heave occurred in elements containing the frost susceptible silty clay. The heave and settlement in the elements limited to tase course material was of a negligible amount being not greater than one-quarter of an inch.

The four elements G, H, J, and K, containing a source of water can be compared for frost action with the four elements L, M,

N, and P, which were sealed off from a source of water and are considered to have water table at infinite depth. The elements exposed to a source of water developed the most heave although element H showed a consistent settlement. The elements with water table at infinite depth, heaved and settled to a lesser degree depending on the original degree of compaction. The appearance of the paved surface of element H and M during settlement are shown on Plate D-19.

the freezing period. The original dry density at which the silty clay was placed in these cylinders was fairly loose ranging from 90 to 98 pounds per cubic foot. Elements G, J, L, and N heaved and returned nearly to their original elevation. The original dry density at which the silty clay was placed in elements which heaved was more dense, ranging from 101 to 107 pounds per cubic foot. It is apparent that a poorly compacted frost-susceptible subgrade will settle during the frost melting period.

SUMMARY OF DATA

1967	80	RPACE		BASE			SUBGRADE		PATER CHI	CONTEST ST DRY WI.	
-	TYPE	(INCHES)	CLASSIFICATION	THICKNES (INCHES)	PER CENT PINER TRAN 0.00 mm	CLASSIFICATION	THICKNESS (INCHES)	PER CENT PIWER THAN 0.00 mm	AS PLACED		AS OCT
4	Stt. Com.	4	Cintere	92	- (A)	•	•	•	1	•	-
•	P.C.C.	6	Cimiere	90	- (A)	•			1		
c	314. Como.		Grand Carl	5/2	<\$	•	•	•	7		
	P.C.C.	•	Grand (on)	90	<5	•	•	•	7		
•	31t. Come.		Crushed Rock (B)	92	<1	•	•		1		
'	P.C.C.	•	Cruebed Book (9)	90	<1	•	•	•	1		
•	Pit. Com.		Send and Gravel (CE)	6	<\$	Silty Clay (CL)	74	51	Base - Subg. 18	:	
•	Bit. Com.		Gravel (CE)	6	<\$	Stity Clay	74	51	Subg. 2	2	1
,	314. Ceas.		Gravel (CE)	6	<\$	Silty Clay (CL)	7.	51	Base - Subg. 16	:	
	Pit, Com.		Gravel (CE)	6	<\$	Silty Clay (CL)	74	51	300 - 300c. 19	b 23	
	Bit. Come.		Gravel (GB)	•	<\$	Silty Clay (CL)	86	51	Base - Subg. 17	:	
	Bit. Come.		Gravel (GE)	6	<\$	Silty Clay (CL)	86	51	Subg. 20	à	
	Bit. Come.		Gravel (GE)	6	<\$	Silty Clay (CL)	86	51	Sabg. 15	:	
,	Bit. Como.		Gravel (CF)	6	<\$	Silty Clay (CL)	86	51	Dass 6 Dobg. 21	25	1
CATION											
	Send and Gravel (CE)	•	•	•	•	Semblet	60	•	•	•	
	Send and Gravel (CE)	6	•	•	•	Dominat	60	•	•		-

⁽A) Claders retained on 1/4" store prior to placing.
(B) Crushed rock 3/6" to 1 1/6" diameter.

HOPES

Precise Index was 965.

Attorborg Limits of Subgredo Cla

	Tacareto	Banco
Liquid Limit	Yaousto	3,530
Plastic Uett	20	19-02
Placticity Tudes	: 13	11-17

Specific Gravity of Subgrade Cla



Y OF DATA

	CONTENT NT DRY WT.		SITY		EGREE OF		GROUND	FREEZING	MAXINUM	MAXIMUM
AS PLACED OCT. 1946	JUNE 1947	AS PLACED OCT. 1946	JUNE 1947	PROPOSED	AS PLACED OCT. 1946	JUNE 1947	DEPTH (INCHES)	TEMPERATURE PEMETRATICS (INCHES)	PAVENENT HEAVE (INCHES)	FAVENCET SETTLEMENT (INCHES)
1		58					Infinity	19	0.1	0,1
1		62						26	0.0	0.2
7		128						20	0.4	0.0
7		130						25	•.1	0.1
1		110						22	0,1	0.1
1		109						30	0.0	0.2
Base - Subg. 18	:	116	:	80 - 90	71.	:	64	12	1.2	0.0
Base . Subg. 29	5 57	118 95	130 100	80 - 90	100	100	84	17	0.0	3.2
Base - Subg. 16	:	123	:	60 - 70	69	:	et.	16	1.5	0.0
Base - Subc. 19	23	121 92	123 103	60 - 70	61	96	Bi ₄	16	0.8	1.2
Base - Subg. 17	:	120 104	:	60 - 90	73	:	Infinity	22	1.0	0.0
Subg. 28	51.	110 94	125	60 - 90	94	93		17	0.1	2.3
Subg. 15	:	115	:	60 - 70	63	:		21	0.6	0.0
Bane 6 Subg. 21	25	124 91	13L 100	60 - 7c	66	97		16	0.5	1.7
				•				12		
								10		

ing Index was 965.

ourg Limits of Subgrade Clay (CL).

Average Range 33 29-36 e Limit 20 19-22 eity Index 13 11-17

No Gravity of Subgrade Clay was 2.74.

FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

SUMMARY OF DATA

Thermo-	Depth In	Type of		oct	DBER	1946				NC	VEND	ER IS	YE												ATE	AN	ID	TI
couple No.		material	0900	100	20	22	31	7 09.50	15	19	21	23	25	27	29	3 0915	7	10	12	14	16	24	MBER 26	27	27	28	30	1 -
2	0.3	Rit Conc Cinders	44	45	6.1	41	62	44	46	24	34	35	4.5	40	.19	10	26	4.	7	32	0920	31	0930	- 4	193	6	2,	/
3	10	11	64	15	50	53	58	32	.03	51	30	35	45	30	40	41	38	38	28	32	74	31	19	-6	18	8	2.	1
4	2.7		38	57	33	16	62	6/	61	60	100	57	33	33	53	51	48	46	46	49	48	F4 .	36	43	34	33	3	1
6	35	10	SV	30	60	1 4/	65	69	70	6.3	64	67	61	65	34	57	60	55	52	53	53	49	49	49	.19	43	48	1
7	48	11	14	34	60	63	66	70	71	71	70	2.7	15	64	6.5	6.3	63	62	8.6	58	58	56	57	55	55	55	59	1
9	75	11	60	10	61	63	65	6.8	7.0	69	55	11	+4	6.3	6.6	65	6.5	64	64	64	64	63	62	62	62	66	62	1
Tempera			55	64	50	44	6.3	44	41	, 8,4	,14	3.1	4.5	48.4	31	6.5	32	40	6 4	0.3	6.3	6.3	7,3	63	# 3	6.5	62	1
urface Ten	nperature		Clear	72	58	41 C/ear	6200	40	319	.10	14	12	14	42	13	17	32	48	3	33	.4	34	24	2	10	6	20	+
	ort Scale)			SW/		1	w3w2		7/51/	A 2	E I	Jaur Nw3	52	NW2	12/0 10%	Clear		210 vez		AW3	AV3	E18 161	5000	0-2a-	ciear	5000	C-UI	c

herms=	Depth in	Type of									J	ANUAR	Y 19	47									_					
ouple No.	feat	material	14.00	20	20	21	21	22	22	23	23	25	25	27	27	20	28	29	30	30	31	31	ı	1	3	3	4	Г
1	02	Bit. Case	34	12	21	1 14	1.1	10	16	-1	2.2		,	21	74 33	1	-270		3433	1433	OSK	1410	0745	1910	04.5	144:	0130	1
2	03	Cincors	12	13	10	24	2.4	11	19	٠,	3		-	31	.55	3/	1.77	6	21	10	31	1.41	25	35	9	37	22	
3	10	10	10	17	127	1 30	91	31	30	25	21		27		14	-4/	1	-7	21.	40	31	14	58	34	9	34	22	
4	1,4		1 11	3.5	3.5	315	3.5	20	15	43	24	20	41	31	31	31	1 1	31	31	31	31	31	3.	31	29	24	.70	
5	2 7		1	40	41	40	1,53	393	40		,U	34	32	3+	34	5.2	3.3	34	34	.33	35	3.5	35	35	35	.25	35	ı
6	7 4		1 7		4	+	10			1.7	40	410	060	40	40	4"	40	40	40	40	40	40	40	40	40	40	40	٢
7	4.0		47		47	47	4/	41	47	46	46	.16	4.	46	46	4.5	46	46	de	46	45	136	45	46	45	46	AC.	1
	4.0		177		52	32	28 9	44	< 7	5'4	54	51	51	51	31	51	51	50	50	50	50	4.7	50	CO	Co	50	Ch	ŀ
		-	18	* 7	17	51	57	47	57	37	38	36	30	96	1.6	56	56	56	33	.5.1	55	4.1	55	55	CC		6.	ŀ
Temper	73	-		3.7	51	57	49	.5 1	.51	31	37	5,4	.5 N	.5,#	30	57	50	CR	59	.50	-	57	52	61	57	53	34	1
Temper			-	11	417	-8	100	-	. 9	-1	12	3/	.29	.9.5	9.4	31	34	16	36	20		20	2/	3,		37	3/	L
	mperature		1	18_	3 10	3.0	34	9	-3	-1	13	23	40	.14	36	3/	36	3.6	7.2	-	32	29	24	27	15	12	27	1.
ther			Cloudy	Trace	tein		Cloudy	Ciasi	CIPY	CABLE	char		RAID				01.	4.9		-67	76	4/	6.7	29	15	30	27	ı
1 (Beau	fort Scole)	we	- ,	EI		14.5	1		-	W 8	Nona	50 1	Mana	1017	Low	Carra,	-land	ching	C/0 rdy			Claur		Clear		Cieur	3
				Time .			The state of	-774	A		113	MANUE	-	779/14	1161	West.	Mone	X10/	None	ME 1		None	NW4	NW3	NW2	A22	None	Ι

hermo-	Depth in	Type of						FEDA	UARY	1947							-	_	_		_							_
couple No.	feet	material	21 2422	21	22	22	24	24	25	25	26	27	27	20	28	1	1	3	4	4	5	5	6	6	7	7	8	
- 1	0.2	Bit Com	4	17	27	UK.	-24	di	17	29	3'4	49	4.	26	1.7	3.0	44	10	21.00	1.200	2450	1210	0200	14.00	1953 h	14.0	27.30	10
2	0.3	Conders	16	18	2.7	-7	20	30	19	2.0	43	4.7	12	- 24	24	2.5	36	- 10	-51	190	27	461	H	50	34	37	32	L
3	3-0	4	31	30	30	37	31	31	31	31	31	31	112	41	3.1		30	37	3.4	-36	30	37	34	47	32	55	32	1
4	118	4	35	1.35	0.6	35	33	33	3.5	43	35	7.4	14	3.0	21	-	101	116	71	- 11	21	36	34	31	31	34	34	1
5	6.7		34	19	39	39	29	35	39	37	I.A	38	39	34	30	30	1.0	4.5	3.5	3.1	33	30	3.5	35	35	3.5	35	1
6	3.6		41	-14	43	44	43	45	43	4.1	4.5	47		41	41				12.00	3.0	.3.8	3.0	30	35	30	30	30	Ι
7	4.6	- 2	47	4.0	47	47	47	41	47	42	47	100	**	100	100	1000	4.	3.2	4.5	43	AL	43	43	43	43	43	43	Ι
8	- 14	100	52	12	102	38	52	53	51	43	1	44		***	46	47	4/	47	47	46	40	47	46	44	46	46	46	I
9	7.5	42	50	15	330	14	2.4	54	64	34	12	-200	0.0	2.4	-	21	31	21	37	51	51	2.	51	51	51	51	51	I
Temper	ature		-	- 17	3.6	28	75	-	12	10	.57	-		24	10	32	14	114	2.5	53	53	31	53	53	53	53	53	I
face Te	emperature	-	10	1.6	29	1/	24	01	34	11	44	30	100		20	23	20	3.7	29	35	从费	36	14	40	36	41	32	Ī
gther	O POSSESSE		Out.	1	er.	Littere	1	12		1.00	100	200	74	21		15	4	19.7	37	37	33	40	Nw.	4.0	34	43	34	1
d (800)	fort Scale	1	1000				Naga	A Pri	Sare	1000	G/404	cong.	CHAP	40	-	1801	240	Clocks	Cienty	Sme	Chew	64 4	country	Sante	chiery	Clear	James	1
			364	(R) 2	per	100	Amon	19.	None	W.	Meril.	Aime.	14.5	None	30.50	Non C	14.4	3,5	More	NEI	W	14.002		VW7	-		NWZ	•

Thermo-	Depth in	Type of				MAI	RCH IS	947											_	_		_	_					
Caupie No	feet	material	26	27	27	20	28	29	29	31	31	0010	/330	2	2	3	3	4		•	•	•	4	4	4	5	5	5
1	0.0	Bit Can	30	19	13.	21	96	28	150		7.	3+	22	47	43	35	2330	24	0.793	0 900	1110	13/0	1500	1700	1050	0.500	0705	090
2	03	Cinders	49	100	-47	24	13	20	42	1.3	100	14	74	7	25	74	42	14	3.0	12	4.	- 11	14		51	5.	30	43
3	1.0	40-	40	29	39	120	38	74	32	34	132	40	19	43	4.7	44	1	- 26	2.5	37	83	67	31	60	59	30	31	41
4	2.0		40	100	140	32	15	130	30	197	149	40	40	100	4.	40	4.	4.		43	4,	42	44	40	41	40	40	44
5	- F.T	0	40	1 40	100	642	40	1 44	43	40	412	1 40	1	**	-	-	4.	41	4.	AL	41	46	42	42	41	44	42	41
6	2.5		197	142	1 40	641	140	(4)	40	1	100	4.	1	201	4.	41	. 0.5	41	41	41	41	41	41	+1	41	42	42	42
7	4.0	- 1	45	100	100	40	44	1 40	1 4	40	43	100	43	4.5	47	41	43	43	44	44	43	44	44	44	41	44	44	44
8	1.8	1	40	44	-00	100	43	42	40	400	45	-	43	4.1	43	43	45	45	45	45	45	45	45	45	45	45	45	4
9	7.6	1 . 1	10		10	45	100	197	100	1	48	44	48	49	42	49	47	40	48	40	44	40	40	40	40	41	48	1
r Temper	ature	_	42		100	-	100	100	100	-	50	200	11	30	50	80	50	30	50	50	81	50	50	50	50	SV	.00	1
-	emperature	1	1	1	100	100	46	130	100	100	96	38	50	41	32	31	43	2.7	2 -	2.5	41	48	50	48	42	26	31	1
egther	-	1	100	120	36	26	100	4	51	200	150	40	20	44	60	33	51	24	24	44	50	61	50	54	45	2.7	12	1
	outort Scale	-5	andres.	Car.		GMA.	-tar	400000	HIGH .	Salte:	-	5001	Sper		(20.0)	1100	E/1000	Liver		Clear	digner.	cour	Lieur	deer	c/000	Giras	char	tz
	Brist L. Selde	1	400	W. W.	100	444.5	MEE	Aure	200	Arr.	NW	we !	WZ	-14/	SWI	NW3	N3	None	Ause.	WI	N.A.	14.0	AL A	No.	None			ŧ

Thermo-	Depith in	Type of													APRIL	1947												
oupre No.	feet	material	16	16	17	17	18	18	19	19	21	21	21	21 1	21	22	22	23	23	24	24	25	25	26	26	26	28	28
}	01	Rit Cons	10		100	10	1	1 21		2.0	1	time.	- lea	4 com	1.55	1000	4,	- Car	4	Sec.	12.	30 4	2"5	350	32/1		21.	14/2
2	0.4	Cinsers	23		1.1	101	1 31	1.	100	100	de .	12.	72	1	1 63	2	Julian .	70	1-20-	122-	122	14	2.	120	73.	11.	2,	.1
3	1.2	10	.,	9.7	94	1	1 1/4	77	14.	1 41	45	16	22	74	20	77	(/	7	12/	31	ا العقد ا	107	20		21 -		12	Z
•	1.4	11	4.	v .	(e,	۰,	45	75	7.	ar.	4.	1	v.	10.	4,	77	1	1.	70	15	11.	1 2	- 21	71.	71	72	*	1.1:
5	21		d,	1/1	4/	45	190	4,	95	4.	4.	2.	7.	97	7/	7,	7.	1 4	97	3:	7.	1 1	12/	72	27	2'	40	-
- 65	0 9		0.	, Ve	V.		¥.	4.	w 2	4.	42	1	47	1 42	7.	7.	140	1 41	127	100	(1)	100	41	Za .	70	7.	7/	12
7	- 4 1	9	10.	-	4.	90	47	42	47	141	97	1 7 7	47	42	7.	1.	100	1 27 9	97	9.	(3)	107	40	-2'	16	47	17	17.
9	1		1 41		41	40	47	70	40	-2	. 40	1 70	40	w	48	40	40	2/	11	40	41	1 40	74	7/	-2/	T.	47	41
Temper	atura	P 20	1	-	41	1	-	4	49	44	9.0	70	199	20	90	40	40	1 71	1 47	111	40	91	71	76	15	77	11	77
ofern To	mperature		34		130	10	3.	130	124	45	100	4 he	1.1	Je	4-		de	13.	10	177	54	. 11	100	11		7.	2	77
eigther	mperururu e		44	0 7	**	100	10	102	42	1	1 -7	4-10	1 46	+ 77	4 . 4	42	70	44	1.12	3.	164	11		311	7.	7.	2	
	efort Scale	.)	4				and diefe	1-44	and a	an Anda	212	buinds			أرفعتسه				aufada.	made	aliada !	Andada .	6 man	madde.	100	. 4		1
			1		100	1000	100	100 8	Neg	400 2 8		MEU	4.450	Año	146-	1.100.	4000	4	Last 2.	And man				labit.			stat I	

2	-	٠	
)		ı	ME
-		•	

30	30	31	31	2	10	1 3	13	1 4		-	-				J	ANUAF	RY 19	47			_	-	_	_	-	-			
9.6	1500	0845	1000	03,0	1548	29.12	1615	0840	1500	0840	16/3	Hee	9 07/0	9	10	10	11	11	13	13	14	14	15	15	16	16	17	17	18
-19	42	+0	1	12	119	33	20	10	24	14	Jo	47	-2	14	-11	14	-	1000	0740	1200	O gate	1400	0940	1530	4913	1555	0850	1530	0730
(H	80	1.5		20	/5	31	21	12	25	14	31	22	-1	14	-10	10	-	-	12	27	4	2.5	30	3.5	30	33	27	33	20
	21	1 5	30	28	128	-4	30	29	2.7	28	28	10	20	100	-	10	-3	- 2	13	27	-3	25	24	33	29	33	28	34	20
	41	41	40	40	40	39	3#	30	38	37	37	37	17.0	61	25	24	24	2.3	26	26	24	24	28	27	3/	31	31	31	31
*	48	47	47	44	40	44	46	45	45	42	44	1/4	37	37	11	36	76	30	3.5	35	35	3.5	34	35	34	35	3.5	3.5	1 14
	54	54	54	53	5.1	53	52	52	52	111	50		44	4	43	43	4.5	41	42	42	AL	42	41	41	4.	41	41	4	
g.	59	58	28	58	58	59	-1	57	51	-0	2.0	31	37		50	50	50	50	49	44	44	49	44	41	48	48	48	48	4.7
2	82	62	62	6	61	41	16.	21	41	2.0	1850	3.6	2.0		35	55	5.5	5%	35	55	44	54	54	54	50	12	13	53	171
0	é2	65	64	42	1.62	44	12	4.5	1	-	9-0	-60	60	10	51	40	25.4	59	34	59	19	1.9	39	57	Co.		3.0	100	100
Y	10	15	12	2	-3	2.2	18	20	62	21	2.4	-07	4.0	44	61	41	61	6.1	60	60	60	66	44	60	60	28-	38	38	58
	7	7	13		24	74	23	-	25	4.5	20	24	-1	11		0	-6	7	-2	20	-2	21	34	3.5		40			60
	-	100	1000	-	-	-	23	01	-	2.5	- 6	100	1	4	-2	E	0	6	7	14		24	35	1	200	30	23	32	30
	1 45 %	miets.	Minage.		BREK.	Sprac		(MIN)	Strat	drie.	ting	400	2744	dese	211.11	Sese	2.2.48	Corne	die	ches	Mar.	-	10.	30	33	34	2.5	31	24
	141	750	(E)	100	162.0	WE!	W.	MI	W.	742	W	NW	WE	www	Sep.	NA	No.	1.2	at most	2000	Land C	11.01	100	Ren	dar	Consy	2/447	C/000	Ciear
					-	_							-6	100.00	.4.4.4	1000	11-	116	MAL	1144	NEI	1461	None.	MEI	SEL	561	WZ	WE	SWI

4							_	_			FE	BRUA	RY IS	947															
# Jn	1400	0831	1525	6	42.4	7 04,	.40	0,400	1425	0345	10	11 1722	11 . 4. 2	12	12	13	13	14	14	15	15	17	17	18	18	19	19	20	20
	74	7.	40	24	20	2,	23		25	20	33	1.6	4'0	1	34	7	34	16	34	32	20	29	23	30	49	25	1530	0125	1900
10	30	3,	3,	3,	3	3	131	32	34	21	14	1.00	38	니	34	J	31	12	32	3/	36	31	32	3/	43	27	36	12	37
C	35	35	ar.	134	1.0	25	is	35	35	185	37	10	150	38	3"	3.	29	30	29	3/	31	31	3/	32	32	31	32	32	32
2	47.7	4.	20	40	7:	70	37	33	33	31	33	39	39	74	107	35	J5	35	34	35	35	35	35	35	35	35	36	36	35
5	46	40	40	45	75	45	15	44	45	44	77	44	44	41	41	33	49	37	38	39	39	39	39	39	39	38	49	.19	39
-	50	57	50	50	33	79	29	172	49	49	48	48	28	44	46	98	78	48	48	48	48	11	44	48	44	13	44	43	75
7	57	SA.	53	57	59	54	54	54	57	53	-3	53	53	53	52	53	53	53	52	52	53	52	52	52	48	+7 52	18	47	47
7	42	77	3.	15	37	24	72	30	37	50	36	50	518	55	55	55	55	55	55	55	55	55	55	55	35	54	55	52	55
7	41	45	39	17	29	. 4	34	3+	32	22	29	24	33	9	26	12	29	20	35	39	41	25	37	27	.37	20	20	6	23
eur	Cicac	Kain	wice.	Cien-	Crew	Snow	Snow	Bare	0	Cicar		Clear	Class	Cien	30		35	23	37	36	46	28	33	30	40	2/	23	7	26
040	546	3/3	562		1100	1. 3	22	Nove			1.413		144	-145	-6 44		NW4	Coury.		None.		NE 2					Cleur	Clear	- CH-
																	كالتفا		THE .	77.77.62	34/	NEZ	NE4	NW2	UMS	1842	1143	1.42	NW3

								MAF	CH IS	947																			
8	8	10	10	- 11	11	12	12	13	13	14	14	15	15	17	17	18	18	19	19	20	20	21	21	22	22	24	24	25	26
72.	1405	14	1.700	24	10-4"	32.7	1430	1.6	1480	3701	1400	1, 11	1345	0815	1977	0100	1500	0715	1500	0705	1900	07 0	1330	0830	1400	0350	1530	0 730	26
12.	37	32	43	30	1 '7	-,	h /	32	//	.13	43	-4	57	32	57	32	51	25	64	28	56	31	60	33	60	46	59	41	44
4	32	34	34	32	132	34	31	.1/	32	33	1	2	32	31	28	11.7	47	27	60	29	60	31	56	32	55	44	58	41	42
15	35	3.5	36	35	33	3.5	35	33	35	35	35	35	35	35	3	35	33	34	34	Js	35	37	37	31	36	4/	41	42	40
	38	38	39	38	3.8	1.0	17	100	3.2	30	34	112	38	38	37	34	37	39	36	36	30	36	37	37	37	39	39	39	40
3	43	43	43	112	43	43	42	4.	-1	42	12	1	42	.12	4	42	4.	42	42	3	32	38	38	38	18	39	40	39	40
-	50	46	47	46	46	46	10	45	***	45	٠٠٠	r	46	de	45"	4.	45"		45	35	45	45"	45	45	.14-	45	45	42	45
18	53	50	51	50	50	30	50	25 %	5.	50	5.	-	50	50	4.9	3,50	44	47	49	49	49	49	49	49	49	49	49	+	49
12.	31	33	35	22	52	5 1	1	-51	52	32	52	51	52	52	ر2,	52	,C,	52	212	Ş.,		51	51	51	51	51	51	51	51
4	41	33	3.	20	40	7,	./	3=2	51	37	45	-3/	4/	35	30	2 14	26	4.5	43	-47	97	30	47	3/		45	52	41	27
~	Snow	Snow	Run	Clear	Glear	6 . 00	130	Tue	5/29	Aun	Alizon	140		-	70	1/2	70	the i	53	111	57	3/	54	35	47	51	54	43	34
12	NEZ	NI	NFZ.	NZ	NEZ	None		SIVI	341	33	33	W 4	W2	madds.	-3400	41.7	relieds"	and do	manda.	None	m b dt.	C/ea-	Clear	Clear		Clouds			Clouds
																d al.	41.5	atths.	1 2 ab 50 1	77.72	<u></u>	147	NW3	NWE	NW3	4	None.	A. L	SWA

						AP	RIL I	947												-									
22.	5	5	5	7	7	8	8	9	9	10	10	H	11	12	12	12	12	12	12	14	14	15	15	15	15	15	16	16	16
2	0900	1100	1300	0805	/330	07.5	1423	0800	1335	0800	1400	0715	1335	0505	0700	0800	1100	1300	1505	0815	1400	20203	0700	3 700	10	1100	1600	15	15
-	43	84	54	46	67	35	72	35	48	33	187	12	79	5	34	55	51	74	28	38	43	11	3 4	11	44	0,	0.4	1000	64
-	4/	51	53	46	64	32	69	35	98	33	68	1 4 2	15	53	50	55	51	11	15	31	43	17	32	46		10	12.	20	1
-	44	43	43	43	43	45	43	45	44	43	42	41	46	49	49	49	50	50	41	47	46	45	45	44	44	44	44	45	46
4	42	42	42	42	43	43	43	41	44	43	43	44	44	45	45	45	4.5	45	45	45	47	46	46				46		1
4	42	42	42	42	42	42	43	43	43	43	43	4.1	43	43	43	12	41	41	43	45	45	1	-	46	46	46			46
	44	44	44	44	44	44	43	45	44	4.	45	45	43	43	45	85	43	45	45	45	46	- 5	45	43	45	43	45	45	43
	45	45	45	45	45	45	45	25	46	25	46	40	46	46	41	46				1	-	4.	46	46	46	46	46	46	46
1	11	41	48	46	49	49	48	44	48	49	28	44	44	48	40	4.4	4	46	41	46	46	,	46	46	A4	46	46	41	46
0	50	50	50	50	50	50	443	50	50	570	50	50	33	50	50	C	41	43	48	48	40	AR	44	4.9	40	48	4.8	48	48
	43	45	45	45	44	32	45	33	36	7.3	Ca	44	61	55	6.1	5.5	25	40	30	30	50	30	50	50	5.0	50	50	50	S
	41	62	41	46	58	35	60	43	47	.9	77	1	20	20		3)	59	D.	67	35	37	37	34	47	25	55	55	51	47
,, †	Clear	01-	61.	01	-	C	30		6	4/1	17	4.	10		55	37	6-9	10	11	37	18	27	43	41	6/	6/	67	♦ Ø	50
1	55 2	CA 3	HIS WA	Links,	witter .	Cloudy.	refeur.	-1340p	Kain	Gillar.	Liear	Cloudy	-10007	44.4	Turn	12.9	Clouds	10004	at 281	Llowly	Agres	w/o aky	-lear	-lair	-log-	1000	wier	1000	. 10
	F. C	15	22	14'5	W h	MW.S.	1344	SEZ	SW2	1642	2 = 5	145	300 20	VAL 5	170 6	ev l	341	SWL		SEL	55 6	None	NWI	14WZ	NEG	NES	NWS	1442	W 2

Air temperature obtained from thermograph record.

Surface temperature obtained from mercury
thermometer laid on the surface of the povement.



FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

TEMPERATURE RECORD OF TEST ELEMENT A

DAT	E	AND	TI
10.46			

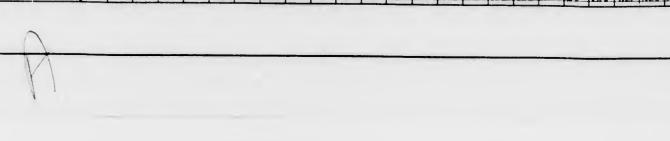
Thermo-	Depth in	Type of		OCTO	BER	1946				NO	VEMB	ER IS	46					********				DECE	MBER	1946	5			_
couple No.	feet	material	1030	16	20 /345	22	31	7	15	19	21	23	25	27	29	3	7 /020	10	12	14	16	24	26	27	27	28	30	T
1	03	Coment Conc		49	54	43	57	43	41	3/	37	35	40	43	39	19	24	26	29	28	3	20	18	-4	10	8	22	۲
2	05	Cinders	60	49	47	43	57	40	41	32.	37	38	34	45	31	18	27	32	3/	3/	19	24	19	1.3	10	10	22	1
3	/-3	"	60	53	40	51	58	47	49	45	45	40	40	49	43	36	22	35	30	29	120	31	12	27	126	23	20	1
4	2.2	И	57	57	59	60	66	62	62	60	58	58	55	44	54	51	40	16	42	47	92	02	97	42	42	42	40	1
5	30	н	57	59	64	64	69	20	67	66	64	64	62	61	61	50	55	54	54	54	10	So	50	49	79	46	41	1
6	3.8	et	58	60	65	67	71	73	72	70	70	68	67	65	65	63	1,	60	69	49	C	ce	55	59	54	51	CI	1
7	52		58	61	0.5	67	71	74	73	73	71	20	70	68	68	67	0.00	64	60	64	4.7	61	61	61	100	60	40	1
8	6.5	4	59	61	6.5		59	71	72	70	70	69	69	68	68	67	1.7	66	66	66	10	4.1	48	62	62	62	61	1
9	7.1		61	61	6.2	64	65	67	67	67	67	66	66	66	67	45		65	45	65	B		1/2	/1	1.7	62	1.2	1
ir Temper	ature		65	59	.50	49	66	50	40	40	3.4	34	52	47	36	19	2.	27	34	22		122	22	-6	100	-0	23	+
surface Ter	mperature		73	61	51	50	59	49	45	38	38	25	40	41	36	22	30	45	37	30	7 -	12	20	12	10		20	1
Veather Vind (Beau	fort Scale)		Clear	5.41	Cloor	61000	Choudy war z		Clear N 3	Gless.	Clordy E		desdy	Clear NW Z	Clear	Clear V 2	Cicar	-		Cear	Con		Saew	-			Clear	9

Thermo-	Depth in	Type of									J	ANUAR	Y 19	47									T					
couple No.	feet	moterial	18	20	20	21	21	22	22	23	23	25	25	27	27	28	28	29	30	30	31	31		1	3	3	4	T.
	03	CementCon	28	11	19	35	35	13	14	-2	12	16	24-	72	1433	31	36	45	27	24.5F	37	22	26	1411	0815	37	21	1/4
2	05	Cinders	25	12	24	32	23	20	18	3	10	16	19	30	31	31	3.2	28	20	2.4	29	3	30	20	12	24	11	1 2
3	13		28	20	26	Jo	31	٦9	26	10	11	2.0	21	30	3/	31	31	32	30	30	30	2	31	V	21	23	126	1
4	22		34	34	34	33	34	3.5	34	34	33.	33	33	33	.3.3	.3.5	33	33	33	33	34	31	29	34	34	34	34-	1
5	30		40	40	40	40	A0	40	40	40	Ao	40	40	34	39	39	39	39	39	39	39	3	39	39	39	39	39	19
6			16	46	46	46	46	45	46	45	46	45	45	45	45	45	44	44	44	44	44	4	44.	44.	44	44	44.	19
7	5.2		54	53	53	23	53	52	53	52	52	52	52	52	52	51	52	51	51	51	.51	01	51	51	51	51	50	7
- 8	65	4	57	56	56	56	36	56	56	56	57.	55	55	55	55	55	55	55	55	55	51	33	64	64	54	54	54	15
9	78		59	59	54	50	58	51	50	58	58	58	58	57	51	57	51	51	57	57	57	57	57	67	56	56	54	1
ir Temper			28	11	40	40	34	6	3	-3	12	3/	39	33	36	3/	34	26	26	20	32.	31	24	27	15	7,	22	-
	mperature		28	17	3#	38	33	9	0	-1	13	26	34	32	34	32	37	27	26	21	32	31	24	28	16	29	27	1 4
Veather		•	cloudy	cloudy	Ruin	clear	Cloudy	Oleor	Gloor	Clear	clogr	cloud	Rain	Rain	Rain	Clouds	cloudy	Mondy	Cloudy			chi				Con	1	Ku
rind (Beau	itort Scale)	WA	EI	EI	342	14 3	NWZ	NW3	wz	ws	None	SE 1	None	NEI	None	None	Novi	None	NEI				NWI		NEL		

hermo-	Depth in	Type of						FEBR	JARY	1947																		_
ouple No.	feet	material	21	21	22	22	24	24	25	25	26	27	27	20	28	1		3	4	4	5	5	6	6	7	7	8	Т
			30 3	1913	0800	1330	0140	1910	DAIS	1240	1420	0126	1900	0810	1505	0135	1410	1350	0800	1500	0830	150	0800	1400	4830	1440	0730	1
1	03	Comont Cons	-12_	18	22	29	25	15	18	25	42	25	39	22	20	21	38	32	١.	.70	28	.11	32	41	33	47	3/	Ť
2	05	Ginders	10	1	21	26	26	21	20	22	2/	25	3/0	22	30	10	2.8	32	31	31	30	33	32	35	31	3,5	32	4
3	13	- 11	25	11	. 7	26	22	21	28	22	29	20	2/	20	29	21	27	32	3/	31	32	12	12	12	32	32	32.	-
4	5.2	13	4.5	iz.	1	24	34	24	30	34	34	24	25	30	34	34	34	34	34	3)	34	3+	34	34	34	34	34	1
5	3.0	15		3/	441	38	35	20	20	28	10	28	22	30	20	38	38	38	15	38	35	31	17	38	28	38	38	î
6	3.8	, P	74	12	12	12	1:	42		42		42	42	1/2	42	42	42	12	41	41	41	41	42	41	41	41	42	Ì
7	52		7"	40	44	48_	de	14	40	41	41	48	11	42	42	47	48	47	47	47	47	47	47	47	41	47	47	
8	65	- 6		57	51	51	5.	57	50	51	50	50	51	60	Sr.	50	10	50	50	50	49	11	50	49	149	49	49	i
9	7.8		19	51	54	54	.C2	.62	62	53	51	53	.54	62	2	53	53	53	53	52	52	-63	52	52	52	52		
Temper	ature			12	21	29	22	28	12	34	30	24		22	21	25	34	37	29	35	280	3.	14	de	36	41	32	۱
rface Te	mperature		.7	15	30	31	24	37	2.2	24	10	2/	27	26	3/	35	41	37	36	7.5	.30	31	25	44	38	43	35	١
ather				Sugar	San	Saem		A .	Sam	Clean	Clear	Claude			Clear	Clour	clow	Chody	dodo	Some	· lane				Joely			
nd (Beau	fort Scale)	NEZ						None							None		SJ			wz	W = 3		MING	NWJ	41443	NWZ	į

Thermo-	Depth in	Type of				MAF	CH I	947															-					_
couple No.	feet	material	26	27	27	28	28	29	29	31	31	1	T	2	2	3	3	4	4	4	4	4	4	4	4	5	5	Т
			1350	AKED	1400	0.75	1400	01.0	14	0700	1410	0810	13:0	0815	1445	0800	1330	0510	01.5	6405	1110	1310	1500	1700	1850	0500	0705	1
_	03	Compat Con.	4	4.	1.1	4.7	41	- 6	4:	-1	52	32.	33	41	53	32	49	27	28	34	46	55	250	35	51	34	14	T
2	0.5	Ginders	- 1	on 5 "	1	-1	32	. 2	.1	31	40	3 .	42	.41	47	33	42	32	31	.1 4	35	44	46	50	50	37	Jr	1
3	_ (.3	**	38	. 1		. 1	34	.14	2.4	36	15	156	36	41	41	11	39	40	40	37	38	37	37	40	41	12	AL	1
4	2.2	- 19	40	4"	1	9	3	. , •	.38	38	37	38	38	39	34	40	40	40	40	40	40	41	41	40	40	AL	41	+
5	310	12	40	.10	JA.	1	14	40	40	40	31	10	34	40	40	40	44	41	40	40	40	41	41	41	AI	41	41	1
6	3.8	41	42	42	1.	.,	12	.12	40	42	41	-12	42	42	12	44	44	4.	44	11	42	14	12	42	42	AL	42	1
7	J' L		270	41	4,	-1-	.10	46	46	46	4.5	46	46	46	4.	46	45	4.	46	46	46	16	46	.46	45	46	46	†
8	6.5		41	1,	38	11	41	48	11	41	47	41	41	41	21	41	41	41	41	41	47	41	4/	41	47	4.7	47	†
9	7.8		50		50	31	50	50	30	44	50	50	50	49	50	.59	37	20	50	50	49	. 0	-	50	50	50	50	1
r Temper			30	17	30	at"	35	2.5	37	28	AL	38	52	41	56	31	43	21	31	31	47	A.	עריי	32.5		26	30	+
urface Te	mperature		35	20	38	and .	42	21	46	24	31	39	52	49	56	7.1	49	-	14	40		57	-	4.1	14	32	4.	+
eather		A distance beautiful	- 6	(ear	1011	"11971	Lan	104"			class		cion,	-		lan	-	10			1	-	34	44	44	+	710	+
ind (Bea	ufort Scale)	5.85	16 2.	11.4		NEL				A land of the land of the land of	NW		-	-			1000			1041	1641	T		Jear		1000	+
and the second contract	And p. 1		-				NEE	177	3	77-	MAL	100	176	0.4	341	Niv.	NS	None	None	NJ	NJ	NJ	NJ	MMS	Nose	None	31	N,

hermo-	Depth in	Type of													APRIL	1947	7											-
ouple No.	feet	material	16	16	17	17	18	18	19	19	21	21	21	21	21	22	22	23	23	24	24	25	25	26	26	26	28	T
ı	0.3	Cament Con.	_12	di.	22	51	32	50	25	41	27	73	32	44	SI	27	4.2	41	-7	+5	59	22	14.5	40	27.3.		7	t
2	05	Gilbers	21	15.	-t-	11	25	17	32	11	25	34	35	22	12	35	-71	39	16	+)	1	41	44	44	12	1	10	1
3	/ 3		20	45	7.	t	12	45	11	12	12	12	4	11	70	41	1.	45	74	45	15	4,	45	11	4)	1	11	1
4	1 2	4,	.77	15	11	71	11	12	11	11	44	11	11	11	42	12	42	44	44	45	15	11	1		45	1	-	1
5	30	- 11	27	11	11	11	71	11	11	11	11	12	11	11	112	11	11	11	11	15	75	11	44	A.	4.	17	12	1
6	3.8	16	#7.		45	15	10	11			15	25	15	40	15	15	4.	4.	45	45	4C	4.	45	40	45	4.	14-	1
7	5-2	r _b	te.	7.	10	+1	47	+2	12	47	47	12		42	16	16	47	72	17	12	192	4	40	37	47	127	42	i
. 0	65	10	41	12	12	ti.	#7	1:	41	47	47	12	47	49	47	42	4)	72	42	77	72	-Ta	47	42	42	42	12	1
9	7.0	*,	11	11	41	7.	11	11	77	49	49	41	41	49	10	45	du	41	AV	47	30	44	77	44	31	7/	10	1
Temper			.79	24	.26	15	3.	5.	21	15	24	24	3/	24	4,	7.4	C7	40	41	42	C	44	76	70	77	70	3.9	4
face Te	mperature		17	41	47	17	.79		4	SS	27	29	34	47	42	15	62	122	CC	53	12	77	12	11	11	39	- 61	4
other				i.u.				-	~	-	Cease				2			47	123	-	52	77	58	37	45	21	-1	4
id (Beat	utort Scale)		444	40	-	-	The same	NWZ		-	- Jack	A.E.		Claray			7		1244	Cara.					-	- ledi	4
					-	M 4	b	-47		1107	ANES.	THE H	NEL	1164	NAI.	MEG.	VIEL	-	SW3	JW2	-	12	Set's	NE		va'a.	india.	į



AI	ND	TIM	E																								-		-	-
8	30	30	131	T 31	2	1 0	-	Т.		-	-	-	_		_	J	ANUA	RY 19	47				_	_	_	-	_	_	_	_
96	0915	1500	0195	1400	09/12	1595	0925	1615	0190	500	014-	LINE	8	9	9	10	10	11	11	13	13	14	14	15	15	16	16	17	17	18
9	22	27	1	12	9	12	19	20	13	21	12	26	25	1	11	-7	7	-3	1	12	19	6	100	20	15,10	30	3555	20	36 11	0730
	20	20	21	22	2/	22	20	24	16	20	19	22	1	6	#	-5	6	-1	2	14	18	10	14	14	20	29	31	30	36	38
	45	40	39	21	30	30	27	37	37	29	76	36	2	2/	36	24	35	15	14	14	34	34	240	24	27	30	31	31	3.5	3.9
-	41	41	47	47	47	46	*	74	4	95	44	97	44	49	9:	42	47	43	43	+2	42	41	41	41	41	33	40	40	14	40
,	60	60	19	59	59	59	62	50	50	50	57	9	99	6	77	11	49	49	49	48	48	42	42	47	47	46	47	46	46	46
4	61	61	41		61	4/	60	61	60	60	0	17	57	56	59	59	56	59	50	55	55	53	55	55	55	54	24	54	.504	54
_	23	58	52	62	62	62	6,	62	1	64	4	100	6	61	4	60	60	60	60	60	60	60	60	40	59	59	51	57	57	57
	20	17	P	12	14	15	21	23	12	2.	23	29	29	7	11	-7	6	-6	7	-1	2.0	= 1	2.5	34	36	32	15	25	32	30
100	Citar	Car	Cue,	Court	Saew	Same	Same	3000	Ciear	Cice	Colon	Chi	Crar	Char	Clean	Clear	Cia.	Cloody	50	Chan	15	4	20	33	3)	31	32	27	29	26
2	42	MAL	SHI	NEL	NEI	NE	NEI	NEZ	Ni.	W-1	Swe	102	ANT	NA	NW	AWZ	Nes	NZ	NE	NWI	NWZ	NAI	NE	None	NEI	561	SEL	Clear	clear ur 2	Sw3

	_			T								FE	BRUA	RY IS	947						***************************************									
5	0121	1400	0830	5 4526	6 0820	6	7	1900	8	8	10	10	11	11	12	12	13	13	14	14	15	15	17	17	18	18	19	19	20	20
7	21	35	12	40	21	11	11	24	11	3/	20	22	.7	"	9	22	P	28	11	20	20	1400	2011	1925	0/30	1345	0830	1500	DRES	1900
<u>-</u>	21	23	36	32	29_	7:	21	22	29	30	21	25	10	28	12	22	12	21	12	24	2.0	30	21	2/	22	41	22	21	_2	22_
4	26	27	2/_	21_	22	32	29_	20	20	20	27	12	26_	22	32_	24	22	22	20	25	29	20	32	22	32	22	27	1/	./2	2/
-	39	90	34	170	34	34	34	24	74	24	74	72	24	24	24	24	31	24	11	22	12	29	24	11	24	39	.22	34	24	20
4-	44	99	99	99	21	19	79	90	71	39	19	22	22_	32_	.22	32_	22_	.72	19	38.	38	38	21	39	28	20	28	39	20	29
/	50	57	GI	50	50	S	Co	102	50	19	42	13	42	47	47	.41_	13	12_	92	42	47	91	12	42	92	12	12	42	16	42
4.	52	57	59	51	52	52	57	52	52	SI	S	52	52	172	44	49_	19	41_	-19	99	12	99	49	11	At.	11	11	91	#	41
6	56	56	56	56	56	S	56	56	cr	CC	.0	55	55	SS	C-	SS	35	C.	CC.	52	52	52	CA	51	51_	52	50	52	52	2
+	27	92	42	24	15	12	20	22	26	25_	22	27_	2	27_	10	26	12	21	20	30	24	41	25	22	29	.72	20	20	6	27
-	27	40	45	36	18	20	12	24	22	22	22	21	11	21	10	22	10	30	20	25	25	12	29	32	30	39	20	24	,	2.5
2	Noge	SWZ	CH2		Cur	Creat			Rain	Bain_	Class	Snow		Cher								Clear.	Clouds	Cloude	Clouds	Clear		Cleer	Char	
		UN 6	100	SW2		TAND	NEI	EL	HONE	MALE	SW2.	VINS		NAME	MW3.	Nucl_	NWA	NW1	NUL	NUZ.	None	SWL	NEZ	NET	NUZ	NWZ				NUZ

									MAR	CH IS	947																			
10	0130	1405	10	10	H	11	12	12	13	13	14	14	15	15	17	17	18	18	19	19	20	20	21	21	22	22	24	24	25	26
7	3/	18	112	41	0715	144)	0120	MIC	0320		0700	1400	0700	1345	0815	1400	0800	1,500	0715	1500	0705	1400	0720	1330	0830	1400	OPSU	1530	0100	1100
5-	32	12	12	2,	27	43	27	31	11/	53	16	43	36	47	30	14	29	45	25	49	27	51	31	50	31	44	43	54	12	36
7.		32	3 -	31	31	26	31	.11	31	16	35	34	36	29	3/	34	31	J4	30	35	31	36	12	40	.30	39	-51	47	12	14
1	34	34	34	32	whereaster would	14	32	31	JL	32	33	33	34	34	33	31	23	31	32	32	33	33	36	35	.3.	35	40	12	43	37
7		-		37	14	14	34	34	34	14	14	34	34	34	35	33	34	34	35	35	35	35	35	36	37	36	3.9	19	39	40
-	38	38	38	18	38	18	38	37	38	31	22	38	38	37	38	37	38	18	37	38	38	38	15	38	3.8	2.0	29	40	10	4-
-	42	42	41	12	41	41	41	41	42	41	41	41	41	41	41	40	41	41	41	41	41	41	41	41	4/	40	1/2	4 3	11	42
7	47	47	47	47	47	47	47	47	46	46	46	47	44	46	4-6	45	46	46	46	AC	46	46	46	AL	4.	15	71	42	7/	-
_	49	49	41	50	49	44	49	41	48	41	49	41	49	49	49	41	48	41	41	48	49	40	48	10	10	40	40	46	73	46
-	2.5	5-1	52	52	52	51	50	51	51	51	37	51	51	51	51	50	51	51	57	51	51	5/	50	-	30	50	50	49	4/	48
/	75	37	13	35	47	39	29	47	33	.50	17	44	17	47	29	14	-	10	37	13	37	42	24	47	2	50	20	3/	30	50
9	35	40	33	33	28	40	34	51	33	37	37	44	32	52	12	45	14	44	27	43	21	65	.10	4/	- 2	47	<u>45</u>	52	41	28
100	Snow	Saew	Snow	Rain	ter	Jan	cher	clear	clow	-	Para	+	clas	-	11.1			-		41	31	55	32	49	33	48	50	51	40	31
NS	NWZ	NEL	NI	NE2		NEZ	None		SWI		S3	Bu			Le Marie	Merdy.				Chor		cley	-	Jasi	Year .	· karr	Joursy	Cloudy	Run	Goods
\Box							77.7	631	5 7	34/	23	22	wz	WL			WS	NZ	1 W3	NW 2	None	52	N3	143	NWZ	NW3	El	None	NEI	5W4

							AP	RIL I	947																					
5	5	5	5	5	7	7	8	8	9	9	10	10	- 11	11	12	12	12	12	12	12	14	14	15	15	15	15	15	15	116	15
00	0/05	0900	1100	1300	0105	1330	0715	1400	opro	1335	0100	1400	3215	1325	2505	0200	2420	1100	1340	1505	08.5	1400	OSOC	1200	2000	1100	/ 7/2	1500	1400	15
-	14	40	11	Ar	14	25	34	53	16	43	3,5	61	43	69	59	51	G	57	65	62	20	41	.70	70	40	C7	1.4	60	65	172
	Jr	38	40	AL	13	145	37	42	36	40	22	12	19	57	51	57	17		57	60	Ja	4	29	29	39	44	50	Car.	CA	
	AL	41	4	41	43	43	AI	41	12	41	40	10	15	15	50		50	O		.57	#	42	42	12	42	46	42	20	15	43
	41	41	41	41	41	41	42	41	42	42	42	42	42	12	12	12	12	44	11	11	45	45	40	40	AC.	11	11	77	72	7/
-	41	41	AL.	41	41	41	42	41	44	42	42	12	42	42	1/2	12		12		42	11	41	11	11	11		+	11	11_	32.
-	41	44	46	12	43	4.	41	43	43	43	42	42	42	42	1)	12	12	12	42	42	40	44	44	44		11		-14	7.1	11
-	46	46	4.	46	16	42	46	45	46	46	14	40	16	4.	7.	44	44	4.	1	44	4/	4	4	4/	12	71	17	77	77	II.
7	47	47	4	11	11	141	41	47	41	47	42	47	47	47	4)	42	12	41	29	12	42	42	40	42	74	70	74	4.	7.0	74
1	50	50	50	30	49	41	41	44	47	44	49	49	41	44	47	40	41	44	20	40	A	10	10	7/	9/	7/	7/	41	11	17
6	31	4)	43	43	45	41	112	45	, 1	36	29	50	44	. 1	127	_(2)	ra-	1 7	1	62	7.0	22	7.2	39	-10	47	41	77	- 17	di.
4	40	-47	31	10	15	10	. 4	37	45	45	40	40	45	. 2	/	~	.7		100		27	7.4	120		7/	**	بالداد	20	-17	7/
47	1900	1000	Lords	Janks	leady	Jeir	1.19	10:40	doods	Run	Clear	4.	See	Citat	***************************************	ti sat	ń.	C		Care		20	22_	14	76		42	31	0	40
1 6-	31	361	363		W.	we	NWJ	Nuch	SEZ		ANE	Jul 2	Ju12	111	1'2	10	Mares	in tally.	SW E	Car.		JE2	Mana	1	1111	as been	1.12	muse.	Aut &	1100

		1 00
	28	28
_	20	120
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au i	12	t.
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	12	47
	17	47
	45	41
	27	4.
	31	5.0
	Jan.	12
	411	14

Air temperature obtained from thermograph record.

Surface temperature obtained from mercury
thermometer laid on the surface of the pavement.

FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

TEMPERATURE RECORD

OF

TEST ELEMENT B

-	-	A	-
- 112		AND	1114

Thermo-	Depth In	Type of		OCTO	MER	1946				NO	VEMB	ER IS	146									a:F	MAER	1946				
couple No.	feet	material	111	16	20	22	31	7	15	110	21	23	25	1035	29	3	7	10	12	14 a (ia)	16	26	26	27	27	20	30	30
	02	Bit Cope	82	6.5	51	61	56	54	43	45	42	44	43	51	45	31	37	31	36	21-	17	-	2.5	10	34	C-043		1
2	03	Sand & Grave	76	60	50	54	56	57	48	42	12	44	43	48	45	24	34	38	35	31	29	4 "	29	10	26	17	11	3
3	1.0	21	61	60	50	53	34	51	52	45	51	41	46	51	48	39	40	12	40	42	18.	p°	31	30	33	22	37	13/
4	_1.3	- 11	17	66	56	51	64	56	51	56	53	53	51	54	53	48	45	16	46	AF	45	41	43	42	41	80	11	31
5	2.1	41	57	THE RESIDENCE !	59	59	63	61	60	60	39	51	57	56	57	33	31	51	51	51	37	A,	46	4.5	4.5	44	43	10
6	3 5	- 11	57	60	61	62	66	60	63	64	6:	61	60	59	59	58	.50	54	54	54	51		50	49	46	49	45	47
7	4.5	а	57	60	62	62	66	66	66	65	65	63	63	62	62	61	60	59	54	58	57		55	.64	34	.00	50	34
8	6-2	71	59	61	62	63	65	66	66	6.6	66	64	64	65	63	63	62	61	60	61	61	0	59	57	59	59	53	51
9	1.5	et.	60	61	61	63	64	64	66	66	64	64	64	63	64	GA	63	0.5	61	63	62		50	24	4.5	60	600	100
Air Tempero			70	66	52	64	60	51	42	4.	34	35	45	49	40	24	42	31	39	24	4.5	-	3.5		10	-1	2.1	1
Surface Ten	nperature		77	73	57	66	61	60	42	47	37	31	47	46	41	24	30	31	40	24	20	-	23	2	789		2.0	17
Neather Nind (Beauf	lort Scale)		Clear	Clear .	Chaor		1	Clos	Clear N3	Cloor N2	Cloudy El	Clear NW3	Glordy 53	Clour NW2	Clar	Clear	Clour	1600	Clear Ser 2	Char	Gine.	14	SAT	(viger		Sara	100-	640

Thermo-	Depth in	Type of								**********	J	ANUAR	Y 19	47														
couple No.	feet	material	18	20	20	21	21	22	22	23	23	25	25	27	27	28	28	29	30	30	31	3	1		3	3		4
1	02	C.L. ins	25	20	32	25	35	25	22	-/	20	27	33	22	25	22	34	20	29	2	34	.70	20	74	20	74.0	31	120
2	03	Signal Grane!	21	21	20	27	39	22	25	2	25	27	30	32	3)	32	23	1	29	3/	3/	13	3/	34	21	22	20	1
3	1.0	81	30	29	20	31	21	31	11	27	27	21	21	31	21	3/	21_	31	21	21	31	2	22	31	21	2.	34	14
	1.5		15	26	35	31	25	20	29	25	21	33	34	29	30	34	20	M	25	25	2/	1	24	24	N	3/	N	A
3	1.7	44	31	38	31	38_	2/0	22	20	30	21	27	37	21	31	27	12	30	20	21_	29	F	21	20	20	21	34	111
- 0	3.3	- 11	42	12	72	12	71	16	41	45	49	11	4	91		41	41	11	9	41	11	1	12	12	41	92	41	4/
-	4.8	7	41	98	49	71	11	11	21	48	11_	41	11	97	47	47	47	10	41	41	49		42	42	41	47	#7	27
	6.2	0	50	53	17	52	117	13	12	50	17	+3	50	58	62	£4	52	54	EL	10	52	14.	52	52	82	52	81	112
3	7.5	- 11	5.	55	53	30	35	44	4	55	85	24"	85	50	59	84	59	10	59	51	89	50	59	22	54		13	11
Air Temper Surface Te	moerature		20	"-	30	40	11		2	.)	13	. Je	39	1)	A	21	N_	24	24	Jt_	JA	11	de.	21_	15	24.	17	4/
Weather	ifort Scale)	Cloudy W4	Cade.	Reca	There .		Line.			in the	A SOC	ina.		Sec.	Goods .			Clark.		laa Laa)) Linds Of the	Car Car	Clear Arra		ge Olan Add	Class	in the same of the

Thermo-	Depth in	Type of						FERR	VARY	1947																		
couple No.		material	21	21	22	22	24	24	25	25	26	27	27	20	20	1	1	3	-			5	6	4	7	7	•	
1	0.2	Set Cars	21		-/		-1	40	0.1	3	4.8.	31	1	21		11.8	4.	11	- 1			4.1	1111	200	-1-01			191
2		Sand A Grove!	24		- 7		24	34	26	لياق	31	31	,50	- 1		of the same	4.0	Transferringer		7		14	111	2/-	.22	1	A. F	-
3	10		32		Ji	41	J1	1	.17	.11	JJ	01	63 3	.1 -		8	al a	113				-	-	· 6	10/200-000			10
4	10	16	36	6	ه ال	30	35	3.5	13	.10	14.	3.5	.16	30	3.6	10.7	26	1.6		.10	10		2.5	1	-	-	38	17
5	47	- 12	31	l'	31	30	38	ir	JP	.15	18	57	21.9	30	3.2	10	27			37	310	37	2/	32	12	7	3.8	2.0
6	35	_81	4,	31	41	4.	4	41	41	ali	201	30	41	4 1	4/1	40	410	-67	4	411	411	40	4	14.	10-	-6	4 "	4"
7	16	47	46	44	45	4.	40	40	45	45	15	45	46	ad-5	45	4F.5	4	18.5	40	1.	41	41	.01	40	1	4	di	
- 8	62		50	104	50	33	19	,61	50	11	19	44	50	49		41	47	20	01	111	41	00	100	40	49	4.9	49	40
9	7.5		51	1.	1, 1	4	47	31	50	51	57	6.7	50	25	- 1	51	9	2	10			400	1 2	10	-	3 0	440-0000A	-
r Temper	ature		1	-13	45	- y-		. 1	1	35	27	-4	(Carry Contract			0.0			- 1					0.7		8		37
	mperature		10	13	31_	-11	24	41	24	31	44	21	41	-7	33	3.3	41_	110	31	11	11	40		47	3	45		1
eather ind (Beau	fort Scale)	Gloody AE 3		16.	3183	AFRE	SHL	ASAB	19.2	CHAN	PAR.		A640	19 8	11/401 Aba a	h//	1. 60		Soon	Hear.	122		Same di		الأثب	Jeew	

hermo-	Depth in	Type of				MAR	CH I	047			_																	
ouple No.	feet	moterial	26	27 C \$ 30	27	28	28	29	23	31	31	1	1	2	2	3	3	4	4	1		4	4	4	4	5	3	5
	02	Det Copy	41	26	5.7	20	770	30	(4	22.20	CA	36	1370	CAL	190	COL.	1736	2510	0701	0.600	1100	182	\$ 10	334	1850	16000	17 Day	Par
2		SAN ASPORT	96	24	43	30	53	22	45	32	57	36	54	47	85	2.	55	27	34	25	34			01	10	do	Au	111
3	12_	N	4,	3,	37	37	37	30	20	20	29	40	10	41	100	41	40	47	42	T from	West	4	4/	3	1. Carrie	Elm	44	7
-	1.1	**	43	of a	4.	+	*	42	41	11	11	92	11	*	00	41	44	4	41	41	42	44	34	40	20	The state of the	41	
2	17	- 42	11.	4.	47	9=	42	41	41	42	12	42	42	44	47	44	49	11	44	44	40	00	1 40	40	30	47	40	1
?	33		44	41	44	0-7	44	41		11	12	47	11	99	40	01	44	45	16	45	46	41	10	41	41	40	0	1
A	40	It	4.	4.	4.	10	46	46	16	16	10	1.	16	4	46	*	70	12	47	42	12	47	41	47	1	07	42	ŀ
9	2 6	b	3/	77	17	71	47	44	17	41	1/	17	17	11_	49	41	11		11	11	40	49	42	41	1	90	01	1
Temper	oture		2,	7	7	2.	39	25	34	70	77	77	6.0	-	44	4	40	41	41	91	47	45	40		41	42	91	
face Te	mperature		76	-	30	25	46	80	31	70	1/3	90	-	40	41	21	22	10	22		47	12	£ 2	40		al Record	A.	10
ather			riture.		L	-		Gen	Luc	1	Leu	- EAC	Chan	-	100	C day	6		-	44	45-	14	do -	21	-	311	-	11
id (Bee	ufort Scol	•)	Je1		14'1	ANK		bone	1	100	1116		11	C.	Ca	14.02		A COL	Almic	- Can	and the same	-det	-	-	Adva.	43.0	-	-

Thermo-	Depth in	Type of													PRIL	1947				_	_	-	_	_	_	_	_	_
couple No.	feet	material	16	16	0730	1345	18	1600	19	19	21	21	21	21	21	22	22	23	23	24	14	21	25	26	26	26	20	20
	01	Mr. Gen	-le	51	35	51	32	-44.	110	10	36	24	41	711	29	1	31	2.6	100	1	-	1	10	-	100	166	-	-
2	0.5	Semi 16mm	40	81	2.7	58	37	24	40	48	11	17.	4)	40	22	100	45	1	30	T	-	17	100	-	de	-	-	
3	10		147	47	43	44	v4.5	45	36	ALC:	44	4	-24	44	16.00	.14	44	44	47	35	25	12	1	1	4	100	77	100
4	11		44	48	41	-47	47	47	49	11/	47	47	det	4.7	-7x-	47	46	45	41	36	177	17	100	100	4	de	-	100
5	21	-	47	47	47	47	47	47	27	47	4	47	4.0	47.	43	42	-17	167	47	77	137	12	+400	1000	de	44	44	100
8	3.5	- 0	47	11	47	47.	41	47	4.	49	47	47.	4.7	17	47	42	45	AT.	47	200	170	12	+	135-	100	450	100	100
7	AP		-1	45	45	44	41	45	4.5	48	45	abs.	48	1	40	45	21	15	17	40	150	† 2)	+	4-55-4	100	100	100	15
8	o L	4	29	47	1.7.	-3.7	49	47	45	+7	41	112	-6)	32.	84	47	10.7	32	1300	40	120	+2	12	35	-55-	1	16	400
9	7.5		47	.29	3.1	17	47	47	42	49	47	25	47	13	40.	4.7	-74	31	44	41	120	15	1-56-	4-55	1364	44	155	150
Air Temper			140	1	100	4.5	0.00	11	29	1de	er.	SF	-21.7	200	47	100	2.7	1,514	100	30	2.0	++	-	-	-		77	-
Surface Te	mperature		45	-0.5	44	1,500	-7	100	4.	70.3	25	48	38	75	475	100	411		-	155	100	+£	4-	-		100	-35-	100
Weather			COL	A Stale	12.20	3.5%	-40	-	Style?	144	240	200	285	200	Conn	100	7.00	-	175	-	-	+5:	4	17.	47	40	-3	100
Wind (Bea	utort Scal	•)		167	1/2	1	100		HWL	2.44	100	400.3	545	8.5.	100	15.5		and the	200	-	en ca	+0	-	Same.	Selfa.			10
		-					-	-		100				-	-	9600	and the		A Company	A SECTION AND ADDRESS.		1.5		1.00	100	Larr	100	200

A	ND	TIN	1E																											
20	30	30	31	31	1	2	3	3	4	4	1 7	7	0			10	ANUA	Y 19	47											
140	0920	130	0251	1400	000	45.45	084	14 5	0.140	1500	0005	14.5	1100	0910	1500	2811	1515	0715	14.00	13	13	14	1400	15	15	16	16	17	17	18
7		15	100	23	1.21	34	27	24	12	1.42	1	31	30	9	23	4	20		/1	20	10	11	200	094	1540	0915	1585	1130	1530	071
- 1		37	15	2.5	27	25	ar	26	20	27	10	31	29	12	31	7	120	10	14	21	29	15	27	34	37	32	33	30	34	25
2	10	1 12	38	30	47	29	30	,10	31	30	34	30	31	21	21	25	135	23	21	31	27	26	34	21	20	1	37	30	12	25
	41	47	41	41	136	27	1	38	34	36	36	34	36	Ji	30	15	34	Je	33	33	31	31	31	11	111	111	34	14	31	3/
	41	47	41	47	47	11	4	1	40	10	47	40	40	do	40	37	31	37	35	37	17	17	37	37	12	37	17	77	31	3.5
	.63	.50	1.54	51	4.1	4.4		48	45	43	44	4.	44	45	44	*	44	46	43	40	45	11	42	42	41	41	42	42	12	36
,	31	57	152	51	57	82		52	34	6.0	51	-	-1	31	81	30	50	50	10	49	49	49	49	15	41	AF.	48	48	41	45
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	31	15	9	12	7	17				4.5	AP	39		31	57		1	57	51	57	57	57	57	52	37	56	58	56	.52	.56
	20	12	1	12	70	20	2.0	21	11	40	21	29	3.7		1			- 6	7		30	-1	23	.14	34	32	ds	25	31	30
	. 100	600	, b w	100-09	Sara	To the	1000	form	-	" mar	1000	and .	, pA11	149-	100	Com	Cian.	42			17	3	51	33	35	34	32	25	31	26
11	11	4-1	3 -	Wel	148	40	40	582	A	19.3	300	141	102	24	1	1 200	Cipa-	Stordy	Same	1000	C1001		closely	Bur	Alpin	Com	Cloudy	Cinar	- in	Clour
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34	10	1	2	35	3	12	n	12	32	14	25	31	26	34	12	M	17	32	24	M.	34	29	2/	N	W	15	29	34	20	40
	A.	Ä	14	14	14	Jir.	AL.	M	14	24	21	14	34	22	a.	H	31	1/	2	à	2	22	14	31	71	22	20	25	22	23
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**	12	17	47	0	4	52	*	5	*	4			4	*	*	*	4	*			11	1		*		1	40	11	1	41
	17	14	6	n	12	11	11	12	ti.	12	S)	B	B.	12	20	12	10	10	B	D.	50	B	0	10	52	a.	11	50	62	50
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	Qu.	ine.	An.	500	-	-	Sarw	in.	lau.	des	Com.	inen	Che.	Can.	Cen	Care	Char	Cier	Conte	Class	Mar.	Gran	Chil	Cont	Jo Chris	Char	21	22	7	Co
	100.5	47.5		-	_	de.	M.	44	Abre	****	2012	102		ALC:	Mel	ARE	Aut.	het.	AM.	NEZ.	Abc	300	ALL	Me	MEL	Nw2	Aus	Nez.	Aire	Art.

									MA	IGH I	147																			
7	× 2 2 0	des	10	10	11	11	15	12	13	13	14	400	0 100	145	17	17	10	4000	19	19	20	20	21	21	22	22	24	24	25	26
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No. 14 P	A.S	114	1-	4		20		13.	10	7.1	4		42	ile	al.	37	-	1	1	da .	112		1	4	2.8	51	45	30,3	4.1	1
Second Street	37	12	and the	1000	-	-	2	-	411	417	1	di	d	4	.3	do	4	41	ade	40	41	41	4:	1	41	31	41	44	44	44
	41	4.	1	0	4				7-	40	41	4	4	1	A.	41	4.5	4	40	41	11	41	41	41	16	42	40	11	4,	44
Д	4.1	•	10,	35	di.		47	100	4/	a	3	11	41.5	40	41	di	41	10	42	4.0	47	di.	43	4	11	43	43	114	43	45
-	41	Sin.	2.5	- 1	21	47	41		40	21	d:	37	21.	00	40	41	do	11	11	41	45	45	41	15	4.	12	41	49	48	49
		31			4.7		49	47		100		20	17	47		28.9	-579	30	100	15.75	41	47	49	49		49	40	570	4	4
5		41	33		0"	10.1			30		-	81	30	13	10	do	73	46	A Bur	35	27	57	51	53	380	47	4.	54	4	37
Mt.	3000			4	CE.		mez	- 00	100	all .	201	dest.	Ø0			mill file.	200	LIM	Stor	1010	100	- 411	Per	. 011	all or	10.01	ichnes	- Cake	111	Mary
	100	-				-				1 to	-6	or other	24	The same	-	-	Side .	A.	400	15.	401 6	Andrews.	N	2 * 1	Atta	341		Adde.	14	Sin F

							4		947																					
3	3		2	5	7	7				9	10	10	11	11	12	12	12	15	12	12	14	14	15	15	15	IA.	14	15.	15	15
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	-	22	60	21	2.	-	20		dia.	*		12	20	-	13		29	12	32	11	4	111	de	42	41	40	2	74	20	100
	40	7	-	-	-	20	32	87	400	41	-0	11	#	4	11	and .	4.3	11	21			20	4		44	53	42	80	4	61
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	40	40	1 5		-	-	19		10	20	4	10	42	12	4	40	#	0,	47	41	70	40	#1	47	47	47	41	41	42	42
2			190	-	-	-	-5	g/s	de	100	41	197	90	. Fa	4	0	16	12	#	47	4	47	1.	47	1	47	4)	47	41	11
-			100		dip	-	-		de	00	do	40	40	46	4	10	do	47	42	(4)	40	41	01	42	47	41	#2	42	97	42
	and the same of	2	1		-	-			47	41	47		07	40	41	47	47	#1	100	47	-	47	10	47	40	41	45	41	46	40
-				-	4/1	41		97	42		4.	-	49		27	20		40	77	41	49	41	47	40	40	41	41	4	49	40
			-	41	-	22	41	400	80	41	do	41	47	27	44	100	40	00	22	-	41	49	30	49		49	49	42	44	91
Acres	Same?	Marin	-3.	Zt	.Dt	0.	da	ed	d'L	do	37		44	ad .	dt.	10	II.	29		67	25	27	11	77	42	11	81	ST	54	42
	1000	22	42.		-	5.	135	47		4	44		1/4	34	11	55	17	94	24	10	21	27		42	52	4.7	67	60	50	40
-	-	100 th	neinda.		-	an age	and in the	W-000	and still g	-	-	2 to 600		-	See	6768	ides .	-bde	Case	-	Care.		September 1		Charles	100	Care	200	Laur	Class
	1.5	00.6	-00	0 0	and a	16	1640	417	Se 66	-6-	dian.	0.00		406			.46	in.	101		1	122	None.	.101	401	1107	201	Aus	14.012	100

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Hibrari		
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	- Control	4 "
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Air temperature obtained from thermograph record.
Surface temperatures obtained from mercury thermometer laid on the surface of the pavement.

FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

TEMPERATURE REGORD
OF
TEST ELEMENT C

hermo-	Depth In	Type of		OCT	OBER	1946				NO	VEMB	ER 19	46	$\overline{}$								DECE	MAER	1946	DATE	====	MD
couple No.	feet	materiai	11	16	20	22	31	7	15	19	21	23	25	27	29	3	7	10	12	14	16	24	26	27	27	28	30
			1-45	1420	1245	10.50	1040	1005	1045	0940	1035	09.50	1045	0940	1420	1020	10/0	IN SE	1020		1050	0840					1
1		General Conc		57	.56	53	600	17	15	3,5	41	139	45	45	41	23	33	14	33	37	22	21	25	9	19	097	
2 _	0 3	Sand & Garel	59	55	5/	50	57	26	47	39	1.1.1	det	43	47	42	28	34	1	35	25	29	1 35	30	25	24	23	24
5	1.3	-	50	1.56	5.5	56	61	52	53	50	51	52	17	52	48	41	4/	14	111	44	29	34	38	35	24	27	30
4	2 2	<u> </u>	50 J	57	61	61	6.5	601	58	57	.50.	51	53	55	1 5	50	1 42	48	19	50	46	137	42	42	10 1	40	33
5	30	V	1.5.5	61	64	65	67	69	63	62	61	60	59	50	50 6	56	54	53	54	1	52	111	49	49	42	170	34
6	3 9	(- h	51	62	166	67	67	67	66	65	64	63	62	61	62	60	5.0	50	56	53	50	77	-	77	-	1 7/	45
7	52	1 h	1.57	63	67	66	68	68	68	67	64	65	10	1	64	60	60	56	60	100	56	CC	52	51	-57	5)	50
8	65	. ,	59	63	1 4.5	67	67	68	48	67	1	66	1	64_	66	-07	100	6/		1 40	37	100	57	3.60	56	120	55
9	7.8	. 1	60	61	64	65	65	46	67	67	4	66	60	65	60	6.0	22-	63	63	6	62	63	60	63	50	.59	57
Temperal	ture		67	66	15	64	10	CO	41	-	20	7.	6.7	68	65			64	64	68	67	03	52	0.2	62	42	12.
	nperature	1	+	1 7	1-3-	50	13	63	7/		10	1	1	(1)	13.	. Asia		37	37	27	14	135	64	-6	12	-47	43
ther	P 31 01010	h	C 100	6/	12.4	59	1		51	-	1	(;)	4,5	46	1 37	-3	1	75	38	22	16	35	24	2	(12)	0	2.1
	fort Scale)	h	Clear	12831	itago	200	lanas	1./18149	2 7 84 .	- 011	17 6.27	16877	in Cours		Cloudy	add to Mill a		THE PERSON		Clear	Clear	Cing	SEON	Cicar	Clear	S,204 1	Clear
2 104001	or 1 Scole)	-	4	1,114	·	-	18 24/5	1,26	1	Lke- 1	4:	1783	52	1302		W.	SW			142	NW3		151	243		1323	4.2

Thermo-	Depth in	Type of									J	ANUAR	Y 19	47														_
couple No.		material	410	20	20	21	21	22	22	23	23	25	25	27	27	28	28	29	30	30	31	31	1	T	3	3	4	T
1	0.3	Coment Com		0540	19.15	Cre	1420	2150	1575	1137	15.00	1731	.7	3075	735	0920	1700	2012	CECT	147	2793	14.1	2240		A. Second	1445	3013	10
2	h	wider universal	29	18	20	34	34	17		1	11	. 25	7.4	3:	34	31	34		_r	24	3.	.,	20		. =	3/	1	T_{j}
1	62.	Sa Ma AGraige		22	27	.04	33_	40	41	7	2/	20	28	31	1.	32	33	10	20	31	.7.	33	3	32	-1	-/	1	1
3	13	*1	31	21	30	1-	J.	22	.7,	21	41	24	27	31	2.	3	32	32	34	12	3.	3,	33	33	3.	1	32	1
		£1	J	15	35	35	25	Jo"	25	25	35	24	2.3	34	24	. 5	35	35	1.0	.7.	10				36		7,	1 3
5	3.0	**	4"	4.	4.	10	49	40	4"	4.9	90	70	40	#2	3.	70	40	40	40	43	23	20	4.	,	1	1	411	1
6	38	Fg.	47	41	44	43	43	43	42	42	43	13	43	72	4.	42	42	43	47	42	43	2	-/-	43	73	1:	+3	1.7
7	.52	ti .	47	41	44	47	49	44	49	45	71	.7.	41	40	40	48	41	43	48	41	7.	_T	43			71		7
8	6.5	l _t	54	54	54	61	53	51	62	57	53	51	63	57	(2)	12	52	54	7	32	1		47	41	40	10	411	1
9	7.8	- 1	0 '	57	57	57	57	57	57	7	57	5.	56	5.	50	S		24	-	34	2.5	4 4	3.		3	22	7.5	15
r Temper	ature		.,	11	42	40	2.1	*	,	~)		7.	21	1.	-	2.0	56	- 6	-	3 1	32	. *	5.5	555	55	55	35	3
wisce Te	mperature	-		.7	30	39	22	-	-	-	-	3.	-			11	37	62	4.13	Er	20	+ 1 June		27	15	10	-7	4
eather	The same of the same		1	0		-			-	-	13	40	22	11	.14	32	- 27	21	n/	-1	3		-7		1.	-1.	12	1 4.
	fort Scale		G 642			maille.	weddit.		Gine.	with all					Litera	Saide.	-ida	-1200,	None	Galada.	Lidad	Cloudy	-lade	inchala'	6182	L. 4.	S. 5-4-	1.4
(0000			-	2/	21	2 K' 6	L' 1	11/2	JW3.	10	14.3	None	A 444	None	Lan	Nonc	None	11.65	None	10E		None		hill		10	None	1

Thermo-	Depth in	Type of						FEBR	UARY	1947											-			_				
couple No.	feet	material	21	21	22	22	24	24	25	25	26	27	27	28	28	-	1	3	4	4	5	5	6	6	7	7	B	1
			2870	14.0	4800	1230	0 - 15	14.0	0 = -	. 340	1920	OYSE	92-	31.		115	. + 1	1.75	25.00	1500	0110	15.0	0833	1450	2015	41-	11/4	
	03	enent Con	12	23	29	34	28	22	22	28	43	26	4.	72			41	. '7		34	29	.74	32	4	33	47		17
2	03	Sant & Graves	.25	25		29	23	32	26	29	12	24	33	21		- 1	-1	7.	7	33	22	11	32	25	7.	14	2 /	13
3	1.3		32	22	,73	33	31	3/	31	,3,	3/	3.	7.7	Ja				-1	31	31	23	05	11	33	1		33	140
4	2.2	1	2.	36	35	35	347	3:	75	35	35	_1,		2.		-		2	7.	35	35	.1.	Ja	2,	120	-7	,	5/2
5	3.0	9	41	46.9	39	31	39	12	31	31	31	31	11	37			,	1,	7.	31	37	34	رد	14	1.	1.7	120	4
6	38		7.3	40	4.	7.	4,	41	7.	41	7.	7	7.	~	Ψ,	4	1	4,		43	4.	4	1	3,	4		4	1
7	52		7.0	70	76	+4	10	70	4.	10	4.	7.	10	2		,		41	71	45	15	45	41	4	7	A =		7
8	6.5		53	125	47	53	49	41	12	+1	91	49		21	-	Lt.	± 41.	7,	-Z	49	411	41	47	44	175	10	70	1 %
9	7 8		5'6	. 2	.02	5.	52	12	52	54								11		51	51	6	5;			77	11	1 1
ir Teinper	oture		1	. 3	. "	28	2.	21	12	.13	2.7						20	37	**	25	-	3,		*31	2,	٧.	2	1
urface Te	emperature		9	-	. 4	3,	2)	37	22	30	39	31	271		31	-24	38		-/-	39	21	70	30	43	0 0	1.	14	100
eather	Temperature face Temperature other d (Beaufort Scale)		ne?dily	rules .	Jane of	Sura	w. z.4	C.	إستبيا		GEL.		infant.	2,	man de		39	37	alan.		Cien		Cloudy	44	all ada	4.5	3104	4
no (Bed	utort Scale	1	1657	01027	462	242	None	34'-	None	,4 °m	MAG	None		None		None					N -	and is		-	anda			1

Thermo-	Depth in	Type of				MAF	CH I	947								•												_
ouple No	feet	material	26	27	27	28	26	29	29	31	31	-	-	2	2	3	3	4	4	4	4	4	4	4	4	5	5	Т
			. 7		1700	* 7/5	200	177.	17:	1111	1. 1.3	***			. 77.	3000	130	5.0	0705	112.	2						124	
1	03	Coment Cin	1.	P.A.	4.	~7	11	37	P#	135		17		11	27	. 1	J.	./	31	4)	7	5,	17		,	- 1		+
2	0.5	Sant 6,016.	di.	A	20	1	17	J.	34	10	4)	+7.		7.	t.	13	++		36	37	13	7,	10	***	1	7 -		1
3	/ 3	4)	1	11	32	30	35	182_	3.	. Se	33	61	50	1-	7	1.	7 '	42	41	7.	1.	11	1.	2.	1-	1.	7,	1
4	4		1	11	- 10	31	11	12	34	11	101	1'	15	#1	4.	1	7-	7.	٧.	4.	1-	7-	7.	7	6-	-		1
5	4 1	11	- Line	43	77	1-	7-	7.	7-	10	7.	1.	1	43	7	7.	1	21	77	41	17	47	77	77	77	78	12	1
6	5 A	11	.11	ti.	\$3	43	73	77	1.5	72	1-	13	fn.	7,	15	45	05	13	15	45	10	175	77	1.	77	,	11	1
- /		- "	1.	11	70	1.	4.	7.	7.	1.	4.	ø.	4,	у.	P	10	P+	9 9	7.	7,0	40	7,	7.	1,	71		7.0	1
8	0.5		1.	4"	10	77	40	10	41	10	+1	97		40	90	**	7"	78	40	++	7.	4.	+1	41			7.	1
3	7 1		3			2)	53	- %	3 9	33	7.	45	-	177	50	2.5	7.	5.4	23	5.	, "p	. 3			J	23	. 1	1
Temper			.7	111	5	2,1	30	6.3	20		1.	3-	V 0	r.	5.	J.	75	- 3			4)	7.	., ,	**	1			+
	mperature			-1	. 1	-1	42	28	4.	24	41	34	55	41	3.	32	. >	25	32	40	50	21	"1	70	7	2.	N'	+
other			وله أسم	asac .	- sale	antinde"		- ste	nand.	Trotte	2100	Step & .	-		184.18			atu te		mi mali	226	1.404	-		Mada		-	1
id (Beo	utort Scal	•)		1.1	11.7	bald's		Noni		Lest's	446	40.06 %	de		1			None	dienes						None		me mde	İ

hermo-	Depth in	Type of													APRIL	1947										-		
ouple No.	feet	material	16	16	17	17	18	18	19	19	21	21	21	21	21	22	22	23	23	24	24	25	25	26	26	26	28	2
1	(* }	Cameal Care	30	1"	30		7725		31	+1		33		7.	2 -	31	2	7.	- 1	30	0.7	17						+
2	03	Sandt Greet!	v.	11	Se	11	41	11	4.	43	4	,		2	11	31	P 1	1	1.	74		14				:		1
3	- 1	PI .	1.	70	71_	11	44	15	15	79	11	14	10	7,	72	23	40	71	15			1.	1.	7		11		1,
4	P.,	A	+.	7.	75_	15	75	45	10	20	white	7.	2-	10	13	75	72		79			7.		7.		1.	۴,	1
5			+1	#2	12	17	11	4.	10	7.	1.		10	12	7,	10	7.	17	7.		1.		1,	1		+	P	1
h			4.	70	10	4.	1-	40	45	to.		71	7.	4.	46		11	10	t.	P 2		11	7.	1.	1,	4,		T
7	1.0		0.	11	77	11	41	+1	4:	47	1.		7.	4"	7.	1.	7.	٧.	zi.	r,		17	,	71		1	10	1
8		*	1 .	11	11	17	41	11 -	10	11	11	11	1 .	4"	7"	.2"	71	12	71	11		10	"			P-	11	1
9		p3	-	1.	- 3	23	.")	11	Ċ.	50	. 5,	2	2		2.1	12.	7,	7,	7/	11		7	7	7	11	1	**	
Temper		-	21		41	4,	00	5.	1 . /	11		~ 0	1	اه	,	-1	-	7	7	+1		11	52	w,	de	.5,	1,2	Τ,
	mperature		11	+7	47_	4-		40	4.	50			N. a.	ø.	1.	77		150		2.7	59	52	50	1.5	71	11	-3,	1
other				. وعملت	mends .		olimbe	وانفنته	مالند	Cloudy		20.00	142			Plant.	E/Ad.	11/1/2	4.44		Charle	Charle	and the same	Clark	C	CHAR	Sec.	1.
a IHear	fort Scole	1			16" m	a da	F 11			18.07	224					44.0	1 1 al a					1.00	SWZ	-			1.15 7	1

T	30	30	3.1	31	2	1 2	1 3	7 78								J	ANUA	RY 19	47											
	095		284		01.	1545	5990	1615	0840	1500	2840	, 620	1100	9	9	10	10	11	11	13	13	14	14	15	15	16	16	17	17	18
+	ZF	25	12	20	18	21	13	24	18	25	19	28		9	17	3	1777	070	1900	0740	1500	0830	1400	0993	1570	085	1855	0830	1530	102
1	30	31	21	24	22	24	5.	28	24	27	22	28	12	17	21	/3	111	12	15	20	25	114	22	29	12	29	33	21	21	22
ł	34	33	32	32	30	30	3/	31	30	30	30	30	1	30	24	27	26	25	24	29	20	27	27	28	30	30	32	31	31	27
1	46	75	39	39	37	37	37	-2 L.	37	37	30	36	30	37	36	35	35	34	34	23	33	27	22	22	30	3/	31	2/	32	21
1	50	79	76	49	45	44	44	44	42	43	43	43	43	42	43	42	42	42	11	40	40	40	40	40	39	40	34	34	25	28
1	55	55	55	55	54	54	54	49	47	4/_	46	16	14	47	40	16	46	45	145	44	14	++	44	43	43	43	40	43	40	40
1	57	59	59	58	59	50	58	53	33	33	32	52	52	52	52	51	52	57	51	50	50	50	50	50	49	49	49	49	17	12
1	e.		1 ./	5,	6/	61	61	60	60	57	58	56	56	57	50	56	56	56	56	55	55	55	55	55	55	54	54	34	44	79
T	. 7	18	-5	2	7	17	22	18	O.	25	60	54	59	59	59	59	59	59	57	58	50	50	58	58	54	58	50	52	54	57
I	23	. 7	8	12	4	15	21	23	12	23	9	9 pt	24	-/	7	-0	6	- 0	7	1	20	7	23	34	1,	32	25	25	32	30
1	183:	Clear	Chas		Same	Sasa	Snow	Sana	Clear		Ciana	Claudy	43	Clear	-	-5	5	7	7	6	15	1	20	3)	22	12	32	27	29	26
	AJ .	1.13	SAY	131	16	NEI	NEL	182	111	W3	5 M/3		A W 7	A/T	A dis	Clear	Ciesa	6,090,	Sugar :	GIGAT	Gur.	Car	Clouds	Ran	Rym	Gear	Com.	Gicar		Cle
٦										10	212	54%	1111	113	14 27 7	NW 2	143	152	,1/2	NWI	1842	121	INE .	None	15.	SE.	SEI	We	112	5,4

. 1	_				,							FE	BRUA	RY IS	47															
2	3421	4	5	5	6	6	7	7	8	8	10	10	11	11	12	12	13	13	14	. 14	15	15	17	17	10	1 10				
	3.0	21	4.	1563	377	1.470.0	ari	1433	24.0	1405	3440	1970	07:0	14.1	27.5	14.5	37.13	1250	0120	41.	120	711	04.2-	142.	1032	18	19	19	20	20
	-	34	37	3	23	34	47	21	3.7	22	27	31	2,	12	15	31	1.4	2.	٠.		2	., , =	21	27	148	-	287	12 35	200	74.35
	32	32	33	Js	3.3	22	126-	100	32	34	31	31	27	2,	4	28	6.6	-7	6.6	40	30	3-	132	1	2	-	21	30	-73	-27
	.7.).	120	3.	-	1 7	-	9.	30	34	23	33	22	2.	1.	31	31	20	22	214	3.	1.,	132	1	72	22	2	23	24	27
	40	4.	40	10	41	20	42	115	30	Ĵ p	20	10	38	30	36	38	25	35	30	119	34	25	2	1.	7.	2.	2,-	21	,	2/
	43	10	45	73	4.	177		4.	7.	-	23	20	43	4:	40	43	N	40	40	27	10.	40			70	12	31	40	40	40
	41	11	7	48	47	47	47	42	41	22	42	42	42	4.	42	42	42	42	42	4,	12	+	4.	1.	7.	41	4:	42	42	1
	12	52	٥.	1		5".	3:	13:	70	7	25	4/	6	4	4/	47	42	47	46	1.	4.	10	14.	4.	10_	44	45	1.	4,	40
	18	43"	."7	5.	274	3"."	5"4	19	519		14	34	54	54	54	3/	50	50	50	30	50	50	50	5	570	59	44	50	50	49
	. 7	7.	4)	3.	10	- 13	23	33		1.0	11	27	2		12	3/	53	54	54	57	57	51	57	57	53	57	25	57	57	57
	47	4,	45"		1.0	-1	- 1	32	.14	3.	-	27	19	-1	10	10	14	30	20	24	24_	11	55	32	27	37	20	20	,	23
	See.	illat .	10.		ales.	WANA	Luca	Sular	4 4 4 10 7	Lane	-	-		-	(CI	61		25	.24	14-	12	27	1	.30	31.	20	24	7	25
E A	lone	ad'a.	1.5	1.15		12012			None			2.22W		11.41			Clear .		Carren .	CICAL	Gear	ciear	Glaudy	Casada	wednes.	- Car	C. C.	Cen	Cisa	Cisac
								1						41.71.	4142	N4.	11.00	Net.	11012	14145	Maye	SW5-	NEZ.	unt.	42	NWE	18.002	103	1.02	442

-									MAF	ICH IS	947																			
	8	8	10	10	11	11	12	12	13	13	14	14	15	15	17	17	18	18	19	19	20	20	21	21	22	22	24	24	25	0.0
-	42	1 435	270	1450	1/4	215	2824	+12	10-11	422	1/11	123	0722	1395	08.5	1400	1800	1500	220	1500	3200	1400	1221	177.	22	140	24	24	25	26
		21-		1		#2	30	51	33	51	21	4'	17	1-	2	47	24	45	23	49	29	52	72	-	22	40	OFF	11.13	0133	1115
	15	الواب.	wt	40	111	11	133-	7-	33	11	11_	40	31	7.	33	40	32	34	12	11	33	4.	2.	1	23	45	1	47	**	40
,	-	12		4.4	14	1.2	2 2	22	47	39	J. B.	31	20	31	24	32	37	32	27	130	30_	31	34	30	34	38	1.	12	41	
	7	43	112		17	121	1.7	5/	27	-1-	4"	73	42	1.	43	39	40	29	31	39	40	34	40	45	40	31	41	11	4.7	12
		4	73	7.	73	73	1.	43	4,	7.	1/	1.	4.	4.	12	1.	12	41	42	42	42	4,	12	42	12	42	42	41	41	44
-		1	179	7.00	3	7.	4,	71	7.	y	7-	42	46	+.	<i>il</i>	<i>\$.</i>	42	42	42	42	42	42	42	42	43	12	45	13	13	44
1 %	7	30	1.	13	72.	7.	10	1.	70	10	45	45	10	15	fo.	7.	7:	1.	45	45	45	45	45	15	45	45	45	10	70	10
13		-	5		1	1	77	7/	7	2"	7,7	42	4"	70	ir.	17	j'r	40	70	46	40	4×	40	40	45	18	45	19	4,4	42
	-		31	1,-	-	7,	,	- 1	1 3 1	2		27	J 3	-	D.	41	31	50	5	5		50	5"	50	50	50	279	373	5-1	5
	15	40	34	31	-1	40	14	51	2-		3/	44	-/	47	-1	3,		3:	64	11	-1	7/	21	47	31	41	15	5.	4,	
5	20.4	. 4.2.0			1	1	1	1		- 3	1	44	37	54	Ja	4-2	-	4-3	-2	17	J	575	3.	11	33	11	50	51	40	37
Ι.	1 -6	16 -	NI	a o data	T A seed.	140.47	None	acc &	= 1 2 41	51 c & .	i due	ritul.	adha.	- 1 5 da	militado.	. ولماند	بالمسته	as add	ad a 60		To make .	was week	w = 4_	Cate .	College	Senda	Jour	-/244	Ru	- 2422
1				r		11,00		and .	n 15 a	444	b . M		مكاه	· 1			16 1	14 8	ald L.	add.	None	20	at 2	AND.	NW2		21	None	116.	· N2

						AP	RIL I	947																					
3	5	5	5	7	7	8	8	9	9	10	10	11	II	12	12	12	12	12	12	14	14	15	15	15	15	15	15	15	15
	10	4.	90	7.4	1	1	-	34	77	77	17.5	4/	1.66	335	0215	2120	42.	1303	23.	281	1400	0505	0.700	.1/A	1/30	1300	1530	1641	10,00
	2.1	7	11:	17	de		7	141	77	37	41	75	T	and the same	32.	59	72	33	65	42	43	34	40	45	51	64	46_	w.J	56
T .	10	7-	1-	11	11.		1	70	12	4,	4.	15	10	40	75	47	41	40	41	di'	45	11	42	12	45	50	27	00	35
1.	*-	13	43	11	10	11	73	27	77	13	45	74	it	7-9	7.1	7.	4.	10	1.	10	45	45	45	45	72	11	12	46	1/
17	77 -	1 7	12	40	77	11	4.	73	7.	4.	1/3	7	70	10	40	16	te	to	40	41	11	17	47	47	97	43	15	45	45
77	12	7.0	17	77	72	-11	11	71	Popl	y."	45	43	٧,	7 -	45	15	45	45	45	1/3	to	70	46	10	1.	40	46	4.	10
10	- Id	10	10-	T.	19	te	t.	7,	70	70	41	40	4	4.	42	41	41	41	47	47	4:	41	41	7/	47	17	47	41	41
1 1	11	1,0		76	7.	17.	1	15	75	70	50	27	48	70	47	45	41	4.	+5	40	7. 7	4)	10	40	41	11	1)	40	411
1	13	4,	73	73	77	1	1	-/	2	77	-	44	12	1.	44	+1	41	11	7,	50	4/	50	50	50	10	275	55	13	50
7,1	7.	15	47	43	1 - 5	1.1.	1	7/	13	1.	62	44		2,	24	23	37	-00	2.	32	3,	37	31	4.1	34	3.3	33.	54	47
- and	to a under	1		I wait I	1	1		and data	La.			200	7.3.1	1	12	1	45	/.		37	38	37		41	50	1.	21.	55	40
1.1.	120-	SE3	1	1.1'-	1 st 42		21117		14'4	1142	JWm	Sela	a dia	446	2 Sedent	3382	sendag .		alda.	- 246p	dada.	Alene	- 64	*44°.	- lade .	23 may	14 hate .	idadi.	
			-	-			1				-			2112		7 87 6	216	2.6		of St. St.		~	184/		1149	4843	11/3	and.	Wa

11 17 18

Air temperature obtained from thermograph record. Surface temperature obtained from mercury thermometer loid on the surface of the povement.

FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

TEMPERATURE RECORD OF TEST ELEMENT D

hermo-	Depth in	Type of		OCTO	BER	1946				NO	VEMB	ER 19	46									DECE	MER	1946			
ouple No.	feet	material	- 11	16	20	22	31	7	15	19	21	23	25	27	29	3	7	10	12	14	16	24	26	27	27	28	30
			1030	1045	1355	0915	1430	1045	1000	0920	1045	0925	1100	0915	1450	0155	0950	1100	0950	10.15	2000	Apac	1940	0840	, 500	0450	2214
-	0.2	Bit Conc	74	52	66	51	64	55	46	37	41	32	48	41	11	21	34	43	34	35	2,	6473	24	()	22		24
2	03	Crushea Rou	6/	51	59	40	63	47	46	33	12	39	45	++	44	22	30	12	32	32	2	35	25	0	2.7	13	25
3	10		59	56	54	55	60	58	53	49	51	51	45	51	49	40	39	45	42	47	38	CP	711	35	34	32	1
4	. 3		51	57	57	58	64	57	58	55	55	54	51	53	54	47	15	46	40	48	45	4.	42	41	40	34	31
5	27		5.	54	61	60	66	63	62	57	60	58	58	57	58	54	51	5.	5.	52	C	47	17	4.	1	15	32
6	35		59	59	62	64	64	66	66	64	64	61	62	60	61	59	56	56	56		55	T1	6	52	40	-	11
7	4.8		59	60	63	65	67	68		66	66	65	64	62	63	62	60	59	62	57	58	50	60	55	or	SE	17
8	0.2		59	58	63	63	66	67	62	67	66	66	67	65	65	64	62	52	62	-		33	Ca	54	59	600	
9	15		60	61	63	63	67	65	66	67	66	64	64	64	65	63	64	63	63	63	62	61	37	- 0		554	37
Tempero	iture		65	10 B	56	48	06	50	47	34	7.0	in	53	46	16	00		4			66	***	46	8.4	64	6/	10/
face Ten	nperature		75	69	60	49	42	54	49	4.	38	33	62	40	30	2/	3 h	15	39	- 7.	-	3,				1	43
ther	•		Clear	Car		1000								Printer a second					1/4	1.	-4	155	2.0				-
d (Beauf	ort Scale)			JW/	W. P. M.		14/5 W.		1 3	Clear			6.2103		Ladua,			Coudy.	meder	ne Cali.		- Lane	tauldi	made.	a habit	142 M	am 4 als
				DIE			WSW2	18.40	16.3	176	E	AWJ.	52	VM5		N2.	241	NE I	W	1141.	345	1.		10.00 5.	1 .	1163	1.1

Thermo-	Depth in	Type of									J	ANUAF	Y 19	47														
couple Na	feet	material	18	20	20	21	21	22	22	23	23	25	25	27	27	28	28	29	30	30	31	31	1	!	3	3	4	Τ
1	02	Bit Conc	35	. 7	33	Ja	34		-	-	1				1	3/	1.	100	0155	14.10	2842	1417	2745	1410	3815	1445	0130	-
2		CryshedRoca		18	3.	33	34		44	1	14	-7	AT.		14	-	15	17	48	30	3/	43	24	45	13	41	26	1
3	10		33	J.	.74	32		. (1.	1	1	15-	R. San	- T	32	.3	33	33	21	37	39	28	38	1//	34	26	17
4	. 8	el .	25	35	15	15			1	1.	35		1 3	11	-	1 3.5	13	35	1 .	37	33	34	70	34	137	33	33	1
5	27	n	32	4.	4.	39	31	31	41	7.	31	1	1-2-	-	1	38	119	10	39	36	Ju	10	42	36	73	30	36	13
6	35	19	77	44	17	44	77	44	71	71	79	7.	43	-		42	43	17	37	41	192_		-	40	43	43	40	1
7	48		10	40	47	77	11	-	+>	10	4.5	**	12	11	47	47	1	47	47	47	47	43	4/3	47	43	44	43	19
8	62		.7	-4	54	54	59	. 7	."9	82	-3	155	-1	11	1	43	51	53	52	177	52	53	41		₹ ?	47	47	17
9	75			57	57	57	57	51	57	.7	57	1 2 4	CI	1		3.	5.	3 10	-	50	50	-	76	n	36	25	32	5
r Temper			.1	12	43	7.	-14	•	3	-2			31	7.	J.	11	3+	26	20		2.0	36	25	56	3.4	5 h	56	50
urface Te	mperature		.0	5	30	37	33	1	,		12	1	4.1	1	35	31	,la	28		. ,	36	39	64	27	15	23	27	40
odher nd (Beaufort Scale)			Clausy.	Eldp.	-1443	sua.	13.	Senda.	and a	vitae.	Sade	Nene.	that	1	Itul	Cloydy	-10441	Closon		- 540,	.Page	Nonc.	-15 m/2	31 -140 Add.		Jo Clear A E 2	Cieus None	CI

Thermo-	Depth in	Type of						FEBR	UARY	1947	•			-														
couple No.	feet	material	21	21	22	22	24	24	25	25	26	27	27	28	28	- 1	1	3	4	4	5	5	6	6	7	7	R	T
			0831	15,0	0800	1330	0840	140	0815	1340	1420	0825	1900	0813	505	0925	14,0	,350	2420	و د اد	24.7	, .	1000	1	A	120	0274	
	02	BIT. CONC.	10	19	24	3/	26	47	20	35	53	29	53	27	44	20	50	10	30	#7	70	41	1.25	62	7.0	200	77	+
2	03	CLUSTERRA	-14_	15	24	29	22	40	22	30	45	27	44	20	41	28	49	30	3/	43	21	42	1 .,	21	70	3	11	1
3	10		ננ	33	3/	31	32	32	33	32	12	32	34	31	32	34	32	4	35	35	75	3,	1-1-	77	30	72	70	17
4	/ 8		Jo	Jo	35	35	34	35	35	35	35	35	36	35	15	35	35	30	30	3.	37	,	37	77	72	70	70	7
5	27	4	37	34	34	39	38	78	38	38	39	38	39	38	35	37	38	19	39	7	19	39	27	39	39	1.	12	1
6	35		43	42	42	42	12	42	42	42	42	12	43	12	12	12	12	12	12	1/2	12	12	12	12	12	70	12	1
7	_ 48	-	40	40	45	15	45	45	45	15	45	45	46	45	45	15	15	15	45	15	41	W	I di	41"	45	46	40	17
8	62.	-	50	50	50	50	50	50	50	50	50	50	50	50	511	50	50	50	40	17	++	+4	42	170	44	170	40	173
9	75		53	53	53	57	5)	53	53	53	57	53	53	53	53	52	33	57	52	52	52	32	1	25	52	-	52	12
r Temper			7	13	28	28	22		14	20	39	21		212	28	25	38	12	21	25	J. a	7.	1.4	34	32	32	12	+ =
rface Te	emperature		1	3	29	31	24	41	24	32	42	30	45	27	33	25	12	12	37	+2	33	40	31	7		*	- 2	13
eather			C. Na.	Sugar	Snow	Same	Ciear			-		10.1	Clear		4						33	40	36		.'7	17	34	1 3
nd (Beau	ufort Scale)			AEJ		None		Mone	112	Aug 2	None	a reur	Alana.		a feet	willet.	meddly.	iea,	- 12 at	m.Call		وجمديا	alsay	miles.	184	Snew	268
			41000	1124	1143	SWI		346		WE	1100	Mone	103	POTE	4 4	Nonu	47	25	None	1864	N2-	1142	-	- 4 W.L.	Aws.	1001	1112	1 3.

Thermo-	Depth in	Type of				MAI	CH I	947																				_
ouple No.	feet	material	26	27	2.7	28	28	29	29	31	31	1	1	2	2	3	3	4	4	4	4	4	4	4	4	5	5	7
			di	1500	1711	1205	1433	0730	1405	2730	14,2	2813	1330	2815	.115	0 200	1510	5.0	025	-1.	1112	1313	Can	1700	1463	2611	-	١,
1	02	Bit Conc	J7	20	57	23	76	30	64	33	23	41	77	45	07	+0	75	25	33	5.	p "	22	7.	144	483	1000		+-
-	03	GrusheaRock	-47	21	11	24	54	28	46	31	62	35	45	43	60	35	1 02	31	32	4.1	5 4	6.	10	63	22	20	200	+
3	10	N	42	40	78	30	31	40	38	40	39	42	41	14	45	14	45	45	94	43	43	44	41		15	23	35	+
4	18		43	42	41	12	40	41	41	41	40	42	42	43	41	11	11	4.	44	44	++	44	44	11	15	70	46	+
5	27		44	79	43	47	43	42	42	43	42	43	1	13	41	11	11	14	44	44	++	44	44	44	44	15	45	+
6	35		4,	75	45	11	44	44	44	11	41	44	++	44	44	43	15	45	45	40	40	45"	77		-	24	44	+
7	+8		44	10	4.	44	40	46	46	46	45	10	10	10	46	10	do.	fo	40	-T2	1	_	Ji_	15	45	15	45	+
8	62		44	49	44	19	41	49	49	49	48	40	40	44	49	19	41	4.0	47	47	44	49	12	10	10	70	44	+
9	75		51	51		51	5/	51		5.	50	50	- Z /	570	0	C2	77	50	17/	St.	47	77	47	17	44	44	+1	+
Temper	ature		30	17	30	20	38	25	79	28	12	16	(3	4	1.7	1	4.6	2.	22	24	3.0	100	10	30	30	50	50	+
rface Te	mperature		35	23	36	24	46	30	51	30	52	41	CE	73	10	23	52	24	33	3/	7.	48	2.3	41	42	26	31	1
ather	8 62				-	1	+		-	•	-	41_	3,		57	-	1			73	23	31	30-	53	4.	33	17	1
nd (Bea	ufort Scal	. 5	dade.	-41			Cleur.	cleer	1001	ALCO.		Luk.		-Judy	Lacay		we bed!	Car.	1	mi sti	acc	as tall		at see.	5.64.	tule .	Clear	1
			of the T	# 2	WI	A she a	ALE.	None	346	14x2	ald a	13013	A'E	208'0	-S#1	distal.	12 1	Noze	Nose.	43	1185	141	u.J	116/4	Abre	None	1.51	1

	Type of												-	PRIL	1947												
feet	material	16	16	0.274	17	18	18	19	19	21	21	21	21	21	22	22	23	23	24	24	25	25	26	26	26	28	1
02	Bit. Con.	1	2	33	65	17	68	40	59	32	35	47	37	67	51	84	16	61	50	64	44	7/	93.65	-		100	/4
03	Crushed Rock	3	4	35	55	35	63	40	50	36	36	41	33	57	12	70	43	34	16	.14	45	11	4/	11	79	12	
10		41	42	45	44	de	45	47	4.	40	16	13	43	15	15	45	19	47	18	18	40	18	30	372	34	AP	1
3		7 4	77	47	40	16	46	47	40	#7	47	36	16	16	16	16	48	48	18	18	30	JA	-	41	10	-	1
27		#7	+7	47	47	47	47	47	47	47	47	07	47	47	47	47	47	41	47	47	47	19	14	43.83	10	19	1
35	-	47	4	77	47	#7	47	47	#2	47	17	47	47	47	47	47	47	47	17	18	47	40		25	15	14	1
48		41	177	77	47	47	#17	47	+7	17	17	47	47	48	47	47	48	41	48	18	47	44	11	21	02	29	1
6.		47	47	44	47	41	44	11	11	44	46	14	44	44	24	21	24	44	14	14	11	14	19	44	47	14	1
75		50	3	50	510	50	5.	51.9	S	53	30	30	30	30	50	30	50	50	30	31	30	30	30	100	36	50	1
iture		12 -	i ir	38	41	16	25	31	45	26	26	31	16	40	3.9	52	10	16	17	34	21	52	-11	14	23	27	13
nperature		#3	40	44	54	1.79	34	43	52	20	27	42	34	25	14	41	.17	54	52	. /	ad it		2.0	-	-		1
		1040				-	-	-		4	10 1	land	10.4	-	-	with rolls of relation to		-					Account to the	47	13	1.00	6
fort Scale)					The state of the s									STATE OF THE PERSON.	-	. 10.00			de las eles					1286	STOW	
1	03 10 8 27 35 48 6. 75 iture	03 Crushed/Rock 8 27 35 48 6. 75 iture	0	0 3	0 3 Crushed/Roca 1 . 35 8 . 78 77 47 27 . 47 47 47 48 . 47 47 77 6. 47 49 49 75 50 5 50 thure 1	0 3	0 3 Crushed Roca 1 . 35 55 JS 8 7 7 7 7 47 46 46 27 9 7 97 47 47 47 47 35 9 7 97 47 47 47 47 48 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	0 3	0 3	0 3	03	0.3 Crushed/Roca 1	0.3 Crushed/Roca 1 . 35 55 35 63 40 50 36 64 61 60 63 63 64 64 65 63 63 64 65 65 65 65 65 65 65 65 65 65 65 65 65	03 Crushed Rocal 1 . 35 55 J5 63 40 55 36 56 41 43 10 10 10 10 10 10 10 10 10 10 10 10 10	03 Crushed Rock 1	03	02 Bit. Com. 4	02 Bit. Cone. 7 22 JJ 65 JJ 69 40 57 J2 35 47 57 67 51 89 46 03 Crushed Roca 1 . 33 55 J5 63 40 50 J6 56 41 43 57 42 70 43 10 41 41 45 44 46 45 47 40 46 45 43 43 45 43 45 45 45 45 27 47 47 46 46 46 47 47 47 47 47 47 47 47 47 47 47 47 47	02 Bit. Cone 7 22 JJ 65 J7 69 43 57 32 35 47 57 67 51 89 36 61 03 CrushedRock 1 . 35 55 J8 63 40 50 36 56 41 45 57 42 70 43 54 61 61 61 61 61 61 61 61 61 61 61 61 61	02 Bit. Cone. 7	02 Bit. Cone. 7	02 Bit. Cone. 4 .2 JJ 65 J2 67 40 57 J2 35 47 57 67 51 89 J6 61 50 64 49 03 Crushed Roct. 4 . J5 55 J5 63 40 50 J6 36 41 40 57 42 70 43 54 46 59 45 47 40 46 45 47 40 46 45 47 40 46 45 47 40 46 45 47 40 46 46 46 46 46 46 46 48 48 48 48 48 48 48 48 48 48 48 48 48	02 Bit. Cone. 1	02 Bil. Cone. 1	02 Bit. Cone. 4 22 JJ 65 JZ 69 40 57 32 35 47 57 67 51 89 16 61 50 64 49 71 JY JY JY OS CrushedRock 1 2 JJ 65 JZ 69 40 57 JZ 35 47 57 67 51 89 16 61 50 64 49 71 JY	02 Bil. Cone 7 22 JJ 65 J7 69 40 57 32 35 47 57 67 51 89 46 61 50 64 49 7/ J7	02 Bil. Cone. 4 2 JJ 65 JZ 68 40 59 JZ 35 47 57 67 51 89 J6 61 50 64 49 7/ J7 J7 J8 91 03 CrushedRock 1 35 55 JS 63 40 50 J6 45 45 45 45 45 46 45 45 46 45 47 47 47 47 47 47 47 47 47 47 47 47 47



D	TIM	E																											
30	30	31	31	2	2	T 3	1 1			1 -					J	ANUA	RY 19	47											
720	150	0850	1100	0920	1555	1 1 1 1 1	1 20	104	Cac	3040	7	8	9	9	10	10	-11	11	13	13	14	14	15	15	16	16	17	17	18
24	21	5	18	15	17	12	2,	1	1222	1 2	10/1	14.5	2115		30.15	1	21.	7.	2.7	1505	25.84	415	2940	.535	2922	. 630	3485	1576	12710
5	-1	7	19	10	20	4	1.1	10	-4	2	13		1		- "	18	-	- :	15	34_		30	30	15	13	22	22	35	22
1	32	131	3.0	24	29	32	30	33	35	1	6	,	1 1	7, -		1	.7	-	-	21.	-	La	34	23	30	33	20	34	20
7	37	37	37	35	35	35		35	15	3.5	35	7-	1	-	1	34	4.		-7		47	27	27	27	32	32	32	32	32
	44	+3	13_	42	46	12	42	4.	42	J.		1	1 13	1	-	1	33	Ja	1.	· ·	12	32	10	35	33	34	14	35	35
12 -	49	19	17	48	عند	40	18	47	47	1/2	f2	4.	21	1	1	7.	46	40	17		37	30	30	38	,()	30	30	37	37
4	24	53	53	53	53	52	52		52	5	5	1./	50	15	.0	50	30	46	77	11	44	40	44	44	77	44	44	44	**
P	5'H	20	1	58	58	50	57	57	35	0.3	F		1.	50	1	5,	Ca	50	1	47	41	49	12.	41	49	47	40	48	41
1/		4,	la,	81	6.3		9 ^	. 1	. 1	0.3	17	51			50	21	21	54	-	55	3:	33	55	52	53	88	34	04	51
3	1	-5	1.	2	.7		1	i		. 2	-1	~ 7	-		-	1		7	-1	37	3.0	3.8	31	311	57	50	50	50	50
	1	-1-	-	.0		23		12	~7	.3	37		1	13	-7	1	,	6	3	40	1		30	a	20	35	45	36	30
hate .	23.20	-cial,	midde.	16201	Jan de	2,32	200	7. Cu.	reliation.	me is de	-12ala.	Ta-meta		- 1.						-	,	23	31,	٠ ' ' '	33	J_{ϵ}	26	3.	2.
•	1144	SAL	16.	de.	1661	151	1166	31	.43		4 6 4	4'2'	11.	andi.	in Side	14 "	readly.	16 300	Sall.	4164C	64,	-duily	120	I dale .	ur lade	- 346,	Car	Guir !	St bide .
																1	17.6			1124	16	11016	4626	chen .	341	121	A'2 .	.5'2	34

-											FE	BRUA	RY IS	947															
130	1400	0830	5 /525	0830	1400	7	7	8	8	10	10	0720	11	12	12	13	13	14	14	15	15	17	17	18	18	19	19	20	20
6	45	43	43	20	40	22	30	33	32	22	3.4	10	17	10	4.72	100	1330	ORZO	1950	0920	1400	010	1425	0730	145	20.15	C13	0.825	1433
6	40	39	40	23	37	24	29	32	32	27	77	12/	76	10	20	10	70	20	39	16	16	21	35	30	56	25	42	15	46
13	33	33	33	34	34	34	34	71	34	33	72	33	32	13	28	14	38	21	34	35	39	30	13	.30	44	26	40	16	Jy
16	38	36	36	35	36	36	36	36	36	36	36	36	-	36	31	3/	30	3/	30	32	32	33	33	34	31	33	35	34	33
U	40	39	40	39	39	39	39	10	39	39	-	-	36	35	35	35	35	34	34	34	34	35	35	J6	36	35	36	Ja	3.
3	44	43	43	43	43	42	41	43		33	39	39	39	33	39	39	39	J8	10	38	18	39	39	39	39	30	40	39	39
7	47	47	47	46	46	46	46	73	43	43	13	43	45	43	43	43	42	42	42	42	48	42	42	42	42	42	41	42	42
2	52	52	52	51	51	CZ	5/	76	76	76	46	46	16	18	16	46	46	46	45	46	46	46	45	46	40	45	76	45	15
6	56	56	55	55	55	66	55	3/	3/	54	51_	5/	51	51	51	51	51	51	50	51	51	5.	51	50	10	50	5	5	573
7		47	36	15	27	20	32	55	55	34	34	54	54	54	54	54	54	54	54	54	50	54	53	54	54	5.8	30	51	CI
7	42	45	36	14	28	20	33	26	35	22	27	21	27	10	26_	12	29	21	36	34	41	25	33	27	37	2/3	25	4	22
	-	-		/6		4	32	35	32	21	29	20	30	10	29	11	32	24	35	35	45	27	34	30	3,6	21	21	2	24
409	crear!	Muin.					Snow.			Clear	Snow	Clear	Clear	Clear	Clear	Clear	Clear	Church	Clear	Clear	Claus	Cours	C/0.00	Claus	-	-	_		-
one	245	SWJ	2M5		AWJ.	NE3	£2	None	Vone	SW3	AH3		NWA	443	ART	XNA	AWE	141	NW2	None	SM	1.53	10.64	A S		Cicar	-Car	not 69/	CIEVE.
																						1924	416.7	1100 6	300	1147	AMI	11.115	AWS.

-								MAF	CH I	947																			
8	8	10	10	П	- 11	12	12	13	13	14	14	15	15	17	17	18	18	19	19	20	20	21	21	22	22	24	24	25	2.5
7.4	135	2895	400	224	275	2120	11:	25.20	220	275-	122	2210	.722	W	100	2802	11.22	37/1	500	2215	1. 400	3 220	17.70	78.20	400	0410	6.70	23	26
, -	11.	7,	43	1.32	2	-H_	24	31	71_	Li	45	36	1	34	61	34	.2	22	59	29	1.2	77	. 7	2.7	-	44	1.0	42	40
	30	27	122	11	34	-51	42	33	04	39	14_	22	51	32	55	1	51	20	159	70	59	33	5.	33	20	45	-	42	117
5		32	74	177	37	39	31	40	10	11	42	11-	1.	de	72	40	24	1:	1.79	11	34	1		1	do.	44	1	45	12
-	39		1	11	-	39	31	4.	412	11	11	11	1,	42	93	10	10	11	29	11	1	1	1.	+2		#3	47	45	47
2	49	#1	71	10	10	10	10	41	4	41	12	12	12	13	4	12	2.3	42	1/2	42	10	42	12	02	42	97	43	41	44
-	12		47	17	1)	41	45	43	17	13	17	13	43	71	al."	11	12	14	14	14	111	44	#	11	44	44	45	4	41
	15	7,	10	-26_	15	45	45	15	15	45	45	15	\$5	45	15	15	45	16	124	15	in	45	4	4.	0,	1.			1 4
-	52	-7	11	19	12	44	44	17	12	47	49	17	77	107	11	49	17	14	44	14	40	49	42	40	W	41	**	40	40
2	25	52	25	5%	25	52	51	52	5.	52	52	5,	52	3	51	51	5	52	51	51	5.	5	6	45.		6		6	4
	11	11_	31	22	JI	29	14	3,	50	37	4	37	17	04	3.	20	37	24	12	20	49	30	42	2	42	45	27	-	2.0
	*	33	10	20	42	32	50	38	27	37	05	30	53	35	45	33	46	27	15	30	57	3/	52	34	42	52	C	4.2	21
70-	11°W	Jan	a latera	- car	160	Wear	Gene	un find ".	Cher	Rea	3202	See-	1600	a dada	Carre	- Ear	Citar	1	~	Cales	-	~			4/	36		43	
42	16.	NI	1122	AL	1162	None	E	uns	144	52	52	14/2	wz	1			NZ	103	And	None	f 2		- Cast	or Cath	- a Cash			idead	- ands
											-								- CAPE	110.16	1 2	44.	48.4.1	1804	Lu L	96	Nena	- ma	Jeg

						AP	RIL I	947																					
5	5	5	5	7	7	8	8	9	9	10	10	- 11	11	12	12	12	12	12	12	14	14	15	1.15	15	15.	1 15	1	1.00	
-	A CASE	422	1333	0135	1112	22.5	43"	OFOO	SIC	0810	1400	250	1735	10115	2702	0930	43.5	300	1530	C'M'S	4.50	2501	2200	1633	10	10	10	15	12
-	7" -	34	35	10	46	35	18	1.	5:	44	83	13	1/2	57	52	55	11/	70	1 801	41	11	Se	AL.	3.5	9(3	6.7	10.7	72	1.24
3	72	31	36	45	2	-0	04	10	49	30	70	44	0.7	53	53	53	55	6	68	41	#U	41	4	-	1 (2	# d	1 40	-	1 20
6	73	44	44	45	-1	10	44	40	4.	100	13	11	47	5,	51	3.	5,	51	SI	47	47	1	ds	4.	41	4:	4	10	600
3	45	15	44	44	45	45	75	≠ e9	7.	45	44	40	10	41	48	19	49	48	48	44	47	40	47	1	40	4.	-		100
	74	10	11	45	44	45	45	1	45	45	45	15	46	4,	40	4.	4.	1.	4	47	47	47	42	70	42	41	42	45	20
5	25	45	45	45	45	40	45	1/10	70	40	10	7.	46	4,	16	10	10	10	4	42	47	41	41	42	47	42	412	42	#2
	te	16	40	40	10	10	10	48	4	47	47	47	47	47	42	1 2	47	47	47	47	47	100	#2	42	1	10	1	-	07
1	41	44	44	14	49	44	44	14	17	#4	49	44	47	41	49	14	49	49	47	41	41	40	49	47	17/		-	47	11
	50	50	57.0	50	50	<i>3</i> ∏2	50	"CA	53	50	50	20	3 (2	31	50	59	n	50	50	0.0	63	(3)	Ca	1	- 77	100	17	12	47
-	13	45	45	45	41	-72	22	33	1 e	37	50	44	n#	\$7		rr	54	2.0	-7	35	EP.	22	LA.	12	61	CC	1	2.5	-
1	48	50	45	47	54	Se	54	37	11	44	20	41	64	35	55	34	63	2/	74	72	Se.	22	-	-	1	23	13-	22	#2
cer .	- add	Care	وقنعشت	المقاد	- sads	2015	and Code	1 Parks	Rain	المائية المائية	market .	G tada	* 3.ds	Rain	24	2.			-	-	0	-	-	13	-	-	#3	29	71
	- E2	162	23	4 1	100	443		in	S#2	442		50.	- 100		for 2	1444	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	and and also	and a	- 8434	Lades 6	Alees	ted Sight	to ouds	-	n i 4	made.	a lab	184
						-		1								1	2	1044	1	1 466	162	ING	13.04	chan'.	1.98	-100	1000	114 a	41

Air temperature obtained from thermograph record. Surface temperature obtained from mercury thermometer laid on the surface of the povement

FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

TEMPERATURE RECORD OF TEST ELEMENT E



Thermo-	Depth In	Type of		OCT	OBER	1946				NO	VEMB	ER 19	146						-			DECE	MER	1946			
couple No.	feet	material	11	16	20	22	31	7	15	19	21	23	25	27	29	3	7	10	12	14	16	2.4	26	27	27	28	30
	03	Caneat Can	6/	47	34	244	35	41	4.5	31	377	200	40,30	CALL	110	exic.	2700	2014	CREC	2003	1005	2839	5933	011:	1015	CZEL	22.5
2	05	Grassi Rad		03	50	100	37	10	11	33	30	37	dir	10	40	17	30	37	1.11	31	14	11	13	6	14	1 "	122
3	13		30	34	52	33	37	31	14.2	17	39	30	ds	30	01	38	15	01	41	100	137	136	25	31	49	27	20
4	2.2		36	37	31	37	69	51	0.0	30	30	193	30	34	34	15	45	10	31	37	00	0.	01	00	20	31	.96
5	20		34	1 34	60	61	66	64	0.3	04	00	60	38	38	39	33	31	32	32	34	31	107	07	00	100	15	13
6	30		36	37	1	60	67	68	66		0.5	63	40	62	66	31	30	36	35	33	23	31	31	30	30	50	101
7	5.2		6 37	4 61	64	63	68	70	67	6.8	0.5	08	67	6.5	66	00	6.3	61	01	61	60	30	18	38	30	37	36
8	65		54	61	64	65	06	61	47	68	0.7	45	67	67	0/	0.5	66	63	40	64	6.0	101	60	61	60	60	60
9	7.0		161	61	38	163	6.5	65	48	67	0.7	67	6.9	0.6	67	45	0.6	4.5	4.5	4.5	165	00	4.0	63	0.4	0.0	4.8
ur Tempera			00	60	33	1 45	100	47	11	+16	135	135	33	0.5	,9,7	15	100	37	.13	27	10	1.1.5	4 4 4	- 0	10	1.3	13
urface Ten	peratura		65	66	37	38	61		101	10	.00	132	100	12	33	15	36	10	.09	28	115	30	20	2	100	10	100
reather rind (Beaut	tort Scole)		Cleur	د داند د مو چ	coner	0/200	shine a		100	Mar	214	.40r	52	der	Aug	17500 10 2	180	(Aug	190	wer	Med 3	112.4	d 700	1666	steer.	1900	149

Thermo-	Depth in	Type of									-	ANJAR	Y 19	47						_		_	_	_	-	_	_	-
couple No.	feet	material	10	50	20	21	21	22	22	23	23	25	25	27	27	20	28	59	30	30	31	31	1	1	3	3	4	T
1	0.5	Conerd Care	11	12	29	34	30	15	32	1	-		21	-	74	2	14	21	10	21	31	37	0.00	7 8 /E	100	2.5	100	۳
2	05	CHARLEN	26	14	21	33	.74	4	41		3		2	1	14	2	33	277	40	22	M	130	30	30	100	24	153	ŧ
3	1	1. 10.	30	10.	A.	34		31	Att	14	14	24	24	20	Nr.	3.	A	.32	31	.31	30	AL	14	M	Je	4.9	45	т
4	22		.79	22	11	.33	21	34	34	30	10	3:	3	32	34	25	72	74	34	34	14	33	35	35	.85	135	34	1
2	30		.78	18.	.74	79	70	.78	38	34	19	100	37	34	37	177	.52	36	36	15	11	N	11	18	35	35	35	Ť
6	24		47	42	41	42	11	42	41	112	15.	N.	41			*	4	41	41	41	#1	4	41	44	44	di	42	1
7	22		.00	50	19	100	10	17	11	50	22:	- 17	40	75	407	47.	48	af.	4.	47	de	45	41	41	40	41	16	1
	0.2		£F.	EF.	11	54	FW.	EX.	14	19	14	69	50	10.	15	4-1	28	55	12	28	53	55	13	35	100	15	33	1
9	28		58	526	59	79	19	SE.	.5%	10	12	77	17	17	47	-25	11	175	54	16	54	14	10	11	54	34	14	л
ir Temper	@fure		19	18	#	40	32		200	- 10	18.	345.	.77	34	16	20	. 4	a.	.19.	16	M	,89	46	27	15	24	1.7	Ŧ
veginer	mperature	-	A.	1	.18	39	16.	7	1	1	100		25	áT.	Je.	Ab.	E.	1.2	24	4	M	M.	44	10	15	29	47	F
	fort Scale	1		- Pade	L	Sec.	41	300	-day 442	Gran.	47	None None	-	None.	AZ	Noss.	Men's	inter-	Charles Canada	Charles .	Aict.	E In-reg	that.	Signal William	1912	Gar.	Alexander .	ł

hermo-	Depth in	Type of						I FIRE	LARY	1947															-		
auple No.		material	21	21	22	5.5	24	24	25	25	26	27	27	50	20	1		3	4	4	5	5	6	6	7	7	
			Party.	181	Differ.	931	110	42	2112	281	1821	3435	1526	200	1.7.25	2840	42	13/0	Jane .	120	34.5	2.5	300	12	3400	4.5	132.0
		Connect Con.		106	53	20	16	JU	12	dif	16	24	37	Table 1	20	23		27		10	2.7	20		10.7	.54	43	20
2	- 11	Countre last	-2	1.1	22	25	24	20	-22	24	32	21	22	24	1	22	119	30	.1	3	21	24		20	17		2.5
3	- (J		21	27	20	20	20	30	20	24	30	24	30	JV		1.0		.52	12		100	1	1	7.0	-	2	1 20
4	4 .		21	39	21	22	22	23	24	33	39	22	20	33	3.1	10	19	20	10	-	35		1	2	0	22	22
5	14		20	20	2.9	32	32	32	.52	22	37	37	39	17	32	20	10	1	3				1	-01	2	10	1 4
6	71		4	A	42	40	40	#0	41	#12	40	40	4.	40	30	46.7				-Aa-		41	1	100		40	1 .00
7	34		0.	Pa .	30	-	40	46	10	de	10	40	70	di.	41	05		er.	ar I		-	1	1	1		41	100
8	0.5		37	4.	0.0	310	JEGS	13	22	60	42	41	A.P	00	41	411	-0.0	40	44		42	4	1	0.		44	91
9	71		10	10	1	12	99	11	11	11	63	12	119	10		63	11	7.7	4.7		-	1	1 "	01	-		-
Temper	ature		9		20	24	3.3	20		4.						4.0	-		143	-			-	1	3.5	28	18
fece Ye	mperature		4	1	6.7	Jo	P.O.		All son	40	28	- Charles	AD.	.64	A.F.	AT.	all.	4	44	-	-			-	- Mary	44.	1/2
other			nh.	400			4.0	37	-	30	34	31	42	4.0		26	22	111	-			177	1	29	Len	97	15
	fort Scale	1	no bridge		mark a	-inda	m 44.	- da	audet .	-da		dade.	-	- da	-	with .		- Ande	no made	Same	mark.			-date	ninds.	Car	Same
. 1940	31001 36 ata		1366		1647	201	APRE	3 = 6	Mona	46	ined	Non to	10.3	Non t.	48	Alen L	01	33	Alvedi	and.	-			1	440		100

	_	_	_			_	_	-	_	_									947	CH I	w				Type of	Depth in	Thermo-
13	•	•	•	*		*	•			3	3	2	2	in	1	31	07.00	20	0730	20	20	27	27	26	material	feet	couple No.
7 74	77	50	77	14	76	37	4	7	4.9	54	14	50	4	35	34	87	27	41	45	47	21	41	20	Al.	Consul Care	0.1	1
1 70	10	61	11	44	20	34	35	41	34	43	33	45	41	45	X.	41	31	33	49	14	11		44		Countricities	-25	2
-4	40	40	19	34	39	w'si	00	6. 1	41	40	41	40	41	31	70	33	37	38	M		35	15	37	12		14	
42	41	41	41	4	16	16	14	c L	44	40	14	41		38		M	39	38	39	M	39	40	40			22	•
40	40	41	16	64	41	44	44		44	40	#1	41	40	40	40.	40	10	40	40		41	40	44	AL.		24	
42	41	45	41	10	42	41	12		43	44	44	40	#4	44	11.	12	44	46	42	40	**	47	41	.62		74	
	14	44	14		45	16	41			40	45	15	4	**	#	05	44	**	44	44	44	44	44	. 100	-	52	-
2 21	42	42	47	47	47	47	07	r. 1	11	42	12	47	47	47	42	47	67	47	47	47	41		4	35.		4.5	-
0 60	50	16	20	10	Je.	16	50	6	110	80	10	10	40	49	78	50	39	30	50	30	30	30	AL.	27.		7.8	Temper
74	14	11	50	AL.	11	16.	11	4.1	11.1	4	16	-56	41	14	. 30	200	M		44	N.	20	30	-9	D.			Control Control Control
E 78	44	41	10	20	20	40	14	F .	21	50	31	27	44	42		47	37	-	20		44	30	40	30		mperature	
er Care G	See	140	. 60	260	de		Sec	-	-	Sinc.	A.D.	Chap	Simp	(fac.	Carlo	6.80	E kee	Citar	Cher	1.50	Char	inter.	far.	-	•1	wfort Scale	- A
4		i der	40	Star.	Ast.	11	Ser.	10.0	a to	State Add	in.	Sung.	Seed M.C.	Sec.	ANI	Liv.	i kur	Char for a	Chec Grea	100	Char may d	idar.	Mar.		•)	wfort Scale	Weather Wind Bea

hermo-	Clepto in	Type of													MAIL	11 (4)											
ouple No.	1001	material	16	16	17	17	10	10	19	19	21	21	21	51	21	55	55	23	5.3	24	54	25	2.5	26	26	24	20
	4.2	const Car	11		22	51	,34	17	31	20	10	24	11	63	11	200	0.2	1	1 11	AU.	12	1 41	15	CALL.	2001	436	222
2	19.0	LIVER ADLA	30	16	15	05	.26	12	10	12	N	37	27	00	02	10	21		do	11	22		45	40	01	3.0	314
3	13		00	00	13	00	11	12	11	00	13	11	13	111	00	00	11	100	34	11	10	15	15	11	40	3.	41
		-	*	10	01	13	13	15	05	15	115	11	15	00	11	00	00	. 00	100	15	10	100	. 40	11	41	27	4 5
3	22		63	102	15	. 45	15	15	15	15		15	15	10	01	15	100	105	10	105	106	102	. 34	16	0.	44	41
	**		16	100	10	40	16	do	10	10	17	16	10	10	10	10	15	10	100	10	10	15	10	4.	181	34	
	12		1.	01	01	101	11	13	12	10	111	11	10	10	0.	01	00	10	100	4.0	101	11	37	22	100	42	44
	4.7		31	9.	100	11	11	15	108	101	11	11	01	11	31	111	11:	11	111	101	111	10	48	11	41	4	40
Temper	10		1	14	100	10	10	12	11:	10	18	12	40	10	31	119	10	100	10	12	42	12	11	10	40	40	4.
			20	111	121	10	1.76	22	1 37	115	126	1.00	21	200	30	11	20	100	de	10!	. 10	. 30	0.4		11		
tece Te	mperature		41	102	47	111	100	20	11	23	136	12	40	234	11	11	4.	1:	10	19	11	111	21	14	11	The state of	20
	utert Scot		60	p-Hode	Cher	10.00	to E40	and well	ling	Sam Daniel	Salari P	Libra	al Bretta	te Berge	cheep		200	-	-	make and	ine of	du des	is the	C Street	- the	Patie	-
	3.0			DH &	10 8	Section 16	1 4	249	98 7 K	41.40	MES	15.2	10 11 11	AT 2	100 3	26 1	100	à.	10 m	200	1	JE 2	. 42	341	002	m 2	2.0

30	30	5 1	31	31	2	2	1 3		1	1 4	1 .					-	ANUA	14 19	47											_
	1150		145	1400	0910	114	30.00	3	cen	1112	1 tage	1		,	9	10	10	11	11	13	13	14	14	15	15	16	16	17	17	18
31	112	2	1	10	0.0	-	21			2/2	7	de	-	1	100	19.55	-	SHE	-	2342	180	108.00	1932	5242	.230	Albert	ILEE.	GEN	12.30	421
2.2	129		11	15	12	15	21	1	1 2	1	1	20	1 4	1	+4	and -	1	Low	12	12.	fred	12	18	48	1.3	30	32	89	30	2.5
21	27			21	25	23	10	1.27	22	22	22	2.1	20	1	6 -	1-	1	1 .	1	10		111		14	10	30	Br	30	30	22
36	31	•	36	10	33	24	34	2.9	10	34	14	24	37	20	- 60	44	40	4	1	127	12	22	53	25	14	31	An	34	3.	Ju
13	12	- 1	12	02	22		4	10	42	10	1	40		-0	100	30	27	28	14	20	34	Bo	30	30	21	22	22	33	2.2	34
11			07	11	41	00	4	1 .	or	or		00	61	91	9.0	411	40	39	32	37	27	27	27	S	22	27	17	37	37	20
36		- 1	16	36	53	n	57	1/	50		172	1	- 09	-	. 24	44	00	09	00	101	03	42	42	45	42	di	42	41	102	02
	100	1	40		39	50	19	1 .	100	11	1	1	63	1 27	12	13	10	23	53	25	20	12	5	51	51	fo	21	10	10	Fu
6.8	62	1	64	4.2	62	1 .			1			1	1	10	112	12	177	12	52	24	76	76	55	17	17	FF	10	17	11	Er
18	10		.5	22	7		41	-		-	-	-	-	-	40	4.2	100	7 '	100	59	27	177	la	19	37	14	50	10	10	IP
10	111	1	8	12	44	-	1	1	1 .	1	1 0	417		-	1	-		-6	7	-	80	4	11	34	N	100	31	PI	92	80
6/20		1					1	1.		47	1	10	9-	-		1	1	-1	7		12	1	20	72	23	28	30	27	50	34
NJ			-	"Hade	00.	ola .	a self-m	and all	hold .	-60	***	who,	on Addi	- 0.	4.34		641	no Serda	- wein	64	Cer	Killer	Care.	San	Rea	Can	Const	100	Can	1
	277 111	10		Marie I	-	- oud	1141	46 4		43	100		A + 2			1046	401	ad .	14	.be.	1000	144	160	Non-G	AZ.	34.	144	-	a I	100

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22	23	39	31	2/	,10	17	40	.27		10	21	11	.57	11	11	18	27	17	11	47	20	20	115	27	10	23	34	10	sacc.
27	29	21	14	.24	12	31	31	31	31	32	27	30	30	16	34	13	32-	19	42	21	30	30	31	30	10	17	12	16	23
138	30	10	34	15	.09	13	32	25	at t	de.	.03	11	13	32	20	30	32	32	26	.52	33	34	34	15	15	34	36	21	30
12	12	111	0:	31	21	38	11	26	21	21	34	31	36	11	3/	11	27	.17	22	27	187	1	11	27	21	12	26	N	11
13	dy	dy	11	11	17	11	11	18	11	11	11	1	41	37	11	11	47	47	10	47	01	41	30	10	46	10	47	10	12
3.	36	57	(a) وال	34	33	23	33	24	34	21	3	25	50	30	21	21	21	30	30	20	30	30	10	20	200	50	50	JV	30
1.7	12	4.	15	13"	11	12 42	31	34	12		27	11	17	10	26	14	27	41	36	34	19	20	33	33	53	30	30	32	33
ikur	10	do	. ***	14	25	17	P.2	30	.12	24	26	14	11	11	47	11	30	100	13	32	4.1	11	32	30	10	10	20		43
Nese		100	THE ST	100	400	- Tum	12	None	Pare.	43	gin J	Mar		4 # 3	000	100	1114	100	002	Syes	100	124	17 3	-	antin	100	Chier	6000	LANCE
																							لناد	12.4		تنت			

		-						MAR	ICH I	247																			
2225	8	10	10	3328	11	12	12	13	13	14 0.0s	14	15	15	17 Otes	17	10	10	19	19	50	10	21	51	5.5	2.2	24		25	26
22	31	33	40 37	29	95 39	27	17	17	53	22	41	72	01	.9_	41	12	11	16	fo.	22	112	77	25	32	40	7/12	f2	02	29
76	15	JU.	136	25	25	36	30	12	30	39	4/	17	20	20	di	, Jo	34	10	80		4	31		32	00	77	49	12	16
12	37	37	30	22	22	11	22	39	30	39	17	17	29	11	32	77	24	24	39	7	30	39	21	44	31	90	9	1	42
20	30	27	10	37	37	32	30	41	90	40	4	di		9	41	**	40	do	de	*	de.	ø.	*	#1	•	40	4	4	11
ør .	ar.	05	05	11	45	25	11	10	0.7	47	di	41	01	#	45	41	32	47		15	41	46	40	47	4/	41	41	41	41
52	49	91	52	-	01	40	4	17	00	0.0	**	40	49	12	47	4	1	11_	W	11	*	47	40	#2	07	02	00	#7	14
12	22	13	-35	17	38	12	89	14	12	27	00	22	Ø1	49	76	29	74	As-	11	0	#1	33	92		50	f >	6	30	4
Same	-		31	20	40	10	-2	33	7	12	01	17	54	34	19		48	4.2	47	31	12	36	27	W	40	30	31	do	32
Snow	452	NI	- Address -	STATE OF	352	Maga	1	- 54d	Sal.	/lan	Tan-	miliat.	0.0	no desta	-items,	-	امدت	101	San.	alles e.	Side of the State	Color	Com	Char	Own	-	-	last.	Sart.

						AP	HL I	947																				-	
5	5	5	5	7	7			9	9	10	Ю	11	8.0	18	12	15	12	12	12	14	14	15	15	15	15	15	15	15	15
43.597	10307	444	ides	COME.	1381	225	4.825	Adas.	221	DAMO,	A22.	244	250	25.7	a had	JOINE.	m46	300	Sec.	3644	492	atie.	of Boy	eter	MARK	1205	1505	1655	1855
34	134	1 77	-	40	38	31	7	27	00	.16	50	01		133	10	50	84		10	24	47	10	40	15	31		66	66	21
37	1 77		40	47	41	71	40	17	43	35	44	00	1.2	16	13	52	13	16	60	40	9.0	40	40	41	13	31	33	U	127
42	10	4:	91	45	di	42	45	41	42	4	40		40	01	44	40	47	#	49	di	44	45	12	41	11	13	43	11	105
02	1 42	42	4/	42	41	07	41	40	41	#1	41	90	40	dl.	47	45	45	#6	-	00	-	do	15	45'	45	05	13	41	45
1 45	1 45	11	42	41	41	07	47	+3	41	47	41	40	44	90	49	00	22	**	40	4	47	do	45	45	41	45	45	12	45
47	41	41	41	43	41	00	41		40	00	-	00	00	90	40	60	00	00	00	di	di	46	44	da	J.	44	45	14	44
10	1 46	-	46	44	4	44	46	40	40	00	46	40	da.	-	10	40		de	da	42	42	41	47	07	42	47	41	47	177
47	47	47	07	47	47	47	97	47	41	4)	41	47	4)	dif	#7	42	47	47	100	47	40	41	40	40	40	41	11	40	1
1 80	In	10	/b	D	07	50	49	50	10	97	49	170	£700	49	64	02	-		40	00		40	44	40	10		49	70	1
3,	11	di	ar	45	PI	27	0.1	21	20	90	1.	00		15	20	2.7	19	94	4.7	11	3 2	21	00	22	2.2	43	1	12	157
	10	10	45	46	50	,Eg:	11	41		41	40	01	42	10	64	17	6.2	4.0	No	22	10	.44	100	19	40	44	40	4.7	155
Cen	Car	-	Card	Care.	C	- mail	Same	-	2					18		2	C. Barrier	Cons	· mar	Carro			The same	Con.	100	7	CARRE	2	-
31	342	201	11	100	2.4	401	S a A	Dr. O	1	had	241	1000	Sant	100	David	Sauce		Total Control	-	Der Mile	-	The party	1000		-	6.40	700	441	and .
1	1000		-	100	100	-		-	4.00	130.0	346	2000	-	-	100	000	416.1	200		25.0	164	T. s. el	100	-	IN 4		-	484	76

NOTE

Air temperature obtained from thermograph record
Surface temperature obtained from mercury
thermometer loid on the surface of the passment

FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

TEMPERATURE RECORD

OF

TEST ELEMENT F

hermo-	Death Is	7.00.04		OCT		1946						-												U	AIL	: Al	ND	i
	Depth In	Type of	4.1	991	AL.	344				MC	VEMB	EK IS	146									DECE	MAER	1946				
ouple No.	foot	material	1430	1115	1245	22	1415	0930	15	19	21	23	25	27	29	3	7	10	12	14	16	24	26	27	27	28	30	1 7
	. 04	Bit. Conc.	14	21	61	52	64	47	46	41	41	11	46	4.3	13	10	32	1130	9729	1110	0935	0155	0955	0845	14.55	0450	0420	16
3	0.5	Sana three	11	31	57	39	62	45	44	37	62	11	44	44	44	21	32	10	32	31	23	32	29	13	12	10	29	1
3	- 00	Sulty Clay	61	36	34	36	60	53	23	50	49	19	15	11	18	41	10	12	142	43	10	31	38	37	37	36	29	1
3			4.7	36	60	3/	13	36	36	54	52	52	18	50	51	15	44	11	15	46	44	11	41	10	40	40	38	1
6	J2		51	12	40	47	64	64	54	37	3/	55	55	144	14	52	50	50	50	50	50	47	47	47	41	47	46	1
7	44	0	31	59	6/	62	44	65	4.5	64	4.3	40	36	57	38	36	55	54	52	53	53	32	32	52	51_	51	50	I
0	57		38	.57	61	63	66	45	65	64	63	62	60	40	61	27	10	57	50	56	57	35	55	55	55	55	54	1
,	69		60	60	62	.2	63	64	6.5	65	64	61	61	60	62	41	57	08	60	60	60	11	5/	5/	57	51	56	+
Tempere			70	47	30	50	60	48	41	40	31	35	.59	40	7.	17	21	4	-	2.0	-	27	27	27	37	57	37	1
	mpereture		25	7/	60	50	62	16	51	45	37	35	30	31	35	1.0	35	47	31	30	12	33	22	-6	10	-4	23	+
other nd (Beaut	lort Scale)		Glese.	CHEST.	Chal	Cieac.	Chore	Maria.	Char	Char		ctor	Grown	Stear	Lieude	Char			Glear				24.	Clear	6/60	inen	Ligar	1
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3	23		41	40	40	40	40	10	40	40	40	40	41	43	40	40	40	70	44	40	40	40	40	33	77	35	35	35
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2	24	43	22	27_	46	-6/	35	12	40	50	40	71	43	27	92	53	54	58	79	77	41	45	39	41	50	63	76	77	70	57
7	15	12	J.P	32	13	3/	36	64_	37	41	31	62	44	69	52	52	53	57	68	70	11	45"	40	41	46	54	67	70	68	60
	44	11	11	14	44	14	45	44	45	43	14	43	16	46	48	48	18	48	48	41	17	47	46	46	46	45	15	46	46	46
	45	11	44	Mark St. St.	11	44	15	44	45	15	44	44	45	45	46	46	16	47	17	47	47	47	46	16	46	46	46	41	44	46
		45	45	45	45	45	45	45	46	16	44	46	46	46	\$6	46	46	46	16	14	17	47	41	47	17	4.7	4.7	47	47	42
		46	46	46	46	46	46	46	46	46	47	41	47	47	47	47	17	47	17	17	17	45	44	10	40	4	18	40	10	7/
1	47	17	91	17	47	47	47	47	47	47	4,	71	47	47	47	47	47	47	47	17	17	17	74	70	71	7/		10	90	48
,	48	11	45	48	48	48	48	48	11	45	44	48	48	19	40	48	40	40	10	10	10	49	76	91	40	71	10	40	91	47
,	49	14	41	49	49	44	19	14	19	19	119	19	19	50		10		10	-0	40	10	7	77	47	17	47	47	97	47	47
5	31	41	15	45	45	19	39	15	22	.36	24	51	21	10	7	50	47	9/	4/	97	97	47	47	47	49	17	47	49	47	19
0	40	50	C	10	16	51	35	54	43	13	4 5	70	45	9.6	55	54	33	37	66	IZ.	33	37	37	31	47	32	55	55	54	47
	4		2/1	21	Marrie Hallmanning	1	-				73	67	45	67	55	35	.7	64	72	77	38	40	38	12	50	63	65	63	60	19
	51	SIESE.	and	1024.	Claude.	Star !	Com	10/201	Low	Kain	Glegr	GRAZ	Claudy	Lawy			1010				Cloud	Kain	Cloudy	Clear	Clear	Chear	Char	Clear	Char	Cher
90	71	262	2 4	22	113	10	WA	NNA	SEX	NW5	NW2	JW2	SHZ	SHE	JN2	SW2	JW2	SWI	SNI		SE 2	SF2	None	MWI	WWZ	N'W4	NW3	NW3	NW2	W2

6	28	28
11	0115	1.5
?	32	67
5	.7 ₆ 18	47
,	18	17
2	19	48
1	+9	H
9	49	49
p	44	44
19	17	44
1	50	3 3
,	27	41
,	25	5.
2.	Cen	- Jak

Air temperature obtained from thermograph record. Surface temperature obtained from mercury thermometer laid on the surface of the pavement.

FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

TEMPERATURE RECORD OF TEST ELEMENT G

SOILS LABORATORY

Thermo-	Depth In	Type of		OCTO	BER	1946				NO	VEMB	ER IS	46									DECE	MBER	1946				-
couple No.	feet	material	- 11	16	20	22	31	7	15	19	21	23	25	27	29	3	7	10	12	14	16	24	26	27	27	28	30	
			1515	1112	1235	1100	1050	1105	2940	1335	0942	1030	1000	1015	1350	1055	1055	0950	1055	3975	10.02	2822	1,005	0840	1505	100C	2430	14
_	0.2	Bit Conc.	78	64	60	61	58	51	45	44	42	13	41	19	14	32	75	38	35	32	31	12	30	1.8	28	25	31	
2	03	Sand + Gravel	70	52	51	52	57	40	15	19	44	44	12	46	++	33	34	34	35	15	71	77	72	25	30	22	3,	
3	08	SiltyClay	61	54	51	52	58	50	51	47	18	49	11	11	46	39	J.C	70	40	#1	30	1 3	34	35	34	33	22	
4	15	<i>h</i>	51	56	55	56	61	56	56	55	54	59	51	53	52	47	73	75	10	17	45	92	12	4	41	4-	4.2	1
5	23	41	57	57	SA	59	63	60	60	59	58	57	55	50	55	52	50	14	49	50	11	4	10	76	1/4	45	111	1
6	32	4	56	57	59	60	63	63	42	41	61	100	59	50	57	54	55	52	52	52	52	de	18	119	11	44	42	
7	44	4	57	57	60	61	64	66	4.0	25	105	64	68	63	63	62	41		50	100	0	11	55	55	55	.55	100	3
8	57	b	58	59	60	61	64	65	67	0		65	65	65	60	63	• 1	62		1 ,	1	-	54	50	50	50	52	1
9	69		58	59	50	62	43	64		6.	64	1.5	155	65	35			1.1	43	1 . 7	1 . 7	* 4	-1	50	4.2	-	1.0	
ir Temper	ature		68	65	54	63	63	50	45	44	37	36	50	49	39	23	41	.75"	-75	25	-	20	23	-6	1	-3	17	1
urface Ter	mperature		78	63	58	67	62	56	4	47	37	36	48	44	Jo	24	39	42	40	29	11	27	24	2	1.3	2	2.5	1
/eather			Clear	Cen	Cear	Cear		- 0/20		Cicar	C 200.		Cara	-	1		,	1 -	1000	10	1.4	-	1	-	-	9	7	1
Vind (Beaut	fort Scale			Swi		-	4542			12	6	AWI	52	142		1				melade		-		L. 6.31		1000	THE BE	1
				- MILL	******	1	TUNE.	INE	18.5	414	on E	1114 7	- L	-146		-	-14	2550	SWZ	seel a	1842	1 30	w 15's	10003	d w	163	1000	10

Thermo-	Depth in	Type of									J	ANUAR	Y 194	17														
couple No.		material	18	20	20	21	21	22	22	23	23	25	25	27	27	28	28	29	30	30	31	31	1	1	3	3	4	T
			1405	1550	1640	1840	14:0	0440	1545	0845	1505	0710	1420	0855	1445	0.845	1420	0880	1335	1440	0850	1420	1755	1420	0520	1355	0840	110
1	0.2	Bit Care	32	2/	31	33	.12	11	20	12	24	26	31	32	32	32	32	28	27	30	31	.1,1	29	33	14	22	27	
2	03	Sand & Gravel	30	22	27_	31	31	24	25	16	24	21	27	31	3/	31	31	27	27	36	31	31	31	31	22	10	27	1.
3	0.8	Suit, Clay	31	20	27	31	31	31	30	27	27	27	27	31	31	31	3	12	3/	31	31	31	31	31	36	20	31	1.2
4	15		3.	15	35	35	35	35	35	35	35	34	34	34	32	35	33	35	20	33	33	33	35	35	3.5	35	35	1.2
5	23		31	38	38	38	38	38	38	38	38	38	38	38	4.5	38	38	38	38	35	38	.23	38	38	33	33	38	1.4
6	32		4/	41	41	41	41	41	41	41	41	40	40	40	1 .	10	4:	40	30	10	40	10	30	10	40	40	40	1
7	44	A	11	27	47	135	18	48	48	45	48	.21	45	47	17	47	27	1)	47	37	47	4	37	37	4,	47	47	1
8	57	er .	32	32	52	52	32	51	52	51	51	51	51	51	31	1.0	20	30	50	1.0	20,0	50	30	.50	10	-	13	1 5
9	69	p.	54	.53	55	33	.13	-13	55	.55	22	34	:4	14	1 . ,	34	. 1	34	5.3	3.3	39	33	.53		. 3			1
ir Temper			2 %	12	40	40	13		3	-2	12	31	37	34		31	14	26	20	28	./2	36	34	7	15	32	4.2	1
Surface Te	mperature		30	13	.3	25	32	11	1	2	16	:6	27	34	150	31	31	2.5	12	2.8	.9 2	,	24	11	1:	30	47	1 4
Veather			chods.	West	Visier	1/11	Chorte	11.11	1:41	il or	2/1	01 1	Kisa.	1117	100	1	1/1/	% 1.	"wel	1	1.00	Cons,		3/1.	Street	T	10,00	10,
Vind (Beau	fort Scale)						N'N'Z		W2	W3	None	351	None	Nel		No 16			NEI			24.7	4'11'		NE 2	1	1

Thermo-	Depth in	Type of						FEBR	UARY	1947																· · · · · · · · · · · · · · · · · · ·		
couple No.		material	21	21	22	22	24	24	25	25	26	27	27	28	28	1	1	3	4	4	5	5	6	6	7	7	8	T
			0835	1420	0810	.37:	0850	1923	2872	.35	110	2835	. 7. "	0825	158	2742	100	120	oras	15%	14		4.	1923	30.75	1470	02.22	
	0.2	Bit Conc.	2.1	2.	24_	-7_	27	.7.1	24	Je	10	7:	37	7,	22	30	10_	it	1	Js	1/4	2		40	12	R	72	I
2	03	Sand + Gree 2/	24	27	27	27	29	1	4-	29	32.	27	24_	ir	2	21	J.	2.	3.	3.	22	100	12	12.	132	L	12	Ī
3	25	Silt, Clay	3,	J.	3,	12	Je	J.	32.	1.	20	Ja	22	52	1.	36	72	3.	3/	21	72	1	1	17,	122	1.4	32	I
4	15		30	Ja	.74"	ر2ي	15	35	15	35	35	35	Ju	35	35	1	72	1.5	35	1/2	4/=			3-		1.1.	35	I
5	23		34	38	37	38	32	37	37	12	32.	27	12	37	32	31	.17	.12	37	27	.'2	17	12	27	+1	.*2	37	I
6	35		Jį	# .5	34	32	37	17	39	34	37	27	7"	27	11	37	12	2 4	.24	27	11	3,	1.	121	12:	3.	37	I
7	44		45	9.5	1-	75	15	25	+3	45	#5	47	11	1.	12	111	10	11	4+	40	tt	t:	77	11	sż.	21	74	1
8	57	de .	43	45	46	14		75	42	11	12	42	40	47	77	47	12	1	47	47	+2	4.	1.	11	7.	4.	4	1
9	.69	4	570	51	3.72	82	5 .	53	50	50	F**	51	57	1.1	111	1	133	13	53	5.	37	21		111	1.	71	47	T
r Temper			1	13	20	28	22	29	12	J:	38	24		22	39	7,7	7.0	7.7		.75			1	1.	7.	4.	32	1
irface Te	mperature		9	r d	29	20	24	-4	24	32	34	1.	3,	22	11	35	42	.12	37	44	33	7	3.	45	37	25	34	1
eather			Giddle	Same	Jula .	Sugar		Cen	Sur		1	-	n. bat		Sau			1	militarily	See		-		1	1		Snow	.1
nd (Beau	fort Scale	}	1523	ALT.	1.1		None				1002			None		1/274	1	- participe	4516	41			The state of	To de al	and it	201	1	- 1

Thermo-	Depth in	Type of				MAR	ICH IS	947																				
couple No.	feet	material	26	27	27	28	28	29	29	31	31			2	2	3	3	4	4	4	4	4	4	4	4	5	5	Г
			. 44.	Gris	10	02.5	11.3	5235	14.5	0240	1425	ORIS	13.22	1125	1455	28.2	1343	0520	2715	34.3	2,23	1013	512		19:5	25.2	27.3	14
1	1:	Bit. Cone	++	20	34	31	50	32	45	22	57	72	0/	12	57	3.	54	22	22	++	51		32	52	47	21	25	Г
2	2.3	SameGrand	11	33	22	.22	37	33	31.	.30	45	24	40	#II	44	3.	47	28	26	70	12	41	17	52	57	25	32	L,
3	73	Suty Clay	21	27.	2.	25	25	36	A	37	37	10	70	4,	4,	40	966	10	pla .	39	32	4.	1	47	47	12	11.	
4	15	h	70	43	40	74	27	34	39	34	39	40	37	#1	41	72	41	42	42	92	12	4,	71	4,	4,	43	42	1
_5	23		+.,	71	11	1 1/2	+0	10	42	43	40	43	40	41	4,	*,	4,	42	42	42	42	4	74	42	12	42	42	1
6	32		1-	42	42	12	4,	4/	1,	41	d,	44	7	41	4,	20	12	12	42	42	12	4 2	7.	42	42	40	12	1
7	44		++	45	45	45	45	45	45	15	44	44	40	45	+5	45	45	45	gl)	45	45	47	15	45	45	15	45	1.
8	57	Rg .	.74	10	10	70	1.	47	47	177	4.	10	1.	16	1.	4.	46	14	di	1.	1.	44	1.	+4	40	4.	4.	1
9,	69	-	41	41	4)	18	4.0	40	48	14	10	AL.	11	44	4.4	40	49	40	40	42	160	40	48	41	48	46	18	1 4
r Temper			33		. An	25	38	25	20	22	12	24	52	4,	52	1	42	22	27	36	1)	dir	52	41	42	26	7.	
	mperature		2)	2/	34	25	J.,	28	47	25	53	41	58	46	58	22	51	24	73	43	50	102	57	44	4,	3,	41	
eather			" inter	See .	Com	-		Cen	Cur	Geur		-			1		Clear	7	-			1.000	- Car	-	-	6	,	1
ind (Bea	ufort Scal	• }	walia.	42	50	عامنت	10.52									12 11/2		None	1024	1 2	1.7	17	4.2	1	None	Nonc	Anna Anna .	T

Thermo-	Depth in	Type of													APRIL	1947												
couple No-	feat	material	16	16	17	17	18	18	19	19	51	21	21	21	21	22	22	23	23	24	24	25	25	26	20	26	28	Γ
-			2715	1240	0241	1755	0.7/4	19.4	27.5	LISTE	3525	0725	0425	445	1322	3125	dia	1141	140	0825	110	1014	14.0	12:30	3255	345	28.93	L
-	0.	Bit Cone	74	4.	34	40	22	49	74	+1	34	75	#	+2	54	#	70	41	54	41	1.7	100	17	34	45	75	134	I
2	2.	Sand + burne	32	*	22	44	34	4%	37	40	20	37	34	#7	+1	30	45	4	42	+7	5	10	50	47	47	59	37	Ι
3	0.0	Silly Glay	47	43	40	4	4.	*	4	40	*	#1	4/	45	4/	4	40	44	42	#	45	1.	15	44	45	4.	147	Г
4	15		45	40	44	11	44	43	11	47	+3	42	45	43	42	47	43	15	15	W.	45	4	45	44	1:	44	43	I
5	23		45	**	41	46	44	**	**	**	44	++	++	*	44	11	4	*	45	45	40	AT.	45	41	45	45	4.	L
6	32		45	45	15	45	15	45	44	44	di.	**	**	14	**	4	**	45	45	115	15	-45	45	45	45	45	44	E
7	44		44	14	44	46	76	**	4.	44	**	44	1.		4.	4	146	1.	45	de	47	48	4.		44	44	45	1
8	57		41	+2	41	47	47	47	47	+7	+7	42	47	+7	47	.7	67		47	42	41	42	47	47	47	47	140	T
9	67		45	#	11	48	40	41	46	40	40	41	40	4.5	-	46	**	44	40	149	49	48	40	40	45	40	40	1
r Temper	ature		24	3/	30	45	34	12	24	45	2.	20	7.			-	-	43	1	40	74	46	-372	7.	39	50	2.9	-
rtace Te	mperature		4	*	40	59	35	240	4)	47	25	28	40	15	45	41	100	42	50	100	150	1	CR	2	-	100	1 3	+
eather			Com	Car	Pine.	Come	1000				0.	-	-		-	40	10	9%	100	1	1	1	1	-		-7	1	+
nd (Beau	fort Scale)	-	222		Cont	7.	- valy	miledo.	month day		-tuide	-		in steday	her all	-	mide	-	-		+	distant.	- wileste	-	Lighter	Audit.	+
	ocus	A			int	wee.	156	ENT .	MAL.	stat.	-wal	steed.	and	1162	acs.	1346	1000	-	242	166	-	06	++44	4466	1366	2.86	14.00	4



	7.0	7.0	-													J.	ANUAF	RY 19	47											
	20	30	31	31	2	2	3	3	4	4	7	7	8	9	9	10	10	- 11	11	13	13	14	14	15	15	16	16	17	17	18
	3/	22	19	24	25	76/3	27	1625	0845	1505	0850	1620	.110	0915	1505	0815	1520	0220	115	0245	1505	0835	1405	0990	1535	0920	.400	0075	1535	
	3,	30	23	21	26	28	30	29	29	30	24	-36-	Jo 29	11	22	9_	20	43	.12	22	29	16.	29	30	32	J2	32	30	J2	25
1	22	34	33	32	32	72	32	72	72	32	3/	30	32	7/	70	28	22 -	16	19	25	.22	20	27	.22	1/_	31	3/	30	3/	22
	10	40	J9	39	30	78	30	33	30	.0	J7	37	37	37	37	38	28	36	36	29	29	28	28	30	30	3/	3/	3/	3/	31
-	11	11	11	44-	4.	43	42	42	42	42	41	4/	41	11	1.	41	11_	10	40	35	35	35	35		35	35	35	35	35	35
	42	17 _	17	10.	1.	10	45	15	15	45	44 _	44	44	11	11	13		17	13	12	12	42	13	39	39	30	Ja .	38	38	39
	51	57	54	51	-KJ	54	53	53	53	2)	52	25	52	52	57	51	51	Si	51	50	50	50	50	50	50	49	19	49	49	7
		3	Co	50	20	9.	00	56	56	56	55	55	55	ST	55	54	51	54	54	54	54	51	57	53	53	50	53	52	53	52
1	23	11	-5-	1.1	2	17	22	10	9	58 25	5 8	29	58	58	51	57	.C7	57	57	C	56	5	56	5%	56	56	5%	52	5	57
\perp	20	17	7	12		20	23	21	/2	23	23	29	21		-11_	-7	6		7		20	-7	23	31	36	32	75	25	32	30
	C.C.	Sucar	رسدب	ر ميد ت	Snow	Saaw	Sagar	Snow	Con	0	Cear	0	Cicer		^ 4	1000	2	2	6	7	15	0	20	75	35	34	32	22	3/	26
1	iJ.	1101	Sw.	of the	16.50	.¿E.	J'EL	122	11	WJ	SW2	SWZ	142	A.P.	ma lode	Lear.	LIEBY.	Laudy	a saw	wear.	Cuseur.	Cient.	Cloudy.	Nerge	80112	Ciear .	Couse	Car	Clear.	Ciea

												FE	BRUA	RY IS	147															
3	4	4	5	5	6	6	7	7	8	8	10	10	- 11	-11	12	12	13	13	14	14	15	15	17	17	18	18	19	10	20	20
	6510	101	1040	1500		141	182.	1413	0920	1410	0850	1420	730	1430	0720	1425	0720	1355	0830	1455	0930	1410	0135	1430	07.15	1400	0840	15/2	1825	20
4	21	20	31	. 77	30	2	26	31	31	32	25	32	23	33	11	.72	16.	32	24	31	32	33	32	32	21	36	.10	33	19	24
-	2/		31	22_	32	32	28	30	21	31	26	31	25	30	21	30	21	24	25	24	30	31	.32	31	31	32	27	32	23	3.0
2	3/	31	3/	22	31	91	31	.72	32	11	31	.41	21	31	30	30	30	30	30	307	31	31	32	31	22	32	31	72	32	30
7.5	33		.,,	35	13	33	35	3.5	25	.45	P (35	35	35	.2	.33"	33	35	35	31	35	3.5	35	3.5	20	35	34	36	35	30
3	38	-13	18	-21	38	23	18	-23	30	33	38	18	38	38	30	8	23	38	38	37	38	27	37	37	38	37	37	10	27	23
0	40	40	40	50	40	40	10	40	40	40	40	40	40	40	40	40	4.	40	40	37	10	40	37	24		39	24	33	39	37
Z	47	4	17	16	40	4.	16	166	10	46	16	26	16	46	46	46	45	14	46	45	46	46	15	15	40		24	40	31	3/
ji.	_40	36	4.	50	4	4:	47	149	47	25 7	64	19	47	49	84	47	18	13	18	18	48	45	12	12	45	12	47	10	47	43
	2	1 .2	4,2	53	53	52	12	152	32	.72	52	.42	." 2	.52	31	52	31	31	52	51	5/	51	48	51	51	44	50		41	41
2_	413	112	47	36	13	2	20	53	36	.35	22.	27	11	27	10	ماد	/2	27	22	36	34	11	25	22	2.7	38	30	19	3/	51
É.	-1	40	15	35	11	23	14	32	35	32	23	2,5	21	£ 7	4	27	11	33	23	35	71	4	20	34	20	20	10	-/4	6	23
46	1/11	Luck.	lucia.	Leve.	ilial	141	1000	2000	Kuin	Ruin	7/100	Jene	Care	1/0	2/10 10	11 .	Clear	Mark Committee of the last of			0	11	24		30	23	11	23	1	23
12	Azec	102		J N'2			NE 3		Nonc	Nose	10.3	100	4.7.4	Ma A.	NN 3	Wald.	N'W/	Vand	Carried B.	1.47	CLEUI	-Riar	- dury	Jord's	Leusy.	Kyr.	Cleyr	1841	161	Shar
								1	تخلق	-	-	, ,,		4.	77 7	77 77 77	77(4	114	N.W.L.	NW2	None	JW!	NEZ	NE4	NHZ.	NWZ	NN3	NW3	NW2	NW3

									MAR	CH IS	47																			
7	8	8	10	10	11	- 11	12	12	13	13	14	14	15	15	17	17	18	18	19	19	20	20	21	21	22	22	24	24	25	26
170	62.23	205	ONE	14.	2221	14.5	0 125	1970	0825	1442	02.0	4.2	07.2	4.22	0825	1435	2010	150	0225	Sis	0215	14:0	0770	1.340	05.40	1410	DESS	1540	0341	20
	12	11.	32	31	32	72	72	2.	22	34	34	40	35	47	12	12	22	11	12	52	32	56	31	57	32	57	15	52	2	
·	34		32	22_	J2 .	35	32	23	32	42	31	20	35	34	1)	40	22	40	23	12	32	17_	34	4	24	42	4.	11	1	7.0
	70	3/4	32	32	32		34.	J4	-22	-32	33	11	35	34	24	15	26	25	36	26	26_	2.	76	38	36	27	4/	12	1	29
	22		35	- 10	وكو	35	25	31_	36	35	35	36	24	J	20	37	30	37	24	71	30	2	19	.39	40	29	4.	41	4.	1.
1	37	74	377	Jr	12	Wa	22	32	30	37	37 _	27	37	37	22	31_	39	31	24	29	29	29	39	10	40	40	40	4	1.	42
,			37	31	17	J_{I}	07	30	39	39	29	74_	79	J9	10	27	11	13	41	10	10	40	10	40	40	40	11	42	11	42
-	44	27	11	71	17	11.	iz	4,	41	19	44	44	11	14	44	4)	_tt	11	11	11	11	#	11	44	44	44	44	N	71	45
	41	TA	41	-	- 11	24	7		-17	14	10	12	40	14	10	15	14	10	1.	-tu	14	to.	16	46	1,	40	16	47	10	+7
	32	7.0	77	75*	27	77	7/	7	75	77	77	41	49	#	17	10	11	10	48	15	12	48	48	49	40	11	41	19	48	19
3	74	40	33	J.	20	42	27	60	37	50	22	.11	38	41	-37	34	2	_Y	4.	+1_	22.	12	30	47	32	17	15	52	4)	28
	Snew		8	1.	28	-14	33	-	37	7	. 12 .	25	24	21	25	47	15	45	47	50	3/	57.	1/3	10	24	17	53	51	17	33
inden e	144	MANUAL TO STATE OF THE PARTY OF	Pr 1	2 1444	7	- 2 -	None		william.	C. Carlo	Alten .	Select	-itali.	allide .	ing.	wade.	State.	MEAR	Seller.	-inte	Cuesar.	weer.	Scar.	CLERT	- we	Cour	milas.	Cana	Bes	a sup.
			4.2	****	4.6		1-0.15		341	2 16'1	d 2	11.	42	sh di			marked.	112	WW.	18 42	None	12	412	disting.	wint.	1242	4	No . C	NEL.	SMI.

							AP	RIL I	947																					
5	5	5	5	5	7	7	8	8	9	9	10	10	11	11	12	12	12	12	12	12	14	14	15	15	15	15	15	15	15	15
2	27.1	11.	1115	35	CK.S.	290	0735	1925	2815	215	38/2	1212	3725	1395	515	3710	2812	40.2	1310	5.5	2825	12.0	0520	220	09.0	11.0	13.0	5.0	2.15	19.30
13	20	77 -		5:	1-	50	35	54	39	4.	10	42	13		51	52	52	51	-	.2	20	41	29	13	45	50	55	56	SC	48
1	32	31	12	15	12	27	1 22	50	. 22	12	26	53	1)	54	50	570	50	51	55	52	10	4	1/2	40	4.	44	18	62	50	19
-	Ai .	71	41	4.	12	17	11	11_	1/_	41	37	11.	44	11	47	12	47	12_	48	10	12	12	12	12	12	12	47	44	15	15
3	42	42	42	12	42	42	17	12	12.	17	12	42	43	11	45	15	15	15	15	15	10	15	45	11	44	14	11	14	11	11
5	12	10	12	42	12	12	17_	12	73	47	12	73	13	17	44	ft.	44	11	44	44	15	45	15	15	15	45	15	15	45	45
-	12-	1.	42	12	13	7)	12	22	13	47	12	43	43	1)	17	43	47	14	44	+1	15	45	45	15	45	15	45	45	45	15
- 10-	15	75_	15	10	15	45	te	1:-	15_	15	45	45	15	1.	45	15	45	15	45	15	16	16	#7	16	10	1,	4.	15	1.	46
0	20	10	1,	1/2	17	16	172	12	47	16	47	47	17	17	47	11	17	47	47	12	17	42	47	12	47	42	47	47	47	47
"	48	40	10	11	15	11	146	18	4)	18	18	48	49	10	43	10	18	48	10	#	11	18	10	48	45	15	41	41	41	40
£	3	47	15	1 05	45	77	12	15	23.	2.	39	50	17	المتو	15	54	25	57	66	2.2	25	27	27	39	17	52	55	55	54	17
u	4.		31	4.	1.	50	Ji.	15	43	11	41	62	45	52	154	54	57_	64	69	21	22	20	36_	12	12	5,	57	54	57	45
بالماسة	un tede .	-64		وه مانت	- de bit	74 hole	C243.	- Cul	Culary,	12.12	Sicar	Clear	Caus,	Gue,	Para .	Eva-	THE	2300	Lacar	Cleas	Cardo	Asia	Ciscop.	Cen	Celar.	Cer	- 440	Care	Cear	Clear
ه ۲۰		36 4	SEZ	- 1	43	60	- Back	diet	Se-2	JW2	142	S42.	SWZ.	SNZ	242	Sa2	SWZ.	SHI	Sw2.		SEZ	SEL.	None	Au.	N'42	Not.	באני.	Auz.	NW2	42

28 -3.45 -7.2 -4.2 -4.2 -4.2 -4.6 -4.6 -4.7 -3.6	
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Air temperatures obtained from thermograph record.

Surface temperatures obtained from merculy thermometer laid on the surface of the pavement.

FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

TEMPERATURE RECORD

OF

TEST ELEMENT H



Thermo-	Depth In	Type of		OCTO	BER	1946		_		NO	VEMB	ER IS	146									14 68	MDER.	DA	IE	ANI	0 1	IM
couple No.		material	11	16	20	22	31	7	15	19	21	23	25	27	29	3	7	10	12	14	16	5.4	54	27	27	2.0	30	1
	0.2	Bit. Conc.	1120	64	61	53	57	57	2957	1020	1001	10.0	220	2812	26		222	1000	30	2011	241	Alst.	100	9.0	did.		-	and
2	03	Sandremere		61	56	50	57	18	94	37	2	1	12	**	94	2/0	38	31	32	30	21	30	20		45	1	28	1 4
3	08	Silty Clay		58	54	50	59	51	51	41	1)	40	41	00	11	211	4	4	1	62	3.	2	22	M	di	20	.1	1 2
5	23		57	59	58	56	64	56	59	25	50	52	18	17	11	6.1	42	41	44	di	1)	1 12	1.	di	40	39	27	1 1
6	32		57	5	61	62	65	66_	67	6.	60	12	177	sr	57		50	12	32	10			12	10	47	40	92	60
7	4.4		51	59		12	a.	66	65	67	57	100	52	50	52		13	1.	11	12	27	10	17	50	17	11	14	1 1
9	57		59	58	61	.2	60	.66	67	65	60	4	0.4	17	-2		SV.	11	51	31	10	10	50	1	12		8.	1 3
Tempero			64	67	56	52	64	49	41	43	17	Je	-	42	20	43	41	-	Sa.	27	-	1	17	12	9 "		12	-
urface Ten		man arrow b	41	C	60	55	04	59	42	47	1	1	100	43	37	27	20	The state of	40	32		2	20	1	p		40	1
Veather Vind (Beaut	lort Scale		agar	Clear	Clear	ital	Comp.		Siew.	Can.	Coop,	500	Clouds		- 40 44	inga-	Cea	Cea	-ta	60	dath	o-Jody	Lan	200	48		60	-

Thermo-	Depth in	Type of									J	ANUAR	Y 19	47								-	r					
couple No.	feet	material	18	20	20	21	21	22	22	23	23	25	25	27	27	28	28	29	30	30	31	31	ı	1	3	3	-	1
			1410	1.347	1441	7.340	1-2-50	1447	1145	244	11:15	3723	1400	141	Legal	1945	1000	34.0	3350	1445	20.	4	25.22	1000		and !		1
1	_ 02	Bit. Conc.	1.1	1-11	34	34	1+	13	وند	4	14	141	33	34	3.0	34	1	1 4.0		01	1 11	2.0		0.0		-		-
2	03	SandeGrand	121	10	30	33	12	4	15	13	and.	11	. 3	31	1.50	34-	30	47	100	0,	100	6 4, 4	40	100	00	1	-	1
3	0.0	Silt, Clay	31	30	30	31	0,	21	21	30	49	10	-0		1	31	11	130		4 4		000	30	30	-	0 000	44	100
4			34	13	34	34	1 cm	1 de	1.4	4.5	34	1	4.		14	1 44	14	1		-5-2	1		34	00	-	9 486 4	31	1
5	2.3		. 33	13.	.38	38	18	14	14	1	33	14		4.8	1	100		-			- della	. 2.	and the	-	44		-	1 33
6	3.2		14.	1-1-1	44	44	14	2000	44	44	14	.3.0	44		4.6	10		30		23	78	. 3.	38	38	3.4	199	25	138
7	44		4.4	4.5	45	12	43			1		-8-4		19.5	4-	10	1	24		W.	4.1	. 25	6,0	41	43	3 413	92	1-41
8	57		6.4		14	-		T.	12	1	-to	1 46	20	47	41	10/		1 0%	71	11/	4.	, al.	107	10	41	14.	4.	47
9	6.9		-11	Car	- 12 -	27	2 0	2 4	-	-	20 00			100	-7	1				1	3	-51	al	34	-6	150	1	131
Air Temper			1	-		-	3.3		-	-	10.00	-	2-			-			- 1	2.4	9"4		10	10	1	1.14	L. A.	1
Surface Te		M04	13	12	10		33			= 2	las	1-1	-17	34	-	+ 1	-	1 9 1	00	-13	20	1.1	de	01	10	1 da	00	1 00
Weather				1-4-	33	10	33	-7-			15	16	+1.	4	10	41		- "	-	08	- " 11	12	10	20	15	34	47	20
	fort Scale	1	med de	we direct and a	184411	reside.	wy	-tede	-1-00	A5"	- Cade	-wade	16000	liain.	Harry	well-by	-1 1	100 4	and a	-	Sa.	C . A	W. 8"	-10-	42.00	A. JUNE	-04	100
ALUIG (DEG	101 2001		14.4	m.l.	E1	a Min	- Na	hai	MAG.	pl'in	1200	None	361	Nese	124	Alena		49.971	Vone	2.1			San de	AN !	Alal A	100	ARRO	1

Thermo-	Depth in	Type of						FERR	VARY	1947											-	-						-
couple No.	feet	material	21	21	22	22	24	24	25	25	26	27	27	20	20	1		3	4		5	5	•		7			
		2	08.72	112	2135	1880	CETO	1922	3812	153	220	2011	1.0	100000	1.8	Saut.	1000	100	29.50		-14		1.	1.4.			-	4
	02	Bit. Conc.	10	4	25	31	21	d/a	24	24	15	20	47	29	32	1	da						- 11					
2	03_	SandGrand	42	22	2.	29	20	32	20	Ji:	31	22	30	29	22	20	10	73	1	4								
3	08	Sitty Clay	22	32	22	22	22	22	22	32	Ja	22	21	32	1/2	32	22	12	34	49	0.01		9 *		-	-	240	-
_ 4	1.5		38	25	25	25	35	25	25	15	34	15	25	35	,	1a	Ja	1	1	0.0	-	4 .	9 44		-10	100	10	
5	23		27	32	20	28	12	70	33	30	17	7.	30	3.5	27	22	20	20	40	7.	14	-		-	29		31	-
6	3.2		62	42	12	12	12	#2	42	12	42	42	41	42	4	13	41	41	ra.		d		17	1	411			-6-
7	44	,	15	45	15	15	15	45	05	dr	or	44		41	44		4.		20		94		1	-	46	40	0	
8	57 .		49	49	47	11	40	41		40	40	11	10	01	40		-	1.0	00	**	00	70	40	4.0	25	~	00	816
9	69		52	C.	52	52		81	-	-		-	-27	-	-	2			100	"	20	0"	00	- 10	44		200	6/7
ir Temper	ature		,	. /	25	21	12	29	-	-	1		26			1		8	2	1			120	4 3				
urface Ye	mperature		-		24	20	30	4	48	all	31	44		40	38	- 44	30	10	4.4	28	20		8 40	0	60	0	.0	
/egther	and corollinations of the	***************************************	-	,			24	Z	24	34	39	30	16	47	11	Ja"	44	27	20	44	2)	47	10		41	\$11	reb .	90
	fort Scale	1	3-03.		Siller				Victoria.	24		-maly		medi.	L'an	-			un debige .	190-		100		. ber		-	300w	de red to
	3001	'	1663	dill	1860	Sais	None.	Sal	4000	#2	106	None	03	Non	11	Name	1.	34	denn	ada	12			Las				100

Thermo-	Depth in	Type of				MAI	RCH I	947																				
couple No.		material	26	27	27	28	28	29	29	31	31	1		2	5		3	4	4		4	4	4	4		5	3	-
1	02	Bit. Conc.	43	ال ال	50	0.00	51	100	57.4	31	(files	4 4 4 4	114		1922		and the	Hal	2010	Adri.	Max	Link	diam	dada.	100	بمماعاتما	-	- La
2		Sand-Grave		11	14	21	27	13	94	34	2.1	14	54	#.S		. ص	40	00.00	31	40	35	100	01	44	34	00	20	41
3	_ 08	Silt, Clay		.20	23	33	31	34	23	13	31	10	40	4	4./	43	10	3	100	100	7.	1	-7	4.0	00		0	de
	1.5		ild	ito	+1-	40	+0	.pu	10	PL	10	1:	40	14	-J	3.4	10	41	01	113	4.	4	1	1	,	11	24	0.0
6	4.0	-	45	43	for	44	16	4-da.	4 30	44	-da .	440	1 da	14	-8	13	13	11	11	2.4	15	1	45	1				
7	J.2.		11	-34	TT	4.2		1.1	44	**	12	10	41	194		. 11	1.3		dr.	b.e.	Pry	20	14	11	10	1	0.0	
8	57		4)	47	43	47	de	2.	4-	1840	te.	1-	185	15	F to.	1		f.	19	1	1.	10	1	-	10	10	1	10
9	6.9	,	17	19	12	100	12		41	12.04	17/		4.	4	4.	1.	11	1	2.	10	10	7	20	2.	10.	100		10
Temper			.M	.16.	30	alas).	23	dis	11	42	Cale		5	21	-		1			77			-da	-de-	-	-	-	-da
ortace is	mperature		33	da	35	21	47	20	9 44	22	54	41	51	45	. 3			20.0		21	1				A	and the same	1	Aug.
	ufort Scal		do do	-lud"	4.95	and to ad "	400	wica:	dias	-	-	-6 = 01	minute.	men ale			-	and	-40.0	21.0	-44	1000				-	-	
	01017 3001	,	4 4 3	10	10 %	14	el on de	Nerv	.40	Nod	1110	And.	400			60.	ad.	Aluen.		18.0		10			Sine .	880 m		

Thermo-	Depth in	Type of													MAIL	1947						-						
couple No.	feet	material	16 2211	16	17	17	18	18	19	19	51	51	51	51	21	55	22	5.2	5.2	24	24	25	52	74	24	24	20	20
	02	Bit. Conc.	32	y.		12	- 1		139	11.0	35		-			44	-l-	- Allin	-	ander.	-	and a	-	-	-	-11	12	- din
2		Sundi Gravel	31	J.	1 1	12	3_		2.	9/	31	127	- an	4.	17	40	1	1		1 "			-	1	-	Es .	-	
3	- 08	Silt, Clay	. 17	t.	10	12	7.	11	7	1.	145	1.	22	44	77	111				1 .				Pa.	44	0.00	-	
4	1.5		27	177	1.	1	10	15	10	1/0	196	0.		45	e/		44	10		1	1	1	1	- 00	277			-
3	23		10	10	17	1 1.	1.	1/4	F .	1.	46		46	4.	4.	0.	0.	1		1 12	1	1	1	100			-	-
6	J.2		1.	0.	127	12	1.	. 7.	1	7.	+1	21	97	9.1	41	91	4	1 41	1	1			1 10			do	-	
- /	44		1977	1	17	11	1	27	1.	ø,		P.	2.	41	49					1 0				1		1		1
8	5.7		21	10	10	14	90	11	go.	4.0	70	9	7.0	11	100	47	0.0	1 00	1 00	1 00	me	1		100		15		1
Temper	6.9		9.7	1	99	91	90	01	111	9.5	1/2	1.1		2.1	40	do	gu	100	00	1	-	4.5	907			1	100	
wince To	mperature	_	12-	.1		1	200	11	13:	di u	180	120			3.	17		0	1 0.	1 0	1 "	- 19	1 14					pude.
eather	mper ature		-1	Zi .	0,0	1.0		1	45	ida	1 35				6		70			1 .	19		1		-	- A. A.	1	
	fort Scale		whole	وعصاديب	-						-	- colpete			-			1					1	-	1	with the	0.7	
(0000	3601	/			oli a	200		34 8		100 2			0 0 0mm			126.			10.2		-			-		and the same	0100	TO HOST



_	IM	_	_	_	_	_	_																							
50	80	T	1	51	2	1	13	I	13	13	I	TY	1	1		10	10	33	11	16	13	14	14	16	16	16	16	17	17	
200	41	#		444	44.00	44	14.4	44	14.0	4	17.	10.7	27	1	7	7.	4	-	4	ale:	di di	2	42 40	20	Jr.	20	32 38	4	JI.	24
日秋秋	18.80	1			4 4	2 4 8	4.5	4 4 2	20	4.0	6.	40	100	2.5	4	44.6	44.4	2 4	11	4	4 17	11	11 14	21	11 11	33	31	11 11	74	34
14.	100	12		A.	4	0	3	10	15	150	100	65	200	0.00	2	000	20	15	12	2.2.2	17	11		12	9	4 0	200		200	6 6 0
10 . 3m		12		1	4	4	47	15	1		2	66	2		-	y Gas	inter.			-	4		41	34	W.	22 23	Sr M Ones	17.	2	2000
-	-	-	-	_		1.0	-	1-4-	941	147	Pint.	1144	100	1.00	100	100	347	36	16.6	det.	1000	46.	de	MAN.	Ad.	de	200	as.	40	100

-		-			-		_		_	_	. 1	200	AT 194				_	-	_	_	_	_	_	_	_			_
-	17	27	4	100		de	clu.	200	18.22	10	150	224	100	1 13	13	13	14	14	15	1511	17	17	18	180	19	19	20	20
27	14	4	Je Ja	4	11	12	45	30	34	44	34	40	3	2	100	41 34	34	4	30	30	34	31	11	M	44	11.	17	31
14	44	44	3	14	18	32.	1	10	14	44	44	38	4	1 34		36	38	12	44	10	11	12	30	15	31	36	15	1
	47.	44	3	Ö.	*	11	10	1	44	77	*	44	2		15	12	1	41	*	4	45	46	44	44	4	*	4	44
3	**	4	1	16	30	芳	7	12	芸	44	27	41	44	4 54	14	Ja .	15	34	H.	41	44	12	4	10	40	1	71	57
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								MAR	ICH I	947							-												
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40		0.00		40	fig.	4		. 7				4		0 m2 m2 m		Ja d	17	1) 20	1	100	17	J)	1/2	30	10	15 11	16	42	44
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		A									er.			0.7		-		. Allen	00	**	de	49		50	27	00	85° 40°	01	11
base	**	7		100 1100 1100 1100	e i				60		dy Sec	9		0.		A	dit.	47		7	4	31	00	10	#2 #2	11	53.	12	11
	140		100	-	-	Mese	0	Beg	.0			10	21			42	31	800	200	Repre	7.	33		100	Jac.	2.	Ales o	No.	53

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mar.	-	etic.	parte.	-	بالاصلية	rising	illu,	Alle	4434	iritie	and said	-	وهائشه	100	die	of Art	Asia.		7.4	A STATE OF	10.2	STATE OF	al file		1112	1310	42 M	1300	1414
100	20	150	100	100	905		20	100	2.0	100	18				-X4.	24.	46.	.71	- 0	01			31		100		10	-01	
44	7	201	25	7 10			-		111		00	20	200	1-72		100	de.	*4	-	91	10	19.2	#4	NF a	23	04		05	59
-14	74	-	7		20			00	9.0		4/	- FW	100		20.	.01	14	-44.	42.	47	do	A _b	44"	40	20	mb 0"	40	de	40
47.	14	-	71				10		20		00	40	0.0	3.25	-		150	126		Øa .		-		20	to.	No.	4.	6	de
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10	0.	20		0	100	40	4			10	10		10.0	4		120	27	133	170	10.0				24	0.0		-/-	14	1
-	- military		- Amor	Amo	and a	profitor p	dies	ala.	-day	83_	12	10		- 0			7	41	et.	4.5	10	45		0.0	07		20	10	70
	- Aur		80	4.	. 4 2	-	1	0.0		- 1	-		23	10.0		100	43	**	100				andribas (10	and the		4.4		
00	*4	90		40	10-1	00		4.0	10	24	08	. 74	-		20	65	43	60.	16			-	-		- 7	1	11	118	5.5
othe 0	perc d	1000	44	-	-	posts of	1000	1/4		mis. P	er fult	p ing	-	100	-0.0	Wat !	-	-	Mer.	and the		4 10	-0 0	100 0		40 0	-010	4.5	red ger
	29	-		1	-	0 00			0.0	2	100	470	14.0	-	200	100	484	100	-		10.0	-	-	-	A = 8	200	4 -	Fra	44

Air temperatures obtained than increageout record Surface temperatures appeared from mercury mermaneter told on the surface of the personnel

FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

TEMPERATURE RECORD OF TEST ELEMENT J

SOLS LABORATORY NEW ENGLAND ENVISION BOSTON, MASS. AUG. 1947

TABLE &

hermo-	Depth In	Type of		OCTO	SER	1946			_	NO	NEMB	ER 19	46		-						-	DE CE	MER	1946		
ouple No.	feet	material	11	16	20	22	31	7	15	19	21	23	25	27	29	3	7	10	12	14	16	24	26	27	27	28
1	9.2	BIL Conc.		62	.52	67	56	52	41	13	10	43	4-3	19	45	30	33	31	36	32	27	72	29	12	25	16
2	03	Sand (Grave)		64	50	55	55	19	44	10	12	13	41	47	41	3/	32	37	32	22	22	22	30	20	27	21
3	04	SultyChy	58	57	50	52	57	11	19	44	16	46	42	18	4	35	35	38	32	31	25	4	34	32	32	20
4	15		55	51	56	56	62	59	55	57	52	51	49	51	51	15	42	43	43	15	43	40	40	39	39	28
5	2.3		55	57	60	60	61	60	59	58	57	56	55	53	55	51	50	48	11	19	43	15	15	45	45	44
6	J2	•	55	57	61	62	64	64	62	61	61	60	50	57	50	55	55	53	52	52	57	50	49	49	47	49
7	94		56	52	61	62	66	66	67	65	65	65	65	61	61	61	57	58	57	58	37	56	55	55	55	55
8	57		57	58	61	62	65	66	67	66	66	66	64	64	63	67	61	61	60	61	60	59	58	58	58	58
9	6.7	•	57	57	6/	60	64	66	67	67	66	66	65	4	64	63	63	62	62	62	62	61	60	61	60	60
Tempere			67	65	54	65	52	51	11	45	177	37	17	42	79	27	41	35	37	26	.4	37	27	-6	10	-1
	nperature		79	65	57	67	52	52	16	45	39	37	47	44	36	24	40	40	40	30	17	24	24	2	10	6
ather	-	Contraction of the Contraction of the	Clear	Clear	Car	Clear	Cloudy	Clear	Clear	Clear.	Clarely	Gear_	Clouds	Clear	Cards	Clear	Coper	Clouds	CIER	Clear	Circ	C.sea.	Soon	Cer	Clear	Snow
nd (Beaut	lort Scale			Sel			WSWZ	NWZ	N3_	NZ		NWZ.					SHL							NWS		

Thermo-	Depth in	Type of									1	NUAR	Y 19	17									T				
couple No.		material	18	20	20	21	21	22	22	23	23	25	25	27	27	28	28	29	30	30	31	31	1	1	3	3	4
	02	Bit Cons	24	20	3/	24	33	12	20	10	22	26	32	32	33	22	34	21	21	.70	7	7,	28	25	11	34	26
2	0.1	Sande Grane	10_	21_	21	22	32	.22	24	13_	21	26	21	21	21	22	22	24	29	20	21	124	12/	12	20	21	22
3	0.0	Silly Clay	30	26	21	2/	21_	30	21	22	21	27_	22	21	21	31	2,	3,	20	21	2	3	21	3,	27	21	28
4	1.5_		22_	32	22	22	12	32	22	22	22	32	22	32	12	22	37	27	32	22	22	22	22	22	24	34	22
5	2.7		34	32	11	28	21	38	21	37	21		22		22	32	27	27	27	22	32	1-	32	27	31	28	20
6	3.2	-	11	12_	11	: 14	42	12	42	12	12	12	12	12	12	12	41	42	4,	1/	12	42	42	12_	42	42	12
7	11		50	44	49	11	11	11	11	19	12	12	19	11	10	#2	47	40	43	12	18	4)	48	49	41	48	42
	57		53	53	53	52	52	52	52	50	52	52	52	51	51	53	51	51	51	51	51	2	51	57	51	51	51
9	69	4	56	56	56	54	5%	23	56	55	55	55	23	55	55	55	55	55	54	54	54	10	51	50	54	50	54
Air Temper			28	12	10	10	رر	4	1	-2.	12	2/	22	34	36	31	34	27	24	21	22	38	24	22	15	22	20
The second second second second	emperature		70	15	20	40	22_	9	1	5	15	26	10	24	25	3/	37	28	26	29	22	12	21	29	15	20	27
Weather			Claus	Clari	Rea	Car	Clark	Cienc	Cien	Cien					Roin	Clouds	Claude	Cloud.	Clouds	Cardo		Lloudy	1		Sear	Cea-	Ces
Wind (Beau	ufort Scale)	1414	EL	EL							Nene											ilus.	1			

hermo-	Depth in	Type of						FEOR	JARY	1947																	-
couple No.		mate, ial	21	21	22	22	24	24	25	25	26	27	27	28	28		ı	3	4	4	5	5	6	6	7	7	
			3840	.220	0210	1344	0850	1920	0220	1350	1350	0825	120	0825	1510	0945	1420	1900	0805	1510	3840	35.5	3812	1405	arsc.	1450	12:
1	_02	But Cook	12	21	25	29	28	22	24	20	41	20	10	29	35	21	42	26	12	29	21	27	22	47	31	11	L
2	01	Sand Grove!	22	42	26	71	28	31	25	29	32	29	22	29	32	20	21	32	22	22	72	22	32	26	22	40	
3	20	Silty Clay.	29	28	28	29	20	20	30	20_	30	24	32	34	3/	20	20	21	21	21	32	3/	32	21	22	22	1
4	15		21	21	24_	29	24	24	24	20	24	22	30	33	27	24	4	34	24	24	24	22_	22	34	34	34	Ι.
5	23	•	22	22	37	27	22	.22	22	22	32	27	30	37	22	22	.22	22	12	27	22	32_	22	27	22	37	Τ.
6	. 32		#_	11	.11	41	10	40	10	11	17 40	10	41	42	40	10	10	10	do	10	10	10	10	10	10	40	T
7	4.4		16	16	16	16	*	16	16	16_	16	15	46	45	45	15	15	45	15	15	41	45	4	15	45	45	1
	5.7		48	12	10	12	11	4)	11	10	11	11	44	15	47	11	18	12	42	47	10	47	11	#7	47	47	1
9	69		57	5	51	51	51	51	57	51	51	51	52	51	51	57	51	50	50	53	2	Ch	50	50	50	50	
Temper	ature		1	13	20	20	22	19	12	20	31	24		22	24	26	22	37	29	25	20	2.	34	40	34	4/	١.
riace To	emperature		12	15	29	Jo	24	1	44	22	29	20	36	27	23	35	42	37	22	10	33	30	34	44	27	12	13
ether			Claus	S)	Saaw	Same	Cer	Cear	Same	Creac	Clear	Clear	Clear	Ciere	Cen	Cicar	-	Care	Sauce	Cana	Care	Care		Claude	Same		~ .
d (Been	ufort Scale	1	wil	364			Noce	Swa	Nose	1/2	ANZ	None		NOAL	4/2	Abe to		1	None			342		ANI			

Thermo-	Depth in	Type of				MAR	IÇH I	947																			
couple No.	feet	material	26	27	27	28	28	29	29	31	31	1	1	2	2	3	3	4	4	4	4	4	4	4	4	5	5
			1355	0810	1420	0710	1410	0735	1110	0890	1420	150	1340	520	1150	0816	1.44	-320	6710	0410	1120	1.520	1510	1710	18:5	0510	071
1	-96	Bit. Conc	41	25	11	27	59	31	SE	32	62	36	67	43	61	36	4.3	28	32	14	58	65	66	61	.53	32	3.
2	-01	Sande Grare!	.44	27	12	30	18	32	10	12	52	33	56	41	35	. 4.5	33	39	33	35	46	14	57	57	5.0	36	130
3	-00	Sul's Clay	10	36	36	15	36	26	36	.17	35	.15	30	42	13	4	41	41	10	10	40	41	13	.1-	16	4.1	10
4	1.5		1	11	40	39	39	10	39	10	37	4.	12	42	12	4 1	12	63	43	43	42	42	42.	12	1/2	43	1
5	2.3		50.8	12	12	41	41	41	11	41	11	41	41	41	12	12	42	43	43	43	43	23	25	43	23	43	10:
6	3.2	0	13	43	43	43	42	13	43	43	12	12	12	43	13	43	33	43	43	43	43	43	43	4.4	43	43	4
7	4.1	lą .	15	15	45	45	15	45	45	15	15	45	45	15	45	45	45	3:	15	45	15	15	45	45	33	43	1
	5.7		10	47	42	47	11	47	47	47	16	47	46	47	47	47	36	47	19	47	47	11	47	47	47	47	1
9	69		17	19	17	12	19	49	49	12	48	17	48	49	37	49	45	47	44	44	49	49	47	49	49	19	4
ir Temper			29	17	10	20	38	25	38	25	12	38	32	41	52	31	13	23	33	35	13	25	.10	48	32	26	7,
urface Te	mperature		33	20	35	25	44	30	52	30	54	41	56	45	57	34	50	24	33	44	17	14	57	51	23	.0	11
eather	- "		Clarks	Clear	Sour	Sugar	cker	Cicar	Clear	Char	Cher	Crown	cher	Church	Cloud	Clear	Clear	SKHE	CLENC	ikur	1245	Lege	Char	Clear	Cleur		Cle
ind (Bea	ufort Scol	•)	5125				-	Neg 2	SN2			NW3				1143		Moge			13	N'3	13	NH2	None.	Alan	10

The/mo-	Depth in	Type of								_					APRIL	1947											
couple No.	feet	material	16	16	2700	17	18	18	19	19	0340	21	21	21	21	22	22	23	23	24	24	25	25	26	26	26	2
	04	Bit Cox	32	50	14	37	10	4.5	39	52	34	35	43	27	ke	45	19	45	27	er	41	di.	.01	37	44	12.4	_
2	0.1	Sanistant/	37	42	37	10	31	46	40	44	30	37	40	44	52	10	65	45	37	11	46	12	33	43	41	67	3
3	0.	Sulta Clas	41	de	44	45	14	44	44	44	40	43	42	114	13	42	14	4.1	40	4.5	17	1	40	47	42	45	4.
•	1.5	Section 2	44	10	4.5	45	4	10	45	45	4.	1.	15	40	45	15	44	41	4.	1	46	102	46	47	47	47	1
	2.3		46	44	46	44	45	42	45	45	15	4.	10	AL	41	40	15	44	40	66	16	41	46	44	47	11	1
	3.2		40	+.	46	46	1.	4.	44	40	06	44	di	46	44	44	46	06	46	46	47	10	46	46	47	11	
7	4.4		42	47	47	47	41	47	47	67	47	47	47	47	47	47	07	47	47	47	42	47	42	47	41	12	1
	57		62	42	4.5	41	45	07	47	45	47	40	48	41	21	45	17	15	4.8	41	11	47	17	45	18	4	1
	69	.0	47	47	44	42	47	47	49	47	42	49	43	41	17	15	48	40	19	41	47	18	45	49	44	15	4
r Temper	alure		36	15	35	41	16	54	39	1)	25	24	31	34	10	31	52	×	44	47	54	11	52	30	39	150F	2/
urface Te	mperature		44	111	44	40	15	150	41	50	20	15	45	14	17	10	20	41	50	52	60	15	37	32	47	4.7	30
deather			100	Link	Sugar	200	Ber	Lend	Cardo	Gents	and	Cheele	Buch	Church	Check	Clear	Cheer	Clarita	Cheste	Buch			Cour	Chance	CAR	Char	15.
Find (Bees	viori Scale	•)		VE A	W 2	10:2	22	i note	NWZ	NHA	NE.S	NES	NES	AC.A	NE 3	NE 2	NWI			24/2			102				100

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A	NI	D	T	IM	15
	м	u		1100	Е.

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2	8 :	30	30	31	31	2	2	3	3	4	A	7	T 7	1 0			- V	ANUA	RY 19	71											
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116	1	20	41_	11	27	21	21	22	24	18	28	20	30	29	- //	20	-	12	0	14	/0	27	12.00	1770	277	1390	0925	16.05	OFF	1540	0240
21	1.	29	20	11	24	27	25	21	22	22	28	21	120	24	-44	122	1	1/2		17	10-		- 12	27	20	12_	3/	22	21_	22	24
20)	3/	3/	21	20	27	28	30	20	27	29	27		28	15	66	11	17	12	15	20	26	12_	25	29	2/	30	21	30	2	26
31		22	32	72	27	26	36	36		4			21	30	25	26	22	23	20	21_	25_	24	24_	24	28	29	30	30	3/	3/	20
-		47	41	43			40	-20-	15	25	35	35	34	. 25	136	25	24	34	.33	33	32	32	32	32	32	32	22	32	32		7.
1		7/	77	47	12	12	42	-11	4/	11	41	40	10	10	10	40	40	40	40	39	JA	38	38	7.0	30	- 74	- Charles				-11
4		11	11	10	47	47	47	12	47	16	16	46	15	45	ac.	45	Ar	Ac	40	40	44		- 40	-	11	12		37	31_	2	36_
53		59	54	50	54	54	54	51	54	52	53	52	52	1	60	52	-T?	77	T2	73	7.7	77	77	4/	47	12	47	41_	12	12	42
50	9	57	57	57	57	57	52	58	58			CC	26	52	52	32	52	52	52	2/	3/	51	51	51	50	20	n	50	50	so	So
10	+-	60	60	40		20	-1-1-	30	30	20	20	55	-55	5	55	55	55	55	55	55	54	54	54	54	54	57	53	53	62	62	C2 1
00	_	~	60	00	61	60	27	54	54	59	59	58	58	58	58	58	58	58	5.8	58	57	57	57	57	C7	C2	C2	Ch	- 22 -	57	-12
1 -9	-	2)	18	-4	12	. 2	17	22	12	9	25	.24	29	24	-/	//	-8	4	-6	7	0	20			24			-	-36	34	-26
6		20	17	7	12	16	20	22	2,	12	22	2)	29	27	-4	0	-3	-	0	1	2			6/		_26	32	-75	25_	12	30
300	w 0	1000	Clear	Ciana	Claus	Snow	\$ 14	Sugar	Same.	Cia	0	The second secon		THE CHAPTER		- 0		2	-	-0-		19	- 6	4_	34	35	35	31	25	31	26
					41.49			2000	- W	LIEGE.	LAC.	Licar.	Cardy.	Clear	Cinar.	Clear	Clear	Clear.	Claudy	Snow	Clear	Clear	Clear	Claude	Row	Real	Clear	Cloud	Clear	Clase	Clean
1.12	2_11	11	NH2.	Silla	المشاد	NEL	NE!	NEL	NEZ.	ALL.	.43	SW2.	SHZ	N#2.	122	NW	NW2	NWS	N2	N2	NW	NW2	NE !	NEL	None	NAI		34/	44.9	4/2	CALLED
																										2347	Ja 7 .	DE.1.	W	WZ	782

												FE	BRUA	RY IS	47															
1500	2100	1410	5	1525	6	6	7	7	8	8	10	10	11	11	12	12	13	13	14	14	15	15	17	17	18	18	19	19	20	20
34	26	22_	20	31	27	35	25	3/	22	22	24	32	27.10	35	0723	7723	2720	32	23	1505	20 30	140	2241	1935	2225	ner	0240	1510	0215	1910
11	22	21_	22	34	21	22	27	20	2	.2	25	3/	24	31	24	21	20	29	24	30	1	2	75	31	2/	1/	20	4	12	25
21	29	20	11	21_	¥	31	3/	1/	2_	2	29	20	21	20	25	29	27	20	4	20	1/	2/	2/	31	2	2/	2/	72	20	1
24	22	24	77	34	34	24	3/	34	24	24	24	34	21	31	34	4	24	34	4	22	34	26	22	21	21	34	2,	25	*	20
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18	12	49	47	41	47	47	17	42	17	42	42	11	11	12	12	1/	1/	41	41	10	11	11	#	#	41	#	40	12	11	4
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20	67	20_	75	25	17	31	21	32	25	22	25	29		20	4	12	12.	20	24	15	25	**	40	20	29	12	19	21	7	24
Sea.	Man.		SWI		Sieur	Bac		Jan.		Run			Ciear !			CHA	Clear		Cardo.	Citar	Clear	Cicar	Crafy.	Chady	Cieras	Crean	Clear.	Car	Clear	Clase
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150	2240	1415	085	1420	0725	1450	0125	1430	-530	1440	0720	1410	0710	1355	0520	141:	0810	1510	0720	1510	1710	10.0	0730	1340	0840	14.0	0255	1540	1420	1/2.0
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10	.J2	32		37_	32	13	32	48	33	50	31	40	37	14	33	47	32	46	31	17	32	50	34	41	27	48	13	53	42	40
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	45	45	15	45	2	15	45	46	45	11	12	11	44	44	44	24.	44	44	44	41.	15	45	44	45	45	44	15	45	15	16
50	47	47	47	41	37	47	47	47	47	47	47	47	47	47	47	46	46	26	46	46	46	46	16	46	47	46	47	47	47	67
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what.	Snow	Sacre	Sraw.	Kein.	Clear_	Cear	Clear	Clear	Siene	Clear		8417	Cieze			Claude	Clear	Clear	ar	Clear	clear	Clear	clear	cker	Ciear.	Clear	Clouds	Cloudy	Rea	Claude
***2.	Adl.	AE2	-NI-	1E 2	N.	NEZ	None	EI	JWI	JN1	13	13	W1	1/2			W3	N2	NW3	NW2	None	52	N3	NW3	NW2			None	4	SW4

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25	10	710	6910	1110	1310	0810	1340	0715	1410	0810	1340	0810	1410	0730	134	0510	0110	0910	1110	1:10	1510	0820	1410	0510	0710	1710	1110	1310	1510	1635	1100
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1 2	6	.6	40	16	19	44	53	37	57	39	16	37	61	44	6/	31	51	52	53	10	64	41	05	40	41	24	57	67	6.5	63	59
13	14	12	41	11	12	13	45	42.	44	12	43	40	44	46	47	30	50	50	50	20	51	44	44	43	43	43	44	45	41	19	50
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1	3	13	43	13	43	13	43	43	62	14	44	14	44	44	100	45	45	45	15	45	43	46	46	46	16	46	46	16	46	44	46
10	1,5	43	.23	43	43	44	41	44	44	44	44	15	44	45	135	15	45	45	45	15	45	46	46	146	16	46	46	46	46	46	16
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Air temperature obtained from thermograph record.

Surface temperature obtained from mercury
thermometer laid on the surface of the pavement.

FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

OF TEST ELEMENT K

																								DA	TE	AN	DT	IME	
hermo-	Depth In	Type of			DBER						VEMB											DECE	MBER	1946		-			
couple No.	feet	material	1415	16	1245	22	31 /230	7	15	19	21	23	25		29	3	7	10	12	14	16	24	26	27	27	28	30	30	31
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4	13		56	51	53		• 1	34	32	52	52		15	51	50	43	42	43	44	45	41	39	37	18	38	31	35	15	31
5	23	10	56	5%	2.8	34	64	61	60	34	.4	. 7	.4.	. 3"	25	32	50	48	49	49	47	45	45	45	45	44	43	42	42
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Thermo- couple No	02	material	1410	19	/645 33	0140 33 33 30	1430	22 3745	22 1550 16	23 0745	23 1505 21	25 01/0 26	Y 194 25 /41	7 27 3/03 34	27	28 0540 32 31	30	27	30 0905 28 29	30 /440	31 0450 34	31 /420	26	1 /4 4 9	3 0710 11	3 1000	4	4 /4:0	5
Thermo-couple No.	02 03 08	B.t Co.k.	1410 30 49 32	19	30	1540 13	1430 32 32	22 3745 15	22 1550 16	23 0745 1	23 1505 21 21	25 01/0 26	Y 194 25 /41°	7 27 3/03 34 31	27 /435	28 0540 32 31	33	27	30 0905 24	30 /44c Jo	31 0450 34	31 /420	26	1 /4 4 0	3 0710	3 1450	4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4 /410	# 3.
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Thermo-couple No.	1001 02 03 98 (2) 23 32 43 57	B.t Co.x Sepus Grave Sirry Gray	1410 30 49 31 31 43 41 53	0850 19 20 27 31 38 43 48 53	/645 33 30 -r 31 34 47 53	32' 31' 31' 43' 41'	1430 32 31 31 31 31 43	22 3745 15 41 31 34 43	22 1550 14 22 30 31 31	23 0745 7 10 44 32 34 44 44 52	23 1505 21 21 15 32 11 43	25 0100 26 26 26 27 27 27 47 52	Y 194 25 /41° 35 30 26 31 37	7 27 3/63 32 31 31 31 42 47	27 /44) 33 31 31 31 44 47	28 0740 32 31 32 37 42 47	33 31 31 31 31 42	0140 27 25 31 32 31 42	30 0905 24 29 30 32	30 /44c 30 31 32 37 42	31 0450 32 31 31 31 32	31 /420 Jy 33 31 J2	26 27 31 31	1 /4.4 • 37 35 31 3 £	3 0710 11 18 24 13	3 /450 11 3 = 47 11 13	4 0 rec ab ab a y J 3 a 7 4 2	4 /4/0 40 33 30 31 31 40	: 11 1 1 1 1 1 1 1
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Veather			Close	Snew	Snew	Snew	GIOUT	Clear	Spow	Mear	Cleur		Claur			cleur	claur	Close	Closes			char	Such	-	Colored	+ +		110	-
ind (Beau	fort Scale		NES	NET							NAL					None	WI	13	1000			Nes	and .	Ned		+	Nul	21/24	3 110

Thermo-	Depth in	Type of				MAR	CH IS	947																					
ouple No		material	26	27	27	28	28	25	29	31	31	1	1	2	2	3	3	4	4	4	4	4	4	4	4	5	5	5	5
	4 .	Bit ware	49	12	53	2 5		0/4	1013	0 /25	1420	0813	1380			08 13	1343	05,0	0101	0100	1110	1310	25.00	1710	1900	0510	312	21/3	1110
2	00-	1	4 4			43	66	31	37	23	6/	37	61	41	63	30	45	15	3-	47	96	20	67	61	51	31	13	43	54
3		and & Graves		26	45	47	54	30	45	34	57	33	54	17	51	الداء	57	10	21	4.	2,4	50	6	200	34	Ju	30	40	10
3		Silty Gloy	40	37	36	36	36	31	36	37	31	28	40	4 -	43	43 /	41	41	4.	e3 [4"	41	1	12	45	41	46	41	41
4		- 41	41	40	39	39	37	39	34	40	39	4.0	. 2	+ 4	-41	43	42	43	4-	4-	al les	+1	1	4-	41	.8	4)	47	4 5
5	23_	11	AL	41	41	41	41	41		4	41	+,	+1	4,	42	44	+4	43	4-	4.	49.5	40		45	43	1.			4.1
6	3 2		43	43	A3	43	43	43	-ind	.53	42	43	41	40	43	3.3	45	1.13	45	.00	+	4		-1.7	1	~			4.1
7	43		44	45	45	45	45	45	.35	45	44	43	+4	163	13	43	45	-40	4.	40	15	of 0	1	1	-			,	el 1
8	5.8	10	47	47	47	47	47	47	47	4 !		47	+1	1 7	· 0 j	47	11	14/	12	4/			1 37	100	AP	-			-
9	7 2	1.5	49	49	49	44	41	49	49	23.1	1 .1	47	100	4.1	re 1	-47	4.	1 3.	42	41	41		1	1			-	-	0 /
r Temper	ature		19	17	3.	20	38	25	35	-	12	37	5.5		.1	27	11	21	33	1 50	43	1	-	47	47	31	11	.0	97
urface Te	mperature		33	20	35	25	4+	30	52	3.	53	41	33	45	57	34		-43 	-		4 3		-		35	26	21	-17	00
eather			610001	Claur	Ciago	Claur	-		April 19 Company	-10.	-	1.00				1	43	-	33	& Gra	4/	B W	-		1 1	-	1	11	
ind (Bea	ufort Scale	15	3 45	WL	W4	NAWE	merchants are a	0 10					-10-1	13341	100		. / 248	+	- 11	11/241	1001			1 .	10 10	4 .	190	2,0	3' 4
			33			1144.8	1466	K219	3 7 6	14 14	1446	wws.	m5	2101	2 41	NWS	14.	Noc	1/29:	A .	415	A .	1 5 5	A	1425	4421	61	-4 9-	# 8

hermo-	Depth in	Type of													APRIL	1947												
uple No	feet	material	0705	16	17	17	18	18	19	19	21	21	21	21	21	22	22	23	23	24	24	25	25	26	26	26	28	28
_ 1	06	Bit Conc.	30	11	27	63	25	45	40	SI	-72	26	45	CC	6.	C'4	65	40	-	7.	-	10	-64	-	2015	1.50	215	offer.
2		Sand & Grayel	30	11.	25	10	M	61	31	11	7-	21	90	22	-5	42	22	47	rr	4	170	.86	+ al	32	1	22	- de :	-00-
3	08	Sitty Clay	to.	10	12	12	12	15	112	11	11	12	47	16		11	di.	4,	d'a	4	12	2.	42	-2	44	-34		- 2
4	15	- 11	47	te	15	H	15	tt.	1.	4)	18	14	45	29	H	15	10	27	Po.	4.	42	12	1	42	-81	112	22 -	Ze
5	2.3	- 11	to-	.14	1-	ta .	15	15	ti	15	11	10	11	2	45	05	1	4.	1-	1	01	fo	10	22		772	20	
6	3.2	#1	14	.1.	7.	1-	to	10-	16	1.	46	7.	4.	fi.	. fo	t.		4.	10		4.	At	Ta	92	12	16	N.	07
7	4.5	11	12	1.	12	47	.#2	12	1.22	47	47	47	42	27	47	07	47		10	-	49	0.7	1	47		fo	2.	100
9	51	#1	.17	12	12	2	12	11	1.12_	11	47	97	ti	11	41	11	10	45	45	45	40	1.42	100	de		12	12	
3	7 2.	- 11	41	11	17	19	49	12	12	40	49	42	24	00	4	12	40	22	49	99	41	40	1 44	43	70	22	. 86	-
Temper			31	20	1 20	45	24	2.	31	12	26	250	.7	7	#11	32	52	1	1	4.	50	40		71		-		
	mperature		00	10	12	57	20	17	12	. "	26		30	43		40	92		-	150		44	100	-	-22	2	100-1	111
ther			v. car	Can.	Sex	Cen	* 4.20		-	-		ρ	-		24	74	- d	2	-		-	-24	1-7		20 -	. A S		24
Beau	utort Scale)		200	44	142	12	-14	142	461		1		alani	- bout	1/2	Jan.			and and		di adam	-4-	ر المعادد الماد الم	146	and last	mand!	- 64



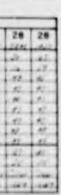
ND TIME

															J	ANUAF	RY 19	47											
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1 25	5.5	1 11	1.0	70	44	146	24	16	27	19	30	20	8	19	1	15	7	13	5	24	-	3.0	37	14	11				-
121	1 35	14	2)	21	24	47	25	19	28	30	30	ar	18	30	6	11	10	14	10	24	1.1	16	3.0	11	10	1		2.	4.7
1 31	11	24	27	28	28	10	30	25	29	22	24	30	47	26	34	44	2.1	a.J	24	2.4	44		27	3.4		3.0	31	31	25
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54	54	54	54	CL	53	-2	53	6.1	0.	-	-	-	46	46	45	46	-15	43	44	44	44	44	AL	41	43	47	43	4.1	43.3
15	54	51	51	C.	62	57	57	3 .	36	37	51	51	51	51	51	57	31	51	50	50	41	49	41	49	49	23 9	4F	41	45
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13	-	-		60	+0	60	6	3 7	59	59	31	57	59	51	.57	51	J	3%	51	51	57	57	52	57	57	37	57	57	67
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dear	Clear	Loods	cloudy	SARW	Sin	Snow	5000	1001	21001	clear	cloudy	11001	1001	Glear	Clear	Less	chards	200~	Clear	Ciede	. 1001	Goods	#town	Acres	class	da. d	2100		4.5
N3	1 NW3	541	NEI	NEI	NEI	NEI	NEL	NI	W.I	SW2	swi	A .7	N3	N64	AWE	NWJ	AL	NE	NWI	APL	Me.		-	4000			10. 1		1001
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44	40	41	100	24	137	45	3	31	32	44	31	41	39	111	24	13	19	43	137	/a	0.0	20	JA		4.2	11	15		
1 46	33	35	16	27	23	27	31	34	Je	25	31	21	33	1.4	Ja	+3"	21	31	11	31	41	111	34	20	11	100	11		47
24	10		11	3.	11	3.	31	21	31	30	30	29	30	4.1	49	27	-dF	48	39	11	11	37	11	1.7	34	37	13		10
33	33	داد	33	31	11	11	2)	37	33	الر	11	33	31	11	11	31	27	21	12	1.1	111	111	12	35	32	34	10		47
37	31	37	38	11	117	1.7	37	11	37	37	31	37	17	17	37	37	31	17	37	11	1.1	1.7	2	37	1.0		10	100	1
1 42	10	aL	46	1-	42	100	42	42	44	4 2	AL	46	10	41	42	41	41	46	41	ai	41	all		41	41	36	4.	11	7
47	47	4.	46	46	يا قد	44	46	4-	di	44	AL	46	48	46	4.	4.5	4.	de	45	44	45	45	4.5	44	45	44	41	4.5	200
1 21	5.	51	2.	5	50	50	30	50	59	50	33	50	3"1	11		50	51	50	41	19	10	40	41	41	41	41	-65	4	
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-d	Ac	4,	Ju	17	27	43	12	33	36	45	29	4.0	29	7	40	12	31	21	36	11	44	4.5	34	33			20		4
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A0- C	b ◆ c	-43	2.4.5		202	20	16	None	4046	301	12.013		444	Nel			A-4	A	N.	Alen L	1	nes	404	400	400	1500	N=3	201	2 4 1
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0740	-	15.62.7	1-635	6170	1400	19 Fat 1	100,3	014	440	1211.5	1415	0715	1355	0825	1422	13415	11 11	0130	1511	10113	1020	0233	1,544	A 140	dest	0000	124.	0.048	27.2.3
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10	10	2-	Jr	30	47	36	34	11	57	17	42	16	49	32	31	34	41	0.7	54	31			3.1	14					
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17	17	= 7	37	37	37	132		17	37	30	35	- 7	7	40	19	40	19	40	40	40	41	40	04	40	40		-	-	
40	40	40	41	43	40	4+	40	40	140	184	40	141	141	41	3,	di	41	41	44	41	41	41	4.5			1			11.1
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47	37	47	45	47	47	47	47	47	47	47	4.7	47	47	41	41	42	12	47	47	4.1	4.2	47	4.1	2.0			AJ.	1	361
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30	40	14	150	4.2	41	35	50	30	57	12	46	37	1			15	20	45	472	-				34		40.7	3 - 1	4	13
3000	. 110 -	1000	-	100	1000	L 1001		- and		A 417	Rain	1001	1100	-	2	-	40	47	-	200	2.0	31	44	-		2.	J. din.	44	31
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	أشنبة	~		-	-	W24 6			9		23		-	-			20	W41	7-2	Atra		PL	4.03	300	441	-61	40 105	0.0	/ med
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111	35		. K.A.	44	. 4.4	24.	11/15	4/	4.5	4.	78	41.	25	31	51	85	6.1	7.6	78	41	-04	39	42	FL	47	74	74	45	25
J.C.	40	15	20	A.I	3.74	34	45	SZF	4.3	32	4.7	44	42	32	80	65	15	6.5		41	46	40	41	44	200	48	20	44	120
45	41	4,	40		44	-41	-44	and.	47	41	A.I.	46	41	100	50	30	44	.50	21	45	45	41	43	41	44	46	41	44	100
4.	44	41	.4:	44	14-1	44	4.1	4.4	42	44	4.1	4.5	Al.	4.5	47	47	41	41	40	44	44	41	46	46	44	40	40	46	201
100	4	47	4.7	40	1.40	41	40	44	44	44	40		44	41	45	45	45	45	45	44	44	44	45	44	44	44	44	44	201
0.11	4.	45	16.2	14	-19	44	44	3.0	4-	41	44	44	45	45	45	46	45	40.5	41	44	44	41	18.	81	de	44	-	44	34
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(K)	A.	47	4	40	41.	100	4.7	4/	47	47	47	47	47	41	47	4.7	42	47	40	41	47	41	40	42	48	41		41	41
72.	41	97	47	4.1"	-7	14.7	42.	- + 1	12	4.	a.F.		41	41	41	48	45	41	45		AT	49	41	42	45	41	41	aF.	4.5
131	4.0	ar.	100	140	42	24	45	747	34	34	12.		H.	56	318	55	58	**	4.7	2.8	22	33	39		20	15	15	79	40
1.	34.	47	357.0	4.2	53	200	12	400	44	4 10	17	44	48	15	15	KP.	45	28	34	33	39	18	41	AL.	47		100	4.5	45
100	Links.	26.00	Se. 44	15.14	dier	comp.	CAR	49	Air.	Sw.	-	-ine	Charg	Spet	Same	Fair	-	124	Citor	d body	Ac-	diam	1000	Liber	-	C Barri	-	- Fee	d lane
76		dis	ist.	43	*	day	15.74	150	600	17.00	100	int	491	let	Zex	les.	3.51	400		115	15.0	dien.	ATL.	Am)	-	100	And	ANT	ar b



NOTES

Air temperature obtained from thermograph record.

Surface temperature obtained from mercury
thermometer laid on the surface of the payement.

PROST INVESTIGATION DOW FIELD, BANGOR, MAINE

TEMPERATURE REGORD
OF
TEST ELEMENT L



Thermo-	Depth in	Type of	_	02 Y	-	1544	_	-	_	-	VIII.											70.00		194	DAT	E	AND	TIM	E
covere No.	1001	materius	XII CH.74	16	20	55	31 part	1	15	19	21	23	25	27	29	3	1	10	1E	14	16	12	76	27	27	26	30	30	81
2	04	Sest filmer		0	걁	2	17.	41	**	*	4	*	40	45	41	先	岩	11.	4	14	45	17	27	11	20	24	2	24	1/2
•	72	palita.	27.	13	100	EF.	177	51	11	4/	1	15	44	100	<u>A.</u>	17	27	3	4	4	10	18	N.	30	M	ia.	34	A	18
	10		14.	100	17	47	41	41	15	2	-13	13	10	10	42.	10.	16	E.	130	133	18	12	0	93	*	*	10	16	12
1	45		12	183	10		130	36	贫	46	11	14	鏸	120	33	33.	47	2	3	12	100	12	0	11	II.	0	2	14	10
	11		4	56	17	10	100	10	17	15	10	10	N.	**	25	14	13	47	*	*	5	45	4	40	14	12.	-	10	10
Air Temper Surface Te			4	*	33	替	150	4	41	#	4	100	10	43.	-53-	35	0	25	15	4	-35	1E	44	3	10	130	12.	100	15
Feether Find (Bets)			Car	ious	in.	-	100	A	-	de	29	en.	Chr	in fear	San	Clear	1	1.7%	i de	ion.	100	A.	4	-	in.	-	inter-	100	13
	W. 1144		-	-	-	-	14.1-1		37	AL.	100	1000	du	, M.	-	Ma	AMI.	AR.	140	are:	100	4:	A	Sei	**	.42.7	dis	Jones.	Ju
Thermo-	Depth in	Type of	F	1 10	1 24	_	-	_	_	_	_			47	_	_	_	_	_	_	_	-		_	_	_	_	_	
TOTAL PRO	Teet	Acces	-	1912	10	-	20	OLE .	100	25	23	634	25	27	27	29	20	29	30	30	At .	100	4	1	N.	1	-		5
1	22	Josephus	13	10	5	10	A	4	4	2	2		0	4	4	4	22.	4.	42	2	5	× 4	42.	4.	×Y.	· a	4	46	44.
	27	Sing child	3	5	10	2	34		10	34	5	4	48	4.	A	4.	4	4	4	A.	4	1.5	20	4.1	dt,	4	at.	2	4
- 1	2.1	- 1	*			*	100		4	4	31	4	Al.	2.	4	4	4.	a.	42	11.	3	10	18.	4	4	0	12	11	清
1	41	-	6.	R.		0	12	a.	a.	W.	ñ.	2	A	6	-	-	2	11	6	2	4	M.	19	M.	H.	84	*	K.	10
-	-11	2	100	12	a	in.	0.	.07	0	10.	a.	.0.	3	44.	4.	0	45	0.	R.	44	a.	4	u.	4	4	44	á.	4.	12
to Temper Serious Te			×	150	4	5	12		3.	7-	6	4	at.	4.	24	16.	4	AF).	14.	16.	14	2.	10.	41	it.	.2	14	AL.	11
Since Back	day's Reals	1	-	-	Sec.	Or.	in	ive.	í.u	au.	in.	-	ia.	5	in.	20	11	-	-	in.	*	Sand	# C.	5	50	415	4	-	4
	OF L. STOR		-	16.	100	-ing	**	APE.	and.	-tar	42.	- April	Mr.	en.	M.	ARTS.	Atte		aire.	-	-	No. 1	- 21	Sec.	146	nie.	dire.	100	-
Theres-	Degitio 14	Table of		-	-			TIM		1947		_	-				_	_	_	_			_			_			
- Ingle for	1841	Ad com	-	-	22	122	183	2.	25	25	24	27	27	28	20	- Admi	-	-300	*						2				10
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-	24	Sand Street	83	18	138	1.45	18	5	*	쏲	18	20	8	41	20	A.	32	df.	38	Œ	.00	133	意	楚	30.	38	2	8	AX
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-	24	-0	*	2	41	7	10	41	15	AL.	20	41	**	-	45	7	**	46	45	45	30	138	30	36.	41	36	35.	30	46
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PROST INVESTIGATION DOW FIELD, HANGON, MAINE

TEMPERATURE REGORD OF TEST ELEMENT M

NEW COVER AND DEVISION BOSTON, MARK ALLS HAT

hermo-	Degth In	Type of		OCT	ACA.	1946	10.71			NO	WEMB	ER IS	46					_	_	_	_	DE AR	-	DA		_	_
couple No.	feet	material	H	16	20	25	31	7	15	19	21	23	25	27	29	3	7	10	12	14	16	24	26	27	27	28	30
1	01	B. R. Gara	-	10	4/	17	61	50	45	37	41	40	47	AJ	41	41	14	43	30	1050	0945	0920	1945	0140	1505	1005	093
2	04	Same Sand		65	28	54	51	4	45	AF	34	41	43	44	44	24	35	41	10	-14	22	14	37	11	21	14	20
	4.0	Story (day	SF	ST	Sec	85		50	50	45	49	50	44	51	AT	37	39	44	41	-41	12	551	NE.	11	11	14	13/
•	15	46.77		58	58	50	44	12	51	58	54	.50	50	54	54	45	44	45	45	45	42	150	40	19	32	32	34
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1			60	40	4.1	*4	4.7	74	41	67	AP.	64		65	65	4.3	40	-	117	40	59	Sitt	53	13	5	.03	1 6
	68	A	*0	AC.	4.1	44	**	47	10	78	67	65	47	47	47	65	44	94	44	4.2	44	17.1	44		10	58	10
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-	101 HUMA	_	-	ARX.	-	-	man h	Ares	*/	N.A.	El	NW.	52	ART		ar b	5 87	MEL	See :	AVI	NW	100	.70	400	10 2	Nel	1

harma-	Depth in	Tupe of										ANDAR	F 19	7								_		_		-	$\overline{}$
couple No	feet	material	10	50	20	21	21	22	22	23	53	25	25	27	27	28	20	29	30	30	31	31	1	1	3	3	
4	00	Bet Care	100	34	3.0	14	1.32	114	100	7	4.5	11	75	24	3.8	20	The same	127	200	30	2140	1000	21201	1944	DF40	1300	CPAN
- 2	0.8	Canada Commit	M	4	31	200	4317	40	4-	110	44	42	60	637	1	100	1	100	1	35	100	4 50	100	100	20	100	26
3	08	5-7-17	100	100	AT	1950	128	100	100	44	42	40	AF.	100	11	100	100	150	1 11	1 11	1	400	400	100	4.9	1.00	100
•	18		128	M	1.03	34.7	129	20	100	10	114	29	100	201	124	114	100	100	100	1.42		1	15		33	130	4 33
2	A. A. Birm	-	12	1.2	4.32	137.7	128	42	38	38	12.0	2.6	100	1	-	1.0	100	100	24	Jan.	1	1	22	12	41	10	1777
-		-	. 112	PL.	44	44	41	9.5	44	41	481	-87	-91	-01	47	100	AL	E at	4.41	41	4.	100	41	41	41	41	1 10
1	4.5	4	4.00	- 45	38.	94	41	49	42	45	Ar	4.7	47	100	47	4.	100	41	444	25	4.	100	127	42	42	42	140
-	4-64-	4	10.	112	100	22	23	40	44	F4.	4.0	2.5	54		-1				150	100	102	A series	21		40	80	422
-	7.4		100	42	1000	100	10.00	1000	de	-00 k	3.8	100	100	100			(XX)		(Car	1.00	100	EAS	3.8	122	54	54	34
1	State		100	4.12	4.85	135	20				12	29	100	35.3		200	(F)	27	- 48	2617	12	35	Jan.	122	15	JA	21
77858 T	ALC: UNK		A Print	A STORY	Adding!	150	4300	400		10	4.55	44	40	100	30	18	41	10	200			-1	44	49	13	29	26
and Television	CHICAGO WIND		Section 1	-	(Sec.)	1000	(All the last)	distance.	A Street	place.	See!	i day	Acres 1	400	Acres	dist	Stray	South	See	Chay	Aug	Chel	cour	in	100	100	100
No. I WHEN	Mary Stewa	/	442	Allen .	652	1000	ALTERIA	15.83	(PRE)	No. of	300	Sime.	100	Alex	A	Abes	Aben.	Mari	Atten	int.		Sinne	Awa	1000	AWA	WEL	Nie.

Narmo-	Depth in	Type of						TO S	MAY	19-17								_	-	_				_	_		
marie No.	feet	material	21	21	22	25	24	24	25	25	26	27	27	26	20	1	1	3		•	5	5	6	6	7	7	
4.	04	All lim.		40	7		12	-	2.2	-	-	100	-	-	100	-	124	1100	- Carre	1446	165	140	-	150	216	1417	ALG
	2.2	Sew Allend	M	77	10	-	44	-	-	70	- 00	10	-	45	-3	20.	2	1-22	-	-	- 20		100	2	- 24	J.	12
3		Authorites	Jan	74	10	1	100	-	-	42.	-	14	-	-	-	-	-57	100	4		-	10.	126	1	32	42	44
	1.7	*********	100	2.0	100	12	-	-	-	-	-	+4	-	14.	-	A	A	14	-	44	-	4.	1.4	A.	14	32	22
			19-	1-22	120	14	-	AL.	24	11.	-24	AL.	-10	22	M	-22	11	-22	41	22	.32	44.	12.	-72	12	23	-12
2			-0.0			- 44	-	-			-	35	-	-	35	- N	N.	- 44	15	21	14	15		35	-44	34	34
-	-		100	- NE.				42.	4	22	10	42.		32	38	39	38		39	34	28	100	38	39	.75	39	30
-	1000	-		-		44	-		.51	M.	40	46	AC.	40	46	**		45		40	111	100	47	47	*	41	41
•		-	×-	45					#		42	12	11	RT.		*1	**	40	#7		47		100	40		.0	47
-			56	177	54	-0	E.	1		45	10	41	6	42	.0	-	Di	6	220	42	400	125	10.	12	.79	10	100
	STATE			14		42	44	21		.0	de	44		44	40	die	34	39	10	20	14	100	26	10	1	-	10
THE T	-		46.	4	4.5	Ju.	24		45	40	24	49	40	42.	22	20	40	N.	30	34	15	13		47	12	-	1 **
-			-	S.m.	-	Daniel .	Chim	Char	2	100	200	Bear	Can.	Com	Cont	Acres 1	70.00	100	-	100	2	1	100	7	100	-	12
6 Sec.	after? Scotte	1	444	464	-	200.2	Abri.	Deck.	Mar.	22	1	-	40.0	About.		45.0	-	-	200	-	mer six	1	-	-	-	-car	1300
			200		-	-	-	-				1	-		44	4000	Acres	466	anti-	4166		4 000		13.00	44.00	diam'.	440

-	Capts in	Type of moterns	14	17	27	1	-	10	29	51	31		1	1	1	3	3	•	•	•	•		•	•	•	5	5
- 1		A4.00	-	100	-	-	-	-	-	-	+	-	-	-	-	40	white.	ALAK.	4040	-	-		+	-	- Total	5520	17.741
		See The see	12	100	120	100	1.50	te:	12.	10.		- 4-	-		46	-40		45.	4.	-	-33		sec.	44	100	2	24
		All Mary Clark	120	120	+*	100	100	+e-	- 10	10.	+45-		-	14	22.		64		44	100	E	200	date.	1.0.	14	15	30
-		Secretary.		120	+		-	+		40.	+*		a.		Ni.							45		45.	24	. 0	44
		_	150	100	4-76			14.	10.	1.2.	10.	. et	AL.			AL.		AL.	M	N.			46.		44	*	-
-2-4	and the same	-		4.5%	4.8.		. A.						Ar.			W		-	-64	N.	160		- 54	-	-		
-		-		Les.	AMA.			A.								N			M	W.	44	1.5	40	. 10	162	42	41
-2-4		-	.ne.	- 25		M.	-0.			a.	m.				.00		44			er.	AV.	Line	- 01				-
		-		1.0	1.6							N.		-							-		- 0				1 -
-		1		-		44		A.	44		100		M		**		44	40		W		1.46	- 11	W	1 44	12	1
1000	EWS.		LH.	42.	.34			340	100		34		44	40	44	201	81	ZV.	31.	100	42	100	1/2:	100	100	-	-
PARCE TO	manufacture.	11.1	.42	44	34	40	44	a.	20	15.	16	41	46	45	0		44	Ar.	30	44	-	100	100	100	100	-	+-
METTINE.					Life.			150	Sec.	II.	150		100	1000		-	1	1	-		-	150	-	200	to:	100	-
nd I Bee	whert form	•	Seed.	Sec. 2	124	Asia	ALC:	Alles	limit.	Ann	100	in.		100	des	1000	2.5	46.	1	-	1	1.5	1	-	-	-	14.00
			-		100	_		-		-		400		-	PART -		100 m	games,	A POST A	400	and.	100	211	4000	APPLIES.	Aims	4.60

name des	-	Type of moteros		14	3	125			10	# *	#1 4634	#1 ****/	21	P.	1	野	22	2.5	28	24	24	15	10	26	26	26	20
1	24	Ar con	Ť.	8	8	袋	2	4	4	4	\$	5	*	3	1	£	1	1	2	41	£.		77	4	4	F1	#
1	15		6	5	10	41	45	2	#	**	1		AL.	20	4	A	整	2	8	1	5		4			#1 #1	紫
	11		ti.	61	1	2	2	21		1	41	1	1		4	2	81	1	*	41	2		盗	**	2	4	4
famous alone fo	-		è	搓	佐	拉	2	公	生	*	壶	3	盐	4	*	经	춮	2	41	北北北	经		44		12.	41.	1
-	day! Engl	1		擂	127	100	100	1			All I		1	and the	1	1000	CON	a Date	17.29	-	-	400	and the same	Sidney.	1000	450	per l

AND	TIME
WILL O	1 1141

								7								_		-												
28	30	30	31	31	2	2	3	3		-	7			-	_		ANUA	RY IS	47							_		_	_	
1005	0930	1530	2945	1125	0935	1550	0945	1630	0850	1515	0150	1620	11116	9	9	10	10	11	11	13	13	14	14	15	15	16	16	17	17	Tia
13	27	21	10	21	19	23	26	14	16	34	11	30	29	7	18	0	1323	0725	/42	0750	1518	0140	1415	0945	1545	0930	1615	0845	1505	0741
17	29	25	11	27	21	24	27	25	20	24	20	Jo	29	111	20	1 4	14	1 4	12	5	25	9	29	31	35	34	2)	39	33	35
120	3/	32	30	39	30	20	30	31	30	10	49	30	3/	27	25	24	10	10	14	10	24	13	26	29	33	31	12	31	12	27
37	36	37	26	30	35	35	34	34	34	14	.34	34	34	34	34	111	11	10	25	26	26	26	20	21	29	31	71	31	31	31
44	44	12	12	12	AI	45	40	40	41	40	39	37	31	39	19	19	10	120	31	31	31	31	30	Je	31	31	11	32	16	12
50	40	11	12	11	47	47	44	46	44	46	45	45	45	45	45	44	44	44	44	37	17	34	36	36	36	36	36	34	38	36
20	55	55	15	55	54	54	54	54	51	51	52	52	52	52	52	51	51	67	0	43	43	4)	AJ	42	41	41	42	44	42	42
37	59	59	50	50	38	5)	58	57	51	57	56	56	58	56	56	SI	60	er	31	30	30	50	50	49	41	49	41	41	49	49
100	6/	61	41	6/	•1	61	60	60	60	60	59	59	59	59	59	59	10	0	174	100	51	50	54	50	54	50	54	54	53	53
-	22		7	12		11	22	18	9	24	24	37	24	-1	11	-8	6	-5	7	1	11	31	51	37	57	58	51	57	57	57
-	20	17	10	12	16	9.0	2.1	21	12	2/	N	29	27	1	8	-3	5	0	4		20	-1	20	34	36	32	15	25	32	30
distance.	Liter.	Sie	erang.	Care	3000	SHOW	(1000	Jave.	Clear	clear	clear	Goods	Clear	Clear	CA	cieer	clear	clouds	Same	***	Plane.		-	33	35	34	15	25	31	20
1401	N.J	482	581	NL.	NEI	NEI	NEI	MEL	WI	w3	Swi	500	NWY	N3	Wast	NW	Aw)	N.L	N's	No.	Copper.	Clear	Chang	Nain		clear	Cloudy	clear	clear	Clear
									_	_	_	-	-	-	-		3.112	74.	74.5	NW	Na.r	NEI	NEI	Nese	NE	SEI	501	10.1	**	543

_	_				_	_						FE	BRUA	RY IS	947	_	_	_	_	_	_			_						
1500	circo	1410	ares	15,33	6	1420	oks	1415	8	8	10	10	0740	11/425	0745	12	13	13	14	1505	15	15	17	17	18	18	19	19	20	20
14	26	.13	36	J40	-1	37	23	31	14	36	25	33	21	3K	15	34	14	38	W	31	35	43	29	34	Je	45	76	35	15	1430
3.0	30		31	31	a) i	JL	34	32	12	34	31	31	30	31	20	10	29	27	29	29	31	34	3/	31	3/	35	34	37	19	34
11	11		116	36	37	32	36	34	33	37	34	33	3.5	33	35	33	1.	33	,12	32	23	11	33	33	33	3/	32	34	33	30
4,	41	41	41	40	4	4:	41	40	40	40	41	40	40	40	40	40	40	40	40	40	40	40	40	36	36	36	35	37	36	36
50	10	50	3.6	179	52	a h	57	50	49	47	45	47	45	45	45	45	45	45	45	45	45	45	45	45	45	45	44	45	45	45
34	3.0	44	54	54	54	154	42.	53	53	53	51	53	,st	51	51	53	52	53	53	S	52	52	54	5 L	SL	50	51	54	51	52
29	28	42	45	35	14	25	20	Ja	35	33	24	27	10	31	10	38	1)	30	22	36	35	42	15	33	27	37	20	19	6	23
150	- Var	Joyr.	Run	Clear	don	Sher	Serve	Men	Rais	Rain	Clear	A	Utar	Clear	Cira	clear	Cleur	clear	Cloud	Clear	Clear	den	Cloudy	Charle	29 Chu4	Class	Clear	clear	clear	Clou
-2.5	25.75		243	24.		***3	NE.	2.5	Abra	None	5W2	NWO		Ned	Nº.	Net	Mad	Nw4	NM	ANT	More	SWI	NEL	Ne4	MWL	NWL	NW)	Nes	NWZ	N#3

_									MAR	CH IS	947					_	_	_	_	_	_	_	_	_	_	_	_	_		_
7	AN.CO		10	10	11	11	12	12	13	13	14	14	15	15	17	17	18	18	19	19	20	20	21	21	22	22	24	24	25	26
10	11	-	71		34		14		200	10.10	0720	100	0720	1410	0130	1930	0820	1515	2775	1520	0720	140	17.204	1250	0.040	1415	0900	15.14	0840	1125
42	34	34	22	12	1	45	12	-	-	55	27	21	36	-57	31	35	22	55	22	60	29	65	11	61	.26	61	#7	57	42	45
20	32	34	72	32	4	12	24	*		36	21	20	-	77	16	-7	N.	*	21	52	20	52	22	51	22	51	12	54	#2	90
22	. 17	21	34	24	-	14	34	**	30	10	-/-	22	120	30	32	37	22	JZ	36	22	27	27	59	30	20	39	12	42	#3	40
34	Tri.	2	24	30	22	-	-	35	-	34	1	-17	31	28	36	27	11	22	20	37	21	31	39	29	10	20	#	4	42	*
10	-	30	34	41	30	70	10	72	32	20	31	37	N.	N	34	11	Jf	29	21	77	29	39	27	40	#	39	41	4/	*	12
21	44	4	-	-	0	42	- 27	-	er.	41		27	-	40	10	40	te	11	4	11	11	40	10	#	#	40	*	42	41	42
-	42	40	4	-	42		-	-	#	12	11	41	42	43	42	47	**	12	**	47	**	11	12	44	#	4)	##	#	*	4
-		100	-	0	-2	-			91	-	76	14	16	-	16	15	46	46	fi.	16	46	16	*	46	16	16	*	47	46	*
*	216	24	**	40	30	14	22		10	77	11	11	47	49	49	**	49	-	11	49	47	49	19	19	11	#	#	12	+1	49
41	-	-	14	34		4.5	21	-	30	10	27	++	31	17	27	26	4	29	26	42	22	47	10	17	32	#7	45	72	41	20
-	-	the o			F	7.6	et.	200	21	20		12	-	14	25	47	34	21	27	.50	30	56	32	49	35	48	22	54	#2	25
-	200m	and the	all.	-	-	Linar.	- chief.	in this re-	m diale.	war.	dia.	Nina.	-100	Line	Citate	Carp	Clar.	Clear	Citar	Ciear.	Clear.	Clear.	Citar	Cient	Ciar	Clear	Clead	Chief.	Rau	Charle
HPRIX.	12.4	166	-NL	ace	al.	466	M216	54	281	484	2.4	24	WZ.	42	-		112	NZ	As1	HWZ	Non c	52	AZ	NAT.	AML	Aires	21	Mas	MA	Dere

							API	RIL I	147																					
5	5	5	5	5		7			,	9	10	10	- 11	11	12	12	12	12	12	12	14	14	15	15	15	15	15	15	18.	15
STALL	ALL C	2634	20	1325	all	195	27.75	-44	2830	12/2	CELT	1800	0225	1255	2522	1222	200	115	1320	1524	arm	1920	0530	2224	2024	ME	1320	1515	17/0	1910
2	29	10	31	52		62	15	20	#	#	12	-2k	.11		.62	12	51_	2	72	24	11	11	12	12	52	62	177	24	62	S
36	W.		12	13	77	- £2"	36	1.	2	-	12	10	11	62_	E	£2	£2	12	62	45	*	#	10	4	#	A	60	64	67	51
22	12	-	- Il-	-14	. Ida.	.22	42	#1	.11	-12	4	42	45	11	11	.15	11	11_	11	11	45	1	-11	12	17	49	-	*	42	28
01	12	67	12	12	47	.12	12	12	11	177		11	44	11_		16	11	4	1	£.	fi.	15.	15	N.	11_	41	11_	11	11	15.
**	12	12	72	74	74	42	43	77	47	73	1/2	27	4.3	12	11	11.	11	41	15	11.	16	20	1	16	15	45	15_	15	15	24
12	72	72	-34	122		4 d	26	-16		12/		75	.77	11	22	#f	22	11	11_	-##	45	46	15	.25	15	1	-21_	15	11	15
*	- 11	77	77	100		7d	- 24	4	4		-				11	-M-	-M	11	25	N.	.H	16	16	1	10	1	14	26	14	16
40	-	-	2	70	44	44	1	Te	44	-	W	-	40	.76	-							- 12	#2	11	12	#-		46	96	11
20	1		41	-	44	42	30	dic	21	2	39	C	40	74	- 17	64	CC	1/		-	300	-	-	32	-	-	11	11	11	14
1	10	-	12	0.7	- 17	52	9 -	32	42		11	3/	A	- 2	-Church	de	Cl	46	- 88	20	20	24	16	-42	2/	44	H	17	39	27
	S.A.			Cara					Can	240	1	- de			Ou.	All a	7	Chia	Cara	Ober	Class	200	Charle	Class	Cia	Clare	Char	62	-	77
A 20 a	11	144	10.1	33	40	-	1000	300	504		1944	Se 4	500	Sal.	Cald	Ja 2	Pie 2	far.	See 2		85.2	200	Abor	A.M.		A links		4	CARL.	Jida.
A 29 5	11	144	303	33	473	and an	10 mg	301	804	See d	a wid	Sold	Sed	Sac.	Sal.	Jug.	Sec.	Corne.	Sul	Clav.	SEA.	Sea.	Abac.	Class.	Claus.	Class.		Can Am	뇌	Cime 3 Aug

24	20	20
91	18	0.6
24		17
47	att	407
di	47	d,
do.	35-	47
4	4	41
00		01
1	1	05
0	41	01
		40
		2.0
-	- 0	graph.
. 0		Sec.

Air temperature obtained from the magraph record.

Surface temperature obtained from mercury
thermometer laid on the surface of the perement.

FROST INVESTIGATION
DOW FIELD, BANGOR, MAINE

TEMPERATURE RECORD

OF

TEST ELEMENT N

																									ATE	E AI	ND 1	TIME	
Thermo- couple No.	Depth in	Type of material	11	OCT I6	20		31	7	15	19	21	23	25	27	29			Lin	T 10					194				_	_
			1915	1345	1100	1150	0860	1135	0900	100	093	1055	094	1010	/330	1100	lies	10	12	092	16	24	26	27	27	28	30	30	31
2	0.2	Sand & Grand		57	57	54	56	55	#	10	42	11	41	50	15	25	25	31	11	21	27	22	27	1	24	16	111	24	12
3	0.8	Silty Clay	60	56	47	51	57	45	11	10	44	12	40	16	#	33	32	37	27	32	27	31	25	12	26	20	30	20	10
5	23		56	58	54	55	61	55	sr	57	53	57	11	11	10	45	12	#	++	16	10	*	22	10	29	30	27	37	24
6	3.4		59	53	57	59	69	65	5	62	58	17	6	60	0	53	55	50	49	50	11	16	16	14	15	ti	+1	47	41
7	5.8		60	57	60	61	61	66	67	66	66	64	69	63	62	62	13	107	0	59	59	100	IT	50	50	10	11	14	11
9	7.2		62	60	60	61	64	60	67	61	67	66	65	65	64	64	64	62	61	62	62	60	19	17	59	57	10	a	57
Air Temper			70	61	52	61	57	51	47	15	37	30	49	11	10	23	12	35	32	25	41	20	14	4	10	14	60	40	60
Surface Tel Weather	mperature	-	75	68	38	67	59	59	15	15	38	30	17	14	10	24	40	20	to	20	14	15	27	2	10	6	20	a	4
Wind (Beau	fort Scale		Clear	Sie	Clear	Clear	W Said	Awz	NJ	N2	Ei	Cient	Cons	Ciear	Gear	Clear	Chor	Ch-67	Clear	Cina	Clear.	Ciava	Same	Char	Cien	Sasur	Clea	Cen	Car
										-		-		-	1	-		1		1.142	1462	1 4/	144	1.782	14.2	NE Z	NJ	AM2.	200
Thermo- couple No	Depth in	Type of material	18	20	20	21	21	22	22	23	23	ANUA 25	25	27	27	28	29	29	30	30	31	31	-	11	3	3	4	4	5
- 1	0.2	B. J. Conc	34	11	32	34	33	orac	15:45	0745	21	26	142-	34	1450	32	1425	0145	47/0	1405	orsi	1445	0100	1430	at 35	1505	0805	1415	0130
2	03	Sand Game)0	21	21	32	12	24	45	13	22	146	28	31	31	34	35 JL	29	27	Je	31	32	3/	37	18	35	27	19	39
4	1.5	Silty Clay	32	15	32	3/	31	30	ar	12	24	2.6	2.1	31	31	31	31	31	30	30	11	34	21	31	24	25	29	30	37
5	23		11	11	11	37	37	37	35	37	37	34	31	34	32	31	12	32	37	15	3.7	12	17	33	33	33	27	31	33
6	3.2	N.	43	AL	42	+1	42	12	4L	42	42	4:	41	41	41	41	41	41	41	41	41	41	41	37	41	41	41	41	37
8	58		53	5)	53	51	53	41	4¥	47	41	47	47	41	47	47	44	46	46		46	46	+6	46	46	46	46	46	46
9	7.2		56	36	56	56	56	56	36	30	52	55	55	33	51	+5	54	51	54	54	51	51	51	54	51	50	52	50	50
Air Temper Surface Te	ature		30	13	40	40	33	6	3	-1	12	31	37	1.5	16	31	14	27	26	24	14	10	26	27	15	34	29	53	47
Weather	mperorure.		Chudy	chees	Naut	Clear	Close,	Clear	Clear	clear	clear	26	40	24	24	11	31	21	26	25	14	21	-4	2/	15	47	27	41	45
Wind (Beau	fort Scale)	1464	164	EI	542	w3	NWZ	NWS	wi	wi	None	SEI	None	NE	Mars	Alens	Ginds.	None	nel	Ren	Man s	Clear	NWS	NWL	1000	None	CHAI	Rec
																						-		1"	M.S.S.		7712		-
Thermo- couple No.	Depth in feet	Type of material	21	21	22	22	24	FEBR	25	25	26	27	27	28	28	1		3	4	4	5	5	6	6	1 7	7			10
1	02	Bit. Com	19	1425	25	1345	0700	40	27	1355	1443	30	475	*7	1515	0950	1425	1400	0820	1515	0845	1520	0420	1410	0840	1450	0400	1420	0900
2	93	Sanda Grand	23	زد	26	29		31	26	29	15	-7	12	27	37	25	32	37	3/	37	11	37	33	14	35	49	32	35	32
-3-	00	Sitty Clay	37	33	27	29		30	29	24	31	31	.12	31	31	30	30	3/	31	31	32	1	12	31	12	36	32	32	32
5	2.3	10	17	17	33	33		37	31	33	31	33	34	33	71	33	33	31	33	31	34	37	33	33	33	33	33	33	33
6	3.2	89	41	40	40	41		40	41	40	.10	39	AC	1	39	40	39	39	39	39	40	J1	3.	36	36	37	36	36	35
8	518	•	41	44	44	44		48	44	44	44	43	44	14	43	44	44	43	43	43	43	43	43	43	43	43	43	43	41
ğ	7. 1	44	51	51	51	51		51	51	51	31	50	33	31	47 50	50	51	50	50	50	50	47	47	47	47	47	47	47	47
Air Temper Surface Te	Gture		2	_11_	27	36		29	12	30	35	14		22	27	26	31	31	37	35	35	36	31	49	36	41	32	49	31
Weather			Cloudy	Snow	29_ Snow	Snow		clear	Spen	31	clour	30	1)	31	33	15	42	38	37	11	37	J,	J.	43	19	34	11	40	34
Wind (Beau	fort Scale)	NE3	NI4	Nº 3	SW3			None	WL	Not	None	10 1	1:11 None		1/276	WI	53	None	SAI~	WL	Clouds.	Cloudy	Cleads Nos	Cloudy	NW3	238 m.	NEZ	SPOW
																									L				
Thermo- couple No.	Depth in feet	Type of material	26	27	27	MAR 28	28	29	29	31	31	-	1	2	2	3	3	4	4	4	4	4	4	4	4	5	5	5	5
1	- 03	BIY Case	1500	25	51	27.24	1420	2345	1120	2260	121	3822	+00	00.22	See	28.20	150	2520	.1220	3922	125	.725	120	.720	.7.0	2522	SHE	24.5	
2	03	Sand Grant	42	22	41	21	42	.J2	A.	22	54	20-	-27_	#2	53	غلا فالس	51	21	22_	14	20	12	55	ál.	53_	22	W.	15	21
3	98	Sitty Clay	.dt	25		25	22	-24	Je	20_	10	22_	11_	1-	44	29	12	11	39	22	10	12	11	12	12	12	22	#3 #	10
5	2.3		12	40	-28 -	28	29	28	21	-10	22	.H	11	. ži	24	12	42	\$2	14	12_	12.	12 .	t	1.	ti.	12_	17	12	12_
6	3.2	14	12	12	12	12	42	12	12	12	11.	12	12	12	1	1-	32	12	12	42	12	12	12	12	12	.12_	14	R.	12
8	5.8	16	11_	71	11	#1_	11	H	11	12	12	_11	11_	-11	11_	-	11_	11	11	11	11	22	11	49	22	11	12	11	12
9	7 2	14	11	10	N	10	10	14	25	10	10	#	14	10-	10	11	30	14	1.	1	1.	./a	1	12_	fa_	ta	ta_	4	10 -
Air Temper Surface Te	Gture		21	12.	20	26_	22_	25	30	21	12	28	G.	26	12	2	12	22	32	20	12	32		22	12	47	7	12	10
Weather	per drur e		33	21	Cor	25	1	22	13-	20	12	M	55	1	52	Y	11	21	27	A	21_	40.40	44	50	22	20	10	50	52
Wind (Flee	ufort Scale	6)		42	41.	المادات المادات	intale.	Nega	South	seaude.	APIA.	A	10 2	- day	Janes.	Saule.	1	None.	diam.	15	Clier.	- Leuch	- Ledin	Sea	Marc	and det	Jake .	iler.	
													1	***	addl.d	Althou	41		Vaze.	146.2	122		181	daz	Neza	Vezs	£4	262	-E2

Thermo-	Depth in	Type of													APRIL	1947	_	_	_	_	_	_	_	_			_	_
cauple No.	feet	material	16	16	17	17	10	18	0720	19	21	21	21	21	21	22	22	23	23	24	24	25	25	26	26	26	28	28
1	24	A. F Care	20	XY.	.04	20	21	64	40	54	14	15	44	51	63	47	79	44	113	4.	61	47	4.5	In.	0/40	1148	25.00	1900
2	24	Sandfürmi	*	50	15	15.6	27	FX	40	45	29	1.5	40	44	30	40	44	47	50	1	51	44	54	4.7	17			-
3	0.0	Silly Chy	di	47	41	44	41	**	41	43	41	AL	41		4/	40	47	47	44	45	49	All	37	4.7	41	1	41	-
-	13		10	4.	45	44	45	44	45	4.5	45	45	45	44	44	.0.5	33	46	44	44	44	46	de	41	47	41	47	34
	23		. 50	45	145	45	Ar	45	45	45	45	45	45	45	45	45	45	45	45	14	45	45	100	44	4.	4.	47	41
		10	*	46	45	1.46	45	41	AL	145		45	43	45	-0	100	25	41	45	45	46	4	4/	4.0	Al	4.1		44
		- 4			46	44	44	44	44	46	.46	46	44	AL	.44	-05	45	41	46	44	44	45		44	de	145	4.	-
	- 11		-	-	47	41	42	. 47	46	46	47	47	4.	47	47	47	4/	4/	42	47	47	44	44	47	41	45.	1.	1
Air Temper	okas.	- 0	•/		47	47	47	4)	41	41	41	47.	47	47	41	47	41	47	44	41	Ar	47	41	41	47.	30	4.	19
orface To	mperature			15	37	44	1-12-	165	12	45	14	34	11	7.6	40	36	52	140	46	47	54	41	14	- 14	1.7	10	-7	10
Feather	migrar dribe a	-	-	-	44	-	112	300		100	44	12,	25	45	5	44	60	47	4.	60	19	Ar	37	.76	118	19	1	-1
	fort Scale	.1	witer.	100.01	77	78-21	100	100.12	200	rmg	16.12	former,	des	Steller	14.5	17.79	1. 11	# 14	-0.04	down	Lind,	do	1.600	Same	(8) e	der	200	100
1000	order acom			160	WE		136	1-4	4.24	800	- Nie	100.2	Re.J.	1977	190	Miller	N.C.		Sec.	iwn		in	48.6	Ari	Acc	In	4:1	400



AND TIME

									_	_	_	_		_		-														
28	30	30	31	31	2	2	13	1 2			-	_	_	_			ANUAR	TY 19	47										_	_
000	0925	1515	0905	140	0920	Was	0050	16.35	OPE	1515	pers	1625	8	9	9	10	10	11	11	13	13	14	14	15	15	16	16	17	17	18
16	4	24	12	22	20	24	26	24	11	30	10	7.	20	10	120	1000	14.10	07.30	1920	9155	1520	0845	1415	10045	USAF	09.20	1615	OME	1545	mes
20	22	24	10	24	23	25	24	29	22	30	1		100	10	-	1	12		12	1	24	11	22	10	24	2/	72	20	.1/	34
85	30	30	26	35	27	20	1 24	24	4.	4	4	30	24	15	122_	10	20	/2	14	111	25	16	25	20	21	20	2			1
	37	39	27	35	17	2	30	20	27	21	26	29	29	25	26	22	23	21	2/	22	24	22	24	3.	24	14	34	-	Ji.	-
	43	41		-	-	-	-25	135	35	15	25	15	34	25	34	34	24	21	22	24	20	70	7/	1	-	120	30	-	2/	21
-		72	41	72	42	12	12	12	4	41	#	40	*	40	40	40	40	4	4		14	24	- 41	14	4	12	20	.22	22	32
10	11	41	11	1	17	12	47	47	46	16	45	45	45	45	ar	ar	40	4.	40	1	40	-	36	20	12	20	32	2	27:	22
ar.	14	54	52	54	52	10	D	12	52	57	12	0	0	0	0	100	124	- 24	12	#	12	12	12	12	43	11	*	12	42	42
19	60	17	57	59	50	57	57	57	57	100	s	-	0	1 "	20	11	51	20	17	.00	S	50	12	12	11	49	+	*	49	4
61	60	60	60	40	60	100	LA	10	0	~	- 26	34	24	10	16	Ir	56	55	ST	55	55	14	54	54	14	54	50		.0	0
4	22	10	4	1/2		12	24	1.5	27	44	47	57	57	39	50	50	17	50	SA	57	57	57	52	17	0	CI	179	a	72	60
2	20	29	100	1.0		1.		112	7	24	24	29	24	7	11	-8	6	-5	2	0	20	4	22	24	2.	3.	300	100	- 12	47
	attended in		-	16	-	20	22	122	12	2/	21	29	27	1		-7	5	0	E	3	20	1		1	- 44	-14	J.	4	12	30
and and	Lieu	With.	- dide	(and	Sam.	Lizam	Same	Same	Char	Char	Ciner	Claud.	Clase	Clase	Com	Chan	Flore	m/4. 1		46	26		- 66		11	25	22	2	21	24
15.4	N3_	AMI.	4.50	NEL	NEI	NEI	NEL	NEZ	M	413	Swa	Sug	Aug	N.I	Marie	40.3	Albert .	- andy	distan.	Citar	Crear.	Great.	Citado	/late	Aga	Clear	County	Cione	Cipar	CINA
			_	_	_				-	-			16.00	414	ANA.	MARK.	See J	NA	NZ	NW.	NW2	NEL	NEL	Mac	NEL	241	Sei	WZ	WE	SwI

	-	-	-	-		_						FE	BRUA	RY IS	47				_	_		_	_	_	_					
	eres	1415	0150	1540	6 0/45	6	0820	1415	8 0915	8	10	10	11 074	11	12	12	13	13	14	14	15	15	17	17	18	18	19	19	20	20
15	26	37	39	39	24	33	27	31	T)	32	25	33	22	27	14	15	250	250	31	25/1	0740	1413	014	1100	0740	1400	0141	1500	opes	1420
30	21	31	33	M	31	Ju	Je	11	14	34	26	31	45	31	al		20	30	100	33	-44	34	10	34	31	44	29	35	17	37
28	29	30	41	12	12	Ju	32	34	16	14	21	31	25	20	3.5	36	-0	30	24	30	3/	36	12	12	21	36	31	33	21	31
8.16	37	33	33	33	.13	12	33	12	11	21	4.1	11	21	21	-	21	21	21	27	-29	31	21	12	31	12	12	31	107	24	29
17	21	39	37	31	137	37	12.	17	27	- 17	27	33	22	33	3.0	34	33	33	31	34	33	33	31	27	33	13	33	.14	33	3.3
41	41	41	41	4	41	41	.to	40	3,	1.7	37	31	31	37	21	31	17	21	37	16	37	37	37	37	37	37	34	37	27	37
14	36	16	46	40		4.	46	40			40	10	41	40	40	41	AU	460	40	40	40	40	40	44	40	40	37	40	40	de
50	50	50	50	12	45		50	45	-13	45	45	45	45	45	45	45	45	45	45	44	45	45	45	45	45	45	44	45	44	AA
. 4	.ca	22	30	3.4	2.9	-	50	50	26	50	41	49	49	47	47	41	49	47	44	dr	47	44	49	49	45	41	41	46	7.0	A.
14		33	- 5.5	211	25	32	33	5	3.3	33	54	54	52	51	5 L	54	56	52	54	59	52	52	52	51	67	57	61	5.	24	51
-	29	44	4/	41.4	1.	27	10	33	200	35	77	27	21	27	11	26	/3	10	27	14	J.F	12	2.5	11	23	10	3-		-/	42.5
4.00	27	41	45	30	4-	29	50	34	35	JL	25	30	20	34	11	28	13	31	25	37	15	44	22	14	14/		20	11		21
104	clear	CIME	Kee	Clour	Close	clair	Spien	\$ 77.00c	huin	Kent	Clour	Snew	clear	clear	clase	clear	- lan-	clar	- had	de	-14.0	-1	Chut	-1 4	-1	4.0	1.1	2.5	-	7.0
in	None	SWE	343	Jay L		NWJ	NE	EL	None	None	1995	NW		NWA	NW	AMA	NWA	NES	NWI	NWE	None	SWI	MEL	HF 4	Aw L	NW 2	Clear	clear	Nez	

_	_	_	_						MAR	ICH IS	947									_	_	_	_	_		_	_		_	_
150	0,000	1420	0900	10	0735	1155	12	12	0710	13	0740	1420	0720	15	0730	17	18	18	0735	1520	20	20	21	21	22	22	24	24	25	26
40	33	38	32	4/	3/	53	35	62	36	60	35	42	36	54	73	52	34	54	30	60	2/	63	3.1	60	34	57	47	53	42	44
16	32	32	32	35	36	13	35	14	37	47	35	36	36	41	33	45	32	45	32	48	22	49	11	47	34	47	41	52	+1	39
11	33	33	33	34	34	34	34	33	14	34	35	35	34	34	12	37	1)-	37	15	38	26	49	17	40	37	11	42	44	41	11
17	36	36	36	3/	36	36	37	36	37	34	36	16	17	37	39	38	39	34	19	17	20	10	37	37	40	40	41	42	41	41
7	42	39	39	40	24	39	39	38	40	21	39	39	39	19	40	31	40	40	40	40	**	40	41	41	41	40	41	44	41	AL
17	47	47	47	47	47	43	43	42	43	43	43	43	43	#3	45	42	43	42	44	43	42	4)	4)	41	4)	41	41	44	45	44
7	49	49	49	50	49	49	41	41	40	45	49	44	40	46	44	45	46	46	46	44	16	46	46	46	45	46	46	44	40	4.
1)	32	37	3.1	35	27	.19	27	41	15	51	37	44	39	47	21	16	21-	45	26	47	11	41	41	AT	49	41	ar.	41	AV	41
4	11	40	34	36	26	44	33	55	19	60	31	45	34	55	3/	47	24	41	17	50	22	57	12	4/	10	4/	41	31	41	21
eur	234-	Sam	Snow	Sun	class	de	clar	de	clear	Clout	Ben	Rose	Citar	clear	doug	-leady	clear	clear	cier	clear	Class	cie	Clear	clear	clear	class	ches	Chude	Char	Chun
W.3	Nort	NEL	WI	NEL	N'L	NEL	4000	61	201	341	51	33	ne t	WZ	-		WJ	NZ	NWS	NWZ	None	52	NJ	NWS	NWI	Ner	Bi	Aber	1001	Swe

							AP	RIL I	947						***															
5	5	5	5	5	7	7	8	8	9	9	10	10	11	11	12	12	12	12	12	12	14	14	15	15	15	15	15	15	15	15
-	2111	25.		225	200	1115	0725	1913	3830	1353	BUC	-	2225	1355	2522	3220	20		.322	1520	0/10	.420	2.520	0.220	2430	400	1720	SIC		10
12	25	15	21	SI	. 1-	44	16	st.	*	11	12	-25	#	22	52	12_	12	11	44	25	1	Ar.	39		57	-22	22	2	40	27
·	-22-	4.9	10	10	11	12-	30	55	17	10	22	, 5%	15	52	£.	SL	81	12	54.	11.2	1	45	*	12	47	47	12	-	61	11
93		£	1	22_	12_	-15	-ta-	05	4	.12	39	15	46	11	52	50	53	0	54	12	12	4	43	12	43	44	4	59	.0	52
-	1)	12	12	12	22_	12	11	13	11	12	-17	12	tt	11	. fa	to.	12	16	10_	16.	4	10	15	15	45	40	45	15	15	15
4	14	R.	12	162	12	12	12_	12	17	4)	42	12	13	11	4	11	11 _	11	11	14	15	N	05	45	15	45	65	45	or	45
×	12-	-12	12	22	12	12	11_	12	12	-12	*7	11	11.	21	44	11	11	11	11	111	45	W-	45	45"	95	45	15	45	15	15
1	-	14	12	-11_	18	- 11	11	11.	15	11	25	15	45	25	16.	05	15	18	15.	15	15	4	4.	06	1	1.		4.	4.	4.
fa	te.	4	10	.fe	-te	10	ti.	16	16.	· 16 .	10_	. de	di	15.	10	15	10	16	16	16	14	10	*	*	46	46	40	4		40
-	-	42	97	42	47	12	1	12	11	42	-1	12	12	47	11	12	42	22	22	62	42	07	47	42	47	4.5	41	49	41	#2
-		12.	25	ti -	1i.	11	12_	15	.22	À	13	A	#	£1	\$5	39	ST.	54	46	42	25	22	22	39	10	177	51	51	Sign	47
-	-	56	- dd	-	07	12.	20-	50	12_	10	11_	70	26	21	52	51	50	at	.20	25	31	30	36	42	57	60	we		50	12
Aid.		malar.				- inter	- Cane	and dealer .	-	iles .	in dut	Can	- and	may.	a Ballera	Rea	Tan -	Coas	Gas	Car.	Colan	Jea.	Cinc	Car	Sur	Car	Car	Con	Car	See.
P2 64	Adam .	252	262	00	- a 2.	4.	1842	about.	122	142	102	Jaz.	J#2.	Sa12	JAZ.	Je 2.	ins	-Ai	302		202	322	Voce	100	200	And	301	102	Sal	77

26	28	28
:1	39	
22	31	24
5	46	46
de.	,17	-3.6
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142	40.0	- W.A

NOTES

Air temperatures obtained from thermograph record.

Surface temperatures obtained from mercury
thermometer laid on the surface of the parement.

FROST INVESTIGATION
DOW FIELD, BANGOR, MAINE

TEMPERATURE RECORD

OF

TEST ELEMENT P



DATE AND TIM

Thermo-	Depth in	Type of		OCTO	DER	1946				NC	VEMO	ER I	44							-		100	allo y	15.62				_
couple No.	feet	material	1533	1145	20	55	31	7 1415	15	0703	21	23	25	27	29	3	OFFE	10	12	14	16	24	20				36	I
1	0.5	Sand & Groval	60	60	59	34	67	50	41	AS	4.	46	41	47	44	41	7	18	37	EN.	de	40	14				100	+
2	13	_ Soudust	61	50	74	29	94	78	51	60	64	37	6.0	6.6	6.5	46	+1	61	60	0.7	6.0	81		A .	4.77		4.0	t
3	7.2	"	69	74	10	F3	9.7	102	54	6.4	16	4.6	4.0	60	71	24	27	12	74	1.	2.1				40	4.5	44	an.
4	30	86	77	81	12	20	93	97	50		41	64	70	12-	-)4	12	10	17	14	-7	74	10	-	00	20	Ji-p	4.9	80
5	3.8		65	74	76	99	133	86	54	14	6.5	69	70	21	70	7.6	25	3.1	10	Fi	13				4.0	-		
6	4:7	Đi .	53	43	67	68	24	74	50	65	60	67	68	67	68	62	10		17			100			4.0	100	177	ш
7	J .3	Sandlbore	35	61	63	4.3	67	69	54	6.0	96	st.	62	66	67	6 /		2	0.3	6	the c	61		41	.5	• 1	**	
r Tempero	iture	L	66	66	35	31	47	5)	40	Ja	,53		33	da		13	10	3.5							Sel 1800Y			L
urface Ten	nperature			64	58	33	37	33	41	37	100		40	100	16	all	12	AF	3	No.	See See	1	1	-	- 170		-	10
feether	ort Scale)		Clear	Gion	644.	Marie Street	4 1511	Girer	4000	6 001	Long	100	1000	1.0	. 4	-	1000	(week)	Repr	and the last	- Chapt	-	- Liphan and	d Hear	9 100	See III	-	
Ting (Begui	OFT SCUIM			30/		-	434 F	APL	NJ	AF	61	AW	54	7-	ev. 1114	10'0	2001	WF L	100	B	10 000	Sept 19	1.50	1000	40	447	4.7	1

Thermo-	Depth in	Type of									- 1	MALLEY A	Y 15	47								_	_					_
couple No.		material	10	50	20	21	21	22	22	25	23	25	25	27	27	20	20	29	30	30	31	30	4	1	2.	1		13
	03	Sand & Gran	13.6	14.0	6.1	100	22	31	20	:F	17	44	45	15	41	27	20	14	11	41	4.	100	-	41	1	17	140	-
2	1.)	Saudist	48	49	AF	-37	47.	45	41.	45	45	44	44	40	**	44	44	44	Al	45	41	43	44	43	127	25	120	12
3	2.2		128	1.63	52	1.0	100	23	50	10	50	52.	4.5	36.	AT.	50	50	50	60	10	1.0	11	42	49	42			-
•	3.0		-		1	4.5	47	ar.	41	27	57	44	45	4.5	D	11	10	11	15	4.0	26	11	10	15	50	4.0	24	10
6	47	-	-	60	**	1	67	47	21	4.1		12	11	10	18	ST.	11	10.	.12.	43.	52.	10.	12	55	18	10	A	53
7	55	Sand Green	10	41	17	19	94	37	14	59	17	17	11	18	et	14	17	14	11	18	F2.	41	쑵	17	#	47.	47	5
9 Air Temper	ature		.7			.14	31	-	-	-																		
Surface Te				16	W	38	31	70		0		4/	25	12	12	-	12	- 67	-27	-35	-55-	45.	.55.	54	-55-1	45.0	155	40
Weather Wind (Beau)	CT-19 ht d	E.	1	182	in it	383	10	* 6		***		Dete.	Acres .	Similar Silva	all to	1	circu.	ATT.	die:	Carlo Carlo	100	AND AND AND	Carl C	48. (3%)	AL.	AT.

Thermo-	Depth in	Type of						FERM	HARY	1541				_		_		_	_	_	_	_	_		_			
coup e No.		material	21	21	22	22	24	24	25	25	26	27	27	20	20	X.	1.	3		•	.2				14.0	M		
1	05	Sand & Grand	2.1	31	u	25	4	4	24	16.	25	12	.lx	42	3	14.	4	1	4	2	Ja.	31	1.6	111	23.	16	32	
3	2.2	-Jandes!	45	45	#	N.	#	22	21	22	17	35	4.	**	H.	-2-	2	4		4	1/4	14,	12	15	22	33	45-	13
:	3.0		30	50	ef	*	H	et	#	42	**	47	10			.00	44	3			41	at.	11	10	16	96	4	100
6	4.7	0	30	51	17	11	17.	17	12	SI	15	12	12	17	10	17	18		-E	2	2	-	150	11.	45	5	4-	3
	5.5	Saltman	29	12,	19	4	12	.12	54	11.	17	12	ix.	12.	42	2.	44	14.	.0	14	23	2)	10.	25.	25	15	1-	(8)
Air Temper	ature		-	12	4	2.5	20	24		-				77														
Surface To Weather	inpersture		9	26	-		20	21	44	20	30	34	34	4	31	7	50	47	4	5	45	-0.	100	4	4	2:	35	
Wind (Beau	fort Scale	1	Arrive S	Star .	Stew.	307	Mark.		dan.	Case.	Ster.	Albert.	-00	-	Chap	-	-	-	24	-	4/37	1.00	and a	2.00	0.49	uttet	200	10

Thermo-	Depth in	Type of				MA	ACH IS	947								-		/		_		<i></i>		-	-	-	_	_
couple No		material	26	21	27	20	20	25	26	31	18 20	- 7-0	1	3		1		•		1		•					6.57	
2 3	1 2 2	Sandfurd Sandon?		7 4 4	3 a 32 4	3.	1 4	70	7 27		37		12				11		**									
5 6	30	9	1	41	40	4)	4)		12		4.		Al al		4		4		41 198	2	41	41	2) 20	45	**	-11	**	
7 8	1,5	Sallows	2.	1	20	30	,.	50	50	30	4	**				7:	31	**	are are	47	-	#3	. # 2	# 2	21	.5	j=1	
Air Temper Surface Te Weather	erature emperature	-	35		- 7	**	11	7.	29	30	da da	28	73	5	7.	推	41) 27	0.5	73	**	rik	41						100
	outers Scale	10)	190		-	(No. 4)		abro	100	900	1	10 mg	0.40	SWIT THE	20	100	25	iffer			-35	-		1	Maryla Maryla	19140	-	1

Therms	Depre in	Type of	-												HALL	194)												
oughe No.	· ·	moterial	#Tr:	141	7 (2)	100	-	100	19	10	71	24	21	201	211	3.2	##	4.5	23	-	*	48	13	28	4	64	2.4	
-	28	Law fire (.42.	17		1.3	.11	180	do		1			2.0					-			-	-				-	
-	- 10	- destat	120	11	25	130	10.0	10	100		10	20			110		1	1 4		-F	35	111	100					20
-	4.4		-25		Al	1 3	4		-	10	4				4	161	100	100		-	- 1	4				-	1	812
-	- 4.5	-	41	Al	41	AL	22	4.1	-0	A	-0.1	E A	44	84	a l	0:0	46	5 14	4.1	4	4		1	4-	7		200	1
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-	55	-	11	100	48	40	47	40	-	40	88	10	100	47	64	45	10	20	40	4	-	100	To f				40	1
		and farming	49	47	- dy	30	4.	40	46	de	Al.	**	118	-1		4.7	41	46	el	24	7.1		(A)			I	100	1
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ne Ben	whert Som	•1		100		1100		1000				600	1	-	1		-	-		jut "	-	-	100	- 1	-	- 11	1 000	
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Additional and		40000	100000	1000	22-11	*****	THE PARTY NAMED	47.681.61	がはない	47.44.74	7	171111111111111111111111111111111111111	1747.03	共大な なんない	大きませい	九 经公	新教教	421111	おいた	100 m	14 14 14 14	4	1.02.7	なないい	100 to 10	おけれれれ	12 12 13 14 14 14 14 14 14 14 14 14 14 14 14 14	2 4 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	N	200
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	104500	0.0000000000000000000000000000000000000	\$4000C	2000年	5000 C	\$14450 C	100 M	1000000	がなれる	经验	ながれた	2000 C	小乔公姓.	公公司	行送をおけた	以上は大変ない	公社会社	かななない	分析教徒	北京教育	公五 替代之	行動を行	行和起程以	经验	22年十年17日	拉拉	1111111	******	经验	2. 红色
13.41	1	1	41		11	は	14.15	#1 #1 #1	ない	A400	放射	10 mm	分置	以見を	AL Sen	4.	扩 爱	社	かった	4 4 4 5 5	弘明	经	対の機	北西	11	41.	19	47 47 (0.00)	4.0	48 31 00 mg

				L	H				1	4	10	100	11/207	9	No.	4	9	4	4	10	18	4		8			2.	(B)	
	の北京教育者	12211	ないない	1.0	**	47	***	0 0 0 0	9		# # # *	24.55.64	44.44	126292	江川を被いれた	11 4 4 4 4 4	行為就是此	24.24.9 22	日本の 大大大大	大学の大学の	100000	は、日本代数数	一 のおもれ	様なのだれれば	おけ まだり 枝枝	1000	B R 2.4 + 2.2	11 M 41 M 41 M	在此也就以此社
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En l'emperatures obtaines finns managnaph record. Eurlisse l'emperatures obtaines fion managraph thermometer laid on the surface of my empowement.

FROST INVESTIGATION

TEMPERATURE RECORD OF EMBANKMENT R

EMBANKMENT H

herme-	Depth In	Type of		0046	144	PHU				NO	VEMB	R 19	46					-				OF CEN	MER	1946	ATE		
ouple No.	feet	meterial	11	1545	1030	1015	31	1400	15	19	21	23	25	27	29	3 1455	7 0050	10	12	0030	16	24	26	27	27	28	30
	01	Seal of time of		84	70	75	75	65	51	53	53	40	41	47	44	47	40	4+	42	42	42	41	40	40	-3 5	40	14
-3	11	Jantest	-01	67	81	88	84	7.3	67	64	62	51	53	53	5.5	50	54	52	51	50	5.0	44	49	49	48	49	51
-	- 27		-	00	94	43	81	84	76	69	09	56	50	59	61	65	62	60	59	58	56	55	55	54	54	54	53
	-30	•	61	92	42	41	80	8+	79	74	7.3	65	65	64	66	66	67	64	64	63	62	60	60	60	59	59	50
3	76	0	90	71	81	81	81	75	75	7.3	71	66	66	66	68	68	67	66		65	64	63	62	62	02	62	61
-	47		57	94	-	00	04	. 4	•	6.5	64	63	64	64		65	65	63	64	63	63	62	61	61	61	61	61
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Tempere			05	57	50	50	67	53	41	34	35	35	6.3	46	33	14	19	35	26	7.5			14	- 4	(0	-4	-
wfoce Ten	merature.			05	87	35	57	54	49	30	34	34	4.4	4	35	27	32	30	30	23	23	35	15	,	10	-	23
-	_	AND DESCRIPTION OF THE PARTY OF	Close	C100	CHOP	CHA	Londy	LHOT	CHO	clear	Lloudy	Clear	Cloudy	ulco	410	clear	Cloudy	chuly	clear	Cloud	Clear	. Joud		rear		Snow	
ind (Beaut	fort Scale)			341			ع لهو س	NWA	N3	NZ	EI	NWJ	32	NUZ		WE	SWI	NEI	SWI	Nw 3	E WB	wi				NES	

Therme-	Depth in	Type of									y	ANUAR	RY 194	47														_
couple No		meterial	10	50	20	21	31	22	55	23	23	25	25	27	27	28	28	29	30	30	31	31	0.800	1430	3	3	4	4
	35	soul poor	34	1 30	10	31	31	31	31	30	14	20	4.9	31	31	31	32	34	32	32	31	1 34	13	1730	30	30	10	30
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,	24		40	40	40	43	45	45	45	45	45	44	44	44	44	43	43	43	43	43	+4	1 43	43	43	43	143	143	141
	30	-	53	SA	54	51	31	1 51	51	51	51	50	50	9.0	טינ	50	49	44	44	44	44	1 49	44	-94	48	48	40	48
3	1	•	50	10	10	50	35	35	55	35	05	54	54	54	154	04	54	54	53	53	5.3	53	53	53	53	52	53	51
	43		50	1 87	57	50	50	50	50	50	56	56	50	55	55	55	55	55	55	55	55	1 55	55	55	5.4	54	-	-
	15	Sand & Grant	50	57	37	51	37	37	37	57	50	36	56	56	56	56	56	56	5.5	55	55	55	35	55	55	55	-	-
Air Temper			2.0	13	40	43	33	0	1	-1	12	11	10	15	26	1	34	12	7.4		22	22	76		100	1	1	1
Surface To	imperature		25	15	38	34	31	10	0	17	13	17	340	32	13	11	and the same	اعدا	115	1 22	35	10	120	27	10	30	14	40
-			Landy	10 di	Rain	1000	loudy	1100	Lice	LAN	1100	and,	Rain		Rain	loudy			Chods	2.1010	200		1100	cher	Liear	-	LHO	-
Bind (Beer	wtert Scale)		V-	181	RI	SWL	wis	NUL	NUS!	WA	W3	None							Nonc						LUUL			

Thermo-	Depth in	Type of						H(E	VARY	1947		-	-															
Couple No.	feet	meterial	21	21	22	1305	24	1430	25	25	26	27	27	20	20	0915	1445	3	OAIO	4	5	1420	6	6	7	7	8	
	91	ford man	.00	10	al y	29	30	31	31	30	30	31	11	31	-11	21	10	41	4.1	41	11	23	22	21	23	()	32	1
2	13	300 deed	30	20	30	10	30	1 36	10	30	30	30	37	34	30	36	1-	45	25	3.5	1	15	2 4	35	1	26	28	3
3	22	0.	41	31	4/1	31	41	41	41	41	-04	40	41	40	41	40	40	40	4.7	40	40	40	40	40	40	40	40	4
•	30	4	de	40	de	40	40	44	40	44	40	45	40	45	45	40	45	45	45	45	45	45	45	45	45	45	45	4.
-3	30		50	30	30	30	50	30	50	30	50	44	50	44	49	30	44	44	49	49	41	44	44	44	4.8	44	44	4
	-3 7		34	34	12	54	54	52	34	31	81	51	عد	51	51	1/	51	51	51	51	51	51	31	51	50	51	51	5
	4'	San Claric	3.9	33	99	34	52	53	52	51	34	32	9.3	27	3/	54	5'00	34	52.	53.	51	51	51	51	51	51	51	5
· Yangar			7	13	الإيداد	20	24	-47	//	10	30	77.4		-1	224	da	74	41	1 1		2.4	1	44	44	200	41		_
	mperatura		7	13	San age	1000-000	-4)	30	23		35	14	32	46	31	15	30	15	30	37	33	30	35	44	17	42	14	3
-			4 4	3000	-	1000	- 1000	100	سيمر	LICAT	1800	South	and the	100	200	Her	LAST	Aw,	1000	1904	143"	LANNY	Loud	-	lowy	clear	1904	390
Me IBee	utart Scolo		40.3	214	50,5	3 41.5	Lieb.	200 ch	At-	W.L.	WW 2	spac.	W3	46		None	WI	33	45.4	134	W.L.	402	1	AWS	NW 3	NV3		

Therme-	Dogth in	Type of				MAA	ICH I	947																				
Couple No.	toot	meterial	6 (a)	MIR			18.00	07.00	1440	31	31	0020	1330	2 0415	2	0000	1330	0510	0701	3423	1/25	3.5	1520	1740	1410	5	5	3
	21 6	hand & brough	31	34	30.	31	11	34	34	11	31	11	34	32	34	34	34	34	12	34	32		32	34.	92	32	32	3
	12	Souther	30	30	So.	Je .	10	10	30	3.	35	20	30	30	30	10	30	10	30	10	10	20	30	30	20	30	30	3
	22		de.	40	27	37	4	.40	40	40	37	40	40	40	ale)	10	p. 3	rest!	40	40	40	40	40	alw	40	-AU	de	4
	20	-	44	44	40	44	44	44	44	44	-64	44	44	44	44	aliab.	-6-4	+4	44	44	44	34	44	-3-4	44	44	42	4
_ 1 _ 1	7.0	-	12	42.	41	42	+2.	11	41	.41	12	47	47	47	47	47	10.	47	47	-37	41	47	47	47	37	47	47	4
	42	-	47.	47	42	47	47	+4	10.2	-11	-87	:27	44	44	abed	44	41	44	44	24	.84	44	44	44	-34	44	-84	189
	**	inside was a l	10	Jo.	10	20	50	35		30	47	2.0	47	30	-pay	33	-7	30	30	47	30	20	ro	14)	13	10	10	
	aliur e		p19	11	202	ela)	10	A)	.4.5	12.00	44	3.4	34	1.	5.	51	41	1.1	0.0		43	18,4	110	44			4.4	
ierfote Te Feetige	-		31"	44	1	4	44	30	44.	30	4	J.F	34	47	17	14	50	25	23	0.6	day	14	3.4	47	40	40	27	4
	where he do	1	3-3	to do	100	0000	150	AND B	Pa da	AN'A	400	mar J	دو دو د فرانها	Swil	301	1000 J	1000	4000	4500	NJ.	43	93	AJ	WEST.	1915	Alia a	31	58

Proprieta in	Degree in	Type of													PAIL	1947												-
MA M	feet	material	2000	14	17	11	300	10	19 (3.6m)	19	31	21	21	21	21	22	22	23	23	24	24	25	25	26	26	26	20	13
	0.0	Total & prompt	34	JA	Jul	30	do	11	24	11	14	30	10	34	10	10	10	114	12	12.	31	24	34	30		34.	35	Ė
- 4	1.0	. and wat	37	2:	37	M.	28	28	21	37	0.00	32	23	20	1.5	29	0,00	14	14	14	74	.19	3	24	17	14	do	П
-1-1	4.0	-	19	+	41.		41	41		81	4	-81	41	+6.1	41	10 7	41	44.	44.	42	4.1	44	1.	-8.7	43	ad plus	41	L
	20	-	165	20	100	182	40	4.1	4	45	100	40	.3.5	100	40	+4.0	10.7	45	de	40	40	62	40	18 40	-60		de	П
1	- 0	-	142	41	17.	41	W	10	41	47	100	140	20.0	100	-digit	40	40.5	444	1349	44	-614	de	44	46/8	ng B	4.9	dy	Ι
- 1	4		47	47		-00	40	38	-3-	-67	1-94	109	02	41	17	69	16.1	-64	44	10.4	1879	4	47	144	44	-64	47	L
		Janety granny	44	34	-	40	34	45	44	-17	-94	107	no y	44	17	47	44	-849	34	44	44	34	44	- 17	44	4	11	I
Parmyani	1500		2.4	0.0	2.0		6-		100		200					0.40											-	1
ofe & Te			oh.	10	40	11		14	3.1	2.3	100	100	1	40	100	40	-	martine and	4.0	30	100	64	54	2.00	100		25	ŧ
ditti gu			- Car	1 7		-	-	1		11 4	9 0,	-			and de	10 0		Money	To make	- Second	2		1000		0.45	Marine.	C. 10	1
10 10 en	Aurt Seme			100 6	wist.	144	400		100	mone	1 4 4	1		400	200	81.0	Bw I			San Full		1 0	400		1	1		ı

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9	51	51	50	18	52	57	46	46	36	36.	34	34	34	34	14		33	JJ	32	31	31	30	Jo	30	30	31	31	37	15.45	3-
_	53	53	53	53		53	53	52	52	25	51	50	50	50	50	50	50	44	45	43	43	42	42	41	41	40	40	44	43	42
14	50	58	50	58	51	58	50	57	57	57	56	56	50	50) 60	55	55	55	-	54	54	45 54	54	54	3.3	43	47	47	47	40
2	61	61	61	61	60	61	60	60	60	60	59	59	3.4	57	24	58	88	50	58	50	58	57	57	57	57	The real Property lies	57	50	51	5'5
i	60	60		60	60	60	60	60	60	60	59.	59	59	59	59	59	59	59	59	58	50	ra	50	58	50	53	50	57	57	50
				A												- '	3,	- 7	37	53	28	58	50	58	58	50	58	5)	51	58
4	23	18	-4	12	8	17	12	18	9	24	14	29	14	-/	11	-5	6	-5	7	,	10									
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3	C1695	C1600	341		· Market ·	Snow			THE RESERVE AND ADDRESS OF THE PARTY NAMED IN	THE RESIDENCE	CHIP	A STREET	The statement of		-	Clear	Clear	loudy	3000	CROP	weer!	Cies	Charly	Rain	Rain	Cror	Chui	Management &	Jea-	100
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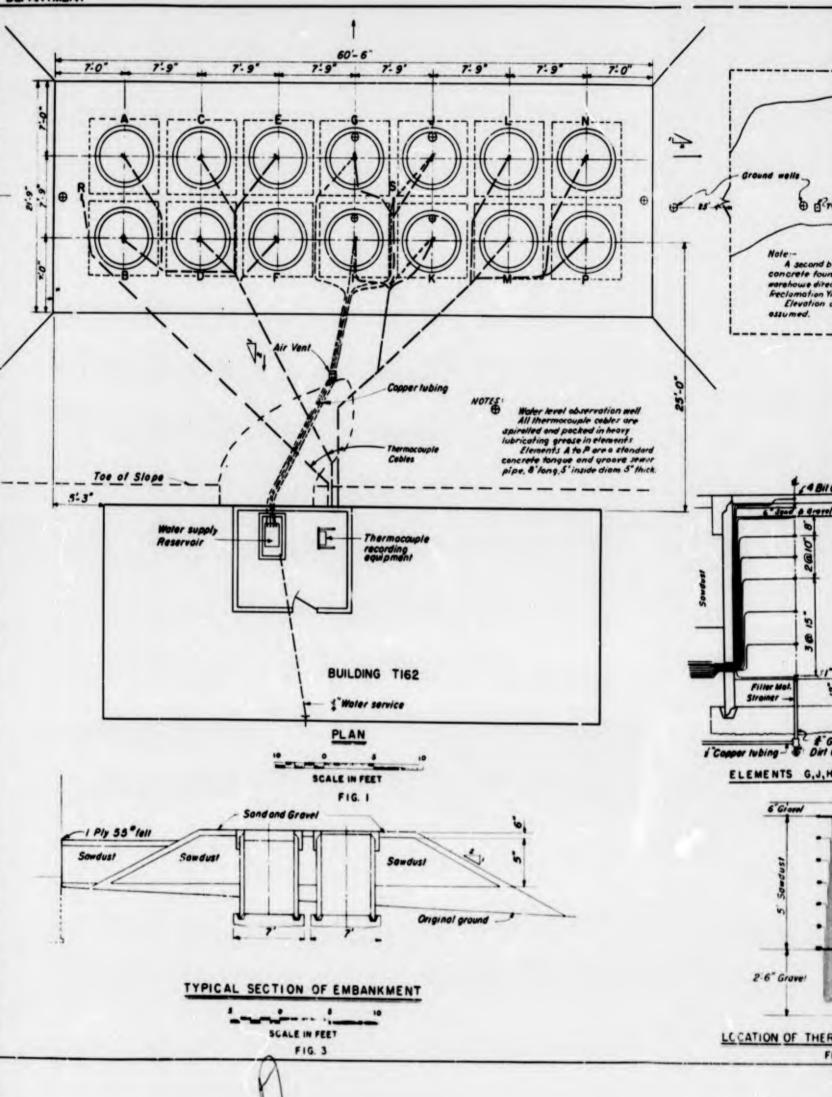
Air temperatures obtained from mermograph record.

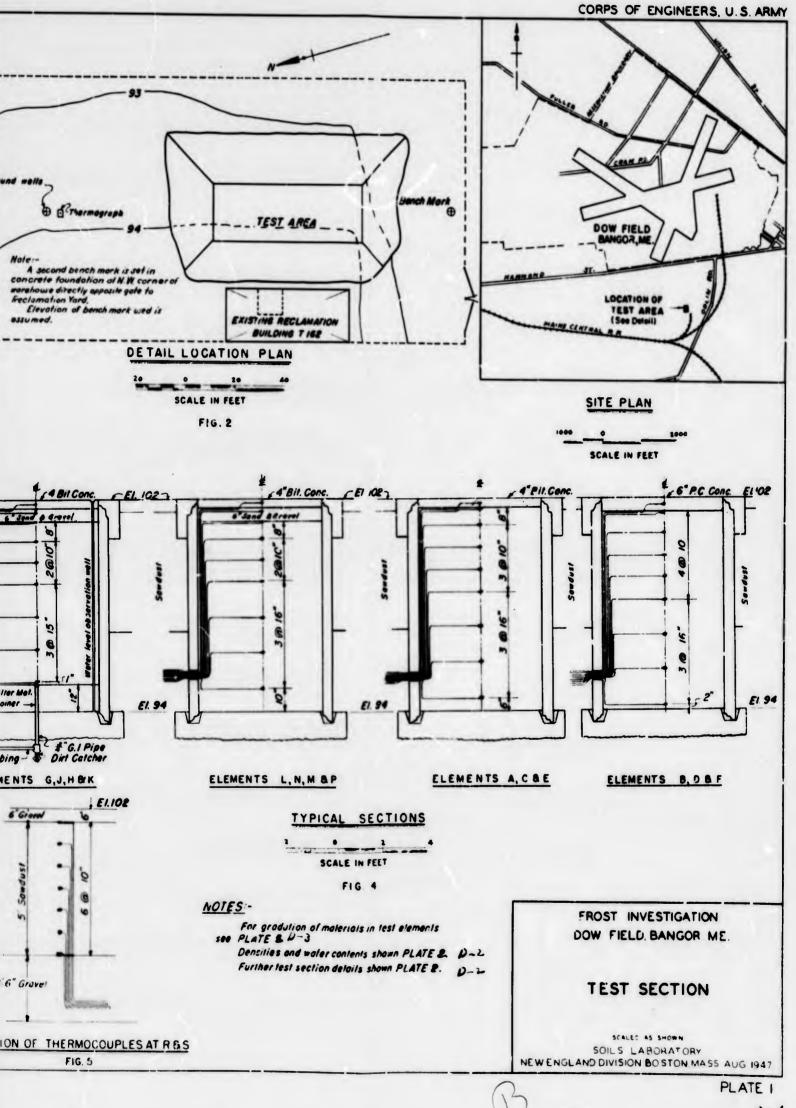
Surface temperature obtained from mercury.

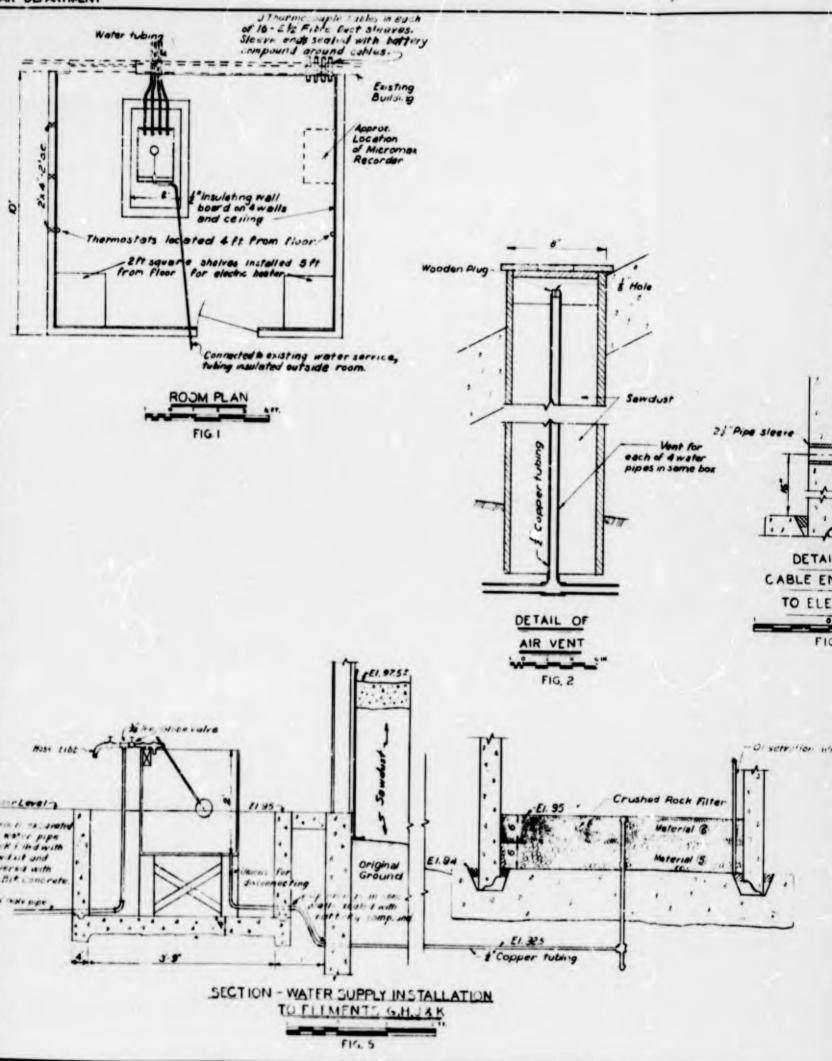
Thermometer laid on the surface of the embankment.

FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

TEMPERATURE RECORD OF EMBANKMENT S







A

2 plies 15* asphalt felt mupped
anto Plyword cover

INZ apacers

1 Plyword

Hollow area filled

n th an dust

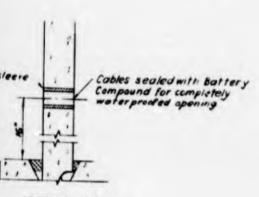
INA spacers
10 ac

Sand
Gravel

Gravel

Y

Committant



DETAIL OF

FIG.3

Servicion will

DETAIL OF BASE & TOP

clement	Specimen Material	Parement	Placed in PCF Dry Wt	for Required % Saturation	"Start of Tests
A	Cinders	4 Bit Conc	53 -60	Ito 2	
0	Cinders	o'PC Con	60-64	1103	
C	So of A word	4 Bit Conc	127-131	6 to 8	-
0	Sand& Grant	6 PC Conc	128 131	6 to 5	
E	Cr Rock	4 Ort Conc	110	4 to 6	
F	Cr Rock	6 PC. Conc	107-112	4 to 6	
G	Sirry Clay	4 Bit Conc	102 105	11 to 25	10-90
H	SiltyClay	4 Bit Con	92.98	25to 29	30 - 90
J	Silty Clay	# Dit Com	102 107	15 to 19	60-70
H	Sitty Jay	& Bit Conc	90 95	151024	60-70
4	Silly Clay	4 Bit Conc.	104-107	19+119	80-80
M	Silly Clay	4 Bit Conc	92-94	25 to 30	80-90
N	Silly Clay	4" Bit Conc	121 105	11 to 19	20-70
•	Silly Clay	4 Bit Conc.	90 93	18 tc 23	60-70

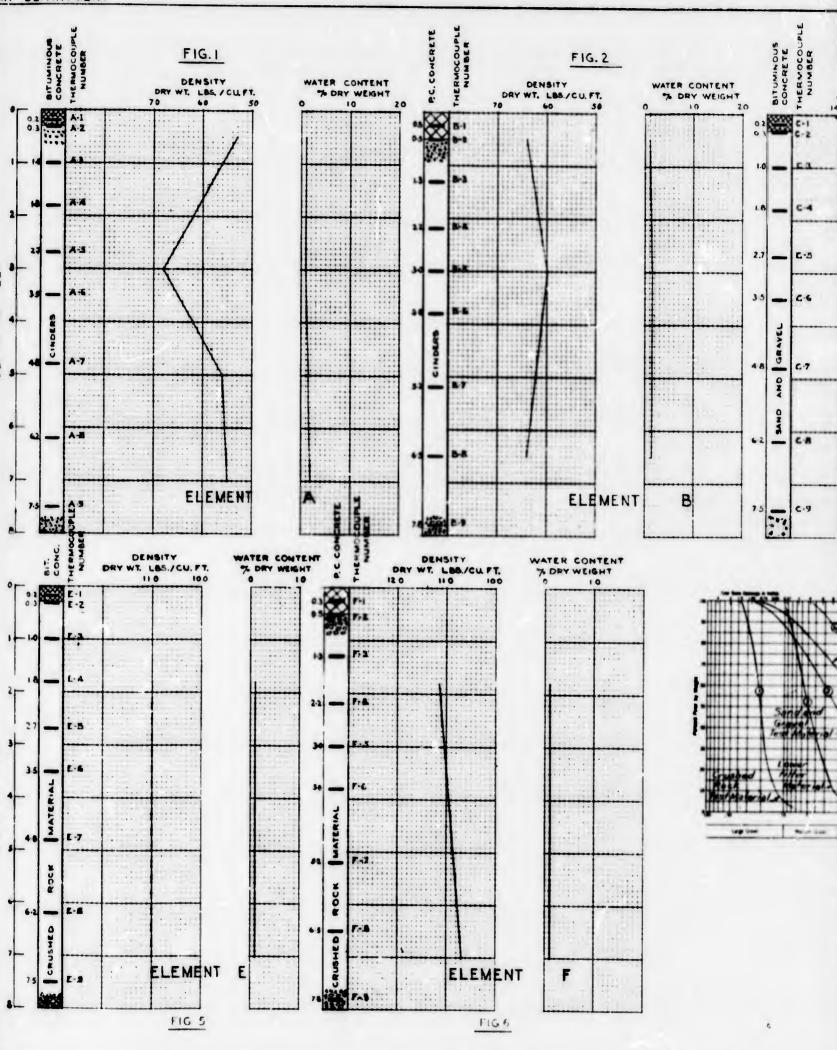
Note for gradation of materials see Plate \$ 0-3

TABLE - ELEMENT VARIABLES

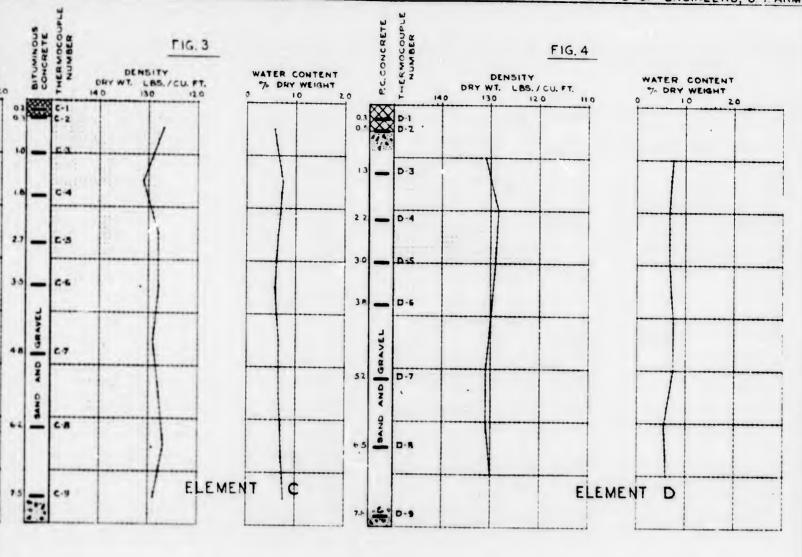
FROST INVESTIGATION DOW FIELD, BANGOR, ME.

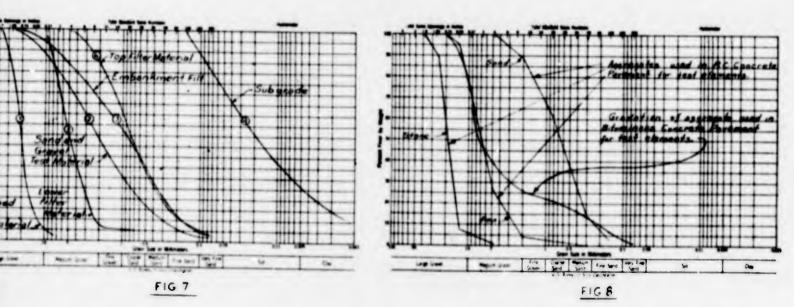
DETAILS OF TEST SECTION





V





NOTE :-

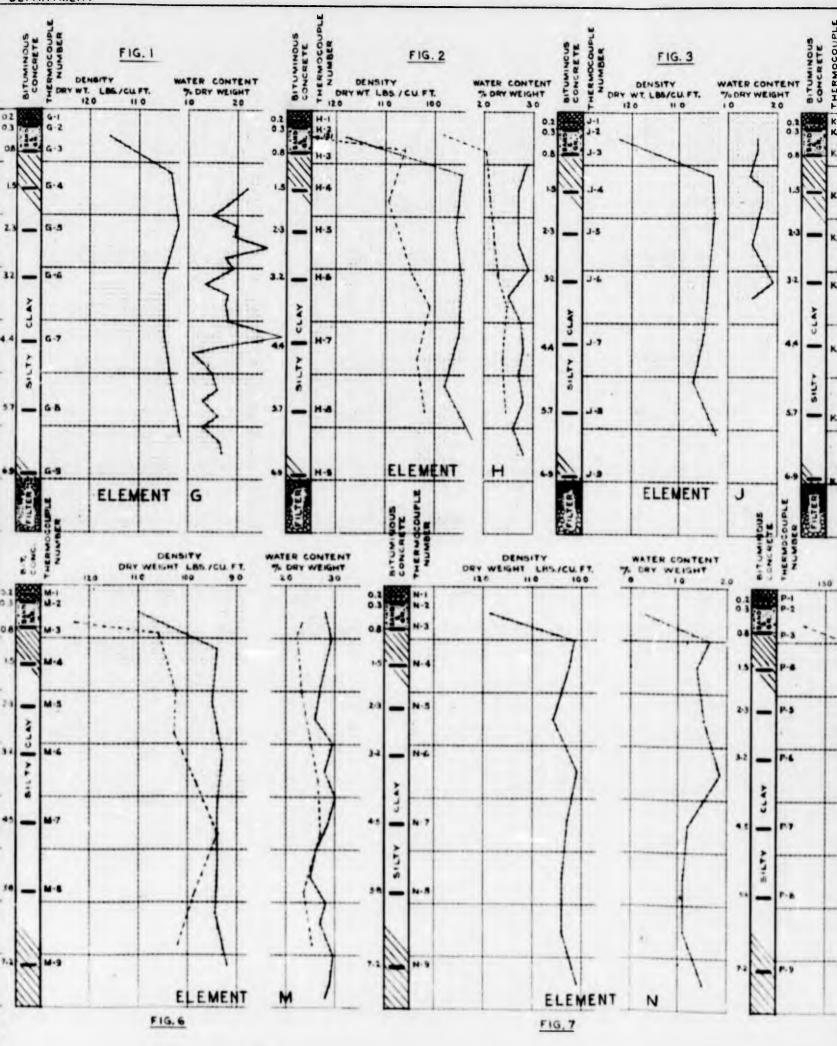
Density of crushed rock in element & determined by pre-weighing material before compecting in place in layers of predetermined thickness.

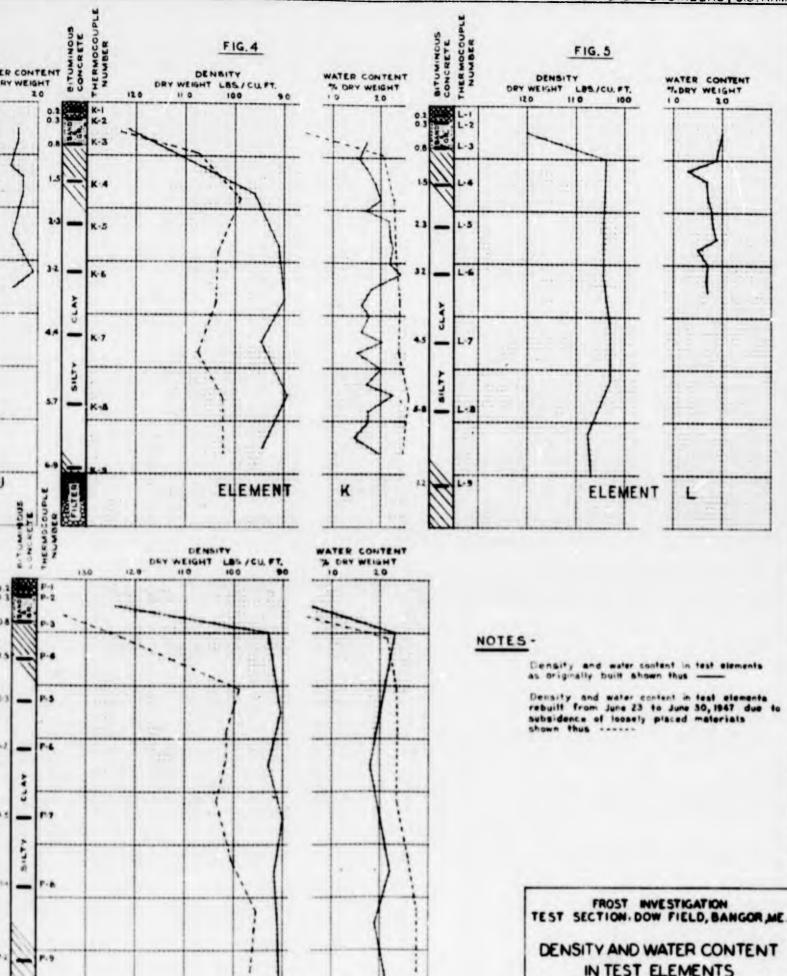
FROST INVESTIGATION TEST SECTION DOW FIELD, BANGOR, ME.

DENSITY AND WATER CONTENT IN TEST ELEMENTS WITH LOCATION OF THERMOCOUPLES

SOIL CLASSIFICATION DATA







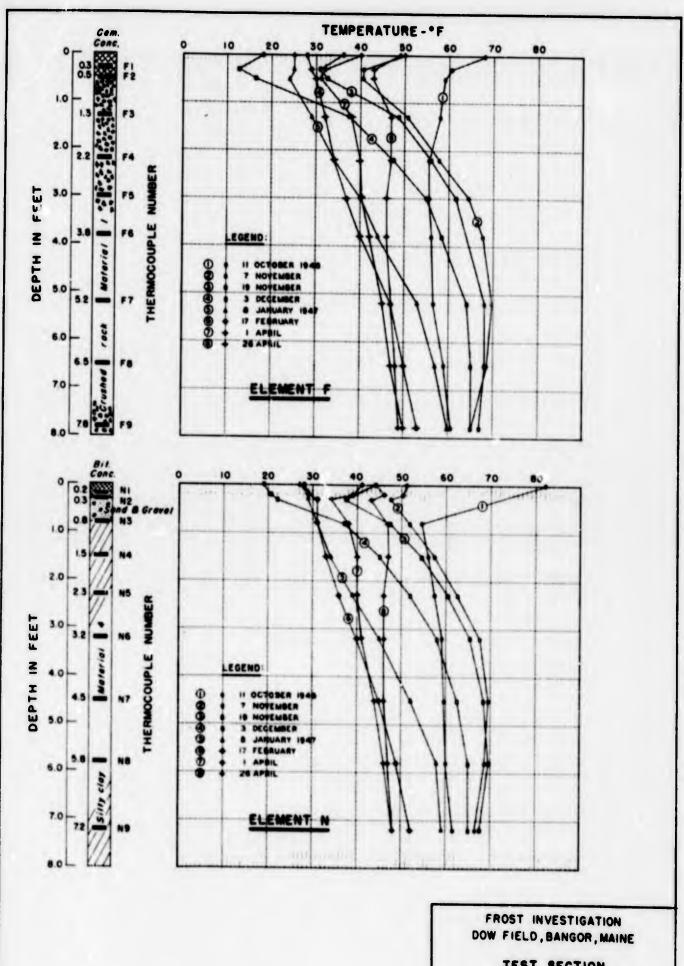
ELEMENT

FIG. 8

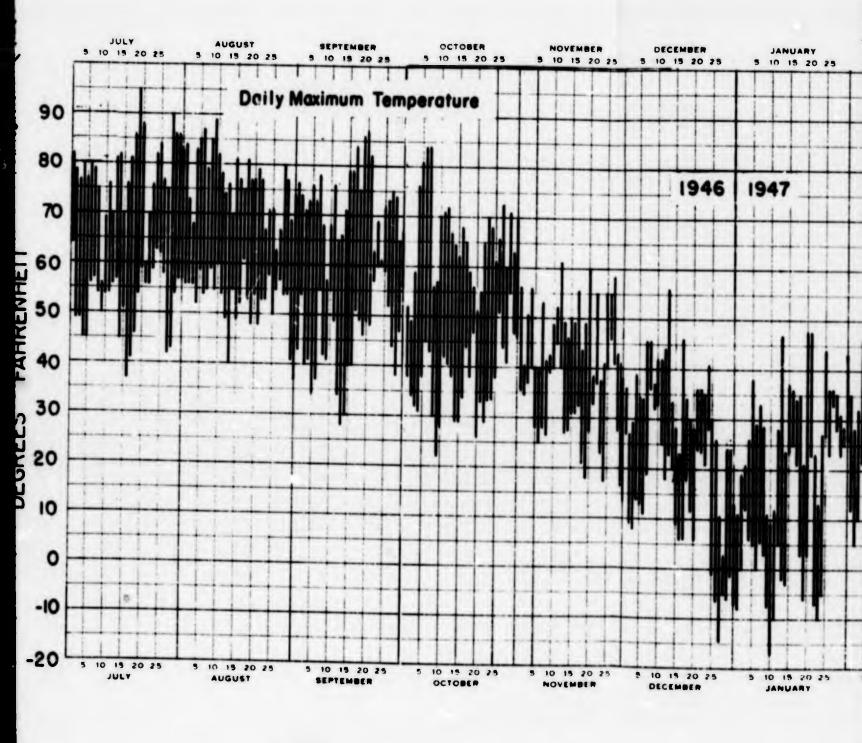
P

PLATE 4

WITH LOCATION OF THERMOCOUPLES

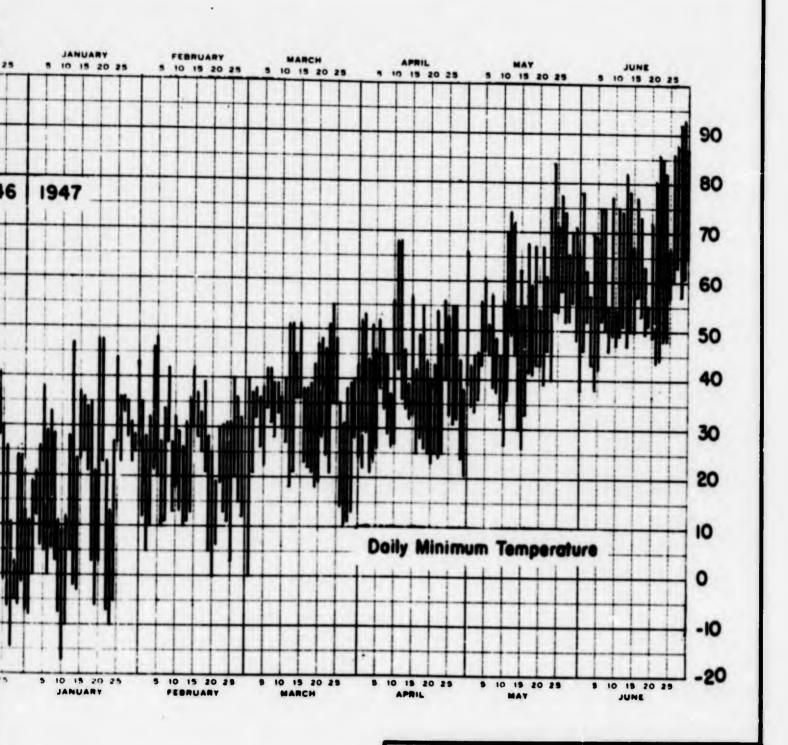


TEST SECTION
TYPICAL SUBSURFACE
TEMPERATURES



NOTES:
SOURCE OF DATA
U.S. Weather Bureau(Bangor Hydro-Electric
Company) I July-10 October, 1946.
New England Division Thermograph, continuous
record, II October, 1946—I July, 1947.

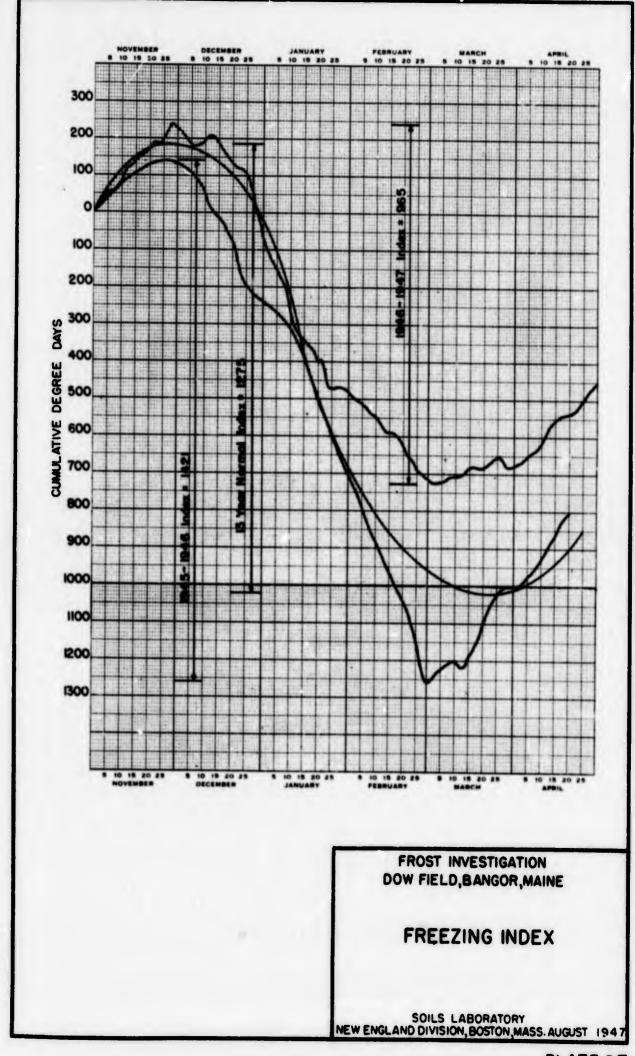


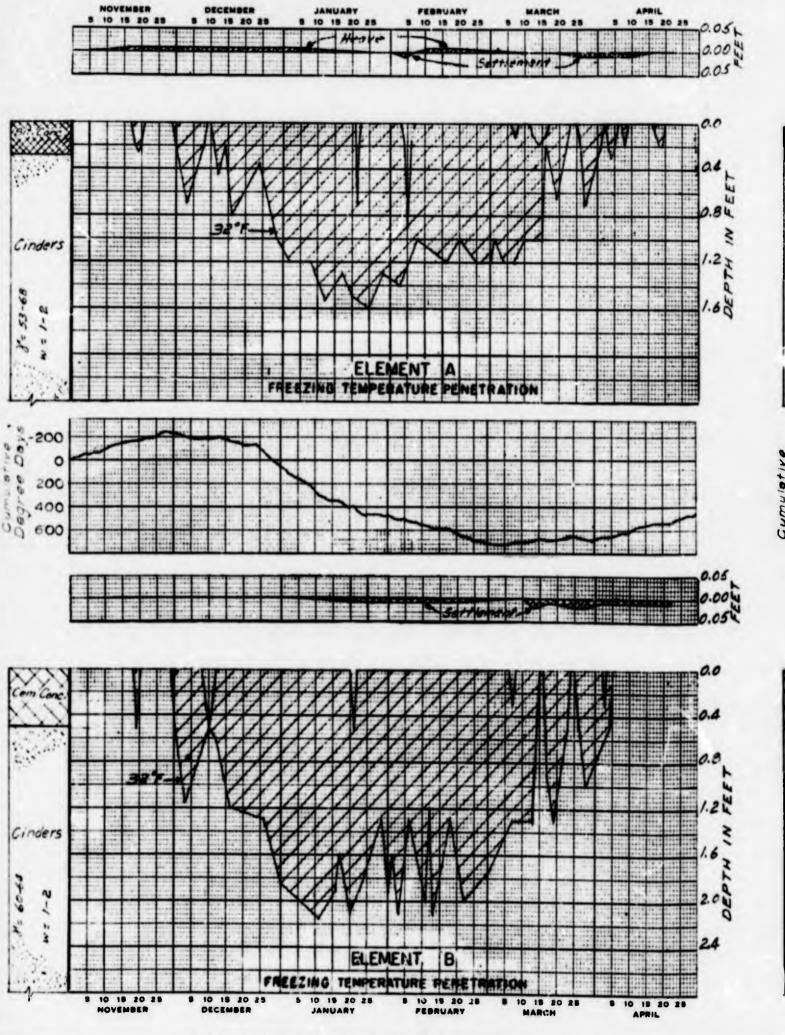


FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

MAXIMUM AND MINIMUM DAILY TEMPERATURES







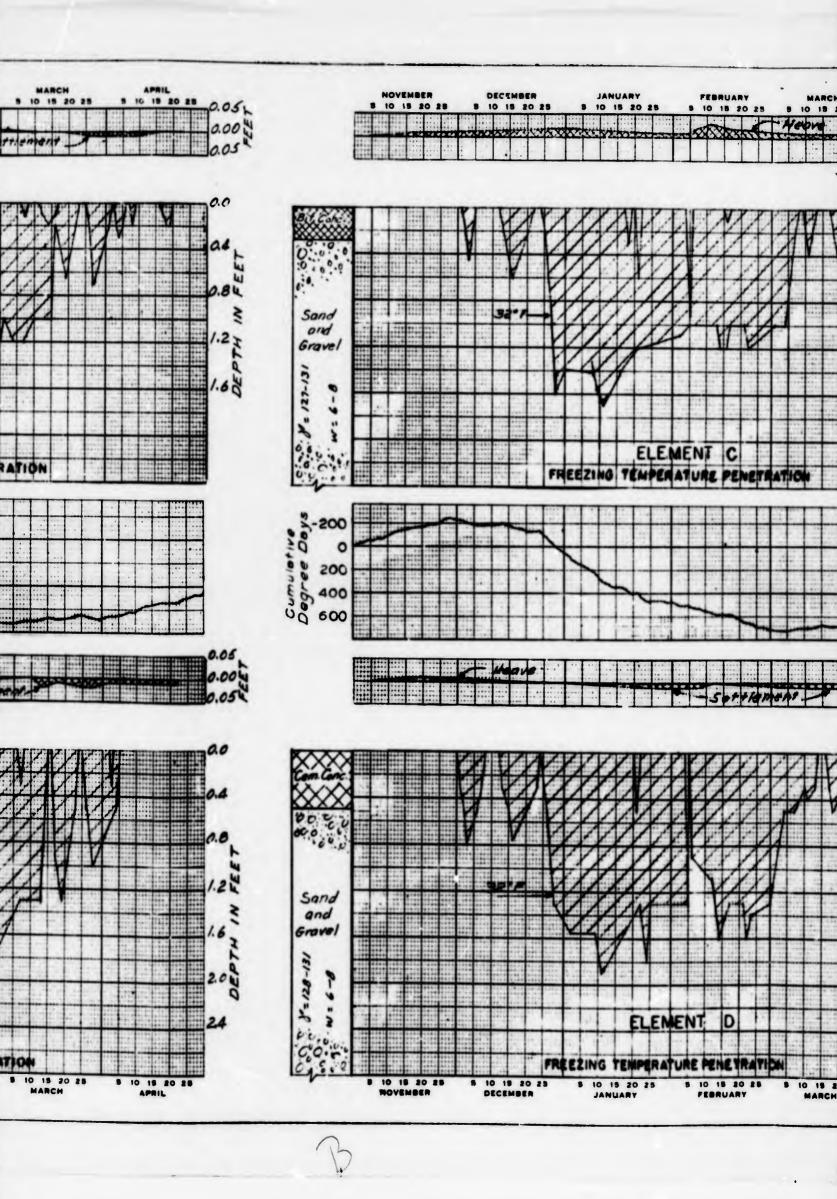
Sand ond Grave

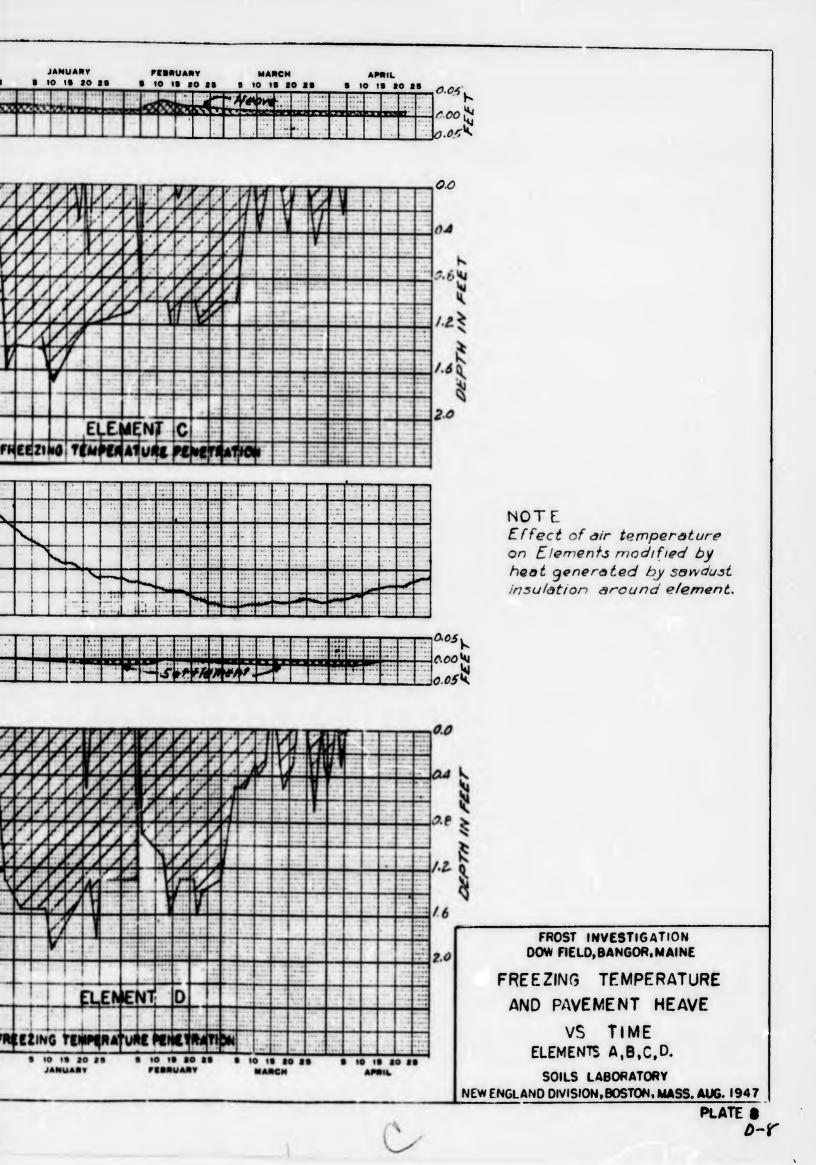
Cumulative
Degree Days
0 0 0 0

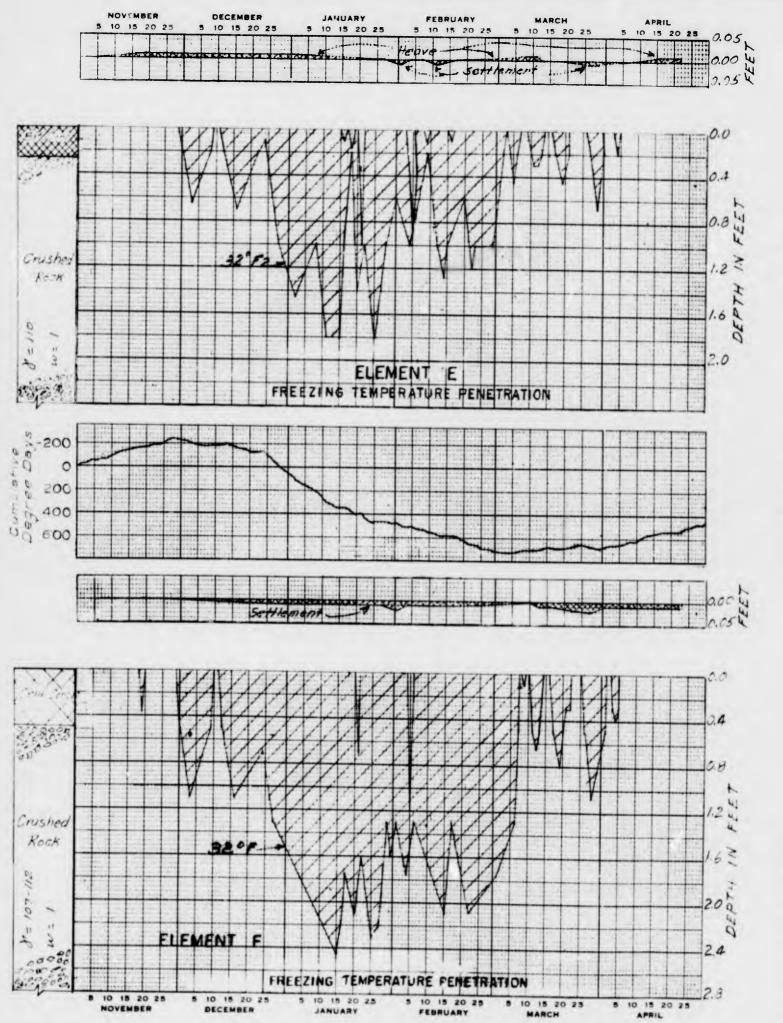
Com Con

Sand and Grave

181-821-8 000 N





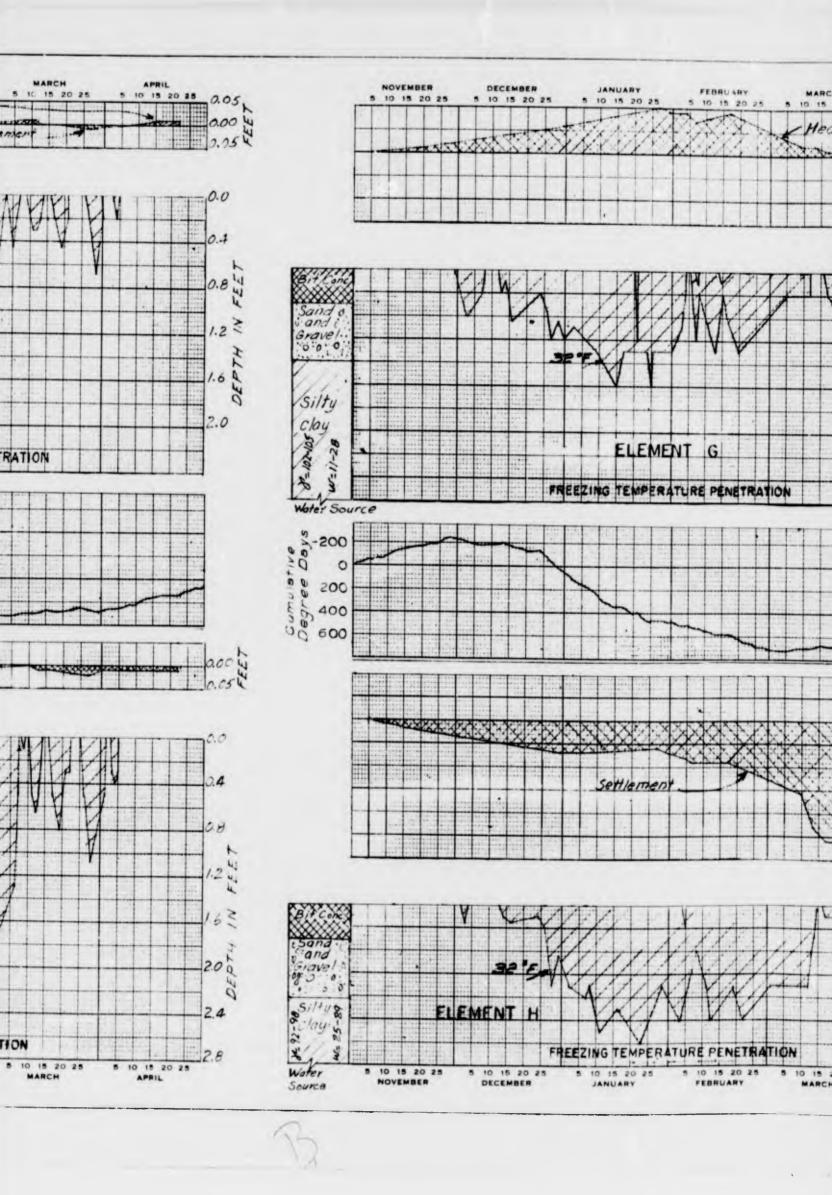


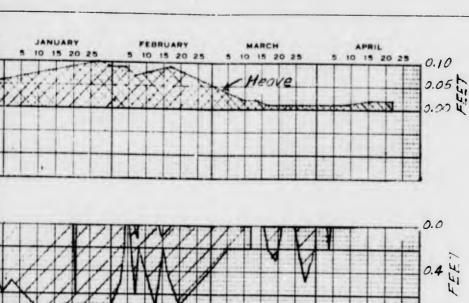
\$3000 Silla Work

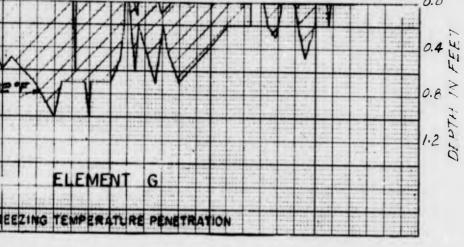
San

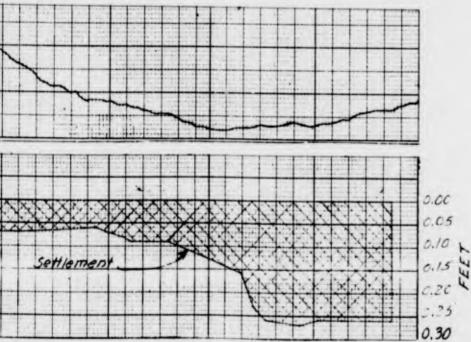
Si Che Solizor - A Work

Cumulative Degree Days

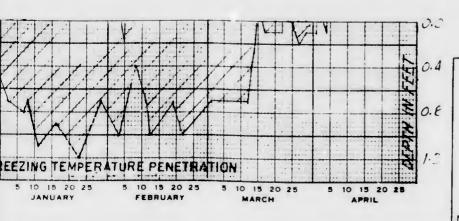




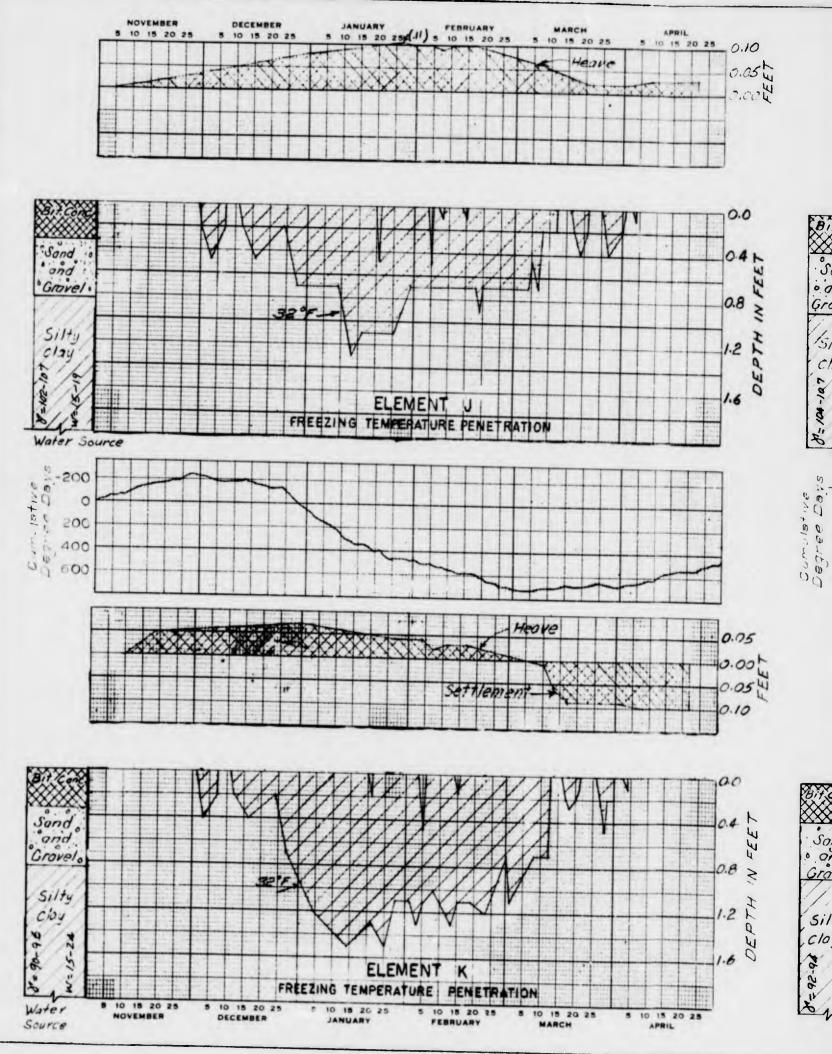




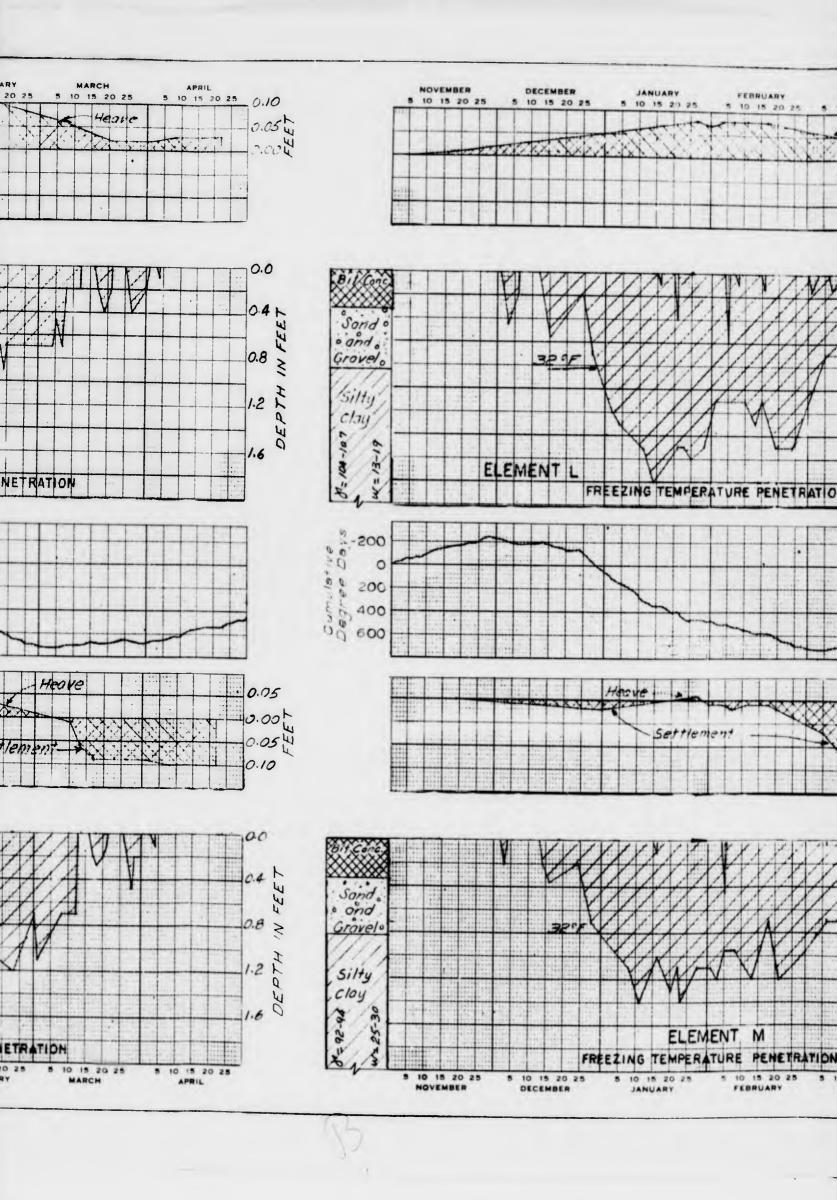
NOTE Effect of air temperature on Elements modified by heat generated by sawdust insulation around element.

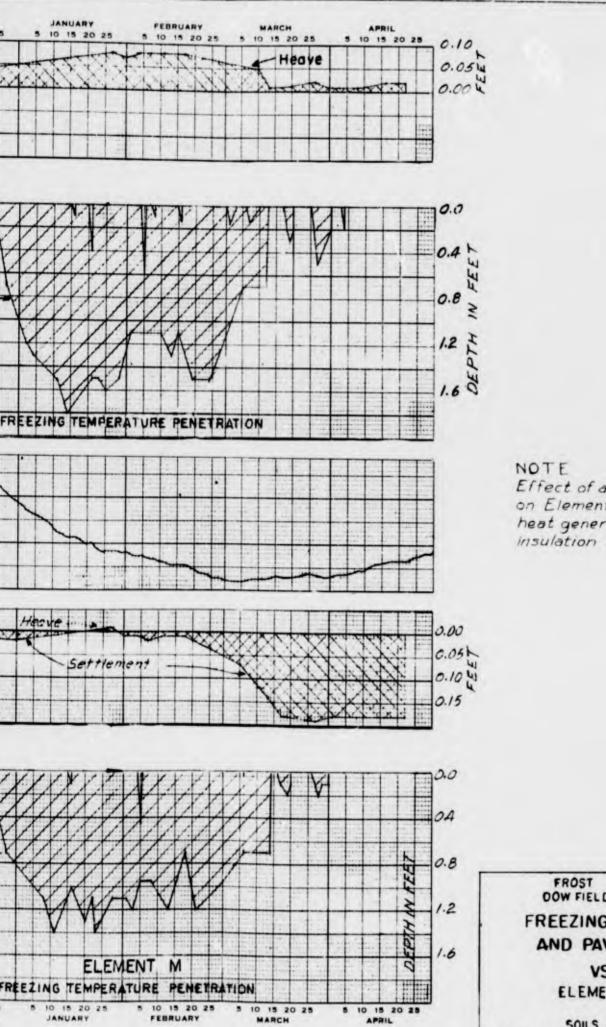


FROST INVESTIGATION
DOW FIELD, BANGOR, MAINE
FREEZING TEMPERATURE
AND FAVEMENT HEAVE
VS TIME
ELEMENTS E, F, G, H.



\



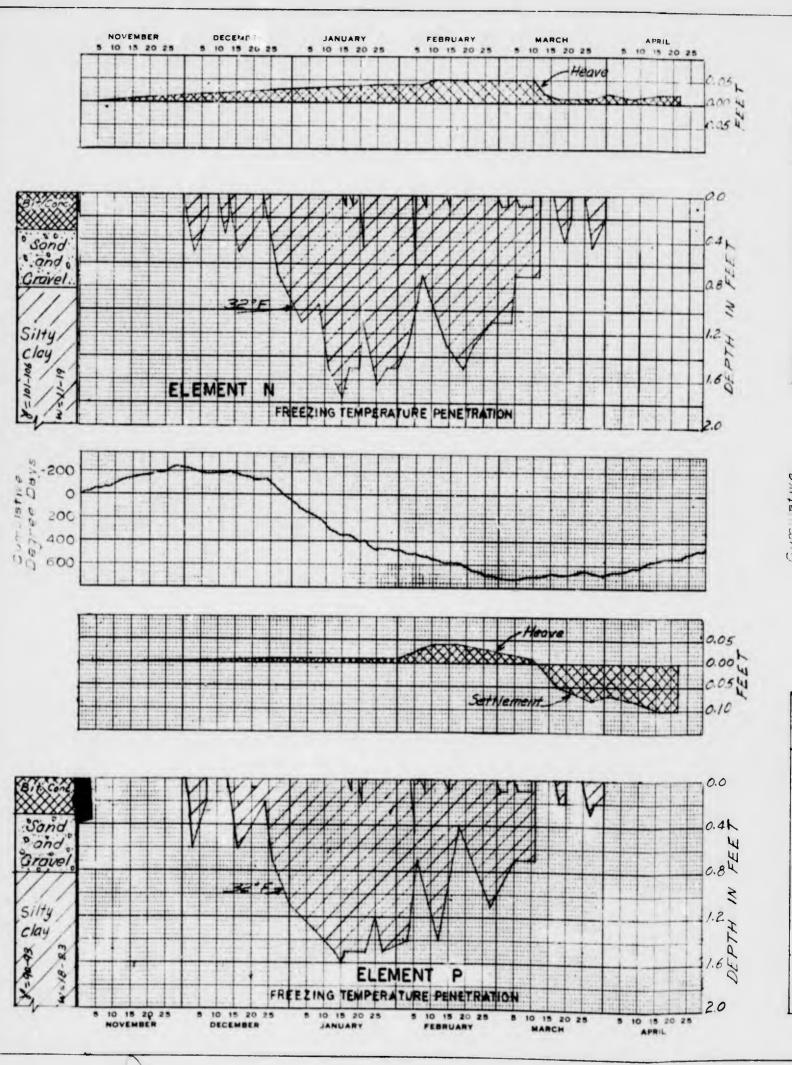


Effect of air temperature on Elements modified by heat generated by sawdust insulation around element.

PROST INVESTIGATION

FREEZING TEMPERATURE
AND PAVEMENT HEAVE

VS TIME ELEMENTS J,K,L,M



o ore San e and er ne or o

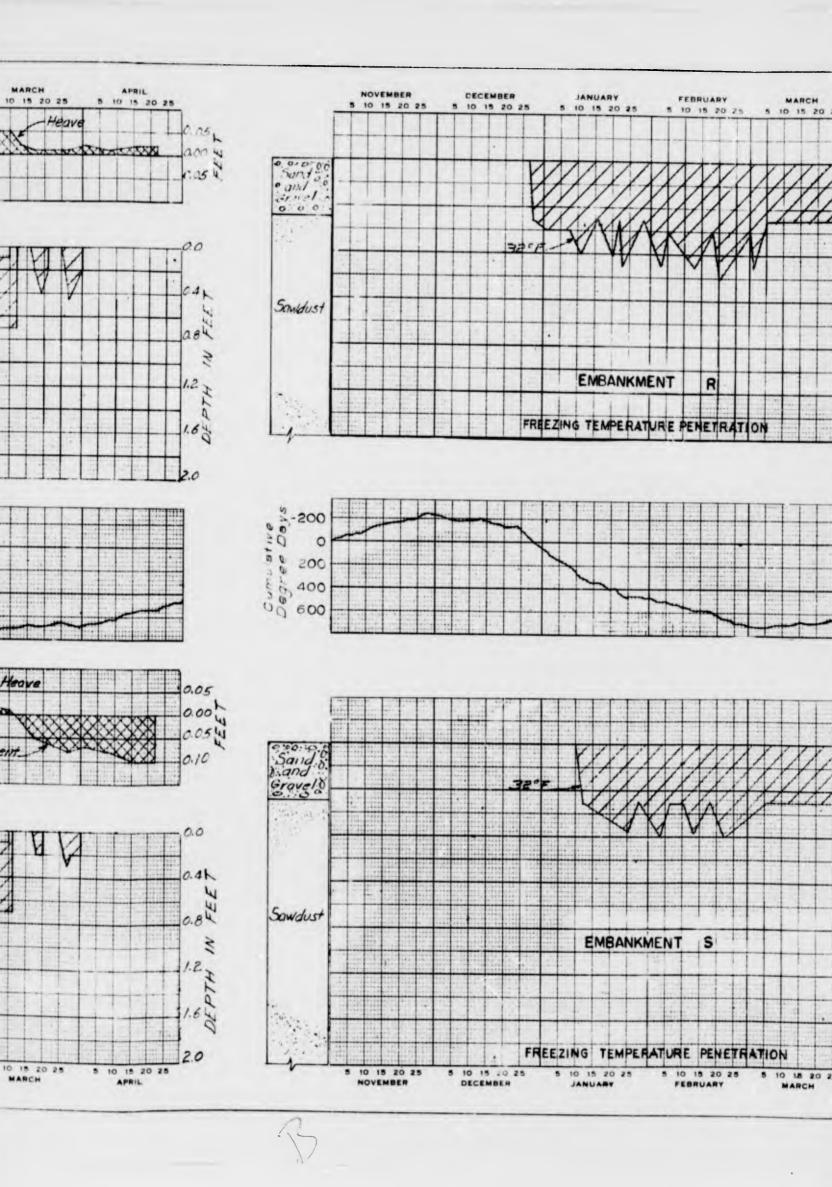
Sawd

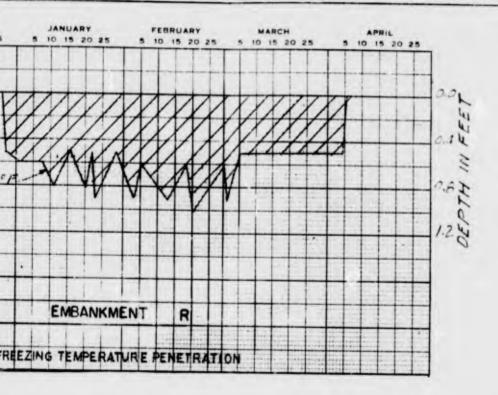
5/0/2

Degree Days

Sand Sand Grave

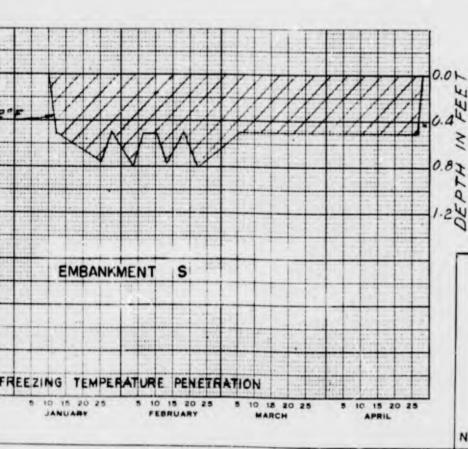
Sawdo







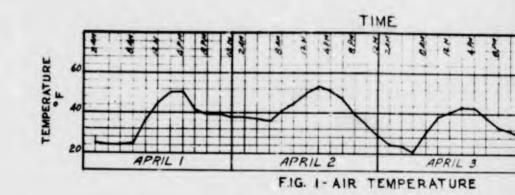
NOTE Effect of air temperature on Elements modified by heat generated by sawdust insulation around element.



FROST INVESTIGATION

FREEZING TEMPERATURE AND PAVEMENT HEAVE VS TIME

> ELEMENTS N, P EMBANKMENTS R , S



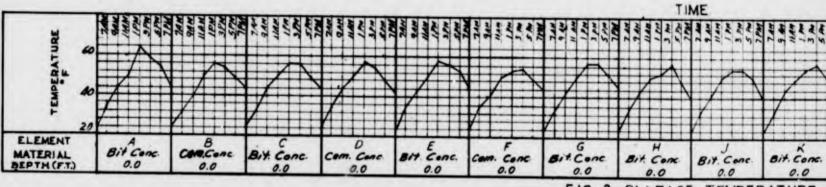


FIG. 2-SURFACE TEMPERATURE

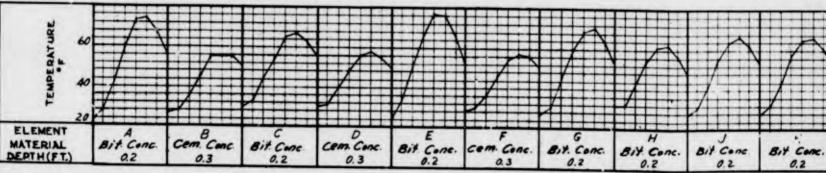


FIG. 3-MID-POINT OF PAVEMENT

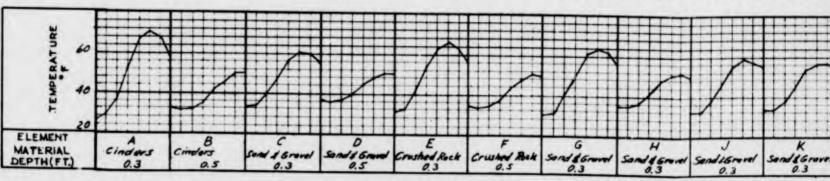


FIG. 4-TOP OF BASE COURSE

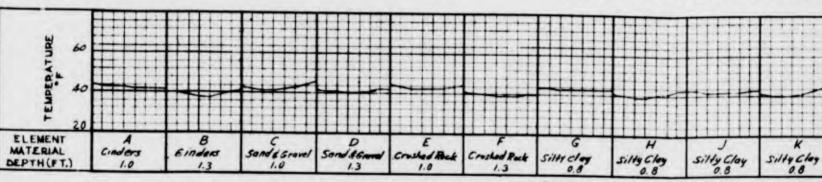


FIG. 5-BASE COURSE OR SUBGRAD

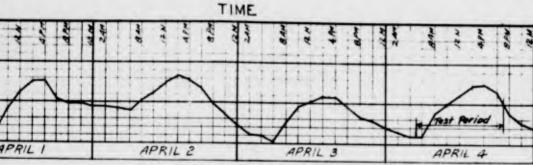


FIG. I- AIR TEMPERATURE

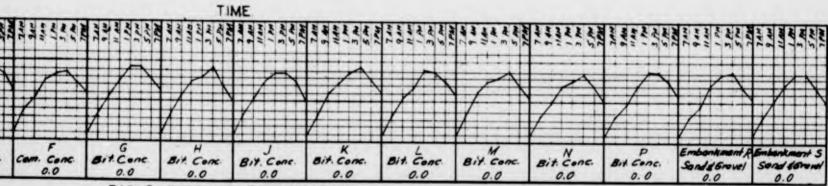


FIG. 2-SURFACE TEMPERATURE

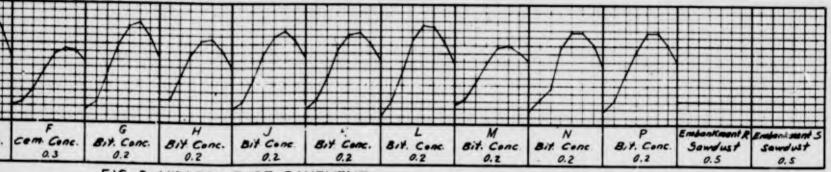


FIG. 3-MID-POINT OF PAVEMENT

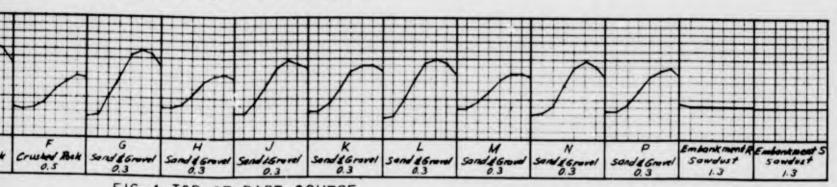


FIG. 4-TOP OF BASE COURSE

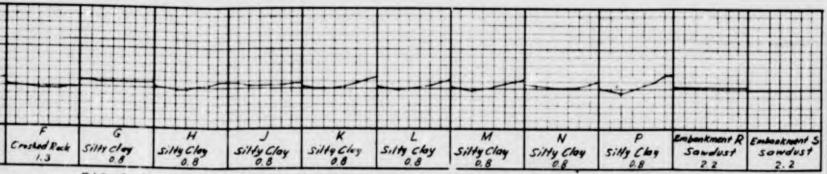
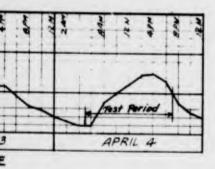
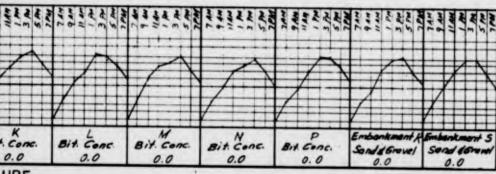
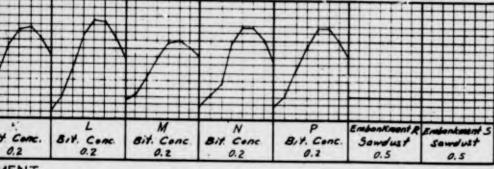


FIG. 5-BASE COURSE OR SUBGRADE



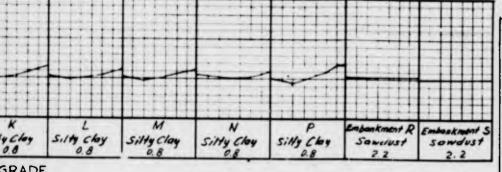


URE



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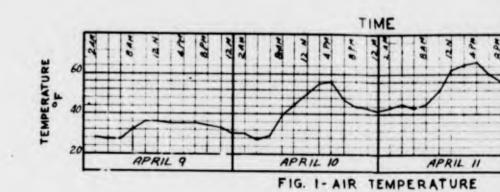
GRADE

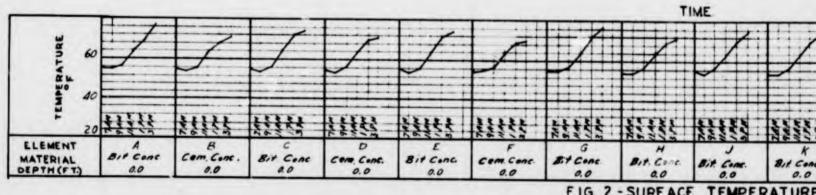
Notes:-

Weather: Clear, northerly wind, gentle breeze. Air temperatures, Fig I, taken from thermograph records. Surface temperatures, Fig 2, mode by mercury thermometer placed on pavenient surface and shielded from the sun. Subsurface temperatures, Figs. 3,4,5, made by thermocouple installations. Readings made every two hours, beginning at SAM. and ending at 7 P.M. 4 April 1947.

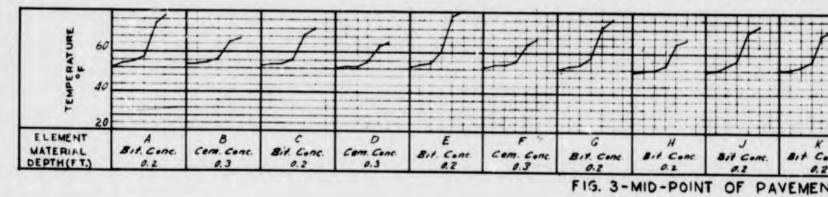
FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

TEMPERATURES TWO HOUR INTERVALS ON 4 APRIL 1947



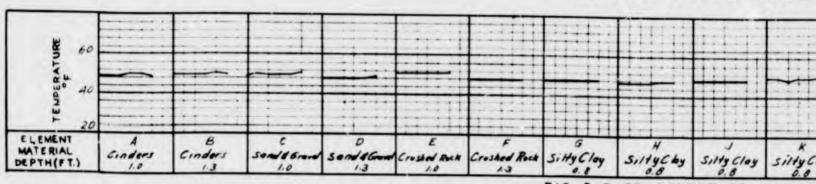


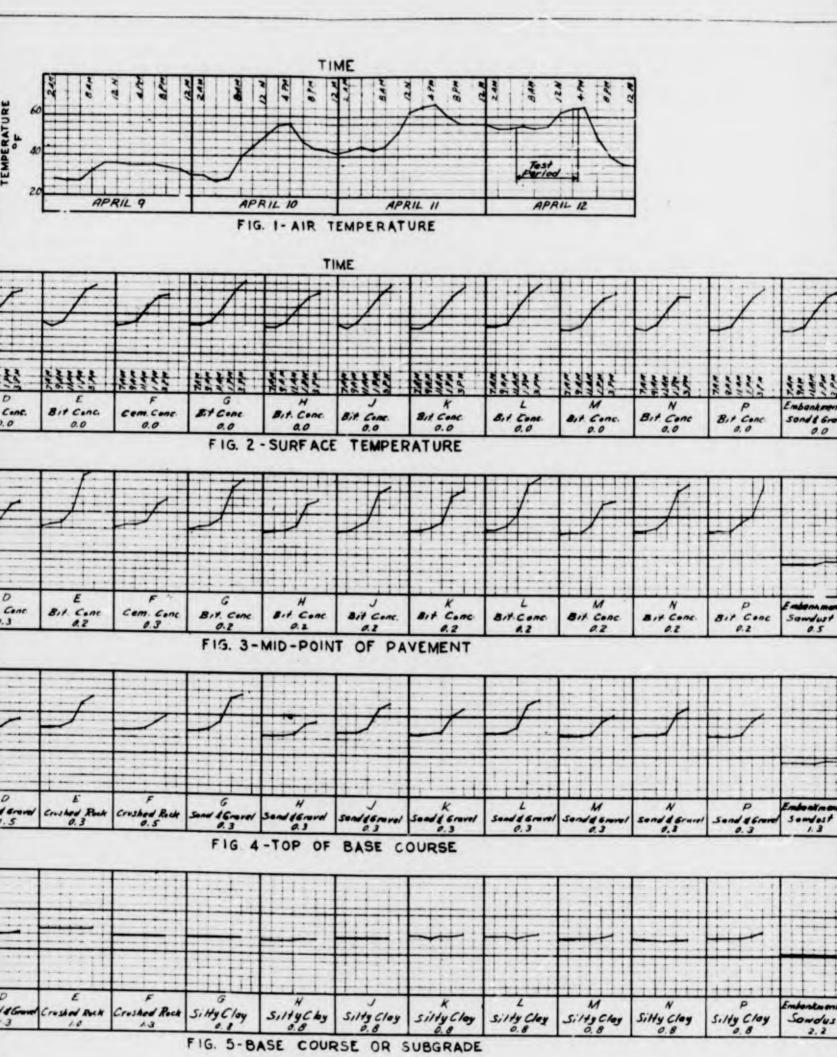
TEMPERATURE FIG. 2 - SURFACE

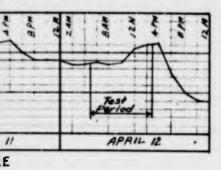


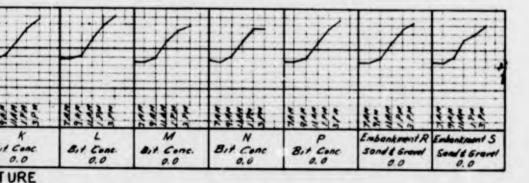
TEMPERATURE "F 60 140 ELEMENT 0 K Cinders 0.3 0.5 Crushed Ro 0.3

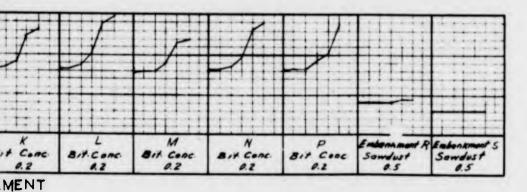
FIG. 4-TOP OF BASE COURSE

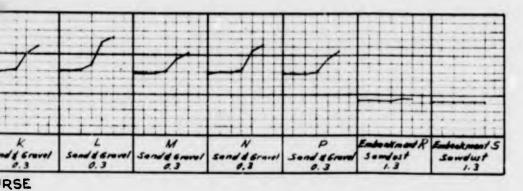


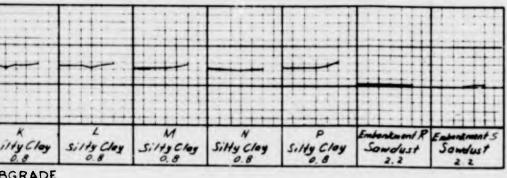












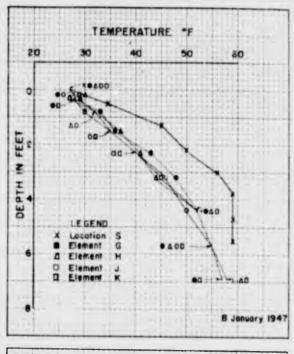
BGRADE

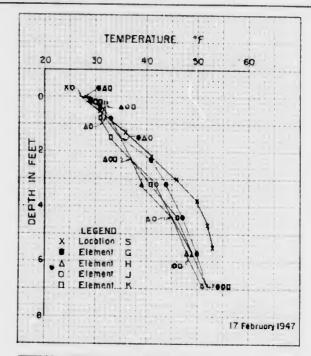
Notes :-

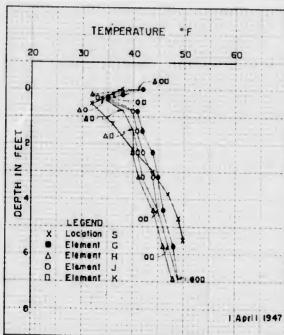
Weather: Southwesterly wind, gentle breeze. Cloudy from 5 A.M. to 7 A.M. Rain from 7A.M. to 11 A.M. Clearing at 2 P.M. Air temperatures, Fig I taken from thermograph records Surface temperatures Fig 2 mode by miscury therinometer placed on pivement surface and shielded from the sun. Subsurface temperatures. Figs 3. 4,5 made by therinocouple installations. Readings made every two hours, beginning at 5 AM and ending at 3 PM 12 April 1947

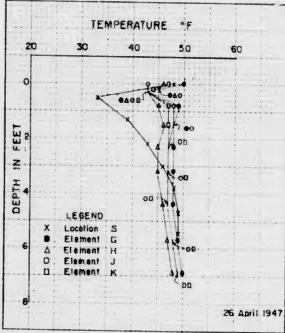
FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

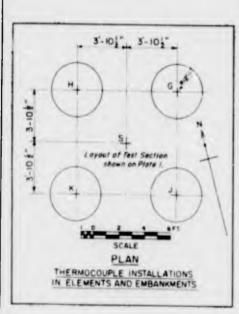
TEMPERATURES AT TWO HOUR INTERVALS ON 12 APRIL 1947

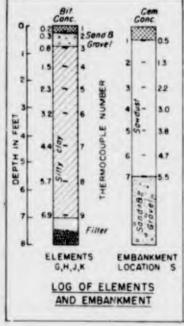












FROST INVESTIGATION DOW FIELD, BANGOR, MAINE

COMPARATIVE TEMPERATURE GRADIENTS IN ELEMENTS AND EMBANKMENT

MAX. PENETRATION OF 32°F. TEMP. DEPTH IN FEET O O O O O O O O O O O O O	CEMENT CONG. GINDERS	BIT CONG. SAND & GRAVELO	GEMENT CONC. SAND B. GRAVEL CO	BIT C.CRUSHED ROCK A W	CEMENT CONG CRUSHED ROCK	BIT G. SILTY CLAYS Z	BIT C. SILTY CLAYE	BIT CONC. SILTY CLAYER	BIT CONG SILTY CLAY X	BIT: COND. SILTY CLAY F	BIT CONG. SILTY CLAY S	BIT CONC. SILTY GLAY Z	BIT. CONC. SILTY CLAY &	SAWBUST &
	M.	AX I MIL	JM FF	REEZI	NG T				PENE	TRAT	ION			
DURING JANUARY 1947 IN. FEET CONG. GINDERS	CONC.CINDERS (B)	S. SAND & GRAVEL	C. SAND. GRAVEL CO	6. CRUSHED ROCK III III	CRUSHED ROCK TO	BIT C. SHLTYCLAND S	BHT. G. SILTY CLAY I B	BHEG.SILTYCLAY C. R.	CONG. SILTY GLAY X	CONG. SILTY CLAY. F	BIT C. SILTY CLAY =	CONG SILTY GLAY Z	ONG. SILTY GLAY TO	SAWDUST
TH TH	GEMENT	9HT: 0	CEMENT	<u>#</u>	EMENT CONC.				BIT	BHT CO		BIT CO	BIT.C.	
DEPTH DEPTH BHT		· · · ·			<u> </u>									
MEAN PENETRATION OF 32	0	MEAN	FRE	EZIN DUR	3	IANU	ARY	RE PE	ENETF	RATI	ON			
MEAN PENETRATION O	9	MEAN	FRE		G TE		ARY	i	7			TIGATI	ON	
MEAN PENETRATION O	s G,H,J	,K,L,M	1. N. ar	DUR	G TE	IANU	ARY	i	7 FR	OST I	NVES	TIGATI NGOR,N		
MEAN PENETRATION O	s G,H,J yer of s	,K,L,M	I,N. dr	DUR nd Pha	G TE	IANU	ARY	9'4'	FR DOW	OST I	NVES D,BAN		AINE ZIN G	6



figure 1. Detail of concrete foundation for Element A

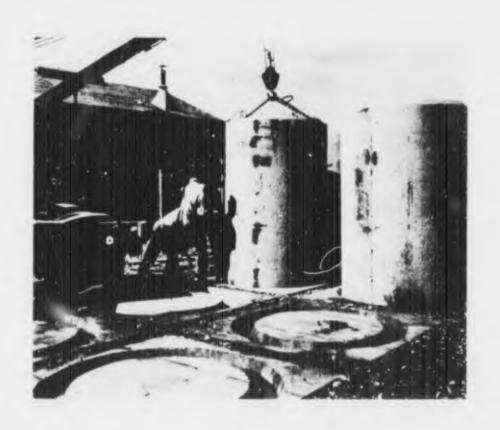


Figure 2. Placing a concrete cylinder during test section construction

DOW FIELD TEST SECTION DETAILS OF CONSTRUCTION



figure 1. All concrete cylinders and gravel backfill in place



rigure 2. General view of test section prior to placing sawdust insulation backfill

DOW FIELD TEST SECTION
GENERAL VIEW OF TEST ELEMENTS DURING CONSTRUCTION



Figure 1. Subgrade material in clement w, placed at density of 90 PCF and at water content of 25 per cent



rigure 2. Subgrade material in Element F, placed at density of 30 For and at water content of 22 nercent

DUW FIELD TEST SECTION SUBGRAVE MATERIAL IN ELEMENTS



Figure i. Settlement of pavement in Element H on 11 April 1947

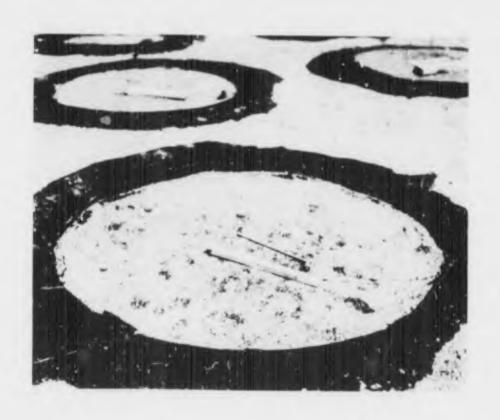


Figure 2. Settlement of pavement in Element & on 11 April 1947

DOW FIELD TEST SECTION
TEST ELEMENTS SHOWING PAVEMENT SETTLEMENT

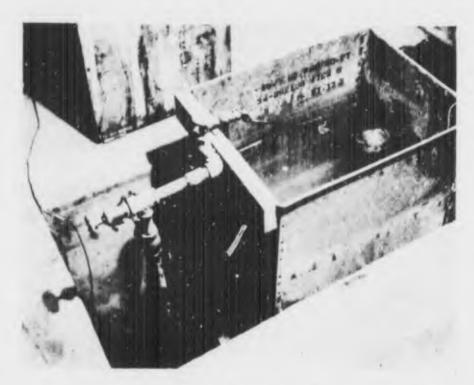


Figure 1. ball and cork valve control for constant ground water level in tlements G, H, J, and k

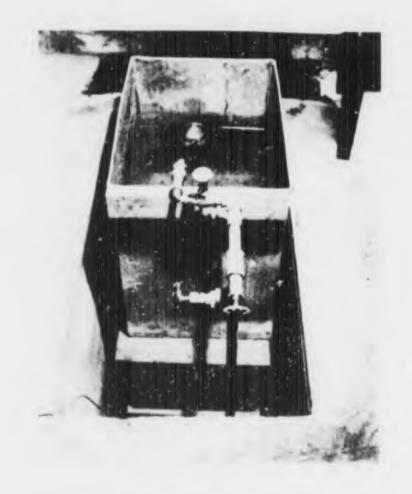


Figure 2. tall and cork valve control for constant ground water level in tlements G. H. J. and K

DOW FIELD TEST SECTION WATER SUPPLY TANK



rigure 1. Thermocouples in element prior to placing soil



Figure 2. Thermocouple leads in trench from elements to instrument room

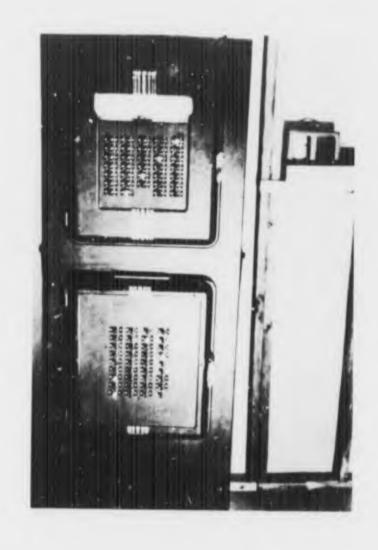
DOW FIELD TEST SECTION
THERMOCOUPLES AND THERMOCOUPLE LEADS



Figure 1. Entrance of thermocouple leads to instrument room



DGW FIELD TEST SECTION
THERMOCOUPLE LEADS TO INSTRUMENT ROOM



DOW FIELD TEST SECTION
THERMOCOUPLE SWITCHING PANEL AND TEMPERATURE
INDICATING SCALE

NEW ENGLAND DIVISION CORPS OF ENGINEERS, U. S. ARMY BOSTON, MASSACHUSETTS

DATA REPORT OF FROST INVESTIGATIONS FISCAL YEARS 1943 - 1949

APPENDIX E

LABORATORY AND FIELD TEST PROCEDURES
1944 - 1945

Part 1, MISSOURI RIVER DIVISION Part 2, GREAT LAKES DIVISION Part 3, NEW ENGLAND DIVISION

FROST EFFECTS LABORATORY

JUNE 1949

DATA REPORT OF FROST INVESTIGATIONS FISCAL YEARS 1943 - 1949

APPENDIX E

Part 1

LABORATORY AND FIELD TEST PROCEDURES
FOR
MISSOURI RIVER DIVISION
1944 - 1945

APPENDIX E

Part 1

LABORATORY AND FIELD TEST PROCEDURES

MISSOURI RIVER DIVISION

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LETTER APPENDIX

APPENDIX E

Part 1

LABORATORY AND FIELD TEST PROCEDURES MISSCURI RIVER DIVISION

1. Introduction.

The methods and procedures given herein have been used in connection with the investigation of frost action beneath airfield pavements as authorized by the letter dated 7 July 1944 from the Office, Chief of Engineers, to the Division Engineer, New England Division, subject: "Frost Investigation," and by the letter dated 28 August 1944, from the District Engineer, Boston District, to the Division Engineer, Missouri River Division, subject: "Frost Investigation".

2. Test Methods.

by the American Society for Testing Materials, hereinafter referred to as A.S.T.M. Standard Methods; and the methods established by the Chief of Engineers, U.S. Engineer Department as given in the Engineering Manual, and the amendments thereto, hereinafter referred to as the Engineer Manual Method, and referred to by paragraph and chapter, or as amendments thereto.

3. Plate Load Bearing Tests.

Plate load boaring tests, using steel plates varying from 24 to 30 inches in diameter, were made on the surfaces of both rigid and flexible pavements and on the surfaces of base courses, subbases, and subgrades to determine the load-deformation characteristics of these surfaces under relatively large areas of loading. Plate load bearing tests are divided into the following groups:

a. Pavement Surface Tests.

(1) Concrete Pavement Tests.

(a) Method.

The concrete pavement load bearing test procedures used in determining the load-deformation characteristics of the surfaces of concrete pavements, were established by the Missouri River Division.

(b) Equipment.

Two semi-trailer rigs were used to supply the static leads necessary to perform the surface tests. One rig was leaded with sand or gravel, giving a lead in excess of 75,000 pounds on the bearing plate. The second rig, leaded by means of an 11,000-gallon water tank, gave static leads in excess of 100,000 pounds. Leads were applied with a hydraulic jack arrangement, using 30 or 50-ten capacity jacks. A 24-inch diameter circular steel bearing plate, approximately 3/4 inches in thickness, was used for the leading tests. Approximately 3 smaller plates were stacked above the bearing plate in order to secure greater stiffness and rigidity in the larger plate.

(c) Procedure.

The surface tests were generally made in the corners of concrete slabs, adjacent to construction, contraction or transverse expansion joints, in order to test the pavement at its weakest point. The steel bearing plate was scated on the surface of the pavement with a thin layer of plaster of Paris, the plate being placed before setting of the plaster occurred. The hydraulic jack was placed over the center of the bearing plate, reacting against a central point on the leaded semi-trailer. Deformations were obtained from two oppositely mounted extensemeters, the pins of the extensemeters resting on the plate, while the extensemeters were mounted on a beam supported by rods driven through holes in the concrete pavement. The rods supporting the beam were greater than 8 feet distand from the edge of

the plate, and were supported entirely by the soil beneath the pavement.

Loadings were applied in increments of 18,000 pounds, each load being held constant until a movement of loss than 0.01 inches per minute was obtained before an additional increment was applied. Extensemeter readings were recorded at the end-point of each increment of load. Repeated increments of loading were added until either cracking of the pavement occurred, or the maximum loading of the test rig was obtained. After completion of the first cycle, if cracking did not occur, 9 additional cycles of loading and unloading were made, unless failure of the pavement occurred in loss than this number of cycles. Repeated loadings were made by rapidly loading the plate to the maximum loading permitted by the rig, and then unloading rapidly, recording only the zero and maximum load deformations.

(2) Floxible Pavement Tests.

(a) Mothod.

The mothods used in obtaining single and repeated cycle loadings on flexible pavements were established at a conference held on 10 March 1945 at Pierre, South Dakota, between representatives of the Boston District and the Missouri River Division.

(b) Equipment.

The equipment used in the flexible pavement tests is the same as that described in Paragraph 3a(1) (b) above, with the exception that a 24-inch diameter bearing plate was used for repeated loading tests, and a 30-inch diameter bearing plate was used for the single cycle loading tests.

(c) Procedure.

1. Single Cycle Tests.

A 30-inch diameter bearing plate was used for a single cycle loading, the plate being scated on the surface of the pavement

by means of plaster of Paris. Loading increments of 18,000 pounds were applied. Each increment was held constant until the rate of deformation was less than 0.001 inches per minute. Leadings were applied to the maximum of the testing equipment. Deformations were measured by means of extensemeters, mounted on a beam which was in turn supported on reds driven into the subgrade through holes bered in the pavement at a distance of not less than 8 feet from the edge of the bearing plate. Extensemeter readings were obtained prior to leading and at the end-point of each lead increment. Readings were also obtained after removal of the lead, in order to secure permanent deformation data.

2. Repeated Cycle Tests.

the repeated cycle tests were made with a 24-inch diameter bearing plate. The plates were scated on the surface of the pavement and the extensemeters were mounted in the manner described in Paragraph 3a(2) (c) 1 above. A total load of 25,000 pounds was used in the repeated loading tests, the load being rapidly applied in one increment, and then held constant for ten minutes before releasing. The loads were released rapidly and extensemeter readings were then obtained at the end of a ten minute period. The cycle was then repeated, ten cycles of loading and unloading being used in the repeated cycle test. Permanent deformations were determined from extensemeter readings obtained ten minutes after completion of the final cycle of loading.

b. Subgrade Moduli Tosts.

Subgrade moduli tests are made to determine the pavement supporting properties of subgrades or base courses under rigid pavements.

(1) Method.

Subgrade moduli tests were made in accordance with

Paragraph 41, Chapter 20 of the Engineering Manual, titled: "Evaluation of Subgrade Reaction," and as amended by a teletype dated 22 January 1944, from the Office, Chief of Engineers, to the Missouri River Division, a copy of which is included in the letter appendix to this report.

(2) Equipment.

Equipment used to obtain subgrade moduli was similar to the equipment described in Paragraph 3a(1) (b) above except that a 30-inch diameter plate was used for all subgrade moduli tests.

(3) Procedure.

The surface of the pavement supporting medium was carefully leveled and a thin layer of fine, dry sand, all of which passed the 40-mesh sieve, was used to seat the bearing plate. A load equivalent to five (5) pounds per square inch, rapidly applied and instantaneously released, was used to obtain additional scating of the plate before beginning the test. Deformations were measured by two extensemeters bearing on opposite sides of the bearing plate, the extensometers being mounted on a beam supported by rods driven into the subgrade holes bored in the surface of the pavement. The rods supporting the beam were located at a minimum distance of 8 feet from the edge of the plate. Lead increments were applied at the rate of 5 pounds per square inch, each increment being held constant until the increase in deformation for that increment of loading, during a five minute period, was less than 3% of the total deformation for that increment. Loadings were applied until either a total deformation of 0.3 inches was obtained or the capacity of the loading equipment reached. Only single cycle loadings were used to determine subgrade moduli. The lead-deformation data obtained from the field moduli test were used to determine subgrade moduli by means of

the formula $K_u = \frac{P}{0.05}$; where " K_u " is the subgrade modulus and "P" is equal to the pressure in pounds per square inch required to give a vertical deformation of 0.05 inches in the plate load bearing test. The subgrade moduli obtained from field bearing tests were further modified to compensate for the effect of saturation by the methods hereinafter described under consolidation tests.

4. Consolidation Tests.

Consolidation test data are used for the purpose of correcting the subgrade moduli obtained from field tests for the effects of saturation. The tests consist essentially of recording the load-deformation characteristics of an undistrubed specimen at field moisture content, and the loaddeformation characteristics of an undisturbed saturated specimen of the same material. The deformation in the undisturbed unsoaked specimen, produced by a unit loading corresponding to the unit loading used to obtain a defermation of 0.05 inches in the original field test, is used to determine the pressure required to produce a similar deformation in the saturated sample. The unit pressure required to give the required deformation in the saturated sample divided by the unit pressure required to give the same deformation in the undisturbed sample at field moisture content is used as a ratio to modify the field bearing modulus in the formula: $K = K_u \frac{P_s}{P}$, where K is the saturation corrected modulus, Ku is the field modulus, Ps is the saturated specimen unit pressure, and P is the unit pressure required to give a deformation of 0.05 inches in the 30-inch diameter plate bearing test.

a. Mothod.

Consolidation tosts were performed in accordance with Paragraph Lic, Chapter 20 of the Engineering Manual.

b. Equipment.

Tests were conducted in Zanesville type consolidation apparatus. The test specimen was $4\frac{1}{4}$ inches in diameter and $1\frac{1}{4}$ inches thick. The ring assembly was placed in a water tight pan to saturate the specimens. Consolidation was measured by extensometers reading to 0.0001 inch.

c. Procedure.

(1) Consolidation of Unsocked Specimen.

The unscaked specimen was placed under a load corresponding to the pavement load over the sample for a period of 30 minutes. The sample was then consolidated by adding increments of load. Each increment of load was impressed for 30 minutes and the resulting consolidation was recorded. Sufficient increments of loading were added to define a consolidation curve from zero load to a unit load greater than the unit load producing a 0.05 inch deflection in the field bearing test. The initial pavement load was taken as zero load in plotting the consolidation curve.

d. Consolidation of Soaked Specimen.

ment load over the sample, and soaked. Water was then introduced into the specimen from the bottom only, under a head of approximately 2 inches, in order to facilitate the excape of air and to decrease the soaking time. The specimen was allowed to stand until swelling was completed. Swelling was considered completed when the vertical movement was less than 0.001 inches in 30 minutes. Approximately 2 days soaking time was required for typical soils. The thickness of the specimen at the end of the soaking period was taken as the initial condition and the specimen was consolidated in a manner similar to that used for the unsoaked specimen.

5. Concrete Flexural Strength Tests.

Concrete flexural strength tests were made on beams, sawed from slabs removed from the concrete pavement. Concrete flexural strengths were determined to supply data for the calculation of the load carrying capacities of the pavements.

a. Method.

Concrete flexural strength tests were performed in accordance with A.S.T.M. Standard Method C78-39, as amended by Paragraph 6 of the letter dated 10 January 1944, from the Office, Chief of Engineers, to the Division Engineer, Missouri River Division, subject: "Airfield Pavement Evaluation," a copy of which is included in the letter appendix to this report.

b. Equipment.

Concrete flexural strength tests were made in a government laboratory equipped with a diamond saw and a hydraulic compression machine.

c. Procedure.

Beams were sawed to a 6-inch width, the depth corresponding to the original pavement depth, and to a length meeting the requirements of the test. The beams were immersed in water for 24 hours prior to testing. The beams were tested with the original surface in the "up" position.

6. Density Tests.

a. Laboratory Maximum Density Tests.

Laboratory maximum density tests are made for the purpose of determining the maximum densities of soils or aggregate mixtures obtainable under standard compaction conditions in the laboratory.

(1) Method.

Maximum density tests were made in accordance with the

methods given in Paragraph 14, Chapter 20, of the Engineering Manual, and as amended by the letter dated 12 May 1944, from the Office, Chief of Engineers, to the Division Engineer, Missouri River Division, subject: "California Bearing Ratio Procedure," a copy of which is included in the letter appendix to this report.

(2) Equipment.

The equipment used for the determination of the laboratory maximum density, consisted of a 6-inch diameter mold, 5 inches high and fitted with a 2-inch removable extension. The metal tamper weighed 10 pounds, and a tamper drop of 18 inches was used. The sample was compacted in 5 approximately equal layers to a height of approximately 5 inches, with 55 blows per layer.

(3) Procedure.

The test procedures used were those given in the letter referenced in Paragraph 6a(1) above.

b. Field Density Tests.

Field density tests were made to determine the density of "inplace" materials in the field.

(1) Method.

A sand-density method was used to determine the field density of the "in-place" materials.

(2) Equipment.

The equipment used in the test consisted of a glass jar filled with a volume-weight calibrated sand, and fitted with a special funnel and cut-off.

(3) Procedure.

Material was carefully removed from the element to be

tested, forming a hole approximately 4 inches in diameter and 6 inches deep in the upper 6 inches of the pavement element. The material removed from the hole was carefully weighed and the moisture content and weight of dry soil determined. The volume of the hole was determined by weighing the quantity of dry sand required to exactly replace the material excavated, this volume being determined by the prior volume-weight calibration of the sand used. With the weight of dry material removed from the hole, and the volume of hole, as data, the dry density of the element was then calculated.

7. California Bearing Ratio Tests.

California Bearing Ratio Test Data are used for the design of flexible type pavements. The method was originally developed and used by the California State Highway Department. The test methods described herein are based on the original methods and revisions thereto made by the U.S. Engineer Department as hereinafter listed.

a. Laboratory Tests.

(1) Undisturbed Specimens.

(a) Source

Laboratory California Bearing Ratio Tests are mede in accordance with the procedures and methods given in Paragraph 18, Chapter 20, of the Engineering Manual, and by the revised methods set forth in the letter dated 12 May 1944, from the Chief of Engineers to the Division Engineer, Missouri River Division, subject: "California Bearing Ratio Procedure," a copy of which is included in the letter appendix to this report.

(b) Equipment.

The equipment used for the California Bearing Ratio Tests, conforms to the equipment described in Paragraph 18, Chapter 20, of the Engineering Manual.

(c) Procedure.

Cchesive subgrade materials are obtained as undisturbed samples by incasing the "in-place" soil in 6-inch diameter, 7-inch high steel cylinders. The cylinders are provided with α cutting shoe on the lower edge and a compression head at the upper end. A rough pillar of "in-place" soil is first formed by cutting away exterior portions. The steel sampling cylinder, with cutting edge and compression head "in-place," is then placed on the soil pillar and light pressure applied to the cylinder by a hydraulic jack operating against a truck bumper. The soil is cut away from the cylinder edge by a trowel or knife to allow inclosure of the specimen without exertion of large pressures or disturbance of the specimen. After inclosure of the specimen, the ends are trimmed, capped with Masonite disks and sealed with paraffin for shipment to the laboratory. Undisturbed specimens received in the laboratory are uncapped and immersed until saturated. The upper surfaces of the specimens are surcharged during immersion by circular weights, giving pressures calculated to be equal to the unit pressure exerted on the surfaces by the unloaded pavement in the field, or to a minimum unit load of ten (10) pounds total load on the specimen. The bottoms of the immersed specimens rest on perforated plates which also admit water to the lower portions of the specimens. Swell of the immersed specimens is measured by initial, daily and final extensometer readings; cessation of swell generally being used as an indication of saturation. The specimens are tested for bearing ratio after they are completely saturated.

(2) Disturbed Specimens.

Disturbed specimens are recompacted in the laboratory for bearing ratio tests in accordance with the methods and procedures outlined under Paragraph 7a(3) given below.

(3) Compaction Studies.

compaction-bearing ratio studies were mage; to determine the effect of density and various water contents for various compaction on the bearing ratio values of given materials. The studies consist of a series of bearing ratio tests made with materials compacted to various densities at a given water content by varying the work input into the compaction procedure, and other similar series of tests with other moisture contents,

(a) Source.

Compaction studies were performed in accordance with instructions given in the letter dated 12 May 1944, from the Chief of Engineers, to the Division Engineer, Missouri River Division, subject:
"California Bearing Ratio Procedure," a copy of which is included in the letter appendix to this report.

b. Field "In-place" Bearing Ratio Tests.

Field "in-place" bearing ratio tests were made in the field on the freshly exposed base, subbase and various subgrade element surfaces to determine the actual existing bearing ratio of the material "in-place".

(1) Source.

"In-place" field bearing ratio tests were performed in accordance with instructions given in Paragraph 18, Chapter 20, of the Engineering Manual.

(2) Equipment.

The equipment used in performing field "in-place" bearing ratio tests consisted of a 7-ton hydraulic jack, gages for the measurement of the load, the piston movement and a 3-square inch circular vertical piston together with the necessary attachments for use under the rear end of a heavily

loaded truck, A special spring device was used to measure loads of less than 200 pounds.

(3) Procedure.

Loading was applied by means of a hydraulic jack, the loads being applied at a rate giving a rate of penetration to the piston of approximately 0.05 inches per minute. Readings were taken of the loadings giving deformations of 0.025 inches, 0.050 inches, 0.075 inches, 0.100 inches, 0.200 inches, 0.300 inches, 0.400 inches, and 0.500 inches. In accordance with the procedures described in Chapter 20 of the Engineering Manual, the bearing ratio was taken to be the ratio of the pressure required to give a penetration of 0.10 inches divided by 1000.

8. Soil Tests.

a. Liquid Limit.

Liquid limit tests were performed in accordance with the procedures and equipment given in A.S.T.M. Standard Mothod DL23-39.

b. Plastic Limit.

Plastic limit tests were performed in accordance with the procedures and equipment given in A.S.T.M. Standard Method DL24-39.

c. Mechanical Analysis.

(1) Sieve Analysis.

Sieve analysis tests were performed in accordance with the procedures and equipment given in A.S.T.M. Standard Method DL22-39.

(2) Hydrometer Analysis.

Hydrometer analysis tests were performed in accordance with the procedures and equipment given in A.S.T.M. Standard Method DL22-39.

d. Classification of Soils.

Soils were classified in accordance with the method described in Paragraph 11, Part 2, Chapter 20, of the Engineering Manual, and as further described in Exhibit 1, Part 2, Chapter 20, of the Engineering Manual.

LETTER APPENDIX

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APPENDIX E

Part 2

LABORATORY AND FIELD TEST PROCEDURES

CONF WA 500 OCE REFERENCE SPENM 104

FROM ROBINS ACTING CHIEF OF ENGRS WASH DC 221850Z Jan 1944

CONF TO GLD MAD MRD NED NAD ORD PD SAD AND SWD

Reference made to paragraph twenty—four one six four Engineering Manual.

To be certain that practically complete deformation occurs, loading for each increment should remain until deformation change in five minute period is less than three percent of the total change for load increment. Reference made to paragraph 5, letter dated five August 1943, subject: "Airfield Pavement Evaluation." Tests should be made in strict accordance with paragraph. CBR tests on remolded samples of cohesive type soils should be made for correlation purposes only.

END

SPENM 104

Date 5 August 1943

File No, CE SPENO 686.61 (Miss. Riv. Div.)

Office of Origin: War Department, Office, Chief of Engineers

Address: Washington, D. C.

To: The Division Engineer

Missouri River Division

Farm Credit Building

19th & Douglas

Omaha (1), Nebraska

Priority

Routine

Deferred

Precedence

Essential Mil. Mail

Airmail x

Spec. Delivery x

Ordinary

Registered

Message:

- 1. A field survey and an investigation will be made by the Division Engineer at each airfield within the Division and under the jurisdiction of the Army Air Forces to determine the maximum gross weight of airplane that can operate at capacity operation and limited operation at the field without overstressing the pavements. In view of the increasing weights of planes and the necessity for designating fields to be used by certain type of aircraft in the program of the Army Air Forces, it is essential to obtain a record of the evaluation of each field based on the carrying capacities of the pavement as actually constructed. The evaluation of each field will be supplied to the Army Air Forces to serve as a basis for determining the type of aircraft to be assigned thereto. The survey and investigation will be in conformance with the instructions in the following paragraphs.
- 2. Field Evaluation. Each airfield will be evaluated based on the carrying capacities of the controlling pavements or sections of pavements. The evaluations will be stated as the gross weight of the airplanes that can satisfactorily operate at the field and should not be limited to the standard loadings given in the Engineering Manual. In making the over-all evaluation, consideration must be given to the probable frequency of operation on the various parts of the field as such operation may be controlled by the windrose or by other conditions poculiar to the field. For example, let it be assumed that all runways of a particular field are satisfactory for capacity operation of 80,000 pound planes except one runway which is rated as satisfactory for limited operation of such planes only. If, because of the windrose or other local conditions, the weak runway will in fact only be used for limited operation, then the proper over-all rating for capacity operation of the field is 80,000 pounds regardless of the weakness of the one runway. Consideration should also be given to the fact that weak sections of pavements of limited extent need not control the overall evaluation of the field. It may be more economical to accept excessive maintenance or reconstruction of a limited pavement area than to abandon a portion of the investment in the greater part of the field.
- 3. Standards for Evaluation. To evaluate a field will involve the following.

- a. Determine the controlling pavements.
- b. Determine the carrying capacity of the controlling pavements.

To determine the controlling pavements or sections of pavements, the study will include a review of all existing records and data; a review of the construction methods and conditions, and if not available, a complete soil survey of the subgrade conditions, and tests on the subgrades, base courses and paving materials. ...ftor the controlling pavements are determined, they will be evaluated in accordance with the design methods described in Chapter XX of the Engineering Manual. To determine the carrying capacity of the controlling pavements at many airfields will require complete field and laboratory investigations. At some fields, sufficient data has been obtained during design and construction to determine the carrying capacity of the controlling pavements. Complete physical tests as described in paragraphs 4 and 5 of "inplace conditions" will not be required if construction control data, inspection of existing conditions and necessary check tests support the validity of the design assumptions and providing the designs were in accordance with the methods described in the Engineering Manual.

- 4. Evaluation of Flexible Pavements. To evaluate flexible pavements, the California Method of accelerated traffic tests will be used. Accelerated traffic tests as described in Chapter XX, Engineering Manual, should be used only after approval is obtained from the Office; Chief of Engineers. Since the adoption of the California Method of Design, several changes in testing technique of the CBR test procedure have been made. These changes have been incorporated in paragraph 20-18 of Chapter XX, E. M. (Merch 1943). The revised CBR test procedure described in paragraph 20-18 of Chapter XX, will be used to evaluate flexible prements in all cases where tests in addition to those for design must be made in compliance with this Circular Letter. It has come to the attention of this office that certain offices have adopted CBR testing procedures at variance with that prescribed in Chapter XX. Although it is not desired to stifle improvement in the CBR test, and Districts and Divisions are urged to investigate all possible improvements, it is apparent that correlation of pavement performance with the purely empirical CBR procedure can only be accomplished by uniform methods of testing. Changes in CBR test procedure where found advisable as a result of this or other investigations will be referred to the Engineering Division, Office, Chief of Engineer, for investigation with the view to incorporation in Chapter XX, E.M.
- 5. CBR tests should be on remolded samples, undisturbed samples, or on soils in place (see paragraph 20-18, Chapter XX, E.M.) according to the type and moisture condition of the scil. Field tests on cohesive soils in place are considered the most satisfactory if the soil is at or near saturation, but, in all other cases, tests should be made on soaked undistrubed samples. Experience has shown that cohesionless soil will compact under traffic. If the cohesionless soil in place has a unit weight equal to or greater than the density stated in paragraph 20-18, b, 4, Chapter XX, E.M., CBR tests may be conducted with field equipment.
- 6. Evaluation of Rigid Pavements. For the evaluation of concrete pavements, portions of the pavements shall be removed and the modulus of soil reaction for a saturated condition beneath the pavement will be determined by field plate bearing tests in accordance with paragraph 20-41 and 20-45, Chapter XX, E.M. If the 28-day flexural strength of the concrete as placed

and cured in actual construction was not determined by the A.S.T.M. Test C-78-39, beams should be taken from the pavement for tests. The flexural strength at the time of evaluation should be adjusted according to the characteristics of the concrete to account for the increase in strength that has developed after the 28-day period. A Circular Letter will be issued in the immediate future giving data for the determination of the 28-day flexural strength.

- 7. Frost and Other Special Features. Consideration should be given to the reduction in load carrying capacity by reason of probable frost action when determining the carrying capacity of both flexible and rigid pavement.
- 8. Report. A report for each airfield will be prepared, describing in complete detail, the investigation and tests, character of materials involved, method of evaluation, construction features affecting the carrying capacity, and giving test results, justification for selection of controlling pavement conclusions, and all other pertinent information necessary for a complete review of the report. Data and information contained in design analyses and specifications, which are considered pertinent to the evaluations, may be referred to, and these documents or sections of them should be included as appendices to the required report. The report should also include the following plates:
 - a. A historical and data summary sheet similar to Figure 1 attached.
 - b. A plan (18 x 21) of the airfield, summarizing the carrying capacity and showing the paved areas similar to Figure 2 attached.
- 9. Deta Summary Sheet. The data summary sheet (Figure 1) should include a maximum of information on a minimum of space. Notes regarding special conditions of subgrade, type of pavement surface, pavement conditions, traffic history, and any item of special interest will be recorded. An unusual maintenance should be described and recorded on the data summary sheet. All reconstruction and resurfacing are to be recorded on separate lines of the data sheet.
- pavements and show the recommended maximum gross weight of plane that may be used at each airfield. If pavement areas of limited extent (see paragraph 2 above) are not satisfactory for the specified weight of plane, a note will be placed on the plan stating pertinent facts similar to the note on Figure 2. The symbols and colors shown on Figure 2 will be used. If necessary, additional pavement symbols will be used. It is not intended for the pavement symbols to indicate the specific type of pavement but to indicate the general type (such as bituminous treatment, bituminous pavement and concrete pavement). All pertinent dimensions such as length and width of runways, width of major taxiways, width of shoulders, deemed necessary should be shown on the layout plane.
- ll. Distribution of Report. It is requested that four copies of the detailed report for each field, five additional copies of data summary sheet, Figure 1, and six additional copies of the summary plan, Figure 2, be submitted to this office as seen as the report is prepared. Prior to submittal, each report shall be reviewed and approved by the Division Engineer. For the

duration of the war, all reports shall be marked "Restricted". Although copies will be forwarded to this office, one copy of the report shall be marked for the Embankment, Foundation and Pavement Division, U. S. Waterways Experiment Station, Vicksburg, Mississippi, Copies of the summary plan will be submitted to the Army Air Forces by this office.

- 12. Order of Fvaluation. To establish the order in which fields in each Division should be evaluated, it is requested that Division Engineers confer with the Commanding Officers of the Army Air Force Commands. In general, airfields should first be evaluated for which the construction airective specified the design of pavements for planes weighing in excess of 50,000 pounds or fields designed for lesser loads, but believed to be capable of supporting plans weighing more than 50,000 pounds. It is requested that the Division Engineer submit to this office not later than 10 September 1943, a report stating the estimated dates of completion of investigations for each airfield.
- 13. Addenda. It is desired that the reports required herein be maintained current. To this end, addenda to the present report will be submitted as future changes in physical characteristics of the airfield or additional information require.
- 1/4. Work will be initiated from funds locally available. Request for authorization for use of funds will be made to the Chief of Engineers, attention SPEKM, at such time as sufficient data are available to permit an estimate of cost.

By order of the Chief of Engineers:

/s/ JAMES H. STRATTON Colonel, Corps of Engineers Chief, Engineering Division

2 Incls. -

#1 - Historical Data & Record Sheet (Figure No. 1)

#2 - Map - Evaluation of
Typical diffield (Figure No. 2)

10 January 1944

File No. CE SPENM 686.61 (Missouri River Division)

Office of Origin: War Department, Office, Chief of Engineers

Lddress:

Washington, D. C.

To: The Division Engineer
Missouri River Division
Farm Credit Building
19th & Douglas
Omaha (1), Nebraska

Wire or Radio Urgent Priority Routine Deferred Week and Precedence
Essential Mil. Mail
Airmail x
Spec. Delivery x
Ordinary
Registered

Subject: Airfield Pavement Evaluation.

- 1. Reference is made to previous letters of same subject, dated 5 August and 20 August 1043. The evaluation program must be considered as a War Emergency Program and the final evaluation should be submitted with the least possible delay. The 'rmy Air Forces have immediate need for the information in order to properly prosecute the War Training and Transport Program. Every effort should be made to obtain maximum use of all available qualified personnel and suitable testing equipment.
- 2. To obtain the final results as soon aspossible, it is requested that:
- a. Final detailed studies be made only on controlling pavements for runway and all-over field evaluations (see paragraph 3 below). Carrying capacities of pavements other than controlling pavements shown on the "Data Summary Sheet" of the final report should be based on present existing factual data. All values estimated should be noted.
- b. Further tests should not be made at field where existing data or observations definitely show that the carrying capacity of the runways for limited operation is less than a gross load of 20,000 pounds.

Many offices desire to make detailed studies of every pavement at a field. The program should be curtailed as above until the present evaluation program is completed. Data and results of additional detailed studies should be submitted in supplementary reports to the final report if such detailed studies are made.

3. The letters referred to in paragraph I requested that the program be conducted to determine the "field evaluation". In many cases the field evaluation is controlled by a taxiway or apron. Since repairs or reconstruction of a taxiway or apron will not bar operation at the field, it is considered advisable to also submit to the Army Lir Forces the "runway evaluation" which will be controlled by the critical runway. Therefore, it is requested that all reports and summary plans show two evaluations; one as "field evaluation" and the other as "runway evaluation". It is requested that all evaluation values

Letter Appendix

be shown in near the unper right hand corner of the summary plan, in a tubular form as follows:

GROSS WEIGHT OF PLANES IN POUNDS

	Capacity Operation	Limited Operation
Field Evaluation		
Runway Evaluation		

- 4. Recent traffic tests and observations have shown that many flexible pavements will settle due to traffic compaction. In new construction, subgrade and base materials should be compacted by heavy compaction equipment to insure that settlement due to traffic compaction will be less than 1 or 2 inches. However, no criteria limiting the permissible settlement due to traffic compaction will be established for the present evaluation program. The Irmy Air Forces have been advised (see attached letter) that flexible pavements will roughen and settle due to traffic compaction and that maintenance in many cases will be considerable. If, subsequent to the present evaluation program, it is considered desirable to conduct accelerated traffic tests on certain pavements to determine the effect of traffic compaction, requests should be submitted to this office in each particular case.
- 5. Attached is a copy of a letter, subject: "Airfield Pavement Evaluation," dated 5 January 1914, from this office, to the Commanding General, Army Air Forces, defining the terms and meaning of criteria used in the evaluation program. It is requested that the information contained in this letter be forwarded to interested personnel and the definitions stated in paragraph 2 be inserted in final reports. In addition, the final reports should include very detailed description of the procedures of the tests used. Reference to the Engineering Manual is not considered sufficient.
- 6. To obtain comparable flexural strength results, it is requested that concrete beams sawed from pavement slabs be tested after immersion for a period of at least 24 hours and with the wearing face up. The testing machine should be equipped with a swivel edge to prevent torsion of the beam. Beams should be kept moist until tested. Many offices are "capping" the underside of the beam in the area of the knife edges to produce a smooth surface. To compute the flexural strength, the usual formulas, although not strictly valid for non-uniform concrete, should be used.
- 7. The evaluation program affords an excellent opportunity to obtain data regarding the moisture conditions below concrete and flexible pavements. It is suggested that such data be obtained at locations which do not interfere with the evaluation program, and special reports be prepared. For the data to be useful, complete information regarding previous weather conditions, densities, water table, construction history, soil characteristics, etc., is required.

By order of the Chief of Engineers:

/s/ E. R. O'BRIEN
Lt. Colonel, Corps of Engineers
Chief, Troop Frcilities Branch
Military Construction Division

1 Incl.
Cy. 1tr to AF dated 5 Jan. 44

Letter Appendix,

WAR DEPARTMENT Cffice of the Chief of Engineers Washington

CE (12 May ЦД) SPENM

12 May 1944

Subject: California Bearing Ratio Procedure

To: The Division Engineer
Missouri River Division

P. O. Box 1216 Omaha (1) Nebraska

- 1. Reference is made to Paragraph 20-18b, Chapter XX, Engineering Manual, March 1943 revision. It is requested that the following changes be made in the California Bearing Test Procedure:
- Step 2. Conduct all compaction tests in the 6-inch diameter CBR mold. To avoid correcting the density and optimum moisture for stones, the total sample should be used. Soil should not be reused. A sufficient number of specimens should be compacted to definitely establish optimum moisture. Four (4) or five (5) specimens should be compacted with moisture contents with +2% of optimum moisture for all except cohesionless soils and high swelling clays. In the Modified A.A.S.H.O. test, the height of fall of the hammer must be carefully controlled and the blows must be uniformly distributed. The optimum condition must be rigidly established.
- Step 4. When results are required for a soil at 95% Modified A.A. S.H.O. density, compact three (3) specimens at optimum moisture for 100% Modified A.A.S.H.O. compaction using a different number of blows for each specimen, i.e., at 55, 25 and 10 blows per layer. The maximum allowable variation in the molding water content shall be ±0.5% of Modified A.A.S.H.O. optimum moisture. Any specimens not falling within this range shall be discarded and a new specimen compacted that does meet this requirement.
- Step 8. Increase the moisture of the specimen, by immersion, to a maximum that might be obtained in the field (porous openings on top and bottom) for a period of four (4) days. The immersion period for previous soils may be less if it is apparent that maximum moisture content is obtained. If the surface of the specimen becomes so soft by immersion that the test results would be unrepresentative of the sample, this method should not be used but the moisture of the specimen should be increased to a maximum by permitting the water to rise upward through the specimen by capillarity until free water appears on the surface.
- Step 10. ipply a penetration surcharge weight on all soils equal to the soaking surcharge weight, except that the penetration surcharge weight should not exceed 30 pounds.

Letter Appendix

- Step 15. When three (3) specimens are prepared as described above under revision for "Step 4", the results of tests on all samples should be plotted to show the relation between density and CBR. For design purposes, the CBR for 95% Modified A.A.S.H.O. density should be used. (See Fig. 1)
- 2. CBR test results are affected by the density and molding moisture content of the soil specimen. The effects are great for some low plastic soils. It is desired that the variation of test results with molding moisture and density be determined for at least one typical specimen of each soil encountered, except soils which readily compact under traffic. (See Step 4 in Par. 20-18b, E.M.). The following procedure is recommended:
- a. Perform the penetration test, after increasing moisture, on each specimen used to develop the compaction curves for the following compactive efforts; 55, 25 and 10 blows per layer on each of 5 layers using the 10-pound hammer with 18-inch drop and the CBR mold. In many cases, tests on three (3) or four (4) specimens prepared by each compaction effort will be sufficient.
- b. Plot the data from these tests as shown on Figure 2. The above procedure is valuable to obtain test results on soils which are greatly affected by small changes in density and molding moisture content and gives a picture of the CBR characteristics, within the range of the expected field control, which will be useful in establishing the limiting CBR values. The test results, as obtained by the above method, should be used in connection with the design curves with the full understanding that the variations obtained may be only qualitatively valid.
- 3. The above changes will be incorporated in the next revision of Chapter XX of the Engineering Manual. The changes are based on the results of investigations conducted by various laboratories and on information obtained by discussions and conferences in various offices. An extensive investigation of the effect of all factors in the CBR tests and of the method of preparation of specimens for the design test has been made at the U. S. Waterways Experiment Station. A report of this investigation will be prepared for publication in the near future.
- 4. Due to the difficulties of preparing the sample for the design test to simulate the moisture, density, and structure of the prototype, CBR tests during construction should be made on unlisturbed soils, except in the case of those soils which will compact readily under traffic. (See Step 4 in Par. 20-18b, E.M.). If the tests on undisturbed samples do not check the design tests, changes should be made either in the construction methods or design.
- 5. The present CBR test procedure has not proven entirely satisfactory for testing samples containing particles larger than 1/4 inch in size. It had been found necessary to conduct a great number of tests in order to determine the most reasonable value. This method should be followed until a more satisfactory procedure is developed. However, in some cases, inconsistent results can be avoided by removing the stones which do not affect the stability of the soil.

- 6. The procedure for taking undisturbed samples, outlined in Paragraph 20-18c, Engineering Manual, has not been found satisfactory for all soils. Several Districts have adopted the method generally used to obtain undisturbed samples for shear and consolidation tests. In this method a soil pedestal about 6 inches in diameter and 6 inches high is formed by excavating the surrounding material, an expandable sheet metal cylinder about 7 inches in diameter is placed around the pedestal and paraffin or other suitable material is poured around the sample. The ends are sealed with about 1 inch of paraffin. In the laboratory, the paraffin coat and metal cylinder serve as the sample container during the penetration test.
- 7. Test results submitted to this office in the past have not always included complete data. Because of minor differences in laboratory technique employed by different divisions soils laboratories for preparing samples for the design tests, it has been extremely difficult to analyze and correlate these results. During the development of any test, the effect of all factors must be studied, and until these are determined, an entirely satisfactory precedure cannot be established. To aid in the development and to maintain a complete record, it is requested that detailed data and a complete description of test procedure be recorded and submitted with all test results.

By order of the Chief of Engineers:

2 Incls /s/ HIBBERT HILL #1- Fig. 1 (In quint.) Lt. Colonel, Corps of Engineers #2- Fig. 2 (in quint.) Deputy Chief, Engineering & Development Divi

DATA REPORT OF FROST INVESTIGATIONS FISCAL YEARS 1943 - 1949

APPENDIX E

Part 2

LABORATORY AND FIELD TEST PROCEDURES
FOR
CREAT LAKES DIVISION
1944 - 1945

APPENDIX E

Part 2

LABORATORY AND FIELD TEST PROCEDURES

GREAT LAKES DIVISION

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APPENDIX E

Part 2

LABORATORY AND FIELD TEST PROCEDURES

GREAT LAKES DIVISION

1. Introduction. - It is the purpose of this appendix to describe the test methods which were used to obtain the results presented in the Data Report of Frost Investigations, Volume III, for investigations conducted at Truax Field, Madison, Wisconsin, and Selfridge Field, Mt. Clemens, Michigan. The following tests were performed on the pavement, base, subbase, and subgrade materials.

Field Tests:

- (1) Plate Bearing Tests
 - (a) Static Load
 - (b) Repeating Load
- (2) California Bearing Ratio (CBR)
- Water Content
- Densi ty

Laboratory Tests:

- (1) Classification Tests

 - (a) Sieve analysis(b) Hydrometer analysis
 - (c) Atterberg limits
 - (d) Specific gravity
- (2) CBR
- (3) Pavement Tests
 - (a) Extraction of bitumen for bituminous concrete
 - (b) Gradation of aggregate in bituminous concrete
 - (c) Modulus of Rupture for cement concrete
- (4) Soil Compaction Test

Where the procedure followed is a standard of the Engineering Manual or the American Society for Testing Materials (A.S.T.M.), the reference or test number will be given, followed by a description of deviations from the standard, if any.

2. Field Tests.

a. Plate Bearing Test - Static Load .- Engineering Manual, Chapter XX, Paragraph 20-41. In this test, care was taken that loading for each increment remained until deformation change in five minute period was less than 3 per cent of the total change for the load increment. The standard procedure was followed to determine the modulus of soil reaction "k" of

the base material under the cement concrete pavement in Test Area C. The equipment shown in photographs, Plate 186, Volume III, was used, arranged in the manner shown except that the 30-inch diameter plate was placed directly on the base material.

- Application of test to flexible pavement. Pavement bearing tests on bituminous concrete pavements were made in the manner described in the Engineering Manual, except that the 30-inch diameter plate was placed directly on top of the pavement. A thin layer of plaster of paris was used to seat the plate to insure uniform bearing. One arrangement of the apparatus using soil anchors is shown in Figure 1, Plate 186, Volume III. Another arrangement employing a loaded trailer for reaction was also used, and is shown in Figure 2 on same plate.
- (2) Application of Test to corner of rigid pavement slabs.-The standard procedure was used in attempting to determine the load required to fracture corners of cement concrete pavement slabs, with the exception that a plate 24 inches in diameter was used. The plate was seated with a thin layer of plaster of paris on the corner of the slab, one inch from each edge. Three extensometers were placed equidistant around the circumference of the bearing plate and arranged so that one was at a point nearest the corner of the slab. The load was applied in increments of 10 pounds per square inch, except the first which was 20, 25, or 30 pounds per square inch. As the available load was not sufficient to cause failure, the loading was released in decrements of the same magnitude as the corresponding increments. The equipment for this test was assembled as shown in Figure 3, Plate 186, Volume III.
- b. Flate Bearing Test Repeating Load .- The same type and arrangement of testing apparatus as required for the standard Static Load Test described in the Engineering Manual, Chapter XX, Paragraph 20-41 was used. A 24-inch diameter bearing plate was placed on top of the bituminous concrete pavement. To insure uniform hearing, plaster of paris was used to seat the plate. The test was conducted in the following manners A seating load of 3500 pounds was applied for five minutes and released. A load of 20,000 pounds was then applied and the deformation measured. The load was maintuined for ten minutes during which the deformation was measured at the end of the first, fourth, and seventh and tenth minute. The load was then released for a period of five minutes, and the deformation readings taken immediately after release of the loud, and after the first and fifth minutes had elapsed. The foregoing procedure was then repeated until ten repetitions had been made. The results of the test were shown graphically by plotting a continuous graph of the deformation messurements taken at the end of the five minute period under no load and at the end of the ten minute period under 20,000 pound load, the deformations being shown as ordinates and the load as abscissae.

- c. CBR Test. Engineering Manual, Chapter XX, Paragraph 20-18d, "CBR Test on Soils in Place". The standard CBR piston was secured to the base of an 8-ton hydraulic jack equipped with gage having a large dial. The load was applied through a swivel head and adjustable column against a steel beam secured to the back end of a light truck. When the spring tests were made, the size of the annular plate through which the penetration surcharge is applied was increased from 6 inches to 12 inches, outside diameter. At each location in the soil profile where the test was conducted, the water content and density tests were also made.
- d. Water Content. The quantity of water contained in the soil was determined by weighing a small representative sample of the soil (50 to 100 grams) before and after drying it in an oven at a temperature of about 110°C. Four ounce content tins, a balance sensitive to 0.01 gram, and a small electric thermostatically controlled oven was used. When it was not practical to weigh the samples immediately, the tins were scaled with scotch cellulose tape. Stones larger than 3/4-inch in diameter were not included in the samples. The water content was reported as a per cent of the dry weight of the soil.
- e. Donsity of Soil in Place .- The unit weight of the soil was determined by weighing the soil removed from a hole, about 3 inches in dismeter and 3 inches doep, and measuring the volume of the hole from which the soil was dug. The procedure is as follows: A trowel was used to cut away enough of the undisturbed soil to provide a horizontal area four or five inches in diameter. A pan, 24 inches square and 2 inches deep, with a 4-inch diemeter hole cut out of the center, was next placed over the spot proviously prepared. By means of the trowel and spoon (an ordinary table spoon) the soil was removed and placed in the pan. In digging, care was exercised to cut toward the center of the hole and to avoid compressing the sides of the hole. When enough soil had been removed to provide a hole of the desired size, the pan and scil removed was weighed on a scale sensitive to one gram. To find the volume of the soil thus removed, the hole was filled level full with Ottawa sand of known density, and the weight of container plus Ottawa sand before filling the hole and the weight of container plus Ottawa sand after filling the hole. The unit weight of the Ottawa sand was frequently determined by weighing a known volume of the sand which had been poured in the same manner as employed when pouring the sand into the hole in the soil. Stones larger than 3/4-inch in diameter encountered when digging the hole were set uside and returned to the hole as the sand was poured into the hole. A representative portion of the soil removed from the hole wes used for determining the water content as explained in Paragraph 2d above.
- f. Density of Frozen Soil. The density of frozen soil was found by weighing a representative sample and measuring the volume of Ottawa sand displaced by the sample from a container of the sand. The sample weighing about 3 kilograms was broken or cut from a large piece of frozen soil, care being taken to disturb the soil mass as little as possible. The sample was weighed and then placed in a container of known volume, 6 inches in diameter and 8 inches deep, partly filled with Ottawa sand. Ottawa sand was then poured around the sample until the container was full. The container was

then rapped with a hammer 10 blows on each side. A small amount of sand was then added and struck off with a straight edge. The container with sand and sample was weighed on a scale sensitive to one gram. Before each series of density determinations were made, the container was filled with Ottawa sand and rapped in the same manner as described above in order to obtain the density of sand required to fill the known volume. The weight of the sand displaced by the sample was calculated by adding the weight of the soil sample, to the weight of a full container of sand (previously determined) and subtracting from this quantity the weight of the sample and sand filling the container. Knowing the density and weight of the sand displaced, its volume was then computed. A portion of the frozen sample, about 100 grams was used for determining the water content as described in Paragraph 2d above.

3. Laboratory Tests.

- a. Classification Tests. All soil samples used in the classification tests were first air dried, and then dried in an oven at 110°C. The sand and clay samples were divided into fractions by quartering until the desired size of sample necessary for the tests was obtained. Each of these selected samples was then ground in a mortar by means of a pestle until all aggregations had been broken down into particles.
 - (1) Sieve Analysis. The sand-clay-gravel samples were screened on 1 1/2, 3/4, 3/8 inch and No. 4 sieves and the amount retained on each noted. In all screening operations, all lumps of soil were broken down. The material passing the No. 4 screen was quartered until desired size of sample required for the remaining tests was obtained. Each sample was split into two portions which were sieved on a No. 10 screen. The material passing the No. 10 screen was ground in a mortar to break down all remaining lumps. The percentages of soil particles passing screens having openings of 0.84, 0.43, 0.25, 0.147, and 0.074 millimeters was obtained by following the standard A.S.T.M. test DL22-39. The test was performed on both portions of each sample. Sieve analyses of two portions of each sand and clay sample were also made according to the standard test.
 - Hydrometer Analysis. A.S.T.M. DL22-39. Two tests were performed on each sample of soil submitted to the laboratory. In the case of samples taken from test areas B and C, one test was run using 20 cc of L% sodium silicate solution, specified in the standard procedure, and in the other test 5 cc of the same solution was added. Apparently, as complete dispersion was obtained in one test as in the other, as no appreciable difference between the results of the two tests could be noted. In reporting the final results, a mean of the two tests was taken. Since no constant temperature bath was available, the temperatures were noted and corrections for temperature applied.

- (3) Atterberg Limits. A.S.T.M. DL23-39 and DL24-39. Each value reported is the average of at least two tests.
- (4) Specific Gravity. This test was performed by placing between 10 and 20 grams of over-dried soil, passing the No. 10 sieve, in a pycnometer. After weighing and adding some distilled water to the sample, it was de-aerated. The pycnometer was then completely filled with water and its weight and the temperature of the water were recorded. The specific gravity was calculated by dividing the weight of the soil in grams by the volume of the soil in cc. The values reported are a mean of at least two tests.
- b. CBR Test.- The procedure for this test is that given in the Engineering Manual, Chapter XX, Paragraph 20-18, as revised by letter from 0. C. of E. dated 12 May 1944 (CE-12 May 44-SPENM). A 12 pound soaking surcharge was used on granular soil samples and a 24 pound surcharge on clay. The penetration surcharge was 24 pounds. Stone not passing the 3/4-inch sieve was replaced with similar stone passing the 3/4 inch sieve and retained on a 1/4 inch sieve. The water content of each compacted specimen containing gravel depends on the mean result of tests on four 75-gram samples. When compacting sand and clay specimens, the mean of two tests for water content was used.

c. Pavement Tests.

- (1) Extraction and Recovery of Bitumen. A.S.T.M. D762-LLT.
- (2) Gradation of Aggregate (extracted from bituminous concrete).A.S.T.M. C 136-39
- (3) Modulus of Rupture for Cement Concrete .- A.S.T.M. C 78-39.
- d. Compaction Test. Engineering Manual, Chapter XX, Paragraph 20-14a (1). This test, called the "Modified AASHO Compaction Test," is performed in the same manner as that portion of the CBR test procedure specified for determination of maximum density and optimum water content. The soil was compacted into a standard CBR mold in five layers, each approximately one inch in thickness under fifty-five blows of the 10 pound hammer dropped from a height of eighteen inches.

DATA REPORT OF FROST INVESTIGATIONS FISCAL YEARS 1943 - 1949

APPENDIX E

Part 3

LABORATORY AND FIELD TEST PROCEDURES
FOR
NEW ENGLAND DIVISION
1944 - 1945

APPENDIX E

Part 3

LABORATORY AND FIELD TEST PROCEDURES

NEW ENGLAND DIVISION

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APPENDIX E

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NEW ENGLAND DIVISION

1. <u>Introduction</u>. It is the purpose of this appendix to describe the test methods which were used to obtain the results presented in the Report on Frost Investigation where applicable at Dow Field, Bangor, Maine, Presque Isle Airfield, Presque Isle, Maine, Otis Field, Sandwich, Mass., and Houlton Airfield, Houlton, Maine. The following tests were performed:

Field Tests:

- (1) Plate Bearing Tests
 - (a) Static Load
 - (b) Repeating Load
- (2) California Bearing Ratio (CBR)
- (3) Water Content
- (4) Density (Unit dry weight)

Laboratory Tests:

- (1) Classification Tests
 - (a) Sieve analysis
 - (b) Hydrometer analysis
 - (c) Atterberg limits
 - (d) Specific gravity
- (2) California Bearing Ratio (CBR)
- (3) Pavement Tests
 - (a) Extraction of bitumen for bituminous concrete
 - (b) Gradation of aggregate in bituminous concrete
 - (c) Modulus of Rupture for cement concrete
 - (d) Compressive Strength
- (4) Soil Compaction Test (Moisture Density)

Where the procedure followed is a standard of the Engineering Manual or the American Society for Testing Materials (A.S.T.M.), the reference or test number will be given, followed by a description of deviations from the standard, if any.

2. Field Tests.

a. Plate Bearing Test - Static Load. - Engineering Manual, Chapter XX, paragraph 20-41. The load for each increment was maintained constant until deformation change in five minute period was less than 3 percent of the total change for the load increment. For each increment of load, deflection readings were taken at time intervals 1/4, 1, 3, 5, 7, 9, 10, 12, 14, 15, 17, 19, 20, 22, 24, 25, 27, 29 and 30 minutes. The equipment illustrated in Photographs on Plate 3E-1 in this appendix.

- (1) Subgrade Modulus Test. Tests were performed on top of the gravel base directly beneath the rigid pavement using a 30-inch diameter plate as outlined above.
- Application of test to flexible pavement. Pavement bearing tests on bituminous concrete pavement were made in the manner described in the Engineering Manual, except that the 30-inch diameter plate was placed directly on top of the pavement. A thin layer of sand was used to seat the plate to insure uniform bearing.
- Application of test to corner of rigid pavement slab.—
 The standard procedure was used to determine the load required to fracture corners of cement concrete pavement slabs, with the exception that a plate 24 inches in diameter was used. The plate was scated on a thin layer of sand at the corner of the slab. The edge of the plate was 3 inches from the slab edges. Two extensometers were placed in a line bisecting the right angle formed by the pavement joints. The load was applied in increments of 20, 30, 35, 40, 45, 50, 55 and 60 thousand pounds. If the available load was not sufficient to cause failure, the loading was released in one decrement and reloaded by increments to the maximum total load. This procedure was repeated until rupture occurred or for a total of 5 repetitions.
- b. Plate Bearing Test Repeating Load .- The same type and arrangement of testing apparatus as required for the static load test described in paragraph 2a above was used. A 24-inch diameter bearing plate was placed on top of the bituminous pavement. To insure uniform bearing, a thin layer of sand was used to seat the plate. The test was conducted in the following manner: A seating load of 3500 pounds was applied for five minutes and released. A load of 20,000 pounds was then rapidly applied in one increment. The load was maintained for ten minutes during which the deformation was measured at the end 1/4, 1, $2\frac{1}{4}$, $6\frac{1}{4}$, and 10 minutes. The load was rapidly released and deformation readings taken at the end of a 5-minute period. The foregoing procedure was then repeated until ten load repetitions had been made. The results of the test were shown graphically by plotting a continuous graph of the deformation measurements taken at the end of the five minute period under no load and at the end of the ten minute period under 20,000 pound load, the deformations being shown as ordinates and the load as abscissae.
- C. Field-in-Place CBR Test. The method described in Engineering Manual, Chapter XX, paragraph 20-18d, "CBR Test on Soils in Place" was used. The standard CBR test head was secured to the piston of a 10-ton hydraulic jack equipped with 3 pressure gages for low, medium and high pressure. The reaction was furnished by two 6-foot lengths of 80-lb. steel rail bolted together and weighted by the front wheels of a $2\frac{1}{2}$ ton truck. The surcharge weights consisted of thin annular lead plates 6 inches in diameter with 2-inch diameter hole in center. At each location in the soil

profile where a Field CBR test was conducted, water content and density of the soil were also determined.

- d. Water Content. The quantity of water contained in the soil was determined by weighing a small representative sample of the soil (200 to 300 grams) before and after drying it in an oven at a temperature of about 110°C. Eight ounce ointment tins, a scale sensitive to 0.01 gram and an electric oven were used. Stones larger than \(\frac{1}{4} \) inch in diameter were not included in the samples. The water content was reported as a percent of the dry weight of the soil.
- e. Density of Soil in Place .- Density of a soil is defined as its unit dry weight. The density of the soil was determined by weighing the soil removed from a hole, 6 to 8 inches in diameter and 6 to 8 inches deep, and measuring the volume of the hole from which the soil was removed. The procedure used is as follows: The surface of the undisturbed soil was leveled using a trowel to provide a horizontal area about 12 inches in diameter. By means of a trowel and spoon, a hole was excavated and the soil was removed and placed in gallon size cardboard cartons. In digging, care was exercised to out toward the center of the hole and to avoid compressing or loosening the sides of the hole. When enough soil had been removed to provide a hole of the desired size, the soil removed was weighed on a scale sensitive to 0.01 gram. The volume of the soil thus removed, was determined from the weight of beach sand of known density required to completely fill the hole. The density of the beach sand was frequently determined by weighing a known volume of the sand which had been poured in the same manner as employed when pouring the sand into the hole in the soil. A representative portion of the soil removed from the hole was used for determining the water content using the procedure described in paragraph 2d above.

3. Laboratory Tests

- a. Sieve Analysis. Coarse material was separated from fine on 1/4-inch sieve. Material passing 1/4-inch sieve was quartered to 100 grams and washed on No. 100 (Tyler) sieve using not more than 1000 C.C. of water. The portion retained in wash water was used for hydrometer analysis. Material 1/4-inch to No. 100 (Tyler) was dried and sieved in mechanical shaker. Material retained on 1/4-inch sieve was sieved by hand. The complete sieve analysis was computed and plotted.
- b. Hydrometer Analysis. The dry weight of sample contained in wash water described in above Paragraph 3 (a) was determined by the pycnometer method. The total dry weight was computed and grain size and percent finer of material passing No. 100 mesh sieve was determined as described in "Notes on Soil Testing for Engineering Purposes" by A. Casagrande and R.E. Fadum, Harvard University, 1940.
- c. Atterberg Limits. Procedure described in "Notes on Soil Testing for Engineering purposes" by A. Casagrande and R.E. Fadum, Harvard University, 1940 was followed. Each value reported is

the average of at least two tests;

d. Specific Gravity .-

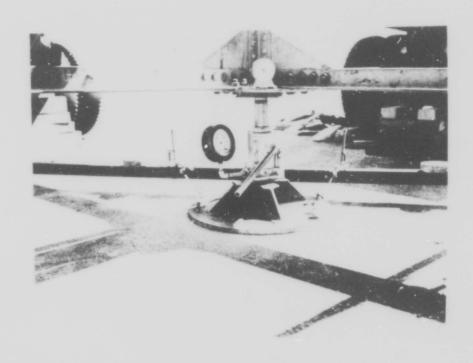
- (1) Material Passing 1/4" Screen. Approximately one hundred grams of material were placed in calibrated volumetric flask approximately two-thirds full of water and the mixture boiled for ten minutes to remove air. The flask was cooled and water was added to bring contents to calibration mark. Outside of the volumetric flask and the inside of the neck was thoroughly dried and weighted to .01 gram and temperature determined. Weight of bottle and water was determined from calibration curve. Dry weight of soil determined and specific gravity computed.
- (2) <u>Material Retained on 1/4" Screen.</u> The specific gravity was determined in accordance with A.S.T.M. Designation C-127-42.
- e. CBR Test.- The procedure for this test is that given in the Engineering Manual, Chapter XX, paragraph 20-18, as revised by letter from 0.C. of E. dated 12 May 1944 (CE-12 May 44-SPENM). Weight of surcharge during soaking and penetration was equivalent to the weight of overlying materials in situ, except that during penetration the surcharge weight was not greater than 30 lbs. or less than 10 lbs. Stone not passing the 3/4-inch sieve was replaced with similar stone passing the 3/4-inch sieve and retained on a 1/4 inch sieve. The water content of each compacted specimen is the average determined in the bottom, center and top.

f. Pavement Tests .-

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- (3) Modulus of Rupture for Cement Concrete. A.S.T.M. C 78-39.
- (4) Compressive strength of modified cubes tested in accordance with A.S.T.M. Designation C 116-39 and cores in accordance with A.S.T.M. C 39-42.
- g. Soil Compaction Test. The procedure is described in Engineering Manual, Chapter XX, paragraph 20-14a (1). This test, called the "Modified AASHO Compaction Test", is performed in the same manner as that portion of the CBR test procedure specified for determination of maximum density and optimum water content. The soil was compacted into a standard CBR mold in five layers, each approximately one inch in thickness under fifty-five blows of the 10 pound rammer dropped from a height of eighteen inches.



General View of Pavement Bearing Test



Close Up View of Pavement Bearing Test

NEW ENGIAND DIVISION CORPS OF ENGINEERS, U. S. ARMY BOSTON, MASSACHUSETTS

DATA REPORT OF FROST INVESTIGATIONS FISCAL YEARS 1943 - 1949

APPENDIX F

BIBLICGRAPHY (REVISED JUNE 1949)

FROST EFFECTS LABORATORY

JUNE 1949

FOREWORD

The following bibliography contains references to published articles on subject matter related to frost phenomena. It is limited only to those articles which deal with seasonal frost and its effect on soils. Publications or articles dealing with permanently frozen ground or "Permafrost" are not within the scope of this bibliography.

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