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## Methods in Structural Dynamics

for

### Thin Shell Clustered Launch Vehicles

TECHNICAL DOCUMENTARY REPORT NO. FIL-TIB-64-105

APRIL 1965

J. S. Keith, et al LTV Astronautics





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## Methods in Structural Dynamics for Thin Shell Clustered Launch Vehicles

. TECHNICAL DOCUMENTARY REPORT NO. FDL-TDR-64-105

APRIL 1965

J. S. Keith, et al LTV Astronautics

AIR FORCE FLIGHT EYNAMICE LABORATOFY RESEARCH AND TECHNOLOGY DIVISION AIR FORCE SYSTEMS COMMUNI WRIGHT-PATTERSCH AIR FORCE BAGE, OHIO

#### FOREWORD

This report covers research conducted by the LTV Astronautics Division, Ling-Temco-Vought, Inc., Dallas, Texas, for the Aerospace Dynamics Branch, Vehicle Dynamics Division, AF Flight Dynamics Laboratory, Wright-Patterson Air Force Base, Ohio, under Contract No. AF33(657)-9146. This work was performed to advance the dynamic loads state of the art for flight vehicles as part of the Research and Technology Division, Air Force Systems Command's exploratory development program. The research was conducted under Project No. 1370, "Dynamic Broblems in Flight Vehicles," and Task No. 137008, "Prediction and Prevention of Dynamic Load Problems". Mr. Lynn C. Rogers and Later Mr. T. D. Lemley of the Vehicle Dynamics Division, AF Flight Dynamics Laboratory were the project engineers.

The project engineer for LTV Astronautics was Mr. J. Stuart Keith. The principal authors were Mr. J. S. Keith and Mr. J. W. Lincoln; Mrs. Susan P. Shrader was responsible for coding the computer program in Appendix I. Mrs. Shrader was also the author of that part of the report. Mr. J. D. Chaney was responsible for the vibration analysis of the Saturn vehicle and he was the author of Appendix II which describes this analysis.

The cooperation of the NASA at Langley Field, Virginia, is gratefully acknowledged. LTV Astronautics is especially grateful to NASA, Langley for making available a complete set of blueprints for the 1/5 scale model of the Saturn vehicle and in particular for the close cooperation of Mr. Homer Morgan and Mr. John Mixson of the Dynamic Loads Division.

This is the final report on Contract AF33(657)-9146.

This report has been reviewed and is approved.

WALTER J. WYKYTOW

WALTER, J. WYKYTOW Asst. for Research & Technology Vehicle Dynamics Division AF Flight Dynamics Laboratory

#### ABSTRACT

A general methodology in Structural Dynamics based on the use of generalized coordinates is presented. These general methods are demonstrated by analysis of some of the problems of slender, conventional, launch vehicles. Applications of the general methodology are also given for complex configurations employing thin shell tanks in clustered arrangements. The methods of structural analysis and vibration analysis that are presented are not restricted to any particular geometry and apply to any complex redundantly coupled structure.

To further demonstrate the general methods, a vibration analysis of the clustered Saturn I launch vehicle is presented.

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1.0 INTRODUCTION

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#### 1.1 THE NATURE AND PURPOSE OF THIS REPORT

The purpose of this report is to provide a consistent methodology for solving structural dynamics problems associated with the design and operation of large, clustered, launch vehicles. The methods are motivated by the need to obtain:

- o Structural load design criteria
- o Control system design criteria
- o Operational capability and performance boundaries for boosted flight through the atmosphere.

The emphasis has been more on the methodology than on the selection of detailed design criteria. The attempt to establish rational design criteria, however, has been the guiding motivation for the methods that are documented herein.

This report was written with the intention of fully documenting a specific methodology which is in common use in the aerospace industry, but which has received limited treatment in the published literature. To be specific, the methodology referred to could be called "a finite degree-of-freedom approach to structural dynamics." The major part of this report is devoted to the detailed development of this general methodology. In subsequent sections of the report this methodology is demonstrated to be applicable to the launch vehicle dynamics problems associated with complex clustered configurations. Sufficient information is given in these latter sections to show that the methods of this report are general enough to cover the structural dynamics problems associated with non-beam-like launch vehicles (such as fitan III C and Saturn I B).

An attempt has leen made in writing this report to show the extensive generality of the methods. This motive influenced the arrangement of the subject material. The following steps were taken to achieve this:

- (1) Section 2.0 provides a complete development of the methods from basic principles of mechanics for a continuum.
- (2) Section 3.0 demonstrates the applications of these methods to conventional, slender, launch vehicles.
- (3) Section 4.1 demonstrates the applications of the methods of Section 2.0 to an arbitrary deformable body.
- (4) Section 4.2 specializes the development in Section
   4.1 to an "arbitrary" launch vehicle in boosted flight through the atmosphere.

- (5) Section 5.0 considers in detail some aspects of structural dynamics concerned with thin-wall tanks and clustered arrangements.
- (6) Finally, Appendix II provides an example numerical analysis of a specific clustered configuration to further demonstrate the general application of the methods, and to clarify the discussion in Section 5.0.

The example analysis in Appendix II is a detailed documentation of a vibration analysis. The configuration chosen is that of the Saturn I launch vehicle. The vibration modes and frequencies are calculated for data corresponding to the NASA Langley one-fifth scale structural model of the first Saturn vehicle (serial designation SA-1) that was launched in October, 1961.

In addition to the Saturn analysis, several numerical examples are given where data have been conveniently available. The examples in Section 3.0 are primarily based on data for the NASA Scout solid-propellant launch vehicle. Practical limitations have made it necessary to omit additional numerical examples.

#### 1.2 HISTORICAL INTRODUCTION TO THE METHODS OF THES REPORT

The methods of this report are not novel, nor are they revolutionary or new to the aerospace industry. They have been used under many different names. Some typical examples are:

- o matrix method
- o generalized coordinate approach
- o collocation method
- o energy method
- o Lagrangian approach
- o Rayleigh-Ritz method
- o modal method

This report attempts to put these methods in order and show how they are all generated from the same basic notions. Since the scope of this report is necessarily restricted to launch vehicle dynamics this purpose is limited and the need still exists for a broader synthesis of these basic methods in structural dynamics.

It has been suggested in Faragraph 1.1 that we call this general method a "finite degree-of-freedom approach to structural dynamics," a name which must suffice until we can indicate more specifically what is involved.

The methods of this report rely completely on the principles of Analytical Mechanics; namely, the Principle of Virtual Work and its extension to dynamics by D'Alembert's Principle. These principles were used by Lagrange to develop Analytical Mechanics by use of his method of generalized coordinates. Lagrange's boast was that his methods were independent of geometry and he took pride in noting that his treatise, <u>Mécanique Analytique</u>, did not contain a single figure. It seems remote, but it is this characteristic of Lagrange's method which makes it useful in structural dynamics. The usefulness stems from the fact that equations of motion are developed which apply to any configuration because they are essentially independent of the geometry of any particular configuration.

The engineering discipline which is currently called structural dynamics and aeroelasticity dates back to the beginning of powered flight and received impetus with the first mathematical analysis of the flutter mechanism by Theodorsen and Garrick in 1934. During this period of development, Lagrange's approach has been used repeatedly with success in vibration and aeroelastic problems. The British, however, have tended to use the method more faithfully than it has been used in this country.

The extensive use of the Lagrangian method at LTV Astronautics is principally due to S. J. Loring. As early as 1936, Loring had developed the use of the method in a general finite degree-of-freedom approach utilizing matrices. Loring came to Chance Vought Aircraft<sup>1</sup> in 1936 and exerted a significant influence on the structures capability of that company in the period from 1936 to 1948. Due to the obscurity of his publications<sup>2</sup>, Loring's work did not influence the general trends in aeroelasticity during that period. Outside of Chance Vought, the subject evolved independently from the following sources:

- NASA, Langley Field, Virginia, due to the contributions of T. Theodorsen and I. E. Garrick: NACA TR No. 496, General Theory of Aerodynamic Instability and the Mechanism of Flutter, 1934.
- Wright Field, Dayton, Ohio, due to the basic approach documented by Smilg and Wasserman: Air Force Technical Report 4798, Application of Three-Dimensional Flutter Theory to Aircraft Structures, 1942. (The existing military specifications on aeroelastic problems have been largely influenced by this document.)
- o Massachusetts Institute of Technology, due to the contributions of R. L. Bisplinghoff, H. Ashley, G. Zartarian, R. L. Halfman and others in the Aeronautical Department and in the Aeroelastic and Structures Research Laboratory.

<sup>&</sup>lt;sup>1</sup> Chance Vought is a parent organization of LIV Astronautics, a division of Ling-Temco-Vought, Inc.

<sup>&</sup>lt;sup>2</sup> For example, Loring's publications in the SAE Journal are not included in the extensive bibliography of the book <u>Aercelasticity</u> (Addison-Wesley, 1955) by Bisplin noff, Ashley and Halfman.

 California Institute of Technology, due to the contributions of Y. C. Fung: Elastostatic and Aeroelastic Problems Relating to Thin Wings of High-Speed Airplanes, (Ph.D. Thesis, 1943) and Theory of Aeroelasticity (Wiley, 1955).

The methods of this report do not reflect directly the extensive and significant contributions of the above sources, but are almost entirely devoted to the independent contributions of Loring and the subsequent development of his ideas by M. J. Turner, Dr. W. W. Soroka, S. Rabinowitz, Dr. Conrad C. Wan, J. E. Stevens, R. Simon, Dr. H. A. Wood, Dr. Ta C. H. Li, Dr. J. K. Haviland, A. L. Head, Jr., and others who have been connected with Chance Vought over the past twenty years. In spite of the obscurity of his publications<sup>1</sup>, Loring's work has, nevertheless, influenced the industry through the engineers which have left Chance Vought and gone to other companies.

<sup>1 3.</sup> J. Loring, General Approach to the Flutter Froblem. SAN Journal (Transactions) volume 49, No. 2, January 1, 1940 and Une of Generalized Coordinates in Flutter Analysis, SAN Journal, volume 54, No. 4, April 1944.

2.0 INTRODUCTION TO METHODS IN DYNAMICS FOR FINITE DEGREE-OF-FREEDOM MECHANICAL SYSTEMS •

#### 2.1 PRINCIPLES OF ANALYTICAL MECHANICS FOR CONTINUOUS FINITE DEGREE-OF-FREEDOM SYSTEMS

#### 2.1.1 An Introduction to Continuum Mechanics

In dealing with the motion of a flexible launch vehicle interacting with its fuel and the surrounding atmosphere there are numerous opportunities to call upon fundamental principles related to the dynamics of a continuum of mass particles. For this reason we want to review these principles, in this section. in a form and notation which will clarify discussions in subsequent sections.<sup>1</sup> Most of the conceptual difficulties with the dynamics of "open" systems (such as a vehicle losing mass) arise because care is not exercised in analyzing the detailed motions of the particles of the system. It is felt that these diffficulties and others may be avoided by a preliminary consideration of the equations of continuum mechanics.

#### 2.1.1.1 The Eulerian and Lagrangian Coordinate Descriptions

Consider an arbitrary, finite portion of a continuum of particles at a time, say, t = 0.



FIGURE T A PORTION OF THE CONTINUUM

Let I, J, and K be a set of inertial unit base vectors directed along the axes of a rectangular coordinate system, (x,y,z) whose origin is fixed. At t = 0 let the arbitrary surface bounding this set of particles be given in

For a more complete discussion, reference should be made to Green and Zerna, Theoretical Elasticity, Oxford, 1954, or Truesdell, Principles of Continuum Mechanics, Colloquim Lectures in Pure and Applied Science No. 5 Socony Mobil Oil Co. Field Research Laboratory, Dallas, Texas, February 1960.

the implicit form

$$f(x, y, z) = 0 \tag{2-1}$$

We may "tag" or give an identity to each of the continuum of particles by associating that particle with the coordinates, (x,y,z), of the point occupied by the particle at t = 0. Even though the particle is displaced from this point in subsequent times, we continue to call it the "x-y-z particle." A set of coordinates, such as these, which label particles are termed "Iagrangian coordinates."

The particles which, at time t = 0, have coordinates,  $(x_xy_yz)_x$  are continuously displaced to new positions which have coordinates, say,  $(\xi_x \eta_x \zeta)$ referred to the original inertial reference frame. The kinematics of the motion is completely described by the set of functions

$$\begin{split} s &= s(x, q, z, t) \\ \eta &= \eta(x, q, z, t) \\ \zeta &= \zeta(x, q, z, t) \end{split}$$

$$(2-2)$$

These equations give the coordinates of a point which, at time t, is occupied by the x-y-z particle. Because of our definitions of x, y, and z, these equations satisfy the peculiar relations,

$$\begin{aligned} x &= f(x, y, z, 0) \\ y &= \eta(x, y, z, 0) \\ z &= f(x, y, z, 0) \end{aligned}$$
(2-3)

In a concise manner we have the position vector,  $\mathcal{J}(x,y,z,t)$ , of the x-y-z particle at time, t, given by

$$I(x, y, z, t) = S(x, y, z, t)I + \gamma(x, y, z, t)I + J(x, y, z, t)K$$
(2-4)

The coordinates,  $(\xi, \eta, \zeta)$ , are termed "Eulerian coordinates" for the particles.

The velocity of the x-y-z particle is defined by

$$\Psi(x,q,z,t) = \frac{\partial \Pi}{\partial t}(x,q,z,t) \qquad (2-5)$$

and likewise the acceleration of the x-y-z particle is defined by

$$\Omega_{t}(x,y_{\tau}z,t) = \frac{\partial^{2}\Pi}{\partial t^{2}}(x,y_{\tau}z,t) \qquad (2-6)$$

The Eulerian and Lagrangian coordinate descriptions are summarized in Figure 2.



FIGURE 2 EULERIAN AND LAGRANGIAN COORDINATES.

#### 2.1.1.2 The Equations of Continuity and Momentum

Let the mass per unit of volume in the neighborhood of the point (x,y,z) at time, t = 0, be denoted by

$$\rho = \rho(\mathbf{x}, \mathbf{y}, \mathbf{z}) \tag{2-7}$$

The total mass of the arbitrary portion inside the surface, f(x,y,z) = 0, which bounds this fixed set of particles is given by<sup>1</sup>

$$\int_{f(x,y,z)=0} e(x,y,z) dV$$
 (2-8)

The conservation of mass in nonrelativistic continuum mechanics is expressed by the trivial relation

$$\frac{d}{dt} \int \varrho \, dV = 0 \tag{2-9}$$

$$f(x,y,z) = 0$$

The conservation of momentum is similar and is derived from a form of Newton's second law in classical mechanics. The problem, however, is not conceptually straightforward because of the nondifferentiable nature of the forces on a mass-point in a continuum. Invoking Newton's second law, we have

$$\int \frac{\partial^2 m}{\partial t^2}(x, q, z, t) g(x, q, z) dV = \int dF(x, q, z, t)$$
(2-10)

where the integral on the right should be interpreted as a generalization of the Stieltjes definition of an integral. Upon integrating over the fixed set of particles inside f(x,y,z) = 0 at t = 0, we have

. .

$$\int \frac{\partial^2 \mathbf{m}}{\partial t^2} e^{d\mathbf{V}} = \mathbf{F}$$
(2-11)  
$$f(\mathbf{x}, \mathbf{u}, \mathbf{z}) = 0$$

The total force,  $\mathbb{F}$ , on this finite set of particles is given by contributions from a surface and a volume integral

$$\mathbf{F} = \oiint \sum \cdot dS + \int \mathbf{P} \, dV \qquad (2-12)$$

$$\int (dV) dV dx dy dz$$
 () dx dy dz

where  $\sum$  is the stress dyadic<sup>1</sup> and  $\mathbb{P}$  is the so-called body-force per unit of volume.



FIGURE 3 SURFACE AND BODY FORCES

The identity,

$$\frac{d}{dt} \int \left( \frac{\partial \Pi}{\partial t} dV = \int \frac{\partial^2 \Pi}{\partial t^2} e^{dV} \right)$$

$$f(x,y,z) = 0 \qquad f(x,y,z) = 0 \qquad (2-13)$$

is true because our consideration of a <u>fixed</u> set of particles makes the limits of integration independent of time. Using this, along with the definition of velocity (Equation 2-5), in Equation 2-11 yields

$$\frac{d}{dt} \int \varrho V dV = \bigoplus \sum dS + \int P dV \qquad (2-14)$$

$$f(x,y,z) = 0$$

Equations 2-9 and 2-14 express the principles of the conservation of mass and momentum in terms of Lagrangian coordinates. Our aim will be to transform these to expressions in Eulerian coordinates; but, before this, we want to introduce the Eulerian notion of mass density. The relations which give the "label," (x,y,z), of a particle which at time, t, is at the point ( $\xi$ ,  $\eta$ ,  $\zeta$ ) are given by the inverse of Equations 2-2

$$\begin{aligned} x &= x (s, \eta, s, t) \\ y &= y (s, \eta, s, t) \\ z &= z (s, \eta, s, t) \end{aligned}$$
 (2-15)

<sup>1</sup>See Constant, Theoretical Physics, Addison-Wesley, 1954 p. 40 and p. 201.

If we use these equations to make a change of variable in Equation 2-9, we have

$$\int_{f(x_{4},z)=0} g(x_{1}(x_{1},y_{1},z_{1},z_{1}),y_{1}(z_{1},y_{1},z_{1},z_{1}),z_{1}(z_{1},y_{1},z_{1})) \frac{f(x_{1},y_{1},z_{1})}{f(x_{4},z)=0} \frac{f(x_{1},y_{1},z_{1},z_{1}),y_{1}(z_{1},y_{1},z_{1},z_{1})}{f(x_{4},z)=0} \frac{f(x_{1},y_{1},z_{1},z_{1}),y_{1}(z_{1},y_{1},z_{1},z_{1})}{f(x_{4},z)=0} \frac{f(x_{1},y_{1},z_{1},z_{1}),y_{1}(z_{1},y_{1},z_{1},z_{1})}{f(x_{4},z)=0} \frac{f(x_{1},y_{1},z_{1},z_{1}),y_{1}(z_{1},y_{1},z_{1},z_{1})}{f(x_{4},z)=0} \frac{f(x_{1},y_{1},z_{1},z_{1}),y_{1}(z_{1},y_{1},z_{1},z_{1})}{f(x_{4},z)=0} \frac{f(x_{1},y_{1},z_{1}),y_{1}(z_{1},y_{1},z_{1},z_{1})}{f(x_{4},z)} \frac{f(x_{1},y_{1},z_{1},z_{1}),y_{1}(z_{1},y_{1},z_{1},z_{1})}{f(x_{4},y_{1},z_{1},z_{1}),y_{1}(z_{1},y_{1},z_{1},z_{1})} \frac{f(x_{1},y_{1},z_{1})}{f(x_{4},y_{1},z_{1})} \frac{f(x_{1},y_{1},z_{1})}{f(x_{1},y_{1},z_{1})} \frac{f(x_{1},y_{1},z_{1})}{f(x_{1},y_{1},z_{1})} \frac{f(x_{1},y_{1},z_{1})}{f(x_{1},y_{1},z_{1})} \frac{f(x_{1},y_{1},z_{1})}{f(x_{1},y_{1},z_{1})} \frac{f(x_{1},y_{1},z_{1})}{f(x_{1},y_{1},z_{1})} \frac{f(x_{1},y_{1},z_{1})}{f(x_{1},y_{1},z_{1})} \frac{f(x_{1},y_{1},z_{1})}{f(x_{1},y_{1},z_{1})} \frac{f(x_{1},y_{1},z_{1})}{f(x_{1},z_{1})} \frac{f(x_{1}$$

 $\frac{\partial(x,y,z)}{\partial(x,\gamma,z)}$  is the Jacobian associated with the transformation expressed by Equations 2-15. The nature of the Jacobian plays an important part in the Eulerian notion of density. The Jacobian is the ratio of the volume element, dxdydz, to the "deformed" volume element,  $d\xi d\eta d\zeta$ . From this it follows that

$$\psi(x, y, z) \frac{\partial(x, y, z)}{\partial(z, y, s)}$$

has the physical interpretation of the mass per unit of deformed volume. We then define a density function for Eulerian variables by

$$\varrho(\mathfrak{S},\eta,\mathfrak{I},\mathfrak{t}) = \varrho(\mathfrak{x},\eta,\mathfrak{t}) \frac{\partial(\mathfrak{x},\eta,\mathfrak{t})}{\partial(\mathfrak{S},\eta,\mathfrak{t})}$$
(2-17)

In like manner, we define a body force per unit mass for Eulerian variables by

$$\mathbb{P}(s, \gamma, z, t) = \frac{\mathbb{P}(z, q, z, t)}{\frac{\partial(z, q, z)}{\partial(s, \gamma, z)}}$$
(2-18)

For conciseness, we denote the instantaneous surface bounding the fixed set of particles by

.

$$F(5, \gamma, 5, t) = f(x(3, \gamma, 5, t), \gamma(3, \gamma, 5, t), z(5, \gamma, 5, t)) = 0$$
(2-19)

If we use Equation 2-15 to transform Equations 2-9 and 2-14 to Eulerian variables, we obtain

$$\frac{1}{2t} \int dV = 0 \qquad (2-20)$$

$$\frac{d}{dt} \int_{\mathcal{C}} \mathcal{V} \, dV' = \oiint \sum \cdot dS + \int_{z_{s, p, z, t}} P_{z, z, t} \, dV \qquad (2-21)$$
The surface integral in Equation 2-21 is a vector invariant in form and hence unaltered by this consideration of a transformation to Eulerian coordinates. Also, it should be emphasized that the  $\rho$ , and P, in Equations 2-20 and 2-21 are the Eulerian counterparts defined in Equations 2-17 and 2-18.

To derive the Eulerian differential equations of continuity and momentum, we must carry out the time differentiation indicated in Equations 2-20 and 2-21. It should be noted that this operation is not as trivial as for the case when the integral was expressed in Lagrangian coordinates. The integrals have time-dependent limits of integration when expressed in Eulerian coordinates. We must invoke a generalization of Leibnitz's rule for differentiating an integral. We state this theorem without proof<sup>I</sup>.

$$\frac{d}{dt} \int (dt) = \int \frac{d}{dt} (dt) + \int \frac{d}{dt}$$

In this expression,  $\mathbb{V}$  is the velocity of points on the surface that describe the limits of integration.

In our applications, the velocity of the surface coincides with the velocity of particles (since no mass crosses the bounding surface). Also, the last term in Equation 2-22 can be transformed into a volume integral by use of the divergence theorem.

$$\frac{d}{dt}\left[ \left( \begin{array}{c} 0 \end{array}\right) dV = \int \left( \begin{array}{c} \frac{d}{dt} \left( \begin{array}{c} 0 \end{array}\right) + \nabla \cdot \left( \begin{array}{c} V \end{array}\right) \right) dV \qquad (2-23)$$

If we apply this to the left-hand side of Equations 2-20 and 2-21, we obtain

$$\frac{d}{dt} \int_{F(I,T),T,T} e^{\nabla} dV = \int_{F(I,T),T,T} \left( \frac{\partial}{\partial t} (e^{\nabla}) + \nabla \cdot (e^{\nabla} \nabla) \right) dV$$
(2-24)

$$\frac{d}{dt} \int e \, dV = \int \left( \frac{\partial e}{\partial t} + \nabla \cdot (e^{\nabla t}) \right) dV$$
(2-25)

If we use the divergence theorem on the surface forces in Equation 2-21, we may write Equations 2-24 and 2-25 as

$$\int \left( \frac{\partial p}{\partial t} + \nabla \cdot (p \nabla) \right) dV = 0$$
(2-26)

A heuristic proof for this theorem is given in several places. In particular it is discussed in <u>lectures in Fluid Mechanics</u> by Sydney Goldstein, Interscience 1960.

$$\int \left( \frac{\partial(\rho \Psi)}{\partial t} + \nabla \cdot (\rho \Psi \Psi) - \Psi \cdot \sum -\rho P \right) d\Psi = 0$$
 (2-27)

Since the portion of the continuum we considered was arbitrary, we must conclude that the integrands of the above expressions are zero at each point,  $(\xi, \eta, \zeta)$ , of the continuum of mass particles. The Eulerian equations of continuity and momentum are then

$$\frac{\partial t}{\partial E} + \nabla \cdot (e \nabla E) = 0 \qquad (2-28)$$

$$\frac{\partial t}{\partial E} + \nabla \cdot (e \nabla E) = \nabla \cdot \Sigma + e \mathbb{P} \qquad (2-29)$$

The Lagrangian counterpart of these equations is derived by applying the divergence theorem to the surface forces in Equation 2-12. The Lagrangian equations of continuity and momentum are then

$$\frac{\partial \rho}{\partial t} = 0 \qquad (2-30)$$

$$q \frac{\partial^2 m}{\partial t^2} = \nabla \cdot \sum + P \qquad (2-31)$$

The first equation just expresses the trivial fact that the Lagrangian density function is not dependent on time. It should be noted, again, that  $\rho$ , P, and  $\sum$  are defined differently in the above sets of equations, but there is rarely any occasion for using both the Eulerian and Lagrangian equations together so that no attempt will be made to give them different notations.

We will have numerous opportunities in this report to use Equations 2-28 and 2-29 or Equation 2-31. In particular, we want to use Equation 2-31 to derive a form of the Principle of Virtual Work which is useful for the derivation of the equations of motion of a flexible vehicle.

## 2.1.1.] A Formulation of the Principle of Virtual Work for a Continuous System

In the conventional manner we define a virtual displacement as one which carries each particle of the system into an imagined configuration in the "neighborhood" of the true configuration of the system at time t.



FIGURE 4 A VIRTUAL DISPLACEMENT OF THE SYSTEM

If we denote this virtual displacement by  $\delta \Pi$ , the position vector for the x-y-z particle in the neighboring configuration is  $\Pi + \delta \Pi$ . The virtual work of all the forces of the system (including the D'Alembert inertia forces) is

$$sw = \int_{1}^{1} \left( \nabla \cdot \sum + \mathbb{P} - e^{\frac{\partial^2 \Pi}{\partial t^2}} \right) \cdot s\pi dV$$
 (2-32)

which is zero because of Equation 2-31. We may elevate this statement to the level of a principle, or an axiom, by postulating that  $\delta W = 0$  even when constraint forces are excluded from  $\sum$  and p. With the understanding that constraint forces are not to be included in the definition of p and  $\sum$ , we have

 $\delta W = \int \left( \nabla \cdot \sum + P - e \frac{\partial^2 \Pi}{\partial t^2} \right) \cdot \delta \Pi \, dV = \Omega \qquad (2-33)$ 

which is the Principle of Virtual Work for a closed system (i.e., a fixed set of particles).

The Principle of Virtual Work can be extended to open systems (where mass crossess the boundary of the system) by considering the Eulerian equations of continuity and momentum (Equations 2-28 and 2-29). If we again exclude constraint forces from the body and surface forces and integrate over a fixed region in space, the total virtual work becomes

$$\delta W = \int_{F(3, \gamma, s, t) = 0} (\nabla \cdot \sum + e P - \frac{\partial}{\partial t} (e^{\nabla}) - \nabla \cdot (e^{\nabla} \nabla)) \cdot \delta dt dV = 0 \qquad (2-34)$$

If we now transform this back to Lagrangian coordinates, then

$$F(S(x,y,z,t), \eta(x,y,z,t), S(x,y,z,t)) = f(x,y,z,t) = 0$$
(2-35)

is the time varying surface bounding the particles which at time, t, are inside the fixed region in space described by F(  $\xi$  ,  $\eta$  ,  $\zeta$  ) = 0.1



FIGURE 5 ILLUSTRATING THE REGION OF INTEGRATION

Let T the language of modern mathematical analysis, f(x,y,z,t) is the "inverse image" of the region of the continuum inside  $F(\xi, \eta, \zeta) = 0$  at time, t.

The integral (Equation 2-34) becomes (the derivation follows closely that given for the transformation leading to Equations 2-26 and 2-27)

$$\delta W = \int \left( \nabla \cdot \sum + \mathcal{P} - e^{\frac{\partial^2 m}{\partial t^2}} \right) \cdot \delta m \, dV = 0 \qquad (2-36)$$

From this we must conclude that the Principle of Virtual Work holds for open systems. This must be qualified by noting that forces acting at the boundary of the system that are constraint forces for the interaction of the system with its surroundings must, nevertheless, be included in the virtual work for the system alone.

Following the convention of Whittaker and Ianczos<sup>1</sup>, we shall refer to the formulation of mechanics conceived by Iagrange and Hamilton as Analytical Mechanics in contrast to Newton's formulation which we shall call Vectorial Mechanics. The term "vectorial" refers to the fact that Newton's laws are relations between vectors. The governing equations in Analytical Mechanics, however, involve scalars such as kinetic energy and potential energy. In this report, the methods of Analytical Mechanics will be used exclusively. The fundamental principle of Analytical Mechanics is the Principle of Virtual Work.

#### 2.1.2 Lagrange's Equations for Continuous Elastic Systems

A more practical formulation of the Frinciple of Virtual Work can be obtained for a system with a finite (or countably infinite) number of degrees-offreedom. In those cases the configuration of the system can be prescribed by a finite number of functions of time. A set of such functions is called generalized coordinates. We shall call them independent generalized coordinates when they may be independently varied without violating the kinematical constraints of the system. In this case, the number of functions required is equal to the number of degrees-of-freedom. If the number of generalized coordinates is greater than the number of degrees-of-freedom, there must exist constraint relations between these coordinates which insure that the kinematical constraints of the system are not violated. We will call these "redundant generalized coordinates."

#### 2.1.2.1 Lagrange's Equations for an Independent Set of Generalized Coordinates

If we denote the generalized coordinates by  $p_j(t)$ , j = 1, 2...N, the position vector of the x-y-z-particle can be written as a function of these N coordinates<sup>2</sup> (N is the number of degrees-of-freedom).

$$m = m(p_1, p_2... p_N; x, y, z, t)$$
(2-37)

See E. T. Whittaker, A Treatise on the Analytical Dynamics of Particles and Rigid Bodies, Cambridge, 1961, or Cornelius Ianczos, The Variational Principles of Mechanics, Toronto Press, 1949, p. 3.

<sup>2</sup>The assumption is tacitly made here that the constraints of the system are holonomic (see Ianczos, The Variational Frinciples of Mechanics, p. 24).

The explicit dependence on time is included to account for the case of time dependent constraints.

A completely general and arbitrary virtual displacement can be imagined by giving an arbitrary variation,  $\delta p_j$ , to each of the generalized coordinates. The position vector of each particle in this neighboring configuration is

$$\pi(p_1 + sp_1, p_2 + sp_2, \dots p_N + sp_N; x, y, z, t)$$
(2-38)

If we attribute a differential nature to the  $\delta p_i$ , the virtual displacement of each particle is

$$\delta \mathbf{m} = \sum_{j=1}^{N} \frac{\partial \mathbf{m}}{\partial \mathbf{p}_{j}} \delta \mathbf{p}_{j}$$
(2-39)

.

Substituting this into Equation 2-33 we obtain

$$\delta W = \sum_{j=1}^{N} \int \left( \nabla \cdot \sum + \mathbb{P} - e^{\frac{\partial^2 \pi}{\partial t^2}} \right) \cdot \frac{\partial \pi}{\partial p_j} dV \ \delta p_j = 0$$
(2-40)

Care must be exercised at this point because the partial derivatives of M are defined differently in Equations 2-33 and 2-39. Difficulty may be avoided by regarding x, y, and z as only of parametric significance. By careful manipulation it can be shown that the following identity is true.

$$\int_{f(x,y,z)=0} \frac{\partial^2 m}{\partial t^2} \cdot \frac{\partial m}{\partial p_j} dV = \frac{d}{dt} \left( \frac{\partial}{\partial p_j} \int_{z} \frac{\partial}{z} \frac{\partial m}{\partial t} \cdot \frac{\partial m}{\partial t} dV \right) - \frac{\partial}{\partial p_j} \int_{z} \frac{\partial}{\partial t} \frac{\partial m}{\partial t} \cdot \frac{\partial m}{\partial t} dV \quad (2-41)$$

The derivation depends upon the limits of integration being independent of time so that our considerations here apply only to closed systems. Equation 2-41. can be written more concisely by introducing the definition of kinetic energy.

$$\tau = \sum_{z} \int_{z} \frac{\partial \pi}{\partial t} dV \qquad (2-42)$$

We may then write

$$\int e \frac{\partial^{2} \pi}{\partial t^{2}} \cdot \frac{\partial \pi}{\partial p_{j}} dt' = \frac{d}{dt} \left( \frac{\partial T}{\partial p_{j}} \right) - \frac{\partial T}{\partial p_{j}}$$
(2-43)

Let us now direct our attentions to the other terms in Equation 2-40. The expression

$$\int \left( \nabla \cdot \Sigma + P \right) \cdot \frac{\partial n}{\partial p_j} dV$$
 (2-44)

is, by definition, the generalized force associated with the j<sup>th</sup> generalized coordinate. We may separate this into internal and external forces by adding and subtracting the term

$$\int \nabla \cdot \left( \sum \cdot \frac{\partial \pi}{\partial p_j} \right) dV$$
(2-45)

We then have

$$\int (\nabla \cdot \Sigma + \mathbb{P}) \cdot \frac{\partial u}{\partial p_{j}} dV = \int \left( \mathbb{P} \cdot \frac{\partial u}{\partial p_{j}} + \nabla \cdot \left( \Sigma \cdot \frac{\partial u}{\partial p_{j}} \right) \right) dV \qquad (2-46)$$
$$+ \int \left( \nabla \cdot \Sigma \cdot \frac{\partial u}{\partial p_{j}} - \nabla \cdot \left( \Sigma \cdot \frac{\partial u}{\partial p_{j}} \right) \right) dV$$

The divergence theorem can be applied to the first term which we then recognize as the contribution from externally applied forces. Thus we define  $P_j$  to be the generalized forces other than internal forces.

$$P_{j} = \int \mathbb{P} \cdot \frac{\partial p_{j}}{\partial m} dV + \oiint \frac{\partial p_{j}}{\partial m} \cdot \mathbb{Z} \cdot d\mathcal{I}$$
(2-47)

The external generalized forces can always be derived from the virtual work of the external forces in the form

$$\delta W = \int \delta m \cdot \mathbb{P} \, dV + \bigoplus \delta m \cdot \mathbb{\Sigma} \cdot d\mathcal{T} = \sum_{j=1}^{N} \delta p_j P_j \qquad (2-48)$$

which is equivalent to Equation 2-47 because of Equation 2-39. Equation 2-46 then becomes

$$\int (\nabla \cdot \Sigma + \mathbb{P}) \cdot \frac{\partial \mathbb{I}}{\partial P_{j}} dV = P_{j} + \int \left( \nabla \cdot \Sigma \cdot \frac{\partial \mathbb{I}}{\partial P_{j}} - \nabla \cdot \left( \Sigma \cdot \frac{\partial \mathbb{I}}{\partial P_{j}} \right) \right) dV \qquad (2-49)$$

The second term is the contribution of internal forces to the  $j^{th}$  generalized force. The integrand of this term can be written slightly more concisely by using the notation of cartesian tensors<sup>1</sup>.

$$\nabla \cdot \sum \cdot \frac{\partial \pi}{\partial p_j} - \nabla \cdot \left( \sum \cdot \frac{\partial \pi}{\partial p_j} \right) \longrightarrow \frac{\partial \nabla \iota_k}{\partial x_i} \frac{\partial 5k}{\partial p_j} - \frac{\partial}{\partial x_i} \left( \sigma_{ik} \frac{\partial 5k}{\partial p_j} \right) = - \sigma_{ik} \frac{\partial}{\partial p_j} \left( \frac{\partial 5k}{\partial x_i} \right) \quad (2-50)$$

In the case of small motions and linear one-dimensional stress-strain relations, we can write

$$\sigma_{ik}\frac{\partial}{\partial p_{j}}\left(\frac{\partial f_{k}}{\partial x_{i}}\right) = \sigma\frac{\partial}{\partial p_{j}}\left(\frac{\sigma}{E}\right) = \frac{\partial}{\partial p_{j}}\left(\frac{\sigma^{2}}{2E}\right) = \frac{\partial}{\partial p_{j}}\left(\frac{1}{2}Ee^{2}\right)$$
(2-51)

which indicates the existence of a potential,  $\mu = \frac{1}{2} \int e^{2}$  (E is Young's modulus). In the general case we can do little more than postulate the existence of a potential for the internal forces. On the basis that the internal forces are conservative we assume that u exists such that<sup>2</sup>

$$\nabla \cdot \overline{\Sigma} \cdot \frac{\partial p_{j}}{\partial p_{j}} - \nabla \cdot \left( \overline{\Sigma} \cdot \frac{\partial p_{j}}{\partial p_{j}} \right) = - \frac{\partial u}{\partial p_{j}}$$
(2-52)

(u is called the specific internal energy for the x-y-z-particle at time, t.)

To account for non-conservative internal forces, we may achieve a little more generality by introducing a "dissipation function" r, such that

$$\nabla \cdot \sum \cdot \frac{\partial \pi}{\partial p_i} - \nabla \cdot \left( \sum \cdot \frac{\partial \pi}{\partial p_i} \right) = - \frac{\partial u}{\partial p_i} - \frac{\partial u}{\partial p_i}$$
(2-53)

A dissipation function will exist when the stress-strain relations are a generalization of the one-dimensional relation<sup>3</sup>

$$\sigma = \varepsilon(\varepsilon + \varepsilon \dot{\varepsilon}) \tag{2-54}$$

If we introduce Equation 2-53 into 2-49, we obtain

$$\int (\nabla \cdot \Sigma + \mathbb{P}) \cdot \frac{\partial \pi}{\partial p_j} dV = P_j - \frac{\partial U}{\partial p_j} - \frac{\partial \mathcal{R}}{\partial p_j}$$
(2-55)

## 1H. Jeffreys, Cartesian Tensors, 1931.

<sup>2</sup>See Green and Zerna, <u>Theoretical Elasticity</u> Oxford, 1954, section 2.6, p. 71. This assumption is closely related to the First Principle of Thermodynamics. <sup>3</sup>A rational generalization of Equation 2-54 has been given by Enrico Volterra, <u>On Elastic Continua with Hereditary Characteristics</u>, Journal of Applied <u>Mechanics</u>, September 1951. See also Section 4.1.5, Equations 4-182 through 4-187 in this report. where

$$U = \int u \, dV \tag{2-56}$$

and

 $R = \int \pi \, dV \tag{2-57}$ 

# U is the total internal or strain energy of the system and R is Rayleigh's dissipation function.

To summarize, we have obtained the following

$$\int (\nabla \cdot \Sigma + P) \cdot \frac{\partial m}{\partial p_j} dV - \int e^{\frac{\partial^2 m}{\partial t^2}} \cdot \frac{\partial m}{\partial p_j} dV$$

$$= P_j - \frac{\partial U}{\partial p_j} - \frac{\partial R}{\partial p_j} - \frac{d}{dt} (\frac{\partial T}{\partial p_j}) + \frac{\partial T}{\partial p_j}$$
(2-58)

where

$$P_{j} = \int \mathbb{P} \cdot \frac{\partial \mathbb{I}}{\partial p_{j}} dV + \oiint \frac{\partial \mathbb{I}}{\partial p_{j}} \cdot \mathbb{\Sigma} \cdot dS \qquad (2-59)$$

$$U = \int u \, \mathrm{d} V \tag{2-60}$$

$$R = \int r_1 dV \tag{2-61}$$

$$\tau = \frac{1}{2} \int e \frac{\partial \pi}{\partial t} \cdot \frac{\partial \pi}{\partial t} dV \qquad (2-62)$$

If we introduce this into Equation 2-40, we obtain

$$\delta W = \sum_{j=1}^{N} \left( P_{j} - \frac{\partial U}{\partial p_{j}} - \frac{\partial R}{\partial p_{j}} - \frac{d}{dt} \left( \frac{\partial T}{\partial p_{j}} \right) + \frac{\partial T}{\partial p_{j}} \right) \delta p_{j} = 0$$
 (2-63)

We have assumed the  $p_j$  to be independent and the  $\delta p_j$  can be arbitrarily assigned, so that the only way the above sum can be zero is for each of the coefficients of the  $\delta p_j$  to be individually zero. The result is Lagrange's equations

$$\frac{d}{dt}\left(\frac{\partial T}{\partial \dot{p}_{j}}\right) - \frac{\partial T}{\partial \dot{p}_{j}} + \frac{\partial U}{\partial \dot{p}_{j}} + \frac{\partial R}{\partial \dot{p}_{j}} = P_{j}$$

$$i = 1, 2, ..., II$$
(2-64)

If part of the external body forces,  $\mathbb P$  , is conservative, it too may be derived from a potential function. For example, if

$$\nabla \times \mathbf{P} = 0 \tag{2-65}$$

then there exists a potential per unit volume,  $\vartheta$ , such that

$$P = -\nabla \vartheta \qquad (2-66)$$

(Note, V is the Eulerian gradient)

and we have

$$P_{j} = \int \mathbb{P} \cdot \frac{\partial n}{\partial p_{j}} dV \qquad (2-67)$$
$$= -\int \nabla \vartheta \cdot \frac{\partial n}{\partial p_{j}} dV$$
$$= -\int \frac{\partial \vartheta}{\partial p_{j}} dV$$
$$= -\frac{\partial V}{\partial p_{j}}$$

where

$$V(p_1, p_2 - p_N) = \int d^2 dV$$
 (2-68)

A typical example of such a force is the force of gravity.

## 2.1.2.2 Legrange's Equations for a Redundant Set of Generalized Coordinates

If there is occasion to express the virtual work in terms of M generalized coordinates  $(M > N_r)$  the number of degrees-of-freedom), then

$$\delta W = \sum_{j=1}^{N} \left( P_j - \frac{\partial U}{\partial p_j} - \frac{\partial R}{\partial p_j} - \frac{d}{dt} \left( \frac{\partial T}{\partial p_j} \right) + \frac{\partial T}{\partial p_j} \right) \delta p_j = 0 \quad (2-69)$$

The  $p_1$  form a set of independent generalized coordinates only if  $M = N_*$  If  $M > N_2$  we must recognize that M-N relations exist between these M coordinates which insure that the constraints are not violated.

$$F_{\hat{i}}(\hat{p}_{i}, \hat{p}_{2}, \dots, \hat{p}_{M_{i}}) = 0$$
  
$$i = I_{n} 2 \dots M - N$$

Since the  $p_j$  are not independent, we are not permitted to imply that the coefficients of  $\delta p_j$  in Equation 2-69 are zero. We may, instead, use Lagrange's method of undetermined multipliers. The derivation of Lagrange's equations in this case proceeds as follows.

The notion that the virtual displacements are consistent with the constraints is expressed by

$$F_{\tilde{L}}(p_{1}+\delta p_{1}, p_{2}+\delta p_{2}, \dots, p_{M}+\delta p_{M}) = 0$$
 (2-71)

## i = 1, 2...M-N

Again, using the differential nature of virtual displacements, we obtain

$$\sum_{j=1}^{M} \frac{\partial F_i}{\partial p_j} s p_j = 0$$
 (2-72)

If we introduce arbitrary multipliers,  $\lambda_{\pm z}$  we may also say that

$$\lambda_{i} \sum_{j=1}^{M} \frac{\partial F_{i}}{\partial p_{j}} \delta p_{j} = 0$$
(2-73)

It is equally true that

.

$$\sum_{i=1}^{M-N} \sum_{j=1}^{M} \lambda_{\bar{i}} \frac{\partial F_{\bar{i}}}{\partial p_{\bar{j}}} \delta p_{j} = 0$$
 (2-74)

If we add this zero-term to  $\delta \mathbf{W}_{x}$  we have

$$SW_{i} = \sum_{j=1}^{M} \left( P_{j} - \frac{\partial U}{\partial p_{j}} - \frac{\partial R}{\partial p_{j}} - \frac{d}{dE} \left( \frac{\partial T}{\partial p_{j}} \right) + \frac{\partial T}{\partial p_{j}} \right) Sp_{j} \qquad (2-75)$$
$$+ \sum_{j=1}^{M} \sum_{i=1}^{M} \lambda_{i} \frac{\partial F_{i}}{\partial p_{j}} Sp_{j} = 0$$

We may choose values of  $\lambda_{\tilde{1}}$  that insure that the first M-N coefficients of  $\delta p_{\tilde{j}}$  are zero; that is

$$P_{j} - \frac{\partial U}{\partial p_{j}} - \frac{\partial R}{\partial p_{j}} - \frac{d}{dt} \left( \frac{\partial T}{\partial p_{j}} \right) + \frac{\partial T}{\partial p_{j}} + \sum_{i=1}^{M-H} \lambda_{i} \frac{\partial F_{i}}{\partial p_{j}} = 0 \qquad (2-76)$$

 $j = L_x 2 \dots M - N$ 

The last N  $\delta p_j$ 's can be independently chosen so that

$$\sum_{j=M-N+1}^{M} \left( P_{j} - \frac{\partial U}{\partial p_{j}} - \frac{\partial R}{\partial p_{j}} - \frac{d_{i}}{dt} \left( \frac{\partial T}{\partial p_{j}} \right) + \frac{\partial T}{\partial p_{j}} + \sum_{k=1}^{M-N} \lambda_{k} \frac{\partial F_{k}}{\partial p_{j}} \right) sp_{j} = 0 \qquad (2-77)$$

implies

$$P_{j} - \frac{\partial U}{\partial p_{j}} - \frac{\partial R}{\partial p_{j}} - \frac{d}{dt} \left( \frac{\partial T}{\partial p_{j}} \right) + \frac{\partial T}{\partial p_{j}} + \sum_{i=1}^{M-N} \lambda_{i} \frac{\partial F_{i}}{\partial p_{j}} = 0 \qquad (2-78)$$

 $\mathbf{J} = \mathbf{M} - \mathbf{N} + \mathbf{L}, \dots \mathbf{N}$ 

Equations 2-76 and 2-78 can very simply be written together as

$$\frac{d_{i}(\frac{\partial T}{\partial \dot{p}_{j}}) - \frac{\partial T}{\partial \dot{p}_{j}} + \frac{\partial U}{\partial \dot{p}_{j}} + \frac{\partial R}{\partial \dot{p}_{j}} = P_{j} + \sum_{\bar{i}=1}^{M-M} \lambda_{\bar{i}} \frac{\partial F_{\bar{i}}}{\partial \dot{p}_{j}} \qquad (2-79)$$

$$F_{\bar{i}}(\dot{p}_{1x} \dot{p}_{2} \cdots \dot{p}_{N}) = O \qquad (2-80)$$

$$j = I_{2} 2_{y} \cdots M$$

$$i = I_{y} 2_{y} \cdots M$$

This is the form of Lagrange's equations which must be used when the generalized coordinates do not satisfy the constraints explicitly. It constitutes a set of 2M-N equations in the M redundant coordinates and the M-N multipliers,  $\lambda_{\perp}$ .

#### 2.1.2.3 Lagrange's Equations for Quasi-Coordinates

Occasionally in dynamics it is desirable to work with the kinetic energy expressed in terms of nonintegrable velocity components. For example, the kinetic energy of a rigid body can be expressed in terms of the velocity components of the mass-center referred to an axis system fixed in the body. Such considerations lead one to assume that the kinetic energy can be expressed in terms of N quantities,  $v_{i,r}$  which are related to generalized coordinates for the system, by equations of the form

$$V_{\tilde{i}} = \sum_{j=1}^{N} \alpha_{\tilde{i}\tilde{j}} (p_{i}, p_{x} \dots p_{N}) \hat{p}_{j}$$
(2-81)

An example is given by the motion of a rigid body in a plane. In this case we have

$$T = \frac{1}{2} \left( M v_1^2 + M v_2^2 + I v_3^2 \right)$$
 (2-82)



FIGURE 6 RIGID BODY IN PLANE MOTION

In this expression  $v_1$  and  $v_2$  are the components of the velocity vector referred to principal axes. They are related to the generalized coordinates ,  $p_{1,r} p_{2,r}$  and  $p_3$  by

$$V_{1} = \cos \beta_{3} \dot{p}_{1} + \sin \beta_{3} \dot{p}_{2} \qquad (2-83)$$

$$V_{2} = -\sin \beta_{3} \dot{p}_{1} + \cos \beta_{3} \dot{p}_{3} \qquad (2-83)$$

$$V_{3} = \ddot{p}_{3}$$

which is the same form as that indicated in Equation 2-81. This example is also characterized by the fact that the set of first order equations, 2-83, are not integrable. That is, there does not exist a set of coordinates,  $s_{I,r}$ ,  $s_{2,r}$ , and  $s_3$  such that

 $\begin{aligned} \gamma_{i} &= \tilde{S}_{i} \\ \gamma_{z} &= \tilde{S}_{z} \\ \gamma_{z} &= \tilde{S}_{z} \end{aligned} \tag{2-84}$ 

To illustrate the point, the following similar set of equations is integrable in the above sense

$$\begin{split} \gamma_{i} &= \cos \beta_{2} \dot{\beta}_{i} - \beta_{i} \sin \beta_{2} \dot{\beta}_{2} \qquad (2-85) \\ V_{z} &= \sin \beta_{z} \dot{\beta}_{i} + \beta_{i} \cos \beta_{z} \dot{\beta}_{z} \\ V_{z} &= \dot{\beta}_{z} \end{split}$$

It is easily verified, in fact, that in this case  $v_{i} = \tilde{s}_{i}$  where

$$S_{1} = p_{1} \cos p_{2} \qquad (2-86)$$
$$S_{2} = p_{3} \sin p_{3}$$
$$S_{3} = p_{3}$$

which corresponds to a simple change from one set of generalized coordinates to another set of generalized coordinates. Our interests are specifically directed toward the case when the equations are nonintegrable.

In the general case we suppose that  $v_{\pm}$  is one of a number of variables that may be appropriate for the description of the motion of a dynamical system. In particular we assume that

$$T = T(Y_{11}, Y_{22}, ..., Y_{N}; p_{11}, p_{22}, ..., p_{N})$$
(2-67)

where

$$V_{\bar{L}} = \sum_{j=1}^{N} \alpha_{\bar{L}j} \dot{\mu}_{\bar{J}} \qquad (2-88)$$

Further, we suppose that  $v_{i}dt$  is not an exact differential (i.e., there do not exist  $s_{i}$ , such that  $ds_{i} = v_{i}dt$ ). It follows from this that

$$\frac{\partial \sigma_{ij}}{\partial \rho_{ki}} = \frac{\partial \sigma_{ij}}{\partial \rho_{ij}} \tag{2-89}$$

does not generally hold for all i, j, and k. (It can be shown that Equations 2-89 are necessarily true if  $s_1$  exists such that

$$\vec{s}_{t} = \sum_{j=1}^{N} \alpha_{tj} \vec{p}_{j} \qquad (2-90)$$

If Equations 2-89 were true, our considerations here would reduce to a trivial change from one set of generalized coordinates to another.

In the nonintegrable case it is convenient to introduce the notion of a quasi-coordinate. The differentials of the quasi-coordinates are defined by

$$\Im_{i} = \sum_{j=1}^{N} \alpha_{ij} \Im_{j}$$
 (2-91)

The quantity,  $\delta s_i$ , thus defined is not an exact differential and the existence of  $s_i$  is not implied by Equation 2-91.

If we can solve for  $\tilde{p}_j$  in Equation 2-88, then

$$\dot{p}_{j} = \sum_{i=1}^{N} \beta_{ij} v_{i} \qquad (2-92)$$

where  $\alpha_{i,j}$  are the elements in the inverse of the N by N matrix whose elements are  $\beta_{k,j}$  . It follows that

$$\sum_{k=1}^{N} \alpha_{k} (2-93)$$

$$\left[ \begin{array}{c} \alpha \\ i \neq j \end{array} \right]$$

$$\left[ \begin{array}{c} \alpha \\ i \neq j \end{array} \right]$$

$$\left[ \begin{array}{c} \alpha \\ i \neq j \end{array} \right]$$

If we premultiply Lagrange's equations (Equation 2-64) by  $\,\beta_{\rm ji}$  and sum over j, we obtain

$$\sum_{j=1}^{N} e_{ji}\left(\frac{d}{dt}\left(\frac{\partial T}{\partial \mu_{j}}\right) - \frac{\partial T}{\partial \mu_{j}}\right) = \sum_{j=1}^{N} g_{ji}\left(P_{j} - \frac{\partial U}{\partial \mu_{j}} - \frac{\partial R}{\partial \mu_{j}}\right)$$
(2-94)

The right-hand side is termed the "generalized forces associated with the quasicoordinates." This follows from the fact that

$$S_{i} = \sum_{j=1}^{N} e_{ji} \left( P_{j} - \frac{\partial U}{\partial P_{j}} - \frac{\partial R}{\partial P_{j}} \right)$$
(2-95)

is the coefficient of  $\delta s_{j}$  in the expression for the virtual work of the applied fonces

$$\delta \mathcal{H} = \sum_{j=1}^{N} \operatorname{Sep}\left( \left( \frac{P_{j}}{j} - \frac{\partial U}{\partial \mu_{j}} - \frac{\partial \mathcal{R}}{\partial \mu_{j}} \right) = \sum_{i=1}^{N} \delta \operatorname{Si}_{i} \sum_{j=1}^{N} \left( \operatorname{Ci}_{i} \left( \frac{P_{j}}{j} - \frac{\partial U}{\partial \mu_{j}} - \frac{\partial \mathcal{R}}{\partial \mu_{j}} \right) \right)$$
(2-96)

where use has been made of Equation 2-91 to write

$$\delta p_{j} = \sum_{i=1}^{N} \beta_{ji} \delta s_{i}$$
 (2-97)

To express Equations 2-94 in terms of the vi, we note that

$$\frac{\partial T}{\partial \dot{p}_{j}} = \sum_{k=1}^{N} \frac{\partial T}{\partial v_{k}} \frac{\partial v_{k}}{\partial \dot{p}_{j}}$$
(2-98)

and

$$\frac{\partial T}{\partial p_j} = \sum_{k=1}^{N} \frac{\partial T}{\partial V_k} \frac{\partial Y_k}{\partial p_j} + \frac{\partial T}{\partial p_j}$$
(2-99)

In these relations we may use Equation 2-88 to write

$$\frac{\partial \dot{k}}{\partial b_{j}} = \kappa_{kj}$$
 (2-100)

$$\frac{\partial V_k}{\partial \dot{p}_j} = \sum_{l=1}^{N} \frac{\partial \alpha_{Rl}}{\partial \dot{p}_j} \dot{p}_l = \sum_{l=1}^{N} \sum_{m=1}^{N} \frac{\partial \alpha_{Rl}}{\partial \dot{p}_j} \partial \dot{p}_l \partial \dot{p}_l$$

Substituting these into Equations 2-98 and 2-99, we obtain

$$\frac{\partial T}{\partial \dot{p}_{j}} = \sum_{R=1}^{N} \frac{\partial T}{\partial V_{R}} \alpha_{Rj}$$
(2-102)

$$\frac{\partial T}{\partial p_j} = \sum_{k=1}^{N} \sum_{\ell=1}^{N} \frac{\partial T}{\partial p_k} \frac{\partial T}{\partial p_j} \frac{\partial \alpha_{k\ell}}{\partial p_j} \beta_{\ell n} V_{n} + \frac{\partial T}{\partial p_j}$$
(2-103)

Substituting into Equations 2-94, we find that

$$\sum_{j=1}^{N} \left( \frac{\partial j}{\partial k_{j}} \sum_{k=1}^{N} \sum_{k=1}^{N} \left( \alpha_{kj} \frac{d_{i}}{dk} \left( \frac{\partial T}{\partial v_{k}} \right) + \frac{\partial T}{\partial v_{k}} \left( \frac{\partial \alpha_{kj}}{\partial k_{k}} - \frac{\partial \alpha_{kk}}{\partial k_{k}} \right) \beta_{\ell n} v_{n} - \frac{\partial T}{\partial k_{j}} \right) = S_{i}$$

$$(2-10^{L})$$

This can be simplified by using Equation 2-93

$$\frac{d}{dt}\left(\frac{\partial T}{\partial V_{i}}\right) - \sum_{j=1}^{N} \sum_{k=1}^{N} \sum_{k=1}^{N} \left(\frac{\partial \alpha_{j} \ell}{\partial p_{k}} - \frac{\partial \alpha_{j} k}{\partial p_{k}}\right) e_{\ell n} V_{n} \frac{\partial T}{\partial V_{j}} + e_{ji} \frac{\partial T}{\partial p_{j}} = S_{i}$$
(2-105)

A further simplification results if the following definition is made

$$\Omega_{ij} = \sum_{k=L}^{N} \sum_{j=1}^{N} \sum_{k=1}^{N} \beta_{ki} \left( \frac{\partial \alpha}{\partial \beta_{k}} j \ell - \frac{\partial \alpha}{\partial \beta_{\ell}} j k \right) \beta_{\ell n} V_{n}$$
(2-106)

Y

and

#### 2.2. THE GENERAL THEORY OF SMALL MOTIONS ABOUT A POINT OF MINIMUM POTENTIAL

The theory of vibrations which is the subject of this section plays an extremely important part in the methods of dynamic analysis which have been developed for elastic airframes and spacecraft. The generality of the concept of a mode of vibration is often obscured by fixing attention on special problems like beams and plates. It is possible, as we shall show, to introduce the theory of vibrations as a very general application of the fundamental principles of Analytical Mechanics. More specifically, we shall specialize the general principles of the preceding section by making the following assumptions:

- 1. The system has a finite number of degrees-of-freedom.
- 2. The system has a static or indifferent position of equilibrium when the external forces are zero.
- There are no time dependent constraints (we assume this for simplicity only; actually, the theory we consider here can be generalized to include time dependent constraints.)
- 4. The displacements of the system from the equilibrium position are small in the sense that third order terms are negligible in comparison with quadratic terms.

With no essential loss in generality we shall assume that the position of equilibrium is given by

$$p_i = 0$$
 (2-113)

This is equivalent to saying that Lagrange's equations in the static case with no external forces (see Equation 2-64),

$$\frac{\partial U}{\partial p_i} = 0 \tag{2-114}$$

are satisfied by  $p_1 = 0$ ; that is,

$$\frac{\partial U}{\partial p_i}(o, \alpha, \dots \sigma) = \alpha \qquad (2-115)$$

Hecause Equation 2-115 is a necessary condition for U having a minimum (actually a stationary) value, it is commonly said that  $p_i = 0$  (in this case) is a "point of minimum potential."

#### 2.2.1 The Kinetic and Potential Energies

If we denote the velocity of the x-y-z-particle by V, then

 $V = \frac{\partial \mathbf{n}}{\partial \mathbf{t}}$ 

#### 2.2. THE GENERAL THEORY OF SMALL MOTIONS ABOUT A POINT OF MINIMUM POTENTIAL.

The theory of vibrations which is the subject of this section plays an extremely important part in the methods of dynamic analysis which have been developed for elastic airframes and spacecraft. The generality of the concept of a mode of vibration is often obscured by fixing attention on special problems like beams and plates. It is possible, as we shall show, to introduce the theory of vibrations as a very general application of the fundamental principles of Analytical Mechanics. More specifically, we shall specialize the general principles of the preceding section by making the following assumptions:

- 1. The system has a finite number of degrees of freedom.
- 2. The system has a static or indifferent position of equilibrium when the external forces are zero.
- 3. There are no time dependent constraints (we assume this for simplicity only; actually, the theory we consider here can be generalized to include time dependent constraints.)
- 4. The displacements of the system from the equilibrium position are small in the sense that third order terms are negligible in comparison with quadratic terms.

With no essential loss in generality we shall assume that the position of equilibrium is given by

$$\dot{p}_i = 0$$
 (2-113)

$$i = 1, 2...M$$

This is equivalent to saying that Lagrange's equations in the static case with no external forces (see Equation 2-64),

$$\frac{\partial U}{\partial p_i} = 0 \tag{2-114}$$

are satisfied by  $p_1 = 0$ ; that is,

$$\frac{\partial U}{\partial p_i}(0,0,\dots 0) \equiv 0 \qquad (2-115)$$

Because Equation 2-115 is a necessary condition for U having a minimum (actually a stationary) value, it is commonly said that  $p_1 = 0$  (in this case) is a "point of minimum potential."

#### 2.2.1 The Kinetic and Potential Energies

If we denote the velocity of the x-y-z-particle by  $\vee$ , then

and the kinetic energy (Equation 2-62) is

$$T = \frac{1}{2} \int e^{\sqrt{2}} dV \qquad (2-116)$$

We want to show first that the kinetic energy is quadratic in the "general-ized velocities,"  $\dot{p}_{\rm j}.$  We have, by differentiating Equation 2-37,

$$\Psi = \sum_{j=1}^{N} \frac{\partial \pi}{\partial \dot{p}_{j}} \dot{p}_{j} + \frac{\partial \pi}{\partial t}$$
(2-117)

The second term is zero if there are no time dependent constraints (assumption 3) because, in that case, time does not appear explicitly in Equation 2-37.

If we introduce Equation 2-117 into Equation 2-116, we obtain

$$\tau = \frac{1}{2} \int \sum_{i=1}^{N} e^{i \frac{\partial m}{\partial p_i}} \cdot \frac{\partial m}{\partial p_j} \dot{p}_i \dot{p}_j dV$$
(2-118)

Integration and summation may be interchanged to obtain

$$\tau = \frac{1}{2} \sum_{j=1}^{N} \sum_{j=1}^{N} \int \frac{\partial n}{\partial p_{i}} \cdot \frac{\partial n}{\partial p_{j}} e^{dV} \dot{p}_{i} \dot{p}_{j}$$
(2-119)

If we introduce

$$a_{ij}(p_{n}, p_{2} \dots p_{N}) = \int \frac{\partial p_{i}}{\partial p_{i}} \cdot \frac{\partial p_{j}}{\partial p_{j}} e dV \qquad (2-120)$$

then

$$T = \sum_{i=1}^{N} \sum_{j=1}^{N} a_{ij} \dot{p}_{i} \dot{p}_{j}$$
(2-121)

The generalized coordinates can generally be chosen so that

$$\frac{\partial M}{\partial p_i}$$
 = a constant, independent of the  $p_i$ . (2-122)

In this case, the a<sub>ii</sub> are constants.

We have thus shown that the kinetic energy is a homogeneous quadratic expression in the generalized velocities. Note from Equation 2-120 that

$$a_{ij} = a_{ji} \tag{2-123}$$

Considering now the potential strain energy<sup>1</sup>, we want to show that under assumptions 2 and 4 the strain energy is also a quadratic form. To do this we expand the strain energy in an N-dimensional Taylor's series about the point of equilibrium,  $p_i = 0$ .

$$U(b_{1}, b_{2} \dots b_{N}) = U(0, 0, \dots 0) + \sum_{i=1}^{N} \frac{\partial U}{\partial b_{i}}(0, 0, \dots 0) b_{i} + \frac{1}{2!} \sum_{\substack{i=1 \\ j=1}}^{N} \frac{\partial^{2} U}{\partial b_{i} \partial b_{j}}(0, 0, \dots 0) b_{i} b_{j} + \dots$$
(2-124)

If the arbitrary reference for the potential is taken as zero at the equilibrium position, then

$$U(0,0,...0) = 0$$
 (2-125)

Also, from Equation 2-115,

$$\frac{\partial U}{\partial p_i}(0,0...0) = 0$$
 (2-126)

Further, if we invoke assumption 4 and neglect terms in the series that are of a higher order than the quadratic terms,  $p_i p_j$ , then

$$U = \frac{1}{2} \sum_{i=1}^{N} \frac{\partial^{2} U}{\partial b_{i} \partial b_{j}} (0, 0 \dots 0) p_{i} p_{j}$$
(2-127)

<sup>&</sup>lt;sup>1</sup>If part of the <u>external</u> forces is conservative, then their potential may also be included. For example, gravity forces are important to the vibration of a pendulum.

If we introduce

$$k_{ij} = \frac{g_{j}}{g_{i}} (0, 0... 0)$$
 (5-158)

then the strain energy for small motions is

$$U = \frac{1}{2} \sum_{i=1}^{N} \sum_{j=1}^{N} k_{ij} p_{i} p_{j}$$
(2-129)

Since U is continuous and otherwise well-behaved at  $p_1 = 0$ , we must have

$$\frac{\partial^2 U}{\partial p_i \partial p_j} = \frac{\partial^2 U}{\partial p_j \partial p_i}$$
(2-130)

Consequently,

 $R_{ij} = R_{ji}$  (2-131)

Using the definition of matrix algebra, we can write Equation 2-121 and Equation 2-129 as

$$T = \frac{1}{2} \left\{ \dot{p} \right\}$$
 (2-132)

and

$$\cup = \frac{1}{2} i \beta f(\kappa) f(\beta)$$
 (2-133)

where [A] is the N by N matrix of inertia coefficients,  $a_{i,j}$ , and [K] is the N by N matrix of stiffness coefficients,  $k_{i,j}$ . It follows from Equations 2-123 and 2-131 that the inertia matrix and stiffness matrix are symmetric matrices, that is

$$[A]' = [A] \tag{2-134}$$

and.

$$[\kappa]' = [\kappa]$$
 (2-135)

In Equations 2-132 and 2-133, {p} is a column matrix of the N generalized coordinates,  $p_j$ , j = 1, 2, ... N.

$$\begin{aligned} \mathbf{f} \mathbf{p} \mathbf{F} &= \begin{bmatrix} \mathbf{p}_1 \\ \mathbf{p}_2 \\ \vdots \\ \mathbf{p}_N \end{bmatrix} \end{aligned} \tag{2-136}$$

#### 2.2.2 The Equations of Motion

We may employ lagrange's equations (Equation 2-64) to derive the equations governing the motion of the system described by Equations 2-132 and 2-133. It can be shown that in the case where  $\tilde{p}_1, \tilde{p}_2, >> \tilde{p}_1, \tilde{p}_2, \tilde{p}_4$  (in addition to assumptions 1 thru 4), the Eayleigh dissipation function can be approximated by

$$R = \sum_{i=1}^{N} \sum_{j=1}^{N} dy \dot{\mu}_{i} \dot{\mu}_{j} = \sum_{i=1}^{N} \hat{\mu}_{i} \dot{\mu}_{i} \dot{\mu}_{i} = \sum_{i=1}^{N} \hat{\mu}_{i} \dot{\mu}_{i} \dot{\mu}_{i}$$
(2-137)

where the elements,  $b_{ij}$ , of the damping matrix, [B], are constants. Also, the existence and continuity of the dissipation function in the neighborhood of the equilibrium position ( $p_i = 0$ ) require that

$$[\mathcal{F}]' = [\mathcal{F}] \tag{2-138}$$

1

The virtual work  $\delta W$  of the external forces defines the generalized forces, P<sub>j</sub>, associated with the generalized coordinates, p<sub>j</sub> (see Equation 2-43).

$$\sum_{j=1}^{n} S_{p_j} P_j = \{ S_{p_j} P_j \}$$
 (2-139)

Substituting Equations 2-132, 2-133, and 2-137 into Equation 2-64 using 2-139, we obtain

$$[A][\ddot{p}] + [B][\dot{p}] + [K][\dot{p}] = \{p\}$$
(2-140)

where use has been made of the fact that

$$\frac{\partial T}{\partial \dot{p}_{k}} = \frac{1}{2} \sum_{j=1}^{N} \sum_{j=1}^{N} \alpha_{ij} \frac{\partial (\dot{p}_{i} \dot{p}_{j})}{\partial \dot{p}_{k}} = \sum_{j=1}^{N} \alpha_{kj} \dot{p}_{j}$$
(2-I41)

$$\frac{\partial T}{\partial p_k} = 0 \tag{2-142}$$

$$\frac{\partial U}{\partial p_{R}} = \frac{1}{2} \sum_{i=1}^{N} \sum_{j=1}^{N} k_{ij} \frac{\partial (p_i p_j)}{\partial p_{R}} = \sum_{j=1}^{N} k_{kj} p_j$$
(2-I43)

$$\frac{\partial R}{\partial \dot{p}_{R}} = \frac{1}{2} \sum_{l=1}^{N} \sum_{j=1}^{N} \psi_{ij} \frac{\partial (\dot{p}_{l} \dot{p}_{j})}{\partial \dot{p}_{k}} = \sum_{j=1}^{N} \psi_{kj} \dot{p}_{j}$$
(2-144)

Equations 2-140 are the classical equations of the theory of vibrations that were first derived by Lagrange and subsequently studied by Lord Rayleigh.

#### 2.2.3 General Solutions to the Vibration Equations

#### 2.2.3.1 The Homogeneous Equations of Free Vibration

We will first consider the case of free vibrations with no damping because of the importance these solutions have in the cases where  $\{P\} \neq \{0\}$  and dissipation is present. We will consider then

$$[A][\dot{p}] + [K][\dot{p}] = \{o\}$$
(2-145)

These equations are a linear simultaneous set of coupled second-order differential equations. We may find a solution to these equations by assuming a "product" solution of the form

$$\{p_{(t)}\} = \{\phi\} \{t\}$$
 (2-146)

where the elements of  $\{\phi\}$  are not functions of time. Substituting this into Equation 2-145, we obtain

$$[A]{\varphi} {\dot{\varphi}} + [K]{\varphi} {\dot{\varphi}} = \{0\}$$
(2-147)

 $\mathbf{or}$ 

$$[A][\phi] = -\frac{k}{3}[K][\phi] \qquad (2-148)$$

Since the left side of the equation is independent of time, the right side must be also, so that

$$-\frac{0}{3} = \lambda = a \text{ constant}$$
 (2-149)

Equations 2-145 have then been "separated" into the equations

$$([A] - \lambda[\kappa])\{\varphi\} = \{0\}$$

$$(2-150)$$

and

$$\ddot{q} + \frac{1}{\lambda}q = 0$$
 (2-151)

In order that solutions (other than the trivial one,  $\{\phi\}=\{O\}$ )exist, it is necessary that the determinant of the coefficients in Equation 2-150 be zero

$$\Delta(\lambda) = |[A] - \lambda[k]| = 0 \qquad (2-152)$$

This equation is an N<sup>th</sup> order polynomial in  $\lambda$  which determines N discrete values of  $\lambda$  for which a product solution of the form in Equation 2-146 exists. It can be shown that, due to the symmetry properties of [A] and [K], the roots,  $\lambda_{i}$ , of  $\Delta(\lambda) = 0$  are all real; and, further, they are positive because of the positive definite character of [A] and [K]. For each of the N roots there corresponds a solution to Equation 2-150.

$$(A] - \lambda_{i}[K]) \{ \varphi \}_{i} = \{ 0 \}$$
 (2-153)

It may be noted that any constant multiple of a solution is also a solution. To make the solution unique, an arbitrary normalizing condition can be imposed. Most often it is convenient to assume that

$$\{\varphi\}_{i}^{\prime}[A]\{\varphi\}_{i} = 1 \qquad (2-154)$$

It is evident that if  $\{\phi\}_i$  is any solution to Equation 2-153, then

$$\left(\frac{1}{\sqrt{\frac{1}{2}\varphi_{i}^{2}\left[AH\varphi_{i}^{2}\right]}}\right)\xi\varphi_{i}^{2}$$
(2-155)

is a normalized solution; that is, one that satisfies Equation 2-154 as well as Equation 2-153.

Solutions to Equation 2-151 for each  $\lambda_i$  are

$$q_{i}(t) = q_{i} \cos \omega_{i} t + \delta_{i} \sin \omega_{i} t \qquad (2-156)$$

where

$$\omega_{i} = \frac{1}{\sqrt{\lambda_{i}}}$$
(2-157)

By a theorem of linear differential equations the general solution to the homogeneous equations (Equation 2-145) is a linear combination of the N particular solutions,

$$f\varphi f_{i} q_{i}(t) \qquad (2-158)$$

$$i = L, 2, \dots N$$

$$\{\beta'(t)\} = \sum_{i=1}^{N} \{\varphi f_{i} (q_{i} \cos \omega_{i} t + f_{i} \sin \omega_{i} t) \qquad (2-159)$$

The constants,  $a_i$  and  $b_i$ , can be expressed in terms of initial conditions; but we will postpone this until the "orthogonality" relations are established.

#### 2.2.3.2 Orthogonality Relations for the Modal Columns

Any two different solutions to Equation 2-150 corresponding to  $\lambda_{\,i}$  and  $\lambda_{\,j}$  (i  $\neq$  j) must satisfy

$$[A] \{\varphi\}_{l} = [K] \{\varphi\}_{l} \wedge_{l}$$
(2-100)

anđ

$$[A] \{\varphi\}_{j} = [K H \varphi\}_{j} \lambda_{j} \qquad (2-16L)$$

If we premultiply the first equation by  $\{\phi_j\}_j'$  and the second equation by  $\{\phi_j\}_i'$  then transpose the first equation, we obtain

$$f \varphi f'_{i} [A] \{ \varphi \}_{j} = \{ \varphi \}_{i} [K ] \{ \varphi \}_{j} \lambda_{i}$$
(2-162)

$$\{\varphi F'_{i} [A] f \varphi \}_{j} = \{\varphi F'_{i} [K] f \varphi \}_{j} \lambda_{j}$$
(2-163)

If Equation 2-162 is subtracted from Equation 2-163 and we note that [A]' = [A] and [K]'' = [K], then

$$\{\varphi F_{i}[\kappa] \{\varphi, f_{j}(\lambda_{i} - \lambda_{j}) = \alpha \qquad (2-164)$$

Since  $i \neq j$ ,  $\lambda_j = \lambda_j \neq 0$  so long as there are no repeated roots to the characteristic equation,  $\Delta(\lambda) = 0$ . Our discussion here will apply to the case of no repeated roots; however, some practical systems can have repeated roots although they usually present no problem.

If 
$$\lambda_{i} - \lambda_{j} \neq 0$$
, then  
 $\{\varphi F_{i}' [\kappa \mathbb{H}\varphi F_{i} = 0$  (2-165)

and from Equation 2-162

$$\{\varphi\}_{i}^{\prime} [A] \{\varphi\}_{j}^{\prime} = \alpha \qquad (2-166)$$

If we premultiply Equation 2-160 by  $\int f_1'$ , we obtain

$$i\varphi f'_{i}[\kappa F_{\varphi} F_{z} = \frac{1}{\lambda_{z}} i\varphi f'_{z}[A]i\varphi f_{z}$$
(2-167)

Using Equation 2-154, we get

$$\{\varphi F_{i}'[\kappa] \{\varphi F_{i} = \chi_{i} = \omega_{i}^{2}$$

$$(2-168)$$

In summary, the orthogonality relations are

$$\begin{cases} f\varphi f'_{i} [A H\varphi f_{j} = \{ i \ i = j \\ 0 \ i \neq j \end{cases}$$

$$f\varphi f'_{i} [K H\varphi f_{j} = \{ i \lambda_{i} \ i = j \\ 0 \ i \neq j \end{cases}$$

$$(2-169)$$

$$(2-170)$$

In Equation 2-159 we may use the above equations to express the arbitrary constants,  $e_1$  and  $b_1$ , in terms of the initial values of the generalized coordinates and generalized velocities. Setting t = 0 in Equation 2-159, we get

$$\sharp(\alpha) f = \sum_{i=1}^{N} \{\varphi\}_{i} a_{i} \qquad (2-171)$$

and similarly

$$\{\dot{p}(\alpha)\} = \sum_{i=1}^{N} \{ \varphi \}_{i} L_{i} \omega_{i}$$
(2-172)

To solve for  $a_i$  and  $b_i$ , we premultiply by  $\{\phi f_j''[A]$ 

$$\{\varphi, f_j [A] \{ p(\alpha) \} = \sum_{i=1}^{N} \{ \varphi f_j [A] \} \{ \varphi F_i a_i \}$$
(2-173)

$$\{\varphi\}_{j}^{\prime} [A \mathbb{H}\dot{p}(\alpha)\} = \sum_{i=1}^{N} \{\varphi\}_{j}^{\prime} [A \mathbb{H}\varphi\}_{i} \psi_{i} \psi_{i} \psi_{i} \qquad (2-174)$$

Using the orthogonality relations (Equation 2-169 and 2-170), these simplify to

$$a_{i} = \{\varphi\}_{i}^{\prime} [A ]\{\varphi(o)\}$$
(2-175)

$$\psi_{i} = \frac{1}{\omega_{i}} i \varphi f_{i}[A] i \dot{p}(a) f \qquad (2-176)$$

We obtain the complete solution to the homogeneous equations of free vibration by substituting Equations 2-175 and 2-176 into Equation 2-159.

$$\{p(t)\} = \sum_{i=1}^{N} \{\varphi\}_{i} \{\varphi\}_{i} \{\varphi\}_{i} [A] \left(\{p(a)\} : x w_{i}t + \{p(a)\} : \frac{5m w_{i}t}{w_{i}}\right)$$

$$(2-177)$$

Finally, we conclude by noting that Equations 2-169 and 2-170 can be written concisely as

$$[\varphi]'[A][\varphi] = [_{1}]$$
 (2-178)

and

$$[\varphi]'[K][\varphi] = \Gamma'_{\lambda \perp}$$
 (2-179)

where  $[\phi]$  is the matrix of model columns

$$[\varphi] = [\{\varphi\}_{1}, \{\varphi\}_{2}, \dots, \{\varphi\}_{N}]$$
(2-180)

commonly called the "modal matrix."

## 2.2.3.3 Response of the Undamped System to External Forces Which Are a Function of Time Only

In Equation 2-140 we assume, again, that [B] = [0] but that the generalized forces,  $\{P\}$ , are a function of time explicitly.

$$[A ] \frac{1}{b} \frac{1}{2} + [K ] \frac{1}{2} \frac{1}{2} = \frac{1}{2} \frac{1}{2$$

Let us assume as a solution, the series

$$\{p(t)\} = \sum_{i=1}^{N} \{\varphi\}_{i} q_{i}(t)$$
 (2-182)

where the  $q_i$  are to be determined by the external forces. Equation 2-182 may also be written as (2.182)

$$\{\mathfrak{p}(\mathfrak{t})\} = [\varphi]\{\mathfrak{q}(\mathfrak{t})\}$$
(2-103)

where  $[\phi]$  is the modal matrix (Equation 2-180). The latter expression emphasizes the role of the  $q_i$ 's as coordinates to describe the motion of the system. Equation 2-183 is commonly called the normal coordinate transformation and the  $q_i$ 's are called normal coordinates. They are, in fact, a set of generalized coordinates which can be used to specify the configuration of the system in the same way the  $p_j$ 's do. The inverse transformation corresponding to Equation 2-183 can be obtained by premultiplying Equation 2-183 by  $[\phi]'[A]$ 

$$[\varphi][A]{[\varphi]} = [\varphi][A][\varphi]{[\varphi]}$$
(2-184)

Using Equation 2-178, we have

$$\{g(t)\} = [\phi]'[A + \phi(t)\}$$
 (2-185)

Equations 2-181 are simplified when the  $q_1$ 's are used as generalized coordinates. To transform to normal coordinates let us substitute Equation 2-183 into Equation 2-181

$$[A [[q] H \ddot{q}] + [\kappa [[q] H q] = \{P\}$$
(2-186)

We will transform the generalized forces consistently if we premultiply this equation by  $\lceil \phi \rceil^{\prime}$ 

$$[\varphi][A][\varphi]{\bar{q}} + [\varphi][K][\varphi]{\bar{q}} = [\varphi][\bar{q}]$$
(2-187)

Making use of Equations 2-17d and 2-179, we obtain

 $\{\tilde{i}\} + [\chi] \{\eta\} = [\varphi] \{\eta\}$  (2-188)

The differential equations are uncoupled when expressed in normal coordinates. The  $i^{\mathrm{th}}$  equation is

$$\frac{d^2q}{dt^2} + \frac{1}{\lambda_i} = \{q\}_i \{P(t)\}$$
 (2-189)

These differential equations may be solved by a variety of methods, but the laplace transform has specific advantages in this case. We define

$$\overline{z}_{i}(s) = \int_{a}^{\pi} \overline{z}_{i}(t) e^{-st} xt \qquad (2-19a)$$

Operating on Equation 2-189, we obtain

$$s^{2}\bar{q}_{1}(s) + \frac{1}{\lambda_{1}}\bar{q}_{1}(s) = \{\varphi\}_{1}^{\prime}\{\bar{P}(s)\} + sq_{1}(s) + \dot{q}_{1}(s)$$
 (2-191)

or

$$\overline{q}_{i}(s) = \frac{1}{s^{2} + \omega_{i}^{2}} \pm \left( \varphi \right)_{i}^{2} \pm \overline{p}(s) \pm \frac{5q_{i}(\sigma) + \dot{q}_{i}(\sigma)}{s^{2} + \omega_{i}^{2}}$$
(2-192)

If we identify the following transforms

$$\int_{a}^{\infty} \cos\omega_{i} t \ e^{-st} dt = \frac{s}{s^{2} + \omega_{i}^{2}}$$
(2-193)

$$\int_{\alpha}^{\infty} \frac{s_{m}\omega_{i}t}{\omega_{i}} e^{-st} dt = \frac{1}{s^{2} + \omega_{i}}$$
(2-194)

we can use the convolution theorem to write the solution

$$q_i(t) = \int_{\alpha}^{t} \frac{\sin \omega_i(t-\alpha)}{\omega_i} \left\{ q F_i \left\{ P(\alpha) \right\} d\alpha + \dot{q}_i(\alpha) \cos \omega_i t + \dot{q}_i(\alpha) \frac{\sin \omega_i t}{\omega_i} \right\}$$

Substituting into Equation 2-182 using Equation 2-185, we obtain

$$\begin{aligned} \{\mathfrak{p}(t)\} &= \sum_{i=1}^{N} \{\varphi\}_{i} \{\varphi\}_{i}^{\prime} \{\varphi\}_{i}^{t} \int_{\alpha}^{t} \frac{\sin \omega_{i}(t-\tau)}{\omega_{i}} \{\mathcal{P}(\tau)\} d\tau \\ &+ \sum_{t=1}^{N} \{\varphi\}_{i} \{\varphi\}_{i}^{\prime} \{\varphi\}_{i}^{\prime} [A] (\{\mathfrak{p}(0)\} \cos \omega_{i} t + \{\mathfrak{p}(0)\}] \frac{\sin \omega_{i} t}{\omega_{i}} ) \end{aligned}$$

$$(2-196)$$

The Green's function for the system is

$$[G(t)] = \sum_{i=1}^{N} \{\varphi\}_{i} \{\varphi\}_{i} \frac{\sin \omega_{i} t}{\omega_{i}}$$
(2-197)

which has the property that the solution with zero initial energy is

$$\{p(t)\} = \int_0^t [G(t-t)]\{P(t)\} dt \qquad (2-198)$$

The transform of the Green's function is the admittance matrix which, in this case, is

$$[H(s)] = \sum_{i=1}^{N} \frac{[\psi]_{i}}{s^{i} + \omega_{i}^{2}}$$
(2-199)

where

$$[\psi]_{i} = \{\psi\}_{i} \neq \psi\}_{i}$$
(2-200)

It can also be shown that the admittance matrix has the property that

$$[H(s)] = \left( \begin{bmatrix} A \end{bmatrix} s^{2} + \begin{bmatrix} K \end{bmatrix} \right)^{-1}$$
(2-201)

## 2.2.3.4 The Zero-Frequency Modes of an Unrestrained System

All of the modes and frequencies must satisfy Equation 2-153 which may be written as

$$(-\omega_{i}^{2}[A] + [K]) \{\varphi\}_{i} = \{0\}$$
 (2-202)

Possible solutions for which  $\omega_{i} = 0$  are called zero-frequency (or "rigid body") modes, and they must satisfy

$$[\kappa]_i \varphi_i = i\sigma_i$$
 (2-203)

Modes which satisfy Equation 2-203 represent possible displacements for which the potential energy is zero. If

$$\{p\} = \{\varphi\}_{i}$$
 (2-204)

where  $\{\phi\}_{i}$  satisfies Equation 2-203, then

$$U = \frac{1}{2} \{ p \}' [\kappa] \{ p \} = \frac{1}{2} \{ p \}' ([\kappa] \{ \varphi \}_i) = 0$$
 (2-205)

These are then possible displacements which result in no elastic deformation. Thus, the term "rigid-body" modes.

In order that solutions other than the trivial one,

$$\{\varphi\}_{i} = \{0\}$$
 (2-206)

exist, it is necessary that the determinant of the coefficients of  $\{\phi\}_i$  be zero. That is

$$|[\kappa]| = 0$$
 (2-207)

The stiffness matrix then is necessarily singular when there are rigid body modes. If we denote the rigid body modes by  $\{\phi_R\}_i$  and premultiply Equation 2-202 by  $\{\phi_R\}'_i$  then

$$\omega_{i}^{2} \{\varphi_{R}\}_{j}^{2} [A H \varphi]_{i} - \{\varphi_{R}\}_{j}^{2} [K H \varphi]_{i} = 0 \qquad (2-208)$$

but

$$\{\varphi_{R}\}_{j}^{\prime}[\kappa]\{\varphi\}_{i}^{\prime} = \{\varphi\}_{i}^{\prime}([\kappa]\{\varphi_{R}\}_{j}) = 0 \qquad (2-209)$$

hence for  $\omega_i \neq 0$ ,

$$\{\varphi_{R}\}_{j}'[A]\{\psi\}_{i} = 0$$
 (2-210)

We thus conclude that all of the zero-frequency modes are orthogonal to the elastic modes for which  $\omega_i \neq 0$ .

If the system is restrained so that the only possible displacements are ones for which the system is deformed and strain energy is stored, then rigid-body modes are not present and there are no solutions to Equation 2-202 corresponding to  $\omega_{i} = 0$ . In this case

and  $[K]^{-1}$  exists. Lagrange's equations (Equation 2-64) in the static case give

$$\frac{\partial p_i}{\partial p_i} = p_i \tag{2-212}$$

or

$$[\kappa]{p} = {p}$$
 (2-213)

.

If there are no zero-frequency modes, then

$$\{b\} = [\kappa]^{i} \{p\}$$
 (2-214)

which indicates that the displacements of an elastic system are linearly related to the loads acting on it (for small displacements). The matrix

$$[E] = [\kappa]^{'}$$
(2-215)

is called the influence matrix. The(i, j) element is the contribution to the  $i^{th}$  generalized coordinate by a unit value of the  $j^{th}$  generalized force.

 $p_i = \sum_{j=1}^{N} e_{ij} p_j \qquad (2-216)$ 

For a restrained system the strain energy may be expressed in terms of the applied loads acting on the system by using

$$\{b\} = [E]\{P\}$$
 (2-217)

$$U = \frac{1}{2} \left\{ \frac{1}{2} \right\} \left[ \frac{1}{2} \right] \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \left[ \frac{1}{2} \right] \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \left[ \frac{1}{$$

From this, Castigliano's theorem is easily proven:

$$\frac{\partial U}{\partial P_i} = \sum_{j=1}^{N} \alpha_{ij} P_j = p_i \qquad (2-219)$$

For restrained systems, Equation 2-150 can be written as

$$[E][A]{\varphi} = \lambda \{\varphi\} \qquad (2-220)$$

an expression which is suitable for a numerical solution by an iteration procedure. The practical importance of iterative procedures makes it desirable to derive a relation suitable for iteration of systems that are unrestrained. For such systems we have shown that |[K]| = 0 and thus  $[E] = [K]^{-1}$  does not exist.

The essence of this problem can be posed as: Are there solutions to the static equations (Equations 2-213),

$$[\kappa]{p} = {p}$$
 (2-221)

even when |[K]| = 0 because of the unrestrained conditions?

From the theory of linear equations it is known that there are solutions to the set of equations, Equations 2-221, provided the right-hand side satisfies certain conditions. In particular, we note that the homogeneous equations,

$$[\kappa Hp] = \{0\}$$
 (2-222)

have a general solution which is a linear combination of all the distinct zero-frequency modes. That is,

$$\{ p \} = \{ \varphi_R \}_1 c_1 + \{ \varphi_R \}_2 c_2 + \dots + \{ \varphi_R \}_M c_M , \qquad (2-223)$$

is a solution to Equations 2-222 for an arbitrary choice of the constants,  $c_1$ ,  $c_2$ ... $c_M$  (where 1 is the number of zero-frequency modes).

Also, a theorem of the theory of linear equations states that the general solution to Equations 2-221 is the sum of the solution to the homogeneous equations (Equations 2-222) and a particular solution to the nonhomogeneous equations.

We will try to find a particular solution to Equations 2-221 in the form

$$[S]{p^{3}}$$
(2-224)

Let us then assume the general solution to be

$$\{ \flat \} = \sum_{i=1}^{M} \{ \varphi_{R} \}_{i} c_{i} + \{ S \} \{ \flat^{*} \}$$
(2-225)

and substitute this into Equations 2-221.

$$[\kappa]\left(\sum_{i=1}^{M} \{\varphi_{R}\}_{i} c_{i} + [S] \} p^{*}\}\right) = \{p\}$$
(2-226)

The definition of zero-frequency modes (Equation 2-203) gives

$$[\kappa]_{i} \varphi_{R}_{i} = \{0\}$$
 (2-227)

$$i = 1, 2, ...M$$

so that Equation 2-226 becomes

$$[\kappa][S]{p^*} = \{P\}$$
(2-228)

It is evident that if these equations are to have a solution, then

$$\{\varphi_{R}\}_{i}^{i}[\kappa][S]\{p^{*}\} = \{\varphi_{R}\}_{i}^{i}\{p\}$$
(2-229)

But the left side is zero from Equation 2-227; thus, it is necessary that

$$i = 1, 2, \dots M$$
 (2-230)
This is a condition which must be satisfied by  $\{P\}$  in order that Equations 2-221 have a solution.

Assuming Equation 2-230 is true, we will attempt to obtain a solution to Equation 2-228 by premultiplying that equation by [S].

$$[S]'[K][S]{p^*} = [S]{P}$$
(2-231)

Up to this point we have said very little about the matrix, [S]'. We now want to assume that [S] is an N x N - M matrix, such that

$$[S]'[K][S]$$
 (2-232)

is non-singular. It follows, that the columns of the [S] -matrix must necessarily be linearly independent, but this is not sufficient to insure that Equation 2-232 is non-singular. A sufficient condition is provided by considering M arbitrary but independent constraints which would prevent rigidbody motion of the system. For a linear system these constraints can be expressed generally as

$$[L]{p} = \{\alpha\}$$
 (2-233)

We can pose a physical argument that at least M of the coordinates,  $p_j = 1, 2...N$ , must be involved in these constraints in such a way that there exist M columns of the [L]-matrix that are linearly independent. If we assume that these columns appear as the first M columns of [L], then

$$[\{L\}_{1},\{L\}_{2},...,\{L\}_{M}]$$
 (2-234)

is a square, non-singular matrix. Stated differently, it is possible to partition Equations 2-233

$$\begin{bmatrix} [L,], [L_2] \end{bmatrix} \begin{bmatrix} \{p_i\} \\ \{p_i\} \end{bmatrix} = \{0\}$$
 (2-235)

so that

$$\{b_i\} = -[L_i]^{\dagger}[L_2]\{b_2\}$$
(2-236)

or

 $\{p\} = \begin{bmatrix} -[L, j'[L_2]] \\ [1] \end{bmatrix} \{p_2\}$  (2-237)

The coefficient matrix in Equation 2-237 can be taken as an [S] -matrix. If Equations 2-233 represent true constraints of rigid-body motion, it is physically evident that

$$[S] = \begin{bmatrix} -[L_1] \\ [L_2] \end{bmatrix}$$
 (2-238)  
(s]'[K][S] with 
$$\begin{bmatrix} S \\ I_1 \end{bmatrix}$$

is a stiffness matrix for the constrained system and hence should be non-singular.

A condition on [S] which is sufficient but not necessary is that

$$[\varphi_{R}]'[S] = [O] \tag{2-239}$$

The proof that this is sufficient to insure that

$$[S]'[K][S]$$
 (2-240)

is positive definite is long and involved and is omitted here. For completeness we note that an [S] -matrix satisfying Equation 2-239 is easily constructed. For this purpose, partition  $[\phi_{\rm R}]$  so that

$$[\varphi_{\mathsf{R}}] = \begin{bmatrix} \varphi_{\mathsf{R}_1} \\ \varphi_{\mathsf{R}_2} \end{bmatrix}$$
 (2-241)

and let

$$[S] = \begin{bmatrix} -\left[ \varphi_{R}, \right] \right] \begin{bmatrix} \varphi_{R} \\ 1 \end{bmatrix}$$
(2-242)

We then have

$$[S]'[\varphi_{R}] = [\varphi_{R_{2}}][\varphi_{R_{1}}]^{\dagger}[\varphi_{R_{1}}] - [\varphi_{R_{2}}] = [O]$$
(2-243)

Returning to Equation 2-231, we have

$$ip^{*} = ([s]'[\kappa][s])^{-1} [s]' \{p\}$$
 (2-244)

Substituting this into Equation 2-225, we obtain

$$\{p\} = \sum_{i=1}^{M} \{\varphi_{R}\}_{i} c_{i} + [S]([S]'[\kappa][S])'[S]'\{p\}$$
(2-245)

which is the general solution to Equation 2-221 provided Equat on 2-230 holds.

The matrix,

.

$$[E] = [S]([S]'[K][S])^{-1}(S]' \qquad (2-246)$$

is a set of influence coefficients corresponding to some arbitrary constraint of the rigid-body motion. It is desirable for later discussions to express the  $c_i$  in Equation 2-245 in terms of the zero-frequency normal coordinates. This we proceed to do.

The general solution to the undamped vibration equations (Equations 2-145) is a linear combination of all the solutions to Equation 2-202 (including the modes corresponding to  $\omega_i = 0$ ).

$$\{p't\} = \sum_{t=1}^{M} \{\varphi_{R}\}_{t} q_{R}^{(n)}(t) + \sum_{i=1}^{N-M} \{\varphi_{i}\}_{i} q_{i}^{(r)}(t)$$
(2-247)

If we premultiply this by  $\{\phi_R\}'_j[A]$ , then

$$\{\varphi_{R}\}_{j}^{\prime}[A]\{p\} = \sum_{l=1}^{M} \{\varphi_{R}\}_{j}^{\prime}[A]\{\varphi_{R}\}_{i} q_{R}^{(i)}$$
(2-248)

where use has been made of Equation 2-210. If we introduce the matrix of zero-frequency modes

$$[\varphi_{\mathsf{R}}] = [\{\varphi_{\mathsf{R}}\}, \{\varphi_{\mathsf{R}}\}, [\varphi_{\mathsf{R}}\}, \dots \{\varphi_{\mathsf{R}}\}, [2-249]$$

then Equation 2-248 can be written as

$$[\varphi_{R}]'[A] \{ p \} = [\varphi_{R}]'[A] [\varphi_{R}] \{ q_{R} \}$$
(2-250)

If we also premultiply Equation 2-245 by  $[\phi_{\rm R}]'_{\rm A}]$  , then

$$[\varphi_{R}]'[A][p] = [\varphi_{R}]'[A][\varphi_{R}][c] + [\varphi_{R}]'[A][E][p]$$
(2-251)

and

$$fc_{f} = ([\varphi_{R}]'[A][\varphi_{R}])^{-1}[\varphi_{R}]'[A][p_{F}] - ([\varphi_{R}]'[A][\varphi_{R}])^{-1}[\varphi_{R}]'[A][e][p_{F}] - (2-252)$$

Substituting from Equation 2-250, we have

$$\{c\} = \{q_R\} - ([\phi_R]'[A][\phi_R])'[\phi_R]'[A][E] \neq p\}$$
(2-253)

Substituting this into Equation 2-245, we have

$$[\varphi_{R}] = [\varphi_{R}] \{q_{R}\} + [E] \{P\} - [\varphi_{R}] / [\varphi_{R}] / [A] [\varphi_{R}] / [A] [E] \{P\}$$
 (2-254)

which can be simplified if we introduce the matrix

$$[\Gamma] = \lceil 1, - \lceil A \rceil [\varphi_R]' [A \rceil [\varphi_R]' [A \rceil [\varphi_R]' ]$$

$$(2-255)$$

We then have

$$\{p\} = [\varphi_R]\{q_R\} + [\Gamma]'[E]\{P\}$$
 (2-256)

which is the general solution to the static equations,

.

$$[\kappa]{p} = {P}$$
 (2-257)

provided.

.

$$[\varphi_{R}]'\{P\} = \{Q\}$$
 (2-258)

Many of the aeroelastic problems associated with unrestrained bodies are approximately solved by imposing the constraint,

$$\{q_{R}\} = \{a\}$$
 (2-259)

so that the system can only take on a configuration that is a linear combination of its elastic modes,

$$\{p\} = \sum_{i=1}^{N-M} \{\varphi\}_{i} q_{i} \qquad (2-26\alpha)$$

In particular, it is useful to impose this constraint in order to derive an equation governing only the elastic modes for the purpose of numerical solution. Equation 2-259 leads to the following conditions of constraint on the generalized coordinates, p<sub>i</sub>,

$$[\mu_R]'[A]{\mu} = \{a\}$$
 (2-26L)

which follows by setting  $\{q_R\} = \{0\}$  in Equation 2-250. If we recall that the kinetic and potential energy is

$$\tau = \frac{1}{2} \{\dot{p}\} \{a \} \dot{p}\}$$
 (2-262)

$$U = \frac{1}{2} \frac{1}{2} \frac{1}{12} (K Hp)$$
 (2-263)

then we may use Lagrange's equations for coordinates which are not independent (Equation 2-79). The relations of constraint are

$$F_{i}(p_{r}, p_{2}, ..., p_{N}) = i \varphi_{R} f_{i}(A I \{ p \} = 0$$
(2-264)

 $i = 1, 2, \dots M$ 

and the generalized constraint forces are

$$\sum_{i=1}^{M} \lambda_{i} \frac{\partial F_{i}}{\partial \mu_{i}} = \sum_{i=1}^{M} \lambda_{i} \frac{\partial}{\partial \mu_{i}} + i \varphi_{R} F_{i}(A) \pm \mu F$$
(2-265)

 $\mathbf{or}$ 

$$\left[\frac{\partial F}{\partial p}[f\lambda]\right] = \left[A\right]\left[\varphi_{R}[f\lambda]\right]$$
(2-266)

Substituting this into Lagrange's equation, we obtain

$$[A Hp] + [\kappa Hp] - [A ][\varphi_R H\lambda] = \{p\}$$
(2-267)

The Lagrangian multipliers,  $\lambda_{\rm i}$ , may be eliminated by the following procedure. Fremultiply Equation 2-267 by  $[\phi_{\rm E}]'$  .

.

$$[\varphi_{R}]'[A] \{ \ddot{P} \} + [\varphi_{R}] [K K P \} - [\varphi_{R}]'[A] [\varphi_{R} K N ] = [\varphi_{R}]' \{ P \}$$
(2-268)

The second term is zero because of Equation 2-227. Solving for  $\{\lambda\}$ , we have

$$i\lambda F = [[\phi_R]'[\Delta][[\phi_R]]''[A]FPF - iPF. \qquad (2-269)$$

If this is substituted into Equation 2-267, we obtain

$$[\kappa Hp] = \{P\} - [A Hp] - [A Hqe] [[qe] [[A Hqe] ][qe] ][qe] - [A Hp] - [A$$

Using Equation 2-255, this can be written as

$$[\kappa \frac{1}{p}] = [\Gamma]^{(p)} - [\Lambda \frac{1}{p}]$$
 (2-271)

This set of linear equations can be solved for the p's in terms of the righthand side by the procedure which led to Equation 2-256. The only difference is that the right-hand side is

$$[\Gamma](\{P\} - [A \} + [P])$$
 (2-272)

in this case; whereas in the static case, considered before, the right-hand side was simply {P}. Condition 2-258 in the present case, is

. .

$$[\varphi_{R}]'[\Gamma](\{P\}-[A]\{p\}) = \{o\}$$
(2-273)

which is identically satisfied because

$$[\varphi_{\mathsf{R}}]'[\Gamma] = [\varphi_{\mathsf{R}}]'[\Gamma_{1} - [A][\varphi_{\mathsf{R}}]'[\varphi_{\mathsf{R}}]'[A][\varphi_{\mathsf{R}}]]'[\varphi_{\mathsf{R}}]'$$

$$= [\varphi_{\mathsf{R}}]' - [\varphi_{\mathsf{R}}]'$$

$$= [0]$$

$$(2-274)$$

We may therefore write Equation 2-271 as

$$\{p\} = [\varphi_{R}]\{q_{R}\} + [\Gamma]'[E][\Gamma] \{p\} - [A]\{p\}^{n}$$
(2-275)

Consistent with our previous assumptions, however, we have  $\{q_R\} = \{0\}$  (Equation 2-259), so that

The coefficients,  $[\Gamma][E][\Gamma]$ , come as close as anything to a generalized notion of influence coefficients. They appear to be the discrete analogy to the "generalized Green's function" introduced by Courant and discussed by Bisplinghoff in relation to aircraft structural analyses<sup>1</sup>. In this report we

<sup>&</sup>lt;sup>1</sup>See R. Courant and D. Hilbert <u>Methods of Mathematical Physics</u>, Interscience, 1953, Vol. I, p. 354, and Bisplinghoff, Ashley, and Halfman <u>Aeroelasticity</u>, Addison-Wesley, 1955, p. 24.

shall refer to the coefficients in Equation 2-276 as the "free-body" influence coefficients<sup>1</sup>. It is a curious consequence of the derivation given previously that the free-body influence coefficients are unique and independent of the arbitrary constraints assumed for the calculation of [E]. In some instances, a constrained influence coefficient matrix is calculated directly without using Equation 2-246. Such is the case when the "complementary strain energy" method is used (see Section 5.1.1.2 of this report).

Equation 2-276 is important for its application to some aeroelastic loads problems for unrestrained systems where the constraint,  $\{q_R\} = \{0\}$ , is not a serious one. We are presently interested, however, in its application to the problem of free vibrations.

For free vibrations we have  $\{P\} = \{0\}$ ; and, as before, we assume a. "product" solution,

$$\{p(t)\} = \{p\}q(t)$$
 (2-277)

in Equation 2-276 and arrive at

$$-\frac{q}{\tilde{g}} \{ \varphi \} = [\Gamma] [E] [\Gamma] [A] \{ \varphi \}$$
 (2-278)

which separates into the two equations

$$[\Gamma I'[E][\Gamma][A]{\phi} = \lambda {\phi}$$
 (2-279)

$$\ddot{q}_{i} + \dot{y}_{i} q = 0 \tag{2-280}$$

Equation 2-279 is in a form suitable for a numerical solution by iteration. The results are the solutions to Equation 2-202 corresponding to  $\omega_{i} \neq 0$ . The zero-frequency modes, or rigid-body modes, can usually be written down immediately; however, in some rare cases Equation 2-203 must be used to calculate them.

In conclusion, we note that the response to time dependent forces (Equation 2-196) must be modified slightly in the case where there are zero-frequency modes. It can be shown that the Green's function, in this case, is

$$[G(t)] = \sum_{i=1}^{M} i\varphi_{R} f_{i}' [\varphi_{R}]' [A] [\varphi_{R}] f_{i}' \varphi_{R} f_{i}' \lim_{\omega_{1} \to 0} \lim_{\omega_{1} \to 0} \frac{im\omega_{1}t}{\omega_{1}}$$

$$+ \sum_{i=1}^{N-M} i\varphi_{i} f_{i} f_{i} \varphi_{i}' \lim_{\omega_{1} \to 0} \frac{im\omega_{1}t}{\omega_{1}}$$

$$(2-2d1)$$

See also the very interesting paper by B. M. Fraeys de Veubike, <u>Iteration in</u> <u>Semidefinite Eigenvalue Problems</u>, Journal of the Aeronautical Sciences, October, 1955.

If we introduce the definitions,

$$[\psi]_{\alpha} = \sum_{i=1}^{M} \{\varphi_{R}\}_{L} \left[ [\varphi_{R}] \right] [A] [\varphi_{R}] \right]^{-1} \varphi_{R} j_{L}$$
(2-282)

$$[\psi]_{i} = \{\psi\}_{i} \{\psi\}_{i}^{\prime}$$
(2-283)

Τ.

this can be written as

$$[\varphi(t)] = [\psi]_{\alpha} t + \sum_{i=1}^{N-M} [\psi]_{i} \frac{s_{M}\omega_{i}t}{\omega_{i}}$$
(2-284)

and the general solution is

**\_** 

$$fp(t)f = \int_{\alpha}^{t} [G(t-t)]fP(t)F dt + [\psi]_{\alpha}[A][fp(t)]F + tip(t)]F$$

$$+ \sum_{i=1}^{N-M} [\psi]_{i}[A][fp(t)F dt w_{i}t]$$

$$+ fp(t)F \frac{dww_{i}t}{w_{i}}$$

$$+ fp(t)F \frac{dww_{i}t}{w_{i}}$$

# 2.2.3.5 Solutions to the Linearly Damped Vibration Equations

The previous sections have dealt with Equation 2-140 in the case where the damping is zero. We want to consider, in this section, the damped equations with external forces which are a function of time only,

$$[^{A}HF] + [BHF] + [KHF] = \{P(u)\}$$
 (2-286)

In an attempt to solve these equations, we might be led to believe that the normal coordinate transformation (Equation 2-163) would uncouple these equations in the same way it uncoupled the equations with no damping. If we substitute

$$ip f = [p] + g f \qquad (2-287)$$

into Equation 2-286 and premultiply by  $\left[\phi\right]'$ , we obtain

.

$$\{\hat{q}\} + [\phi]'[B][\phi]\{\hat{q}\} + [\psi_{\lambda}]\{\hat{q}\} = [\phi]'\{P\}$$
(2-288)

or

$$\{\ddot{q}\} + [R]\{\dot{q}\} + [\chi_1\{q\}] = [\psi]^{\{p\}}$$
(2-289)

where

$$[R] = [\phi][B][\phi] \qquad (2-290)$$

is the damping matrix corresponding to the normal coordinates. The i<sup>th</sup> equation is

$$\frac{d^{2}g_{i}}{dt^{2}} + \sum_{j=1}^{N} \tau_{ij} \frac{d_{ij}}{dt^{2}} + \omega_{i}^{2} g_{i} = \{\psi_{i}\}_{i} \{\mathcal{P}_{i}^{1}\}$$
(2-291)

These equations are only uncoupled when the modal damping matrix, [R], is diagonal. There are some mechanical systems where the damping matrix is approximately proportional to the stiffness matrix (see also Section . . ),

$$e_{1} = a[\kappa_{1}]$$
 (2-292)

and in this case

$$[R] = [+][B][+] = 3[+][K][+] = 5 [. \qquad (2-2y3)$$

30 that

$$\exists z_{j} = 3 z_{j}^{2} \quad z_{j}^{2}$$

$$(2-2j+)$$

and Education 2-231 becomes

$$\frac{x^{2}}{x^{2}} + 3u_{1} \frac{t_{1}}{t_{2}} + u_{1} \frac{t_{1}}{t_{2}} = \frac{1}{1} \frac{t_{1}}{t_{2}} + \frac{1}{2} \frac{t_{1}}{t_{2}} = \frac{1}{1} \frac{t_{1}}{t_{2}} + \frac{1}{2} \frac{t_{1}}{t_{2}} + \frac$$

If we express this in terms of the "model critical damping factor,"  $\zeta_{\rm i},$  we have

$$\frac{d^2q_i}{dt^2} + z_{ji}\omega_i\frac{dq_i}{dt} + \omega_i^2 q_i = \{q\}_i^2 \{P\}$$
(2-296)

with

$$J_{i} = \frac{\beta}{z} \omega_{i} \qquad (2-297)$$

For this type of damping the critical damping factor is higher in the higher frequency modes.

Lord Rayleigh has given a general proof that the solutions to Equation 2-291 are very little affected by assuming that

$$\pi_{ij} = \sigma \qquad (2-298)$$

$$i \neq j$$

when the damping is small . We then have the fairly general result that

$$\frac{d^2q_i}{dt^2} + 2r_i\omega_i\frac{dq_i}{dt} + \omega_i^2 + i = \{\varphi\}_i^{\prime}\{P\}$$
(2-299)

with

$$S_{i} = \frac{i\psi f_{i}^{\prime}[B] f_{i}\psi}{2\omega_{i}}$$
(2-300)

For structures that are common in the aerospace industry, the damping factor defined in Equation 2-300 can be a very complicated function of the discrete undamped frequencies,  $\omega_{\perp}$ . (Note,  $\omega_{\perp}$  is a description, to some extent, of the stiffness distribution of the structure; $\omega_{\perp}^{\mathcal{I}}$  is sometimes called the generalized stiffness.) Figure 7 shows the type of empirical relations that are obtained from resonant frequency and decay measurements on a wide variety of structures. The last graph is indicative of systems characterized by Equation 2-292.

See Rayleigh, Theory of Sound Dover, 1945, Vol. I, Sec. 102, p. 136 and 137. Recently, Dr. T. K. Caughey has given a much more direct proof of this important result by using modern perturbation methods. See equation (44) in Effect of Damping on the Natural Frequencies of Linear Dynamic Systems by T. K. Caughey and M. E. J. O'Kelly; Journal of the Acoustical Society of America, Vol. 33, No. 11, pp. 1458-1461, November 1961.



FIGURE 7 DAMPING CHARACTERISTICS FROM MEASUREMENTS OF  $\zeta_i \omega_i$  and  $\sqrt{1-\zeta_i^z} \omega_i$ 

The solution to Equation 2-299 is fairly straightforward and we give only the final result so that we may direct our attention to the case where the [R]-matrix is not diagonal and the damping is not small. The Green's function for the system described by Equation 2-299 turns out to be

$$[\dot{u}(t_1)] = \sum_{i=1}^{N} \frac{1}{2} \phi_i \frac{1}{2} \phi_i^2 \phi_i^2 e^{-s_i \omega_L t} - \frac{m \sqrt{1-s_L^2} \omega_i t}{\sqrt{1-s_L^2} \omega_L}$$
(2-30L)

In the general case we must use a method devised by K. A. Foss<sup>1</sup> to find the solution for time dependent forces. A discussion of the general case follows.

<sup>1</sup>K. A. Foss, Coordinates Which Uncouple the Equations of Motion of Damped Linear Dynamic Systems, Journal of Applied Mechanics, Sept., 1958. If we operate on Equation 2-286 with the Laplace transform, we obtain

$$(s^{2}[A] + s[B] + [K]) \{\bar{p}(s)\} = \{\bar{p}(s)\} + s[A] \{\bar{p}(0)\} + [A] \{\bar{p}(0)\} + [B] \{\bar{p}(0)\} \quad (2-302)$$

The formal solution to these equations is given by

$$\{\bar{p}(s)\} = (s^{*}[A] + s[B] + [K])^{-1} (\{\bar{p}(s)\} + s[A]\{\bar{p}(o)\} + [B]\{\bar{p}(o)\})$$

$$(2-303)$$

We recall that, in the case of no damping, the admittance matrix (Equations 2-199 and 2-201) was given by

$$(J^{2}[A] + [K])^{-i} = \sum_{l=1}^{N} \frac{[\psi]_{i}}{s^{2} + \omega_{l}^{2}}$$
 (2-304)

Also, it can be shown that in the case leading to Equation 2-301, we have

$$(s^{2}[A] + s[B] + [K])^{-1} = \sum_{i=1}^{N} \frac{[\Psi]_{i}}{s^{2} + 2s_{i}\omega_{i}s + \omega_{i}^{2}}$$
 (2-305)

where

$$s_{i} = \frac{i\varphi_{i}^{2}(B)i\varphi_{i}}{2\omega_{i}} \qquad (2-306)$$

In ooth of these equations

$$[\psi]_{i} = \{\phi\}_{i} \hat{i}\phi\}_{i}$$
(2-307)

Jur intentions are to show that in the general case of Equation 2-286, we have

$$(2-308)$$

The principal difference betwee: equations 2- 7; and - 3 reflects on the existence of the physical notion of a mode of vibration. This is discussed by F. K. Caughey in <u>Classical Normal Modes in Damped Linear Dynamic Systems</u>, Journal of Applied Mechanics, 27 E. (1960).

where  $[\psi_1]_i$  and  $[\psi_0]_i$  are real matrices and  $\sigma_i + i\omega_i$  is the ith complex root of the characteristic equation

$$\Delta(5) = |S^{2}[A] + S[B] + [K]| = 0 \quad . \tag{2-309}$$

We can reduce Equation 2-286 to a set of first order equations by introducing the generalized velocities as additional "coordinates." If we let

$$\{h\} = \{p\}$$
 (2-310)

then Equation 2-286 can be expressed as

$$[A]{\dot{h}} + [B]{h} + [K]{p} = {p}$$
(2-311)

with

$$\{h_i\} - \{\dot{p}\} = \{o\}$$
 (2-312)

This set of first order equations can also be written as

$$[V]\{\frac{dp^{*}}{dt}\} + [W]\{p^{*}\} = \{p^{*}(t)\}$$
(2-313)

where

$$\begin{bmatrix} V \end{bmatrix} = \begin{bmatrix} A & D \\ 0 & D \\ 0 & D \end{bmatrix}$$
(2-314)

$$[W] = \begin{bmatrix} [B] & [K] \\ - \begin{bmatrix} 1 \end{bmatrix} & (2-315) \end{bmatrix}$$

$$\{p^{\mathbf{t}}\} = \begin{bmatrix} \{\mathbf{h}\} \\ \{\mathbf{p}\} \end{bmatrix}$$
(2-316)

and

$$\{P^{*}\} = \begin{bmatrix} \{P\}\\ \{o\}\end{bmatrix}$$
(2-317)

The Laplace transform of Equation 2-313 is

$$(s[v] + [w]) \{\bar{p}^{*}(s)\} = \{\bar{p}^{*}(s)\}$$
 (2-318)

We have assumed zero initial energy because we can generalize our final result by using Equation 2-303.

Equations 2-318 are easier to solve than the equations

$$(s^{2}[A] + s[B] + [K]) \{\bar{p}(s)\} = \{\bar{p}(s)\}$$
 (2-319)

although it is the latter set in which we are primarily interested.

To solve Equations 2-318 let us first consider the homogeneous equations

$$(s[V] + [W]) \{ \bar{p}^* \} = \{ 0 \}$$
 (2-320)

For non-trivial solutions we must have

$$\Delta(s) = |s[V] + [W]| = |s^{2}[A] + s[B] + [K]| = 0$$
(2-321)

Corresponding to each root (which is, in general, complex), we have a solution to Equations 2-319 or 2-320. If  $s = s_i$ ,  $i = 1, 2 \dots 2N$ , are the solutions to  $\Delta(s) = 0$ , then

$$(s_i [V] + [W]) \{ \varphi^* \}_i = \{ 0 \}$$
 (2-322)

There are also solutions to the transposed equations

$$(s_i [v] + [w]') \{\eta^*\}_i = \{0\}$$
 (2-323)

[V]' = [V] from Equation 2-314

From these equations we can establish the following orthogonality relations

$$\{\chi^*\}_{j}[V] \{\varphi^*\}_{i} = 0 \quad i \neq j$$
 (2-324)

$$\{\eta^*\}'_{i}[W]\{\varphi^*\}_{i} = 0 \quad i \neq j$$
 (2-325)

$$s_i \{\eta^*\}_i [v] \{\varphi^*\}_i + \{\eta^*\}_i [W] \{\varphi^*\}_i = 0$$
 (2-326)

Iet

$$[\varphi^*] = [\{\psi^*\}_1, \{\psi^*\}_2, \dots, \{\psi^*\}_N, \{\bar{\psi}^*\}_1, \{\bar{\psi}^*\}_2, \dots, \{\bar{\psi}^*\}_N]$$
(2-327)

$$[\eta^*] = [\{\eta^*\}, \eta^*\}_{2}, \dots \{\eta^*\}_{N}, \{\eta^*\}_{2}, \dots \{\eta^*\}_{N}]$$
(2-328)

In these matrices, the last N columns are the complex conjugate of the first N columns because the roots to  $\Delta(s) = 0$  are generally complex and will occur in conjugate pairs. If we now consider the nonhomogeneous equations (Equations 2-318) and make the transformation,

$$\{p^*\} = \{\psi^*\}\{\bar{q}^*\}$$
 (2-329)

in Equations 2-310, we obtain

$$[v] + [w] [\psi^*] \{ \bar{\psi}^* \} = \{ \bar{P}^* \}$$
 (2-330)

Premultiply by [r\*]

.

$$[x^*]' = [y] + [w] = [y^*] + [q^*] = [y^*] + [q^*]$$
 (2-331)

From the orthogonality relations (Equations 2-324 and 2-325), we have

$$[v^*] [-[-] + (N] [v^*] = [-+t^*f_1] V H v^*f_1 + \frac{1}{2} v^*f_2]$$
(2-332)

Using Equation 2-320, we have

$$[\eta^*] = [s_{-1}]$$
 (2-333)

where we have assumed the solutions to be normalized so that

Using Equation 2-333 in Equation 2-331, we can solve for  $\{\bar{q}^*\}$ 

$$\{\bar{q}^*\} = \lceil \frac{1}{5} \cdot s_i \perp [\gamma^*]' \{\bar{P}^*\}$$
(2-335)

Substituting this into Equation  $2-329_x$  we have

$$\{\bar{p}^{*}\} = [\phi^{*}]^{\Gamma} \leq s_{*} \rfloor [\gamma^{*}]^{T} \bar{p}^{*} \}$$
(2-33b)

We can relate the modes of Equation 2-322 to the modes which satisfy

$$(s_{i}^{2} [A] + s_{i} [B] + [K]) \{ \varphi \}_{i} = \{ o \}$$
 (2-337)

By comparing Equation 2-337 with Equations 2-322 and 2-323, it can be shown that

$$\{\varphi^*\}_{i} = \begin{bmatrix} s_{i} \{\varphi\}_{i} \\ f \varphi \}_{i} \end{bmatrix}$$
 (2-338)

and

for  $s_i = 0$ 

Substituting these in Equation 2-336 and using Equation 2-31c, we have

$$\{\bar{h}\} = [\varphi]^{\lceil s_{L} \rceil \lceil s_{\neg s_{L}} \rceil \lfloor \varphi \rceil \rceil \langle \bar{f}\bar{P}\}$$

$$(2-34L)$$

.

$$x_i \neq 0$$

$$\{\bar{\mathbf{p}}\} = [\psi]^{\top} \frac{1}{s} \frac{1}{s} \frac{1}{s} [\psi]^{\top} \{\bar{\mathbf{p}}\}$$
(2-342)

.

The following identity is easily verified

$$[\varphi]^{\Gamma} :_{\mathfrak{I}-\mathfrak{s}_{1}}[\varphi]' = \sum_{i=1}^{\mathfrak{Z}} : :_{\mathfrak{I}-\mathfrak{s}_{1}} : \{\varphi\}_{i} : \{\varphi\}_{i} : \{\varphi\}_{i} : (2-343)$$

We can then express Equation 2-342 as

$$\begin{aligned} f\bar{p}f &= \sum_{i=1}^{N} \left( \frac{f\varphi f_{i} f\varphi f_{i}}{z - \bar{z}_{i}} + \frac{f\bar{\varphi} f_{i} f\bar{\varphi} f_{i}}{s - \bar{z}_{i}} \right) \bar{f}\bar{p}f \\ &= \sum_{i=1}^{N} \frac{\left( s - \bar{s}_{i}, \bar{f}\varphi f_{i} f_{i} + (s - s_{i}) f\bar{\varphi} f_{i} f\bar{\varphi} f_{i}}{(s - z_{i})(s - \bar{z}_{i})} \right) \bar{f}\bar{p}f \end{aligned}$$

$$(2-344)$$

If we let

$$z_1 = \tau_1 + L\omega_1$$
 (2-345)

ard

$$[\psi]_{L} = 2 \mathcal{F}_{2} + \psi f_{L} + \psi f_{L} + \psi f_{L} ] \qquad (2-346)$$

$$[\psi]_{L} = -2 \mathcal{R}_{2} (\exists_{L} = \psi f_{L} + \psi f_{L}$$

then

$$\{\bar{p}\} = \int_{-\infty}^{N} \frac{s[\psi_{1}]_{1} + [\psi_{3}]_{2}}{z^{2} - zz_{2}z_{1} + z_{2}^{2} + z_{1}^{2}} + \bar{p}\}$$
(2-346)

Comparing this with Equation 2-303, we must conclude that

$$(s^{2}[A] + s[B] + [K])^{-1} = \sum_{t=1}^{N} \frac{s[\psi_{t}]_{t} + [\psi_{a}]_{t}}{s^{2} - 2\pi_{t}s + \pi_{t}^{2} + \omega_{t}^{2}}$$
(2-349)

The above relation is for restrained systems only and it has been assumed that there are no zero roots. The case of common interest, however, is the case where the zero-frequency modes discussed in Paragraph 2.2.3.4 satisfy the relation

$$[\mathcal{B}] \{ \varphi_{\mathcal{R}} \}_{\tilde{L}} = \{ \alpha \}$$

$$i = L_{x} 2_{x} \cdots M$$

which is characteristic of internal damping in that no dissipation results from a mode in which there is no elastic deformation. The theoretical development is fairly complex in this case, but the result is what would be expected

$$(s^{2}[A] + s[B] + [K])^{-1} = \frac{1}{s^{2}} [\psi]_{\sigma} + \sum_{l=1}^{N-M} \frac{z(\psi_{l}]_{l} + [\psi_{\sigma}]_{l}}{c^{2} - z\sigma_{L}^{2} + \frac{1}{z^{2}} + \omega_{L}^{2}}$$
(2-351)

where  $[\psi]_{a}$  is defined by Equation 2-202.

Numerical methods to solve either Equation 2-322 or Equation 2-337  $^{\rm I}$  are discussed in Argentic III.

In conclusion, we note that the Green's function corresponding to Equation 2-3+9 is

$$[\exists t_{i}] = \sum_{i=1}^{N} \frac{e^{\sigma_{i}t}}{\omega_{i}} (\tau_{i} \circ n \omega_{i}t + \omega_{i} \simeq \omega_{i}t) [\psi_{i}]_{i} + \frac{s^{\tau_{i}t}}{\omega_{i}} \sin \omega_{i}t [\psi_{i}]_{i} \qquad (2-352)$$

and the general solution is the inverse transform of Equation 2-340

. . . . . .

$$fp(t) = \int_{0}^{t} \left\{ G(t-t) \right\} dt \qquad (2-353)$$

<sup>1</sup>If unnormalized solutions to Equation 2-337 are obtained, then the following formulas must be used instead of Equations 2-34t and 2-347.

$$[\psi_{i}]_{i} = -z \,\mathcal{R}_{e} \left( \frac{i\psi_{i}i_{i}\psi_{i}}{i\psi_{i}'[BH\psi_{i}]_{i}} \right) \qquad [\psi_{o}]_{i} = z \,\mathcal{R}_{e} \left( \frac{i\psi_{i}i_{i}\psi_{i}}{i\psi_{i}'[BH\psi_{i}]_{i}} \right)$$

$$fp t_{1}F = \int_{2}^{t} \frac{e^{iy_{2}-t}}{z_{1}} = \frac{e^{iy_{2}-t}}{z_{1}}$$

•

or

.

.

## 2.3 THE APPLICATIONS OF AN INTERPOLATION PROCEDURE FOR OBTAINING A FINITE DEGREE-OF-FREEDOM APPROXIMATION FOR CONTINUOUS ELASTIC STRUCTURES

In Sections 2.1 and 2.2 very little was said about the procedure for replacing a continuous system (with an infinite number of degrees-of-freedom) by one with only a finite number of degrees-of-freedom. In this section, attention will be given to discussion of the specific form of Equation 2-37 in the case of small motions.

## 2.3.1 The Kinematics of Small Motions

Let us suppose that the continuum of particles is in its equilibrium position at time, t = 0. Consistent with the assumptions of Section 2.2, the system of particles executes small motions about the equilibrium configuration. If we denote the position vector of the x-y-z particle at t = 0 by  $\mathbb{L}(x_xy_yz)_y$  then

$$[\pi(x,y,z,t) = \pi(x,y,z,t) - L(x,y,z)$$
 (2-355)

is the displacement vector of the x-y-z particle. Referred to the inertial base vectors introduced in Section 2.1, the position vector, L , has components x, y, and z.

$$\mathbb{L}(x,q,z) = x \mathbb{I} + q \mathcal{I} + z \mathbb{K}$$
 (3-256)

The instantaneous position vector of the x-y-z particle is then

$$\pi(x, q, z, t) = x I + q J + z K_{t} + p(x, q, z, t)$$

$$= (x + p_{x}(x, q, z, t)) I + (q + p_{q}(x, q, z, t)) J + (z + p_{z}(x, q, z, t)) K_{t}$$

$$(2-357)$$



FIGURE 8 A PORTION OF THE SYSTEM DISPLACED FROM ITS EQUILIBRIUM POSITION

All of the assumptions of Section 2.2 are satisfied if we assume that

$$[p(x,y,z,t) = \sum_{t=1}^{N} [t_{t}(x,y,z) p_{t}(t)]$$
(2-350)

We then have

$$\mathbb{L}(p_{1},p_{2},\dots,p_{N};x_{v},y_{v},z) = \mathbb{L}(x_{v},y_{v},z) + \sum_{i=1}^{N} \mathbb{L}_{i}(x_{v},y_{v},z) p_{i} \qquad (2-359)$$

la particular,

$$\frac{\partial \pi}{\partial p_i} = h_i = a \text{ constant}, \text{ independent of the } p_i$$
 (2-300)

as was assumed in Equation 2-122. Equation 2-358, expressed in components referred to I, J, and K, can be written as

$$p_{x}(x, y, z, t) = \sum_{i=1}^{N} n_{x}^{(i)}(x, y, z) p_{i}(t)$$
 (2-3.1)

$$p_{\psi}(x, y, z, t) = \sum_{i=1}^{N} h_{\psi}^{(i)}(x, y, z) p_{i}(t)$$
 (2-362)

$$p_{\mathbf{z}}(\mathbf{x}, \mathbf{y}, \mathbf{z}, \mathbf{t}) = \sum_{i=1}^{N} h_{\mathbf{z}}^{(i)}(\mathbf{x}, \mathbf{y}, \mathbf{z}) p_{i}(\mathbf{t})$$
(2-363)

The form of Equation 2-359 might be justified on the basis of small motions; that is, JL can be expanded in a Taylor's series,

$$\mathbb{I}(p_i, p_{z_r} \dots p_N; x, q_r z) = \mathbb{I}(0, 0, \dots 0; x_r q_r z) + \sum_{i=1}^{N_r} \frac{\partial \mathbb{I}}{\partial p_i} (0, 0, \dots 0; x_r q_r z) p_i$$

$$+ \frac{1}{z!} \sum_{i=1}^{N_r} \sum_{j=1}^{N_r} \frac{\partial^2 \mathbb{I}}{\partial p_i \partial p_j} (0, 0, \dots 0; x, q_r z) p_i p_i p_j$$

$$(2-364)$$

If the pi are "small," then

$$\mathbb{I}_{L}(p_{1}, p_{2}, ..., p_{N}; x, q, z) = \mathbb{L}(x_{1}q_{1}z) + \sum_{i=1}^{N} \frac{\partial \pi}{\partial p_{i}}(2, 0, ..., 0; x, q, z) p_{i}$$
(2-365)

which is the same as Equation 2-359. There is some indication, however, that the form of Equation 2-365 is justifiable even for "large"  $p_i$ .

The choice of the functions,  $h_i(x,y,z)$ , appears to be quite arbitrary; however, we must recall that the generalized coordinates can be independently varied without violating the constraints of the system. If, for example, the system is displaced so that  $p_j = 1$  and  $p_i = 0$  for  $i \neq j$  then

$$p(x,y,z,t) = h_{1}(x,y,z)$$
 (2-366)

From this we conclude that the  $h_{i}$  must satisfy all the physical constraints that are imposed on the displacements, p(x,y,z,t). It also follows that no c of the  $h_{i}$  can be linear combination of the others; for if this were the the generalized coordinates would not be independent. These general conclusions were obtained by Rayleigh and were used as the basis of an approximate method devised by Ritz. The interpolation procedure to be described below is concerned with an appropriate choice of the functions,  $[h_1]$ . This procedure has been considered only briefly in the literature although it has been used at Chance Vought (a parent organization of LTV Astronautics) for many years. The method had its origins in a paper by S. J. Loring published in 1941. By way of introduction, the method might be called a Rayleigh-Ritz approximation "in-the-small." In a very general sense, all finite degree-of-freedom approximations are Rayleigh-Ritz approximations in that the deformations of the structure are considered as a finite linear combination of known (assumed) deformation shapes. In the interpolation procedure the assumed functions are considered to be valid only over small regions of the system. In contrast to the conventional Rayleigh-Ritz technique, very little engineering judgement is required to pick approximate "assumed modes."

## 2.3.2 Interpolation in One-Dimension

To describe the values of a function in a region between points where the function is specified requires that some interpolation procedure be used. Any interpolation formula makes use of an assumed function for describing the ordinates between points where the ordinates are specified. To fix ideas we will consider first a very simple interpolation formula.

If a function, p(x), is specified at a finite number of points in an interval, say (0, L), then it is required to furnish values of the function at other points on the assumption that

$$p(x) = a_{t} x + 4t$$
 (2-367)

for  $x_{i-1} \leq x \leq x_{i}$ 

If  $P_i$  is the value of the function at  $x = x_i$ , then we also require that

$$p(x_i) = p_i$$
 (2-368)

This is commonly called "trapezoidal interpolation" and it approximates the function p(x) by a series of straight lines. Figure 9 shows the interval (0, L) divided into N regions by N + 1 points, 0,  $x_1, x_2...x_{N-1}$ , L.

Loring, S. J., <u>A General Approach to the Flutter Problem</u>, Society of Automotive Engineering Journal, August 1941. This very fundamental paper, although specifically motivated by the wing flutter problem, developed a general approach to all vioration and aeroelastic problems. Loring developed his ideas while at Chance Vought in the period from 1936 to 1948. The general methodology presented in Section 2.0 of this report has principally evolved from Loring's original papers.





We may determine  $e_1$  in  $b_1$  in Equation 2-367 by using Equation 2-368.

$$p_{i+1} = p(x_{i+1}) = u_i x_{i+1} + u_i \qquad (2-369),$$

٠

$$p_{i} = p(x_{i}) = a_{i} x_{i} + 4i \qquad (2-370)$$

Solving for  $a_i$  and  $b_i$  we obtain

•

$$\begin{bmatrix} \alpha_{i} \\ \sigma_{i} \end{bmatrix} = \begin{bmatrix} \chi_{i-1} & i \\ \chi_{i} & i \end{bmatrix}^{-1} \begin{bmatrix} p_{i-1} \\ p_{i} \end{bmatrix}$$

$$= \begin{bmatrix} \frac{-i}{\chi_{i} - \chi_{i-1}} & \frac{i}{\chi_{i} - \chi_{i-1}} \\ \frac{\chi_{i}}{\chi_{i} - \chi_{i-1}} & \frac{-\chi_{i}}{\chi_{i} - \chi_{i-1}} \end{bmatrix} \begin{bmatrix} p_{i-1} \\ p_{i} \end{bmatrix}$$

$$(2-371)$$

If we substitute this into Equation 2-367, we have

$$p(x) = \{x \ i\} \begin{bmatrix} \frac{-i}{x_{i} - x_{i-r}} & \frac{i}{x_{i} - x_{i-r}} \\ \frac{x_{i}}{x_{i} - x_{i-r}} & \frac{-x_{i}}{x_{i} - x_{i-r}} \end{bmatrix} \begin{bmatrix} p_{i-r} \\ p_{i} \\ p_{i} \end{bmatrix} (2-372)$$

Equation 2-372 relates the continuous function, p(x), to the discrete values,  $p_1$ , on the interval (0, L). If the matrix products are expanded, we obtain the more familiar form of the trapezoidal interpolation rule,

$$p(x) = b_{i-1} + \frac{x - x_{i-1}}{x_i - x_{i-1}} (b_i - b_{i-1})$$
for  $x_{i-1} \le x \le x_i$ 
(2-373)

In subsequent discussions we will find it very convenient to replace x by a "local" veriable,  $\xi$ , which varies from 0 to 1 in the region where the interpolation formula is valid

$$S = \frac{\hat{x} - x_{i-1}}{x_{i} - \hat{x}_{i-1}} = \frac{\hat{x} - x_{i-1}}{\ell_{i}}$$
(2-374)

In terms of this variable, Equation 2-373 can be written as

$$p(x) = p(x_{i-1} + \mathcal{I}_{L}) = p_{i-1} + \mathcal{I}(p_{i-1} - p_{i-1})$$
for  $0 \leq \mathcal{I} \leq 1$ 
and  $x_{i-1} \leq x \leq x_{i}$ 

If this is expressed in the form of Equation  $2-372_x$  we have

$$b(x) = b(x_{1-i} + f\ell_1) = fi \quad f \mid f \mid f_1 - f_{1-i} \mid p_{1-i} \mid p_{1-i}$$

or,

where:

$$\begin{bmatrix} \boldsymbol{\zeta} \end{bmatrix}_{\mathbf{\hat{L}}}^{\mathbf{I}} = \begin{bmatrix} \boldsymbol{\mathcal{E}}_{\mathbf{\hat{L}}} & -\boldsymbol{\mathcal{E}}_{\mathbf{\hat{L}}-\mathbf{\hat{I}}} \\ -\mathbf{I} & \mathbf{I} \end{bmatrix}^{\mathbf{I}} = \begin{bmatrix} \mathbf{I} & \boldsymbol{\boldsymbol{\Omega}} \\ -\mathbf{I} & \mathbf{I} \end{bmatrix}$$
(2-378),

for 
$$i = L_2 \dots N$$

For applications to be considered in this report the trapezoidal formula is not adequate. A formula with more desirable properties has been developed by J. A. Griffin, Jr.<sup>1</sup> The formula, which he calls "diparabolic," has a simplicity, flexibility, and stability that is not achieved by many of the common interpolation methods.

The diparabolic formula is defined by averaging the "weighted" parabolic curves through each set of three adjacent points. More specifically, if  $f_{\underline{i}}(\xi)$  is the parabola through the points defined by the ordinates,  $p_{\underline{i}}-2_x p_{\underline{i}}-1_y$  and  $p_{\underline{i}}$ ; and  $f_{\underline{i}}+1$  ( $\xi$ ) is the parabola through the points defined by  $p_{\underline{i}}-1_x p_{\underline{i}}y$  and  $p_{\underline{i}}+1_y$  then the diparabolic formula approximates the function,  $p(x)_y$  by

$$p(x) = (1-3) f_{i}(s) + s f_{i+1}(s)$$
(2-379)

for xit < x < xi

where, as before,

$$s = \frac{x - x_{i-1}}{x_i - x_{i-1}}$$
(2-380)

See Griffin, J. A., A Diparabolic Method of Four-Point Interpolation, Journal of the Aeronautical Sciences, Vol. 28, No. 2, Reader's Forum, February, 1961.



FIGURE TO DIPARABOLIC INTERPOLATION

It is fairly straightforward to show that

$$f_{1}(s) = \{ : s \ s^{z} \ f_{1}(s) = \{ : s \ s^{z} \ f_{1}(s) = \{ : s \ s^{z} \ f_{1}(s) = s \ s^{z} \ s^{z} \ f_{1}(s) = s \ s^{z} \ s^{z}$$

Also, to simplify the discussion given here, we assume that the interval  $(O_r, L)$  is divided into N equal intervals so that

$$x_{i} - x_{i} = i$$
 (2-382)  
for all i

We then have

 $S_{i-2} = -1 \qquad (2-383)$   $S_{i-1} = 0$   $S_{i} = 1$   $S_{i+1} = Z$ 

and

$$f_{\overline{i}}(\underline{s}) = f_{1} \underline{s} \underline{s}^{-1} F \begin{bmatrix} \sigma & i & \sigma \\ -\frac{1}{2} & \sigma & \frac{1}{2} \end{bmatrix} \begin{bmatrix} \mu_{\overline{i}-z} \\ \mu_{\overline{i}-1} \\ \frac{1}{2} & -i & \frac{1}{2} \end{bmatrix} \begin{bmatrix} \mu_{\overline{i}} \\ \mu_{\overline{i}} \end{bmatrix}$$
(2-384)

$$f_{t+1}(\vec{x}) = \{1 \ \vec{x} \ \vec{x}^{-1}\} \begin{bmatrix} 1 \ Q \ Q \\ -\frac{3}{2} \ 2 \ -\frac{3}{2} \ 2 \ -\frac{3}{2} \end{bmatrix} \begin{bmatrix} \mu_{1-2} \\ \mu_{1-1} \end{bmatrix}$$
(2-385)

If we multiply Equation 2-384 by  $(1 - \xi)_r$  then we may rearrange the terms in the form

$$(1-\xi)f_{i}(\xi) = \frac{1}{\xi} + \frac{1}{\xi} \xi^{2} \xi^{2} \frac{1}{\xi} \begin{bmatrix} \sigma & 1 & \sigma \\ -\frac{1}{\xi} & -\frac{1}{\xi} \end{bmatrix} \begin{bmatrix} \mu_{i-1} \\ \mu_{i} \end{bmatrix}$$
(2-386)  
$$\begin{bmatrix} -\frac{1}{\xi} & -\frac{1}{\xi} \\ -\frac{1}{\xi} & 1 & -\frac{1}{\xi} \end{bmatrix}$$

and in a similar manner,

.

$$\mathcal{S}_{f_{i}}(\mathcal{S}) = \frac{1}{2} | \mathcal{S}_{\mathcal{S}}^{*} \mathcal{S}^{*}_{f_{i}} \left[ \begin{array}{ccc} \alpha & \alpha & \alpha \\ \alpha & \alpha & \alpha \\ \beta & \alpha & \alpha \\ -\frac{3}{2} & z & -\frac{1}{2} \\ \frac{1}{2} & -\frac{1}{2} & z \\ \frac{1}{2} & -\frac{1}{2} & z \end{array} \right] \left[ \begin{array}{c} p_{i_{+}} \\ p_{i_{+}} \\ p_{i_{+}} \\ p_{i_{+}} \\ \frac{1}{2} & z \end{array} \right]$$
(2-387)

Substitution of these results in Equation 2-379 gives the desired form of the diperabolic formula.

$$p(x) = \{ : \mathcal{S} \ \mathcal{S}^{\perp} \mathcal{S}^{\perp} \} \begin{bmatrix} \alpha & : & \alpha & 0 \\ -\frac{1}{2} & \alpha & \frac{1}{2} & \alpha \\ : & -\frac{5}{2} & 2 & -\frac{1}{2} \\ -\frac{1}{2} & \frac{\pi}{2} & -\frac{\pi}{2} & \frac{1}{2} \end{bmatrix} \begin{bmatrix} p_{i-z} \\ p_{i-1} \\ p_{i-1} \\ p_{i-1} \\ p_{i+1} \end{bmatrix}$$

$$veIid \text{ for } x_{i-1} \leqslant x \leqslant x_{i-1}$$

$$\alpha \leqslant \mathcal{S} = \frac{x - x_{i-1}}{\mathcal{L}} \leqslant 1$$

The above formula may be written more concisely as

$$p(x) = \{i \in \mathcal{L}^{*} \} [i] \{ p \}_{i} \qquad \text{for} \quad x_{i+1} \leq x \leq x_{i} \qquad (2-38g)$$

where

When unequal intervals are used, the interpolation coefficients, [, -], are not the same for every interval; however, the set of 4 x 4 matrices,

 $\begin{cases} \mu_{i} \\ \mu_{i} \\ \mu_{i} \\ \mu_{i} \\ \mu_{i} \\ \mu_{i} \end{cases}$ 

$$[J]_{i}$$
  $i = 1, 2... N$  (2-391)

(2 - 390)

can be calculated when the coordinates, xi, are specified.

It can be shown that the diparabolic formula defines a continuous curve with a continuous derivative on the interval from x = 0 to  $x = L_{\star}$ . This important property of "smoothness" follows from the fact that Equation 2-389 satisfies

$$\lim_{5 \neq 1} \frac{dp}{dx}(x_{i}, +5L) = \lim_{5 \neq a} \frac{db}{dx}(x_{i} + 5L)$$
(2-392)

Special consideration must be given to the end points if it is undesirable to have the collocation points,  $x = x_i$ , fall outside of the interval (0, L). These points may be eliminated by artibrarily imposing the conditions

$$\frac{d^2 b}{dx^2}(0) = \frac{d^2 b}{dx^2}(L) = 0$$
 (2-393)

In most applications this constraint introduces very little error<sup>1</sup>. Using the relations

$$\frac{d^{2} b}{d x^{2}} (\alpha) = \frac{1}{l^{2}} \{ \alpha \ \sigma \ \lambda \ \alpha \} [ x ] \begin{bmatrix} p_{1} \\ p_{0} \\ p_{1} \\ p_{2} \end{bmatrix} = 0$$

$$(2-394)$$

$$\frac{d^{2}p}{dx^{2}}(L) = \frac{1}{\ell^{2}} \left\{ \sigma \ 0 \ z \ 6 \ \right\} \begin{bmatrix} p_{N-2} \\ p_{N+1} \\ p_{N} \\ p_{N+1} \end{bmatrix} = 0$$

$$(2-395)$$

we can derive the following results which are appropriate for the first and  $\ensuremath{\mathbb{N}}^{\mathrm{th}}$  intervals

$$p(x) = \frac{1}{4} + \frac{5}{5} + \frac{5^{2}}{5^{2}} \frac{5^{3}}{5^{2}} \begin{bmatrix} 1 & 0 & 0 \\ -\frac{5}{4} & \frac{3}{2} & -\frac{1}{4} \\ 0 & 0 & 0 \\ -\frac{1}{4} & -\frac{1}{2} & \frac{1}{4} \end{bmatrix} \begin{bmatrix} p_{0} \\ p_{1} \\ p_{2} \end{bmatrix}$$
(2-396)
$$for = 0 \le x \le x_{1}$$

$$p(x) = \frac{1}{4} + \frac{5}{5} + \frac{5^{3}}{5^{3}} \begin{bmatrix} \alpha & 0 & 0 \\ -\frac{1}{2} & 0 & \frac{1}{2} \\ -\frac{1}{2} & 0 & \frac{1}{2} \\ -\frac{1}{2} & -\frac{1}{2} & \frac{1}{2} \end{bmatrix} \begin{bmatrix} p_{N-2} \\ p_{N-1} \\ p_{N} \end{bmatrix}$$
(2-397)
$$\begin{bmatrix} -\frac{1}{4} & -\frac{1}{4} & -\frac{1}{4} \\ -\frac{1}{4} & -\frac{1}{4} \end{bmatrix}$$
for  $X_{N-1} \le x \le L$ 

.

<sup>&</sup>lt;sup>L</sup>No attempt is being made here to satisfy the "natural" boundary conditions for any particular problem. It is a consequence of the use of the Principles of Analytical Mechanics that the natural boundary conditions do not have to be considered, but are automatically satisfied.

#### 2.3.3 <u>Application of the Interpolation Technique to Some Typical Structures</u> Problems

The concept of relating a continuous function to discrete values, as described above, can be generalized and applied to structures of widely varying geometries. To illustrate the technique we will consider briefly three problems:

- (1) the vibration of a uniform beam,
- (2) the vibration of a uniform simply-supported plate, and
- (3) the vibration of a uniform thin ring.

The numerical solution to these problems is compared with the exact solutions for the continuous case which is governed by a partial differential equation.

#### 2.3.3.1 The Vibration of a Uniform Beam

The specific internal (strain) energy for a particle of a beam is

$$u(x,y,z,t) = \frac{1}{2} \in e_{rx}^{2}$$
 (2-398)

If  $\mathtt{P}_Z(x,t)$  is the lateral displacement of the neutral axis of the beam, the strain is given by

$$\epsilon_{xx} = - \pm \frac{\partial^2 \beta_z}{\partial x^2}$$
(2-399)



FIGURE 11 UNIFORM BEAM

The total strain energy (Equation 2-56) is

$$U = \int u \, dV \qquad (2-400)$$
  
= 
$$\iiint_{\lambda} E \left(z \frac{\partial^{2} \beta z}{\partial x^{2}} \pm x \, dy \, dz\right)$$
  
= 
$$\sum_{\lambda} \int_{0}^{L} \iint E z^{2} dy \, dz \left(\frac{\partial^{2} \beta z}{\partial x^{2}}\right)^{2} dx$$
  
= 
$$\sum_{\lambda} \int_{0}^{L} EI(x) \left(\frac{\partial^{2} \beta z}{\partial x^{2}}\right)^{2} dx$$

The total kinetic energy (Equation 2-42) is

$$T = \frac{1}{2} \int \frac{2\pi^2}{2\pi^2} dx,$$

$$= \frac{1}{2\pi^2} \int \frac{4\pi^2}{2\pi^2} dx + \frac{1}{2\pi^2} \int dx$$

$$= \frac{1}{2\pi^2} \int \frac{4\pi^2}{2\pi^2} dx$$

$$= \frac{1}{2\pi^2} \int \frac{4\pi^2}{2\pi^2} dx$$

$$= \frac{1}{2\pi^2} \int \frac{4\pi^2}{2\pi^2} dx$$

In these expressions, EI(x) is the beam "binding rigidity" and m(x) is the mass per unit of length.

We want the elastic displacement of the continuous beam to be approximated by displacements at a finite number of discrete points. The procedure makes use of the diparabolic interpolation formula developed in Paragraph 2.3.2. For convenience, we will consider the points to be spaced at N equal intervals. The length of these intervals is

$$i = \frac{1}{x_1} = x_1 - x_{1-1}$$
 (2-402)

If we denote the displacements at the points,  $x = x_i$ , by  $p_i$ , then

$$\dot{p}_i(t) = \dot{p}_{\mathcal{E}}(x_i, t) \tag{2-403}$$

and from Equations 2-389, 2-396, and 2-397 we have

$$p_{z}(x,t) = \{i \ s \ s^{2} \ s^{3} \ t \ [s]_{i} \ \{b\}_{i}$$
(2-404)

for 
$$x_{\overline{i}-1} \le x \le x_{\overline{i}}$$
  
$$\overline{z} = \frac{1}{k} (x_{-} x_{\overline{i}-1})$$

where

$$\begin{bmatrix} J \end{bmatrix}_{i} = \begin{pmatrix} 1 & 0 & 0 \\ -54 & \frac{3}{2} & -44 \\ 0 & 0 & 0 \\ -4 & -5 & 44 \end{bmatrix}$$
 for  $i = 1$  (2-405)  
$$\begin{cases} 0 & 1 & 0 & 0 \\ -\frac{1}{2} & 0 & \frac{1}{2} & 0 \\ 1 & -55 & 2 & -54 \\ -\frac{1}{2} & \frac{3}{2} & -\frac{3}{2} & \frac{1}{2} \end{bmatrix}$$
 for  $i = 2, 3 \dots N-1$   
$$\begin{bmatrix} 0 & 1 & 0 \\ -\frac{1}{2} & 0 & \frac{1}{2} \\ \frac{3}{4} & -\frac{3}{2} & \frac{3}{4} \\ -\frac{1}{4} & \frac{1}{2} & -\frac{1}{4} \end{bmatrix}$$

for i = N

We can use Equation 2-404 to express the kinetic energy and strain energy in terms of the discrete displacements,  $p_i$ . Let us first write Equations 2-401 and 2-400 as

$$\tau = \sum_{i=1}^{N} \int_{x_{i-i}}^{x_i} \frac{\partial p_i}{\partial t} dx$$
 (2-406)

$$U = \frac{1}{2} \sum_{i=1}^{N} \int_{x_{i-1}}^{x_i} \left( \frac{\partial^2 p_z}{\partial x^2} \right)^2 dx \qquad (2-407)$$

and then make the following change of variable-of-integration

$$x = x_{i-1} + \xi s$$
 (2-408)

for which

$$dx = \ell d\mathcal{E} \tag{2-409}$$

and

.

$$\frac{\partial^2 b_x}{\partial x^2} = \frac{1}{\ell^2} \frac{\partial^2 b_z}{\partial z^2}$$
(2-410)

On substituting these into Equations 2-406 and 2-407, we find

$$\tau = \frac{1}{2} \sum_{t=1}^{N} \int_{a}^{b} m \left( \frac{2b_{E}}{\partial t} \right)^{2} \ell ds \qquad (2-411)$$

$$U = \frac{1}{2} \sum_{j=1}^{N} \int_{0}^{1} E \left[ \frac{d^{2} p_{z}}{dx^{2}} \right]^{2} \frac{1}{\ell^{3}} dz \qquad (2-412)$$

From Equation 2-404 we have

$$\frac{\partial p_z}{\partial t} = \frac{1}{2} (z^2 z^3) [z]_1 \dot{z} \dot{p} \dot{z}_i$$
(2-413)

$$\frac{\partial^2 P_z}{\partial s^2} = \{0 \ 0 \ z \ os \ \{[s]_1 \ i \ b\}_i \quad \text{for } x_{1-i} < x \le x_1 \quad (2-414)$$

By transposing Equations 2-413 and 2-414 and multiplying them by themselves, we obtain

$$\left(\frac{\partial^{2} p_{z}}{\partial z^{z}}\right)^{n} = \frac{1}{2} p_{t} \int_{t}^{t} \left[ \mathcal{F}_{t} \right]_{t}^{t} \left[ \begin{array}{c} 0 \\ 0 \\ z \\ \epsilon \end{array} \right] = \frac{1}{2} p_{t} \int_{t}^{t} \left[ \mathcal{F}_{t} \right]_{t}^{t} \left[ \begin{array}{c} 0 \\ 0 \\ z \\ \epsilon \end{array} \right] = \frac{1}{2} \left[ \begin{array}{c} 2 - 4 + 16 \\ 0 \\ z \\ \epsilon \end{array} \right]$$

$$(2-4+16)$$

.

Substitution of Equations 2-415 and 2-416 into Equations 2-411 and 2-412 gives

$$T = \frac{1}{2} \sum_{i=1}^{N} \{\hat{p}\}_{i} [J]_{i}^{\prime} \int_{0}^{1} m\ell \begin{bmatrix} 1 & g & g^{2} & g^{3} \\ g & g^{2} & g^{3} & g^{4} \\ g^{2} & g^{3} & g^{4} & g^{5} & g^{4} \end{bmatrix} df [J]_{i} \{\hat{p}\}_{i}$$
(2-417)

and

If we define

$$[A]_{i} = [J]_{i}^{\prime} \int_{0}^{1} m \mathcal{L} \begin{bmatrix} i \ \xi \ \xi^{2} \ \xi^{3} \\ \xi \ \xi^{2} \ \xi^{3} \ \xi^{4} \\ \xi^{2} \ \xi^{3} \ \xi^{4} \ \xi^{5} \\ \xi^{3} \ \xi^{4} \ \xi^{5} \ \xi^{5} \end{bmatrix}$$
(2-419)
$$\begin{bmatrix} w \\ i \end{bmatrix}_{i} = \begin{bmatrix} z \end{bmatrix}_{i=1_{0}}^{i} \begin{bmatrix} e \\ Ei \end{bmatrix} \begin{bmatrix} e \\ c \\ 0 \end{bmatrix} \begin{bmatrix} c \\ 0 \end{bmatrix} \begin{bmatrix} e \\ e \end{bmatrix}_{i} \begin{bmatrix} e \\ e \end{bmatrix}_{i}$$

$$\begin{bmatrix} c \\ 0 \end{bmatrix} \begin{bmatrix} e \\ e \end{bmatrix}_{i}$$

$$\begin{bmatrix} e \\ e \end{bmatrix}_{i}$$

then a laborious numerical calculation gives

.

$$\begin{bmatrix} A \end{bmatrix}_{i} = \begin{cases} \frac{m_{i} f}{6720} \begin{bmatrix} 1880 & 1224 & -164 \\ 1224 & 2264 & -288 \\ -164 & -286 & 32 \end{bmatrix} \quad \text{for } i = I. \quad (2-42I)$$

$$\begin{cases} \frac{m_{i} f}{672a} \begin{bmatrix} 6 & -186 & -12a & 12 \\ -168 & 212a & 1226 & -12a \\ -168 & 212a & 1226 & -12a \\ -12a & 1228 & 272a & -188 \\ 12 & -12a & -168 & 16 \end{bmatrix} \quad \text{for } i = 2_{3}3 \cdots N-I.$$

$$\begin{cases} \frac{m_{i} f}{672a} \begin{bmatrix} 32 & -286 & -164 \\ -226 & 324 & -226 \\ -164 & 1224 & 1680 \end{bmatrix} \quad \text{for } i = N$$

$$\begin{bmatrix} \mathsf{K} \end{bmatrix}_{\mathbf{i}} = \begin{cases} \frac{\Xi \mathbf{i}}{4E^{2}} \begin{bmatrix} \bar{s} & -6 & \bar{s} \\ -6 & iz & -6 \\ \bar{s} & -4 & \bar{s} \end{bmatrix} & \text{for } \mathbf{i} = \mathbf{I} \\ \frac{\Xi \mathbf{i}}{4E^{2}} \begin{bmatrix} 4 & -6 & 6 & -2 \\ -c & 25 & -26 & 8 \\ g & 26 & 25 & -16 \\ -2 & g & -16 & 4 \end{bmatrix} & (2-422) \\ \frac{\Xi \mathbf{i}}{4E^{2}} \begin{bmatrix} \bar{s} & -6 & \bar{s} \\ -6 & 25 & -16 & 4 \\ -7 & 5 & -16 & 4 \end{bmatrix} & (2-422) \\ \frac{\Xi \mathbf{i}}{4E^{2}} \begin{bmatrix} \bar{s} & -6 & \bar{s} \\ -6 & 2 & -6 \\ -6 & 3 \end{bmatrix} & 37 \end{cases}$$

.

and

and

.

For a nonuniform beam  $[A]_i$  and  $[K]_i$  must be calculated using a numerical integration (this is discussed in Paragraph 2.3.3.4). In either case, Equations 2-417 and 2-418 can be written as

$$\tau = \frac{1}{2} \sum_{i=1}^{N_{k}} \hat{i} \hat{p} \hat{f}_{i} [A]_{i} \hat{p} \hat{f}_{i}$$
(2-423)

.

$$u = \frac{1}{2} \sum_{i=1}^{N} \frac{1}{2} p f_{\hat{i}} [K]_{\hat{i}} \frac{1}{2} p f_{\hat{i}}$$
(2-4.24)

If we define

.

.

$$\{ p \} = \begin{bmatrix} p_{\alpha} \\ p_{1} \\ \vdots \\ p_{N_{\alpha}} \\ \vdots \\ p_{N_{\alpha}} \end{bmatrix}$$
 (2-425)

then we can introduce transformation matrices that are defined by the relations

$$\{ p \}_{i} = [ T ]_{i} \{ p \}$$
 (2-426)

٠

It is fairly evident that this transformation has the following form

$$[T]_{i} = \begin{bmatrix} 2 & 2 & 2 & 2 & 2 & 2 & 2 & 2 \\ 0 & 0 & 1 & 2 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$(2-427)$$

i<sup>th</sup> column

If we substitute Equation 2-426 into Equations 2-423 and 2-424, we obtain

$$\tau = \frac{1}{2} \{ \hat{p} \} [A \mathbb{H} \hat{p} ]$$
 (2-428)

$$-= itpl[k][p]$$
 (2-429)

where

.

$$[x] = \sum_{j=1}^{N_{t}} [T]_{t} [A]_{t} [T]_{t} \qquad (2-430)$$

enđ

$$[r_{1}] = \sum_{l=1}^{N} [r_{l_{1}}, r_{l_{1}}] = ]_{l_{1}}$$
 (2-431)

For N = 10, we obtain the following result by using Equations2-421 and 2-422.

$$\begin{bmatrix} A \end{bmatrix} = \underbrace{\mathbf{mL}}_{67,200}$$
 1896 1036 -284 12  
1036 5990 752 -240 12  
-284 752 5488 752 -240 12  
12 -240 752 5472 752 -240 12  
12 -240 752 5472 752 -240 12  
12 -240 752 5472 752 -240 12  
12 -240 752 5472 752 -240 12  
12 -240 752 5472 752 -240 12  
12 -240 752 5472 752 -240 12  
12 -240 752 5472 752 -240 12  
12 -240 752 5472 752 -240 12  
12 -240 752 5472 752 -240 12  
12 -240 752 5488 752 284  
12 -240 752 5990 1036  
12 -284 1036 1896

(2-432)

Using Equations 2-428 and 2-429 in Legrange's equations, we obtain

$$[A + b] + [K + b] = \{a\}$$
 (2-434)

which are the equations considered in Paragraph 2.2.3.1 (Equation 2-145). For the unrestrained beam, there are zero-frequency solutions to the eigenvalue problem:

$$(-\omega^{2}[A] + [K]) \frac{1}{2}\phi = \frac{1}{2}0$$
 (2-435)

This is evidenced by the fact that the stiffness matrix (Equation 2-433) is singular. The problem requires the special consideration given in Paragraph  $2\cdot2\cdot3\cdot4\cdot$ . There are two zero-frequency modes and they may be chosen as



The first mode represents an equal translation of every point on the beam,  $P_0 = P_I = \dots P_{N-I} = P_N = 1$ . The second mode represents a rigid-body rotation about the mid-point of the beam, x = L/2. It is easily verified that these are zero-frequency modes by premultiplying them by the stiffness matrix (Equation 2-433) and noting that

$$\{k, k\}_{q, k} = \{1\}$$
  $\{k, k\}_{q, k} = \{1\}$   $\{2-437\}$ 

For the purpose of calculating an influence coefficient matrix, we assume the following constraints

$$p_z(L,t) = 0$$
 (2-438)

$$\frac{\partial p_z}{\partial x} (2-439)$$

which are just sufficient to prevent rigid-body motion. These constraints "cantilever" the beam at x = L. Using Equation 2-404, Equations 2-438 and 2-439 lead to

$$\begin{aligned}
\varphi_{z}(L_{r}L) &= \begin{cases} 1 & 1 & 1 \\ 1 & 1 & 1 \end{cases} \begin{bmatrix} 0 & 1 & 0 \\ -\frac{1}{2} & 0 & \frac{1}{2} \\ \frac{5}{4} & \frac{3}{2} & \frac{3}{4} \\ -\frac{1}{4} & \frac{1}{2} & -\frac{1}{4} \end{bmatrix} \begin{bmatrix} \varphi_{N-z} \\ \varphi_{N-1} \\ \varphi_{N} \end{bmatrix} = 0 \end{aligned} \tag{2-440}$$

$$\frac{3p_{z}}{3x}(L,L) = \frac{1}{2} \{0 + 25\} \left[ \begin{array}{cc} 0 + 0 \\ -\frac{1}{2} & 0 \\ \frac{3}{2} & \frac{3}{2} \\ \frac{3}{2} & \frac{3}{2} \\ \frac{3}{4} & \frac{3}{4} \\ \frac{3}{4} & \frac{3}{2} \\ \frac{3}{4} & \frac{3}{4} \\ \frac{3}{4} & \frac{3}$$

or

.

$$\begin{bmatrix} 2 & 1 & 0 \\ p_{N-2} & = 1 & 0 \end{bmatrix}$$

$$\begin{bmatrix} 2 & -\frac{1}{2} & \frac{2}{3} \\ p_{N-1} & p_{N-1} \end{bmatrix}$$

$$\begin{bmatrix} p_{N-1} & p_{N-1} \\ p_{N-1} \end{bmatrix}$$

which we may solve to find that

$$p_{N} = 0$$
 (2-443)

$$p_{N-1} = \frac{1}{6} p_{N-2}$$
 (2-444)

We may also write these constraints in the form of the [S]-matrix discussed in Paragraph 2.2.3.4 (Equation 2-225).

$$\begin{bmatrix} p_{\sigma} \\ p_{i} \\ p_{z} \\ \vdots \\ p_{N-z} \\ p_{N-z} \\ p_{N-q} \end{bmatrix} = \begin{bmatrix} 1 & 0 & 0 & \dots & 0 \\ 0 & i & 0 & \dots & 0 \\ 0 & 0 & 1 & \dots & 0 \\ \vdots & & \vdots & \vdots \\ 0 & 0 & 0 & \dots & i \\ 0 & 0 & 0 & \dots & 0 \end{bmatrix}$$

If we let the coefficient matrix in the above equation be  $\left[ S \right]$  , then

$$[E] = [S] [S] [K] [S] [S]$$
(2-446)

is an influence matrix for the beam cantilevered at x = L. The  $[\Gamma]$ -matrix defined in Equation 2-255, in this particular case, is

$$\begin{bmatrix} - & \\ -$$

Solution of the equation,

.

by iteration gave the following values for the non-dimensional frequency parameter,  $\omega_{1}^{2}\,\frac{\mathrm{mL}}{\mathrm{EI}}$ 

TABLE T FREQUENCIES OF UNIFORM BEAM, COLLOCATION METHOD

	i = l	ĭ = 2	i = 3
WI TEI	517.66	4246.77	17,819.1

The exact solution of the partial differential equation,

$$EI\frac{\partial^{4}p_{z}}{\partial x^{4}} = m\frac{\partial^{4}p_{z}}{\partial t^{2}} \qquad (2-449)$$

for the case of free-free boundary conditions gives 1

RABLE Z FREQUENCIES OF UNIFORM DEAM, EAACT SOLUTIO	TABL	E 2	FREQUENCIES	OF UNIFORM	BEAM. EX	ACT SOLUTION
--	------	-----	-------------	------------	----------	--------------

7 mil.4	i = 1	i=2	i = 3
र्ष्य हो	500-42	3303 .19	14,616.8

Also, for comparison, the first two vibration modes are given in Table 3

#### TABLE 3 MODES OF UNIFORM BEAM

Exact SolutionExact Solution $\begin{bmatrix} \phi_1^2 \\ = \\ 0.093 \\ 0.093 \\ 0.0977 \\ 0.271 \\ 0.0271 \\ 0.0514 \\ 0.599 \\ 0.678 \\ 0.599 \\ 0.6078 \\ 0.0977 \\ 0.514 \\ 0.5203 \\ 0.000 \\ 0$	FIRST MODE			SECOND MODE	
$ \begin{bmatrix} \phi_1^2 \\ 0.531 \\ 0.093 \\ 0.0977 \\ 0.271 \\ 0.0514 \\ 0.5503 \\ 0.5512 \\ 0.0271 \\ 0.0271 \\ 0.0514 \\ 0.5503 \\ 0.5514 \\ 0.5503 \\ 0.000 \\$		Exact Solution			Exact Solution
[1.000] 1.0000 [-1.000] -1.0000	$ \begin{cases} \phi_1^7 = \begin{bmatrix} 1.000 \\ 0.531 \\ 0.093 \\ -0.271 \\ -0.514 \\ -0.599 \\ -0.514 \\ -0.271 \\ 0.093 \\ 0.531 \\ 1.000 \end{bmatrix} $	L.0000 0.5372 0.0977 -0.2720 -0.5203 -0.6078 -0.5203 -0.2720 0.5272 L.0000	$\{\phi\}_2 =$	L.000 0.212 -0.393 -0.639 -0.462 0.000 0.462 0.639 0.393 -0.212 -1.000	1.0000 0.2274 -0.3973 -0.6620 -0.4830 0.0000 0.4830 0.6620 0.3973 -0.2274 -1.0000

<sup>1</sup>These values are taken from Scanlan and Rosenbaum, <u>An Introduction to the</u> Study of Aircraft Vibration and Flutter, Macmillan, 1951, p. 146.

The exact modes were obtained from <u>Tables of Characteristic Functions Repre</u>senting Normal Modes of Vibration of a Beam, University of Texas Publication No. 4913, July 1, 1941, by Dana Young and Robert P. Felgar, Jr. The comparison, for this case of 10 intervals, shows the diparabolic method to give favorable results. The agreement can be expected to be much better when more intervals are used. These results should also be compared with the "complementary strain energy" method discussed in Section 5.1.1.2. (A numerical comparison is considered in Section 5.2.1 of this report.)

In conclusion, we note that the interpolation scheme can be viewed as a Rayleigh-Ritz method. Equation 2-363, in the case of a beam, is

$$p_{z}(x,t) = \sum_{i=\alpha}^{N+1} h_{z}^{(i)}(x) p_{i}(t)$$
 (2-450)

Comparison of this with Equation 2-404 will show that the "assumed modes,"  $h_{x}(1)_{(x)}$ , have the form indicated in Figure 12.





The function in Figure 12 is defined by

$$\begin{aligned} h_{\mathcal{L}}^{(1)}(x) &= \left\{ \begin{array}{cccc} 0 & \text{when } x < x_{1-x} & (2-451) \\ -0.5\left(\frac{x-x}{L}t^{2}\right) + 0.5\left(\frac{x-x}{L}t^{2}\right)^{2} & 2t-x < x_{1-1} \\ 0 & z \cdot 5 + 2\left(\frac{x-x}{L}t^{2}\right)^{2} + 1.5\left(\frac{x-x}{L}t^{2}\right)^{3} & x_{1-1} \leq x \leq x_{1-1} \\ 1.0 - 2t \cdot 5\left(\frac{x-x}{L}t^{2}\right)^{2} + 1.5\left(\frac{x-x}{L}t^{2}\right)^{3} & 2t \leq x \leq x_{1-1} \\ -0.5 + \left(\frac{x-x}{L}t^{2}\right)^{2} - 0.5\left(\frac{x-x}{L}t^{2}\right)^{4} & x_{1+1} \leq x \leq x_{1+2} \\ 0 & z > x_{1+2} \end{aligned}$$

For comparison, Figure 13 shows the "assumed modes" corresponding to the trapezoidal interpolation assumption.



FIGURE 13 THE "ASSUMED MODES" CORRESPONDING TO THE TRAPEZOIDAL INTERPOLATION METHOD

# 2.3.3.2 The Vibration of a Uniform Simply-Supported Plate

The specific internal energy for a particle of a plate is

$$u(x, y, z, t) = \pm \frac{E}{1 - \gamma^2} \left( e_{xx}^2 + e_{yy}^2 + 2\gamma e_{xx} e_{yy} \right) + \pm \frac{E}{2 \cdot (+\gamma)} e_{xy}^2 \qquad (2-452)$$

If  $P_Z(x,y,t)$  is the lateral displacement of the neutral surface of the plate, the components of strain are given by

$$E_{ix} = -E \frac{\partial^2 p_e}{\partial x^2} \qquad (2-453)$$

$$z_{qq} = -z \frac{z^2 p_z}{z q^2}$$
 (2-454)

$$z_{xy} = -z_z \frac{z^2 p_z}{z_x z_y}$$
 (2-455)



FIGURE 14 UNIFORM PLATE

The total strain energy is

(2-456)

$$U = \int u \, dV$$

$$= \iiint \left[ \frac{Ez^2}{2(1-\gamma^2)} \left( \frac{\partial^2 p_z}{\partial x^2} \right)^2 + \left( \frac{\partial^2 p_z}{\partial q^2} \right)^2 + 2\gamma \left( \frac{\partial^2 p_z}{\partial x^2} \right) \left( \frac{\partial^2 p_z}{\partial q^2} \right) + 2(1-\gamma) \left( \frac{\partial^2 p_z}{\partial x \partial q} \right)^2 \right) dx dq dz$$

$$= \frac{1}{2} \int_0^{\infty} \int_{-6}^{6} E\Gamma(x,q) \left( \frac{\partial^2 p_z}{\partial x^2} \right)^2 + \frac{\partial^2 p_z}{\partial q^2} + 2\gamma \left( \frac{\partial^2 p_z}{\partial x^2} \right) \left( \frac{\partial^2 p_z}{\partial q^2} \right)^2 \right) dx dq$$

where

$$\exists I : x, y = \int \frac{\exists z^{2}}{1 - y^{2}} dz \qquad (2-457)$$

is the plate "bending rigidity." For a plate of uniform thickness,  $\boldsymbol{\tau}$  , we have

$$ET = \frac{E\tau^{3}}{(z^{-1} - y^{2})}$$
 (2-458)

The total kinetic energy of the plate is

$$\tau = \frac{1}{2} \int \rho \left( \frac{dp_z}{dt} \right)^z dV$$

$$= \frac{1}{2} \int_{0}^{1} \int_{-t}^{t} \int \rho tz \left( \frac{dp_z}{dt} \right)^z dx dy$$

$$= \frac{1}{2} \int_{0}^{1} \int_{-t}^{t} m(x,y) \left( \frac{dp_z}{dt} \right)^z dx dy$$
(2-459)

where m(x,y) is the mass per unit of area which, for a uniform plate, is

$$m/x, q = m = p_{c}$$
 (2-460)

In analogy to the procedures in Paragraph 2.3.3.1, we will divide the plate into a number of equal regions as shown in Figure 15.



FIGURE 15 COLLOCATION POINTS ON THE PLATE

If we let N be the number of equal intervals in the y-direction and M be the number of intervals in the x-direction, the length of these intervals will be

$$w = \frac{L}{N}$$
(2-461)

$$\ell = \frac{zk}{M} \tag{2-462}$$

We then introduce the following "local" coordinates

$$\xi = \frac{x - x_i}{g} \qquad (2 - \frac{1}{2} 63)$$

$$\gamma = \frac{q - q_t}{ar}$$
(2-464)

where  $(x_i, y_i)$  are the coordinates of the upper left corner (in Figure 16) of the 1<sup>th</sup> region.

For the i<sup>th</sup> region it is possible to construct an interpolation formula which is a two-dimensional generalization of Equation 2-404.

(2-465)

$$p_{\Xi'}x_{u_{1}+1} = \{f(\Xi_{u})\} [J]_{u} \{p\}_{u}$$

where

 $\dot{z} \, \dot{z} \,$ 

The displacement at any point in the region is given in terms of the discrete displacements at 16 neighboring points as shown in Figure 16. The resulting matrix of interpolation coefficients is 16 by 16.



FIGURE 16 NUMBERING OF POINTS IN THE ith REGION

The interpolation coefficients for edge regions are derived by eliminating the virtual points with the arbitrary conditions,

$$\frac{\partial^2 p_z}{\partial s^2} + 0.3 \frac{\partial^2 p_z}{\partial \eta^2} = 0$$
 (2-467)

anà

$$\frac{\delta^2 p_{\pm}}{\delta \xi \delta \eta} = 0. \tag{2-1468}$$

evaluated at the two discrete points in the region which fall on the edge. This leads to

for edges on the top (x = -b), in this case). In this relation,  $|\zeta|$  is the twodimensional equal interval interpolation matrix for interior regions which is tabulated in Appendix IV. Equation 2-469 can be used to eliminate points 1, 2, 3 and 4, and arrive at an interpolation matrix which is 16 by 12. A similar procedure can be used to obtain interpolation coefficients for the bottom, left-hand, and right-hand edges. The edge interpolation coefficients are also tabulated in Appendix IV.

<sup>1</sup>The comments made previously about Equation 2-393 apply here also. The motive for choosing Equations 2-467 and 2-468 stems from the Kirchoff conditions for a plate (see Section 22 on page 83 of Timoshenko and Woinowsky-Krieger, Theory of Plates and Shells, McGraw-Hill, 1959). From Equation 2-465 we have

$$\frac{2p_{z}}{2t} x_{i} x_{i} t = \{ f(s, 7) \} \{ j \}_{i} \{ j \}_{i} \}$$
(2--70)

•

and from this,

$$\frac{\partial E}{\partial t} = ip I_1 [I_1]_1 + r s \eta i t (s s \eta i b [I_1]_1 + p I_1$$

$$(2-471)$$

Iet us write Equation 2-459 as

$$\tau = \sum_{\substack{i=1\\j=1}}^{k \cdot n} \iint \pi \left( \frac{2}{2k} + ix \right)$$
 (2-472)

and make the following change of variable-of-integration

$$x = l_{2}^{2} - x_{1}^{2}$$
 (2-473)

$$x_{t} = x_{t} - x_{t}$$
 (2-474)

for which

•

$$x_{14} = x_{13}^2 x_{7}^2 \qquad (2-x_{13}^2)$$

Equation 2-72 Lecomes

Substituting Equation 2--- 1, we get

$$= -\sum_{i=1}^{\infty} \frac{1}{12} \sum_{i=1}^{\infty} \frac{1}{12} \sum_{i$$

where

$$[A]_{i} = [J]_{i} \int_{0}^{0} m w L \{ i \in \eta \} \{ i \in \eta \} \}$$
 (2-478)

The matrix,

$$[7,] = \int_{0}^{1} \int_{0}^{1} \frac{1}{3} f(s, \gamma) f$$

is tabulated in Appendix IV of this report. We can also write the strain energy in a similar manner. From Equation 2-456, we write

$$u = z_{1} \sum_{j=1}^{n-1} \sum_{j=1}^{n-1} z_{j} \sum_{i=1}^{n-1} z_{i} + \frac{z_{i}^{2}}{z_{i}^{2}} + z_{i}^{2} \frac{z_{i}^{2}}{z_{i}^{2}} + z_{i}^{2$$

From Equation 2-465, we have

$$\frac{z_{F_{E}}}{z_{F}} = \frac{1}{2^{2}} + \frac{z_{F}}{z_{F}} + \frac{z_{F}}{z$$

.

$$\frac{2}{2} = \frac{3}{2} \frac{3}{2} \frac{3}{2} \frac{1}{2} \frac{$$

$$\frac{2^{2}F_{e}}{3^{2}\delta_{1}} = \frac{1}{w-1} \left\{ \frac{3^{2}}{3^{2}\delta_{1}} \right\} \left[ \frac{1}{2} \right]_{i} \left[ p_{i}^{2} \right]_{i} \left[ \frac{1}{2} \right]_{$$

We then define

$$\left[ n_{1i} = \frac{EI}{wL} \left[ \sum_{i=1}^{i} \frac{w_{i}}{L} \right]^{2} \left[ \Gamma_{2}^{2} \right] + \frac{2}{w}^{2} \left[ \Gamma_{3}^{2} \right] + 2\sqrt{[\Gamma_{3}]} + 2\sqrt{[\Gamma_{3}]} \left[ \frac{1}{2} \right]_{1i}$$
(2-4.64)

.

where

$$[\Gamma_{z}] = \int_{0}^{1} \int_{0}^{1} \frac{3^{2} f}{3s^{2}} F \frac{3^{2} f}{3s^{2}} F \frac{3^{2} f}{3s^{2}} f' ds d\eta \qquad (2-405)$$

$$[\Gamma_{3}] = \int_{0}^{1} \int_{0}^{1} \frac{\xi \frac{\partial^{2} f}{\partial \eta^{2}}}{\xi \frac{\partial^{2} f}{\partial \eta^{2}}} F \frac{\partial^{2} f}{\partial \eta^{2}} F' ds d\eta \qquad (2-486)$$

$$[\Gamma_{*}] = \int_{\alpha}^{\beta} \int_{\alpha}^{\alpha} \frac{1}{2} \left( \frac{3\frac{2}{2}}{3\frac{2}{2}} \frac{1}{2} \frac{3\frac{2}{2}}{3\frac{2}{2}} \frac{1}{2} \frac{3\frac{2}{2}}{3\frac{2}{2}} + \frac{3\frac{2}{2}}{3\frac{2}{2}\frac{2}{2}} \frac{1}{2} \frac{3\frac{2}{2}}{3\frac{2}{2}\frac{2}{2}} \right) \frac{1}{3\frac{2}{2}} \frac{1}{2} \frac{3\frac{2}{2}}{3\frac{2}{2}} \frac{1}{2} \frac{1}{3\frac{2}{2}} \frac{1}{2} \frac{1}{3\frac{2}{2}} \frac{1}{2} \frac{1}{3\frac{2}{2}} \frac{1}{2} \frac{1}{3\frac{2}{2}} \frac{1$$

$$[\Gamma_{5}] = \int_{\alpha} \int_{\alpha} \frac{\xi \frac{2f}{2}}{353\pi} f \frac{2^{\frac{2}{3}}}{353\pi} f \frac{454\pi}{454\pi} \qquad (2-488)$$

(These matrices are also tabulated in Appendix IV.)

Equation 2-480 can then be written as

$$= \sum_{i=1}^{n-m} i p F_i [\kappa I_i + p F_i$$
 (2-489)

As in Paragraph 2.3.3.1, we introduce transformation matrices defined by

$$\frac{1}{2} F_{i} = [T_{i} + F]$$
 (2-430)

For the case of N = 4 and M = 4, there are 25 points (see Figure 15). For the case of simple supports on all edges, it can be shown that the interpolation formula (Equation 2-4t5) dictates that  $p_i = 0$  for all points on the edges. If the remaining points are numbered as shown in Figure 17, then

$$f F F = \begin{bmatrix} c \\ p_z \end{bmatrix}$$

$$\begin{bmatrix} p_z \\ \vdots \\ p_z \end{bmatrix}$$

and the transformation matrices in Equation 2-490 have zeros in the appropriate rows corresponding to the boundary points.



FIGURE 17 COLLOCATION POINTS FOR SIMPLY-SUPPORTED PLATE

Equations 2-477 and 2-489 then become

$$\tau = \frac{1}{2} \{ \hat{p} \}$$
 (2-492)

and

$$v = \frac{1}{2} \{p\}'[\kappa] \{p\}$$
 (2-493)

where

$$[A] = \sum_{i=1}^{N-M} [T]_{i} [A]_{i} [T]_{i}$$
(2-494)

and

$$[\kappa] = \sum_{i=1}^{n-M} [\tau]'_i[\kappa]_i[\tau]_i \qquad (2-495)$$

For this problem the inertia and stiffness matrices are 9 by 9. The influence coefficients are given by

$$[E] = [\kappa \vec{l}]$$
 (2-496)

These matrices are listed in Tables 6 and 7 for a steel plate (  $\nu = 0.3$ ) with L/b = 3. The following tables compare the frequencies calculated from the equation,

$$[E][A][\{\varphi\} = \lambda \{\varphi\}$$
(2-497)

with the "exact" solutions1.

wimLt	1 = 1	i=2	i=3	1 = 4	i=5	
εI	1,098	4,378	11,827	16,316	19,600	

TABLE 4 FREQUENCIES OF SIMPLY - SUPPORTED PLATE, L/b = 3,  $\nu = 0.3$ , DIPARABOLIC COLLOCATION

TABLE 5 FREQUENCIES OF SIMPLY - SUPPORTED PLATE, L b = 3, EXACT SOLUTION

<u>ت</u> بن 104 ا. <sup>4</sup>	i = 1	i=2	i=3	i=4	i=5
=_	1,029	<u>5</u> 05, 3	9,740	12,328	15,585

The exact solutions are obtained from Timoshenko and Woinowsky-Krieger; Theory of Plates and Shells, McGraw-Hill, 1959, Section 20, by using equation (g) on page 335.

	/									
	1.413	0+904	0+364	1.355	1.109	Q.487	074a	0.708	0+337	-
•	0.904	1.806	α.,904.	1.12	1.846	1-112	0.708	1.080	a. 708	* 10- <u>1</u>
	0-36h.	a.;;c4	1-413	0.487	L-109	1-355	0-337'	0.708	a74a	
	1-355	1.12	0.487	5.13	L-57	a.683	L-350	1.12	0.487	
Ì	1.109	1.846	1.109	1.568	2.811	1.56ã	1-109	1.846	1.109	
	a_497	1.12	1-355	£83.0	1-568	2,129	7.487	1.12	1-355	
1	0-740	0708	0-337	1-355	1.109	<b>0.</b> 487	1.413	₫+3cj <sup>n</sup>	0-364	
	0.,708	1.080	0-708	1.12	1.846	1.12	a-30r	1.806	0-904,	
ĺ	0-337	0.708	a_740.	0.487	1.109	1.355	0-364.	0-904	L.hI3	
	<b>~</b>									

### TABLE 6 INFLUENCE MATRIX FOR A SIMPLY - SUPPORTED PLATE

TABLE 7 INERTIA MATRIX FOR A SIMPLY - SUPPORTED PLATE

-	~									
ſ	1.,+5	ن		u-e71	للوفارب	-1.020	-0-079.	-0.010	500.0	-
	<b>a.</b> 25⊥	يتلدا	u.291	للوقاءت	<u>्र</u> स्य]	0-73T	-3.010	0.072	-0.010	x 10 <sup>-1</sup>
ļ.	-2.273	0.271	L-993	-0-510	u.831	0.491	0.003	-0.010	-3-379	
ŀ	C.251	125.0	-1-414	1-81-	0.237	-1-072	0.251	0.031	-0.010	
ł	a.331	J.227	لغدمه	3-2-1	1.00	9.227	3.012	0.227	0.J]I	
ŀ	-3-313	J.631	3.291	-0.072	4.227	1.511	-2.010	a.031	0.251	
	-ù	-3.310	0.003	1.2.1	للوديي	-4-610	1.353	0.251	-13-1374	
ţ.	-0.010	0.07Z	-3.010	3-031	4.227	2.031	251	1.81	3	
ľ	0.001	-1.CLI	-0-575	-0-010	2, 192	in L		6-242	1. ·* ي	
-	•									-

## 2.3.3.3 The Vibration of a Uniform Thin Ring

.

In the general development and also in the specific problems we have considered, it has been convenient to use rectangular coordinates, (x,y,z), as the Lagrangian particle variables. For a thin ring, however, the geometry is better suited to a set of cylindrical coordinates for use as Lagrangian variables. For this purpose, we introduce  $(r, \theta, x)$  such that

-

$$\begin{aligned} x &= x \\ y &= rtcos \Theta \\ z &= rtsin \Theta \end{aligned}$$
 (2-498)

The specific internal energy for a particle of a thin ring is then

$$\mathcal{U}(\pi,\Theta,\chi,t) = \frac{1}{2} \frac{E}{1-\gamma^2 z} \epsilon_{\Theta\Theta}^2 \qquad (2-499).$$



FIGURE 18 UNIFORM THIN RING

A position vector for the  $r-\theta-x$  particle is

$$\mathbb{I}(\pi, \theta, x, t) = \mathbb{L}(\pi, \theta, x) + \mathbb{P}(\theta, t)$$
(2-500)

where

•

.

$$p = p_{rc}(e_{r}t) \dot{F}_{r} + p_{\theta}(e_{r}t) \dot{F}_{\theta}$$
 (2-501)

The strain is related to the tangential and radial components of displacement by

$$\epsilon_{\mathbf{e}\mathbf{e}_{i}} = \frac{1}{\delta_{\mathbf{r}}} \left( \frac{\partial \mathbf{p}_{\mathbf{e}_{i}}}{\partial \mathbf{e}_{i}} + \mathbf{p}_{\mathbf{r}_{i}} \right) - \left( \frac{\mathbf{r}_{\mathbf{r}} - \mathbf{f}_{\mathbf{r}}}{\delta_{\mathbf{e}} \mathbf{r}_{i}} \left( \frac{\partial^{2} \mathbf{p}_{\mathbf{r}_{i}}}{\delta_{\mathbf{e}} \mathbf{r}_{i}} + \frac{\partial \mathbf{p}_{\mathbf{e}_{i}}}{\delta_{\mathbf{e}} \mathbf{r}_{i}} \right)$$

$$(2-502)$$

The total strain energy is

$$U = \int_{0}^{2\pi} u \, dV \qquad (2-503)$$

$$= \frac{1}{2} \int_{0}^{2\pi} \int_{0}^{L} \int_{\xi-\frac{\pi}{2}}^{\xi+\frac{\pi}{2}} \frac{E}{1-\gamma^{2}} \left( \frac{1}{4r} \left( \frac{\partial p_{\theta}}{\partial \theta} + p_{\pi} \right) - \left( \frac{\pi-b}{b^{2}} \left( \frac{\partial^{2} p_{\pi}}{\partial \theta^{2}} + \frac{\partial p_{\theta}}{\partial \theta} \right) \right)^{2} dr dr dr dr d\theta$$

$$= \frac{1}{2} \int_{0}^{2\pi} \frac{EL}{1-\gamma^{2}} \left( \frac{1}{4r} \left( \frac{\partial p_{\theta}}{\partial \theta} + p_{\pi} \right)^{2} + \frac{\tau^{3}}{ikt^{4}} \left( \frac{\partial^{2} p_{\pi}}{\partial \theta^{2}} + \frac{\partial p_{\theta}}{\partial \theta} \right)^{2} dr d\theta$$

$$= \frac{1}{2} \int_{0}^{2\pi} \frac{EL}{(2(1-\gamma^{2}))} \left( \frac{\frac{d^{2} p_{\pi}}{b^{2}} - \frac{\partial p_{\theta}}{b^{2}}}{\frac{1}{b^{2}} \left( \frac{1}{b^{2}} - \frac{\partial p_{\theta}}{b^{2}} + \frac{1}{b^{2}} \right)^{2} dr d\theta + \frac{1}{2} \int_{0}^{2\pi} \frac{ELT}{(1-\gamma^{2})} \left( \frac{dp_{\theta}}{dr\theta} + \frac{1}{b^{2}} \right)^{2} dr d\theta$$

bending energy

### tensile energy

The total kinetic energy is

$$T = \frac{1}{2} \int_{0}^{2\pi} e^{\frac{\partial \pi}{\partial t}} - \frac{\partial \pi}{\partial t} dV \qquad (2-504)$$
$$= \frac{1}{2} \int_{0}^{2\pi} \int_{0}^{L} \int_{6-\frac{\pi}{2}}^{6+\frac{\pi}{2}} e^{\left(\left(\frac{\partial p}{\partial t}\right)^{2} + \left(\frac{\partial p}{\partial t}\right)^{2}\right) + \left(\frac{\partial p}{\partial t}\right)^{2}} d\sigma d\sigma$$
$$= \frac{1}{2} \int_{0}^{2\pi} e^{\tau L} \left(\left(\frac{\partial p}{\partial t}\right)^{2} + \left(\frac{\partial p}{\partial t}\right)^{2}\right) d\sigma d\sigma.$$

Let the circumferential length be divided into N equal segments each of arc length,

$$w = \frac{2\pi b}{N} \qquad (2-505)$$



FIGURE 19 COLLOCATION POINTS

The location of the ith collocation point is given by

$$\Theta = \Theta_{i} = \frac{3\pi}{N} i$$
(2-506)

We then write Equation 2-504 as

$$\tau = \sum_{z=1}^{N} \int_{e_{\overline{z}-1}}^{e_{\overline{z}}} e^{z} L\left(\left(\frac{\partial p_{\overline{z}}}{\partial t}\right)^{z} + \left(\frac{\partial p_{\overline{z}}}{\partial t}\right)^{z}\right) L de \qquad (2-507)$$

and introduce a local variable,  $\eta$ , such that

$$\eta = \frac{k_{\rm C} - k_{\rm C}}{n_{\rm c}}$$
 (2-508)

Then, in Equation 2-507, we make the change of variable

$$lr \theta = w \eta + k \theta_{\tilde{L}} \tag{2-509}$$

$$\tau = \frac{1}{2} \sum_{i=1}^{N} \int_{a}^{b} m \left( \left( \frac{dpe}{dt} \right)^{2} + \left( \frac{dpe}{dt} \right)^{2} \right) m \ln \eta \qquad (2-510)$$

where

$$m = ; cl$$
 (2-511)

If we define

$$p_{t}^{(t)}(t) = b_{t}^{(t)}(\theta_{t}, t)$$
 (2-512)

enă.

$$\mathbf{c}_{\boldsymbol{\Theta}}^{(\mathbf{t})}(\mathbf{t}) = \mathbf{p}_{\boldsymbol{\Theta}}^{(\mathbf{t})}(\mathbf{t}) \tag{2-513}$$

then we may approximate the functions  $p_r(\theta_r t)$  and  $p_{\theta}(\theta_r t)$  by use of the diparabolic formula.

$$p_{\eta}(\boldsymbol{\alpha},\boldsymbol{L}) = \{ \boldsymbol{\gamma} \mid \eta \mid \eta^{L} \eta^{3} \boldsymbol{L} \{ \boldsymbol{J} \mid \boldsymbol{J} \mid \boldsymbol{\gamma}_{L} \}$$

$$(2-5L4)$$

$$p_{\sigma}(a_{t}t) = f_{1} \eta \eta^{2} \eta^{3} f[s] f_{\rho_{\sigma}} f_{1}$$
(2-515)

for 
$$\theta_{i}, \leq \theta \leq \theta_{i}$$

where (see Equation 2-388)

$$\begin{bmatrix} x \end{bmatrix} = \begin{bmatrix} 0 & i & 0 & 0 \\ -\frac{1}{2} & 0 & \frac{1}{2} & 0 \end{bmatrix}$$
(2-516)  
$$\begin{bmatrix} -\frac{1}{2} & \frac{1}{2} & -\frac{1}{2} \\ -\frac{1}{2} & \frac{1}{2} & -\frac{1}{2} \end{bmatrix}$$

for every interval, and

$$+ \tau_{\tau} \Big]_{L} = \begin{bmatrix} \rho_{\tau}^{(1)} & \vdots & \rho_{\Xi}^{(1)} \\ \rho_{\tau}^{(1)} & & \rho_{\Xi}^{(1)} \\ \rho_{\tau}^{(1)} & & \rho_{\Xi}^{(1)} \\ \sigma_{\tau}^{(1)} & & \rho_{\Xi}^{(1)} \\ \end{bmatrix}$$
(2-517)

Substitution into Equation 2-510 gives

$$\tau = \sum_{i=1}^{N} \left( \frac{1}{2} \dot{p}_{\pi} f_{i} \left[ A_{\pi\pi} \right] \frac{1}{2} \dot{p}_{\pi} f_{i} + \frac{1}{2} \dot{p}_{\pi} f_{i} \left[ A_{GG} \right] \frac{1}{2} \dot{p}_{\pi} f_{i} \right]$$
(2-518)

.

$$\begin{bmatrix} A_{n\pi} \end{bmatrix}_{i} = \begin{bmatrix} A_{\theta\theta} \end{bmatrix}_{1} = \begin{bmatrix} r \end{bmatrix}^{\prime} \int_{0}^{1} m \begin{bmatrix} 1 \\ \eta \\ \eta^{2} \\ \eta^{3} \end{bmatrix} = \begin{bmatrix} n & \eta^{2} & \eta^{3} \end{bmatrix} \text{ were } \eta \begin{bmatrix} r \\ r \\ \eta^{3} \end{bmatrix}$$
(2-513)

$$= \frac{W'W'}{672,\alpha} \begin{bmatrix} 16\alpha & -158 & -12\alpha & :z \\ -158 & 2720 & 1228 & -120 \\ -150 & 1228 & 2720 & -188 \\ 12 & -12\alpha & -189 & 16 \end{bmatrix}$$
for all i,  $I_{1}, I_{2}, I_{2}, I_{2}, I_{2}, I_{2}$ 

For the strain energy, we have

.

$$J = \frac{1}{2} \sum_{k=1}^{N} \int_{-\pi}^{1} EE \left( \frac{1}{w^2} \frac{\partial^2 p_{\pi}}{\partial \eta^2} + \frac{1}{wt} \frac{\partial^2 p_{\eta}}{\partial \eta} \right)^2 w d\eta + \int_{-\pi}^{1} \frac{ELE}{1-\gamma^2} \left( \frac{1}{w} \frac{\partial^2 p_{\eta}}{\partial \eta} + \frac{\mu_{\pi}}{v} \right)^2 w d\eta$$

$$= \frac{1}{2} \sum_{\tau=1}^{N} \int_{-\pi}^{1} EE \left( \frac{1}{w^3} \left( \frac{\partial^2 p_{\pi}}{\partial \eta^2} + \frac{w}{v} \frac{\partial p_{\eta}}{\partial \eta} \right)^2 + \frac{12}{v^2 w} \left( \frac{\partial^2 p_{\eta}}{\partial \eta} + \frac{w}{v} \frac{p_{\pi}}{v} \right)^2 \right) d\eta$$
(2-520)

where

$$\Xi r = \frac{\varepsilon^2}{\varepsilon (r^2)}$$
 (2-521)

and from Equation - 25,

$$\frac{w}{b} = \frac{1}{2}$$
 (2-522)

Uning Equations 2-74 and 2-515, we have

$$\frac{dp_{n}}{d\eta} + \frac{b_{n}}{d\eta} = \frac{1}{2}\eta + \lambda\eta + \eta^{2}f(z)f(p_{n}) + \frac{d}{d\eta} + \eta^{2}\eta^{3}f(z) + p_{n} \frac{1}{d\eta}$$
(2-523)

and.

.

.

$$\frac{\partial^2 b_n}{\partial \eta^2} + \frac{w}{b} \frac{\partial b_0}{\partial \eta} = \{0 \ 0 \ z \ 6\eta \ f[z]f[p_n]_i + \frac{\lambda \pi}{N} \{0 + z\eta \ 3\eta^2\}[z]hp_0\}, \quad (2-524)$$

Substituting these into the strain energy, we obtain

$$U = \frac{1}{2} \sum_{\mu=1}^{N} \frac{1}{2} p_{\mu} f'_{\mu} [\kappa_{\pi\pi}]_{i} f \mu_{\pi} f_{i} + 2 \frac{1}{2} p_{\mu} f'_{i} [\kappa_{\pi\pi}]_{i} f \mu_{\theta} f_{i} + \frac{1}{2} p_{\theta} f'_{i} [\kappa_{\theta\theta}]_{i} f \mu_{\theta} f_{i}$$

$$(2-525)$$

where

$$[\kappa_{qn}]_{i} = [J]' \int_{0}^{1} \frac{H}{W^{3}} \begin{bmatrix} 0 \\ 0 \\ z \\ 6\eta \end{bmatrix} \{ 0 \ 0 \ 2 \ 6\eta \} d\eta + \int_{0}^{1} \frac{ET}{W^{3}} (z \frac{E}{T})^{2} (\frac{2\pi}{N})^{4} \begin{bmatrix} 1 \\ \frac{\pi}{N^{2}} \\ \frac{\pi}{N^{3}} \end{bmatrix} \{ 1 \ \eta \ \eta^{2} \eta^{3} \} d\eta \setminus [J]$$
 (2-526)

$$\begin{bmatrix} \kappa_{n0} \end{bmatrix}_{i} = \begin{bmatrix} r \end{bmatrix}^{\prime \prime} \begin{bmatrix} 1 \\ 0 \\ ws \end{bmatrix}^{\prime} \begin{bmatrix} w \\ N \end{bmatrix}_{0} \begin{bmatrix} v \\ ws \end{bmatrix}^{\prime} \begin{bmatrix} v \\ N \end{bmatrix}_{0} \begin{bmatrix} v \\$$

$$[\kappa_{\Theta\Theta}]_{i} = [x]^{\prime} \left( \int_{0}^{1} \frac{\varepsilon_{1}}{w_{3}} \left( 1 + 12 \frac{k^{2}}{v_{3}} \right) \left( \frac{2\pi}{N} \right)^{2} \left[ 0 + i 0 + 2\pi 3\eta^{2} \right] t_{ij} \left( \frac{1}{2} \right) \left( \frac{2}{2} - 528 \right)$$

If the definite integral in these expressions is evaluated and the matrix products are evaluated, the following numerical results are obtained

$$\begin{bmatrix} k_{\Pi,1} \end{bmatrix}_{1} = \frac{EI}{103} + \frac{1}{4} \begin{bmatrix} 4 - 10 & 8 - 2 \\ -10 & 18 - 12 & 8 \\ 8 - 26 & 23 - 16 \\ -2 & 3 - 10 & 4 \end{bmatrix} + \frac{12}{12} \left[ \frac{L}{2} \right]^{\frac{1}{2} + \frac{1}{N}} \left\{ \begin{array}{c} 18 - 128 & -120 & -12 \\ -88 & 2720 & -128 & -120 \\ -20 & 1228 & 2720 & -128 \\ -12 & -124 & -168 \\ -12 & -124 & -124$$

$$\left[ K_{TLG} \right]_{i} = \frac{EI}{W^{3}} \left( \frac{1}{16} \begin{bmatrix} -1 & -3 & 5 & -1 \\ 3 & 1 & 7 & 3 \\ -3 & 7 & -1 & -3 \\ 1 & -5 & 3 & 1 \end{bmatrix} + \frac{12(\frac{1}{C})^{2}(\frac{2\pi}{N})^{3}}{240} \begin{bmatrix} 0 & 11 & -12 & 1 \\ -11 & -120 & 143 & 12 \\ 12 & -140 & 120 & 11 \\ -1 & 12 & -11 & 0 \end{bmatrix} \right)$$
(2-530)

$$[\kappa_{\Theta\Theta}]_{i} = \frac{EI}{W^{3}} \frac{(1+12(\frac{L}{2})^{2})(\frac{2T}{N})^{2}}{120} \begin{bmatrix} 4 & -7 & 2 & 1 \\ -7 & 12b & -13i & 2 \\ 2 & -13i & 13b & -7 \\ 1 & 2 & -7 & 4 \end{bmatrix}$$
 (2-531)

Finally, we introduce the definitions

.

$$\begin{bmatrix} \{ \mathfrak{p}_n \}_i \end{bmatrix} = \{ \top \}_i \{ \mathfrak{p}_0 \}_i$$

$$\begin{bmatrix} \{ \mathfrak{p}_n \}_i \end{bmatrix}$$

$$(2-532)$$

in which we identify (see Figure 20),

$$\Theta_{-1} = \Theta_{N-1}$$
 (2-533)

$$e_{N+1} = 0,$$
 (2-534)



FIGURE 20 RELATION BETWEEN THE FIRST AND LAST INTERVAL

If we define,

.

$$\{p\} = \begin{cases} p_{t}^{(1)} \\ p_{t}^{(2)} \\ \vdots \\ p_{t}^{(N)} \\ \vdots \\ p_{t}^{(N)} \\ z_{3}^{(N)} \\ z_{5}^{(N)} \\ \vdots \\ z_{6}^{(N)} \\ z_{6}^{(N)} \end{bmatrix}$$
 (2-535)

then

$$\tau = \frac{1}{2} \left\{ p \right\} \left[ A + p \right]$$
 (2-536)

and

$$u = \frac{1}{2} \{p\}'[\kappa H p\}$$
 (2-537)

where

and

.

$$\begin{bmatrix} A \end{bmatrix} = \sum_{i=1}^{N} \begin{bmatrix} T \end{bmatrix}_{i}^{\prime} \begin{bmatrix} A_{ren} \end{bmatrix}_{i} \quad \begin{bmatrix} r \circ_{i} \\ \sigma_{i} \end{bmatrix} \begin{bmatrix} T \end{bmatrix}_{i}^{\prime} \quad (2-538)^{\prime}$$

$$[\kappa] = \sum_{i=1}^{N} [\tau]'_{i} \begin{bmatrix} \kappa_{n\tau}]_{i} & \kappa_{n\theta}]_{i} \end{bmatrix} [\tau]_{i} \\ \begin{bmatrix} \kappa_{n\theta}\end{bmatrix}'_{i} & \kappa_{\theta\theta}\end{bmatrix}_{i} \end{bmatrix} (2-539)$$

It can be verified that there are three zero-frequency modes for the unrestrained ring which represent displacements of the ring as a rigid body. These modes can be taken as:

The first represents a unit rotation while the last two represent unit translations in the y and z directions.

The solution to the eigenvalue problem,

$$(-\omega^{2}[A] + [K] + [\psi] = \{0\}$$
 (2-541)

follows that outlined in Paragraph 2.2.3.4. For N = 8,  $(\frac{2\pi}{4} = 0.78539816)$  the following results are obtained

$$\begin{bmatrix} A_{\Pi\Pi} \end{bmatrix} = \begin{bmatrix} A_{\Pi\Theta} \end{bmatrix} = \frac{m_{\Lambda}^{2} \pi^{\frac{1}{2}} \kappa_{\Lambda}^{2}}{\frac{5\pi^{2}}{6720}} \begin{bmatrix} \frac{5m_{\mu}}{8\pi_{\mu}} & \frac{\pi}{82} & \frac{\pi}{8} & \frac{\pi}{$$

•

#### 2.3.3.4 The Analysis of Structures with Nonuniform Properties

When the inertia and stiffness properties of the structure are nonumiform, then integrals like those in Equations 2-419, 2-420, 2-478, 2-519, 2-526, 2-527, and 2-528 must be evaluated by some numerical process. This may easily be done with the aid of a digital computer by using, for example, a Simpson's rule integration. Another alternative is to use an approximate method based on the mean value theorem. To illustrate, consider Equation 2-419 in the case where m(x) is not constant.

$$[A]_{i} = [J]_{i} \int_{0}^{n} m(x) \begin{bmatrix} 1 & 3 & 3^{2} & 5^{3} \\ 5 & 3^{2} & 5^{3} \end{bmatrix} L ds [J]_{i}$$

$$[S = [J]_{i} \int_{0}^{n} m(x) \begin{bmatrix} 1 & 3 & 3^{2} & 5^{3} \\ 5 & 3^{2} & 5^{3} & 5^{4} \\ 5^{2} & 5^{3} & 5^{4} \end{bmatrix}$$

$$(2-546)$$

By the mean value theorem for integrals, there are values of m(x) in the interval  $x_{i-1} \leqslant x \leqslant x_i$  such that

$$\int_{0}^{1} m(x) d\xi = m_{i} \int_{0}^{1} d\xi = m_{i} \qquad (2-5!+7)$$

$$\int_{0}^{1} m(x) \, \xi \, d\xi = m_{t}^{(1)} \int_{0}^{1} \xi \, d\xi = \xi \, m_{t}^{(1)} \tag{2-548}$$

$$\int_{a}^{t} m(x) s^{2} ds = m_{t}^{(z)} \int_{a}^{t} s^{2} ds = \frac{1}{3} m_{t}^{(z)}$$

$$(2-549)$$

$$\int_{0}^{1} w(x) \, s^{b} ds = m_{1}^{(b)} \int_{0}^{1} s^{b} ds = \frac{1}{7} m_{1}^{(b)}$$
(2-550)

From Equation 2-547 we note that  $m_i$  is the average value of m(x) on the interval,  $x_{i-1} \leq x \leq x_i$ . On the basis that m(x) is a "slowly" varying function, we may make the approximation

$$m_{1}^{(i)} = m_{1}^{(2)} = \dots = m_{1}^{(6)} = m_{1}^{(6)}$$
 (2-551)

We then have

$$[A]_{i} = [J]'_{i} m_{i} \ell \begin{bmatrix} 1 & \frac{1}{2} & \frac{1}{3} & \frac{1}{4} \\ \frac{1}{2} & \frac{1}{3} & \frac{1}{4} & \frac{1}{3} \\ \frac{1}{3} & \frac{1}{4} & \frac{1}{5} & \frac{1}{6} \\ \frac{1}{4} & \frac{1}{5} & \frac{1}{5} & \frac{1}{6} \end{bmatrix}$$
(2-552)

which reduces to Equation 2-421 with m replaced by  $m_i$ . This approximation is not good when there are "concentrated" mass items, but these can be considered separately, on the basis that they act like Dirac delta-functions. Say, for example, that

$$M'x = m + m_{R} \tilde{S}(x - x_{R})$$
 (2-553)

that is, m(x) is constant with the exception of a single concentrated mass,  $m_k$ , at  $x = x_k$ .  $\mu(x)$  is the Dirac "function" with the property

$$\int \dot{U}^{2} x - x_{e} \sqrt{2} x^{2} dx = f(x_{e})$$
(2-554)

in this case Equation 2-546 becomes

.

where

Using the property described in Equation 2-554, we have

as the contribution to [A] , from the concentrated mass at  $x = x_k$ .

In a practical case the actual distribution, m(x), can be broken down into two parts: one is a slowly varying part which can be approximated as in Equation 2-522; the second is a part which can be idealized as a concentrated item. Figure 21 is a typical example



FIGURE 21 APPROXIMATION TO DISTRIBUTION OF STRUCTURAL PROPERTIES

3.0 METHODS IN DYNAMICS AND AERCELASTICITY FOR SLIMPLER LAUXCE VEHICLES IN FLAME MOTION

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#### 3.1 THE LINEAR AEROELASTIC EQUATIONS GOVERNING THE SMALL LATERAL MOTIONS OF A SLENDER LAUNCH VEHICLE

Most aspects of launch vehicle dynamics are adequately described by a set of linear equations. The linear analyses also form a firm conceptual basis for the understanding of dynamics problems involving nonlinearities. Traditionally, nonlinear dynamic analyses have only been considered in missile performance where the determination of the trajectory has assumed the missile to be rigid. There has been some question, in the past, whether the linear equations, in terms of small motions from an inertial-axis, are equivalent to a set of nonlinear body-axis equations which have been linearized. This question is of fundamental importance in the discussion of Section 3.2. In this section, however, we will review the methods of aeroelastic analysis for small motions from an inertial frame-of-reference<sup>1</sup>.

For the purposes of illustration, we will confine our attentions to the dynamics of a slender launch vehicle whose aeroelastic properties are sufficiently described as functions of a single coordinate, x, measured from the nose of the missile and considered positive in the aft direction. We will also assume that there is a mechanism for control and guidance. This might be a gimbaled engine, jet vane, and/or aerodynamic control. We will assume, however, that it is an aerodynamic control device for the purposes of concentrating on the method of analysis. Further we will assume that the control surface is rigid. Many of the assumptions made in this section are not essential but are made only to simplify the preliminary discussion. It is an advantage of the methods of Analytical Mechanics that many of the derived relations are independent of the geometry of the particular system being considered.

### 3.1.1 The Kinetic and Potential Energy and the Virtual Work of Aerodynamics and Servo Forces

## 3.1.1.1 The Calculation of Basic Data for a Slender Controlled Vehicle

#### 3.1.1.1.1 The Kinetic Energy

If we let the displacement of the axis of the missile (from an inertial frame) be denoted by  $P_{z}(x,t)$ , the kinetic energy of the missile is

$$= \frac{1}{2} \int_{0}^{\infty} \pi x + \frac{(p_{z})^{2}}{3E} dx$$
(3-1)

where

m(x) = mass/unit of length along the vehicle

<sup>&</sup>lt;sup>1</sup>In order to distinguish the linear analyses from the flexible-body trajectory analyses described in Section 3.2, the term "time-slice analysis" is coming into common use. The advantages and disadvantages of both the "time-slice" and "flexible-trajectory" analyses were recently aired in a NASA informal conference on Winds Aloft and Application to Launch Vehicle Design held at Langley Research Center April 22-23, 1964 (Proceedings not suitable for referencing).



FIGURE 22 SLENDER CONTROLLED VEHICLE

Using the interpolation methods developed in Section 2.3, we may divide the missile into a number of intervals or "bays" and approximate the continuous displacement curve,  $P_z$ , by displacements at discrete points. The result may be expressed as (see Paragraph 2.3.3.1, Equation 2-404).

$$t_{2}(x,t) = \{ z : z^{2} : z^{3} \} [z]_{1} [T]_{1} \{ z \} \}$$
 (3-2)

for the X & X

Substituting these expressions into the kinetic energy, we obtain

where

$$\Delta_{j} = \sum_{k=1}^{n} [\tau_{j}] [\tau_{j}]$$

124
Figure 23 shows a typical mass distribution for a slender launch vehicle.



FIGURE 23 MASS DISTRIBUTION FOR A SLENDER LAUNCH VEHICLE

### 3.1.1.1.2 The Strain Energy

The strain energy in bending is

$$= \frac{1}{\sqrt{2}} = \frac{1}{\sqrt{2}} \frac{1}{$$

where EI(x) is the bending rigidity of the missile<sup>L</sup>. The interpolation procedure may also be used to express this in terms of a finite number of degrees-of-freedom.

$$\int_{0}^{\infty} \frac{d^{2}r^{2}}{r^{2}} dr = -c^{2} (4, F_{0})$$
(3-6)

The total strain energy is then

$$= \sum \{c\} \{c\} \{c\}\}$$
 (3-7)

where [K] is the unrestrained, "free-free", stiffness matrix,

LThe effect of axial loads is discussed in Paragraph 3.1.2.5. The effect of shear energy and rotary inertia is considered in Paragraph 5.2.2.1. Also, it must be noted that the "complementary energy" techniques give much more accurate results than the diparabolic interpolation (see Paragraph 5.2.1). The method is only used here for its conceptual simplicity. Figure 24 shows a typical bending rigidity distribution for a slender launch vehicle



FIGURE 24 BENDING RIGIDITY DISTRIBUTION FOR A SLENDER LAUNCH VEHICLE

3.1.1.1.3 Aerodynamic Forces

For the purposes of illustration we shall assume that the aerodynamic forces can be described by the following expression.

$$L(x,t) = \frac{1}{\lambda} \log \log \frac{\partial D_{x}(x)}{\partial x_{x}} \chi(x,t)$$
(3-9)

where L(x,t) is the lift (positive "up") per unit of length along the missile;  $L/2 = \frac{1}{2}v_{p}^{2}$  is the free stream dynamic pressure; and

is the running lift coefficient for the rigid missile at unit angle-of-attack. The assumption in Equation 3-9 that the local lift is only dependent on the local angle-of-attack is actually not valid except at Mach numbers nominally greater than 2.5. There is, however, some empirical evidence that the adverse effect of this assumption on missile loads and stability is not great and Equation 3-9 may even be used at subsonic Mach numbers. It is more important that the lift distribution reflect the proper total lift and aerodynamic center for the rigid missile. The use of Equation 3-9 in dynamic analyses is based on the "quasi-steady" assumption that Equation 3-9 is valid in the case of unsteady flow if  $\alpha$  is interpreted as the ratio of induced downwash to the forward velocity.

$$\alpha = \frac{W}{V_{\infty}} \tag{3-10}$$

The boundary condition of tangent flow gives the following relation for the fluid velocity, or downwash, normal to the free stream:

$$w^{-} = v_{m} \frac{\partial h_{z}}{\partial x} + \frac{\partial h_{z}}{\partial k}$$
(3-11.)

The quasi-steady lift is then given by

$$L(x,z) = \frac{1}{2} \rho_{III} v_{III}^2 \frac{\zeta C_L}{\partial R} (i) \left( \frac{\partial \rho_Z}{\partial x} + \frac{1}{V_{III}} \frac{\partial \rho_Z}{\partial L} \right)$$
(3-12)

A more direct interpretation is that the angle-of-attack is composed of two parts: one, due to the slope,  $\frac{b_2}{2}$ , of the missile; and, two, due to the shift in the direction of the relative wind caused by the velocity of the missile normal to the free stream,  $\frac{c_2}{2}$ .



FIGURE 25 LOCAL ANGLE-OF-ATTACK

The virtual work of the aerodynamic forces on the missile is then

$$\exists w = -\sum_{x \neq y} \sqrt{\frac{1}{2}} \left[ \frac{1}{2} \frac{1}{$$

Equation 3-13 can be expressed in terms of the discrete displacements on the body in much the same way as was done for the kinetic and potential energies. From the interpolation formula,

$$p_{z}(x, L) = \{i \in \mathbb{I}^{2} \mid \mathbb{I}^{3} \mid [f]_{1}[f]_{1}[f]_{1}[f] \}$$
(3-14)

we have

Also, we may interpolate on the angle-of-attack

.

.

$$f(x,t) = \{i \le i \le i \le j \} [S]_i [T]_i \{\pi\}$$
(3-10)

where  $\{\alpha\}$  is the matrix of angle-of-attack's at the collocation points. Substitution into Equation 3-13 results in

$$\varepsilon_{W} = -\frac{1}{2} \left( \omega V_{\omega}^{2} \left\{ 5 \right\} \right)^{2} \sum_{i=1}^{N} \left[ T \right]_{i}^{i} \left[ J \right]_{i}^{i} \int_{X_{i-1}}^{X_{i-1}} \frac{1}{2} \left[ \frac{1}{2} \sum_{i=1}^{N} \frac{1}{2^{i}} \sum_{j=1}^{N} \frac{1}{2^{j}} \right]^{2} \left[ x \left[ J \right]_{i} \left[ T \right]_{i}^{j} \left\{ x, j \right\} \right]$$
(3-17)

It is convenient to introduce the matrix of aerodynamic influence coefficients,

$$[A_{1}] = \underbrace{\frac{1}{2}}_{i} [T]_{1} [S]_{1} [\int_{1}^{1} \frac{1}{2s} \left[ \frac{1}{2s} S_{1}^{2} S_{2}^{3} \right]_{i} dx [S]_{i} [T]_{i} (S-1c)$$

We can then write Equation 3-17 as

$$\delta W = -\frac{1}{2}\rho_0 v_0^2 \left\{ \delta \varphi \right\}' \left[ \Lambda \right] \left\{ \kappa \right\}$$
(3-19)

Figure 26 shows a typical air load distribution.



FIGURE 26 RUNNING AIR LOAD DISTRIBUTION FOR A SLENDER LAUNCH VEHICLE

Using the interpolation formula, we can calculate a "differentiation" matrix,  $[\Delta]$  , with the property

$$\frac{\left[\frac{p}{2}\right]}{\left[\frac{p}{2}\right]} = \left[\frac{p}{2\pi}\right]_{c,t} = \left[\frac{1}{2}\right]_{p}$$

$$\frac{\left[\frac{p}{2}\right]}{\left[\frac{p}{2}\right]}_{c,t} = \left[\frac{1}{2}\right]_{p}$$

$$\left[\frac{p}{2\pi}\right]_{c,t} = \left[\frac{1}{2}\right]_{p}$$

$$\left[\frac{p}{2\pi}\right]_{c,t} = \left[\frac{1}{2}\right]_{p}$$

and the relation

$$x(x,t) = \frac{\partial p_z}{\partial x} + \frac{1}{v_z} \frac{\partial p_z}{\partial t}$$
(3-21)

can be used to derive

$$\{\lambda\} = [\Delta] \{b\} + \frac{1}{\sqrt{\alpha}} \{b\}$$

$$(3-22)$$

## 3.1.1.1.4 Control Forces

We must also include the virtual work done by the moment,  $\rho$  , exerted on the control surface by the serve or other means of power control.

$$E_N = ST \qquad (3-23)$$

where  $\mathcal{V}$  is the control rotation relative to the body. The control rotation is not an independent generalized coordinate, but is related to the  $p_i$  by:

$$\dot{r} = -\left(\frac{\beta r_{z}}{13}\right)_{x=0} \qquad (3-24)$$
(on body) (on control surface)

or

We then have the virtual work of the servo moment given by

(- - X

To summarize this section, we have

$$= -2^{-2} - 2^{-2}$$

where

$$-1.1 = [-1.1 + 1$$

If we substitute Equation 3-30 into Equation 3-29 and define

$$\lfloor -\pm \rfloor = \lfloor \mathcal{I} \rfloor \tag{3-32}$$

we obtain

$$S_{n} = -\sum_{k=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{k=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}$$

### 3.1.1.2 Rigid Body Check on the Besic Dete

A numerical check can be made on the proper calculation of the matrices in Equations 3-33, 3-34, 3-35, and 3-30. This check is based on comparing the expressions with the corresponding ones for a rigid missile (the check can also be performed for the data associated with the control, but the discussion below will suffice to illustrate the procedure). For the rigid missile with locked control we have

$$= 1 + 12^{-1}$$

$$S_{n} = -\sum_{\alpha,\alpha} v_{\alpha}^{2} S_{\alpha}^{\alpha} S_{\alpha}^{\alpha} + \sum_{\alpha} \frac{1}{v_{\alpha}} \left( 3 - 3 \frac{1}{2} \right)$$

$$= \sum_{\alpha} v_{\alpha}^{\alpha} S_{\alpha}^{\alpha} + \sum_{\alpha} \frac{1}{v_{\alpha}} \left( \frac{1}{v_{\alpha}} + \frac{1}{v_{\alpha}} \right) \left( \frac{1}{v_{\alpha}} + \frac{1}{v_{\alpha}} + \frac{1}{v_{\alpha}} \right) \left( \frac$$

(3-40)

た = - = - 二 ご



FIGURE 27 GENERALIZED COORDINATES FOR THE RIGID MISSILE

We may also note that for the rigid missile, with locked control,



FIGURE 28 COLLOCATION POINT DISPLACEMENTS FOR THE RIGID MISSILE

Substitution of Equation 3--1 into Equations 3-33, 3-3-, 3-35, and 3-3t gives

$$\begin{aligned} \xi_{N} &= -\chi_{1,2} \frac{1}{2} - \xi_{N} \frac{1}{2}$$

By comparing Equation 3-37 with Equation 3-42, we must conclude that

.

$$\underline{z} = -\overline{z} - x \overline{z} - x \overline{$$

<u>also</u>

$$\{x,x\}$$
 [A]  $\{z,z\}$  (3-47)

gives

Equations 3-45, 3-46, and 3-46 are independent checks on the proper calculation of the mass matrix.

By concerning Equation 3-43 with Equation 3-30, we conclude that

(3-49) 2000

We also have the following relations

$$[-z] - z - z = -[-z] + [-z] + [-z]$$

.

which is true because of properties of the differentiation matrix like:

and

Equation 3-44 then becomes

By comperison with Equations 3-39 and 3-40 we must conclude that

$$h_{3} = -1 h_{1} h_{2} h_{3} h_{3} + -1 h_{3} h_{3}$$

$$1_{2} = -1_{2} + \frac{1}{2} + \frac{1}{2}$$

which are several independent checks on the proper calculation of the seroivnamic matrices.

Under provinsiones in which the zerodynamic forces can be expressed completely by:

$$sw = -\frac{1}{2} \frac{1}{2} \frac{1}{2$$

$$\frac{1}{2}\chi_{\tilde{f}}^{2} = \left[ \Delta_{\tilde{f}}^{2} \rho_{\tilde{f}}^{2} + \frac{1}{v_{z}} \rho_{\tilde{f}}^{2} \right]$$
(3-61)

we have

$$\mathcal{L}_{z} = \{i\} [A] [i]$$
 (3-62)

$$M_{\mathcal{I}} = \{ \hat{x} = \{ \hat{x} \in \mathcal{X} \} \}$$
 (3-63)

.

$$L_{xy} = -\frac{1}{2} - x_{1}^{2} \left[ \sum_{i=1}^{N} \frac{1}{2} - x_{1}^{2} \right]$$
(3-65)

as checks on the zerodynamic influence coefficients with the following checks on the differentiation matrix

All of the properties in Equations 3-45 through 3-48 and 3-56 through 3-59 may be calculated from the original expressions for the continuous missile. For example, the total mass is

the total moment of inertia is

.

and the total rigid body moment coefficient is

$$h_{\chi} = \left[ \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right]$$
 (3-70)

In a similar manner the damping derivatives are

Lastly,

$$= \frac{1}{2}$$
 (3-73)

may be checked by comparison with Equation 3-4c.

3.1.2 The Equations for Determining "Loads" and Transient Motion

3.1.2.1 The Equations for Calculating Transient Internal Loads

The equations relating internal stresses to the generalized coordinates, velocities, and accelerations are derived by applying Lagrange's equations to the expressions for the kinetic and potential energy. Using Equations 3-33, 3-34 and 3-35 in Lagrange's equations (Equation 2-64), we obtain

We may arrange this equation as

These are the "loads" equations for the missile; the left-hand side is the "effective" loads which are directly related to the internal stresses of the structure<sup>1</sup>. If we introduce the definition

$$\{r\} = [r] Rr\}$$
 (3-76)

then we can derive a matrix which relates the shears and bending moments along the missile to the "effective" loads, {F}. The shears and bending moments just to the right of the collocation points are

$$f_{i} = \prod_{j=1}^{n} F_{j} + M_{i} = \prod_{j=1}^{n} f_{j} - f_{j} + \frac{1}{2}$$
 (3-77)





Equations 3-77 can be written as



<sup>1</sup>If internal damping is included, and described by the Rayleigh dissipation function, then the internal stresses are related to

see also Paragraph 5.1.1.1, Equation 5-1-4

. .

or simply as

$$\begin{bmatrix} \cdot \cdot \\ \cdot \\ \cdot \end{bmatrix} = \begin{bmatrix} -1 \\ -1 \end{bmatrix}$$
(3-79)

If the time histories of the generalized coordinates are known, then either side of Equation 3-75 may be used in Equation 3-79 to compute the internal loads. When the model approximation is made, however, the right-hand side gives better results. This is the so-called "modal-acceleration" method.<sup>1</sup> Substituting Equation 3-75 into Equation  $3-79_x$  we obtain

$$z_{k} = -\left[R\right] \left[A \right] \left[A \right] \left[F \right] \left[A \right] \left[A \right] \left[F \left[A \right] \left[A \right] \left[F \left[A \right] \left[A \right] \left[F \right] \left[A \right] \left[A \right] \left[A \right] \left[F \left[A \right] \left[A \right] \left[A \right] \left[F \left[A \right] \left[A$$

In addition to the inertia Loads and aerodynamic loads, there are generally external loads forcing the system and these forces must be included as additional terms in Equation 3-80. Additional forces are considered in Paragraph  $3 \cdot 1 \cdot 2 \cdot c \cdot$ 

It is conceivable that Equation 3-74 could be integrated to obtain the time histories of the quantities which appear in Equation 3-80. This is not usually done, however, because it is expedient to reduce the number of degrees-of-freedom of the problem by transforming to "modal" generalized coordinates. This is the subject of the next section.

#### 3.1.2.2 The Modal Equations of Motion

The vibration modes of the missile are obtained from Equation 3-74 with no external forces and the control surface locked in the position,  $\gamma = 0$ .

The solution of these equations is important for deriving a transformation which simplifies the solution of the differential equations governing the motion of the system (see Faragraph 2.2.3.1). If we assume a separated solution to these equations,

See, for example, Bisplinghoff, Ashley, and Halfman Aeroelasticity pp. 642.

we obtain

$$(-\omega^2 [A] + [-] + [+] = \{0\}$$
 (3-83)

In the case of an unrestrained missile we have two zero-frequency modes (see Paragraph 2.2.3.4),

$$\{\psi_{R}\}_{r} = \{i\}; \quad \{\psi_{R}\}_{2} = \{\bar{x} - x\}$$
 (3-84)

which represent a rigid-body displacement and rotation about the center of mess. From Equation  $3-83_x$  we have

$$[\kappa + 1] = \{ : \}$$
 (3-85)

$$[\pi, ] + [x - x] = + ]$$
 (3-86)

The non-zero frequency modes are calculated from

$$[T_{F_{+}}^{T}] = \{F_{+}\}$$
 (3-87)

as discussed in Paragraph 2.2.3.4. In the present case, we have

(also, the influence coefficients are derived with the control locked,  $\mathcal{P}=0$ ). The solutions to Equations 3-63 are used to form the following transformation of couldinates

Where  $\{\phi_{\gamma}\}$  are the values of the generalized coordinates for a unit rotation of the rigid control relative to the rigid body.

An interpretation of the significance of the zero-frequency modal coordinates,  $\zeta$  and  $\theta$ , is given by premultiplying Equation 3-89 by grand -7.774] and making use of the orthogonality conditions for the modal columns

(see Equation 2-210, Paragraph 2.2.3.4).

$$f(F_{A}) = f(F_{A}) + f(F_{A}) + f(F_{A}) + f(F_{A}) = (3-90)$$

$$\{x - x\} [A] \{b\} = \{x - x\} [A] \{b\} + \{z - x^{2} - x\} [A] \}$$
(3-91)

Making use of Equations 3-45, 3-46, and 3-47 we have

.

In the first of these equations  $\zeta$  can be interpreted as the lateral displacement of the instantaneous center of mass of the missile. The analogous relation for the "continuous" description of the missile is

A corresponding interpretation of  $\theta$  is not so direct, nevertheless we have the relation,

$$z_{\pm} = \frac{1}{2} \left[ \frac{1}{2} - \frac{1}{2} \frac{1}{2} + \frac{1}{2} \frac{1}{2} \frac{1}{2} \right]$$
 (3-95)

in analogy to Equation 3-93. We can write Equation 3-89 as

where

$$(3-97)$$

and

The modal transformation matrix,  $[\phi]$ , is not generally square because the high frequency modes are omitted on the basis that they have little effect on the dynamics of the system. Generally the number of degrees-of-freedom in Equation 3-96 can be reduced in this way without compromising the description of the dynamics of the missile. This constraint is not serious in most response problems; however, due to this approximation, Equation 3-75 can never be satisfied exactly. This results in the difference in accuracy between the "modal-displacement" and "modal-acceleration" method of loads analysis.

To transform to model coordinates we substitute Equation 3-96 into Equations 3-33, 3-34, and 3-35 which gives

$$= \frac{1}{2} - \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^$$

$$u = \{1, 1, 2\}$$
 (3-100)

$$S_{ii} = - \sum_{i=1}^{n} \sum_{j=1}^{n} \left( \sum_{i=1}^{n} \frac{1}{1} + \frac{1}{1} \sum_{i=1}^{n} \frac{1}{1} + \frac{1}{1} \sum_{i=1}^{n} \sum_{j=1}^{n} \frac{1}{1} + \frac{1}{1} \sum_{i=1}^{n} \frac{1}{1} + \frac{1}{1} \sum_$$

where

$$[M] = [1]'[4][2] \qquad (3-102)$$

$$[C_{R}] = [\varphi]^{\prime}[L_{R}][\varphi] \qquad (3-104)$$
$$[C_{L}] = [\varphi]^{\prime}[L_{L}][\varphi] \qquad (3-105)$$

The final form of Equations 3-99, 3-100, and 3-101 is quite general, being independent of many of the simplifying assumptions that were made in the derivation. It is often important to account for flexibility of the control surface, particularly when investigating problems of "control effectiveness." It can be shown that Equation 3-35 is the same, in this case, although the coefficients are different. Also, steady-state aerodynamic interference between control surface and body can be accounted for "exactly" in the case of a flexible control surface by including a full matrix of aerodynamic influence coefficients.

The equations of motion are obtained by using relations 3-99, 3-100, and 3-101 in Lagrange's equations (Equation 2-64, Paragraph 2.1.2.1) regarding  $\zeta$ ,  $\theta$ ,  $\gamma$ , and the  $q_{\underline{i}}$ 's as independent generalized coordinates. The result is

$$[M]+j! + [=H_{3}! + \underline{b}_{0} \times \underline{a}^{*}([2n]+j!) - \underline{a}^{*}([2n]+j!) = \underline{b}_{0}!' \cdot \underline{b}_{0} + \underline{b}_{0} \times \underline{b}_{0} \times \underline{b}_{0} \times \underline{b}_{0} + \underline{b}_{0} \times \underline{$$

The solution to the above differential equations can be used in Equation 3-80 to obtain the transient shears and bending moments<sup>1</sup>. If  $\zeta$ ,  $\theta$ ,  $\gamma$ , and the  $q_1$ 's have been obtained as functions of time, then the collocation point coordinates are obtained from:

$$\{b\} = [(t)]\{q(t)\}$$
 (3-107)

Substituting this into Equation 3-80, we obtain the final form of the transient loads equations

$$\begin{bmatrix} \langle I \rangle \\ I \rangle \end{bmatrix} = - \begin{bmatrix} P & [A & [P] A & [P] I \rangle \\ I \rangle \\ I \rangle \end{bmatrix} = \begin{bmatrix} P & [A & [P] A & [P] A & [P] P \\ I \rangle \\ I \rangle \\ I \rangle \end{bmatrix} = \begin{bmatrix} P & [A & [P] A & [P] A & [P] \\ I \rangle \\ I \rangle \\ I \rangle \\ I \rangle \end{bmatrix} = \begin{bmatrix} P & [A & [P] A & [P] A & [P] \\ I \rangle \\ I \rangle$$

<sup>&</sup>lt;sup>1</sup>Transient loads problems such as response to gusts and impulsive control motions can be handled effectively by a computer program which solves a general set of equations similar to Equations 3-106. The second part of Appendix VI describes such a set of equations and a method for their solution.

## 3.1.2.3 The "Quasi-Rigid" Approximation and Aeroelastic Corrections to the Rigid-Body Stability Derivatives

It is sometimes undesirable to have to solve the differential equations, 3-106, in order to obtain transient loads. This is particularly true in view of the fact that an approximate solution of Equations 3-106 can be used to obtain preliminary loads for structural design purposes.

For the purpose of obtaining an approximate solution to Equations 3-106, we partition the equations into a rigid body part and an elastic part.

To simplify the discussion we will, in this section, redefine  $[\phi]$ , and  $\{q\}$ -so that they are the elastic modes and coordinates only. We then have

$$\frac{1}{2} = \frac{1}{2} \left( \frac{1}{2} + \frac{1$$

for Equation 3-89. By expanding the products in Equations 3-102, 3-103, 3-104 and 3-105, it can be shown that

(3-110)

[M] =	113 [A][1]	{1}{A Hx-x}	{1}'{A][φ]	{I}{A]{φ <sub>2</sub> }	
	{x-x}'[A]{1}	\x-x}`{A}{x-x}	{x-x}'[A][φ]	{x-x}`[A`H\$##}}	
	[φ] <sup>'</sup> [A]{;}	{x-x}{A]{x-x}	[φ][Α][φ]	φİtAHψ <sub>γ</sub> }	
	+φ <sub>2</sub> 3'[A][+]	iq+}'{A]{x-x}	(47) [A][4]	iφ#}[A]{ φ#}	

 $[F] = \begin{cases} i \cdot j'(\kappa Hi) & i \cdot j'(\kappa H\bar{x} - x) + i \cdot j'(\kappa H\varphi) & i \cdot j'(\kappa H\varphi_p) \\ i \cdot z \cdot z \cdot j'(\kappa Hi) & i \cdot \bar{x} - x \cdot j'(\kappa H\bar{x} - x) + i \cdot x \cdot j'(\kappa H\varphi_p) \\ i \cdot \varphi \cdot j'(\kappa Hi) & i \cdot \varphi \cdot j'(\kappa H\bar{x} - x) + i \cdot \varphi \cdot j'(\kappa H\varphi_p) \\ i \cdot \varphi \cdot j'(\kappa Hi) & i \cdot \varphi_p \cdot j'(\kappa H\bar{x} - x) + i \cdot \varphi_p \cdot j'(\kappa H\varphi_p) \\ i \cdot \varphi_p \cdot j'(\kappa Hi) & i \cdot \varphi_p \cdot j'(\kappa H\bar{x} - x) + i \cdot \varphi_p \cdot j'(\kappa H\varphi_p) \end{cases}$  (3-111)

$$\begin{bmatrix} C_{R} \end{bmatrix} = \begin{bmatrix} i_{1}i_{1}'[\iota_{R}H_{1}] & i_{1}i_{1}'(\iota_{R}H_{\bar{x}}-x) & i_{1}i_{1}'(\iota_{R}](\varphi) & i_{1}i_{1}'(\iota_{R}H_{\varphi}) \\ i_{\bar{x}}-x_{3}i_{1}'[\iota_{R}H_{1}] & i_{\bar{x}}-x_{3}i_{1}'(\iota_{R}H_{\bar{x}}-x) & i_{\bar{x}}-x_{3}i_{1}'(\iota_{R}H_{\varphi}) \\ i_{\varphi}i_{1}'(\iota_{R}H_{1}) & (\varphi)i_{1}'(\iota_{R}H_{\bar{x}}-x) & (\varphi)i_{1}'(\iota_{R}H_{\varphi}) & (\varphi)i_{1}'(\iota_{R}H_{\varphi}) \\ i_{\varphi}i_{1}'(\iota_{R}H_{1}) & i_{\varphi}i_{1}'(\iota_{R}H_{\bar{x}}-x) & i_{\varphi}i_{1}'(\iota_{R}H_{\varphi}) & i_{\varphi}i_{1}'(\iota_{R}H_{\varphi}) \end{bmatrix}$$

$$(3-112)$$

.

$$\begin{bmatrix} c_{I} \end{bmatrix} = \begin{cases} \{i\}'_{[L_{I}}\}_{i}\} & \{i\}'_{[L_{I}}\}_{i}\}_{i} \\ \{i\}'_{[L_{I}}\}_{i}\} & \{i\}'_{[L_{I}}\}_{i}\}_{i} \\ \{i\}'_{[L_{I}}\}_{i}\} & \{i\}'_{[L_{I}}\}_{i}\}_{i} \\ \{i\}'_{[L_{I}}\}_{i}\} & \{i\}'_{[L_{I}}\}_{i}\}_{i} \\ \{i\varphi\}'_{[L_{I}}\}_{i}\} & \{i\rangle'_{[L_{I}}\}_{i}\}_{i} \\ \{i\varphi\}'_{[L_{I}}\}_{i}\} & \{i\varphi\}'_{[L_{I}}\}_{i}\}_{i} \\ \{i\varphi\}'_{i}\}'_{i} \\ \{i\varphi\}'_{i} \\ \{i\varphi\}'_{i}\}'_{i} \\ \{i\varphi\}'_{i} \\ \{i\varphi$$

.

Using Equations 3-45 through 3-48 and the orthogonality relations (Equation 2-169, 2-170 and 2-210 of Paragraph 2.2.3), we can write these more simply as

$$[M] = \begin{bmatrix} M & O & ioi' & O \\ O & I & ioi' & O \\ ioi & ioi' & I \\ ioi' & ioi' & ioi' \\ O & i \neq_{\varphi} i'[A][\psi] & J \end{bmatrix}$$
(3-114)

$$\{i_{1}^{2}(A)^{\frac{1}{2}}i_{1}^{\frac{1}{2}}=1.4$$

$$\{i_{2}^{2}(A)^{\frac{1}{2}}x_{2}x_{3}^{\frac{1}{2}}=1.4$$

$$\{i_{2}^{2}(A)^{\frac{1}{2}}i_{3}^{\frac{1}{2}}x_{3}^{\frac{1}{2}}=1$$

where

.

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(We have assumed that the control mode is orthogonal to the rigid body modes. For further discussion see Paragraph 3.1.3.5, Equations 3-Jud and 3-509).

$$[F] = \begin{bmatrix} 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$
(3-115)  
$$\begin{bmatrix} 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$
(3-115)  
$$\begin{bmatrix} 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

$$\begin{bmatrix} \mathbf{J}_{\mathbf{R}} \end{bmatrix} = \begin{bmatrix} \mathbf{J}_{\mathbf{-}} - \mathbf{L}_{\mathbf{x}} & \mathbf{i} \begin{bmatrix} \mathbf{J}_{\mathbf{-}} + \mathbf{R} \mathbf{L} \mathbf{q} \end{bmatrix} & \mathbf{L}_{\mathbf{1}} \mathbf{j}_{\mathbf{-}} \\ \mathbf{J}_{\mathbf{-}} - \mathbf{L}_{\mathbf{R}} & \mathbf{i} \mathbf{x} \cdot \mathbf{x} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{R}} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} & \mathbf{L}_{\mathbf{n}} \mathbf{j}_{\mathbf{-}} \\ -\mathbf{L}_{\mathbf{1}} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{L}_{\mathbf{1}} \end{bmatrix} \end{bmatrix} \end{bmatrix} \begin{bmatrix}$$





We may now use these results to rewrite Equations 3-106 as:

$$\frac{1}{2} + \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} + \frac{1}{2} \frac{1}{2} \frac{1}{2} + \frac{1}{2} \frac{1}{$$

$$\frac{13 + z_{1}y_{2}y_{2}}{z_{1}z_{2}z_{2}} = \frac{1}{z_{1}} + \frac{1}{z_{1}} + \frac{1}{z_{1}} + \frac{1}{z_{1}} + \frac{1}{z_{2}} + \frac{1}{z_{2}} + \frac{1}{z_{1}} +$$

$$\frac{1}{2} + \left[ \frac{1}{2} + \frac$$

$$J\vec{x} + i_{\vec{x}}\vec{z}\left[A\left[i_{\vec{x}}\right]\vec{z}\right] + i_{\vec{x}}\vec{z}\left[\frac{\beta}{2}\right] $

where we have used the fact that

$$[\eta] \hat{\eta} = 0 \tag{3-122}$$

$$\{n\},\{7,-x\} = 0 \tag{3-123}$$

$$-nfrires = (3-124)$$

$$\{y\}[y] = -2\}$$
 (3-125)

Up to this point we have done little more than change the form of Equations 3-106; the above equations are still a set of linear, second order, differential equations. On the basis that we are interested only in response to "slowly" varying forces, we make the approximation

$$-\frac{1}{2} = -\frac{1}{2} = -\frac{1}{2}$$
 (3-126)

which we shall call the "quasi-rigid" approximation. In addition, we will assume that

Equations 3-118 through 3-121 can then be replaced by

$$\frac{1}{2} = 4 \frac{1}{2} \frac{1}{\sqrt{2}} - e^{\frac{1}{2}} + \frac{1}{\sqrt{2}} \frac{1}{\sqrt{2}} + \frac{1}{\sqrt{2}} \frac{1}{\sqrt{2}} + \frac{1}{\sqrt{2}} \frac{1}{\sqrt{2}} + \frac{1}{\sqrt{2}} \frac{1}{\sqrt{2}} \frac{1}{\sqrt{2}} + \frac{1}{\sqrt{2}} \frac{1}{\sqrt{2}$$

$$1 = \sum_{k=1}^{\infty} \sum_{i=1}^{\infty} \sum_{j=1}^{\infty} \sum_{i=1}^{\infty}$$

$$\begin{pmatrix} \frac{2}{\ell_{0}V_{0}} \Gamma_{X} \end{bmatrix} + [\varphi]'[L_{R}][\varphi] \end{pmatrix} \{q\}$$

$$= -[\varphi]'[L_{I}]_{1} + [\varphi]'[L_{R}][\varphi] \end{pmatrix} = -[\varphi]'[L_{I}]_{X-X} + \frac{\tilde{\Theta}}{V_{0}}$$

$$- [\varphi]'[L_{R}]_{1} + [\varphi]_{X-X} + \frac{\tilde{\Theta}}{V_{0}} + \frac{\tilde{\Theta}}$$

$$\frac{1}{2} \left( \partial_{\mu} V_{\mathcal{D}}^{r} \left( \zeta_{\mu}_{\mathcal{K}} \left( \frac{\tilde{r}}{V_{\mathcal{D}}} - \theta \right) + C_{\mu}_{\tilde{\Theta}} \frac{\tilde{\Theta}}{V_{\mathcal{D}}} + C_{\mu}_{\mathcal{F}} \mathcal{F} + \frac{1}{2} \varphi_{\mathcal{F}} \frac{1}{2} \left[ L_{\mathcal{R}} \right] \left[ \psi \right] \left\{ q_{\mathcal{F}} \right\} \right) = \Gamma$$

$$(3-131)$$

When the quasi-rigid approximation is made, the modal equations are no longer differential equations and may be solved algebraically for the q's.

$$\{q_{\mathbf{L}}\} = -\left(\frac{z}{\varrho_{\mathbf{v}} v_{\mathbf{m}}^{*}} \Gamma_{\boldsymbol{\lambda}_{\perp}}^{\perp} + [\varphi]^{\prime} [L_{\mathbf{R}}] [\varphi]\right)^{-1} [\varphi]^{\prime} [L_{\mathbf{L}}] \frac{z}{[\psi]} \frac{z}{[\psi]} \left(\frac{z}{V_{\mathbf{m}}} - \varphi\right)$$

$$- \left(\frac{z}{\varrho_{\mathbf{w}} v_{\mathbf{m}}^{*}} \Gamma_{\boldsymbol{\lambda}_{\perp}}^{\perp} + [\varphi]^{\prime} [L_{\mathbf{R}}] [\varphi]\right)^{-1} [\varphi]^{\prime} [L_{\mathbf{L}}] \frac{z}{[\psi]} \frac{z}{v_{\mathbf{m}}}$$

$$- \left(\frac{z}{\rho_{\mathbf{w}} v_{\mathbf{m}}^{*}} \Gamma_{\boldsymbol{\lambda}_{\perp}}^{\perp} + [\varphi]^{\prime} [L_{\mathbf{R}}] [\varphi]\right)^{-1} [\varphi]^{\prime} [L_{\mathbf{R}}] \frac{z}{[\psi]} \frac{z}{v_{\mathbf{m}}}$$

$$- \left(\frac{z}{\rho_{\mathbf{w}} v_{\mathbf{m}}^{*}} \Gamma_{\boldsymbol{\lambda}_{\perp}}^{\perp} + [\varphi]^{\prime} [L_{\mathbf{R}}] [\varphi]\right)^{-1} [\varphi]^{\prime} [L_{\mathbf{R}}] \frac{z}{[\psi]} \frac{z}{v_{\mathbf{m}}}$$

$$+ \left[\frac{z}{\rho_{\mathbf{w}} v_{\mathbf{m}}^{*}} \Gamma_{\boldsymbol{\lambda}_{\perp}}^{\perp} + [\varphi]^{\prime} [L_{\mathbf{R}}] \frac{z}{v_{\mathbf{m}}} \right]^{-1} [\varphi]^{\prime} [L_{\mathbf{R}}] \frac{z}{v_{\mathbf{m}}}$$

Equation 3-132 can be used to eliminate the q's from the rigid-body equations

$$M \tilde{J} + \frac{1}{2} \chi_{0} v_{0}^{2} \left( C_{L_{\alpha}} \left( \frac{\tilde{I}}{Y_{0}} - \Theta \right) + C_{L_{\alpha}} \frac{\tilde{\Theta}}{Y_{0}} + C_{L_{\alpha}} \mathcal{Y} \right)$$

$$- \frac{1}{2} \chi_{0} v_{0}^{2} \tilde{J}_{1} \left[ L_{\alpha} I \varphi \right] \left( \frac{\tilde{I}}{2} \chi_{0}^{2} - G \right) + \left[ \varphi \right] \left[ L_{\alpha} I \varphi \right] \left[ \frac{\tilde{\Theta}}{Y_{0}} \right] \left( \left( L_{\alpha} I f_{1} \right) \frac{\tilde{I}}{Y_{0}} - \Theta \right) + \left[ L_{\alpha} I \tilde{I} \frac{\tilde{I}}{Y_{0}} - \Theta \right] + \left[ L_{\alpha} I \tilde{I} \frac{\tilde{I}}{Y_{0}} \right] \left( \frac{\tilde{I}}{Y_{0}} - \Theta \right) + \left[ L_{\alpha} I \tilde{I} \frac{\tilde{I}}{Y_{0}} + C_{L_{\alpha}} \frac{\tilde{I}}{Y_{0}} \right] = 0$$

$$(3-133)$$

.

$$I = \pm \frac{1}{2} \exp \frac{1}{2} \left( C_{M_{R}} \left( \frac{1}{V_{R}} - \Theta \right) + C_{M_{E}} \frac{1}{2} + C_{M_{F}} T \right)$$

$$= \pm \frac{1}{2} \exp \frac{1}{2} \left\{ \frac{1}{2} \exp \left[ \frac{1}{2} \exp$$

$$z^{*}r^{*}r^{*} = \left( \frac{1}{v_{m}} - e \right) + \left( \frac{1}{v_{m}} + \frac{1}{v_{m}}$$

If we introduce the notion of "flexible aerodynamic coefficients," we can write Equations 3-133 and 3-134 as

$$M_{r}^{2} + 2 \left[ \frac{1}{2} \frac{1}{2} \frac{1}{2} - \frac{1}{2} \right] - 2 \left[ \frac{1}{2} \frac{1}{2} \frac{1}{2} + 2 \left[ \frac{1}{2} \frac{1}{2} + \frac{1}{2} \frac{1}{2} + 2 \left[ \frac{1}{2} \frac{1}{2} + \frac{1}{2} \frac{1}{2} + 2 \left[ \frac{1}{2} \frac{1}{2} + \frac{1}{2} \frac{1}{2} + 2 \left[ \frac{1}{2} \frac{1}{2} \frac{1}{2} + 2 \left[ \frac{1}{2} \frac{1}{2} \frac{1}{2} + 2 \left[ \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} + 2 \left[ \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} + 2 \left[ \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} + 2 \left[ \frac{1}{2} \frac{$$

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$$I\ddot{e} + \frac{1}{2} \sqrt{2} \sqrt{2} \left( C_{M_{R}}^{F} \left( \frac{1}{M_{0}} - e \right) + C_{M_{\tilde{e}}}^{F} \frac{e}{M_{0}} + C_{M_{\tilde{e}}}^{F} \frac{e}{M_{0}} + C_{M_{\tilde{e}}}^{F} \frac{e}{M_{0}} + C_{M_{\tilde{e}}}^{F} \frac{e}{M_{0}} \right) = 0$$
(3-137)

$$z_{\theta} \sqrt{\sigma} \left( C_{\mu \kappa}^{\mu} \left( \frac{1}{V_{\theta}} - \Theta \right) + C_{\mu \phi}^{\mu} \frac{1}{V_{\theta}} + C_{\mu \phi}^{\mu} \frac{1}{V_{\theta}} + C_{\mu \phi}^{\mu} \frac{1}{V_{\theta}} + C_{\mu \phi}^{\mu} \frac{1}{V_{\theta}} \right) = -$$
(3-135)

where, the flexible coefficients are given by

$$c_{x}^{F} = c_{x} - \{i\} [c_{R}[\phi]] (\frac{1}{1000} [c_{1}] + [\phi]] [\phi] [\phi] [\phi] [\phi] (c_{1}] + [\phi]] (3-13)$$

$$C_{\underline{z}}^{\mathsf{F}} = C_{\underline{z}} - \left\{ i \operatorname{f} \left[ \operatorname{k} \left[ \left[ \psi \right] \right] \left[ \frac{z}{\psi a \psi_{\underline{a}}} \right]^{-1} + \left[ \varphi \right] \left[ \left[ \operatorname{k} \left[ \left[ \psi \right] \right] \right]^{-1} \right] \left[ \psi \right] \left[ \left[ \operatorname{k} \left[ \left[ \frac{z}{\psi} \right] \right]^{-1} \right] \right] \right] \right] \right\}$$

$$c_{\text{NG}}^{\text{E}} = C_{\text{NG}} - \frac{1}{2} \bar{x}_{-x} E[L_{\text{E}} I[\Psi]]^{\frac{1}{2}} |x_{1}| + [\Psi]^{\frac{1}{2}} [L_{\text{E}} I[\Psi]]^{\frac{1}{2}} |\Psi|^{\frac{1}{2}} $

$$C_{\mathcal{F}}^{\mathsf{F}} = C_{\mathcal{F}} - \{i\}'[\mathsf{L}_{\mathsf{R}}][\varphi](\frac{\lambda}{\sqrt{\omega}v_{\alpha}} \mathcal{F}'_{\mathsf{X}} + [\varphi]'[\mathsf{L}_{\mathsf{R}}][\varphi])'[\varphi]'[\mathsf{L}_{\mathsf{R}}]\{\varphi_{\mathcal{F}}\}$$
(3-143)

$$C_{kr_{\mathcal{F}}}^{\mathcal{F}} = C_{kr_{\mathcal{F}}} - \frac{1}{2} \overline{k} - \chi \dot{f} [L_{\mathcal{R}}] [\Psi] \left( \frac{z}{k_{u}k_{u}} \Gamma'_{x,1} + [\Psi]' [L_{\mathcal{R}}] [\Psi] \right)^{\dagger} [\Psi]' [L_{\mathcal{R}}] \frac{1}{2} \Psi_{\mathcal{F}}^{\dagger}$$

$$(3 - \underline{I} L_{u}^{\dagger} + )$$

$$C_{\mathcal{R}}^{\mathsf{F}} = C_{\mathcal{R}} - \{\varphi_{2} \{ \mathsf{L}_{\mathsf{R}} \mathbb{T} \varphi \} \left( \frac{2}{\varrho_{\mathsf{W}} \varphi_{\mathsf{G}}} \mathbb{T}_{\mathsf{X}} + [\varphi] \left[ \mathsf{L}_{\mathsf{R}} \mathbb{T} \varphi \right] \right) \left[ \varphi \right] \left[ \mathsf{L}_{\mathsf{T}} \right] \{\mathsf{L}_{\mathsf{T}} \} \left[ \varphi \right] \left[ \mathsf{L}_{\mathsf{T}} \right] \{\mathsf{L}_{\mathsf{T}} \} \right]$$

$$= C_{+\underline{a}} - \frac{1}{2} \varphi_{\overline{z}} E'[L_{R}][\varphi](\underline{z}_{\overline{z}} - \frac{1}{2} \varphi_{\overline{z}} - \frac{1}{2} \varphi_{\overline{z}} + [\varphi](L_{R}) - [\varphi](L_{R}) + [\varphi](L_{R})$$

$$C_{H_{2}}^{F} = C_{H_{2}} - f \varphi_{2} f \left[ L_{\#} I \left[ \varphi \right] \left[ \frac{1}{\sqrt{2} \sqrt{2}} \int \sum_{i} + \left[ \varphi \right] \left[ L_{\#} I \left[ \varphi \right] \right] \left[ \frac{1}{\sqrt{2}} \int \varphi_{2} f \left[ \frac{1}{\sqrt{2}} f \left[ \frac{1}{\sqrt{2}} \int$$

# 3.1.2.4 "Unit' Internal Loads Based on the Quasi-Rigid Approximation

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We can now use the solutions to the quasi-rigid equations to obtain loads consistent with this approximation. In Equation 3-109 we make the quasi-rigid assumption and obtain

$$iFJ = iJJJ + ix - xJ \vec{e}$$
 (3-148)

$$\{\hat{p}\} = \{i\} = \{i\} = \{x, x\} \in \{3, -1\} \in \{3,$$

$$fp f = f(f, r) + f(r) $

but from Equations 3-136, 3-137, and 3-132 we have

$$F = -\frac{i}{2M}\left(\sum_{k=1}^{F} a_{k} + \sum_{k=1}^{F} \frac{a_{k}}{v_{m}} + \sum_{j=1}^{F} p\right)$$
(3-151)

$$\ddot{\boldsymbol{\varphi}} = -\frac{\hbar \Sigma \dot{\boldsymbol{\psi}}_{\mathcal{R}}}{\lambda I} \left( \sum_{k=1}^{F} \boldsymbol{\xi}_{k} + \sum_{k=1}^{F} \dot{\boldsymbol{\xi}}_{k} + C_{k} \boldsymbol{\gamma}_{\mathcal{R}} + C_{k} \boldsymbol{\gamma}_{\mathcal{R}} \right)$$
(3-152)

(The control surface relation given in Equation 3-138 is of interest in defining the demands on the serve to produce a required control surface deflection,  $\gamma_*$ )

$$\begin{aligned} \{i_{j}\} &= -\left(\frac{2}{4\pi N_{z}} \left[\frac{1}{N_{z}}\right] + \left[\varphi\right] \left[\left(L_{z}\right) \left[\varphi\right] \right] \left(\left[L_{z}\right] \left[\frac{1}{N}\right] + \left[L_{z}\left[\frac{1}{N}-\lambda\right] \left[\frac{1}{N_{z}}\right] + \left[L_{z}\left[\frac{1}{N}-\lambda\right] \left[\frac{1}{N}\right] + \left[L_{z}\left[\frac{1}{N}-\lambda\right] \left[\frac{1}{N}\right] + \left[L_{z}\left[\frac{1}{N}-\lambda\right] \left[\frac{1}{N}\right] + \left[L_{z}\left[\frac{1}{N}\right] + \left$$

where we have introduced the rigid-body angle-of-attack

$$\alpha = \frac{j}{\gamma_{\infty}} - \Theta \qquad (3-154)$$

We may simplify the discussion further by introducing the following notation:

$$\frac{\partial q}{\partial x} = -\left(\frac{z}{\partial x} \Gamma_{x,1} + [\varphi] \left( L_{x}[\varphi] \right)^{-1} \left( \varphi \right)^{-1} \left( L_{x}[\varphi] \right)$$

$$(3-155)$$

$$\frac{1}{26} = -\left(\frac{1}{60} \left[ \frac{1}{2} + \left[ \varphi \right] \left[ \frac{1}{2} \right] \right]^{-1} \left[ \frac{1}{2} \left[ \frac{1}{2} - x \right] \right]$$
(3-156)

$$\{\frac{1}{2}\varphi\} = -\left(\frac{z}{2} \nabla_{\mathcal{K}_{1}} + [\varphi]'[L_{\mathcal{K}}][\varphi]\right)^{-1} [\varphi]'[L_{\mathcal{K}}][\varphi_{2}]$$
(3-157)

so that

.

.

$$+\frac{1}{4}I = \frac{1}{4}\frac{\partial q}{\partial x} x + \frac{1}{5}\frac{\partial q}{\partial x} + \frac{\partial q}{\partial x} + \frac{\partial q}{\partial x} x + \frac{\partial q}{\partial x} $

These equations may now be substituted into Equation 3-80 to obtain the quasirigid approximation to the loads

$$\{ L \} = \begin{bmatrix} \{ V \} \\ \{ M \} \end{bmatrix} = \begin{bmatrix} R \end{bmatrix} \begin{bmatrix} A \end{bmatrix} \{ I \end{bmatrix} \xrightarrow{M_{0}} \sum_{2M} \left( C_{L_{A}}^{F} \mathcal{R} + C_{L_{0}}^{F} \stackrel{\circ}{\xrightarrow{V_{D}}} + C_{P}^{F} \right)$$

$$+ \begin{bmatrix} R \end{bmatrix} \begin{bmatrix} A \end{bmatrix} \{ \overline{X} - X \} \xrightarrow{M_{0}} \sum_{2M} \left( C_{M_{A}}^{F} \mathcal{R} + C_{L_{0}}^{F} \stackrel{\circ}{\xrightarrow{V_{D}}} + C_{M_{P}}^{F} \stackrel{\circ}{\xrightarrow{V_{D}}} \right)$$

$$- \xrightarrow{M_{0}} \sum_{2} \begin{bmatrix} R \end{bmatrix} \begin{bmatrix} L_{I} \end{bmatrix} \stackrel{\circ}{\xrightarrow{V_{0}}} + \frac{M_{0}}{2} \begin{bmatrix} R \end{bmatrix} \begin{bmatrix} L_{I} \end{bmatrix} \stackrel{\circ}{\xrightarrow{V_{0}}} \stackrel{\circ}{\xrightarrow{V_{0}}} \stackrel{\circ}{\xrightarrow{V_{0}}} \stackrel{\circ}{\xrightarrow{V_{0}}} \stackrel{\circ}{\xrightarrow{V_{0}}} \stackrel{\circ}{\xrightarrow{V_{0}}} \begin{bmatrix} R \end{bmatrix} \begin{bmatrix} L_{Q} \end{bmatrix} \stackrel{\circ}{\xrightarrow{V_{0}}} \stackrel{\circ}{\xrightarrow{V_{0}} \stackrel{\circ}{\xrightarrow{V_{0}}} \stackrel{\circ}{\xrightarrow{V_{0}} \stackrel{\circ}{\xrightarrow{V_{0}}} \stackrel{}$$

Now, since

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$$\Gamma = \frac{\Theta V_{0}^{z}}{2} \left( C_{H_{0}}^{F} \kappa + C_{H_{0}}^{F} \frac{\tilde{\Theta}}{V_{0}} + C_{H_{1}}^{F} \tau \right)$$
(3-160)

we can write the loads in the form of "unit solutions."

$$\{L\} = \{\frac{\partial L}{\partial \alpha}\} \alpha + \{\frac{\partial L}{\partial \phi}\} \frac{\dot{\phi}}{v_{\omega}} + \{\frac{\partial L}{\partial \phi}\}\}$$
(3-161)

where

.

$$\begin{split} & \{\frac{\partial L}{\partial \alpha} \frac{1}{2} = (\frac{\partial V^{2}}{\partial \alpha} [R] \left( [A] \frac{1}{2} i \frac{1}{N} \frac{C_{R}^{F}}{M} + [A] \frac{1}{2} \overline{x} - \overline{x} \frac{C_{M}^{F}}{L} + [L_{E}] \frac{1}{2} i \frac{1}{N} \frac{1}{2} C_{H_{K_{j}}}^{F} \right) \\ & - [L_{R}] [Q] \frac{1}{2} \frac{\partial Q}{\partial \alpha} \frac{1}{2} + \frac{1}{2} n \frac{1}{2} C_{H_{K_{j}}}^{F} \right) \\ & \{\frac{\partial L}{\partial Q} \frac{1}{2} = \frac{n_{M}^{2} \sqrt{n}}{2} [R] \left( [A] \frac{1}{2} i \frac{1}{2} \frac{C_{L}^{F}}{M} + [A] \frac{1}{2} \overline{x} - x \frac{1}{2} \frac{C_{M}^{F}}{L} + [L_{E}] \frac{1}{2} \overline{x} - x \frac{1}{2} - [L_{R}] \frac{1}{2} \frac{\partial Q}{\partial \alpha} \frac{1}{2} + \frac{1}{2} n \frac{1}{2} C_{H_{K_{j}}}^{F} \right) \\ & \left(\frac{1}{2} \frac{\partial L}{\partial \beta} \frac{1}{2} = \frac{n_{M}^{2} \sqrt{n}}{2} [R] \left( [A] \frac{1}{2} i \frac{1}{2} \frac{C_{L}^{F}}{M} + [A] \frac{1}{2} \overline{x} - x \frac{1}{2} \frac{C_{M}^{F}}{L} - [L_{R}] \frac{1}{2} \frac{\partial Q}{\partial \beta} \frac{1}{2} + \frac{1}{2} n \frac{1}{2} C_{H_{K_{j}}}^{F} \right) \\ & \left(\frac{1}{2} \frac{\partial L}{\partial \beta} \frac{1}{2} + \frac{1}{2} n \frac{1}{2} C_{H_{K_{j}}}^{F} + [A] \frac{1}{2} \overline{x} - x \frac{1}{2} \frac{C_{M}^{F}}{L} - [L_{R}] \frac{1}{2} \frac{\partial Q}{\partial \beta} \frac{1}{2} + \frac{1}{2} n \frac{1}{2} C_{H_{K_{j}}}^{F} \right) \\ & \left(\frac{1}{2} \frac{\partial L}{\partial \beta} \frac{1}{2} + \frac{1}{2} n \frac{1}{2} C_{H_{K_{j}}}^{F} + [A] \frac{1}{2} \frac{1}{2} \frac{C_{L}^{F}}{M} + [A] \frac{1}{2} \frac{1}{2} \frac{C_{L}^{F}}{L} + \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{C_{L}^{F}}{L} \right) \\ & \left(\frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{C_{L}^{F}}{M} + [A] \frac{1}{2} \frac{1}{2} \frac{C_{L}^{F}}{L} + \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{C_{L}^{F}}{L} \right) \\ & \left(\frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{C_{L}^{F}}{L} + [A] \frac{1}{2} \frac{1}{2} \frac{C_{L}^{F}}{L} + \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{C_{L}^{F}}{L} + \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{C_{L}^{F}}{L} + \frac{1}{2} \frac{1}{$$

3.1.2.5 The Aeroelastic Effects of Large Axial Loads on Lateral Motions

In our discussions of beam theory in Paragraph 2.3.3.1, we assumed that the specific strain energy for a particle of a beam is given by

$$u(x, y, z, t) = z \in f_{xx}^{\perp}$$
 (3-105)

end that the strain is related to the displacements by

$$\dot{z}_{xx} = -z \frac{16z}{2x^2}$$
 (3-166)

In the presence of large axial loads, however, a more precise description of strain must be used. The Lagrangian-coordinate strain for arbitrary displacements of a one-dimensional beam (whose curvature is small) isl

 $z_{w} = -z \frac{1}{z^{y_{z}}} - \frac{1}{z^{y_{z}}} - \frac{1}{z^{y_{z}}} + \frac{1}{z^{y_{z}}} - \frac{1}{z^{y_{z}}} + \frac{1}{z^{y_{z}}} -  

The last three terms are the contribution from longitudinal strain of the neutral surface. On the basis that the longitudinal strain is small, we have

$$\frac{1}{2} \ll \frac{1}{2\pi}$$
(3-16°)

so that

Using Equation 2-56, the total strain energy is

$$= \frac{1}{2} \int_{-\infty}^{\infty} Ez^{2} dq z = \frac{1}{2} \int_{-\infty}^{\infty} \frac{1}{2} \int_{-\infty}$$

For symmetrical cross sections, the second term is zero because

$$\int z z d d z = 1 \qquad (3-171)$$

If we neglect the fourth order terms,  $\frac{\frac{1}{2}}{\frac{1}{2}}$ , then Equation 3-170 becomes

$$= 2 \left[ \frac{1}{2} \left[ \frac{1}{2} + \frac{1}{2} \right] - \frac{1}{2} \left[ \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right] \frac{1}{2} + \frac{1}{2} \left[ \frac{1}{2} + $

13ee Green and Zerna Theoretical Elasticity, Oxford, 1954.

The virtual work of the internal forces can be obtained from

If we write the virtual work of external forces as

• •

.

$$\sum_{z=1}^{n} \sum_{z=1}^{n} \sum_{z=1}^{n} \sum_{x=1}^{n} \sum_{x$$

then using D'Alembert's Principle, the Principle of Virtual Work, in this case, becomes

In this expression  $A_{\rm X}$  is the uniform rectilinear acceleration of every particle parallel to the  ${\tt j}$  -axis.



FIGURE 31 AXIALLY LOADED LAUNCH VEHICLE

Since  $\delta \mathbf{P}_{\mathtt{r}}$  and  $\delta \mathbf{P}_{\mathtt{r}}$  are arbitrary, we must have

$$m \frac{d^{2} p_{z}}{dz^{2}} = p_{z} + \frac{d^{2}}{dx^{2}} (ez \frac{d^{2} p_{z}}{dx^{2}}) - \frac{d}{dx} = A \frac{d^{2} x}{dz^{2}} \frac{d^{2} z}{dz^{2}}$$
(3-176)

$$m A_{\mathbf{x}} = -\frac{1}{2} - \frac{1}{2x} = A \left[ \frac{b_{\mathbf{x}}}{2x} - \frac{b_{\mathbf{x}}}{2x} - \frac{b_{\mathbf{x}}}{2x} \right]$$
(3-177)

We cannot solve these nonlinear equations exactly; however, we can use the last equation to solve for  $\frac{1}{2}$  and eliminate this term in the nonlinear part of the strain energy (Equation 3-172). This procedure would be equivalent to the first iteration in a Ficard solution to the nonlinear Equations 3-176 and 3-177.

From Equation 3-177, we have then

$$\Xi^{A} \frac{\partial A}{\partial x} = -\int_{-1}^{1} F_{x} - MA_{x} dx - \frac{GA}{2} \frac{\partial E}{\partial x} dx \qquad (3-178).$$

If we introduce the definition,

$$h_{x}(x) = -\int_{0}^{\infty} (P_{x} - m_{x} \Delta_{x}) (x)$$
 (3-179)

then

$$\frac{i}{3} = \frac{N}{4} - \frac{1}{2} \left( \frac{3}{25} \right)^{2} \qquad (3-160)$$

Substituting this into the nonlinear term (only) in Equation 3-172, we obtain<sup>1</sup>  $\frac{1}{11}$  we denote the order-of-magnitude of N by  $\kappa$  and the order-of-magnitude of

 $\frac{2}{2}$  by  $\epsilon$  , then the limiting process is defined by

$$\frac{1}{2}\frac{1}{2}\frac{1}{2} = x = a = "constant"$$

and

EA = order-of-magnitude of 
$$\kappa^2$$

If we then let  $\epsilon \rightarrow 0$  and  $\kappa \rightarrow \infty$ , we have from Equation3-L0 that

 $e = \frac{k}{k^2} - \frac{w_{ic}}{w_{ic}}$ 

and hence

Thus  $\frac{1}{2}$  is of the same order-of-magnitude as  $\frac{1}{2}$ , and  $\frac{1}{2}$  is of the order-of-magnitude of

$$(\kappa \epsilon)^{\perp} = (-\alpha)^{\perp} = \alpha^{"} \text{constant"}$$

$$U = \frac{1}{2} \int_{0}^{1} \left( EI(x) \left( \frac{\partial^{2} p_{z}}{\partial x^{2}} \right)^{2} + EA(x) - \frac{\partial p}{\partial x} \right)^{2} + N(x) - \frac{\partial p}{\partial x} \right)^{2} = 0.$$
 (3-181)

where, consistent with approximations already made, we have neglected terms in the strain energy of fourth order.

In the case of a slender missile the axial loads arise from distributed dragy, thrust, and gravity (see Figure 35)

$$P_{x} = \frac{1}{2} p_{int} v_{int}^{x} C_{D}(x) - T(x) cost(x) + M(x) q_{SM} \theta$$
(3-182)

where T(x) is the mightude of the thrust per unit of length along the missile and  $\epsilon(x)$  is the misslightent of the thrust distribution from the body exis.



FIGURE 32 DISTRIBUTION OF AXIAL COMPONENT OF THRUST



FIGURE 33 DISTRIBUTION OF LATERAL COMPONENT OF THRUST

Substituting Equation 3-102 into Equation 3-179, we obtain

 $f_{x,x}(x) = -\int_{0}^{x} \frac{dm_{0}^{2}}{2} e_{-D(x)} - \frac{\pi(x)}{2} \cos(x) - m(x) \sin^{2}(x)^{2}$   $-m(x) A_{x} = d_{x}$ (3-183)

We then not a that

$$N(L) = -\int_{-\infty}^{L} \frac{\partial u_{x}^{2}}{\partial z} J_{E}(x) - T(x) \cos(x) - m(x) A_{x} + m(x) q \sin^{-2} dx = F_{x} - MA_{x} \qquad (3-184)$$

where

 $F_{\mu} = \sum_{i=1}^{n} \frac{1}{2} \sum_{i=1}^{n} \frac{1}$ 

= tor sotal applied actual force

ani

$$h = \int_{a}^{b} m(x) dx = to(a) muss \qquad (3-186)$$

From Long\_tucina' e mil briun

$$F_{x} = MA_{x}$$
 (3-187)

SQ

We car us the relation

 $h_x = \int m^2 - \frac{m^2}{2} \int \frac{m^2}{2} \frac{m^2}{2} \int \frac{m^2}{2} \frac{m^2}$ 

to eliminate the axial equivalentian the regulation p-105 and express N(x) in terms of the drag and throat only.

-

$$(3-190)$$

The third term in Equation 3-181 can then be treated in the same manner as the first term (the second term which is involved in longitudinal dynamics does not concern us here). Using the interpolation formula, we have

$$\frac{\partial P_{i}}{\partial x} = \frac{1}{y_{i}} \{ 0 \mid x_{i} \neq y_{i} \}_{i}$$
(3-191)  
$$x_{i-1} \leq x \leq x_{i}$$

Using this in Equation 3-181, we obtain

.

$$U = \frac{1}{2} \int_{0}^{L} N(X) \left( \frac{\partial p_{2}}{\partial X} \right)^{2} dX + \frac{1}{2} \int_{0}^{L} EI(X) \left( \frac{\partial p_{2}}{\partial X^{2}} \right)^{2} dX$$

$$= \frac{1}{2} \sum_{i=1}^{N} \frac{1}{2} \frac{p_{1}^{2}}{i} \left[ \left( K \right)_{i} \frac{1}{2} \frac{p_{1}}{i} + \frac{1}{2} \frac{p_{1}^{2}}{i} \left[ N \right]_{i} \frac{1}{2} \frac{1}{2} \frac{p_{2}^{2}}{i} \right]$$

$$(3-192)$$

where, if we make the same approximations discussed in Paragraph 2.3.3.4, we obtain

$$[N]_{\hat{i}} = \frac{N_{\hat{i}}}{120J_{\hat{i}}} \begin{bmatrix} 4 & -7 & 2 & i \\ -7 & 36 & -131 & 2 \\ 2 & -131 & 136 & -7 \\ i & 2 & -7 & 4 \end{bmatrix}$$
(3-193)

with 
$$N_i$$
 = average value of  $N(x)$  on the i<sup>th</sup> in-

(3-194)

terval.

.

The appropriate expression for the contribution of axial loads that is valid in the first and last bay is

•

$$[N_{i}]_{\hat{i}} = \frac{N_{\hat{i}}}{N_{i}} \begin{bmatrix} 2.6 & -132 & 6 \\ -13.2 & 144 & -12 \\ 6 & -12 & 6 \end{bmatrix} \text{ for } \hat{i} = 1.$$

(3-195)

 $[N]_{i} = \frac{N_{i}}{120 l_{i}} \begin{bmatrix} 126 & -132 & 6 \\ -132 & 144 & -12 \\ 6 & -12 & 6 \end{bmatrix} \text{ for } i = N$ 

Using

$$\{b\}_{\bar{i}} = [\tau]_{\bar{i}}\{b\}$$
 (3-196)

we have

$$U = \frac{1}{2} \left( \frac{1}{2} \beta^{2} [K] \frac{1}{2} \beta^{2} + \frac{1}{2} \frac{1}{2} \beta^{2} [N] \frac{1}{2} \beta^{2} \right)$$
(3-197)

where

$$[N] = \sum_{i=1}^{N} [\tau]'_{i}[N]_{i}[\tau]_{i}$$
(3-198)

## 3.1.2.6 Modilication of the Loads Equations to Include Gravity, Thrust, Drag, and Air Loads Due to Asymmetries

In our considerations of axial loads in the preceding section, we included only the component of thrust parallel to the x-axis. If we were to include only this effect, we would obtain unrealistic results in the consideration of loads and stability of the missile. To illustrate, for the rigid missile if we were to assume that the thrust acts tangent to the missile axis, then the moment of the thrust (assumed concentrated at x=L) about the center of mass is given by a contribution from the axial component,  $T_{(X,\Theta)}(-x)_{SM\Theta}$ 

and a contribution from the lateral component,  $-\pi i w e^{(r_{-} - \vec{x})} cose$ 



FIGURE 34 MOMENT OF THE THRUST FOR A RIGID MISSILE

The sotal moment of the same is the sum of two efforts

The competences

which is identically zero, reflecting the fact that the thrust, in this case, acts through the center of mass. It can be shown that the moment contributed by the axial component of thrust, in the case of a flatible missile, is exactly accounted for when the axial effects are considered as in the previous section. The moment contributed by the lateral component is computed in a straightforward fashion from the virtual work of the distributed lateral forces. The argument presented here applies only to forces, like the thrust, which act tangent to the body. The drag, for example, is assumed to act parallel to the x-exis (i.e., to the free-stream) independently of the motions of the body.

#### 3.1.2.6.1 The Virtual Work of Distributed Lateral Forces

The virtual work of gravity, thrust, and air loads at zero angle-of-attack is given by

$$(= -\frac{2}{2} \int \frac{dx}{dx} \int \frac{dy}{dx} \int \frac{$$



FIGURE 35 DISTRIBUTED LATERAL FORCES
If the thrust is assumed to be concentrated, then

 $-\int_{-\infty}^{\infty} \frac{\partial P_{\text{eff}}}{\partial r} = \int_{-\infty}^{\infty} \frac{\partial P_{\text{eff}}}{\partial r} = \int_{$ 

is replaced by









In order to compute the generalized forces, we may use the interpolation equation (Equation 3-2) to express Equation 3-199 in terms of the generalized coordinates,  $p_{1}$ .

$$\phi_{z}(x,t) = \{ : s \ s^{2} \ s^{2} \ f[z]_{\bar{t}}[\tau]_{\bar{t}} \ t \}$$
(3-200)

which we may write as

$$b_{\Xi}(x,t) = \{ h_{\Xi}(x) \} \{ b \}$$
(3-201)

where

$$\{h_{z}(x)\}' = \{i \in \mathcal{E}^{x} \mathcal{E}^{3}\}[\mathcal{I}]_{z}[\mathcal{I}]_{i}$$
(3-202)

for 
$$x_{i-1} \leq x \leq x_{i-1}$$

we then can write

$$\varepsilon_{W} = -\frac{1}{5}\varepsilon_{D}^{L} \frac{1}{2} \frac{$$

Now, when

$$\{b\} = \{1\}$$
 (3-204)

we have

$$\phi(x,t) = | \text{ for all } x. \tag{3-205}$$

so that

$$= -\pi_{z} [+] [-]$$
 (3-206)

and the virtual work of gravity forces can be written as

but

$$[A] = \int_{a}^{b} n_z h_z h_z + m_z dz = \text{the inertia matrix} \qquad (3-207)$$

so that the gravity forces are

$$SW = -1555 [AH]_{+-10}^{+-10$$

If we introduce

$$\int -\frac{1}{2} = \int \frac{1}{2} \int \frac{1}{2\pi e^2} T x^2 T x e^{-\frac{1}{2}} dx \qquad (3-209)$$

and

.

$$[-] = \begin{bmatrix} 1 & \frac{1}{2} & \frac{1}{2} & \frac{1}{2} \end{bmatrix}$$
 (3-210)

anđ

the virtual work of the distributed lateral forces in Equation 3-199 becomes

3.1.2.6.2 The Loads Equations Including Axial and Lateral Forces

The kinetic energy, from Equation 3-33, is

$$T = \frac{1}{2} F F F A F F$$
 (3-213)

The strain energy, including axial load effects, is

$$c = (-25)^{2} + 25^$$

and the virtual work of all external forces is

$$SW = - s_{\text{EV}} = - \frac{1}{2} s_{\text{E}} = - \frac{1}{2}$$

The control deflection is related to the generalized coordinates of the system Ъγ

We may also generalize the previous results by accounting for energy dissipation in the structure through Rayleigh's dissipation function (see Paragraph 2.2.2)

\_

$$P = g + \beta F [B] + \delta f$$
 (3-217)

Employing Lagrange's equations in conjunction with the above system of equations, we obtain

$$[-FB] = [E] [FF] = [FF] = [FB]$$

$$= - \frac{1}{2} [FB] = \frac{1}{2} [FB$$

If we assume that the structural loads are given by

$$f = [K K + i 3 H ]$$
 (3-219)

then the loads equations are:

.

.

.

$$FF = -[ARF] - [NFF] - \frac{1}{20} \sqrt{2} ([L_RFF] + \frac{1}{2} (L_FFF] + \frac{1}{2} L_FFF] - \frac{3}{20})$$
$$- [ARF] - \frac{1}{20} + \frac{1}{$$

The internal loads, member loads, and stresses are related to these loads by a simple transformation. The modal equations of motion that are compatible with the above equations are given by

where

$$[-] = [\phi]'([N] + [\phi])[\phi]$$
 (3-222)

anđ

$$[F] = [\phi][B][\phi]$$
 (3-223)

#### 3.1.3 Aeroelastic Stability

## 3.1.3.1 "Static" Stability of a Free Missile with Locked Controls

Our preliminary discussion will exclude the effect of axial loads even though these effects are a source of destabilizing moments on the unrestrained missile. We will assume the system to be completely described, for stability purposes, by its kinetic energy, strain energy, and the virtual work done by aerodynamic forces. In this section the control surface is assumed to be constrained at the  $\gamma = 0$  position.



FIGURE 38 THE UNCONTROLLED MISSILE

The equations governing the motion of the system in this case are obtained from Equations 3-27, 3-28, and 3-29 by setting  $\gamma = 0$ 

$$\mathbf{T} = \frac{1}{2} + p_2 \left[ \mathbf{A} \right] + p_2$$

$$\mathbf{U} = \frac{1}{2} i \mathbf{p} \mathbf{j}' [\mathbf{K}]^{\dagger} \mathbf{p} \mathbf{j}$$
(3-225)

$$\delta W = -\frac{1}{2\pi m^2} + \frac{1}{2} \beta \frac{1}{2} \left[ \Lambda \frac{1}{2} \pi \right]$$
(3-226)

where

$$dt = [-dz^2 + \sqrt{z} dz^2]$$
(3-227)

The fact that the body is unrestrained is evidenced by (Equations 3-65 and 3-66).

$$[k, j_{\ell}] = \{z\}$$
 (3-226)

$$[h, b, k-k] = -b t \qquad (3-229)$$

Because of the subtle nature of the stability of an unrestrained body, it is best to introduce the subject by briefly discussing the complete dynamic stability which should cover static stability as a special case.

Lagrange's equations (Equation 2-64) may be applied to Equations 3-224, 3-225, and 3-226 to obtain the following equations of motion

$$[A]{b} + [K]{b} + \frac{1}{2} e^{-\frac{1}{2}} [A][A]{b} + \frac{1}{2} [A][A]{b} + \frac{1}{2} [A][b] = \frac$$

To investigate the stability of the system described by these linear differential equations, we shall use the conventional Laplace transform techniques. Application of the Laplace transform,

$$f_{p}a_{t}^{2} = \int_{0}^{x} f_{p}b_{t}^{2}e^{St}dt , \qquad (3-231)$$

to Equation 3-230 yields

 $\frac{1}{2} \left[ A \right] + \frac{1}{2} \left[ A \right] + \left[ A \right] + \frac{1}{2} \left[ A \right] +$ 

The stability of the system is governed by the equation

$$\Delta = \frac{\pi e^2}{2} = \frac{e^2 [A]}{e} = \frac{\pi e^2}{2} [1] - [A] - \frac{\pi e^2}{2} [1] = 0$$
(3-233)

In the conventional sense the static stability would be governed by the special case,

$$\lim_{x \to 1} \frac{1}{2} = \frac{1}{2} \frac{1}{2} \frac{1}{2} = \frac{1}{2} \frac{1}{2} \frac{1}{2} = \frac{1}{2}$$
(3-234)

which yields

The associated eigenvalue problem is given by setting s = 0 in Equation 3-232

$$[[k] - \frac{1}{2} a v_{D} [A][A]] = \frac{1}{2} b = \frac{1}{2} (3-236)$$

It is quickly shown that this problem has an infinity of eigenvalues. To illustrate, for any value of the dynamic pressure,  $\frac{1}{24M_{\odot}^2}$ , we have

$$[\kappa]_{-\frac{1}{2}} a_{\alpha} a_{\alpha} [\Lambda [[\Delta I]] + i]_{2} = \{c\}$$
(3-237)

This is true because

$$[\kappa]_{1} = \{c\}$$
 (3-238)

from Equation 3-228, and

$$(3-239)$$

from Equation 3-67. As a consequence, all the coefficients in the polynomial  $\Delta(0, \xi_{0}, \xi_{0})$  are zero. It is more convenient, perhaps, to say that the problem has no eigenvalues.

The physical significance of these conclusions is that the system governed by Equation 3-230 is, in the strictest sense, incipiently unstable at any airspeed or altitude. The system, however, acts passively and the only result of the "instability" is the fact that the missile can translate laterally in a quasi-static fashion without producing forces which would restore it to its initial position.

The straightforwardness of this problem is also obscured in the dynamic case. We will find that Equation 3-233 has a repeated zero root in s (for any fixed value of the dynamic pressure). The result of our discussion in this section will show that the logical criterion for "static" stability is that

$$\frac{1}{2} \frac{2}{\sqrt{2}} = 2$$
 (3-240)

The lowest value of dynamic pressure which satisfies this equation will be called the "dynamic pressure of divergence."

### 3.1.3.1.1 Divergence for the Vehicle in Rectilinear Flight

In spite of the fact that the missile is inherently unstable in the static case, it is instructive to look at the problem from a physical standpoint and imagine an artificial set of forces which constantly maintain lateral equilibrium. To insure that the problem is uniquely defined, we will require that the true center of mass of the missile move with constant velocity,  $V_{\infty}$ , along a straight path. The instantaneous displacement of the center of mass is given by (see the comments regarding Equation 3-92)

$$s = \frac{1}{M} i I I [A] [b]$$
 (3-241)

The requirement of rectilinear flight is then equivalent to  $\zeta = 0$  which leads to the following constraint on the generalized coordinates,  $p_i$ .

$$\frac{1}{2} \frac{1}{2} = 0$$
 (3-242)

In the static case,

$$(3-243)$$

and the governing equations are given by Equations 3-225 and 3-226

$$c = \frac{1}{2} $

$$s_{ii} = -\frac{1}{2} \frac{1}{2} \frac{$$

subject to the constraint,

$$+:: [A]+p! = +: : [3-246]$$

### 3.1.3.1.1.1 Collocation Method

Using Lagrange's equations for a redundant set of coordinates (Equation 2-79 of Paragraph 2.1.2.2), we have, in this case,

$$\frac{1}{10i} = \frac{1}{10} \frac{1}{10$$

$$z_{N} = \sum_{i} z_{P_i} P_i \qquad (3-248)$$

where  $\lambda$  is Lagrange's undetermined multiplier corresponding to the constraint,  $\zeta = 0$ . Using Equations 3-244 and 3-245 in Equation 3-247, we obtain

$$[\kappa]\{\flat\} = \lambda [\Lambda]\{I\} - \mathbb{I}_{[\omega V_{\omega}]} [\Lambda][\Delta]\{\flat\}$$
(3-249)

For the purpose of arriving at an eigenvalue problem governing the effective loads instead of displacements, we introduce

$$\{F\} = [K] \}$$
 (3-250)

We can then write Equation 3-249 as

.

$$\{F\} = \lambda [A]\{H\} - \sum_{\alpha} v_{\alpha}^{2} [A][\Delta] \{\varphi\}$$
(3-251)

Even though [K] is singular it is possible to rewrite Equation 3-250 as

$$\{i_{j}\} = \{i_{j}\} + \{i_{x} - i_{y}\} = -[\Gamma] [\in ]\{F\}$$
 (3-252)

provided

$$\{i\}\{r\} = 0$$
 (3-253)

and

$$\{\bar{x} - x\} \{\bar{z}\} = 0$$
 (3-254)

This is a special application of the result we obtained in Equation 2-256 of Paragraph 2.2.3.4. Equations 3-253 and 3-254, in this case, correspond to Equation 2-258 of the same paragraph.

.

Using Equation 3-251, Equations 3-253 and 3-254 lead to

$$A_{1}^{2}[A_{1}]_{1}^{2} - \frac{1}{2}e^{2} + \frac{1}{2}[A_{1}]_{1}^{2} = 0 \qquad (3-255)$$

$$x_{x-x} [A_{h}] = \sum_{x \neq x} [x_{x-x}] [A_{h}] = 0$$
 (3-256)

We can use the first equation to eliminate  $\lambda$  from Equation 3-251

Substituting this into Equation 3-251, we obtain

$$\{F_{j}^{*}=-\frac{1}{2}\sqrt{2}\left[ \begin{bmatrix} 1\\ 1 \end{bmatrix} - \frac{1}{M} [AHISIS] \right] [A][\Delta][A][A][b]$$
(3-258)

where we have used Equation 3-45,

$$-1f[A]_{21} = M$$
 (3-259)

Using Equation 3-47,

$$\frac{1}{3-260}$$

in Equation 3-256 along with Equation 3-252, we obtain

$$ix - xi[\Lambda][\Lambda][i]: +ix - xi + [\Gamma][EEFi] = 3$$
 (3-201)

Further, if we make use of (Equations 3-66 and 3-67)

$$[\Delta]_{ij} = \{2\}$$
 (3-202)

and

.

$$[\Delta]_{3,-263}$$

then from Equation 3-201, we obtain

$$g = \frac{1}{12-45} (7-45) r (14) r (14) r (3-264)$$

where we may recognize (from Equation 3-63) that

$$\pm \bar{x} - \pm \left[ A \right] = C_{n_{for}} \qquad (3-265)$$

Since  $\zeta = 0$ , we have

$$\{p\} = \{\bar{*}, \bar{*}\} \oplus + [r] [e] \{\bar{r}\}$$
 (3-266)

and using Equation 3-252

and from Equation 3-264,

$$[A \parallel \Delta H p H = [r_1 - \frac{1}{2} P H H m m H h m m h] [A \parallel \Delta \Pi - 1 [r_1 - 1 m h]] (3-266)$$

Substituting this into Equation 3-258, we obtain

$$[z][r][4][r][2][r][2][r] = \frac{z}{z_{1}z_{2}} r^{2}$$

where

$$[z] = [x - \sqrt{|x|}] + [z - \sqrt{|z|}] + [z - \sqrt{|z|}]$$
(3-270)

This equation can be solved by iteration in much the same way as the equation governing the vibration modes (Equations 2-220 or 2-279 of Paragraph 2.2.3.4). The result converges to the lowest value of dynamic pressure and the corresponding distribution of effective loads<sup>1</sup>. Figure 39 shows the results of the application of this method to data for the NASA Scout solid-propellant launch vehicle.

<sup>&</sup>lt;sup>1</sup>The method, presented here, of introducing the loads as eigenvectors instead of the displacements is due to Vernon L. Alley, Jr. of the MASA Langley Research Center. We will make use of it again in the next section.



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FIGURE 39 DISTRIBUTION OF LOADS IN THE DIVERGENCE MODE – CASE OF RECTILINEAR FLIGHT

# 3.1.3.1.1.2 Modal Method

.

An alternative to the above approach is to use a modal approximation. If we use Equation 3-109, we can write (for  $\gamma=0$ )

$$(3-271)$$

Substituting this into Equations 3-224, 3-225, and 3-226, we obtain

$$= 2 (3-272)$$

$$\begin{aligned} \nabla w_{l} &= -\frac{1}{2} \frac{1}{2} \sqrt{\frac{1}{2}} + \frac{1}{2} \frac{1}{2} \left[ \frac{1}{2} + \frac{1}{2} \right] \left[ \frac{1}{2} + \frac{1}{2}$$

In the static case, we have  $\dot{\xi} = 0$ ,  $\dot{\theta} = 0$ , and also, consistent with the assumptions made in the first method, we constraint the center-of-mass to move in a straight line by taking

.

$$\zeta = c \qquad (3-275)$$

Using Equation 2-179 of Paragraph 2.2.3.2

$$f[k][k] = [1]$$
 (3-276)

we obtain

$$5N = -\frac{1}{2} \log \frac{1}{2} = \frac{1}{2} \log \frac{1}{2} + \frac{1}{2} \log \frac{1}{2} + \frac{1}{2} \log \frac{1}{2} = \frac{1}{2} \log \frac{1}{2} + \frac{1}{2} \log \frac{1}{2}$$

The constraint,  $\zeta = 0$ , is satisfied explicitly in terms of modal generalized coordinates so that we may use Lagrange's equations for a set of independent generalized coordinates (Equation 2-64 of Paragraph 2.1.2.1) to obtain

$$\int \frac{1}{2\pi m_{e}^{2}} \left[ \frac{1}{4} \int \frac{1}{4} \right] + \frac{1}{4} = \frac{1}{4} \ln \frac{1}{4} \int \frac{1}{4} \ln \frac{1}{4} + \frac{1}{4}$$

$$4\bar{x} - 4 \left[ \Lambda \right] \left[ \psi \left[ \frac{1}{2} \right] - 4\bar{x} - 4 \left[ \Lambda \right] \left[ \frac{1}{2} \right] + \frac{1}{2} = 0$$
 (3-280)

using

$$C_{M_{R}} = \{i - x\} [\Lambda] \{i\}$$
 (3-28L)

we can eliminate  $\theta$  from Equation 3-279 and substitute it into Equation 3-280 with the result that

$$\left( \left[ \left[ \tau_{\lambda} \right] + \left[ \frac{1}{2\rho_{0}} \gamma_{0}^{2} \right] \left[ \left[ \frac{1}{2} \right] - \left[ \frac{1}{2\rho_{0}} \left[ \Lambda \right] \left[ \frac{1}{2} \right] \left[ \frac{1}{2\rho_{0}} \left[ \frac{1}{2\rho_{0}} \left[ \frac{1}{2\rho_{0}} \left[ \frac{1}{2\rho_{0}} \right] \left[ \frac{1}{2\rho_{0}} \left[$$

which we can also write as

$$-1\times1.1^{1}(1)-\frac{1}{2}(1)+1(1+2-x)\left[1\times1(y)+1(y)\right] = \frac{y}{2\pi^2}(1)+1(1+2-x)\left[1\times1(y)+1(y)\right]$$

This equation can also be solved by the method of iteration, and the eigenvalues will approach the eigenvalues of Equation 3-269 as the number of modes considered approaches the number of degrees-of-freedom used in Equation 3-269. Figure 40 illustrates this for the case where 25 collocation points are used in Equation 3-269. In the modal case, the number of degrees-of-freedom is equal to 2 plus the number of elastic modes.



FIGURE 40 DYNAMIC PRESSURE OF DIVERGENCE AS A FUNCTION OF THE NUMBER OF MODES CONSIDERED – CASE OF RECTILINEAR FLIGHT

As a matter of practical computation, Equation 3-283 can be expressed as,

$$- \sum_{i=1}^{n} \left\{ \sum_{\substack{k=1\\k\neq k}}^{n} \right\} - \frac{1}{\frac{1}{2}} \sum_{\substack{k=1\\k\neq k}}^{n} \sum_{j=1}^{k} \sum_{\substack{k=1\\k\neq k}}^{n} \sum_{\substack{k=1\\k\neq k}}^$$

where the  $C^{\mathbf{R}}$ 's are elements from the matrix

$$\begin{bmatrix} 1 \\ 1 \end{bmatrix} \begin{bmatrix} \Delta \end{bmatrix} \begin{bmatrix} \Delta \end{bmatrix} \begin{bmatrix} 1 \\ 1 \end{bmatrix} \begin{bmatrix} 1 \end{bmatrix} \begin{bmatrix} 1 \end{bmatrix} = \begin{bmatrix} 1 \\ 2 \end{bmatrix} \begin{bmatrix} R \\ -2 \end{bmatrix} \begin{bmatrix} 1 \\ -2 \end{bmatrix} \begin{bmatrix} 1 \\ 2 \end{bmatrix} \begin{bmatrix} 2 \\ -2 \end{bmatrix} \begin{bmatrix} 1 \\ -2 \end{bmatrix} \begin{bmatrix} 2 \\ -2 \end{bmatrix} \begin{bmatrix} 1 \\ -2 \end{bmatrix} \begin{bmatrix} 2 \\ -2$$

Also, from Equation 3-284 we may derive the following approximate formula based on using only one mode:

.

$$\frac{1}{\lambda \rho_{\rm e} v_{\rm e}^2} = \frac{1}{\lambda} \frac{1}{\frac{c_{\rm e}^2 + c_{\rm e}^2}{c_{\rm e}^2} - c_{\rm e}^2}}{\frac{c_{\rm e}^2}{c_{\rm e}^2} - c_{\rm e}^2}$$
(3-286)

For preliminary design purposes, these terms can be calculated from

.

$$-\frac{1}{22} = -\int_{0}^{1} \frac{dx}{dx} x \, \bar{x} - \bar{x}^{2} \, dx \qquad (3-287)$$

$$\frac{1}{10} = \frac{1}{10} $

$$\frac{1}{2} = \frac{1}{2} x + \frac{1}{2$$

.

with  $\phi(\mathbf{x})$  normalized so that

$$\int_{-\infty}^{\infty} |t| x = 1$$
 (3-292)

These formulas are fairly useful even when the shape of the first mode,  $\phi(\mathbf{x})$ , is essumed. If an assumed mode is used, it should satisfy the "rigid-body" orthogonality conditions:

$$-3$$
 (3-293) (3-293)

$$\int_{-\infty}^{\infty} |\dot{x} - x_{1}| |\psi(x_{1} - x_{1}) dx = 0$$
 (3-294)

The approximate formula, Equation 3-286, should be used with caution since Figure 40 shows the one-mode formula (three degrees-of-freedom) to give results on the unconservative side. For example, the one-mode formula gives

$$z_{\alpha}^{2} y_{\alpha}^{2} = 17.429$$
 is with (3-295)

whereas the "exact" collocation method gives

.

$$2\lambda_2 v_2^2 = 3 \lambda_2 \varepsilon_4 \lambda_{10} v_4 \qquad (3-296)$$

The actual shape of the missile in the divergence mode may be calculated from the eigenvector in Equation 3-284 by using Equation 3-271 with

.

.

anđ.

.

$$\dot{e} = -\frac{1}{2} \left\{ \frac{1}{2} + \frac{1}{2} \right\}$$
 (3-298)

(See Equation 3-260)

so that

$$(3-299)$$



Using this relation along with the eigenvector obtained by iteration of Equation 3-284, the result shown by Figure 41 was obtained.

### FIGURE 41 SHAPE OF THE MISSILE IN THE DIVERGENCE MODE - RECTILINEAR FLIGHT CASE

# 3.1.3.1.2 Divergence for the Vehicle in Steady Flight in a Circular Path

We have noted that it is impossible for an uncontrolled vehicle to be completely stable, in the strictest sense, when perturbed from a straight line path. It is possible, however, that the missile can achieve a configuration of stable equilibrium when the center of mass follows a trajectory which is a circle of radius, say, R.

The collocation method of analysis in this section is due principally to Vernon L. Alley, Jr., of the NASA Langley Research Center.

### 3.1.3.1.2.1 Collocation Method

The complete dynamic equations of motion are given by Equation 3-230. If we introduce the definition,

$$\{r\} = [K, Hp]$$
 (3-300)

then we can write Equation 3-230 as

$$FF = -[A][p] - \frac{1}{2}m^{\frac{1}{2}}[A][A][A][CA][CA] - \frac{1}{2}m^{\frac{1}{2}}[A][CA][CA][CA]]$$

Τf

and

then we can write Equation 3-300 as

 $r_{2} = r_{1} r_{2} - r_{1} r_{1} + -[r_{1}] r_{2} r_{2} = \frac{1}{2} r_{1} r_{2} r_{2} r_{3} r_{4} r_{$ 

(This follows the same line of reasoning as the procedure in Paragraph 3.1.3.1.1.1.)

We can satisfy both Equations 3-302 and 3-303 by assuming that the trajectory of the center of mass is a circle of radius, R; and that the motion is otherwise steady; and, in particular,  $\ddot{\theta} = 0$ . These assumptions imply

$$\dot{z} = \frac{\gamma_{z}}{r_{c}}$$
(3-305)

$$\dot{z} = \frac{v_{\Xi}}{z} \tag{3-306}$$

If we integrate these equations, we obtain

$$\Sigma'_{t} = \frac{j_{t}}{2M} z^{2} + j_{(0)} z + j_{(0)}$$
 (3-307)

$$\frac{\partial z}{\partial t} = \frac{\partial z}{\partial t} (z + \frac{\partial z}{\partial t})$$
 (3-308)

Further, we note that the "rigid-body" angle-of-attack, lpha, is given by

$$\begin{aligned} x &= \frac{\dot{T} + \dot{T}}{v_{ct}} - \frac{\partial}{\partial t} \\ &= \frac{v_{ct} t}{R} + \frac{\dot{T} c}{R} - \frac{v_{ct} t}{R} - \frac{\partial}{R} - \frac{\partial}{R} \end{aligned}$$

$$= \frac{\dot{T} c}{R} - \frac{\partial}{\partial t} - \frac{\partial}{\partial t} \end{aligned}$$
(3-309)

= a constant

With no essential loss in generality, we assume

$$T'\sigma_1 = J'c_1 = 0$$
 (3-310)

so that

$$f_{c} = -\chi \tag{3-311}$$

Þ



# FIGURE 42 THE MISSILE IN STEADY CIRCULAR FLIGHT

The trajectory of the collocation points is given by

$$\dot{p}_{t}(t) = \dot{p}_{t}(s) + \frac{v_{at}^{T}}{2R}t^{T} + (\bar{x} - x_{t})\frac{v_{at}}{R}t$$
(3-312)

or

.

$$f_{t}(t) = \{ f(0) \} + \{ i \} \frac{v_{\omega}^{z}}{2R} t^{z} + \{ \overline{x} - x \} \frac{v_{\omega}t}{R}$$
(3-313)

where

.

$$\frac{1}{2} [\alpha_1] = -\frac{1}{2} - \frac{1}{2} \alpha + [\Gamma]' [E] \{F\}$$
 (3-314)

Under these assumptions, we have

$$\{\ddot{p}\} = \{i\} \frac{V_{a}}{R}$$
(3-315)

.

$$\begin{aligned} \{x\} &= [\Delta]\{b\} + \frac{1}{\gamma_{\alpha}} i b\} \\ &= [\Delta][\Gamma][E]\{F\} - \frac{1}{2} i \frac{\gamma_{\alpha}}{E} t - x] \\ &+ \frac{1}{2} i \frac{\gamma_{\alpha}}{R} t + \frac{1}{2} - x \frac{1}{R} \end{aligned}$$
 (3-316)

or

$$\{\alpha\} = \{i\} \alpha + \{i-x\} = \{\Delta[\neg] \in [i]\}$$
 (3-317)

.

Substitution of Equations 3-315 and 3-317 into Equation 3-301, we obtain

.

.

$$= - \left[A \right] \left[ \frac{1}{R} - \frac{1}{2R} \right] \left[ A \right] \left[ \frac{1}{R} - \frac{1}{2R} \right] \left[ \frac{1}{R} $

$$iF_{j}^{2} = -\frac{1}{2} \frac{1}{2} \frac{1}{2} \left( [A][i] \frac{2}{2R} + [A][i] \frac{2}{R} + [A][i] \frac{1}{2} \frac{1}{R} \right) - \frac{1}{2} \frac{1}$$

In this expression, for a given value of  $\rho_{\infty}$  (i.e., at a given altitude), the flight curvature and angle of attack can be chosen so that Equations 3-302 and 3-303 are satisfied. Premultiplying by {1} and { $\bar{x}$ -x}, we obtain

$$\frac{1}{2} \left[ \frac{1}{2} F \right] = -\frac{1}{2} \left[ \frac{2M}{R_0 R} + C_{0} \frac{1}{R} + C_{1} \frac{1}{R} x \right] = \frac{1}{2} \left[ \frac{1}{2} \sqrt{2} \frac{1}{2} \left[ A \right] \left[ A \right] \left[ A \right] \left[ F \right] = 0 \quad (3-320)$$

$$\left[\tilde{x} - x \right]^{2} \left[F\right] = -\frac{1}{2} \left[x + x^{2}\right]^{2} \left(C_{M_{E}} + C_{M_{Z}} E\right) - \frac{1}{2} \left[x + x^{2}\right]^{2} \left[\tilde{x} - x^{2}\right]^{2} \left[\Delta \left[\left[P\right]^{2}\right] \left[E\right] \left[F\right]^{2} = 0 \quad (3-321)$$

If we solve these equations for 1/R and , we obtain

$$\begin{bmatrix} x \\ z \end{bmatrix} = \cdot \begin{bmatrix} z_{w} & \frac{2M}{2k} + z_{w} \end{bmatrix} \begin{bmatrix} z_{1}z & [\Delta ][\Delta ][\Delta ][\Delta ][\Delta ][\Delta ][\Delta ][\Delta ][\Delta ]] = j \\ z_{w}z & z_{w}z \end{bmatrix}$$
(3-322)

Substitution back into Equation 3-319 gives

$$\{z \, [A] [A] [\Gamma]' [E] \neq F \} = \frac{\lambda}{2\pi \hbar^2} \neq F \}$$
(3-323)

where

$$[z] = -i_{1,1} + [[\Lambda]] + [\Lambda] $

or

Equation 3-323 can be solved by the same procedure used for Equation 3-269 in the case of level flight. Curiously enough, the results for the two cases are not significantly different. This bears out the fact that the approximation of artificially constraining the true center of mass of the missile does not introduce serious error. Again, for data corresponding to the NASA Scout, Equation 3-323 gives

$$\sum_{k=0}^{\infty} |u_{k}|^{2} = 13.986 \ \beta_{k} = 1/m^{2} , \qquad (3-325)$$

for the curved flight case; whereas, Equation 3-269, for the rectilinear flight case, gave

$${}^{1}_{2} {}_$$

The curved flight case was calculated at an altitude corresponding to:

$$c_{\sigma} = 1.2 + 1x 0^{-5} lb_{\mu} / m^{3}$$
(3-327)

There is some danger in generalizing the conclusions drawn here for the Scout vehicle, since they are based solely on the evidence of numerical results. Also, the curved flight effects are relatively easy to incorporate, particularly when a model method is used as explained in the next paragraph.

#### 3.1.3.1.2.2 Modal Method

The complete model equations of motion can be obtained by applying Lagrange's equations to Equations 3-272, 3-273, and 3-274 with the result that

$$M_{i}^{2} + \frac{1}{2} e^{i \frac{1}{6}} \left( C_{k} \left( \frac{1}{7_{0}} - \varphi \right) + C_{k} \frac{1}{7_{0}} \right)$$

$$- \frac{1}{2} e^{i \frac{1}{6}} \left( A_{k}^{2} + \frac{1}{2} \frac{1}{2} + \frac{1}{6} \frac{1}{2} + \frac{1}{6} A_{k}^{2} + \frac{1}{6} \frac{1}{2} + \frac{1}{6} + \frac{1}$$

$$I\vec{\theta} = \frac{1}{\lambda^{2}} i \vec{\theta} = \frac{1}{\gamma_{z}} (\lambda_{z} - \theta) + C_{M_{\theta}} \frac{\dot{\theta}}{\gamma_{z}}$$

$$= \frac{1}{\lambda^{2}} i \vec{\theta} = \frac{1}{\lambda^{2}} (\lambda_{z} - \lambda_{z}) [\lambda_{z}] (\lambda_{z} - \lambda_{z}) [\lambda_{z}] (\lambda_{z}) (\lambda_{z}) = 0$$
(3-329)

$$\frac{1}{2} + \frac{1}{2} + \frac{1}$$

The circular flight conditions are

$$\vec{r} = \frac{i\pi}{\beta}$$
, a constant (3-331)

$$\frac{1}{2} = \frac{1}{2}$$
, a constant (3-332)

$$-it = -it = a \text{ constant}$$
(3-333)

which implies, as before,

$$\frac{2}{y_1} - \varphi = \alpha$$
, a constant (3-334)

$$-\frac{1}{2} = -\frac{1}{2} = -\frac{1}{2}$$
 (3-335)

$$\ddot{z} = z \tag{3-336}$$

Introducing these equations into Equations 3-325, 3-329, and 3-330, we obtain

$$\frac{1}{2} = \frac{1}{2} \frac{$$

$$\frac{z^{1/2}}{2} \lim_{R \to \infty} R + \lim_{R \to \infty} \frac{1}{R} + \frac{1}{2} - x \frac{1}{2} [\Lambda] [z] \tau x \frac{1}{2} = 0 \qquad (3-338)$$

$$1 + \frac{32}{2} = -5$$
 (3-339)

If we solve the lift and moment equations for  $\sigma$  and 1/R and substitute into the elastic equations, we obtain

.

•

As a matter of practical computation, Equation 3-340 can be written as

$$- \left[ \chi_{1} \right] \left[ \left[ C_{\frac{2}{3}\frac{1}{4}}^{R} \right] - \left[ \frac{1}{2} C_{\frac{2}{3}\frac{1}{4}}^{R} \right] \left[ \frac{1}{2} C_{\frac{2}{3}\frac{1}{4}}^{R} + \frac{1}{2} \frac{1}{2} C_{\frac{2}{3}\frac{1}{4}}^{R} \right] \left[ \frac{1}{2} C_{\frac{2}{3}\frac{1}{4}}^{$$

where the  $C^{\rm R*}s$  and  $C^{\rm I*}s$  are elements from the matrices

.

· .

$$\begin{bmatrix} \pm 1 \mathbf{k} \\ \pm \mathbf{k} \end{bmatrix} \begin{bmatrix} \Delta \end{bmatrix} \begin{bmatrix} \pm 1 \mathbf{k} \\ \pm \mathbf{k} \\ \pm \mathbf{k} \end{bmatrix} = \begin{bmatrix} \pm \frac{\mathbf{k}}{22} \\ \pm \frac{\mathbf{k}}{22} \\ \pm \frac{\mathbf{k}}{22} \end{bmatrix}$$

$$\begin{bmatrix} \pm \frac{\mathbf{k}}{22} \\ \pm \frac{\mathbf{k}}{22} \end{bmatrix}$$

.

The results of the solution of Equation 3-341 and comparison with solution of Equation 3-323 are shown by Figure 43.



FIGURE 43 DYNAMIC PRESSURE OF DIVERGENCE AS A FUNCTION OF THE NUMBER OF MODES CONSIDERED – CIRCULAR FLIGHT CASE

To summarize the results of Paragraph 3.1.3.1, the following table is given

	VEHICLE CONSTRAINED TO RECTILINEAR FLIGHT (lb <sub>E</sub> /in <sup>2</sup> )	VEHICLE IN CIRCULAR FLIGHT (lb <sub>F</sub> /in <sup>2</sup> )
MODAL METHOD		
one elastic mode two elastic modes five elastic modes six elastic modes	17.322977 15.744623 14.568644 14.157779	17.428000 15.857967 14.673300 14.259390
COLLOCATION METHOD	13.886316	13.986051

TABLE 8 DYNAMIC PRESSURE OF DIVERGENCE

### 3.1.3.2 Dynamic Stability with Locked Controls

In the dynamic case, we are interested in solving the equation (see Equation 3-232),

$$\left(s^{T}[A] + s \underbrace{e_{T}}_{\Sigma}[A] + [K] + \underbrace{e_{T}}_{\Sigma}[A][\Delta]\right) \underbrace{f}_{\overline{p}} \underbrace{f}_{\overline{r}} = f \underbrace{s}_{\overline{r}}$$
(3-3<sup>4</sup>4<sup>4</sup>)

We want to prove, first, that this equation has a repeated zero root. To do this, let us transform to a complete set of normal coordinates by introducting the square model matrix.

or

$$\{\bar{\varphi}\} = [\varphi]\{\bar{q}\}$$
(3-346)

(The notation here departs slightly from Paragraphs 3.1.1 and 3.1.2 in that  $[\phi]$  includes the rigid-body modes.) In Equation 3-346,  $[\phi]$  is an N x N matrix of all the eigenvectors of the equation

$$\int z^{z} [A] + [K] \frac{1}{2} \dot{z} \dot{z} = \frac{1}{2} o \dot{z}$$
(3-347)

If we substitute Equation 3-346 into Equation 3-344 and premultiply by  $[\phi]^{\prime}$ , we obtain

$$\mathcal{L}^{2} = \mathcal{L}_{\mathbf{M}} + \mathcal{L}_{\mathbf{X}} \frac{\partial \mathcal{L}_{\mathbf{X}}}{\partial \mathbf{X}} [\mathcal{C}_{\mathbf{X}}] + [\mathbf{F}_{\mathbf{J}} + \frac{\partial \mathcal{L}_{\mathbf{X}}}{\partial \mathbf{X}} [\mathcal{C}_{\mathbf{F}}] + \frac{\partial}{\partial \mathbf{X}} \frac{\partial}{\partial \mathbf{X}} \frac{\partial}{\partial \mathbf{X}} - \frac{\partial}{\partial \mathbf{X}} - \frac{\partial}{\partial \mathbf{X}} \frac{\partial}{\partial \mathbf{X}} - \frac{\partial}{\partial \mathbf{X} - \frac{\partial}{\partial \mathbf{X}} - \frac{\partial}{\partial \mathbf{X} - \frac{\partial}{\partial \mathbf{X}} - \frac{\partial}{\partial \mathbf{X}$$

where

$$[\forall \mathbf{I}'[\mathbf{A}][\boldsymbol{\psi}] = [\mathbf{M}] \qquad (3-342)$$

$$[\varphi]'[\kappa][\varphi] = \lceil \varepsilon_{\perp} \qquad (3-350)$$

$$|\varphi['[\Lambda]][\Delta][\varphi] = [\mathbb{I}_{R}] \qquad (3-35L)$$

$$[\psi_1]'[\Lambda][\varphi] = [C_{\pi}]$$
 (3-352)

It can be shown that the roots to Equation 3-233 are inveriant under this transformation, so that

$$\mathbb{A}(s, \underline{z}_{l}, w_{\alpha}) = \left| z^{z} [w_{l\perp} + z \frac{w_{\alpha}}{z} [c_{z}] + [f_{\perp} + \frac{w_{\alpha}}{z} [c_{R}] \right| = c \qquad (3-353)$$

where, from previous results, we have

.

$$\mathbf{F}_{\mathbf{M}^{*}\mathbf{I}} = \begin{bmatrix} \mathbf{W} & \mathbf{C} & +\mathbf{C}_{\mathbf{I}}^{*} \\ \mathbf{C} & \mathbf{I} & +\mathbf{C}_{\mathbf{I}}^{*} \\ \mathbf{f}_{\mathbf{C}}^{*}\mathbf{I} & +\mathbf{C}_{\mathbf{I}}^{*} \end{bmatrix}$$
 (3-354)

$$[r] = \begin{bmatrix} c & c & to \\ c & to \\ c & to \\ rch(o) & r_{1} \end{bmatrix}$$
(3-355)

$$[C_R] = \begin{bmatrix} z - a_R & ic_{R}^R i \\ z - a_R & ic_{R}^R i \\ iz + ic_{R}^R i & [c_{R}^R] \end{bmatrix}$$
(3-356)

Reference to Equations 3-116 and 3-117 will indicate that

$$ic_{qe}^{R} = -ic_{gr}^{T}$$
 (3-358)

a fact we will want to refer to presently. Equation 3-353 then has the form

$$\Delta(s, \frac{1}{2}, \underline{\rho}, \underline{v}_{\underline{\alpha}}^{\mathrm{E}}) = \left| \begin{array}{ccc} \mathsf{M}s^{\mathrm{E}} + s, \underline{\rho}_{\underline{\alpha}}, \underline{v}_{\underline{\alpha}} \in \mathcal{L}_{\underline{\alpha}}, & s\underline{f}_{\underline{\alpha}}, \underline{v}_{\underline{\alpha}} \in \mathcal{L}_{\underline{\alpha}}, & s\underline{f}_{\underline{\alpha}}, \underline{v}_{\underline{\alpha}} \in \mathcal{L}_{\underline{\alpha}}, \\ \overline{\mathcal{L}}(s, \frac{1}{2}, \underline{\rho}, \underline{v}_{\underline{\alpha}}^{\mathrm{E}}) = \left| \begin{array}{ccc} \mathsf{M}s^{\mathrm{E}} + s, \underline{\rho}_{\underline{\alpha}}, \underline{v}_{\underline{\alpha}} \in \mathcal{L}_{\underline{\alpha}}, & \underline{\sigma}_{\underline{\alpha}}, \underline{v}_{\underline{\alpha}} \in \mathcal{L}_{\underline{\alpha}}, \\ \overline{\mathcal{L}}(s, \frac{1}{2}, \underline{\rho}, \underline{v}_{\underline{\alpha}}^{\mathrm{E}}) = \mathcal{L}_{\underline{\alpha}}, \\ \overline{\mathcal{L}}(s, \frac{1}{2}, \underline{\rho}, \underline{v}_{\underline{\alpha}}^{\mathrm{E}}) = \mathcal{L}_{\underline{\alpha}}, & \underline{s}, $

Inspection will show that s appears as a common factor in the first column of this determinant. If we factor  $s/v_{co}$  out of the determinant and then add the first column to the second column and make note of Equation 3-358, we obtain

$$\Delta(s, \frac{1}{2}, \underline{P}, v_{\underline{u}}^{c}) = \frac{s}{v_{\underline{u}}} \left[ v_{\underline{u}} s M' + \frac{P_{\underline{u}}, v_{\underline{u}}^{b}}{2} C_{\underline{u}} + \frac{s}{2} \frac{P_{\underline{u}}, v_{\underline{u}}}{2} C_{\underline{u}} + v_{\underline{u}} s M - \frac{s}{2} \frac{P_{\underline{u}}, v_{\underline{u}}}{2} \left\{ C_{\underline{x}\underline{x}}^{T} \right\}' + \frac{P_{\underline{u}}, v_{\underline{u}}^{b}}{2} \left\{ C_{\underline{x}\underline{x}}^{T} \right\}' - \frac{P_{\underline{u}}, v_{\underline{u}}}{2} \left\{$$

Inspection now shows that s is a common factor in the second column. Factoring out s/ $\gamma_{DD}$  again, we finally obtain

$$\Delta(s, \frac{L}{2} P_{\omega}, v_{\omega}^{L}) = \left(\frac{s}{\sqrt{\omega}}\right)^{2} \left| v_{\omega,s} M + \frac{P_{\omega} v_{\omega}^{L}}{2} c_{L_{d}} - v_{\omega}^{L} M + \frac{P_{\omega} v_{\omega}^{L}}{2} c_{L_{d}}^{L} - s \frac{P_{\omega} v_{\omega}}{2} \left\{ c_{33}^{L} \right] + \frac{P_{\omega} v_{\omega}^{L}}{2} \left\{ c_{33}^{R} \right] - \frac{P_{\omega} v_{\omega}}{2} \left\{ c_{33}^{R} \right\} - \frac{P_{\omega} v_{\omega}} - \frac{P_{\omega} v_{\omega}}{2} \left\{ c_{33}^{R} \right\} - \frac{P_{\omega} v_{\omega}} - \frac$$

The remaining determinant is still a polynomial in s, now of order 2N-2. We have then shown that there is, in general, a repeated zero root to the dynamic stability determinant. The remaining polynomial governs the short period and elastic roots. In the case of the rigid missile, we have

$$\frac{\Delta(\overline{s},\underline{z}_{20}v_{\omega}^{2})}{(\underline{s}_{10})^{2}} = \begin{vmatrix} M_{1}v_{\omega}s + \frac{\hbar}{2}v_{\omega}^{2} C_{L_{\infty}} & M_{1}v_{\omega}^{2} + \frac{\hbar}{2}v_{\omega}^{2} C_{L_{\infty}} \\ \frac{\hbar}{2}v_{\omega}^{2}}{(\underline{s}_{10})^{2}} = 0 \end{cases} (3-362)$$

which is nothing more than the conventional "short-period quadratic,"

•

$$\left(S + \frac{\partial \underline{w} Y \underline{w}}{2M} C_{L_{\underline{w}}}\right) \leq + \frac{\partial \underline{w} Y \underline{w}}{2L} C_{\underline{M}_{\underline{w}}} - \frac{\partial \underline{w} Y \underline{w}}{2L} C_{\underline{M}_{\underline{w}}} + \frac{\partial \underline{w}}{2L} C_{\underline{L}_{\underline{w}}} = 0$$
(3-363)

In the case of the rigid missile, "static" stability (i.e., s = 0) is given by

$$L_{M_{\mathcal{X}}} + \frac{\partial \omega}{\partial M} \left( C_{M_{\mathcal{X}}} C_{L_{\Theta}} - C_{L_{\mathcal{X}}} C_{M_{\Theta}} \right) = O \qquad (3-364)$$

The second term is usually small and the criterion for static stability of a rigid body is taken as

$$C_{M_{\chi}} < 0$$
 (3-365)

For the case of an elastic body, the natural generalization for the criteria of static stability is to take

$$\lim_{s \to 0} \frac{\Delta(s, \frac{1}{2}\rho_0 v_0^2)}{\left(\frac{s}{2}\rho_0\right)^2} = 0$$
(3-366)

as an equation governing the dynamic pressure of marginal static stability. From Equation 3-361, this is the condition

$$\begin{array}{c|c} C_{\mu} & \frac{IM}{\ell c} + C_{\mu} & \frac{1}{\ell c} R_{\mu}^{R} f \\ \hline C_{MR} & C_{Me} & \frac{1}{\ell c} C_{eq}^{R} f \\ \hline C_{MR} & C_{Me} & \frac{1}{\ell c} C_{eq}^{R} f \\ \hline C_{fq}^{T} f & \frac{1}{\ell c} C_{eq}^{T} f & \frac{1}{2 \pi V_{eq}} \Gamma_{\lambda_{1}} + \left[ C_{eq}^{R} f \right] \end{array}$$

$$(3-367)$$

The lowest dynamic pressure for which this determinant is zero is exactly the same as the dynamic pressure of divergence in the <u>curved flight</u> case considered in Paragraph 3.1.3.1.2. This is easily shown by observing that Equation 3-367 is the determinant of the matrix of coefficients in Equations 3-337, 3-338, and 3-339.

Using the above results, we may draw some general conclusions about the complete dynamic stability determinant,

$$\sum_{\lambda \in \frac{1}{2}} e^{y_{\alpha}^{\lambda}} = \left| \frac{1}{2} [\Lambda] + \frac{1}{2} \frac{y_{\alpha}}{2} [\Lambda] + [\kappa] + \frac{1}{2} \frac{y_{\alpha}}{2} [\Lambda] [\Lambda] \right| = 0$$
(3-368)

1. This is a 2N<sup>th</sup> order polynomial in s with real coefficients having a repeated zero root for any dynamic pressure.

2. The remainder of the roots vary with dynamic pressure in such a way that at least one root passes into the unstable part of the "root-plane" at the dynamic pressure of circular-flight divergence.

It could be proved, although it seems fairly evident, that the root which passes into the unstable part of the plane is the "short-period root" for the flexible missile.

The results of solving the stability determinant for a parametric variation of the dynamic pressure is shown by Figure 44. These results were obtained using the modal approximation and solving Equation 3-353 (instead of Equation 3-368) expressed in terms of only six elastic modes. (This equation was first expanded into a polynomial and then solved by appropriate numerical techniques.)



## FIGURE 44 LOCUS OF MISSILE STABILITY ROOTS FOR VARYING DYNAMIC PRESSURE

The critical value of dynamic pressure is indicated better if the real and imaginary parts of the short period root are plotted versus the dynamic pressure as shown by Figure 45.



FIGURE 45 DAMPING AND FREQUENCY OF THE SHORT PERIOD MODE

The estimated point where  $\sigma = 0$  in Figure 45 should agree with the six-elastic mode approximation for the circular-flight case. Reference to Table 8 gives

$$\sin^2 = 14.26 \ln / \ln^2$$
 (3-369)

while from Figure 45, we have

$$z_{r} = 14.4 \text{ lb} / \text{in}^2$$
 (3-370)

The difference results from the error produced by expanding the determinant in Equation 3-353.

The stability roots may be determined more accurately by a method which does not require the expansion of a determinant. Also, in some applications, the eigenvectors, as well as the eigenvalues, are required.

Because the locked-control eigenvectors can be used in the analysis of a missile with active control, we want to consider, in the next paragraph, a method of solving the eigenvalue problem associated with Equation 3-353. We will consider then

$$(3-371) = \frac{1}{2} \left[ 2 - \frac{1}{2} \right] - \frac{1}{2} \left[ 2 - \frac{1}{2} \right] = \frac{1}{2} \left[ 2 - \frac{1}{2} \left[ 2 - \frac{1}{2} \right] = \frac{1}{2} \left[ 2 - \frac{1}{2} \left[ 2 - \frac{1}{2} \right] = \frac{1}{2} \left[ 2 - \frac{1}{2} \left[ 2 - \frac{1}{2} \right] = \frac{1}{2} \left[ 2 - \frac{1}{2} \left[ 2 - \frac{1}{2} \right] = \frac{1}{2} \left[ 2 - \frac{1}{2} \left[ 2 - \frac{1}{2} \right] =$$

This problem is similar to that considered in Paragraph 2.2.3.5. In particular, Equation 3-371 should be compared with Equation 2-319. The only essential difference is that in Equation 3-371 the matrices are not all symmetric. We have already shown that there is a repeated zero root to this problem; it will be important to show that there is only one independent eigenvector corresponding to this zero root.

### 3.1.3.3 Solution of the Eigenvalue Problem for the Aeroelastic System

As in Paragraph 2.2.3.5, we will transform Equation 3-371 to a set of first-order equations. Consider the differential equations

$$[u]_{\{ij\}}^{N\times F} = \frac{1}{2} [C_{I}]_{\{ij\}} + \frac{f(F)_{i}}{2} [C_{R}]_{\{ij\}}^{i} = \frac{1}{2} [C_{R}]_{\{ij$$

Let us introduce

$$\{:\} = \{i\}$$
 (3-373)

Then we can write Equation 3-372 as

$$[v_{1}]_{1}^{2} + \frac{2}{2} \left[ \sum_{k=1}^{n} \frac{1}{2} + \left\{ [F_{k}] + \frac{2}{2} \sum_{k=1}^{n} \left[ C_{R} \right] + \frac{2}{3} \right\} = -\frac{1}{3}$$
(3-374)

or

where

$$[x] = [M] [0] [1] [1] [3-376]$$

and

$$[w] = \begin{bmatrix} \frac{1}{2} p \begin{bmatrix} 0 \\ 0 \end{bmatrix} \\ -\frac{1}{2} \end{bmatrix} \begin{bmatrix} p \end{bmatrix} + \frac{1}{2} \frac{1}{2} p \begin{bmatrix} 0 \\ 0 \end{bmatrix} \end{bmatrix}$$
(3-377)

The Laplace transform of the homogeneous equations is

$$\left( s\left[v\right] + \left[w\right] \right) \begin{bmatrix} iz_{j} \\ iz_{j} \end{bmatrix} = \frac{1}{2} o_{j}^{2}$$
 (3-378)

The stability determinant,  $\Delta = 0$  , in Equation 3-353 can also be written as

$$\Delta^{\prime}s, \, \underline{b}_{\beta}^{\prime}w_{\alpha}^{2} = \left| s\left[ \gamma \right] + \left[ \gamma \gamma \right] \right| = 0 \tag{3-379}$$

which has 2N roots, two of which are zero. We will suppose that they are arranged in the following order

$$s = 0, 0, s_2, s_3, \dots, s_N, \bar{s}_2, \bar{s}_3, \dots, s_N$$
 (3-380)

where  $s_i$ , i = 2,3...N, are complex roots with the conjugate denoted by  $\tilde{s}_i$ . The eigenvectors corresponding to these roots are defined by

$$(s_i[v] + [w]) \{ \phi_i^* \}_i = \{c\}$$
 (3-381)

$$(s_i[v] + [w]') + y^* + i = \{o\}$$
 (3-382)

Corresponding to the zero root, we have

$$[w] \exists v^* f_c = \{ o \}$$
 (3-383)

$$[N]'i\gamma^{*}j_{2} = \{j_{1}\}, \qquad (3-384)$$

It can be shown that [W] is only simply degenerate<sup>1</sup> provided  $\frac{1}{2} \rho_{\omega} \sqrt{\omega} \neq$  the divergence dynamic pressure. Thus, there is only one independent eigenvector corresponding to the repeated zero root. We can, however, introduce a pseudo zero-root eigenvector defined by

<sup>&</sup>lt;sup>1</sup>See Frazer, Duncan, and Collar, <u>Elementary Matrices</u>, Cambridge Univ. Press, 1950, for a definition of simple degeneracy.
$$[w]\{\varphi^*\}_{i} = \{\varphi^*\}_{j}$$
(3-385)

.

$$[w]' \{\gamma^*\}_{I} = \{\gamma^*\}_{0}$$
(3-386)

The following orthogonality conditions can be derived from Equations 3-381, 3-382, 3-383, 3-384, 3-385, and 3-386.

a

$$\{\gamma^{i}\}_{i}^{\prime}[V]\{\gamma^{i}\}_{j}^{\prime}=0 \qquad (3-387)$$

$$\{\gamma^{*}\}_{1} [w] \{\varphi^{*}\}_{j} = 0$$
 (3-388)

for 
$$i = j$$
  $i = 2, 3...N$   
 $j = 2, 3...N$   
 $\{j_{0}^{\dagger}\}_{0}^{\prime}[W] \\ i \\ \xi_{0}^{\dagger}\}_{0}^{\dagger} = j$  (3-389)

.

$$\frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} = 0$$
(3-390)

$$= j^{*} j_{0}^{*} [w]^{*} j_{0}^{*} = 0$$
 (3-391)

$$\{\varphi^*\}'_1[w]'\{\gamma^*\}_0 = 0$$
 (3-392)

We shall also show, below, that, in this problem

$$\frac{1}{2} \left[ \left[ V \right] \right] \left[ \frac{1}{2} \left[ V \right] \right] = 0$$
 (3-393)

The eigenvectors of the original second-order system are

$$\left( \begin{array}{c} s_{i}^{2} \left[ M \right] + s_{i} c_{I}^{2} \left[ c_{I} \right] + \left[ F \right] + \frac{c_{I} s_{I}^{2}}{I} \left[ c_{R} \right] \right) \left[ i \right] \left[ i \right] \left[ c_{I} \right] \right] \left[ i \right] \left[ c_{I} \left[ c_{I} \right] \left[ c_{I} \left[ c_{I} \right] \left[ c_{I} \left[ c_{I} \left[ c_{I} \right] \left[ c_{I} \left$$

$$\left\{s_{1}^{1}[M] + s_{2}\left(\frac{w^{\gamma_{m}}}{L}[C_{L}] + [F] + \frac{w^{\gamma_{m}}}{L}[C_{R}]^{\prime}\right) \neq \gamma_{1}^{2} = \{s\}$$
(3-395)

$$\left( [F] + \frac{\partial w_0}{2} [C_R] \right) \{ \psi \}_{j} = \{ o \}$$
(3-396)

Comparison of these equations with Equations 3-381, 3-382, 3-383, and 3-384, using Equations 3-376 and 3-377, will give the following relation between the eigenvectors of the two systems of equations.

$$\left\{\varphi^{*}\right\}_{i} = \left\{\begin{array}{c}s_{i}\left\{\varphi\right\}_{i}\\i\varphi\right\}_{i}\\i\varphi\right\}_{i}\right\}$$
(3-398)

$$\frac{1}{2} \psi^{+}_{i} = \begin{bmatrix} i \psi_{i} \\ \vdots \\ \vdots \end{bmatrix} \begin{bmatrix} F_{i} + \frac{1}{2} \psi_{i}^{*} \\ \vdots \\ \vdots \end{bmatrix} (3-399)$$

for i = 2, 3... N

$$\left\{ \begin{array}{l} \left\{ \gamma^{*} \right\}_{b} = \left[ \begin{array}{l} \left\{ \gamma \right\}_{3} \\ \left[ \begin{array}{c} \left\{ \alpha \right\}_{b} \right] \\ \left[ \left\{ c_{L} \right\}_{b} \right] \\ \left[$$

If we introduce pseudo eigenvectors for the original system, defined by

$$[F] - \frac{v_{\mathrm{r}}v_{\mathrm{r}}^{2}}{2}[C_{\mathrm{R}}] + \frac{v_{\mathrm{r}}v_{\mathrm{r}}}{2}[C_{\mathrm{r}}] + \frac{v_{\mathrm{r}}v_{\mathrm{r}}} + \frac{v_{\mathrm{r}}v_{\mathrm{r}}}{2}[C_{\mathrm{r}}] + \frac{v_{\mathrm{r}}v_{\mathrm{r}$$

$$\left[ F \right] + \frac{2\pi}{2} \left[ c_{R} \right]' + \frac{3\pi}{2} \left[ c_{R} \right]' + \frac{3\pi}{2} \left[ c_{L} \right]' \left\{ \eta \right\}_{0}$$
 (3-403)

then we can relate these to the pseudo eigenvectors defined in Equations 3-385 and 3-386.

$$(3-405)$$

and

.

.

We are then in a position to show that Equation 3-393 is true. From Equations 3-400, 3-401, and 3-376, we have

$$i\eta' \frac{\gamma'}{c} [V] \{ \varphi^{*} \}_{o} = \left[ \{ \eta \}_{o}' \quad \frac{\varphi_{2} v_{0}}{2} \{ \eta \}_{o}' [C_{T}] \right] \left[ [M] \{ o \} \\ \left\{ \varphi \}_{o} \right] = \frac{1}{2} \frac{V_{0}}{2} \{ \eta \}_{o}' [C_{T}] \{ \varphi \}_{o} \quad (3-406)$$

.

but from Equation 3-402,

$$\frac{2\pi}{2} \left\{ \gamma_{0}^{*} \left[ c_{I} \right]_{i}^{*} \left\{ i \right\}_{o}^{*} = \left\{ \gamma_{0}^{*} \right\}^{'} \left[ F \right]_{i}^{*} \left[ c_{I}^{*} \left[ c_{I}^{*} \right]$$

because of Equation 3-397.

.

.

Let us consider the nonhomogeneous equations,

$$\begin{bmatrix} s[v] + [w] \\ \{\bar{v}\} \end{bmatrix} = \begin{bmatrix} \bar{v}\bar{v} \end{bmatrix} \begin{bmatrix} v \\ \bar{v} \end{bmatrix}$$
(3-408)

and make the following transformation of coordinates

$$\begin{bmatrix} \left\{ \bar{x} \right\} \\ \left\{ \bar{q} \right\} \end{bmatrix} = \begin{bmatrix} \varphi^* \end{bmatrix} \{ \bar{q}^* \}$$
 (3-409)

where

.

$$[\psi^*] = \left[ \{\psi^*\}_0 \{\psi^*\}_1 \{\psi^*\}_2 \dots \{\psi^*\}_N \{\bar{\psi}^*\}_2 \dots \{\bar{\psi}^*\}_N \right]$$
 (3-410)

.

and then premultiply the equations by

$$[\gamma^*] = \left[\{\gamma^*\}, \{\gamma^*\}, \{\gamma^*\}, \{\gamma^*\}, \{\bar{\gamma}^*\}, \{\bar{\gamma}^$$

We then obtain

$$( {}^{5} [n^{*}]'[v][q^{*}] + [n^{*}][w][q^{*}] ) { \{ \bar{q} \} }^{*} = [n^{*}]' [ { \{ \bar{q} \} }^{*} ]$$

$$(3-412)$$

From the orthogonality conditions, we have

$$[v^{*}]'[w][q^{*}] = \begin{bmatrix} i\eta^{*}i_{1}^{*}[w]i_{1}^{*}i_{2}^{*}i_{1}^{*}[w]i_{1}^{*}i_{1}^{*}\\ i\eta^{*}i_{2}^{*}[w]i_{2}^{*}i_{2}^{*}[w]i_{1}^{*}i_{1}^{*}\\ i\eta^{*}i_{2}^{*}[w]i_{2}^{*}i_{2}^{*}[w]i_{1}^{*}i_{1}^{*}\\ i\eta^{*}i_{2}^{*}[w]i_{2}^{*}i_{2}^{*}[w]i_{2}^{*}i_{1}^{*}i_{1}^{*}]$$

$$(3-413)$$

$$[\chi^{*}][\chi][\chi^{*}] = \begin{bmatrix} \chi^{*}_{1}[\chi][\chi^{*}]_{0} & \chi^{*}_{1}[\chi][\chi^{*}]_{0} & \chi^{*}_{1}[\chi^{*}]_{0} & \chi^{*}_{1}[\chi^{*}$$

and from Equation 3-381

$$s_{i} \{ \psi_{j}^{\dagger} \}_{i}^{\prime} [v] \{ \psi_{j}^{\dagger} \}_{i}^{\prime} + \{ \psi_{j}^{\dagger} \}_{i}^{\prime} [w] \} \psi_{j}^{\dagger} \psi_{j}^{\dagger} \}_{i}^{\prime} = 0$$
 (3-415)

i = 2,3...N

Now it is possible to choose  $\{\eta^*\}_i$  , such that Equation 3-386 is satisfied and at the same time

$$\{\eta^{*}\}_{1}^{\ell}[\forall]\{\varphi^{*}\}_{1} = 0$$
 (3-416)

This is true because

$$\{z_1^{*}\}_{1} = u : z_1^{*}\}_{3}$$
(3-417)

satisfies Equation 3-386 for any value of  $\mu$  , and  $\mu$  can be chosen so that

$$\{\cdot^* \}' \cdot u_{-1} \{\cdot\}'_{i} = 0$$
 (3-418)

Then

$$(3-419)$$

satisfies both Equation 3-386 and Equation 3-416. We note also that from Equations 3-376 and 3-400

$$[x_{1},y^{*}]_{2} = [y^{*}]_{2} \qquad (3-420)$$

and hence, from Equation 3-385

$$2\eta^{*} \frac{1}{2} \left[ \sqrt{\frac{1}{2}} \frac{1}{2} $

We then have

$$= \left\{ \begin{array}{c} \sum_{i=1}^{n} \left[ \left[ \psi_{i} \right] \left[ \psi_{i} \right] \left[ \psi_{i} \right] \left[ \psi_{i} \right] \left[ \psi_{i}^{*} \right] \right] = \left\{ \sum_{i=1}^{n} \left[ \left[ \psi_{i} \right] \left[ \psi_{i}^{*} \left[ \psi_{i}^{*} \right] \left[ \psi_{i}^{*} \left[ \psi_{i}^{*} \right] \left[ \psi_{i}^{*} \right] \left[ \psi_{i}^{*} \right] \left[ \psi_{i}^{*} \left[ \psi_{i}^{*} \right] \left[ \psi_{i}^{*} \right] \left[ \psi_{i}^{*} \left[ \psi_{i}^{*} \left[ \psi_{i}^{*} \right] \left[ \psi_{i}^{*} \left[ \psi_{i}^{$$

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If we normalize the eigenvectors so that

$$E(f_{1}^{*}) \neq f_{5} = 1$$
 (3-423)

(3-424)

$$i_{\eta} F_{i} [V] i_{\eta} S_{i}^{=}$$
 (3-425)  
 $i = 2, 3 \dots N$ 

and use Equation 3-415, then

We then have, from Equation 3-1412,

$$-\frac{1}{2} = -\frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \right] \right] + \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \right] \right] + \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \right] + \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \right] + \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \right] + \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \right] + \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \right] + \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \right] + \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \right] + \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \right] + \frac{1}{2} \left[ \frac{1}{2$$

(3-427)

Substitution into Equation 3-409 gives

.

.

$$\begin{bmatrix} \overline{z} \\ \overline{z} \\ \overline{z} \\ \overline{z} \end{bmatrix} = \begin{bmatrix} \varphi^* \end{bmatrix} \begin{bmatrix} \frac{1}{2} & -\frac{1}{2^L} \\ \alpha & \frac{1}{2} \\ \vdots & -\frac{1}{2^{-L_{\overline{z}}}} \end{bmatrix} \begin{bmatrix} \gamma^* \end{bmatrix} \begin{bmatrix} \gamma^* \end{bmatrix} \begin{bmatrix} z \\ \overline{z} \end{bmatrix}$$
(3-428)

We can partition these equations, using the bottom balf of  $[\phi^*]$  and the top half of  $[\eta^*]$  to obtain

,

.

where, from Equations 3-398, 3-399, 3-400, 3-401, 3-404, and 3-405, we have

$$[1] = [+25, +25, -245, -252, -375, -275,$$

By expanding the indicated products, we have the following identity

$$\frac{1}{1-\frac{1}{2}} = \frac{1}{1-\frac{1}{2}} \frac{1}{1+\frac{1}{2}} \frac{1}{1+\frac{1$$

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In this expression we have

$$\sum_{i=z}^{n} \frac{\xi \psi f_{L} \xi \eta f_{L}}{s - s_{L}} + \frac{\xi \overline{\varphi} f_{L} \xi \overline{\eta} f_{L}}{s - \overline{s}_{L}} = \sum_{i=z}^{n} \frac{(s - \overline{s}_{L}) \xi \eta f_{L} \xi \eta f_{L}}{(s - s_{L}) (s - \overline{s}_{L})} \qquad (3 - 433)$$

If we let

$$s = 7\frac{1}{2} + 1\omega_{1}$$
 (3-434)

$$[1_{2}] = -i_{4} i_{5} i_{4} i_{5}$$
 (3-435)

$$[4, ]_{1} = -\frac{1}{4} \int_{2} \frac{1}{2} \frac{1}{4} \int_{-\infty}^{\infty} \frac{1}{4} \int_{-\infty}$$

$$[x_{2}]_{2} = -2 F_{2} = \frac{1}{2} (\frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2})$$
 (3-437)

$$[x_1]_{i} = z \mathcal{F}_2 + \mu \frac{1}{2} + \frac{1}{2} \frac{1}{3} \frac{1}{2}$$
 (3-438)

then

.

$$\{\tilde{j}_{j}\} = \frac{1}{2} \frac{1}{2} \frac{1}{2} - \frac{1}{2} \frac{1}{2} \frac{1}{2} + \sum_{t=2}^{N_{c}} \frac{1}{2^{t} - 2\tau_{c}^{2} + \tau_{c}^{2} + \tau_{c}^{2}} + \frac{1}{2\tau_{c}^{2}} + \frac{1}{2\tau_{c}^$$

Comparing this with Equation 3-371, we must conclude that

.

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$$\left(z^{L}[M] + z^{N}\underline{\omega}_{\alpha}^{T}[z^{L}] + [E] + \overline{\omega}_{\alpha}^{N}[z^{L}]\right) \qquad (3-4+0)$$

$$= \frac{z[\phi, I, +[\phi_{\alpha}]]}{z^{L}} + \sum_{\underline{i}=z}^{N} \frac{z^{L}}{z^{L}} \frac{z^{L}}{z^{T}} \frac{z^{L}}{z^{T$$

The  $[\psi]$  matrices in the above equation are all real matrices. We shall have occasion to use Equation 3-440 in the closed-loop stability analysis in the next section. Numerical methods for obtaining the eigenvectors are given in Appendix III of this report.

### 3.1.3.4 General "Point" Stability with Closed Control Loop

In the case where the control surface is active, the stability problem is far more involved than that considered in the previous section. When the control position is governed by information gained from sensors of the vehicle's attitude, the whole question of stability depends on the characteristics of the sensing elements and the power source for positioning the control mechanism. The equations governing the airframe, in this case, are given by Equation 3-106,

$$\begin{bmatrix} \mathbf{w} \\ \mathbf{w}$$

where use has been made of Equations 3-122 through 3-125.

The derivation of these equations in Paragraph 3.1.2 was based on the use of an aerodynamic surface control. The general form of these equations, however, is valid for most other important cases. The particular aspects of gimbaled engine control are considered in Paragraph 3.1.3.5.

The control moment,  $\Gamma$ , is derived from

$$S_{n}^{*} = S_{n}^{*} S_{n}^{*}$$

$$(3-442)$$

which is the virtual work of the forces exerted on the control by the control servo-mechanism.

The Leplace transform of Equation 3-441 is

$$\int_{-\frac{1}{2}}^{2} [M] + [F] + \frac{1}{2} \frac{1}{\sqrt{\alpha}} \sqrt{\frac{1}{\alpha}} \left[ \frac{1}{\sqrt{\alpha}} \left[ \frac{1}{\sqrt{\alpha}} \left[ \frac{1}{\sqrt{\alpha}} \right] - \frac{1}{\sqrt{\alpha}} \right] \right] = \begin{bmatrix} \frac{1}{\sqrt{\alpha}} \\ \frac{1}{\sqrt{2}} \\ \frac{1}{\sqrt{2}} \end{bmatrix} = \begin{bmatrix} \frac{1}{\sqrt{2}} \\ \frac{1}{\sqrt{2}} \\ \frac{1}{\sqrt{2}} \end{bmatrix} = \begin{bmatrix} \frac{1}{\sqrt{2}} \\ \frac{1}{\sqrt{2}} \\ \frac{1}{\sqrt{2}} \end{bmatrix}$$
(3-44-3)

. .

If we partition Equation 3-443 into sinframe equations and the single control equation, we have

$$\begin{bmatrix} 1 & 1 & 1 \\ 1 & 1 & 1 \end{bmatrix} \begin{bmatrix} 1 & 1 & 1 \\ 1$$

where

•

and the coefficients are

$$[1 - 1] = \frac{1}{2} \left[ \frac{1}{2} - \frac{1}{2} + \frac{1}{2} \right] = \frac{1}{2} \left[ \frac{1}{2}$$

$$f_{2i} = -f_{2i} + F_{2i} + F_{2i} + \frac{1}{2} + \frac{1}{2} \frac{1}{2} + \frac{1}{2} \frac{1}{2} \frac{1}{2} + \frac{1}{2} \frac$$

$$N_{20}(z_1) = J^2 N_{12}^2 + F_{22} + \frac{1}{2} \log J_2 + \frac{3}{20} \log_{12} + \frac{3}{20} \log$$

More explicitly, Equation 3-444 can be written as

$$(14_{1} \circ 1+7 \circ 1+7)_{2} \circ 1 = -0$$
 (3-451)

$$\frac{1}{2} \left( \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=$$

It may be noted that Equation 3-451 is expressed in terms of the locked-control coefficients considered in Faragraph 3-1-3-3. In fact, we may use the result of Equation 3-440 to write

where the  $[\psi]$ 's are obtained from solving the locked control eigenvalue problem as described in Paragraph 3.1.3.3. This formulation of the problem makes it feasible to solve Equation 3-451 for the airframe coordinates,

$$-3 = -3$$
,  $-3 = -3$ ,

Jubstituting this into the control equation, we obtain

$$h_{12} = -\{h_{2}, h_{2}, h_{3}\} = F$$
 (3-455)

If we introduce the "aero-inertia impedance" of the control,

$$N(s) = N_{22}(s) - \frac{1}{2}N_{21}(s) \frac{1}{2} [N_{11}(s)] \frac{1}{2} [N_{12}(s)] \frac{1}{2}$$

then we can write Equation 3-455 as

$$N(\hat{s}) \quad \overline{\varphi}(s) = \overline{\Gamma}(s) \qquad (3-457)^{2}$$

The moment from the control servo is usually governed by a mechanical or electrical signal which dictates a given control deflection, say,  $\epsilon$ . Because of the impendance the control mechanism faces, the signal deflection,  $\epsilon$ , is never equal to the actual control deflection,  $\gamma$ . A fairly general expression for the control moment developed is given by

$$\overline{\Gamma}(s) = -I(s) \left( \overline{\mathcal{J}}(s) - \overline{\mathcal{J}}(s) - \overline{\mathcal{J}}(s) \right)$$
 (3-458)

where I(s) and G(s) can be given "empirical" definitions which may be used to measure them experimentally. The "power control impedance" can be defined by

$$\Xi_{s} = -\left(\frac{\overline{P}(s)}{\overline{d}^{2}(s)}\right) = 0$$
 (3-459)

This is obtained experimentally by applying an oscillatory load on the control and measuring the response of the control with zero signal input to the servo. The "servo no-load impedance" can be defined by

$$G'_{5} = \left(\frac{\bar{\beta}_{(5)}}{\bar{\epsilon}_{(5)}}\right) = 0 \qquad (3-460)$$

which can be obtained from measurements of the response of the unloaded surface to oscillatory signals to the servo. In most cases, the control mechanism must be dismantled because its own inertia will load the servo at high frequencies. In many instances, theoretical expressions for G(s) and I(s) may be obtained from analysis of the servo<sup>1</sup>. A useful approximation for preliminary analysis is the assumption

$$(3-461)$$

Also, the power control impedance can usually be approximated by the impedance of an equivalent spring-damper system with undamped frequency,  $\omega_{y,x}$  and critical damping factor,  $\zeta_{y}$ 

$$z_{12} = z_{23}^{2} + z_{13}^{2} + z_{13}^$$

(J is the control hinge-line moment of inertia).

The complete control loop is closed when the signal to the zervo,  $\epsilon$ , is described in terms of the vehicle's attitude as seen by the gyros, and other sensing elements like angle-of-attack vanes and accelerometers. The sensing elements'estimate of the missile attitude can generally be expressed as

For example, a single displacement gyro at a point  $x = x_D$  on the missile would sense an attitude,  $\theta_p$ , given by

Using the interpolation formula (see Equation 3-2), this can be expressed in terms of collocation point displacements

where 
$$x_{i} \leq x_{D} \leq x_{i}$$

Expressions for G(s) and I(s) for an electrically energized hydraulic servo are given in <u>Aeroelastic Analyses of Multi-Stage Rocket Systems</u>, AGARD Report 390 July, 1961, equations A-65 and A-66.

Substituting

$$i = i F_{2} + i x_{-x} = + [\phi]_{1,x}$$
 (3-465)

we obtain

$$\begin{aligned} \mathcal{E}_{\mathbf{n}} &= \frac{\partial p_{\mathbf{n}}}{\partial \mathbf{x}} (\mathbf{x}_{\mathbf{n}}, \mathbf{t}) = -\mathcal{E} + \frac{1}{\mathbf{n}} \left\{ \mathcal{A} + \mathcal{I}_{\mathbf{n}_{\mathbf{n}}}^{2} \mathcal{I}_{\mathbf{n}_{\mathbf{n}}}^{2} \mathbf{I}_{\mathbf{n}_{\mathbf{n}}}^{2} \mathbf{I}_{\mathbf{n}}^{2} \mathbf{I}_{\mathbf{n}}^{$$

Also, outputs from several displacement end/or rate gyros might be filtered and combined to arrive at an estimate of the vehicle attitude so that, in general, T(s) in Equation 3-467 will depend upon the characteristics of all of the sensing elements and the shaping and filtering networks.

If  $\mathcal{V}(t)$  is the required program of the vehicle's attitude, then the Laplace transform of the instantaneous attitude error is

This error is monitored and used to command a control deflection,  $\epsilon$ , according to some control law which, in a fairly general form, can be expressed as

where K(s) is a gain function with the units of radians of control deflection per unit radian of attitude error. An example, representative of a rigid missile with a perfect gyro is,

$$z_{1} = z_{0} - y$$
 (3-469)

where  $K_{n}$  is a constant.

Equation 3-454 can be used to write the sensed attitude as

$$\Xi_{T} = -\Xi_{T} = -=$$

The function,

$$- z = -\frac{1}{2} = \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} = \frac{1}{2} + \frac{1}{2}$$

is usually termed the "sirframe transfer function." It gives the sensed attitude of the missile in terms of control deflection. By way of summary, we have the following equations

$$4 + \overline{1} = - \frac{1}{2}, \qquad (3-472)$$

$$\overline{\Gamma}_{\pm} = -\underline{1}_{\pm} \quad \overline{F}_{\pm} \quad (3-473)$$

The only functions that depend on the zeroelastic parameters of the missile are N(s) and R(s), and these are both independent of the more important parameters involved in the design of the control system. N(s) and R(s) may be calculated in terms of polynomials in s by using Equation 3-453.

 $= \frac{1}{2} + \frac{$ 

$$P(z) = -\frac{1}{2} \tau_{(z)} F' \left( \sum_{i=1}^{N} \frac{z(\psi_i)_i + (\psi_2)_i}{z^2 - z \sigma_i z + \sigma_i^2 + \omega_i^2} \right)^{2^2 + N_{12}} F^{-1} = 2F + 2\psi_0 v_0^2 + \psi_{i,2} F + \frac{3}{v_0} E^{-1} \sigma_{i,2} F' \right)$$
(3-476)

If we eliminate  $\bar{\mathcal{V}}$  and  $\bar{\Gamma}$  in Equations 3-472 and 3-473, we find

$$\overline{\mathcal{F}}_{-1} = -\frac{(1-2)}{2} = \frac{1}{2}$$
 (3-477)

Substituting this into Equation 3-474 gives

$$\Xi_{11} = - \varepsilon F_{12}, \frac{(2 - 1)}{1 - \frac{m_{22}}{r_{12}}} = \Xi_{21} - - - \varepsilon_{22}, \frac{1}{r_{22}}, \qquad (3-478)$$

or

$$E_{1} = E_{2} = E_{1} = E_{2} = E_{2} = E_{1} = E_{2} = E_{2} = E_{1} = E_{2} = E_{1} = E_{2} = E_{2$$

The stability of the system is governed by the equation

$$\left(x_{2}, y_{1}, z_{2}, -\frac{x_{2}}{1, z^{2}}, -y\right) = 2$$
 (3-480)

The three functions, K(s), G(s), and I(s), associated with the control system can usually be written as rational functions of s (i.e., as the ratio of two polynomials in s). It is also clear from Equations 3-475 and 3-476 that the two functions, N(s) and R(s), associated with the zeroelastic system can be written as rational functions of s. It is then possible, by multiplying and adding polynomials coefficients, to express Equation 3-480 as a single polynomial equation which may be solved for the stability roots of the whole system. The principal advantage of this technique is that a characteristic polynomial may be developed which has important control system gains appearing explicitly so that they may be varied without having to reconsider the whole zeroelastic system. It is characteristic of mass-balanced, aerodynamically-balanced control surfaces that

$$\frac{N(s)}{I(s)} \to 0 \tag{3-481}$$

so that Equation 3-480, in this case, reduces the the equation

$$E(s) G(s) R(s) - c = 0$$
 (3-482)

This is far from the actual fact in the case of a gimbaled engine, but for the aerodynamically controlled NASA Scout launch vehicle, it proves to be a valid approximation. The control equation

$$x_{12}, \bar{y} = \bar{z} = -ie(\bar{y} - E_{12})\bar{z}$$
 (3-483)

in this case, reduces to

$$\bar{x} = \frac{1}{2} + \frac{3}{2}$$
 (3-484)

In subsequent sections, this approximation is referred to as the perfect servo assumption."

#### 3.1.3.5 Gimbaled Engine Considerations in Closei Loop Stability

There are some important dynamic effects of gimbaled, thrusting engines which have not been considered in the previous sections. The most important of these effects is the loss in control effectiveness due to engine inertia at high control frequencies. The particular frequency where the control force is zero is commonly called the "dog-wags-tail" frequency.

In the discussion below, we will assume the vehicle in a vacuum with no external forces other than the thrust and the control force from the actuator.

Let  $f_{\sigma}(x,t)$  be the continuous displacement of the missile



FIGURE 46 SLENDER LAUNCH VEHICLE WITH GIMBALED ENGINE

The total kinetic energy of the system is

$$T = \frac{1}{2} \int_{0}^{1} \frac{d^2 t}{dt} = \frac{1}{2} \frac{1}{2} \frac{d^2 t}{dt} = \frac{1}{2} $

and the total strain energy is

 $L = \sum_{i=1}^{n} \frac{1}{2} \sum_{i=1}^{n} \frac{1}{2} x_{i} + \sum_{i=1}^{n} \frac{1}{2} \sum_{i=1}^{n} x_{i}$  (3-466)

where the axial resultant, M(x), is given by

$$x_{x}(x) = \int_{-\infty}^{\infty} \frac{1}{x^{2}} \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \frac{1}{x^{2}} \int_{-\infty}^{\infty} \frac{1}{x^{2}} \frac{1}{x^{2$$

The virtual work of external forces is

$$SN = -\left[Sp_{\pm} T(x) + \frac{G}{2x} + x + 5 + T\right] \qquad (3-486)$$

The gimbal angle  $\gamma$  is given by the jump in the slope at the gimbal,  $x=x_{\rm fl}$ 

$$\mathcal{Y} = \left(\frac{\partial \mathcal{P}_{z}}{\partial x}\right)_{x = x_{c}^{+}} - \left(\frac{\partial \mathcal{P}_{z}}{\partial x}\right)_{x = x_{c}^{-}}$$
(3-489)

We will suppose that the system can be approximated by one with a finite number of degrees-of-freedom as in the previous sections.

$$p_{\Xi}(x,t) = \{h_{\Xi}(x)\}^{T} \{\beta(t)\}$$
(3-490)

.

Expressed in terms of the generalized coordinates,  $\boldsymbol{p}_{\tilde{t}}$  , the kinetic energy is

$$T = \left\{ \left\{ \frac{1}{2} + \frac{1}$$

where

$$[A] = \int_{-1}^{1} [k_{E} x] \frac{1}{2} [h_{E} \hat{x}] \frac{1}{2} w^{2} dx \qquad (3-492)$$

The strain energy is

$$= \frac{1}{2} + \frac{1}{2} \sum_{k=1}^{2} \frac{1}{2k} \sum_{k=1}^$$

$$= \left[ \frac{1}{2} - \frac{1}{2} + $

216

where

Equation 3-489 becomes

$$\vec{r} = i \frac{4}{5} \vec{r} \cdot \vec{r}$$

where

$$\{ \{ \{ j \} \} = \{ \frac{dh_{\pi}}{dx} (x_{c}^{*}) \} - \{ \frac{dh_{\pi}}{dx} (x_{c}^{*}) \}$$
(3-497)

If the control moment,  $\Gamma$ , from the servo is zero, the system has three zero-frequency modes. One is a translation mode defined by

$$b_{z}(x,t) = i = \{b_{z}(x)\} \{ \psi_{z} \}$$
(3-498)

.

(In the case the generalized coordinates are collocation point displacements, we have

 $\{\sigma_{\mathbf{R}}\} = \{\mathbf{i}\}$ 

A second rigid-body node is defined by

$$p_{z}(x,t) = -x = -th_{z}(x) + th_{z}(t)$$
 (3-499)

A third zero-frequency mode is given by

$$p_{\Xi}(x,t) = \begin{cases} 0 & x < x_c \\ x - x_c & x > x_c \end{cases} = \frac{1}{2} \frac$$

which represents a unit deflection of the control.

An important simplification has resulted in previous sections because the rigid-body modes were mutually orthogonal. We can introduce a set of orthogonal rigid-body modes by taking

$$\{\varphi_{\mathcal{E}}\} = \{\varphi_{\mathcal{E}}\}$$
(3-501)

and

$$\{\varphi_{\theta}\} = \{\varphi_{R}\}_{2} + c \{\varphi_{R}\}_{1}$$
(3-502)

such that

$$i\varphi_{x} f[A] H \varphi_{0} f = 0$$
 (3-503)

This gives

$$c \{\varphi_{R}\}_{i} [A ] H \varphi_{R} P_{i} + i \varphi_{R} P_{i} [A ] H \varphi_{R} P_{R} = 0 \qquad (3-504)$$

or

$$c = - \frac{i\varphi_{R}F_{L}[A]F\varphi_{R}F_{2}}{i\varphi_{R}F_{L}[A]F\varphi_{R}F_{3}}$$
(3-505)

$$\{\varphi_{\mathbf{r}}\} = - \frac{\{\varphi_{\mathbf{r}}\}_{i}[A]\{\varphi_{\mathbf{r}}\}_{i}}{\{\varphi_{\mathbf{r}}\}_{i}[A]\{\varphi_{\mathbf{r}}\}_{i}} \{\varphi_{\mathbf{r}}\}_{i} + \{\varphi_{\mathbf{r}}\}_{i}$$
(3-506)

# An orthogonal control mode is constructed in a similar fashion. Take

$$\{\varphi_{F}\} = \{\varphi_{R}\}_{F} + c_{1}\{\varphi_{F}\} + c_{2}\{\varphi_{\theta}\}$$
(3-507)

and require

$$\{\varphi_{z}\}'[A]\{\varphi_{z}\} = 0$$
 (3+508)

$$\{\varphi_{\theta}\}'[A]\{\varphi_{\gamma}\} = 0$$
 (3-509)

This gives

$$z_{2} = 1_{1} + 1_{2} + 1_{2} + 1_{3} + 1_{4$$

$$\{ f_{2} \} = \{ \varphi_{\theta} \} = \{ \varphi_$$

where

$$\mu = \{\varphi_{J}\} [A] \{\varphi_{J}\}$$
(3-512)

.

$$I = \{\varphi_{\theta}\} [A\} \{\varphi_{\theta}\}$$
(3-513)

We then have

$$c_1 = -\frac{1}{M} \{ q_2 \}' (A H q_R F_2$$
 (3-514)

$$c_2 = -\frac{1}{2} \{ \varphi_0 \}' [A ] H \varphi_0 \}_3$$
 (3-515)

Substitution into Equation 3-507 gives

$$\begin{aligned} \vdots_{j_{1}} \vdots &= i \gamma_{P} f_{3} - \frac{1}{M} \left[ i \varphi_{S} f'[A H \varphi_{R} f_{3}] + i \varphi_{S} \right] \\ &- \frac{1}{T} i \varphi_{S} f'[A H \varphi_{R} f_{3} + i \varphi_{S} f] \end{aligned} (3-516) \\ &= i f_{1} - \frac{1}{M} i \varphi_{S} f'[A] \pm i \gamma_{S} f \varphi_{S} f'[A] \right]^{2} \varphi_{R} f_{3} \\ &= [P] \left[ i f_{R} f_{3} \right] \end{aligned}$$

The virtual work of external forces is

$$sw = -fspf' \int_{a}^{b} fh_{z} f \tau_{x} f \frac{dh_{z}}{dx} f' dx fpf + sfr'' = -fspf' [H]fpf + sfr'' (3-517)$$

where

$$[H] = \int_{0}^{L} \{h_{z}(x)\} T(x) \{ \frac{dh_{z}}{dx}(x)\}' dx \qquad (3-518)$$

To derive the elastic modes, we impose the constraint  $\gamma = 0$ . From Lagrange's equations we obtain

$$[r''[E][\Gamma'][A H \varphi] = \lambda \{\varphi\}$$
(3-519)

where [E] is an influence matrix for the missile with locked controls and

$$[r] = r_{\perp} - \frac{1}{M} \{ \varphi_{\perp} H \varphi_{\perp} \}' - \frac{1}{L} \{ \varphi_{\Theta} H \varphi_{\Theta} L'$$
(3-520)

We then make the following transformation of coordinates

$$\{p\} = \{q_{r}\} + \{q_{e}\} + \{\varphi\} + \{\varphi\} + \{\varphi_{e}\} + \{\varphi_{e}$$

where  $[\phi]$  is the matrix of locked-control elastic modes. By this transformation we obtain

$$T = \frac{1}{2} \begin{bmatrix} \hat{J} \div \hat{L} \hat{q} \hat{J}' \hat{J} \end{bmatrix} \begin{bmatrix} M \end{bmatrix} \begin{bmatrix} \hat{J} \\ \hat{\sigma} \\ \hat{L} \hat{q} \hat{J} \\ \hat{J} \end{bmatrix}$$
(3-522)

$$U = \frac{1}{2} \begin{bmatrix} s & q \\ r \end{bmatrix} \begin{bmatrix} r \\ 0 \\ q \\ r \end{bmatrix}$$
(3-523)

$$S_{\mathcal{H}} = -\left[ c_{\Gamma} c_{\Theta} f_{\sigma} c_{\varphi} f_{\sigma}^{T} c_{\varphi} \right] \left[ \bigcup_{\substack{\Theta \\ \{q,l\}\\ \mathcal{F}}} \right] + \frac{\delta \mathcal{F}}{\delta} f_{\sigma}^{T}$$

$$(3-52^{l_{\varphi}})$$

where the axial load part of the strain energy has been included in [U]. In these expressions we have

$$\begin{bmatrix} M \end{bmatrix} = \begin{bmatrix} M & 0 \\ 0 & L \\ & \Gamma_{1} & [\varphi][A][\varphi_{2}] \end{bmatrix}$$
(3-525)  
$$\{q_{2}\}[A][\varphi] = J$$

where

-

.

$$J = \{\varphi_{\mathcal{F}}\} \{A \mid \{\varphi_{\mathcal{F}}\}$$
(3-526)

$$\begin{bmatrix} F \end{bmatrix} = \begin{bmatrix} a & \sigma & & \\ \sigma & \sigma & & \\ & & \Gamma_{\lambda_{j}} & \\ & & & \sigma \end{bmatrix}$$
(3-527)

.

$$\begin{bmatrix} [u] = \begin{bmatrix} 0 & 0 & fof & 0 \\ c & fq_0 F[NKq_0 F & fq_0 F[NKq_1 & fq_0 F[NKq_2 F \\ fof [q]'[NKq_0 F & [q]'[NKq_1 & [q]'[NKq_2 F \\ ] & fof [q]'[NKq_0 F & [q]'[NKq_1 & [q]'[NKq_2 F \\ ] & fq_2 F[NKq_0 F & [q]'[NKq_1 & [q]'[NKq_2 F \\ ] & fq_2 F[NKq_0 F & fq_2 F[NKq_1 & [q]'[NKq_2 F \\ ] & fq_2 F[NKq_0 F & fq_2 F[NKq_1 & [q]'[KKq_2 F \\ ] & fq_2 F[NKq_0 F & fq_2 F[NKq_1 & [q]'[KKq_2 F \\ ] & fq_2 F[NKq_0 F & fq_0 F[NKq_1 & [q]'[KKq_2 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F & [q]'[KKq_2 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F \\ ] & fq_2 F[NKq_0 F & [q]'[KKq_0 F \\ ] & fq_2 F[NKq_0 F \\ ] & fq_2 F[NKqq_0 F \\ ] & fq_2 F[NKqq_0 F \\ ] & fq_2 F[NKqq_0 F \\ ] & fq_$$

(3-528)

Now,

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£

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error.

•

$$\begin{aligned} f_{fe}f[\mathbf{N}] \left\{ \tilde{\mathbf{q}}_{e}F + f_{ij} f[\mathbf{H}] \right\} \tilde{\mathbf{q}}_{e}F \\ &= \int_{2}^{L} \mathbf{N} \mathbf{x}_{i} d\mathbf{x} - \int_{2}^{L} (\bar{\mathbf{x}}_{-\mathbf{x}}) \mathbf{T}(\mathbf{x}) d\mathbf{x} \\ &= \int_{2}^{L} \mathbf{N}(\mathbf{x}) d\mathbf{x} - (\bar{\mathbf{x}}_{-\mathbf{L}}) \int_{2}^{L} \mathbf{T}(\mathbf{x}) d\mathbf{x} - \int_{0}^{L} \int_{0}^{T} \mathbf{g}_{i} d\mathbf{g} d\mathbf{x} \\ &= - \int_{0}^{L} \int_{0}^{\mathbf{x}} \mathbf{m}(\mathbf{g}) \frac{\mathbf{T}}{\mathbf{M}} d\mathbf{g} d\mathbf{x} - (\bar{\mathbf{x}}_{-\mathbf{L}})^{T} \\ &= - \frac{\mathbf{T}}{\mathbf{M}} (\mathbf{L} - \bar{\mathbf{x}}) \mathbf{M} - (\bar{\mathbf{x}}_{-\mathbf{L}})^{T} \end{aligned}$$
(3-529)

From Lagrange's equations we obtain

.

$$M_{J}^{z} + U_{te} \Theta + \{U_{ty}\}^{t} + U_{ty} + = c \qquad (3-530)$$

$$I = \{ U_{69} \} \{ \{ \} \} + U_{67} \} = 2$$
 (3-531)

$$\frac{1}{2} M_{q_2} + \frac{1}{2} + \frac{1}{2$$

$$\vec{z} \vec{x} + u_{2e} e + \{u_{2q} f \{q\} + u_{2q} r = r$$

$$+ \{w_{2q} f \{q\}\}$$

$$(3-533)$$

Eliminating  $\{\ddot{q}_j\}$  from the control equation by use of Equation 3-532, we obtain

$$\begin{array}{l} \left[ (J - \frac{1}{2} M_{2q} J_{1}^{2} M_{2q} J_{1}^{2} J_{1}^{2} + (J_{1q} J_{1}^{2} J_{1}^{2} J_{1}^{2} J_{1}^{2} + (J_{1q} J_{1}^{2} J_{1}^{2$$

The "dog-wags-tail" frequency is determined from

$$(J - \{M_{yq}\} \hat{J} M_{qy} \hat{J}) \hat{y} + (U_{yq} - \{M_{yq}\} \hat{J} U_{qy} \hat{J}) \hat{y} = 0$$
(3-535)

At this frequency the shear force across the gimbal point approaches zero, and the effective control moment is reduced.

In concluding these comments on ginbaled engine effects, we note that the equations of motion can be used to obtain the response to "hard-over" engine. In this case, we assume that the signal to the servo is a step input to command a constant ginbal angle,  $\gamma^*$ .



FIGURE 47 TIME HISTORY OF SIGNAL TO SERVO

The actuator moment is then given by

$$\Gamma = J\omega_{p}^{*} \left( e - F \right)$$
  
=  $-J\omega_{p}^{*} F + J\omega_{p}^{*} F^{*} H(t)$  (3-536)

The equations of motion are

$$\begin{bmatrix} \mathsf{M} \end{bmatrix} \begin{bmatrix} \mathbf{\tilde{s}} \\ \mathbf{\tilde{s}} \\ \mathbf{\tilde{s}} \end{bmatrix} + \left( [\mathsf{F}] + [\mathsf{U}] + \begin{bmatrix} \mathfrak{g} & \mathfrak{g} \\ \mathfrak{g} & \mathfrak{g} \end{bmatrix} \right) \begin{bmatrix} \mathbf{\tilde{s}} \\ \mathfrak{g} \\ \mathfrak{g} \\ \mathbf{\tilde{s}} \end{bmatrix} = \begin{bmatrix} \mathfrak{g} \\ \mathfrak{g} \\ \mathfrak{g} \\ \mathfrak{g} \end{bmatrix} \begin{bmatrix} \mathfrak{g} \\ \mathfrak{g} \\ \mathfrak{g} \\ \mathfrak{g} \\ \mathfrak{g} \end{bmatrix}$$

$$= \begin{bmatrix} \mathfrak{g} \\ \mathfrak{g} \\ \mathfrak{g} \\ \mathfrak{g} \\ \mathfrak{g} \\ \mathfrak{g} \end{bmatrix}$$

$$(3-537)$$

which can be solved by the routine discussed in the second part of Appendix VI.

In the case that the thrust forces can be ignored in comparison with the inertia forces, the above equations reduce to the following simple forms

$$I\ddot{\Theta} = 0 \tag{3-539}$$

$$\{\frac{1}{2}\} + \lceil \omega^2 \}_{12}^{2} + \{M_{12}\}_{2}^{2} = \{5\}$$
 (3-540)

$$J\left(\tilde{T} + \omega_{F}^{2}\tilde{T}\right) + \{M_{FQ}\}_{i}^{2}\tilde{J}_{i}^{2} = T\omega_{F}^{2}\tilde{z}^{i}H(E)$$
(3-541)

# 3.1.3.6 Some General Considerations of the Influence of Fuel Slosh on the Interal Motions of a Slender Launch Vehicle

We will only consider a vehicle having a single, unsegmented tank as shown by Figure 48.



FIGURE 48 LIQUID FUEL TANK AND LAUNCH VEHICLE

The displacement normal to the tank walls is given by

$$p_{i} = n \cdot p = n \cdot \mathbb{R} p_{z} = n_{z} p_{z}$$
(3-5/42)

where  $\mathbb{R}$  is a unit vector normal to the wall.

The virtual work of the wall forces is given by

$$s_{W} = \oiint s_{\mathbb{P}} \cdot \mathbb{Z} \cdot ds \qquad (3-543)$$

For an inviscid fluid

where p is the fluid pressure.

For the launch vehicle with empty fuel tank we have

$$T = \frac{1}{2} \int \pi(x) \left( \frac{\partial b}{\partial t} \right)^2 dx \qquad (3-5!+5)$$

$$\upsilon = \frac{1}{2} \int E_{\text{II}}(x) \left( \frac{\partial^2 p_2}{\partial x^2} \right)^2 dx \qquad (3-546)$$

The influence of the fuel and its inertia is described in the virtual work of the wall pressure (Equation 3-543).

$$S_{W} = \iint S_{P_{W}} b da.dk$$
 (3-547)

If  $p_i$ , i = 1,2...N are a set of generalized coordinates for the launch vehicle, then the transformation

$$h_{\mathcal{E}} = \{ n_{\mathcal{E}} \}$$
 (3-548)

gives

$$T = \frac{1}{2} \frac{1}{4} \hat{p} F \left[ A H \hat{p} \right]$$
(3-549)

$$U = \frac{1}{2} \{ b \}' [\kappa] \{ b \}$$
 (3-550)

$$S_W = \{S_0\}^{10}\} \tag{3-551}$$

where

$$[A] = \int 44 (x) \{ h_{z} \} i h_{z} \} i x \qquad (3-552)$$

$$[k] = \int E[x_1 \left\{ \frac{d^2}{dx} \right\} \frac{dx_2}{dx} \left\{ \frac{d^2}{dx} \right\} \frac{dx_2}{dx}$$
(3-553)

$$\int \partial f = \int \left\{ n_z \circ n_z \circ t_{\rm H} \right\}$$
 (3-554)

Since the pressure will depend on the motion of the walls, the fuel motion is coupled in a complicated way with the launch vehicle motion.

## 3.1.3.6.1 Dynamics of the Motion of the Fuel

If we let [p(x,y,z,t) be the displacement of the x-y-z particle of fluid, then we have the problem of determining the displacements of the fluid in terms of the displacements of the walls of the tank.



FIGURE 49 DISPLACEMENTS OF SLOSHING FUEL

The equations governing the motion are (see Paragraph 2.1.1.2, Equation 2-31)

$$y = \frac{1}{2} = \frac{1}{2} = \frac{1}{2} = \frac{1}{2} = \frac{1}{2}$$
 (3-555)

In the present case

and thus

If it is assumed that the displacements are small, the Eulerian coordinates

$$z = x + \beta_x + z_{1,2} + z_{2,3} + z_{3,2} +$$

are approximately equal to the Lagrangian coordinates; and, moreover

.

$$\frac{\partial f}{\partial x} = \frac{\partial f}{\partial x} \frac{\partial f}{\partial x} + \frac{\partial f}{\partial y} \frac{\partial g}{\partial x} + \frac{\partial (f)}{\partial y} \frac{\partial g}{\partial x} + \frac{\partial (f)}{\partial y} \frac{\partial f}{\partial x}$$

$$= \frac{\partial f}{\partial x} f (1 + \frac{\partial f x}{\partial x}) + \frac{\partial (f)}{\partial y} \frac{\partial f g}{\partial x} + \frac{\partial (f)}{\partial y} \frac{\partial f g}{\partial x}$$

$$= \frac{\partial (f)}{\partial x}$$
(3-559)

when the displacement gradients are small.

If the fluid is inviscid, then

 $\overline{c} = -\frac{1}{2}$  (p is the fluid pressure) (3-560)

for the Eulerian representation. The only body forces are those of gravity which are assumed to act parallel to the x-axis.

$$\mathbf{E} = -\frac{1}{2}\mathbf{I} \tag{3-561}$$

and Equation 3-557 becomes

$$\frac{2^{2}F}{2^{2}} = -\frac{2^{2}F}{2^{2}} + \frac{2^{2}F}{2^{2}}$$
 (3-562)

If the fluid is assumed incompressible, the Jacobian of the deformation must be constant.

$$\overline{z} = \frac{3}{24}, \frac{3}{25}, \frac{3}{25} = [1 - \frac{3}{24}], - [5 + \frac{3}{24}] \times [3 - \frac{3}{24}], \quad (3 - 563)$$

for small displacements

$$J = I \cdot J \times \mathbb{K} + I \cdot \frac{3\mu}{\delta y} $
(3-564)

If the Jacobian is constant, then

$$\nabla \cdot \mathbf{p} = 0 \tag{3-565}$$

If the fluid has uniform density in the undeformed state, the density is constant in space and time and the body forces have a potential,

such that

$$P = -\nabla \psi \quad \left( \nabla = \mathbf{I}_{\mathcal{U}}^{2} + \mathbf{J}_{\mathcal{U}}^{2} + \mathbf{K}_{\mathcal{U}}^{2} \right) \tag{3-567}$$

Equation 3-562 then becomes

$$\rho^{\frac{3^2}{2}}_{\delta t^*} = - \nabla(\rho + \vartheta)$$
(3-568)

Finally, if the deformation is assumed to be irrotational,

$$\nabla x \beta = 0 \tag{3-569}$$

then there exists a displacement potential,  $\boldsymbol{\theta}$  , such that

From Equation 3-565 we conclude that  $\theta$  satisfies Laplace's equation,

and, further, Equation 3-568 becomes

$$\forall \left( \frac{\partial^2 \Theta}{\partial t^2} \right) = - \forall (b \div \vartheta)$$
 (3-572)

or

$$e^{\frac{\partial t_{\pm}}{\partial t^2}} + \beta + \vartheta = \Im$$
 (3-573)

The boundary condition is that the normal component of the displacement is specified on the boundary.

Since the walls and free surface constitute a closed volume, we can introduce a set of curvilinear coordinates  $(\mu, \nu, \kappa)$  such that  $\nu(x, y, z) = 0$  describes the undeformed surface of this volume. The transformation from the cartesian coordinates (x, y, z) is

$$\begin{array}{l} x = x \ p, \ p_{1}, \ y^{(1)} \\ x = y \ (x_{0}, \ p_{1}, \ y^{(1)}) \\ z = z \ p \ (x_{0}, \ p, \ y^{(1)}) \end{array}$$
 (3-574)

A point of the surface is described by  $\mu$  and  $\kappa$  in the sense that

are the parametric equations of the surface bounding the volume of fluid.

If N is a normal to this surface, then

$$r_{1} = r_{1} = r_{1} = r_{1} = r_{1} = r_{1}$$
 (3-576)

is the normal component of the displacement at the boundary.

At those points of the boundary constituting the tank walls,  $p_{p}$  is assumed to be known in terms of generalized coordinates describing the configuration of the rest of the launch vehicle.

At those points of the boundary which constitute the free surface,  $p_{\nu}(\mu,\kappa,t)$  is not known; but on these boundaries the pressure is zero and, consequently, from Equation 3-573

$$e^{\frac{\partial^2 \sigma}{\partial E^2} + \frac{\partial}{\partial F} = \sigma}$$

(3-577)

on the free surface

but

$$v^{2} = -\varrho q (x + \beta_{x}(x,y,z,t))$$
 (3-578)

If  $\mathcal S$  is defined so that  $\mathcal S=0$  at the free surface, then

$$\vartheta = -\ell q \left( x - \chi_{f} + \left| b_{x}(x, y, z, \epsilon) \right. \right)$$

$$(3-579)$$

where  $x = x_s$  is the equation of the particles on the free surface. Since

$$p_x = \beta \cdot n \qquad (3-580)$$

on the free surface, we have

$$\frac{36}{32^2} = i \neq \beta \cdot n \qquad (3-581)$$

Thus, if we write,

$$p \cdot n - \nabla \vartheta \cdot n - \frac{j2}{\vartheta r}$$
(3-582)

we have

$$\frac{1}{2} = \begin{cases} \frac{1}{2} \sqrt{2} & y(t) & \text{on the walls} \\ \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} & \text{on the free surface} \end{cases}$$
(3-583)

Now, the general solution to Laplace's equation which is defined on the interior of a closed volume and has its normal derivative specified on the boundary is

$$\exists (\mu, \tau, \tau, t) = \bigoplus G(\mu, \kappa, \tau; \tau\tau) \frac{\Im}{\partial r} (\tau, \tau, z, t) d\tau dz \qquad (3-584)$$

This is the Neumann problem for the closed region bounded by the walls and the free surface<sup>1</sup>. G is the Green's function of the second kind.

Use of Equation 3-583 yields

$$\Theta(\mu,\kappa,\nu,t) = \iint_{(W)} \widehat{\pi}(\mu,\kappa,\nu;\tau;t) \frac{1}{2} \varphi(\eta,\kappa,\nu;\tau;t) \frac{1}{2t^2} \pi(\tau,t) \frac{1}{2$$

<sup>1</sup>See Kellogg, O. D., Foundations of Potential Theory, Dover, 1953, p. 246.
We then have the following integral equation for  $\theta$ 

$$\frac{\partial (u, n, n, t)}{(s)} = \iint_{(s)} \frac{1}{2} G(u, n, v; \tau, \tau) \frac{\partial G}{\partial t} (\sigma, \tau, 0, t) d\tau d\tau.$$

$$= \iint_{(s)} G(u, x, n; \tau, \tau) \phi_{n} (\pi; \tau, t) d\tau d\tau.$$
(3-586)

At points of the interior, the pressure is given by

$$b = -2\frac{3\frac{2}{5}}{5\frac{2}{5}} - 2\frac{3}{5}(x-x_5) - 2\frac{3\frac{2}{5}}{5\frac{2}{5}}$$
(3-587)

# 3.1.3.6.2 Solution of the Integral Equation

Consider the homogeneous equation

$$f'(x, y, c) = \pm \int_{0}^{\infty} f(x, y, c, c) \frac{f'_{0}}{ft'} (T, t, c, t) dt dt$$
(3-588)

and let

$$K(r_{K},\tau,\tau) = -\kappa_{K} \frac{1}{2} \frac{1}{2$$

and assume a separated solution

$$f(x, \mu, \chi, z, t) = z(\mu, \kappa) q(t)$$
 (3-590)

Then

$$y'' + y + y' = \int_{-\infty}^{-\infty} \psi(x, \pi \tau) \psi(\sigma, \tau) d\sigma d\tau d\tau d\tau$$
(3-591)

which requires

$$-\frac{q}{\ddot{q}} = a \text{ constant, say, } \lambda \tag{3-592}$$

then we have the following integral equation for  $\psi(\mu,\kappa)$ 

$$\iint_{(3)} \mathcal{K}(\mu,\kappa;\sigma,\tau) \psi(\kappa,\tau) d\sigma d\tau = \lambda \psi(\mu,\kappa)$$
(3-593)

From general properties of the Green's function, we have the following symmetry condition for the kernel function, K,

$$K(\mu_{\mu}\kappa_{\nu}\sigma_{\mu}\tau) = K(\sigma_{\tau}\tau_{\mu}\mu_{\nu}r) \qquad (3-594)$$

Equation 3-593 has a sequence of solutions

.

(3-595)

From the symmetry condition of K, one can derive the following orthogonality conditions for the solutions

$$\iint_{(\mathbf{x})} \psi_{\tilde{\mathbf{z}}}(\mu,\kappa) \psi_{\tilde{\mathbf{z}}}(\mu,\kappa) \quad \text{dynd}\kappa = 0 \quad \tilde{\mathbf{z}} = j \quad (3-596)$$

$$\iint_{(\Delta)} \iint_{(\Delta)} \psi_{\tilde{L}}(\mu,\kappa) K(\mu,\kappa;\sigma,\tau) \psi_{\tilde{L}}(\tau,\tau) d\tau d\tau d\tau d\mu d\kappa = J \qquad (3-597)$$

A normalizing condition can be imposed by considering the kinetic energy of the fluid  $^{\rm L}{\rm \cdot}$ 

$$=-\frac{1}{2} \bigoplus \frac{3}{28} \frac{3}{2} \frac{1}{2} $

for the homogeneous solution

$$\frac{dG}{dr} = \begin{bmatrix} \alpha & \\ 0$$

<sup>1</sup>See Lamb, H., Hydrodynamics, Dover, 1945 , r. 46, Equation (4).

so that

 $\tau = -\frac{1}{2} \iint_{(S)} \stackrel{\sim}{=} \frac{7}{7} \stackrel{\sim}{=} \frac{1}{2} u d^{H} \mathcal{K} \qquad (3-600)$ 

when

$$\boldsymbol{\varepsilon} = \boldsymbol{\psi} \boldsymbol{z} \qquad (\boldsymbol{\boldsymbol{3-601}})$$

$$\tau = - \lim_{z \to 0} \frac{1}{z} \lim_{z$$

We then choose the normelizing condition to be

or

$$\iint_{(3)} \psi_i^z \, d\mu d\nu = - \underbrace{\mathcal{Z}}_z \qquad (3-604)$$

Since

$$\iint \mathcal{K} = \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} = \frac{1}{2} \frac$$

we have

Τπ

.

summary 
$$\iint_{(z)} \iint_{(z)} \psi_{\overline{z}} \mathbb{K} \psi_{\overline{z}} d\sigma dz d\omega d\kappa = \begin{bmatrix} 0 & \overline{i} \neq \underline{j} \\ -\frac{\overline{i}}{2} \lambda \overline{i} \\ -\frac{\overline{i}}{2} \lambda \overline{i} \end{bmatrix}$$
(3-607)

$$\int_{iJ} +i \psi_j \, dx = \begin{bmatrix} J & i+j \\ -\frac{1}{2} & i-j \end{bmatrix} \quad (3-608)$$

.

Returning to the nonhomogeneous equation, we have

$$\begin{split} \mathfrak{S}(\mu, \mathfrak{n}, \mathfrak{a}, \mathfrak{t}) &+ \iint_{(\mathfrak{a})} \mathcal{K}(\mu, \mathfrak{n}; \mathfrak{a}; \tau) \frac{\partial \mathfrak{L}}{\partial \mathfrak{t}^{\sharp}} (\mathfrak{a}, \mathfrak{r}, \mathfrak{a}, \mathfrak{t}) d\mathfrak{s} d\mathfrak{s} d\mathfrak{s} \\ &= -\iint_{(\mathfrak{n})} \mathfrak{P}_{\mathcal{K}}(\mu, \mathfrak{n}; \mathfrak{n}; \mathfrak{a}) | \mathfrak{p}_{\mathcal{H}}(\mathfrak{a}, \mathfrak{r}; \mathfrak{t}) d\mathfrak{s} d\mathfrak{s} d\mathfrak{s} \end{split}$$
(3-609)

We will try to find a solution in the form

.

$$\begin{split} \mathfrak{S}(\mu,\kappa,\sigma,\mathfrak{t}) &= \sum_{\hat{\mathfrak{l}}} \psi_{\widehat{\mathfrak{l}}}(\mu,\kappa) q_{\widehat{\mathfrak{l}}}(\mathfrak{t}) \\ &- \iint_{(\mathfrak{n})} q_{\widehat{\mathfrak{l}}} \mathcal{K}(\mu,\kappa;\sigma,\tau) \dot{\mathfrak{p}}_{\mathcal{H}}(\sigma,\tau,\mathfrak{t}) d\mathfrak{d} \mathfrak{t} \end{split} \tag{3-610}$$

Substituting this into Equation 3-609, we obtain

$$\int_{i}^{T} \psi_{\bar{i}} q_{\bar{i}} + \iint_{(s)} K \int_{i}^{T} \psi_{\bar{i}} ds ds \bar{i} q_{\bar{i}}$$

$$-\iint_{(s)} K \iint_{(s)} q_{\bar{i}} K \frac{3^{2}b}{\delta t} ds ds ds ds ds = 0$$
(3-611)

but

$$\psi_{\bar{L}} = \frac{1}{\lambda_{\bar{L}}} \iint_{(s)} \mathcal{K} \psi_{\bar{L}} dz dz \qquad (3-612)$$

so that

$$\iint_{(S)} \left( \sum_{i} \frac{1}{\lambda_{i}} \kappa \psi_{i} q_{i} + \kappa \sum_{i} \psi_{i} \tilde{q}_{i} - \kappa \iint_{(W)} q_{i} \kappa \frac{\partial \psi_{i}}{\partial t^{i}} x_{\mu} d_{\mu} \right) ddr = 0$$
(3-613)

or

.

$$\frac{\sum_{i} \sum_{i} \psi_{i} q_{i} + \psi_{i} \sum_{i} = + \sum_{i} \frac{1}{4} K \frac{\partial \psi_{i}}{\partial t^{i}} a_{TAT}$$
(3-614)

On multiplying by  $\psi_{\mathbf{j}}(\mu_{\mathbf{x}}\kappa)$  and integrating over (S), we obtain

$$\frac{\prod_{i=1}^{n} \prod_{i=1}^{n} \psi_{i} \psi_{i} du dx_{i} \eta_{i} \frac{1}{\lambda_{1}} + \frac{1}{\eta_{i}}}{= \prod_{i=1}^{n} \psi_{i} \prod_{i=1}^{n} \frac{1}{\lambda_{1}} K \frac{\lambda_{1}}{\lambda_{1}} + \frac{1}{\eta_{i}}}{236} (3-615)$$

Using the orthogonality conditions, we obtain

 $\ddot{q}_{i} + \dot{\zeta}_{i} = -2 \iint_{(s)} \psi_{i} \iint_{(m)} K \frac{\partial f_{i}}{\partial t} t t d d c d \mu d \kappa \qquad (3-616)$ 

Interchanging the order of integration on the right, we obtain

. ..

$$\iint_{(S)} \mathcal{F}_{i} \iint_{(M)} \mathcal{K} \frac{\partial^{2} h_{i}}{\partial t^{2}} d\sigma d\tau d\mu d\kappa$$

$$= \iint_{(M)} \frac{\partial^{2} h_{i}}{\partial t^{2}} \iint_{(S)} \psi_{i} \mathcal{K} d\sigma d\tau d\mu d\kappa \qquad (3-617)$$

$$= \iint_{(M)} \frac{\partial^{2} h_{i}}{\partial t^{2}} \lambda_{i} \psi_{i} d\mu d\kappa$$

Using this result in Equation 3-616, we finally obtain

$$\frac{d^{2}q_{i}}{dt^{2}} + \frac{1}{\lambda_{i}} \eta_{i} = \rho_{i} \prod_{(w)} \psi_{i}(\mu,\kappa) \frac{\partial \mu}{\partial t^{i}}(\mu,\kappa,t) u_{i} d\kappa \qquad (3-618)$$

Now the pressure on the walls is, from Equation 3-587,

$$\dot{\beta} = - \varrho \frac{\partial \varphi}{\partial t^{2}} + \varrho q(x - x_{2} + \beta_{x})$$
 (3-619)

in which we neglect the contribution of  $\mathbf{p}_{\mathbf{X}}$  to the total force on the walls, so that

$$\dot{p} = -e \frac{\partial^2 \varepsilon}{\partial t^2} + e q (x - x_s)$$
(3-620)

Now, from Equation 3-610,

$$:\frac{\partial \mathcal{L}}{\partial \underline{z}} = \int \sum_{i} \psi_{i} \frac{\partial}{\partial y} - \psi_{i} \iint_{(\mathbf{w})} \mathbf{K} \frac{\partial^{2} p_{i}}{\partial z^{2}} d\sigma dz \qquad (3-621)$$

but from Equation 3-614,

$$F \prod_{(w)} \mathcal{K} \stackrel{\text{abp}}{\text{at}} d r d r = \sum_{i} \frac{1}{\lambda_{i}} \psi_{i} g_{i} + \psi_{i} \tilde{g}_{i} \qquad (3-622)$$

so that

.

$$e^{\frac{\partial^2 \theta}{\partial t^2}} = -e^{\sum_{i} \frac{1}{\lambda_i}} \psi_i \psi_i \psi_i \qquad (3-623)$$

The wall pressure is then

$$b = c \sum_{i} \frac{1}{\lambda_{i}} \psi_{i} q_{i} + i q_{i} (x - x_{s})$$
(3-624)

# 3.1.3.6.3 Coupling of Sloshing Fuel with Launch Vehicle Motion

In order to express these results in terms of the launch vehicle generalized coordinates, we use Equations 3-542 and 3-54d

$$b_{\gamma} = n_{z} \{ n_{z} f'_{1} \neq \}$$
 (3-625)

and write Equation 3-113 as

and Equation 3-547 becomes

$$SW = \iint_{(W)} Sp_{\mu} p d\mu d\kappa$$
  
=  $i SpF' \iint_{(W)} i n_{z} F i \psi F n_{z} d\mu d\kappa_{2} f'_{x_{1}} f i_{z} F$   
+  $i q f SpF' \iint_{(W)} i n_{z} F n_{z} (r - r_{a}) d\mu d\kappa$  (3-627)

If the tank has axial symmetry, the net hydrostatic force in the lateral direction is zero.

$$\iint_{(w)} \{h_{\pm}\}(x-x_s) n_{\pm} d\mu d\kappa = \{a\}$$
(3-628.)

The generalized forces defined in Equation 3-554 are then

,

$$f^{2}F = \rho \iint_{(w)} \{h_{z} \} f \psi f' n_{z} d\mu d\kappa \Gamma'_{x \perp} \{q_{z}\}$$
(3-629)

If we introduce

$$[\eta] = \iint_{(w)} \{h_{\Xi}\} \{\psi\}' n_{\Xi} \} \psi dx. \qquad (3-630)$$

then we have

.

$$-\frac{1}{4}\dot{F} + \frac{1}{4}\dot{I}\dot{F} = -\frac{1}{2}\lambda_{1}\left[\sqrt{1}\dot{F}\dot{F}\right]$$
 (3-631)

and Lagrange's equations for expressions (Equations 3-549, 3-550, and 3-551) give

$$[A H \mu] + [K H \mu] = J [Y ]^{1} J^{1} J^{$$

These may be rearranged by substituting Equation 3-631 into Equation 3-632.

$$([A] + e^{z} [\gamma] [\lambda_1[\gamma]') f \ddot{\mu} f + [\kappa E \mu f ]$$

$$= -e [\gamma] E \ddot{q} f$$

$$(3-633)$$

.

$$+ij_{+}+r_{x_{1}+j_{+}}=-\rho r_{x_{1}}[\eta ]+ij_{+}$$
 (3-634)

.

These equations can be somewhat simplified if we introduce the transformation

$$\{q\} = \rho \left[ x^{2} \left[ \gamma \right]^{\prime} (\{s\} - \{\beta\}) \right]$$
(3-635)

where the  $s_{1,r}$  i = 1,2...N are the "sloshing coordinates." Equations 3-633 and 3-634 become

$$([A]+[A_R]+[A_S])\{\tilde{\beta}\} + [\kappa H\beta] = -[A_S]\{\tilde{s}\}$$
(3-636)

$$[A_{\mathfrak{S}}H\mathfrak{S}] + [\kappa_{\mathfrak{S}}H\mathfrak{S}] = -[\kappa_{\mathfrak{S}}H\mathfrak{S}] \qquad (3-637)$$

1- (--)

where

$$[A_{R}] = e^{i} [\eta]^{r} \lambda_{J} [\eta]^{l}$$
 = the rigid fuel mass matrix (3-638)  

$$[A_{S}] = e^{i} [\eta]^{r} \lambda_{J} [\eta]^{l}$$
 = the sloshing fuel mass matrix (3-639)  

$$[\kappa_{S}] = e^{i} [\eta]^{r} \lambda_{J} [\eta]^{l}$$
 = the sloshing fuel stiffness (3-640)  
matrix

Equations 3-636 and 3-637 are the basis for a mechanical analogy in which the motion of the fuel is represented by the motion of an equivalent set of masses and springs.

## 3.1.3.7 Flutter and Divergence of Launch Vehicle Lifting Surfaces

#### 3.1.3.7.1 A General Method of Flutter and Divergence Analysis

In considering the aeroelastic stability of stabilizing fins or other Launch vehicle lifting surfaces, the generalized coordinates chosen to describe the system may include coordinates defining the motion of the launch vehicle itself although constraint of this motion usually has very little effect on the flutter speed. In any case, the equations are of the same form and are derived from Lagrange's equations.

$$\frac{d_i}{dt} \frac{\partial T}{\partial p_i} - \frac{\partial T}{\partial p_i} + \frac{\partial \omega}{\partial p_i} + \frac{\partial E}{\partial p_i} = D_i \qquad (3-641)$$

where, for small motions,

 $T = kinetic energy = \frac{1}{2} \frac{1}{p} \frac{1}{p} [A] \frac{1}{p}$ 

U = potential energy = zipikkipi

R = Rayleigh dissipation function =  $\frac{1}{2} \frac{1}{2}  

 $P_{i}$  = generalized external forces

- $i_p = matrix of displacements$
- [A] = inertial coefficient matrix
- [k] = stiffness coefficient matrix
- [B] = viscous damping coefficient matrix

The damping may be assumed to have the same distribution as the structural stiffness, so that the damping coefficient matrix is proportional to the stiffness matrix (see also Paragraph 2.2.3.5).

$$[8] = \mathfrak{z}[\kappa] \tag{3-642}$$

The vibration modes of the surface may be derived and used as a transformation to reduce the number of degrees-of-freedom:

$$\{p\} = [\phi] \{q\}$$
 (3-643)

where {q} is the column of "modal" generalized coordinates.

Lagrange's equations in terms of modal generalized coordinates are:

$$\frac{1}{2\pi} \left( \frac{2\pi}{2\eta_i} - \frac{2\pi}{2\eta_i} + \frac{2\eta_i}{2\eta_i} + \frac{2\eta_i}{2\eta_i} + \frac{2\eta_i}{2\eta_i} \right) = Q_i \qquad (3-644)$$

Then:

$$r = \frac{1}{2} \{ \hat{p} \} [A H \hat{p} \} = \frac{1}{2} \{ \hat{i}_{i} \} [M H \hat{i}_{i} \}$$
 (3-645)

$$J = \frac{1}{2} \{p\} \{\kappa\} = \frac{1}{2} \{j\} \{F\} \}$$
 (3-646)

$$R = \frac{1}{2} \frac{1}{p} \left[ B \right] \frac{1}{p} = \frac{1}{2} \frac{1}{2} \frac{1}{p} \left[ R \right] \frac{1}{p} \left[ R \left[ R \right] \frac{1}{p} \left[ R \right] \frac{1}{p} \left[ R \left[ R \right] \frac{1}{p} \left[ R \right] \frac{1}{p} \left[ R \left[ R \left[ R \right] \frac{1}{p} \left[ R \left[ R \left[ R \left[ R \right] \frac{1}{p} \left[$$

$$sw = \{s_p\}\{p\} = \{s_2\}\{z\}$$
 (3-648)

where

$$[M] = [r] [A] [\varphi]$$
(3-649)

$$[F] = [x] [K] [\phi]$$
(3-650)

$$[R] = [\varphi] [B] [\varphi]$$
 (3-651)

$$\{x\} = [\psi] \{p\}$$
 (3-652)

Substitution of these relations into Lagrange's equations gives

$$[M H \bar{g} ] + [R H \bar{g} ] + [F ] +$$

Since the solution to Equation 3-653 is extremely complex with Q a function of time, in the flutter equations it is convenient to calculate forces as a function of the frequency,  $\omega$ . This can be done by using Fourier transforms as shown below.

The Fourier transform is defined by

$$fq(t) F = \int_{-\infty}^{\infty} f \bar{q}(\omega) F e^{i\omega t} d\omega \qquad (3-65!+)$$

Now, the virtual work of aerodynamic forces can be expressed as:

$$\delta w = - \frac{1}{2} \frac{1}$$

where:

 $\omega =$  Fourier transform variable (circular frequency)

(• = air density

 $\gamma_{\alpha} = airspeed$ 

Ma = Mach number

 $[C(\mathcal{M}_{w_{m}}, M_{w})] = matrix of generalized sinforces$ 

The generalized forces associated with the  $\mathbf{q}_{\tilde{\tau}}$  are

$$iq(t_1) = -iq_{int} \int_{-\infty}^{\infty} [C' \frac{w}{v_{or}}, w_{or}] iq_{int} e^{i\omega t} d\omega \qquad (3-656)$$

and the Fourier transform of these forces is

$$\overline{z}_{\omega(\omega)} F = -\frac{1}{2} \overline{z}_{\omega} \overline{z}_{\omega} \left[ C(\mathcal{W}_{\omega}, \mathcal{M}_{\omega}) \right] \overline{z} \overline{j}(\omega) F$$
(3-657)

Equation 3-655 can be taken as an expression defining the zerodynamic matrix, [C]. If quasi-steady sirforces are used, then

$$S_{M} = -\frac{1}{2} \rho_{0} v_{0}^{2} i S_{0} F^{\prime} [C_{R}] i Q_{1} + \frac{1}{v_{0}} (C_{1}] i Q_{1} F) \qquad (3-658)$$

and, for this case, the complex zerodynamic matrix is

$$\left[ \mathbb{C} \left( \frac{\omega_{\mu}}{V_{\mu}} \cdot M_{\mu} \right) \right] = \left[ \mathbb{C}_{R} \right] + \left[ \mathbb{C}_{n} \right] \quad (3-659)$$

Substitution of Equations 3-654 and 3-656 into Equation 3-653 gives

$$\int_{-\pi}^{\infty} -u^{2} [w | \overline{z} \, \overline{q} \, \overline{F} \, e^{i u t} du$$

$$+ \int_{-\pi}^{\infty} \omega [\mathbb{R} \, \overline{H} \, \overline{q} \, \overline{F} \, e^{i u t} du$$

$$+ \int_{-\pi}^{\infty} [\mathbb{R} \, \overline{H} \, \overline{q} \, \overline{F} \, e^{i u t} du$$

$$= \cdot - \frac{1}{2} v_{\pi} v_{\pi}^{2} \int_{-\pi}^{\infty} [\mathbb{C} \, \overline{h} \, \overline{z} \, \overline{F} \, e^{i u t} du$$
(3-660)

OF

$$\frac{\pi}{1-\pi} = 1^{-1} [\pi] + [\pi]$$

which implies

$$-\frac{1}{2} = \frac{1}{2} = \frac{1$$

Now, consider

By using Equations 3-642, 3-650, and 3-651, we have

Then, substituting, we have

$$\frac{1}{2} [c] + \frac{1}{2} [c] = \frac{1}{2} [c] + \frac{1}{2} [c]$$
(3-664)

Rewriting Equation 3-662, and substituting Equation 3-664, we have:

$$-[w] + \frac{1}{2} \frac{1}{$$

Premultiplying by

[G] = [F] (3-666)

 $\chi = \frac{1 + i\omega_0 \varepsilon}{\omega^2}$ (3-667)

and introducing

we obtain

$$(\lambda [1] - [N]) \{ \bar{q} \} = \{ a \}$$
 (3-668)

$$\left[\mathbb{N}\left(\frac{\omega}{v_{\alpha}}, w_{\alpha}, \ell_{\alpha}\right)\right] = \left[\mathbb{G}\left[\left(\mathbb{M}\right] - \frac{\ell^{2}}{\chi}\left(\frac{\omega}{v_{\alpha}}\right)^{2}\left[\mathbb{C}\left(\frac{\omega}{v_{\alpha}}, w_{\pi}\right)\right]\right]\right]$$
(3-669)

The solution to Equation 3-668 yields the following:

$$f_{\overline{i}} = \{a\} \tag{3-670}$$

or

where

$$\Delta(\lambda) = |\lambda| [n_1 - [N]| = 0 \qquad (3-671)$$

For other than the trivial solution  $\{\bar{q}(\omega)\} = \{0\}$ , the determinant must be equal to zero. This can be expended for each value of  $\omega/v_{\infty}$  as an N<sup>th</sup> order polynomial in  $\lambda$  with complex coefficients. Several methods of expension are available; for example, Danielewski's method. The roots of the polynomial,  $\Delta(\lambda) = 0$ , are obtained by Newton's method<sup>2</sup>. The damping,  $\beta$ , frequency,  $\omega$ , and speed,  $\omega_{\alpha}$ , are then calculated from these roots by the following relationships.

From Equation 3-667, we have

 $\lambda_{\mu} = \lambda_{\mu\mu} + i \lambda_{\mu\mu} = \frac{i + i \omega_{\mu\mu}}{\omega^2}$  (3-672)

$$x_{\rm H} = \frac{1}{4^2}$$
 (3-673)

The flutter equations can be reduced to the form of Ventth h (-) even when  $[F]^{-1}$  does not exist so that the method is also accordinate for unrestrained systems.

<sup>&</sup>quot;A method employing litert solution of the eigenvalue workler has lately been used with success on systems requiring a large number of moles for their description. See W. 1. Rodden, et al. Flutter and Vibration Analysis by a Model Method: Analytical Development and Computational Procedure. Aprospace Compontion, Remort No. IDR-109(3230-11) IN-10. 31 July 1903.

$$\lambda_{I} = \frac{4}{\omega}$$
(3-674)

So that:

$$\lambda = \frac{\lambda_{\rm I}}{\sqrt{\lambda_{\rm R}}} \tag{3-675}$$

$$\omega = \frac{1}{\sqrt{\lambda_{R}}}$$
(3-676)

The airspeed is then calculated from the value of  $\omega \not \sim_{\infty}$  which has been selected:

$$v_{\alpha} = \frac{v_{\alpha}}{w} \omega = \frac{1}{\left(\frac{w}{v_{\alpha}}\right)} \sqrt{\frac{1}{\lambda_{z}}}$$
(3-677)

The flutter solution is obtained when the value of  $\beta$  calculated from Equation 3-657 agrees with experimental or theoretical estimates of  $\beta$ . This is the airspeed at which neutral stability exists. True flutter speed occurs where the airspeed calculated in Equation 3-677 coincides with the assumed Mach number.

To determine the aeroelastic divergence speed, that is, where neutral stability exists as the frequency approaches zero, refer to Equation 3-661, and rewrite with  $\omega = 0$ .

$$\left(2^{2} x_{1}^{2} \right) \left(1 - \frac{1}{5}\right) \left(\frac{1}{5}\right) = 10\right)$$
(3-678)

Iet

This becomes a simple relationship of real equations dependent upon the dynamic pressure and the internal restoring forces. If we assume a solution other than the trivial solution  $\{\bar{q}(\omega)\} = \{0\}$ , the following determinant can be expanded as an N<sup>th</sup> order polynomial with real coefficients in  $\mathfrak{W}^{\omega}_{\mathcal{I}}$  for each value of Mach number.

$$\frac{1}{|\psi|_{\pi}^{2}} [\mathbf{1}_{j} + [\Im][2]_{3}, \mathbf{w}_{\pi}] = 0$$
(3-680)

From this relationship the solution for  $\frac{1}{2}\sqrt[3]{\omega}\sqrt{\omega}$  can be easily obtained. It should be pointed out that this is a special case in the flutter equations whereby selection of the parameter  $(\omega/\gamma_{\omega}) = 0$  (frequency equal to zero) will also yield the aeroelastic divergence speed.

To correlate the modal solutions with test data for checking flutter analyses, consider the following. The equations of motion, including viscous damping, can be written in the following form (see Equation 2-296):

$$\frac{d^2 \pi}{dt^2} + Z J_n \omega_n \frac{d q_n}{dt^2} + \omega_n^2 J_n = J_n \qquad (3-681)$$

where:

 $q_r$  = the normal coordinate for the n<sup>th</sup> mode of vibration

From Equation 3-653

$$[MH\bar{q}I + [FH\bar{q}I + [FH\bar{q}I = Eq.]$$
 (3-682)

By comparison of Equations 3-681 and 3-682, we must have

$$[\mathsf{M}] = \lceil \mathsf{1} \rfloor \tag{3-683}$$

$$[R] = [-i_{runj}] \qquad (3-684)$$

$$[= [u_m] \qquad (3-685)$$

But, from Equation 3-663,

[R] = 5[F] (3-686)

Hence:

$$-Inum = k In$$
(R-687)

or

$$z = \frac{z_{\infty}}{z_{\infty}} \tag{(3-683)}$$

This implies that there is a different percent of critical damping present in each mode which varies with the natural frequency and mode number. For structures where the damping distribution is proportional to the stiffness distribution, the critical damping factor,  $:_n$ , is higher in the higher frequency modes, that is:

$$i_n = i_{n-n}$$
 (3-689)

An example of determining  $\beta$  experimentally is shown by Figure 50. The data have been obtained from a restrained surface ground vibration test. These data are listed in Table 9 and pictured in Figure 50.

Figure 51 shows the graphical representation of the solution of the flutter determinant.

Mode Number M	Critical Demping Fector \$_	Frequency (Red/Sec) 47 <sub>m</sub>	25 <u>m</u> x 10 <sup>3</sup> w <sub>m</sub> ((Sec))
F & m_# In@	0.00945	41.94	0.451
	0.0087	104.74	0.166
	0.0196	139.61	0.281
	0.0154	176.56	0.174
	Not available	206.15	Not availeble
	0.0152	255.10	0.1192

TABLE 9 TABULATION OF EXPERIMENTAL DATA FROM GROUND VIERATION TEST



FIGURE 50 ESTIMATION OF  $oldsymbol{eta}$  FROM GROUND VIERATION TEST DATA



FIGURE 51 GRAPHICAL REPRESENTATION OF SOLUTION OF FLUTTER DETERMINANT

#### 3.1.3.7.2 A Hoot-Flene Method of Flutter Anelysis for Quesi-Steedy Aerodynemic Forces

If quest-steedy zerodynamic forces are assumed, the equations of the previous section (Equation 3-653) can be written as

$$W_{EGF} + [R_{EGF} +$$

with

$$f \alpha F = - \pm_{\text{HM}} \left( [i_{\pi} F_{\eta}] F + \pm_{\text{H}} [c_{\pi} F \ddot{q}] \right)$$
(3-691)

(see Equation 3-658)

In the present case we will not assume any relation between the damping and stiffness matrix (such as Equation 3-642)

TH we introduce

$$i\pi F = iF$$
 (3-692)

Then the above equations can be written as

$$-35 - 4\pi F = -3F$$
 (3-694)

These equations may be rewritten as a single matrix equation

$$\begin{bmatrix} 1 & 1 \\ - & 1 \\ 1 & 1 \end{bmatrix} = \begin{bmatrix} x \\ 1 & 1 \end{bmatrix} = \begin{bmatrix} x \\ -1 & 1 \end{bmatrix} = \begin{bmatrix} 1 \\ -1 & 1 \end{bmatrix} = \begin{bmatrix} 1 \\ -1 & 1 \end{bmatrix} = \begin{bmatrix} 1 \\ -1 & 1 \\ -1 & 1 \end{bmatrix} =$$

where

$$[N_{*}] = (-[N_{*}]^{2} [\mp ] - \underline{w}_{\underline{\lambda}}^{\underline{\mu}} [\mp_{\underline{\lambda}}] - [N_{*}]^{2} (\mp_{\underline{\lambda}} - \underline{w}_{\underline{\lambda}}^{\underline{\mu}} ]_{\underline{\lambda}})$$

$$(3-696)$$

$$[1]$$

243

If we take the Laplace transform of these equations, we obtain

$$\left( \begin{array}{c} \left[ \left( S \right]_{1} - \left[ N \right] \right) \right| \left\{ \overline{n} \right\} = \left\{ 0 \right\}$$

$$\left( \begin{array}{c} \left[ \left\{ \overline{q} \right\} \right] \right\} = \left\{ 0 \right\}$$

$$\left( \begin{array}{c} \left( 3 - 697 \right) \right\} \right)$$

The stability of the system is described by the characteristic equation

$$\Delta(s) = \left| S \left[ t_{j} - \left[ N \right] \right| = 0 \qquad (3-698)$$

Unlike the polynomial in Equation 3-671, the above polynomial has real coefficients and thus the roots of this polynomial occur in conjugate pairs.

The matrix,  $[N]_r$  is a function of airspeed  $V_{00}$ , Mach number,  $M_{00}$ , and air density, (a. For the trajectory of a launch vehicle, these parameters can be related to flight time. For a given trajectory then, the matrix,  $[N]_r$  can be related to a single variable. Thus, the roots to Equation 3-698 can be regarded as changing continuously with a parametric variation of flight time along the boost trajectory.

If we let  $\sigma$  and  $\omega$  be the real and imaginary parts of the i<sup>th</sup> root to Equation 3-698, then

$$S_i = T_i + i \omega_i \tag{3-699}$$

which may be plotted as a point in the  $(\sigma, \omega)$  plane.

Flutter or divergence is indicated when one of the roots passes into the unstable part of the root plane  $(\sigma > 0)$ . Figure 52 is a typical plot.



FIGURE 52 LOCUS OF THE FLUTTER ROOTS FOR A PARAMETRIC VARIATION OF FLIGHT TIME

The stability of the system is more evident from a plot of the real part of the root as shown in Figure 53.



FIGURE 53 DAMPING VERSUS FLIGHT TIME

#### 3.1.4 Response of the Missile to Continuous Random Turbulence

If it is assumed that the control system does not respond to the frequencies of turbulence in the atmosphere, the loads due to atmospheric turbulence may be obtained by assuming the control locked in the  $\gamma = 0$  position. The appropriate equations may be obtained from Equations 3-27, 3-28, and 3-29 by setting  $\gamma = 0$ .

$$\tau = \frac{1}{2} + $

$$u = \frac{1}{2} + b + (1 - 1) + (3 - 701)$$

$$\varepsilon w = -\frac{1}{2} \left\{ \omega v_{\alpha} i \delta \beta \right\} [\Lambda] [\pi]$$
(3-702)

$$\{x\} = [\Delta]\{\flat\} + \frac{1}{v_m} \{\flat\}$$
(3-703)

In addition, we have the virtual work done by the gust forces

$$SW = -\frac{1}{3} \frac{1}{2} v_{iz}^{2} \int_{-\infty}^{\infty} \frac{M(x,z)}{2} \frac{M(x,z)}{2} dx \qquad (3-704)$$

where w(x,t) is the downwash of the turbulent atmosphere at a point,  $x_{i}$  on the missile. Assuming that the velocity of atmospheric particles is stationary in space, we have

$$h(x,t) = -(1,t-x)$$
 (3-705)

where  $f(\lambda)$  is a function giving the distribution of gust velocities as a function of the distance,  $\lambda_r$  along a fixed path in space.





The function  $f(\lambda)$  is assumed to be a random function whose statistical characteristics are described by von Karman's well-known formula for the power spectrum of fluid velocities in a locally turbulent fluid<sup>I</sup>.

$$\Phi[f^{(i)}; -2] = \tau \frac{1}{\tau} \frac{(1+3)T^{2}T^{2}}{(1+s)^{2}T^{2}}$$
(3-706)

The power spectrum is a "functional" of the random function,  $f(\lambda)_r$  and an ordinary function of the frequency variable,  $\Omega_*$ . In this expression,  $\Omega$  has the units of (length)<sup>-1</sup>; and corresponds to a "wave length" variable for the oscillations in space of the gust velocities. Also,  $\sigma$  is the root mean square value of  $f(\lambda)$  and L is the "scale" of the turbulence<sup>2</sup>.

The gust forces can be written for the discrete system by using the interpolation method which gives a relation of the form

$$\phi_{z'(x,t)} = \frac{1}{2} h_{z'(x)} f_{z} + f_{z'(x)} f_{z'(x)} + f_{z'(x)} f_{z'(x)} + f_{z'(x)} f_{z'(x)} + f_{z'(x)} f_{z'(x)} + f_{z'(x)$$

1- ----

This is discussed in several sources; in particular, H. S. Isien, Engineering Cybernetics, McGraw-Hill, 1954.

<sup>&</sup>lt;sup>2</sup>See MASA Report 1272, <u>A Reevaluation of Data on Atmospheric Turbulence for</u> <u>Application in Spectural Calculations</u>, by Harry Press, May T. Meadows, and Ivan Hallock, 1956.

The functions,  $h_{Z'}(x)$ , for the diparabolic formula are given in Equation 2-451 of Paragraph 2.3.3.1 of this report. Equation 3-704 can then be written as

$$sw = \frac{1}{2} m v_{m}^{2} + \delta p \frac{f'}{2} \int_{0}^{1} \frac{f_{h_{\overline{n}}}(x) \frac{y C_{L}(x)}{y c_{L}}}{\sqrt{k}} \frac{W'(x, t)}{V_{m}} dx \qquad (3-708)$$

The gust downwash at the ith collocation point can be written as

$$W_{L}(t) = W(x_{L}, t) = f(v_{0}t - x_{L})$$
 (3-709)

and the interpolation formula can be used to write

$$wr'x_{1}t_{1} = \{h_{z}(x)\}'\{w(t_{1})\}$$
  
=  $\{h_{z}(x)\}'\{f(v_{0}t_{1}-x_{t_{1}})\}$  (3-710)

We then have

$$\delta W = \frac{1}{2} \log v_{\alpha}^{2} + \delta \beta \left[ \Lambda \left[ \frac{1}{2} \frac{f(v_{\alpha} t - x_{i})}{v_{\alpha}} \right] \right]$$
(3-711)

where

$$[\Lambda] = \int_{\alpha}^{L} \frac{1}{2} h_{z}(x) \frac{1}{3\kappa_{z}} \frac{\chi_{L}}{\chi_{L}}(x) \frac{1}{2} h_{z}(x) \frac{1}{2} dx \qquad (3-712)$$

which is the matrix of aerodynamic influence coefficients introduced in Equation 3-18.

Using Lagrange's equations, we obtain

.

$$[\Delta Tip J + [\kappa Hp J + \frac{1}{2} m m (\Lambda T'[\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m m (\Lambda T' [\Delta Hp J + \frac{1}{2} m (\Lambda T' $

The effective loads are

$$\{F\} = -[A]_{2} \dot{F}_{1} - \frac{1}{2} \dot{F}_{0} v_{0} \cdot [A] \left( [\Delta F_{1} \dot{F}_{1} + \frac{1}{V_{0}} \dot{F}_{1} \right) + \frac{1}{2} \dot{F}_{0} v_{0} \cdot [A]_{1} f + \frac{1}{V_{0}} \dot{F}_{1} \right)$$

$$(3-714)$$

To solve the equations of motion, we make the modal transformation

$$i \neq l = [ \varphi ] i j$$
 (3-715)

which is assumed to include rigid body modes; i.e.,

$$\{\psi\}_{i} = \{i\}, q_{i} = j, \{\psi\}_{i} = \{\bar{x} - x\}, q_{i} = 0$$
 (3-716)

Substituting this into Equation 3-713 and premultiplying by  $\left[\phi\right]'$ , we obtain

$$[M]\{\hat{q}\} + [F]\{\hat{q}\} + \frac{1}{2} e^{\pi i \hat{\sigma}} ([C_R]\{\hat{q}\} + \frac{1}{V_{\omega}} [C_{\mathrm{T}}]\{\hat{q}\}) = \{Q\} \qquad (3-717)$$

where

$$\{q(\mathbf{k})\} = \pm_{q(\mathbf{k})} [q]'[\Lambda] \{\frac{f(v_{0}t-x_{1})}{v_{0}}\}$$
(3-718)

and

$$[M] = [\phi]'[A][\psi] \qquad (3-719)$$

$$[F] = [\varphi]'[K][\varphi]' \qquad (3-720)$$

$$[C_{\mathsf{R}}] = [\varphi]'[\Lambda][\Delta][\varphi] \qquad (3-721)$$

$$[C_{I}] = [\phi]'[\Lambda][\phi]$$
 (3-722)

In order to solve Equation 3-717 and obtain appropriate transfer functions, we assume  $\{q(t)\}$  to be expressed in terms of its Fourier transform as follows:

$$\{q_i\} = \int_{-\infty}^{\infty} \{\bar{q}_i \mid \omega\} e^{i\omega t} d\omega \qquad (3-723)$$

The corresponding inverse transform is 1

$$\{\bar{j}_{\omega}\} = \lim_{d \in \mathbb{T}} \int_{-\infty}^{\infty} \{\bar{j}_{\nu}\} e^{i\omega t} dt \qquad (3-724)$$

Introducing Equation 3-723 into Equation 3-717, we obtain

$$\int_{-\infty}^{\infty} \left( -\omega^{1} [M] + [F] + \frac{1}{2} \mu \alpha V_{\infty}^{2} [\Omega_{R}] + \frac{1}{V_{\infty}} [\Omega_{T}] \right) \frac{1}{2} \frac{1}{2} (\omega) \frac{1}{2} e^{i\omega t} d\omega \qquad (3-725)$$

By definition of the Fourier transform. we have

$$\frac{fQ(t)}{f} = \int_{-\infty}^{\omega} f \frac{1}{2} \sin f e^{i\omega t} d\omega \qquad (3-726)$$

<sup>&</sup>lt;sup>1</sup>The convention for the form of the Fourier transform used here agrees with the engineering artifice of assuming "harmonic motion" of the form  $\{q_i(t)\} = \{\bar{q}_i\} e^{i\omega t}$ .

and by comparison with Equation 3-725,

.

$$\left(-\omega^{L}[M] + [F] + \frac{1}{2} \partial_{\alpha} \gamma_{\alpha}^{L} \left( \left[ C_{R} \right] + \frac{1}{\gamma_{\alpha}} \left[ C_{L} \right] \right) \right) \hat{i}_{\hat{y}} (\omega) \hat{j} = \hat{i} \tilde{\omega}(\omega) \hat{j}$$
(3-727)

Using Equation 3-718, we obtain the inverse transform

$$\begin{aligned} \bar{q}(\omega) ]_{j} &= \frac{1}{2\pi} \int_{-\infty}^{\infty} \frac{i\omega v_{\omega}}{2} [\varphi]'[\Lambda]_{1} \left[ i v_{\omega} t - x_{i} \right]_{j}^{j} e^{-i\omega t} dt \\ &= \frac{i\omega v_{\omega}}{2} [\varphi]'[\Lambda]_{1} \left[ \frac{1}{2\pi} \int_{-\infty}^{\infty} \{f(v_{\omega} t - x_{i})\} e^{-i\omega t} dt \end{aligned}$$
(3-728)

If we change the variable of integration from t to

$$\lambda = v_{x} t - x_{i} \tag{3-729}$$

then we have

.

$$\frac{1}{2\pi} \int_{-\infty}^{\infty} \{f(x_{0}t-x_{1})\} e^{-i\omega t} dt = \int_{-\infty}^{\infty} \frac{1}{2\pi} \int_{-\infty}^{\infty}$$

Now, in this expression, we recognize the Fourier transform of the spacial distribution of fluid velocities based on the "spacial" frequency

$$I = \frac{\omega}{v_{\varpi}}$$
(3-731)

That is

 $\overline{f}(\lambda) = \frac{1}{4\pi} \frac{f(\lambda)}{f(\lambda)} e^{-i\pi\lambda} d\lambda$  (3-732)

We then have

$$\{\hat{q}_{(2)}\} = \frac{i\pi}{2} [\hat{q}_{1}]'[\Lambda] \{\hat{e}^{\frac{1}{2}} \hat{e}_{2}^{\frac{1}{2}} \} [\hat{q}_{-2}]$$
(3-733)

We have the theorem<sup>1</sup> that any variable linearly related to  $\overline{f}(\varrho)$ , say

$$\frac{1}{2}\omega_1 = \tau_{,\omega} + \frac{1}{2} \cdot 2^{3}$$
 (3-734)

See Truxal, John G., Control System Synthesis, McGraw-Hill, 1955, Chaps. 7 and 8 in particular, equation 8.3, p. 455.

has a power spectrum given by

.

$$\sum [f(t); \omega] = |T(\omega)|^2 \quad \sum [f(t); z]$$
(3-735)

Using Equations 3-727 and 3-733, we obtain

$$\frac{1}{\sqrt{2}} z = - \frac{\omega_{e}}{\sqrt{2}} \sum_{n=1}^{\infty} \left[ e_{n} \right] + \frac{1}{\sqrt{2}} $

The loads equations (3-714) in the frequency domain are

$$\frac{1}{2} = -\omega^{2} [A] + \frac{1}{2} = \frac{1}{2} \left[ A \left[ \frac{1}{2} + \frac{1}{2} \right] \left[ A \left[ \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right] \right] + \frac{1}{2} = \frac{1}{2} \left[ A \left[ \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right] \right]$$
(3-737)

For the purpose of being specific let us take the bending moment at the center of mass,  $x = \overline{x}$ , as a typical load

Making use of

.

and Equation 3-737, we obtain

$$\overline{w}_{\perp} = \{x\} - \frac{1}{2\pi}, [A] + \frac{1}{2\pi} [A] +$$

which we can write as

$$\bar{w}_{+} = -\frac{1}{2} \frac{1}{4\pi} (3-7+2)$$

where

$$T(\omega) = \{Rf\| (-(z)^{T}[A] - \underline{g}^{T}[A]] + i(\underline{z})[A]\} - (1f)^{T} - \frac{1}{2} f^{T} + \frac{1}{2} f$$

The power spectural density of the bending noment is given by

$$\overline{\Phi}\left[\left[M(t);\omega\right] = \left[T(\omega)\right]^{2} \left[T \frac{u}{\pi} \frac{u+3\left[\frac{\omega}{2}\right]^{2} \left[1\right]}{\left[f_{1}+\left(\frac{\omega}{2}\right)^{2}\right]^{2}}\right]$$
(3-743)

where use has been made of Equations 3-706 and 3-735. The above procedure is typical for finding the transfer function of any zeroelastic variable that is important to the design of a missile for strength and/or low cycle fatigue considerations.



FIGURE 55 TRANSFER FUNCTIONS FOR ATMOSPHERIC TURBULENCE

#### 3.1.5 Dynamic Loads during Ground Transportation

The results of this section are more specialized in their application than the results of the previous sections, but this section is included to illustrate further the applications of the general methods of Section 2.0 to a wide variety of structural dynamics problems.

The specific problem considered is described by Figure 56. The system is assumed to be a flexible launch vehicle supported on three shock mounts by a rigid trailer. The rigid body motion of the trailer is assumed to be known as a function of time. The shock mounts are idealized by a linear spring and a linear dashpot in parallel as shown by Figure 57.



FIGURE 56 MISSILE TRANSPORTER SYSTEM

In terms of a number of collocation point displacements on the missile, we have

$$T = \frac{1}{2} \frac{1}{2} \dot{p} \frac{1}{2} \dot{p} \frac{1}{2} \dot{p} \frac{1}{2} \dot{p} \frac{1}{2}$$
(3-744)

$$R = \frac{1}{2} \frac{1}{10} \frac{1}{10$$

ani

$$U = \underset{i=1}{1} \underset{j=1}{1} $

where [K] is the stiffness matrix for the unrestrained launch vehicle and [B] is the unrestrained damping matrix<sup>1</sup>.

In most cases, damping in the vehicle structure can be described adequately by  $[B] = \beta[K]$ . See Also Paragraph 3.1.3.7.1.

The extension of the	i <sup>th</sup> spring is
よ ÷ (x-×i)ラ	$-p_{\pm}(x_{i}t)$
<u> </u>	- Topic
displacement	displacement
of lower end	of upper end

. .



FIGURE 57 IDEALIZATION OF SHOCK MOUNT

The displacement,  $\zeta$ , and rotation,  $\theta$ , of the trailer are assumed to be known functions of time.

The force,  $\ensuremath{\mathbb{N}}_i$  , that the ith shock mount exerts on the missile in the z-direction is given by

$$f_{1} = f_{2} \left[ (-x_{1}) - f_{2} (x_{1}, t) \right] + c_{1} \left[ (z - x_{1}) - \frac{2t}{2t} (x_{1}, t) \right]$$

$$(3 - 7 + 7)$$

.

where  $\mathbf{k}_i$  and  $\mathbf{c}_i$  are the spring and damping constants. The total virtual work of these forces is

$$\varepsilon_{W} = \int_{i=1}^{M} \varepsilon_{p_{z}}(x_{i}, t) N_{i} \qquad (3-748)$$

for M different shock mounts.

The missile displacement is related to the generalized coordinates by a relation of the form:

$$\oint_{z} (x_{1} t) = \{ h_{z}(x) \} \{ p \} \quad (3-749)$$

Using this in Equations 3-747 and 3-748, we obtain

$$5x = \frac{1}{2} 5p \int_{-\frac{1}{2}}^{\frac{M}{2}} \frac{1}{2} k_{Z}(x_{2})^{\frac{1}{2}} p_{\overline{2}}^{\frac{1}{2}} J + (\overline{x} - x_{2})^{\frac{1}{2}} - \frac{1}{2} h_{Z}(x_{2})^{\frac{1}{2}} + p_{\overline{2}}^{\frac{1}{2}} J + (\overline{x} - x_{2})^{\frac{1}{2}} - \frac{1}{2} h_{Z}(x_{2})^{\frac{1}{2}} + p_{\overline{2}}^{\frac{1}{2}} J + (\overline{x} - x_{2})^{\frac{1}{2}} - \frac{1}{2} h_{Z}(x_{2})^{\frac{1}{2}} + p_{\overline{2}}^{\frac{1}{2}} J + (\overline{x} - x_{2})^{\frac{1}{2}} - \frac{1}{2} h_{Z}(x_{2})^{\frac{1}{2}} + p_{\overline{2}}^{\frac{1}{2}} J + (\overline{x} - x_{2})^{\frac{1}{2}} - \frac{1}{2} h_{Z}(x_{2})^{\frac{1}{2}} + p_{\overline{2}}^{\frac{1}{2}} J + p_{\overline{2}}^{\frac{1}{2}}$$

If we use

$$x = \{h_{z'} : x\} \{1\}$$
 (3-751)

$$\bar{x} - x = \{ h_{z} : x^{2} \} \{ \bar{x} - x \}$$
 (3-752)

and introduce

$$[r_{25}] = \frac{M}{2^{2}} e^{ik_{2}} r_{2} e^{ik_{2}} e^$$

$$[B_{s}] = \frac{1}{12} + \kappa_{z} +$$

we can write Equation 3-750 as

$$s_{M} = f s_{0} f f h_{0} f f h_{0} + f s_{0} $

In order to reduce the number of degrees-of-freedom, it is expedient to transform to model generalized coordinates. For this purpose, we introduce

where  $[\phi]$  is a matrix of unrestrained modes for the launch vehicle (including rigid-body modes). Jubstituting Equation 3-755 into Equations 3-744, 3-745, 3-746, and 3-755, gives

$$R = \pm \frac{1}{2} \frac{1}{2$$

$$\begin{split} \varepsilon_{W} &= -i \varepsilon_{3} f' [\varphi] [\kappa_{s} [\varphi] f_{2} f - i \varepsilon_{3} f(\varphi] [\varepsilon_{s} [[\varphi] f_{3}]) & (3-760) \\ &+ i \varepsilon_{2} f' [\varphi] [\kappa_{s} [f_{1} f_{3} + [\varphi] [\kappa_{s} [f_{3} - x] f_{6}] \\ &+ [\varphi] [B_{s} [f_{1} f_{3} f + [\varphi] [\kappa_{s} [f_{3} - x] f_{6}] ] \end{split}$$

Lagrange's equations (Equations (2-64) are

$$\frac{1}{2t}\frac{\partial f}{\partial j_{t}} - \frac{\partial f}{\partial q_{t}} + \frac{\partial Q}{\partial q_{t}} + \frac{\partial Q}{\partial j_{t}} = \hat{z}_{t}$$
(3-761)

where

.

$$\delta w = \int_{i} \frac{\omega_{i}}{\sqrt{2}} \tilde{\omega}_{i} \qquad (3-762)$$

and, in the present case, these equations yield

where

.

$$[*] = [:][B][:] + [:][B_{e}][:]$$
(3-765)

$$[F] = [\mu I[k, I[\mu] + [\nu]][k_{\rm e}I[\mu]]$$
 (3-766)

These equations may be put in the form

$$[Mh_{j}] + [Rh_{j}] + [Fh_{j}] = [\frac{39}{24}H_{F} + 1]$$
 (3-767)

where

$$\frac{[13]}{[7]} = [13][7:1], [3][7:5], [4][8:1]$$

ani

$$\frac{4\pi}{2} \left[ \frac{3}{2} \left[ \frac{1}{2} \right]^2 \right] = \begin{bmatrix} 3 \cdot t \\ 0 \cdot t \\ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \right] \\ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \right]$$
 (3-769)

These equations can be solved by the general program mentioned in the second part of Appendix VI.

The transient shears and bending moments on the vehicle are determined from (see also Paragraph 5.1.1.1, Equation 5-54).

$$\{F\} = [K]\{ p\} + [B]\{ p\} \}$$
 (3-770)

$$Y_{i} = \sum_{i < j} F_{j}$$
(3-771)

$$M_i = \prod_{i < j} (x_i - x_j) F_j$$
 (3-772)

The force in the ith shock mount is given by Equation 3-747

$$N_{z} = k_{i} \{ k_{z}(x_{0}) \} \{ \{i\} \} + \{ \bar{x} - x \} = -\{ b \} \}$$
  
+  $c_{\bar{i}} \{ k_{z}(x_{0}) \} \{ \{i\} \} + \{ \bar{x} - x \} = -\{ b \} \}$  (3-773)

From Lagrange's equations for the coordinates, p,, we can obtain

. .

$$[\kappa \Im_{p}] + [\beta \Im_{p}] + [\kappa_{s} \Im_{1}] + [\kappa_{s} \Im_{2} - \kappa_{s} \Im_{p}] + [\kappa_{s} \Im_{2}] + [\kappa_{s} \Im_{2} - \kappa_{s} \Im_{p}] + [\kappa_{s} \Im_{2}] + [\kappa_{s}$$

If we introduce a matrix of all the internal loads (shears, bending moments, and shock mount forces) defined by



$$\{L\} = \left[\frac{\partial L}{\partial F}\right] \{F \in J\} + \left[\frac{\partial L}{\partial \overline{C}} \widetilde{K} \widetilde{C}\right] + \left[\frac{\partial L}{\partial \overline{C}} \widetilde{K} \widetilde{C}\right] + \left[\frac{\partial L}{\partial \overline{C}} \widetilde{K} \widetilde{C}\right]$$
(3-776)

where, as before

(3-777)

The transient loads, expressed in the form of Equation 3-776 can be calculated along with the integration of Equation 3-763 as discussed in the second part of Appendix VI.

#### 3.2 THE MONLINGAR AEROFLASTIC EQUATIONS GOVERNING THE PLANE MOTION OF A FLEXIBLE MISSILE EXECUTING ARBITRATILY LARGE "RIGID-BODY" DISPLACEMENTS

The motivation for including the nonlinearities associated with large displacements of a slender missile stens from the need for calculating the trajectory simultaneously with the calculations of loads (shears and bending moments). In this section, only plane motion is considered; these concepts serve as an introduction to Section 4.0, where the general case is treated.

#### 3.2.1 The Kineratics of the Plane Motion of a Flexible Missile

The geometry of the system considered is shown in Figure 58.



FIGURE 58 INERTIAL AND BODY REFERENCE SYSTEMS

The relation between the two reference systems is given by

$$Y = 123 = \mathbb{I} - 1 = \mathbb{K}$$
 (3-778)

$$k = i \text{wel} + i \text{sec} k \tag{3-779}$$

from which

$$\frac{di}{dt} = -\frac{1}{2}k \qquad (3-780)$$

$$\frac{1}{2L} = \frac{1}{2} \hat{I} \qquad (3-781)$$

The position vector for the x-y-z particle is given by

$$\pi(x,y,z,z) = Pt' + (x,z)t + yj + (z-z)k + p(x,z,z)$$
(3-782)

where  $\mathbb{R}^{-1}$  is the position vector for the center-of-mass of the body.

$$R(t) = \frac{\iint I x_1 u_{z_1} t}{\lim i x_1 u_{z_1} t} \frac{dxdydz}{dxdydz} = F(t) I + F(t) K \qquad (3-783)$$

and x, z are defined by

.

 $\xi(t)$  and  $\zeta(t)$  are the components of  $\mathbb{R}(t)$  resolved in the ground (inertial) reference system. The displacement of particles relative to the  $(\bar{\tau}, j, \bar{\kappa})^{n}$  body" reference system is described by  $\mathfrak{D}(x, z, t)$ . It may be noted that  $(\bar{x}, o, \bar{z})$  are the Lagrangian coordinates of the particle which is at the center-of-sums of the undeformed body.

The velocity of the x-y-z particle is given by

The displacement resolved in the body reference is

$$\beta_{1,2,2} = \beta_{1,2,2,2} + \beta_{2,2,2,2} + \beta_{2,2,2} + \beta_{2,2,2$$

so that

$$\frac{1}{12} = \frac{12}{12}7 + \frac{12}{12}E + \frac{1}{12}E + \frac{1$$

If we denote the velocity of the center-of-mass by 
$$/$$
 , then

$$\frac{dE}{dt} = V \qquad (3-769)$$

and

$$V(x, y, z, t) = V(t + y, z - \overline{z} + y_{\overline{z}} + \frac{1}{2t}) + \frac{1}{2t} + \frac{1}{2t} + \frac{1}{2t}$$

$$= (x - z + y_{\overline{z}} - \frac{1}{2t}) + (3 - 750)$$

## 3.2.2 The Minetic Energy

## The kinetic energy of the body is

$$T = 2 \int \left( \frac{\lambda m}{2 \pi} \right)^{2} \rho^{2} V$$

$$= \frac{1}{2} \iint V \cdot V \left( \frac{\lambda m}{2 \pi} \right)^{2} \rho^{2} V \qquad (3-791)$$

From Equation 3-790

$$\mathbf{v} \cdot \mathbf{v} = \nabla^2 + 2\nabla \cdot \mathbf{v} \quad \frac{\partial \mathbf{k}}{\partial t} + z \cdot z \cdot \dot{z} \cdot \dot{u} + \mathbf{p}_{z} \cdot \dot{u} + \mathbf{p}_{z} \cdot \dot{u} + z \cdot \nabla \cdot \mathbf{k} \quad \frac{\partial \mathbf{k}}{\partial t} - z \cdot z \cdot \dot{z} + \mathbf{p}_{z} \cdot \dot{u} +$$

In order to separate the "rigid-body" and "elastic" motion we will impose the following constraints on the displacement relative to the body-axis reference system:

$$F_{1} = f_{1} = f_{1} = f_{1} = f_{2} = f_{1} = f_{1$$

The first condition is always true when  $\mathbb{R}$  is the position vector to the true center-of-mass for the deformed body. The motive for imposing the second condition is not basic and any further discussion will have to be deferred to Section 4.0 (see Equation 4-18). Equation 3-794 is the "second-moment" of [5] in the same sense that Equation 3-793 is the "first-moment" of [5].

The scalar equations corresponding to Equations 3-793 and 3-794 are

$$\mathcal{C}_{2} = \mathcal{C}_{2} $

$$\iiint P_{z}(x_{1}z_{1}z) P(x_{1}y_{1}z, dx dy dz = 0)$$
 (3-796)

$$\iiint \left( (x-\overline{x}) \beta_{z} - (z-\overline{z}) \beta_{x} \right) \rho(x,y,z) \text{ axdyd} z = 0$$
(3-797)

# By differentiating these expressions, we also have:

.

$$\iiint \frac{\partial p_{x}}{\partial t}(x, z, t) \cdot x, y, z) dx dy dz = 0$$
 (3-798)

$$\iiint \frac{\partial p_{\Xi}}{\partial t}(x, \Xi, t) p(x, y, z) dxdy d\underline{z} = 0$$
(3-799)

$$\iint (x-\overline{x}) \stackrel{\text{def}}{=} i_{x,y,z} dx dy dz = - \iint z - \widehat{z} \stackrel{\text{def}}{=} i_{x,y,z} dx dy d\overline{z} = 0$$
 (3-800)

# Substituting Equation 3-787 into Equation 3-786 using Equations 3-795 through 3-800, we obtain

$$= \frac{1}{2} \left( \frac{1}{2} \right)^{2} - \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} = \frac{1}{2} \frac{1}{2} \frac{1}{2} = \frac{1}{2} \frac{$$

We can resolve  $\mathbb{V}$  in the body reference

$$V = V_{e} \hat{r} + V_{z} k$$
 (3-802)

and we recognize the total mass,

$$M' = \iiint e^{dx} dy dz \qquad ((3-803))$$

and the total "pitch" moment of inertia about the center of mass of the undeformed body

$$I = \iiint \left( (z-z)^{T} + (x-z)^{T} \right)_{\mathbb{R}} dx dy dz \qquad ((3-804))$$

The kinetic energy is then

$$T = \frac{1}{2} \left( M \nabla_{x}^{L} + M \nabla_{z}^{L} + I \tilde{\Theta}^{L} + I \tilde{\Theta}^{L} + I \tilde{\Theta}^{L} + I \tilde{\Theta}^{L} \int \left( (z - \bar{z}) \dot{p}_{z} + (x - \bar{x}) \dot{p}_{x} \right) \dot{p}_{z} - y_{z} dy dz \right)$$

$$+ \frac{1}{2} \tilde{\Theta} \left[ \int (\dot{p}_{x}^{L} + \dot{p}_{z}^{L}) \dot{\theta} dx dy dz \right]$$

$$+ 2 \tilde{\Theta} \left[ \int \dot{p}_{z} \frac{\partial \dot{p}_{z}}{\partial t} - \dot{p}_{x} \frac{\partial \dot{p}_{z}}{\partial t} \right] dx dy dz$$

$$+ \int \int \left( \frac{\partial \dot{p}_{z}}{\partial t} \right)^{2} + \left( \frac{\partial \dot{p}_{z}}{\partial t} \right)^{2} dx dy dz$$

$$(3-805)$$

# 3.2.3 The Approximation of a Finite Number of Degrees-of-Freedom

In order to obtain a rational solution to the general problem, we make a finite degree-of-freedom approximation (see Equations 2-361, 2-363 and also 2-451 in Section 2.3).
$$b_{x}(x, z, t) = \frac{N}{\sum_{i=1}^{N}} h_{x}^{(i)}(x, z) p_{i}(t) = \frac{1}{2} h_{x} \frac{1}{2} \frac{1}{2} p_{i}^{2}$$
(3-806)

$$p_{\overline{z}}(x_{r}z_{r}t) = \sum_{\overline{i}=1}^{N_{r}} h_{\overline{z}}^{(\overline{b})}(x_{r}z) p_{\overline{i}}(t) = \{h_{\overline{z}}\}_{\overline{z}}^{\overline{z}}p_{\overline{i}}\}$$
(3-807)

Rigid body displacements relative to the body reference system are given by

.

• ·

.

.

.

$$b_{x} = i = \pm h_{x} F \pm \psi_{R} F, \qquad (3-808)$$

$$f_{x} = f_{x} + f_{x} + f_{x}$$
 (3-810)

$$\mathbf{s}_{\pm} = t = -\mathbf{h}_{\Xi} + \mathbf{h}_{\Xi}$$
 (3-811.)

$$= -h_{2} - h_{3}$$
 (3-813)

Using these relations we can express the terms in the kinetic energy in terms of the generalized coordinates,  $p_{\pm}$ .

$$\iiint p_{x}^{2} p dx dy dz = \frac{1}{2} p_{y}^{2} \qquad (3-814)$$

$$\iiint p_{e}^{*} dx du de = -p_{f}^{*} \iiint \{h_{E}\} \{h_{E}\} dx dy de + p_{f}^{*}$$
(3-E15)

$$\iiint F_{x} \frac{\partial F_{z}}{\partial t} > t \times dy dz = -\mu F \iiint -h_{x} + h_{z} + u \times dy dz + \frac{1}{2}\mu \}$$
(3-316)

$$\prod_{i=1}^{m} F_{z} \underbrace{\frac{h_{z}}{h_{z}}}_{i=1}^{i=1} t_{z} t_{z} = i p \int_{1}^{n} \underbrace{\frac{h_{z}}{h_{z}}}_{i=1}^{i=1} h_{z} f_{z} t_{z} t_{z} t_{z} dt_{z} = p f_{z}$$
(3-817)

$$\iiint (z-\overline{z}) \beta_{z} e^{dx du dz} = \frac{1}{2} \langle g_{R} j_{z} \parallel j \parallel th_{x} j_{z}^{2} h_{z} j_{z} + j_{z} \qquad (3-818)$$

$$\iiint (x-\tilde{x})p_x \text{ odx} dy dE = -\frac{1}{2} \{p_R\}_3 \iiint \{n_2 \in \mathbb{N} \mid n_2 \in \mathbb{N} \}$$
(3-819)

$$\iiint \frac{1}{2} \frac{$$

$$\iint \left[ \frac{d}{dE} \right]^{p} dx dy dz = \pm p \int \left[ \int t n_{e} - n_{e} \right]^{p} dx dy dz = \pm p \int \left[ \int t n_{e} - n_{e} \right]^{p} dx dy dz = \pm p \int \left[ \int t n_{e} - n_{e} \right]^{p} dx dy dz = \pm p \int \left[ \int t n_{e} - n_{e} \right]^{p} dx dy dz = \pm p \int \left[ \int t n_{e} - n_{e} \right]^{p} dx dy dz = \pm p \int \left[ \int t n_{e} - n_{e} \right]^{p} dx dy dz = \pm p \int \left[ \int t n_{e} - n_{e} \right]^{p} dx dy dz = \pm p \int t n_{e} dx dy dz$$

These equations can be simplified if we introduce

.

$$[A_{xx}] = \iiint \bot_{h_x} \vdash h_x \vdash \dots \qquad (3-822)$$

$$[t_{zz}] = \iint \{h_z + h_z \} \text{ is zyd} z$$
(3-823)

$$[A_{xz}] = \iint \{h_x\} \{h_z\} \} Axtydz \qquad (3-824)$$

.

and the kinetic energy can be written as

$$T = \sum_{k=1}^{N} M_{k}^{2} + M_{k}^{2} + I \frac{\partial}{\partial x_{k}}^{2}$$

$$= \sum_{k=1}^{2} M_{k}^{2} + M_{k}^{2} + I \frac{\partial}{\partial x_{k}}^{2} - I \frac{\partial x_{k}}{\partial x_{k}} - I$$

We can further introduce

.

$$[A] = [A_{xx}] - [A_{zz}]$$
 (3-826)

$$[G_{T}] = \frac{[A_{XZ}] - [A_{XZ}]'}{2}$$
(3-827)

and note that

.

$$[A] = [A]'$$
 (3-328)

and

.

$$[G] = -[G]' \tag{3-829}$$

so that [A] is symmetric and [G] is anti-symmetric.

Using Equations 3-808 through 3-813, it can be shown that

$$M = \{\varphi_{R}\}_{1}^{\prime} [A] \{\varphi_{R}\}_{1} = \{\varphi_{R}\}_{2}^{\prime} [A] \{\varphi_{R}\}_{2}$$
(3-830)

$$I = \{ \varphi_{R} \}_{3}^{'} [A] \{ \varphi_{R} \}_{3}$$
(3-831)

.

It is fairly evident that the [A] matrix is the inertia matrix of small vibration theory (see Equation 2-132 or Section 2.2).

The final expression for the kinetic energy is

$$= \frac{1}{2}MV_{x}^{2} + \frac{1}{2}MV_{z}^{2}$$

$$+ \frac{1}{2}I + \frac{1}{2}\beta F[A]F + \frac{1}{2}V_{x} + \frac{1}{2}$$

where we have introduced  $\Omega_y = \theta$ . 3.2.4 The Strain Energy and the Virtual Work of External Forces

.

The strain energy of the system is assumed to be of the form

$$U = \frac{1}{2} \{ b \} [K] \{ b \} - \frac{1}{2} \{ b \} [N] \{ b \} \}$$
(3-833)

where the second term is the contribution to the strain energy from "column" loading (see Paragraph 3.1.2.5, Equation 3-197). The strains in the body are assumed to be independent of the "rigid-body" coordinates,  $\xi$ ,  $\zeta$ ,  $\theta$ ; so that the discussion of the strain energy in Section 3.1 applies here as well.

The damping in the structure is described by Rayleich's dissipation function:

$$8 = \pm 101[BHp]$$
 (3-934)

External forces are introduced in the virtual work of these forces

$$sw = \{sp\}'\{p\} + ss \mathcal{R} + sf \mathcal{I} + s\omega \Theta \qquad (3-535)$$

which defines the generalized forces associated with the generalized coordinates,  $p_i$  ,  $\xi$  ,  $\zeta$  , and  $-\theta$  .

The generalized coordinates,  $p_i$ , are subject to the following constraints:

$$F_{1} = \xi \psi_{R} \xi' [A J_{2} p J = 0$$
 (3-836)

$$F_{z} = \{q_{R}\}_{z}^{'} [A] \{ j \} = 0$$
(3-837)

$$F_{\rm H} = \{q_{\rm R}\}_{\rm S}^{*} [{\rm A}]_{\rm S}^{*} | {\rm E}_{\rm S}^{*} = 0$$
(3-838)

which follow from Equations 3-795, 3-796, and 3-797.

# 3.2.5 The Equations for Transient Loads

The equations for determining transient loads are derived from the Lagrange equations corresponding to  $p_1, p_2 \cdots p_N$ . From Equation 2-79 of Paragraph 2.1.2.2, we have

$$\frac{\frac{d}{dt}\left(\frac{\partial T}{\partial F_{1}}\right) - \frac{\partial T}{\partial F_{1}} \pm \frac{\partial U}{\partial F_{1}} + \frac{\partial R}{\partial F_{1}} = \prod_{j=1}^{2} \frac{\partial F_{j}}{\partial F_{1}} \lambda_{j} + P_{1}$$
(3-839)

Using Equations 3-333 through 3-338, we obtain

$$[AH\bar{p}] + 2\lambda_{g}[\Delta Hp] + 4\lambda_{g}[\Delta Hp]$$

$$= 0_{\Phi}^{L}[AH\bar{p}] + 2\lambda_{g}[\Delta Hp] h_{g}h_{g}$$

$$+ [KH\bar{p}] - [NH\bar{p}] + [BH\bar{p}]$$

$$= [AH\bar{q}R_{h}h_{g} + [AH\bar{q}R_{h}h_{g}] + [AH\bar{q}R_{h}h_{g}]$$

$$+ \frac{1}{2}P]$$

$$(3-840)$$

Because of the relations

$$[\kappa]_{f_{q_R}} = \{0\}$$
 (3-841)

and

$$[6]_{\{\psi_{\mathcal{H}}\}_{\tilde{t}}} = \{0\}$$
(3-842)

we may eliminate the multipliers  $\lambda_1$ ,  $\lambda_2$ , and  $\lambda_3$  (see Faragraph 2.2.3.4, Equation 2-271). Then

$$[KH_{p}] + [BH_{p}] + 2\bar{\alpha}_{y}[GH_{p}] + 4\alpha_{y}[GH_{p}]$$

$$= - [\Gamma]'[AH_{p}] + 2\bar{\alpha}_{y}[GH_{p}] + 4\alpha_{y}[GH_{p}]$$

$$(3-843)$$

$$(3-843)$$

where

$$[\Gamma] = \Gamma_{1} - [A][\varphi_{R}]'[\varphi_{R}]'[A][\varphi_{R}]]'[\varphi_{R}]' \qquad (3-844)$$

and

$$-\varphi_{R}] = [+\varphi_{P} f_{r}, + \varphi_{R} f_{2}, + \varphi_{R} f_{3}] \qquad (3-345)$$

.

The structural loads (shears and bending moments) can be related to the effective loads,

[KH\$]+[BH\$]

by a transformation, [R], like the one considered in Paragraph 3.1.2.1, Equation 3-79. If we denote the shears, bending moments, and any other pertinent stress resultants by  $L_i$ , then

$$\{L\} = [R]'[K]\{\dot{p}\} + [B]\{\dot{p}\}$$
 (3-846)

or

$$\{L\} = [R][\Gamma] \{P\} - [A][\bar{p}] - 2\dot{a}_{y} [\bar{a}][\bar{p}] \}$$
  
- + 2y [ $\bar{a}$ ][ $\bar{p}$ ] + 2y [ $\bar{a}$ ][ $\bar{a}$ ][ $\bar{a}$ ][ $\bar{a}$ ][ $\bar{a}$ ]]  
- 22y [ $\bar{a}$ ][ $\bar{a}$ ][ $\bar{a}$ ][ $\bar{a}$ ]] (3-847)

# 3.2.6 The Equations for the Trajectory

The equations for determining the trajectory are derived from Lagrange's equations corresponding to the generalized coordinates,  $\xi$ ,  $\zeta$ , and  $\theta$ .

$$\frac{1}{2}\left(\frac{7}{2}-\frac{7}{22}\right) = \hat{\epsilon} \qquad (3-\hat{\epsilon}+\hat{\epsilon})$$

$$\frac{d}{dt}\left(\frac{\partial T}{\partial t}\right) - \frac{\partial T}{\partial t} = \frac{1}{2}$$
(3-849)

$$\frac{d}{dt}\left(\frac{\partial T}{\partial \theta}\right) - \frac{\partial T}{\partial \theta} = \theta$$
(3-850)

Because the kinetic energy (Equation 3-832) is expressed in terms of  $V_X$ ,  $V_Z$ , and  $\Omega_Y$ , it will be convenient to replace the above equations by a set of Lagrange equations for quasi-coordinates (see Paragraph 2.1.2.3).

From the relation,

.

$$V = \frac{dR}{dt} = V_{x} \gamma + V_{z} k = \dot{s} \mathbf{1} + \dot{j} \mathbf{k}$$
(3-851)

and Equations 3-778 and 3-779, we obtain

$$V_x = (\omega \hat{e} \hat{e} - s \hat{u} \hat{e} \hat{e})$$
 (3-852)

We complete the transformation with the trivial definition

$$l_{\rm g} = 2$$
 (3-854)

Equations 3-852, 3-853, and 3-854 are of the same form as Equation 2-81 of Paragraph 2.1.2.3. In the present case we have:

 $x_1 = x_1 + x_2 = -iw^2, \quad x_{13} = 0$   $x_1 = w^2, \quad x_{23} = -i^2\theta, \quad x_{23} = 0$  $x_2 = 1, \quad x_{23} = 1$ 

.

The coefficients defined by Equation 2-106 of Paragraph 2.1.2.3 are

$$\Omega_{\mathbf{x}} = \begin{bmatrix} 1 & 1 \\ 1 & 2 \end{bmatrix} = \begin{bmatrix} 1 \\ 1 & 2 \end{bmatrix} = \begin{bmatrix} 1 \\ 1 & 2 \end{bmatrix} = \begin{bmatrix} 1 \\ 2 & 2 \end{bmatrix}$$

The "quasi-Lagrange equations" (Equations 2-107) corresponding to the quasi-coordinates,  $V_{\rm X},~V_{\rm Z},~$  and  $~\Omega_{\rm Y}$  are then given by

$$\frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} \right) = F_{*}$$
(3-855)

$$\frac{1}{4\pi}\left(\frac{\partial T}{\partial V_{z}}\right) - \mathcal{I}_{y}\left(\frac{\partial T}{\partial V_{z}}\right) = F_{z}$$
(3-856)

$$V_{\pm} \frac{\partial T}{\partial V_{\pm}} - \frac{V_{\pm}}{\partial V_{\pm}} \frac{\partial T}{\partial V_{\pm}} + \frac{d}{dt} \left( \frac{\partial T}{\partial Q_{\pm}} \right) = G_{y}$$
(3-857)

where

$$\mathbf{F} = \lim_{n \to \infty} \mathbf{Z} \qquad (3-853)$$

$$F_{\pm} = \frac{1}{166} + \frac{1}{166$$

$$u_{\rm J} = w_{\rm J} \tag{3-860}$$

From the kinetic energy expressed in Equation 3-332, we obtain

$$\sum_{k=1}^{k} \frac{d^{2}k}{dk} + \frac{1}{2} \frac{d^{2}k}{dk} = F_{*}$$
 (3-B61)

$$\lim_{n \to \infty} \frac{2\sqrt{2}}{2} = -2 \sqrt{2} \sqrt{2} = \frac{1}{2}$$
 (3-862)

# Additional differential equations which govern the trajectory are obtained from Equations 3-852, 3-853, and 3-854.

$$\frac{42}{32} = -\frac{1}{4}$$
 (3-866)

These equations determine the range,  $\xi$ , eltitude,  $\zeta$ , and attitude,  $\theta$ . (See Figure 58.)

# 3.2.7 The Relation between the Total Forces, Fr., Fr., and Gy and the

## Generalized Forces

When the generalized coordinates  $p_1, p_2 \dots p_m$  do not satisfy the constraints (Equations 3-856, 3-837 and 3-838) explicitly, it is possible to derive a convenient relation between the  $P_1$  and the total forces,  $F_X$ ,  $F_X$  and  $G_y$ . The relations which we want to derive in this section are

$$F_{x} = \{ \psi_{R} \}, \{ z \}$$
 (3-867)

$$G_{g} = \frac{1}{4} G_{R} \frac{1}{3} \frac{1}{2} P_{F}^{2} - \frac{1}{4} G_{R} \frac{1}{3} [N]_{F}^{2}$$
 (3-869)

The derivation proceeds as follows. Consider the virtual work of external forces as given in Paragraph 2.1.2.1, Equation 2-48.

$$Sw = \int Su \cdot P dv + \bigoplus Er \cdot \mathbb{C} \cdot ds \qquad (3-870)$$

From Equation 3-782, we have

$$sn = sR + se'(z-z)k + (z-z)l', \qquad (3-871)$$
$$+ se'(z-z)k + sp_z l' + sp_z k$$

and

$$\delta W = SR \cdot i \left[ i \cdot P \cdot W - \frac{H}{2} i \cdot D \cdot IS \right] + SR \cdot k \int P \cdot P \cdot W + \frac{H}{2} i \cdot D \cdot IS \\+ SF \cdot k \int SR \cdot k - \frac{1}{2} - \frac{1}{2} i \cdot P \cdot IS \\+ \frac{H}{2} i \cdot hk - \frac{1}{2} - \frac{1}{2} i \cdot D \cdot IS \\- \int SR \cdot P \cdot k - P \cdot K - P \cdot K \\- \frac{1}{2} - \frac{1}{2} i \cdot D \cdot P \cdot I - \frac{1}{2} - \frac{1}{2} i \cdot D \cdot IS \\- \frac{1}{2} - \frac{1}{2} i \cdot D \cdot P \cdot I - \frac{1}{2} - \frac{1}{2} i \cdot D \cdot IS \\- \frac{1}{2} - \frac{1}{2} i \cdot D \cdot P \cdot I - \frac{1}{2} - \frac{1}{2} i \cdot D \cdot IS \\- \frac{1}{2} - \frac{1}{2} i \cdot D \cdot P \cdot I - \frac{1}{2} - \frac{1}{2} i \cdot D \cdot IS \\- \frac{1}{2} - \frac{1}{2} i \cdot D \cdot P \cdot I - \frac{1}{2} - \frac{1}{2} i \cdot D \cdot IS \\- \frac{1}{2} - \frac{1}{2} i \cdot D \cdot P \cdot I - \frac{1}{2} - \frac{1}{2} i \cdot D \cdot IS \\- \frac{1}{2} - \frac{1}{2} i \cdot D \cdot P \cdot I - \frac{1}{2} - \frac{1}{2} i \cdot D \cdot IS \\- \frac{1}{2} - \frac{1}{2} i \cdot D \cdot D \cdot I - \frac{1}{2} - \frac{1}{2} i \cdot D \cdot IS \\- \frac{1}{2} - \frac{1}{2} i \cdot D \cdot D \cdot D \cdot I - \frac{1}{2} - \frac{1}{2} i \cdot D \cdot IS \\- \frac{1}{2} - \frac{1}{2} i \cdot D \cdot D \cdot D \cdot D \cdot D \cdot IS \\- \frac{1}{2} - \frac$$

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By comparing this with

.

$$\delta W = \delta R \cdot F_x + \delta R \cdot k F_z + \delta G_y + \{\delta p\} \{p\}$$
(3-873)

we conclude that

.

$$F_{x} = \int \dot{\mathbf{r}} \cdot \mathbf{P} d\mathbf{Y} + \oiint \dot{\mathbf{r}} \cdot \mathbf{\Gamma} \cdot d\mathbf{S}$$
 (3-874)

$$F_{z} = \int k \cdot \mathbb{P} dV + \oiint k \cdot \mathbb{L} \cdot ds \qquad (3-875)$$

$$G_{q} = \int \widehat{\gamma}_{\hat{x}-x} \mathbf{k} + (z-\overline{z})\widehat{j} \cdot \mathbf{P} d\mathbf{V} + \widehat{j} \cdot \int \mathbf{p}_{x} \mathbf{P} d\mathbf{v} + \underbrace{\oplus}_{z} \mathbf{F} * \mathbf{\Sigma} \cdot d\widehat{z} + \underbrace{\oplus}_{z-\hat{x}} \mathbf{F} + (z-\overline{z})\mathbf{I} \cdot \mathbf{\Sigma} \cdot d\widehat{z}$$
(3-876)

$$iFF = \int [h_{x}F_{7} - E + h_{2}F_{7} + P_{1}]_{x}$$

$$- \tilde{G} = ir_{x}F_{7} - E - ir_{z}F_{7} - f_{3}$$
(3-877)

Now, consider

.

.

where use has been made of Equations 3-808 and 3-809. In a similar menner

$$\{p_i\}_{i=1}^{i} = \{F_i, F_{i+1}, F_{i$$

.

by using Equations 3-810 and 3-811.

.

.

For the total moment, Gy, we have

It can be shown for the results in Paragraph 3.1.2.5 that in the case of a one-dimensional body:

$$-\int L F_{\pm} - r_{\pm} = r_{\pm} \int c_{\pm} r_{\pm} - r_{\pm} k + k_{\mp} + r_{\pm} k_{\pm} k + k_{\mp} + r_{\pm} k_{\pm} k + k_{\mp} + r_{\pm} k + k_{\pm} + r_{\pm} + r_{\pm} k + k_{\pm} + r_{\pm} +$$

There is some indication that this relation holds for more general considerations than that of a one-dimensional body. On intuitive grounds we speculate that in the present, more general case, we have

$$\int ||\mathbf{p} \times \mathbf{P} d\mathbf{v}| + \oint |\mathbf{p} \times \mathbf{E} \cdot d\mathbf{S}| \cdot \mathbf{j} = -\{q_{\mathbf{p}} \mathbf{f}_{\mathbf{z}}' | \cdot \mathbf{F} \mathbf{p}\}$$
(3-882)

where [N] is defined in Equation 3-833 and from Equation 3-880

$$f_{4\pi}f_{3}f_{5}F_{5} = \tilde{x}_{4} + f_{4\pi}f_{5}[N]f_{5}F$$
 (3-883):

In summery, we have

$$F_{\mathbf{x}} = f \varphi_{\mathbf{x}} \xi'_{\mathbf{z}} p \xi \qquad ((3-884))$$

$$F_{z} = \{ \phi_{R} \}_{z} \{ P \} \qquad (3-885)$$

$$G_{\mu} = \{ \varphi_{\mu} f_{a}^{'} \{ p \} = - \frac{1}{4\pi} f_{a}^{'} [n] \}$$
 (3-886)

### 3.2.8 Detailed Description of External Forces

In each of the sections below the generalized forces will be derived from the virtual work of external forces from one of sevenal sources. The separate expressions for the generalized forces will be combined in Faragraph 3-2-9.

### 3.2.8.1 Aerodynamic Forces

As in Feregraph 3.1.1, we will assume the aerodynamic forces to be sufficiently described by the "quasi-steady" assumption. The virtual work of the aerodynamic forces is then

$$\exists n = -\frac{1}{2} a a b \int d \mathbf{k} - \frac{1}{2} \mathbf{r} \cdot \mathbf{r} \mathbf{p} \, \mathbf{x} \qquad (3-887)$$

where  $C_{\rm L}$  and  $C_{\rm D}$  are local drag and lift coefficients per unit of length along the vehicle. The "free stream" direction is arbitrarily taken as parallel to the  $\ddot{y}$  -axis, so that

$$v_{\alpha} = \left( \frac{W}{dx} - \frac{\chi R}{dx} \right) - \dot{R}$$
 (3-888)

where W is the wind vector.



FIGURE 59 AERODYNAMIC ASSUMPTIONS

If  $\mathbb H$  and  $\mathbb K$  are unit vectors parallel and normal to the zero angle-of-attack axis at each point along the body, then the local angle-of-attack is

.

$$\alpha_{(x+t)} = -\frac{\left(\frac{\partial m}{\partial t} - W\right) \cdot Ik}{\left(\frac{\partial m}{\partial t} - W\right) \cdot Ik}$$
(3-889)



FIGURE 60 LOCAL ANGLE-OF-ATTACK

From Figure 60 we can write

$$\mu = \dot{\ell} + \frac{\partial k}{\partial x} k \qquad (3-890)$$

$$\kappa = -\frac{\partial k_z}{\partial x}\dot{y} + k$$
(3-891)

and also

$$W = W_{\chi} \dot{v} + W_{\chi} k \qquad (3-892)$$

Substituting these into Equation 3-889 and using Equation 3-786, we obtain

$$\mathcal{A}(x,t) = -\frac{\left(\overline{\mathbb{V}} - \overline{\mathbb{W}}\right) \cdot \dot{k}}{\left(\overline{\mathbb{V}} - \overline{\mathbb{W}}\right) \cdot \dot{\gamma}} + \frac{\partial \dot{p}_{z}}{\partial x} + \frac{\frac{\partial \dot{p}_{z}}{\partial t} - (x - \bar{x})\dot{\phi}}{\left(\overline{\mathbb{V}} - \overline{\mathbb{W}}\right) \cdot \dot{\gamma}}$$
(3-893)

•

where the assumption has been made that the angle-of-attack is small. If we introduce the "rigid-body" angle-of-attack,

$$\alpha = -\left(\frac{V-W \cdot k}{(V-W) \cdot i}\right)$$
(3-894)

then

.

$$\mathcal{L}(x, \mathbf{r}) = \mathcal{X} + \frac{\overline{\lambda} - \mathbf{Y}_{1}}{\mathbf{v}_{C}} \stackrel{!}{\Rightarrow} + \frac{\mathbf{b}\mathbf{p}_{c}}{\mathbf{b}\mathbf{x}} + \frac{\mathbf{I}}{\mathbf{v}_{C}} \frac{\mathbf{b}\mathbf{p}_{c}}{\mathbf{b}\mathbf{t}}$$
(3-895)

The aerodynamic coefficients,  $C_D$  and  $C_L$ , are assumed to be a function only of the local angle-of-attack. Linearized aerodynamics would predict these to be

$$z_{2} = a \text{ constant}, \text{ independent of } \alpha(x,t)$$
 (3-896)

$$C_{L} = \int L(x,s) \alpha(s,t) ds + C_{L_{0}}(x)$$
 (3-897)

.

At high Mach numbers,  $M_{co} > 3$ , the kernal, L (x,  $\xi$ ), approaches the form

$$L(x, g) = \frac{1}{2\pi} (x; \bar{C}(x-g))$$
 (3-898)

where  $\beta$  (x) is the Dirac function. Equation 3-897 is then replaced by

$$C_{L} = \frac{\overline{z_{LL}}(x)}{\partial z}(x) + \overline{z_{LJ}(x)}$$
 (3-899)

For the more general case we have

$$\exists w = -i j_{z} i_{z}^{2} \int Sp_{z}(x,z) L_{x}(z) dz St j dz dx$$

$$= \int Sp_{x} L_{y} dx$$

$$= \int Sp_{z} L_{y} dx$$
(3-900)

Using Equations 3-806, 3-807, 3-811, and 3-813, we can write

$$\varepsilon_{r_x} = -\varepsilon_{r_x} + \varepsilon_{r_x} + (3-901)$$

$$SF_{2} = -SE_{1}F_{2}F_{2}$$
 (3-902)

$$\frac{1}{2} = \frac{1}{2} \frac{$$

## Substituting these into Equation 3-900 gives

where

$$[1 + ] = \iint \{ \frac{1}{2} + \frac{1}{2} \times \frac{1}{2} \times \frac{1}{2} + \frac{1}{2} \times \frac{1}{2} \times \frac{1}{2} \times \frac{1}{2} + \frac{1}{2} \times \frac$$

$$[L_{\Sigma}] = ((-h_{\Xi}(x)) + L(x, S) + h_{\Xi}(S) + dS dx$$
 (3-906)

$$i_{-2} \bar{s} = \int c_{\alpha}(x) f h_{x} F dx + \int c_{\alpha}(x) f h_{z} s dx \qquad (3-907)$$

and also from Equations 3-888 and 3-894, we have

.

$$v_x = W_x - V_x$$
 (3-908)

$$x_{z} = -\frac{W_{z} - V_{z}}{W_{x} - V_{x}}$$
(3-909)

The components of wind in the body axis are calculated from

$$M_{\rm H} = 1.57 \oplus M_{\rm H} = 1.04 \oplus M_{\rm H}$$
 (3-910)

$$W_{e} = m = W_{r} + m = W_{r}$$
 (3-911)

where  $W_{\xi}$  is the down-range component of wind parallel to the earth's surface and  $W_{\xi}$  'is the "up-draft" component normal to the earth's surface



# FIGURE 61 WINDS RESOLVED IN THE INERTIAL REFERENCE SYSTEM

The vinds,  $W_{\xi}$  and  $W_{\zeta}$ , and the density,  $\rho_{w}$ , are assumed to be known functions of altitude,  $\zeta$ . The sound speed,  $C_{w}$ , is also a function of and is mediad for the calculation of Mach number,

$$M_{\rm m} = \frac{V_{\rm m}}{\zeta_{\rm m}} \tag{3-912}$$

3.2.8.2 Gravity Forces

The gravity field will be assumed to vary with altitude but be uniformly parallel to the inertial axis,  $\mathbb K$  .



FIGURE 62 APPROXIMATIONS TO INVERSE SQUARE GRAVITY FIELD The force per unit of mass would be

$$\frac{G_{\rm E}M_{\rm c}}{(J+R)^2}$$

where G is the universal gravitational constant , is the mass of the gravitating body, and R is the radial distance from the center of the earth to the origin of the ( $\xi, \, \zeta$ ) coordinates.

If we introduce the local acceleration of gravity, g, at the origin of the (  $\xi,\zeta$  ) coordinates, then

$$y = \frac{GW}{R^2}$$
 (3-913)

and we can write the gravity vector as

If we assume this acts uniformly over the entire vehicle, we are neglecting the "gravity gradient torque." The virtual work of the gravity force is then

$$z_{k} = \int \frac{1}{1 + z_{k}} z_{k} + z_{k} + z_{k} z_{k} + z_{k$$

Using Equations 3-806, 3-807, 3-808 and 3-811, we have

$$\int \mathcal{E}_{x,y} dx = \{ \mathcal{E}_{p} \} [A \quad h \notin k \}$$
(3-915)

$$\int Sp_{2,p} dV = +Sp_{1,p} [A ]_{Sp_{2,p}}$$
 (3-916)

Also, from Equations 3-778 and 3-779

$$\mathbf{K} \cdot \mathbf{i} = -\mathbf{s} \mathbf{u} \mathbf{g} \tag{3-917}$$

and

$$k \cdot k = \cos \theta \qquad (3-918)$$

We then have

$$\varepsilon_{W} = -\frac{2}{\left(1+\frac{1}{K}\right)^{2}} \left\{\varepsilon_{F}\right\} \left[A\right] \left(\frac{1}{1+\frac{1}{K}}\right)^{2} \left(1+\frac{1}{K}\right)^{2} \left(1+\frac{1}{$$

### 3.2.8.3 Thrust Forces

To allow for complete generality, we assume that the distributed thrust on each particle of the system is given by a function of x, y, z, and t as well as the generalized coordinates,  $p_1$ ,  $p_2$ ... $p_N$ . It is generally not a function explicitly of  $\xi$ ,  $\zeta$ , or  $\theta$ .

$$= \exists (x, y \in t; p, p \dots p_n)$$
 (3-920)

The dependency on the generalized coordinates is included to allow for the fact that the local thrust vector is redistributed when the body is distorted. In addition, for a ginbaled engine, the thrust depends on the generalized coordinates describing the engine swiveling.

The total thrust force is

$$T = \int \nabla \vec{x}.$$
 (3-921)

and the virtual work of the thrust forces is

$$z_{ii} = \int z \cdot z_{ij} \, \mathcal{X}_{ij} \tag{3-922}$$

On the basis of the "smallness" of the generalized coordinates, we may write:

$$\mathcal{I} = \sum_{i=1}^{N} \frac{3\mathcal{I}}{2\mu_{i}} (x_{i} q_{i} z_{i} \tau_{i} \tau_{$$

Since

$$f(c = -f(z) + f(z) +$$

we can write

$$S_{N} = i s_{P} \int \int \sum_{k=1}^{\infty} \sum_{j=1}^{\infty} \sum_{j=1}^{\infty} \sum_{j=1}^{\infty} \sum_{k=1}^{\infty} \sum_{j=1}^{\infty} \sum_{k=1}^{\infty} \sum_{j=1}^{\infty} \sum_{j=1}^$$

where

.

The above matrices are generally functions of time and the ambient pressure,  $p_{\rm T}$  . If the ambient density and speed of sound are assumed to be a function of altitude only, then

$$z_{2} = \frac{1}{2} \frac{1}{2}$$
 (3-928)

where 7 is the adiabatic constant and

$$j_x = j_x(x)$$
 (3-929)

## 3.2.8.4 Control System Forces

We will assume that there is only one control coordinate,  $\gamma$ , which may be the rotation of a shaft supporting a jet vane, or the gimbal angle of a sviveling engine, or in a generalized case it might be the displacement of a valve controlling the flow rate in a fuel injection mechanism for thrust vectoring. It might also be the rotation at the hinge-line of an aerodynamic surface.

In any case, the control coordinate can be related to the generalized coordinates describing the configuration of the system:

$$f = \frac{1}{2} \int $

For the case of the fuel injection system, the mechanical generalized coordinates,  $p_i$ , must include the description of the valve position; and the variation of thrust with valve position must be included in Equation 3-925.

The control force from the servo is defined by the virtual work of the servo in a virtual displacement of the control coordinate, 7.

The statement describing how  $\Gamma$  depends on the outputs of sensors in the control system is called the "control law" and this is discussed in Paragraph 3.1.3.4.1

If Equation 3-951 is substituted into Equation 3-932, we could

$$x_{4} = -3x_{5}^{2} + 3x_{5}^{2}$$
 (3-933)

<sup>&</sup>lt;sup>1</sup> A clear statement of this definition of control law, in a sufficiently general form for use in flexible body analysis, appears to have first been given in The Dynamic Response of Advanced Vehicles, WADD TR 60-518, by Q. R. Bohme, et. al. Sept. 1960, Appendix C. Section 22, p. 176.

so that  $\{\eta_{\gamma}\}\Gamma$  are the generalized forces contributed by the control servo.

2

# 3.2.9 Summary of Equations of Motion

.

From the virtual work expressions of the previous section, the total generalized forces from aerodynamics, thrust, gravity and control are given by

$$sw = \{sp\}\{P\}$$
 (3-934)

where

.

$$\frac{1}{2} = -\frac{1}{2} \sum_{x} \frac{1}{x} \left( \frac{1}{2} \sum_{R} \frac{1}{R} p_{R}^{2} - \frac{1}{2} \sum_{x} \frac{1}{2} \sum_{x} \frac{1}{(1 + \frac{1}{2})^{2}} + \frac{1}{2} \sum_{x} \frac{1}{(1 + \frac{1}{2})^{2}} + \frac{1}{2} \sum_{x} \frac{1}{(1 + \frac{1}{2})^{2}} + \frac{1}{2} \sum_{x} \frac{1}{2$$

If we note that

which follows from

$$[U_R][A][b] = \{0\}$$
 (3-937)

then we can rewrite Equation 3-843 as

.

$$[A H \dot{p} + [6 H \dot{p} + [K H \dot{p}]$$
  
= [r]  $\{P\} - 2\lambda_{y} [G] \hat{p} + -4\lambda_{y} [G] \hat{p} \hat{p}$   
-  $\lambda_{y} [A H \dot{p} + 2\lambda_{y} [G] \hat{p} \hat{q}_{y} - [N H \dot{p}]$ (3-938)

.

These equations must be integrated simultaneously with (see Equations 3-861, 3-862, 3-863, 3-884, 3-885, and 3-886).

$$\frac{dV_x}{dt} = -\frac{1}{2}yV_z + \frac{1}{M} \frac{1}{2}\frac{1}{2}z^2 \frac{1}{2}$$
(3-939)

$$\frac{dV_{z}}{dt} = -\frac{1}{2}V_{x} + \frac{1}{2}\frac{1}{10}\frac{1}{2}$$
 (3-940)

$$\frac{d^{2}}{dt} = \frac{1}{2} \frac{1}{$$

where  ${\rm H}_{\rm y}$  is the y-component of the total angular momentum,

In addition, we have the differential equations

.

.

$$\frac{45}{44} = \omega_{10} V_{\chi} + \omega_{10} V_{\xi} \qquad (3-943)$$

.

$$\frac{t}{tt} = -z_{xx} + z_{xz} + z_{zz}$$
(3-944)

$$\frac{4\pi}{44} = \frac{1}{12}$$
 (3-945)

### 3.2.10 Control System Equations

Let  $\epsilon$  be a signal to the serve to command a control displacement  $\gamma$ . This signal is assumed to depend on the sensors' estimate of the vehicle's attitude, acceleration, and/or angle-of-attack. It also depends on the configuration of the vehicle that is programmed by the guidance system. The particular equation relating  $\epsilon$  to the vehicle motion can vary widely in form depending on the choice of type and number of sensors and how the sensed signals are mixed and filtered to achieve a stable system. A fairly representative expression for an attitude control system is a simple rateplus-displacement feedback:

$$\epsilon = \kappa_{\mathcal{O}_{\mathcal{O}_{\mathcal{O}}}} - \mathfrak{V}^{-1} + \kappa_{\mathcal{O}_{\mathcal{O}_{\mathcal{O}}}}$$
(3-946)

where  $\vartheta$  is a prescribed function of time for the vehicle attitude,  $\theta_{\rm D}$  is the sensed attitude at the environment of a displacement gyro, and  $\theta_{\rm R}$  is the sensed attitude at the environment of a rate gyro. The gains,  $K_{\rm D}$  and  $K_{\rm R}$ , are constants. For "perfect" gyros:

$$\varphi_{0} = \varphi_{-\frac{1}{2\chi}} \sum_{k=r_{0}}^{k} (3-947)$$

$$\frac{1}{2} = \frac{1}{2} - \frac{1}{2} $

where  $x_D$  and  $x_R$  are x coordinates of the displacement and rate sensors, respectively. Using Equations 3-947 and 3-948, we have

In this report, an attempt will be made to generalize the above expression to angle-of-attack feedback or other schemes of stabilization, but it must be stressed that any particular scheme may be easily expressed and, in any case, involves only one equation for the variable,  $\epsilon$ . This equation may be an integro-differential equation when the dynamical characteristics of the sensors are included.

As in Paragraph 3.1.3.4, we may express the moment developed by the servo as

$$\vec{I}(3) = I_{ab} \left( \vec{G}(3) \vec{E}(3) - \vec{F}(3) \right)$$
(3-950)

where the bars denote Laplace transforms and I(s) is the "power control impedance" and G(s) is the "no-load servo impedance." One of the simplest forms of the above equation is

$$\Gamma = \operatorname{Jep}(\epsilon - r) \tag{3-951}$$

where  $J\omega_{\gamma}^2$  is the "zero-frequency back-off stiffness" of the serve actuator (see Paragraph 3.1.3.4, Equation 3-462).

If we let  $h_i$  be one of a number of rigid-body parameters, then we might, for example, have

$$h_1 = 4 - t^2$$
  
 $h_2 = \frac{3}{2}$   
 $h_3 = \frac{3}{2}$   
 $h_4 = \frac{3}{2} - \frac{3}{2} t_4$   
(3-952)

and Equation 3-949 can be generalized to

$$\bar{z} = -\frac{1}{2} \frac{1}{2} \frac{1}$$

where, again, the bars denote Laplace transforms.

The general form of the control law is then given by

$$\vec{\Gamma} = I_{(3)} / G_{(3-954)}$$
 (3-954)

where

.

$$\vec{e} = \{ \vec{k} \in \vec{k} \} + \{ \vec{k} \in \vec{k} \}$$
(3-955)

I(s), G(s),  $K_{I}(s)$ ,  $L_{I}(s)$  are generally rational functions of s.

## 3.2.11 The Transformation to Modal Coordinates

In order to reduce the number of degrees-of-freedom (and thus the number of differential equations to be integrated), it is expedient to make a transformation to modal generalized coordinates. It will prove convenient in this transformation for the control coord.,  $\gamma$ , to appear explicitly. The vibration modes for locked control are governed by the following equations

$$ITI(E)[T](A H \phi F = X H \phi F) \qquad (3-956)$$

where  $[\Sigma]$  is the influence matrix for the vehicle with locked controls. We can use the solutions of this equation together with the control mode to form a complete transformation to generalized modal coordinates

wasre

and  $\{\mathcal{O}_{i}\}$  is assumed to be orthogonal to the other rigid body modes so that

$$\{4_{2}, 5_{1}, 5_{2},$$

In transforming to model coordinates, we will make one modification which will greatly simplify the equations from a machine computations stanapoint. For this purpose, we will redefine  $\{P\}^2$ , the generalized forces, so that they include the axial load contribution and the nonlinear part of the inertia forces. Thus,

$$\begin{aligned} \{p\} &= -\frac{1}{2} \frac{1}{2} \sqrt{2} \left( [L_{P}H_{P}] + \frac{1}{V_{D}} (L_{T}] \frac{1}{2} \frac{1}{p} \right) + \frac{1}{2} L_{D} + \frac{1}{2} \left( L_{T}H_{P} \frac{1}{p} \frac{1}{p} + \frac{1}{V_{D}} \right) \\ &- \frac{1}{(1+\frac{1}{2}N_{P})} \left[ A \right] \left( \frac{1}{2} \sqrt{2} + \frac{1}{2} - \frac{1}{2} \sqrt{2} + \frac{1}$$

If Equation 3-957 is substituted into Equation 3-938 and that equation is premultiplied by  $[\phi]',$  we obtain

$$[\varphi]^{t}[A][\varphi]H\ddot{\varphi}H + [\varphi]^{t}[AH\varphi\bar{z}\bar{z}]$$

$$+ [\varphi]^{t}[B][\varphi]H\dot{\varphi}H$$

$$+ [\varphi]^{t}[B][\varphi]H\dot{\varphi}H$$

$$+ [\varphi]^{t}[K][\varphi]H\dot{\varphi}H$$

$$= [\varphi]^{t}[F]H\bar{z}H$$

$$= [\varphi]^{t}[F]H\bar{z}H$$

If Equation 3-938 is premultiplied by  $\{\phi_\gamma\}'$  , we obtain

.

$$i\varphi_{\mathcal{F}} f[A][\varphi F \ddot{q}] + i\gamma_{\mathcal{F}} f[A F \varphi_{\mathcal{F}}] \ddot{\mathcal{F}} = i\varphi_{\mathcal{F}} f[\Gamma F \rho]$$
 (3-962)

We note that

.

$$+ \frac{1}{2} y_{2}^{2} [\Gamma] = + \frac{1}{2}$$
 (3-963)

.

and

and solve for  $\ddot{\gamma}$  in Equation 3-962 and substitute it into Equation 3-961 . We then obtain

(3-965)

where

and

Also, Equation 3-962 can be written as

where

One of the advantages of the transformation to modal coordinates is that the approximation of a perfect serve (see Paragraph 3.1.3.4, Equation 3-484) can be handled without restricting the generality of the equations. This is important because the conditions where the perfect serve assumption is valid (Equation 3-481) are equivalent to a high frequency associated with the Lagrange equation for the generalized coordinate,  $\gamma$ , (Equation 3-970). In the numerical integration of the differential equations of motion, the stepsize will be dictated by this frequency and will slow down the integration to the extent of making the procedure unfeasible.

When the perfect serve assumption is made, Equation 3-970 is ignored and that equation for determining Y is replaced by

$$\dot{x} = \dot{\varphi}(a) \in (3-972!)$$

and  $\Gamma_{p}$ , the actuator moment, is either calculated as zero from Equation 3-954 or set to zero automatically.

The equation for determining loads (Equation 3-847) can be rewritten as

$$\frac{1}{2} = [R \Pi r I (IPI - [A \Pi \phi H] I - [A H \phi_{\theta} I] F)$$
(3-973)

where {P} is given by Equation 3-960. We may use Equation 3-970 to eliminate y with the result that

$$-1 + = CP I + Try I (2P + -1 + Ty + 2)$$
 (3-974)

where  $\left[ \Gamma_{v} \right]$  given by Equation 3-969.

Finally, the control system equations in terms of model coordinates are summarized in the "control law"

$$F = II_{1} + I_{1} + II_{1} + II_{2} + II_{3}$$
 (3-975)

The convolution theorem offers the formal solution to this equation as

$$\Gamma = \int_{-1}^{1} \frac{1}{2} \left[ \int_{-1}^{1} \frac{1}{2} \left[ \frac{1}{2} \left( \frac{1}{2} - \frac{1}{2} \right) + \frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \left( \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) + \frac{1}{2} \left( \frac{1}{2} + \frac$$

which can be made the basis for a scheme to include a general control system description in a computer program describing Launch vehicle dynamics.

For the single representation of an attitude control system mentioned carlier, we have

$$F = J\omega_{\mu}^{2} \kappa_{D}(\theta - \vartheta) + J\omega_{\mu}^{2} \kappa_{R} \dot{\oplus} - J\omega_{\mu}^{2} \vartheta$$

$$- J\omega_{\mu}^{2} \kappa_{D} \frac{dh_{R}}{dx}(x_{D}) \dot{F}[\dot{\varphi} F_{q}] f$$

$$- J\omega_{\mu}^{2} \kappa_{R} \frac{dh_{R}}{dx}(x_{R}) \dot{F}[\dot{\varphi} F_{q}] f$$
(3-977)

A summery of the equations derived in this section is given in Figure 63.

### 3.2.12 The Quast-Rigid Approximation

The equations derived in this section can be greatly simplified if the quasi-rigid assumption is made (see Paragraph 3.1.2.3). In the present equations this approximation is:

$$\{\tilde{q}\} = \{\tilde{q}\} = \{o\}$$
 (3-978)

With this approximation the more significant nonlinear inertia terms are zero. We further neglect the terms

by comparison with I, the undeformed vehicle moment of inerties. We then have

$$+y = I - iy$$
 (3-980)

If we recognize that

$$[r] = r'_{\lambda,1} \qquad (3-981)$$

the equations of motion become

$$M\left(\frac{d\nabla_{\mathbf{x}}}{dt} + \frac{1}{2}, \nabla_{\mathbf{z}} - \frac{1}{(1+d_{\mathbf{x}})^2}\right) = \{\varphi_{\mathbf{z}}\}^{-1}[\mathbf{p}\}$$
(3-982)

$W_{x} = \cos \Theta W_{\xi} - \sin \Theta W_{f}$ $W_{z} = \sin \Theta W_{5} + \cos \Theta W_{5}$ $IpI = [q]IqI + Iq_{3}I d$ $IpI = [q]IqI + Iq_{3}I d$	$\begin{split} & \frac{1}{2} - \sin \Theta \left\{ \phi_{5}  t \right\} - 2  \dot{\Omega}_{4} \left[ G  1 \xi  \beta \right\} - 4  \Omega_{4} \left[ G  1 \xi  \dot{\beta} \right] \\ & + \Omega_{4}^{1} \left[ A  1 \xi  \beta  \frac{1}{2} - 2  \Omega_{4}^{2} \left[ G  1 \xi  \phi_{0}  1 \right] \\ & + \Omega_{4}^{1} \left[ A  1 \xi  \beta  \frac{1}{2} + 2  \Omega_{4}^{2} \left[ G  1 \xi  \phi_{0}  1 \right] \\ & - 1  0  1 \xi  \beta  1 + 1  \Omega_{2}  1  \Gamma \\ & - 1  0  1  1  \Omega_{2}  \Omega_{1}^{2}  1 + 1  \Omega_{2}  1  \Gamma \\ & \left[ \eta_{4}  1  \alpha + \frac{1}{4}  \phi_{0}  \frac{1}{2}  \frac{\Omega_{4}}{10} + \frac{1}{2}  \frac{1}{2}  \dot{\beta}  1  \right] \right] - \frac{1}{2} \rho_{0}  \Omega_{4}^{2}  1  L_{0}  J \end{split}$	
$V_{nn} = W_s - V_s$ $\alpha_r = - \frac{W_s - V_s}{W_s - V_s}$ $H_{H_1} - 2 \frac{1}{2} \frac{1}{2} \frac{1}{3} 1$	$\frac{1}{3} \int \frac{1}{4} \left[ P_{1}^{2} = -\frac{1}{(1+\frac{1}{2})^{2}} \left[ A_{1}^{2} \right] \left( \cos \frac{1}{2} q_{1}^{2} - \frac{1}{2} H_{0} - \frac{1}{2} H_{0} - \frac{1}{2} H_{0} \right) - \frac{1}{2} \left[ e_{0} v_{0}^{2} \left( \left[ L_{R} H p_{3}^{2} + \left[ L_{L}^{2} \right] \right] \right]$	$\{L\} = [R][\Gamma][\Gamma_{2}](\{P\} - [A][\phi]\{\dot{q}\})$
$M\left(\frac{dV}{dt}x + rt_{t}V_{z}\right) = \frac{1}{2}q_{z}^{2}f^{2}P^{2}$ $M\left(\frac{dV}{dt}x + rt_{t}V_{z}\right) = \frac{1}{2}q_{z}f^{2}f^{2}P^{2}$ $M\left(\frac{dV}{dt}x + rt_{t}V_{z}\right)$	$M \left[ i \ddot{y} \dot{y} + \left[ R \left[ i \dot{y} \dot{y} \right] + \left[ F \right] \left[ f \dot{y} \right] \right] = \left[ \psi \right] \left[ \left[ r \right]_{\tau} \left[ F \right] \right] \left[ \psi \right]$ $J \frac{d^2 \dot{y}}{dt^2} = i \left\{ \psi_{\mu} \right\}^{\prime} \left( \left[ F p \right] - \left[ A \right] \left[ \psi \right] \right] \left[ \ddot{y} \right]$ $\frac{d \delta}{dt} = \cos \theta V_x + \sin \theta V_{\epsilon}$ $\frac{d \delta}{dt} = -\sin \theta V_x + \cos \theta V_{\epsilon}$	hu = th

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FIGURE 63 EQUATIONS FOR PLANE MOTION OF A FLEXIBLE LAUNCH VEHICLE

,

$$M\left(\frac{dV_{z}}{dt} - \dot{\Theta} V_{x} + \frac{q\cos\theta}{(1+x_{0})^{2}}\right) = \{\varphi_{s}\}\left(2\right)$$
(3-983)

$$I \frac{d^2 \Theta}{dt^2} = \{\varphi_{\Theta}\}^{2}\{P\}$$
 (3-984)

$$J \frac{d^{2} d^{2}}{dt^{2}} = \{ \varphi_{p} \}^{2} \{ p \}$$
(3-985)

$$\{q_{3} = r_{\lambda} [\phi] (r_{3} H P) \}$$
 (3-986)

.

$$\frac{ds}{dt} = \cos \theta V_x + \sin \theta V_z \qquad (3-987)$$

$$\frac{dr}{dt} = -j\omega \Theta V_x + j\omega \Theta V_z \qquad (3-988)$$

The expressions for the forces reduce to

.

$$\begin{aligned} \{\rho\} &= -\frac{1}{2} e^{\gamma} v_{\omega}^{2} \left[ L_{z} \right] \left[ \frac{1}{2} \psi_{z} \frac{1}{2} \alpha + \frac{1}{2} e_{0} \frac{1}{2} \frac{9}{\sqrt{\omega}} \right] - \frac{1}{2} e^{\gamma} v_{\omega}^{2} \left[ L_{0} \right] \\ &- \frac{1}{2} H_{0} \frac{1}{2} - \left[ (H] + [N] + \frac{1}{2} e^{\gamma} v_{\omega}^{2} \left[ L_{R} \right] \right] \left[ (\psi]_{1} \frac{1}{2} \frac{1}{2} + \frac{1}{2} \psi_{z} \frac{1}{2} \frac{9}{2} \right] \\ &+ \frac{1}{2} \eta_{z} \frac{1}{2} \Gamma \end{aligned}$$

$$(3-989)$$

and the equation for internal loads reduces to

$$\{L\} = [R][\Gamma][\Gamma_{+}]\{P\}$$
 (3-990)

The simple attitude control law (Equation 3-971) becomes

$$f' = J\omega_{\mathcal{J}}^{*} \left( \kappa_{D}(\partial - \vartheta) + \kappa_{R} \dot{\partial} - \mathcal{F} \right)$$

$$- J\omega_{\mathcal{J}}^{*} \kappa_{D} f \frac{dh^{*}}{dx} (x_{D}) \dot{\mathcal{F}} [\varphi] f g f$$
(3-991)

When  $\omega_\gamma \to \infty$  , the perfect servo assumption, becomes valid and Equation 3-985 is replaced by

.

$$\mathcal{Y} = \kappa_{\mathrm{D}}(\Theta - \vartheta) + \kappa_{\mathrm{R}}\dot{\Theta} - \kappa_{\mathrm{D}}\left\{\frac{\mathrm{d}h_{\mathrm{P}}}{\mathrm{d}x}(x_{0})\right\}\left[\varphi + \varphi\right] \qquad (3-992)$$

.

and  $\Gamma$  is set to zero in Equation 3-991.

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4.0 THE DYNAMICS OF AN UNRESTRAINED ELASTIC STRUCTURE IN GENERAL SIX-DEGREE-OF-FREEDOM MOTION

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### 4.1 THE EQUATIONS OF MOTION OF A SINGLE ELASTIC BODY CONSTITUTING A FIXED SET OF PARTICLES AND EXECUTING LARGE "RIGID BODY" MOTIONS

The object of this section is to derive a practical and general set of equations for an unrestrained elastic body in general motion. The theory presented here is a rational generalization of the work of Euler on the motion of a rigid body and of the work of Lagrange on the small motions of a body having any number of degrees-of-freedom. We have discussed Lagrange's theory in Section 2.2 leading to Equation 2-140.

$$[A H p] + [B H p] + [\kappa] \{p\} = \{p\}$$
 (4-1)

Euler's theory leads to the following equations

$$M\left(\frac{dV_x}{dt} - \Omega_z V_q + \Omega_q V_z\right) = F_x$$

$$M\left(\frac{dV_x}{dt} + \Omega_z V_x - \Omega_x V_z\right) = F_q$$

$$M\left(\frac{dV_z}{dt} - \Omega_q V_x + \Omega_x V_q\right) = F_z$$

$$I_x \frac{d\Omega_z}{dt} - \left(\frac{1}{2zz} - \frac{L_{qq}}{2z}\right) \cdot \left(\frac{1}{q} - \Omega_z\right) = G_x$$

$$I_{qq} \frac{d\Omega_q}{dt} + I_{qq} - I_{zz}\right) \Omega_x \Omega_z = G_q$$

$$I_{qq} \frac{d\Omega_q}{dt} - \left(\frac{1}{2zz} - \frac{L_{qq}}{2z}\right) \cdot \left(\frac{1}{q} - \frac{1}{2zz}\right) = F_z$$

$$I_{qq} \frac{d\Omega_q}{dt} = I_{qq} - I_{qz} \cdot \Omega_q = G_q$$

where

 $(V_{\rm X},~V_{\rm y},~V_{\rm z})$  are the body-fixed components of the velocity of one center of mass

 $(\Omega_{\rm X},\Omega_{\rm Y},\Omega_{\rm Z})$  are the body-fixed components of the angular velocity of the body

 $(F_X,\ F_Y,\ F_Z)$  are the body-fixed components of the total force  $(G_X,\ G_Y,\ G_Z)$  are the body-fixed components of the total moment of forces about the center of mass

Our interests are in the case where neither of these theories is valid but both are obtained as special cases of a more general theory to be developed.

The results of this section will be used in Section 4.2 to develop a general set of equations describing launch vehicle dynamics.

Following the development in Faragraph 2.1.1 of this report, we will consider that each of the continuum of particles of the body is labeled with coordinates (x, y, z) which correspond to the rectangular coordinates of the point occupied by the x-y-z particle at time, t = 0. The Lagrangian coordinates of the particles on the boundary of the body satisfy the equation, say,

$$f'(x_i,y_i,z_i) = 0 \tag{4-3}$$

Stated differently, f(x, y, z) = 0 is the equation of the bounding surface of the body when it is in its position at t = 0.





In deriving the equations of motion we shall make use of the Principle of Virtual Work (see Equation 2-33 of Paragraph 2.1.1.3).

$$\delta W = \int_{\mathcal{A}, q, \mathbf{z} \geq 0} \delta \mathbf{R} \cdot \left( \frac{\mathbf{p}}{\mathbf{r}} + \nabla \sum_{\mathbf{z} \geq -1} - \frac{\lambda_{\mathbf{z}}}{\lambda_{\mathbf{z}}} \right) \, \mathbf{x}^{\mathbf{y}} = 0 \tag{4-4}$$

Preliminary to this, however, we want to discuss some of the details of the kinematics of the motion.

## 4.1.1 The Kinematics of an Elastic Body Executing Arbitrarily Large Displacements

It will be convenient to arbitrarily decompose the position vector,  $\mathbb{M}(x, y, z, t)$ , into a sum of vectors describing the "rigid body" motion and the "elastic" motion. There is no à priori way of assessing how much of the motion of a given particle is due to rigid body motion and how much is due to elastic motion. It should be emphasized that the procedure we shall describe is just one of a number of arbitrary criteria that might be used to separate the motion.

An important property of the gross motion of the body is the path taken by the center-of-mass of the body. The position vector for the center-ofmass is defined by

$$\mathbb{R}'t) = \frac{\int \mathbb{R}(x,q,\bar{z},t) \rho(x_{q},\bar{z}) d\bar{z}}{\int \rho(x_{q},\bar{z}) d\bar{z}} \qquad (h-5)$$

(The region of integration, unless otherwise noted, is the whole fixed set of particles inside f(x, y, z) = 0.)



# FIGURE 65 CENTER-OF-MASS FOR AN ELASTIC BODY

If we arrange the x-y-z coordinate system so that the origin is at the point which is the center of mass at t = 0, then x = 0, y = 0, z = 0 labels the particle which, at time t = 0, is on top of the center of mass. We then have

$$\int x = x = c \qquad (4-6)$$

$$\int f(x,y,z)dV = 0 \qquad (4-7)$$

$$\int z \rho(x + z) dt = 0$$
(4-8)

The inertial reference system (I, J, K) is, in some ways, inconvenient for describing the motion of a body. Let us introduce instead a reference system,  $(\dot{r}, \dot{j}, \dot{k})$ , which is neither fixed in space nor fixed in the body. We will assume it to be arbitrary for the present. The relation between this reference system and the inertial reference system can be specified by a set of Euler angles,  $\phi, \theta, \psi$ , (see Figure 67).

We then introduce a position vector that is fixed in this frame of reference and whose length is constantly equal to the distance of the x-y-z particle from the origin (x = 0, y = 0, z = 0) at time, t = 0.

$$L^{i_{x}} q z z^{i_{y}} = xT + q j + zk$$
 (4-9)

The direction of L depends on the orientation of the "body" axis system,  $(\dot{r}, \dot{j}, k)$ . The position vector of a point fixed in the  $(\ddot{r}, \dot{j}, k)$  frame of reference is then



#### FIGURE 66 A POINT FIXED IN "BODY" AXIS FRAME OF REFERENCE

We define the "elastic" displacement vector ("relative" displacement might be a better description) of the x-y-z particle to be

$$\mathbf{E}^{(1)} = \mathbf{E} \cdot \mathbf{A} \mathbf{z}_{1} \mathbf{E}^{(1)} - \mathbf{E} \mathbf{z}_{1} \mathbf{z}_{2} \mathbf{E}^{(1)} - \mathbf{E} \mathbf{z}_{2} \mathbf{z$$

with the result that the position vector of the x-y-z particle can be written as

$$\mathbb{H}^{(x,y_{1},z_{1},z_{1})} = \mathbb{E} [z_{1} + [z_{1},z_{1},z_{2},z_{1}] - \mathbb{P}^{(x,y_{1},z_{1},z_{1})}]$$
(4-11)

This definition of elastic displacement leads to the important relation,

$$\int f'' dz z' : I = : \qquad (4-12)$$

This follows from using Equation 4-10 and the relations defining the center of mass (Equation 4-5).

$$\int F_{2} \dot{n} = \int I_{2} I_{2} I_{2} - \int L_{2} I_{2} V - E_{2} \int I_{2} I_{2} I_{2}$$

$$= E_{2} \int F_{2} I_{2} I_{2} - E_{2} \int I_{2} I_{2} I_{2} - E_{2} \int I_{2} I_{2} I_{2} I_{2}$$

$$(4-13)$$

The first and last terms cancel leaving

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which is zero because of Equations 4-5, 4-7, and 4-3.

$$\int L_{2} L_{2} = \int x^{2} - y J_{2} = L \quad I = 2 \qquad (4-24)$$

The elastic displacement is not completely specified until we say that the  $(\tilde{r}, \tilde{j}, \tilde{k})$  frame of reference is. There are a number of possibilities for defining  $(\tilde{r}, \tilde{j}, \tilde{k})$  which would tend to make them follow the gross rotational notion of the body. It can be shown that the condition,

Along with the requirement that  $|\hat{I}| = |\hat{J}| = |\hat{K}| = 1$  and

$$I_x j = k \tag{4-16}$$

$$j = i$$
 (4-17)

completely specify the  $(\forall, j, k)$  frame of reference as a set of unit, orthogonal, right-handed base vectors.

Equation 4-15 leads immediately to the condition

$$\int L x p_{\uparrow} dY = 0 \qquad (4-18)$$

which is true because

$$\int \mathbf{L} \cdot \mathbf{n} e^{tt} \mathbf{Y} = \int \mathbf{L} e^{tt} \mathbf{Y} \cdot \mathbf{R} - \int \mathbf{L} \cdot \mathbf{L} \cdot d\mathbf{Y} = 0 \qquad (4-19)$$

from Equation 4-14 and the fact that  $L \propto L = 0$ .

In summary, we have

$$\mathbb{L}_{x,y} = \mathbb{L}_{x,y} = \mathbb{L$$

where

$$\mathcal{R} = \frac{\int \mathfrak{A} \rho dY}{\int \mathfrak{K} \mathfrak{A}'} \tag{4-21}$$

and

$$L = \chi \dot{\tilde{I}}_{\pm} q \dot{J}_{\pm \pm} \tilde{E} \qquad (4-22)$$

and p satisfies the following conditions

$$\int p_{2} dx = 0$$
 (4-23)

$$\int \mathbb{L} \times \mathbb{P} \left[ \frac{d_{4}}{d_{4}} = \alpha \right]$$
(4-24)

The kinematics of the "rigid-body" reference system is described by the velocity of the center of mass,

$$\frac{\partial \mathcal{L}}{\partial t} = \frac{\partial \mathcal{L}}{\partial t}$$
 (4-25)

and the angular velocity of the  $(\bar{f}, j, \bar{k})$  axis system,

$$J_{L,2}^{i} = \frac{i \dot{t}_{j}}{i t} \cdot \frac{1}{k} \cdot \dot{t} - \frac{i \dot{t}}{i t} \cdot \frac{1}{k} \cdot \frac{i \dot{t}}{j} + (\frac{i \dot{t}}{i t} \cdot \frac{i}{j}) k \qquad (4-26)$$

This vector has the property that

.

$$\frac{1}{\partial z}(x,q,z,t) = 1 \times L(x,q,z,t) \qquad (4-27)$$

which is characteristic of a vector that is fixed in the (I, j, k) frame of reference.

The velocity of the x-y-z particle is given by differentiating Equation 4-20 and using Equation 4--27

$$F_{L} = \frac{T}{L} (x, y, z),$$

$$= \frac{T}{2L} - \frac{T}{L} - \frac{T}{L}$$
(4-28)

If we let  $p_{Z}(z, y, z, t)$ ,  $p_{y}(x, y, z, t)$ , and  $p_{Z}(x, y, z, t)$  denote the components of p referred to  $(\tilde{\gamma}, \tilde{j}, k)$ , then

and

$$\frac{\mathcal{I}_{[x_{n},y_{1},z_{1},z_{1}]}}{\mathcal{I}_{z}} = \frac{\mathcal{I}_{z}}{\mathcal{I}_{z}} \left[ x + \frac{\mathcal{I}_{z}}{\mathcal{I}_{z}} \right] + \frac{\mathcal{I}_{z}}{\mathcal{I}_{z}} \left[ x + \frac{\mathcal{I}_$$

If we introduce the notation

then

$$\frac{-1}{2} = \frac{1}{2} + 1.1 \times \frac{1}{2} \qquad (12-32)$$

and Equation 4-28 becomes

$$y = z = \frac{e^2}{4t} + -x L + F = F$$
 (4-33)

In the Frinciple of Virtual Work reference is made to the virtual displacement,  $\delta \text{DI}$ , of the x-y-z particle. This can be considered to be due to virtual displacements of the "rigid body" reference system and due to the "elastic" motion relative to this reference system.

It can be shown that there exists a vector,  $\delta \Theta_{s}$  such that

$$\begin{aligned} s &= c \cdot s \cdot y \\ -s &= c \cdot s \cdot s \\ s &= s s \cdot s \\ s &$$

We then have

$$\operatorname{SIL} = \operatorname{SIL} + \operatorname{SID} \times \operatorname{L} + \operatorname{SID} \times \operatorname{P} + \operatorname{SID} \tag{4-36}$$

where we have let

$$-\mathbf{p} = = \mathbf{p} \mathbf{x} \mathbf{x} + \mathbf{p} \mathbf{y} \mathbf{y} + \mathbf{p} \mathbf{z} \mathbf{k}$$
 (4-37)

In the Principle of Virtual Work the virtual displacements must satisfy all the kinematical constraints of the system. In particular the virtual displacements must satisfy Equations 4-23 and 4-24.

$$\int_{U} S[r a, W] = 0 \tag{4-38}$$

$$\int_{\mathcal{L}} \mathbb{L} \cdot \mathbb{S}[\mathbf{p} : \mathbf{X}] = \mathbf{D}$$

$$(4-39)$$

# 4.1.2 Derivation of the Equations of Motion from the Frinciple of Virtual Work

From Equation 4-4 we have

$$s_{M} = \int sr \cdot \left( P + v \cdot \underline{\Gamma} - v \cdot \underline{R} \right) = 1$$
 (4-40)

L See Synge and Griffith, Frinciples of Mechanics, McGraw-Hill, Third Edition, 1959, F. 253.

Substituting from Equation 4-36 into Equation 4-40, we obtain

$$s_{\mathcal{N}} = \int_{\mathbb{R}} \left( s_{\mathcal{R}} + s_{\mathcal{D}^{\mathcal{N}}} , \mathbb{L}_{+} | \mathbf{p} \right) - s_{\mathcal{P}} \right) \cdot \mathbb{R}_{+} \mathbb{V} \cdot \sum_{\mathcal{D}} \frac{J_{\mathcal{D}}^{T} \mathbf{m}}{J_{\mathcal{D}}^{T}} dV = J$$

$$(4-41)$$

subject to the following constraints on  $\delta \mathbb{R}$  ,  $\delta \mathbb{P}$  , and  $\delta | \mathfrak{p}$  .

$$\int \mathcal{E}[\mathbf{p}, dy = \mathbf{J} \qquad (4-42)$$

and

$$\int \mathbb{L}_{x} \mathbb{E}[x \cdot tV = \sigma] \qquad (1 - 1 + 3)$$

Following Lagrange's method of undetermined multipliers, we let  $\lambda_1$ , and  $\lambda_2$  be, as yet, undetermined vectors and add

to the virtual work. Since this term is zero, we still have  $\delta W = 0$ .

$$= \int (\Xi \mathbf{R} + S \mathbf{E} \cdot (\mathbf{I} + \mathbf{p}) + \Xi \mathbf{p}) \cdot (\mathbf{P} + \mathbf{w} - \sum_{j \in \mathcal{I}} -\frac{1}{2} \frac{d\mathbf{r}}{d\mathbf{r}})$$

$$+ \lambda_{i_1} - S \mathbf{p}_{j_1} + \lambda_{j_2} \cdot \mathbf{L} \times S \mathbf{p}_{i_1} / d\mathbf{V} = 0$$

$$(4-44)$$

This can be also written as

$$SW = \int_{U} (F + \nabla \cdot \Sigma - s \frac{d\pi}{dt^{2}}) tV + TE \qquad (4-45)$$

$$\int_{U} (I + P) \times (F - \nabla \cdot \Sigma - s \frac{d\pi}{dt^{2}}) tV + SE$$

$$+ \int_{U} SP - (P + \nabla \cdot \Sigma - s \frac{d\pi}{dt^{2}} - s \Delta_{u} - s \Delta_{u} \times L) tV = 0$$

By the usual arguments concerning the independency of the virtual displacements, we obtain

$$\int \left( \mathbb{P}_{+} \nabla \cdot \sum_{-1} \frac{\partial^{2} \pi}{\partial t^{2}} \right) dV = 0$$
 (4-46)

$$\int \left( \mathbb{L} + \mathbb{P} \right) \times \left( \mathbb{P} + \mathbb{V} \cdot \mathbb{Z} - 2 \frac{\partial^2 \mathfrak{A}}{\partial \mathfrak{t}^2} \right) d\mathcal{V} = 0$$
 (4-47)

$$\int \varepsilon \mathbf{p} \cdot \left( \mathbf{P} + \nabla \cdot \sum_{k=0}^{\infty} - \frac{\partial^{2} \mathbf{M}}{\partial t^{2}} + \frac{\partial}{\partial t^{2}} \lambda_{1} + \frac{\partial}{\partial t^{2}} \lambda_{2} \times \mathbf{L} \right) \, dV = 0 \tag{4-48}$$

From these relations we may derive all the equations governing the motion of the system.

Since we are considering a fixed set of particles, we can write Equation 4-46 as

$$\frac{d^{2}}{dt^{2}}\int_{\mathbb{C}}p \, \mathrm{d} V = \int (\mathbb{P} + \nabla \cdot \Sigma) \, \mathrm{d} V \qquad (4-49)$$

The total external force on the particles is

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$$\mathbb{F} = \int \mathbb{P} \, dV + \oiint \Sigma \cdot dS \tag{4-50}$$

Using the divergence theorem we have, from Equation 4-49,

$$\frac{\alpha^{L}}{dt^{L}}\int \varrho H dV = F$$
 (4-51)

From the definition of the center of mass (Equation 4-5), we can then write Equation 4-51 as

$$\int_{\mathbb{R}^d} \frac{d^{T} \mathbb{R}}{dt^{T}} = \mathbb{F}$$
 (14-52)

If we let

$$M = \int e^{dY}$$
 (1:-53)

denote the total mass of the body, then

$$M\frac{d^{L}R}{dt^{2}} = F \qquad (4-54)$$

Equation 4-47 is treated in a similar manner. We first note that

$$\mathbb{L} + \mathbb{P} = \mathfrak{n} - \mathbb{R} \tag{4-55}$$

Then Equation 4-47 can be written as

$$\int (\mathbf{u} - \mathbf{R}) = \frac{\sqrt{2\pi}}{2t^2} d\mathbf{V} = \int (\mathbf{u} - \mathbf{R}) = (\mathbf{P} + \mathbf{\nabla} \cdot \mathbf{\Sigma}) d\mathbf{V}$$
 (4-56)

We can reduce the left-hand side by using the identity

$$\frac{d}{dt}\int (\mathbf{n}-\mathbf{R}) \times \frac{\partial}{\partial t} (\mathbf{n}-\mathbf{R}) \, \mathbf{e} \, d\mathbf{V} = \int (\mathbf{n}-\mathbf{R}) \times \mathbf{e} \, \frac{\partial^2 \mathbf{n}}{\partial t^2} \, d\mathbf{V} \qquad (4-57)$$

and introducing the total angular momentum of the body calculated about the mass center,

$$\mathbb{H} = \int (\mathbf{n} - \mathbf{R}) \times \frac{1}{2^2} (\mathbf{n} - \mathbf{R}) e^{i \mathbf{M}}, \qquad (12-58)$$

We obtain

We also have

$$\int (m-R) \times (P + \nabla - \Sigma) dV$$

$$= \int \pi \times (P + \nabla - \Sigma) dV - R \times F$$

$$= \int m \times P dV + \int \pi \times \nabla - \Sigma dV - P \times F$$

$$= \int \pi \times P dV + \int \nabla - (m \times \Sigma) dV - R \times F$$
(4+-60)

where we have used

.

$$\int \mathbb{R} \cdot \nabla \cdot \mathbb{Z} \, dt = \int \nabla \cdot (\mathfrak{m} \times \mathbb{Z}) \, dt \qquad (4-61)$$

which is true because the integral can be expressed in coordinates for which

$$\pi * \nabla \cdot \Xi = \nabla \cdot (\pi * \Xi) \qquad (1 - 62)$$

(It can be shown that this is true when the integral is transformed to Eulerian coordinates.)

We then have

$$\frac{dH}{dt} = \int \mathbf{m} \times \mathbf{F} \, d\mathbf{Y} - \int \mathbf{\nabla} \cdot (\mathbf{m} \times \mathbf{\Sigma}) \, d\mathbf{V} - \mathbf{R} \times \mathbf{F}$$
(4-63)

Using the divergence theorem on the second term, we obtain

$$\underbrace{\overset{\text{UH}}{\text{III}}}_{\text{III}} = \int \mathbf{n} \cdot \mathbf{P} \cdot \mathbf{N} + \bigoplus \mathbf{n} \cdot \mathbf{\Sigma} \cdot \mathbf{1S} - \mathbf{P} \cdot \mathbf{F}$$

$$= \int (\mathbf{n} - \mathbf{R}) \cdot \mathbf{F} \cdot \mathbf{M} + \bigoplus (\mathbf{n} - \mathbf{R}) \cdot \mathbf{\Sigma} \cdot \mathbf{LS}$$

$$(4-64)$$

The right-hand side can now be recognized as the total moment of external forces calculated about the mass center.

$$\mathcal{L} = \int (\mathcal{L} - \mathcal{R}) * \mathcal{P} d\mathcal{V} - \bigoplus (\mathcal{L} - \mathcal{R}) * \sum \mathcal{L} - \mathcal{L} \mathcal{S} \qquad (4-65)$$

We then have

$$\frac{\Delta F}{M} = G \qquad (4-66)$$

Turning our attention to Equation 4-43, we assume that [P can be specified by N generalized coordinates,  $p_i(t)$ , i = 1, 2, ... N. Further, we assume that on the basis of [P being "small" (see also Paragraph 2.3.1), we can write

$$P * + 2t = \frac{1}{2} E_{t} * + 2 E_{t}$$
 (4-57)

Then

$$I'_{x,y}(z,z) = P(z, + 1, z) + \prod_{i=1}^{N} I(z, + y, z) p_{i}(z)$$
  
(4-62)

We are thus approximating the continuous elastic body by one with N+6 degrees-of-freedom. The six generalized coordinates describing the "rigid body" motion could be taken as  $\xi$ ,  $\eta$ ,  $\zeta$ ,  $\phi$ ,  $\theta$ ,  $\psi$ , where  $\xi$ ,  $\eta$ , and  $\zeta$  are the inertial coordinates of the center of mass,

$$\mathbb{E} t = \mathbb{E} \mathbb{E} + $

and  $\phi$ ,  $\theta$ , and  $\psi$  are Euler angles for the ([,j],k) frame of reference.

We then have

$$S_{i}^{b} = \underbrace{\sum_{i=1}^{b} \sum_{i=1}^{b} \sum_$$

but from Equation 4-63, we also have

$$\frac{\Delta r}{\lambda k_{i}} = h_{i} \qquad (4-71)$$

so that

$$S_{\mathcal{D}}^{*} = \frac{1}{\frac{1}{2}} $

Introducing this into Equation 4-45, we have

$$\sum_{i=1}^{n} \frac{1}{p_{t}} \int \frac{dt}{dp_{t}} \cdot \frac{dt}{dp_{t}} \cdot \frac{1}{p_{t}} $

The  $\delta p_i$  can be independently varied and  $A_1$ , and  $A_2$  can be chosen so that

$$\int \frac{dT}{dt} \cdot \left[ \mathbf{P} \cdot \nabla \cdot \boldsymbol{\Sigma} - z \frac{dT}{dt^2} - z \mathbf{A} + z \lambda_2 \times \mathbf{L} \right] dt = 0$$
(4-74)

i = 1,2...1

Using the identity expressed in Equation 2-43 of Faragraph 2.1.2.1, we have

 $\int \frac{1}{\sqrt{2}} \frac{d}{dt} = \frac{1}{2t} \frac{d}{dt} - \frac{d}{dt} = \frac{1}{2t} \frac{d}{dt} - \frac{d}{dt}$ (4-75)

where

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$$\tau = \frac{1}{2} \int \frac{M}{2\pi} \cdot \frac{M}{2\pi} \frac{M$$

Also, from Equation 2-55 of Faragraph 2.1.2.1, we have

$$\int \frac{\partial f}{\partial t_{1}} \cdot \vec{F} \cdot \vec$$

where

$$P_{i} = \int \frac{2\pi i}{\partial p_{i}} \cdot P \, dV \rightarrow \iint \frac{2\pi}{\partial p_{i}} \cdot \sum \cdot dS \qquad (4-7^{\frac{3}{2}})$$

= generalized forces derived from the virtual work of only the external for bes.

we then can write Equation 4-74 as

$$\frac{1}{2} \frac{1}{2r_{1}} - \frac{1}{2r_{2}} \frac{1}{2r_{2}} = \frac{1}{2} - \int r_{1} r_{2} r_{3} + \lambda_{1} r_{2} - \lambda_{2} r_{3} + \lambda_{2} r_{3} r_{3} + \lambda_{3} + \lambda_{3} r_{3} +$$

The complete equations of motion are then

$$m\frac{d^{2}\mathcal{R}}{dt^{2}}=\mathbb{F}_{0}$$

$$\frac{dH}{dt} = C_{t} \qquad (t_{t-81})$$

$$\frac{1}{2!}\frac{\partial T}{\partial t_{1}} - \frac{\partial T}{\partial t_{1}} + \frac{\partial U}{\partial t_{1}} + \frac{\partial F}{\partial t_{1}} = P_{1} + \int \mathbb{E}_{t_{1}} \cdot (\lambda_{1} + \lambda_{2} \times \mathbb{E}) dV \qquad (4-82)$$

1 = 1, 2...N

## 4.1.3 The Kinetic Energy of the Body

From Equation 4-79 the kinetic energy is

and from Equation 4-33, we have

ani

If we introduce the dyadic,

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 $( \stackrel{f}{=} is the Iterdyalis<sup>1</sup>)$ 

then we can write

<sup>1</sup>Also called Riemfactor and Unit Durine: see A. F. Wills <u>Vector Analysis</u> with an Introduction to Tensor Analysis, Dover, 1950, p. 130.

$$2r 2 + p = 2 + (2 + p)^2 + (2 + p)^2 + (4 - 87)$$

Substituting this in Equation 4-85 and then into Equation 4-83, we obtain

Since  $\ensuremath{ \Lambda}$  and  $\ensuremath{ R}$  do not depend on x, y or z, we may take them out of the integral.

From Equations 4-14 and 4-23, we have

Also, since (from Equation 4-2-

we have, on differentiating,

From which we conclude that

In a similar manner

Using Equations 4-90, 4-93, and 4-94 we may simplify the kinetic energy

where we have introduced the instantaneous inertia dyadic for the deformed body,

and the total mass

.

It will be convenient to express the kinetic energy explicitly in terms of the components of p referred to the "body axis" reference system, (  $\dot{\gamma}$  ,  $\dot{j}$  , k )

$$t = c_1 7 + c_2 + c_2 c_1$$
 (4-98)

We then have

$$\int \overline{p} x \left[ \overline{p} \cdot d\overline{y} \right] = \int c_{x} \frac{c_{x}}{c_{x}} - c_{z} \frac{c_{x}}{c_{x}} - c_{x} \frac{c_{y}}{c_{x}} + c_{x} \frac{c_{y}}{c_{x}} \right] e^{\frac{1}{2}t} \int e^{\frac{1}{2}$$

. and

$$\int \frac{1}{2} \dot{r}_{2} \, \alpha = \int \frac{\frac{1}{2} \dot{r}_{2}}{c t} \, \left( c + \int \frac{\frac{1}{2} \dot{r}_{2}}{c t} \right)^{2} dV + \int \frac{\frac{1}{2} \dot{r}_{2}}{2 t} \frac{\frac{1}{2} \dot{r}_{2}}{2 t} dV$$
(4-100)

The inertia dyadic can be expressed as

$$\frac{1}{2} = \int (\frac{1}{2} + \frac{1}{2} + \frac{1}{2})^{2} + (\frac{1}{2} + \frac{1}{2})^{2} + \frac{1}{2} +$$

This expression can be simplified by making use of the constraint relations, Equations 4-23 and 4-24, written in terms of components. These equations are:

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$$\int p_{1,p} dV = 0 \tag{4-102}$$

$$\int p_{+2} dV = 0 \tag{4-103}$$

$$\int r_z r dt = r$$
 (4-104)

$$\int \frac{\partial r_z}{\partial t_z} - z b_y \cdot z b_z = 0 \qquad (4-105)$$

$$\int r_{\varepsilon} - \varepsilon r_{x} = 0 \qquad (4-106)$$

For the sake of brevity we write Equation 4-101 as

.

$$J = v_{11} t_{1}^{2} + v_{21} t_{2}^{2} + v_{21} t_{2}^{2} + v_{21} t_{2}^{2} + v_{21} t_{2}^{2} + v_{22} t_{2}^{2} + v_{2} t_{2} + v_{2} t_{2} + v_{2} + v_{2} + v_{2} + v_{2} +$$

Then, using Equations 4-102 through 4-107 in Equation 4-101, we have

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$$m = \int (1 - z^2) dt - \int dy dy + dz dz = (d_1 - f) dy - dz dy$$
 (4-109)

$$\delta v_{\mu} = -\int v_{\mu} z \dot{x}_{\mu} - \int \delta v v_{\mu} + S_{\mu} z_{\mu} dv \qquad (4-120)$$

$$\lambda_{xz} = -\int e_{z} (z_{y} - \int \lambda z_{y} + p_{x} \phi_{z}) g(z)$$
(4-111)

$$\sum_{i=1}^{n} = -\int_{i} z_{i} r_{i} - \int_{i} z_{i} z_{i} + z_{i}$$

In these expressions we may recognize the moments and products of inertia of the undeformed body.

$$b_{1} = \int z_{1}^{1} z_{2}^{1} z_{3}^{1} dt \qquad (4-115)$$

.

$$z_{i,z} = y_{i,z} z_{i,z}$$
 (4-117)

.

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. .

$$I_{\gamma\gamma} = \int x^{2} + z^{2} j_{\xi} d\xi$$
 (4-118)

$$I_{42} = \int f_{20} x^{3}$$
 (4-119)

$$I_{22} = \int (x^{1} v^{2})^{\frac{1}{2}} (4-120)$$

As in Section 3.2, we want to make a finite degree-of-freedom approximation. From Equation 4-67 we have

Expressed, component wise,

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and Equation 4-121 can be replaced by

$$y = \sum_{i=1}^{n} x_{i}^{2} + x_{i}^{2} = -i x_{i}^{2} + i z_{i}^{2} + i$$

$$t_{z} = \sum_{i=1}^{n} n_{z}^{(i)} x_{ii}, z_{i} y_{i}^{(i)} = \sqrt{r_{z}(x_{i}y_{i})} \frac{1}{2} \frac{1$$

Using these expressions we can write

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$$\int \frac{h_{x}}{E} \left[ \frac{1}{2} + \frac{1}{2}$$

$$\int \left[\frac{4}{2}\eta^{2} dY - \frac{1}{2}\rho^{2} \int \frac{1}{2} + \frac{1}{2} \int \frac{1}{2} + \frac{1}{2} \int \frac{1}{2} + \frac{1}{2} \int \frac{1}{2} \frac{1$$

$$\int \frac{h_z}{H} \frac{1}{2} dx = \frac{1}{2} \int \frac{1}{2} \ln z \frac{1}{$$

$$\int p_{x} \frac{dp_{y}}{dt} dV = \{p_{y}^{1} \int \frac{1}{2} k_{x} \frac{1}{2} h_{y} \frac{1}{2} dx \{0\}$$
(4-129)

$$\int p_{x} \int_{E} \frac{1}{2E} \left[ Ax - \frac{1}{2E} \right] \int \frac{1}{2E} r_{x} \left[ \frac{1}{2E} r_{x} \right] \frac{1}{2E} \left[ \frac{1}{2E} + \frac{1}{2E} \right] \frac{1$$

$$\int t_{4} \frac{b_{z}}{b_{z}} dV = + \frac{c}{s} \int \frac{dv}{dv} \frac{b_{z}}{dv} dV = -\frac{c}{s} \int \frac{dv}{dv} \frac{dv}{dv} \frac{dv}{dv} \frac{dv}{dv} + \frac{c}{s} \int \frac{dv}{dv} \frac{$$

We can write these expressions more concisely if we define

$$[4_{xx}] = \int \frac{1}{16x^{3}} h_{x} f(x) dx \qquad (4-132)$$

$$A_{44} = \int \frac{1}{16} $

$$[F_{zz}] = \iint \{n_z \in [\mu, z] \mid \mu \}$$

$$[2m_1] = \int \{h_x\} h_y r_y/h$$

$$[A_{xz}] = \int [h_{xz}] f(x_z) dt$$

$$[a_{2}] = [a_{1} - b_{2}] = [a_{1}, a_{2}]$$

Equation 4-99 can then be written as

and Equation 4-100 as

(4-139)

It will be convenient to introduce rigid body modes,  $\{\phi_{e}\}_{i}$ , which represent values of  $\{p\}$  corresponding to displacements relative to the ([i,j],[.]) reference system as a rigid body.

A rigid body translation parallel to the x-axis is given by

for each particle. There then exists  $\{ \phi_{\rm E}, F_{\rm c}, {\rm such that} \}$ 

$$i = fh_{x} f' f_{x} f_{y}$$

$$(4-141)$$

$$i = fh_{y} f' f_{y} f_{y}$$

$$i = fh_{z} f' f_{y} f_{y}$$

A rigid body rotation about the x-axis, positive according to the righthand rule, is given by

$$y_{1} = -z \qquad \qquad ((1-1)+2)$$

$$y_{2} = -z$$

.

$$= -\frac{1}{2} \left[ \frac{1}{2} + \frac{1}{2} \right]$$

In general there are six possible independent rigid body modes. If these are taken as

- (I) translation perallel to V
- (2) translation parallel to J
- (3) translation parallel to k
- (4) rotation about V
- (5) rotation about J
- (6) rotation about &

Then we have

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$$\frac{1}{2} \frac{1}{2} \frac{1}$$

.

$$-in_{1} = \frac{1}{16} + \frac{1}{16} + \frac{1}{16} = \frac{1}{16} + \frac{1}{16} + \frac{1}{16} + \frac{1}{16} = \frac{1}{16} + $

Using these relations, we find that

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$$I_{xx} = \left[ (j + z) p dV = -j_{R} i_{4} ([A_{xx}] + [A_{yy}] + [A_{zz}]) + j_{R} i_{4} \right]$$
 (4-146)

and

.

.

$$w = \int dv = -\frac{1}{4\pi} \frac{d^2}{dt} \left( \frac{d_{Axx}}{dt} - \frac{d_{Ayy}}{dt} + \frac{d_{Azz}}{dt} \right) \frac{1}{2} \frac{d_{Azz}}{dt} \left( \frac{d_{Azz}}{dt} \right) \frac{1}{2} \frac{d_{Azz}}{dt}$$
(4-147)

and, in general

$$\begin{bmatrix} \mathbf{M} & \mathbf{J} \\ \mathbf{J} & \mathbf{M} \\ \mathbf{J} & \mathbf{M} \end{bmatrix} = \begin{bmatrix} \mathbf{M} \\ \mathbf{K} \end{bmatrix}^{T} \begin{bmatrix} \mathbf{A} \\ \mathbf{K} \end{bmatrix}^{T} \begin{bmatrix} \mathbf{M} \\ \mathbf{J} \end{bmatrix}^{T} \begin{bmatrix} \mathbf{M} \\ \mathbf{K} \end{bmatrix}^{T} \end{bmatrix}^{T} \begin{bmatrix} \mathbf{M} \\ \mathbf{K} \end{bmatrix}^{T} \begin{bmatrix} \mathbf{M} \\ \mathbf{K} \end{bmatrix}^{T} \end{bmatrix}^{T} \begin{bmatrix} \mathbf{M} \\ \mathbf{K} \end{bmatrix}^{T} \begin{bmatrix} \mathbf{M} \\ \mathbf{K} \end{bmatrix}^{T} \end{bmatrix}^{T} \end{bmatrix}^{T} \begin{bmatrix} \mathbf{M} \\ \mathbf{K} \end{bmatrix}^{T} \end{bmatrix}^{T} \begin{bmatrix} \mathbf{M} \\ \mathbf{K} \end{bmatrix}^{T} \end{bmatrix}^$$

Where  $[\phi_{\rm R}]$  is the N x 6 matrix of rigid body modes

$$[\varphi_R] = [\{\varphi_R\}, \{\varphi_R\}_{2, \dots}, \{\varphi_R\}_{n}]$$

$$(4-149)$$

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The components of the inertia dyadic can be written as

$$\lambda_{xx} = I_{xx} + a \left[ \frac{1}{2} \frac{1}{2} \left[ A_{yz} \right] + a \left[ \frac{1}{2} \frac{1}{2} \left[ A_{xz} \right] + \frac{1}{2} \frac{1}{2} \right] + \frac{1}{2} \frac{1}{2} \frac{1}{2} \left[ A_{xz} \right] + \frac{1}{2} \frac{1}{2} \frac{1}{2} \left[ \frac{1}{2} \frac{1}{2} \frac{1}{2} \right] + \frac{1}{2} \frac{1}{2$$

$$\lambda_{xy} = -I_{xy} + 2 \lambda_{yx} \lambda_{y} [A_{yz}]^{2} [B_{yz}]^{2} [A_{xy}] \{b\}$$
 (4-151)

$$\lambda_{iz} = -I_{iz} - i \cdot i \cdot [-i, i \cdot z] - i \cdot [\lambda_{iz}] \cdot z]$$

$$(4-152)$$

$$A_{\mu\mu} = A_{\mu\mu} + A_{\mu\nu} + A$$

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$$\Delta z_{2} = z_{22} + k \int [A_{12}] + [A_{22}] + [A_{23}] $

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The kinetic energy (Equation 4-95) for the finite degree-of-freedom system is

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$$T = \frac{1}{2} \left( \frac{12}{24} + 2 \frac{1}{2} $

where  $\Omega_x$ ,  $\Omega_y$ , and  $\Omega_z$  are the components of the angular velocity vector referred to ( $\dot{r}$ ,  $\dot{j}$ ,  $\kappa$ ).

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## 4.1.4 The Strain Energy of the Body

The specific internal strain energy of a particle of a continuous elastic body can be written in a very general form as  $^{1}$ 

$$\frac{12[x,y,z,z]}{(z+y)} = \frac{1}{2} \left( \frac{z}{z+y} \frac{z}{(-z+y)} + \frac{z}{(-z+y)} + \frac{z}{(-z+y)} + \frac{z}{(-z+y)} \right)$$
$$= \frac{1}{2} \frac{z}{(-z+y)} + \frac{z}{(-z+y)} + \frac{z}{(-z+y)} + \frac{z}{(-z+z)} + \frac{z}{(-z+y)} + \frac{z}{(-$$

The Lagrangian-coordinate components of strain can be written directly in terms of the displacements relative to the  $(\dot{y}, j, k)$  frame of reference<sup>2</sup>

$$z_{r_{k}} = \frac{1}{r_{k}}$$
 (4-159)

$$=\frac{2x_{4}}{2}$$
 (4-160)

$$\frac{b_1}{b_2} = \frac{b_1}{b_2}$$
 (4-161)

<sup>1</sup>See Timoshenko and Goodier, <u>Theory of Elasticity</u>, McGraw-Hill, 1951, p. 148 equation (85).

<sup>2</sup>This statement requires proof. It can be shown that the "exact" definition of strain used in conjunction with the assumption of small displacements relative to  $(\dot{y}, \dot{y}, k)$  would give the result stated in Equations 4-159 through 4-164. For the "exact" definition of the Lagrangian strains, reference should be made to Green and Zerna <u>Theoretical Elasticity</u>, Oxford, 1954, section 2.2, p. 77.

$$e_{xz} = \frac{10x}{5z} + \frac{10z}{1x}$$
 (4-163)

$$r_{1} = \frac{r_{0}}{2} + \frac{r_{0}}{4}$$
 (4-164)

Using Equations 4-123, 4-124, and 4-125, these can be written in terms of the generalized coordinates,  $P_{i}$ .

,

$$\epsilon_{xx} = \{ \frac{\partial hx}{\partial x} j' \{ p \}$$
 (4-165)

$$\epsilon_{44} = \{ \frac{\lambda h_4}{\lambda_4} \}^{\prime}_{1} p \}$$
 (4-166)

.

$$\epsilon_{\epsilon\epsilon} = \left\{ \frac{3\hbar\epsilon}{3\epsilon} \right\}$$
 (4-167)

$$f_{eq} = f_{eq} + f$$

Substituting this into frate and a larger ing,

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we obtain the following expression for the total strain energy

$$\begin{bmatrix} k_{+} \end{bmatrix} = \int \left( \frac{E_{2}}{(1+Y_{1})^{2} + 2Y_{2}} \left( \frac{1}{1+Y_{1}} \frac{1+Y_{2}}{1+Y_{2}} + \frac{1}{2Y_{2}} \frac{1+Y_{2}}{1+Y_{$$

In practical structural analyses Equation 4-173 would never be used directly but the general approach would be the same except the strain-displacement relations would be replaced by approximate ones appropriate to beams, plates, shells, etc.

# 4.1.5 The Dissipation Function for the Body

In a similar manner we may derive an expression for Rayleigh's dissiption function. The general form of H distance for a static stress state is

$$J_{xx} = \Lambda [z_{xx} + c_{yy} + d_{zz}] + \lambda z = c_{xx} \qquad (4-174)$$

 $f_{ij} = \sum_{i=1}^{n} f_{ii} + f_{ij}  

where

$$\mathcal{T}_{zz} = \sum \left( \hat{e}_{xx} + \hat{z}_{yy} + \hat{e}_{zz} \right) + \hat{a} \hat{z} \hat{e}_{zz} \qquad (4-176)$$

$$\tau_{ij} = G \in \mathcal{I}_{ij}$$
(4-177)

$$\tau_{x_{\mathcal{E}}} = \mathcal{J} \in_{x_{\mathcal{E}}}$$
 (4-1.78)

$$r_{jz} = \hat{j} \hat{e}_{jz} \tag{4-179}$$

where  $\lambda$  is Lame's constant

$$\cdot = \frac{i\varepsilon}{i\tau' - i\tau'}$$
(4-180)

and G is the shear modulus

$$J_{\pi} = \frac{J_{\pi}}{\lambda_{1} + \tau^{2}} \qquad (4-181)$$

Following Volterra<sup>1</sup> we may generalize these relations, in the dynamic case, to

-

$$i_{1} = -i_{2} + i_{2} + i_{2} + i_{3} + i_{5} + i_{7} + i_{$$

$$(4-184)$$

$$\bar{z}_{i,j} = (i - jk) \bar{z}_{i,j}$$
 (4-185)

where the bar denotes the Laplace transform with respect to time,

<sup>1</sup>\_\_\_\_\_ Enrico Volterra, <u>On Elastic Continua with Hereditary Characteristics</u>, Journal of Applied Mechanics, September, 1951, equation (14).

Volterra has shown that invariancy under a rotation of axes indicates that only two parameters are pertinenet to the description of the strain rate terms. The functions

$$\mu(x_{ij}, z_{j}, t)$$
 (4-189)

and

$$R(x_{ii}, z, t)$$
 (4-190)

are called hereditary functions. We suppose that in the frequency range of interest in structural vibrations we can say that

$$\overline{\mu}^{(i)}(s,j,z,i) = \mu(s,j,z)$$
 independent of s (4-191)

$$\bar{R}_{s,t}(x_{0}, z_{1}, z) = R_{s,t}(x_{1}, z)$$
 independent of S (4-192)

(This classifies the internal energy dissipation as what is generally called viscous damping.)

The generalized stress-strain relations are then given by

$$J_{xx} = \lambda \left( \hat{z}_{xx} + \hat{z}_{yy} + \hat{z}_{zz} \right) + \omega \left( \hat{e}_{xx} + \hat{e}_{yy} + \hat{z}_{zz} \right)$$

$$+ \lambda \left( \hat{z}_{xx} + \lambda \nabla_{x} \hat{e}_{xx} \right)$$

$$(4-193)$$

$$T_{3'j} = \sqrt{-i_{2x} + i_{2z}} + u (i_{2x} + i_{3'j} - i_{2z}) \qquad (4-194)$$

$$T_{42} = x \dot{e}_{42} + k \dot{e}_{32} \qquad (4-197)$$

$$\tau_{x_{2}} = \hat{J} \hat{c}_{x_{2}} + R \hat{c}_{x_{2}}$$
(4-193)

The virtual work of the internal forces is given by

$$SW = -\int \nabla_{x} \Xi \epsilon_{xx} + \nabla_{eg} S \epsilon_{gg} + \nabla_{eg} S \epsilon_{gg}$$
$$- \nabla_{eg} \Sigma \epsilon_{eg} + \nabla_{eg} S \epsilon_{xg} + \nabla_{gg} S \epsilon_{gg} \right) IV$$
(4-199)

Substituting the stresses in Equations 4-193 through 4-198, we obtain

$$\begin{aligned} f_{N} &= -\int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \frac{1}{2} \frac{$$

The first integral is  $-\delta U$  and has already been accounted for in deriving Equation 4-173. Making use of Equations 4-123, 4-124, and 4-125, Equation 4-200 becomes

$$s_{N} = -s_{0} - \{s_{p}\}\{s_{p}\}\}$$
 (4-201)

Where

.

$$\begin{aligned} [3] &= \int_{-\infty}^{\infty} \frac{1}{(1+x_{1})^{2}} \frac{1$$

(4-202)

In the very special case that  $\mu = \beta \lambda$  and  $R = \beta G$ , we have, by comparing Equation 4-173 with Equation 4-202,

.

$$[n] = u[n]$$
 (4-203)

and the comments concerning Equation 2-292 of Paragraph 2.2.3.5 apply.

From Equation 4-201 and the symmetry of [B], it can be shown that a dissipation function exists such that
# 4.1.6 The Complete Set of Differential Equations Governing the Motion of the Body

The total angular momentum of the system (Equation 4-58) is

$$H = \int (\pi - R) \times \frac{\Im}{J_{k}} (\pi - R) \,_{e} \, dV \qquad (4-205)$$

In this expression

$$1 - P_{c} = L + P_{c}$$
 (4-206)

and

$$\frac{1}{2\pi}(\mathbf{1}-\mathbf{R}) = \mathbf{1} \times (\mathbf{L}+\mathbf{p}) + \mathbf{p}$$
 (4-207)

so that

$$H = \int (\mathbf{L} \cdot \mathbf{p}) \times \int \mathcal{L} \cdot (\mathbf{L} + \mathbf{p}) = \chi \mathbf{v} + \int (\mathbf{L} + \mathbf{p}) \cdot \mathbf{x} + \int (\mathbf{L} + \mathbf{p$$

The first term can be written as  $\Lambda \cdot \Lambda$  where  $\Lambda$  is given by Equation 4-96. In the second term we can use Equation 4-93 so that

$$H = \lambda \cdot R + \int p \cdot \dot{p} \cdot (t) \qquad (4-209)$$

which may be expressed in terms of the generalized coordinates,  $\rm p_{i},$  by using Equations 4-138 and 4-150 through 4-155.

Using the results of the previous section in Equation 4-82, we will have the Lagrange equations corresponding to  $p_i$ , i = 1, 2...N. From Equation 4-156, we have

$$\frac{1}{4} = \frac{1}{4} = \frac{1$$

and

.

$$\begin{aligned} \frac{1}{2} \frac{\partial \Gamma}{\partial p_{L}} \hat{f} &= -\partial_{x} \left( \left[ A_{q Z J}^{-1} - \left[ A_{q Z} \right]^{2} \right] \hat{f} \hat{p} \hat{f} \\ &= -\partial_{x} \left[ \left[ A_{X Z} \right] - \left[ A_{X Z} \right]^{2} \right] \hat{f} \hat{p} \hat{f} \\ &= -\partial_{z} \left[ \left[ A_{X Z} \right] - \left[ A_{X Z} \right]^{2} \right] \hat{f} \hat{p} \hat{f} \\ &= -\partial_{z} \left[ \left[ A_{X Z} \right]^{2} - \left[ A_{X Z} \right]^{2} \hat{f} \hat{p} \hat{f} \\ &= -\partial_{x} - \partial_{z} \left[ \frac{\partial A_{X X}}{\partial p_{L}} \hat{f} + -\partial_{x} - \partial_{z} \right] \hat{f} \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X X}}{\partial p_{L}} \hat{f} + -\partial_{x} - \partial_{z} \right] \hat{f} \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \\ &= -\partial_{x} - \partial_{z} \left[ \frac{\partial A_{X X}}{\partial p_{L}} \hat{f} + -\partial_{x} - \partial_{z} \right] \hat{f} \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} + -\partial_{y} - \partial_{z} \right] \hat{f} \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \\ &= \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat{f} \right] \hat{f} \\ \\ &= -\partial_{x} \left[ \frac{\partial A_{X Z}}{\partial p_{L}} \hat$$

(4-211)

Also, from Equation 4-172

,

$$\frac{100}{1000} = [10] \frac{100}{100}$$
 (4-212)

and from Equation 4.-204

$$i\frac{3}{3p_1}i = [5]p_1^{2}i$$
 (4-213)

If we let

$$\lambda = \frac{1}{1 + 1} \frac{1}{2} + \frac{1}{2} \frac{1}{2}$$
 (4-214)

and

•

 $y_{i} = x_{i} \frac{1}{1 + x_{i}} \frac{1}{1 + x_{i}$ 

then we can write the generalized constraint forces as

$$\int \mathbf{h}_{2} \cdot \mathbf{h}_{1} + \mathbf{h}_{2} \cdot \mathbf{L}^{p} dd$$

$$= \int \left\{ \mathbf{h}_{2}^{(1)} \cdot \mathbf{h}_{1} + \mathbf{h}_{2}^{(1)} \cdot \mathbf{h}_{2}^{(1)} + \mathbf{h}_{2}^{(1)} \cdot \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{2}^{(1)} \cdot \mathbf{h}_{2}^{(1)} + \mathbf{h}_{2}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{2}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{2}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{2}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{2}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)} \right\} dd$$

$$+ \int \left\{ \mathbf{h}_{3}^{(1)} \cdot \mathbf{h}_{3}^{(1)} + \mathbf{h}_{3}^{(1)$$

Using Equations 4-144, we have

(4-217)

where

Before substituting these relations into Eq.ations 4-32, let us express the constraints (Equations 4-102 through 4-107) in terms of the  $p_1$ . Again, using Equations 4-123, 4-124, and 4-125, and, also, 4-144 and 4-145, we have

$$\int z_{r}(x) = -z_{r}(z_{r}) = z$$
 (4-219)

$$\int f_{4} e^{ikt} = -e^{ikt} f_{2}(kt) = 0 \qquad (4-220)$$

$$\int \phi_{z} - z_{P_{1}} \phi_{z} dt = \frac{1}{2} \phi_{z} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \left[ \frac{1}{2} \right] \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \right] \left[ \frac{1}{2} \left[ \frac{1$$

$$\int (v_{Fz} - z p_x) dV = f \varphi_R E_s[A] [A] = 0$$
(4-223)

$$\int k * k y - y b_{\pi} \partial_{2} d k = f \varphi_{\pi} E_{\epsilon}' [A] f b F = \sigma \qquad (4 - 224)$$

which can be written concisely (from Equation 4-149) as

$$[\varphi_R]'[A][b] = \{c\}$$
 (4-225)

which bears a remarkable similarity to Equation 2-261 of Paragraph 2.2.3.4. <sup>1</sup> We also have

$$[\kappa]i\varphi_{\kappa}]_{i} = \{0\} \qquad (!z-226)$$

and

$$l = H q_{R_{L}} = i o I \qquad (l + -227)$$

which follows because the strains (Equations 4-165 through 4-170) are zero for  $\{p\} = \{\phi_R\}_i$ 

$$\frac{1}{100} \operatorname{high}_{1} = 0 \qquad (4-228)$$

$$t_{14}^{h_4} f_{14} f_{1} = 0$$
 (4-229)

$$\left(\frac{2\pi}{23\pi}b'+\frac{3\pi}{23\pi}b'\right)+\frac{1}{2}frt=2$$
(4-231)

$$\left[ \frac{1}{12} \frac{1}{12} + \frac{1}{2} \frac{1}{22} + \frac{1}{2} \frac{1}{2} + \frac{1}{2} \frac{1}{2} + \frac{1}{2} \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \frac{1}{2} + $

<sup>1</sup>This similarity is a consequence of the choice of

 $\int \mathbf{L} \times \mathbf{p} \, e^{dY} = \mathbf{a}$ 

as a constraint on  $\,p$  (see Equation 4-18). This gives a motive, a posteriori, for making this choice.

$$\left(\frac{1}{5}\right) + \left(\frac{1}{2}\right) +$$

Using Equation 4-217, Equation 4-12 cam be written as

$$\frac{d}{dt} + \frac{dT}{dp} \frac{1}{t} + \frac{T}{t} \frac{1}{p} + \frac{200}{t} + \frac{1}{t} \frac{p}{p} \frac{1}{t} = \frac{1}{t} F \frac{1}{t} + [A][\varphi_R][\lambda]$$
(4-234)

Premultiply by  $[\phi_1]'$  and use

$$[4_{F_{1}}]^{2} + \{\frac{1}{p_{1}}\} + \{\frac{1}{p_{1}}\} = \{c\}$$
 (4-235)

which is equivalent to Equations 4-226 and 4-227. Then

Solving for the  $\lambda$ 's, we have

(4-237)

Substituting this into Relation 4-234 yields

where

$$= [-] + [-$$

Following Hamilton we can reduce the system to a set of first order equations by introducing the generalized momenta as additional coordinates. If the generalized momenta are denoted by  $h_{i,r}$  then

and we can write Equation 4-210 as

(4-241)

where the [G] 's are the anti-symmetrical part of the [A] 's

$$[z_{1,z}] = \frac{[z_{4z}] \cdot [z_{5z}]}{z}$$
 (4-242)

$$[c_{\tau_{\lambda 2}}] = \frac{[A_{\lambda 2}]}{2} - \frac{[C_{\lambda 2}]}{2}$$
 (4-243)

We can also write Equation 4-241 as

.

Using Equations 4-211, 4-212, and 4-213 in Equation 4-238, we have

(4-246)

Using Equations 4-150, through 4-155, we have

$$\frac{4}{247} = \frac{4}{247} + \frac{1}{247} + \frac{1}$$

$$\frac{\partial \lambda_{s} \tau_{1}}{\partial \mu_{t}} = \lambda \left[ \lambda_{4t} \right] \tau_{7t} F_{s} - \left[ \left[ A_{s} \right] + \left[ A_{s} \right] \right] t F_{s}$$

$$(4-248)$$

$$\frac{1}{2} \sum_{i=1}^{n} \left[ \sum_{i=1}^{n} \sum_{i=$$

$$\frac{1}{1+1} = 2 \left[ A_{xy} \right] \frac{1}{1+1} + \left[ A_{xz} \right] \frac{1}{1+1} \frac{1}{1+1} \left[ A_{xz} \right] \frac{1}{1+1} \frac{1}{1+1} \left[ A_{xz} \right] \frac{1}{1+1} \frac{1}{1$$

$$z_{p_{z}}^{\text{A}\neq z} = z \left[ A_{xz} \right]^{2} \varphi_{x} \xi_{z} - \left( \left[ A_{yz} \right] + \left[ A_{yz} \right]^{2} \right) \left\{ b \right\}.$$
 (4-251)

$$\frac{2^{A_{12}}}{p_{c}} = 2 \left[ (A_{x_{1}}]_{Y_{18}} + \frac{1}{2} A_{y_{2}} \right]_{Y_{18}} + \frac{1}{2} A_{y_{1}} + \frac$$

## For simplicity we introduce the following symmetric matrices

•

•

•

$$-\frac{1}{2} = \frac{4\pi 1 + [4\pi 2]}{2}$$
 (4-253)

$$\begin{bmatrix} -\frac{1}{2} \end{bmatrix} = \begin{bmatrix} \frac{1}{2} + \frac{1}{2} \end{bmatrix}$$
(4-254)

$$[-_{44}] = \frac{[4n] + [4_{44}]}{2}$$
 (4-255)

$$\left[ \left[ \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum$$

(4-259)

This equation is to be solved similaneously with Equation 4-245

(4-25.)

The rigid body equations (Equations 4-30 and 4-31) must also be solved simultaneously with the above equations.

From Equation  $4^{-2}$ , we have

where

.

.

$$V = \frac{tE}{t\bar{t}}$$
(4-262)

If we denote the components of  $\mathbb V$  by  $\mathtt V_x,$   $\mathtt V_y,$  and  $\mathtt V_z,$  then

$$V = V_{z} j + V_{z} k$$
 (4-263)

and

$$\frac{dV}{dt} = \frac{dV_x}{dt} \mathbf{r} + \frac{dV_y}{dt} \mathbf{j} + \frac{dV_z}{dt} \mathbf{k}$$

$$+ \frac{V_x}{dt} \mathbf{r} + \frac{1}{2} \mathbf{r} + \frac{1}{2} \mathbf{r} + \frac{1}{2} \mathbf{k}$$

$$= \frac{dV_x}{dt} - \frac{1}{2} \mathbf{r} + \frac{1}$$

Equation 4-261 is then equivalent to the following three scalar equations

$$\frac{w_{z}}{v_{t}} = -\frac{1}{2}w_{z} - \frac{1}{2}v_{z} - \frac{1}{2}v_{z}$$
 (4-265)

$$\frac{1}{24} = -\frac{1}{2} \frac{1}{2} $

$$\frac{x_{iz}}{x_{iz}} = -x_{ix} - x_{iy} - \overline{x} F_{z} \qquad (4-267)$$

where .

$$\overline{F} = \overline{\zeta}_{1} - \overline{\zeta}_{2} \qquad (4-268)$$

In a similar manner, Equation 4-81 leads to the following scalar equations

$$\frac{1+x}{3t} = \frac{1}{2}H_{3} - \frac{1}{2}H_{2} - \frac{1}{3}x$$
 (4-269)

$$\frac{d\mu}{dt} = -1_{z} H_{x} + 1_{z} H_{z} + 1_{z}$$
 (4-270)

$$\frac{t}{4t} = -\frac{1}{2} H_x - \frac{1}{2} H_y + \frac{1}{2} = (4-271)$$

where

$$H = H_{x} \gamma + H_{1} j + H_{z} k \qquad (4-272)$$

The relation between the total angular momentum, H , and the angular velocity vector,  $\Lambda$  , is obtained from Equation 4-209 using Equation 4-138

$$H_{\chi} = \lambda_{\chi_{\xi}} \Omega_{\chi} + \lambda_{\chi_{f}} \Omega_{\xi} + \lambda_{\xi} \beta_{\xi}^{2} [G_{\eta_{\xi}}] \{\dot{p}\} \qquad (4-273)$$

$$H_{y} = \lambda_{xy} \Omega_{x} + \lambda_{yy} \Omega_{y} + \lambda_{yz} \Omega_{z} - \lambda \{\beta \}' [G_{xz}] \{\beta \}$$

$$(4-274)$$

$$H_{z} = \lambda_{xz} \mathcal{A}_{x} + \lambda_{yz} \mathcal{A}_{y} + \lambda_{zz} \mathcal{A}_{z} + 2 \mathbb{I} p^{2} \mathcal{A}_{xy} + 2 \mathbb{I}$$

where the  $\lambda$ 's are given by Equations 4-150 through 4-155.

Finally, we want to derive a useful relation between the total force and moment and the generalized forces,  $P_i$ . From Equations 4-50, 4-65, and 4-78, we have

$$F = \int P dV + \oiint \Sigma \cdot iS$$
 (4-276)

$$G = \int (\mathbf{L} + \mathbf{p}) \times \mathbf{P} d\mathbf{Y} + \bigoplus (\mathbf{L} + \mathbf{p}) \times \mathbf{\Sigma} \cdot d\mathbf{S}$$
 (4-277)

$$P_{i} = \int h_{i} \cdot \mathbb{P} dV + \oiint h_{i} \cdot \Sigma \cdot dS \qquad (4-278)$$

From Equation 4-278, we have

$$\begin{split} \dot{i} \{ \hat{q}_{k} \}_{i}^{i} \{ P \} + j \{ q_{k} \}_{a}^{i} \{ P \} + k \{ q_{k} \}_{a}^{i} \{ P \} \\ &= \int (\dot{i} \{ q_{k} \}_{i}^{i} \{ h \} + j \{ q_{k} \}_{a}^{i} \{ h \} + k \{ q_{k} \}_{a}^{i} \{ h \} \} ) \cdot \mathbb{P} dV \\ &+ \oiint (\dot{i} \{ q_{k} \}_{i}^{i} \{ h \} + j \{ q_{k} \}_{a}^{i} \{ h \} + k \{ q_{k} \}_{a}^{i} \{ h \} ) \cdot \mathbb{E} \cdot dS \\ &= (\dot{i} \hat{i} + j j + k k) \cdot (\int \mathbb{P} dV + \oiint \mathbb{E} \cdot dS) \\ &= 1 \cdot \mathbb{F} \\ &= \mathbb{F} \end{split}$$

$$(4-279)$$

where use has been made of Equations 4-144 and 4-145. From this we conclude, by equating components, that

$$F_{\mu} = \{1, n\} + P\}$$
 (4-280)

$$F_2 = \{\{e_{ij}, j\}\}$$
(4-282)

The moment can be treated in a similar fashion if an approximation is made. From Equation 4-277, we have

$$G = \int L \times P dY - \iint L \times \Xi \cdot d\Xi$$
  
+  $\int g \cdot P dx - \iint g \cdot \Xi \cdot d\Xi$  (4-283)

The contribution of the last two terms to the total moment of forces is negligible if the displacements relative to the (i, j, k) reference are small. Approximately, then

$$G = \int C \cdot P dt + \frac{1}{2} E \times C \cdot ds$$
 (4-284)

Consider,

$$\begin{aligned} \hat{f}_{1} = \int F_{1} + \int f_{1} + \int f_{2} + \int$$

From this we conclude that

.

$$\bar{a}_{t} = \frac{1}{2} $

$$x_{y} = \frac{1}{2} \frac{1}$$

$$\dot{x}_{z} = -\frac{1}{2} \dot{\varphi}_{z} \dot{\xi}_{z}^{\prime} \dot{\xi} \rho_{J}^{\prime}$$

$$(h-268)$$

In summary, the complete set of equations governing the motion of the body are

$$\frac{t_{k_{z}}}{t_{z}} = -\frac{1}{2}t_{z} - \frac{1}{2}t_{z} + \frac{1}{t_{z}}t_{z}$$
 (4-289)

$$\frac{\lambda x_{2}}{\lambda z} = -\frac{1}{2}x_{2} + \frac{1}{2}x_{2} + \frac{1}{2}F_{2} +$$

$$\frac{2x_z}{x_z} = -\frac{1}{x_z} \frac{1}{x_z} - \frac{1}{x_z} \frac{1}{x$$

$$\frac{1}{35} = \frac{1}{2} - \frac{1}{2} - \frac{1}{3} + \frac{1$$

and

.

(4-295)

.

This can also be written as the single second order equation:

$$[A]{P} + [B]{P} + [K]{P} = [7]{P} - [3]{P} - \frac{1}{2} [3]{P} + [H]{P} + [K]$$
(4-297)

where we have

.

.

.

$$[\mathbf{F}] = -2 \sum_{x} [(\mathbf{x}_{yz}] + 2 \sum_{y} [\mathbf{x}_{xz}] - 2 \sum_{z} [(\mathbf{x}_{xy})]$$
(4-298)

$$[H] = \lambda \Gamma_{x}^{1}[H_{xy}] - 2 \Gamma_{x} \Gamma_{y}[H_{xy}] - 2 \Gamma_{x} \Gamma_{z}[H_{xz}]$$

$$\chi_{\tau_{y}}^{2}[H_{yy}] - 2 \Gamma_{y} \Gamma_{z}[H_{yz}]$$

$$\chi_{\tau_{z}}^{2}[H_{zz}]$$

$$(4-299)$$

anđ

$$-x = x_{4} - i_{x} + c_{xy} - i_{y} - i_{xz} - z + p + i_{yz} + z + (4-301)$$

$$-_{a} = \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^$$

.

where

$$\lambda_{xx} = I_{xx} + 2 \left[ I_{xx} \left[ A_{yz} \right] + \frac{1}{2} I_{xx} \left[ A_{yz} \right] + \frac{1}{2} I_{xx} \left[ A_{xz} \right] \left[ B \right] + 2 \left[ A_{xx} \right] \left[ A_{xx} \right] \left[ B \right] + 2 \left[ A_{xx} \right] \left[ A_{xx} \right] \left[ B \right] + 2 \left[ A_{xx} \right] \left[ A_{xx} \left[ A_{xx} \right] \left[ A_{xx} \right] \left[ A_{xx} \right] \left[ A_{xx} \left[ A_{xx} \right] \left[ A_{xx} \right] \left[ A_{xx} \right] \left[ A_{xx} \left[ A_{xx} \right] \left[ A_{xx} \right] \left[ A_{xx} \left[ A_{xx} \left[ A_{xx} \left[ A_{xx} \left[ A_{xx} \right] \left[ A_{xx} \left[ A_$$

$$\lambda_{xy} = -I_{xy} - I i_{y} i_{s} [Ay_{z}] i_{s} - i_{s} i [H_{y}] i_{s} i_{s}$$
 (4-305)

$$\chi_{x_{\overline{x}}} = -\sum_{x_{\overline{x}}} + \sum_{x_{\overline{x}}} \frac{1}{2} [A_{x_{x}}] \{p\} - \{p\} [H_{x_{\overline{x}}}] \{p\}$$
(4-306)

$$\lambda_{yy} = I_{yy} + 2 \left( \{ \varphi_R \}_{\delta}' [A_{xy}]' + \{ \varphi_R \}_{\delta}' [A_{xz}] \right) \{ p \} + 2 \{ p \}' [H_{yy}] \{ p \}$$
(4-307)

$$\lambda_{yz} = -I_{yz} + 2 \left\{ \{ \varphi_R \}_{b}^{\prime} [A_{xz}] \{ b \} - \{ b \}^{\prime} [H_{yz}] \{ b \} \right\}$$
(4-308)

$$\lambda_{zz} = I_{zz} + 2 \left\{ \left\{ \varphi_{R} \right\}_{6}^{\prime} \left[ A_{xy} \right]^{\prime} + \left\{ \varphi_{R} \right\}_{4}^{\prime} \left[ A_{yz} \right]^{\prime} \right\} \left\{ b \right\} + 2 \left\{ b \right\}^{\prime} \left[ H_{zz} \right] \left\{ b \right\}$$
(4-309)

and, finally, we have

•

.

.

•

•

$$F_x = \{\varphi_x\}_i'\{P\}$$
 (4-310)

$$\bar{\mathbf{y}} = \{\varphi_{\mathbf{R}}\}_{2} \{\mathbf{P}\} \tag{4-311}$$

$$F_{z} = \{\varphi_{R}\}_{\delta}^{\prime} \{P\}$$

$$(4-312)$$

$$G_{x} = \{ \varphi_{R} \}_{+}^{\prime} \{ P \}$$
 (4-313)

$$G_{y} = \{\varphi_{R}\}_{S} \{P\}$$
 (4-314)

$$G_{\ell} = \{\varphi_{R}\}_{\delta}^{\ell} \{P\}$$
 (4-315)

## 4.2 THE COMPLETE SIMULATION OF THE DYNAMICS OF A FLEXIBLE BOOST VEHICLE DURING THE "IN-FLIGHT" PHASE

In order to completely simulate the motion and stresses of a flexible boost vehicle from launch to a point outside of the earth's atmosphere, it is necessary to supplement the equations of motion developed in Section 4.1 with detailed descriptions of the forces. In addition, the trajectory and orientation of the body are required as well as the relations giving the internal "loads" which would be required, for example, in the detailed design of the structure.

## 4.2.1 The Differential Equations Governing the Orientation of the Body and the Trajectory of the Center of Mass

## 4.2.1.1 Euler Angles for the $(\nabla, \mathbf{J}, [k])$ Frame of Reference

It is convenient to use what are commonly called "pitch-roll-yaw" Euler angles. These angles are shown in Figure 67 and defined by the following equations:

$$\dot{f} = \omega_{S} \in \omega_{S} \notin I + sin \notin U - sin \in cos \# IK$$

$$(4-316)$$

-----

$$k = (\omega \varphi \sin \theta + (m \varphi \cos \theta \varphi))^{2} - \sin \varphi \cos \varphi J \qquad (4-318)$$
  
+  $(\omega \varphi \cos \theta - \sin \varphi \sin \theta \sin \varphi) K$ 



FIGURE 67 EULER ANGLES

These equations, in conjunction with Equations 4-26 and 4-157, lead to

.

$$\Omega_{x} = \dot{\varphi} + s_{in} \psi \dot{\varphi} \qquad (4-319)$$

$$\mathcal{A}_{q} = \cos \varphi \cos \varphi \, \hat{\Theta} + \sin \varphi \, \hat{\psi} \qquad (4-320)$$

$$n_z = (\omega \varphi \, \dot{\psi} - s m \varphi \, \omega s \varphi \, \dot{\varphi} \qquad (4-321)$$

Solving these equations for the Euler-angle rates, we obtain the following differential equations

$$\frac{dc}{dt} = R_x - \cos \varphi \tan z \, \lambda_y + \sin \varphi \tan \varphi \, \lambda_z \qquad (4-322)$$

$$\frac{dE}{d\Theta} = \frac{\cos \phi}{\cos \phi} x^2 - \frac{\cos \phi}{\sin \phi} x^2 \qquad (F-353)$$

$$\frac{d\psi}{dt} = \sin\varphi \mathcal{A}_{1} + \cos\varphi \mathcal{A}_{2} \qquad (4-324)$$

#### 4.2.1.2 Trajectory of the Center of Mass

A similar set of equations may be derived for the inertial coordinates of the center of mass. Using Equations 4-69 and 4-262, we have

$$I_{I} = \frac{\Delta E}{\Delta t} = \frac{\Delta E}{\Delta t} I_{I} + \frac{\Delta E}{\Delta t} J_{I} + \frac{\Delta E}{\Delta t} K_{I}$$
(4-325)

also,

$$V = V_x I - V_y J + V_z I_z$$
 (4-326)

Using Equations 4-316, 4-317 acl 4-313, we obtain

$$\frac{d3}{db} = \cos \theta \cos \psi V_x + (\sin \theta \sin \theta - \cos \theta \cos \theta \sin \psi) V_{\psi} + (\cos \theta \sin \theta + \sin \theta \cos \theta \sin \psi) V_z \qquad (4-327)$$

$$\frac{dI}{dt} = \sin \varphi V_x + \sin \varphi \cos \varphi V_y - \sin \varphi \cos \varphi V_z \qquad (4-323)$$

$$\frac{dI}{dt} = -\sin \varphi \cos \varphi V_x + \sin \varphi \sin \varphi + \sin \varphi \sin \varphi \sin \varphi \sin \varphi V_y \qquad (4-329)$$

Integration of these first order differential equations along with Equations 4-322, 4-323, and 4-324 will result in the time history of  $\phi$ ,  $\theta$ ,  $\psi$ ,  $\xi$ ,  $\eta$ , and  $\zeta$ ; and hence define the configuration of the "rigid body" reference at each instant of time.

#### 4.2.2 External Forces

The generalized forces, F<sub>i</sub>, arising from specific "forcing functions" such as gravity, zerodynamics, and control system can be derived separately and combined in an expression for the total virtual work of external forces.

The separate expressions for the generalized forces may be derived from the virtual work contribution by  $\delta p_1$ ,  $\delta p_2 \dots \delta p$ . The contribution of "rigid body" virtual displacements,  $\delta R$  and  $\delta \Theta$ , meed not be considered since the rigid body forces,  $F_X$ ,  $F_Y$ ,  $F_Z$ ,  $G_X$ ,  $G_Y$ , and  $G_Z$  are related to  $P_1$  by Equations 4-310 through 4-315. This is a distinct "conceptual" advantage (and a practical advantage from a machine computations standpoint) that results from using a redundant set of generalized coordinates. We only have to consider, then, the virtual work arising from virtual displacements,  $\delta [D$ , relative to the "rigid body" frame of reference. 4.2.2.1 Gravity Forces

If we assume that the origin is at the center of the earth and that this point is an inertial point, then the virtual work of gravity forces is given by

$$\delta W = -\int \frac{G \mathcal{X}_0 \mathbb{I} \cdot \delta \mathbb{P}}{|\Pi|^3} \varrho \, dV \qquad (4-330)$$

where  $\mathcal{M}$  is the mass of the earth.



FIGURE 68 GRAVITY FORCES

In the region of integration over the body, we have the very good approximation,

$$\mathbb{R} \doteq \mathbb{R}$$
  $|\mathbb{R}|^3 \doteq |\mathbb{R}|^3 = \mathbb{R}^3$  (4-331)

where

$$R = |R| = \sqrt{\xi^2 + \eta^2 + \zeta^2}$$
 (4-332)

so that

$$\delta W = -\frac{G \mathcal{M}}{R^3} \int \mathbb{R} \cdot \delta p \, e^{\frac{1}{2}} dV \qquad (4-333)$$

From Equations 4-123, 4-124, and 4-125

$$\delta p = \delta p_{x} \dot{i} + \delta p_{y} \dot{j} + \delta p_{z} k$$
  
= {\delta p}'{\delta\_{x}} \dot{i} + {\delta p}' \delta\_{y} d\_{y} d\_{

Substituting

.

$$s_{W} = -\frac{G_{N}}{R^{3}} \{ s_{p} \}' \left( \int \mathbb{R} \cdot \tilde{r} \{ h_{x} \}_{z} dV + \int \mathbb{R} \cdot \tilde{j} \{ h_{y} \}_{z} dV + \int \mathbb{R} \cdot \mathbb{R}$$

Using Equations 4-144 and 4-145, we can write

$$\int -r_{1} = -r_{2} =$$

$$\int dx_{2} dx_{3} = \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_$$

so that the virtual work of gravity forces is given by

$$s_{k} = -\frac{1}{k_{0}} + \frac{1}{2} + \frac$$

where, from Equations 4-316, 4-317, and 4-318, we have

$$\mathbb{E} - \tilde{p} = \mathcal{Z} = \mathcal{Z} + $

$$\begin{aligned} \mathbf{E} \cdot \mathbf{j} &= \mathbf{F} \cdot \mathbf{i} \cdot \mathbf{n} \cdot \mathbf{F} - \mathbf{c} \cdot \mathbf{c$$

`

and

.

$$R^{3} = \left( \tilde{s}^{2} + \eta^{2} + J^{2} \right)^{3/2}$$
 (14-3)+3)

4.2.2.2 Aerodynamic Forces

If we can idealize the vehicle by two "flat plates" as in Figure 69, then the distributed lift is

the side for ce is

and the drag can be written as



FIGURE 69 IDEALIZED DISTRIBUTION OF AERODYNAMICS FORCES

The total virtual work of aprodynamic forces is then

$$S_{N_{k}} = -\frac{1}{2} \frac{1}{2} \sqrt{n} \int C_{k} - C_{0} \dot{F} = \frac{1}{2} \frac{1$$

The "free-stream" direction is arbitrarily taken parallel to the V- axis. If W is the wind velocity, we have

$$v_{\rm m} = -\frac{4F}{25} + i$$
 (4-345)

If we assume linear, quasi-steady, zerodynamics then we have the following linear integral relations:

where  $\alpha(\xi, \eta, t)$  is the local angle-of-attack, and  $\beta(\xi, \zeta, t)$  is the local angle-of-sideslip. If we introduce

$$\begin{aligned} \mu(\mathbf{x}, \mathbf{y}, \mathbf{z}, t) &= \dot{\mathbf{p}} + \frac{\partial \mu_{i}}{\partial \mathbf{x}} \mathbf{j} + \frac{\partial \mu_{z}}{\partial \mathbf{x}} \mathbf{k} \\ \mathbf{p}(\mathbf{x}, \mathbf{y}, \mathbf{z}, t) &= \mathbf{j} - \frac{\partial \mu_{z}}{\partial \mathbf{x}} \mathbf{\bar{p}} \\ \mathbf{k}(\mathbf{x}, \mathbf{y}, \mathbf{z}, t) &= \mathbf{k} - \frac{\partial \mu_{z}}{\partial \mathbf{x}} \mathbf{\bar{p}} \end{aligned}$$

$$(4t-347)$$

which is a local set of unit vectors in the deformed body, then we can write

$$\mathcal{X}(\mathbf{x}, \mathbf{y}, \mathbf{r}) = - \frac{\left(\frac{\partial \mathbf{x}}{\partial \mathbf{r}} - \mathbf{W}\right) \cdot \mathbf{R}}{\left(\frac{\partial \mathbf{x}}{\partial \mathbf{r}} - \mathbf{W}\right) \cdot \mathbf{R}} \qquad (1 + -3 + 8)$$

and

$$\beta(x,z,t) = -\frac{\left(\frac{\partial u}{\partial t} - W\right) - V}{\left(\frac{\partial u}{\partial t} - W\right) - V} \qquad (4-349)$$



FIGURE 70 THE LOCAL ANGLE-OF-ATTACK



FIGURE 71 THE LOCAL ANGLE OF SIDESLIP

Now, from Equation 4-33

$$\frac{t}{t} = \frac{tP}{xt} + \mathcal{I} \times (\mathbb{L} + \mathbb{p}) + \dot{\mathbb{p}}$$
(4-350)

and

$$\frac{\partial h_{z}}{\partial b} = W_{z} + h_{z} = -\left(V_{x} - W_{x}\right) \frac{h_{z}}{h_{x}} + \left(V_{z} - W_{z}\right)$$

$$- \frac{\partial h_{z}}{\partial x} \left(-h_{y}\left(z + \frac{h_{z}}{z}\right) - h_{z}\left(y + \frac{h_{y}}{y}\right)\right)$$

$$+ -h_{x}\left(y + \frac{h_{y}}{y}\right) = -h_{y}\left(x + \frac{h_{x}}{z}\right)$$

$$- \frac{h_{x}}{zz} - \frac{h_{z}}{yz} + \frac{h_{z}}{zz}$$

$$+ \frac{h_{z}}{zz} + \frac{h_{z}}{z} + \frac{h_$$

(4-352)

If we neglect the nonlinear terms in the small displacements, we have, approximately,

$$(\frac{\partial T_{z}}{\partial E} - W) = \mathcal{K}_{z=0}$$

$$= - \mathcal{K}_{z} - \mathcal{K}_{z} \frac{\partial F_{z}}{\partial x} + (\mathcal{K}_{z} - \mathcal{K}_{z} - \mathcal{K}_{z}) + (\mathcal{K}_{z} - \mathcal{K}_{z})$$

$$(4-353)$$

$$\left[\frac{\partial \pi}{\partial \varepsilon} - W\right] \cdot \mathbb{P}_{y=0}$$

$$= -\left(V_x - W_x \cdot \frac{\partial \xi_y}{\partial x} + (V_y - W_y) - \varepsilon \mathbb{I}_x + x \mathbb{I}_z - \mathbb{I}_z\right]$$
(4-354)

Also, to this approximation in the expressions for  $\alpha$  and  $\beta$ , we have

$$\left(\frac{\partial \pi}{\partial t} - \gamma_{t}\right) = i_{t} - \gamma_{t} \qquad (4-355)$$

Substituting these experssions into Equations 4-348 and 4-349, we obtain

$$\alpha(x,y,t) = \frac{1}{2} - \frac{1}{2} + \frac{1$$

$$v_{\pm} = \frac{1}{2^{2}} - \frac{1}{12^{-1}} - \frac{1}{12^{-1}} - \frac{1}{2^{2}} - \frac{1}{12^{-1}} - \frac{1}{12^{-1}} - \frac{1}{12^{-1}}$$
(4-357)

The "rigid body" angle-of-attack and angle-of-sideslip are defined by

$$\alpha = \frac{V_{\pm}}{(4-358)}$$

We then have

•

.

$$2(x_1 z) = 2 - 4 \frac{2x}{y_1} - x \frac{1}{y_2} - \frac{1}{2x} + \frac{1}{2x} \frac{1}{y_1} \frac{1}{y_2}$$
 (4-360)

$$\frac{1}{2} = \frac{1}{2} + \frac{1}$$

Using Equations 4-144 and 4-145, we have

.

$$= -\frac{1}{r_{e}} - \frac{1}{r_{e}} - \frac{1}{r_{e}} - \frac{1}{r_{e}} + \frac{1}{r_{e}}$$

$$z + z = \frac{1}{2\pi} \frac{1}{2\pi} \frac{1}{2} \frac{1}{2\pi} + \frac{1}{2\pi} \frac{1}{2\pi} \frac{1}{2\pi} + \frac{1}{2\pi} $

The virtual work from Equation 4-344 is

.

$$\begin{aligned} \varepsilon_{R_{t}} &= -\frac{1}{2} \frac{1}{2} \left[ \int \zeta_{L} \varepsilon_{F_{z}}^{2} - \frac{1}{2} \varepsilon_{F_{x}}^{2} \varepsilon_{z=0} \right]^{d_{x}} ty \\ &= -\frac{1}{2} \frac{1}{2} $

Use of Equations 4-346 gives

.

•

$$SA = - \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{$$

Substituting Equations 4-362 and 4-363 yields

$$SW = -\frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_$$

where

$$\{L_{0}\} = \iint \{h_{z}\} C_{L_{0}} - \{h_{x}\} C_{p_{0}}\}_{z=0} dx dy$$
  
+ 
$$\iint \{h_{y}\} C_{Y_{0}}\}_{y=0} dx dz$$
(4-367)

$$[L_R] = \iiint \{h_z\} \{ \frac{\partial h_z}{\partial x} \}' L(x,y;s,r) dzdy cxdy$$

$$+ \iiint \{ h_y\} \{ \frac{\partial h_y}{\partial x} \}' Y(x,z;s,r) dzdy dxdz$$

$$(4-368)$$

$$[L_{I}] = \iiint \{h_{z}\}\{h_{z}\}' L(x,y;5,7) dzdydxdy \qquad (4-369)$$
  
+ 
$$\iiint \{h_{y}\}\{h_{y}\}' Y(x,z;3,s) dzdz dxdz$$

and use has been made of Equations 4-144 and 4-145.

## 4.2.2.3 Thrust Forces

General consideration of the distributed thrust forces is simplified because it can be assumed that they do not depend explicitly on the orientation of the "rigid body" reference. That is, they do not depend on  $\xi$ ,  $\eta$ ,  $\zeta$ ,  $\phi$ ,  $\theta$  or  $\psi$ . There is one exception to this, in that the ambient pressure,  $p_{\alpha}$ , depends on altitude,  $\sqrt{\xi^2 \cdot \eta^2 \cdot \zeta^2}$ , and the thrust forces are slightly influenced by the ambient pressure in the neighborhood of the rocket exhaust. On the basis that the generalized coordinates,  $p_1$ ,  $p_2$ ...mi, are "small," the following general linear expression can be assumed for the thrust forces.

$$Sw = -\frac{1}{5}Sp_{s}^{3} \left( \frac{1}{10} + \frac{1}{10} + \frac{1}{10} \right)$$
 (4-370)

where the coefficients  $\{H_0\}$  and [H] depend linearly on  $p_m$ 

The two important variations of thrust distribution with the generalized coordinates are:

o Change in thrust direction with body bending, and

o Change in thrust direction due to gimbaling of motor nozzles.

## 4.2.2.4 Control System Forces

It will be assumed that there are only three primary control coordinates: a roll control coordinate,  $\mu$ ; a pitch control coordinate,  $\gamma$ ; and a yaw control coordinate,  $\lambda$ . All three of these are related to the generalized coordinates describing the instantaneous configuration of the body relative to the rigid reference frame.

$$p = \frac{1}{2} + $

$$t^{2} = -(1)t^{2}t^{2}$$
 (4-372)

$$\lambda = f_{\lambda} \chi_{\lambda} \chi_{\lambda}$$

The servos that operate the control mechanisms exert forces on the structure. The virtual work of these forces is

$$\delta w = \delta \mu M + \delta \gamma \Gamma + \delta \lambda \Lambda \qquad (4-374)$$

The general statement of a "control law" specifies how  $\mathcal{M}$ ,  $\Gamma$ , and  $\Lambda$  depend on the outputs of sensors in the control system.

Substitution of Equations 4-371, 4-372, and 4-373 into Equation 4-374 gives the following expression for the virtual work of servo forces.

$$SN = \left\{ i \beta \right\} \left( \left\{ i \left[ \mu \right\} I \Lambda + i \left[ i \right]_{2} \right\} \Gamma + i \left[ \eta \right\} \Lambda \right) \right)$$

$$(4-375)$$

4.2.2.5 Summary of External Generalized Forces

In summary, the generalized forces defined in Equation 4-78 are obtained by adding the virtual work of the separate forces (Equations 4-339, 4-366, 4-370, and 4-375) to obtain

## 4.2.3 The Transformation to Modal Coordinates

In order to judiciously reduce the number of degrees-of-freedom, we consider Equations 4-297 in the case where

$$[\Gamma][A]{\dot{p}} = - \{\kappa]{\dot{p}} - [B]{\dot{p}} + \{p\}$$
(4-377)

Further, in order to derive the modes of free vibration, we consider the dissipation and external forces to be zero.

--

$$[\Gamma][A][b] = -[K][b]$$
 (4-378)

It will be convenient also to have a set of vibration modes with locked controls so that

$$\mu = i \{\mu\} \{ \beta\} = 0$$
 (4-379)

$$\hat{j} = \{z_{i}, j\} \{z_{i}\} = 0$$
 (4-380)

$$\lambda_{1} = \frac{1}{2} [\lambda_{1}^{2} + \beta_{1}^{2}] = 1$$
 (4-381)

A set of influence coefficients for the restrained system with locked controls can be derived in the form

$$[\varepsilon] = [s] [s]'[\kappa][s]'[s]'$$
(4-382)

where [S] is a constraint matrix that constrains the rigid body motion as well as the control motion. Then, from relations developed in Paragraph 2.2.3.4

 $\{\flat\} = -[\Gamma]'[\varepsilon][\Lambda][\Lambda][\check{\rho}]]$  (4-383)

separating variables,

$$\{ b \} = \{ i \} \}$$
 (4-384)

leads to the eigenvalue problem:

$$[P]'[E][P][A]= \sqrt{\frac{1}{2}}$$
(4-385)

whose solutions are the elastic modes and frequencies with locked controls

$$\exists \vec{i} \exists_{\vec{i}} \quad \omega_{\vec{i}} = \int_{\vec{\lambda}_{\vec{i}}} \quad \mathbf{i} = \mathbf{1}, 2 \dots \qquad (4-386)$$

The control modes are any values of the generalized coordinates representing displacements of the rigid system with a unit displacement in the control coordinate. Such a displacement is

$$(t_{1}) = \{ \psi_{r} \}_{r} \times - \{ \psi_{r} \}_{r} \times \{ \psi_{r} \}_{r} \}$$
 (4-387)

which are "rigid body" modes in the sense

$$[F_{i}]_{i} = [F_{i}]_{i} = [F_{i}]_{i} = [F_{i}]_{i} = \{0\}$$
(4-388)

These modes are not necessarily orthogonal to the rigid body modes, but a set can be constructed which is orthogonal to the rigid body moles. The derivation is similar to that in Paragraph 3.1.3.5, in particular, Equation 3-516.

We then consider the following transformation of coordinates

(4-392)

where

and

ι <sup>1</sup> .

The new coordinates satisfy the constraints, Equation 4-225, explicitly.

If we make this change of coordinates in Equations 4-295 and 4-296, we obtain

$$\{ \dot{a} \} = - \{ \phi \} [ \{ G \} \} \phi \} + \{ \phi \} [ \{ H \} \} \phi \}$$

$$- \{ \phi \} [ \{ \kappa \} \} \phi \} + \{ \phi \} \{ \kappa \}$$

$$- \{ \phi \} [ \{ \kappa \} \} \phi \} + \{ \phi \}$$

$$+ \{ \phi \} \}$$

$$(4-394)$$

where

$$\{\pi\} = [\alpha][A][\phi][\beta] + [\phi][G][\phi][\beta]$$
(4-395)

and

$$f_{4} = [1]_{-2}^{2}$$
 (4-396)

## 4.2.4 Transient Loads and Stresses

In the analysis of the structure by either the complementary energy method or the direct stiffness method (see Paragraph 5.1.1), there are two important results. First, the determination of the influence coefficients which are defined by the strain energy in terms of the generalized forces associated with the generalized coordinates  $p_1, p_2...p_1$ .

$$v = \frac{1}{2} \frac{1}{10} \frac{1}{10$$

(When the body is unrestrained, arbitrary constraints must be imposed to prevent rigid body motion and

$$[E] = [S] [ST[K][S]) [S]'$$

as discussed in Paragraph 2.2.3.4)

Second, the determination of the coefficients relating internal stress resultants (or "member loads") to the generalized forces,  $P_i$ .

The stress resultants,  $L_i$ , may be shears, moments, or torques in structural members like beams, plates, shells, and rods representing idealized portions of a complex built-up structure. In the case of a simply determinant structure, [R] depends only on the geometry of the structure (see, for example,

Paragraph 3.1.2.1, Equations 3-78 and 3-79). For a redundant structure, however, [R] depends also on the elastic characteristics of the system.

It is perhaps, intuitively clear that Equation 4-398 would lead to

$$\{L\} = [R][K][p] + [R][B][p]$$
 (4-399)

although this is very difficult to show systematically (see Paragraph 5.1.1.1 and the discussion leading to Equation 5-54).

From Equation 4-295, we have

Substituting this into Equation 4-399 using Equations 4-296 and 4-392, we obtain

$$\{L\} = [R][\Gamma](\{P\} - 2[G][\varphi]; \{i\} + [H][\varphi]; \{i\}]$$

$$-[G][\varphi][i] + [A][\varphi]; \{i\} - [X][\varphi]; \{i\} - [X][\varphi]; (4-401)$$

## 4.2.5 Final Equations of Motion

A summary of the equations of motion is given in Figure 72 for the case where the centrifugal and Coriolis effects of elastic deflection are neglected. In these equations a set of rigid modes which are mutually orthogonal has been introduced. The procedure for constructing such a set of modes belongs to the theory of the inertia dyadic for a rigid body. The orthogonal rigid body modes are denoted by:

$$\{\varphi_5\}, \{\varphi_7\}, \{\varphi_5\}, \{\varphi_6\}, \{\varphi_6\}, and \{\varphi_7\}$$
 (4-402)

Using the orthogonality properties of the rigid body modes, it can be shown that the gravity forces only influence the "translation" equations. The generalized forces,  $P_i$ , in Figure 72 have been redefined to take advantage of this.

$[p] [mH\ddot{g}_{1} + [RH\dot{g}_{1} + [F]fg_{1} = [g]^{l}fP]$	$+ \frac{1}{M} \{ \varphi_n \}' \{ p \}$	$D + \frac{1}{M} \{ q_x \}^{i_1^* j_1} P \} \qquad	$ \begin{array}{l} \left( L_{R} H P J + \{L_{O} J \right) \\ \mathfrak{R}_{O}^{d} \left[ L_{I} \right] \left( f \left\{ \boldsymbol{p}_{J} J \right\}  \alpha  + \left\{ \boldsymbol{p}_{N} J R + \left\{ \boldsymbol{q}_{\varphi} \right\}  \frac{\Delta x}{V_{D}}  + \left\{ q_{\varphi} \right\}  \frac{\Omega y}{V_{D}} + \left\{ q_{\varphi} \right\}  \frac{\Omega y}{V_{D}} \\ \left( L_{I} \right) \left( f \left\{ \boldsymbol{p}_{J} \right\}  \alpha  + \left\{ Y p_{P} \right\}  \Gamma  + \left\{ Y p_{A} \right\}  \Delta \\ \left( H_{O} \right)  +  \left\{ Y p_{P} \right\}  \mathcal{M}  +  \left\{ Y p_{P} \right\}  \Gamma  +  \left\{ Y p_{A} \right\}  \Delta \end{array} $		$W_{x} = \cos \varphi  W_{\xi} + \sin \psi  W_{\eta} - \sin \varphi  \cos \psi  W_{\xi}$	W <sub>4</sub> = /sinφsin θ - cosq cose sin φ)W5 + cosq cos φW <sub>4</sub> + (sinφ cose + cosq sin e sin φ)W5	W <sub>ε</sub> = (carpsime + sin φ case sin ψ)W <sub>ε</sub> - sin φ cas ψ W <sub>η</sub> + ( case care - sin φ sin ψ)W <sub>τ</sub>	$v_{\rm m} = W_{\rm s} - V_{\rm s}$	- = [R](נף} – [A](לפּא)			FOR THE GENERAL MOTION OF A FLEXIBLE LAUNCH VENICLE
$\frac{dV_x}{dt} = \Omega_x V_{ty} - \Omega_y V_z - \frac{G_{\lambda L}}{M} \frac{3\cos \theta \cos \psi + \gamma \sin \psi}{(s^3 + \gamma^2 + \gamma^2)^{3/2}} + \frac{1}{M} \xi \rho_3^{3/2} f^{-1/2}$	$\frac{dV_{H}}{dt} = \Omega_{X}V_{E} - \Omega_{z}V_{X} - \frac{q_{z}M_{z}}{M} - \frac{1}{2}(\log \cos^{2}\psi) + \frac{1}{2}(\sin \psi \cos \theta + \cos^{2}\psi \sin \theta)^{3}$	$\frac{dV_{z}}{dt} = \alpha_{y} V_{x} - \alpha_{x} V_{y} - \frac{G}{M} \frac{M}{(5^{2} + \gamma^{2} + \gamma^{2})^{3}}$	$\frac{\varphi_{1}}{\varphi_{1}} = \frac{1}{1} $	$-\frac{4\Omega_{\rm H}}{2} - \frac{4\Omega_{\rm H}}{2} - \frac{4\Omega_{\rm H}}{2} + \frac{1}{2} + $	$\frac{d\Omega_{z}}{dt} = \frac{I_{xx} - I_{yx}}{1 - t_{xx}} \Omega_{y} \Omega_{x} + \frac{1}{T_{xx}} \{\varphi_{y}\} \{P\}$	$\sum_{dt} \frac{d\xi}{dt} = \cos \cos \sqrt{\lambda_x} + (\sin \varphi_{1,m} - \cos \varphi_{1} - \cos \varphi_{1,m} - (\cos \varphi_{1,m} - \varphi_{1,m}) V_{t}$	<del>άν</del> – simpV <sub>x</sub> + cosp cos φVy - simp cos φVz	dJ = -simecosφV <sub>x</sub> +(simφcore + cosφsimesimφ)Vy dt = -simpeosφV <sub>x</sub> +(cosφcore - simpsimesimφ)V <sub>z</sub>	$\frac{dt}{dt} = \Lambda_x - \cosh \log \psi  \Omega_y + \sin \varphi  \log \psi  \Omega_t$	$\frac{d\mathbf{r}}{d\mathbf{r}} = \frac{\cos \phi}{\cos \phi} \mathcal{D}_{\mathbf{r}} - \frac{\sin \phi}{\sin \phi} \mathcal{D}_{\mathbf{s}}$	$z_{U} h scn + h_{U} h m s = \frac{m}{m}$	FIGHRE 72 EQUATIONS

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5.0 METHODS TO OBTAIN VIBRATION MODES FOR LARGE REDUNDANTLY COUPLED STRUCTURES

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## 5.1 THE PROBLEM OF VIBRATION ANALYSIS IN ANALYTICAL MECHANICS

In the light of the general principles of analytical mechanics, the problem of vibration analysis is distinctly divided into two problems of lesser scope. The first problem involves the consideration of the strain energy of the system, which defines the stiffness matrix or the matrix of structural influence coefficients. The second problem involves consideration of the kinetic energy of the system, which defines the inertia matrix. The problem of vibrations in these general terms has been considered in Section 2.2.

In Paragraph 5.1.1 below, we want to consider the first of these problems. We shall use the term "structural analysis" to mean the determination of structural influence coefficients as well as the determination of the transformation matrix which defines the internal stress resultants or "member" loads in terms of the applied generalized forces.

In Paragraph 5.1.2, we turn to the second problem in vibrations and consider some techniques for deriving inertia matrices for specific types of structures.

We shall restrict our attention, in this section, to the linear analysis of structures because the general theory of vibrations is predicated on the assumption of linear motions from an equilibrium position. This is not intended to imply, however, that the problem of large deflections is unimportant in strength calculations for some flexible aerospace structures.

#### 5.1.1 General Methods of Structural Analysis

The general study of the deformation of linear structures is largely divided into two basic methods. The first method deals with the strain energy expressed in terms of generalized coordinates and is commonly called the "Direct Stiffness" method (or "Matrix-Displacement" method). The second method deals with the strain energy expressed in terms of applied loads and is commonly called the "Complementary Energy" method (or "Matrix-Force" method).

Both of these basic methods can be applied in a routine manner to arbitrary linear structures and both methods have their advantages and disadvantages. It can be safely said that any problem that can be worked by one method can be worked by the other method. The equivalence in generality of the two approaches is demonstrated in Paragraphs 5.1.1.1 and 5.1.1.2.

Both methods employ the philosophy of dividing a complicated structure into a number of small, simple, structural elements for which the stiffness properties are known. The following definitions are appropriate for both methods:

"element" - one- or two-dimensional structure whose motion and stressstate are well defined by loads acting only at the "ends." An element is a subdivision of a member. "member" - a piece of a structure with no redundant load paths. A member is a subdivision of the whole structure.

The mode of subdivision is arbitrary and is left, somewhat, to engineering judgement.



FIGURE 73 SUBDIVISIONS OF STRUCTURE INTO "MEMBERS" AND "ELEMENTS"

If u is the specific internal energy of a particle of the structure, the total strain energy of the whole structure is

$$U = \int u \, dV \tag{5-1}$$

This leads to the important additive property that the total strain energy is the sum of the energy of the members which is in turn the sum of the energies of the elements. That is, if

$$\cup_{ij} = \int_{(i,j)} \sum_{(i,j)} \sum_{(i,j)} \sum_{(j,j)} \sum_{(j$$

where.

$$\int_{t_{ij}} \int dY$$
(5-3)

is an integration over the region occupied by the  $i^{th}$  element of the  $j^{th}$  member then, since

$$\int ( ) dV = \sum_{j \in i} \sum_{(i,j)} ( ) dV$$
(5-4)

we have

$$U = \sum_{j \in i} \sum_{i} U_{ij}$$
 (5-5)

Now, if  $\{Q_{j}\}_{ij}$  is a set of generalized coordinates which describe the configuration of the i<sup>th</sup> element of the j<sup>th</sup> member then we know that the strain energy of this element is of the general form:

$$U_{ij} = \frac{1}{2} \overline{f}_{ij} \overline{f}_{ij} \overline{f}_{ij} \overline{f}_{ij} \overline{f}_{ij} \overline{f}_{ij} \overline{f}_{ij}$$
(5-6)

which defines the element stiffness matrix,  $[F]_{ij}$ . This matrix is, in general, singular because the coordinates,  $\{q\}_{ij}$ , describe a general motion of the element, in particular, rigid body motion. If we introduce a constraint to prevent rigid body motion, then we may derive an element influence matrix in the form:

$$[C]_{ij} = ([S]'_{ij}[F]_{ij}[S]_{ij})^{-1}$$
(5-7)

From equilibrium of loads on the free element, we can derive a transformation which eliminates some of the loads. Such a transformation can be used to obtain an unrestrained stiffness matrix from the element influence matrix

$$[\mathbf{F}]_{ij} = [\mathbf{x}]_{ij}^{*} [\mathbf{F}]_{ij}^{*} [\mathbf{V}]_{ij}$$
(5-8)

Using Lagrange's equations,

 $\frac{\partial U}{\partial q_{R}} = Q_{R}$  (5-9)

we obtain, from Equation 5-6,

$$[F]_{ij} \{q\}_{ij} = \{Q\}_{ij}$$
 (5-10)

or, subject to the constraint of rigid body motion (see Paragraph 2.2.3.4),

$$\{q\}_{ij} = [S]_{ij} ([S]_{ij} [F]_{ij} [S]_{ij})^{-1} [S]_{ij} \{Q\}_{ij}$$
(5-11)
If we introduce the internal stress resultants:

$$\{L\}_{ij} = [S]_{ij}^{\prime} \{Q\}_{ij}$$
(5-12)

Then we can write

$$\{v_{i}\}_{ij} = [S]_{ij} \{v_{i}\}_{ij}$$
(5-13)

Substitution into Equation 5-6 gives

$$U_{ij} = \frac{1}{2} \{ L_{ij}^{j} [G]_{ij}^{\prime} [S]_{ij}^{\prime} [F]_{ij} [S]_{ij}^{\prime} [G]_{ij}^{\prime} \{L_{ij}^{j}\}$$
(5-14)

but

$$[G]'_{ij}[S]'_{ij}[F]_{ij}[S]_{ij} = -1$$
, (5-15)

so that

$$U_{ij} = \frac{1}{2} \{ L_{ij}^{j} [G]_{ij} \{ L_{ij} \}$$
 (5-16)

which is the element strain energy in terms of internal stress resultants.

It may be worthwhile to mention that the above "complementary energy" expression depends on the arbitrary constraints imposed on the element. The representation in Equation 5-6, however, is unique and, in a sense, more basic than Equation 5-16.

## 5.1.1.1 The Direct Stiffness Method

The aggregate of all the coordinates of all the elements of all the members represents a set of "internal" coordinates which are not consistent with the kinematic constraints demanding displacement and slope continuity between adjacent elements. For each member we can make a transformation which introduces the proper constraints between elements. If  $\{q\}_j$  is a set of generalized coordinates for the  $j^{\text{th}}$  member which is consistent with all the constraints on the  $j^{\text{th}}$  member, then there is a transformation of the form

$$i_{j}i_{ij} = [r]_{ij}i_{ij}i_{j}$$
 (5-17)

The matrix of the transformation is called the element compatibility matrix. Figure 74 illustrates the typical procedure for deriving the compatibility matrices.



FIGURE 74 DISPLACEMENT CONTINUITY AT A JOINT BETWEEN ELEMENTS OF A MEMBER

If Equation 5-17 is substituted into Equation 5-6, we obtain

$$L_{ij} = \sum_{i=1}^{j} \{ j_{ij} \}_{j} [T_{ij}] [T_{ij}] [T_{ij}] [T_{ij}] [T_{ij}] [T_{ij}] ]$$
(5-18)

The total strain energy in the jth member is then

or

$$u_{j} = \frac{1}{2} \xi_{j} $

where

$$[F]_{j} = \prod_{i} [T]_{ij} [F]_{ij} [T]_{ij}$$
(5-21)

which is the "composite" stiffness matrix of the  $j^{th}$  member. Finally, we introduce the constraints between members. If {q} is a set of coordinates consistent with the constraints in the whole structure, then there exists a compatibility transformation of the form

$$\hat{z}_{ij} \hat{z}_{j} = \begin{bmatrix} z \\ z \\ z \end{bmatrix} \hat{z}_{ij} \hat{z}_{j}$$
 (5-22)

such that the total strain energy in the whole structure is

$$= \bigcup_{j=1}^{n} \bigcup_$$

or

where

In the process of eliminating coordinates to insure structural continuity, the final composite set of generalized coordinates is far more than the number necessary to describe the 'external' configuration of the structure. If  $p_i$ , i = 1, 2...N, is a subset of the q's that are sufficient to describe the external configuration of the system, then the number of legrees-of-freedom may be reduced by assuming that the "internal" applied loads are zero. If the coordinates are arranged so that

and  $\{q_2\}$  is the set of coordinates to be eliminated, then Equation 5-24 is partitioned so that

The generalized forces associated with  $\{q_2\}$  are the "internal" applied loads which are assumed to be zero.

$$\{z_{12}\} = \{z_{12}^{N}\} = \{F_{2}, H_{12}^{n}\} + \{F_{12}\} \{z_{12}\} = \{z\}$$
(5-28)

Subject to this constraint, we have

$$\{j_{z}\} = -[F_{az}][F_{a}][i_{\theta}]\}$$
 (5-29)

or

$$\begin{bmatrix} f_{2_{1}} \\ f_{q_{2}} \end{bmatrix} = \begin{bmatrix} f_{2_{1}} \\ - \begin{bmatrix} F_{2_{2}} \end{bmatrix} \begin{bmatrix} f_{p_{2}} \\ F_{2_{2}} \end{bmatrix}$$
 (5-30)

or, finally,

where

$$\begin{bmatrix} T \end{bmatrix} = \begin{bmatrix} r_{1}, \\ -[F_{22}][F_{23}] \end{bmatrix}$$
(5-32)

Substituting Equation 5-30 into Equation 5-27 gives

(5-33)

1- -->

. . .

$$U = \frac{1}{2} \left\{ p \right\}^{\prime} \left[ \begin{bmatrix} f_{1,1} & - \begin{bmatrix} F_{21} \end{bmatrix}^{\prime} \begin{bmatrix} F_{12} \end{bmatrix}^{\prime} \right] \left[ \begin{bmatrix} F_{11} & \begin{bmatrix} F_{12} \end{bmatrix} \end{bmatrix} \begin{bmatrix} f_{1,1} & \\ & &$$

or

where the stiffness matrix for the structure is given by

$$[\kappa] = [\Gamma] - [\Gamma_{\alpha}][\Gamma_{\alpha}][\Gamma_{\alpha}]$$

The structural influence coefficients of the system are given by

$$[\varepsilon] = [\kappa]^{\dagger}$$
 (5-36)

if the structure is constrained. If the structure is not constrained, a set of influence coefficients can be derived for an arbitrary constraint which is just sufficient to prevent rigid body motion (see Faragraph 2.2.3.4).

$$[E] = [S][U][K]S][U]$$
 (5-37)

For statically applied loads, we may relate the internal stress resultants

$$f_{-ij} = \{S\}_{ij} + Q\}_{ij}$$

to the externally applied loads, {P}, which are the generalized forces associated with the generalized coordinates, {p}. This is done in the following manner. By successive substitution of Equations 5-10, 5-17, 5-22, and 5-31 into 5-32, we have

$$\begin{aligned} f_{-j}_{ij} &= [j]_{ij}^{\prime} \{ Q_{j} \}_{ij} \\ &= [j]_{ij}^{\prime} \{ F_{ij} \}_{ij} \{ Q_{j} \}_{ij} \\ &= [j]_{ij}^{\prime} \{ F_{ij} \}_{ij} [T]_{ij} \{ Q_{j} \}_{j} \\ &= [j]_{ij}^{\prime} \{ F_{ij} \}_{ij} [T]_{ij} \{ T_{j} \}_{j} \{ Q_{j} \}_{j} \\ &= [j]_{ij}^{\prime} \{ F_{ij} \}_{ij} [T]_{ij} [T]_{ij} \{ Q_{j} \}_{j} \\ &= [j]_{ij}^{\prime} \{ F_{ij} \}_{ij} [T]_{ij} [T]_{ij} [T]_{ij} \{ Q_{j} \}_{j} \\ &= [j]_{ij}^{\prime} \{ F_{ij} \}_{ij} [T]_{ij} [T]_{ij} [T]_{ij} \{ Q_{j} \}_{j} \\ &= [j]_{ij}^{\prime} \{ F_{ij} \}_{ij} [T]_{ij} [T]_{ij} [T]_{ij} \{ Q_{j} \}_{j} \\ &= [J]_{ij}^{\prime} \{ F_{ij} \}_{ij} [T]_{ij} [T]_{ij} [T]_{ij} \{ Q_{j} \}_{j} \\ &= [J]_{ij}^{\prime} \{ F_{ij} \}_{ij} [T]_{ij} [T]_{ij} [T]_{ij} [T]_{ij} \{ Q_{j} \}_{j} \\ &= [J]_{ij}^{\prime} \{ F_{ij} \}_{ij} [T]_{ij} [T]_{ij} [T]_{ij} [T]_{ij} [T]_{ij} \{ Q_{j} \}_{j} \\ &= [J]_{ij}^{\prime} \{ F_{ij} \}_{ij} [T]_{ij} [T]$$

The last substitution, based on

is valid for restrained or unrestrained systems because in the general solution (see Equation 2-245 in Faragraph 2.2.3.4) the rigid body portion does not contribute to the stress resultants. All of the pertinent internal reactions for computing stresses for a given external loading can be selected from the aggregate of the  $\{L\}_{ij}$  for all the elements of all the members. These can then be related by a single transformation by using Equation 5-39.

$$\{.\} = [R] \{ P \}$$
 (5-41)

where {L} is a matrix whose elements have been selected from all of the  $\{L\}_{ij}$  and the rows of [R] are the corresponding rows from the matrices

(5-42)

For dynamically applied loads, we have from Lagrange's equations for an element

$$\{x\}_{ij} = [x_{ij}, x_{ij}] + [x_{ij}, x_{ij}] + [x_{ij}, x_{ij}] + [x_{ij}, x_{ij}] \}$$
 (5-43)

ani the stress resultants are

where the inertia terms are presumed to vanish on the basis that the stress resultants depend only on the strains and strain rates (see also Faragraph 4.1.4, Equations 4-193 through 4-195). Now, as in the static case,

and, by differentiating

$$-12_{1} = [-1, -1, -1]$$
 (5-46)

Substituting these into Equation 5-44 gives

If we introduce

.

$$[1] = [\kappa][\kappa]$$
 (5-48)

in the first term and

$$[1] = [3][8]$$
 (5-49)

in the second term, then

$$\begin{aligned} \{ L_{ij}^{k} &= \left[ s_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \right] \right] \right] \right] \\ &+ \left[ \left[ s_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \right] \right] \right] \right] \right] \\ &+ \left[ \left[ s_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \right] \right] \right] \right] \right] \\ &+ \left[ \left[ s_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \right] \right] \right] \right] \right] \\ &+ \left[ \left[ s_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \right] \right] \right] \right] \right] \right] \\ &+ \left[ \left[ s_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \right] \right] \right] \right] \right] \\ &+ \left[ \left[ s_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \left[ T_{ij}^{k} \right] \right] \right] \right] \right] \right] \\ &+ \left[ \left[ s_{ij}^{k} \left[ T_{ij}^{k} \right] \right] \right] \right] \right] \right] \right] \\ &+ \left[ \left[ s_{ij}^{k} \left[ T_{ij}^{k} $

Now in the simple case:

$$[R]_{ij} = e^{[F]_{ij}}$$
 (5-51)

we have

$$\begin{bmatrix} \mathcal{R} \end{bmatrix} = \sum_{i} \sum_{j} [\tau J [\tau]_{j}' [\tau]_{j}' [\pi]_{ij} [\tau]_{ij} [\tau]_{j} [\tau]_{j} [\tau]$$

$$= \sum_{i} \sum_{j} [\tau J [\tau]_{j}' [\tau]_{j}' [\tau]_{j} [\tau]_{j} [\tau]_{j} [\tau]$$

$$= p [\kappa]$$
(5-52)

so that

If we suppose this to be generally true, then

$$\exists \iota \}_{ij} = [\Box]_{ij} [F]_{ij} [T]_{ij} [T]_{j} [T] [E] ([\kappa]_{ij} ] + [B] [j],$$

or, for the pertinent internal stress resultants, we have

$$\{L\} = [R]([\kappa]\{b\} + [B]\{b\})$$
 (5-54)

where [R] is the same matrix that appears in Equation 5-41. It is felt intuitively that this could be proved to be true without making the assumption in Equation 5-51.

## 5.1.1.2 The Complementary Energy Method

The fundamental starting point for the complementary energy method is Equation 5-16

$$v_{ij} = \frac{1}{2} \{ L \}_{ij} \{ [G]_{ij} \{ L \}_{ij} \}$$
 (5-55)

In this equation the stress resultants in an element can be related to the applied loads and reactions on the whole member to which the element belongs.

$$\{\iota\}_{ij} = [C]_{ij} \{Q\}_{j}$$
 (5-56)

This transformation is based on consideration of equilibrium of a free body which is a portion of the member excluding the i<sup>th</sup> element. The fact that such a free body exists which is acted upon by only the  $\{L_{ij}^j$  and  $\{Q\}_j$  follows from the definition of a member as a portion of the structure with no redundant load paths. Figure 75 illustrates the typical procedure for deriving the "free-body" matrices.





If Equation 5-56 is substituted into Equation 5-55, we obtain the following expression for the total strain energy in the  $j^{th}$  member.

$$u_{j} = \sum_{i} u_{ij} = \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \sum_{i} [2l_{ij}(a)_{ij$$

or

$$J_{1} = \frac{1}{2} \frac{1}{4} \frac{1}{2} \frac{1}{4} \frac{1}{2} \frac{1}{4} \frac{1}{2} \frac{1}{4} \frac{1}{2} \frac{1}{4} \frac{1}{2} \frac{1}{4} \frac{1}$$

(- ---)

where

 $[\mathbf{x}]_{\mathbf{y}} = \sum_{\mathbf{y}} [\mathbf{x}]_{\mathbf{y}} [\mathbf{x}]_{\mathbf{y}} [\mathbf{z}]_{\mathbf{y}}$ (5-59)

In addition to this we have a relation between the  $\{{\tt l}\}_{,j}$  based on equilibrium of the whole member

The number of rows in  $[L]_j$  will be no greater than six for any given member, corresponding to the six equilibrium relations for an arbitrary free body.

We finally introduce a "bookkeeping" transformation which relates the  $\{z_i\}_j$  to the aggregate of all the loads on all the rembers,  $\{z_i\}$ . These loads

are composed of internal loads and external loads. The external loads,  $\{P\}$ , are those generalized forces associated with generalized coordinates which are sufficient to describe the "external" configuration. For restrained systems, the 2's must include the reactions from either determinant or redundant constraints. For unrestrained systems the Q's must include the reactions at some arbitrary determinant constraint (i.e., at some arbitrary constraint just sufficient to prevent rigid body motion). The "bookkeeping" transformation is of the form

$$\{\varphi_{j}^{*} = [\mathcal{C}]_{j} \{\mathcal{A}\}$$

$$(5-61)$$

Substitution into Equations 5-58 and 5-60 yields

$$J = \frac{1}{2} \{Q\} \{G\} \{Q\} \}$$
 (5-62)

$$[L]{Q} = \{c\}$$
 (5-63)

where

and



If we suppose that the  $\{i,j\}$  can be partitioned so that

where

 $\{l_1\} = \{P\} = generalized forces associated with the "external" generalized coordinates$ 

{ white "redundant" loads

 $\{ \downarrow_3 \}$  = loads that can be determined from the equilibrium equations, then the equilibrium equations can then be written as

$$\{-\frac{1}{2}, \frac{1}{2}, $

and the "determinant" loads may be solved for

$$-3_{3} = -[-_{3}] - [-_{$$

This may be rewritten as

$$\begin{aligned} & \{Q_i\} = \begin{bmatrix} 1 \\ Q_i\} = \begin{bmatrix} 1 \\ Q_i \end{bmatrix}, \begin{bmatrix} 1$$

or as

$$isi = [C, Hs_1i + [C_2, Hs_2]$$
 (5-70)

Substitution into the strain energy gives

$$U = \chi \left[ \{ i_{2}, j \} \{ j_{2}, j \} \right] \left[ j_{3}, j \} \left[ j_{2}, j \} \right] \left[ j_{2}, j \} \left[ j_{2}, j \} \right] \left[ j_{2}, j \} \right]$$
(5-71)

where

.

Now, from Castigliano's theorem

and Castigliano's second theorem essentially requires internal structural continuity by stating that  $\{q_2\} = \{0\}$ .

In Equation 5-71

$$\left[\frac{2}{3x_2}\right] = 13^3$$
 (5-77)

then leads to

$$[x_{1}] = \{-1, x_{21}\} = \{z\}$$
 (5-78)

from which

$$\{z_{1}\} = -[z_{1}, y_{2}, y_{3}] \{z_{0}\}$$
(5-79)

or

$$\begin{bmatrix} -1 & 1 & 1 \\ -1 & 1 & 1 \\ -1 & 1 & 1 \end{bmatrix}$$
 (5-80)  
337

Substitution into Equation 5-71 gives

$$U = \frac{1}{2} \left\{ P_{j}^{2} \left[ \left[ 1 \right]_{j} - \left[ G_{12} \right] \left[ G_{22} \right]^{2} \right] \left[ G_{21} \right] \left[ G_{21} \right] \left[ \left[ G_{21} \right] \right] \left[ -\left[ G_{21} \right]^{2} \left[ G_{22} \right] \right] \right] - \left[ G_{21} \right]^{2} \left[ G_{22} \right] \left[ G_{22} \right] \left[ G_{22} \right] \left[ G_{22} \right] \right] - \left[ G_{22} \right]^{2} \left[ G_{22} \right] \right] - \left[ G_{22} \right] \left[ G_$$

or

$$U = \frac{1}{2} \{P\}^{r}[E]\{P\}$$
 (5-82)

where the matrix of structural influence coefficients is given by

$$[E] = [G_{11}] - [G_{12}][G_{21}][G_{21}]$$
(5-83)

The stiffness matrix for the system is then

$$[k] = [k]'$$
 (5-84)

if the system is restrained. If the structure is not restrained, then the arbitrary constraints imposed on [E] may be uniquely removed by the following procedure.

For the unrestrained structure the loads,  $\{P\}$ , must satisfy equilibrium relations which can be expressed as

Using these equations, some of the loads may be expressed in terms of the rest of the loads

$$\frac{1}{2} = -[L_2][L_1][P_1]$$
 (5-86)

or

$$\{P\} = \begin{bmatrix} \{P_i\} \end{bmatrix} = \begin{bmatrix} r_1 \\ - \lfloor v_2 \rfloor \end{bmatrix} \{P_i\}$$
(5-87)

or

$$\{P\} = [V]\{P\};$$
 (5-88)

Now,

$$\{b\} = [E]\{P\}$$
 (5-89)

and premultiplying by (V)' and substituting Equation 5-88 gives

$$[V]_{P} = [V]_{E} [V]_{R}$$
 (5-90)

or

Substitution into Equation 5-88 gives

$$\{v\} = [v] ([v]'[E][v])^{-1} [v]' \{ \}$$
(5-92)

so that

$$[\kappa] = [v]([v]'[\varepsilon][v])''$$
(5-93)

.

for an unrestrained structure.

.

To relate the internal stress resultants to the applied loads, we note, first, that Equation 5-70 and Equation 5-80 can be combined to give

$$\{o\} = [c]\{p\}$$
 (5-94)

where

$$\begin{bmatrix} C \end{bmatrix} = \begin{bmatrix} [C_1], [C_2] \end{bmatrix} \begin{bmatrix} A_1 \\ - \begin{bmatrix} C_{022} \end{bmatrix} \begin{bmatrix} C_{02} \end{bmatrix} \begin{bmatrix} C_{$$

By successive substitution of Equations 5-ol and 5-94 into Equation 5-56, we have

$$\begin{aligned} \frac{1}{2} L^{2} L_{ij} = \left\{ c \right\}_{ij} \left\{ c \} \left\{ c \right\}_{ij} \left\{ c \} \left\{ c \} \left\{ c \} \left\{ c \right\}_{ij} \left\{ c \} \left\{ c \right\} \left\{ c \} \left\{ c \right\} \left\{ c \right\} \left\{ c \right\} \left\{ c \} \left\{ c \right\} \left\{ c \right\} \left\{ c \right\} \left\{ c \} \left\{ c \} \left\{ c \} \left\{ c \right\} \left\{ c$$

If {L} is a matrix of pertinent stress resultants selected from the aggregate of the  $\{L\}_{i,j}$ , then

$$L_{F} = [F, HP]^{2}$$
 (5-97)

where the rows of [k] are selected from the rows of the matrix

# [c];[c];[c]

Figure 76 summarizes the operations for both the direct stiffness and the complementary energy methods of structural analysis.

$$\begin{split} \upsilon_{ij} &= \frac{1}{2} f_{ij} f_{ij} [r^{1} J_{ij} f_{ij} J_{ij} \\ \upsilon_{ij} &= \frac{1}{2} f_{ij} f_{ij} f_{ij} [r^{1} J_{ij} f_{ij} J_{ij} \\ (r^{1} J_{ij} [r^{1} J_{ij} ]r^{1} J_{ij} ]r^{1} J_{ij} [r^{1} J_{ij} ]r^{1} $

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•

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#### 5.1.1.3 Some General Considerations of Beam Theory

Almost all primary structural-load-carrying components in aerospace designs can be idealized as a beam. Figure 77 is intended to be a typical example.



# FIGURE 77 STRUCTURE IDEALIZED AS A BEAM

The beam model as an approximation affords a convenient structural analysis when time and/or detailed information is limited.

In order to be precise and to indicate the limits of applicability, we shall define a beam to be a structure symmetric about the x-y plane and having a median axis whose radius of curvature is at least greater than the width normal to the median  $axis^{1}$ . The section normal to the axis is assumed to be structurally connected as shown by Figure 70.

This essumption is make so that the median axis will have a well defined normal at each point.



FIGURE 78 GEOMETRY OF A BEAM

We may introduce then a curvilinear coordinate system  $(\mu, \kappa, \nu)$  with  $\mu$  measured along the arc of the median axis and  $\kappa, \nu$  in the plane of a section normal to the axis.

The fundamental assumption of beam theory is that the only nonzero stresses on a section are:

 $\sigma_{\mu\mu}$ , the stress normal to the section

 $\sigma_{\!\mu \, \kappa}$ , vertical shear stress in the plane of the section

 $\sigma_{\mu\nu}$ , lateral shear stress in the plane of the section

The specific strain energy is then

$$u = \frac{1}{2} \left( \frac{\nabla \mu^{\mu}}{E} + \frac{\nabla \mu^{\mu} + \nabla \mu^{\nu}}{F} \right)$$
(5-98)

where E is Young's modulus and G is the shear modulus. If stress distributions satisfying the equations of linear elasticity are assumed then they may be related to the total stress resultants for the whole section M,T, and V where

$$M(\mu) = \iint -r_s \tau_{\mu\mu} dY dx. \qquad (5-99)$$

 $V(\mu) = \iint \sigma_{\mu\kappa} \, d\nu d\kappa \tag{5-100}$ 

$$T(\mu) = \int (\kappa \, \tau_{\mu\nu} - \nu \, \tau_{\mu\kappa}) \, d\nu d\kappa \tag{5-101}$$



#### FIGURE 79 STRESS RESULTANTS ON A SECTION

As an introduction, we assume the distribution of stresses from elementary beam theory.

By substitution into Equations 5-99, 5-100, and 5-101, we find that a,b, and c can be related to M.V, and T

$$M = \iint_{x^{2}} x^{2} x^$$

$$T = \iint x^{1} + y^{2} x^{2} x^{2} x = - \iint x^{2} x^{2} x + 0 \qquad (5-107)$$

If the origin of the  $\kappa, \nu$  coordinates is placed at the "shear center," then

$$\int z + z + z = z \qquad (5-105)$$

and we have

$$L = \frac{V}{\int e^{i} dx dx} = \frac{H}{I}$$
 (5-109)

$$k = \frac{1}{\sqrt{372k}} = \frac{1}{A}$$
 (5-110)

$$= \frac{1}{\sqrt{\kappa^2 t Y^2}} \frac{1}{dY dx} = \frac{1}{J}$$
 (5-111)

where I is the second moment about the neutral surface, A is the section area, and J is the polar moment about the shear center. We then have

$$r_{ure} = - k \frac{M}{2}$$
 (5-112)

$$T_{\mu \kappa} = \frac{V}{4} - \gamma \frac{T}{T}$$
 (5-113)

$$r^{r} = \kappa \mp \qquad (5-1)4)$$

and substitution into the strain energy gives

$$\lambda = \frac{1}{2} \left( \frac{\kappa^2 M^2}{E_{\perp}^2} + \frac{V^2}{G^2} - \lambda \mathcal{P} \frac{VF}{GJ^2} + \mathcal{P} \frac{T^2}{GJ^2} - \kappa^2 \frac{T^2}{GJ^2} \right)$$
(5-115)

The total strain energy in a length  $d\mu$ , of the beam is then

$$dU = \iint \operatorname{Ind} \operatorname{rd} \operatorname{rd} \operatorname{rd} \operatorname{rd} \mu$$
$$= \frac{1}{2} \left( \frac{\mu^2}{E\Gamma} + \frac{V^2}{GA} + \frac{\Gamma^2}{GJ} \right) d\mu \qquad (5-116)$$

and for the whole beam

$$u = \pm \int \left( \frac{M_{\mu}}{ET} + \frac{V_{\mu}}{ET} \right) - \frac{T_{\mu}}{ET} \right) d\mu$$
 (5-117)

Surprisingly enough, the above equation is valid for more exact section stress distributions although in the case of stress distributions other than Equations 5-112, 5-113, and 5-114 the section properties  $\text{EI}(\mu)$ ,  $\text{CA}(\mu)$ , and  $\text{GJ}(\mu)$  cannot be interpreted as simple functions of the geometry of the section. The computation of the section properties and shear center locations for complicated sections is the difficult part of a structural analysis using beam theory.

Conventionally, the notation, EI, GA, GI is used even though these properties no longer have the interpretation they have in the more elementary beam theory.

#### 5.1.1.3.1 The Computation of Beam Section Properties

#### 5.1.1.3.1.1 Bending Rigidity

The preceding paragraph indicated that the normal stress and shear-stress are independent, so that  $EI(\mu)$  can be determined separately from the analysis of shear stresses which gives  $GA(\mu)$ ,  $GJ(\mu)$ , and the shear center location. Also, general experience indicates that for thin beams, the normal stresses are always approximated by Equation 5-112:

$$\sum_{k=1}^{\infty} = -\kappa \frac{k}{\sum_{k=1}^{\infty} \kappa^{2} \pm \gamma^{2} \kappa}$$
(5-11c)

Then the bending strain energy is

$$dU = \iint M dritx.du$$

$$= \int \iint \frac{T_{min}}{E} dr dr du$$

$$= \int \frac{1}{2} \frac{\int \frac{1}{E} \kappa^2 dr d\kappa}{\int \frac{1}{K^2} dr d\kappa} d\mu \qquad (5-213)$$

So that in the case the section is made of different materials (that is, E varies with K and  $\nu$  ), we have

$$\frac{1}{EI(n)} = \frac{\iint \frac{1}{E} \kappa^2 d\vec{r} d\kappa}{(\iint \kappa^2 d\vec{r} d\kappa)^2}$$
(5-120)

so that

$$EI(\mu) = \hat{E} \iint x^{t} dy dx = \hat{E} I \qquad (5-121)$$

where the effective modulus is the following weighted average

$$\hat{E} = \frac{\iint k^2 dr dk}{\iint \frac{1}{E} k^2 dr dn}$$
(5-122)

In the case the section were made of only two materials, we would have

$$\hat{E} = \frac{L_1 + L_2}{E^{1} + E_2 L_2}$$
(5-123)

with

$$I = I_{r} + I_{L}$$
 (5-124)

# 5.1.1.3.1.2 Torsion Rigility for Thin Wall Sections

A common beam section for light weight high strength structures is one composet of a number of thin-wall cells as in Figure 20.



FIGURE 80 MULTI-CELL THIN WALL SECTION

The total torgue on the section is (from Equation 5-101)

Using the thin well assumption we may write this integral as a sum of integrals over the individual cells. For N cells

$$\iint () dv d\kappa = \sum_{i=1}^{N} \oint () \tau ds \qquad (5-126)$$

In this expression s is a curvilinear coordinate around the arc of the cell wall, and  $\tau(s)$  is the thickness of the cell wall at a point, s. At webs adjacent to neighboring cells,  $\tau$  is one-half of the web thickness, as in Figure 81.



FIGURE 81 A SINGLE CELL

The torque,  $\varphi_i$ , contributed by the resultant of stresses over the i<sup>th</sup> cell is given by

$$G_{\bar{t}} = \oint \left( (\nabla \mu v - v \nabla \mu t) + ds \right)$$
 (5-127)

Guided by the theory of elasticity, we make the following assumptions about the stress distribution:

- (1) The component of shear stress normal to the wall is zero
- (2) The component of shear stress tangent to the wall is inversely proportional to the section thickness

If  $\mathscr{V}(s)$  is the angle between the wall tangent and the  $\nu$ -axis, the above assumptions can be expressed as

$$T_{mn} = \tau_{SWV} \qquad (5-128)$$

$$\tau_{07} = \tau_{107} r^{2} \qquad (5-129)$$



FIGURE 82 SHEARING STRESSES

The torque on the ith cell is then

$$A_{i} = \oint T \tau \left( \kappa \cos \vartheta - v \sin \vartheta \right) ds \qquad (5-130)$$

and it is easily shown that the enclosed area,  $\boldsymbol{A}_{i},$  of the  $i^{th}$  cell is

$$A_{i} = \pm \oint \left[ \kappa_{i} \alpha_{i} \dot{\alpha} - i s_{m} \sigma_{j} \right] ds \qquad (5-131)$$

The quantity

$$\frac{Q_{L}}{2A_{C}} = \frac{\oint -c(\kappa \cos \theta - v \sin \theta) ds}{\oint (\kappa \cos \theta - v \sin \theta) ds}$$
(5-132)

is then the average value of  $\sigma au$  around the i<sup>th</sup> cell. We then assume

 $T = \frac{1}{t} \begin{cases} \frac{Q_1}{AA_1} & \text{on upper and lower walls} & (5-133) \\ \frac{1(\frac{Q_1}{AA_1} - \frac{Q_1}{AA_{14}}) & \text{on the wall adjacent to } i + 1^{\text{st}} \text{ cell} \\ \frac{1}{t} \begin{pmatrix} \frac{Q_1}{AA_1} - \frac{Q_1}{AA_{14}} \end{pmatrix} & \text{on the wall adjacent to } i - 1^{\text{st}} \text{ cell} \end{cases}$ 

We may write this as

$$\tau(s) = \{f(s)\} \{ \{ Q \}_{i}$$
 (5-134)

where

$$\left\{ \mathbf{Q} \right\}_{\tilde{\mathbf{i}}} = \begin{bmatrix} \mathbf{Q}_{\tilde{\mathbf{i}}-\mathbf{i}} \\ \mathbf{Q}_{\tilde{\mathbf{i}}} \\ \mathbf{Q}_{\tilde{\mathbf{i}}+\mathbf{i}} \end{bmatrix}$$
 (5-135)

and

(5-136)

.

$$\{f(s)\}' = \begin{cases} \{0, \frac{1}{2A_1}, 0\} & \text{s on bottom} \\ \{0, \frac{1}{4A_1}, \frac{1}{4A_{11}}\} & \text{s on right side} \\ \{0, \frac{1}{4A_1}, \frac{1}{4A_{11}}\} & \text{s on top} \\ \{0, \frac{1}{4A_1}, 0\} & \text{s on left side} \end{cases}$$

The specific internal strain energy (Equation 5-95) is

$$a = \lim_{k \to \infty} \frac{\tau_{kk} + \tau_{kk}}{\tau_{k}}$$
(5-137)

cut, from Equations 5-120 and 5-129

$$T_{4\kappa}^{2} + J_{4\nu}^{2} = T^{2}$$
 (5-136)

Substitution of Equation 5-134 gives

$$u = \frac{1}{2} + \frac{1}{2} \lambda_{1} + \frac{1}{2} \lambda_{1} + \frac{1}{2} \lambda_{2} + \frac{1}{2} \lambda_{1} + \frac{1}{2} \lambda_{2} + \frac{1}{2} \lambda_{1} + \frac{1}{2} \lambda_{2} $

.

The total strain energy up to the point  $\mu$  is

.

which gives

.

(5-141)  $U(\mu) = \int^{\mu} \sum_{i=1}^{N} \oint_{i} u \tau ds d\mu$  $= \int_{a}^{a} \sum_{j=1}^{N} \frac{1}{2} \{Q\}_{j}^{j} \oint \frac{1}{2} \{(s)\} \frac{1}{2} \{s\}_{j} \int \frac{1}{2} \frac{1}{2} \{Q\}_{j}^{j} \int \frac{1}{2} \frac{1}{2} \{s\}_{j} \int \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \int \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \int \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \int \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \int \frac{1}{2}  

•

 $U(\mu) = \int^{\mu} \iint u d r d \kappa d \mu$ 

(5-142)  $[G]_{i} = \oint \{i, s\} + f(s) +$ 

then

If we define

$$U_{i}^{\mu} = \int_{1}^{\mu} \frac{1}{2} \frac{h}{r_{i}} \{Q_{i}^{\mu} [G]_{i} \{Q_{i}^{\mu} ] d\mu$$
 (5-143)

Now

$$\begin{aligned}
f_{2} f_{1} &= [a_{1-1}] = [a_{1-1} = [a_{1-1}] = [a_{1-1} = [a_{1-1}] + [a_{1-1}] = [a_{1-1}] + [a_{1-1}] + [a_{1-1}] = [a_{1-1}] + [a_{1-1}] + [a_{1-1}] = [a_{1-1}] + $

where

$$\frac{1}{2} = \begin{bmatrix} \tilde{\alpha}_1 \\ \tilde{\alpha}_2 \\ \vdots \\ \ddots \end{bmatrix}$$
(5-145)

This defines the transformation

.

$$-\frac{1}{3}\frac{1}{3}-\left[C\right]_{i}+\frac{1}{3}\frac{1}{3}$$

(5-140)

(5-140)

Substitution of this into the strain energy gives

$$U(\mu) = \int_{0}^{\mu} \frac{1}{2} \{Q\}'[G]\{Q\} d\mu \qquad (5-147)$$

where

.

$$[G] = \sum_{i=1}^{N} [C]_{i}'[G]_{i}[C]_{i}$$
(5-148)

Now, the generalized coordinates,  $q_{\rm i},$  associated with the cell torques,  $Q_{\rm i},$  are rotations of the cells at section  $\mu$ , and

$$q_i = \frac{\partial U}{\partial Q_i} \tag{5-149}$$

$$\{q\} = \int^{\mu} [G]\{q\} d\mu$$
 (5-150)

Compatibility requires that the cells all rotate together

$$q_{1} = q_{2} = \dots = q_{N} = \Theta(\mu)$$
 (5-151)

so that

$$\int^{\mu} [G_{f} Q_{f}^{3} d\mu = \{i\} \theta_{\mu}^{n}$$
 (5-152)

 $\mathbf{or}$ 

•

$$[G] \{ Q \} = \{ I \} \frac{dG}{d\mu}$$
 (5-153)

Solving for the  $Q_i$ 's, we obtain

$$f_{u}(\tilde{r}) = [\tilde{v}] \tilde{f} \tilde{f} \frac{19}{4u}$$
(5-154)

.

Now, the total torque is

$$T = \sum_{i=1}^{N} Q_i = -\{i\} \{Q_i\}$$
 (5-155)

or

$$T = \frac{1}{2} \left[ \hat{G} \right] \hat{i} \hat{i} \hat{j} \frac{d\hat{\sigma}}{d\mu_{j}}$$
(5-156)

Substituting Equation 5-154 into Equation 5-147, we obtain

$$U = \frac{1}{2} \int \{i\} [G]^{1} \{i\} (\frac{de}{d\mu})^{2} d\mu$$
 (5-157)

but, from Equation 5-156,

 $\frac{4e}{4\mu} = \frac{1}{\{1\} [(f_1^{-1})]^2}$ (5-158)

so that

$$U = \frac{1}{2} \int \frac{\tau^2}{\{i\}\{i\}\{i\}} d\mu$$
 (5-159)

By comparison with Equation 5-117, we must conclude that the effective torsional rigidity is

$$(5-160)$$

# 5.1.1.3.1.3 Shear Rigidity for Thin-Wall Cylinders

For a cylindrical, thin-wall section as in Figure  $\delta 3$ , we can assume a stress distribution consistent with the theory of elasticity and obtain an expression for the shear rigidity.



## FIGURE 83 SHEAR STRESS ON A THIN-WALL CYLINDER

If we assume

$$T(t) = 1 \quad \text{(5-161)}$$

then we have

$$\mathcal{T}_{h} = \mathcal{T}_{surf} + \mathcal{T}_{surf}$$

$$= \mathcal{L}_{surf} + \mathcal{T}_{surf} +$$

For a circular cylinder

$$\vec{v} = \vec{z} - \frac{1}{2}$$
 (5-103)

so that

$$c_{\mu \kappa} = 1 (\alpha \frac{1}{2} \sin (\frac{\pi}{2} - \frac{\pi}{2}))$$
  
=  $\alpha \cos \frac{\pi}{4}$  (5-164)

and

.

$$V = \iint_{0} \text{ for drift}$$

$$= \oint_{0} 2 \cos^{2} \xi \tau ds \qquad (5-165)$$

$$= a \tau \int_{0}^{2r} \cos^{2} \xi d\xi$$

$$= a \tau \xi \tau$$

so that

$$\lambda = \frac{V}{U_{c}^{2}\pi}$$
(5-166)

and

$$\tau = \frac{1}{\tau_{\text{FF}}} \approx \frac{2}{5} \tag{5-167}$$

and the strain energy is

$$u = \frac{t}{2} \frac{\sigma^{2}}{\zeta_{T}}$$

$$= \frac{1}{2} \frac{(2^{2} \frac{s}{2})}{(2^{2} t)^{2} \pi^{2}} \sqrt{2}$$
(5-168)

$$U = \int_{-\infty}^{\infty} iz \, dz \, dz \, dz$$

$$= \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \frac{zz^2 \cdot z}{z \cdot z^2 \cdot z^2} \, dz \, \forall^2 \, dz$$

$$= \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \frac{zz}{z \cdot z^2} \int_{-\infty}^{\infty} \frac{zz}{z \cdot z^2} \, dz$$

$$= \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \frac{1}{z \cdot z^2} \, dz$$

$$= \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \frac{1}{z \cdot z^2} \, dz$$
(5-169)

Ey comparison with Equation 5-117, we conclude that

$$(3A[\mu]) = 37.47$$
 (5-170)

The result here is interesting in view of the fact that the area of the section is  $2G\tau_B\pi$ , and hence the effective "GA" is one-half of G times A.

# 5.1.1.3.2 Influence Coefficients for a Beam by the Complementary Energy Method

The general expression for the strain energy of a beam from results of the previous section is:

$$U = \frac{1}{2} \int (\frac{M^2}{EI} + \frac{Y^2}{GA} + \frac{T^2}{GJ}) d\mu$$
 (5-171)





If the beam is loaded at a number of points  $(x_i, y_i)$  by vertical loads,  $P_i$ , then an "element" of the beam can be considered to be a portion of the beam between two successive slices normal to the median axis such that loads act only on the boundary.



FIGURE 85 GEOMETRY OF THE BEAM AND SUBDIVISION INTO ELEMENTS

If we apply the general complementary energy method discussed in Paragraph 5.1.1.2, we have first to consider the strain energy in the i<sup>th</sup> element

$$U_{\bar{l}} = \frac{1}{2} \int_{\mu_{l-1}}^{\mu_{\bar{l}}} \left( \frac{M^{2}(\mu)}{EI(\mu)} + \frac{V^{2}(\mu)}{GA(\mu)} + \frac{T^{2}(\mu)}{GJ(\mu)} \right) d\mu$$
 (5-172)

and we introduce

.

 $V_i = V(\mu_i)$  = the shear just to the left of  $\mu = \mu_i$  $M_i = M(\mu_i)$  = the moment just to the left of  $\mu = \mu_i$  $T_i = T(\mu_i)$  = the torque just to the left of  $\mu = \mu_i$ 

If the element is not loaded except at the ends, then Figure 86 shows the loads acting on the  $i^{th}$  element.



FIGURE 86 LOADS AT THE END OF AN ELEMENT

The shear, moment, and torque on the interior of the i<sup>th</sup> element are related by equilibrium to the shear, moment, and torque at the right end.

$$r_{\rm e}^{\rm H} = i_{\rm f}$$
 (5-173)

$$(5-174) = \sqrt{2} + (\kappa_1 - \kappa_1 - \kappa_2)/2$$

$$\tau(\mu) = \tau_{t} + (S(\mu) - S_{t}) V_{t}$$
(5-175)

These relations are evident from Figure 07.



FIGURE 87 FREE BODY OF AN ELEMENT

We may write Equations 5-173, 5-174, and 5-175 as

$$V(\mu) = \{1, j, j\} \{1, L\}_{i}$$
 (5-176)

$$M(\mu) = \{ u_i - \mu, \dots, j \} \{ i \}_{j}$$
 (5-177)

where

.

.

$$\begin{aligned} -\frac{1}{2} &= \begin{bmatrix} V_i \\ M_i \\ T_i \end{bmatrix} \end{aligned}$$
 (5-179)

Substitution of these relations into Equation 5-172 gives

$$u_{\bar{i}} = \frac{1}{2} \{ L_{i}^{j} : G_{\bar{i}} \}_{\bar{i}}$$
(5-180)

where

$$\begin{bmatrix} G_{1} \end{bmatrix}_{i} = \int_{\mu_{j-1}}^{\mu_{i}} \left[ \frac{1}{GR(\mu)} \begin{bmatrix} i & \Im & \Im \\ \Im & \vdots & \Im \end{bmatrix} + \frac{i}{EE(\mu)} \begin{bmatrix} \mu_{1} \cdot \mu^{2} & \mu_{1} \cdot \mu \\ A_{1} \cdot \mu^{2} & \vdots & \Im \end{bmatrix}$$

$$\begin{pmatrix} J_{1} - I_{2} \\ \vdots \\ J_{2} - I_{2} \\ \vdots \\ J_{3} - I_{3} \\ \vdots \\ J_{3} - I_{3} \\ \vdots \\ J_{3} - I_{3} \\ \vdots \\ J_{4} - I_{4} \\ J_{4} - I_{4} \\ J_{4} \\ J_{4} - I_{4} \\ J_{4} - I_{4} \\ J_{4} \\ J_{4} \\ J_{4} - I_{4} \\ J_{4} \\$$

This can be expressed more conveniently in terms of a non-dimensional integration variable,

$$\vec{z} = \frac{\mu_{1} - \mu_{1}}{\mu_{1} - \mu_{1}}$$
,  $\Delta \mu_{1} = \mu_{1} - \mu_{1}$ , (5-182)

so that

$$[J]_{i} = \begin{bmatrix} \frac{1}{24} - \frac{5^{2} \Delta \mu_{i}}{81} - \frac{5^{2} \Delta \mu_{i}}{81} & \frac{5 \Delta \mu_{i}}{81} & \frac{5 \Delta \mu_{i}}{81} & \frac{5 \Delta \mu_{i}}{81} \end{bmatrix}$$
(5-183)

In order to relate the element stress resultants, {L}<sub>i</sub>, to the applied loads, we consider the equilibrium of a free body of the portion of the beam to the right of the section  $\mu = \mu_i$ 



# FIGURE 88 FREE BODY OF STRUCTURE ADJACENT TO ELEMENT

If  $\xi_{\rm i}$  and  $\eta_{\rm i}$  are the x and y coordinates, respectively, of the shear center at the i<sup>th</sup> section, then

These relations can be written more concisely as

where

.

and

$$[1]_{i} = \begin{bmatrix} 1 & 1 & ... \\ 1 & ... & 1 & ... \\ 1 & ... & 1 & ... & 1 & ... \\ 1 & ... & 1 & ... & ... & ... \\ 1 & ... & ... & ... & ... & ... & ... \\ 1 & ... & ... & ... & ... & ... \\ 1 & ... & ... & ... & ... & ... \\ 1 & ... & ... & ... & ... \\ 1 & ... & ... & ... & ... \\ 1 & ... & ... & ... & ... \\ 1 & ... & ... & ... & ... \\ 1 & ... & ... & ... & ... \\ 1 & ... & ... & ... & ... \\ 1 & ... & ... & ... & ... \\ 1 &$$

Substitution of Equation 5-185 into Equation 5-180 gives

and since

.

we have

or

where

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.

which is a set of influence coefficients for the beam cantilevered at  $\mu = 0$ .

# 5.1.2 <u>Procedures for Calculating Inertia Matrices for Components</u> of a Complex Structure

In this section we discuss some of the specific procedures for deriving inertia matrices for parts of a structure in terms of generalized coordinates defining the configuration of those parts. Inertia matrices for a whole system can then be formed by appropriate geometric transformations relating the generalized coordinates of the component to generalized coordinates for the whole system. The fundamental starting point, for each of the discussions below, is the form of the kinetic energy given in Equation 2-42, Paragraph 2.1.2.1.

For small motions, as discussed in Paragraph 2.3.1 (Equation 2-357), we have

and Equation 5-193 reduces to

$$T = \frac{1}{2} \int \frac{d\mathbf{k}^2}{d\mathbf{k}^2} + \frac{d\mathbf{k}^2}{d\mathbf{k}^2} = \frac{1}{2} \frac{d\mathbf{k}^2}{d\mathbf{k}^2} = \frac{1}{2} \frac{d\mathbf{k}^2}{d\mathbf{k}^2} + \frac{1}{2} \frac{d\mathbf{k}^2}{d\mathbf{k}^2} + \frac{1}{2} \frac{d\mathbf{k}^2}{d\mathbf{k}^2} = \frac{1}{2} \frac{d\mathbf{k}^2}{d\mathbf{k}^2} + \frac{1}{2$$

where

$$f_{x} = \frac{1}{2} x_{y} \frac{1}{2} dx_{z} dx_{z} = \text{mass of the x-y-z particle}$$

$$f_{x} (y,z,t) = \text{displacement of the x-y-z particle}$$

$$f_{y} (x,y,z,t) = \text{displacement of the x-y-z particle}$$

$$f_{z} (x,y,z,t) = \text{displacement of the x-y-z particle}$$

$$f_{z} (x,y,z,t) = \text{displacement of the x-y-z particle}$$

$$f_{z} (x,y,z,t) = \text{displacement of the x-y-z particle}$$

#### 5.1.2.1 Rigid Components

For a part of the structure which can be considered to be rigid, we can introduce six generalized coordinates which will define the configuration of the component. If we let these six generalized coordinates be the displacements and rotations at some point of the body, and if we label this point "O," then

(5-196)

These displacements are defined in Figure 89.



FIGURE 89 GENERALIZED COORDINATES FOR A RIGID BODY

The displacements of each of the particles can then be related to these coordinates.

$$z_{1} = z_{2} z_{1} z_{2} z_{1} z_{2} z_{1} z_{2} z_{2} z_{3} z_{4} z_{5} z_$$

$$p_{4}(x_{1}q,z_{1}t) = p_{2}(t) + (x_{1}x_{0})p_{1}(t) - (z-z_{0})p_{1}(t)$$
(5-198)

$$b_{2}(x,y,\epsilon,t) = b_{3}(t) + (y-y_{0})b_{1}(t) - (x-x_{0})b_{5}(t)$$
(5-199)

The velocities of the particles can be written as

(5-200)

$$\frac{cbx}{3c} = \{1, 0, 1, c, z-z_3, (q-q_3)\} \begin{bmatrix} s \\ s_1 \\ s_2 \\ s_3 \end{bmatrix} = \{h_x J'(p)\}$$

$$\frac{3b_{1}}{25} = \frac{1}{2} (3 + 1) (3 - 2 - 2) (3 + 1) (5 - 2)$$

For simplicity we assume that the point 0 is the origin so that  $x_0 = y_0 = z_0 = 0$ . Squaring these expressions and substituting into Equation 5-195, we have

$$T = \frac{1}{2} \left\{ \hat{p} \right\} \left\{ A \right\} \left\{ \hat{p} \right\}$$
(5-203)

•

where

(5-204)

$$[x] = \int (-x_{x} \int (h_{x} \bar{s} + \frac{1}{2} h_{y} \int (h_{y} \bar{s} + \frac{1}{2} h_{y} - \frac{1}{2} h_{y} \frac{1}{2} h_{y} \frac{1}{2} + \frac{1}{2} h_{z} \frac{1}{2} h_{y} \frac{1}{2} \int dx \, dy \, dz$$

$$= \int \begin{bmatrix} -x_{y} & -x_{y} & -x_{y} \\ -x_{y} & -x_{y} & -x_{y} \\ -x_{y} & -x_{y} & -x_{z} \end{bmatrix}$$

The terms in the above matrix can be recognized as common terms describing the inertia characteristics of a rigid body. In fact, we have

$$\begin{bmatrix} A \end{bmatrix} = \begin{bmatrix} M & 2 & 0 & M_{\overline{E}} & -M_{\overline{J}} \\ - & M & 0 & -M_{\overline{E}} & -M_{\overline{J}} \\ - & M & M_{\overline{J}} & -M_{\overline{X}} & 0 \\ - & -M_{\overline{E}} & M_{\overline{J}} & \frac{1}{x_{X}} - \frac{1}{x_{J}} - \frac{1}{x_{L}} \\ -M_{\overline{E}} & -\frac{1}{x_{X}} - \frac{1}{x_{J}} - \frac{1}{x_{L}} \\ -M_{\overline{Z}} & -\frac{1}{x_{X}} - \frac{1}{x_{J}} - \frac{1}{x_{L}} \\ -M_{\overline{J}} & M_{\overline{X}} & 0 & -\frac{1}{x_{Z}} - \frac{1}{x_{J}} & \frac{1}{x_{2e}} \end{bmatrix}$$

where

$$M = \int_{\mathcal{C}} dV = \text{total mass of the body} \qquad (5-206)$$

$$\tilde{x}M = \int_{\mathcal{C}} \ell \tilde{x} dV \qquad \tilde{x} = x-\text{coordinate of center-of-} \qquad (5-207)$$

$$\tilde{y}M = \int_{\mathcal{C}} \ell \tilde{y} dV \qquad \tilde{y} = y-\text{coordinate of center-of-} \qquad (5-208)$$

$$\tilde{z}M = \int_{\mathcal{C}} \ell \tilde{z} dV \qquad \tilde{z} = y-\text{coordinate of center-of-} \qquad (5-209)$$

$$mass$$

$$I_{xx} = \int_{\mathcal{C}} (v_{1}^{x} + \tilde{z}^{2}) \ell dV \qquad (5-210)$$

$$I_{yy} = \int_{\mathcal{C}} v^{x} + \tilde{z}^{2} \ell \tilde{z} dV \qquad (5-211)$$

$$I_{xy} = \int_{\mathcal{C}} v^{x} + \tilde{z}^{2} \ell dV \qquad (5-212)$$

$$I_{xy} = \int_{\mathcal{C}} v^{x} + \tilde{z}^{2} \ell dV \qquad (5-212)$$

$$I_{xy} = \int_{\mathcal{C}} v^{x} \ell dV \qquad (5-213)$$

$$I_{yz} = \int_{\mathcal{C}} v^{x} \ell dV \qquad (5-214)$$

$$I_{yz} = \int_{\mathcal{C}} v^{z} \ell dV \qquad (5-214)$$

$$I_{yz} = \int_{\mathcal{C}} v^{z} \ell dV \qquad (5-215)$$

## 5.1.2.2 One-Dimensional Flexible Components

Lyy

I.22

For a one-dimensional body moving in a plane, the configuration can be defined by generalized coordinates which are displacements at a number of collocation points.



FIGURE 90 GENERALIZED COORDINATES FOR A ONE-DIMENSIONAL

COMPONENT

In the general expression (Equation 5-195) for the kinetic energy, we have, for a one-dimensional body,

$$p_{x}(x,y,z,t) = p_{y}(x,y,z,t) = 0$$
 (5-216)

and  $p_z$  is not a function of y or z, so that

$$T = \left\{ \frac{\mathcal{L}}{\mathcal{L}} \right\}^{2} + \frac{\mathcal{L}}{\mathcal{L}}^{2} + \frac{\mathcal{L}}^{2} + \frac{\mathcal{L}}{\mathcal{L}}^{2} +$$
where  $m(x) = \iint \rho \, dy \, dz$  = mass per unit of length. We can relate the displacement of the particles,  $p_z$ , to the generalized coordinates,  $p_i$ , in a number of ways. A very simple way employs trapezoidal interpolation:

$$p_{z}(x_{1}t) = b_{1-1} + \frac{x - x_{1-1}}{x_{1} - x_{1-1}} (p_{1} - p_{1-1})$$

$$= \left\{ i - \frac{x - x_{1-1}}{x_{1} - x_{1-1}}, \frac{x - x_{1-1}}{x_{1} - x_{1-1}} \right\} \begin{bmatrix} p_{1-1} \\ p_{1} \end{bmatrix}$$
(5-218)

for 
$$x_{i-1} \leq x \leq x_i$$

(Note, 
$$x_0 = 0$$
,  $x_N = L$ )

We can write

.

$$\int ( ) dx = \sum_{i=1}^{N} \int_{x_{i_{i_{1}}}}^{x_{i_{1}}} ( ) dx.$$
 (5-219)

in Equation 5-217 and use Equation 5-218 to obtain

$$T = \frac{\sqrt{2}}{k_{t}} \sum_{i=1}^{N} \left\{ \dot{P}_{t-1} - \dot{P}_{t} \right\} \int_{x_{t}-x_{t}}^{x_{t}} \left[ \left( 1 - \frac{x - x_{t+1}}{x_{t} - x_{t+1}} \right)^{2} - \left( 1 - \frac{x - x_{t+1}}{x_{t} - x_{t+1}} \right) \left( \frac{x - x_{t+1}}{x_{t} - x_{t+1}} \right) \left( \frac{x - x_{t+1}}{x_{t} - x_{t+1}} \right)^{2} - \left( \frac{x - x_{t+1}}{x_{t} - x_{t+1}} \right)^{2} \left[ \dot{P}_{t} \right]$$

$$= \frac{1}{2} \sum_{i=1}^{N} \left\{ \dot{P}_{t-1} - \dot{P}_{t} \right\} \left[ A \right]_{i} \left[ \dot{P}_{t-1} - \dot{P}_{t} \right]$$

$$(5-220)$$

where

For practical computation this expression can be considerably simplified. If we change the variable of integration to

$$s = \frac{\lambda - \chi_{1-1}}{\chi_{1} - \chi_{1-1}}$$
(5-222)

and introduce the length of the interval,

$$\dot{z}_1 = x_1 - x_2$$
 (5-223)

then

$$\begin{bmatrix} A_{1} \\ -5 \end{bmatrix} = \begin{bmatrix} -5 \\ -5$$

In the special case that m(x) is constant over the interval, this can be integrated explicitly to give

_A!_≂	m.k. 3	<u>"i</u> i =	= <u></u> <u>3</u>	Mr	(5-225)
	<u>mi</u> :	* <u>'i</u> i	<u>M</u> 1	Mt ]	

where  ${\tt M}_{i}$  is the total mass of the body between  ${\tt x}_{i-1}$  and  ${\tt x}_{i}$ 

If we write Equation 5-218 as

(5-226)

コント マイ お(の)、 Kg

for 
$$x_{i-1} \leq x \leq x_i$$

where

.

then Equation 5-224 can be further simplified to

$$[A]_{i} = [J]_{i}^{\prime} \int_{0}^{1} \begin{bmatrix} i & J \\ J & J^{\prime} \end{bmatrix} m(x) l_{i} d_{j} [J]_{i}$$
(5-228)

This can be evaluated by numerical integration, or it can be noted that

$$\int_{0}^{t} m(x) ds = \frac{M_{i}}{\ell_{i}}$$
 (5-229)

$$\int_{0}^{1} g_{M(x)} dg = \frac{\bar{x}_{1} - x_{t-1}}{L_{1}} \frac{M_{1}}{L_{1}}$$
 (5-230)

$$\int_{0}^{1} \overline{s}^{2} \mathfrak{m}(x) d\overline{s} = \frac{1}{k!} \frac{\mathrm{I}_{1}}{k!}$$
(5-231)

where

 ${}^{M_{i}}$  is the total mass of the i<sup>th</sup> segment,  $\overline{x}_{i}$  is the x-coordinate of the center of mass of the i<sup>th</sup> segment, and  $I_{i}$  is the total moment of inertia of the i<sup>th</sup> segment about the left end, x = x<sub>i-1</sub>.

If we finally introduce the transformation, [T] i, such that

$$\begin{bmatrix} \aleph_{-1} \\ \flat_{\bar{i}} \end{bmatrix} = [\top]_{\bar{i}} \{ \flat_{\bar{i}} \}$$
 (5-232)

where

$$\begin{cases} p_1 = \begin{bmatrix} p_1 \\ p_2 \\ p_3 \\ \vdots \\ p_{N+1} \end{bmatrix}$$
(5-233)

then

$$T = \frac{1}{2} \sum_{i=1}^{N} \frac{1}{2} \dot{p}_{i+i} \dot{p}_{i} \frac{1}{2} \begin{bmatrix} A \end{bmatrix}_{i} \begin{bmatrix} \dot{P}_{i-1} \\ \dot{p}_{i} \end{bmatrix}$$

$$= \frac{1}{2} \frac{1}{2} \dot{p} \frac{1}{2} \sum_{i=1}^{N} [T]_{i} \begin{bmatrix} A \end{bmatrix}_{i} [T]_{i} \frac{1}{2} \dot{p} \frac{1}{2}$$
(5-234)

or

$$\tau = \frac{1}{2} \{\dot{p}\} [A] \{\dot{p}\}$$
 (5-235)

where

$$[A] = \sum_{i=1}^{N} [T]_{i}^{i} [A]_{i} [T]_{i}$$
(5-236)

# 5.1.2.3 <u>Two-Dimensional Flexible Components</u>

For a plane two-dimensional body moving normal to its own plane, the configuration can again be defined by generalized coordinates which are displacements at discrete points.



FIGURE 91 GENERALIZED COORDINATES FOR A TWO-DIMENSIONAL

#### COMPONENT

In the general expression (Equation 5-195) for the kinetic energy, we have, for a two-dimensional body

$$b_{x}(x_{i}y,z,t) = b_{y}(x_{i}y,z,t) = 0$$
(5-237)

and  $p_z$  is not a function of z, so that

$$\tau = \frac{1}{2} \int \left( \left( \frac{\partial Px}{\partial t} \right)^2 + \left( \frac{\partial Px}{\partial t} \right)^2 + \left( \frac{\partial Px}{\partial t} \right)^2 \right)_{\ell} dV$$

$$= \frac{1}{2} \iint \left( \frac{\partial Pz}{\partial t} \right)^2 \int_{\ell} dz \, dx \, dy$$

$$= \frac{1}{2} \iint \left( \frac{\partial Pz}{\partial t} \right)^2 \int_{\ell} dz \, dx \, dy$$

where  $m(x,y) = \int \rho dz = mass per unit of area.$ 

One of the simplest assumptions that can be made for relating  $\mathbf{p}_z$  to the  $\mathbf{p}_i$  in two dimensions is the "bilinear" interpolation assumption. In the region between four collocation points, we assume

$$p_{\pm}(x_1y_1,t) = x + (x_1 + axy)$$
 (5-239)

where a, b, c, and d are determined so that, at the i<sup>th</sup> collocation point,

$$p_{z}(x_{i}, y_{i}, t) = p_{i}$$
 (5-240)

In order to be systematic, we define, for the i<sup>th</sup> region,

$$\{ \boldsymbol{b} \}_{i} = \left\{ \begin{array}{l} \boldsymbol{b}_{k} (\boldsymbol{x}_{i}^{th}, \boldsymbol{y}_{i}^{th}, \boldsymbol{t}) \\ \boldsymbol{b}_{k} (\boldsymbol{x}_{i}^{th}, \boldsymbol{y}_{i}^{th}, \boldsymbol{t}) \\ \boldsymbol{b}_{k} (\boldsymbol{x}_{i}^{th}, \boldsymbol{q}_{i}^{th}, \boldsymbol{t}) \\ \boldsymbol{b}_{k} (\boldsymbol{x}_{i}^{th}, \boldsymbol{q}_{i}^{th}, \boldsymbol{t}) \end{array} \right\}$$

$$\left\{ \begin{array}{l} \boldsymbol{b}_{k} (\boldsymbol{x}_{i}^{th}, \boldsymbol{y}_{i}^{th}, \boldsymbol{t}) \\ \boldsymbol{b}_{k} (\boldsymbol{x}_{i}^{th}, \boldsymbol{q}_{i}^{th}, \boldsymbol{t}) \\ \boldsymbol{b}_{k} (\boldsymbol{x}_{i}^{th}, \boldsymbol{q}_{i}^{th}, \boldsymbol{t}) \end{array} \right\}$$

$$(5-241)$$

These displacements are related to the generalized coordinates by a transformation of zero's and one's (a "bookkeeping" matrix).

$$\{b\}_i = [T]_i \{b\}$$
 (5-242)

Then, from Equation 5-239,

Solving for a, b, c, and d, we have

Substituting this into Equation 5-239, we have

$$F_{2}(x,y,t) = (1 + x + y + y) = (1 + p)_{1}$$
 (5-245)

for (x,y) on the i<sup>th</sup> region

The kinetic energy can be written as the sum over the individual regions

$$\tau = \sum_{\lambda=1}^{N} \iint_{S_{1}} \operatorname{su}(x,y) \left(\frac{\partial p_{z}}{\partial t}\right)^{2} dx dy \qquad (5-246)$$

In the i<sup>th</sup> region

$$\frac{24x}{2} = -\frac{1}{2} \frac{1}{2} \left[ \frac{1}{2} \right]_{i} \left[ \frac{1}{2} \right]_{i} \times \frac{1}{2} \frac{1}{2} \frac{1}{2} \left[ \frac{1}{2} \right]_{i} \left[ \frac{1}{2$$

.

Substitution into Equation 5-238 gives

$$T' = \sum_{i=1}^{N} \left\{ \frac{1}{p} \int_{i}^{v} \left[ \frac{1}{p} \int_{i}^{v} \left[ \frac{1}{p} \int_{i}^{v} \frac{$$

$$[A]_{i} = \begin{bmatrix} 1 \\ 1 \end{bmatrix}_{i} \begin{bmatrix} 1$$

which can be evaluated by numerical integration.

Using Equation 5-242, we obtain

-= (1); (5-250)

where

$$[A] = \sum_{i=1}^{k} [T]_{i}^{'} [A]_{i} [T]_{i}^{'}$$
(5-251)

## 5.1.2.4 Inertia Matrices for Complex Structures

When the routine method of structural analysis, discussed in Paragraph 5.1.1.1, is used, the inertia matrices for the whole structure may be obtained as a by-product of the direct stiffness approach. If  $\{q\}_{i,j}$  are the coordinates of an element and the kinetic energy of the element is written in the form

$$= \frac{1}{3} = \frac{1}{3} $

where  $[M]_{ij}$  is the element inertia matrix, then from Equation 5-45, we have

$$\{q_{j}\}_{ij} = [T]_{ij}[T]_{j}[T]_{j} \}$$
 (5-253)

and substitution into Equation 5-252, using

T =  $\sum_{j=L}$  Ty

gives

$$T = \frac{1}{2} \frac{1}{10} \frac{1}{10$$

where

$$[A] = \sum_{i} [T_{i}^{T}[T_{i}^{T}] [T_{i}^{T}] [A_{i}]_{i} [T_{i}^{T}]_{i} [T$$

#### 5.1.3 The Methods of Modal Coupling

#### 5.1.3.1 Introduction - A Summary of the General Vibration Problem

If  $\{p\}$  is a set of generalized coordinates for a complete structural system, then the methods of Paragraph 5.1.1 and Paragraph 5.1.2 give the kinetic energy and strain energy in the following form:

The theoretical details involved in the vibration problem have been considered in Section 2.2. The equation governing the vibration modes and frequencies has been shown to be

$$[e_{1}^{*}]_{+}^{*} f = \sqrt{2}$$
 (5-258)

where  $[E] = [K]^{-1}$  for restrained systems. If the system is unrestrained, the elastic vibration modes are governed by

$$([r][E][r]]A]{\phi} = \lambda{\phi}$$
 (5-259)

where

$$[\Gamma] = [I] - [A][\varphi_R] ([\varphi_R] (A][\varphi_R]) [\varphi_R]'$$
(5-260)

and

$$[z] = [z] [z] [r] [r] [s] [z]'$$
(5-261)

If  $\lambda_i$  i = 1,2... are the solutions for  $\lambda$  from Equation 5-258 or Equation 5-259, the vibration frequencies are given by

The solutions of the problem can be used as a transformation to a new set of generalized coordinates, called modal coordinates.

$$\sum_{i=1}^{\infty} \{z_i\}_i = [z_i]_i \}_i$$
 (5-263)

In the analysis for vibration modes and frequencies of very large and complex systems, the number of degrees-of-freedom, N, required often exceeds the capacity of a computer to handle the resulting NxN inertia and influence matrices. In those cases the system can be broken down into component pieces, the modes of the pieces obtained, and then these modes coupled together in a systematic procedure. This procedure is discussed below for two important cases.

#### 5.1.3.2 Modal Coupling of Elastically Uncoupled Components

The concept of modal coupling is introduced, first, for the case where it has the greatest practical utility. This is the case when it is possible to decompose the structure into large components which are "elastically uncoupled." Two systems are said to be "elastically uncoupled" when strain energy can be stored in one system without inducing deformations in the other system. Figure 92 is a schematic description of two elastically uncoupled components of a system.



FIGURE 92 TWO ELASTICALLY UNCOUPLED COMPONENTS

The figure is intended to convey the fact that deformations in (A) produce only rigid body displacements in (B).

If we let  $\{p_A\}$  be a set of generalized coordinates describing the configuration of the first component, and  $\{p_B\}$  be a set of generalized coordinates describing the configuration of the second component, then we have

$$= \{ \{ r_A \} \{ A_A \} \} \}$$
 (5-264)

and

We further suppose that the coordinates,  $\{p_B\}$ , are not consistent with the constraints imposed on (B) by (A). On this assumption,  $\{p_B\}$  includes rigid body degrees-of-freedom and hence  $[K_B]$  is singular.

If (B) is rigid, then its displacements can be uniquely related to (A) by a transformation that depends only on the geometry of the system:

The elastic displacements of (B) when (A) is motionless can be written (see also Figure 93)

where  $[\phi_{B}]$  is a matrix of modes of (B) constrained by (A) which satisfy the equation

$$[\mathsf{E}_{\mathsf{B}}][\mathsf{A}_{\mathsf{B}}]\{\varphi\} = \lambda\{\varphi\} \tag{5-268}$$

where

$$[\varepsilon_{\mathsf{B}}] = [S_{\mathsf{B}}]([S_{\mathsf{B}}]'[\kappa_{\mathsf{B}}][S_{\mathsf{B}}])^{\mathsf{T}}[S_{\mathsf{E}}]'$$
(5-269)

and  $[S_B]$  is a constraint matrix describing the constraints imposed on (B) by (A) when  $\{\,p_A\,\}=~0$ 



FIGURE 93 DISPLACEMENTS OF (B) WITH (A) MOTIONLESS

The total displacement of (B) can then be written

$$\frac{1}{2}p_{\delta} = \begin{bmatrix} -\frac{1}{2} & \frac{1}{2} & \frac{1}{2} \end{bmatrix} + \underbrace{ \begin{bmatrix} \gamma_{\delta} \end{bmatrix} \hat{\gamma}_{\delta} } \\ \text{Displacements of Elastic displacements of} \\ \begin{array}{c} (B) \text{ due to motion} \\ (B) \text{ relative to } (A) \\ \text{of } (A) \\ \end{array}$$

The general form of the geometric transformation matrix can be constructed as follows. If  $\xi$ ,  $\eta$ ,  $\zeta$ ,  $\phi$ ,  $\theta$ , and  $\psi$  are three displacements and three rotations of a point on (A) that is also on the region in contact with (B), then these can be uniquely related to the generalized coordinates describing the configuration of (A):

The coefficients can be derived by a suitable interpolation such as has been discussed in Section 2.3. The displacements of (B) when it is rigid can be written as

$$\begin{aligned} \frac{1}{2} \xi_{\varepsilon} &= \frac{1}{2} \varphi_{\varepsilon} \frac{1}{2} \xi + \frac{1}{2} \varphi_{\varepsilon} \frac{1}{2} \xi + \frac{1}{2} \varphi_{\varepsilon} \frac{1}{2} \xi \\ &+ \frac{1}{2} \xi + \frac{1}{2} \varphi_{\varepsilon} \frac{1}{2} \Theta + \frac{1}{2} \varphi_{\psi} \frac{1}{2} \Psi \end{aligned}$$
(5-272)

where the columns are "rigid body" modes. For example,  $\{\phi_{\xi}\}$ , are the values of the generalized coordinates for  $\xi = 1$  while  $\eta = \zeta = \phi = \theta = \psi = 0$ . Substitution of Equations 5-271 into 5-272 gives

$$\{t_{\mathbf{F}}\} = \left[\{t_{\mathbf{F}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\} + \left\{t_{\mathbf{F}}\}, t_{\mathbf{T}}\} + \left\{t_{\mathbf{F}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\} + \left\{t_{\mathbf{T}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\} + \left\{t_{\mathbf{T}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\} + \left\{t_{\mathbf{T}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\}, t_{\mathbf{T}}\}$$

 $\mathbf{or}$ 

If we assume, for conceptual simplicity, that (A) is constrained, then the influence coefficients for (A) are given by

$$[E_{A}] = [M_{A}]^{2}$$
 (5-275)

Now, the total kinetic energy, when (B) is rigid, is given by

$$T = \frac{1}{2} \left[ \frac{1}{2} A_{A} \right] \left[ \frac{1}{2} A$$

and the vibration modes with (B) rigid are governed by

$$[E_A]([A_A] + [T_{GA}][A_G][T_{GA}]) \{ \psi \} = \lambda \{ \psi \}$$
(5-277)

If we denote the matrix of these solutions by  $[\phi_{\!A}]$  , we have, in summary:

$$\{p_A\} = [\varphi_A]\{q_{,A}\}$$
 (5-278)

$$\{p_{B}\} = [\varphi_{B}]\{q_{B}\} + [T_{BA}][\varphi_{A}\}\{q_{A}\}$$
(5-279)

or

$$\begin{bmatrix} \{p_A\} \\ \{p_B\} \end{bmatrix} = \begin{bmatrix} \Phi \end{bmatrix} \begin{bmatrix} \{q_A\} \\ \{q_B\} \end{bmatrix}$$
 (5-280)

where

$$\begin{bmatrix} \Phi \end{bmatrix} = \begin{bmatrix} [\phi_A], [0] \\ [T_{BA}][\phi_A], [\phi_B] \end{bmatrix}$$
(5-281)

If we introduce

$$\{q_{c}\} = \begin{bmatrix} \{q_{n}\} \\ \{q_{B}\} \end{bmatrix}$$
 (5-282)

then the kinetic energy, using Equation 5-280, is

$$\tau = \frac{1}{2} \left[ \left\{ \dot{p}_{A} \right\}, \left\{ \dot{p}_{B} \right\} \right] \left[ \left[ A_{A} \right] \left[ 0 \right] \right] \left[ \dot{p}_{A} \right] \left[ \dot{p}_{B} \right] \right]$$

$$= \frac{1}{2} \left\{ \dot{q}_{c} \right\}' \left[ \dot{\Phi} \right]' \left[ \left[ A_{A} \right] \left[ 0 \right] \right] \left[ \dot{\Phi} \right] \left\{ \dot{q}_{c} \right\}$$

$$= \frac{1}{2} \left\{ \dot{q}_{c} \right\}' \left[ \dot{\Phi} \right]' \left[ \left[ A_{A} \right] \left[ 0 \right] \right] \left[ \dot{\Phi} \right] \left\{ \dot{q}_{c} \right\}$$

$$= \frac{1}{2} \left\{ \dot{q}_{c} \right\}' \left[ \dot{\Phi} \right]' \left[ \left[ A_{A} \right] \left[ 0 \right] \right] \left[ \dot{\Phi} \right] \left\{ \dot{q}_{c} \right\}$$

or

$$\tau = \frac{t}{2} \{ \dot{q}_{c} \}' [M] \{ \dot{q}_{c} \}$$
 (5-284)

where

$$[M] = [\Phi]' \begin{bmatrix} [A_A] & [o] \\ [o] & [A_a] \end{bmatrix} \begin{bmatrix} \Phi \end{bmatrix}$$
(5-285)

which is called the "modal mass matrix." The modal stiffness matrix is found in an analogous manner.

Since [Ton Kpa]

represents a rigid displacement of (B), we must conclude that

$$[\kappa_{B}]([\tau_{BA}H|h_{A}]) = \{0\}$$
(5-286)

Further, since the coordinates,  $\{p_A\}$  , are independent, we have the much stronger relation

$$[\kappa_6][\tau_{84}] = [0] \tag{5-287}$$

The total strain energy of the system is

$$J = \frac{1}{2} \left[ \frac{1}{2} p_{A} \frac{1}{2} \frac{1}{2} p_{B} \frac{1}{2} \frac$$

 $\mathbf{or}$ 

where

$$[F] = [E] [[K_0] [D]] [E]$$

$$(5-290)$$

$$[[U] [K_0]]$$

which can be expanded using Equation 5-281

$$[F_{1}] = [[F_{0}] [K_{0}] - [F_{0}] [K_{0}] [F_{0}] [V_{0}] - [V_{0}] [K_{0}] [K_{0}] [K_{0}] ]$$

$$[[F_{0}] [F_{0}] [F_{0}] [F_{0}] [K_{0}] [F_{0}] [F_{0}] [F_{0}] ]$$

Using Equation 5-207, this reduces to

Now, from general properties of the vibration equations (see Paragraph 2.2.3.2, Equation 2-179), we have

$$[\psi_{A}] [[K_{A}] [[\psi_{A}]] = []_{\lambda_{A}} ]$$
 (5-293)

$$[\varphi_{\mathsf{B}}]'[\kappa_{\mathsf{B}}][\varphi_{\mathsf{B}}] = \lceil \frac{1}{2} \lambda_{\mathsf{B}} \rfloor$$
(5-294)

So that the total "modal stiffness matrix" for an elastically uncoupled system is a diagonal matrix

$$[\mathsf{F}] = \begin{bmatrix} \mathsf{F}_{\lambda_{A}} \mathsf{J}_{\mathbf{A}} \mathsf{J}$$

We then have

.

$$T = \frac{1}{2} \{ j_c \} \{ M \} \{ j_c \}$$
 (5-296)

$$v = \frac{1}{2} \{ q_{c} \} \{ F \} \{ q_{c} \}$$
 (5-297)

and Lagrange's equations give

.

.

$$[M]_{t} \ddot{q}_{c} + [F]_{t} \dot{q}_{c} = \{0\}$$
 (5-298)

Assuming a separated solution,

$$-j_{c} = \{\pi\} q,$$
 (5-299)

leads to

$$[G[M]_{\pi} = \lambda \{\pi\}$$
 (5-300)

where the "modal influence coefficient matrix" is given by

$$[3] = [2] = [\lambda_{A}$$
 (5-301)

The matrix of eigenvectors to this problem is commonly called "modal modeshapes." This is used to express the natural vibration modes of the coupled system in the form

$$\begin{bmatrix} -P_{A} \\ +P_{B} \end{bmatrix} = \begin{bmatrix} \Phi \end{bmatrix} \{ \pi \} \{ q \}$$
 (5-302)

or simply

$$\{b\} = [\psi]\{z\}$$
 (5-303)

where

$$\frac{1}{2} \frac{1}{2} \frac{1}$$

and

$$[\phi] = \text{matrix of natural modes} = [\phi][\pi]$$
 (5-305)

The generality and practical utility of this procedure cannot be overemphasized. This concept can be used to obtain vibration modes on very complicated structures by iterating moderately small eigenvalue problems, reducing the number of degrees-of-freedom, and then solving the complete problem in terms of modal coordinates for the individual components.

The procedure of modal coupling is used in the example analysis of the Saturn vehicle in Appendix II.

#### 5.1.3.3 Modal Coupling of Elastically Coupled Systems

The procedure in this case is very similar to that of the previous section with the exception that there are additional constraint relations between the coordinates of (A) and the coordinates of (B). Figure 94 is a schematic of a system that is "almost" elastically uncoupled.



FIGURE 94 TWO ELASTICALLY COUPLED COMPONENTS

If  $\{p_A\}$  and  $\{p_B\}$  are coordinates for the components and  $\{p_B\}$  is not consistent with any of the constraints imposed on (B) by (A), we have the following equations which describe the structural continuity between (A) and (B) at points of connection which cause the components to be elastically coupled:

$$[-_{A} H_{PA}] = [-_{B} H_{PB}]$$
 (5-306)

As in the preceding section, we have

$$\{p_{A}\} = [f_{A} + 2p_{A}]$$

$$\{r_{B}\} = [T_{B} + p_{A}\} + [c_{B} + q_{B}]$$

$$(5-307)$$

where both  $[\phi_A]$  and  $[\phi_B]$  are derived with all connections between (A) and (B) relaxed except those just sufficient to restrain (B). It follows then that  $\{q_A\}$  and  $\{q_B\}$  are also not consistent with the constraints.

If we substitute Equation 5-307 into Equation 5-306, we obtain

where  $[\Phi]$  is the matrix introduced in Equation 5-230. We can also write this as

where

$$[-] = [-] + [-]$$

Using these constraint equations, we may select some of the modal coordinates and eliminate them in such a manner that the remaining coordinates satisfy the constraints explicitly. Let  $q_0$  be the coordinates to be eliminated and let  $q_0$  be the remaining coordinates, then

a ne mic remained contained by shen

$$\begin{bmatrix} 1 & 1 & 1 \\ 1 & 2 & 1 \end{bmatrix} = \begin{bmatrix} 1 & 1 & 2 \\ 2 & 3 & 1 \end{bmatrix} = \begin{bmatrix} 1 & 2 & 2 \\ 2 & 3 & 1 \end{bmatrix}$$
(5-311)

where the transformations are appropriate matrices of zero's and one's. Substitution into Equation 5-309 gives

Solving for {a<sub>0</sub>} gives

Substitution into Equation 5-311 gives

or

$$\frac{1}{12} = \begin{bmatrix} -1 & -1 \\ -1 & -1 \end{bmatrix}$$

$$(5-315)$$

The matrix, [T], is a compatibility matrix in the same sense as the compatibility matrices introduced in the Direct Stiffness Method of Structural Analysis (see Paragraph 5.1.1.1, Equation 5-17). As in the previous section, we have

$$\tau = \frac{1}{2} \left[ \left\{ \dot{q}_{A} \right\}^{2} \left\{ \dot{q}_{B} \right\}^{2} \right] \left[ \left[ \dot{\Phi} \right]^{2} \left[ \left[ A_{A} \right] \left[ 0 \right] \right] \left[ \dot{\Phi} \right]^{2} \left\{ \dot{q}_{A} \right\} \right] \left[ \left[ 0 \right] \left[ A_{B} \right] \right] \left[ \left\{ \dot{q}_{B} \right\} \right] \left[ \left\{ \dot{q}_{B} \right$$

...

$$U = \frac{1}{2} \left[ \left\{ \frac{1}{2} \Lambda \right\}' \left\{ \frac{1}{2} q_{B} \right\}' \right] \begin{bmatrix} \Gamma_{\lambda_{AJ}} \\ \Gamma_{\lambda_{AJ}} \\ \Gamma_{\lambda_{B}} \end{bmatrix} \begin{bmatrix} \frac{1}{2} q_{A} \\ \frac{1}{2} q_{B} \end{bmatrix}$$
(5-317)

Substitution of Equation 5-315 into these expressions gives

$$\tau = \frac{1}{2} \{ j_{c} \} [M] \{ j_{c} \}$$
 (5-318)

$$U = \frac{1}{2} \{q_{c}\}'[F] \{q_{c}\}$$
 (5-319)

where

$$[n] = [T]'[\frac{1}{2}] [A_{A}] [0] ][\frac{1}{2}][T]$$
(5-320)  
[0] [A\_{S}]

and

$$[=] = [-] \begin{bmatrix} -1 \\ -1 \end{bmatrix} \begin{bmatrix} 1 \\ -1 \end{bmatrix}$$
 (5-321)

The modal influence coefficients are [G] = [F]. The "modal mode-shapes" are obtained from the equations

$$[\Im][M]_{i} = \chi_{i}$$
 (5-322)

and the displacements in the modes are

$$\begin{bmatrix} i \mid h \rangle \end{bmatrix} = \begin{bmatrix} \phi \end{bmatrix} \begin{bmatrix} \tau \end{bmatrix} \begin{bmatrix} \pi \end{bmatrix} \begin{bmatrix} \eta \\ \eta \\ \eta \end{bmatrix}$$
 (5-323)

or

$$\{\phi\} = [\phi]\{q\}$$
 (5-324)

where

$$[\psi] = [\phi][\tau][\pi]$$
 (5-325)

An example which better illustrates the procedure is given in Appendix II, Paragraph .

#### 5.2 VIBRATION PROBLEMS IN LAUNCH VEHICLE STRUCTURES

#### 5.2.1 <u>A Numerical Study and Comparison of Methods to Obtain</u> Vibration Modes for Straight Beams

This section describes some studies conducted to evaluate and compare several finite degree-of-freedom methods available for the vibration analysis of beams. All of the methods are compared for the case of a straight, uniform ceam. The numerical results are given for a contilever constraint and for the free beam.

The study is restricted to what is commonly called the "Euler-Bernoulli" theory of beams governed by

$$T = \left\{ \begin{bmatrix} x_{1} \\ y_{2} \end{bmatrix} \right\}_{x}$$
(5-326)

which has been briefly discussed in Faragraph 2.3.3.1. In this section we consider only the case of the uniform team where EI(x) = EI, a constant and m(x) = m, a constant. The geometry of the system is shown in Figure 95.

1 Important deviations from this theory are considered in Faragraph 5.2.2.1.



FIGURE 95 UNIFORM STRAIGHT BEAM

In all of the finite degree-of-freedom studies, the generalized coordinates will be the lateral displacements at eleven (11) equally spaced collocation points. The generalized coordinates are then

$$p_i(t) = p_z(x_i, t)$$
 (5-328)  
i = 0,1,2...10

The points divide the beam into ten (10) equal intervals of length,

$$\dot{z} = \frac{1}{2} \qquad (5-329)$$



FIGURE 96 COLLOCATION POINTS

The methods to be considered are:

- (1) Influence coefficients from complementary energy inertia matrix by diparabolic interpolation
- (2) Influence coefficients from complementary energy inertia matrix by trapezoidal interpolation
- (3) Influence coefficients by diparabolic interpolation inertia matrix by diparabolic interpolation
- (4) Influence coefficients from the direct stiffness method inertia matrix consistent with the direct stiffness method
- (5) Influence coefficients from the direct stiffness method inertia matrix by diparabolic interpolation

(6) The exact solution of the partial differential equation for the continuous case.

These methods are considered separately in the sections that follow. The results are summarized and compared in Paragraph 5.2.2.1.2.

#### 5.2.1.1 The Exact Solution for the Continuous Beam

The exact solution may be obtained from Equations 5-326 and 5-327 by the use of Hamilton's Principle. For the case of a system that is both conservative and holonomic, this principle can be obtained from the more general Principle of Virtual Work stated in Paragraph 2.1.1.3 (see Equation 2-33). Hamilton's Principle states:

where, in our particular case,

$$\frac{1}{2} = \frac{1}{2} \frac{\frac{1}{2}}{2} = \frac{1}{2} \frac{\frac{1}{2}}{2} \frac{1}{2}$$
(5-331)

This is a problem in the Calculus of Variations which yields<sup>1</sup>

$$\int_{-\infty}^{\infty} dx \frac{dy}{dx} = \frac{1}{2\pi} + \frac{1}{$$

We want to consider only two cases in this section. First, for the beam clamped at x = L, we have

(5-334)

and Equation 5-332 then requires

The complete details of this problem are discussed by R. Weinstock, <u>Calculus</u> of Variations, McGraw-Hill, 1952, Section 10-5, p. 217. In particular, equation (c<sup>1</sup>).

$$\frac{1}{12} \frac{1}{12} \frac{1}{12} \frac{1}{12} = 0$$
 (5-336)

$$\underset{j \neq i}{\overset{j \neq j}{\underset{z}{z}}} - \exists I \frac{j \dagger j_{z}}{j \times 4} = 0$$
 (5-337)

In the second case, for the beam unconstrained, Equation 5-332 requires

$$\frac{-F_{z}}{M^{4}}(0,t) = 0$$
 (5-338)

$$\frac{3^{3}p_{2}}{3x^{3}}(o,t) = 0$$
 (5-339)

$$\frac{\partial^2 p_{\pm}}{\partial x^2} - b = 0 \qquad (5-340)$$

$$\frac{s_{12}}{s_{12}} = 2$$
 (5-341)

$$\sum_{j=1}^{m} \frac{z^{2} p_{z}}{z z^{1}} - EI \frac{z^{4} p_{z}}{z z^{4}} = 0$$
 (5-342)

The equations can be solved by separation of variables in either case. If we assume

$$c_{\pi} = \varphi(x) y(t) \qquad (5-343)$$

then we have

۰.

$$v_{0,1}(y_{0,1}) = \varepsilon I \frac{i^{4}\varphi}{ix^{4}} y_{1}(t) = 0$$
 (5-344)

which requires that

$$\frac{2}{3} \stackrel{\text{EL}}{\underset{\lambda \neq \nu}{=}} = \text{a constant, say, } \lambda$$
 (5-345)

In the case of the clamped beam, we have

.

.

while in the case of the unrestrained beam, we have

$$\frac{4}{3x^{2}} \frac{4^{3} c}{3x^{2}} + \frac{1}{3x^{2}} = 0 \qquad (5-347)$$

$$\frac{\frac{1}{3x^{2}}}{\frac{1}{3x^{2}}} \frac{1}{3x^{2}} = 0 \qquad \frac{\frac{1}{3x^{2}}}{\frac{1}{3x^{2}}} \frac{1}{3x^{2}} = 0 \qquad (5-347)$$

The solution to these equations is well known and has been tabulated in a convenient form by Dana Young and Robert P. Felgar<sup>1</sup>. The solutions,

(5 - 348)

$$i = 1, 2...$$
 (5-349)

are compared with the finite degree-of-freedom solutions in Paragraph 5.2.1.6.

# 5.2.1.2 Influence Coefficients by the Direct Stiffness Method

If we write

$$-z = z - \frac{1}{x_{1}} - z - \frac{1}{y_{2}} - \frac{1}{x_{2}} - \frac$$

then, from Equation 5-327,

Now, the bending moment in the beam is

$$V = - \int_{0}^{1} z \, \overline{z}_{1}, \, u_{2} dz = \int_{0}^{1} z \, \overline{z}_{2x} \, u_{3} dz \qquad (5-352)$$
$$= - \int_{0}^{1} z \, \overline{z}_{2x} \, u_{3} dz$$
$$= \int_{0}^{1} \overline{z} \, \overline{z}^{2} \, u_{3} dz \quad \overline{z}^{2} \, \overline{z}^{2} = \overline{z} \, \overline{z}^{2} \, \overline{z}^{2} \, \overline{z}^{2}$$

<sup>&</sup>lt;sup>1</sup>Young, D. and, Felgar, R. P., <u>Tacles of Characteristic Functions Representing</u> <u>Normal Modes of Vibration of a Beam</u>, University of Texas Publication No. 4913, July 1, 1949.

and hence,

 $\bigcup_{\hat{L}} = \frac{1}{2} \int_{X_{b1}}^{X_{b1}} \frac{M^2}{EL} dx$  (5-353)

If we let

$$\mathcal{Z} = \chi_{i} - \chi \tag{5-354}$$

then

$$U_{i} = \frac{1}{2} \int_{0}^{\ell} \frac{M^{2}(s)}{E_{i}} ds \qquad (5-355)$$

If we assume the element is only loaded at the ends, then, on the ith interval,

$$M(s) = M_1 + s V_1$$
 (5-356)

•

where  $V_i$  and  $M_i$  are the shear and bending moment just to the left of  $x = x_i$ .

FIGURE 97 TYPICAL ELEMENT

From Equation 5-356,

 $M^{1}(s) = \{ V_{i} \ M_{i} \} \begin{bmatrix} s \\ 1 \end{bmatrix} \{ s \ 1 \} \begin{bmatrix} V_{i} \\ M_{i} \end{bmatrix}$ (5-357)

Substitution into Equation 5-355 gives

$$U_{i} = \frac{1}{2} i v_{i} M_{i} \int_{0}^{1} \frac{1}{EI} \begin{bmatrix} 3^{2} & 3 \\ 5 & i \end{bmatrix} ds \begin{bmatrix} v_{1} \\ M_{i} \end{bmatrix}$$
(5-358)

.



Performing the indicated integration gives

$$U_{i} = \frac{1}{2} \frac{1}{i} \frac{1}$$

where

$$[G]_{i} = \begin{bmatrix} \underbrace{4^{3}}_{3 \in I} & \underbrace{4^{2}}_{4 \in I} \end{bmatrix}$$

$$\begin{pmatrix} \underbrace{4^{4}}_{2 \in I} & \underbrace{4^{2}}_{1} & \underbrace{4^{2}}_{1} \end{bmatrix}$$
(5-360)

and

$$\{L\}_{i} = \left\{ \begin{array}{c} U_{i} \\ U_{i} \\ U_{i} \\ \end{array} \right\}$$
 (5-361)

Now, if we denote all the loads acting on the i<sup>th</sup> element by  $\{u_i\}_i$ , we have

$$i\hat{x}_{t}^{T} = \begin{bmatrix} \hat{x}_{t}^{(t)} \end{bmatrix}$$
 (5-362)  
 $\hat{x}_{t}^{T} = \begin{bmatrix} \hat{x}_{t}^{(t)} \\ \hat{x}_{t}^{(t)} \end{bmatrix}$ 

.

where the loads are shown in Figure 98.



FIGURE 98 LOADS ON AN ELEMENT

We note here that

.

$$\psi_{\tilde{t}} = \mathcal{Q}_{3}^{(t)} \tag{5-363}$$

$$M_{\tilde{t}} = G_t^{(n)} \tag{5-364}$$

Also, from equilibrium of the element, we have

$$Q_{t}^{(l)} = -Q_{t}^{(l)} \tag{5-365}$$

$$g_2^{(i)} = -f_2 g_3^{(i)} - g_4^{(i)}$$
 (5-366)

Using Equations 5-363 and 5-364, this may be written as

$$\{ \hat{\boldsymbol{u}} \}_{\hat{\boldsymbol{i}}} = \begin{bmatrix} \boldsymbol{Q}_{1}^{(1)} \end{bmatrix} = \begin{bmatrix} -1 & 0 \\ \boldsymbol{Q}_{2}^{(1)} & -\hat{\boldsymbol{i}} & -1 \end{bmatrix} \begin{bmatrix} \boldsymbol{V}_{\hat{\boldsymbol{i}}} \end{bmatrix} = \begin{bmatrix} \boldsymbol{V} \end{bmatrix}_{\hat{\boldsymbol{i}}} \{ \boldsymbol{L} \}_{\hat{\boldsymbol{i}}}$$

$$= \begin{bmatrix} \boldsymbol{Q}_{1}^{(2)} \end{bmatrix} \begin{bmatrix} \boldsymbol{V}_{1} \end{bmatrix} \begin{bmatrix} \boldsymbol{V}_{1} \end{bmatrix} = \begin{bmatrix} \boldsymbol{V} \end{bmatrix}_{\hat{\boldsymbol{i}}} \{ \boldsymbol{L} \}_{\hat{\boldsymbol{i}}}$$

$$= \begin{bmatrix} \boldsymbol{Q}_{1}^{(2)} \end{bmatrix} \begin{bmatrix} \boldsymbol{V}_{1} \end{bmatrix} = \begin{bmatrix} \boldsymbol{V} \end{bmatrix}_{\hat{\boldsymbol{i}}} \{ \boldsymbol{L} \}_{\hat{\boldsymbol{i}}}$$

$$= \begin{bmatrix} \boldsymbol{Q}_{1}^{(2)} \end{bmatrix} \begin{bmatrix} \boldsymbol{V}_{1} \end{bmatrix} = \begin{bmatrix} \boldsymbol{V} \end{bmatrix}_{\hat{\boldsymbol{i}}} \{ \boldsymbol{L} \}_{\hat{\boldsymbol{i}}}$$

$$= \begin{bmatrix} \boldsymbol{Q}_{1} \end{bmatrix} \begin{bmatrix} \boldsymbol{V}_{1} \end{bmatrix} = \begin{bmatrix} \boldsymbol{V} \end{bmatrix}_{\hat{\boldsymbol{i}}} \{ \boldsymbol{L} \end{bmatrix}$$

.

If  $\mathtt{v}_i$  and  $\mathtt{m}_i$  are the generalized coordinates associated with  $\mathtt{V}_i$  and  $\mathtt{M}_i$  and we let

$$\chi_i j_i = [\chi_i]$$

$$[\pi_i]$$

$$(5-369)$$

then the virtual work of the stress resultants is

$$SW = \frac{1}{1}Sif_{1}(1)i_{1}$$
 (5-370)

but also

$$\varepsilon_{W} = \{\delta_{Q}\}_{i}^{\prime} \{\hat{u}\}_{i}^{\prime}$$
 (5-371)  
442

.

and, from Equation 5-367,

$$S_{n} = \{S_{ij}\}_{i}^{\prime} [V]_{i} \{L\}_{i}^{2}$$
 (5-372)

By comparison with Equation 5-370, we conclude

$$-\varepsilon_{k} \sharp_{i}^{\prime} = -\varepsilon_{k} \Im_{i} [\Lambda]_{i}^{\prime}$$
(5-373)

or

$$\{z\}_{i} = [v]_{i} \{g_{j}\}_{i}$$
(5-374)

Expanding this, using Equation 5-368, gives

$$m_i = g_4^{(i)} - g_2^{(i)}$$
 (5-376)

which indicates that the generalized coordinates associated with the stress resultants are relative displacements between the ends of the element.

From Equation 5-359 and Castigliano's theorem, we have

so that

$$i L_{i}^{2} = [3]_{i}^{2} i i_{i}^{2}$$
 (5-370)

Using Equation 5-374, we obtain

$$i = [j_1] = [j_1] (j_1 + j_2)$$
 (5-379)

and substitution into Equation 5-359 gives

$$z_{1} = \frac{1}{2} \frac{1}$$

or

$$U_{\bar{i}} = \frac{1}{2} \{q_{\bar{j}}\}_{i} [F_{\bar{i}}]_{\bar{i}} \{q_{\bar{j}}\}_{i}$$
(5-381)

where

$$[F]_{\tilde{i}} = [V]_{\tilde{i}} [G]_{\tilde{i}} [V]_{1}^{\prime}$$
(5-382)

Expanding this using Equations 5-360 and 5-368 gives

$$[F]_{i} = \frac{EI}{\ell^{3}} \begin{bmatrix} 12 & cL & -12 & cL \\ cL & 4\ell^{2} & -6L & \lambda\ell^{2} \end{bmatrix}$$
(5-383)  
$$\begin{bmatrix} -12 & -6L & 12 & -6L \\ cL & 2\ell^{2} & -6\ell & 4\ell^{2} \end{bmatrix}$$

The compatibility relations for the beam are that slope and displacement are continuous across the boundary between elements. If  $q_i$  (i = 1,2...11) are the common displacements at the points x = x<sub>i</sub>, then

$$\begin{aligned}
y_{1}^{(i)} &= y_{1}, & (5-384) \\
y_{1}^{(i)} &= y_{1}^{(i)} &= y_{2}, \\
y_{2}^{(2)} &= y_{1}^{(3)} &= q_{3} \\
\vdots \\
y_{2}^{(i2)} &= y_{1}^{(i2)} &= y_{2}, \\
y_{2}^{(i2)} &= y_{10}, & \vdots
\end{aligned}$$

If  $q_i$  i = 12, 13...22 are the common slopes at  $x = x_i$ , then

(5-388)

.

For example, for i = 1:

 $\{\mathbf{g}_{i}\} = \begin{bmatrix} \mathbf{g}_{01} \\ \mathbf{g}_{1} \\ \vdots \\ \mathbf{g}_{11} \\ \mathbf{g}_{12} \\ \mathbf{g}_{13} \\ \mathbf{g}_{13} \end{bmatrix}$ [ يتر 4 ]

(5-387)

.

These relations can be written as

.

.

where

(5-386)  $-\frac{1}{2} = [\tau]_1 \{q\}$ 

i = 1,2...10

$$\begin{array}{rcl}
q_{12}^{(i)} &=& q_{12} \\
q_{12}^{(i)} &=& q_{12} \\
q_{14}^{(i)} &=& q_{12}^{(i)} &=& q_{13} \\
q_{14}^{(i)} &=& q_{12}^{(i)} &=& q_{14} \\
&\vdots \\
q_{14}^{(i0)} &=& q_{122} \\
q_{14}^{(i0)} &=& q_{122}
\end{array}$$

(5-385)

Substitution of Equation 5-386 into Equation 5-381 gives

$$\cup = \sum_{i=1}^{N} \cup_{i} = \frac{1}{2} \{ q_{j} \}' \sum_{i=1}^{D} [T]'_{i} [F]_{i} [T]_{i} \{ q_{j} \}$$
 (5-389)

or

$$U = \frac{1}{2} \{ q_{3} \} [F] \{ q_{3} \}$$
(5-390)

where

$$[F] = \sum_{i=1}^{N} [T]_{i}^{i} [F]_{i} [T]_{i}^{i}$$
(5-391)

If we choose as external coordinates the eleven displacements at the points  $x = x_i$ , then  $p_i \equiv q_i$  for i = 1,2...ll and from the general theory in Paragraph 5.1.1.1, we have

$$U = \frac{1}{2} \frac{1}{5}$$
 (5-392)

where

$$[\kappa] = [F_{11}] - [F_{12}][F_{22}][F_{21}]$$
(5-393)

This could be used to calculate an influence matrix for the cantilevered condition. However, a scheme that is one degree-of-freedom more accurate is to set  $q_{11}$  and  $q_{22}$  equal to zero in Equation 5-390 by the transformation:

(5-394)

.

$$\begin{array}{c} \overline{z} \left[ \begin{array}{c} \overline{z} \\ \overline{$$

and calculate

$$\begin{bmatrix} 3 \\ 3 \end{bmatrix} = \begin{bmatrix} 8 \\ 1 \end{bmatrix} \begin{bmatrix} 8 \\ 1 \end{bmatrix} \begin{bmatrix} 8 \\ 3 \end{bmatrix} \begin{bmatrix} 1 \\ 8 \end{bmatrix} \begin{bmatrix} 1 \\ 3 \end{bmatrix} \begin{bmatrix}$$

Then it can be shown that

$$U = \frac{1}{2} \int P_{1} \left[ e_{1} \right] \hat{r}$$
 (5-396)

where

## 5.2.1.3 Inertia Matrix Corresponding to the Direct Stiffness Approach

The kinetic energy, Equation 5-326, can be written as

$$T = \sum_{i=1}^{n} \sum_{j=1}^{n} \frac{\sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{j=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{j=1}^{n$$

We want to show that, according to assumptions already made, the deflection on the  $i^{th}$  interval is a cubic curve. This follows from Equation 5-352

$$\varepsilon_{I} \frac{5 \epsilon_{I}}{3 \kappa_{I}} = \kappa$$
 (5-399)

and Equation 5-356

$$M = M_1^2 + (r_1 - x) V_1^2$$
 (5-400)

We then have the following differential equation to integrate:

$$z_{1}^{2} \frac{z_{0}}{y_{1}^{2}} = x_{1} + (x_{1} - x_{1})_{1}$$
 (5-401)

If we introduce the non-dimensional variable,

$$s = \frac{s_{-x}}{2}$$
 (5-402)

then

.

$$\varepsilon r \frac{3}{1}, \frac{3}{2} \varepsilon = \frac{1}{1}, \frac{5}{2} \varepsilon \sqrt{1}$$
 (5-403)

whose solution is obtained by repeated integration:

$$p_{z} = p_{z|_{S=0}} + \frac{2p_{z}}{\delta S} + \frac{\ell^{1}M_{i}S^{2}}{\ell EI} + \frac{\ell^{3}V_{i}}{\ell EI}S^{3}$$
(5:404)

It is more convenient to express this in terms of:

$$b_{z|y=1} = q_{1}^{(1)}$$
 (5-405)

$$-\frac{1}{2} \frac{\partial p_{z}}{\partial s}\Big|_{s=1} = y_{z}^{(t)}$$
(5-406)

$$b_{z|_{J=0}} = q_{J}^{(i)}$$
 (5-407)

$$-\frac{1}{\ell}\frac{\partial\dot{p}_{z}}{\partial\bar{z}}\Big|_{\xi=0}=o_{t}^{(i)}$$
(5-408)

which gives, from Equation 5-404,

.

$$f_{\Xi} = z^{(1)} - z^{2} z^{(3)} - z^{2} z^{(3)} + 2z z^{(1)} - 3 z^{(1)} + z z^{(1)} - 3 z^{(1)} + z z^{(1)} - 3 z^{(1)} + z z^{(1)} - z z^{(1)} + z$$

Now, the kinetic energy can be written as

$$= \sum_{i=1}^{N} \int_{0}^{1} w_{i} \left( \frac{\partial z}{\partial z} \right)^{2} z dz$$
 (5-410)

and, on the ith interval,

$$\frac{1}{3t} = -1, \frac{1}{3t}, $

Substitution into Equation 5-410 gives

$$T = \sum_{i=1}^{n} -i F_i [M]_i - i F_i [M]_i$$

where

.

.

Now, from Equation 5-306,

(5-414)

and

and hence

$$[A] = \sum_{i=1}^{10} [T]'[T]'_{i}[M]_{i}[T]_{i}[T]$$
(5-417)

where

# 5.2.1.4 Influence Coefficients by the Complementary Energy Method

From Equation 5-359, we have

.

$$u_i = \frac{1}{2} \int L_i [G]_i \{L\}_i$$
 (5-418)

where

.

.

$$\begin{bmatrix} G \end{bmatrix}_{i}^{i} = \begin{bmatrix} \frac{\ell^{3}}{3\ell I} & \frac{\ell^{1}}{2\ell I} \\ \frac{\ell^{1}}{2\ell I} & \frac{\ell}{\ell I} \end{bmatrix}$$
(5-419)

Now, consider the equilibrium of the free-body in Figure 99.



# FIGURE 99 LOADS ON THE PORTION OF THE BEAM TO THE LEFT OF x = x;

From equilibrium, we have

$$y_{i} = -2 - P_{i} + \dots - P_{i-1} = \frac{1}{2} - y_{i-1} - y_{i-1} = \frac{1}{2} - y_{i-1} - y_{i-1} = \frac{1}{2} - $

.

$$M_{L} = (x_{L} - x_{0})P_{0} + (x_{L} - x_{1})P_{1} + \dots + (x_{L} - x_{L})P_{L},$$

$$= [x_{L} - x_{0} - x_{L} - x_{1} - x_{1} - x_{1} - x_{1} - x_{1} - x_{1}] \{P\}$$

$$= [x_{1} - x_{1} -$$

We may write these equations as:

$$z_{1}z_{2} = [c]_{1}\{P\}$$
 (5-422)

where

$$\begin{bmatrix} c \end{bmatrix}_{i} = \begin{bmatrix} c_{i} & -c_{i} & c_{i} & 0 & 0 \\ c_{i} $

i<sup>th</sup> column

Substitution of Equation 5-422 into Equation 5-418 gives

where

٠

$$[s] = \sum_{i=1}^{2} [i]_{i} [i]_{i} [c]_{i}$$
 (5-425)

#### 5.2.1.5 Inertia Matrix by Trapezoidal Interpolation

This method has already been considered in Faragraph 5.1.2.2, where it has been shown that

$$[\lambda] = \frac{2}{17} [-1, [\lambda]_1[\tau]]$$
(5-426)
where, for the present case (see Equation 5-225),

$$\begin{bmatrix} A \end{bmatrix}_{i} = m\ell \begin{bmatrix} \frac{i}{3} & \frac{i}{6} \\ \frac{i}{4} & \frac{i}{3} \end{bmatrix}$$
(5-427)

and

$$[T]_{i} = \begin{bmatrix} 0, 0 \dots & 1 & 0, 0 \dots & 0 \\ 0, 0 & 0 & 1, 0 & 0 \end{bmatrix}$$
(5-428)

#### 5.2.1.6 Comparison and Conclusions

Table 10 summarizes the numerical results of this section.

The comparison of the complementary energy method with the direct stiffness method bears out the fact that they are equivalent methods that use the same mathematical model and the same approximations.

The diparabolic methol gives very good results when it is considered that the system is allowed only half as many degrees-of-freedom as for the other cases. When the complementary energy or direct stiffness influence coefficients are used, the results can be improved considerably by using a diparabolic inertia matrix instead of a trapezoidal inertia matrix.

Finally, the direct stiffness method has the distinct advantage that an inertia matrix can be derived consistent with the internal load paths of the structure and the use of this matrix gives modes and frequencies very nearly equal to the exact solution for the continuous structure.

#### 5.2.2 Vibration of Thin-Wall Cyliniers

This section deals with thin-wall, large diameter cylindrical structures that are typical of liquid propellant tanks on current clustered launch vehicles. Fairly slender thin-wall cylinders also find applications as heat shields and payload fairings on both liquid and solid propellant launch vehicles.

Faragraph 5.2.2.1, below, contains some important deviations from the Euler-Bernoulli theory of beams, and comparisons between methods for including the various effects of shear energy and rotary inertia are shown.

Faragraph 5.2.2.2 contains analyses concerned with shorter cylinders where the structure must be considered as a thin shell. The effects of internal pressure and axial loads are considered in the shell study.

Finally, Paragraph 5.2.2.3 deals again with a beam model; however, in this case, the results are derived from shell theory, and thus the important effects of internal pressure are retained.

	7							
	فالمراجب والمراجب والمراجب والمراجب والمراجب والمراجب والمراجب	(†)	14,619	14,980	18.593	17,060	14,655	17,060
	EVERED	(3)	3,806.9	3,833.1	4,454.6	4,096.3	3,809.2	4,096.3
62 i EI	CANTILL	(2)	485.32	485.97	524.84	71.964	485.57	496.12
DENCY,	-	(T)	12.355	12.363	12.520	12.344	12.362	446.21
NSTONAL FREQ		(1)	39,960	42,424	52,958	51,756	40,389	ς42 <b>,</b> Σζ
HEIFTI CINON	FIREL	(3)	14,616.8	14,980.0	17,819.1	U.090.11	14,077.4	17,060.3
	FRAN	(2)	3,803.19	3,830.30	4,246.17	4,094.04	3,807.449	4,094.01
		(1)	500.42	501.05	517.65	510.64	500.63	510 <b>.</b> 64
WETHOD*			ч	01	m	łł	2	<u>د</u> ،

.

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\* 1 Exact Solution

453

- 2 Mass Matrix Diparabolic Influence Matrix - Complementary Energy
- 3 Mass Matrix Diparabolic Influence Matrix - Diparabolic
- 4 Mass Matrix Trapezoidal Influence Matrix - Complementary Energy
- 5 Mass Matrix Direct Stiffness Influence Matrix - Direct Stiffness
- 6 Mass Matrix Trapezoidal Influence Matrix - Direct Stiffness

A COMPARISON OF SEVERAL METHODS TO OBTAIN THE FREQUENCIES OF A STRAIGHT BEAM

# 5.2.2.1 <u>A Study of Vibration Modes of Straight Beams with Thin-Wall</u> Cylindrical Cross Sections

In this section, a number of analyses are compared to evaluate the various effects of shear strain energy and rotary inertia. All of the numerical comparisons are given for a cantilevered thin-wall cylinder idealized as a beam. The geometry is shown in Figure 100.



FIGURE 100 GEOMETRY OF THE STRUCTURE

The equivalent beam section properties are taken to be:

$$EI = E\pi \delta^3 \tau \tag{5-429}$$

$$G_{A} = \frac{E}{z_{(1+7)}} \pi \xi_{\tau} \qquad (see Paragraph 5.1.1.3.1.3, (5-430)$$
  
Equation 5-170)

$$u = 2\pi \delta \tau \ell \tag{5-431}$$

I = moment of inertia/unit length =  $\pi \epsilon^3 \tau \rho$  (5-432)

The numerical analysis is given for a beam that is an idealization of the outer liquid oxygen tanks on the fifth-scale model of the Saturn launch vehicle (see Appendix II).

The values of the beam parameters in this case are:

L = 105.91 inches

Ð

b = 7.0135 inches

 $\tau$  = 0.020 inches

$$\rho = 0.1 \text{ lbm/in}^3$$

$$E = 10.6 \times 10^6 \text{ lbF/in}^2 \qquad (Aluminum)$$

$$\nu = 0.3$$

## 5.2.2.1.1 Equations for a Beam Including Shear Energy and Notary Inertia

The most general beam representation of this particular type of structure has a kinetic energy and strain energy given by the following expressions:

$$\tau = \frac{1}{2} \int_{0}^{1} m \left( \frac{1}{2} \right)^{2} + \frac{1}{2} \left( \frac{1}{2} \right)^{2} dx \qquad (5-433)$$

$$z = z \frac{i^2}{z_1^2} + \frac{i^2}{z_1^2} + \frac{i}{z_1^2} + \frac{i}$$

where  $\theta(x,t)$  is the rotation of a section and  $\zeta(x,t)$  is the deflection of the axis.

It is assumed that

$$\frac{2}{3} \frac{1}{2} = \frac{1}{32} (x_{\pm}, (5-435))$$

that is, sections do not rotate so that they remain perpendicular to the elastic deflection curve. The above expressions define what is commonly called Timoshenko's theory of beams. The partial differential equations for the continuous beam can be obtained, as in Faragraph 5.2.1.1, by using Hamilton's Principle.

$$z^{(2)}_{1}$$
  $\tau_{-1}$   $t = 2$  (5-430)

It can be shown that the stress resultants are related to the team deformation functions by

 $= \frac{3}{37} - 6$  (5-437)

$$M = -E \Xi \frac{3 \Theta}{3 \kappa}$$
 (5-43c)

so that

$$T - U = \int_{a}^{b} \pi(\frac{2a}{2b})^{2} + I(\frac{2a}{2b})^{2} - EI(\frac{2a}{2b})^{2} - A(\frac{2a}{2b}) + I(\frac{2a}{2b})^{2} - EI(\frac{2a}{2b})^{2} - EI(\frac{2a}{2b})^{2$$

and the variational problem gives:

$$\overline{cL}\frac{3^{2}\theta}{3\chi_{2}} + \overline{cA}\frac{4\xi}{3\chi}(-\xi) - L\frac{3\xi}{3\chi_{2}} = 0$$
 (5-h40)

$$= \frac{1}{2\lambda^2} \qquad \mathbf{r}^2 = \frac{\lambda^2}{2\lambda^2} - \frac{2\omega}{2\lambda} = 0 \qquad (5-441)$$

which are the "Timoshenko-Beam" equations<sup>1</sup>. The "Euler-Bernoulli" equations are obtained by using the first equation to eliminate  $\frac{2t}{2x} - c$  in the second equation and then setting

The numerical results of this section are concerned with the following special cases of Equations 5-433 and 5-434:

- Complete Timoshenko Beam analysis for 50 degrees-of-freedom
   Shear energy and rotary inertia included but rotary inertia
- described in terms of lateral displacements by use of
  - $e = \frac{25}{2x}$

for 25 degrees-of-freedom

- (3) Shear energy included but no rotary inertia for 25 degreesof-freedom, I = 0
- (4) No shear energy and no rotary inertia (Euler-Bernoulli theory) for 25 degrees-of-freedom

$$\dot{\varepsilon} = \frac{iS}{iX}$$
 in shear energy term and  $I = 0$ 

In the numerical studies the beam was divided into 24 equally spaced intervals by 25 collocation points, and the generalized coordinates were taken to be the displacement and rotation at each of the collocation points.

$$\begin{array}{c}
\sum_{i=1}^{2} \sum_{j=1}^{2} \left( \sum_{i=1}^{2} \sum_{j=1}^{2} \sum_{i=1}^{2} \left( \sum_{i=1}^{2} \sum_{j=1}^{2} \sum_{j=1}^{2} \sum_{i=1}^{2} \sum_{j=1}^{2} \sum_{j=1}^{2} \sum_{i=1}^{2} \sum_{j=1}^{2} \sum_{j=1}^{2} \sum_{i=1}^{2} \sum_{j$$

For a more complete discussion of these equations for the continuous case, reference should be made to Huang, T. C., The Effect of Rotary Inertia and of Shear Deformation on the Frequency and Normal Mode Equations of a Uniform Beam with Simple End Conditions, Journal Adventical Mode Equations, 1920, 1991.

The generalized forces associated with these generalized coordinates are

$$i D_{j}^{1} = [Z]$$
 where  $Z_{j}$  is the load at the i<sup>th</sup>  
 $z_{i}$  collocation point  
 $\vdots$   
 $z_{j}$   
 $\Rightarrow$  and  $\Theta_{j}$  is the moment at the i<sup>th</sup>  
collocation point

### 5.2.2.1.1.1 Timoshenko Beam

The complementary energy method (see Paragraph 5.1.1.3.2 Equation 5-103), including shear energy, gives the following for the influence coefficients corresponding to these degrees-of-freedom:

$$C_{i} = \sum_{i} C_{i} \sum_{i=1}^{i} \frac{1}{2\pi i} \sum_{j=1}^{i} \frac{1}{2\pi i}$$

and Equation 5-422 of Paragraph 5.2.1.4 is polified to give

$$\begin{bmatrix} \mathbf{k}_{1} \\ \mathbf{k}_{2} \end{bmatrix} = \begin{bmatrix} \mathbf{k}_{1} \\ \mathbf{k}_{2} \end{bmatrix}$$
(5-446)

where

.

The influence coefficients for the Timoshenko beam are then defined by

where

.

$$\begin{bmatrix} s_{1} \\ s_{1} \end{bmatrix} = \underbrace{\overset{\mathcal{H}}{\underset{i=1}{\overset{(c)}{\underset{i=1}{\atopi=1}{\overset{(c)}{\underset{i=1}{\overset{(c)}{\underset{i=1}{\atopi=1}{\overset{(c)}{\underset{i=1}{\overset{(c)}{\underset{i=1}{\atopi=1}{\overset{(c)}{\underset{i=1}{\atopi=1}{\overset{(c)}{\underset{i=1}{\atopi}{\atopi=1}{\underset{i=1}{\atopi=1}{\atopi=1}{\overset{(c)}{\underset{i=1}{\atopi=1}{\atopi}{\underset{i=1}{\atop$$

· .

If we partition this into  $25 \times 25$  square matrices:

$$[3] = [E_{23}] [E_{24}]$$
(5-450)  
$$[E_{24}] [E_{24}]$$

then, from Castigliano's theorem, we have

$$\{z\} = \{E_{JJ}\}\{z\} + \{E_{J0}\}\{\Theta\}$$
(5-451)

$$\{ \Rightarrow \} = [E_{es} Kz\} + [E_{eo} K \ominus ]$$
 (5-452)

which are useful for interpretation of the results of this section.

For the inertia matrix, diparabolic interpolation gives

$$\frac{2}{5t} = \frac{1}{5} \cdot \frac{1$$

$$\frac{2\Theta}{\delta t} = \frac{1}{3} \frac{3}{5} [3]_{1} [T]_{1} \{\dot{\Theta}\}$$
(5-454)

and substitution into Equation 5-433 gives

$$T = \frac{1}{2} \left\{ \frac{1}{2} \left\{ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right] \right] \left[ \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right] \right\}$$

$$S = \frac{1}{2} \left\{ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right] \right] \left[ \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right] \right]$$

$$S = \frac{1}{2} \left\{ \frac{1}{2} \left[ \frac{1}{2} \left[ \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right] \right] \left[ \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right] \\\left[ \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right] \\\left[ \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right] \\\left[ \frac{1}{2} + \frac{1}{2}$$

$$\mathbf{or}$$

(5-450)

(5-457)

where

with

The modes and frequencies for the Timoshenko beam are obtained from

$$E[A]_{A} = (5-459)$$

# 5.2.2.1.1.2 Shear Energy Uncoupled from Rotary Inertia

In the second case Equation 5-442 was used to give

$$\overline{z} = \frac{1}{2\pi} \cdot z^2 \qquad (5-4\varepsilon 0)$$

This, in conjunction with the diparabolic formula, gives

$$\frac{z_1}{z_1} = \frac{1}{2} \frac{z_2}{z_1} \frac{z_2}{z_2} \frac{z_1}{z_1} \frac{z_1}{z_1} \frac{z_1}{z_1} \frac{z_1}{z_1}$$

$$a_t \xi = 0$$

which yields

.

$$= \underbrace{1}_{k} \underbrace{1}_{-} \underbrace{5}_{k} \underbrace{1}_{k} \underbrace{1}_{k} \underbrace{1}_{k} \begin{bmatrix} \vdots \\ \vdots \\ \vdots \end{bmatrix}$$
 (5-401)

$$\Theta_{\overline{1}} = \frac{1}{2} \left[ 1 - \frac{1}{2} \right]_{\overline{2}} \left[ 3 - \frac{1}{2} \right]_{\overline{2}} \left[ \frac{1}{2} \right]_{\overline{1}+1} \left[ \frac{1}{2} \right]_{\overline{1}+$$

$$\Theta_{25} = \frac{1}{2} \begin{bmatrix} 1 & -\frac{3}{2} & \frac{5}{4} & \frac{5}{4} \end{bmatrix} \begin{bmatrix} J_{22} \\ J_{24} \\ J_{32} \end{bmatrix}$$
(5-403)

These equations may be combined to give the following transformation

$$i \Theta F = [\Delta H F F \qquad (5-4e4)$$

Substituting this into the kinetic energy gives

.

.

$$T = \frac{1}{2} + $

or

.

$$T = \frac{1}{2} + \frac{1}{2} \frac{1}{2} - \frac{1}{2} \frac{1}{2} + \frac{1}{2} \frac{$$

where

ani

$$\begin{bmatrix} A_{II} \end{bmatrix} = 2\pi A_{II} \begin{bmatrix} A \end{bmatrix}$$

(5-457)

.

(5-400)

The second term gives the rotary inertia contribution in terms of lateral velocities. The influence coefficients for case (2) are obtained by setting  $\Theta_{\tau} = 0$  which gives

[E<sub>17</sub>]

in Equation 5-450.

The modes and frequencies for the second case are obtained by iterating

$$[\Xi_{rr}][A_{rr}] + [\Delta][A_{rr}][L_{r}] + [2] = \chi^{2} [\rho] \qquad (5-469)$$

#### 5.2.2.1.1.3 Shear Energy with No Rotary Inertia

For case (3) the influence coefficients are the same as for case (2) and the mass matrix is  $[A_{re}]$ . The modes and frequencies are obtained from

$$[E_{22}T_{A,2}H_{\Psi}] = \lambda \{\psi\}$$
 (5-470)

5.2.2.1.1.- Euler-Bernoulli Beam

For case (4) the inertia matrix is the same as case (3) and the influence maurix is computed by using

$$u_{i} = U_{eI} \quad (5-471)$$

That is, with L CA = I.

#### 5.2.2.1.2 Comparison and Conclusions

The results of the study are summarized in Table 11, where the first ten natural frequencies for the four cases are given.

Figure 101 is a plot of the second and third modes showing a comparison of cases (1), (3), and (-) indicating the important contribution of shear deflections for thin-wall cylinders.

The following conclusions are irawn from this study:

(1) For the lower frequency range of structural vibrations, the Timoshenko theory is more sophisticated than is required, and the important results are obtained if only shear energy is included (this result is well known and has, in fact, been established by Timoshenko). The consequence of this, for the finite degree-of-freedom methods, is a saving of one-half the number of degrees-of-freedom and resulting matrices only one-half as large.

- (2) Attempts to include rotary inertia by assuming that the section rotates normal to the elastic curve is detrimental to the results and is generally inconsistent with the fact that shear energy is stored in the beam.
- (3) The results of this section suggest that a more consistent approximation to the Timoshenko beam is to set the applied moments,  $\{\Theta\}$ , to zero and use Equations 5-451 and 5-452 to obtain

$$\{z\} = \{z_{3}\}\{z\}$$
(5-472)  
$$\{s\} = \{z_{0}\}\{z\}$$
(5-473)

.

or

and use method (2) with

$$[\Delta] = [E_{\text{st}}][E_{\text{st}}]^{T}$$
(5-475)

When shear energy is neglected, the above expression will be approximately that given by the diparabolic method.

-14

FREQUENCY (c.p.s.)

1041	
1403	
Į	
	CASE*

	LST MODE	2ND MODE	3RD MODE	4TH MODE	<b>UTH MODE</b>	6TH MODE	ттн море	BTH MODE	ETH MODE	loth mode
(T)	48.640	264.229	630•379	1045.794	1480.935	1916.491	2345.783	2764.122	31.69.884	3541.367
(0)	48.632	263 <b>.</b> 127	617.507	551.266	1359.437	1691.323	1.991.436	2265.697	2522.639	2770.728
(c)	50.045	313-634	878.290	1721.7744	2848.573	4261.980	5968.023	7976.032	10299.871	12957-733
) (±	148.772	267.938	642.957	1069.378	1513.423	1955.979	240.942	2817.016	3235.647	3649.229

\* Case (1) Timoshenko Beam

Shear Energy with Rotary Incrtia Expressed in Terms of Lateral Displacements. Case (2)

Case (3) No Shear Energy - No Rotary Inertia.

Case (4) Shear Energy - No Rotary Inertia.

# TABLE 11 FREQUENCIES FOR A CYLINDRICAL, THIN-WALL, STRAIGHT BEAM SHOWING THE EFFECTS OF SHEAR ENERGY AND ROTARY INERTIA

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#### 5.2.2.2 Some Considerations of the Vibrations of Axially and Circumferentially Loaded Thin Cylindrical Shells

In this section we consider the analysis of a thin cylindrical shell of length, L, radius, b, and wall thickness,  $\tau$ . The approach to the problem is similar to that used on the thin ring in Paragraph 2.3.3.3. In that analysis we found it convenient to use cylindrical coordinates which will also prove to be the case for the shell. The geometry of the shell is shown in Figure 102.



FIGURE 102 THIN CYLINDRICAL SHELL

The difficult part of the analysis is the derivation of an expression for the specific internal energy,  $u(\pi, \theta, x, t)$ , in terms of displacements of the shell "mid-surface,"  $p_r(\theta, x, t)$ ,  $p_{\theta}(\theta, x, t)$ , and  $p_x(\theta, x, t)$ .

#### 5.2.2.2.1 Strain Energy for a Cylindrical Shell

We assume that the shell is in a state of plane stress with the only nonzero stresses being  $\sigma_{gg}$ ,  $\sigma_{cx}$ , and  $\sigma_{cy}$ . The specific internal strain energy is then

$$\frac{1}{2} = \frac{1}{2} \left[ \frac{1}{2} x_{c} \cdot \overline{x}_{x} - \frac{1}{2} x_{c} \cdot \overline{x}_{0} + \frac{1}{2} \overline{x}_{0} \cdot \overline{x}_{0} \right]$$

$$(5-476)$$

and Hooke's law reduces to

$$\tau_{xx} = \frac{\varepsilon}{1 - \gamma^2} \left( \varepsilon_{xx} + \gamma \epsilon_{\Theta \Theta} \right)$$
(5-477)

$$z_{\mathbf{x}\mathbf{\theta}} = \frac{E}{2^{f_{1}}+\gamma} \epsilon_{\mathbf{x}\mathbf{\theta}}$$
(5-478)

$$\tau_{\Theta\Theta} = \frac{\mathcal{E}}{(-\gamma)^2} \left[ \varepsilon_{\Theta\Theta} + \gamma \epsilon_{xx} \right]$$
(5-479)

Substitution into Equation 5-476 gives the specific internal energy of a particle in terms of the strains

$$\mathfrak{u}\left(\mathfrak{n}_{1}\theta_{1}\mathfrak{x},\mathfrak{t}\right) = \frac{\mathcal{E}}{\mathfrak{c}^{1}_{1}-\gamma^{2}}\left(\epsilon_{xx}^{2} + \epsilon_{\theta\theta}^{2} + 2\gamma^{2}\epsilon_{xx}\epsilon_{\theta\theta} + \frac{1-\gamma^{2}}{2}\epsilon_{x\theta}^{2}\right)$$
(5-480)

#### 5.2.2.2.1.1 Strain-Displacement Relations for a Cylindrical Shell

Since the shell surface is described by r = b, the coordinates and x are coordinates in the surface of the shell, and we then can introduce a position vector for the  $\theta$ -x particle at time, t.

$$n \sim n(\theta_1 x, t)$$
 (5-481)

We shall denote the position vector for the  $\theta$ -x particle when in the undeformed state by

.

.

$$\mathbb{L}(\theta, x) = x \mathbb{I}_{x} + \delta \mathbb{I}_{n}$$
(5-482)

where  $V_n$ ,  $V_g$ , and  $V_x$  are a set of cylindrical coordinate unit vectors. The displacement of marticles on the shell mid-surface is then given by

$$\mathbb{P}(\Theta, \mathbf{x}, \mathbf{t}) = \mathfrak{M}(\Theta, \mathbf{x}, \mathbf{t}) - \mathbb{L}(\Theta, \mathbf{x})$$
(5-483)



FIGURE 103 SHELL DISPLACEMENTS

If we denote the components of p in the  $I_\theta$  ,  $I_\lambda$  , and  $I_\Pi$  directions by  $P_\theta$  ,  $p_x$  and  $p_r,$  then

$$p(a,x,z) = r_a e(x,z) \hat{r}_a + \hat{p}_x(a,x,z) \hat{r}_x - \hat{p}_x(a,x,z) \hat{r}_1$$
 (5-484)

The strains of the mid-surface can be defined by

$$un dx - dt \cdot dt \qquad (5-465)$$

$$= 2\left(e_{11} dx dx + de_{20} baxdo - d_{20} b^{2} dx ds\right)$$

$$= c^{2}$$

The total strains are then assumed to be

$$\hat{e}_{xx} = \hat{e}_{xx} - 1 - \hat{e}_{y} \frac{\partial^{2} \hat{p}_{y}}{\partial x^{2}}$$
 (5-486)

$$\varepsilon_{rg} = \varepsilon_{rg} - \frac{1}{2} \frac{$$

 $f_{66} = f_{66} = \frac{1}{260} \frac{1}{12} \frac{1}{3^2} \frac{3^2 \beta_2}{56^2} - \frac{2\beta_2}{30}$ (5-468)

The terms depending on the mid-surface curvatures used here are those proposed by Timoshenko for cylindrical shells in which  $\tau \ll$  b. The more general case has been considered by Love.

Now from Equation 5-482,

$$dL = \frac{3L}{2X} dx + \frac{3L}{33} dG$$
(5-489)  
=  $T_{1} dx + T_{2} dG$ 

.

and from Equation 5-403,

$$\begin{aligned} dx &= \frac{1}{3x} + \frac{12}{3x} $

which gives

$$\frac{1}{3} - \frac{1}{3} = 2 \frac{1}{3} \cdot \frac{1}{3} + \frac{1}{3} \cdot \frac{3}{34} + \frac{1}{34} + \frac$$

By comparing this with Equation 5-465, we conclude

$$\dot{z}_{xx} = \dot{I}_{x} \cdot \frac{\dot{z}_{x}}{\dot{z}_{x}} - \dot{z} \cdot \frac{\dot{z}_{x}}{\dot{z}_{x}} \cdot \frac{\dot{z}_{x}}{\dot{z}_{x}}$$
(5-492)

Using Equation 5-484 together with Equations 5-480, 5-407, and 5-402, results in the final equations for the strains in terms of displacements

$$\varepsilon_{r_{\lambda}} = \frac{1}{12} + \frac{1}{3} \frac{1}{12} \frac{1}{1$$

# 5.2.2.2.1.2 The Total Strain Energy Including Monlinear Jerrs in the Strain-Displacement Relations

In order to simplify the operations, we introduce the following notation for the nonlinear part of the strains

Equations 5-497, 5--91, and 5--97 can then be written as

.

$$\frac{1}{2}\alpha = \frac{1}{2}\frac{\alpha}{4} + \frac{1}{2}\alpha + \frac{1}{2}\frac{\sqrt{2}}{2}$$
 (5-501)

Now, the total strain energy is

.

$$U = \int u \, dV \qquad (5-504)$$
  
= 
$$\int_{C}^{L} \int_{0}^{2\pi} \int_{b-\frac{\pi}{2}}^{b+\frac{\pi}{2}} \frac{\varepsilon}{\overline{z(1-\gamma^{2})}} \left( \varepsilon_{xx}^{2} + \varepsilon_{\varepsilon\Theta}^{2} + 2\gamma \varepsilon_{xx} \varepsilon_{\Theta\Theta} + \frac{1-\gamma^{2}}{2} \varepsilon_{x\Theta}^{2} \right) \delta dr d\Theta dx$$

When Equations 5-501, 5-502, and 5-503 are substituted into this expression, the integration with respect to r can be made explicitly. The result is

(5-505)

$$U = \frac{1}{2} \int_{0}^{1} \int_{0}^{2\pi} \frac{E\tau}{(1-Y^{2})} \left( \frac{(\frac{\partial}{\partial \chi})^{2}}{(\frac{\partial}{\chi})^{2}} + \frac{1}{6^{2}} (\frac{\partial}{\partial \theta} + \frac{1}{2}\pi)^{2} + \frac{1}{2} \frac{\partial}{\partial \theta} + \frac{1}{2}\pi \right)^{2} + \frac{1}{2} \frac{\partial}{\partial \theta} + \frac{1}{2}\pi \right)^{2} + \frac{1}{2} \frac{\partial}{\partial \theta} $

where we have neglected terms of the second order in the  $\Delta$ 's. In the above expression, the first integral is the energy due to stretching of the mid-surface, the "membrane" energy. The second integral is the bending energy, and the last integral is the nonlinear part of the membrane energy.

Let us consider the stress resultants:

$$N_{2x} = \int_{6-3}^{6+3} dx \qquad (5-506)$$

.

$$N_{XG} = \int_{4-\frac{1}{2}}^{6+\frac{1}{2}} \frac{C_{XG}}{C_{XG}} dr$$
 (5-507)

$$N_{\Theta} = \int_{1}^{1+T_{2}} \sigma_{\Theta} dr \qquad (5-508)$$

Then, from Hooke's law, we obtain

.

.

.

$$H_{xx} = \frac{\varepsilon_{\Gamma}}{1-\gamma^2} \left( \varepsilon_{xx} + \gamma' \varepsilon_{\infty} \right)_{\eta=0}$$
 (5-509)

$$N_{xy} = \frac{c}{2(1+\gamma)} \left( \hat{c}_{xy} \right)_{x=y}$$
 (5-510)

$$N_{\theta\theta} = \frac{\varepsilon \tau}{1-\gamma^{1}} \left( \varepsilon_{\theta\theta} + \gamma \varepsilon_{w} \right)_{n=0}$$
 (5-511)

The integral of the bending strains gives no contribution. Solving for the strains gives

.

.

$$\frac{1+\gamma}{3\gamma} + \Delta_{xx} = \frac{1}{10} \left( N_{xy} - \gamma N_{go} \right)$$
 (5-512)

$$\frac{1}{2}\frac{\partial 2}{\partial x} + \frac{1}{2}\frac{\partial 2}{\partial \Theta} + \Delta_{x\Theta} = \frac{2(1+y)}{ET} N_{x\Theta}$$
(5-513)

$$\frac{1}{2} \frac{1}{2} \cdot \frac{1}{2} p_{x} = \frac{1}{2} \cdot $

The linear part of the membrane strains is then

$$\frac{j_{x}}{j_{x}} = \frac{1}{k_{x}} + \frac{1}{k_{x}} + \frac{1}{k_{x}} + \frac{1}{k_{x}} + \frac{1}{k_{x}}$$
(5-515)

$$\frac{z}{2x} + \frac{z}{2x} \frac{z}{2x} + \frac{z}{2z} \frac{z}{2x} + \frac{z}{2z} + \frac{$$

.

If we substitute these expressions into the third integral in Equation 5-505, we obtain, for that integral, the expression:

$$= \int_{0}^{2\pi} \int_{0}^{2\pi} \frac{1}{\sqrt{k}} \left( \frac{1}{\sqrt{k}} \Delta_{x\theta} + \frac{1}{\sqrt{2}} \Delta_{x\theta} + \frac{1}{\sqrt{2}} \Delta_{x\theta} \right) + \lambda e^{-\frac{1}{2}\lambda}$$
(5-518)

where, as before, squared terms in the  $\Delta$ 's have been neglected. An approximate expression for this integral can be obtained by using the stress resultants for the linear membrane equations. These equations are obtained by neglecting bending and the nonlinear terms. The result can be shown to be given by

$$\frac{\gamma_{xx}}{\gamma^{\lambda}} \cdot \frac{\gamma}{\gamma} N_{xy} = -\left[P_{x} - \gamma \frac{\gamma_{x}}{\gamma t^{2}}\right]$$
(5-519)

$$= \frac{1}{6} \frac{\partial N_{ij}}{\partial \theta} + \frac{\partial N_{ij}}{\partial x} = -\left(P_{\theta} - \rho \tau \frac{\partial p_{\theta}}{\partial t}\right)$$
(5-520)

 $h_{111} = (h_{11} - (t_{11}^{2}))$  (5-521)

where  $P_n$  ,  $P_{\sigma}$  , and  $P_{\pi}$  are derived from the virtual work of applied forces,

$$SN = \frac{c_{rel}}{s_{r}} (s_{r} s_{r}^{2} - s_{r} s_{r}^{2} R + s_{r}^{2} R) + lex$$
 (5-522)

If we consider only the case where the inertia terms can be neglected, the above equations may be integrated to give

$$v_{xx} = -\int_{-\infty}^{\infty} P_x \, dx \, + \frac{1}{5} \int_{-\infty}^{-\infty} \frac{1}{35} - \frac{1}{35} \int_{-\infty}^{\infty} \frac{1}{35} \, dx \, dx \qquad (5-523)$$

$$N_{xy} = -\int_{0}^{x} P_{y} + \frac{2P_{z}}{2y} jx$$
 (5-524)

A case of common interest is that of an axially loaded shell that is internally pressurized. If  $\mathbf{P}$  is the total axial load (positive for compression) and  $\mathbf{p}_{\mathbf{o}}$  is the internal pressure, we have

$$h_c - t_x$$
 (5-526)

$$P_{g} = 0$$
 (5-527)

$$f_{x} = c_{x} f_{x} - c_{x}$$

where  $\mathcal{P}(\mathbf{x})$  is the Dirac function. For this case, substitution into Equations 5-523, 5-524, and 5-525 gives

.

$$N_{1x} = P_{1x} + \frac{p}{r_{1x}} + C \times x < L$$
 (5-529)

$$v_{43} = fr \frac{\tau}{2}$$
 (5-531)

The final expression for the strain energy, using Equation 5-518 and Equations 5-498, 5-499, and 5-500, is

$$U^{2} = \left[ \frac{1}{2} \int_{0}^{2} \int_{0}^{2} \frac{\Xi r}{-\nu^{2}} \left( \left( \frac{\partial F_{1}}{\partial x} \right)^{2} + \left( \frac{\partial}{\partial x} \frac{\partial F_{0}}{\partial y} + \frac{F_{1}}{\partial x} \right)^{2} + \frac{2P_{1}^{2} A_{1}}{\partial x} \left( \frac{\partial}{\partial x} \frac{\partial F_{0}}{\partial y} + \frac{F_{1}}{\partial x} \right) \right] + \frac{2P_{1}^{2} A_{2}}{2} \left( \frac{\partial F_{0}}{\partial x} + \frac{F_{1}}{\partial x} \right)^{2} + \frac{2P_{1}^{2} A_{2}}{\partial x} \left( \frac{\partial F_{0}}{\partial x} + \frac{F_{1}}{\partial x} \right)^{2} \right) + d\sigma dx$$

$$+ \left[ \frac{2}{2} \int_{0}^{2} \int_{0}^{2} \frac{Z r}{(2 + \nu)^{1/2}} \left( \left( \frac{\partial F_{1}}{\partial x} \right)^{2} + \left( \frac{\partial F_{0}}{\partial x} + \frac{F_{1}}{\partial x} - \frac{\partial F_{0}}{\partial x} \right)^{2} + \frac{2P_{1}^{2} A_{2}}{\partial x} \left( \frac{\partial F_{0}}{\partial x} - \frac{F_{1}}{\partial x} \frac{\partial F_{0}}{\partial x} \right)^{2} \right) + d\sigma dx$$

$$+ \left[ \frac{2}{2} \int_{0}^{2} \int_{0}^{2} \frac{Z r}{(2 + \nu)^{1/2}} \left( \left( \frac{\partial F_{1}}{\partial x} + \frac{\partial F_{1}}{\partial x} + \frac{\partial F_{0}}{\partial x} - \frac{F_{1}}{\partial x} \frac{\partial F_{0}}{\partial x} \right)^{2} \right) + \frac{A\sigma d}{d\sigma dx}$$

$$+ \left[ \frac{2}{2} \int_{0}^{2} \int_{0}^{2} \frac{A r}{(2 + \nu)^{1/2}} \left( \frac{A r}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} \right)^{2} \right] d\sigma dx$$

$$+ \left[ \frac{A r}{2} \int_{0}^{2} \frac{A r}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} \right] d\sigma dx$$

$$+ \left[ \frac{A r}{2} \int_{0}^{2} \frac{A r}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} \right] d\sigma dx$$

$$+ \left[ \frac{A r}{2} \int_{0}^{2} \frac{A r}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} \right] d\sigma dx$$

$$+ \left[ \frac{A r}{2} \int_{0}^{2} \frac{A r}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} \right] d\sigma dx$$

$$+ \left[ \frac{A r}{2} \int_{0}^{2} \frac{A r}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} + \frac{A F_{0}}{(2 + \nu)^{1/2}} \right] d\sigma dx$$

•

## 5.2.2.2.2 The Kinetic Energy

The total kinetic energy of the shell is

$$T = \frac{1}{2} \int \left(\frac{\partial p}{\partial t}\right)^{2} \rho \, dV$$
  
=  $\frac{1}{2} \int_{0}^{1} \left(\frac{\partial r}{\partial t}\right)^{2} \left(\frac{\partial r}{\partial t}\right)^{2} + \left(\frac{\partial p}{\partial t}\right)^{2} + \left(\frac{\partial p}{\partial t}\right)^{2} + \left(\frac{\partial p}{\partial t}\right)^{2} \right) \rho \, \delta \, dn \, d\theta \, dx$ 

 $\mathbf{or}$ 

$$T = \frac{1}{2} \int_{-\frac{1}{2}}^{\sqrt{2}} \frac{1}{2} \frac{dp}{dt} + \left(\frac{dp}{dt}\right)^2 + \left(\frac{dp}{dt}\right$$

# 5.2.2.3 <u>A Finite Degree-of-Freedom Representation of a Cylindrical</u> Shell using Diparabolic Interpolation

# 5.2.2.3.1 Description of the Method

If we choose collocation points spaced at equal intervals on the shell, we can divide the shell surface into a number of regions as in Figure 104.





In Figure 105 a typical region is shown on the inside surface of the developed shell.



FIGURE 105 INSIDE OF DEVELOPED SHELL SHOWING TYPICAL

REGION

For the i<sup>th</sup> region, let the coordinates of the upper left corner be  $x = x_i$  and  $\theta = \theta_i$  and introduce non-dimensional coordinates defined by the following equations

$$s = \frac{s-3i}{2}$$
 :  $i = \frac{1}{4}$  (5-534)

$$\gamma = \frac{4\pi^2 - \frac{1}{2} + \frac{1}{2}}{w}$$
 (5-535)

The total number of regions is  $\mathbb{N}\cdot\mathbb{M}$  , where  $\mathbb{N}$  and  $\mathbb{M}$  are integers.

The kinetic energy can then be written as a sum over the N.M regions

$$\tau = \sum_{i=1}^{N-M} \iint_{\mathcal{T}_{i}} \left( \frac{(1-1)^{2}}{2t} + \frac{(1-1)^{2}}{2t} + \frac{(1-1)^{2}}{2t} \right)^{2} \text{ (odeax}$$
(5-536)

If we change the variable of integration to  $(\xi,\eta)$ , then

$$fd\theta dx = wldsay$$
 (5-537)

and

$$T = \frac{1}{2} \sum_{l=1}^{N-M} \int_{0}^{l} \int_{0}^{l} \rho_{T} w t \left( \frac{\partial p_{T}}{\partial t} \right)^{2} + $

The procedure is very similar to that followed for the plate in Paragraph 2.3.3.2. We assume

$$p_{1}(\theta, \mathbf{x}, t) = \{ f(\mathbf{x}, \mathbf{y}) \} [ f ]_{1} [ b_{1} \}_{1}$$
(5-539)

$$p_{\theta}(\theta, \mathbf{x}, \mathbf{t}) = \{ j(\mathbf{s}, \mathbf{y}) \} [ j ]_{i} \{ p_{\theta} \}_{i}$$
(5-540)

$$p_{x} = \{j \in \{1\}, j\} \{j\}_{i} \{j_{x}\}_{i}$$
(5-541)

where  $\{f(\xi,\eta)\}$  is given in Equation 2-466 of Paragraph 2.3.3.2 and the interpolation coefficients are the same for every region except those regions adjacent to the top and bottom edges. The construction of the edge interpolation coefficients is briefly discussed in Paragraph 2.3.3.2. At the right and left hand edges, the points falling off the surface are not eliminated but will be eliminated later by a compatibility condition at the cut,  $\theta = 0$ .

Substituting Equations 5-539, 5-540, and 5-541 into the expression for the kinetic energy, we obtain

$$T = \sum_{i=1}^{N-N} 2\pi w \ell \left( i \beta_{i} l_{i} [\mathcal{D}_{i}]_{i} [\mathcal{D}_{$$

where

$$[T_{1}] = \int_{0}^{1} e^{-\frac{1}{2}(s,\gamma)\frac{1}{2}(s,\gamma)\frac{1}{2}} \frac{1}{2}s^{\frac{1}{2}}\gamma}$$
 (5-543)

which is the matrix previously introduced in the plate analysis (see Equation 2-479) and is listed in Appendix IV.

A compatibility matrix can be constructed which relates the displacements of a region to the displacements of the whole shell and sets the displacements along the cut,  $\theta = 0$ , equal

$$\frac{1}{2} F_{\tau} \frac{1}{3} = \left( T \right) \frac{1}{2} \frac{1}{2} F_{\tau} \frac{1}{3}$$

.

The same transformation relates the other components of displacement

$$(5-5+5)$$

$$\frac{1}{2} \left\{ x_{i}^{2} = [T]_{i}^{2} \left\{ x_{i}^{2} \right\}$$
 (5-546)

Substitution of these relations into the kinetic energy gives

where

$$\begin{bmatrix} 2 & -1 \\ -1$$

The strain energy of the shell is considered in the same way. From Equation 5-532, we obtain

$$U = \frac{i/2}{\sum_{k=1}^{N-M}} \iint_{S_{k}} \frac{\sum_{i} T^{i}}{\int_{I} (\frac{E_{i} T^{i}}{2(1-p^{2})} \left( \left(\frac{\partial^{2} p_{i}}{\partial x^{2}}\right)^{2} + \left(\frac{\partial^{2} p_{i}}{d^{2} \partial \sigma^{2}} - \frac{i}{d} - \frac{\partial P_{o}}{d \partial \sigma}\right)^{2} + 2 \gamma \left(\frac{\partial^{2} p_{i}}{\partial x^{2}}\right) \left(\frac{\partial^{2} p_{i}}{d^{2} \partial \sigma^{2}} - \frac{i}{d} - \frac{\partial P_{o}}{d \partial \sigma}\right) + 2 \left((-\gamma) \left(\frac{\partial^{2} p_{i}}{d \partial x \partial \sigma} - \frac{i}{d} - \frac{\partial P_{o}}{\partial x}\right)^{2}\right) d\sigma dx$$

477

When we make the change of variable in Equations 5-534 and 5-535, the derivatives transform as follows:

.

$$\frac{\partial^{2}p_{T}}{\partial x^{2}} = \frac{1}{\lambda^{2}} \frac{\partial^{2}p_{T}}{\partial x^{2}} : \qquad \frac{\partial^{2}x}{\partial x} = \frac{1}{\lambda} \frac{\partial^{2}x}{\partial x} \qquad (5-550)$$

$$\frac{\partial^{2}p_{T}}{\partial x^{2}} = \frac{1}{\lambda^{2}} \frac{\partial^{2}p_{T}}{\partial y^{2}} : \qquad \frac{\partial^{2}x}{\partial x^{2}} = \frac{1}{\lambda} \frac{\partial^{2}x}{\partial y}$$

$$\frac{\partial^{2}p_{T}}{\partial x^{2}} = \frac{1}{\lambda^{2}} \frac{\partial^{2}p_{T}}{\partial y^{2}} : \qquad \frac{\partial^{2}p_{T}}{\partial x^{2}} = \frac{1}{\lambda^{2}} \frac{\partial^{2}p_{T}}{\partial y}$$

$$\frac{\partial^{2}p_{T}}{\partial x^{2}} = \frac{1}{\lambda^{2}} \frac{\partial^{2}p_{T}}{\partial x^{2}} : \qquad \frac{\partial^{2}p_{T}}{\partial x^{2}} = \frac{1}{\lambda^{2}} \frac{\partial^{2}p_{T}}{\partial y}$$

also,

.

\*

The strain energy then becomes

.

$$U = \frac{M}{Z} \sum_{k=1}^{M,M} \int_{0}^{1} \left( \frac{1}{F_{2,1}} \frac{T^{3}}{F_{2,1}} \left( \frac{M}{F_{2}} \left( \frac{\partial^{2} P}{\partial g^{3}} \right)^{2} + \frac{f}{M^{3}} \left( \frac{\partial^{2} P}{\partial \eta^{1}} - \frac{M}{F} \frac{\partial P}{\partial \eta} \right)^{2} \right) \right) dS d\eta$$

$$+ \frac{2}{Wf} \left( \frac{\partial^{2} P}{\partial g^{3}} \right) \left( \frac{\partial^{2} P}{\partial \eta^{1}} - \frac{M}{F} \frac{\partial P}{\partial \eta} \right) + \frac{2(r-V)}{Wf} \left( \frac{\partial^{2} P}{\partial g^{3} \partial \eta} - \frac{M}{F} \frac{\partial P}{\partial g} \right)^{2} \right) dS d\eta$$

$$+ \frac{1}{V_{Z}} \sum_{k=1}^{M,M} \int_{0}^{1} \left( \frac{r}{r-V^{2}} \right) \left( \frac{M}{f} \left( \frac{\partial P}{\partial g} \right)^{k} + \frac{f}{M} \left( \frac{\partial P}{\partial g} + \frac{M}{F} \right)^{2} \right)^{2}$$

$$+ \frac{2}{V} \left( \frac{\partial P}{\partial g} \right) \left( \frac{\partial P}{\partial \eta} + \frac{M}{F} \right)^{2} + \frac{f}{M} \left( \frac{\partial P}{\partial g} + \frac{M}{F} \right)^{2}$$

$$+ \frac{2}{V} \left( \frac{\partial P}{\partial g} \right) \left( \frac{\partial P}{\partial \eta} + \frac{M}{F} \right)^{2} + \left( \frac{r-V}{2} \right) \frac{M}{f} \left( \frac{1}{2} \frac{\partial P}{\partial \eta} + \frac{M}{F} \right)^{2} \right)^{2} dS d\eta$$

$$+ \frac{1}{V_{Z}} \sum_{k=1}^{M,M} \int_{0}^{1} \left( \frac{\partial P}{\partial \eta} + \frac{M}{F} \right)^{2} + \left( \frac{\partial P}{\partial g} \right)^{k} + \left( \frac{\partial P}{\partial \eta} \right)^{k} + \frac{f}{2} \frac{\partial P}{\partial \eta} \right)^{2} dS d\eta$$

$$+ \frac{1}{V_{Z}} \sum_{k=1}^{M,M} \int_{0}^{1} \left( \frac{\partial P}{\partial \eta} + \frac{M}{F} \right)^{2} + \left( \frac{\partial P}{\partial g} \right)^{k} + \left( \frac{\partial P}{\partial g} \right)^{k} + \left( \frac{\partial P}{\partial g} \right)^{k} + \left( \frac{\partial P}{\partial \eta} \right)^{2} \right)^{2} dS d\eta$$

$$+ \frac{1}{V_{Z}} \sum_{k=1}^{M,M} \int_{0}^{1} \left( \frac{\partial P}{\partial \eta} + \frac{M}{F} \right)^{2} + \left( \frac{\partial P}{\partial g} \right)^{k} + \left( \frac{\partial P}{\partial g} \right)^{k} + \left( \frac{\partial P}{\partial g} \right)^{k} \right)^{k} + \left( \frac{\partial P}{\partial g} \right)^{k} \right)^{k}$$

$$+ \frac{1}{V_{Z}} \sum_{k=1}^{M,M} \int_{0}^{1} \left( \frac{\partial P}{\partial \eta} + \frac{M}{F} \right)^{2} + \left( \frac{\partial P}{\partial g} \right)^{k} + \left( \frac{\partial P}{\partial g} \right)^{k} + \left( \frac{\partial P}{\partial g} \right)^{k} \right)^{k} + \frac{1}{2} \frac{\partial P}{\partial \eta} \right)^{k}$$

$$+ \frac{1}{V_{Z}} \sum_{k=1}^{M,M} \int_{0}^{1} \left( \frac{\partial P}{\partial \eta} + \frac{P}{F} \right)^{2} + \left( \frac{\partial P}{\partial g} \right)^{k} + \frac{1}{2} \frac{\partial P}{\partial \eta} \right)^{k}$$

$$+ \frac{1}{2} \sum_{k=1}^{N} \int_{0}^{1} \left( \frac{\partial P}{\partial \eta} + \frac{1}{F} \right)^{k} + \frac{1}{2} \frac{\partial P}{\partial \eta} \right)^{k} + \frac{1}{2} \frac{\partial P}{\partial$$

Using Equations 5-539, 5-540, and 5-541, we may then express this in terms of a finite number of degrees-of-freedom.

$$\frac{\partial^{4} \mathfrak{p}_{n}}{\partial \mathfrak{s}^{2}} = \left\{ \frac{\partial^{2} \mathfrak{f}}{\partial \mathfrak{s}^{2}} (\mathfrak{s}, \eta) \right\}^{\ell} [\mathfrak{I}]_{\mathfrak{i}} \left\{ \mathfrak{p}_{n} \mathfrak{s}_{\mathfrak{i}} \right\}$$
(5-553)

$$\frac{\delta^{t} b_{n}}{\delta t^{2}} = \frac{1}{2} \frac{\delta^{t} f}{\delta t^{2}} (s, \eta) f(s)_{i} \{b_{n}\}_{i}$$
(5-554)

$$\frac{2}{2} = \{ \frac{2}{2} (s, \gamma) \}^{\prime} [s]_{i} \{ b o \}_{i}$$
 (5-555)

$$\frac{\partial^{1} h_{n}}{\partial s_{2} v_{1}} = i \frac{\partial^{2} f_{1}}{\partial s_{1} v_{1}} (s, \eta) f(s]_{i} \{h_{n}\}_{i}$$
(5-556)

$$\frac{2}{3} = \left\{ \frac{1}{3} (s, v) \right\} (s]_{i} \left\{ \frac{1}{3} \right\}_{i}$$
(5-557)

$$\frac{\partial p_x}{\partial s} = \frac{1}{\partial s} \frac{\partial f}{\partial s}(s,\eta) \left[ (s,\eta) + \frac{1}{2} \right]_{i} \left[ \frac{1}{2} \right]$$

$$p_n = \frac{1}{2} f(s, \eta) \hat{s}'(s)_i \hat{z} p_n \hat{z}_i$$
(5-559)

$$\frac{dx}{dy} = -\frac{dy}{dy} (x_{1}) \frac{dy}{dy} (x_{1}) \frac{dy}{dy}$$
(5-560)

When these terms are substituted into the strain energy, the following type of integrals will result:

$$[r_i] = \int_{0}^{1} \frac{1}{2} f_i^2 dd \eta \qquad (5-561)$$

$$[\Gamma_{2}] = \int_{0}^{1} \int_{0}^{1} \frac{i^{2}}{i^{2}} H_{22}^{2} \int dedu \qquad (5-562)$$

$$[\Gamma_{3}] = \int_{0}^{1} \int_{0}^{1} \frac{\partial Y}{\partial \eta} \left\{ \frac{\partial Y}{\partial \eta} \right\}' dt d\eta \qquad (5-563)$$

$$[r_{+}] = \int_{0}^{1} \int_{0}^{1} \frac{1}{2} \left\{ \frac{\partial Y}{\partial t}, \frac{\partial Y}{\partial t}, \frac{1}{2} + \frac{\partial Y}{\partial t}, \frac{\partial$$

$$[\Gamma_{s}] = \int_{0}^{1} \int_{0}^{1} \{\frac{b^{2}}{33a_{1}}\}^{2} \frac{1}{a_{s}a_{1}} \int_{0}^{1} ds dy \qquad (5-565)$$

$$[\Gamma_{7}] = \int_{0}^{1} \int_{0}^{1} \{ \frac{\partial f}{\partial s} \} \{ \frac{\partial f}{\partial s} \} dsa\eta \qquad (5-566)$$

$$[\Gamma_{\bullet}] = \int_{0} \int_{0}^{1} \left\{ \frac{\partial f}{\partial \eta} \right\} \left\{ \frac{\partial f}{\partial \eta} \right\} ds d\eta \qquad (5-567)$$

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$$[\Gamma_{4}] = \int_{0}^{1} \int_{0}^{1} \frac{1}{2} \left( \frac{1}{23} \frac{1}{2} \frac{1}{3} \frac{1}{3} \frac{1}{3} \frac{1}{3} + \frac{1}{3} $

$$[\Gamma_{i}] = \int_{0}^{1} \int_{0}^{1} \{\frac{2i}{32}\} \{\frac{2i}{34}\} dsd_{1}$$
 (5-569)

$$[\Gamma_{12}] = \int_{0}^{1} \int_{0}^{1} \frac{\partial^{2}f}{\partial \eta_{1}} \frac{\partial^{2}f}{\partial \eta_{2}} $

$$[\Gamma_{u}] = \int_{0}^{1} \int_{0}^{1} \left\{ \frac{\partial^{2} f}{\partial s} \right\} \left\{ \frac{\partial f}{\partial \eta} \right\}^{2} ds d\eta \qquad (5-571)$$

$$[\Gamma_{+}] = \int_{0}^{1} \int_{0}^{1} \frac{\partial^{2} f}{\partial s \partial \eta} + \frac{\partial f}{\partial s} \frac{\partial f}{\partial s} + \frac{\partial f}{\partial s} + \frac{\partial f}{\partial s} \frac{\partial f}{\partial s}$$

$$[\Gamma_{G}] = \int_{0}^{1} \int_{0}^{1} \frac{4f \mathcal{J} \left[\frac{\Delta f}{\delta m}\right]' - \left\{\frac{\Delta f}{\delta \eta}\right\} \left\{f\right\}}{2} \, ds \, d\eta \qquad (5-573)$$

$$[\Pi_{b}] = \int_{0}^{1} \int_{0}^{1} \frac{\{f\}\{\frac{2f}{\delta g}\}' - \{\frac{2f}{\delta g}\}\{f\}'}{2} \, ds \, d\eta \qquad (5-574)$$

$$[\Gamma_{\eta}] = \int_{0}^{1} \int_{0}^{1} \left\{ \frac{\partial f}{\partial \eta} \right\} \left\{ \frac{\partial f}{\partial s} \right\}^{\prime} ds d\eta \qquad (5-575)$$

These integrals of polynomials can be easily evaluated independently of the geometry of the shell and are tabulated in Appendix IV.

Substitution of Equations 5-553 through 5-560 into Equation 5-552 gives

$$U = \frac{1}{2} \sum_{i=1}^{N^{M}} \left( \frac{1}{2} p_{n} \beta_{i}^{'} [s]_{i}^{'} [r_{nn}] [s]_{i}^{'} \frac{1}{2} p_{n} \beta_{i} \right)$$

$$+ 2 \frac{1}{2} p_{n} \beta_{i}^{'} [s]_{i}^{'} [r_{ne}] [s]_{i}^{'} \frac{1}{2} p_{e} \beta_{i}$$

$$+ 2 \frac{1}{2} p_{n} \beta_{i}^{'} [s]_{i}^{'} [r_{nn}] [s]_{i}^{'} \frac{1}{2} p_{n} \beta_{i}$$

$$+ \frac{1}{2} \frac{1}{2} p_{e} \beta_{i}^{'} [s]_{i}^{'} [r_{nn}] [s]_{i}^{'} \frac{1}{2} p_{n} \beta_{i}$$

$$+ \frac{1}{2} \frac{1}{2} p_{e} \beta_{i}^{'} [s]_{i}^{'} [r_{nn}] [s]_{i}^{'} \frac{1}{2} p_{n} \beta_{i}$$

$$+ \frac{1}{2} \frac{1}{2} p_{e} \beta_{i}^{'} [s]_{i}^{'} [r_{nn}] [s]_{i}^{'} \frac{1}{2} p_{n} \beta_{i}$$

$$+ \frac{1}{2} \frac{1}{2} p_{e} \beta_{i}^{'} [s]_{i}^{'} [r_{nn}] [s]_{i}^{'} \frac{1}{2} p_{n} \beta_{i}$$

$$\begin{bmatrix} 1 \\ 1^{n} \end{bmatrix} = \frac{2\pi^{3}}{(cL-2)(c_{1},c_{1})} = \frac{4e^{-2}}{N} \left[ \Gamma_{1} \right] + \frac{4e^{-1}}{L} \left[ \Gamma_{2} \right] - \frac{cL}{N} \left[ \Gamma_{3} \right] + 2\gamma \left[ \Gamma_{4} \right] + 2(c_{1},c_{1}) \left[ \Gamma_{3} \right] + 2(c_{1},c_{2}) \left[ \Gamma_{3} \right]$$

$$\sum_{i=1}^{n} = \frac{z\tau^{i}}{(z^{i} - v^{i})} \cdot \frac{\mathcal{H}\left(\frac{z}{v}\right)^{2} \frac{z^{i}}{z^{i}}\left[\Gamma_{0}\right] - \frac{z}{\frac{z}{v}} \frac{z^{i}}{v^{i}}\left[\Gamma_{2}\right] - z\gamma \frac{z}{v}\left[\Gamma_{2}\right]}{(z^{i})^{2}} - z(-\gamma)\left(\frac{z}{v}\right)\left[\Gamma_{0}\right]$$

$$= z(-\gamma)\left(\frac{z}{v}\right)\left[\Gamma_{0}\right]$$

$$= z(-\gamma)\left(\frac{z}{v}\right)\left[\Gamma_{0}\right]$$

$$= z(-\gamma)\left(\frac{z}{v}\right)\left[\Gamma_{0}\right]$$

$$= z(-\gamma)\left(\frac{z}{v}\right)\left[\Gamma_{0}\right]$$

$$= z(-\gamma)\left(\frac{z}{v}\right)\left[\Gamma_{0}\right]$$

$$[\Gamma_{r_{Y}}] = \frac{E_{T}}{\nu I} \frac{1}{(1-\nu^{2})} \frac{1}{\nu} \left(\frac{L}{\nu}\right)^{2} \left(\frac{2\pi}{N}\right)^{3} \nu \left[\Gamma_{b}\right]$$
(5-579)

$$\begin{bmatrix} \frac{1}{92} \end{bmatrix} = \frac{\frac{1}{2} \frac{1}{2} \frac{1}{2} - \frac{1}{1 + 2^{2}}}{\frac{1}{2} \frac{1}{2} $

$$+ \frac{1}{2V} N_{\Theta\Theta} \left( \frac{2V}{N} \right)^{2} \left[ \Gamma_{i} \right] + \frac{2V}{L} N_{NY} \left[ \Gamma_{j} \right] + \frac{1}{2V} N_{\Theta\Theta} \left[ \Gamma_{i}^{2} \right] + N_{N\Theta} \left[ \Gamma_{i}^{2} \right]$$

$$\left[ \frac{2\pi^{3}}{2\pi} + \frac{2\pi^{3}}{2\pi} + \frac{2\pi^{2}}{2\pi} + \frac{2\pi^{2}}$$

$$\begin{bmatrix} \sum_{\tau \in \mathcal{A}} \frac{1}{\tau_{1}} \begin{bmatrix} z_{1} & z_{1} \\ z_{2} & z_{1} & z_{1} \end{bmatrix} = z_{1} + z_{1$$

New, using the compatibility transformations (Equations 5-544, 5-545, and 5-546), we have

where

$$[\kappa_{vv}] = \sum_{i=1}^{N-M} [\tau]_{i} [j]_{i} [\tau_{vv}] [j]_{i} [\tau]_{i}$$
(5-584)

$$[x_{10}] = \sum_{i=1}^{N} [T_{i}^{1}(z)_{i}^{1}(\Omega_{y})] \ge l_{i}^{1}[T_{i}^{1}]$$
(5-585)

where

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$$[\kappa_{n_{2}}] = \sum_{i=1}^{MM} [\tau_{i}^{i}(z_{1})_{i}^{i}(z_{2})_{i}^{i}(z_{1})_{i}] (\tau_{i})$$
(5-586)

$$[\kappa_{\mathfrak{H}}] = \sum_{j=1}^{N-M} [\mathsf{T}]_{i}^{j} [\mathcal{T}]_{i}^{j} [\mathcal{T}]_{i}^{j} [\mathcal{T}]_{i}^{j} [\mathcal{T}]_{i}^{j} (\mathcal{T}]_{i}^{j} (\mathcal{T})_{i}^{j} (\mathcal{T})_$$

$$[\kappa_{\alpha x}] = \sum_{i=1}^{N} [\tau_{i}^{i}(\tau_{i})_{i}^{i}(\tau_{\alpha x})] \cup i_{i}^{i}(\tau_{\alpha x}) ] (\tau_{i}^{i}(\tau_{\alpha x}))$$
(5-588)

$$[\kappa_{xx}] = \sum_{i=1}^{N} [\tau_i]_i [\Sigma_i [\Omega_{x_i}] [\Sigma_i]_i^{-1}]_1$$
 (5-589)

#### 5.2.2.3.2 Analysis of a Copper Shell

In order to demonstrate the method, we consider the following problem:

Determine the vibration modes of a "freely supported" shell with no internal pressure and no axial load. The geometrical parameters for the shell are

> L = length = 15.5 inches b = radius = 8 inches  $\tau$  = thickness = 0.0032 inches

The shell is made of homogeneous copper for which  $\rho = 0.322 \text{ lb}_{WP}/\text{in}^3$ .

$$V = 0.3$$
  
E = 13 × 10<sup>6</sup> 1b<sub>F</sub> / m<sup>2</sup>.

The interval  $(0,2\pi)$  of  $\theta$ , was divided into 4 equally spaced intervals and the interval (0,L) was divided in 5 equally spaced intervals. The developed shell surface was therefore divided into 20 regions of length,

and width,

$$\mu^{-} = \frac{2\pi 6}{4}$$
 (5-591)

There are then 24 collocation points and 72 degrees-of-freedom. To describe the freely supported condition, all components of displacement are set to zero for the points on the ends. These 12 conditions at each end reduce the system from 72 degrees-of-freedom to 48 degrees-of-freedom. These constraints can be described by a transformation of the form

$$\{b_{\theta}\} = [T_{\theta}]\{b\}$$
 (5-593)

The kinetic energy and strain energy for this problem are then

$$T = \frac{1}{2} $

where

$$\begin{bmatrix} A_{3} = [T_{\tau}] \left[ A_{nn} \right] [T_{\tau}] + [T_{\theta}] \left[ A_{3\theta} \right] [T_{\theta}] + [T_{x}] \left[ A_{xx} \right] [T_{x}]$$
 (5-597)

and

.

$$\begin{aligned} & \stackrel{*}{[K]} = [T_{\tau_{1}}]'[K_{\tau_{1}}][T_{\tau_{1}}] + [T_{\tau_{1}}]'[K_{\tau_{6}}][T_{6}] + [T_{\tau_{1}}]'[K_{\tau_{8}}][T_{8}] \\ & + [T_{\theta}]'[K_{\tau_{0}}]'[T_{1}] + [T_{\theta}]'[K_{\theta_{0}}][T_{\theta}] - [T_{\theta}]'[K_{\theta_{8}}][T_{8}] \\ & + [T_{4}]'[K_{\tau_{3}}]'[T_{1}] + [T_{8}]'[K_{\theta_{3}}][T_{\theta}] - [T_{8}]'[K_{\tau_{8}}][T_{8x}] \end{aligned}$$

$$(5-598)$$

The influence coefficients are then

$$[e] = [\kappa]^{"}$$
 (5-599)

The vibration modes and frequencies are obtained from

$$[ETAKq] = \lambda iq$$
 (5-600)

TABLE 12 FREQUENCIES FOR A THIN COPPER SHELL (c.p.s.)

1 2 3 4 55.89 70.69 92.00 92.00 115.59 115.59 129.02 129.02 138.29 139.56 141.44 141.97 142.13



484

5.2.2.3 <u>The Effects of Axial Load and Internal Pressure on the</u> Vibration of Beams as Derived from Thin Shell Theory

The significant thin shell effects of internal pressure and axial load can be retained in an approximation which leads to a beam theory model appropriate for slender cylindrical shells.



FIGURE 107 SLENDER CYLINDRICAL SHELL

For an axial compression load, P, and an internal pressure,  $p_{xx}$ , the membrane theory stress resultants are (from Equations 5-529, 5-530, and 5-531)

$$N_{rx} = t_x \frac{L}{2} - \frac{p}{4\pi r}$$
 (5-601)

$$N_{xe} = 0 \tag{5-602}$$

$$N_{00} = P_{x} \frac{T_{x}}{2}$$
 (5-603)

The expression for the strain energy derived in the previous section can be used to obtain an approximate expression for the strain energy involving integration with respect to x only and thus of the same form as the strain energy for a beam. Using Equations 5-601, 5-602, and 5-603 in Equation 5-532, we obtain the following expression for the strain energy.

$$U = \frac{1}{2} \int_{0}^{1} \int_{0}^{1} \frac{\varepsilon \tau}{1 - \gamma^{2}} \left( \left( \frac{\partial P_{X}}{\partial t} \right)^{2} + \left( \frac{1}{6} \frac{\partial P_{X}}{\partial \theta} + \frac{P_{X}}{\phi} \right)^{2} + 2\gamma \frac{\partial P_{X}}{\partial t} \right)^{2} \frac{i}{6} \frac{d \rho}{\partial \theta} + \frac{P_{X}}{\phi} \right)$$

$$+ \frac{1 - \gamma}{2} \left( \frac{1}{2} \frac{\partial P_{X}}{\partial x} + \frac{1}{26} \frac{\partial P_{X}}{\partial \theta} \right)^{2} \right) + \frac{\varepsilon \varepsilon^{3}}{12(1 - \gamma^{2})} \left( \left( \frac{i \nu P_{X}}{\partial \tau^{2}} \right)^{2} + \left( \frac{\partial P_{X}}{\partial \tau^{2}} - \frac{1}{6} \frac{\partial P_{X}}{\partial \theta} \right)^{2} \right)$$

$$+ 2\gamma \left( \frac{2^{2} P_{X}}{3x^{2}} \left( \frac{\partial P_{X}}{\partial t^{2}} - \frac{1}{6} \frac{\partial P_{X}}{\partial \theta} \right) + 2(1 - \gamma) \left( \frac{\partial P_{X}}{(3120)} - \frac{1}{6} \frac{\partial P_{X}}{\partial x} \right)^{2} \right)$$

$$+ \left( \left( P_{X} \frac{\Gamma_{X}}{2} - \frac{P_{X}}{200} \right) \left( \left( \frac{\partial P_{X}}{\partial \tau} \right)^{2} + \left( \frac{\partial P_{X}}{\partial \tau} \right)^{2} + \left( \frac{\partial P_{X}}{\partial \tau} \right)^{2} + \left( \frac{\partial P_{X}}{\partial \tau} \right)^{2} \right) \right)$$

$$+ \left( P_{X} \frac{\Gamma_{X}}{2} \left( \left( \frac{\partial P_{X}}{\partial \tau} \right)^{2} + \left( \frac{\partial P_{X}}{\partial \tau} \right)^{2} + \left( \frac{\partial P_{X}}{\partial \tau} \right)^{2} + \left( \frac{\partial P_{X}}{\partial \tau} \right)^{2} \right) \right)$$

$$+ \left( P_{X} \frac{\Gamma_{X}}{2} \left( \left( \frac{\partial P_{X}}{\partial \tau} \right)^{2} + \left( \frac{\partial P_{X}}{\partial \tau} \right)^{2} + \left( \frac{\partial P_{X}}{\partial \tau} \right)^{2} \right) \right)$$

If we now constrain the displacements so that sections normal to the x-axis remain plane during the deformation, the result should yield an Euler-Bernoulli beam theory model<sup>1</sup>. The shell displacements in this case are

$$p_x = x_x = p_x(x,t) - b = 0 = 0 = (x,t)$$
 (5-605)

$$[r_1'\theta, x, t] = \sup \phi_{\varepsilon}(x, t)$$
(5-606)

$$p_{\Theta}(\theta_1 x, t) = \cos \theta p_{\varepsilon}(x, t) \qquad (5-607)$$



FIGURE 108 BEAM DISPLACEMENTS FOR A CYLINDRICAL SHELL

<sup>&</sup>lt;sup>1</sup>This procedure is similar to that suggested by Enrico Volterra in his "Method of Internal Constraints" (see Volterra, E., <u>The Method of Internal Constraints</u> and Its Application to Static and Dynamic Problems Journal of the Engineering Mechanics Division, A.S.C.E., Aug. 1961, pp 103-127) Volterra's method shows promise for a more systematic and realistic reduction of a shell-like structure to a beam.

The derivatives in the strain energy are then

.

.

$$\frac{\partial \mathbf{p}_x}{\partial \mathbf{x}}(\mathbf{q},\mathbf{t}) = \frac{\partial \mathbf{p}_x}{\partial \mathbf{x}}(\mathbf{x},\mathbf{t}) - b\mathbf{q}\mathbf{u} \oplus \frac{\partial^2 \mathbf{p}_z}{\partial \mathbf{x}^2}(\mathbf{x},\mathbf{t})$$
(5-608)

$$\frac{\partial p_{\theta}}{\partial \theta}(\theta, x, t) + p_{\pi}(\theta, x, t) = -x_{\theta} \theta p_{\xi} + \sin \theta p_{\xi} = 0 \qquad (5-609)$$

$$\frac{\partial \phi}{\partial x} (\phi_{x,x}, \phi) = (1 + \frac{\partial x}{\partial x} (\phi_{x,x}, \phi)) = (1 + \frac{\partial \phi}{\partial x} - \frac{\partial \phi}{\partial x} (\phi_{x}, \phi)) = (1 + \frac{\partial \phi}{\partial x} (\phi_{x}, \phi))$$

$$\frac{\partial^2 p_z}{\partial \theta_z} (\theta, x, t) = -\sin \theta p_z = \sin \theta p_z = 0$$
 (5-611)

$$\frac{d^{2}E_{x}}{dx^{2}} = 0, x \in \frac{d^{2}E_{x}}{dx^{2}} (x, t)$$
(5-612)

$$\frac{\partial E_{2}}{\partial x_{2}} = (x,z) - \frac{\partial E_{2}}{\partial x} = (x,z) = (x,z) + \frac{\partial E_{2}}{\partial x} - x,z) = 0$$
 (5-613)

$$\sum_{i=1}^{2\pi} \varphi_i(\mathbf{x}, \varepsilon) = \sum_{i=1}^{2\pi} \varphi_i(\mathbf{x}, \varepsilon)$$
 (5-614)

$$\frac{d^2}{dt} = \frac{d^2}{dt} = \frac{d$$

$$\frac{\partial \mathcal{L}_{\mathbf{x}}}{\partial \mathbf{x}} \left( -\mathbf{x}, \mathbf{c} \right) = \frac{\partial \mathcal{L}_{\mathbf{x}}}{\partial \mathbf{x}} \left( \mathbf{x}, \mathbf{c}^{T} - \mathbf{A} \right) \left( \mathbf{x} + \mathbf{x} \right)$$
(5-616)

$$\frac{32}{2} = 3 \times 10^{-1} = 33 \times 10^{-1} = 33 \times 10^{-1} \times$$

Substituting these in the strain energy gives

$$U = \frac{1}{2} \left( \frac{1-\sqrt{3}}{\sqrt{3}} + \frac{1}{\sqrt{3}} \right)^{-\frac{1}{2}\sqrt{3}} - \frac{1}{2\sqrt{3}} \left( \frac{1}{2\sqrt{3}} + \frac{1}{\sqrt{3}} + \frac{1}{\sqrt{3}} + \frac{1}{\sqrt{3}} \right)^{\frac{1}{2}}$$

$$+ \frac{1}{2} \left( \frac{1}{\sqrt{3}} + $

.
We can now make an explicite integration with respect to  $\Theta$ , using:

$$\int_{0}^{2\pi} \sin \theta d\theta = 0 \qquad (5-621)$$

$$\int_{-5}^{27} \sin^2\theta \, d\theta = -$$
 (5-622)

$$\int_{c}^{t_{1}} \omega^{1} \phi \, d\phi = \tau \qquad (5-623)$$

The result is

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•

$$U = \frac{1}{2} \int_{0}^{L} \left( \frac{\varepsilon\tau}{1-\gamma^{2}} \left( \lambda \pi \frac{\partial bx}{\partial \lambda} \right)^{2} + \frac{\sqrt{2}\pi}{\partial \lambda} \frac{\partial^{2} by}{\partial \lambda^{2}} \right)$$

$$+ \frac{\varepsilon\tau^{3}}{12(1-\gamma^{2})} \left( \frac{\partial^{3} by}{\partial \lambda^{2}} \right)^{2}$$

$$+ i \frac{c}{2\pi} \int_{0}^{L} \frac{D}{\partial \tau^{2}} \left( 2\pi \pi \frac{\partial by}{\partial \lambda} \right)^{2} + b^{2} \pi \left( \frac{\partial^{2} b}{\partial \lambda^{2}} \right)^{2}$$

$$+ b^{2} \pi \left( \frac{\partial by}{\partial \lambda} \right)^{2} \int_{0}^{L} \frac{\partial b}{\partial \lambda} \right)^{2} \int_{0}^{L} dx$$

$$(5-624)$$

On collecting terms, we can write this as

$$U = \sum_{n=1}^{n} \left( E \left( \frac{2n}{2x} \right)^{2} + E \left( \frac{2n}{2x} \right)^{2} + N \left( \frac{2n}{2x} \right)^{2} \right) dx$$
 (5-625)

where the equivalent beam section properties are

$$\varepsilon I = \frac{\varepsilon t^{2} t^{3}}{(-r^{3})} + \frac{\varepsilon r^{3} \tau b}{z(r-r^{3})} + \varepsilon t^{2} t^{2} - \frac{F}{z_{3} b} t^{2} t^{3}$$
(5-626)

.

$$v = v_{2} + v_{3} - \rho + v_{1} - \frac{-24}{2} = p_{2} f^{2}(\pi + \frac{\pi}{2}) - \rho$$
 (5-628)

In order to include the important effects of shear energy, the Timoshenko-Beam displacements could be assumed instead of the Euler-Bernoulli assumptions made in Equations 5-605, 5-606, and 5-607.

### 6.0 CONCLUSIONS AND RECOMMENDATIONS

During the course of the investigations that are documented in this report, two distinct problems were explored which appear to be fruitful for additional development.

The first area for future work is the completion of a "production" digital program which will give complete dynamic simulation for a complex (clustered, for example) configuration in general six (rigid body) degree-of-freedom motion. This would necessitate incorporation of coordinate dependent forces into the existing six degree-of-freedom flexible body trajectory routine (see Appendix II). In addition, careful consideration should be given to selecting analytical schemes and governing differential equations for control systems of a general and variable nature. Using relations for detailed forces developed in section 4.2, subroutines should be coded which calculate generalized forces that appear in the equations of motion for the existing computer program. In summary, the result would be the development of a single computer routine to completely simulate the mission of a complex clustered booster including detailed dynamic simulation of a general linear or non-linear control system, as well as simulation of atmospheric turbulence, proces wind profiles, thrust, drag, gravity, and fuel slosh.

The second area for future work is the further investigation of the interpolation procedure (see section 2.3 and 5.2.2.2.3) for the dynamic analysis of shell-like structures with complex geometries. The method should be extended and dem. strated in the analysis of conical shells and shells with additional stiffening from rings and longerons. Detailed demonstrations of the method in the case of axial loads and pressurization should be performed to compare with available theory and test data. 7.0 LIST OF REFERENCES

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#### APPENDIX I

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#### A DIGITAL ROUTINE FOR SOLVING THE EQUATIONS OF MOTION OF A SINGLE ELASTIC BODY EXECUTING LARGE "RIGID-BODY" MOTIONS AND ACTED UPON BY FORCES WHICH ARE A FUNCTION OF TIME ALONE

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#### 1.0 INTRODUCTION

The equations which were coded are given in Section 4.0 equations 4-289 through 4-315.

For the purposes of numerical integration it is desirable to isolate the highest derivatives. In Equation 4-295 we have

Since  $[\Gamma]$  is singular (it only has rank N-6) it is desirable to define  $h_i$  so that instead of

$$\{t_{j}\} = \{\frac{1}{21}\},$$
 (I-2)

we have

$$\{k\} = \{\Gamma_1 \{\frac{2T}{2}\}$$
(1-3)

Then Equation I-1, above, is replaced by

.

$$\frac{1}{1} = -\frac{1}{1} = \frac{1}{1} = \frac{1$$

Equation 4-296 is replaced by

$$\{x_i\} = [n] [AR] \{i \in [GR]\}$$
 (1-5)

but

$$[\Gamma [[A]] \dot{\phi}_{F}] = (\Gamma_{1} - [A]] \langle \varphi_{R}] / [\varphi_{R}] / [A] [\varphi_{R}] / [A] [\dot{\phi}_{R}] / [$$

and from Equation 4-225

.

$$[\varphi_{\kappa}]' [A H \models] = \{o\}$$
 (I-7)

so that

Then we can write Equation I-5 as

and solve for the  $p_i$ 's

$$i\hat{p}\hat{s} = [A\hat{I}(\hat{s}h) - (\Gamma)(G\hat{s}h))$$
(1-10)

In Equations 4-301, 4-302, and 4-303 the  $\Omega$ 's were solved for, obtaining

$$\begin{aligned} \mathcal{I}_{x} &= \frac{1}{\lambda} (\lambda_{yy} \lambda_{zz} - \lambda_{yz}^{L}) (H_{x} - 2\xi \beta F[G_{yz} H \beta F]) \\ &+ \frac{1}{\lambda} (\lambda_{yz} \lambda_{xz} - \lambda_{xy} \lambda_{zz}) (H_{y} + 2\xi \beta F[G_{xz} H \beta F]) \\ &+ \frac{1}{\lambda} (\lambda_{xy} \lambda_{zz} - \lambda_{xy} \lambda_{zz}) (H_{z} - 2\xi \beta F[G_{xy}] f \beta F) \end{aligned}$$

$$(I-12)$$

$$= \frac{1}{\sqrt{2\pi}} \left\{ \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_$$

$$= \frac{1}{2} \left[ \frac{\lambda_{x_{1}}}{\lambda_{z_{2}}} - \frac{\lambda_{x_{2}}}{\lambda_{x_{2}}} \frac{1}{2} \left[ \frac{1}{2} + \frac{1}{2} \left[ \frac{1}{2} + $

.

where 
$$\lambda = \frac{1}{12} + $

.

# 2.0 EQUATIONS SOLVED BY THE ROUTINE

The input for the program is:

 $\begin{bmatrix} A_{xx} \\ A_{xx} \end{bmatrix}$ ,  $[A_{yy}]$ ,  $[A_{zz}]$ ,  $[A_{xy}]$ ,  $[A_{xz}]$ ,  $[A_{yz}]$   $\begin{bmatrix} K \\ B \end{bmatrix}$  $\begin{bmatrix} N_{x6} \\ [ \phi_{K} \end{bmatrix}$ 

and initial conditions:

.

Preliminary calculations, internal to the program, are:

$$[\Gamma] = \Gamma_{1,j} - [\Lambda][\varphi_R]([\varphi_R]'[\Lambda][\varphi_R])^{-1}[\varphi_R]'$$

$$(I-17)$$

$$[G_{xy}] = \frac{[A_{xy}] - [A_{yy}]'}{2}$$

$$[G_{xz}] = \frac{[A_{xz}] - [A_{xz}]'}{2}$$

$$[G_{yz}] = \frac{[A_{yz}] - [A_{yz}]'}{2}$$

$$[H_{xx}] = \frac{[A_{yy}] + [A_{zz}]}{2}$$

$$[H_{xy}] = \frac{[A_{xx}] + [A_{zz}]}{2}$$

$$[H_{zz}] = \frac{[A_{xx}] + [A_{yy}]}{2}$$

$$[H_{xy}] = \frac{[A_{xy}] + [A_{xy}]'}{2}$$

$$[H_{xz}] = \frac{[A_{xy}] + [A_{xy}]'}{2}$$

$$[H_{yz}] = \frac{[A_{yz}] + [A_{xy}]'}{2}$$

$$[H_{yz}] = \frac{[A_{yz}] + [A_{yz}]'}{2}$$

The equations to be integrated are

.

$$\frac{dV_x}{dt} = \int_{\mathcal{X}} Y_y - \int_{\mathcal{Y}} V_z + F_x / M$$
(I-20)
$$\frac{dV_y}{dt} = \int_{\mathcal{X}} V_z - \int_{\mathcal{Z}} V_x + F_y / M$$

$$\frac{dH_x}{dt} = \Omega_y Y_x - \Omega_x Y_y + F_z M$$

$$\frac{dH_x}{dt} = \Omega_z H_y - \Omega_y H_z + G_x$$

$$\frac{dH_y}{dt} = \Omega_x H_z - \Omega_z H_x + G_y$$

$$\frac{dH_z}{dt} = \Omega_y H_x - \Omega_x H_y + G_z$$

$$\{\dot{p}\} = [A]'(\{h\} - [\Gamma][G]\{\dot{p}\})$$

(1-21)

$$\{i_{k}\} = -\{r_{k}\}\{i_{k}\} + \{r_{k}\}\{i_{k}\} + \{r_{k}\}\{i_{k}\} + \{r_{k}\}\{i_{k}\} + \{r_{k}\}\{i_{k}\}\} + \{r_{k}\}\{i_{k}\}\{i_{k}\}\} + \{r_{k}\}\{i_{k}\}\{i_{k}\}\{i_{k}\}\} + \{r_{k}\}\{i_{k}\}\{i_{k}\}\{i_{k}\}\} + \{r_{k}\}\{i_{k}\}\{i_{k}\}\{i_{k}\}\} + \{r_{k}\}\{i_{k}\}\{i_{k}\}\} + \{r_{k}\}\{i_{k}\}\} + \{r_{k}\}\{i_{k}\}\{i_{k}\}\} + \{r_{k}\}\{i_{k}\}\} + \{r_{k}\}\{i_{k}\}\{i_{k}\}\} + \{r_{k}\}\{i_{k}\}$$

The following intermediate calculations are made at each integration step.

$$\begin{aligned} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_$$

·

$$\begin{split} \lambda &= \lambda_{xx} \lambda_{yy} \lambda_{zz} + z \lambda_{xy} \lambda_{xz} \lambda_{yz} - \lambda_{yz} \lambda_{yz}^{2} - \lambda_{yy} \lambda_{zz}^{1} - \lambda_{zz} \lambda_{zy}^{2} \end{split}$$
(I-24)  
$$\begin{aligned} \kappa_{\alpha} &= (\lambda_{yy} \lambda_{zz} - \lambda_{yz}^{2}) - \lambda \\ \kappa_{yj} &= (\lambda_{xx} \lambda_{yy} - \lambda_{xz}^{2}) - \lambda \\ \kappa_{zz} &= (\lambda_{xx} \lambda_{yy} - \lambda_{xz}^{2}) - \lambda \\ \kappa_{zz} &= (\lambda_{xy} \lambda_{yz} - \lambda_{xz} \lambda_{yy}) - \lambda \\ \kappa_{zz} &= (\lambda_{xy} \lambda_{yz} - \lambda_{xz} \lambda_{yy}) - \lambda \\ \kappa_{zz} &= (\lambda_{yz} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zy} \lambda_{zz} - \lambda_{zy} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zz} \lambda_{zz} - \lambda_{zz} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zz} \lambda_{zz} - \lambda_{zz} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zz} \lambda_{zz} - \lambda_{zz} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zz} \lambda_{zz} - \lambda_{zz} \lambda_{zz}) - \lambda \\ \kappa_{zz} &= (\lambda_{zz} \lambda_{zz} - \lambda_{zz} \lambda_{zz}) - \lambda \\ \kappa_{zz} &$$

.

$$I_{x} = r_{xx} (H_{x} - 2ipi[G_{yz}]!\dot{p}i) + k_{xy} (H_{y} + 2ipi[G_{xz}]!\dot{p}i) + k_{xz} (H_{z} - 2ipi[G_{xz}]!\dot{p}i)$$
 (I-26)

$$-2y = \kappa_{xy} + \kappa_{x} - \lambda \{p\}' [\Im_{yz}] \{\dot{p}\} \}$$

$$+ \kappa_{yy} (H_{y} + \lambda \{p\}' [\Im_{xz}] \{\dot{p}\} )$$

$$+ \kappa_{yz} (H_{z} - \lambda \{p\}' [\Im_{xy}] \{\dot{p}\} )$$

$$(1-27)$$

$$\begin{aligned} 
\mathbf{A}_{\mathbf{z}} &= \mathbf{K}_{\mathbf{z}} \left( \mathbf{H}_{\mathbf{v}} - \mathbf{z} \{ \mathbf{p} \} [ \mathbf{G}_{\mathbf{y} \mathbf{z}} \} \mathbf{x} \mathbf{p} \} \right) \\ &+ \mathbf{K}_{\mathbf{y} \mathbf{z}} \left( \mathbf{H}_{\mathbf{y}} - \mathbf{z} \{ \mathbf{p} \} [ \mathbf{G}_{\mathbf{x} \mathbf{z}} \} \mathbf{x} \mathbf{p} \} \right) \\ &+ \mathbf{K}_{\mathbf{z} \mathbf{z}} \left( \mathbf{H}_{\mathbf{z}} - \mathbf{z} \{ \mathbf{p} \} [ \mathbf{G}_{\mathbf{x} \mathbf{y}} \} \mathbf{x} \mathbf{p} \} \right) \end{aligned}$$

$$(\mathbf{I} - \mathbf{28})$$

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$$[G_{I}] = 2\left(-\Omega_{x}[G_{yz}] + \Omega_{y}[G_{xz}] - \Omega_{z}[G_{xy}]\right) \qquad (I-29)$$

$$[H] = 2 \left( \Omega_{x}^{2} [H_{xx}] + \Omega_{y}^{1} [H_{yy}] + \Omega_{z}^{1} [H_{zz}] - \Omega_{x} \Omega_{yz} [H_{yz}] \right)$$

$$(I-30)$$

$$\{\kappa\} = (\mathcal{A}_{y}^{2} + \mathcal{A}_{z}^{1}) [A_{xy}] \{\varphi_{x}\}_{6} + 2 \mathcal{A}_{x} \mathcal{A}_{y} [A_{yz}] \{\varphi_{x}\}_{5} + 2 \mathcal{D}_{x} \mathcal{D}_{z} [A_{xy}] \{\varphi_{x}\}_{4} + (\mathcal{D}_{x}^{2} + \mathcal{D}_{z}^{1}) [A_{yz}] \{\varphi_{x}\}_{4} + 2 \mathcal{D}_{y} \mathcal{D}_{z} [A_{xz}] \{\varphi_{x}\}_{6} + (\mathcal{D}_{x}^{2} + \mathcal{D}_{z}^{1}) [A_{xz}] \{\xi_{x}\}_{5}$$

$$(I-31)$$

$$\begin{bmatrix} F_{k} \\ F_{y} \\ F_{z} \\ G_{k} \\ G_{y} \\ G_{z} \end{bmatrix} = [\phi_{p}]^{\prime} \{P\}$$
(I-32)

The generalized forces,  $P_i$ , may be supplied as a table or generated by subroutines in the program.

The output is the time history of  $V_x$ ,  $V_y$ ,  $V_z$ ,  $\Omega_x$ ,  $\Omega_y$ ,  $\Omega_z$ and the generalized coordinates,  $p_1$ ,  $p_2$ .... $p_N$ .

#### 3.0 DESCRIPTION OF DIGITAL ROUTINE NO. LVV420 - FANDORA

The following is a description of the digital routine that has been coded to determine the motion and configuration for a general elastic body executing large "rigid-body" displacements. A description of the data, order in which data is presented to the routine, sample problems, listings of the coding, and core storage allocations are included.

The routine has been coded in 709/7090 FORTRAN II language and is compatible with the FORTRAN Monitor System for a machine with a minimum of 32,768 storage locations.

Integration of the differential equations in this program is accomplished by means of a four point Gill-Runge-Kutta numerical integration procedure (see Appendix VI) which is generally considered to have good convergence qualities. The equations of motion have been expressed in the Hamiltonian form which are first order and more amenable to solution by the Range-Kutta scheme. This has resulted in streamlining the numerical problem as compared with the alternative of the Lagrangian approach. Lagrange's equations, which are second order, must be artificially reduced to a set of first order equations before the Runge-Kutta method can be applied. The symmetry and simplicity of the Hamiltonian equations is lost by this circuitous approach.

In order to achieve the capability of handling up to 30 elastic degrees of freedom, the problem of data handling has been optimized. Most of the matrices in the program are either symmetric  $(a_{ij} = a_{ji})$  or antisymmetric  $(a_{ij} = -a_{ji})$ . Due to this fact, it was necessary to store only half of these matrices.

#### 3.1 The Order in Which Data is Presented to the Machine

The data is read into the machine in the following order:

- (1) One IBM card of control numbers for the routine.
- (2) Two IBM cards of descriptive information about the run.
- (3) As many cards as necessary to read in the single parameters for the run.
- (4) As many cards as necessary to read in the initial conditions of the integration variables for the run.
- (5) As many cards as necessary to read in the blocks of parameters for the run.

There are two features of this routine which greatly minimize the effect of reading in information for parameter studies, etc. First, the machine initially sets to zero all the parameters. This means that if any parameters of the system are zero, they need not be read in since they are already zero in the computer. Second, parameters retain their values throughout the time the routine is in the computer except as modified by new values read in. Since this routine allows parameters to be read in selectively, for successive runs only values that change for that run need be read in. To accomplish this selectivity in reading data into the machine, it is necessary to make a one to one correspondence between parameters of the system and a subscripted symbol "p". Below is a list which defines this correspondence with typical consistent units of data in parenthesis.

The following initial conditions are also supplied as parameters if unequal to zero or unchanged from the previous run.

P(o1) - F(90) - p(0) generalized coordinates (in.)

F(91) - F(120) - h(0) generalized momenta (lb.-sec.)

The remaining initial conditions are entered in the same manner as the above "P" initial conditions, but have a correspondence to the subscripted symbol "FIRSTF", thus;

- FIRSTY(1) t<sub>o</sub>, initial value of time (usually this is zero, hence it need not be read into the machine)(sec.)
- FIRSTY(2)  $V_x(0)$ , initial value of c.g. velocity in the x direction (in./sec.)
- FIRSTY(3)  $V_y(0)$ , initial value of c.g. velocity in the y direction (in. sec.)

FIRSTY(4) -  $V_z(0)$ , initial value of c.g. velocity in the z direction (in./sec.)

- FIRSTY(5)  $H_x(C)$ , initial value of angular momentum in the x direction (1t.-sec.-in.)
- FIRSTY(6)  $H_y(0)$ , initial value of angular momentum in the y direction (1b.-sec.-in.)
- FIRSTY(7)  $H_z(0)$ , initial value of angular momentum in the z direction (lb.-sec.-in.)

As far as the machine is concerned it reads in values of the subscripted variables. There are two methods by which the computer reads in these values. The first method allows for a completely random selection of variables to be read in, for instance, F(17), F(15), F(23), F(2). The second method allows for selected blocks of successive parameters to be read in, or for selected parameters separated by equal numbers of parameters to be read in. For instance, this method could be used to read in F(3), F(4), F(5), F(6), and F(11), F(12), F(13), F(14), F(15), or to read in (F(6), F(9), F(12), F(15), F(18). This method allows up to seven parameters to be read in on each IBM punched card, while the first method requires an IBM punched card for every parameter read in. A combination of the two methods may be used for reading data into the computer. The initial conditions which correspond to the subscripted symbol "FIRSTY" are read in exclusively by the first method.

## 3.2 Details of the Mechanics of Reading Data into the Computer

The first IBM punched card presented in the data contains the control numbers for the routine (see Figure 109). Seven of these are determined by the user, while the remaining must stay fixed as shown in Figure 109.

The control numbers are denoted and allocated columns on the first two cards as shown below.

IDENT - Card columns 1-5 NF - Card columns 6-10 NFIRST - Card columns 16-20 NFS - Card columns 36-40 NFORSF - Card columns 51-55 NFSKIF - Card columns 61-65

These numbers are never written with a decimal point and are always placed to the extreme right in their allotted number of columns.

IDEMI - This number is "1" for the first run, "2" for the second run, etc., or any identification number that is desired.

NP - This number controls the reading in of data. NP equals the number of single parameter cards to be read in by the method previously described.

NFIRST - This number controls the reading of initial conditions. NFIRST = 0 results in no new initial conditions being read in, while if it is desired to read in initial conditions, NFIRST equals the number of initial conditions to be read in.

NFS - NFS equals the number of generalized coordinates to be used in the calculation.

NFORSP - This integer selects the subroutine by which the generalized forces are to be calculated. Allowance has been made to include up to nine methods. At the present time, none have been coded, and the generalized forces are assumed to be zero or constant. This number must be supplied as an integer between "1" and "9".

NPT - This number controls the reading in of parameters by the block method. NPT equals the number of blocks of parameters to be read in by the method previously described.

MTSKIP - This number is used to indicate at what times it is desired to have results of the integration printed out. Noting that the computer prints out automatically at time = 0, MTSKIP is the number of integration steps minus "1" required to progress from the time of the last printout up to the time where the next printout is desired. For example, if the integration stepsize is 1 second and answers are desired at 0, 5, 10, 15 sec., etc., MTSKIP would equal 4. MTSKIP = 0 causes a printout for each integration step.

The second and third IBM punched cards allow descriptive material to be read into the machine which will later be printed out with the answers. Each card has columns 2 through 72 available for entering either alphabetic or numeric characters. It is necessary that cards 1, 2, and 3 always be supplied with each run. The cards following the first three depend upon the values of the control numbers, NF, NFIRS, and NFT, on the first card which is read in at the beginning of the run. To determine the cards that follow, first consider NF. If NP = 0, no parameters are to be read in. In NP = `m`, there will be "m" separate parameter cards read in. Each parameter will have its own card. The form of the card will be as follows:

Card columns 1 through 5 will contain the number N (with no decimal point and placed to the extreme right of the field).

Card columns 6 through 20 (preferably 11 through 20) will contain the desired value of the Nth parameter P(N).

Card columns 21 through 80 are ignored by the computer. For convenience, descriptive material may be entered here for future reference.

There will be 'm' of these parameter cards.

To determine the next cards to be read in, consider NFIRST. If NFIRST = L, there will be supplied "L" cards of the same format as the above NF parameter cards.

The last cards to be read in are determined by NPT. If NPT = 0, the input data is complete and no additional cards need be supplied. If NFT -"n", then "n" blocks of parameters will be read in. The first card in the block will contain three integer control numbers (written without decimal points): NP1 - card columns 1 through 5, NP2 - card columns 6-10, and NP3 card columns 11 through 15. These control numbers tell the computer that the next card(s) will contain parameters P(NP1) through F(NP2) and that they should be read in in steps of NP3. The card(s) that follow will then contain the desired parameters. Each card must contain 7 parameters except the last card of the block which may contain from 1 to 7 parameters. The first 70 card columns are available for supplying parameters. Each parameter is allotted 10 card columns, hence, the first parameter entered on a coard uses columns 1 through 10, the next, 11 through 20, etc. The numbers must have a decimal point. A parameter may be written down in either of two forms, decimal or exponential, ie., the number 3562.2 may be entered on the IBM card either as card col.

3 3 h 5 6 7 9 10 1 2 3 5 2 6 2 or 5 2 6 2 3 3

In the last case, the "+" (or "-" in the case of a number less than one), is mandatory, and the exponent must occupy the far right hand column. To enter a block of parameters, suppose NP1 = 8, NF2 = 16, and NF3 = 1. The cards neccessary to supply these parameters are

21 - 30 31 - 40 h1 - 5051 - 60 61 - 70 card col. 1 - 10 11 - 20 Card 1, å 16 1 P(8) P(9) P(10) P(11) P(12) F(13) P(北) Card 2, Card 3, P(15) P(16)

Had NP3 equaled 2, the second card would be as shown below, and there would be no third card.

 $C_{ard} 2$ , P(3) P(10) P(12) P(1L) P(16)

There will be "n" cards of control numbers NP1, NF2, NF3, each followed by the card(s) of parameters dictated by the control numbers.

The blocks of parameters are the last cards to be read into the computer.

#### 3.2.1 Mechanics of Entering Arrays

All matrices which are input data have a correspondence to the subscripted symbol "P". These arrays are arranged in storage in the manner dictated by a Fortran generated program<sup>1</sup>. It is suggested that arrays be entered by the block parameter method for efficiency.

#### 3.2.2 General Form of Data Sheets for Routine Pandora

Figure 109 depicts the general order and form in which data is presented to routine Pandora. It also indicates that the first 3 cards must always be presented for a run. Note that the first card (the integer-control card) has both integers and symbols denoted on it. The symbols denote those fields of the control card into which numbers can be entered at will by the user of the routine. Those fields which contain given numbers must retain these indicated values run after run.

<sup>&</sup>lt;sup>1</sup> Reference Manual 709/7090 FORTRAN Programming System, C28-6054-2, IEM Corporation, 1961, p. 8 and p. 56.

RM FOR PRESENTING DATA TO ROUTINE	
FOR	
FIGURE 109	

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#### 3.2.3 Core Storage Allocation

The following is a complete description of the core storage allocation for Routine PANDORA. This is included for use in possible program modification and for debugging. These variables which are input data are denoted by an asterisk.

3.2.3.1	Core Storage	Allocations	fo	or Subscripte	d Variabl	es
		*P(61)	- 1	PP0	P(12211)	- AZXF5
		*P(91)	- 1	HHO	P(12221)	- AZXF6
		*P(121)	- I	PF	P(12271)	- AYXFL
		*P(151)	- I	FER	P(12301)	<b>-</b> AYXF6
		*P(331)	- 2	AXX	P(12331)	- DK
*Denote	es input data	*P(1231)	- 2	AYY	2(12361)	- HGAMGP
		*P(2131)	- 2	AZI	P(12956)	- GZY
		*P(3031)	- 1	лұх	P(13321)	- GZX
		*P(3731)	- 2	AZ.C	P(13795)	– GYX
		*P(1831)	- 2	1CT	P(11251)	- HZY
		*P(5751,			P(14716)	- HZX
		*? <b>(</b> 5531)	. – E	3	P(15191)	- HYX
		P(7531)		1	P(15565)	- HX.(
		P(9L31)	- 4	AINV	r <b>(1</b> 5111)	- HYY
		P(9331)	- 6	MAB	P(16576)	- HZZ
		P(10231)	- 0	hanh	P(17401)	- G
		P(11131)	- 1	νB	P(175%)	- H
		P(12031)	- E	22 2	P(1?971)	- F
		P(12061)	- H	Ħ	P(17977)	- FAF
		P(12091)	- 5	22	P(17993)	- FAFI
		P(12121)	- F	Ð	P(17933)	- 0025
		P <b>(12151)</b>	- 2	koyfl	b(50071)	- GAM
		P(12131)	- 4	LEYF5	P(20921)	- FYESS

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# 3.2.3.2 Equivalent "P" Allocations for Data Entered in Array Form

First element of single subscripted variables:

- PPO P(61)
- HHO P(91)
- PF P(121)

First element of each column of double subscripted variables:

Column	FER	XXA	AYY	AZZ	ΑΥΧ	AZX	AZY	SK	В
Column 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20	FER 151 181 211 241 271 301	AXX 331 361 391 421 451 451 511 511 571 601 631 631 631 721 751 781 811 811 901	AYY 1231 1261 1291 1321 1351 1351 1381 1441 1471 1501 1531 1561 1591 1621 1651 1651 1651 1651 1711 1741 1771 1501	AZZ 2131 2161 2191 2221 2251 2311 2311 2311 2431 2451 2451 2451 2451 2451 2451 2451 2451 2451 2451 2451 2451 2451 2451 2451 2551 2551 2551 2551 2551 2551 2551 2651	AYX 3031 3061 3091 3121 3151 3211 3211 3211 3211 3211 321	AZX 3931 3961 3991 4021 4051 4051 4051 4111 4141 4141 4201 4231 4351	AZY 4831 4861 4891 4921 4951 4951 4951 5011 5001 5101 5101 5101 5101 5121 5221 52	SK 5731 5761 5791 5851 5851 5851 5911 5911 5911 5911 6001 6031 6091 6121 6151 6181 6211 6211 6211 6201	B 6631 6661 6721 6751 6751 6751 6751 6811 6811 6811 6901 6931 6901 6931 6961 6971 7051 7051 7051 7051 7111 7111 7171 7201
22		931 961	1831 1961	2731 2761	3631 3661	4531 4561	5431 5761	6331 6361	7231
23		991	1871	2791	3691	4591	5491	6391	7291
2h 25		1021 1051	1921 1951	2321 2351	3721 3751	4621 1651	5521	6421	7351
26		1081	1781	2881	3731	4681	5581	6).81	7381
27 28		1111	5071 5071	2911 2911	3841	4711 4711	5611	6541	7411 7441
29		1171	2071	2971	3871	1771	5571	6571	7571
30		1201	5101	300T	⊥∪رز	4001	5701	57 JT	1201

ROUT	INE	PANDORA

.

Integer 1 to 100

.

\*Denotes input data

* 1_IDENT	34 NEOL(2)	67
* 2 NP	35 NEOL(3)	68
* <u>S NINT = 0</u>	36 NEOL(L)	59
* • NFIRST	37 NEOL(5)	70
* S NTABLE • O	38 NEOL(6)	71
* <b>C = X = O</b>	39 NCUTP	72
* 7 NMORE = 0	40 NLO	73
* B. NPS	41 LPRINT	74
- 9 NPASS = 0	12 MLINE	75
- 10 NFCR3P	43 NSAIP	76
11 NPT	44 NPAGE	77
- 12 KTCh = G	45 NETLE	78
- 13 NTSKIP	46 NPSL	79
14	47	80
16 NIND	48	81
13 MINDI	49	82
17_NIND2	50	83
בדבוי 18	51	84
ורדיין 13	52	25
20 NOTO	53	36
21	54	97
22	56	88
23	56	39
24	57	90
25	68	91
28	59	92
27	60	93
23	61	94
29	62	95
30	63	36
31	54	97
32	65	98
35 NEOL(1)	66	99
		00

ROUTINE	PANDORA
-	

Parameter 1 to 100

\*Denotes input data

1			
*	1 DELTAT	34 CK	67
*	2 TIMAX	* 35 D	68
	3	36 HMPGPX	69
1	4	37 HARGPY	70
ĺ	5	38 HMPGPZ	71
	6	39	72
ļ	7	40	73
	8	41	74
	9 -	42	75
	10	43	76
	11 <u>Din</u>	44	77
	12 0122	45	78
	13 DL33	46	79
	14 01.12	4.7	80
	15 DL13	3.P	81
	16 DL23	40	82
	17 DETERM	50	83
	18 2111	51	F4
	19 DLI22	52	85
	20 DII33	53	86
(	21 געוור 21	54	87
	22 MII3	55	99
	23 DLI23	56	63
	24 CMI	57	90
	25 CMI	50	<u>01</u>
1	<u>26 0XC</u>	59	92
	27 CMA	45	93
1	28 CMT	<i>F</i> 1	94
1	29 0733	62	95
ļ	20 0222	63	96
L	31 CMC	61	97
	32 CMT3	65	or
L	33 EVES	66	29
L			00

#### ROUTINE PANDORA

# Integration Variables

#### \*Denotes input data

T's	DYDI 'e	FIRSTY's
1 1	1 1.0	1 I(0)
2 VI	2 710	2 710
s VI	S TD	5 TYO
4 VZ	4 7ZD	<b>4 V</b> ZO
5 HI	5 HID	* 5 HIO
e HI	6 EYD	* 6 HTO
9 HI	7 HZD	* 7 HZO
8 PP(1)	8 PD(1)	5 PPO(1)
9 ===(1)	9 HD(1)	9 ====0(1)
10 PP(2)	10 PD(2)	10 PPO(2)
11 ==(2)	11 ED(2)	11 HEO(2)
12	12 P3(3)	12 ppc(3)
13 邑())	15 (3)	13 HEO(3)
14 PP(L)	14 PD(4)	14 250(2)
15 ==(!.)	15 三〇	15 EHD(L)
15 = (5)	13 PD(5)	16 PP0(5)
17 ==(5)	17 ED(5)	17 臣3(5)
15 22(5)	18 PD(5)	18 220(6)
13 HH(6)	13 HD(6)	19 HEQ(6)
20 22(7)	20 20(?)	20 200(7)
21 ==(7)	21 ===(7)	21 EE3(7)
22 99(8)	22 F2(9)	22 EPO(5)
23 82(5)	23 ED(B)	23 EE0(8)
24 77(?)	24 93(9)	24 290(9)
25 ==(?)	25 == (9)	25 =====(?)
26 PP(10)	26 PT(10)	26 PP0(10)
27 ==(12)	27 57(10)	27 450(20)
23 pp(11)	25 571(71)	23_FPG(11)
29 EI(11)	29_HD(11)	29_323(11)
30 PP(12)	30 FD(12)	30 PP0(12)
<u>31 EE(12)</u>	31 E(12)	31 5=2(12)
32 rp(23)	32 20(13)	32 220(23)
33 EE(13)	<u>55</u> E(13)	33 EHO(13)
34 (1)	34 PB(14)	PP0(11)

# ROUTINE PANDORA

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# Integration Variables

		FIRSTY .
<u>Y</u> '•		35 HHQ(14)
35 HH(止)	35 HD(14)	36 P.0(15)
36 PP(15)	36 PD(15)	37 HHO(15)
37 HH(15)	37 HD(15)	38 PP0(15)
38 P2(16)	38 PD(16)	<b>19</b> HHO(16)
39 (15)	39 FD(16)	40 220(17)
40 pz(17)	40 PD(17)	
41 HH(17)	41 HD(17)	42 220(13)
(2) P2(18)	42 PJ(13)	47 HEO(18)
47 HH(18)	43 HE(13)	
<b>4</b> 3 (19)	44 ?)(12)	44 570(12)
<b>1</b> (.)(-//	45 HD(19)	<u>40</u> nno(2/
45 0(2)	46 20(20)	<b>40</b> <u>220(20)</u>
	47 HD(20)	47 (10(2))
<b>4</b> 7 (21)	48 PD(21)	48 pp(1-1)
48 (21)	49 HD(21)	
49 :::(22)	50 FT (2?)	<u>50</u> (22)
50 PP(22)	51 HD(22)	51 .HO(22)
51 E.a(22)	52(23)	52 =5 (13)
<u>62 PP(23)</u>	53 :: (23)	53 = 3(23)
53 HH(23)	54 20(25)	54 PPC(2)
54 PF(2L)	55 HD (24)	55 HH (12)
55 ==== (201	56 20(25)	56 220(23)
58 24(25)	57 HT (25)	57 99-102
<b>57</b> HH(25)	58 == (?=)	58 - 2-(
58 = 2(26)	59 HD(25)	59 HEC(25)
59 HH(26)	60 FJ(27)	60 FPU(27)
60 PP(27)	81 HD(27)	61 (H1(27)
61 HH(27)	62 ± (29)	<u>62 F2C(23)</u>
62 PP(CT)	63 EJ(23)	63 HHC(29)
63 HH(25)	(cc)ra 4	64 FP. (23)
64 FF(29)	65 H2(2?)	65 王(27)
65 HH(27)	((1))	66 FF0(30)
66 FF(30)	67 HD(32)	67 HD(12)
67 HE (32)		68
68		

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## 3.2.4 Source Program Listings

The following is a complete set of listings of the main program and subroutines. This can be useful in debugging and in program modification. It should be noted that two subroutines are included which have not previously been discussed. These are subroutine TAPEIN and subroutine FORCE1.

Subroutine TAPEIN was coded to allow the user to enter large matrices by means of a special tape input. These matrices were previously computed and written on the tape to be used as input to PANDORA. To use the tape input method at various computing facilities, it will probably be necessary to recode the subroutine to allow for inconsistencies between systems.

Subroutine FORCEL was coded to calculate the generalized forces due to a gust. It is of a very specialized nature. For this reason, the option NFORSP = 1 will not be used in normal operation of routine PANDORA.

# TABLE 13 FORTRAN SOURCE PROGRAM LISTINGS OF ROUTINE "PANDORA"

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+	LIST		•
•	SYMBOL	TABLE	
CHAIN	-R		
			CONMON VAR
			DIMENSION VAR(24000)+DYDX(75)
			EQUIVALENCE (VAR(76), DYDX(1))
10			00201 = 1,24000
20		•	VAR(I) = 0.0
30			DYDX(1) = 1.0
40			CALL RK
			END
0011			

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LIST SYMBOL TABLE CINPUT SUBROUTINE INPUT COMMON VAR DIMENSION VAR(24000), P(23400), Y(75), NTEGER(225), 1 Q(75) + FIRSTY(75) + NT(30) + NT1(30) + NT2(30) EQUIVALENCE (VAR(1),Y(1)), (VAR(376), NTEGER(1)), (VAR(601),P(1)),(VAR(151),Q(1)),(VAR(226),FIRSTY(1)), 1 (NTEGER(101),NT(1)),(NTEGER(131),NT1(1)),(NTEGER(161),NT2(1)), 2 (NTEGER(2),NP), (NTEGER(3),NINT), (NTEGER(4),NFIRST), ٦ (NTEGER (5) + NTABLE) + (NTEGER (7) + NMORE) + (NTEGER (11) + NPT) + 4 5 (NTEGER(12), NTCR), (NTEGER(13), NTSKIP), (NTEGER(44), NPAGE), (NTEGER(6),N) EQUIVALENCE (NTEGER(15), NIND), (NTEGER(16), NIND1), 1 (NTEGER(17),NIND2) ¢ SET THE PAGE NUMBER FOR THE FIRST PAGE. 10 NPAGE = 0 READ CONTROL NUMBERS INTO THE PROBLEM. c 20 READ INPUT TAPE 5,30, (NTEGER(1), 1=1,14) 30 FORMAT (1415) IF (NMORE) 60,60,50 READ INPUT TAPE 5,30, ("TEGER(I), I=15,NMORE) 40 50 с PLACE HEADING AT TOP OF WRITE OUT. CALL PAGEHD 60 с READ AND WRITE TWO CARDS OF ARBITRARY RUN INFORMATION. 70 READ INPUT TAPE 5,80 80 FORMAT (72H 1 /72H 2 90 WRITE OUTPUT TAPE 6,80 CHECK FOR FLOATING POINT PARAMETER ENTRY AND THEN MAKE ENTRY c с ACCORDING TO SINGLE PARAMETER READ IN METHOD (TYPE A ENTRY) 100 IF (NP) 150,150,110 110 DO 140 J = 1 .NP READ INPUT TAPE 5,130,1,(P(1)) 120 130 FORMAT (15,E15.7) CONTINUE 140 c CHECK FOR FIXED POINT NUMBER ENTRY AND THEN MAKE ENTRY с ACCORDING TO SINGLE INTEGER READ IN METHOD (TYPE B ENTRY) 150 IF (NINT) 200,200,160 DO 190 K = 1,NINT READ INPUT TAPE 5,180,K,(NTEGER(I)) 160 170 FORMAT (15,115) 180 190 CONTINUE c CHECK FOR NEW INITIAL CONDITIONS TO BE READ INTO THE PROBLEM ¢ AND READ IN ACCORDING TO PRESCRIBED FORWAT (TYPE C ENTRY) 200 IF (NFIRST) 242,242,210 210 DO 240 L = 1,NFIRST READ INPUT TAPE 5,230,1,(FIRSTY(1)) 220 FORMAT (15,E15.7) 230 240 CONTINUE 242 IF (NPT) 250,250,244 244 DO 248 I=1,NPT READ INPUT TAPE 5,30, NP1, NP2, NP3 READ INPUT TAPE 5,360, (P(J), J≈NP1, NP2, NP3) 246 248 c c CHECK IF TABLE ENTRIES ARE TO BE MADE AND READ IN TABLE ENTRIES ACCORDING TO PRESCRIEED INPUTS (TYPE D ENTRY) 250 IF (NTABLE) 380,380,260 NT1(1) = NTCR DO 370 M = 1,NTABLE 260 270 28. IF (NT(M)) 290,370,310 290 NT1(M+1) = NT1(M)

300		GO TO 310
310		NT1(M+1) = NT1(M) + NT(M)
320		NT2(M) = NT1(M) - 1 + NT(M)
330		NT11 = NT1(M)
340		NT12 = NT2(M)
350		READ INPUT TAPE 5,360, (P(N), N=NT11,NT12)
360		FORMAT (7E10.7)
370		CONTINUE
c		CALL IN INPUT DATA WRITING ROUTINE
380		CALL INAID
c		ZERO THE Q AND SET THE Y TO INITIAL VALUES
390		DO 420 N1 = 1 + N
400		Q(N1) = 0.0
410		Y(N1) = FIRSTY(N1)
420		CONTINUE
430		IF(NIND) 450,450,440
440		CALL PDUMP (VAR, VAR(150), 1, VAR(226), VAR(300), 1,
	1	NTEGER, NTEGER(46), 2, P, P(7530), 1)
430		NTEGER(42) = 0
460		RETURN
		END
0084		

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LÍST
SYMBOL TABLE
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CTAPEIN
                                                    SUBROUTINE TAPEIN
           SUBROUTINE TAPEIN

COMMON VAR

DIMENSION VAR(24000),P(23400),NTEGER(225)

EQUIVALENCE (VAR(601),P(1)),(P(34),CK),

1 (VAR(376),NTEGER(1)),(NTEGER(45),NFILE)

NFILE = NFILE

IF (NFILE) 40,30,40

CALL FWOFS (9,1)

CALL RDTB (9,CK;F(6631),900,P(5731),900,P(4831),900,

1 P(3931),900,P(3031),900,P(2131);900,P(1231),900,P(331),900,

2 P(131),180)
     10
     20
     30
     40
             2 P(151)+180)
     50
                                                    NFILE # 1
                                                   RETURN
     60
                                                                                                                                                                                .
0017
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٠ LIST × SYMBOL TABLE CINAIO SUBROUTINE INAID COMMON VAR DIMENSION VAR(24000), P(23400), NTEGER(225), AYX(30,30), AZX(30,30), AZY(30,30), PP(30), HH(30), AZYF4(30), AZYF5(30), 1 AZXF5(30), AZXF6(30), AYXF4(30), AYXF6(30), GZY (465) 2 GZX(465),GYX(465),HZY(465),HZX(465),HYX(465),HXX(465),HYY(465), 3 HZZ(465), FAF(6), GAM(30, 30), FMESS(30, 30) 4 EQUIVALENCE (P(2004),GAN(1)),(P(20941),FMESS(1)) EQUIVALENCE (VAR(376),NTEGER(1)),(VAR(601),P(1)), 1 (P(3031),AYX(1)),(P(3931),AZX(1)),(P(4831),AZY(1)), (P(12031), PP(1)), (P(12061), HH(1)), (P(12151), AZYF4(1)), 2 (P(12181),AZYF5(T)),(P(12211),AZXF5(1)),(P(12241),AZXF6(1)), 3 (P(12271), AYXF4(1)), (P(12301), AYXF6(1)), (P(35), D), 4 5 (P(12856),GZY(1)),(P(13321),GZX(1)),(P(13786),GYX(1)), (P(14251),HZY(1)),(P(14716),HZX(1)),(P(15181),HYX(1)), 6 (P(15546),HXX(1)),(P(15111),HYY(1)),(P(16576),HZZ(1)), 8 (P(17977), FAF(1)), (P(33), FNPS), (NTEGER(8), NPS), (NTEGER(46), NPSL) DIWENSION FIRSTY(75), PPO(30), HH0(30), FER(30,6), 1 AXX (30, 30; , AYY (30, 33) , AZZ (30, 30) , AINV (30, 30) , VB (30, 30) , A(30, 30) r 2 FAFI(6),E(30),X(30,30),FER1(30),FER2(30),FER3(30),FER4(30), 3 FER5(30) + FER6(30) + NEOL(6) EQUIVALENCE (VAR(226), FIRSTY(1)), (P(61), PPO(1)), (P(91), HHO(1)), (P(151), FER(1)), (P(331), AXX(1)), (P(1231), AYY(1)), Ľ (P(2131), AZZ(1)), (P(7531), A(1)), (P(8431), AI"V(1)), (AINV, X), 2 Ī (R(11131), #B(1)), (P(17983), FAF1(1)), (FER(1), FER1(1)), (FER(31), FER2(1)), (FER(61), FEP3(1)), (FER(91), FER4(1)), 4 (FER(121), FER5(1)), (FER(151), FEP6(1)), ("TEGER(9), NPASS), 5 (NTEGER(33), NEOL(1)), ("TEGER(39), NOUTP), (NTEGER(6), N) 6 DETERMINE IF PASTRAN TAPE IS TO BE READ с 10 IF (NPASS) 30,30,20 20 CALL TAPEIN с CALCULATE MASS AND MOMENTS OF INERTIA 30 00 50 I=1,NPS DO 50 J=1,NPS 40  $\dot{A}(I,J) = AXX(I,J) + AYY(I,J) + AZZ(I,J)$ 50 60 DO 110 I=1+6 70 FAF(I) = 0.080 00 100 J=1,NPS 90 00 100 K=1,NPS FAF(I) = FAF(I) + FER(J,I)\*A(J,K)\*FER(K,I)100 110 FAFI(I) = 1.0/FAF(I)120 DO 160 I=1,NPS 130 DO 160 J=1,NPS 140 F¥ESS(1+J) = 0+0 150 00 160 X=1,6  $F^{SS}(I,J) = F^{SS}(I,J) + FER(I,K) * FAFI(K) * FER(J,K)$ 160 CALCULATE GAMMA MATRIX c 179 00 200 I=1, SPS 180 00 200 J =1:NPS 190 IF (I-J) 192,196,192 192 GAM(I)J = 0.0194 GO TO 198 GA4(I) = 1.0 196 198 DO 200 X=1,NPS 200 GAM(I)J) = GAM(I)J) - A(I,K)\*FMESS(K,J)c SET UP LOOP FOR CALCULATING TRIANGULAR MATRICES 201 IF (NTEGER(14)) 202,202,430 00 420 I=1,NPS 202 204 DO 290 J=1+NPS 206 L = (J\*(J-1))/2+I

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c	CALCULATE G AND H TRIANGULAR MATRIX
210	GZY(L) = (AZY(I,J)-AZY(J,E))*.5
220	$GZX(L) = \{AZX\{I,J\}-AZX\{J,I\}\} + s$
230	GYX(L) = (AYX(I,J) - AYX(J,I)) + 5
240	HZY(L) = (AZY(L)J) + AZY(J)LIJ = 0.5
250	HZX(L) = (AZX(I,J)+AZX(J,I)+0.5
260	HYX(L) = (AYX(I,J)+AYX(J,I))*0.5
270	HXX(L) = (XYY(I,J)+AZZ(I,J))*0.5
280	HYY(L) = (AXX(I,J)+AZZ(I,J))+0.5
290	HZZ(L) = (AXX(I,J)+AYY(I,J))+0.5
c	CALCULATE APHI MATRICES
300	AZYF4(I) = 0.0
310	AZYF5(1) = 0.0
320	AZXF5(I) = 0.0
330	AZXF6(I) = 0.0
340	AYXF4(I) = 0.0
350	AYXF6(I) = 0.0
360	DO 420 JJ=1,NPS
370	AZYF4[E] = AZYF4[E] + AZY[J]+ER4[J]
380	AZYF5(I) = AZYF5(I) + AZY(JJ,I)*FER5(JJ)
390	AZXF5(I) = AZXF5(I) + AZX(I,JJ)*FER5(JJ)
400	AZXF6(I) = AZXF6(I) + AZX(I,JJ)*FER6(JJ)
<b>410</b>	AYXF4(I) = AYXF4(I) + AYX(JJ,I)*FER4(JJ)
420	AYXF6(I) = AYXF6(I) + AYX(JJ,I)*FER6(JJ)
c	INVERT MASS MATRIX
430	00 470 I=1,NPS
<b>4</b> 40	DO 450 J=L,NPS
450	WB(I,J) = 0.0
460	AINV(I,J) = A(I,J)
470	$WB(I \cdot I) = I \cdot 0$
<b>490</b>	M = XSIMEOF(30,NPS,MPS,AINV,WS,D,E)
500	GO TO (570,510,540),M
510	WRITE OUTPUT TAPE 6,520
520	FORMAT (46H UNDER/OVERFLOW IN MASS MATRIX INVERSION)
530	CALL RK
540	WRITE OUTPUT TAPE 6,550
550	FORMAT (30H MASS MATRIX IS SINGULAR)
560	CALL RK
570	$N = 7 + 2 \neq NPS$
580	DO 640 I=1.NP5
270	
~~~~	$X = 1 + 2\pi Z$
LA10	SET P AND R EQUAL TO INTITAL CUMULTIONS
610	PP(1) = PPO(1)
~~~~	$RR\{1\} = RHU\{1\}$
<b>`</b> 430	SET P AND A EUGAL TO PROPER INTEGRATION VARIABLES
830	PIRSIT(J) = PP(1)
0.40	FINSIT(K) = HH(L)
67V	$\frac{1}{2} \frac{1}{2} \frac{1}$
470	rard = rloair (ard)
610	NJUIN = (MES-11/S+1)
400	UU DYU ITLINUUHY
970	ACULII = 0 NEOL (NONTO) - NOS STUDITO : S
710	AEAFLANAINI = UNO - DINUNIN + 9
* 7.0	AC IVAA ENA
0110	
wii7	

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LIST SYMBOL TABLE COYDXS SUBROUTINE DYDXS COMMON VAR DIMENSION VAR(24000), P(23400), NTEGER(225), AYX(30,30), AZX(30,301,AZY(30,30),PP(30),HH(30),AZYF4(30),AZYF5(30), ł AZXF5(30),AZXF6(30),AYXF4(30),AYXF6(30), GZY(465). 2 GZX(465),GYX(465),HZY(465),HZX(465),HYX(465),HXX(465),HYY(465), 3 á. HZZ(465), FAF(6), GAM(30, 30), FMESS(30, 30) EQUIVALENCE (P(20041), GAM(1)), (P(20941), FHESS(1)) EQUIVALENCE (VAR(376), NTEGER(1)), (VAR(601),P(1)), (P(3031),AYX(1)),(P(3931),AZX(1)),(P(4831),AZY(1)), 1 (P(12031), PP(1)), (P(12061), HH(1)), (P(12151), AZYF4(1)), 2 3 (P(12181),AZYF5(1)),(P(12211),AZXF5(1)),(P(12241),AZXF6(1)), (P(12271),AYXF4(1)),(P(12301),AYXF6(1)), 5 (P(12856),GZY(1)),(P(13321),GZX(1)),(P(13786),GYX(1)), (P(14251),HZY(1)),(P(14716),HZX(1)),(P(15181),HYX(1)), 6 7 (P(15646),HXX(1)),(P(16111),HYY(1)),(P(16576),HZZ(1)), (P(17977), FAF(1)), (P(33), FNPS), (NTEGER(8), NPS), (NTEGER(46), NPSL) 8 DIMENSION Y(75), DYDX(75), PF(30), SK(30,30), B(30,30), GAMG(30,30),GAMH(30,30),PD(30),HD(30),DK(30),HGAMGP(30),G(465), Ŀ Ż H(465),F(6) ,AINV(30,30),FER(30,6) EQUIVALENCE (VAR(1),Y(1)), (VAR(76), DYDX(1)), (P(121), PF(1)), (P(5731), SK(1)), (P(6631), B(1)), (P(9331), GAMG(1)), L (P(10231),GANH(1)),(P(12091),PD(1)),(P(12121),HD(1)), 2 (P(12331), DK(1)), (P(12361), HGAMGP(1)), (P(17041), G(1)), 3 (P(17506),H(1)),(P(17971),F(1)),(FAF(3),CM),(FAF(4),CIXX), 4 (FAF(5), CIYY), (FAF(6), CIZZ), (P(11), DL11), (P(12), DL22), 5 6 (P(13), 0L33), (P(14), 0L12), (P(15), 0L13), (P(16), 0L23), (P(17), DETERM), (P(18), OLI11), (P(19), DL122), (P(20), DL133), 7 (P(21), DL1121, (P(22), DL113), (P(23), DL123), 8 Ģ (P(24), OMX), (P(25), OMY), (P(26), OMZ), (P(27), OMXX), (P(28), OMYY) EQUIVALENCE (P(29), OMZZ1, (P(30), OMXY), (P(31), OMXZ), ł (P(32),0MYZ1,(Y(2),VX),(Y(31,VY),(Y(4),VZ),(Y(5),HX),(Y(6),HY), (Y(7),HZ), (DYDX(2),VXD), (DYDX(3),VYD), (DYDX(4),VZD), 2 3 (DYDX(5),HX)),(DYDX(6),HYD),(DYDX(7),HZD),(P(8431),AINV(1)), (P(151), FER(1)) 4 EQUIVALENCE (P(36), HMPGPXI, (P(37), HMPGPY), (P(38), HMPGPZ) Ŀ DETERMINE P AND H FROM INTEGRATION VARIABLES ¢ 10 DO 50 I=1,NPS 20 J = 6 + 1 + 2X = 7+1+2 30 40 PP(I) = Y(J)50 HH(E) = Y(K) c CALCULATE TERMS IN LAMBDA MATRIX 62 OLII = CIXX 70 DL22 = CIYY 80 DL33 . CIZZ 90 DL12 = 0.0 100 DL13 = 0.0110 DL23 = 0.0HMPGPX = HX 112 HMPGPY \* HY 114 116 HMPGPZ + HZ IF (NTEGER(14)) 120,120,240 118 120 00 230 I=1,NPS 130 DO 230 J=1,NPS 140 IF (I-J) 150,150,170 150 L = (J\*(J-1))/2+1C = 1.0 255 160 GO TO 172

170  $E = \{[*(I-1)]/2+\}$ C = -1.0 HMPGPX = HMPGPX + Z.0\*C\*PP([]\*GZY(L)\*PO(J) HMPGPY = HMPGPY - Z.G\*C\*PP([]\*GZX(L)\*PO(J) HMPGPZ = HMPGPZ + 2.0\*C\*PP([]\*GYX(L)\*PD(J) 171 172 174 176 DL11 = UL11+(2.0/FNPS)\*(52YF4(1)+AZXF5(11)\*PP(1) 180 1 + PP(1)\*HXX(L)\*PP(J)\*2.0 190 DL22 = DL22+(2.0/FNPS)\*(4YXF6(1)+AZXF5(1))\*PP(1) + PP([]+HYY(L)+PP(J]+2+] 1 200 DL33 = DL33+(2.0/FNPS)\*(AYXF6(1)+AZYF4(1))\*PP(1) + PP(1)\*HZZ(L)\*PP(J)\*2.0 ĩ 210 DL12 = DL12+(2.0/FNPS; #47YF5(1) \*PP(1) - PP(I)\*AYX(I+J)\*PP(J) 1 220 DL13 = DL13+(2.0/FNPS1\*AYXF4(1)\*PP(1) - PP(1)\*AZX([+J)\*PP(J) ł 230 DL23 = DL23+(2.0/FNP5)\*AZXF6(1)\*PP(1) - PP(1)\*AZY(1,J)\*PP(J) ł CALCULATE TERMS IN INVERSE LAMHDA MATRIX c 240 DETERM # DL11\*DL22\*DL33 + DL12\*DL13\*DL23\*2.0 1 - IDL11\*DL23\*DL23 + DL22\*DL13\*DL13 + DL33\*DL12\*DL12) 250 DL111 = (DL22\*DL33 - DL23\*DL23)/DETERM 260 DL122 = (DL11\*DL33 - DL13\*DL13)/DETERM 270 DL133 = (DL11+DL22 - DL12+DL12)/DETERM 280 DL112 = (DL23+DL13 - DL12+DL33)/DETERM 290 DL113 = (DL12\*DL23 - DL13\*DL22)/DETERM DL123 = (DL12\*DL13 - DL23\*DL11)/DETERM 300 C CALCULATE OMEGA TERMS CMX = HMPGPX+DL111 + HMPGPY+DL112 + HMPGPZ+DL113 CMX = HMPGPX+DL112 + HMPGPY+DL127 + HMPGPZ+DL123 310 320 330 OM2 \* HMPGPX\*DL113 + HMPGPY&0L123 + HMPGPZ\*DL133 340 0MXX = 0MX + GMX 350 OMYY \* OMY \* OMY 360 OM2Z \* OMZ \* CMZ 370 OMXY = OMX + OMY 380 OMXZ = OMX + OM! 390 OMYZ = DMY + OM? ¢ CALL SUBPOUTINE WHICH DETERMINES METHIC FOR FORCE CALCULATIONS 400 CALL FORSEL 00 440 [=],6 F(I) = '.0 410 420 00 440 J=1+MP5 430 440 F(1) = F(1) + FES(1)(1) + PF(1)CALCULATE PERIVATIVES OF  $\sqrt{4}$ ,  $\sqrt{5}$ ,  $\sqrt{5}$ ,  $\sqrt{6}$ ,  $\sqrt{6$ c 450 460 470 HXD = 0-2+HY - 244+HZ + F(4) 480 490 HYD = 0"X+HZ - 0M2+HX + F(5) 500 HZD = ONY+HX - ONX+HY + F(6) С CALCULATE & AND H TREAMBLAR MATRICES 575 1F (NTEGER(14)) 510,510,700 510 00 530 L=1+APSE GILI = IOMX+GZYILI - OMY+GZXILI + OMZ+GYXILI)+2.0 520 H(L) = (JMXX+HXX(L) + JMYY+HYY(L) + OMZZ+HZZ(L) 530 1 - OMXY\*HYX(L) - GMYZ\*HZY(L) - CMXZ\*HZX(L))\*2.0 c CALCULATE GAMMA TIMES G, CAMMA TIMES 11, AND & MATRICES 54 1 00 692 I=1+NP5 550 DO 680 J=1+NPS 560 GAMGEI,JI = 0.7 GAMH(1,J1 = 0.0 570 580 00 680 K=1+NPS 630 IF (K-J) 640,640,661 64.1 L2 = (J+(J-11)/2+K

526

٥
645		C = 1 Q
650		GO TO 670
660		L2 = (K*(K-1))/2+J
665		C = -1.0
670		$GAMG(I_{\bullet}J) = GAMG(I_{\bullet}J) + C*GAM(I_{\bullet}K)*G(L_{2})$
680		$GAMH(I_{\bullet}J) = GAMH(I_{\bullet}J) + GAM(I_{\bullet}K) + H(L_2)$
690		DK(1) = (OMXX+OMZZ) * AZYF4(1) + (OMXX+OMYY) * AZXF5(1)
	1	◆ (OMYY+OMZZ)*AYXF6(1) + 2.0*(OMXZ*AYXF4(1) + OMXY*AZYF5(1)
	Ž	+ OMYZ#AZXF6(I))
¢		CALCULATE H MINUS GAMMA TIMES G TIMES PF
700		DO 735 I=1.NPS
710		HGAMGP(I) = HH(I)
715		IF (NTEGER(14)) 720,720,735
720		DO 730 J=1+NPS
730		HGAMGP(1) = HGAMGP(1)-GAMG(1,J)*PP(J)
735		CONTINUE
c		CALCULATE DERIVATIVES OF P
740		DO 850 I≠1,NPS
750		PD(I) = 0.0
760		HD(1) = 0.0
770		DO 830 J=1.NP5
780		PD(I) = PD(I) + AINV(I)J)*HGAMGP(J)
830		$HD(I) = HD(I) + GAM(I_{9}J) * (DX(J) + PF(J))$
c		SET P EQUAL TO PROPER INTEGRATION VARIABLE
840		K = 6 + I + 2
850		DYDX(K) = PD(1)
860		DO 900 I=1.NPS
c		CALCULATE DERIVATIVES OF H
870		DO 880 J=1.0PS
880		$HD(I) = HD(I) + (GA^{HH}(I,J)-SK(I,J)) + PP(J)$
	1	→ (GAMG(I,J)+B(I,J))*PD(J)
¢		SET H EQUAL TO PROPER INTEGRATION VARIABLE
890		K =7+I*2
900		DYDX(K) = HD(I)
910		RETURN
		END
0162		

-

```
*
                          LIST
                           SYMBOL TAPLE
CRK
                                                                                  SUBROUTINE RK
                                                                                   CONTON VAP
                                                                                   ntvension VAR(24000)+0904(74)+9(75)+9(75)+0(75)+
                      1 MTEGEP(225)+P(23400)
                                                                                   FOULVALENCE (VAR(1), Y(1)), (VAP(76), DYDX(1)),
                             (VAR(141),0(1)),(VAR(301),0(1)),(VAP(376),NTEGER(1)),
                      1
                     1 (VAQ(1~[]),(V1)),(V1A(4)]),(1)),(V1Q(3))

2 (VAQ(5^]),0[]),(VTFCF2(5),*)

LOAD [NOUT NATA [NTO "10"HIVE,

CALL I"PUT

CALCHLITE THE DELTY(J) AT Y(1) = 0.00
C
        11
Ċ
        S.
                                                                                   CALL PYOXS
                                  Determine the obtaint of the interaction J(i) = JAUX(I) = J(i)
DO 4U I = 144
        3-
        4 ^
C
        51
                                                                                   CALL OUTDUT
r
                                  CALCULATE THE Y(J) AT Y(1) = A.C.
        61
                                                                                   17 21 J # 1. 1
        7^
                                                                                   9 = .5+(n(J) = n(J))
         • ^
                                                                                  V(J) = V(J) + 9
                                  9
  11,
                                                                                   CALL TYTES
                                 CALL PYPXS

PP 120 [ = 1+

P(1) = PYPX(1)+P(1)

CALCULATT FUE Y(1) AT Y(1) = P(L STEP SIZE+

PP 160 J = 1+0

P 160 J = 1+0

P (J) = Y(J) + P

P(J) = Y(J) + P

P(J) = P(J) + P
    in
   120
   12.
   141
    15
                                                                             ika
~
                                   ~~L~~L***
   17.
  ١٩٦
                                                                                  nn lan I = 1+"
  197
                                                                                 U(1) = UAUA([]#0(J)
                                  \frac{1}{23\Gamma(2)\Gamma(24E-21E-21(2))} = \frac{1}{24} + 
۲<u>.</u>
  21.
21.
                                                                                 P = 1.77717670+(1(J) - 1(J))
                                                                                 Y(J) = Y(J) + 0

T(J) = T(J) + 3_00 = 1_0707105738^{(J)}
· ...
                                   TALTULITE THE DELTAY (U) IT Y(1) + STED SIZE.
                                CALL OVIC

CALL OVIC

OC 240 [ = 1.0"

O(1) = OVIC()=D(1)

CALCULATE THE Y(J) AT V(1) = CTED CT2E
   251
242
```

270		DO 300 J = 1 + N
280		R = ±1656666667+(D(J) → 2+0+Q(J))
290		$-\gamma(J) = \gamma(J) + R$
300		Q(J) = Q(J) + 3₀0#R → ₀5#9(J)
1	PROCEED T	O THE NEXT INTEGRATION STEP.
310		NGO = 1
32^		GO TO (20,330),NGO
330		RETHRN
		END
055		
310 320 330	PROCEED	NGO = 1 GO TO (20,330)+NGO RETURN END

LIST ٠ . SYMBOL TABLE COUTAID SUBROUTINE OUTAID COMMON VAR DIMENSION VAR(24000)+P(23400)+NTEGER(225)+ 1 QUTP(6+57+6)+NEOL(6)+Y(75)+DYDX(75)+PP(30) EQUIVALENCE (VAR(601) .P(1)) .(VAR(376) .NTEGER(1)) . (P(17989) .OUTP(1)) .(NTEGER(33) .NEOL(1)) .(NTEGER(39) .NOUTP) . . 1 (NTEGER (40) +NL0) + (NTEGER (42) +NLINE) + (Y(1) +T) + (Y(2) +VX) + (Y(3) +VY) + (Y(4) +VZ) + (DYDX(2) +VX0) + (DYDX(3) +VYD) + (DYDX(4) +VZD) + 2 3 (P(24)+OMX)+(P(25)+OMY)+(P(26)+OMZ)+(P(12031)+PP(1))+ 4 5 (VAR(1), Y(1)), (VAR(76), DYDX(1)) NOUTP = NOUTP 10 IF (NLINE) 30+30+50 WRITE OUTPUT TAPE 6+40 20 30 40 FORMAT(//81H TIME ٧X V۲ ٧Z 0 1MEGAX OMEGAY OMEGAZ/ SEC IN/SI RAD/SEC//) 82H IN/SEC IN/SEC R IN/SEC 2 3AD/SEC RAD/SEC 50 WRITE OUTPUT TAPE 6+60+T+VX+VY+VZ+OMX+OMY+OMZ 60 FORMAT(F11.3.1P6E12.4) 70 NLO = NLINE+1 DO 130 NPO-1,NOUTP OUTP(NPO,NLO,1) = T 80 90 100 NEOS = NEOL(NPO) DO 130 NEO=2 .NEOS 110 NE = 5+NPO+NEO-6 OUTP(NPO+NLO+NEO) = PP(NE) 120 130 140 IF(NLINE-54) 160+150+150 150 160 CALL LASOUT END

0033

```
LIST
       SYMBOL TABLE
G. ASOUT
                      SUBROUTINE LASOUT
                      COMMON VAR
                      DIMENSION VAR(24000) . P(23400) . NTEGER(225) .
      1
         OUTP (6, 57, 6), NEOL (6), Y(75), DYDX(75), PP (30)
                      EQUIVALENCE (VAR(601).P(1)), (VAR(376).NTEGER(1)).
         (P(17989).OUTP(1)). (NTEGER(33).NEOL(1)). (NTEGER(39).NOUTP).
      1
         (NTEGER(40),NLO). (NTEGER(42),NLINE), (Y(1),T). (Y(2).VX).
      2
         (Y(3) + VY) + (Y(4) + VZ) + (DYDX(2) + VXD) + (DYDX(3) + VYD) + (DYDX(4) + VZD) +
      3
         (P(24).OMX).(P(25).OMY).(P(26).OMZ)
      4
                      NLO = NLO
NOUTP = NOUTP
DO 170 NPO = 1.NOUTP
  10
20
  30
                      CALL PAGEHD
NE = 5+(NPO-1)
  40
  50
                      NE1 = NE+1
NE2 = NE+2
NE3 = NE+3
  60
70
  80
  90
                      NE4 = NE+4
 100
                      NE5 = NE+5
 110
                      WRITE OUTPUT TAPE 6,120,NE1,NE2,NE3,NE4,NE5
                                                                 P-12+
         FORMAT(///18H
                                TIME
 120
                                            P-12+11H
                                            P-12+11H
                                                                 P-12//)
      1
         11H
                       P-12,11H
                      DO 170 J=1+NLO
MEOS = NEOL(NPO)
 130
 140
 150
                      WRITE OUTPUT TAPE 6,160, (OUTP(NPO+ J,NEO)+
         NEO = 1.NEOS)
      1
 160
         FORMAT(F11.3.1P5E13.5)
                      CONTINUE
RETURN
 170
 180
                      END
0033
```

```
LIST
•
       SYMBOL TABLE
COUTPUT
                       SUBROUTINE OUTPUT
                       COMMON VAR
                       DIMENSION VAR(24000).P(23400).Y(75).NTEGER(225)
EQUIVALENCE (VAR(1).Y(1)).(VAR(376).NTEGER(1)).
          (VAR(601),P(1)), (NTEGER(13),NTSKIP), (NTEGER(41), LPRINT),
      1
2
          (NTEGER(42) .NLINE) . (NTEGER(43) .NSKIP)
                       EQUIVALENCE (NTEGER(18),NOTD), (NTEGER(19),NOTD1),
          (NTEGER (20) +NOTD2)
      1
  10
                       IF (Y(1) - P(2)) 20+ 50+ 50
c
          DETERMINE WHETHER OR NOT TO PRINT ON THIS INTEGRATION STEP
  20
                       IF (NSKIP - NTSKIP) 30+ 50+ 50
                       NSKIP = NSKIP + 1
  30
                       GO TO 150
  60
с
         DETERMINE IF A NEW PAGE IS REQUIRED FOR PRINTING OF RESULTS
  50
                       IF (NLINE - 55) 90.70.70
c
         HEAD A NEW PAGE
  70
                       CALL PAGEHD
  80
                       NLINE = 0
  90
                       CALL OUTAID
 100
                       NLINE = NLINE + LPRINT + 1
NSKIP = 0
 110
         ON THE LAST STEP OF THE INTEGRATION GO TO NEXT RUN IF (Y(1) - P(2)) 150. 150. 130
c
 120
 130
                       CALL LASOUT
 135
                       IF (NOTD) 140+140+136
         CALL PDUMP (VAR. VAR (150) + 1 + VAR (226) + VAR (300) + 1 + NTEGER + NTEGER (46) + 2 + P (7531) + P (21840) + 1 }
 136
      1
 140
                       CALL RK
 150
                       RETURN
                       END
0033
```

¥	LIST	
¥	SYMBO	DL TABLE
CPAGE	HD	
		SUBROUTINE PAGEHD
		COMMON VAR
		DIMENSION VAR(24000) NTEGER(225)
		EQUIVALENCE (VAR(376),NTEGER(1)),(NTEGER(1),IDENT),
	1 (N1	EGER(42), NLINE), (NTEGER(44), NPAGE)
10		NPAGE = NPAGE + 1
20		WRITE OUTPUT TAPE 6,30, IDENT, NPAGE
30		FORMAT (13H1 RUN NO 15+53H
	1	PAGE NO 151
50	-	RETURN
		END
0014		

LIST	
. SYNBOL	TABLE
CFORSEL	
	SUBROUTINE FORSEL
	COMMON VAR
	DIMENSION VAR(24000) +NTEGER(225)
	EQUIVALENCE (VAR(376) + NTEGER(1))
1	NSUBR = NTEGER(10)
2	GO_TO_(10+20+30+40+50+60+70+80+90)+NSUBR
10	CALL FORCE1
15	GO TO 95
20	CALL FORCE2
25	GO TO 95
30	CALL FORCES
35	GO TO 95
40	CALL FORCEA
45	GO TO 95
50	CALL FORCES
55	GO TO 95
60	CALL FORCES
65	GO TO 95
70	CALL FORCE7
75	GO TO 95
80	CALL FORCES
85	GO TO 95
90	CALL FORCES
95	RETURN
	END

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0028

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.

LIST SYMBOL TABLE ٠ ٠ CFORCE1 SUBROUTINE FORCE1 COMMON VAR DIMENSION VAR(24000) +P(23400) +PF(30) +NTEGER(225) UITERSIUN VAR(24000)+P(23400)+Pf(30)+NTEGER(225) EQUIVALENCE (VAR(1)+T)+(VAR(601)+P(1))+(P(4)+C5)+ 1 (VAR(376)+NTEGER(1))+(P(121)+Pf(1))+(NTEGER(8)+NPS)+(P(3)+C3)+ 2 (P(5)+TS) 00 20 I=1.NP5 PF(I) = 0.0 10 20 30 IF(T-T5) 40.40.60 FT = 1.0 GO TO 70 FT = 0.0 PF(3) = C3+FT PF(5) = C5+FT 40 60 70 80 90 RETURN END 0019

* LIST	a,		
		SUBROUTINE RETURN END	FORCE2
CFORCES		SUBROUTINE RETURN END	FORCE3
* LIST CEORCE4			
		SUBROUTINE RETURN END	FORCE4
* LIST CFORCE5			
		SUBROUTINE RETURN END	FORCE5
<pre># LIST CEORCE6</pre>			
		SUBROUTINE RETURN END	FORCE6
# LIST CEORCE7			
		SUBROUTINE RETURN END	FORCE7
+ LIST CFORCE8			
		SUBROUTINE RETURN END	FORCE8
<pre># LIST CEORCE9</pre>			
		SUBROUTINE RETUPN END	FORCE9
0040			

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0040

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# 4.0 SAMPLE ANALYSIS USING DIGITAL ROUTINE

4.1 <u>Geometry and Basic Data for Multi-Cylinder Model to be Used</u> in Checking Digital Routine

The geometry of the idealized missile is shown in Figure 110.



FIGURE 110 EXAMPLE VEHICLE

For the purposes of dynamic analysis the model is assumed to consist of a rigid body supporting two homogenous circular cylinders that are cantilevered from a uniform, homogenous, flat plate. The cylinders are secured at top by a uniform thin rod which is cantilevered from the rigid body and pinned to the top of the cylinders.

The relative dimensions of the system are shown in Figure 111.





The cylinders will be allowed parabolic deformations in two directions. The bottom plate can deform so that slices parallel to the Z-axis remain rigid. The rods can deform axially and bend in two directions.

The result is a system with 14 degrees-of-freedom. The 14 generalized coordinates are shown in Figure 112.



FIGURE 112 GENERALIZED COORDINATES

In Figure 112  $p_1$ ,  $p_2$ ,  $p_3$ ,  $p_4$ ,  $p_5$ , and  $p_6$  ar displacements and rotations of the rigid body at the origin of coordinates, x = 0, y = 0, z = 0. The generalized coordinates  $p_7$ ,  $p_6$ ,  $p_9$ , and  $p_{13}$  are displacements of the right cylinder <u>relative</u> to the rigid body in the center. Likewise,  $p_{10}$ ,  $p_{11}$ ,  $p_{12}$ and  $p_{14}$  are relative displacements for the left cylinder.

Figures 113 and 114 describe the deflection assumptions in detail.



FIGURE 113 DEFLECTIONS IN THE SYMMETRIC PLANE





These assumptions are described analytically in the following relations which relate continuous displacements to the finite number of generalized coordinates

(I-33)



(1-34)



These relations define  $\{h_x(x,y,z)\}, \{h_y(x,y,z)\}, and \{h_z(x,y,z)\}\$  which were introduced in Section 4.0.

(I-36)

$$p_{x}(x,y,z,t) = \frac{1}{2}h_{x}(x,y,z)\frac{1}{2}p(t)\frac{1}{2}$$

$$p_{y}(x,y,z,t) = \frac{1}{2}h_{y}(x,y,z)\frac{1}{2}\frac{1}{2}p(t)\frac{1}{2}p$$

$$P_{\mathbf{z}}(\mathbf{x}, q, z, t) = \{h_{\mathbf{z}}(h_1 q, z)\} \{p_{\mathbf{z}}\} \{p_{\mathbf{z}}\} \}$$
(I-37)

(I-38)

The inertia matrices can then be calculated

(I-39)

$$[A_{xx}] = \int \frac{1}{1+x} \frac{1}{2} \frac{1}{x^2}$$
(I-40)  
$$[A_{xx}] = \int \frac{1}{1+x} \frac{1}{2} \frac{1}{x^2} \frac{1}{x^2}$$

$$[A_{22}] = \int \{a_2\} \{b_2\} \{cd\}$$

$$[A_{22}] = \int \{a_2\} \{b_2\} \{cd\}$$

$$[A_{24}] = \int \{a_2\} \{b_3\} \{cd\}$$

$$(I-41)$$

$$[A_{xx}] = \int [n_x f + h_x] = M$$

$$[A_{xx}] = \int [n_y f + n_x] = M$$

$$(I-42)$$

(1-43)

The integration is broken down over the rigid body, the two cylinders, and the two parts of the plate as follows:

٠

In these expressions A is the cross-sectional area of the cylinders,  $\tau$  is the thickness of the plate and 2b is the width of the plate.

These matrices have been calculated and are listed in Table 13. The calculations were based on the dimensions in Figure 111 with

$$R = 340$$
 ins. (I-46)

and

$$\varrho = 1.2 \times 10^{-3} \frac{d_{10}}{m^3} = 3.1088 \times 10^{-6} \frac{slimber}{m^3}$$
 (I-47)

The results are expressed in the "slinch-inch-sec" system of units. The "slinch", which is the mass unit, is one  $lb_p$ -sec<sup>2</sup>/in. This is the only consistent set of units using inches for length and pounds for force. The digital routine, Pandora, is coded for accepting input data in any consistent set of units.

The strain energy of the system is

$$= z \frac{z^{2}}{z_{x}} = \frac{1}{z} \frac{z^{2}}{z_{x}} + \frac{z^{2}}{z_{x}}$$

where r is the radius and 1 is the length of the rod.

When equations I-36, I-37, and I-30 are introduced we obtain

$$= (1-4)$$

The stiffness matrix is listed in Table 14 for the following

$$\boldsymbol{\nu} = 0.25 \tag{I-50}$$

and

$$E = 9.637 \times 10^2 \ U_F \ /m^2$$
 (I-51)

The fictitious values of  $\rho$ , and E are introduced to make the homogenous model representative of a missile about 170 feet long with a total mass of 950,600 lb<sub>M</sub> and a fundamental free-free frequency of about 1.0 cps.

The damping matrix was taken as

$$[B] = \rho[K] \qquad (I-53)$$

The vibration modes are listed in Table 17.

# TABLE 14 INERTIA MATRICES OF MULTI-CYLINDER MODEL

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# INERTIA MATRICES FOR MULTI-CYLINDER MODEL

$\begin{bmatrix} A_{xx} \end{bmatrix}$
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COLUMN

	1	2	3	4	5
1	2.46269E 03	• 0•	0.	. 0.	0.
5	0.	0.	0.	0.	6.69156E 07
7	6.75281E 01	0.	0.	0.	0.
10	6.75281E 01	0.	0.	0.	0.
13	0.	0.	0.	0.	5.88542E 04
14.	0.	0.	0.	0.	5.88542E 04

# ROW

COLUMN

	6	7	8	9	10
1	0.	6.75281E 01	G.	0.	6.752810 01
6	1.01691E 08	-3.36604E 04	0.	0.	3.36604E 04
7	-3.36604E 04	6.50843. 01	0.	с.	0.
10	3.36604E 04	0.	0.	0.	6.50843E 01

ROW

COLUMN

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	11	12	13	14
5	0.	0.	5.88542E 04	5.88542E 04
13	0.	0.	3.53125E 04	0.
14	0.	0.	0.	3.53125E 04

# INERTIA MATRICES FOR MULTI-CYLINDER MODEL

# $\begin{bmatrix} A_{yy} \end{bmatrix}$

# COLUMN

R	0	W
	-	••

	1	. 2			3	4.		5
2	0.	2.46269E	03	0.		0.	0.	
4	0.	0.		0.		6.69156E 07	0.	
6	0.	7.91544E	04	0.		0.	0.	
7	0.	5.45944E	01	0.		0.	0.	
8	0.	2.04729E	01	0.		0.	0.	
10	0.	-5.45944E	01	0.		0.	0.	
11	0.	2.04729E	01	0.		0.	с.	

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## ROW

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# COLUMN

	6	7	8	У	10
2	7.91544E 04	5.45944E 01	2.04729E 01	0.	-5.45944E 01
6	8.70021E 08	1.85621E 04	0.	0.	-1.85621E 04
7	1.85621E 04	5.82340ĕ 01	1.63783E 01	0.	0.
8	0.	1.63783E 01	ľ.22837E 01	0.	0.
10	-1.85621E 04	0.	0.	0.	5.82340E 01
. 11	0.	0.	0.	0.	1.63783E 01

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# ROW

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COLUMN

	11	12	13	14
2	2.04729E 01	0.	0.	0.
10	1.63783E 01	0.	0.	0.
11	1.22837E 01	0.	0.	0.

### INERTIA MATRICES FOR MULTI-CYLINDER MODEL

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		[/	A <sub>zz</sub> ]		
ROW		co	LUMN		
	1	2	3	4	5
3	0.	0.	2.46269E	03 0.	-7.91544E 04
4	0.	0.	0.	1.01691E 08	0.
5	0.	0.	-7.91544E	0.	8.70021E 08
9	0.	Ũ.	2.04729E	01 1.04412E 04	Ū.
12	0.	0.	2.04729E	01 -1.04412E 04	0.
13	0.	. 0.	1.39216E	04 7.1000LE 06	-4.73333E 06
14	0.	0.	1.39216E	64 -7.1000CE 66	-4.73333E 06
ROW		C	BLUMN		
	6	7	8	9	10
3	0.	0.	0.	2.04729E 01 0.	
4	0.	0.	С.	1.04412F 04 G.	
9	0.	0.	с.	1.228378 01 0.	
13	0.	с.	0.	4.17647E 03 0.	
ROW		CO	LUMN		
	11	12	13	14	
3	0.	2.04729E 01	1.39216E 04	1.39216E 04	
4	0.	-1.04412E 04	7.10000E 06	-7.10000E 06	
5	0.	0.	-4.73333E 06	-4.73333E 06	
9	0.	0.	4.17647E 03	0.	

12 1.22837E 01 0. 4.17647E 03 0.

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0. 3.786678 06 0. 0. ο.

4.17647E 03 0. 3.78667E 06

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# INERTIA MATRICES FOR Multi-cylinder model



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	1		2 3	4	5
-	2 ((2605 03	0.	0.	0.	0.
2	2.462092 03	0	0.	0.	-6.69156E 07
4	0.	0.	0	0.	0.
6	7.91544E 04	0.	0.	0.	0.
7	5.45944E 01	0.	0.	0	0.
8	2.04729E 01	0.	0.	0.	0
10	-5.45944E 01	0.	0.	0.	· · · ·
11	2.04729E 01	0.	0.	0.	U.

# ROW

ROW

ROW

# COLUMN

	6	7	8	9	10
_	0	6.75281F 01	0.	. 0.	6.75281E 01
2	υ.	2 711605 04	0.	0.	2.71140E 04
6	0.	2.711402 04	0	0.	0.
7	-2.78431E 04	5.45944E UI	0•	0	0.
8	-1.04412E 04	2.04729E 01	0.	0.	-5.45944E 01
10	-2.78431E 04	-0.	0.	0.	2 047295 01
11	1.04412E 04	-0.	0.	0.	2.041232 01
•					

### COLUMN

	11	12	13	14
4	0.	0.	-5.88542E 04	-5.88542E 04

INERTIA MATRICES FOR MULTI-CYLINDER MODEL

# $\begin{bmatrix} A_{zx} \end{bmatrix}$ COLUMN

			_	3	4	5
	1		2	0.	(	<b>.</b>
3	2.46269E 03	0.	0•	•		0.
,	7 01544F 04	0.	0.	0.		
5	-1.913446 01	0	0.	0.		0.
9	2.04729E 01	0.	0	0.		0.
12	2.04729E 01	0.	0.	0		0.
	1.39216E 04	э.	0.	0.		0
13	1.07/2202	0	0.	0.		••
14	1.39216E 04	0.				

ROW

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# COLUMN

		-7	8	9	10
	6	7	<u> </u>	· 0.	6.75281E 01
3	0.	6.75281E 01	ζ.		-3.36604E 04
4	-1.01691E 08	3.36604E 04	0.	0.	
-	0	-2.71140E 04	0.	0.	-2.711402 04
5	0.	2 04729F 01	0.	0.	0.
9	-1.04412E 04	2.047292 01	-	0.	2.04729E 01
12	1.04412E 04	0.	0.	0	0.
13	-7.1000CE 06	1.39216E 04	0.	J•	1 202165 64
14	7.10000E 06	0.	0.	0.	1.392100 04
•					

# ROW

# INERTIA MATRICES FOR MULTI-CYLINDER MODEL

[A <sub>zy</sub> ]	
COLUMN	

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ROW

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	. 1	2	3	4	5	•
3	0.	2.46269E (	03 0.	0.	0.	
5	0.	-7.91544E 0	0.	• O <b>•</b>	0.	
9	0.	2.047296 0	01 0.	0.	0.	
12	0.	2.04729E C	01 0.	0.	0.	
13	0.	1.392168 0	0.	0.	С.	
. 14	0.	6.18736E C	03 0.	Ο.	0.	

ROW

## CULUMN

	6	7	8	9	10
.3	7.91544E 04	5.459448 01	2.047296 01	0.	5.45944E 01
4	0.	2.78431E 04	1.04412E 04	0.	2.78431E 04
5	-8.70021E 08	-1.85621E 04	0.	0.	1.85621E 04
9	0.	1.63783E 01	1.22837E 01	0.	0.
12	0.	0.	0.	0.	1.63783E 01
<b>k</b> 3	4.73333E 06	7.42484E 03	4.17647E 03	0.	0.
14	4.73333E 06	0.	0.	0.	7.42484E 03
		•			

ROW

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# COLUMN

	11	12	13	14
3	2.04729E 01	0.	0.	0.
4	-1.04412E 04	0.	0.	0.
12	1.22837E 01	0.	0.	0.
14	4.17647E 03	0.	С.	0.

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TABLE 15
STIFFNESS MATRIX OF MULTI-CYLINDER MODEL

		STIFFNES MULTI-CY	S MATRIX FOR LINDER MODEL		·
ROW			[K] Colůmn		
	6	7	8	9	10
7	0.	7.82137E 0	2 -1.37249E 02	0.	0.
8	0.	-1.37249E 0	2 3.31971E 03	0.	0.
9	0.	·0 •	0.	4.13578E 01	0.
10	С.	U.	0.	0.	7.82137E 02
11	0.	0.	0.	0.	1.37249E 02
13	0.	0.	Ċ.	-3.49985E 04	с <b>.</b>
RON			 Column		
	11	12	13	14	
9	<b>U</b> .	0.	-3.49985E 04	С.	
10	1.37249E 02	0.	J.	э.	
11	3.31971E 03	0.	0.	0.	
12	0.	4.13578E 0	0.	-3.49985E 04	
13	0.	0.	5.24197E 07	0.	
14	0.	-3.49985E 0	)4 0.	5.24197E 07	

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ROW		DAMPING MULTI-CYL	MATRIX FOR INDER MODEL [B] DLUMN		
	6	7	٤	9	10
7	0.	<b>じ</b> •	4.61461E-03	-8.09769E-04	¢•
8	0.	0.	-8.09769E-04	1.95863E-02	С.
9	0.	0.	С.	0.	2.44011E-04
13	0.	0.	Ŭ.∎	<b>5</b> .	-2.05491E-01
ROW		C	DLUMN		
	11	12	13	14	
9	·0 .	0 <b>.</b>	-2.064918-01	0.	
10	8.09769E-04	0.	<b>0.</b>	0.	
11	1.95863E-02	С.	î.	э.	
12	9.	2.44011E-04	٥.	-2.064910-01	
13	0.	0.	3.09276E 02	0.	
14	э.	-2.06491E-01	ί.	3.09276E 02	

TABLE 16 DAMPING MATRIX OF MULTI-CYLINDER MODEL

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## TABLE 17 VIBRATION MODES AND FREQUENCIES OF MULTI-CYLINDER MODEL

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### RIGID BODY MODES OF MULTI-CYLINDER MODEL

COLL: Point	1ST MODE O CPS	2ND MODE O CPS	3RD MODE O CPS
1	1.0000000E 00	0.	0.
2	0.	1.0000000E 00	0.
3	0.	0.	1.000000E 00

COLL. POINT	4TH MODE O CPS	5TH MODE 0 CPS	6TH MODE 0 CPS
2	С.	<b>G</b> .	-3.2141383E 01
3	9.	3.2141383F 01	0.
4	1.0000000E 00	С.	0.
<sup>`</sup> 5	0.	1.00000CGE 00	0.
6	0.	0.	1.0000000E 00

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# ELASTIC VIBRATION MODES OF MULTI-CYLINDER MODEL

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COLU. Point	7TH MODE 0.1525 CPS	8TH MODE 0.1616 CPS	9TH MODE 0.4017 CPS	10TH MODE 0.4064 CPS
•	0	0.	-1.1133801E-04	-1.4099310E-04
1	0.	υ.	-1.1833926E-04	9.3313473E-05
2	-2.0188738E-04	-6.99683228-12	0.	0.
4	3.5300905E-14	-1.6040825E-06	0.	0.
5	4.5499184E-08	1.5349514E-15	0.	0.
6	0.	0.	8.8626299E-08	-7.1413935E-08
7	2	0.	4.57195C4E-C3	5.1748865E-04
r	0.	0.	3.3601228E-04	4.2828283E-05
0 0	7.9674938E-C3	8.4550517E-03	0.	0.
.,,		0.	-5.1154501E-04	4.5244134E-03
10	0.	Ũ.	4.3165546E-07	-3.9645560E-05
17	7.9674943E-03	-8.4550512E-03	0.	0•
.12	6.2691606E-06	6.6124635E-06	0.	э.
14	6.2691609E-06	-6.6124632E-06	0.	э.

# ELASTIC VIBRATION MODES OF MULTI-CYLINDER MODEL

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COLL. Point	11TH MODE 0.9697 CPS	12TH MODE 0.9711 CPS	13TH MODE 2.899 CPS	14TH NODE 2.951 CPS
1	0.	0.	5.6372719E-06	8.8310978E-05
2	0.	0.	6.3957797E-05	-1.9864263E-04
3	7.5097082E-05	2.5339505E-05	0.	0.
4	-2:3025346E-07	6.8238795E-07	0.	0.
<sup>5</sup>	-2.0908947E-07	-7.0551745E-08	0.	0.
·6	0.	0.	-5.4058644E-08	2.0338822E-08
7.	0.	0.	-1.6747193E-03	-1.4765217E-03
8	Э.	0.	1.1784473E-02	1.0951930E-02
9	6.5869496E-03	1.2696821E-02	ũ.	٥.
10	0.	0.	1.46913408-03	-1.7441080E-03
11	0.	0.	-1.0885370E-02	1.2140589E-02
12	1.2934143E-02	-6.1119409E-03	C.	ΰ.
13	-1.4189395E-05	-2.9219000E-05	0.	0.
14	-2.8991421E-05	1.4648770E-05	<b>0</b> .	0.

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## 4.2 Sample Problems

The digital program was demonstrated for three different problems:

(1) Response to simplified gust. Generalized forces were taken as

$$P_{3} \epsilon_{i}^{2} = -\frac{1}{2} \partial_{\mu} V_{\alpha} C_{e_{\mu}} \frac{3\epsilon}{V_{\alpha}} \left( \mathcal{H}(\epsilon) - \mathcal{H}(\epsilon - \epsilon^{*}) \right)$$
(I-54)

$$P_{\mathcal{S}} : t = -\frac{1}{2} \sqrt{2} \sqrt{2} \left( \frac{v_{\mathcal{I}}}{\eta_{\mathcal{D}}} \left( \frac{w_{\mathcal{I}}}{\eta_{\mathcal{D}}} \left( \frac{w_{\mathcal{I}}}{w_{\mathcal{D}}} - \frac{w_{\mathcal{I}}}{w_{\mathcal{D}}} \right) \right)$$
(I-55)

where

$$H(t) = \begin{cases} i & \text{for } t \ge 0 \\ o & \text{for } t < 0 \end{cases}$$
 (I-56)

$$v_{\infty}$$
 = 2.442 x 10<sup>4</sup> in/sec (I-57)  
 $t^{*}$  = 0.5325 secs. (five missile lengths)  
 $v_{\infty}$  = 9.828 x 10<sup>-3</sup> (20 ft/sec. gust)  
 $c_{t_{\infty}}$  = 2.011 x 10<sup>5</sup> in<sup>2</sup>  
 $c_{t_{\infty}}$  = 2.373 x 10<sup>8</sup> in<sup>3</sup>

(2) Response to impulsive spin-up. External forces were zero; all initial conditions zero except Ω<sub>x</sub>/o).

$$\Omega_{x}(0) = 10 \text{ nad./sec.}$$
 (I-58)

All other forces were zero. All initial conditions were zero.

(3) Response to initial displacements in first mode of vibration. All external forces were zero; all initial conditions were zero except {b(a)}.

$$\{p(i)\} = \{q\}_i =$$
 the first mode of (I-59) vibration

The results of these problems are given on the following pages.

TABLE 18 DATA READ INTO THE COMPUTER FOR SAMPLE RUNS

1 5 0 0 0 0 0 14 1 1 PANDORA - LVV420 15 AUG 1963 RESPONSE OF TITAN III MODEL TO GUST 12 •1 22•0 1235.25675 3 4 4 145/615-27 5 5325 2 5 7 1 0 0 1 14 0 1 PANDOPA - LVV420 15 AUG 1663 9550045E OF TITAN III MODEL JO MODELIVE SPIN-JP 123 •05 17•0 0.0 4 0.0 5 DESODAISE DE FITAN ILL MODEL TO INITIAL DISOLACEMENTA IN ITS IST MODE 1 •1 2 5•7 Z 5.° 63 -.2°100737.° 54 3630005.73 65 46400104.07 60 70574038-07 73 53674038-07 73 53674038-07 73 .52501575-75 74 .57501510-75 . **`•**` 5 -----

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TABLE 19						
LISTINGS OF	COMPUTER	RUNS	FOR	TRANSIENT	GUST	RESPONSE

RUN NU PANDORA RESPONSI	0 1 - LVV420 E OF TITAN L	15 AUG II MODEL TO G	1963 UST		PAGE NO	1
TIME		~~~~	 W7	OMEGAY	OHEGAY	ONECAZ
SEC	IN/SEC	IN/SEC	IN/SEC	RAD/SEC	RAD/SEC	RAD/SEC
0.	0.	0.	0.	-0.	0.	0.
0.100	-2.6792E-07	-8.6144E-20	5.0159E-02	-4.5379E-17	1.6025E-04	-5.9227E-20
0.200	-2-14346-00	-0.43206-18	1.00326-01	-9.11895-15	3.2049E-04	1 35595-18
0.400	-1.7147E-05	-1.4760E-15	2.0063E-01	-1.71948-13	6.4098E-04	5.1526E-17
0.500	-3.3490E-05	-7.7982E-15	2.50796-01	-5.2731E-13	8.0123E-04	1.4679E-16
0.600	-5.4963E-05	-2.7318E-14	2.5915E-01	-1.3533E-12	8.2793E-04	2.5777E-16
0.700	-7.6420E-05	-7.1129E-14	2.5915E-01	-2.1089E-12	8.2793E-04	4.7445E-16
0.800	-9.18/01-05	~1.30042+13	2.59155-01	-2.99048-12	8-21935-04	0.12000-10
1.000	-1.4079E-04	-3.3335E-13	2.5915E-01	~4.41218-12	8.27936-04	9.5955E-16
1.100	-1.62241-04	-4.5181E-13	2.59158-01	-4.3517E-12	8.2793E-04	1.4810E-15
1.200	-1.8370E-04	-5.6237E-13	2.59158-01	-3.8636E-12	8.2793E-04	1.5424E-15
1.300	-2.0516E-04	-6.5421E-13	2.5915E-01	-2.8500E-12	8.2793E-04	1.7491E-15
1.400	-2.2661E-04	-7.1529E-13	2.59155-01	-1.3326E-12	8.27936-04	1.8073d-15
1.500	-2-69536-04	-7.06216-13	2.59155-01	2.01896-12	8.27935-04	1.99646-15
1.700	-2.9098E-04	-6.3624E-13	2.5915E-01	3.6613E-12	8.2793E-04	1.7863E-15
1.800	-3.1244E-04	-5.25126-13	2.59156-01	5.1461E-12	8.2793 -04	1.5219E-15
1.900	-3.3389E-04	-3.7707E-13	2.5915E-01	6.38956-12	8.2793E-04	1.38886-15
2.000	-3.5535E-04	-1.9878E-13	2.5915E-01	7.4463E-12	8.2793E-04	3.1005E-16
2.200	-3.98265-04	4.0/99E-13	2.5915E-01	8.86275-12	8-27935-04	-1.76236-16
2.300	-4.1972E-04	4.6282E-13	2.5915E-01	9.0684E-12	8.27931-04	-9.5399E-16
2.400	-4.4117E-04	6.9891E-13	2.59156-01	8.85956-12	8.2793E-04	-1.5077E-15
2.500	-4.6263E-04	9.2514E-13	2.5915E-01	8.20156-12	8.2793E-04	-2.0804E-15
2.600	-4.8409E-04	1.1292E-12	2.59156-01	7.0321E-12	8.27932-04	-2.9436E-15
2.700	-5.05546-04	1.29995-12	2.59156-01	3.01845-12	9.27935-04	-3.23492~15
2.900	-5.4845E-04	1.51475-12	2.59158-01	2.29001-12	8.27936-04	-4.1435E-15
3.000	-5.6991E-04	1.5571E-12	2.59151-01	7.74436-13	8.2793E-04	-4.31896-15
3.100	-5.9137E-04	1.5622E-12	2.59156-01	-4.6982E-13	8.2793E-04	-4.63755-15
3.200	-6.12828-04	1.5381E-12	2.5915E-01	-1.4031E-12	8.27936-04	-4.6673E-15
3.500	-0.34285-04	1.49200-12	2.59158-01	-2.00520-12	8 27935-04	-4.04/25-15
3.500	~6.7719E-04	1.3638E-12	2.5915c-01	-2.8271E-12	3.2793E-04	~4.38712-15
3.600	-6.9865E-04	1.2873E-12	2.59152-01	-3.03132-12	8.27936-04	-4.2641E-15
3.700	-7.2010E-04	1.2050E-12	2.5915E-01	-3.3309E-12	8.2793E-04	-3.9654E-15
3.800	-7.41562-04	1.1176E-12	2.59156-01	-3.5)196-12	8.2793E-04	-3.6774E-15
5.900	-7.84475-04	9 78536-13	2.59155-01	-3.70305-12	8 27935-04	-3.40422-15
4.100	-8.0593E-04	8.25106-13	2.5915E-01	-4.25288-12	8.27936-04	-2.91636-15
4.200	-8.2738E-04	7.12895-13	2.59158-01	-4.6616c-12	8-2793E-04	-2.36138-15
4.300	-8.4884E-04	5.8931E-13	2.59156-01	-5.1356E-12	8.2793E-04	-2.10116-15
4.400	-8.7030E-04	4.5316E-13	2.59156-01	-5.57748-12	8-27936-04	-1.78338-15
4.500	-8.91758-04	3.06451-13	2.59151-01	-5.83641-12	8-27935-04	-1.1316E-15
4.700	-9.34665-04	1.01695-14	2.5915F-01	-5.21508-12	8.27936-04	-5.39746-16
4.800	-9.5612E-04	-1.16886-13	2.59158-01	-4.20708-12	8.2793E-04	-5.91936-16
4.900	-9.7758E-04	-2.1404E-13	2.5915E-01	-2.81956-12	8-2793E-04	-3.7195E-17
5.000	-9.9903E-04	-2.7292E-13	2.59152-01	-1.226512	8.27936-04	1.6635E-16
5.100	-1.0205E-03	-2.9015E-13	2.59158-01	3.7.276-13	8.27935-04	2.06918-1/
5.300	-1.0634F-03	-2.10435-13	2.59156-31	2.95296-12	8.27935-04	-2.4986F-1
5.400	-1.0849E-03	-1.2645E-13	2.5915E-01	3.8390E-12	8.2793E-04	-2.41576-16
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2.300 -5.87578E-08 -4.607ClE-C8 7.10759E-03 6.37362E-14 -1.73753E-06 2.400 5.78495E-07 -3.60808E-08 6.59396E-03 3.94292E-14 -1.06388E-06 2.500 1.14730E-06 -2.17633E-08 6.05744E-03 -7.0468LE-14 -5.40599E-07 2.600 1.60858E-06 -4.32002E-09 5.58808E-03 -1.97743E-13 -3.26250E-07 2.700 1.92668E-06 1.44304E-08 5.19494E-03 -3.14272E-13 -4.63865E-07 2.800 2.07531E-06 3.31237E-08 4.85063E-03 -3.99308E-13 -8.65896E-07 2.900 2.04316E-06 5.04039E-08 4.049274E-03 -4.45763E-13 -1.34606E-06 3.000 1.83687E-06 6.49356E-08 4.04792E-03 -4.15839E-13 -1.68659E-06 3.100 1.47997E-06 7.60545E-08 3.46077E-03 -3.94672E-13 -1.71684E-06 3.200 1.00894E-06 8.35478E-08 2.71244c-03 -3.88662E-13 -1.37468E-06 3.200 1.00894E-06 8.35478E-08 2.71244c-03 -3.88662E-13 -1.37468E-06 3.200 1.00894E-06 8.35478E-08 2.71244c-03 -3.88662E-13 -1.37484E-06 3.300 4.67062E-07 8.17168E-08 1.24435E-05 -7.60039E-13 7.19385E-07 3.600 -1.17910E-06 7.21154E-08 -1.24435E-05 -7.60039E-13 7.19385E-07 3.600 -1.17910E-06 3.78942E-08 -2.05786E-03 -1.29502E-12 8.65615E-07 3.900 -2.28321E-06 1.40047E-08 -2.53788E-03 -1.29502E-12 8.65615E-07 3.900 -2.28321E-06 1.40047E-08 -2.53788E-03 -1.256280E-12 2.77349E-08 4.100 -2.49398E-06 3.78942E-08 -2.05786E-03 -1.29502E-12 8.65615E-07 4.000 -2.49398E-06 -1.09275E-08 -3.01562E-03 -1.56280E-12 2.77349E-08 4.300 -2.17257E-06 -4.09847E-08 -3.55487E-03 -1.43886E-12 4.19371E-07 4.000 -2.49398E-06 -1.09275E-08 -3.01562E-03 -1.56280E-12 2.77349E-08 4.300 -2.17257E-06 -9.09880E-08 -4.89689E-03 -1.29502E-12 8.65615E-07 4.000 -1.81968E-06 -1.08426E-07 -5.62790E-03 -2.04565E-12 1.32824E-06 4.500 -1.36553E-06 -1.08426E-07 -5.62790E-03 -2.85533E-12 2.43321E-06 4.500 -1.36553E-06 -1.18607E-07 -6.22894E-03 -2.31628E-12 2.00057E-06 4.600 -8.48113E-07 -1.20789E-07 -7.38370E-03 -2.87845E-12 2.21435E-06 4.900 5.98614E-07 -5.77849E-08 -7.36237E-03 -3.287845E-12 2.21435E-06 4.900 5.98614E-07 -5.77849E-08 -7.36237E-03 -3.287845E-12 1.69239E-06 5.000 1.05358E-06 -2.91415E-08 -7.25752E-03 -3.123784E-12 1.69239E-06 5.000 1.05358E	2.200	-7.26971E-07	-5-191358-08	7.54761E-03	6.97457E-14	-2.34401E-06
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2.800 2.07531E-06 3.31237E-08 4.85063E-03 -3.99308E-13 -8.65896E-07 2.900 2.04316E-06 5.04039E-08 4.49274E-03 -4.45763E-13 -1.34606E-06 3.000 1.83687E-06 6.49356E-08 4.04792E-03 -4.15839E-13 -1.68659E-06 3.100 1.47997E-06 7.60545E-08 3.46607E-03 -3.94672E-13 -1.71684E-06 3.200 1.00894E-06 8.35478E-08 2.71244:-03 -3.88662E-13 -1.37684E-06 3.200 4.67062E-07 8.71484E-08 1.83568E-03 -4.37276E-13 -7.29979E-07 3.400 -1.02336E-07 8.66081E-08 8.98990E-04 -5.73701E-13 4.15279E-08 3.500 -6.61165E-07 8.71484E-08 -1.24435E-05 -7.60039E-13 7.19385E-07 3.600 -1.17910E-06 7.21154E-08 -8.25755E-04 -9.45728E-13 1.1177E-06 3.700 -1.63333E-06 5.74790E-08 -1.50417E-03 -1.13743E-12 1.15141E-06 3.800 -2.00639E-06 3.78942E-08 -2.05786E-03 -1.29502E-12 8.65615E-07 4.000 -2.28321E-06 1.40047E-08 -2.53788E-03 -1.43886E-12 4.19371E-07 4.000 -2.44939E-06 -1.29275E-08 -3.01562E-03 -1.63481E-12 4.19371E-07 4.000 -2.44939E-06 -4.09847E-08 -3.55477E-03 -1.63481E-12 1.15853E-07 4.000 -2.44939E-06 -1.29275E-08 -3.01562E-03 -1.56280E-12 2.77349E-08 4.100 -2.44939E-06 -1.09847E-08 -4.89689E-03 -1.85996E-12 6.20531E-07 4.200 -2.39987E-06 -6.78070E-08 -4.89689E-03 -1.85996E-12 6.20531E-07 4.200 -2.39987E-06 -1.1807E-07 -6.28984E-03 -2.31628E-12 2.005732E-08 4.300 -2.17257E-06 -9.09880E-08 -4.89689E-03 -2.87645E-12 1.32824E-06 4.600 -1.381968E-06 -1.08426E-07 -5.62790E-03 -2.87645E-12 1.32824E-06 4.600 -3.48113E-07 -1.20789E-07 -6.83270E-03 -2.87645E-12 2.20057E-06 4.600 -3.48113E-07 -1.20789E-07 -7.34386E-03 -3.12378E-12 2.21435E-06 4.600 -8.48113E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06 4.600 -8.98614E-07 -8.24305E-08 -7.30287E-03 -3.12378E-12 1.32824E-06 5.000 8.98614E-07 -5.77849E-08 -7.25752E-03 -3.12378E-12 1.14270E-06 5.000 8.98614E-07 -5.77849E-08 -7.25752E-03 -3.12378E-12 1.14270E-06 5.000 8.96251E-07 -5.77849E-08 -7.25752E-03 -3.15736E-12 7.274811E-07 5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 1.01884E-07 5.400 6.64342E-07 6.751632E-08 -7.31437E-03 -2.971125E-12 1.01884E-07 5.400 6.64	2.700	1.926685-06	1.443045-09	5.194946~03	-3.142726-13	-4-638656-07
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<b>3.</b> 000 1.83687E-06 6.49356E-08 4.04792E-03 -4.15839E-13 -1.68659E-06 <b>3.</b> 100 1.47997E-06 7.60545E-08 3.46607E-03 -3.94672E-13 -1.71684E-06 <b>3.</b> 200 1.00894E-06 8.35478E-08 2.71244=03 -3.88662E-13 -1.37684E-06 <b>3.</b> 300 4.67062E-07 8.71484E-08 1.83568E-03 -4.37276E-13 -7.29979E-07 <b>3.</b> 400 -1.02336E-07 8.71484E-08 1.83568E-03 -4.37276E-13 -7.29979E-07 <b>3.</b> 600 -1.02336E-07 8.17168E-08 -1.24435E-05 -7.60039E-13 7.19385E-07 <b>3.</b> 600 -1.17910E-06 7.21154E-08 -8.25755E-04 -9.45728E-13 1.11777E-06 <b>3.</b> 700 -1.60333E-06 5.74790E-08 -1.50417E-03 -1.13743E-12 1.15141E-06 <b>3.</b> 800 -2.00639E-06 3.78942E-08 -2.05786E-03 -1.29502E-12 8.65615E-07 <b>3.</b> 900 -2.28321E-06 1.40047E-08 -2.53788E-03 -1.43886E-12 4.19371E-07 <b>4.</b> 000 -2.4939E-06 -1.29275E-08 -3.01562E-03 -1.56280E-12 2.77349E-08 <b>4.</b> 100 -2.49398TE-06 -6.78070E-08 -4.18684E-03 -1.73712E-12 9.05732E-08 <b>4.</b> 300 -2.17257E-06 -9.09880E-08 -4.18684E-03 -1.73712E-12 9.05732E-08 <b>4.</b> 300 -2.17257E-06 -9.09880E-08 -4.89689E-03 -1.85996E-12 6.20531E-07 <b>4.</b> 000 -1.81968E-06 -1.08426E-07 -5.62790E-03 -2.04565E+12 1.32824E-06 <b>4.</b> 500 -1.36553E-06 -1.18607E-07 -6.29894E-03 -2.31628E-12 2.00057E-06 <b>4.</b> 600 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12373E-12 2.043321E-06 <b>4.</b> 600 1.82756E-07 -1.01891E-07 -7.34388E-03 -3.12373E-12 2.243321E-06 <b>4.</b> 600 1.82756E-07 -1.01891E-07 -7.34388E-03 -3.12373E-12 2.21435E-06 <b>4.</b> 600 1.82756E-07 -1.01891E-07 -7.34388E-03 -3.12373E-12 2.21435E-06 <b>4.</b> 000 -8.48113E-07 -5.77349E-08 -7.306377E-03 -3.287845E-12 1.32824E-06 <b>5.</b> 000 1.82756E-07 -1.01891E-07 -7.34388E-03 -3.12373E-12 2.21435E-06 <b>5.</b> 000 1.9251E-07 -1.577849E-08 -7.306377E-03 -3.12373E-12 1.4270E-06 <b>5.</b> 000 1.92651E-07 -1.577849E-08 -7.306377E-03 -3.12373E-12 1.4270E-06 <b>5.</b> 000 1.92651E-07 -1.577849E-08 -7.25752E-03 -3.12373E-12 1.4270E-06 <b>5.</b> 000 1.92651E-07 -3.51693E-08 -7.31437E-03 -3.27371E-12 7.74811E-07 <b>5.</b> 200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15733E-12 1.01884E-07 <b>5.</b> 400 <b>5.6642E-07</b> 5.76892E-08 -7.31437E-03 -3.07389E-12 1.01884E-07	2.900	2.043365-06	5.040396-08	4.492746-03	-4.457636-13	-1.346065-06
3.100 1.47997E-06 7.60545E-08 3.46607E-03 -3.94672E-13 -1.71684E-06 3.200 1.00894E-06 8.35478E-08 2.71244c-03 -3.88662E-13 -1.37484E-06 3.300 4.67062E-07 8.71484E-08 1.83568E-03 -4.37276E-13 -7.29979E-07 3.400 -1.02336E-07 8.71484E-08 1.83568E-03 -4.37276E-13 -7.29979E-07 3.500 -6.61165E-07 8.71484E-08 1.24435E-05 -7.60039E-13 7.19385E-07 3.600 -1.17910E-06 7.21154E-08 -1.24435E-05 -7.60039E-13 7.19385E-07 3.600 -1.17910E-06 7.21154E-08 -8.25755E-04 -9.45728E-13 1.11777E-06 3.700 -1.63333E-06 5.74790E-08 -1.50417E-03 -1.13743E-12 1.15141E-06 3.800 -2.00639E-06 3.78942E-08 -2.05786E-03 -1.29502E-12 8.65615E+07 3.900 -2.28321E-06 1.40047E-08 -2.53788E-03 -1.43886E-12 4.19371E-07 4.000 -2.44939E-06 -1.29275E-08 -3.01562E-03 -1.56280E-12 2.77349E-08 4.100 -2.49152E-06 -4.09847E-08 -3.55487E-03 -1.63481E-12 -1.15853E-07 4.200 -2.39987E-06 -6.78070E-08 -4.18684E-03 -1.73712E-1 2 9.05732E-08 4.300 -2.17257E-06 -9.09880E-08 -4.89689E-03 -1.85996E-12 6.20531E-07 4.400 -1.81968E-06 -1.08426E-07 -5.62790E-03 -2.04565E+12 1.32824E-06 4.500 -1.36553E-06 -1.18607E-07 -6.29894E-03 -2.31628E-12 2.00057E-06 4.600 -8.48113E-07 -1.20789E-07 -6.83270E-03 -2.31628E-12 2.5035E+06 4.800 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.5035E+06 4.800 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.5035E+06 4.800 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06 4.900 5.98614E-07 -8.24305E-08 -7.308370E-03 -3.287445E-12 1.4270E+06 5.000 1.9558E-06 -2.91415E-08 -7.308370E-03 -3.28734E+12 1.69239E-06 5.000 1.05358E-06 -2.91415E-08 -7.30837E-03 -3.28737E-12 7.74641E-07 5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E+12 7.27464E-07 5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E+12 7.27464E-07 5.400 6.64342E+07 3.51693E-08 -7.31437E-03 -3.0339E+12 1.01884E-06 5.400 6.66454E+07 3.51693E-08 -7.31437E-03 -3.03939E+12 1.01884E+06 5.400 6.66454E+07 3.51693E-08 -7.31437E-03 -3.03939E+12 1.01884E+06 5.400 6.66454E+07 3.51693E-08 -7.31437E-03 -3.0339E+12 1.01884E+06 5.400 6.66454E+07 3.5	3.000	1.83687E-06	A.49356E-08	4.04797E-03	-4-15839E-13	-1.686596-06
3.200 1.00894E-06 8.35478E-08 2.71244±-03 -3.88662E-13 -1.37484E-06 3.300 4.67062E-07 8.71484E-08 1.83568E-03 -4.37276E-13 -7.29979E-07 3.400 -1.02336E-07 8.66081E-08 8.98990E-04 -5.73701E-13 4.15279E-08 3.500 -6.61165E-07 8.17168E-08 -1.24435E-05 -7.60039E-13 7.19385E-07 3.600 -1.17910E-06 7.21154E-08 -8.25755E-04 -9.45728E-13 1.11777E-06 3.700 -1.63333E-06 5.74790±-08 -1.50417E-03 -1.13743E-12 1.15141E-06 3.800 -2.00639E-06 3.78942E-08 -2.05786E-03 -1.29502E-12 8.65615E+07 3.900 -2.28321E-06 1.40047E-08 -2.53788E-03 -1.43866E-12 4.19371E-07 4.000 -2.44939E-06 -1.29275E-08 -3.01562E-03 -1.63481E-12 4.19371E-07 4.000 -2.44939E-06 -4.09847E-08 -3.55487E-03 -1.63481E-12 -1.15853E-07 4.200 -2.39987E-06 -6.78070E-08 -4.18684E-03 -1.73712E-12 9.05732E-08 4.300 -2.17257E-06 -9.09880E-08 -4.89689E-03 -1.85996E-12 6.20531E-07 4.400 -1.81968E-06 -1.08426E-07 -5.62790E-03 -2.04565E+12 1.32824E-06 4.600 -8.48113E-07 -1.20789E-07 -6.283270E-03 -2.31628E-12 2.43321E-06 4.600 -8.48113E-07 -1.20789E-07 -6.83270E-03 -2.58533E-12 2.43321E-06 4.600 -8.48113E-07 -1.20789E-07 -7.18167E-03 -3.12378E-12 2.21435E-06 4.600 -8.4813E-07 -5.77849E-08 -7.30895E-03 -3.12378E-12 2.21435E-06 4.900 5.98614E-07 -5.77849E-08 -7.30895E-03 -3.287845E-12 1.14270E-06 5.000 1.96251E-07 -5.77849E-08 -7.35752E-03 -3.12378E-12 1.14270E-06 5.000 3.98614E-07 -5.77849E-08 -7.25752E-03 -3.12378E-12 1.14270E-06 5.000 3.986251E-07 -5.77849E-08 -7.25752E-03 -3.287345E-12 1.14270E-06 5.000 3.986251E-07 -5.77849E-08 -7.25752E-03 -3.15736E-12 7.27464E-07 5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 1.01884E-07 5.400 6.46432E-07 6.781692E-08 -7.31437E-03 -3.297371E-12 7.74811E-07 5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 1.01884E-06 5.400 6.46432E-07 6 -7.51693E-08 -7.31437E-03 -3.297371E-12 1.5481E-07 5.400 5.4642E-07 6 -7.51693E-08 -7.31437E-03 -3.297311E-12 1.5481E-07 5.400 6.46432E-07 6 -7.51693E-08 -7.31437E-03 -3.297311E-12 1.54610F-04 5.400 6.46432E-07 6 -7.51693E-08 -7.31437E-03 -3.03395E-12 1.01884E-06	3,100	1.479976-06	7.605656-08	3.466075-03	-3.96672E-13	-1.716845-06
3.300 4.67062E-07 8.71484E-08 1.83568E-03 -4.37276E-13 -7.29979E-07 3.400 -1.02336E-07 8.66081E-08 8.98990E-04 -5.73701E-13 4.15279E-08 3.500 -6.61165E-07 8.71484E-08 1.83568E-03 -4.37276E-13 7.19385E-07 3.600 -1.17910E-06 7.21154E-08 -8.25755E-04 -9.45728E-13 1.11777E-06 3.700 -1.6333E-06 5.74790E-08 -1.50417E-03 -1.13743E-12 1.15141E-06 3.800 -2.00639E-06 3.78942E-08 -2.05786E-03 -1.29502E-12 8.65615E-07 3.900 -2.28321E-06 1.40047E-08 -2.53788E-03 -1.43866E-12 4.19371E-07 4.000 -2.44939E-06 -1.29275E-08 -3.01562E-03 -1.63481E-12 -1.15853E-07 4.000 -2.49152E-06 -4.09847E-08 -3.55487E-03 -1.63481E-12 -1.15853E-07 4.000 -2.49152E-06 -6.78070E-08 -4.18684E-03 -1.73712E-12 9.05732E-08 4.300 -2.17257E-06 -9.09880E-08 -4.89689E-03 -1.85996E-12 6.20531E-07 4.000 -1.81968E-06 -1.08426E-07 -5.62790E-03 -2.04565E+12 1.32824E-06 4.500 -1.3653E-06 -1.18607E-07 -6.29894E-03 -2.31628E-12 2.00057E-06 4.600 -8.48113E-07 -1.20789E-07 -7.18167E-03 -2.87445E-12 2.50355E-06 4.600 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06 4.800 1.82756E-07 -5.77849E-08 -7.30287E-03 -3.12378E-12 1.222455E-06 4.800 1.82756E-07 -5.77849E-08 -7.30895E-03 -3.12378E-12 1.4270E-06 5.000 8.96614E-07 -5.77849E-08 -7.3572E-03 -3.12378E-12 1.14270E-06 5.000 8.9651E-07 -5.77849E-08 -7.30837E-03 -3.287845E-12 1.14270E-06 5.000 8.9651E-07 -5.77849E-08 -7.30837E-03 -3.27371E-12 7.74811E-07 5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 7.27464E-07 5.300 9.29264E-07 3.51693E-08 -7.31437E-03 -3.03939E-12 1.01884E-06 5.400 5.404 5.407462E-07 -7.31437E-03 -3.297112E-12 1.54010E-06 5.400 5.40442E-07 5.76492E-08 -7.30494E-03 -2.97112F-12 1.54010E-06 5.400 5.40462E-07 5.51693E-08 -7.31437E-03 -3.29731E-12 1.01884E-06 5.400 5.40462E-07 5.51693E-08 -7.31437E-03 -3.29731E-12 1.01884E-06 5.400 5.400 5.4046E-07 5	3.200	1.008946-04	8.354786-08	2.71244=-03	-3.886626-13	-1.374845-04
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3.900 -2.28321E-06 1.40047E-08 -2.53788E-03 -1.43886E-12 4.19371E-07 4.000 -2.44939E-06 -1.29275E-08 -3.01562E-03 -1.56280E-12 2.77349E-08 4.100 -2.49152E-06 -4.09847E-08 -3.55487E-03 -1.63481E-12 -1.15853E-07 4.200 -2.39987E-06 -6.78070E-08 -4.18684E-03 -1.73712E-12 9.05732E-08 4.300 -2.17257E-06 -9.09880E-08 -4.89689E-03 -1.85996E-12 6.20531E-07 4.400 -1.81968E-06 -1.08426E-07 -5.62790E-03 -2.04565E+12 1.32824E-06 4.500 -1.36553E-06 -1.18607E-07 -6.29894E-03 -2.31628E-12 2.00057E-06 4.600 -8.48113E-07 -1.20789E-07 -6.83270E-03 -2.31628E-12 2.00057E-06 4.600 -8.48113E-07 -1.20789E-07 -7.34386E-03 -2.387445E-12 2.50355E-06 4.800 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06 4.900 5.98614E-07 -8.24305E-08 -7.30237E-03 -3.287845E-12 1.69239E-06 5.000 8.966251E-07 -5.77849E-08 -7.30237E-03 -3.28734E-12 1.69239E-06 5.100 1.05358E-06 -2.91415E-08 -7.25754E-03 -3.27371E-12 7.74811E-07 5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 7.27464E-07 5.400 6.64342E-07 6.73803E-08 -7.31437E-03 -2.09839E-12 1.01884E-06 5.400 6.64342E-07 -3.7869E-08 -7.39034E-03 -3.09839E-12 1.01884E-07 5.400 6.64342E-07 -3.51693E-08 -7.31437E-03 -3.09839E-12 1.01884E-06 5.400 6.66432E-07 -3.7869E-08 -7.31437E-03 -3.09839E-12 1.01884E-06 5.400 6.66432E-07 -3.51693E-08 -7.31437E-03 -3.09839E-12 1.01884E-06 5.400 6.664342E-07 -3.51693E-08 -7.31437E-03 -3.09389E-12 1.01884E-06 5.400 6.66432E-07 -3.51693E-08 -7.31437E-03 -3.09389E-12 1.01884E-06 5.400 6.66432E+07 -3.51693E-08 -7.31437E-03 -3.09839E-12 1.01884E-06 5.400 6.664342E+07 -3.51693E-08 -7.31437E-03 -3.09839E-12 1.01884E-06 5.400 6.664342E+07 -3.51693E-08 -7.31437E-03 -3.09839E-12 1.01884E-06 5.400 6.664342E+07 -3.51693E-08 -7.31437E-03 -3.09839E-12 1.01884E-06 5.400 5.604342E+07 -3.51693E-08 -7.31437E+03 -3.09839E+12 1.01884E+06 5.4	3.800	-2.006395-06	3.789426-08	-2.057865-03	-1.295026-12	8.656151-07
4.000 -2.44939E-06 -1.29275E-08 -3.01562E-03 -1.56280E-12 2.77349E-08 4.100 -2.49152E-06 -4.09847E-08 -3.55487E-03 -1.63481E-12 -1.15853E-07 4.200 -2.39987E-06 -6.78070E-08 -4.18684E-03 -1.73712E-12 9.05732E-08 4.300 -2.17257E-06 -9.09880E-08 -4.89689E-03 -1.85996E-12 6.20531E-07 4.400 -1.81968E-06 -1.08426E-07 -5.62790E-03 -2.04565E+12 1.32824E-06 4.500 -1.36553E-06 -1.18607E-07 -6.29894E-03 -2.31628E-12 2.00057E-06 4.600 -8.48113E-07 -1.20789E-07 -6.83270E-03 -2.31628E-12 2.43321E-06 4.700 -3.15062E-07 -1.15009E-07 -7.18167E-03 -2.87445E-12 2.50355E-06 4.800 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06 4.900 5.98614E-07 -5.77849E-08 -7.36237E-03 -3.287845E-12 1.69239E-06 5.000 8.96251E-07 -5.77849E-08 -7.30895E-03 -3.27371E-12 7.74811E-07 5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 7.27464E-07 5.300 9.29264E-07 3.51693E-08 -7.31437E-03 -3.03839E-12 1.01884E-06 5.400 6.64342E-07 6.7803E-08 -7.31437E-03 -3.03839E-12 1.01884E-07 5.400 6.664342E-07 7 7.14952E-08 -7.31437E-03 -3.03839E-12 1.01884E-06 5.400 1.05558E-06 -2.91415E-08 -7.25722E-03 -3.15736E-12 7.27464E-07 5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 7.27464E-07 5.400 6.664342E-07 6.78032E-08 -7.31437E-03 -3.03839E-12 1.01884E-06 5.400 5.400 5.9667E-07 7 5.1693E-08 -7.31437E-03 -3.0389E-12 1.01884E-06 5.400 5.400 5.400 5.9672E-08 -7.31437E-03 -3.0389E-12 1.01884E-06 5.400 5.400 5.400 5.4000 5.9672E-08 -7.31437E-03 -3.0389E-12 1.01884E-06 5.400 5.400 5.4000 5.9672E-08 -7.31437E-03 -3.0389E-12 1.01884E-06 5.400 5.400 5.4000 5.9672E-08 -7.31437E-03 -3.0389E-12 1.01884E-06 5.400 5.400 5.4000 5.9672E-08 -7.39034E-03 -7.297112E-12 1.50010E-06 5.400 5.400 5.4000 5.9672E-08 -7.39034E-03 -7.297112E-12 1.50010E-06 5.400 5.400 5.4000 5.9672E-08 -7.39034E-03 -7.297112E-12 1.50010E-06 5.400 5.400 5.4000 5.4000 5.7000E-08 -7.39034E-03 -7.297112E-12 1.50010E-06 5.400 5.4000 5.4000 5.7000E-08 -7.39034E-03 -7.297112E-12 1.50010E-06 5.400 5.4000 5.4000 5.7000E-08 -7.39034E-03 -7.297112E-12 1.50010E-08	3.900	-2.283218-06	1.400476-08	-2-53788E-03	-1-438866-12	4.19371E-07
<ul> <li>4.100 -2.49152E-06 -4.09847E-08 -3.55487E-03 -1.63481E-12 -1.15853E-07</li> <li>4.200 -2.39987E-06 -6.78070E-08 -4.18684E-03 -1.73712E-12 9.05732E-08</li> <li>4.300 -2.17257E-06 -9.09880E-08 -4.89689E-03 -1.85996E-12 6.20531E-07</li> <li>4.400 -1.81968E-06 -1.08426E-07 -5.62790E-03 -2.04565E-12 1.32824E-06</li> <li>4.600 -8.48113E-07 -1.20789E-07 -6.29894E-03 -2.31628E-12 2.00057E-06</li> <li>4.600 -8.48113E-07 -1.20789E-07 -6.383270E-03 -2.387345E-12 2.43321E-06</li> <li>4.700 -3.15062E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06</li> <li>4.800 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06</li> <li>4.900 5.98614E-07 -8.24305E-08 -7.30237E-03 -3.28784E-12 1.69239E-06</li> <li>5.000 8.96251E-07 -5.77849E-08 -7.30895E-03 -3.27371E-12 7.74811E-07</li> <li>5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 7.27464E-07</li> <li>5.300 9.29264E-07 3.51693E-08 -7.31437E-03 -2.97112E-12 1.01884E-06</li> </ul>	4.000	-2.203210-00	-1.29275E-08	-3-015626-03	-1.562805-12	2.77369F+08
<ul> <li>4.200 -2.39987E-06 -6.78070E-08 -4.18684E-03 -1.73712E-12 9.05732E-08</li> <li>4.300 -2.17257E-06 -9.09880E-08 -4.89689E-03 -1.85996E-12 6.20531E-07</li> <li>4.400 -1.81968E-06 -1.08426E-07 -5.62790E-03 -2.04565E+12 1.32824E-06</li> <li>4.500 -1.36553E-06 -1.18607E-07 -6.29894E-03 -2.31628E-12 2.00057E-06</li> <li>4.600 -8.4813E-07 -1.20789E-07 -6.83270E-03 -2.58533E-12 2.43321E-06</li> <li>4.700 -3.15062E-07 -1.15009E-07 -7.18167E-03 -2.87445E-12 2.50355E-06</li> <li>4.800 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06</li> <li>4.900 5.98614E-07 -8.24305E-08 -7.30237E-03 -3.28784E-12 1.69239E-06</li> <li>5.000 8.96651E-07 -5.77849E-08 -7.25754E-03 -3.27371E-12 7.74811E-07</li> <li>5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 1.01884E-06</li> <li>5.400 6.64342E-07 6.7803E-08 -7.31437E-03 -2.97112E-12 1.01884E-06</li> </ul>	4.100	-2.491525-06	-4.098478-08	-3.554875-03	-1-636816-12	-1.15853E-07
<b>4.300</b> -2.17257E-06 -9.09806-08 -4.89689E-03 -1.85996E-12 6.20531E-07 <b>4.400</b> -1.81968E-06 -1.08426E-07 -5.62790E-03 -2.04565E-12 1.32824E-06 <b>4.500</b> -1.36553E-06 -1.18607E-07 -6.29894E-03 -2.31628E-12 2.00057E-06 <b>4.600</b> -8.48113E-07 -1.20789E-07 -6.83270E-03 -2.58533E-12 2.43321E-06 <b>4.700</b> -3.15062E-07 -1.15009E-07 -7.18167E-03 -2.87445E-12 2.50355E-06 <b>4.800</b> 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06 <b>4.900</b> 5.98614E-07 -8.24305E-08 -7.36237E-03 -3.28784E-12 1.69239E-06 <b>5.000</b> 8.96251E-07 -5.77849E-08 -7.30895E-03 -3.32538E-12 1.14270E-06 <b>5.100</b> 1.05358E-06 -2.91415E-08 -7.25754E-03 -3.27371E-12 7.74811E-07 <b>5.200</b> 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 7.27464E-07 <b>5.300</b> 9.29264E-07 3.51693E-08 -7.31437E-03 -2.97112E-12 1.56010E-06	4.200	-2.491526-00	-6 790706-08		-1 737125-12	9 057376-08
<ul> <li>4.400</li> <li>-1.81968E-06</li> <li>-1.08426E-07</li> <li>-6.29894E-03</li> <li>-2.31628E-12</li> <li>2.00057E-06</li> <li>4.600</li> <li>-8.48113E-07</li> <li>-1.20789E-07</li> <li>-6.83270E-03</li> <li>-2.58533E-12</li> <li>2.43321E-06</li> <li>4.600</li> <li>-3.15062E-07</li> <li>-1.01891E-07</li> <li>-7.34386E-03</li> <li>-2.87445E-12</li> <li>2.50355E-06</li> <li>4.800</li> <li>-1.82756E-07</li> <li>-1.01891E-07</li> <li>-7.34386E-03</li> <li>-2.87445E-12</li> <li>2.50355E-06</li> <li>4.800</li> <li>-5.98614E-07</li> <li>-8.24305E-08</li> <li>-7.36237E-03</li> <li>-3.28784E-12</li> <li>1.69239E-06</li> <li>-000</li> <li>8.96251E-07</li> <li>-5.77849E-08</li> <li>-7.30895E-03</li> <li>-3.3253E-12</li> <li>1.14270E-06</li> <li>5.100</li> <li>1.05358E-06</li> <li>-2.91415E-08</li> <li>-7.25754E-03</li> <li>-3.15736E-12</li> <li>7.27464E-07</li> <li>-5.1693E-08</li> <li>-7.31437E-03</li> <li>-3.0339E-12</li> <li>1.01884E-07</li> <li>-5.400</li> <li>-6.4342E-07</li> <li>-5.1693E-08</li> <li>-7.39145E-03</li> <li>-2.97112E-12</li> <li>-5.4010E-04</li> </ul>	4.300	-2.333676-06	-0.008806-08	-4.996896-03	-1.859946-17	A.205316-07
4.500 -1.36553E-06 -1.18607E-07 -6.29894E-03 -2.31628E-12 2.00057E-06 4.600 -8.48113E-07 -1.20789E-07 -6.29894E-03 -2.31628E-12 2.00057E-06 4.700 -3.15062E-07 -1.15009E-07 -7.18167E-03 -2.87445E-12 2.50355E-06 4.800 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06 4.900 5.98614E-07 -8.24305E-08 -7.36237E-03 -3.12378E-12 1.69239E-06 5.000 8.96251E-07 -5.77849E-08 -7.30895E-03 -3.33253E-12 1.14270E-06 5.100 1.05358E-06 -2.91415E-08 -7.25754E-03 -3.27371E-12 7.74811E-07 5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 7.27464E-07 5.300 9.29264E-07 3.51693E-08 -7.31437E-03 -2.97112E-12 1.01884E-06	4.500	-1 819685-06	-).084746-07	-5-627906-03	-2.045456-12	1.328246-04
<b>4.600</b> -8.48113E-07 -1.20789E-07 -6.83270E-03 -2.58533E-12 2.43321E-06 <b>4.700</b> -3.15062E-07 -1.01891E-07 -7.18167E-03 -2.87445E-12 2.50355E-06 <b>4.800</b> 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06 <b>4.900</b> 5.98614E-07 -8.24305E-08 -7.30895E-03 -3.12378E-12 1.69239E-06 <b>5.000</b> 8.96651E-07 -5.77849E-08 -7.30895E-03 -3.33253E-12 1.14270E-06 <b>5.100</b> 1.05358E-06 -2.91415E-08 -7.25754E-03 -3.27371E-12 7.74811E-07 <b>5.200</b> 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 1.01884E-06 <b>5.400</b> 6.64342E-07 6.7893E-08 -7.31437E-03 -2.97112F-12 1.54010F-06	4.500	-1.345536-04	-1.186075-07	-6.298945-03	-2.316295-12	2.000575+06
4.700 -3.15062E-07 -1.15009E-07 -7.18167E-03 -2.87445E-12 2.50355E-06 4.800 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06 4.900 5.98614E-07 -8.24305E-08 -7.36237E-03 -3.28784E-12 1.69239E-06 5.000 8.96251E-07 -5.77849E-08 -7.30895E-03 -3.33253E-12 1.14270E-06 5.100 1.05358E-06 -2.91415E-08 -7.25754E-03 -3.27371E-12 7.74811E-07 5.200 1.06309E-06 2.28972E-09 -7.2572E-03 -3.15736E-12 7.27464E-07 5.300 9.29264E-07 3.51693E-08 -7.31437E-03 -3.03839E-12 1.01884E-06 5.400 6.64342E-07 6.78962E-08 -7.39034E-03 -2.97112E-12 1.56010E-06	4.600	-A.481135-07	-1.207896-07	-6.832705-03	-2.585336-12	2.433216-04
<b>4.800</b> 1.82756E-07 -1.01891E-07 -7.34386E-03 -3.12378E-12 2.21435E-06 <b>4.900</b> 5.98614E-07 -8.24305E-08 -7.36237E-03 -3.28784E-12 1.69239E-06 5.000 8.96251E-07 -5.77849E-08 -7.30895E-03 -3.33253E-12 1.14270E-06 5.100 1.05358E-06 -2.91415E-08 -7.25754E-03 -3.27371E-12 7.74811E-07 5.200 1.06309E-06 2.28972E-09 -7.2572E-03 -3.15736E-12 7.27464E-07 5.300 9.29264E-07 3.51693E-08 -7.31437E-03 -3.03839E-12 1.01884E-06 5.400 6.64342E-07 6.78962E-08 -7.39034E-03 -2.97112E-12 1.56010E-06	4.700	-3-150A2E-07	-1.150096-07	-7.181A7E-03	-2-874455-12	2.503555-04
4.900 5.98614E-07 -8.24305E-08 -7.36237E+03 -3.28784E+12 1.69239E-06 5.000 8.96251E-07 -5.77849E-08 -7.30895E-03 -3.33253E+12 1.14270E-06 5.100 1.05358E-06 -2.91415E-08 -7.25754E-03 -3.27371E+12 7.74811E-07 5.200 1.06309E-06 2.28972E-09 -7.2572E+03 -3.15736E+12 7.27464E+07 5.300 9.29264E-07 3.51693E+08 -7.391437E+03 -3.03839E+12 1.01884E+06 5.400 6.64342E+07 6.78962E+08 -7.39034E+03 -2.97112E+12 1.56010E+06	6.800	1.827566-07	-1.01891F-07	-7.34386F-03	-3-12378F-12	2.214356-04
5.000 8.96251E-07 -5.77849E-08 -7.30895E-03 -3.33253E-12 1.16276540 5.100 1.05358E-06 -2.91415E-08 -7.30895E-03 -3.27371E-12 7.74811E-07 5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 7.27464E-07 5.300 9.29264E-07 3.51693E-08 -7.31437E-03 -3.03839E-12 1.01884E-06 5.400 6.64342E-07 6.78962E-08 -7.39034E-03 -2.971125-12 1.54010E-04	4.000	5.986145-07	-8.243056-09	-7.362376-03	-3.287944-12	1.697396-04
5.100 1.05358E-06 -2.91415E-08 -7.25754E-03 -3.27371E-12 7.74811E-07 5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 7.27464E-07 5.300 9.29264E-07 3.51693E-08 -7.31437E-03 -3.03839E-12 1.01884E-06 5.400 6.64342E-07 6.78962E-08 -7.39034E-03 -2.971125-12 1.54010E-04	5.000	8.962515-07	-5.778405-08	-7.308955-03	-1.112636-12	1.142705-04
5.200 1.06309E-06 2.28972E-09 -7.25722E-03 -3.15736E-12 7.27464E-07 5.300 9.29264E-07 3.51693E-08 -7.31437E-03 -3.03839E-12 1.01884E-06 5.400 6.64342E-07 6.78962E-08 -7.39034E-03 -2.97112E-12 1.54010E-04	5.100	1.053585-04	-2.914156-08	-7 257548-03	-3.273716-12	7.748116-07
5.300 9.29264E-07 3.51693E-08 -7.31437E-03 -3.03839E-12 1.01884E-06 5.400 6.64342E-07 6.78962E-08 -7.39034E-03 -2.97112E-12 1.54010E-04	5.200	1.063095-04	2.289776-00	-7.257228-03	-3.157345-13	7.774445-07
5.400 6.64342F=07 6.78962E=08 =7.39034E=03 =7.97112E=12 1.54010E=06	5.300	9.292445-07	3.516036-09	-7.314376-03	-1.038304-12	1.018845-04
	5.400	6.643425-07	6.78962E-08	-7.390346-03	-7-971126-12	1.54010E-0A

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TINE	P~ 6	P- 7	P~ 8	P- 9	P-10
0.	0.	0.	0.	0.	0.
0.100	-1.94319E-14	8.28623E-09	-1.13762E-08	-7-123828-03	8.50400E-09
0.200	-2.19317E-13	1.26056E-07	-1.18626E-07	-2.690398-02	1.29169E-07
0.300	-7.45905E-13	6.15036E-07	-3.46552E-07	-5.51806E-02	6.29427E-07
0.400	~1.68913E-12	1.91924E-06	-6.23997E-07	-8.66799E-02	1.96249E-06
0.500	-3.28076E-12	4.692536-06	-9.27798E-07	-1.16861E-01	4.79523E-06
0.600	-5.29224E-12	9.55808E-06	-9.39705E-07	-1.38640E-01	9.76C14E-06
0.700	-6.82048E-12	1.63901E-05	-1.16733E-07	-1.44523E-01	1.672148-05
0.800	-9.07251E-12	2.499328-05	4.97229E-07	-1.41739E-01	2.54736E-05
0.900	-1.13408E-11	3.47732E-05	1.07334E-06	-1.40256E-01	3.53972E-05
1.000	-1.24253E-11	4.488Z1E-05	2.10899E-06	-1.49088E-01	4.561591-05
1.100	-1.234/0E-11	5.44820E-05	3.09//42-00	-1.72988E-01	3.3268UE-03
1.200	-1+100222-11	0.2/10/1-03	3.112426-00	-2.100/12-01	0.3400YE-U3
1.500	-7.002712-12	7 310345-05	4. E0704E-06	-7 944916-01	7 750745-05
1.500	-T+17013E-12	7 254225-05	4.530376-06	-3 247095-01	7 262146-05
1.600	7.17985F-17	7.009906-05	4.22500E=06	-3.339606-01	6-978795-05
1.700	3.44258F-11	6.501605-05	3.844745-06	-3.24215E-01	6.47753E-05
1.800	2-16376E-11	5-76936F-05	3.323226-06	-3-01344F-01	5-65215E-05
1.900	2.82086E-11	4-85597E-05	2-69440E-06	-2.74974E-01	6-69907E-05
2.000	3.36600E-11	3.80193E-05	2.071538-06	-2.55020E-01	3.61227E-05
Z.100	3.737036-11	2.64801E-05	1.41646E-06	-2.48126E-01	2.43565E-05
2.200	3.881768-11	1.43691E-05	6.64701E-07	-2.55289E-01	1.21430E-05
2.300	3.77349E-11	2.16495E-06	-1.43630E-07	-2.71515E-01	-2.21012E-08
2.400	3.394176-11	-9.55097E-06	-9.71585E-07	-2.87642E-01	-1.15463E-05
2.500	2.73769E-11	-2.00967E-05	-1.79795E-06	-2.93655E-01	-2.17446E-05
2.600	1.82455E-11	-2.87560E-05	-2.53091E-06	-2.82320E-01	-2.99076E-05
2.700	6.96276E-12	-3.48704E-05	-3.05519E-06	-2.51826E-01	-3.53942E-05
2.800	-5.90547E-12	-3.79456E-05	-3.30677E-06	-2.06479E-01	-3.77392E-05
2.900	-1.95897E-11	-3.77545E-05	-3.25350E-06	-1.55220E-01	-3.67580E-05
3.000	-3.31531E-11	-3.43912E-05	~2.89220E-06	-1.08479E-01	-3.25979E-05
3.100	-4.56063E-11	-2.825491-05	-2.281841-06	-7.450408-02	-2.5/184E-05
3.200	-3.390472-11	-1.997955-05	~1.52265E~U0	-3.048/3E-U2	-1.0815/1-05
3.400	-0.3300/2-11	-1.032335-03	7 360805-08	-5.130186-02	-0.10/092-00
3.500	~0.007/40-11	1 013846-05	7 704985-07	-5.104295-02	3.191132-00
3.600	-6.113755-11	1.972206-05	1 365875-04	-7.910095-02	2 327875-05
3.700	-5.168776-11	2-82705E-05	1-873026+06	5.549195-03	3-129585-05
3.800	-3.83177F-11	3-54519E-05	2-31074E-06	5-46068E-02	3.771966-05
3.900	-2.17368E-11	4-09707E-05	2.68628E-06	1.10263E-01	4.22962E-05
4.000	-2.89161E-12	4.45367E-05	2.98605E-06	1.62178E-01	4.47906E-05
4.100	1.70803E-11	4.58740E-05	3.16962E-06	2.01015E-01	4.49896E-05
4.200	3.69156E-11	4.47709E-05	3.18007E-06	2.21717E-01	4.275056-05
4.300	5.53011E-11	4.11570E-05	2.96747E-06	2.25244E-01	3.80749E-05
4.400	7.09463E-11	3.51799E-05	2-50990E-06	2-18141E-01	3.11823E-05
4.500	8.26643E-11	2.72491E-05	1.82714E-06	2.10119E-01	2.25507E-05
4.600	8-94602E-11	1.80272E-05	9.852518-07	2.10502E-01	1.29028E-05
4.700	9.06146E-11	8.35949E-06	8.60371E-08	2.24824E-01	3.13057E-06
4.800	8-57554E-11	-8.39999E-07	-7.563318-07	2.52795E-01	-5.82496E-06
4.900	7.49125E-11	-8.72091E-06	-1.44131E-06	2.88364E-01	-1.31101E-05
5.000	2.824446-11	-1.40103E-05	-1,90298E-06	3-21856E-01	-1.80752E-05
5.100	3+/32036-11	-1.808042-05	-2.120946-06	3.434201-01	~2.03428E-05
5 300	1+310046-11	-1.0377/96-05	-1 939385-04	3.4075(E-01	-1.9812/6-05
5.400	-11-300611	-1-321146-05	-1.636795-04	J. JUJJZE-UL 7. 999405-01	-1.001435-04
20.000	J470373C-11		***30.05-00	C. 77707C-VI	-1+101046-03

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TIME	P-11	P-12	P-13	P-14	P-13
ο.	0.	0.	°0.	0.	
0.100	-1.16666E-08	-7.12382E-03	7.32265E-06	7.32265E-06	
0.200	-1.24643E-07	-2.690398-02	2.58478E-05	2.58478E-05	
0.300	-3.834888-07	-5.51806E-02	4.66060E-05	4.66060E-05	
0.400	-7.56031E-07	-8.66799E-02	5.83141E-05	5.83141E-05	
0.500	-1.27392E-06	-1.16861E-01	5.14596E-05	5-14596E-05	
0.600	-1.67971E-06	-1-38640E-01	1.66768E-05	1.66768E-05	
0.700	-1.44293E-06	-1.44523E-01	-5.06091E-05	-5.06091E-05	
0.800	-1.556228-06	-1.41739E-01	-1.33669E-04	-1.33669E-04	
0.900	-1.79396E-06	-1.40256E-01	-2.09749E-04	-2.09749E-04	
1.000	-1.61543E-06	-1-49088E-01	-2.58242E-04	-2.582428-04	•
1.100	-1.44880E-06	-1.72988E-01	-2.68008E-04	-2.68008E-04	
1.200	-1.52265E-06	-2-10671E-01	-2.41317E-04	-2.41317E-04	
1.300	-1.50622E-06	-2.55214E-01	-1.92961E-04	-1.92961E-04	
1.400	-1.43886F-06	-2.96481E-01	-1.44903E-04	-1.44903E-04	
1.500	-1.53021E-06	-3.24709E-01	-1.18343E-04	-1.18343E-04	
1.600	-1.61863E-06	-3.33960E-01	-1.260828-04	-1.26082E-04	
1.700	-1.56711E-06	-3.24215E-01	-1.67905E-04	-1.67905E-04	
1.800	-1.47348E-06	-3-01344E-01	-2.30644E-04	-2.30644E-04	
1.900	-1.33319E-06	-2.74974E-01	-2.92893F-04	-2.92893E-04	
2.000	-1.07187E-06	-2.55020E-01	-3.32681E-04	-3.32681E-04	
2.100	-7-65363E-07	-2-48126E-01	-3-35351F-04	-3.35351E-04	
2.200	-5.06355E-07	-2.55289E-01	-2.98816E-04	-2.98816E-04	
2.300	-2.92609E-07	-2.71515E-01	-2.34310E-04	-2.34310E-04	
2.400	-1.40261E-07	-2.87642E-01	-1.62365E-04	-1.62365E-04	
2.500	-8.42543E-08	-2.93655E-01	-1.05464E-04	-1.05464E-04	
2.600	~9.09515E-08	-2.82320E-01	-8.00088E-05	-8.000886-05	
2.700	-1.04288E-07	-2.51826E-01	-9.04718E-05	-9.04718E-05	
2.800	-1.04423E-07	-2.06479E+01	-1.27948E+04	-1.27848E-04	
2.900	-7.64656E-08	-1.55220E-01	-1.72938E-04	-1.72938E-04	
3.000	-8.70546E-09	-1.08479E-01	-2.03300E-04	-2.03300E+04	
3.100	7.33977E-08	-7.45040E-02	-2.01354E-04	-2.01354E-04	
3.200	1.258476-07	-5.64873E-02	-1.60750E-04	-1.60750E-04	
3.300	1.19456E-07	-5.15018E-02	-8.86995E-05	-8.86995E-05	
3.400	3.83086E-08	-5.16429E-02	-3.449908-06	-3.44989E-06	
3.500	-1.17772E-07	-4.69564E-02	7.22006E-05	7.22006E-05	
3.600	-3.19809E-07	-2.91009E-02	1.19012E-04	1.19012E-04	
3.700	-5.20078E-07	5.56919E-03	1.28127E-04	1.28127E-04	
3-800	-6-744278-07	5.46C68E-02	1.04173c-04	1.04173E-04	
3.900	-7.52093E-07	1.10263E-01	6.35157E-05	6.35157E-05	
4.000	-7.42814E-07	1.62178E-01	2.83114E-05	2.83114E-05	
4.100	-6.63981E-07	2.01015E-01	1.848696-05	1.84869E-05	
4.200	-5.53891E-07	2.21717E-01	4.45252E-05	4.452528-05	
4.300	-4.55242E-07	2.25244E-01	1.03648E-04	1.03648E-04	
4-400	-4.01785E-07	2.181416-01	1.80763E-04	1.80763E-04	
4-500	-4.08536E-07	2.10119E-01	2.53825E-04	2.53825E-04	
4.600	-4.66154E-07	2.10502E-01	3.017022-04	3.01702E-04	
4.700	-5.45704E-07	2-24824E-01	3.11706E-04	3.11706E-04	
4.800	-6.11903E-07	2.52795E-01	Z-84096E-04	Z-84096E-04	
4.900	-0.3/0998-07	2-883646-01	2.319426-04	2.319428-04	
>.000	-6.121768-07	3+21856E-01	· 1.76401E-04	1.76401E-04	
5.100	-5.517486-07	3.434206-01	1.390975-04	1.390976-04	
5.200	-4.901125-01	3.465376-01	1-343491-04	1.343496-04	
5.300	-4-9AT095-01	3.303526-01	1+040342-04	1.640346-04	
<b>5.400</b>	-2-233/76-07	2.999696-01	2.169176-04	2.16917E-04	

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TIME	٧X	٧Y	٧Z	OMEGAX	OMEGAY	OMEGAZ
SEC	IN/SEC	IN/SEC	IN/SEC	RAD/SEC	RAD/SEC	RAD/SEC
		•				
5.500	-1.1063E-03	-2.2509E-14	2.5915E-01	4.4642E-12	8.2793E-04	-4.4481E-16
5.600	-1.1278E-03	9.5557E-14	2.5915E-01	4.89256-12	8.2793E-04	-6.1358E-16
5.700	-1.1492E-03	2.2331E-13	2.5915E-01	5.1620E-12	8.2793E-04	-1.0278E-15
5.800	-1.1707E-03	3.57.01E-13	2.5915E-01	5.2801E-12	8.2793E-04	-1.3757E-15
5.900	-1.1921E-03	4.9277E-13	2.5915E-01	5.2323E-12	8.2793E-04	-1.5837E-15
6.000	-1.2136E-03	6.2622E-13	2.5915E-01	5.0364E-12	8.27936-04	-1.7126E-15
6.100	-1.2350E-03	7.52796-13	Z.5915E-01	4.6192E-12	8.27936-04	-2.42/56-15
0.200	-1.23636-03	8.68/UE-13	2.59158-01	4.132/E-12	8.27935-04	-2.53676-15
4 400	-1.27002-03	9.1221E-13	2.59150-01	2.04935-12	0.27935-04	-2.93076-15
A 500	-1.29745-03	1.00420-12	2.59155-01	3.29292-12	8-21936-04	-3.23006-13
6.600	-1.36236-03	1.23506-12	2.59156-01	3 33446-13	8 77935-04	-3.96956-15
6.700	-1.3638E-03	1.37756-12	2.59156-01	3.72556-12	8 27035-04	-3.040326-15
6.800	-1.38526-03	1.43416-12	2.59156-01	4.44406-12	8.27935-04	-4.5000E-15
6.900	-1.40678-03	1.55888-12	2.55156-01	5.19496-12	8.27935-04	-4.91006-15
7.000	-1.4281E-03	1.7023E-12	2.5915E-01	5.8663F-12	8.2793E-04	-5.26798-15
7.100	-1.4496E-03	1.8615E-12	2.5915E-01	6.3605E-12	8.2793E-04	-5.87636-15
7.200	-1.4711E-03	2.0304E-12	2.5915E-01	6.574JE-12	8.2793E-04	-6.3427E-15
7.300	-1.4925E-03	2.2011E-12	2.5915E-01	6.44772-12	8.2793E-04	-6.76482-15
7.400	-1.5140E-03	2.3643E-12	2.5915E-01	5.9449E-12	8.2793E-04	-7.1276E-15
7.500	-1.5354E-03	2.5102E-12	2.5915E-01	5.03536-12	8.2793E-04	-7.61255-15
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7.700	-1.5783E-03	2.7061E-12	2.5915E-01	1.9010E-12	8.2793E-04	-7.7995E-15
7.800	-1.5998E-03	2.7333E-12	2.5915E-01	-2.9491E-13	8-2793E-04	-7.7767E-15
7.900	-1.6212E-03	2.6998E-12	2.5915E-01	-2.77876-12	8.27938-04	-7.7494t-15
8.000	-1.6427E-03	2.6001E-12	2.5915E-01	-5.3425E-12	8.2793E-04	-7.4653E-15
8.100	-1.66426-03	2.4347E-12	2.59156-01	-7.75976-12	8.2793E-04	-6.65901-15
8.200	-1.0000000-03	2.211/E-12	2.59158-01	-9.5713E-12	8.27935-04	-0.41516-15
8 400	-1.77855-03	1.990/0-12	2.59156-01	-1.07576-11	8-21935-04	-2.20136-15
8.500	-1.75006-03	1.37426-12	2.59155-01	-1.13530-11	8-21935-04	-4.07335-17
B-600	-1.77146-03	1.10928-12	2.50156-01	-9.43456-12	8 27936-04	-4.2/346-15
8.700	-1.7929E-03	8-80865-13	2.5915+-01	-7.79116-12	8.27936-04	-3.0305E-15
8.800	-1.8144E-03	6.99618-13	2.59156-01	-5.84235-12	8.27935-04	-2.33556-15
8.900	-1.8358E-03	5.70886-13	2.5915E-01	-3.77308-12	8.27935-34	-1.79656-15
9.000	-1.8573E-03	4.9661E-13	2.5915E-01	-1.67621-12	8.27936-04	-1.4478E-15
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9.200	-1.9002E-03	5.0922E-13	2.59156-01	2.3505E-12	8.2793E-04	-1.4251E-15
9.300	-1.9216E-03	5.9128E-13	2.5915E-01	4.1469E-12	8.2793E-04	-1.93356-15
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9.800	-2.0289E-03	1.4694E-12	2.5914E-01	7.6212E-12	8.2793E-04	-4.3354E-15
9.900	-2.0504E-03	1.662JE-12	2.5714E-01	7.10356-12	8.27936-04	-4.96038-15
10.000	-2.0718E-03	1.83796-12	2.5914E-01	6.3795E-12	8.27936-04	-5.4708E-15
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10.400	-7.15765-03	2.37495-12	2.07140-01	3.13672-12	0.21935-04	-3.95822-15
10.500	-7.17916-03	2.30575-12	2 50146-01	3+13412*12	9 37035-04	-1.42036-15
10.600	-2.20055-03	2.44945-12	2.59146-01	2+42076712	9 27015-04	-7 93455-15
10.700	-2.2220E-03	2.4886E-12	2.59148-01	1.24345-12	8.27936-04	-1.032020-13
10.800	-2.24355-03	2.51368-12	2.59145-11	7.75668-14	3.27936-04	-8-1870-15
10.900	-2.2649E-03	2.52918-12	2.5914E-01	4.17518-13	8.27935-04	-7.7952-15

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RUN NO 1

	TIME	P~ 1	P- 2	P~ 3	P- 4	P- 5
	5.500	2-84101E-07	9-85303F-08	-7-41550F-03	-7-94136F~17	2.094565-06
	5.600	-1-94807E-07	1.248295-07	-7.31466F-03	-2.92895F-12	2.468405-06
	5.700	-7.55182E-07	1.444346-07	-7-03461E-03	-2.90953E-12	2-50748E-06
	5-800	-1-37751E-06	1.551956-07	-6.55405E-03	-7.833746-12	2-17280E-06
	5.900	-2-03720F-06	1-555316-07	-5-93860E-03	-2-61616F-12	1-55463E-06
	6.000	-2.70247E-06	1.44749E-07	-5.77926E-03	-2.20532E-12	8.40327E-07
	6.100	-3-33429E-06	1.23221E-07	-4-51829E-03	-1.71896E-17	2.473526-07
	6.200	-3-88913E-06	9.236366-08	-3.87129E-03	-1-21338E-12	-5.43677E-08
	6.300	-4-32425E-06	5.44403E-08	-3,31542E-03	-7.99989E-13	-2-980405-09
	6.400	-4-60428E-06	1.22234E-08	-2.83155E-03	-5.31255E-13	3-33603E-07
	6.500	-4.70723E-06	-3.13679E-08	-2.36318E-03	-4.53732E-13	7-82613E-07
•	6.600	-4.62832E-06	-7.35736E-08	-1.83883E-03	-4-99206E-13	1-128945-06
	6.700	-4.38029E-06	-1.11973E-07	-1.19989E-03	-5-942718-13	1.192826-06
	6.800	-3.99046E-06	-1.44525E-07	-4-23885E-04	~6.47324E-13	8.948136-07
	6.900	-3.49517E-06	-1.69512E-07	4-65145E-04	-6.41315E-13	2.84718E-07
	7.000	-2.93358E-06	-1.85456E-07	1.403006-03	-5-30768E-13	-4-75821E-07
	7.100	-2.34244E-06	-1.91086E-07	2.30603E-03	-2-69938E-13	-1-17086E-06
	7.200	-1.75325E-06	-1.85417F-07	3.09883E-03	9-542968-14	-1-60715F-06
	7.300	-1-19207E-06	-1-679316-07	3.73899F-03	3-94480F-13	-1.68287E-06
	7.400	-6.81428E-07	-1-38820E-07	4.23009E-03	6-26753E-13	-1-47366F-06
	7.500	-2.42838E-07	-9-92096E-08	4.61825E-03	7-01204F-13	-9.72788F-07
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	7.700	3-29500E-07	1.91518E-09	5.36263E-03	4.394676-13	-3-223676-07
	7.800	4.214768-07	5.65009E-08	5-82373E-03	2-599726-13	-4.397655-07
	7.900	3.652795-07	1-084145-07	6-35059E-03	2.59922F-13	-8-850156-07
	8.000	1.607276-07	1.537675-07	6.893715+03	5.131736-13	-1.529625-06
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	8.600	-2.49859E-06	1.712236-07	7 373625-03	3 489176-12	-1.037732-00
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	9.700	5-142036-07	-2.441975-07	2.527756-03	1 342276-12	- 3.030102-01
	9.800	1.120376-06	-2-133275-07	1.759226-03	5 268665-13	5 144845-07
	9,900	1.639376-06	-1.678415-07	1.099335-03	-1.486515-13	5.385115-07
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1	0.100	2.256501-04	-4-64413E-DA	2.062245-05	-6.784055-13	-1.71)305-07
1	0.200	2.30750F-04	2.15942E-08	-5.18554F-04	-4.961816-13	-5.334336-07
1	0.300	2.18322E-04	8.91347F-DR	-1,141936-03	-2.200026-13	-6-631525-07
i i i i i i i i i i i i i i i i i i i	0.400	1.90060E-06	1.523365-07	-1.877916-03	1.181096-14	-4.043005-07
	0.500	1.48730E-06	2.07601F-07	-2.70967E-03	8.381265-14	1.579296-07
•	0.600	9.76013F-07	2.516136-07	-3.57900F-03	-1-01876F-13	8-825685-07
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	0.800	-2.126455-07	2.945216-07	-5.11089F-03	-8.901055-13	2.030885-04
1	0.900	-8.337096-07	2.89278E-07	-5.64976E-03	-1.19549E-12	2.133796-06
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TIME P-6	P- 7	P~ 8	P- 9	2-10
5-500 -6 666565-1	1 -4 994445-04	-1 268795-06	2 666265-01	-3 366275-06
5.600 -8.595335-1	1 1 112505-04	-7 957636-07	2 343955-01	5 90004E-04
5.700 -1 025655-1	1 1.11330E-00 0 1 08484E-05	-7 3(0) 65-07	2.343036-01	1 660736-00
5.800 -1.130406-1	0 1.00400E-05	-2.34010E-01	2.121365-01	2 924006-06
5 600 -1 14492E-1	0 2.130135-05	4.239240-01	2+151546-01	2.03470CF03
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	0 1.001012-02	2.003402-00	2.201000-01	3.231030-03
4 300 -0 310155-1	0 J.10117E-U)	3.0004(2-00	2+293046-01	7 3314016-03
$6.200 - 6.21013E^{-1}$	1 0.001425-00	3.875.25-00	2+141336=01	1.33190E~03
6.300 -3.74374E*1		4.018032-00	1.305188-01	8.0000025-00
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		5.301422-06	1.6/0962-02	8.5//416-05
6.600 3.77507E~1	1 8.241/46-02	5.18042E-06	2.525918-02	8.33/35E-05
0.100 0.97943E~1	1 8.182528-05	4. 1845AL-08	-1.3/2188-02	1.192036-05
6.600 9.83/60E-1	L /.55512E-US	4.18831E-05	-3.61939E-02	0.99//52-05
6.900 1.21470E-1	0 0.71974E-05	3.480/1E-06	-4-4148/E-02	6.02684E-05
7.000 1.37363E-1	0 5.142678-05	2.74169E-06	-4.4/9/6E-02	4.955846-05
7.100 1.44/88E-1	0 4.68/185-05	2.024296-06	-4.73890E-02	3-85551E-05
7.200 1.430218-1	0 3.60361E-05	1.34390E-06	-6.21189E-02	2.785356-05
7.300 1.31936E-1	0 2.554248-05	1.10/518-07	- 3.192186-02	1.79316E-05
7.400 1.12025E-1	0 1.566666-05	9.55442E-08	-1.357962-01	9.18456E-06
7.500 8.43629E-1	1 6.882755-05	-4.955472-07	-1.357618-01	1.97335E-06
7.000 5.054948-1	1 -3.555818-07	-1.042996-06	-2.34786E-01	-3.34417E-06
7.700 1.20105E-1	1 -3.59/4/2-06	-1.474621-06	-2.70314E-01	-6.419128-06
7.800 -2.71166E-1	1 -9.409942-06	-1.782252-06	-2.87619E-01	-6.96096E-06
7.900 -0.611918-1	1 -8.59420E-06	-1.339515-06	-2.867911-01	-4.817226-06
8.000 -1.01857E-1	0 -3.893426-00	-1.52517E-08	-2.73643E-01	-5.61389E-08
8.100 -1.31921E-1	0 -3.185222-07	-1.140510-08	-2.5/5562-01	0.9/939E-00
		-9.321446-01	-2.44171E-01	1.50/146-05
		2.3-4406-01	-2.010098-01	2.510826-05
	3 373755-05	2 022116-06	-2.03//22-01	3-43300E-V3
	0 3.31213C-03	2.027116-00	-2.943420-01	4.299140-05
	1 4.14124C-V/	2.033.02-00	-3.334336-01	5 4 3 3 5 4 b - 3 5
A.800 -7.45435E-1	5 153075-05	3 331185-06	-3.336236-01	5 531356-05
8.900 -3.16798E-1	5 334175-05	3 3535436	-3.580316-01	5 525136-05
9,000 1,617685-1	5.29/032-05	3 3336006	-7.703126-01	5 217176-05
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9,200 1,03017E-10	A.54404E-05	2.935)36-05	-1.330636-01	3.050056-05
9.300 1.401585-1	3.857695-05	2 592556-06	-1 676108-01	3 059346-05
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9.500 1.874205-16	1.945421-05	1.415418-06	-1.470336-01	8.727C0E-06
9.600 1.94092F-10	7.973536-06	5-875251-07	-1.499498-01	-3.164116-06
9.700 1.882906-10	-3.957318-06	-3.456131-07	-1.469246-01	-1.479536-05
9.800 1.700996-10	-1.55149+-05	-1.304258-06	-1.293985-01	-2.534395-05
9.903 1.40404F-10	-2.581516-05	-2.178238-06	-9.544498-02	-3.397146-05
10.000 1.00859E-10	-3.40353E-CS	-2.3-5038-06	-4.517268-02	-3.99449E-05
10.100 5.38111E-1	-3.953648-05	-3.2-536E-06	1.021316-02	-4.27562E-05
10.203 2.16375E-1	2 -4.19501E-05	-3.407302-06	5.352781-02	-4.22026E-05
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10.500 -1.46912E-10	-3.13037E-05	-2.320936-05	1.372501-01	-2.29370E-05
10.600 -1.83598E-1	-2.304576-05	-1.699721-06	1.352146-01	-1.25487E-05
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10.800 -2.21536E-10	-2.431002-06	-3.773746-27	1.434968-01	1.0236CE-05
10.900 -2.199938-10	8.881476-06	3.051292-07	1.659938-01	2.15232E-05

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TIME	<b>9-11</b>	P-12	P-13	P-14	P-15
5.500	-6.67896E-07	2.64624E-01	2.72697E-04	2.726975-04	
5.600	-8.91425E-07	2-34385E-01	3.09309E-04	3-09309E-04	
5.708	-1.16014E-06	2.16566E+01	3-10854E-04	3-10854E-04	
5.800	-1-42657E-06	2-131345-01	2.73329E-04	2.73329E-04	
5.900	-1-64648E-06	2-20049E-01	2.06082E-04	2.06082E-04	
6.000	-1.781645-06	2.287685-01	1.284905-04	1.28490E-04	
6.100	-1.058056-06	2-293645-01	6.30434E-05	6.304345-05	
6 200	-1.855406-06	2.141536-01	2.732778-05	2.732776-05	
6.300	-1.848076-06	1.80518E-01	2.777126-05	2.77712E-05	
6.600	-1.841218-06	1.319146-01	5.740436+05	5.740435-05	
6 500	-1 847126-06	7 470946-02	0 843076-05	B #410#5-05	
6.600	-1 926526-06	2 525016-02	1.287096-04	1.287095-04	
6.700	-1.998176-06	-1.372185-02	1.297016-04	1.207016-04	
6 800	-1.990170-00	-1.5/2100-02	0 376405-05	9 374405-05	
4 900	-2.040071-00	-5.017576-02	7.J2047E-0J	7.32049C-03	
7 000	-1 946305-06	-4.470746-02	-5 954545-05	-5 954545-05	
7 100	-1.775945-04	-4.4(9)00-02	-1 368676-04	-) 348475-04	
7 200	-1.551555-06	-4. 211895-02	-1. 973035-04	-1 872025-04	
7 300	~1.331332-00	-0.211070-02	-2 003125-04	-2.003125-04	
7.500	-1.310000-06	-1.357066-01	-1 784025-04	-1 784025-04	
7.400	-0 003036-03	-1 847415-01	-1.764720-04	-1 346035-04	
7.500	-9.003030-07	-1.001010-01	-0 415375-05	-0 416375-06	
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7.700		-2.703148-01	-1.111136-05	-1.111132-03	
7.000	-1.040485-06	-2.8/0192-01	-9.3380/8-03	-9.3380/2-03	
7.900	-1+114026-06	-2.80/916-01	-1.424046-04	-1.424045-04	
8.000	-1.140202-00	-2.130486-01	-2.11003E-04	-2.110036-04	
8.100	-1-122090-06	-2.5/5802-01	-2.198222-04	-2.198222-04	
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8.400	-9.009000-07	-2.08//2E-01	-3.033046-04	-3.03304E~04	
8.300	-8.138316-01	-2.943426-01	-2.491832-04	-2.991836-09	
8.000	-9.020296-07	-3-192435-01	-1.802228-04	-1.802225-04	
8.700	-3.152806-01	-3.330236-01	-1.303436-04	-1.303436-04	
8.000	-1.039296-00	-3+304576-01	-1.212426-04	-1.212926-09	
0.900	-1.112335-06	-3.080812-01	-1.38512E-U4	-1.308122-09	
9.000	-1-090806-00	-2-708126-01	-1-818005-04	-1.81800E-04	
9.100	-9.932108-07	-2.2/429E-01	-2.311492-04	-2.311498-04	
9.200	-0.014456-01	-1.880026-01	-2.048//1-04	-2.045//2-04	
9.300	-2*034316-01	-1.60610E-01	-2.66002E-04	~2.66002E-04	
9.400	-3.238696-07	-1.4/9042-01	-2.286/7E-04	-2.280//E-04	
9.500	-1-152/16-07	-1.470385-01	-1.003285-04	~1.60328E-04	
9.000	2.301000-03	-1-44446-01	-1.903502-05	~7.90350E-05	
9.100	9-100445-08	-1.469242-01	-7.102428-06	-7.102402-06	
9.800	3.10348E-08	-1-298906-01	3.088552-45	3.688552-05	
9.900	1.0000000000	-9.344496-02	4.400UIE-US	4.40001E-05	
10.000	0.121816-08	-4.61/06E-02	2.115881-05	2.115876-05	
10.100	1.112016-08	1.021312-02	-1.132396-05	~1.732592-05	
10.200	1.281028-07	0.352/82-02	-4.90/03E-05	-4.90703E-05	
10.300	1-33535-01	1.046978-01	-2.4/883E-05	->.47882E-05	
10.400	2.38/361-07	1-289105-01	-2.46197E-05	-2.46197E-05	
10-200	2.211395-07	1.3/2505-01	3-82670E-05	3-82671E-05	
10.000	1.350826-07	1+30218E-01	1-18/516-04	1-187516-04	
TO*100		1.35303E-01	1.921205-04	1.951206-04	
10-800	-2.820512-07	1.434986-01	2.46803E-04	Z-46803E-04	
10.300	->.43580E-07	1.65993E-01	Z_61678E+04	2.61678E+04	

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TIME	¥X	VY .	٧Z	OMEGAX	OMEGAY	OMEGAZ
SEC	14/580	IN/SEC	IN/SEC	RAD/SEC	RAD/SEC	RAD/SEC
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11.000	~7.7864F-03	2.53736-12	2.59146-01	1.5380E-13	8-27935-04	-7-92226-15
11 100	-7 30786-03	2 54046-12	7 59145-01	-6 28205-14	8 27936-04	-7.97866-15
11 200	A2 23036-03	2.0707070712	2.59146-01	-3 07095-13	9 27025-04	-7 72656-16
11.200	-2.32736-03	2.000000-12	2.59146-01	-5.01700-15	9 27025-04	-7 81905-15
11.500	-2/33075-03	2.55100-12	2.59140-01	-0.00422-12	0.2/702 04	7 64336-16
11.400	-2.3122E-03	2.5132E-12	2.5914E-01	-1,10756-12	8.21935-04	-1.20336-13
11.500	-2.39362-03	2.48116-12	2.59146-01	-1.80286-12	8.27936-04	-7.5140E-15
11.600	-2.4151E-03	Z.4317E-12	2.5914E-01	~2,4/56E-12	8.2793E-04	-/.4419E-15
11.700	-2.4366E-03	2.3649E-12	2.5914E-01	-3.0319E-12	8.2793E-04	-7.35258-15
11.800	-2-458GE-03	2.2853E-12	2.5914E-01	-3.3124E-12	8.2793E-04	-7.3803E-15
11.900	-2.4795E-C3	2.20128-12	2.5914E-01	+3.2102E-12	8.2793E-04	-6.9123E-15
12.000	-2.5009E-03	2.12278-12	2.5914E-01	-2.7147E-12	8-2793E-04	-6.8990E-15
12.100	-2.52246-03	2.0596E-12	2.59146-01	-1.9223E-12	8.27936-04	-6.5487E-15
12.200	-2.5438E-03	2.018GE-12	2.5914E-01	-1.0067E-12	8.2793E-04	~6.6394E-15
12.300	+2.5653E-03	1.99928-12	2.59148-01	-1.6281E-13	8.2793E-04	-6.8507E-15
12.400	-2.58678-03	1.9995E-12	2.5914E-01	4.5266E-13	8.2793E-04	-6-5981E-15
12.500	-2.6082E-C3	2.012GE-12	2.5914E-01	7.5943E-13	8.2793E-04	-6.8672E-15
12.600	-2.6296E-03	2.0287E-12	2.5914E-01	7.57296-13	8.2793E-04	-6.5880E-15
12.700	-2.6511E-03	2.04228-12	2.5914E-01	5.00368-13	8.2793E-04	-6.8645E-15
12.800	-2.6726E-03	2.0464E-12	2.5914E-01	5.95886-14	8-27936-04	-6-74316-15
12.900	-2.69408-03	2.0387F-12	2,59148-01	-5-05118-13	8-2793E-04	-6-61536-15
13,000	-2.7155E-03	2.0156E-12	2.59148-01	-1-15006-12	8.27935-04	-6-4768E-15
13,100	-2.7369F-03	1.97536-12	2.59146-01	-1.8314E-12	8-27935-04	-6.29676-15
13.200	-2.75846-03	1.91936-12	2.59146-01	-7 48155-17	8.27936-04	-6.05126-15
13,300	-2.77986-03	1.84785-12	2.59148-01	-2.99435-12	8.27936-04	-5.89285-15
13.400	-2.20136-03	1.76636-12	2.59146-01	-1.73346-17	8.27935-04	-5.7156F-15
13.500	-2.92276-63	1 68335-12	2 59145-01	-3.06115-12	8.27936-04	-5.69175-15
13.607	-2.84478-03	1.61076-12	2.59146-01	-2-37778-12	8.27936-04	-5.54236-15
13.701	-2.86576-63	1.56716-12	2 59145-01	-1 15446-17	8.27936-04	-5.54755-15
13 800	-2 23716-03	1 55136-17	2 59145-01	5 53616-13	8 27936-04	-5 47016-15
13.901	+2.9086F-03	1 5 56-12	2 59145-01	2 61836-12	8.27936-04	-5 58415-15
16.003	-2 93006-03	1 68466-12	2 50146-01	4 84786-12	8 27936-04	-5 92276-15
14.105	-2 95155-(3	1 93756-12	2 59146-01	7 12156-12	8 27936-04	-6 17176-15
14 205	-7 07706-03	2 04735-12	2 50145-01	9 21755-12	8 27035-04	-6 73895-16
14.30	-2 93446-03	2 303-5-12	2.59132-01	3.21756-12	9 27936-04	-7 57876-15
14.500	-2.33440-03	2.30300-12	2.59136-01	1+10220-11	8 27935-04	~7.92076~15
14 501	-3.01300-03	2.01310-12	2+37136-31	1 23765-11	9 27025-04	-0.05005-16
14 400	-3.65678273	2.34710-12	2.55130-01	1.33100-11	9 27036-04	-1.01146-14
14.000	-3.00072-03	3+3-225-12	2+39135-01	1+2/2/07-11	9 27026 04	
14.103		3.03362~12	2.59132-01	1~3+325-11	8.27935-04	-1.00296-14
19.200	-3+1(11(-0)	2.99105-12	2.59130-01	1.25700-11	8.21956-04	-1.10220-14
14-403	-1.12516-05	4.50125-12	2.59136-51	1.05296-11	8.21932-04	-1.25226-14
15.302	-3.14401-03	1.24845-12	2.59138-01	1.92816-12	8.27936-04	-1.29596-14
15.103	-3.10002-03	4.72072-12	2.59135-01	4.71345-12	8.2193E-04	-1.3/050-14
15.233	-3.10152-03	4.20432-12	2.59136-01	1.13056-12	8.21938-04	-1.30366-14
15.300	-1.26895-03	4.7930E-12	2.59136-01	-2.4397E-12	8.2793E-04	-1.3676E-14
15.403	-3.23042-03	4-68928-12	2.59136-01	-5.8393E-12	8.2793E-04	-1.3589E-14
15.500	-3.2518E-03	4.59402-12	2.5913E-01	-8.5966E-12	8.27936-04	-1.3173E-14
15-600	-3.2733E-03	4.25512-12	2.5913E-01	-1.05918-11	8.2793E-04	-1.2340E-14
15.700	-3.29478-03	3.9639E-12	2.5913E-01	-1.17522-11	8.2793E-04	-1.1739E-14
15.800	-3.31622-03	3.6517E-12	2.5913E-01	-1.21316-11	8.2793E-04	-1.0961E-14
15.900	-3.3377E-03	3.33545-12	2.59136-01	-1.18398-11	8.2793E-04	-1.0018E-14
16.000	-3.35916-03	3.03978-12	·2.5913E-01	-1.10085-11	8.2793E-04	-9.2113E-15
16.100	-3.32048-03	2.76826-12	2.5913E-01	-9.7545E-12	8.2793E-04	-8.44136-15
16.200	-3.40206-03	2.53376-12	2.5913E-01	-8.1737E-12	8-2793E-04	-7.4610E-15
16.300	-3+42356-53	2.34356-12	2.5913E-01	-6.3530E-12	8.2793E-04	-7.0559E-15
16.400	-3.44498-03	2.20286-12	2.5913E-01	-4.37572-12	8.2793E-04	-6.7354E-15

TIME	P= 1	P- 2	P- 3	P- 4	P- 5
11,000	-1.43890E-06	2.65064E-07	-6.01927E-03	-1.26899E-12	1.88988E-06
11,100	-2.00262E-06	2.22618E-07	-6.26090E-03	-1.06180E-12	1.42607E-06
11,200	-2.49697E-06	1.64165E-07	-6.442986-03	-6.33282E-13	9.43837E-07
11.300	-2.89227E-06	9.33610E-08	-6.63560E-03	-1.06655E-13	6.46633E-07
11,400	-3.16006E-06	1.50127E-08	-6.883396-03	3.35709E-13	6,66581E-07
11.500	-3.27794E-06	-6.53814E-08	-7.18888E-03	5.71771E-13	1.01692E-06
11.600	-3,23490E-06	-1.42188E-07	-7.510746-03	4.819326-13	1.587146-06
11.700	-3.035332-08	-2.101/6E-07	-7.015035-03	2.986402-14	2.182778-06
11.900	-2,765355-06	-3.03181E-07	-7 863676-03	-1 485646-17	2.574102-00
12.000	-1.77539E-06	+3.22723E-07	-7-627146-03	-2.141566-12	2.385936-06
12.100	-1.27781E-06	-3.22528E-07	-7.22459E-03	-2.492425-12	1.82297E-06
12.200	-8.15870E-07	-3.025648-07	'-6.72929E-03	-2.49621E-12	1.16958E-06
12.300	-4.23896E-07	-2.63734E-07	-6.219795-03	-2.20441E-12	6.37846E-07
12.400	-1.25394E-07	-2.078556-07	-5.757698-03	-1.75045E-12	3.91105E-07
12.500	6.59849E-08	-1.37699E-07	-5.36663E-03	-1.342746-12	4.85645E-07
12.600	1.435706-07	-5.70321E-C8	-5.02516E-03	-1.17362E-12	8:50880E-07
12.700	1.04445E-07	2.94173E-08	-4.67594E-03	-1.30594E-12	1.31509E-06
12.800	-5.25983E-08	1.16136E-07	-4.247988-03	-1.68340E-12	1.66747E-06
12.900	-3.269898-07	1.97174E-07	-3.68394E-03	-2.05196E-12	1.73416E-06
13.000	-7.14172E-07	2.66646E-07	-2.962576-03	-2.26261E-12	1.44104E-06
13.100	-1.20221E-06	3.19316E-07	-2.10844E-03	-2.0943CE-12	8.40689E-07
13.200	-1.108/41-00	3.511428-07	-1.185146-03	-1.48039E-12	9.393838+08
13.600	-2.001615-06	3.590975-07	-2.14/300-04	-2.140010-13	-1.016035-06
13.400	-2.991010-00	3.443600-07	1 746355-03	1 470766-12	-1.010030-00
13.600	~4.023216~06	2.481646-07	1.820696-03	2.150206-12	~8.556746-07
13.700	-4.356536-06	1.733596-07	2.31760-03	2.381695-12	-4.385776-07
13.800	-4.52996E-06	8.803442-09	2.804828-03	2.17562E-12	-4.96915E-08
13.900	-4.53449E-06	-4.17529E-09	3.34579E-03	1.523136-12	1.177826-07
14.000	-4.37641E-26	-9.74061E-08	3.97553E-03	1.014586-12	-4.76108E-08
14.10Ŭ	-4.07303E-06	-1.862276-07	4.694662-03	5.576120-13	-5.351808-07
14.200	-3.650622-06	-2.652548-07	5.421/32-03	3.49186E+13	-1.21557E-06
14.300	-3.133916-06	-3.29485E-C7	6.10978E-C3	3.64345E-13	~1.8868CE-06
14.400	-2.54870E-06	-3.742776-07	6.67199E-03	5.24156E-13	-2.34642E-06
14.500	-1.91946E-06	-3.96012E-07	7.057782-03	5.56791E-13	-2.46374E+06
14.600	-1.27042E-36	-3.923698-07	7.25753E-03	5.43944E-13	-2.22636E-06
14.700	-0.285491-37	-3.627295-07	7.314132-03	7.467422-14	-1.74426E-06
14.000	5 052905-07	-2 326016-07	7 25554 -01	-1.77336-17	-9 304545-07
15.000	9 264665-07	-1 405746-07	7 265936-03	-2 762505-12	-7 492535-07
15.100	1.206946-06	-3.833731-08	7.331258-03	-3.462816-12	-9.992366-07
15.200	1.32513E-06	6.706591-09	7.418511-03	-3.69446E-12	-1.48759E-06
15.300	1.27432E-06	1.696298-07	7.46276E-03	-3.42579E-12	-2.03302E-06
15.400	1.064932-06	2.593376-67	7.390438-03	-2.76826E-12	-2.42534E-06
15.500	7.232598-07	3.350792-07	7.146336-03	-1.91828E-12	-2.50694E-06
15.600	2.874865-07	3.89372E-07	6.71419E-03	-1.14266E-12	-2.22402E-06
15.700	-1.98877E-07	4.209701-07	6.123396-03	-6.62957E-13	-1.65049E-06
15.800	-6.93342E-C7	4.26395E-C7	5.439592-03	-5.53060E-13	-9.59266E-07
15.900	-1.15947E-06	4.054696-07	4.74245E-03	-7.23590E-13	-3.61048E-07
16.000	-1.56923E-06	3.58876E-07	4.07532-03	-1.06844E-12	-2.90030E-08
16.100	-1.90268E-06	2.83753E-07	3.537372-03	-1.32868E-12	-3.70785E-08
16.200	-2.1400000-06	1.70011L-U7	3.031546-03	~1.3433/E-12	-3.33956E-07
10.300	-2.200331-20	7.42100E-US	2.067308-03	-1.01715E-12	-1.01986E-07
10.400	-2-313426-08	-1.049225-08	2.001305-03	- 2.320/46-13	-1.114286-08

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TIRE	P- 8	P- 7	P- 8	P- 9	P-10
11.000	-7.04189E-10	2.03619E-05	1-02230E-06	2.02240E-01	3.21137E-05
11,100	-1.74967E-10	3-147026-05	1.764616-06	2.46088E-01	4.156376-05
11.200	-1-33974F-10	4-16503E-05	2.50801E-06	2.87977E-01	4-94121E-05
11.300	-R. 36391E-11	5-029265-05	3-19492E-06	3,18083E-01	5.51861E-05
11.400	-2.70745E-11	5-679216-05	3.74607E-06	3.30239E-01	5.84527E-05
11.500	3-23500E-11	6.06415E-05	4.07998E-06	3.23657E-01	5.89022E-05
11.600	9-071636-11	6-15335E-05	4.136436-06	3.03405E-01	5.64405E-05
11.700	1.44289E-10	5.94402E-05	3.89602E-06	2.78513E-01	5.12558E-05
11.800	1.89505E-10	5.46424E-05	3.38881E-06	2.587116-01	4.38351E-05
11.900	2.23266E-10	4.76976E-05	2.68889E-06	2.50936E-01	3.49179E-05
12.000	2.43156E-10	3.93522E-05	1.89640E-06	2.56843E-01	2.539486-05
12.100	2-47622E-10	3.04233E-05	1.11298E-06	2.72226E-01	1.61775E-05
12.200	2.36103E-10	2.16816E-05	4.18678E-07	2.88552E-01	8.072508-06
12.300	2.09089E-10	1.37666E-05	-1.42301E-07	2.96050E-01	1.69252E-06
12.400	1.68104E-10	7.15116E-06	-5.62749E-07	2.872568-01	-2.57314E-06
12.500	1.15616E-10	2.15632E-06	-8.61151E-07	2.59727E-01	-4.56273E-06
12.600	5.48795E-11	-1.00143E-06	-1.06280E-06	2.169716-01	-4.23444E-06
12.700	-1.02766E-11	-2.15809E-06	-1.18095E-06	1.672396-01	~1.65093E-06
12.800	-7.56953E-11	-1.17229E-06	-1.20542E-06	1.206318-01	3.09051E-06
12.900	-1.37157E-10	2.06575E-06	-1.103056-06	8.55376E-02	9.85929E-06
13.000	-1.90644E-10	7.58678E-06	-8.30215E-07	6.57135E-02	1.84585E-05
13.100	-2.32596E-10	1.52740E-05	-3.527686-07	5.901126-02	2.85696E-05
13.200	-2.60134E-10	2.48017E-05	3.340426-07	5.82416E-02	3.9/0232-05
13.300	-2./1252E-10	3.560881-05	1.193196-06	5.384395-02	5.118062-05
13.400	-2.04958E-10	4.692/25-05	2.146245-06	3.749725-02	6.21743E-05
13.500	-2.413012-10	5.78041E-05	3.08/11E-06	4.111962-03	7.179216-05
13.000	-2.01092E-10	0.152175-05	3.905186~00	-4.2309/2-02	1.920146-95
13.100	~1.40230E-10	8 011005-05	4.510436-06	-1. 40262-02	9 609/11-05
13.900	-04430326-11	8 222845-05	4.077665-06	-1.99776-01	9 314085-05
14.000	5. 86434E-11	8 143395-05	4.772005-06	-1.037100-01	7 917036-05
14.100	1.28434E-10	7-792586-05	4.44361E-06	-2.193695-01	7.063625-05
14.200	1.90947E-10	7-20168E-05	4-00077E-06	-2.143935-01	6.11184E-05
14.300	2.42015E-10	6.406806-05	3.483722-06	-2.073596-01	5.02195E-05
14.400	2.78177E-10	5.444518-05	2.906586-06	-2.075026-01	3-850416-05
14.500	2.96904E-10	4.35167E-05	2.261976-06	-2.20716E-01	2.648436-05
14.600	2.96767E-10	3.16870E-05	1.535785-06	-2.474078-01	1.464446-05
14.700	2.77536E-10	1.94415E-05	7.26385E-07	-2.37299t-01	3.48120E-06
14.800	2.40195E-10	7.37659E-06	-1.39735E-07	-3.162868-01	-6.46529E-06
14.900	1.86898E-10	-3.80940E-06	-1.00242E-06	-3.39673E-01	-1.46181E-05
15.000	1.20839E-10	-1.33737E-05	-1.77523E-06	-3.456316-01	-2.04138E-05
15.100	4.60763E-11	-2.06258E-05	-2.36406E-36	-3.32628E-01	-2.33902E-05
15.200	-3.27111E-11	-2.50400E-05	-2.69103E-06	-3.049716-01	-2.32864E-05
15.300	-1.10506E-10	-2.63507E-05	-2.71578E-06	-2.712626-01	-2.01227E-05
15.400	-1.82267E-10	-2.46022E-05	-2.44676E-05	-2.41317E-01	-1.42339E-05
15.500	-2.43259E-10	-2.01379E-05	-1.938888-06	-2.226711-01	-6.23971E-06
15.500	-2.89371E-10	-1-35320E-05	-1.27810E-05	-2.179018-01	3-047658-06
12.100	-3.17402E-10	-5.483886-06	-5.58362E-07	-2-237455-01	1.27368E-05
12.600	-3.232935-10	3.292/62-06	1.41471E-07	-2.32325E-01	2.199298-05
12-200	-3.122851-19	1.215615-05	1.130265-07	-2.349265-01	3-01287E-05
T0*000	-2.109/01-10	2.031116-05	1+321311-06	-2.209555-01	3.005108-05
10.100	-1 604205-10	2+01307E-03	1+140305-00	-1-400396-01	4.12722E-05
16 200	-1.004200~10	3 928405-05	2.213402-00	-1.430490-01	
16.400	1.419716-13	4.77744E-05	2 866775-04	-3 348045-33	4.710102-05
10.400	TIOT211E-16	*****************	-+0001CC-00		4.551116-03

RUK NO 1

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	TINE	.9-11	8-12	P-13	9-14	P-15
	11.000	-7.79403E-07	2-02240E-01	2.40363E-04	2.40363E-04	
	11.100	-9.50764E-07	2.46088E-01	1.95925E-04	1.959258-04	
	11.200	-1-03946F-06	2-87927E-01	1.49139E-04	1.491395-04	
	11.300	-1-052725-06	3-180835-01	1.209855-04	1,20985E-04	
	11.500	-1-019315-06	3.30239E-01	1.250825-04	1.250825-04	
	11.500	-9.782845-07	3 236575-01	1 627785-04	1 627795-04	
	11.600	-9.646376-07	3.034055-01	2 224505-04	7 776505-04	
	11 700	-0 047885-07	7 795125-01	2 845845-04	2 845845-04	
	11 300	-1 070565-06	2.103132-01	2 240075-04	2 260075-04	
	11 000	-1.161665-06	7 \$00245-01	3 345546-04	2 245545-04	
	17 000	-1.735725-04	2,509502-01	3 036905-04	3 024205-04	
	12 100	-1 363165-06	2.300732701	2 /28/25-04	2 438475-04	
	12.100	-1.202102-00	2. 122202-01	2 4 30025-04	2,430022704	
	12.200	-1.12001E-00	2.00000220-01	1.141012-04	1.141012-04	
	12.500	-1.13901E-06	2.900502-01	1.100836-04		
	12.400	-0.30(605-07	2.0/2002-01	8+194312-07	0.410105-05	
	12.500	-9+35+09E-01	2.591215-01	9.410075-05	4.410685-05	
	12.000	-9-105062-01	2.109/12-01	1.2/8195-04	1.210195-04	
	12.100	-9+000102-01	1.012392-01	1./13852-04	1.113832-04	
	12.800	-1.104492-00	1.20631E-01	2.030/62-04	2.030785-04	
•	12.900	-1.302002-06	8+553/85-02	2.049842-04	2.049842-04	
	13.000	-1-516462-06	6.5/1352-02	1.695012-04	1.695002-04	
	13.100	-1.703672-05	5.901122-02	1.02103E-04	1.02103E-04	
	13.200	-1.83110E-06	5.82416E-02	1.94473E-05	1.94472E-05	
	13.300	-1.88776E-06	5-38439E-02	-5.654328-05	-5.65432E-05	
	13.400	-1.88621E-06	3.74072E-02	-1.06477E-04	-1.06477E-04	
	13.500	-1.85602E-06	4.77789E-03	-1.20375E-04	-1.20375E-04	
	13.600	-1.83136E-06	-4-23607E-02	-1.01171E-04	-1.01171E-04	
	13.700	-1.83708E-06	-9.700448-02	-6.357718-05	-6.35770E-05	
	13.800	-1.87856E-06	-1.49253E-01	-2.87288E-05	-2.87287E-05	
	13.900	-1.93888E-06	-1.89776E-01	-1.65268E-05	-1.65268E-05	
	14.000	-1.98456E-06	-2.13069E-01	-3-843316-05	-3.84331E-05	
	14.100	-1.97767E-06	-2.19347E-01	-9.32995E-05	-9.32996E-05	
	14.200	-1-89013E-06	-2.14383E-01	-1.67727E-04	-1.677276-04	
	14.300	-1.71480E-06	-2.07359E-01	-2.40823E-04	-2.40823E-04	
	14.400	-1.46961E-06	-2.07502E-01	-2.916466-04	-2.916465-04	
	14.500	-1.19293E-06	-2.20716E-01	-3.066695-04	-3.066695-04	
	14.000	-9.31817E-07	-2.47407E-01	~2.84588E-04	-2.845886-04	
	14.700	-7.27297E-07	-2.822998-01	-2.36735E-04	-2.367356-04	
	14.800	-6.017395-07	-3.16286E-01	-1.82982E-04	-1.82982E-04	
	14.900	-5.527812-07	-3.39673E-01	-1.44578E-04	-1.44578E-04	
	15.000	-5-55862E+07	-3.45631E-01	-1.36509E-04	-1.36509E-04	
	15.100	-5.743395-07	-3.326285-01	~1.62114E-04	-1.62114E-04	
	15.200	-5.73497E-07	-3.04971E-01	-2.11895E-04	-2.11895E-04	
	15.309	-5-333292-07	-2.71262E-01	-2.66932E-04	-2.66932E-04	
	15.400	-4.55661E-87	-2.41318E-01	-3.05687E-04	-3,05687E-04	
	15.500	-3.628028-07	-2.22671E-01	-3.11748E-04	-3-11748E-04	
	15.600	-2.88553E-07	-2.17901E-01	-2.797368-04	-2.79736E-04	
	15.700	-2.64713E-07	-2-237468-01	-2.17255E-04	-2.17255E-04	
	15.800	-3.08065E-07	-2,323258-01	-1.42193E-04	-1.42193E-04	
	15.900	-4.12646E-07	-2.340206-01	-7.633782-05	-7.63378E-05	
	16.000	-5.50177E-07	-2.209888-01	-3.760948-05	-3.76095E-05	
	16.100	-6.78602E-07	-1-900398-01	-3.36841E-05	-3.36842E-05	
	16.200	-7-55809E-07	-1.438496-01	-5.93063E-05	-5.93064E-05	
	16.300	-7.53653E-07	-9.007338-02	-9.824365-05	-9-824375-05	
	16.400	-6.67580E-07	-3.868066-02	-1.29181E-04	-1.29181E-04	

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TIME	Vac	¥ <sup>3</sup> Y	YET.	OHEGAX	DHEGAY	OMEGAZ
SEC	IN/SEC	IN/SEC	IN/SEC	RAD/SEC	RAD/SEC	RAD/SEC
16.500	-3.,46642-03	2.1135E-12	2.5913E-01	-2.4300E-12	8-2793E-04	-6.5360E-15
16.600	-3+48788-03	2-0742E-12	2.5913E-01	-5.97298-13	8+2793E-04	~6.5097E-15
16.700	-3.5093E-03	2.0799E-12	2.5913E-01	9.78416-13	8.2793E-04	-6.6048E-15
16.800	-3.5307E-03	2-1226E-12	2.5913E-01	2.2228E-12	8.27935-04	-6.7662E-15
16.900	-3.5522E-03	2+1930E-12	2.59138-01	3.1290E-12	8-2793E-04	-6.92378-15
17.000	-3.5736E-03	2+28256-12	2.5913E-01	3.7490E-12	8.2793E-04	-7.2560E-15
17.100	-3.5951E-03	2.3842E-12	2.5913E-01	4.1624E-12	8.2793E-04	-7.5956E-15
17.200	-3.6166E-03	2.4935E-12	2.5913E-01	4.4380E-12	8.2793E-04	-8.0338E-15
17.300	-3.6380E-03	2.6076E-12	2.5913E-01	4.60548-12	8-2793E-04	-8.23528-15
17.400	-3.6595E-03	2.72378-12	2.5913E-01	4.65118-12	8-2793E-04	-8.8630E-15
17.500	-3.6809E-03	2.8387E-12	2.5913E-01	4.5375E-12	8.2793E-04	-9.5344E-15
17.600	-3.7024E-03	2.9483E-12	2.5913E-01	4.2360E-12	8.27935-04	-9.7757E-15
17.700	-3.7238E-03	3.0479E-12	2.5913E-01	3.7554E-12	8.2793E-04	-1.0021E-14
17.800	-3.74536-03	3-1337E-12	2.59128-01	3.1529E-12	8-2793E-04	-1.0319E-14
17.900	-3.7667E-03	3.2036E-12	2.59126-01	2.5217E-12	8-2793E-04	-1.0734E-14
18.000	-3.78826-03	3.25828-12	2.59128-01	1.9621E-12	8.2793E-04	-1.0851E-14
18.100	-3.80965-03	3.3008E-12	2.59126-01	1.5476E-12	8-2793E-04	-1.0630E-14
18.200	-3.8311E-03	3.3356E-12	2.5912E-01	1.3020E-12	8-2793E-04	-1+0946E-14
18.300	-3-85258-03	3.3673E-12	2.5912E-01	1.1951E-12	8.2793E-04	-1.0737E-14
18.400	-3.8740E-03	3.39885-12	2.59128-01	1.1616E-12	8.2793E-04	-1.0873E-14
18.500	-3-8955E-03	3.4314E-12	2.5912E-01	1.1331E-12	8-2793E-04	-1.0820E-14
18.600	-3.9169E-03	3.46446-12	2.59125-01	1.0735E-12	8.27936-04	-1.1036E-14
18.700	-3.9384E-03	3.4967E-12	2.59126-01	1.0312E-12	8.2793E-04	-1.0890E-14
18.800	-3.9598E-03	3.5281E-12	2.5912E-01	9.89778-13	8.2793E-04	-1-1099E-14
18.900	-3.9813E-03	3.5607E-12	2.5912E-01	1.14276-12	8.2793E-04	-1-1474E-14
19.000	-4.00275-03	3.5995E-12	2.5912E-01	1.5520E-12	8.2793E-04	-1.1229E-14
19.100	-4.0242E-03	3.6512E-12	2.59128-01	2.25398-12	8.2793E-04	-1-1504E-14
19.200	-4.0456E-03	3.7230E-12	2.5912E-01	3.2011E-12	8.2793E-04	-1.1828E-14
19.300	-4.0671E-03	3.8199E-12	2.5912E-01	4.2628E-12	8.2793E-04	-1-2004E-14
19.400	-4.0885E-03	3.9431E-12	2.5912E-01	5.2538E-12	8.27936-04	-1.2459E-14
19.500	-4.1100E-03	4.0886E-12	2.5912E-01	5.9812E-12	8.27938-04	-1.2989E-14
19.600	-4.1314E-03	4.24776-12	2.59126-01	6.2903E-12	8.27938-04	-1-3523E-14
19.703	-4.1529E-03	4.4084E-12	2.59126-01	6.6923E-12	8.27935-04	-1.4017E-14
19.800	-4.1743E-03	4.5574E-12	2.59125-01	5.3651E-12	8.2793E-04	-1.4483E-14
19.900	-4.1958E-33	4.6813E-12	2.59128-01	4.1342E-12	8.2793E-04	-1.4791E-14
20.000	-4.2173E-03	4.7678E-12	2.5912E-01	2.44896-12	8.2793E-04	-1.4903E-14
20.100	-4.2387E-03	4-6060E-12	2.5912E-01	3.7117E-13	8.2793E-04	-1.4769E-14

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RUM NOL 1

TIME	6- J	P- 2	P- 3	P- 4	P- 5
16.500	-2.223475-06	~1.31860E-07	1.44423E-03	4.12628E-13	~1.20946E-06
16.600	-2.00581E-06	~2.38050E~07	6.864975-04	1.07802E-12	-9.57156E-07
16.700	-1.665288-06	~3.29563E~07	-1.37121E-04	1.27037E-12	-3.90946E-07
16.800	-1.21603E-06	-3.99916E-07	-1.11809E-03	9.18591E-13	3.43302E-07
16.900	-6.85435E-07	-4,44220E-07	-2.02618E-03	9.939512~14	1.039296-06
17.000	-1.13087E-07	-4.59551E-07	-2.835976-03	-1.04727E-12	1.50380E-06
17.100	4.533236-07	-4.45078E-07	-3.50156E-03	-2.17564E-12	1.625478-06
17.200	9.64364E-07	-4.01964E-07	-4.02081E-03	-3.00699E-12	1.414216-06
17.300	1.375926-06	-3.33103E-07	-4.43396E-03	-3.35027E-12	9.97020E-07
17.400	1.65510E-06	-2.428026-07	-4.80667E-03	-3.165768-12	5.71939E-07
17.500	1.78350E-06	-1.36458E-07	-5.20540E-03	-2.57600E-12	3.36479E-07
17.600	1.75725E-06	-2.02882E-08	-5.67073E-03	-1.87474E-12	4.16382E-07
17.700	1.58425E-06	9.88712E-08	-6.20173E-03	-1.34799E-12	8.201826-07
17.800	1.279808-06	2.13398E-C7	-6.75465E-03	-1.16387E-12	1.43588E-06
17.900	8.62427E-07	3.174686-07	-7.257268-03	-1.32146E-12	2.070928-06
18,000	3.512448-97	4.02654E-07	-7.53369E-03	-1.69506E-12	2.521006-06
18,100	-2.34445E-07	4.63318E07	-7.833645-03	-2.047506-12	2.643228-06
18.200	-8.735226-07	4.94693E-07	-7.835736-03	-2.07598E-12	2.407076-06
18.300	-1.54054E-36	4.93912E-07	-7.681448-03	-1.613036-12	1.90520E-06
18.403	-2.20389E-06	4.60392E-07	-7.43303E-03	-6.645276-13	1.32003E-06
18.500	-2.325898-36	3.95991E-07	-7.155036-03	5.942576-13	8.58237E-07
18.600	-3.36563E-06	3.043925-07	-6.934856-03	1.83204E-12	6.76612E-07
18.700	-3.724025-06	1.93216E-C7	-6.76209E-03	2.72067E-12	8.2595777
18.803	-4.05002E-06	6.844972-03	-6.62260E-03	3.010356-12	1.23306E-06
18.903	-4.14613E-35	-6.12121E-63	-6.457676-03	2.654616-12	1.72721E-06
19.003	-4.07165E-36	-1.87501E-07	-6.196225-03	1.81046E-12	2.10174E-06
19.100	-3.84260E-06	-3.026002-07	-5.781476-03	7.428866-13	2.18844E-06
19.200	-3.488458-06	-3.99532E-07	-5.192696-03	-2.37366E-13	1.91819E-36
19.300	-3.046528-06	-4.724232-07	-4.454152-03	-7.232776-13	1.34603E-06
19.403	-2.55586E-06	-5.16699E-07	-3.62612E-03	-1.13392E-12	6.31694E-07
19.50)	-2.05226E-06	-5.29263E-07	-2.794356-03	-9.406556-13	-1.72904E-08
19.603	-1.56552E-06	-5.036991-07	-2.02346E-03	-5.670586-13	-4.19670E-07
19.700	-1.11944E-36	-4.55484E-07	-1.353546-03	-3.466136-13	-4.84974E-07
19.800	-7.33745E-97	-3.72159E-07	-7.773898-04	-4.77028E-13	-2.45412E-07
19.900	-4.265608-07	-2.633945-07	-2.553626-04	-1.04847E-12	1.56272E-07
20.000	-2.159858-07	-1.35842E-C7	2.94629E-04	-1.920726-12	5.186026-07
20,103	-1.19653E-97	2.225936-09	9.017332-04	-2.89607t-1Z	6.54200E-07

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TIME	P- 6	P- 7	P- 8	P- 9	P-10
16.500	8-652338-11	4-298265-05	3-03867E-06	1.54125E-03	3.81054E-05
16.600	1.66789E-10	4.13183E-05	3.03554E-06	2.60466E-02	3.18319E-05
16.700	2.37208E-10	3.71350E-05	2.80724E-06	3.60692E-02	2.35963E-05
16.800	2.93136E-10	3.05584E-05	2.33191E-06	3.80840E-02	1.378912-05
16.900	3.30793E-10	2.19789E-05	1.63027E~06	4.13885E-02	3.01832E-06
17.000	3.47515E-10	1.20404E-05	7.67927E-07	5.46739E-02	-7.91613E-06
17.100	3.41946E-10	1.57082E-06	-1.56020E-07	8.28174E-02	-1.81031E-05
17.200	3.141516-10	~8.52920E-06	-1.03067E-06	1.25047E-01	-2.66404E-05
17.300	2.656438-10	~1.74142E-05	-1.75844E-06	1.75136E-01	-3.27644E-05
17.400	1.99306E-10	~2.44029E-05	-2.27548E-06	Z.23560E-01	-3.59571E-05
17.500	1.19237E-10	-2.90420E-05	-2.56110E-06	2.60836E-01	-3.60006E-05
17.600	3.048148-11	~3.11150E-05	-2.63358E-06	2.808546-01	-3.29704E-05
17.700	-6.12872E-11	-3.06011E-05	-2.53472E-06	2.82996E-01	-2.71751E-05
17.800	-1.50142E-10	-2.76074E-05	-2.30922E-06	2.72299E-01	-1.90658E-05
17.900	-2.30293E-10	-2.23045E-05	-1.98659E-06	2.57587E-01	-9.14747E-06
18.000	-2.96468E-10	-1.48895E-05	-1.57238E-06	2.48258E-01	2.07991E-06
18.100	-3.44258E-10	-5.58852E-06	-1.05157E-06	2.50877E-01	1.41386E-05
18.200	-3.70400E-10	5.30577E-06	-4.02702E-07	2.667916-01	2.65509E-05
18.300	-3.72993E-10	1.73832E-05	3.828316-07	2.9163CE-01	3.87992E-05
18.400	-3.51618E-10	3.00790E-05	1.284186-06	3.16866E-01	5.029518-05
18.500	-3.07383E-10	4.26756E-05	2.24367E-06	3.32871E-01	6.03824E-05
18.600	-2.42864E-10	5.43549E-05	3.172426-06	3.32376E-01	6.83869E-05
18.700	-1.61964E-10	6.42946E-05	3.96780E-06	3.13091E-01	7-37055E-05
18.800	-6.96910E-11	7.17872E-05	4.53727E-06	2.7854SE-01	7.59136E-05
18.900	2.814805-11	7.63483E-05	4.82109E-06	2.35555E-01	7.485766-05
19.000	1.25304E-10	7.77855E-05	4.80611E-06	1.97838E-01	7.070418-05
19,100	2-15485E-10	7-62093E-05	4.52643E-06	1.69540E-01	6-39272E-05
19.200	2.92//3E-10	7.19844E-05	4.050728-06	1.55412E-01	5-523648-05
19.300	3.520195-10	6.564145-05	3.460995-06	1.53131E-01	4.546265-35
19.400	3-891965-10	5.111492-05	2.83001E-06	1.55504E-01	3-543545-05
19.500	4.01681E-10	4.896076-05	2.205125-06	1.53130E-01	2-248365-05
19.600	3-884385-10	3.9/1298-05	1.003002-00	1.3/84/2-01	1.73802E-05
10 000	3.300945-10	3-045446-05	1.020935-00	1.03/435-01	1.034065-05
10 000	2.003032-10	2.110000-05	-1 -1 -1 -1 - 07	7.30470E-UZ	3-038432-06
20 000	1 142106 10	7 767635.04	-1.21011E-U/	-/ 001225.02	1.133435-00
20.000	1 144076-11	7 547555-00	-0.319002-01	-4.701335-02	0-104046-01
20.100	1+1049/6-11	2.302335-00	-1-092305-00		1-001105-00

RUN NO I

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TIME	P-11	P-12	P-13	9-14	P-15
16.500	-5.17698E-07	1.541256-03	-1.334395-04	-1.334398-04	
16.600	-3.421908-07	2.60465E-02	-1.01738E-04	-1.01738E-04	
16.700	-1.84152E-07	3.606912-02	-3.759736-05	-3.75973E-05	
16.800	-7.732498-08	3.80839E-02	4.38700E-05	4.38701E-05	
16.900	-3.54791E-08	4.138846-02	1.21309E-04	1.213095-04	
17.000	-4.922218-08	5.467398-02	1.747176-04	1.74717E-04	
17.100	-9.12323E-08	8.28174E-02	1-925478-04	1.92547E-04	
17.200	-1.27936E-07	1.250478-01	1.75771E-04	1.75770E-04	
17.300	-1.332998-07	1.75136E-01	1.37458E-04	1.374586-04	
17.400	-9.965512-08	2.23560E-01	9.801376-05	9.80136E-05	
17.500	-4.161182-08	2.60836E-01	7.779205-05	7.77920E-05	
17.600	S.46406E-09	2.808545-01-	8.97394E-05	8.973958-05	
17.700	1.181315-08	2.829968-01	1.34703E-04	1.34703E-04	
17.800	-6.25625E-08	2.72299E-01	2.01089E-04	2.01089E-04	
17.900	-2.26045E-07	2.57587E-01	2.689876-04	2.68987E-04	
18.003	-4.65120E-07	2.482588-01	3.17280E-04	3.172806-04	
18.100	-7.44631E-07	2.50877E-01	3.31196E-04	3.311966-04	
18.200	-1.01851E-06	2.667916-01	3.07573E-04	3.07573E-04	
18.300	-1.24419E-36	2.916305-01	2.559798-04	2.559708-04	
18.400	-1.39572E-06	3.16866E-01	1.952226-04	1.952226-04	
18.500	-1.47095E-06	3.328716-01	1.466746-04	1.46674E-04	
18.600	-1.49032E-36	3.32376E-01	1.265228-04	1.26522E-14	
18.700	-1.48792E-06	3.130916-01	1.400275-04	1.40027E-04	
18.800	-1.49799E-06	2.78548E-01	1.79644E-04	1.79644E-04	
18.903	-1.541825-05	2.368585-01	2.277716-04	2.277718-04	
19.000	-1.619645-06	1.978386-01	2.63092E-04	2.63092E-04	
19-100	-1.71168E-06	1.69540E-01	2.682708-04	2.682766-04	
19+200	-1.78418E-06	1.554128-01	2.362016-04	2.36201E-04	
19-303	-1.80434E-06	1.53131E-01	1.72696E-04	1.72606E-04	
19.403	-1.75267E-26	1.55504E-01	9.40234E-05	9.40234E-05	
19.500	-1.63180E-06	1.531305-01	2.192416-05	2.192418-05	
19.600	-1.46734E-06	1.378476-01	-2.492868-05	-2.49285E-05	
19.700	-1-300478-06	1.05743E-01	-3.714406-05	-3.714395-05	
19.800	-1.17484E-C6	5.864576-02	-1.79979E-05	-1.79978E-05	
19.900	-1.12233E-06	3.54735E-03	1.782146-05	1.78215E-05	
20.003	-1.152765-36	-4.981336-02	4.95616E-05	4.95616E-05	
20.103	-1.25102E-06	-9.235498-02	5.79262E-05	5.79261E-05	

# TABLE 20 LISTINGS OF COMPUTER RUNS FOR IMPULSIVE SPIN-UP RESPONSE

PAGE NO 1

RUN NO	2				
PANDORA - LY	¥ <del>1</del> 20	1	5 AUG 19	163	
RESPONSE OF	TITAN I	I HODEL	TO INPL	ILSIVE	SPIN-UP

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TIME	AX	¥Y	٧Z	OHEGAX	OHEGAY	OMEGAZ
SEC	IN/SEC	IN/SEC	IN/SEC	RAD/SEC	RAD/SEC	RAD/SEC
<b>n</b> .	ß.,	â	0	A. 09985-01	-0.	-0.
0.050	n.	ñ.	0.	A. 9991E-01	-3-3578F-06	-8.10485-09
0.100	Ő.	0.	6.	4.99755-01	-1.69856-05	-7.18046-08
0.150	0.	0.	0.	4.9956E-01	-1.76276-05	-2.61615-07
0.200	n	n 0.	0.	4 99395-01	-1.92235-05	-5.77795-07
0 250	0	0	0	4.00765-01	-1 50066-05	-8 50685-07
0.300	0.	0.	0.	4.99106-01	-1.22226-05	-8.2816E-07
0.350	0. 0	0. 0	0.	4 08955-01	-1 28005-05	-4. 68535-07
0.400	0.	0	0	4.90090-01	-1.98395-05	-1.48995-07
0.450	6.	0.	0	4.98045-01	-3.03395-05	-4 62295-07
0.500	0	0.	0	4 97645-01	-3.91595-05	-1 65666-06
0.550	0.	ວ. ລ	0	4 97285-01	~~~ 25365~05	-3 22945-06
0.600	0.	0.	0	4 96975-01	-4 10815-05	-4 18605-06
0.650	0.	ก.	0.	4. 96686-01	-3 88016-05	-3.89765-06
0.700	<b>0</b> .	n.	0	4 96335-01	-4 0735E-05	-2 82755-04
0.750	ő.	n.	0.	4.95916-01	-4-82865-05	-2.35536-06
0.800	n.	0.	e.	4.95476-01	-5 85456-05	-3 67275-06
0.850	0	0	0	4 95096-01	-6 63756-05	-6 64876-06
0.900	n	0. 0.	0.	4 94815-01	-6.82756-05	-9.73566-06
0.950	<u>.</u>	0.	0. 0.	4.94625-01	-6.49925-05	-1.11636-05
1,000	0. 0.	0.	0.	4.94486-01	-6.08905-05	-1-04616-05
1.050	0.	0.	0.	4.94325-01	-6-05846-05	-8-97815-06
1,100	0.	0.	0.	4.94128-01	-6.54068-05	~8.83326-06
1,150	а.	a.	0.	4.93955-01	-7.23786-05	-1.11218-05
1.200	0.	0.	0. 0.	4-93855-01	-7-64686-05	-1.49445-05
1.250	0.	0.	0. 0.	4-93875-01	-7.44155-05	-1.80995-05
1.300	0.	0.	0.	4-94008-01	-6.7214E-05	-1.88925-05
1.350	0.	0.	0.	4-9418E-01	-5.9361E-05	-1.75175-05
1.400	0.	0.	0.	4.94355-01	-5.5440E-05	-1.58298-05
1.450	0.	0.	0.	4.94495-01	-5.66495-05	-1.57456-05
1.500	0.	0.	0.	4.9463E-01	-5.99206-05	-1.7679E-05
1.550	0.	0.	0.	4.9485E-01	-6-03005-05	-2.0311E-05
1-600	0.	0.	0.	4-9517E-01	-5-47975-05	-2.17985-05
1-650	0.	0.	0.	4.9558E-01	-4-47228-05	-2.12986-05
1.700	0.	0.	0.	4.96035-01	-3.4736E-05	-1.95228-05
1.759	0_	0.	0.	4.96378-01	-2-93258-05	-1.79595-05
1-800	0.	0.	0.	4.96696-01	-2.94158-05	-1.75948-05
1.850	0.	0.	0.	4-95978-01	-3.1737E-05	-1.82368-05
1.900	0.	0.	0.	4.9733E-01	-3.14226-05	-1.8900E-05
1.950	0.	0.	0.	4.97696-01	-2.56318-05	-1.8765E-05
2.000	0.	<b>a.</b>	0.	4.98116-01	-1.6658E-05	-1.7780E-05
2.050	υ.	0.	0.	4.93518-01	-8.64638-06	-1.6525E-05
2.100	0.	0.	0.	4.95818-01	-5.9747E-05	-1.5590E-05
2.150	0.	0.	0.	4.99316-01	-9.0804E-06	-1.5133E-05
2.200	0.	0.	0.	4.99158-01	-1.44008-05	-1.49116-05
2.250	0.	0.	0.	4.9931E-01	-1.71978-05	-1.4631E-05
2.300	0.	0.	0.	4.9349E-01	-1.5271E-05	-1.41916-05
Z.350	G.	0.	0.	4.99676-01	-1.06458-05	-1.36626-05
2.400	0.	0.	0.	4.99785-01	-7.9700E-06	-1.31478-05
2.450	0.	0.	0.	4.9978E-01	-1.0583E-05	-1.2711E-05
2.500	0.	0.	0.	4.93676-01	-1.9190E-05	-1.2421E-05
2.550	0.	0.	0.	4.99508-01	-2.90568-05	-1.23338-05
2.600	0.	0.	0.	4.97345-01	-3.59436-05	-1.23856-05
2.650	0.	0.	0.	4.99208-01	-3.6110E-05	-1.23408-05
2,700	0.	0.	0.	4.99355-01	-3.7305E-05	-1.1954E-05

TIME	P- 1	P- 2	P- 3	P- 4	P- 5
0.	0.	0.	0.	0.	0.
0.050	-4.87135E-03	3.88413E-03	2.81334E-06	3.99347E-07	-1.01574E-08
0.100	-1.87107E-02	1.23698E-02	5.712086-05	2.64692E-06	-7.49454E-08
0.150	-3.98232E-02	1.87109E-02	3.22242E-04	6.85814E-06	-2.29304E-07
0.200	-6.67852E-02	1.78319E-02	1.08035E-03	1.10883E-05	-4.85657E-07
0.250	-9.93202E-02	1.02756E-02	2.65041E-03	1.25778E-05	-8.49351E-07
0.300	-1.38159E-01	1.73206E-03	5.26648E-03	9,63089E-06	-1.34024E-06
0.350	-1.84011E-01	-1.44930E-03	9.01462E-03	2.69247E-06	-2.00317E-06
0.400	-2.36476E-01	2.77606E-03	1.38760E-02	-6.19165E-06	-2.89865E-06
0.450	-2.93774E-01	1.058586-02	1.98389E-02	-1.51029E-05	-4.08092E-06
0.500	-3.53512E-01	1.53227E-02	2.69886E-02	-2.38299E-05	-5.57999E-06
0.550	-4.13882E-01	1.27136E-02	3.55017 <u>-</u> 02	-3.40900E-05	-7.401008-06
0.600	-4.74339E-01	4.18175E-03	4.55490E-02	-4.82357E-05	-9.54011E-06
0.650	-5.35243E-01	-4.31598E-0 <del>3</del>	5.71840E-02	-6.74489E-05	-1.20030E-05
0.700	-5.96768E-01	-6.97864E-03	7.03036E-02	-9.08089E-05	-1.481016-05
0.750	-6.57963E-01	-2.68424E-03	8.47123E-02	-1.15941E-04	-1.79838E-05
0.800	-7.16724E-01	4.18222E-03	1.002376-01	-1.40743E-04	-2.15266E-05
0.850	-7.707128-01	7.20740E-03	1.16803E-01	-1.64854E-04	-2.54084E-05
0.900	-8.18486E-01	2.97904E-03	1.34410E-01	-1.89758E-04	-2.95715E-05
0.950	-8.59963E-01	-6.27371E-03	1.53029E-01	-2.17442E-04	-3.39502E-05
1.000	-8.95853E-01	-1.44656E-02	1.72508E-01	-2.48669E-04	-3.84903E-05
1.050	-9.265368-01	-1.64289E-02	1.925582-01	-2.82170E-04	-4.31528E-05
1+100	-9.512916-01	~1.19215E~02	2.128346-01	-3.15388E-04	-4.79014E-05
1.150	-9.68487E-01	~5.74910E-03	2.33051E-01	-3.46185E-04	-5-268476-05
1.200	-9.76601E-01	-3.94855E-03	2.53043E-01	-3.74200E-04	-5.742928-05
1.250	-9.75232E-01	-9.04947E-03	2.72718E-01	-4.00835E-04	-6.20491E-05
1.300	-9.65322E-01	-1.81214E-02	2.91950E-01	-4.27884E-04	-6.64682E-05
1.350	-9.48410E-01	-2.50992E-02	3.10543t-01	-4.55867E-04	-7.06361E-05
1.400	-9.25515E-01	-2.55270E-02	3.28141E-01	-4.33363E-04	-7.45271E-05
1.450	-8.96542E-01	-1.99937E-02	3.44424E-01	-5.07828E-04	-7.81250E-05
1.500	-8.60673E-01	-1.35979E-02	3.59159E-01	-5.27293E-04	-8.14037E-05
1.550	-8.17431E-01	-1.19024E-02	3.72254E-01	-5.41620E-04	-8.43219E-05
1.600	-7.67543E-01	-1.65759E-02	3.83692E-01	-5.52380E-04	-8.68341E-05
1.650	-7.12924E-01	-2.41223E-02	3.934296-01	-5.61445E-04	-8-89090E-05
1.700	-6.55777E-01	-2.86533E-02	4.01323E-01	-5.69397E-04	-9.05407E-05
1.750	-5.97534E-01	-2.64939E-02	4.071716-01	-5.74974E-04	9.17429E-05
1.800	-5.38452E-01	-1.90102E-02	4.10794E-01	-5.759658-04	-9.25296E-05
1.850	-4.78192E-01	-1.14765E-02	4.12143E-01	-5.70886E-04	-9.28986E-05
1.900	-4.16879E-01	-8.91397E-03	4.11305E-01	-5.60122E-04	-9-28292E-05
1.950	-3.55806E-01	-1.21833E-02	4.08433E-01	-5.45698E-04	-9.22968E-05
2.000	-2.97192E-01	-1.73716E-02	4.03634E-01	-5.29781E-04	-9.12922E-05
2.050	-2.43185E-01	-1.891126-02	3.969262-01	-5.13246E-04	-8.98307E-05
2,100	-1.94902E-01	-1.395016-02	3.88279E-01	~4.95125E-04	-8.79451E-05
2.150	-1.52241E-01	-4.55399E-03	3.777128-01	-4.73628E-04	-8.56669E-05
2.200	-1.14615E-01	3.94830E-03	3.653916-01	-4.47757E-04	-8.30136E-05
2.250	-8.20014E-02	7.135098-03	3.515572-01	-4.13352E-04	-7.99914E-05
2.300	-5.54613E-02	4.89522E-03	3.365465-01	-3.37720E-04	-7.66121E-05
2.350	-3.67080E-02	1.42454E-03	3.205348-01	-3.58171E-04	-7.29104E-05
2.400	-2.106978-02	1.86556E-03	3.03826E-01	-3,30551E-04	-6.89492E-05
2.450	-2.666511-02	8.25479E-03	2.85304E-01	-3.03913E-04	-6.48069E-05
2.500	-3.44454E-02	1.19443E-02	2.63222E-01	-2.765518-04	-6.05571E-05
2.550	-4.906228-02	2.569201-02	2.498185-01	-2.47602E-04	-5.62562E-05
2.500	-6.98680E-02	2.77408E-02	2.314345-01	-2.17975E-04	~5.19465E-05
2.650	-9.72525E-02	2.46820E-C2	2.133742-01	-1.89889E-04	-4.76724E-05
Z.700	-1.32054E-01	2.089286-02	1.959196-01	-1.65402E-04	-4.34933E-05

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PAGE NO 3

TIME	P- 6	P- 7	P- 8	P- 9	P-10
ο.	0.	0.	0.	0.	0.
0.050	-1.62872E-06	4.65973E-02	1.85861E-01	-3.79963E-03	1.31057E-01
0.100	-5.21625E-06	2.05750E-01	6.03804E-01	-2.72022E-02	4.76613E-01
0.150	-7.97882E-06	5.18452E-01	9.50849E-01	-7.95831E-02	9.33867E-01
0.200	-7.78121E-06	1.01415E 00	9.86179E-01	-1.577556-01	1.42145E 00
0.250	-4.772768-06	1.684412 00	7.10808E-01	-2.52649E-01	1.53771E 00
0.300	-1.25159E-06	2.48354E 00	3.65356E-01	-3.59966E-01	2.55501E 00
0.350	1.24796E-07	3.35559E 00	2.48339E-01	-4.36732E-01	3.35515E 00
0.400	-1.60760E-06	4.26759E 00	4.915828-01	-6.48803E-Cl	4.35650E 00
0.450	-4.98524E-06	5.22417E 00	9.61036E-01	-8.61157E-01	5.48952E 00
0.500	-7.25677E-06	6.25279E 00	1.35707E 00	-1.12826E 00	6.63950E 00
0.550	-6.50478E-06	7.37098E 00	1.43879E 00	-1.44172E 00	7.72296E 00
0.600	-3.10914E-06	8.56028E 00	1.204068 00	-1.78700£ 00	8.73846E 00
0.650	5.68695E-07	9.76692E 00	8.90105E-01	-2.15415E 00	9.75295E 00
0.700	2.02289E-06	1.09286E 01	7.94296E-01	-2.54465E 00	1.08350E 01
0.750	5.35677E-07	1.20079E 01	1.04939E 00	-2.96916E 00	1.19875E 01
0.800	-2.28287E-06	1.30067E 01	1.52237E 00	-3.43808E 00	1.31317E 01
0.850	-3.75179E-06	1.39518E 01	1.91313E 00	-3.95189E 00	1.415558 01
0.900	-2.190935-06	1.486218 01	1.97941E 00	-4.49874E 00	1.49874E 01
1 000	5 780745-06	1.5/2250 01	1.710090 00	-5.000900 00	1.000900 01
1.050	7.446456-06	1.709165 01	1.236346 00	-5 192255 00	1.66984E 01
1,100	6.274305-06	1.75170E 01	1.44914E 00	-6.768718 00	1.71757E 01
1.150	3,995156-06	1.77735E 01	1.87443E 00	-7.36343E 00	1.75465E 01
1.200	3.18989E-06	1.79C02E 01	2.20842E 00	-7.97463E 00	1.77156E 01
1.250	5.293828-06	1.79296E 01	2.20730E 00	-3.58794E 00	1.76363E 01
1.300	9.489906-06	1.78601E 01	1.872266 00	-7.18305E 00	1.73444E 01
1.350	1.33427E-05	1.76574E 01	1.45010E 00	-9.74470E 00	1.69303E 01
1.400	1.46702E-05	1.72916E 01	1.24779E 00	-1.02697E 01	1.64712E 01
1.450	1.32031E-05	1.67205E 01	1.39971E 00	-1.07648E 01	1.59756E 01
1.500	1.07582E-05	1.60046E 01	1.76534E 00	-1.12368E 01	1.53835E 01
1.550	9.80633E-06	1.51899E 01	2.03384E 00	-1.16823E 01	1.46212E 01
1.600	1.15512E-05	1.432416 01	1.960122 00	-1.20876E 31	1.36676E 01
1.650	1.500086-05	1.341948 01	1.55247E 00	-1.24311E 01	1.258048 01
1.700	1.777005-05	1.240302 01	1.06908E 00	-1.20993E UI	1.14022E 01
1.000	1.111090-05	1.139820 01	0.241072-01	-1.20923E UL	1.037340 01
1 950	1.152105-05	9 040305 00	1 20/526-01		4.303,JC JU
1.900	9.507985-06	7.884216 00	1.537618 00	-1.312445 01	7.31902E 00
1,950	1.002215-05	6.77855E 00	1.43317E 00	-1.30920F 01	6.19741F 00
2.000	1.19391E-05	5.757832 00	9.99878F-01	-1.29867E 01	5.08051F 00
2.050	1.28993E-05	4.79991E 00	5.09981E-01	-1.28019E 01	4.06886E 00
2.100	1.11377E-05	3.87580E 00	2.84323E-01	-1.25452E 01	3.23212E 00
2.150	6.84164E-06	2.98427E 00	4.48210E-01	-1.22357E 01	2.56782E 00.
2.200	1.99926E-06	2.16464E 00	8.34534E-01	-1.18920E 01	2.01527E 00
2.250	=1.13158E-06	1.47755ë 00	1.10931E 00	-1.15219E 01	1.51297E 30
2.300	-1.74244E-06	9.68073E-01	1.02879: 00	-1.11202E 01	1.05455E JO
2.350	-1.06019E-06	6.38970E-01	6.23823E-01	-1.06764E 01	6.99735E-01
2.400	-1.36123E-06	4.54689E-01	1.833236-01	-1.01872E 01	5.32516E-01
Z-450	-4.18756E-06	3.73111E-01	3-202432-02	-9.66346E JJ	>.99341E-01
2.500	-9.14344E-06	3.809022-01	2.846261-01	- 4.12595E 00	8.75290E-01
2.220	-1.413005-05	7 0310/5-01	1.001945	-3.33391E UU	1.203932 00
2.650	-1.756196-05	1.272926 00	1.057116 00	-7 540546 00	2 273706 00
2.700	-1.65978E-05	1.92860E 00	6.990626-01	-7.04043E 00	2.88728F 00

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PAGE NO 4

TIME	P-11	P-12	P-13	P-14	P-15
Ο.	0.	0.	0.	0	
0.050	-4-215616-01	4-83876F-	03 5.682646-07	-2.651816-06	
0.100	-1.34930F 00	3-091396-	02 3.309136-06	-1.929328-05	
0.152	-2.36297E 00	7-489776-	02 6.443775-06	-5.73663E-05	
0.200	-2.01475E 00	1.04556E-	01 3.37836E-06	-1.19036E-04	
0.250	-1,25295E 00	7.682895-	02 -1.46414E-05	-2.0048JE-04	
0.300	-3.78279E-01	-3.54101E-	02 -5.451656~05	-3.03196E-04	
0.350	-7.56408E-02	-2.28561E-	C1 -1.19215E-34	-4,3494úE-04	
0.400	-5.82207E-01	-4.76699E-	01 -2.07916E-04	-5.08046E-04	
0.450	-1.50754E 00	-7.56397E-	01 -3.20596E-04	-8.33308E-04	
0.500	-2.14096E 00	-1.068495	00 -4.608252-04	-1.11462E-03	
0.550	-2.00433E 00	-1.43999E	00 -6.36177E-04	-1.44842E-03	
0.600	-1.21992E 00	-1.90424E	CD -8.55211E-04	-1.82827E-03	
0.650	-4.10394E-01	-2.47652E	CO -1.12325E-03	-2.25091E-03	
0.700	-2.12265E-01	-3.141376	CO -1.44031E-03	-2.71895E-03	
0.150	-7.829848-01	-3.86389E	00 -1.800/7E-03	-3.2383JE-03	
0.800	-1.68329E 00	-4.61453E	00 -2.199935-03	-3.812135-03	
0.850	-2.222468 00	-2.302045	00 -2.034335-03	-4.430315-03	
0.900	-1.99492E 00	-7.090065	00 - 3.103376-03	-5.099645-03	
1 005	-4 444405-31	-1.050656	00 = 3.00(35) = 03	-5.100020-03	
1.050	-3.371918-01	-9.038655	CO -4.141712-03	-7 212476-03	
1.100	-9.495086-01	-1.003895	01 -5.262302-03	-7.942876-03	
1.150	-1.80361E 00	-1.10.985	01 -5.823346-03	-8.682446-03	
1.200	-2.23804E 00	-1.194496	C1 -6.37408E-03	-9.42162F-03	
1.250	-1.92123E 00	-1.286006	01 -6.90930E-03	-1.01456E-02	
1.300	-1.10447E 00	-1.37727E	01 -7.427476-03	-1.083976-02	
1.350	-4.21524E-01	-1.46805E	01 -7.92473E-03	-1.149138-02	
1.400	-3.94660E-01	-1.555458	01 -8.39380E-03	-1.21007E-02	
1.450	-1.03215E 00	-1.63540E	01 -8.82561E-03	-1.25657E-02	
1.500	-1.82749E GO	-1.705072	01 -9.21300E-03	-1.318500-02	
1.550	-2.15661E GO	-1.76426E	01 -9.553526-03	-1.36512E-32	
1.600	-1.76160E CC	-1.81493E	61 -7.849266-63	-1.405262-02	
1.650	-9.46044E-01	-1.859016	$C1 = 1.01037\xi = 02$	-1.437962-02	
1.700	-3.34257E-31	-1.896462	C1 -1.03177E-C2	-1.46257E-02	
1.753	-3.85378E-01	-1.92484E	01 -1.048335-02	-1.47951E-02	
1.800 -	-1.04074E 00	-1.94095E	01 -1.06038E-02	-1.43926E-J2	
1.850	-1.775392 00	-1.943046	01 -1.35//Jc-02	-1.492132-02	
1.900	-2.019288 00	-1.932728	01 -1 044864 07	~1-45/91E-JZ	
2 000	-1.330090 00	-1.911266	01 -1.00000000-02	-1.410000-JZ	
2.050	-7.346735-01	-1.002902	01 -1+037212-02 01 -1 04477e-17	-1 422305-02	
2.100	-3.657916-11	-1 904355	01 -1.034526-02	-1 303725-02	
2.150	-1.03739F 00	-1.750545	01 -1.003448-02	-1.353236-02	
2.200	-1.71553E CC	-1.685916	01 -9.748798-03	-1-37738-02	
2.250	-1.86926E 00	-1.612576	01 -9.410876-03	~1.25754E-02	
2.300	-1.380305 00	-1.534216	01 -9.028068-03	-1.202601-02	
2.350	-6.29045E-01	-1.45399E	01 -3.60923E-03	-1.143328-02	
2.400	-1.94636E-01	-1.37285E	01 -8.161286-03	-1.080312-02	
2.453	-4.055258-01	-1.28771E	01 -7.59796E-03	-1-014988-02	
2.500	-1.08944E 00	-1.203058	01 -7.191982-03	-9.437868-03	
2.550	-1.712988 00	-1.11322é (	C1 -6.67792E-03	-3-928128-03	
2.600	-1.79793E 00	-1.02206E	G1 -6.15536E-03	-3.17555E-03	
2.653	-1.289258 00	-9.33231E	60 -5.636392-03	-7.531878-03	
2.700	-5.916098-01	-8.51090E	00 -5.134902-03	-6.90030E-03	

PAGE NO 5

RUN HO 2

			47	O"EGAX	GHEGAY	CHEGAZ
TIME	VX TN/SFC	VY IN/SEC	IN/SEC	RAD/SEC	RAD/SEC	RAD/SEC
350	111/ 520		_	6 0885E-01 -3	.9445E-05 -1	12825-05
2.750	0.	0.	0.	4 98545-01 -4	.6077E-05 -1	.0773E-05
2.800	0.	0.	0.	4 98155-01 -5	.6967E-05 -1	.0945E-05
2.850	0.	0.	0.	4 9774F-01 -6	.81338-05 -1	.1831E-05
2,900	0.	0.	0.	4 9736F-01 -7	.5337E-05 -1	-2767E-05
2.950	0.	0.	0.	4 9703E-01 -7	.72618-05 -1	-2850E-05
3.000	0.	0.	0.	4 9672F-01 -7	.6373E-05 -1	.1787E-05
3.050	0.	0.	0.	4 96338-01 -7	.68928-05 -1	.03418-05
3.100	0.	0.	0.	4.95972-01 -8	1441E-05 -9	.8313E-06
3.150	0.	0.	0.	4.95568-01 -9	1.8973E-05 -1	-09882-05
3.200	0.	0.	0.	4.9-168-01 -9	.55488-05 -1	-3167E-05
3.250	0.	0.	0.	4.94845-01 -9	).7314E-05 -1	4730E-05
3.300	0.	0.	· ·	4.9460E-01 -9	9.3416E-05 -1	43978-05
3.350	0.	0.	0.	4.94418-01 -5	3.6532E-05 -1	.24332-05
3.400	0.	<b>e.</b>	0.	4.94238-01 -8	3.09888-05 -1	.05532-05
3.450	0.	0.	0.	4.9403E-01 -	7.90428-05 -1	.05212-05
3.500	0.	0.	0.	4.9384E-01 -	7.9577E-05 -	1.20176-05
3.550	0.	0.	0.	4.93708-01 -	7.88018-05 -	1.53132-03
3.600	0.	0.	0.	4.93668-01 -	7.32728-05 -	1.07240-05
3.650	0.	0.	0.	4.93738-01 -	6.25732-05 -	1.755555-05
3.700	0.	0.	0.	4.93862-01 -	4.97938-05 -	1.11055-05
3.750	0.	0.	0.	4.94008-01 -	3.90142-05 -	1.11032-05
3.800	0.	0.	G.	4.94126-01 -	3.74942-05 -	1 26765-05
3.850	0.	0.	0.	4.94262-01 -	2.89258-35 -	1 52018-05
3.900	0.	0.	0.	4.9445E-01 -	2.45312-05 -	1 52036-05
3.950	0.	0.	0.	4.94736-01 -	1.652/8-00 -	1 33556-05
4.000	0.	0.	0.	4.9503E-31 -	4.82376769 3.057 6-36 -	1.10046-05
4.050	0.	0.	0.	4.95432-01	1.92042-05 -	9-63175-06
4.100	0.	0.	0.	4.95856-01	1.10325-05 -	9.84906-06
4.150	0.	0.	G.	4.961001	2.03230032	1.06821-05
4.200	0.	0.	0.	4.95522-31	2.10306 - 35 -	1.09386-05
4.200	0	0.	C.	4.95841-11	2.12200 22	1.00261-05
4.300	0.	0.	0.	4.97232-01	2.711 0 27	3.3525E-06
4.500	0.	0.	0.	4.97645-01	3 41418-35 -	-6.83626-96
4.450	0.	0.	0.	4.9-00-01	3.45316-05 -	-6.0016E-06
4.500	0.	0.	0.	4.9-395-01	2.93278-05	-5.7317E-06
4.550	0.	0.	0.	4.91202-01	2-14412-05 -	-5.51558-06
4,600	0.	0.	0.	4.9301C -1 1 0517E-31	1.15058-05 -	-4.3845E-06
4.650	0.	0.	0.	4-33012 JL	5. 27218-06	-4.1561E-06
4.700	0.	0.	C.	2 33435-01	7.40332-07	-3.2676E-06
4.750	0.	0.	<b>U</b> .	4.93648-01	-4.41368-05	-2.49126-06
4.800	0.	0.	<b>9.</b>	4.91716-01	-1.34458-05	-1.8336E-06
4.850	) 0.	0.	v.	4.975:5-01	-2.713.2-05	-1.27452-06
4.900	) 0.	0.	0.	4.97536-01	-4.32338-95	-7.97626-07
4.950	) 0.	0.	0.	4.93492-01	-5.93538-35	-3.93132-07
5.000	) 0.	0.	0.	4.993:2-01	-7.00461-05	7-97252-08
5.050	) 0.	0-	0. 0.	4.99252-01	-7.380:1-35	1.76342-31
5.100	0.0.	U.	0.	4.99338-31	-8.73428-25	1.63412-00
5.150	0 0.	U.	0.	4.98832-31	-9.83:72-25	2.49336-00
5.20	0 0.	0.	ŭ.	4.93516-01	-1.12151-04	2.85/22-00
5.25	0 0.	0.	G.	4.74156-21	-1.26645-14	2.14301-00
5.30	0 0.	0	0.	4.975lè-11	-1.38448-34	2.00110-00
5.35	0 0.	0.	0.	4.974±±-01	-1.4553:-24	3+20340-00 / /3416-04
5.40	0 0.	0.	G.	4.97142-01	-1.48528-24	#-42910-00
5.45	U U+	v.				

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TIME	P- 1	₽- 2	P- 3	P- 4	P- 5
2.750	-1-74520E-01	2.10358E~02	1.791065-01	-1-450336-04	-3.94834E-05
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2.850	-2-774895-01	3.38837E-02	1.477166-01	-1-11120E-04	-3.22385E-05
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2.950	-3.92150E-01	3-723968-02	1.203376-01	-7-92205E-05	-2.62242E-05
3.000	-4.518368-01	3.132558-02	1-08645E-01	-6.64759E-05	-2.36796E-05
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3.100	-5.76282E-01	2.31469E-02	8.94674E-02	-5.43659E-05	-1.95362E-05
3.150	-6.39071E-01	2.56501E-02	8.178192-02	-5.34916E-05	-1.79557E-05
3.200	-6.99450E-01	2.93999E-02	7.525968-02	-5.34332E-05	-1.66888E-05
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3.300	-8.06281E-01	2.44798E-02	6.57925E-02	-5.29246E-05	-1.49224E-05
3.350	-8.52103E-01	1.53786E-02	6.28620E-02	-5.46354E-05	-1.43275E-05
3.400	-8.93362E-01	6.93916E-03	6.09719E-02	-5.940348-05	-1.38858E-05
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3.500	-9.59959E-01	2.91983E-03	5.93095E-02	-7.51293E-05	-1.34206E-05
3.550	-9.82229E-01	3.84147E-03	5.91047E-02	-8.19056E-05	-1.33574E-05
3.600	-9.954248-01	1.36749E-03	5.91719E-02	-8.60086E-05	-1.33521E-05
3.650	-9.99645E-01	-6.04392E-03	5.944802-02	-8.79305E-05	-1.335518-05
3.700	-9.95971E-01	-1.60888E-02	5.98032E-02	-8.91576E-05	-1.33293E-05
3.750	-9.85533E-01	-2.44215E-02	6.00017E-02	-9.07347E-05	-1.32561E-05
3.800	-9.686612-01	-2.801198-02	5-97457E-02	-9.219406-05	-1.31269E-05
3.850	-9.44/28E-01	-2.74179E-02	5.87700E-02	-9.167082-05	-1.29248E-05
3.903	-9.129176-01	-2.622745-02	5.891472~02	-8.71203E-05	-1.201232-05
5.950	-8.131862-31	-2.815/52-02	5.411995-02	-1.109842~05	-1.1/3305-05
4.050	-8.20/212-01	-3.411145-02	5.034985-02	-0.425405-05	-1.143392-03
4.000	-7 212825-01	-4.140372-02	3 0/1776-02	-4.830302-03	-1.040042-05
4.150		-4.501616-02	3 187376-02	-3.190332-03	-7 663865-06
4.200	-6.062096-01	-4.169896-02	2.274225-02	6-457616-06	~5.796335-06
4-250	-5.450785-01	-3.717936-62	1.201556-02	3.122635-05	~3.58315E-06
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4.450	-3.01601E-01	-3.90747E-02	-5.534696-02	1.59975E-04	9.189856-06
4.500	-2.493838-01	-3.36132E-02	-6-268306-02	1-928278-04	1-32906E-05
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4.600	-1.57665E-01	-1.67124E-02	-1.015906-01	2.62350E-04	2.23044E-05
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4.700	-8.551348-02	-9.746748-03	-1.43688E-01	3.38554E-04	3.21774E-05
4.750	-6-042565-02	-8.17336E-03	-1.653258-01	3.75139E-04	3.73127E-05
4.800	-4.420035-02	-3.743498-03	-1-871698-01	4.08481E-04	4-24773E-05
4.850	-3.65832E-02	4.601098-03	-2.09098E-01	4-38648E-04	4.75890E-05
4.900	-3.65191E-02	1.49302E-02	-2.30915E-01	4.66875E-04	5.25825E-05
4.950	-4.31008E-02	2.38174E-02	-2.52325E-01	4.94308E-04	5.74147E-05
5.000	-5.629588-02	2.89577E-02	-2.72990E-01	5+20819E-04	6.20554E-05
5.050	-7.687532-02	3.08521E-02	-2.926275-01	5.44841E-04	6.64712E-05
5.100	-1.05606E-01	3-22757E-02	-3.110582-01	5.64322E-04	7.06159E-05
5.150	-1.423498-01	3.599502-02	-3.231925-01	5-78072E-04	7.44364E-05
5.200	-1.85794E-01	4.255132-02	-3.43945E-01	5.85482E-04	7.78878E-05
5.250	-2.340276-01	4.983048-02	-3.581682-01	>.91057E-04	8.09465E-05
2.300	~2.87704E-01	5.4/1002-02	-3.105442-01	5.931458-04	8.36104E-05
5.500	-3.370312-31	5.343492-02	~3,81150E-01	2.92841E-04	8.58851E-05
2.409	-2.545006-01	5 805205 02	-3.842401	5.88913E-04	8.116682-05
3+430		2-002305-02	-2.928145-01	2.14142E-04	8.923562-05

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TIME	P- 6	P- 7	P- 8	P- 9	P-10
2.750	~1.65068E-05	2.70626E 00	3.23182E-01	-6.51024E 00	3.65834E 00
2.800	-1.86341E-05	3.54785E 00	2.57672E-01	-5.98050E 00	4.60876E 00
2.850	-2.24224E-05	4.42717E 00	6.00605E-01	-5.47163E 00	5.69262E 00
2.900	-2.583598-05	5.36023E 00	1.14175E 00	-5.00158E 00	6.82112E 00
2.950	-2.69162E-05	6.38215E 00	1.52089E 00	-4.57511E 00	7.91922E 00
3.000	-2.52656E-05	7.50812É 00	1.50292E 00	-4.18289E 00	8.96996E 00
3.050	-2.232028-05	8.70863E 00	1.15899E 00	-3.81125E 00	1.00128E 01
3.100	-2.021728-05	9.91835E 00	8.13333E-01	-3.45496E 00	1.10982E 01
3.150	-2.014168-05	1.10723E 01	7.94600E-01	-3.12320E 00	1.22341E 01
3.200	-2.142558-05	1.21418E 01	1.18166E 00	-2.83406E 00	1.33666E 01
3.250	-2.20845E-05	1.31430E 01	1,73982F 00	-2.60150E 00	1.44083E 01
3.300	-2.03617E-05	1.41111E 01	2,098218 00	-2.42488E 00	1.52933E 01
3.350	-1.60851E-05	1.50604E 01	2.03378E 00	-2.28889E 00	1.60150E 01
3.400	-1.07993E-05	1.59608E 01	1.643528 00	-2.17415E 00	1.66194E 01
3.450	-6.58408E-06	1.67491E 01	1.27139E 00	-2.07032E 00	1.71583E 01
3.500	-4.48081E-06	1.73679E 01	1.24405E 00	-1.98157E 00	1.76410E 01
3.550	-3.73670E-06	1.78004E 01	1.61806E 00	-1.92035E 00	1.80207E 01
3.600	-2.43693E-06	1.80763E 01	2.13370E 00	-1.89415E 00	1.82260E 01
3.650	9.76846E-07	1.82437E 01	2.41276E 00	-1.89548E 00	1.82125E 01
3.700	6.476276-06	1.83274E 01	2.24936E 00	-1.90309E 00	1.79949E 01
3.750	1.24443E-05	1.83073E 01	1.77000E 00	-1.89358E 00	1.76344E 01
3.800	1.68891E-05	1.81332E 01	1.338086 00	-1.85469E 00	1.71931E 01
3.850	1.89308E-05	1.77640E 01	1.27491E 00	-1.79021E 00	1.66894E 01
3.900	1.94184E-05	1.72028E 01	1.61139E 00	-1.71273E 00	1.60905E 01
3.950	2.02163E-05	1.64989E 01	2.06257E 00	-1.62983E 00	1.53454E 01
4.000	2,27233E-05	1.57167E 01	2.24626E 00	-1.53447E 00	1.44331E 01
4.050	2.67681E-05	1.48935E 01	1.97840E 00	-1.40712E 00	1.33882E 01
4.100	3.07238E-05	1.40198E 01	1.41716E 00	-1,22805E 00	1.22847E 01
4.150	3.27393E-05	1.305718 01	9.43527E-01	-9.90456E-01	1.11907E 01
4.200	3.21327E-05	1.19787E 01	8.68830E-01	-7.04221E-01	1.01293E 01
4.250	2.98768E-05	1.08032E 01	1.19452E 00	-3.87305E-01	9.07530E 00
4.300	2.78186E-05	9.59355E 00	1.61024E 00	-5.12882E-02	7.989936 00
4.350	2.72446E-05	8.42207E 00	1.73318E 00	3.07799E-01	6.864735 00
4.400	2.78987E-05	7.32887E 00	1.40474E 00	7.06270E-01	5.74187E 00
4.450	2.81996E-05	6.304598 00	8.15463E-01	1.15963E 00	4.69455E 00
4.500	2.647658-05	5.31127E 00	3.603182-01	1.66959E 00	3.78349E 00
4.550	2.22643E-05	4.32505E 00	3.35110E~01	2.22138E 00	3.02303E 00
4.600	1.66669E-05	3.36748E 00	7.08022E-01	2.79350E 00	2.38243E 00
4.650	1.15227E-05	2.50104E 00	1.14358E 00	3.37237E 00	1.82061E 00
4.700	8.02322E-06	1.79193E CO	1.26168E 00	3.96067E 00	1.32666E 00
4.750	5.85035E-06	1.26867E 00	9.32295E-01	4.57269E 00	9.34994E-01
4.800	3.48302E-06	9.08238E-01	3.777346-01	5.22026E 00	7.03701E-01
4.850	-5.75381E-07	6.606218-01	2.50662E-03	5.70014E 00	6.73531E-01
4.900	-6.60012E-06	4.92655E-01	8.127486-02	6.59256E 00	8.39159E-01
4.950	-1.33945E-05	4.17326E-01	5.45976E-01	7.27222E 00	1.15452E 00
5.000	-1.91657E-05	4.85690E-01	1.03739E 00	7.92321E 00	1.56736E 00
5.050	-2.284518-05	7.475148-01	1.18220E 00	8.54639E 00	2.05606E 00
5.100	-2.48378E-05	1.211018 00	8.81836E-01	9.15360E 00	2.64033E 00
5.150	-2.66381E-05	1.83283E 00	3.89615E-01	9.75314E 00	3.35852E 00
5.200	-2.96194E-05	2.54628E 00	1.14942E-01	1.03380E 01	4.22947E 00
5.250	-3.39349E-05	3.305998 00	3.06644E-01	1.08861E 01	5.22876E 00
5.300	-3.83680E-05	4.114788 00	8.59015E-01	1.13721E 01	6.29729E 00
5.350	-4.12372E-05	5.01187E 00	1.39207E 00	1.17826E 01	7.37513E 00
5.400	-4.16492E-05	6.03167E 00	1.54434E 00	1.21226E 01	8.43440E 00
5.450	-4.01415E-05	7.16520E 00	1.25199E 00	1.24077E 01	9.48626E 00

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TIME P-11	P-12	P-13	P-14	P-15
2-750 -2-50889F-01	~7.74337E 00	-4.65802E-03	-6.28875E-03	
2.800 -5.31230E-01	-7.02101E 00	-4,20985E-03	-5.708946-03	
2-850 -1-21528F 00	-6.32700F 00	-3-79110E-03	-5.17200E-03	
2,900 -1,77847F 00	-5-66059E 00	~3.40301E-03	-4.68396E-03	
2.950 -1.79756E 00	-5-04179E 00	~3.04950E-03	-4.74448E-03	
3,000 -1,27515E 00	-4.49914F 00	-2-73622E-03	-3.84976E-03	
3-050 -6-31261E-01	-4-050198-00	-2.46732E-03	-3.49720E-03	
3-100 -3-73358E-01	-3.68974F 00	~2.24248E+03	-3-18827E-03	
3-150 -7-04001E-01	-3.39481E 00	-2.05649E-03	-2.92734E-03	
3,200 -1,36924E 00	-3.14227E 00	-1,901896-03	-2-71754E-03	
3,250 -1,860956 00	-2.92518E 00	-1.77284E-03	~2.556898-03	
3-300 -1-81184F 00	-2.75558E 00	-1.667296-03	-2.43779E-03	
3,350 -1,27590E 00	-2.65159E 00	-1.586086-03	~2.35014E-03	
3-400 -6-800825-01	-2.61922E 00	-1.52976E-03	-2.285926-03	
3.450 -4.89744F-01	-2.643018 00	-1.49573F-03	-2.24153E-03	•
3-500 -8-49757E-01	-2.69257E 00	-1.47788E-03	-2.21641E-03	
3.550 -1.47810E 00	-2.73989E 00	-1.46915E-03	-2.20899E-03	
3.600 -1.88966E 00	-2.77411E 00	-1.46490E-03	-2.21332E-03	
3.650 -1.77267E 00	-2.80241E 00	-1.46455E-03	-2.21895E-03	
3.700 -1.22563E 00	-2.83685E 00	-1.47013E-03	-2.21421E-03	
3.750 -6.74745E-01	-2.87765E 00	-1.48275E-03	-2.19029E-03	
3.800 -5.40771E-01	-2.90581E 00	-1.49960E-03	-2.14320E-03	
3.850 -9.15661E-01	-2.89092E 00	-1.51375E-03	-2.07201E-03	
3.900 -1.49756E 00	-2.80796E 00	-1.51674E-03	-1.97499E-03	
3.950 ~1.82961E 00	-2.65014E 00	-1.50197E-03	-1.84665E-03	
4.000 -1.65382E 00	-2.42817E 00	-1.46638E-03	-1.67798E-03	
4.050 -1.10590E 00	-2.15678E 00	-1.40902E-03	-1.45984E-03	
4.100 -6.04257E-01	-1.83922E 00	-1.32786E-03	-1.18676E-03	
4.150 -5.23767E-01	-1.46171E 00	-1.21741E-03	-8.58429E-04	
4.200 -9.08857E-01	-1.00197E 00	-1.06906E-03	-4.77839E-04	
4.250 -1.44551E 00	-4.45178E-01	-8.74153E-04	-4.75020E-05	
4.300 -1.70900E 00	2.05116E-01	-6.27660E-04	4.331045-04	
4.350 -1.492088 00	9.29328E-01	-3.296/8E-04	9.669802-04	
4.400 -9.601438-01	1.705468 00	1.608646-05	1.556336-03	
4.450 -5.1/62/1-01	2.523365 00	4.056405-04	2.199135-03	
+.500 -4.93423E-01	3.388/2E 00	8.3/1825-04	2.88817E-03	
4.330 -0.90249E-01	4.31394C UU	1.026776.03	3.013285-03	
4.500 -1.50072E 00 4.650 -1 50033E 00	5.30323E 00	2 277226-02	5 127/10-03	
4 700 -1 360985 00	7 407805 00	2.371336-03	5 024555-03	
4.750 -8.615345-01	8 440346 00	3 550045-03	5.72835E-03	
4.800 -4.86329E-01	9.513866 00	4 148576-03	7 534825-03	
4-850 -5-19326E-01	1.054245 01	4.74474F-03	8-334576-03	
4-900 -9-27705E-01	1-156195 01	5.335226-03	9 114406-03	
4.950 -1.39336E 00	1.25711E 01	5.91736E-03	9-863346-03	
5.000 -1.56215E 00	1.35535E 01	6.48626E-03	1.057456-02	
5.050 -1.31564E 00	1.44815E 01	7.03398E-03	1.124448-02	
5.100 -8.56789E-01	1.53303E 01	7.55171E-03	1.18695E-02	
5.150 -5.47970E-01	1.60885E 01	8.03311E-03	1.24441E-02	
5.200 -6.30413E-01	1.67601E 01	8.47625E-03	1.29594E-02	
5.250 -1.04217E 00	1.73542E 01	8.88273E-03	1.34069E-02	
5.300 -1.47176E 00	1.78724E 01	9.25430E-03	1.37805E-02	
5.350 -1.60132E 00	1.83034E 01	9.58980E-03	1.40788E-02	
5.400 -1.34949E 00	1.86275E 01	9.88445E-03	1.43035E-02	
5,450 -9.28203E-01	1.88297E 01	1.01320E-02	1.44565E-02	

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PAGE	NO	9

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	TIME	٧X	VY	٧Z	DHEGAX	OHEGAY	OMEGAZ
	SEC	IN/SEC	IN/SEC	IN/SEC	RAD/SEC	RAD/SEC	RAD/SEC
	5 500	0	0	0	4 96815-01	-1 50105-04	5 82776-06
	5 550	0.	0	0	4.96446-01	-1 52826-04	6 7654E-06
	5 600	0	0	0	4 96045-01	-1 57135-04	5 43386-06
	5 450	0. 0	0.	0	4 95666-01	-1 60036-04	4 0444E-04
	5 700	0.	0.	0.	4 95296-01	-1 61215-04	3 39206-06
	5.760	0.	0.	0.	4.95005-01	-1.01212-04	4 21576-06
	5.150	J.	0.	0.	4.950000-01	-1./7036-04	5 07345-04
	5.000	0.	0.	0.	4.94/00-01	-1.47050-04	5.92346-00
-	5.850	0.	0.	0.	4.94000-01	-1.30392-04	0.9004C-00
	5.900	0.	0.	0,	4.94342-01	-1.2/028-04	0.(4)10-00
	5.950	0.	0.	0.	4.94145-01	-1.19476-04	3.72020-00
	6.000	0.	0.	0.	4.93985~01	-1.11802-04	1.32996-00
	6.050	0.	0.	0.	4.9389E-0L	-1.01248-04	0.00935-07
	6.100	0.	0.	0.	4.9388E~01	-8.64072-05	1.79002-08
	6.150	0.	0.	0.	4.93958-01	-6.85448-05	3.50372-06
	6.200	0.	0.	0.	4.9405E-01	-5.07578-05	3.87855-06
	6.250	0.	0.	0.	4+9417E-01	-3.58012-05	2.1/5/8-06
	6.300	0.	Q.	0.	4.9429E-01	-2.4158E-05	-5.8836E-07
	6.350	0.	0.	0.	4.9446E-01	-1.3856E-05	-2.5137E-06
	6.400	0.	0.	0.	4.9469E-01	-2.1736E-06	-2.41308-06
	6.450	0.	0.	0.	4.94992-01	1.2075E-05	-7.47538-07
	6.500	0.	0.	0.	4,95348-01	2.74478-05	8.6021E-07
	6.550	0.	0.	0.	4.95718-01	4.08278-05	1.04256-06
	6.600	0.	Q.	0.	4.96062-01	4.96542-05	-2.24501-07
	6.650	0.	0.	0.	4.9639E-01	5.3776E-05	-1.74225-06
	6.700	0.	0.	0.	4.9573E-01	5.54268-05	-2.20146-06
	6.750	0.	0.	0.	4.9710E-01	5.73748-05	-1.24938-06
	6.800	0.	0.	0.	4.9750E-01	6.08176-95	3.70176-07
	6.850	0.	0.	0.	4-97938-01	6.4235E-05	1.58382-06
	6,900	0.	0.	0.	4.98276-01	6.4/358-05	1.89272-06
	6.950	0.	0.	0.	4.99596-01	6.0182E-05	1.63351-06
	7.000	0.	0.	0.	4.98858-01	5.19/01-05	1.50702-06
	7.050	0.	0.	U.	4.99072-01	3.93432-05	1.9213E-06
	7.100	0.	0.	0.	4.97298-01	2.84641-35	2.14538-95
	7.150	0.	0.	0.	4-99508-01	1.93356-05	3.57302-06
	7.200	0.	0.	0.	4.99685-01	1.05076-05	4.10301-00
	7.250	0.	0.	0.	4.97806-01	-4.60926-07	4.53332-06
	7.300	0.	0.	0.	4.99845-01	-1	4.84011-06
	7.350	0.	0.	0.	4.99816-01	-3.345.2-05	5.14455-06
	7.400	0.	0.	0.	4.99722-01	-5.3053E-05	5.4002E-06
	7.450	0.	0.	0.	4.99628-01	-0.9947E-05	5.58616-05
	7.500	0.	0.	0.	4.99516-01	-3.3543E-05	5.15902-06
	7.550	0.	0.	<b>u</b> .	4.99305-01	~9.555/12~05	2.33325-00
	1.800	0.	U.	0.	4.99166-01	-1.00152-04	6.1811E-06
	7.650	0.	0.	U.	4.98896-01	-1.22998-04	6.1327E-06
	7.700	0.	0.	<b>u.</b>	4.98500-01	-1.39256-04	5.80691-06
	7.750	0.	0.	u.	4.98215-01	-1.54255-04	5.41685-06
	7.800	0.	0.	0.	4.9/860-01	~1.65316-04	5.25791-06
	1.850	0.	ч. О	0.	4.9(232-01	~1.1001-04	5-35355-96
	7.900	v.	<b>u.</b>	V.	4.91232-01	~1.(49)E-04	2.3040F-00
	1.950	v.	0.	v.	4.40845-01	-1.11132-04	4.01735-06
	8.000	v.	v.	0.	4.30406-01	-1.81356-04	3.19/2E-06
	8.050	U.	U.	U.	4.95052-01	~1.83641-04	1.50665-06
	8.100	v.	0.	v.	4.90655-01	-1-21432-34	0.54260-07
	8.150	0.	0.	v.	4.95356-01	~1.24308-04	0.35245-07
	8.200	<b>U.</b>	v.	υ.	+.42685-01	-1.10(02-04	4.31925-07
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TIME	P- 1	P- 2	P- 3	P- 4	P- 5
5.500	-5.18566E-01	4.90458E~02	-4.00017E-01	5.64863E-04	9.02627E-05
5.550	-5.80714E-01	5.01192E-02	-4.022228-01	5.45070E-04	9.08267E-05
5.600	-6.40841E-01	5.11380E-02	-4.02430E-01	5.22366E-04	9.09262E-05
5.650	-6.97390E-01	4.94006E-02	-4.00590E-01	4-98442E-04	9.05771E-05
5.700	-7.49905E-01	4.37679E-02	-3.96666E-01	4.73702E-04	8.98004E-05
5.750	-7.98642E-01	3-553415-02	-3,90718E-01	4.47269E-04	8.86073E-05
5.800	-8.43668E-01	2.73958E-02	-3.82920E-01	4.179998-04	8.69971E-05
5.850	-8,84159E-01	2.13728E-02	-3.73512E-01	3.85710E-04	8.49681E-05
5.900	-9.18477E-01	1.732256-02	-3.62712F-01	3.51674E-04	8.25344E-05
5.950	-9.44957E-01	1.31827E-02	-3.50666E-01	3.18006E-04	7.97355E-05
6.000	-9.62798E-01	6.70409E-03	-3.37468E-01	2.86396E-04	7.66314E-05
6.050	-9.72354E-01	-2.71609E-03	-3.23228E-01	2.57180E-04	7.32868E-05
6.100	-9.74605E-01	-1.35270E-02	-3.08147E-01	2.29428E-04	6.97570E-05
6.150	-9.70232E-01	-2.31982E-02	-2.92514E-01	2.01988E-04	6.60862E-05
6.200	-9.59035E-01	-3.01162E-02	-2.766498-01	1.74662E-04	6.23169E-05
6.250	-9.40148E-01	-3.47096E-02	-2.60814E-01	1.48617E-04	5.850428-05
6.300	-9.12885E-01	-3.89574E-02	-2.451748-01	1.25710E-04	5.47198E-05
6.350	-8.77534E-01	-4.47120E-02	-2.29832E-01	1.07262E-04	5.10427E-05
6.400	-8.35474E-01	-5.21782E-02	-2.14899E-01	9.32027E-05	4.75413E-05
6.450	-7.88512E-01	-5.972498-02	-2.00554E-01	8.22552E-05	4.42607E-05
6.500	-7.37971E-01	-6.51333E-02	-1.87032E-01	7.30030E-05	4.12230E-05
6.550	-6.84231E-01	-6.72687E-02	-1.74559E-01	6.50165E-05	3-84399E-05
6.600	-6.27083E-01	-6.68642E-02	-1.63272E-01	5.91768E-05	3.59268E-05
6.650	-5.66599E-01	-6.58408E-02	-1.531928-01	5.69515E-05	3.37064E-05
6.700	-5.03814E-01	-6.57403E-02	-1.44276E-01	5.918316-05	3.17998E-05
6.750	-4.40673E-01	-6.652096-02	-1.364912-01	6.53228E-05	3.02120E-05
6.800	-3.79271E-01	-6.663535-02	-1.29863E-01	7.36955E-05	2.89238E-05
6.850	-3.20985E-01	-6.42889E-02	-1.24460E-01	8.258826-05	2.78981E-05
6.900	-2.66160E-01	-5.88566E-02	-1.20319E-01	9.13253E-05	2.70953E-05
6.950	-2.14594E-01	-5-135408-02	-1.17377E-01	1.00509E-04	2+64880E-05
7.000	-1.66413E-01	-4.36317E-02	-1.15468E-01	1.112462-04	2.60631E-05
7.050	-1.22637E-01	-3-696416-02	-1.14379E-01	1.23968E-04	2-58115E-05
7.100	-8.491102-02	-3-115245-02	-1.13931E-01	1.377585-04	2+5/1355-05
7.150	-5.501448-02	-2.481628-02	-1.14015E-01	1.506942-04	2.573308-05
1.200	-3-341346-02	-1.00010E-02	-1.145652-01	1.509685-04	2.582365-05
7 200	-1.9/8155-02	-0.325521-03	-1.154802-01	1.6/9002-04	2.594395-05
7.300	~1.325235-92	4.321/86-03	-1.105/48-01	1.721002-04	2.608/41-05
1.000	-1+33/3/2-02	1.010226-02	-1.175392-01	1.750/05 0/	2.018128-05
7 450	-2.004000-02	2.303310-02	-1.192926-01	1.751075-04	2.621165-05
7 500	-5.936676-02	3 7710/5-02	-1.102020-01	1.704505-04	2.031100-03
7.550	-9.334412-02	6 561226-02	-1.162095-01	1.600115-04	2.023405-03
7.600	-1.79745E-01	5 380285-02	-1.132036-01	1+009112-04	2.004200-03
7.650	-1 734135-01	A 170075-02	-1 105165-01	1 763725-04	2.302130-03
7.700	-2.210625-01	A. 77224E-02	-1.058645-01	1.940316-04	7 405575-05
7.750	-2-72461E-01	7.128195-02	-9,97410E-02	7.9688405	2.285515-05
7.800	-3.27806E-01	7.29322E-02	-9.204596-02	5-285906-05	2-13381E-05
7.850	-3.86892E-01	7.381638-02	-8-27791E-02	2-236915-05	1-946235-05
7.900	-4.48550E-01	7.469908-02	-7.19922E-02	-1.26064E-05	1.718725-05
7.950	-5.10826E-01	7.535626-02	-5.971728-02	-5.16853E-05	1.44905E-05
8.000	-5.71753E-01	7.47819E-02	-4-57323E-02	-9.33116E-05	1.13755E-05
8.050	-6.30122E-31	7.20105E-02	-3.05925E-02	-1.35653E-04	7.86600E-06
8.100	-6.856602-01	6.68813E-02	-1.36997E-02	-1.77672E-04	3.99204E-06
8.150	-7.38483E-01	6.01488E-C2	4.64317E-03	-2.19558E-04	-2.20674E-07
8.200	-7.88278E-01	5.28883E-02	2.42380E-02	-2.621948-04	-4.750608-06

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TIME	P- 6	P- 7	P~ 8	P- 9	P-10
5.500	-3-82307E-05	8.35416E 00	7,98058F-01	1.26498E 01	1.055758 01
5.550	-3.72268E-05	9.52252F 00	5.930435-01	1.28454E 01	1.16556E 01
5.600	-3.72278E-05	1.06215F 01	8.56250E-01	1.29776E 01	1.27494E 01
5.650	-3.70615E-05	4.16535E 01	1.44456E 00	1.30284E 01	1.37797E 01
5.700	-3.52057E-05	1.26560E 01	1.96164E 00	1.29933E 01	1.46924E 01
5.750	-3.09725E-05	1.36594E 01	2.06351E 00	1.288588 01	1.54664E 01
5.800	-2.50493E-05	1.46497E 01	1.72416E 00	1.27286E 01	1.61182E 01
5.850	-1.89909E-05	1.55672E 01	1.25523E 00	1.25386E 01	1.66774E 01
5.900	-1.40639E-05	1.63409E 01	1.06423E 00	1.231668 01	1.71552E 01
5.950	-1.03321E-05	1.69330E 01	1.33825E 00	1.20494E 01	1.752888 01
6.000	-6.67851E-06	1.73589E 01	1.89312E 00	1.172418 01	1.77536E 01
6.050	-1.73222E-06	1.76676E 01	2.336298 00	1.13412± 01	1.77933E 01
6.100	5.017386-06	1.78975E 01	2.33196E 00	1.09182E 01	1.76455E OI
6.150	1.27736E-05	1.80420E 01	1.89728E 00	1.04797E 01	1.73415E 01
6,200	1.998576-05	1.80518E 01	1.37640E 00	1.00425E 01	1.69234E 01
6.250	2.54692E-05	1.79718E 01	1.16357E 00	9.60603E 00	1.641468 01
6.300	2.92325E-05	1.74846E 01	1.41315E 00	9.15712E 00	1.58075E 01
6.350	3.23848E-05	1.69264E 01	1.91168E 00	8.68374E 00	1.50765E 01
6.400	3.62084E-05	1.62629E 01	2.24556E 00	8.1384CE 00	1.42061E 01
6.450	4.11138E-05	1.55447E 01	2.121912 00	7.69012E 00	1.32117E 01
6.500	4.627778-05	1.477518 01	1.59596E 00	7.21358E 00	1.21381E 01
6.550	5.02313E-05	1.39168E 01	1.02887E 00	6.77386E 00	1.10366E 01
6.600	5.19405E-05	1.29307E 01	8.09482E-01	6.36853E 00	9.938458 00
6.650	5.154898-05	1.181/9E 01	1.04969E 00	5.93313E 00	8.84546E 00
6.700	5.02226E-05	1.06306E 01	1.50651E 00	5.60579E 00	7.74302E 00
6.750	4.922668-05	9.44519E 00	1.76381E 00	5.23954E 00	6.625/62 00
0.800	4.894/26-05	8.31628E UJ	1.55013E 00	4.902698 00	5.51541E UV
6.850	4.800092-05	1.24805E CU	9.909966-01	4,61680E 00	4.45/40E PO
6.900	4.001002-05	5 159325 00	4.333822-01	4.391396 00	3.49915E 00
7 000	3 702575-05	4 111545 00	5 494974-01	4+213330 00	1 957375 00
7.050	3.04615E-05	3.119586 66	1.01732 00	3.950436 00	1.352906 00
7.100	2.449258-05	2.256016 00	1.254035 00	3.83775E 00	8.430185-01
7,150	·1.95020E-05	1.56567E 00	1.03226E 00	3.751536 00	4.406476-01
7.200	1.47660E-05	1.04096F 00	4.85365+-01	3.70600E 00	1.77664E-01
7.250	9.08835E-06	6.36579E-C1	6.37675E-03	3.702445 00	8.48251E-02
7.300	1.79212E-06	3.11953E-C1	-6.03276E-02	3.724296 00	1.713376-01
7.350	-6.69022E-06	6.82884E-C2	3.18552E-01	3.74570E 00	4.194286-01
7.400	-1.50756E-05	-4.84584E-02	8.383396-01	3.74705E 0J	7.97558E-01
7.450	-2.21315E-05	1.89292E-C2	1.093852 00	3.72545E 00	1.28179E 00
7.500	-2.75277E-05	2.95142E-01	8.93532E-01	3.69231E 0J	1.86910E JO
7.550	-3.19924E-05	7.51927E-01	4.071365-01	3.65990E 00	2.57285E 00
7.600	-3.666328-05	1.32707E 00	3.303866-02	3.62761E 00	3.40462E 00
7.650	-4.21377E-05	1.96787E 00	8.86825E-02	3.57852E 00	4.356348 00
7.700	-4.79510E-05	2.66546E CO	5.64511E-01	3.489022 00	5.39650E JO
7.750	-5.287478-05	3.45358E 00	1.13182d 00	3.344032 00	6.48284E 00
7.800	-5.58000E-05	4.37297E 00	1.39770E CO	3.14597E JO	7.58185E CO
7.850	-5.65303E-05	5.42918E CO	1.208728 00	2.91053E 0J	8.68043E 00
7.900	-5.58795E-05	6.57674E 00	7.68127E-01	2.652912 00	9.78148E 00
7.950	-5.50115E-05	7.74198E 00	4.762296-01	2.37430E 00	1.08874E 01
8.000	-2.453152-05	8.855901 00	6.18/88E-01	2.060438 00	1.19844E 01
8.050	-5.339/75.05	9.94030E CO	1.145198 80	1.07107E 00	1.303978 01
0.100	-2.230412-03	1.099236 01	1 931035 00	7 400234 01	1.401312 01
8 200	-4 28000546-05	1 313216 01	1 703036 00	2 616965-01	1 541875 01
0.200	1.203702-03	Tellore OI	14102736 00	2071770E-01	1-201215 01

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TIME	P-11	P-12	P13	P-14	P-15
5.500	-6.74352E-01	1.89089E 01	1.03281E-02	1.45375E~02	
5.550	-7.89643F-01	1.88784E 01	1.04720E-02	1.45439E-02	
5.600	-1.18947E 00	1.87557E 01	1.056595-02	1.44735E~02	
5-650	-1.57368F 00	1.85509E 01	1.06114E-02	1.43268E-02	
5.700	~1.66013E 00	1.82620F 01	1.060765-02	1.41086E-02	
5.750	-1.39933E 00	1.78806F 01	1.05501E-02	1.38262E-02	
5 800	-1.00670E 00	1.740285 01	1.043456-02	1.34870E-02	
5.850	-7.922436-01	1.68381F 01	1.02593E-02	1.30962E-02	
5.900	-9.22055E-01	1.62076E 01	1.00278E-02	1.26570E-02	
5.950	-1.29511E 00	1.55352E 01	9.74708E-03	1.21728E-02	
6.000	-1.62626E 00	1.48358E 01	9.42452E-03	1.16497E-02	
6.050	-1.66761E 00	1.41128E 01	9.065558-03	1.109716-02	
6.100	-1.39618E 00	1.33628E 01	8.67326E~03	1.05266E-02	
6.150	-1.02607E 00	1.25870E 01	8.25079E-03	9.94854E-03	
6.200	-8.39973E-01	1.17976E 01	7.80426E-03	9.3709CE-03	
6.250	-9.72684E-01	1.10164E 01	7.34385E-03	8.79897E-03	
6.300	-1.31232E 00	1.02653E 01	6.88196E-03	8.23752E-03	
6.350	-1.59138E 00	9.55686E 00	6.42973E-03	7.69293E-03	
6.400	-1.59378E 00	8.891538 00	5.99434E-03	7.17364E-03	
6.450	-1.31768E 00	8.26381E 00	5.57891E-03	6.68859E-03	
6.500	-9.71755E-01	7.67181E 00	5.18495E-03	6.24471E-03	
6.550	-8.11759E-01	7.12334E 00	4.81530E-03	5.84533E-03	
6.600	-9.45628E-01	6.63365E 00	4.47518E-03	5.49060E-03	
6.650	-1.25554E 00	6.21668E 00	4.17053E-03	5.179526-03	
6.700	-1.49308E 00	5+8/669E 00	3.904936-03	4.91199E-03	
6.100	-1.470802 00	5.000368 00	3-0//515-03	4.689316-03	
4 950	-1.202116 00	5.392876 00	3.403396-03	4.312000-03	
6.000	-8.889102-01	5 107025 00	3.3176496-03	4.301072-03	
6.950	-8-990255-01	5.041565 00	3-060276-03	4.236476-03	
7.000	-1.18889E 00	5.03169E 00	2-970846-03	4.208415-03	
7.050	-1.399836 00	5.07299E 00	2-908636-03	4.201746-03	
7.100	-1.36939E 00	5.14990E 00	2.87070E-03	4.212456-03	
7.150	-1.12258E 00	5.24274E 00	2.85143E-03	4.23706E-03	
7.200	-8.48130E-01	5.33605E 00	2.845281-03	4.27069E-03	
7.250	-7.51961E-01	5.42324E 00	2.849496-03	4.30608E-03	
7.300	-9.01597E-01	5.50402E 00	2.86459E-03	4.33426E-03	
7.350	-1.17899E 00	5.57701E 00	2.89213E-03	4.34655E-03	
7.400	-1.37428E 00	5.63339E 00	2.93148E-03	4.33623E-03	
7.450	-1.34612E 00	5.65633E 00	2.97790E-03	4.29882E-03	
7,500	-1.12609E 00	5.626948 00	3.02340E-03	4.23085E-03	
7.550	-8.90298E-01	5.53184E 00	3.05968E-03	4.12803E-03	
7.600	-8.23144E-01	5.36717E 00	3.08078E-03	3.98443E-03	
7.650	-9.78526E-01	5.13583E 00	3.08358E-03	3.793Z2E-03	
7.700	-1.24271E 00	4.84067E 00	3.06576E-03	3.54851E-03	
7.750	~1.42391E 00	4.47866E 00	3.02306E-03	3.24689E-03	
7.800	~1.39800E 00	4.040578 00	2.94/96E-03	2.88763E-03	
7 000	~1+1995/E 00	3.510318 00	2.831185-03	2.4/1512-03	
7 050	-7.91/UIE-UI	2.301815 00	2.004842-03	1.4441(5-03	
9 000	-7.903020-01	1 42042516 00	2 172006-02	1.4/0442-03 9.9/0695-0/	
8 050	-1 333386 00	5.941315-01	1 850865-03	2 632005-04	
8.300	-1.495126 00	-2.973055-01	1.481346-03	-4.497175-04	
8.150	-1.46469E 00	-1.744876 00	1.064735-03	-1.187506-03	
8.200	-1.27691E 00	-2.248898 00	6.00734E-04	-1.96336E-03	

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RUN NO	) 2				PAGE NO	13
TIME SEC	YX IN/SEC	YY IN/SEC	VZ IN/SEC	OMEGAX RAD/SEC	CHEGAY RAD/SEC	OMEGAZ RAD/SEC
8.250	0.	0.	0.	4.9485E-01	-1.65808-04	4.4015E-07
8.300	0.	0.	0.	4.9463E-01	-1.54708-04	-1.45558-06
8,350	0.	0.	0.	4.9442E-01	-1.45008-04	-4.2147E-06
8.400	0.	Û.	0.	4.94238-01	-1.3594E-04	-6.45898-06
8.450	0.	0.	û.	4.9409E-01	-1.2512E-04	-7.0924E-06
8.500	0.	0.	0.	4.94038-01	-1.1052E-04	-6.28038-06
8.550	0.	0.	0.	4.94948-01	-9.22318-05	-5.38438-06
8.600	0.	0.	0.	4.94118-01	-7.2571E-05	-5.84198-06
8.650	0.	0.	0.	4.9420E-01	-5.44718-05	-7.92418-06
8.700	0.	0.	0.	4.94318-01	-3.94142-05	-1.0455E-05
8.750	0.	0.	0.	4.9445E-01	-2.65018-05	-1.1760E-05
8.800	0.	0.	0.	4.94638-01	-1.33668-05	-1.10456-05
8.850	0.	0.	0.	4.9488E-01	1.7758E-06	-9.02548-06
8.900	0.	0.	0.	4.9519E-01	1.85538-05	-7.2983E-06
8.950	0.	0.	0.	4.9553E-01	3.45538-05	-7.01128-06
9.000	û.	0.	0.	4.95888-01	4.7029E-05	-7.97528-06
9.050	0.	0.	0.	4.96202-01	5,48686-05	-8.9247E-06
9.100	ð.	0.	0.	4.96538-01	5.92898-05	-8.62635-06
9.15J	ŋ.	0.	0.	4.9587E-01	6.2756E-05	-6.8579F-06
9,200	· .	6.	0.	4.9724E-01	6.6964E-05	-4.4666E-06
9.250	с.	0.	C.	4.9763E-01	7.14676-05	-2.5804E-05
9.300	C.	0.	0.	4.98005-01	7.39976-05	-1.7104E-06
9.350	<b>c.</b>	0.	<b>0.</b>	4.98335-31	7.22056-05	-1-48936-06
9.400	с.	0.	з.	4.93615-01	6.54648-95	-1.14686-06
9.450	с.	0.	с.	4.9385E-01	5.5345F-35	-2-02046-07
9.500	<b>5.</b>	э.	j.	4.99665-01	4.44338-05	1.22916-06
9.550	э.	0.	0.	4.9327E-01	3.4413E-05	2.57355-96
9.600	ε.	<b>0</b> .	0.	4.99465-01	2.48936-05	3-75356-06
9.650	0.	0.	0.	4-99605-01	1.38546-05	4-45371-36
9.700	э.	0.	C.	4-9963E-01	-6.42328-07	4-95616-06
9.750	0.	6.	<u>.</u>	4.93685-01	-1.83255-05	5.43356-06
9.800	n.	0.	0.	4.99625-01	-3.8884E-35	5-89325-06
9.850	ġ.	0.	0.	4.93535-01	-5.61778-35	6 76166-06
9.900	0.	0.	ġ.	4-99425-01	-7.50456-75	6.4433F-36
9.950	0.	0.	0.	4.93296-01	-8.98406-15	6-526.35-04
10.000	Ġ.	0.	0.	4,99115-01	-1.24476-04	6.52976-04
10.050	0.	0.	0.	4.93848-01	-1.20508-04	5 45785-66
				TO STORE OF		J+77400-00

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RUN NO 2

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TIME	P- 1	P- 2	P- 3	₽ <b>4</b>	P- 5
. 350		1 540495-02	1 105215-02	-7 060975-06	-0 540455-04
8 200	-0.330122-01	3 937946-02	4 470045-02	-3.505755-04	-1 463156-05
0+300	-0+15551E-01	3.032002-02	0.029002-02	7 020676 04	-1.905152-05
0.00	-9-00190E-01	3.002575-02	8.84102E-02	-3.939030-04	~1.900910-05
0.900	-9+300932-01	2.024595-02	1.1111/2-01	-4-34+102-04	~2.520342-05
0++50	-9-1003/2-01	9.119102-03	1.342398~01		~3.000012-05
8.500	-9.56/322-01	-2.30911E-03	1.5/5428-01	-5-038972-04	~3. 390302-05
8.550	-9.60020E-01	-1.329286-02	1-90723E-01	-5.34132E-04	-4.11692E-05
8.600	-9.561552-01	-2+33/512-02	2.03482E~01	-5.621888-04	-4-032/85-05
8.650	-9.44218E-01	-3.28370E-02	2.25585E-01	-5.81415E-04	-5-133365-05
8.700	-9.237118-01	-4.223305-02	2.46876E-01	-6.084742-04	-5-613156-05
8.750	-8.951358-01	-5.17965E-02	2.67235E-01	-6.23669E-04	-6.06671E-05
8.800	-8.59840E-01	-6.11426E-02	2.865118-01	-6.32513E-04	-6-490008-05
8.850	-8.19326E-01	-6.94908E-02	3.04491E-01	~6.356928-04	-6.88093E-05
8.900	-7.744978-01	-7.61994E-02	3.20928E-01	-6.34496E-04	-7.23880E-05
8.950	-7.25480E-01	-8.11654E-02	3.35613E-01	-6.29790E-04	-7.56285E-05
9-000	-8.72120E-01	-8+47759E-02	3.48424E-01	-6.21371E-04	-7-85119E-05
9.050	-6.14753E-01	-8.74792E-02	3.59329E-01	-6.08213E-04	-8.10077E-05
9.100	-5.54641E-01	-8.934866-02	3.68331E-01	-5.89396E-04	-8.30848E-05
9.150	-4.93705E-01	-8.999622-02	3.75406E-01	-5.650178-04	-8.47240E-05
9.200	-4.33766E-01	-8-889016-02	3.80481E-01	-5.36356E-04	-8.59226E-05
9.250	-3+75883E-01	-8.579212-02	3.83471E-01	-5.052026-04	-8.66880E-05
9+300	-3.20297E-01	-8.09459E-02	3.84345E-01	-4.72824E-04	-8.70246E-05
9.350	-2.67003E-01	-7.48746E-02	3.83172E-01	-4.39395E-04	-8.69273E-05
9.400	-2.16486E-01	-6.79680E-02	3.801036-01	-4.04285E-04	-8.63346E-05
9.450	-1.70040E-01	-6.022892-02	3.75313E-01	-3.66992E-04	-8.53921E-05
9.500	-1.29367E-01	~5.13675E-02	3.689402-01	-3.27982E-04	-8.396475-05
9.550	~9.58304E-02	-4.11678E-02	3.61073E-01	-2.88787E-04	-8.21392E-05
9.600	~6.98919E-02	-2.979298-02	3.517978-01	-2.51272E-04	-7.99658E-05
9-650	-5.11861E-02	-1.77698E-02	3.41251E-01	-2.16646E-04	-7.74946E-05
9.700	-3.91658E-02	-5.671408-03	3.296596-01	-1.84952E-04	-7.47689E-05
9.750	-3.37915E-02	6.22903E-03	3.17306E-01	-1.55435E-04	-7.182846-05
9.803	-3.571998-02	1.803136-02	3.044685-01	-1.27474E-04	-6.87204E-05
9.850	-4-584375-02	2.99506E-02	2.91356E-01	-1.01372E-04	-6.55974E-05
9.900	-6.455018-02	4.19529E-02	2.781195-01	-7.83837E-05	-6.22654E-05
9.950	-9.128436-02	5.36056E-02	2.648676-01	-5.99676E-05	-5.907698-65
10.000	-1.247638-01	6.427848-02	2.518276-01	-4.68337E-05	-5.598748-05
10.050	-1.63667E-01	7.35300E-02	2.39167E-01	-3.85254E-05	-5.305822-65

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TIME	P- 6	P- 7	P- 8	P- 9	P-10
8.250	-3.596602-05	1.41677E 01	1.25824E 00	-3.06781E-01	1.62429E 01
8.300	-2.91039E-05	1.50925E 01	9.92851E-01	-8.73145E-01	1.67647E 01
8.350	-2.29484E-05	1.58570E 01	1.15979E 00	-1.469498 00	1.71766E 01
8.400	-1.71435E-05	1.64613E 01	1.66994E 00	-2.11083E 00	1.74585E 01
8.450	-1.07013E-05	1.69449E 01	2.16289E 00	-2.80104E 00	1.75855E 01
8.500	-2.84568E-06	1.73480E 01	2.28671E 00	-3.52674E 00	1.75432E 01
8.550	6.30796E-06	1.76737E 01	1.96976E 00	-4.26402E 00	1.73374E 01
8.600	1.575318-05	1.78807E C1	1.47444E 00	-4.99278E 00	1.69894E 01
8.650	2.420758-05	1.79122E C1	1.152178 00	-5.70755E 00	1.65225E 01
8.790	3.12471E-05	1.77383E 01	1.33969E 00	-6.41654E 00	1.59487E 01
8.750	3.705778-05	1.73791E 01	1.79225E 00	-7.129778 00	1.52657E 01
8.800	4.265372-05	1.68911E 01	2.18221E 00	-7.34566E 00	1.44665E 01
8.850	4.875206-05	1.63267E 01	2.18508E 00	-8.54656E 00	1.35534E 01
8,900	5.520186-05	1.56992E 01	1.77080E 00	-9.20684E 00	1.254608 01
8.950	6.10261E-05	1.49799E 01	1.22583E 00	-9.80725E 00	1.14778E 01
9.000	6.51006E-05	1.41285E 01	9.30226E-01	-1.03447E 01	1.03831E 01
9.050	5.69476E-05	1.31348E 01	1.06378E 00	-1.08298E 01	9.23472E 00
9.103	5.70424E-05	1.20368E 01	1.46890E 00	-1.12747E 01	8.19047E 00
9.153	6.64034E-05	1.09034E 01	1.77545E 00	-1.16801E 01	7.10159E 00
9.203	6.57858E-05	9.79349E 00	1.68936E 00	-1.20319E 01	6.02559E 00
9.250	6.50949E-05	8.72390E 00	1.22053E 00	-1.23108E 01	4.98424E 00
9.300	6.34647E-05	7.67056E 00	6.742428-01	-1.25053E 01	4.01040E 00
9.350	5.99367E-05	6.60115E 00	4.140218-01	-1.26209E 01	3.13621E 00
9.400	5.421692-05	5.514098 00	5.813946-01	-1.26754E 01	2.38106E 00
9.450	4.69346E-05	4.45326E 00	9.87628E-01	-1.26861E 01	1.74816E 00
9.500	3.920992-05	3.48605ë 00	1.26519E 00	-1.26581E 01	1.23183E 00
9.550	3.186266-05	2.66483E 00	1,15182E 00	-1.25817E 01	8.30014E-01
9.600	2.486686-05	1.99655E 00	6.946425-01	-1.24429E 01	5.52340E-01
9.650	1.746376-05	1.44875E 00	2.11915E-01	-1.22364E 01	4.17958E-01
9.700	5.837616-06	9.83755E-01	4.56149E-02	-1.19726c 01	4.44579E-01
9.750	-1.15208E-06	5.95424E-01	2.97379E-01	-1.16727E 01	6.36915E-01
9.800	-1.17758E-05	3.19658E-01	7.521078-01	-1.13553E 01	9.83006E-01
9.850	-2.18684E-05	2.10730E-01	1.047236 90	-1.102548 01	1.46114E JO
9.900	-3.06141E-05	3.01864E-01	9.53861E-01	-1.06745E 01	2.05221E 00
9.950	-3.80494E-05	5.8063CE-01	5.53156E-01	-1.02907E 01	2.74842E 00
10.000	-4.49182E-05	9.98043E-01	1.70799E-01	-9.87212E 00	3.55195E 00
10.050	-5.19918E-05	1.50408E 00	1.231586-01	-9.43225E 00	4.46473E 00

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RUN NO 2

TIME	P-11	P-12		P-13	P-14	P-15
8.250	-1.08230E 00	-3.30480E	00	9.11169E~05	-2.76873E-03	
8.300	-1.03180E 00	-4.40003E	00	-4.58285E-04	-3.59691E-03	
8.350	-1.16414E 00	-5.51678E	00	-1.03801E-03	-4.44167E-03	
8.400	-1.37959E 00	-6.63807E	00	-1.63764E-03	-5.29603E-03	
8.450	-1.51749E 00	-7.,75264E	00	-2.24890E-03	-6.15128E-03	
8.500	-1.47727E 00	~8.85525E	00	-2.86695E-03	-6.99714E-03	
8.550	-1.29191E 00	-9.94223E	00	-3.48907E-03	-7.82299E-03	
8.600	-1.10037E 00	-1.10060E	01	-4.11193E-03	-8.61949E-03	
8.650	-1.04191E 00	~1.20325E	01	-4.72974E-03	-9.37928E-03	
8.700	-1.15260E 00	-1.30041E	01	-5.33460E-03	-1.00966E-02	
8.750	-1.34077E 00	-1.39048E	01	-5.918948-03	-1.07662E-02	
8.800	-1.45786E 00	-1.47251E	01	-6.47811E-03	-1.13818E-02	
8.850	-1.41000E 00	-1.546228	01	-7.01100E-03	-l.19369E-02	
8.900	-1.22682E 00	-1.61160E	01	-7.51825E-03	-1.24253E-02	
8,950	-1.03717E 00	-1.66838E	01	-7.99923E-03	-1.28425E-02	
9.000	-9.71831E-01	-1.71580E	01	-8.45016E-03	-1.31865E-02	
9.050	-1.06654E 00	-1.75286E	01	-8.86457E-03	-1.34569E-02	
9.100	-1.23728E 00	-1.77878E	01	-9.23583E-03	-1.36545E-02	
9.150	-1.34474E 00	-1.79350E	01	-9.55967E-03	-1.37790E-02	
9.200	-1.29873E 00	-1.79764E	01	-9.83483E-03	-1.38300E-02	
9.250	-1.12469E 00	-1.79215E	01	-1.00613E-02	-1.38075E-02	
9.300	-9.43017E-01	-1.77774E	01	-1.02380E-02	-1.37132E-02	
9.350	-8.78958E-01	-1.75472E	01	-1.03609E-02	-1.35509E-02	
9.400	-9.695176-01	-1.72307E	01	-1.04247E-02	-1.33263E-02	
9.450	-1.13722E 00	-1.68303E	01	-1.042516-02	-1.30459E-02	
9.500	-1.24901E OC	-1.63541E	01	-1.03618E-02	-1.27152E-02	
9.550	-1.21576E 00	-1.58166E	01	-1.02385E-02	-1.23394E-02	
9.600	-1.05821E 00	-1.52342E	01	~1.00615E-02	-1.19237E-02	
9.650	-8.906628-01	-1.46208E	Gl	-9.83653E-03	-1.147476-02	
9.700	-8.35380E-01	-1.39847E	01	-9.56773E-03	-1.10009E-02	
9.750	-9.31578t-01	-1.333C2E	01	-9.258222-03	-1.05119E-02	
9.800	-1.10671E 00	-1.26627E	01	-9.91262E-03	-1.00171E-02	
9.350	-1.231028 00	-1.19919E	01	-8.53835E-03	-9.52485E-03	
9.900	-1.21443E 00	-1.13324E	01	-8.147926-03	-9.04182E-03	
9.950	~1.07351E 00	-1.06998E	01	-7.75152E-03	-8.57386E-03	
10.000	-9.18799E-31	-1.01062E	01	-7.35913E-03	-8.12706E-03	
10.050	-8.72041E-01	-9.55686E	00	-6.97653E-03	-7.70816E-03	
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# TABLE 21 LISTINGS OF COMPUTER RUNS FOR RESPONSE TO INITIAL DEFORMATION

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	3	15 AL	C 1963		PAGE NO	1
RESPONSE	OF TITAN	III MODEL TO	INITIAL DIS	PLACEMENTS IN	ITS IST MOD	E
TIME SEC	VX IN/SEC	VY IN/SEC	VZ IN/SEC	DMEGAX RAD/SEC	OMEGAY RAD/SEC	DMEGAZ RAD/SEC
0.	0.	0.	0.	3.1607E-08	-7.5956E-09	4.8966E-13
0.100	0.	0.	0.	-3.4335E-23	-5.6986E-23	2.2879E-23
0.200	0.	· 0.	0.	3.5930E-20	2.8402E-21	-3.3608E-23
0.300	0.	0.	0.	-4.5005E-20	-1.89056-21	-5.90598-23
0.500	0.	0.	0	2 50816-20	2 78156-21	-0.0009E-23
0.600	0.	0.	0.	-1.30416-20	5-0833E-22	-4.4969E-23
0.700	0.	0.	0.	-1.76656-20	2.0467E-22	-2.6333E-23
0.800	0.	0.	0.	6.7122E-21	1.6087E-21	-9.4643E-24
0.900	0.	0.	Ο.	-1.32048-22	1.0621E-21	-1.54456-23
1.000	0.	0.	0.	-9.8119E-21	3.1537E-22	-3.7387E-23
1.100	0.	0.	0.	-1.5407E-21	6.5736E-22	-5.8698E-23
1.200	0.	0.	0.	9.5621E-22	6.5911E-22	-8.9278E-23
1.300	0.	0.	0.	-2.8005E-21	3.0510E-22	-7.5628E-23
1.400	0.	0.	0.	-1.9120E-21	2.6902E-22	-7.03162-23
1.500	0.	0.	0.	-6.8320E-22	2.90532~22	-0.52992-23
1.700	0.	0.	0.	-0.09/15-22	2.57996-22	-0.04496-20
1.800	0.	0.	0.	-1.4168E-21	2.34448-22	-5.69438-23
1,900	ő.	0.	0.	-1.3776E-21	2.37018-22	-5.4815E-23
2.000	0.	0.	0.	-2.8744E-22	2.7902E-22	-4.8226E-23
2.100	<b>0</b> .	0.	0.	-7.2223E-22	1.8981E-22	-5.1501E-23
2.200	0.	0.	ο.	-1.0682E-21	5.89016-23	-5.3456E-23
2.300	0.	0.	с.	4.1439E-22	-1.0077E-23	-5.29886-23
2.400	0.	0.	0.	1.1942E-21	-1.6722E-22	-5.1057E-23
2.500	0.	0.	0.	1.0817E-21	-4.1324E-22	-8.7380E-23
2.600	0.	0.	0.	2.2426E-21	-5.99216-22	-8.38368-23
2.700	0.	U. 0	0.	3.58876-21	-1.10626-22	-9.11266-23
2.800	0.	0.	0.	2.01036-21	-9.84875-22	-9.93116-23
3.000	0.	0.	0.	5.0501E-21	-1.20375-21	-5.82256-23
3,100	0.	0.	0.	5.0672E-21	-1.23761-21	-1.95776-23
3.200	0.	0.	0.	4.63802-21	-1.2134E-21	-1.9857E-23
3.300	0.	0.	0.	4.3373E-21	-1.09426-21	-1.29456-23
3.400	0.	0.	0.	3.82576-21	-9.20871-22	-5.50376-24
3.500	0.	0.	0.	2.80196-21	-7.1698E-22	-1.2010E-23
3.600	0.	0.	0.	1.8389E-21	-4.7236E-22	-2.5300E-23
3.700	0.	0.	0.	9.2397E-22	-2.11596-22	-4.1781E-23
3.800	0.	0.	0.	-1.1101E-22	3.11146-23	-2.12496-23
5.900	0.	0.	0.	-1.03108-21	2+48455-22	-3.2438E-23
4.000	0.	0.	0.	-1.70410-21	4.31315-22	3.20396-23
4.200	0.	0.	0.	-2.58128-21	6.4257E-22	2.33596-23
4.300	0.	0.	0.	-2.7029F-21	6.75651-22	1.96331-23
4.400	0.	0.	0.	-2.6625E-21	6.6863E-22	9.9917E-24
4.500	0.	0.	0.	-2.5279E-21	6.32068-22	3.4893E-24
4.600	0.	0.	0.	-2.3286E-21	5.81576-22	-3.2947E-24
4.700	0.	0.	0.	-2.1228E-21	5.31146-22	-3.8388E-24
4-800	0.	0.	0	-1.9696E-21	4.9261E-22	-9.1628E-24
4.900	0.	0,	0.	-1.9006E-21	4.75276-22	-7.4921E-24
5.000	0.	U. 0	0.	-1.9341E-21	4.83528-22	-7.4096E-24
2+100	<b>U</b> .	0.	U.	-2.10/12-21	2.1020c-22	-1.1011E-23

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PAGE NO 2

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TIME	P- 1	P- 2	P- 3	F- 4	P~ 5
ъ.	0.	0.	-2.01887E-04	3.53009E-14	4.54992E-08
0.100	-7.91742814	5.836366-14	-2.00961E-04	3.503886-14	4.52905E-08
0.200	-4.89893E-14	2.43619E-15	-1.98191E-04	3.40470E-14	4.46662E-08
0.300	-2.04699E-14	-4.27604E-14	~1.93603E-04	3.20930E-14	4.36322E-08
0.400	-3.60898E-14	1.58315E-14	-1.87239E-04	2.95346E~14	4.21978E-08
0.500	-2.55284E-14	2.01237E-14	-1.79156E-04	2.66036E-14	4.03763E-08
0.600	6.66803E-15	-2.11988E-14	-1.69431E-04	2.36676E-14	3.81844E-08
0.700	1.32001E-14	-4.95164E-15	-1.58150E-04	2.13828E-14	3.56421E-08
0.800	1.67673E-14	1.61358E-14	-1.45419E-04	1.94534E-14	3.27729E-08
0.900	3.73197E-14	-3.94288E-15	-1.31353E-04	1.67990E-14	2.96030E-08
1.000	4.87219E-14	-8.58367E-15	-1.16083E-04	1.38384E-14	2.616145-08
1.100	4.780946-14	7.67775E~15	-9.974696-05	9.33828E-15	2.247995-08
1.200	5.200195-14	3.9/2102-15	-8.249615-05	3.723312-15	1.859216-08
1.500	0-02014E-14	-4.8/0392~15	-0.448852-05	~1.02008E-15	1.453372-08
1.500	4.920110-14	2.010095-10		-1,192002-15	1.034202-00
1 600	3 340136-14	-7 072/46-14	-7 403165-04	-1.693605-14	1 712205-09
1 700	2 151396-14	-9 656675-17	1 173445-05	-1.983698-14	-2 644575-09
1 800	6 571286-15	3 55 204 - 15	3 096325-05	-7 366516-14	-6 978166-09
1.90.	-6.202478-15	1 204945-15	2 99680E-05	-2.936956-14	-1.124778-08
2.000	-1.88123F-14	-6.48876F-16	6.83949F-05	-3.239596-14	-1.54141E-08
2.100	-3.18605E-14	1.18158E-15	8-625425-05	-3-625498-14	-1.94390E-08
2.200	-4-19535E-14	8.680316-16	1.033725-04	-4-09326E-14	-2.32856E-28
2.300	-4.86433E-14	-1.07297E-15	1.19442E-04	-4.47814E-14	-2.69186E-08
2.400	-5.314128-14	-8.82162E-16	1.34466E-04	-4.809736-14	-3.03046E-08
2.500	-5.42791E-14	-5.68014E-16	1.49257E-04	-5.046576-14	-3.34125E-08
Z.600	-5.12706E-14	-1.71376E-15	1.60687E-04	-5.1650GE-14	-3.62140E-08
2.700	-4.522898-14	-2.18265E-15	1.716432-04	-5.226668-14	-3.86831E-08
2.800	-3.66095E-14	-1.81732E-15	1.81025E-04	-5.24688E-14	-4.07974E-08
2.903	-2.531338-14	-2.091798-15	1.88745E-04	-5.240451-14	-4.25373E-08
3.000	-1.231726-14	-2.40334E-15	1.94734E-04	-5.22268E-14	~4.38970E-08
3.100	1.214218-15	-2.00.338-15	1.93936E-04	-5-216765-14	-4.48340E-08
3.200	1.473046-14	-1.62036E-15	2.01313E-04	-5.20862E-14	-4.53697E-08
3.300	2.744128-14	-1.445368-15	2.01843E-04	-5.17180E-14	-4.54892E-08
3.400	3.827846-14	-9.141056-16	2.005211-04	-5.08138E-14	-4-51913E-08
3-303	4.0000000-14	-1.990178-10	1.973602-04	-4.9318/E-14	-4.44/88E-08
3 700	5+203305-14	0 202505-10	1.923412-04	-4+/00110-14	-4.333825-08
3 200	5 74410775-14	9.20939E-10 1 60557E-15	1 773105-04	-4.3/939416-14	
3.900	4.78269F-14	2 130025-15	1.671638-04	-3.927005-14	-3.766805-08
4.000	3.996885-14	2.511876-15	1.555436-04	-3.5083814	-3 505476-08
4.100	2.950728-14	2.825376-15	1.425165-04	-3,27567E-14	-3.211885-08
4.200	1.715036-14	2.977105-15	1-281825-04	-2.941631-14	-2-88883E-08
4.300	3.717576-15	2.89505E-15	1.126726-04	-2-627818-14	-2-53927E-08
4.400	-9.96161E-15	2.65367E-15	9.61275E-05	-2.31153E-14	-2.16642E-08
4.500	-2.30098E-14	2.262432-15	7.870148-05	-1.98485E-14	-1.77369E-08
4.600	-3.45648E-14	1.68511E-15	6.055338-05	-1.53974E-14	-1.36468E-08
4.700	-4.38942E-14	9.744856-16	4.18496E-05	-9.94995E-15	-9.43161E-09
4.800	-5.041C5E-14	1.96341E-16	2.276206-05	-4.26561E-15	-5.12985E-09
4.900	-5.36838E-14	-6.30679E-16	3.46556E-06	1.112518-15	-7.81029E-10
5.000	-5.35017E-14	-1.46092E-15	-1.58626E-05	5,49419E-15	3.57495E-09
5.100	-4.98861E-14	-2.22031E-15	-3.50453E-05	9.68808E-15	7.89813E-09

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PAGE NO 3

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TIME	P- 6	P- 7	P- 8	P- 9	P-10
0.	0.	0.	0.	7.967495-03	0.
0.100	-5.24749E-18	1.42784E-12	-3-332485-12	7.93094F-03	1.45958E-12
0.200	1.67454E-19	9-05074E-13	-1.48509E-13	7.821636-03	8-815258-13
0.300	4-42065E-18	4-034265-13	2.46037E-12	7.640568-03	3-43094E-13
0.400	-1.20954E-18	6.60659E-13	-9.01488E-13	7.389398-03	6.55506E-13
0.500	-1.54923E-18	4.68396E-13	-1.18409E-12	7.070425-03	4.62602E-13
0.600	2.34357E-18	-1.01742E-13	1.18737E-12	6.68658E-03	-1.414358-13
0.700	6.75247E-19	-2.31949E-13	2.69993E-13	6.24140E-03	-2.49448E-13
0.800	-1.35213E-18	-3.06958E-13	-9.628005-13	5.73896E-03	-3.04530E-13
0.900	5.04842E-19	-6.74598E-13	1.78879E-13	5.18386E-03	-6.86419E-13
1.000	7.75154E-19	-8.85980E-13	4.67616E-13	4.58120E-03	-8.90867E-13
1.100	-8,99179E-19	-8.80594E-13	-4.61811E-13	3.93652E-03	-8.62975E-13
1.200	-6.34596E-19	-9.68454E-13	-2.553828-13	3.25571E-03	-9.48436E-13
1.300	4.20975E-20	-1.03624E-12	2.736856-13	2.545048-03	-1.01338E-12
1.400	-7.82404E-19	-9.18358E-13	-1.02649E-13	1.81102E-03	-8.788846-13
1.500	-1.16991E-18	-7.68025E-13	-2.79686E-13	1.06038E-03	-7.202116-13
1.600	-6.89380E-19	-6.38384E-13	7.57786E-14	3.00019E-04	-5.90308E-13
1.700	-8.31744E-19	-4.19314E-13	6.38168E-14	-4.63097E-04	-3.65278E-13
1.800	-1.18216E-18	-1.48553E-13	-1.37414E-13	-1.22196E-03	-9.109648-14
1.900	-9.01154E-19	8.72593E-14	-8.64643E-16	-1.96962E-03	1.33940E-13
2.000	-6.46130E-19	3.205426-13	1.12889ć-13	-2.69921E-03	3.655286-13
2.100	-6.81604E-19	5.62128E-13	6.97098E-15	-3.40403E-03	5.998006-13
2.200	-4.43380E-19	7.53012E-13	1.18085E-14	-4.07762E-03	7.76997E-13
2.300	-2.54365E-20	8.83360E-13	1.09638E-13	-4.71379E-03	8.90622E-13
2.400	2.08297E-19	9.73399E-13	8.301516-14	~5.30672E-03	9.64618E-13
2.500	4.58469E-19	1.00302E-12	4.05911E-14	-5.85097E-03	9.76493E-13
2.600	8.43077E-19	9.57536E-13	7.98139E-14	-6.34153E-03	9.12261E-13
2.700	1.12904E-18	8.553416-13	8.31529E-14	-6.77391E-03	7.94120E-13
2.800	1.299732-18	7.04619E-13	3.81104E-14	-7.14414E-03	6.30499E-13
2.900	1.4/9055-18	5.03/09E-13	3.088711-14	-7.44883E-03	4.19447E-13
3.100	1.084905-18	2.69303E-13	3.27229E-14	-7.685182-03	1.79896E-13
3 200	1.0000000000000000000000000000000000000	2+213045-14	3.123055-10	-7.85102E-03	-6.641641-14
3 300	1 220215-10	-2.2/3020-13	-2.508415-14	-7.944832-03	-3-098428-13
3 400	0 006866-10	-4.0490000-10	-2.672662.16	-7.013505.03	-2-3282/2-13
3 500	5 158076-19	-0./11200-13	-4-0/2445-14	-7.913582-03	-7.248015-13
3 600	9 992155-20	-0.557756-15	-0.031012-14	7 503606 03	-0.059208-13
3.700	-3.704535-19	-9.400202-13	-7.113375-14	-7 336405-03	-9.520020-13
3.800	-8,442515-19	-9.901002-10	-7 454646-14	-1.520090-05	-9+110920-13
3,900	-1.27873E-18	-9.07680E-13	-6.925556-14	-6 596316-03	-9-3093325-13
4,000	-1-64869E-18	-7.752836-13	-5.62026E-14	-6 138535-03	-6-903666-13
4.100	-1.93971E-18	-5.93065E-13	-4.37843E-14	-5.624426-03	-4.830395-13
4.200	-2.11930E-18	-3-731235-13	-2-894316-14	-5.058716-03	-7-52335E-13
4.300	-2.16578E-18	-1-29893E-13	-9.26417E-15	-4.446595-03	-5.683156-15
4.400	-2.07846E-18	1.21718E-13	9.89283E-15	-3.793676-03	2-41574E-13
4.500	-1.85604E-18	3.65778E-13	2.68961E-14	-3.10595E-03	4-73370F-13
4.600	-1.50138E-18	5.86379E-13	4.362228-14	-2.38974E-03	6.741728-13
4.700	-1.03487E-18	7.69645E-13	5.79540E-14	-1.65160E-03	8,31140E-13
4.800	-4.84504E-19	9.04138E-13	6.74623E-14	-8.98303E-04	9.342916-13
4.900	1.20608E-19	9.811296-13	'7.29383E-14	-1.36769E-04	9.76674E-13
5.000	7.434708-19	9.95677E-13	7.44283E-14	6.26020E-04	9.55487E-13
5.100	1.34193E-18	9.47014E-13	7.07202E-14	1.38306E-03	8.72289E-13
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TIM	IE P-11	P-12	P-13	P-14	P-15
٥.	0.	7.967498-03	6.269166-06	6.26916E-06	
0.1	00 -3.58316E-12	· 7.93094E-03	6.240408-06	6.24040E-06	
0.2	00 -2.079850-13	7.821636-03	6.15439E-06	6.15439E-06	
0.3	00 2.50532E-12	7.640568-03	6.01191E-06	6.01191E-06	
0.4	00 -1.01196E-12	7.38939E-03	5-81428E-06	5.81428E-06	
0.5	00 -1.24606E-12	7.070426-03	5.56330E-06	5.56330E- <b>06</b>	
0.6	00 1.247741-12	6.68658E-03	5.2h129t-06	5.26129E <b>-06</b>	
0.7	00 2.76369E-13	6.241406-03	4.9110 H-06	4.9110CE-06	
0.8	00 -9.664792-13	5.738962-63	4.515658-06	4.5156>E-06	
0.9	00 2.61941E-13	5.193366-03	4.07983E-06	4.07988E-06	
1.0	00 5.48889t-13	4.58120E-C3	3.604681-06	3.60468E-06	
1.1	00 -4.11289E-13	3.936526-03	3.097471-06	3.097426-06	
1.2	00 -1.665688-13	3.255716-03	2.551731-06	2.561732-06	
1.1	00 3.136851-13	2.545042-03	2.002558-00	2.002558-06	
1.1	00 -3.871000-14	1.511021-03	L 424398-90	1.424998~00	
1.7		1.000078-03	1 34 374t = 17	3.343346-01	
1.0	00 1.40104(-13		2. 5000 (1 -07	-) 463855-07	
1.1	00 - 7.511010 - 14	-1 221071-01		-3 414046-07	
1 0	00 -1.521032-15	-1.271976-03	- 3+010 740-07	-1 540786-06	
2.0	00 8.762001-14	-2.64.212-03	-2.124.151-06	-2.123856-06	
2.1	00 -4.600631-14	-3.494031-03	-1.672431-06	-2.678436-06	
2.2	00 -5-054976-14	-4-07/521-04	-3-21441-06	-1.20844t-06	
2.3	00 3.889511-14	-4.713701-03	- 4- 1.19-11-06	-3.709618-06	
2.4	00 -1.119576-15	-5.300/31-03	4.175558-06	-4.175568-06	
2.5	00 -4.471920-14	-5.850978-03	-1 - 573791-26	-4.603796-06	
2.6	00 2.342980-15	-6.34153F-03	-4.91.9722-06	-4.93978E-06	
2.7	00 1.17782F-14	-6.773911-03	-5.330578-06		
2.8	00 -7.2120 46-14	-7.144146-03	-5.62131L-C6	-5.621318-06	
2.9	00 -9.683216-15	-7.44-038-03	-5.85106.006	-5.861362-06	
3.0	01 1.18319E-14	-1.632131-03	-6.247021 -26	-6.04702E-06	
3.1	00 -1.779191-15	-7.851028-03	-6.177511-06	-6.17751F-06	
3.2	00 -5.354300-15	-7.944432-03	- 5-251421-06	-6.251326-06	
3.3	00 1.024436-14	-7.965741, 03	-6.267196-06	-1.267721-06	
3.4	00 9.85/135-15	-1-713586-03	-6.226741-06	-6.22674E-06	
3.5	00 3.014641-15	-1.186616 (3	-6.12856E-66	-5.12356E-06	
3.6	00 9.645051-15	-7-592586 (3)	-5.974162-00	-5.97416E-06	
3.1	00 1.247751-14	*********		-3.164951-08	
3.0	00 7.143076-15	-6.993596-63	-3+1, 285:-06	-3.502851-00	
3.9	00 6.332732-15				
9.0	00 8.200911-10		**************************************	~~,330078~00	
	00 4.01900L-19				
7.2	00 L+L23001.*13			- 7.700910-00	
4.4	00 -1 467476-16			- 3.490701-00	
4.6	00 -4.95619-15		-2.700022-00	-7.463902L-VO	
4.2	00 -4.200176-12		-1.820356-04	-1.830356-04	
4.0	00 -7.187/96-15	~1.651601-01	-1.2/3555-06	-1.279556-06	
4.8	00 -8.798445-15	-8.991011-04	-1.06-231-01	-1.068236-07	
4.9	00 -9.42088F-15	-1.36/6%2-04	-1.0761607	-1.076156-07	
5.0	00 -8.74078E-15	5.260201-04	4.425171-01	4. 125791-07	
5.1	00 -8.092571-15	1.343061-03	1.024/51-06	1.088256-06	

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## APPENDIX II

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## A VIBRATION ANALYSIS OF A ONE-FIFTH SCALE MODEL OF THE SATURN VEHICLE

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#### 1.0 COMPONENT BREAKDOWN OF SATURN MODEL

#### 1.1 Introduction

For the purpose of analysis, the Saturn Model was divided into eight (8)component structures. Each of these component structures was then analyzed independently by methods applicable to that particular type of structure. The selection of constraints for each of these component structures was based on the idealized constraints imposed by adjacent component structures such that each separate analysis was compatible with the idea of eventually coupling the component structures as described in Section 5.1.3. This final coupling process is presented in Section 3.0 of this Appendix.

#### 1.2 Saturn Model Components

The component structures into which the Saturn Model was divided, their idealized constraints, and their unit designations are presented below. It should be noted that a completely darkened area indicates a portion of the vehicle which was considered rigid for the purpose of aiding in the final coupling of the components.



FIGURE 115 SATURN SA-1 LAUNCH VEHICLE



(4) Spider Beam supported on eight simple supports (points of contact with center lox tank). Designated by (S).





(8) Outrigger and engines considered as a rigid body. Designated as (R).



#### 2.0 ANALYSIS OF INDIVIDUAL COMPONENTS

### 2.1 Upper Stage (U)

#### 2.1.1 Collocation Point Geometry for Upper Stage

One of the basic assumptions in the analysis of the Upper Stage was that this portion of the vehicle could be represented by nine (9) collocation points. These points were spaced equidistant along the x axis (neutral axis) as shown in Figure 117.



FIGURE 117 COLLOCATION POINT GEOMETRY

The x coordinates (body stations) of the assumed collocation points are presented in Table 22.

#### 2.1.2 Analysis of Upper Stage

The analysis of the Upper Stage was based on the assumption that the elastic behavior of this portion of the vehicle could be determined through the use of equivalent beam theory. The use of this theory allowed the structure of this stage to be presented as a beam as shown in Figure 118.


FIGURE 118 EQUIVALENT BEAM

The analysis of this equivalent beam allowed for not only a different total mass in each bay, but also the addition of concentrated mass items. This allowed the consideration of the lead ballast weights as point loads. This consideration displays itself in the computation of the point-mass matrix of the kinetic energy expression.

The kinetic energy of the equivalent beam was expressed in matrix form as,

$$\tau = \frac{1}{2} \{ \hat{p}_{u} \} [A_{u}] \{ \hat{p}_{u} \}$$
(II-1)

where

$$[A_{u}] = \sum_{i=1}^{4} [T]'_{i} [A]_{i} [T]'_{i}$$
(II-2)

where [A]<sub>i</sub> was a function of the individual bay (see Paragraph 5.1.2.2) such that

$$\begin{bmatrix} A \end{bmatrix}_{i} = w_{i} \begin{bmatrix} \bar{A} \end{bmatrix}_{i} + \sum_{j} w_{ij} \begin{bmatrix} J \end{bmatrix}_{j}^{\prime I} + \frac{3}{5} + \frac{3}{5}^{\prime} + \frac{3}{5}^{$$

where  $m_i$  was the total mass of the ith bay,

my, was the jth concentrated mass in the ith bay (perhaps it should be noted that Equation II-3 is somewhat general · in that if any particular bay did not have a concentrated mass, then the summation term was neglected or zero), in the i<sup>th</sup> bay by the relation,

$$z = \frac{x_{i_1} - x_{i_2}}{x_i - x_{i_3}}$$
 (II-4)

such that the matrix distributed the jth concentrated mass diparabolically between four collocation points (two to either side of the bay in which it was located). The  $[\tilde{A}]_i$  matrix in Equation II-3 was a non-dimensional matrix which distributed the total mass of the i<sup>th</sup> bay between four collocation points (two points on either side of the i<sup>th</sup> bay). Due to the absence of a fourth point in the case of the first and last bays, the elements of this matrix varied. This variation is shown below.

(II-5)

$$\begin{bmatrix} \bar{A} \end{bmatrix}_{\hat{L}} = \begin{pmatrix} \frac{1}{6770} \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 1820 & 1224 & -164 \\ 0 & 1224 & 3264 & -258 \\ 0 & -164 & -298 & 32 \end{bmatrix}$$
 for  $1 = 1$   
$$\begin{cases} \frac{1}{6770} \begin{bmatrix} 16 & -198 & -120 & 12 \\ -168 & 2720 & 1228 & -120 \\ -120 & 1228 & 2720 & -168 \\ 12 & -120 & -168 & 16 \\ 12 & -120 & -168 & 16 \\ \end{bmatrix}$$
 for  $1 = 2,3,...,7$   
$$\begin{cases} \frac{1}{6770} \begin{bmatrix} 32 & -228 & -164 & 0 \\ -258 & 3264 & 1224 & 0 \\ -258 & 3264 & 1224 & 0 \\ 0 & 0 & 0 & 0 \\ \end{bmatrix}$$
 for  $1 = 8$ 

The remaining term in expression  $[T]_i$ , was a non-dimensional matrix which positioned the distributed mass of the i<sup>th</sup> bay in the final point-mass matrix  $[A_{it}]$ . This matrix is presented in Table 22.

The strain energy stored in the equivalent beam was expressed in terms of the applied loads  $P_{U_2}$ , as;

$$u = \frac{1}{2} \{ P_{0} \}^{2} [E_{0}] \{ P_{0} \}$$
(II-6)

where  $[E_U]$ , the collocation point influence coefficient matrix for the Upper Stage, was determined from EI and GA slice data (presented in Table 30), through the use of the complementary strain energy method presented in Section 5.1.1.2 of this report. This collocation point influence coefficient matrix for the Upper Stage is presented in Table 22.

The modes of the Upper Stage (cantilevered at  $x = x_0 = 125.306$ ) and their respective eigenvalues were calculated by the iteration of the expression,

$$[\varepsilon_{u}][A_{v}]\{\varphi\} = \lambda\{\varphi\} \qquad (II-7)$$

These modes and their respective frequencies are presented in Table 22.

2.2 Middle Stage (M)

### 2.2.1 Collocation Point Geometry For Middle Stage

In the interest of additional accuracy, in the analysis of the Middle Stage the number of collocation points was increased to fifteen (six points more than were used in the analysis of the Upper Stage). These points were spaced equidistant along the x axis as shown in Figure 119.



FIGURE 119 COLLOCATION POINT GEOMETRY FOR MIDDLE STAGE

The x coordinates of the Middle Stage collocation points are presented in Table 23.

### 2.2.2 Analysis of Middle Stage

The analysis of the Middle Stage was somewhat complicated by the structure found in the adapter portion of the stage (that portion of the stage between body stations 193.345 and 211.320). This portion of the stage did not lend itself to an analysis based on equivalent beam theory while the remaining portion did. It was therefore decided that the analysis of the Middle Stage should consist of two parts; (1) an analysis of the entire structure based on equivalent beam theory in which the adapter portion of the stage was considered rigid, and (2) an analysis of the adapter as an independent structure based on complementry strain energy methods. It is the first part of the analysis which will be presented here.

The assumption that the adapter is rigid and the assumptions implied by the use of the equivalent beam theory lead to the idealization of the Middle Stage structure shown in Figure 120.



FIGURE 120 EQUIVALENT BEAM

In the analysis of this equivalent beam it was considered extremely advantageous to allow for the treatment of structural items such as ring stiffeners, radial members, plates, and tank caps as concentrated mass items. This allowance was made in the computation of the collocation point-mass matrix in the expression for kinetic energy.

The kinetic energy of this equivalent beam for the Middle Stage was expressed in matrix form as,

$$T = \frac{1}{2} \{ \dot{p}_{M} \} [A_{M}] \{ \dot{p}_{M} \}$$
 (II-8)

where

$$[A_{M}] = \sum_{i=1}^{M} [T]'_{i} [A]_{i} [T]_{i}$$
(II-9)

where [A]<sub>i</sub> was a function of the individual bay.

The final point mass matrix for the Middle Stage is presented in Table 23. It should be noted that the assumption that the adapter portion of the stage was rigid has had no effect whatsoever on the analysis thus far. This assumption has no bearing on the calculation of  $[A_M]$ .

The strain energy stored in the equivalent beam was expressed in terms of applied loads,  $P_{\!M_1}$  , as;

$$U = \frac{1}{2} \{ P_{M} \}' [E_{N}] \{ P_{M} \}$$
(II-10)

where  $[E_M]$ , the collocation point influence coefficient matrix for the Middle Stage, was determined from EI and GA slice data (presented in Table 30) through the use of the complementary strain energy method presented in Section 5.1.1.2 of this report. This collocation point influence coefficient matrix for the Middle Stage is presented in Table 24.

The modes of the Middle Stage and their respective frequencies were determined from the iteration of the expression;

$$[\mathsf{E}_{\mathsf{M}}][\mathsf{A}_{\mathsf{M}}]\{\varphi\} = \lambda\{\varphi\} \tag{II-11}$$

These modes and their respective frequencies are presented in Table 24. These modes are actually for the Middle Stage cantilevered at  $x = x_{15} = 211.320$ , but due to the rigidity of the adapter section, they will appear as though the stage were cantilevered at x = 193.345.

2.3 Adapter (A)

### 2.3.1 Generalized Coordinates for the Adapter

One of the basic assumptions in the analysis of the adapter was that the elastic behavior of this portion of the Middle Stage could be described through the use of two degrees-of-freedom ( $\zeta_A$  and  $\theta_A$ ). These degrees-of-freedom are shown in Figure 121.



FIGURE 121 DEGREES-OF-FREEDOM FOR ADAPTER

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As may be noted, the degrees-of-freedom selected here are only those of primary importance. With respect to the final analysis,  $\zeta_A$  and  $\theta_A$  adequately describe the elastic behavior of the adapter.

### 2.3.2 Analysis of the Adapter

The analysis of the adapter was based on the complementary strain energy method as presented in Section 5.1.1.2 of this report. Generalized loads were applied to an idealized version of the adapter structure as shown in Figure 122.



### FIGURE 122 GENERALIZED ADAPTER LOADS

The strain energy stored in the tension members, compression members, and the conical shell was written in terms of the generalized loads,  $Z_A$  and  $\Theta_A$  (associated with the generalized coordinates  $\zeta_A$  and  $\theta_A$ ), with the result that the total strain energy stored in the adapter was written as,

$$\boldsymbol{\omega} = \frac{1}{2} \left\{ \boldsymbol{Z}_{A} \cdot \boldsymbol{\Theta}_{A}^{*} \right\} \begin{bmatrix} \boldsymbol{E}_{A} \end{bmatrix} \begin{bmatrix} \boldsymbol{Z}_{A} \end{bmatrix}$$
(II-12)

where  $[E_A]$  was found to be,

It should be noted that the mass of the adapter was included in the mass of the Middle Stage and therefore need not be considered in this analysis.

### 2.4 Spider Beam (S)

### 2.4.1 Collocation Point Geometry for Spider Beam

The primary interest in the selection of collocation points, and generalized coordinates, for the Spider Beam was the fact that this member coupled together the motions of the adapter, the center lox tank, and the four outer lox tanks. With this consideration in mind, eight (8) collocation points and eleven (11) generalized coordinates (the three additional coordinates were used to describe the rigid body displacements) were chosen and are shown in Figures 123 and 124, respectively.



FIGURE 123 COLLOCATION POINT GEOMETRY FOR SPIDER BEAM

It should be noted that only the three rigid body displacements are displacements (of a point in the plane of the supports).



FIGURE 124 GENERALIZED COORDINATES FOR SPIDER BEAM

606

### 2.4.2 Analysis of Spider Beam

The analysis of the Spider Beam was based on the complementary strain energy method presented in Section 5.1.1.2 of this report. Generalized loads, associated with the generalized coordinates shown in Figure 124 were applied to the structure, and the strain energy stored in each structural member written in terms of these loads. These expressions for strain energy in the individual members were then combined to form an expression for the total strain energy stored in the Spider Beam, which was;

$$\bigcup = \frac{1}{2} \left\{ P_{s} \gtrsim_{s} Z_{s} \Theta_{s} \right\} \begin{bmatrix} \mathsf{E}_{s} \end{bmatrix} \begin{bmatrix} P_{s} \\ \vdots \\ Z_{s} \\ \Theta_{s} \end{bmatrix}$$
 (II-14)

where  $[E_S]$  is the structural influence coefficient matrix presented in Table 25.

The kinetic energy of the Spider Beam was written in terms of the generalized velocities and appeared in matrix form as,

$$T = \frac{1}{2} \left\{ \dot{p}_{s} \dot{s}_{s} \dot{s}_{s} \dot{\varphi}_{s} \right\} \begin{bmatrix} A_{s} \end{bmatrix} \begin{bmatrix} \dot{p}_{s} \\ \dot{s}_{s} \\ \dot{\varphi}_{s} \end{bmatrix}$$
(II-15)  
$$\begin{bmatrix} A_{s} \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{bmatrix}$$
(II-16)  
$$\begin{bmatrix} 11-16 \\ 0 \\ 0 \end{bmatrix}$$
(II-16)

6933.65

where

### 2.5 Center Lox Tank (L)

### 2.5.1 Collocation Point Geometry for Center Lox Tank

In the analysis of the Center Lox Tank, fifteen (15) collocation points were selected to represent this portion of the vehicle. These points were placed equidistant along the x axis as shown in Figure 125.



FIGURE 125 COLLOCATION POINT GEOMETRY FOR CENTER LOX TANK

The x coordinates (body stations) of these points are presented in Table 26.

### 2.5.2 Analysis of Center Lox Tank

The analysis of the Center Lox Tank was based on the assumption that the elastic behavior of this portion of the vehicle could be determined through the use of equivalent beam theory. This assumption lead to the idealization of the structure shown in Figure 126.



FIGURE 126 EQUIVALENT BEAM

In this analysis, it was considered advantageous with respect to accuracy to consider structural members such as ring stiffeners, tank caps, and plates as concentrated weight items. This consideration was made in the computation of the point-mass matrix used in the expression for kinetic energy. The kinetic energy for this equivalent beam was expressed in matrix form as,

$$T = \frac{1}{2} \{ \dot{p}_{L} \} \{ A_{L} \} \{ \dot{p}_{L} \}$$
 (II-17)

where

$$[A_{i}] = \sum_{i=1}^{H} [T]_{i}^{'} [A]_{i} [T]_{i}$$
(II-18)

where  $[A]_i$  was a function of the individual bay. This final point-mass is presented in Table 26.

The strain energy stored in the equivalent beam was expressed in terms of the applied loads,  $P_i$ , as;

$$U = \frac{1}{2} \{ P_{i} \} [E_{i}] \{ P_{i} \}$$
(II-19)

where  $[E_L]$ , the collocation point influence coefficient matrix, was determined from EI and GA slice data (presented in Table 30) through the use of the complementary strain energy method presented in Section 5.1.1.2 of this report. This collocation point influence coefficient matrix for the Center Lox Tank is presented in Table 27.

The mode shapes for the Center Lox Tank were determined by the iteration of the expression,

$$[E_{L}][A_{L}H\varphi] = \lambda \{\varphi\}$$
(II-20)

The mode shapes for the Center Lox Tank cantilevered at  $x = x_{15} = 350.409$  and their respective frequencies are presented in Table 27.

2.6 Fuel Tanks (F)

### 2.6.1 Collocation Point Geometry for Fuel Tanks

Each of the Fuel Tanks were analyzed on the basis of fifteen (15) collocation points. These points were equally spaced along the x axis (neutral axis) as shown in Figure 127. These points were chosen in preparation for an analysis based on equivalent beam theory. It was thought that due to the absence of both axial loading and internal pressure, the application of thin shell vibration theory was not necessary.





It should be noted that the collocation point selection and resulting analysis was the same for each of the four Fuel Tanks. Although a small asymmetry did exist in the complementary bending planes, it was found that this condition did not appreciably affect the final results and was therefore ignored.

### 2.6.2 Analysis of Fuel Tanks

The analysis of each of the four Fuel Tanks was based on the equivalent beam theory. This assumption implied that the elastic behavior of the Fuel Tanks could be determined by analyzing a beam with the same mass distribution along the x axis and identical stiffness in the x-z plane. This idealized, equivalent beam is shown in Figure 128.



FIGURE 128 EQUIVALENT BEAM FOR FUEL TANK

In this analysis of the Fuel Tanks, structural items such as ring stiffeners, tank caps, plates, and fittings were considered concentrated masses. This consideration was incorporated in the computation of the point-mass matrix which appeared in the expression for kinetic energy. The kinetic energy was also expressed in terms of the generalized velocities,  $P_{\rm F_i}$ , as;

$$\tau = \frac{1}{2} \{ \dot{p}_{F} \}^{\prime} [A_{F}] \{ \dot{p}_{F} \}$$
(II-21)

where  $[A_F]$ , the point-mass matrix, was expressed as;

$$[A_{F}] = \sum_{i=1}^{M} [T]_{i} [A]_{i} [T]_{i}$$
(II-22)

where  $[A]_i$  was a function of the individual bay. This final point-mass matrix is presented in Table 28.

The strain energy stored in the equivalent beam was expressed in terms of the generalized loads,  $\mathrm{P}_{\mathrm{F}_{\mathrm{i}}}$ , associated with the generalized coordinates.

$$U = \frac{1}{2} \{ P_{\rm e} \} [E_{\rm e} ] \{ P_{\rm e} \}$$
(II-23)

Where the collocation point influence coefficient matrix for the Fuel Tank,  $[E_F]$ , on two simple supports (as shown in Figure 128, was derived by first calculating an influence matrix for the beam cantilevered at x =  $x_{15}$  = 350.717. Using complementary strain energy principles these influence coefficients were transformed from cantilevered restraints to simply supported constraints. This transformation process was expressed in matrix form as;

$$[E_{F}] = [\top]^{\binom{\text{contlinence}}{(\text{contlinence})}} [\top]$$
(II-24)

The transformation matrix, [T], was derived from equilibrium conditions on the loads and support reactions.

The mode shapes for the Fuel Tanks were determined through the iteration of the expression

$$[\varepsilon_{F}][A_{F}][\phi] = \lambda \{\phi\} \qquad (II-25)$$

It should be noted that the mode shapes for each of the four Fuel Tanks were identical, and that the mode shapes presented in Table 29 are for one tank. It should also be noted that these mode shapes were for the Fuel Tank on two simple supports.

### 2.7.1 Degrees-of-Freedom for Outrigger

The assumptions that the Outrigger was rigid and limited to plane motion restricted the structure to three degrees-of-freedom ( $\xi_{\rm R}$ ,  $\zeta_{\rm R}$ , and  $\theta_{\rm R}$ ) as shown in Figure 129. Also shown, for reference purposes, in Figure 129 are the displacements of the Outrigger center of mass ( $\xi$  and  $\zeta$ ).



### FIGURE 129 OUTRIGGER DEGREES-OF-FREEDOM

### 2.7.2 Outrigger Analysis

The analysis of the Outrigger was based on the assumption that this portion of the structure was a rigid body limited to plane motion. The kinetic energy of this body was expressed in terms of  $\xi$  and  $\zeta$  (deflections of the center of mass) and  $\theta$ (rotation of the body) as,

$$T = \frac{1}{5} M_{R} \left( \dot{s}^{2} + \dot{s}^{2} \right) + \frac{1}{5} I_{R} \dot{\Theta}^{2}$$
 (II-26)

Noting the relations,

$$\begin{aligned} s &= s_{R} - (\bar{s} - s_{R}) \Theta_{R} \\ \bar{s} &= s_{R} \\ \Theta &= \Theta_{R} \end{aligned} \tag{II-27}$$

it was seen that,

$$\dot{j} = \dot{j}_{R} - (\bar{x} - x_{R})\dot{\Theta}_{R}$$
$$\dot{j}^{2} = \dot{j}_{R}^{2} - 2(\bar{x} - x_{R})\dot{\Theta}_{R}\dot{j}_{R} + (\bar{x} - x_{R})^{2}\dot{\Theta}_{R}^{2}$$
(II-28)

and that

$$\dot{\dot{s}}^{2} = \dot{\dot{s}}_{R}^{2}$$
(II-29)  
$$\dot{\dot{\theta}}^{2} = \dot{\theta}_{R}^{2}$$

The combination of Equations II-26, II-27, II-28, and II-29 resulted in an expression for kinetic energy in terms of  $\xi_{\rm R},\ \xi_{\rm R},$  and  $\theta_{\rm R}$ .

$$\tau = \frac{1}{2} M_{R} \left( \dot{j}_{R}^{2} - 2(\tilde{\chi} - \chi_{R}) \dot{\Theta}_{R} \dot{J}_{R} + (\tilde{\chi} - \chi_{R})^{2} \dot{\Theta}_{R}^{2} + \dot{J}_{R}^{2} \right) + \frac{1}{2} I_{R} \dot{\Theta}_{R}^{2}$$
(II-30)

which was then expressed in matrix form as,

$$T = \frac{1}{2} \left\{ \frac{\dot{s}_{R}}{\dot{s}_{R}} \frac{\dot{b}_{R}}{\dot{b}_{R}} \right\} \begin{bmatrix} M_{R} & O & O \\ 0 & M_{R} & -M_{R}(\bar{x} - x_{R}) \\ 0 & -M_{R}(\bar{x} - x_{R}) & I_{R} + M_{R}(\bar{x} - x_{R})^{2} \end{bmatrix} \begin{bmatrix} \dot{s}_{R} \\ \dot{b}_{R} \end{bmatrix}$$
(II-31)

where  $M_R$  was the total mass of the Outrigger (269.3 lb<sub>m</sub>) I +  $M_R(\bar{x}-x_R)^2$  was the moment of inertia about point R (48641.5 lb<sub>m</sub>-in<sup>2</sup>)  $-M_R(\bar{x}-x_R)$  was the first moment about point R (1901.0 lb<sub>m</sub>-in).

Equation II-31 was then written in final matrix form as,

$$\tau = \frac{1}{2} \{ \dot{p}_{R} \} [A_{R} ] \{ \dot{p}_{R} \}$$
(II-32)

where

$$\{\dot{\mathbf{p}}_{R}\} = \begin{bmatrix} \dot{\mathbf{s}}_{R} \\ \dot{\mathbf{s}}_{R} \\ \dot{\mathbf{\Theta}}_{R} \end{bmatrix}$$
(II-33)

### TABLE 22 UPPER STAGE (U) MASS, STIFFNESS, AND CANTILEVERED MODES

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### COLLOCATION POINT GEDWETRY BAY LENGTH - 15.663 INCHES COLLOCATION X-COORDINATE POINT INCHES FROM NOSE)

2	1.56630E 01	
3	3.13260E 01	
4	4.69890E 01	
5	6.26520E 01	
6	7.83160E 01	
7	9.39780E 01	
8	1.09641E 02	
9	1.25306E 02	

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### COLLOCATION POINT MASS MATRIX (TOTAL MASS = 878.585 LBM) (BALLAST TANK FILLED WITH WATER)

COLL. POINT	1	2	3	4	5	6	7		,
Ĩ	7.210456-01	4.01681E-01	~1.09422E-01	4.21880E-03	-0.				
2	4.01681E-01	2.44889E 00	~2.32715E 00	-1.93094E 00	2.04888E~01				
3	-1.09422E-01	-2.32716E 00	3.90913E 01	1.10699E 01	-6.48231E 00	4.51175E-01	0.	0.	٥.
4	4.21880E-03	~1.93094E 00	1.106998 01	1.25876E 02	3.71403E 01	-9.03493E 00	4.52318E-01	ο.	ο.
5	0.	2.048888-01	-6.48231E 00	3.71398E 01	2.055598 02	3.23490E 01	~8.90868E 00	4.38550E-01	0.
4	0.	0.	4.51175E-01	-9.034938 00	3.234858 01	2.025618 02	3.74441E 01	-4.59528E 00	2.09784E-02
7	0.	٥.	0.	4.52318E~01	-8.90868E 00	3.74636E Q1	1.04832E 02	-5.71526E 00	-5.07044E-01
	0.	0,	٥.	0.	4.38550E-01	-4.59528E 00	-5.71529E 00	2.87263E 01	2.362718 00
,						2.09784E-02	-5.07044E-01	2.36271E 00	3.26441E 00

### COLLOCATION POINT STRUCTURAL INFLUENCE COEFFICIENTS MATRIX

COLL. POINT	1	2	3	•	5	6	7	•	
۱	2.52041E-04	1.53918E-04	1.03819E-04	6.69653E-05	3.74016E-05	1.61452E-05	5.58735E-06	1.59701E-06	٥.
2	1.539185-04	1.214665-04	8.55629E-05	5.642612-05	3.22428E-05	1.42898E-G3	5.04918E-04	1.491768-06	۰.
3	1.038195-04	8.35429E-05	6.73064E-05	4,588688-05	2.708396-05	1.24344E~05	4-51100E-06	1.386518-06	٥.
4	6.496538-05	5+642618-05	4.58868E-05	3.53476E-05	2.19250E~05	1.05789E~05	3.97282E-06	1.201268-06	٥.
5	3.740148-05	3.22428E-05	2.70839E-05	2.19250E-05	1.67662E-05	8.723512-06	3.43465E-06	1.17601E-06	٥.
•	1.41452E-05	1.42898E-05	1.24344E-05	1.05789E-05	8.72351E-06	6.86796E-06	2.89644E-06	1.070768-06 -	٥.
7	5.38735E-06	5.04918E-06	4.51100E-06	3.972825-06	3.43465E-06	2.89644E-06	2.358298-06	9.65514E-07	٥.
•	1.59701E-06	1.49176E-06	1.386518-06	1.28126E~06	1.17601E-06	1.07076E-06	9.65514E-07	8.60264E-97	٥.

### HODAL DATA

CANTILEVERED	AT	x	-	125.306

COLL. POINT	15T MODE 27.28 CPS	ZND HODE 89.04 CPS	3RD HODE 174.16 CPS	41H MODE 273.08 CPS
1	9.6424360E-02	1.6796861E-01	2.4422816E-01	5.4789109E-01
2	8.1195670E-02	1.1838275E-01	1.4369598E-01	2.1827023E-01
3	6.63169968-02	7.4647183E-02	7.3377694E-02	4.11972212-02
4	5-0869723E-02	2.5571569E-02	-7.81733965-03	-5.27693978-02
5	3.4241724E-02	-2.3343700E-02	-3.8395116E-02	3.2723531E-02
6	1.78340818-02	-4.1927207E-02	3.32203386-02	6.1911686E-04
7	6.9849456E-03	-2.3426839E-02	3.2963625E-02	-3.1780140E-02
8	2.34682018-03	-9.8140919E-03	1.42397168-02	-1.53880128-02

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																			.sı								•		<b>.</b>	•	2.18371E-02	-3.24670E-01	10-301706.7	3.21628E 00
																			14-÷								.0		5	1.45754E-02	-3.90630E-01	1.67195E 00	Z.10993E 01	10-301196-1
													•						a								••	••	2.83162E-02	-4.16789E-01	8.32278E-01	7.79359£ 00	1.67195E 00	-3.74670F-01
																			12									1.65857E-02	-4.48688E-01	2.07531E 00	7.70715E 00	8.32273E-01	-3.906305-01	C0-314541 C
																			н								1.49930E-01	-1.67997E 00	-7-60684E-01	9.54102E 00	2.07530E 00	-4-16789E-01	1.45754E-02	
																			10			-5		•	<b>0</b> -	1.64322E-01	-3.14341£ 00	1.24562E 01	4-02563E 01	-7.60700E-01	-4-486886-01	2.831625-02		
S MATRIX Erry	CHES	RDINATE From Nose)	44E 02	<b>8</b> 4E 02	02E 02	20E 02	38E 02	57E 02	75E 02	50 3E4	11E 02	296 02	47E 02	65E 02	20 9C	01E 02	20E 02	MATRIX 1 LBM) (M MATER)	•			•	••	••	1.749676-01	-3.38992E 00	1.172415 01	7.28529E 01	1.24560E 01	-1.67997E 00	1.65857E-02	.0		
E (M) MAS on point geom	H - 4.7181 IN	X-CDD I INCHES	1.172	662-1	1.307	1.374	1++1	1.508	1.575	1.42	1.710	1.777	44-1 1	116-1	1.978	2-044	5-113	M POINT MASS ASS - 830,854 Am Filled Wit	-			••	۰.	1.64322E-01	-3.382136 00	1.26013E 01	7.55561E 01	1.172096 01	-3.14341E 00	1.49930E-01	0.	•		
DLE STAG	BAY LENGT	COLLOCATION OINT	1	2	•	•	•	•	•	•	r	•;	11	7	2	•	13	CDLLDCATIO (TOTAL M (BALLAST TA	1			••	1.700516-01	00 31980C.C.	1.136156 01	7.663856 01	1.24011E 01	-3-38992E 00	1.643226+01	•	۰.			
Idiw																			•			1.443226-01	3.34947E 00	1-224456 01 -	7.65296E 01	1.136135 01	3.38213E 00	1.749676-01 -	•	•	••	••		
																			•		10-376+19"	.288896 00	1.15798E 01 _	1.51437E 01	1.224436 01	00 3168EC.6	1.64322E-01 _		•					
																			*		35174E 00	- 10 909221.	1.72990E D1	.15796E 01		- 10-315004-								
																			~	.19920E 00	.38969£ 01 -3	.45087E 01 1	.17761E 01	-266895 00 1	- 04322E-01 -3	,	·							
			•																2	- 06641E 00 -2		10 31696.		- 64436E-01 -]	-									
																			-	1.17812E 00 1	1-06415 00 5	2.19920E 00 1	- 10-361929-1	, Y	c									
																6-			COLL.	-	N	ň	•	~ ~	\$ \$	7	8	e	10	11	12	9	11	5

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# TABLE 24 AIDDLE STAGE (M) INFLUENCE COEFFICIENTS AND MODES CANTILEVERED WITH RIGID ADAPTER

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## MODAL DATA

	CANTILEVENE	0 AY X - 211.320	WITH RIGID ADAP	TEA
COLL.	157 MODE 77.67 CPS	240 MODE 293.91 CPS	340 MODE 589.32 CPS	47H MODE 433.75 CPS
-	6.1469032E-02	7-0145206E-02	1.0011#74E-01	1.4416741E-01
7	5-40991506-02	5.2539498E-02	5.90496176-02	4.4627587E-02
^	5.0194491E-02	2.92116566-02	2.57881126-03	-3.43285176-02
4	4.43353126-02	8-4282113E-03	-2.94141546-02	-4.2202427E-02
'n	3.41791646-02	~1.24519236-02	-4.J625732E-02	-4-30347146-03
•	3.18640636-02	-3-63037546-02	-3,37749386-02	3.54264346-02
۲	2.55442006-02	-4-24289468-02	-4.39190045-03	4.24310566-02
•	1.9398544E-02	-4.49853316-02	2.82301146-02	4.57440196-03
٠	1.34254146-02	-4.34842906-02	4.4791493E-02	-3.42622146-02
9	6.44038052-03	-3.28949456~02	4.18287045-02	-4.41940146-02
11	4.09080495-03	-1.66915965-02	2,43404885-02	-2.44473046-02
12	8.00181416-04	-4-51714236-03	5.44543908-03	-4.74505996-03
9	9.11211456-12	-1.80280876-11	2.36696966-31	11-31617182.5-
1	21-3619929612	-5.02241056-12	4.5879544E-12	-4-4034018E-12

COLLOCATION POINT Structural influence coefficients maynix

51		÷	÷	÷	÷	÷				4				
1	2.079206-10	1.927356-16 0	1.775916-10 (	1.424276-16 0	1.472636-16 (	1.320966-16 0	1.169326-16 6	1.017686-16 0	8.660385-17 (	7.143966-17	5.627546-17 0	4.111126-17 6	2.594716-17 0	1.078296-17
2	8-09995E-E6	7-493485-16	6-88700E-16	6.28052E-16	5.474046-16	5-067486-16	4.461006-16	3.854528-16	3.248046-16	2.641576-16	2.035096-16	1.428616-16	8.221346-17	2-594718-17
21	5.31284E~08	5.25585E-08	5.198846-08	5-141638-08	5.084826-08	\$-027806-08	4.970806-08	4.913796-08	4.856786-00	80-327924.4	4.742766-08	4.68575E-08	1.428618-16	4.1111126-17
"	70-302010-2	2.674126-07	2.778006-07	2.601896-07	2.585746-07	2.409658-07	2.393536-07	.297426-07	2.201306-07	.105196-07	2.009086-07	4.742766-08	2.035096-16	5.62754[-1.7
01	6.49558E-07	6.200456-07	5.905316-07	5.610175-07	5.31504E-07	10-398610.0	4.124726-07	4.429580-07	4.134446-07	70-316968-6	2.105196-07	00-317997.4	2.641576-16	7.43966-17
F	1.098286-06	1.038185-06	9.780776-07	9.179786-07	8.578786-07	10-301770.5	7.376716-07	0-315271.0	6-174726-07	4.134446-07	2.201306-07	4.856786-08	3.248046-16	8.66038[-17
æ	1.63095E-06	1+529786-06	1.428406-06	1.327436-06	1.226256-06	1-12506[-06	1.023885-06	9-227096-07	6.175716-07	4.429586-07	7.297426-07	4.913792-08	3.454526-16	1.017686-16
٢	2.235746-04	2.00)19[-06	1.930645-06	1.776096-06	1.625546-06	L.47297E-06	1-320436-06	1.023886-06	7.376716-07	4.72.726-07	2.393336-07	80=J08010*5	4.461006-16	1.169320-16
•	2.901140-06	2.687126-06	2.473106-06	2.259096-06	2.045075-06	00-320169.	.472976-06	1.125065-06	10-301776-1	5-01986[-07	2.489695-07	5.027805-08	5.067486-16	1.320966-16
*	90-361919-6	3.330746-06	3.045366-06	2.159976-06	2.474586-06	2.045076-06	1.625546-06	4-220256-00	8.578786-07	5.315046-07	2.585785-07	5.08482E-0A	5.674046-16	1.472636-16
*	4-369806-04	40-376600.4	3.636735-96	3.210506-06	2.759976-06	2.259096-06	1.778071-06	1.327436-06	9.179746-07	5.610176-07	2.661895-07	5.141836-08	6.28052E-16	1.024276-16
^	5.151926-06	4.69497f=06	4.234025-06	3.636936-06	40-36(2+0.(	2.473106-06	1.930646-06	1.428605=06	70-311041.0	10-316506.5	7.7789464-07	5.19484E-0A	41=300188.4	1.715916-16
7	6.13398E-06	5.57+22€-06	4.694976-06	4.00337E-06	3.330745-06	2.687126-06	2.083196-06	1.529786-06	90-38186 v* 1	h.29945f-A7	2.074126-07	5,255554-08	7.493485-10	1-927556-16
	8.322425-06	6.133745-04	\$.15192E-06	4,369805-06	40-3E1414.	2.991146-96	2.235745-06	1.630956~06	1.n98286-06	10-185664.4	10-312010.5	6.31286f-0A	91-35666678	51-352620*2
50LL.		~	^	٠	ŝ	q	-	ų	t	5	Ξ	2	2	*1

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TABLE 25 SPIDER BEAM (S) COLLOCATION POINT STRUCTURAL INFLUENCE COEFFICIENTS MATRIX

COLL. POINT	ı	2	3	4	5	6	т	
<b>`1</b>	4.09564655-05	\$.6007798E-06	5.713C059E-06	+. MI 400E-06	4.43688258-06	4.4252460E-05	5.71300595-04	4.4007794E-94
2	6.6007191E-C6	4.0956465E-05	6.6007798E-C6	5-7130059E-06	4.82524002-06	4.43688258-06	4.8252400E-06	5.71300598-04
3	5.713005 <b>4</b> Ę-06	6.6007798E-06	4.09564652-09	6.6047798E-06	5.7130059E-06	4.82524002-06	1.4368825E-G&	4.82524006-84
4	4-8232400E-05	5.7130059E-06	6-4007798E-04	4.0956465E-05	6.6007798E-06	5.71300598-06	4.8252400E-06	4.4368825E-94
5	4.4368825E-06	4.82524008-06	5.7130059E-06	6.6007798E-06	4.0956465E-05	6.6C07798E-04	5.7130059E-06	4,8252408E-86
4	4.82524008-06	4.4368825E-06	4.8252400E-C6	5.71300598-06	6.6007798E-06	4.0956465E-05	6.6007798E-C6	5.71300598-84
7	5.713C059E-06	4.82524COE-06	4.4368825E-06	4.8252400E-06	5.7130059E-06	6.6007792E-06	4.0456465E-05	6.6007798E-06
•	6.6007798E-C6	5.7130059E-06	4.82524002-06	4.43688252-06	4.42524002-06	5.7130059E-06	6.6007798E-06	4.09566656-85
10								
11								

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							•															15											20-368695	10-3+6-01	14954E-01	12343E 00
																						1							•	.0	.0	7042E-03 0.	59256-01 2.5	1320E 00 -3.4	5940E D] 4.1	1-1 10-3+564
						•																							°.		.0 CO-3	16-01 2-9	11-6- 10-3	E 00 2.10	1E 00 1.21	E-01 4.84
																						5							۰.	3 0.	1 5-71084	1-1.16813	0 2.75762	1 4-28915	1 2.10319	46[09-6- 2
																					1	12							<b>.</b> .	5.71084E-0	-1.14217E-0	4-01413E-0	2-6794BE DI	2-757756-0	-3-15925E-0	2-54383E-0
																						=							4.75903E-03	-1.046995-01	4-203926-01	2.60324E 00	4.014076-01	1.168136-01	5.97042E-03	
																					5	2		o.	٥.	٥.	٥.	4.55137E-03	9.310406-02	3.262426-01	2.386876 00	4.20385E-01	1.14217E-01	5.71084E~03 -		
MATRIX			( <b>3</b> 6)																		•	-		÷		÷	551376-03	.102736-02	3-19902E-01 -	2.12406E 00	1.262376-01	10-366940°1		÷		
.E 26 (L) MASS	C GEONETRY	PL INCHES	X-COORDINATE CMES FROM NO	20 326741.5	2.244236 02	2.341146.5	2.430066.2	2-53497E 02	2.431886 02	2.124796 02	2.82570E Q2	2.92262E 02	3-014536 02	3.112446 02	3.213356 02	3.310246 02	3.407146 02	3.50409£ 02	XIATAI X	"1.970 L8H	-	•				.55137E-03	. 102736-02	- 10-355152-	.07572E DO	.198976-01	.310406-02	- 20-360451.				
TABL LOX TANK	LOCATION POINT	LENGTH - 7.41	11) MOI L																ATTON POINT HA	ASS LEMPTY! -	~		•	•	.55137E-03 0	.102736-02 4	- 231556-01 -9	.075446 00 3	.23150E-01 2.	.10273E-02 3.	-551376-03 -9.	÷	•	•		
CENTER	COL	8AY	COLLOCA		7	~	*	•	•	-	e	•	10	1	12	<b>C</b> 1	1	15	כטררסכ	TOTAL M	4			4.55137E-03 0	9.102736-02 4	- 231556-01 -9	.07544E 00 3	3.231506-01 2	9.10273E-02 3	4-551J7E-0J -9	•	•	•	•		
																					~		551376-03	• 102736-02	- 10-355165-1	•01544E 00	.231506-01	- 102736-02	- 551376-03							
	,																				4	*53428E-03 -C	10-311616.	- 10-31476-01 -4	.077766 00 3	•231506-01 2	.102736-02 3	- CO-376122.	•							
																					~	476236-01 7	1- 10-385551	491356 00 2	957626-01 2	102736-02 3	55137E-03 -9	4	0							
																					· 2	10326 00 -2.	40166 00 5.	\$5516-01 2.	1911E-01 2.	°6- 60-32619	*		0.							
																					-	15699E 00 1.C	110326 00 7.6	·7623E-01 5.7	34286-03 -1.3	4.5	.0	•	<b>.</b>							
															f	519	9			. 1102	POINT	1 2.5	2 1.6	3 -2.4	4 7.5	~ •	۰ ۶	•		6	01	11	12	9	3	15

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## TABLE 27 CENTER LOX TANK (L) CANTILEVERED INFLUENCE COEFFICIENTS AND MODES

MODAL DATA

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.

	4 TH MODE 653.90 CPS	1.2972127E-01	2.96393325-02	-9.5330902E-02	-1.86414806-01	-2.0334435E-01	-1-37570366-01	-1.7538858 <del>6-0</del> 2	1.10885336-01	1.99152996-01	2.1267576E-01	1-3294035E-01	4-2107432E-02	-7.8188702E-C2	-1.38342206-01
- 350.409	3RD MODE 414.47 CPS	1.1552918E-01	4.63088816-02	-3.8541665E-02	-1.1600653E-01	-1-6409007E-01	-1.7464052E-01	-1.4671658E-01	-8-6946350E-62	-8.27163776-03	7.14524096-02	1-3271066E-01	1.4932091E-01	1.74114216-01	1.47937466-01
NTILEVERED AT X	2ND MODE 202.81 CPS	-1.5998072E-01	-9.0046691E-02	-1.4888576E-02	5.9339920E-02	1.24173006-01	1.7532994E-01	2.0953250E-01	2.2492085E-01	2.21309886-01	2.0056513E-01	1.44791946-01	1.29033746-01	B.7210849E-02	4.97027396-02
CA	157 MODE 35.84 CPS	2.4654423E-01	2-2224364E-01	1.9758494E-01	1.7285050E-01	1.4857087E-01	1.2505034E-01	1.0242748E-01	8.1665484 <del>5-</del> 02	4.2543984E-02	4.5499371E-02	3.1341342E-02	1.94275756-02	1.0324446-02	3-90294496-03
	COLL.	1	7	n	*	'n	•	~	•	e	2	11	12	2	1

COLLOCATIC POINT STRUCTURAL INFLUENCE COEFFICIENTS MATRIX 15

	•	•	•	5	•	•	ť	•	•	•	•	•	•	•
14	6.11284E-06	5.840486-06	5.568136-06	5.29575E-06	5.02339E-06	4.75104E-06	4.478695-06	4.20633E-06	3.93395E-06	3.66160E-06	3.38924E-06	3.116895-06	2.84454E-06	2.57216E-06
8	1.T0067E-05	1.60035E-05	1.50002E-05	1.39969E-05	1.29937E-05	1.19904E-0≶	1.09872E-05	9.983996-06	8.98065E-06	7.977426-06	6.97419E-06	5.970976-06	4.96774E-06	2,84454E-06
77	3-26610E-05	3.05487E-05	2-84364E-05	2.63238E-05	2.42115E-05	2.20992E-05	1.998696-05	1.78746E-05	1.57620E-05	1.36497E-05	1-15374E-05	9.42506E-06	5.97097E-06	3.11689E-06
Ħ	20-351036-05	4.982086-05	4.61313E-05	4.24414E-05	3.875186-05	3.50623 <del>E-</del> 05	3.13727E-05	2.76832E-05	2.39933E-05	2.03037E-05	1.66142E-05	1.153746-05	6.97419E-06	3.36924E-06
10	7.90481E-05	7.33125E~05	6.75769E-05	6.18407E-05	5.61051E-05	5.03696E-05	4.46340E-05	3.68964E-05	3.31622E-05	2.74266E-05	2.03037E-05	1.36497E-05	7.97742E-06	3.66160E-06
۰	1.09449E-04	1.01158E-04	9.28679E-05	8.45766E-05	7.628620-05	6.79959E-05	5.97055E-05	5.14151E-05	4.31239E-05	3.31622E-05	2.399336-05	1.57620E-05	8.98065E-06	3.93395E-05
æ	1.44597E-04	1.33166E-04	1.21736E-04	1.10304E-04	9.887346-05	8.74429E-05	7.60124E-05.	6.458186-05	5.14151E-05	3.889846-05	2.76832E-05	1.78746E-05	9.983996-06	4.20633E-06
٢	1.83850E-04	1.68692E-04	1.53535E-04	1.38376E-04	1.23219E-04	1.08062E-04	9.29045E-05	7.60124E-05	5.97055E-05	4.46340E-05	3.13727E-05	1.99869E-05	1.09872E-05	4.47869E-36
•	2.266246-04	2-07153E-04	1.87682E-04	1.682095-04	1.487386-04	1.29267E-04	1.08062E-04	8.74429E-05	<i>k</i> .799596-05	5.03696E-05	3.50623E-05	2.20992E-05	1.19904E-05	4.75104E-06
~	2.723336-04	2.47961E-04	2.235906-04	1.99216E-04	1.74845E-04	1.48738E-04	1.232196-04	9.88734E-05	7.62862E-05	5.61051E-05	3.875186-05	2.42115E-05	L.29937E-05	\$.02339E-06
÷	3.203896-04	2-905306-04	2.60671E-04	2.308106-04	1.99216E-04	1.682095-04	1.383766-04	1.103046-04	8.457666-05	6.16407E-05	4.244146-05	2.43238E-05	1.399696-05	5.29575E-06
ſ	3.702116-04	3.342776-04	2.983446-04	2.606716-04	2.23590E-04	1.876826-04	1.535356-04	1.217366-04	9.286796-05	6.757696-05	4.61313E-05	2.843646-05	1.50002E-05	5.5681JE-06
2	4.20556E-04	3.780216-04	3.34277E-04	2.905306-04	2.47961E-04	2.07153E-04	1.68692E-04	1.33166E-04	1.011586-04	7.33125E-05	4.98208E-05	3.054876-05	1.60035E-05	5-840486-06
-	4.70434E-04	4.205566-04	.70211E-04	3.20389E-04	.72333E-04	2.26624E-04	1.838506-04	1.445976-04	1.094496-04	7.90481E-05	5-35103E-05	.266106-05	1.700676-05	6.11284E-06
COLL.	1	2	•	¥	*	¢	1	8	6	01	11	12	6	11

														181												.450966-03	.60504E-01	19-365119-	.15046E 00	
														14								•	•	•	-79893E-04 0	.87571£-D2 8	.934596-01 -1	.61877E 00 5	1 10-3627702	
														13								0	0	798936-04 0	959796-02 9	11111E-02 -8	337436 00 5	-93456E-01 2	60504E-ef 5	
														12								ò	79693E-04 0.	959796-02 9.	95742E-02 -1.	550596-01 -1.	11122E-02 1.	87671E-02 5.	45096E-03 -1.	
														11								440056-04 0.	92390E-02 9.	0,365E-02 -1.	467876-01 6.	95731E-02 4.	95979E-02 ~1,	798936-04 -8.	-	
														01							111296-04	85513E-02 9.	69789E-02	386096-01 7.	01354E-02 4.	95979E-02 6,	79893E-04 -1.			
5191E 02 18186 02	1445E 02	4073E 02	3700E 02	332TE 02	2954E 02	2581E 02	2208E 02	1035E 02	1462E 02	1089E 02	0717E 02	MATRIX	-401 TBH	•			5	-0	.0	1296-04 0.	2266-02 9.1	7685-02 -1.1	0226-01 6.4	7786-02 4.	3406-02 7.6	8936-04 -1.				
2.5	2.5	2.4	2.7	2.0	2.9	3.0	3.1	3.2	3.3	3.4	3.S	N POINT MASS	(EMPTY) - 14				••	•	10 +0-38e	11.0 20-364	056-02 -1 <b>.0</b> 2	20E-01 6.41	58E-02 4.23	136-02 4.49	02E-04 -1.4	67.6	<b>.</b>			
n 4	· •••	*	-	-	•	5	11	12	13	14	15	COLLOCATIO	TOTAL MASS	•			•	E-04 0.	E-02 9.092'	E-02 -1.820	E-01 6.472	E-02 4.155	E-02 6.417	E-04 -1.855	9.440	••	0.			
														-			04 0.	02 8.25173	02 -1.73447	01 6.56511	02 4-14949	02 6.47195	04 -1-82226	9-11129	•	•	ċ			
														٩			8.25173E-	-356039.1 -	5.727095-	3-95465E-	6.58501E-	820436-	9.111296-	••		•	•			
														ŝ	-0.	8.25173E-04	-1.65035E-02	5.856866-02	3.763946-01	5.727006-02	-1.734476-02	9.092986-04								
														÷	2.05970€-03	-3.46394E-02	5,057746-02	3.769646-01	5.85879E-02	-1.650356-02	8.251736-04	<b>.</b> .								
														e	÷.70323E-02	2.40492E-01	4.90310E-01	5-05764E-02	1-65035E-02	8-251736-04	0.	0.								
														~	6.57369E-02 -	1.980816 00	2.404916-01	3.46394E-02	8-251736-04 -	<b>0</b> .	0.	0.								
														-	.624186-01	.57369E-02	.703236-02	- 059706-03 -												
										62	1			LUIDA	1 5	2 8	3 - 6	4	*	¢	7 0	6	6	10	11	12	5	1	51	

TABLE 28 FUEL TANKS (F) MASS MATRIX

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COLLOCATION POINT GEOMETRY Bay Length = 9.6271 INCHES

X-COORDIMATE {!MCHES FAQM MOSE} 2.15937E 02 2.25544E 02 COLLOCATION POINT

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TABLE 29	COLLOCATION POINT
UEL TANKS (F) SIMPLY SUPPORTED INFLUENCE COEFFICIENTS AND MODES	STRUCTURAL INFLUENCE COEFFICIENTS MATRIX

.

15	.96558E-12	<b>89423E-12</b>	.661695-12	.111436-11	.31474E-11	.46659E-11	.560685-11	.587426-11	.53743E-11	-40610E-11	.185956-11	68075E-12	57530E-12	-93179£-19		
1	42663E-05 2.	-83555E-05 5.	.16721E-05 8	.347946-05 1	.327546-05 1	060446-05 1	1 -516466-05 1	1 \$0-3096-05	1.414894-05 1	0.790286-05 1	5.739826-05 1	4.220936-05 8	2.257776-05 4	4.57530E-12 9		
51	.857836-05 1	680256-05 2	1.34814E-05 4	1.67142E-04 5	L.26780E-04 6	1.414856-04 7	1.506526-04 7	1.53362E-04 7	1.487156-04 7	1.362716-04 6	1.15310E-04	8.49808E-05	4.22093E-05	8.680756-12		
12	4.24703E-05	8-440966-05	1.240316-04	1.591256-04	1.88185€-04	2.09845E-04	2.231996-04	2.268626-04	2.19501£-04	2.004/396-04	1.68602E-04	1.153106-04	5.739826-05	1.185956-11		
11	5.508496-05	.094626-04	1.407486-04	2.059986-04	2.43203E-04	2.705416-04	2.868046-04	2.901546-04	2.787916-04	2.518336-04	2.004396-04	1.362716-04	6.79028E-05	1.40610E-11		
10	6.59963E-05	1.311086-04	L-92355E-04	2.46058E-04	2.897046-04	3.210176-04	3.394876-04	3.398146-04	3.22750E-04	2.78791E-04	2.19501E-04	1.487156-04	7.414896-05	1.537436-11		
¢	7.477746-05	1.48493E-04	2.175726-04	2.775976-04	3.25554E-04	3.587116-04	3.75257E-04	3.724336-04	3.398146-04	2.901546-04	2.268686-04	1.53362E-04	7.64960E-05	1.587426-11		
•	A.08368E-05	1-404335-04	7.346246-04	2.982496-04	3.477952-04	1.80075E-04	3.929756-04	3.75257E-04	10-176-04	2.84.8045-04	2.23199F-04	1.506526-04	7.516466-05	1.560686-11	T.A	AT EACH END
۴	A 1447A6-05	10-306-04	2 11000F-04	1.059885-04	1.53894E-04	3 a2048E-04	1.80075E-04	1 587116-04		2 20611-04	70-31-6n/ ·7	1 414855-04	2040465	1.466596-11	MODAL DA	LY SUPPORTED
<b>.</b>	30-310100 0	10-360107.9	10-302040*1	2. 20274E-04	3 415616-04		+0-3-5026.1				2.432035-04		10-309/02-1	· .31474E-11		SIMP
'n		. 105035-10	1.535435-04		+0-311/5/*7		3,0574865-04	10 314264.17	+n-3:66:1.*7	2-4608E-04	2.051986-04	1.591255-04	50-375170°1	CU-3*47*6*C		
		6.747236-05	1.329986-04	1.900245-04	2.217025-0-	2.38291E-U-	2.417906-04		2.175726-04	1.923556-94	1.60748E-04	1.249315-04	8.349146-05	4.18721F-US 4.48149F-12		
۴		5.17552E~05	1.013866-04	1.329986-04	1.535436-04	1.6402BE-04	1.659206-04	1.60435E-04	1.484936-04	1.311086-04	1.99462E-04	8.44996E-05	5.680264-05	2.835556=05		
Я		3.028556-05	5.17551E-05	6.14723E-05	7.76563E-05	8.281056-05	8.366785-05	8.083686-05	7.477746-05	6.599634-05	5.50849E-US	4.24793E-75	2.857836-05	1.426436-95	21-386604.2	
-				÷	2							3	•			
10104 COLL-	-	2	0	4	\$	\$	۲	æ	e	61	Ξ	21	9	41	51	

	3RD MODE 509.16 CPS	-2.88566206-01	-4.1482159E-01	-3.38323106-01	-1.15470246-01	1.64830016-01	3.89225256-01	4.65091006-01	4.1937068E-01	2.2102042E-01	-2.91446366-02	-2.3643684E-01	-3.171263 <b>6E-01</b>	-1.5159000E-01	-3.4684613E-08
RTED AT EACH END	2ND MODE 274.45 CPS	2.11233835-51	3.80642571-01	4.7489748E-01	4.7693008E-01	3.89954016-01	2.3660715€-01	4.57727036-02	-1-4685145E-01	-3.0324433E-01	-3-9514785E-01	-4.03964736-01	-3.2385426E-01	-1.6718566E-01	-3.36063456-08
SIMPLY SUPPO	157 MODE 80.73 CPS	1.11289246-01	2.1915043E-01	3.1693343E-01	3.9751486E-01	4.5674300E-01	4.9203560E-01	5.0270422E-01	4.8795750€-01	4.4841456E-01	3.86928246-01	3.0607492E-01	2.09297936-01	1.04739226-01	2.14899856-08
	COLL-	 2	•	÷	'n	¢	1	•	٠	01	11	12	1	1	5

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TABLE 30 BENDING AND SHEAR RIGIDITY OF BEAM-LIKE COMPONENTS

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GA SLICE DATA

. SLICE VALUE(S) GA VALUE GA VALUE

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### EI SLICE DATA

SLICE 6	DUNDARIES
X-CDORDINATE	X-COORDINATE

SLICE VALUE(S) EI VALUE EI VALUE

SLICE BOUNDARIES X-COORDINATE X-COORDINATE

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		U	PPER	STAGE			
٥.		2.50000008	00	3.5000000E	05	1.000000E	06
2.5000000E	00	5.000000E	00	1.00000008	06	1.1500000E	04
5.000000E	00	2.0000000E	01	1.1500000E	04	2.5000000E	06
2.0000000E	01	3.0000000E	01	2.5000000E	06	3.4000000E	06
3.0000000E	01	3.6000000E	01	3.4000000E	40	4.0000000E	06
3.6000000E	01	4.4000000E	01	4.0000000E	06	4.8500D00E	06
4.4000000E	01	8.8000000E	01	4.8500000E	06	4.8500000E	06
8.8000000E	01	1.000000E	02	1.4550000E	07	1.6600000E	07
1.00000008	02	1.0700000€	oz	1.6600000E	07	1.7750000£	07
1.0700000E	G2	1.1100000E	0 Z	1.7750000E	07	1.845000DE	07
1.1100000E	02	1.1700000E	0 Z	1.8450000E	07	1.9450000E	07
1.1700000E	02	1.2530600E	02	1.9450000E	07	2.1850000E	07
		ж	IODLE	STAGE			
1.1726600E	02	1.212880DE	02	3.3100000E	06	5.3583003E	06
1.2128800E	02	1.2530900E	02	5.3583003E	06	6.6205924E	06
1,2530900E	02	1.42310008	02	4.6110508E	07	4.6110508E	07
1.4231000E	02	1.59313008	02	4.6110508E	07	4.6110508E	07
1.5931300E	02	1.7631700E	02	4.6110508E	07	4.6110508E	07
1.7631700E	0Z	1.93320008	02	4.6110508E	07	4.6110508E	07
1.9332000E	02	2.1132000E	02	9.9499998E	18	9.9999998E	18
		CENT	ER LO	X TANK		•	
2.1473200E	02	2.2529200€	02	1.9837592E	07	1.9837592E	07
2.2529200E	02	2.31602008	02	8.3375920E	06	8.33759208	06
2.31602006	02	3.0023200E	02	5.2879286E	06	5.2879286E	06
3.0053500E	02	3.30112005	02	6.6130525E	06	6.6130525E	06
3.30112005	62	3.3936200E	02	9.9313594E	06	9.9313594E	06
3.39365006	02	3.5040900E	02	4.05381608	06	4.0538160E	06
		Ŧ	UEL T	ANKS			
2.15937008	02	2.3107700E	02	2.29424038	08	2.29424036	08
2.31077008	02	2.64217006	02	I.2669718E	08	1.26697188	. 08
2.6421700E	02	2.97916996	02	1.38185586	08	1.38185581	. 08
2.97976990	65	3.31677006	02	1.49649058	08	1.49669058	08
3.31677900	02	3.38067005	02	2.29424038	80	2.29424038	: 08
3.3806700E	02	3.5071700E	02	5.30169408	08	5.30169408	0.

	UPPER SI	TAGE	
٥.	2.5000000E 00	1.0000000E 05	1.5000000€ 06
2.5000000E 00	5.0000000E 00	1.5000000E 06	6.5000000E 06
5.000000E 00	2.0000000E 01	6.5000000E 06	2.5000000E 08
2.0000000E 01	3.0000000E 01	2.5000000E 08	6.2500000E 08
3.000000E D1	3.6000000E 01	6.2500000E 08	1.0000000E 09
3.6000000E 01	4.4000000E 01	1.0000000E 09	1-8500000E 09
4.4000000E 01	8.8000000£ 01	1.8500000E 09	1.850000DE 09
8.8000000E 01	1.0000000E 02	4.0500000E 09	8.0000000E 09
1.0000000E 02	1.07000008 02	8.0000000E 09	1.0850000E 10
1.0700000E 02	1.1100000E 02	1.0850000E 10	1.3000000E 10
1.1100000E 02	1.1700000E 02	1.30000008 10	1.6500000E 10
1.1700000E 02	1.25306008 02	1.6500000E 10	2.2000000E 10
	MIDDLE	STAGE	
1.1726600E 02	1.212880DE 02	9.7399999E OB	1.0326837E 09
1.2128800E 02	1.2530900E 02	1.0326837E 09	1.9479450E 09
1.2530900E 02	1.4231000E 02	3.3324611E 10	3.1474882E 10
1.4231000E 02	1.593130DE 02	3.1474882E 10	2.9856592E 10
1.5931300E 02	1.763170DE 02	2.9856592E 10	2.8469738E 10
1.7631700E 02	1.9332000E 02	2.8469738E 10	2.7314321E 10
1.93320005 02	2.1132000E 02	9.9999998E 18	9.9999998E 18
	CENTER LO	IX TANK	
2.1473200E 02	2.2529200E 02	4.1438364E 09	4.1438364E 09
2.25292006 02	2.3160200E 02	2.45056938 09	2.45056938 09
2.31602006 02	3.0023200E 02	1.5508256E 09	1.5508256E 09
3.0023200€ 02	3.3011200E 02	1.9412973E 09	1.9412973E 09
3.3011200E 02	3.39362006 02	2.9223334E 09	2.9223334E 09
3.39362006 02	3.50409006 02	1.67086988 09	1.6708698E 09
	FUEL T	ANKS	
2.1593700E 02	2.3107700E 02	1.7618052E 06	1.7618052E 06
2.3107700E 02	2.6427760E 02	9.7030599E 05	9.7030599E 05
2.6427700E 02	2.9797699E 02	1.0584402E 06	1.0584402E 06
2.97976996 02	3.31677000 02	1.1465619E 06	1.1465619E 06
3.3167700E 02	3.3800100E 02	1.7618052E 06	1.7618052F 06
3.3806700E 02	3.5071700E 02	9.88340518 06	9.0834051E 06

### 3.0 COUPLING OF SATURN MODEL COMPONENT MODES

### 3.1 Introduction

The ultimate goal of this analysis of the Saturn Model was to determine the mode shapes and natural frequencies of the 1/5 scale model. The basic idea behind breaking the model down into component structures was the reduction of the number of degrees-of-freedom used in the final coupling process. It can be seen that the component structures of the model have been allowed a total of 251 degrees-of-freedom. The manipulation of matrices of this size would be very difficult not only in computation, but also in the mere analytical expressions preceding the computation. It will be seen here that this number of degrees-of-freedom can be reduced to 24 through the use of component modes as generalized coordinates.

### 3.2 Vehicle Without Outer Lox Tanks

For the purpose of this final coupling analysis, an intermediate vehicle configuration was defined and analyzed through the use of its component structures modes. This intermediate configuration was defined as the "Center Lox Tank Vehicle" and consisted of the entire Saturn Model with only the Outer Lox Tanks removed. This configuration was selected because it essentially eliminated all major redundant load paths. Physically, this allowed any component structure to store strain energy without forcing adjacent structures to also store strain energy. (If the Outer Lox Tanks had been left in, then strain energy in one tank would have forced strain energy into the Spider Beam, Center Lox Tank, and other outer Lox Tanks.) Analytically, this selection of configuration allowed a much simpler intermediate coupling analysis.





### 3.2.1 Components of Vehicle Without Outer Lox Tanks

For the purpose of the analysis of the Vehicle Without Outer Lox Tanks, the configuration shown in Figure 130 was divided into four component structures. These component structures were then analyzed independently to obtain their mode shapes which were then used as generalized coordinates in the final coupling analysis of the Vehicle Without Outer Lox Tanks.

The first component selected was the same as that shown in Figure 117 (Upper Stage, U) and analyzed in Section 2.1 of this appendix. This analysis resulted in the determination of mode shapes as a 9 x  $\frac{1}{4}$  matrix, which are presented in Table 32. It may be recalled that the kinetic energy of the Upper Stage was written as

$$T = \frac{1}{2} \left\{ \dot{p}_{0} \right\}^{\prime} \left[ A_{0} \right] \left\{ \dot{p}_{0} \right\}$$
 (II-34)

The second component selected was the portion of the Vehicle Without Outer Lox Tanks shown in Figure 131, the elastic Middle Stage and rigid Upper Stage.



FIGURE 131 SECOND COMPONENT OF VEHICLE WITHOUT OUTER LOX TANKS

The kinetic energy of this component was written as

$$T = \frac{1}{2} \{ \dot{p}_{M} \}' / [A_{M}] + [T_{UM}]' [A_{U}] [T_{UM}] \} \{ \dot{p}_{M} \}$$
(II-35)

where  $\{\dot{p}_{M}\}$  was the generalized velocities of (M).

- $[A_M]$  was the collocation point mass matrix of (M) derived in Section 2.2.
- [Tum] was a geometric transformation matrix which related the rigid motion of the Upper Stage to the elastic motion of the Middle Stage, and [A<sub>U</sub>] was the collocation point mass matrix of the Upper Stage derived in Section 2.1 of this appendix.

The mode shapes of this component of the Center Lox Tank Vehicle were determined through iteration of the expression,

$$[\mathsf{E}_{\mathsf{M}}]([\mathsf{A}_{\mathsf{N}}] + [\mathsf{T}_{\mathsf{U}\mathsf{N}}]'[\mathsf{A}_{\mathsf{U}}][\mathsf{T}_{\mathsf{U}\mathsf{M}}])\{\varphi\} = \lambda \{\varphi\}$$
(II-36)

where  $[E_M]$  was the collocation point structural influence coefficient matrix derived in Section 2.2, and

 $[\phi_{\rm M}] {\rm was}$  the 15 x 2 mode shape matrix for this portion of the Vehicle (presented in Table 32).

The third component selected was that portion of the Vehicle Without Outer Lox Tanks shown in Figure 132, the elastic adapter (A) and the rigid Middle and Upper Stages.

ATTAII

FIGURE 132 THIRD COMPONENT OF VEHICLE WITHOUT OUTER LOX TANKS

The kinetic energy of this third portion was written in matrix notation as,

$$T = \frac{1}{2} \{\dot{p}_{A}\}'([T_{MA}]'[A_{M}][T_{MA}] + [T_{UA}]'[A_{U}][T_{UA}]\} \\ (II-37)$$

where,

$$[\tau_{UA}] = [\tau_{UM}][\tau_{MA}]$$
(II-38)

and where.

 $\{\dot{p}_A\}$  was the matrix of generalized velocities of (A), and

 $[T_{MA}]$  was a geometric transformation matrix which related the rigid motion of the Middle Stage to the Elastic motion of the Adapter (Note: all the transformation matrices used in this Section are presented in Table 31 ).

The mode shapes of this third component of the Vehicle Without Outer Lox Tanks were determined through iteration of the expression.

$$[E_{A}]([T_{MA}]'[A_{M}][T_{MA}] + [T_{UA}]'[A_{U}][T_{UA}]) \{\psi\} = \lambda \{\psi\}$$
(II-39)

where  $[E_A]$  was the collocation point influence matrix for the Adapter, (A) (derived in Section 2, 3, and

 $\{ \mathcal{P}_A \}$  was a 2 x 1 mode shape matrix for this portion of the Center Lox Tank Vehicle (presented in Table 32).

The fourth component of the Center Lox Tank Vehicle was the Center Lox Tank itself with all other components attached to it as rigid members as shown in Figure 133. Two rigid body modes were included, unit translation and unit pitch about the top of the spider beam.



### FIGURE 133 FOURTH COMPONENT OF VEHICLE WITHOUT

OUTER LOX TANKS

The kinetic energy of the fourth and final component of the Center Lox Tank Vehicle was expressed in matrix form as,

$$\tau = \frac{1}{2} \{\dot{p}_{L}\}' ([A_{L}] + [T_{UL}]'[A_{U}][T_{UL}] + [T_{SL}]'[A_{S}][T_{SL}] + [T_{ML}]'[A_{M}][T_{ML}] + [T_{RL}]'[A_{R}][T_{RL}] + \{[T_{FL}]\}'[A_{F}][T_{FL}]) \{\dot{p}_{L}\}$$
(II-40)

where,

$$[\tau_{ML}] = [\tau_{MS}][\tau_{SL}]$$
(II-41)

and,  $[T_{MS}]$  was a geometric transformation matrix which related the motion of the rigid Middle Stage (M) to the motion of the Spider Beam (S), and

 $[T_{IS}]$  was a geometric transformation matrix which related the motion of the rigid Spider Beam (S) to the elastic motion of the Center Lox Tank (L),

and where,

$$[\mathsf{T}_{\mathsf{UL}}] = [\mathsf{T}_{\mathsf{UM}}][\mathsf{T}_{\mathsf{ML}}] \tag{II-42}$$

and where,

$$[\mathsf{T}_{\mathsf{FL}}] = [\mathsf{T}_{\mathsf{FC}}][\mathsf{T}_{\mathsf{SL}}] + [\mathsf{T}_{\mathsf{FR}}][\mathsf{T}_{\mathsf{RL}}] \qquad (\mathsf{II}-43)$$

where [T<sub>FS</sub>]was a geometric transformation matrix which related the motion of the rigid Fuel Tank (F) to the motion of the Spider Beam (S).  $[T_{rp}]$  was a geometric transformation matrix which related the motion of the rigid Fuel Tank (F) to the motion of the Outrigger (R), and  $[T_{p_T}]$  was a geometric transformation matrix which related the motion of the Outrigger to the motion of the Center Lox Tank, [A<sub>1</sub>,] was the collocation point mass matrix for the and where, Center Lox Tank (derived in Section 2.5 ),  $[A_{G}]$  was the collocation point mass matrix for the Spider Beam (derived in Section 2.4),  $[A_R]$  was the collocation point mass matrix for the Outrigger (derived in Section 2.7),  $[{\rm A}_{\rm F}]$  was the collocation point mass matrix for the Fuel Tanks (derived in Section 2.6 ), and  $\{ \flat_{T_{\cdot}} \}$  was a matrix of generalized velocities for the Center Lox Tank.

The mode shapes of the fourth component of the Vehicle Without Outer Lox Tanks were determined through iteration of the expression,

$$\begin{split} [E_{L}]([A_{L}] + [T_{UL}]'[A_{U}][T_{UL}] + [T_{SL}]'[A_{S}][T_{SL}] & (II-44) \\ &+ [T_{ML}]'[A_{M}](T_{ML}] + [T_{RL}]'[A_{R}][T_{RL}] \\ &+ 4 [T_{FL}]'[A_{F}][T_{FL}]) \{\varphi\} = \lambda \{\varphi\} \end{split}$$

where

 $\begin{bmatrix} E_L \end{bmatrix}$  was the collocation point structural influence coefficient matrix for the Center Lox Tank (derived in Section of this part of the report), and

 $\left[ \varphi_L \right]$  was a 15 x 10 mode shape matrix for this fourth component of the Center Lox Tank Vehicle (presented in Table 32). It should perhaps be noted that two of these modes were rigid body modes while the remaining eight were elastic modes.

### 3.2.2 Modes of Vehicle Without Outer Lox Tanks

The mode shapes and natural frequencies of the entire Vehicle Without Outer Lox Tanks were determined in much the same manner as were the mode shapes and frequencies of its components. The kinetic energy of the Vehicle was expressed as the sum of the kinetic energies of its parts and appeared in matrix form as,

$$T = \frac{1}{2} \left( \frac{1}{2} \dot{p}_{u} \frac{1}{2} \left[ A_{u} \frac{1}{2} \dot{p}_{u} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{1}{2} \right] + \frac{1}{2} \dot{p}_{m} \frac{1}{2} \left[ A_{m} \frac{1}{2} \dot{p}_{m} \frac{$$

It should perhaps be noted that  $\{p\}$  represents the generalized coordinates of a component. Each of these generalized coordinate matrices were then related to modal coordinates,  $\{q\}$ , by the relations;

$$\{ p_{U} \} = \begin{bmatrix} T_{UM} ] \{ p_{M} \} + \begin{bmatrix} \varphi_{U} ] \{ q_{U} \} \\ \hline \\ rigid \ displacements \\ of (0) \ due \ to \\ motion \ of (M) \end{bmatrix} \xrightarrow{elastic notion \\ of (p) \\ relative to (M) \end{bmatrix}} (II-46)$$

$$\{p_{M}\} = [T_{ML}]\{p_{L}\} + [T_{MA}]\{p_{A}\} + [\varphi_{M}]\{q_{M}\}$$
(II-47)

$$\{p_A\} = \{\varphi_A\} \mathcal{Q}_A$$

$$if (A) is a to motion of (A) is a to b to (II-48) is a to (II$$

$$\{b_{s}\} = [T_{s_{L}}]\{b_{L}\}$$
 (II-49)

$$\{p_R\} = [T_{RL}]\{p_L\}$$
 (II-50)

$$\{p_{F}^{(n)}\} = \{T_{FL}\}\{p_{L}\} + [\varphi_{F}]\{q_{F}^{(n)}\}$$
(II-51)

$$\{p_{F}^{(2)}\} = [T_{FL}]\{p_{L}\} + [\varphi_{F}]\{q_{F}^{(2)}\}$$
(II-52)

$$\{p_{F}^{(i)}\} - [T_{FL}]\{p_{L}\} + [\varphi_{F}]\{q_{F}^{(i)}\}$$
(II-53)

$$\{b_{F}^{(4)}\} = [T_{FL}]\{b_{L}\} + [\phi_{F}]\{g_{F}^{(3)}\}$$
(II-54)

$$\{p_{i}\} = [\varphi_{i}]\{q_{i}\}$$
 (II-55)

The modal coordinates associated with the entire Center Lox Tank Vehicle,  $\{g_B\}$ , were then defined by the relation,

$$\{ \mathcal{J}_{B} \} = \begin{cases} \mathcal{J}_{Q} \mathcal{U} \} \\ \mathcal{J}_{Q} \mathcal{M} \} \\ \mathcal{J}_{A} \\ \mathcal{J}_{Q} \mathcal{H} \\ \mathcal{J}_{F} \} \\ \mathcal{J}_{F} \mathcal{J}_{F} \end{cases}$$

$$\left\{ \mathcal{J}_{F}^{(n)} \right\} \\ \mathcal{J}_{F} \mathcal{J}_{F} \mathcal{J}_{F} \end{pmatrix}$$

$$\left\{ \mathcal{J}_{F}^{(n)} \right\}$$

$$\left\{ \mathcal{J}_{F}^{(n)} \right\}$$

$$\left\{ \mathcal{J}_{F}^{(n)} \right\}$$

$$\left\{ \mathcal{J}_{F}^{(n)} \right\}$$

.

Attention should be called to the  $\{q_F^{(1)}\}, \{q_F^{(1)}\}$ , and  $\{q_F^{(3)}\}$  modal coordinates. As shown in Figure 134, the modal coordinates  $\{q_F^{(1)}\}$  were associated with Fuel Tanks number 1 and 3, and the modal coordinates  $\{q_F^{(2)}\}$  and  $\{q_F^{(3)}\}$  were associated with Fuel Tanks 2 and 4 respectively. This constraint restricted the Fuel Tanks to symmetric motion.

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$$\{p_{F}^{(3)}\} = [\varphi_{F}] \{q_{F}^{(1)}\} \qquad (2) \qquad \{p_{F}^{(2)}\} = [\varphi_{F}] \{q_{F}^{(3)}\} \\ \{p_{F}^{(3)}\} = [\varphi_{F}] \{q_{F}^{(1)}\} \qquad (2) \qquad \{p_{F}^{(2)}\} = [\varphi_{F}] \{q_{F}^{(3)}\}$$

FIGURE 134 MODAL COORDINATES OF FUEL TANKS

The generalized coordinates of each part of the vehicle were then expressed in terms of the modal coordinates  $\{q_B\}$  by combining expressions II-46 through II-55 with expression II-56 in the following manner:

(L) 
$$\{ p_{L} \} = [ \Phi_{L} ] \{ q_{B} \}$$
 (II-57)  
 $[ \Phi_{L} ] = ( [ 0 ], [ 0 ], ... [ 0 ], [ P_{L} ] ]$ 

(S) 
$$\{\phi_{s}\} = [\phi_{s}]\{q_{6}\}$$
 (II-58)  
 $[\phi_{s}] = [\tau_{s_{L}}][\phi_{L}] = (0], ..., [0], [\tau_{s_{L}}][\phi_{L}])$ 

.

(M)

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.

.

$$\{ p_{M} \} = \{ \Phi_{M} \} \{ q_{B} \}$$

$$[\Phi_{M}] = \{ [0], [\phi_{M}], [\tau_{MA}] \{ \phi_{A} \}, [0], ... [0], [\tau_{ML}] [\phi_{L}] \}$$

•

(II-59)

(U) (II-61)  $\{p_{u}\} = [\phi_{u}]\{q_{g}\}$   $[\phi_{u}] = [[\phi_{u}], [\tau_{u_{M}}][\phi_{M}], ... [0]]$ 

(R)  

$$\{p_{R}\} = [\Phi_{R}]\{q_{\mu}\}$$

$$[\Phi_{R}] = [[0], ... [0], [T_{RL}][\varphi_{L}]]$$
(II-62)

(F1)  

$$\{ p_{F}^{(n)} \} = [\Phi_{F}^{(n)}] \{ q_{B} \}$$
(II-63)  

$$\{ \Phi_{F}^{(n)} \} = ([0], [0], [0], [\phi_{F}], [0], [0], [\tau_{FL}] [\phi_{L}] )$$

.

.

.

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(F2)  

$$\{ p_{F}^{(2)} \} = [ \phi_{F}^{(2)} ] \{ q_{g} \}$$

$$[ \phi_{F}^{(2)} ] = [ [0], [0], [0], [0], [\phi_{F}], [0], [\tau_{FL}], [\phi_{L}] ]$$
(II-64)

(F3) 
$$\{p_{F}^{(n)}\} = \{\Phi_{F}^{(n)}\}\{q_{B}\}$$
 (II-65)

$$\{p_{F}^{(4)}\} = \{\phi_{F}^{(3)}\} \{q_{yB}\}$$

$$\{\phi_{F}^{(3)}\} = \{(o), [o], [o], [o], [\phi_{F}], [\tau_{FL}][\phi_{L}]\}$$

$$\{\phi_{F}^{(3)}\} = \{(o), [o], [o], [o], [\phi_{F}], [\tau_{FL}][\phi_{L}]\}$$

Such that the kinetic energy was expressed in terms of the modal coordinates  $\{a_B\}$  by combining Equations II-57 through II-66 with II-45 as shown in Equation II-67 below.

$$T = \frac{1}{2} \left\{ \dot{q}_{B} \right\}' \left( \left[ \dot{\Phi}_{L} \right]' \left[ A_{L} \right] \left[ \dot{\Phi}_{L} \right] + \left[ \dot{\Phi}_{S} \right]' \left[ A_{S} \right] \left[ \dot{\Phi}_{S} \right] + \left[ \dot{\Phi}_{M} \right]' \left[ A_{M} \right] \left[ \dot{\Phi}_{M} \right] \\ + \left[ \dot{\Phi}_{U} \right]' \left[ A_{U} \right] \left[ \dot{\Phi}_{U} \right] + \left[ \dot{\Phi}_{R} \right]' \left[ A_{R} \right] \left[ \dot{\Phi}_{R} \right] \\ + 2 \left[ \dot{\Phi}_{F}^{(0)} \right]' \left[ A_{F} \right] \left[ \dot{\Phi}_{F}^{(0)} \right] + \left[ \dot{\Phi}_{F}^{(2)} \right]' \left[ A_{F} \right] \left[ \dot{\Phi}_{F}^{(2)} \right] \\ + \left[ \dot{\Phi}_{F}^{(0)} \right]' \left[ A_{F} \right] \left[ \dot{\Phi}_{F}^{(0)} \right] + \left[ \dot{\Phi}_{F}^{(2)} \right]' \left[ A_{F} \right] \left[ \dot{\Phi}_{F}^{(0)} \right] \right] \left\{ \dot{q}_{B} \right\}$$

This expression was then written in terms of the individual modal mass matrices as,

$$\tau = \frac{1}{2} \left\{ \dot{q}_{B} \right\}^{\prime} \left( \left[ M_{L} \right] + \left[ M_{S} \right] + \left[ M_{M} \right] + \left[ M_{U} \right] \right] \\ + \left[ M_{R} \right] + 2 \left[ M_{F}^{(1)} \right] + \left[ M_{F}^{(2)} \right] + \left[ M_{F}^{(3)} \right] \right\} \left\{ \dot{q}_{B} \right\}$$
(II-68)

where the modal mass matrix of a component was expressed in the following generic form

$$[M] = [\phi]'[A][\phi]$$
(II-69)

The sum of the individual modal mass matrices was defined as the "Center Lox Tank Vehicle" modal mass matrix,  $[M_{\rm B}]$ , such that the kinetic energy of the complete "Center Lox Tank Vehicle" was finally written as,

$$T = \frac{1}{2} \{ \dot{q}_{B} \}' [M_{B} ] \{ \dot{q}_{B} \}$$
(II-70)

where  $[M_8]$  was the modal mass matrix for the Center Lox Tank Vehicle, and

.

 $\{q_{B}\}$  was the modal coordinate matrix for the Center Lox Tank Vehicle

The strain energy stored in the Center Lox Tank Vehicle was written in matrix form as,

$$U = \frac{1}{2} \{ q_{B} \}^{2} [F_{B}] \{ q_{B} \}$$
 (II-71)

where  $[{\tt F}_{\rm B}],$  the modal stiffness matrix for the Center Lox Tank Vehicle, was;

,

$$[F_{g}] = \begin{bmatrix} \dot{\lambda}_{u} & & \\ & \dot{\lambda}_{m} & \\ & & \dot{\lambda}_{r} $

TABLE 31
GEOMETRIC TRANSFORMATION MATRICES FOR THE
ELASTICALLY UNCOUPLED COMPONENTS

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GEC	ELASTIC	ALLY UN	COUPLED		MPO)	IEN.	TS	•	GEIMF Fispla Oi	TRIC TRANSP CEMENTS OF SPLACEMENTS	SPHATION FROM MIDSLE STAGE SP ADAPTER	, to
0	SECHETRIC TRANS	SFORMATION PP UPPER STAGE	134 13 14.5						ruy	CQ	LUNN	
	DISPLACEMENTS		~ 35							1	2	
		LUNA							1	1.000002 00	7.60790E 0	1
1	2	3	4		5				2	1.000008 60	6.93610E 0	1
3.00517E 00	1.40163E 01 -	-2.059298 01	2.57143E 00	3.					3	1.00000E 00	6.26430E 0	L
1.42139E 00	1.41286E 01 -	-1.79983E 01	2.24806E 00	0.					٠	1.000000 00	5.59250E Q	1
1.23802E DQ	1.22409E 01 -	-1.54037E 01	1.92459E 00	0.					5	1.000000 60	4.92070E 0	L
1.854448 00	1.035338 01 -	-1.28090E 01	1.40133E 00	٥.					6	1.00000E 00	4.24840E 0	L
1.47087E 00	8.46559E 00 -	-1.02144E 01	1.277968 00	0.					۲	1.000008 00	3-57700E 01	L
1.06727E 00	6.57779E 00 -	-7.61963E 00	9.545712-01	٥.						1.00000E 00	2.90520E 01	L
T.03720E-01	4.69023E 00 -	-5.02517E QO	6.31224E-01	٥.					9	1.000008 00	2.23340E 01	L
3.201445-01	2.80254E CO -	-2.43055E 00	3.07857E-01	٥.					10	1.00000E 00	1.54160E 01	L
-6,347788-02	9.14622E-01	1-64407E-01	~1.55515E-02	٥.					11	1.00000E GO	8.89800E OG	1
GEOMETRIC DISPLACEME	TRANSFORMATIC	N FROM STATE TO							12	1.000008 00	2.18000E 00	)
<b>QISPLACE</b>	MENIS OF SPICE	R PEAM							13	1.00000E 00	-4.53800E 00	)
	Cu	ILU#N							14	1.000006 00	-1.125608 01	L
9	10	11							15	1.00000E 00	-1.79750E 01	
0.	1.00000E CO	9.54660E 01						GEOMET	RIC TRA	NSFORMATION	FROM	
۰.	1.000008 60	8.8748GE 01						DISPLAC	EMENTS	OF CENTER L	DA TANG	
0.	1.00000E CO	8.203008 01			RC	×			C	OLUMA		
۰.	1.00000E CO	7.53120E 01					1		2	з	•	5
<b>0.</b>	1.00000E 00	*4.859408 01			1	0 1	-25577E	00 -3.05	164E-01	4_93959E-0	2 0.	0.
0.	1.00000E 00	6.18750E 01			1	1 I	.25407E-	-01 -1.48	188E-01	2.25004E-0	oz 0.	۰.
0.	1.00000E 06	5.515TOE 01										•
Q.	1.000008 00	4.84390E 01										
0.	1.000008 00	4.17210E 01							GEOMETI	RIC TRANSFOR	WATION FROM	
0.	1.000005 00	3.500308 01							DISPL	ACEHENTS OF	SPIDER BEAM	
٥.	1.00000E CO	2.82850E 01						AUE				
٥.	1.03000E 00	2.15470E 01								•	10	11
a.	1.000008 00	1.484908 01						1	۰.	9.71	550E-01 Q.	
0.	1.000005 00	\$,13100E CO						2	٥.	9-14	1205-01 0.	
0.	1.0000056 00	1.412008 00						3	٥.	8.41	690E-01 0.	
DISPLACEMEN	IS OF FUEL TAN	x 10						٠	0.	7.45	240E-01 0.	
DISPLACE	CO	ILUMM						3	٥.	7.20	#30E-01 0.	
								٠	٥.	4.54	393E-01 0.	
1	2	3						7	٥.	5.91	9632-01 0.	
٥.	2.144996-02	0.							٥.	5.27	533E-01 0.	
۰.	8.587998-02	0.						9	۰.	4.63	103E-01 0.	
0.	1.50310E-01	٥.						10	٥.	3,94	\$74E-01 0.	
٥.	2-147405-01	٥.						11	۰.	3.34	244E-01 0.	
٥.	2.79170E-01	٥.						12	٥.	2.69	\$14E-01 0.	
٥.	3.436078-01	e.						13	٥.	2.05	384E-91 0.	
0.	4.08037E-01	u.						14	٥.	1.40	9545-01 0.	
o.	4.12467E-01	v.						15	٥.	7.65	1446-02 0.	
	2.308916-01	v.										
~.		~						GEOMETR 1	C TRANSP	FORMATION FR	-9	
0.	4.01324E-01	o.										
o. o.	4.013246-01	0. 0.						CISPLACE	MENTS OF	F OUTRINGER	TO FANE	
o. o. o.	4.01326E-01 4.65756E-01 7.30186E-01	0. a. 0.			ROw			CISPLACE CISPLACE	MENTS OF	F CUTRINGER F CENTER LOX	TO FANE	
0. 0. 0. 0.	4.01326E-01 4.65756E-01 7.30184E-01 7.94516E-01	a. a. a.			Rûw		11	CISPLACE	MENTS OF	F OUTRIISER F CENTER LOX UPA 13	TO TANE 14	15
0. 0. 0. 0. 0. 0.	4.01326E-01 4.63756E-01 7.30184E-01 7.94516E-01 8.55046E-01 8.23481E-01	0. 0. 0. 0.			2 2	٥.	ц	CISPLACE TISPLACE	MENTS OF MENTS OF COLU	E OUTRIISER F CENTER LOX UMA 13 5.05123E-02	14 -3.12536E-01	15 1.26202E 90
0. 0. 0. 0. 0. 0.	4.01324E-01 4.657585-01 7.30184E-01 7.94516E-01 8.59046E-01 9.23483E-01	0. 0. 0. 0.			R04 2 3	a. o.	ų	CISPLACE CISPLACE 12 0. 0.	MENTS OF MENTS OF COLU	E OUTRINGER E CENTER LOX JAN 13 5.05123E-02 2.23344E-02	14 -3.12536E-01 1.47655E-01	15 1+26202E 00 -1-25521E-01

.
TABLE 32 COMPONENT MODES OF VEHICLE WITHOUT OUTER LOX TANKS

.

		FIRST COMPON	ENT				
		FODES OF	U)			SECOND COP	PONENT
CGLL.	15T MODE	280 4006	3RD HODE	41H MODE	0K	DES OF (M) WITH	(U) RIGID
,0141	21428 (75	ay.UA LPS	174.16 CPS	273.08 CP5	COLL. Point	151 MODE 14-35 3PS	2ND HODE 89.95 CPS
-	9.8.2.3602-02	1.07965615-01	2.4422816E-01	5.4789109E-01	001	1.0368176E-02	3.90024300-00
2	8+11958/0E-02	1.1838275E-01	1.4369598E-01	Z.1827023E-01	z	6.1357344E-09	4.21178265-02
	6.5316995E-02	7.46471638-02	7.33776948-02	4.1197221E-0Z	3	5.34984346-03	4.61456148-02
•	5.0869723E-02	2.5571569E-02	-7.8173396E-03	-5.2769397E-02	, 4	4-53452786-03	4+1368621F+02
3	3-42417246-02	-2.33437008-02	-3.8395116E-02	3.27235316-02	5	3.67238765-03	3-65013145-02
6	1.78340818-02	-4.19272078-02	3.3220338E-02	6-1911646E-04	•	2.8791005E~03	3-11782275-02
7	6.9849456E-03	-2.34268398-02	3.2963625E-02	-3.1780140E-02	7	2.16293565-03	2.55621525-02
8	2.34682018-03	-9.51409198-03	1.4239716E-02	-1.5388012E-02	r	1.53213986-03	1.000000000000
		•			•	9 96272295-04	1.9851100E-02
					,	1.7321719E-U4	1+42685408-02
			ONDOWENT		10	5.01102352-04	9.0626127E-03
			v-reatar 		11	2.3887028E-04	4.5261999E-03
	,	UCES OF TAL WITH	1 (A) AND 101 K10	510	12	3.7481043E-05	9-2269560E-04
		8.995	CPS		13	8.0862670E-13	8.96518392-12
		1 5.14430	696-03		14	2.0572569E-13	2.3409367E-12
		2 2.04880	936-04				

#### FOURTH COMPONENT MODES OF ILL XITH ALL OTHER COMPONENTS RIGID

COLL. POINT	IST MODE O CPS	2ND HODE O CPS	3R0 MODE 14.67 CPS	41H MODE 70.92 CPS	STH MODE 214.58 CPS	6TH #00E 519.59 CPS	7TH NODE 881.54 CPS	8TH #00E 1226 CPS
£	1.000000000 00	-2.00000008 00	4.23529556-02	-1.12012446-02	-8.0286235E-03	-6.4647484E-03	6.4918731E-03	-3.7809066E-04
2	1.000000028 00	-1.1691000E 01	4.5449349E-02	-1.0168233E-02	9.45727988-04	1-2988802E-02	-1.5081862E-02	-6.1849724E-03
3	1.000000000 00	-2.1382000E 01	4.5520352E-02	-1-6736109E-04	5.3939579E-02	1.2262895E-01	-1.3364180E-01	-4.0611693E-02
*	1.00000008 00	-3-1074001E 01	4.49941938-02	1.09394946-02	1,0833035E-01	2.1077425E-01	-1.7579772E-01	-1.1775114E-01
5	1.00000005 00	-4.0763001E 01	4.3249546E-02	2.3551196E-02	1.6000935E-01	2+4853520E-01	-1.0716090E-01	-1.92651222-01
5	1.00000028 00	-5.04560018 01	4.0377785E-02	3.6770886E-02	2.0230714E-01	2.2359037E-01	3.6415722E-02	-1.8239162E-01
7	1.0000003E 00	-5.0146999E 03	3.6475152E-02	4.9715651E-02	2-2988843E-01	1.41798416-01	1.73282068-01	-4.8190587E-02
4	1.00000002E 00	-6.9538001E 01	3.14378338-02	6.1531699E-02	2.3910762E-01	2-4504056E-02	2.2527492E-01	1.4329112E-01
,	1.000000E CO	-7,95299985 01	2-5%32156-42	7-1404417E-02	2.28384598-01	-9.5721021E-02	1.3521370E-01	2,5994834E-01
10	1.00000000 00	-8.9221001E 01	1.95645858-82	7.8503517E-02	1.983761*E-01	-1.#385224E-01	5.8083205E-04	1.97105098-01
11	1.00000028 00	-9.8911998E C1	1.26609738-42	8.2335412E-02	1.5435452E-01	-2-1495/10E-01	-1.4077358E-01	7-41433246-03
12	1.02220015 00	-1.08603026 02	5.3284484E- <b>4</b> 3	8-28515118-02	9.96396102-02	-1.8781658E-01	-2.1031508E-01	-1.84382958-01
13	1.00000088 00	-1.18294028 C2	-2.3442015E-03	7.9696603E-02	3.9691722E-02	~1.1113700E-01	-1-\$906330E-01	-2.2693398E-01
14	1.00000028 00	-1.27986002E 02	-1.01372736-02	7.5076619E-02	-2.3239111E-02	4-1482524E-G4	-1.26778955-02	-1.9673689E-02
13	1.0000000000000000000000000000000000000	-1.3767702E C2	-1.90326168-02	3.8356383E-02	-2.1000764E-02	1.36543618-02	9.24331748-03	1.21240916-02

#### TABLE 33 FREQUENCIES OF THE VEHICLE WITHOUT OUTER LOX TANKS

		ANALYSIS OF THIS REPORT	VIBRATION TEST* RESULTS (WITH OUTER LOX TANKS)
l <sup>st</sup>	mode	(c.p.s.) 12.3 <sup>14</sup>	(c.p.s.) 13.4
2 <sup>nd</sup>	mode	45.29	44.7
3 <sup>rd</sup>	mode	71.13	
4th	mode	80.09	
$5^{th}$	mode	80.59	
$6^{th}$	mode	82.97	
$7^{th}$	mode	93.31	
8 <sup>th</sup>	mode	166.07	

\*Taken from NASA TN D 1593, Investigation of the Lateral Vibration Characteristics of a 1/5-scale Model of Saturn SA-1 by John S. Mixson, John J. Catherine, and Ali Arman, January, 1963.

The modal influence coefficient matrix,  $[G_B]$ , was then formed from the modal stiffness matrix by the method presented in Section 2.2.3.4 of this report, such that the "modal-mode shapes" and modal frequencies of the Center Lox Tank Vehicle were determined through the iteration of the expression,

$$[G_{B}][M_{B}]\{\pi\} = \lambda \{\pi\} \qquad (II-73)$$

such that  $[\pi_c]$ , the "modal-mode shapes" of the Center Lox Tank Vehicle, was a 26 x 10 matrix containing two rigid body modes and eight elastic modes.

It is somewhat disconcerting to find that the "modal-mode shape" matrix,  $[\pi_c]$ , has little or no direct physical interpretation. However, gratification is obtained through the use of an expression such as II-74. The deflections of the component structures may be determined in this manner and plotted to yield a picture of the complete vehicle in a particular bending mode. However, it should not be forgotten that the primary purpose of  $[\pi_c]$  is the reduction of the number of degrees-of-freedom in the final coupling analysis.

$$\{b\} = [\Phi][\pi_c]\{q_c\}$$
 (II-74)

or

$$\{p\} = [\varphi_c]\{q_{c}\}$$
 (II-75)

where

$$[\varphi_c] = [\Phi][\pi_c]$$
(II-76)

It should also be noted here that,

$$\{q_{B}\} = [\pi_{c}]\{q_{c}\}$$
 (II-77)

where  $\{g_c\}$  was the final modal generalized coordinates of the Center Lox Tank Vehicle.

#### 3.3 Coupling of Outer Lox Tanks With the Rest of the Vehicle

#### 3.3.1 Analysis of Outer Lox Tanks

3.3.1.1 Outer Lox Tank Geometry and Basic Data



FIGURE 135 GEOMETRY OF OUTER LOX TANK

The inertia properties of the ends of the tank are:

$$M_{T} = \text{mass} = 6.0202 \text{ lb}_{M}$$

$$I_{T} = \text{moment of inertia about T.}$$

$$= 490.51505 \text{ lb}_{M} - \text{in}$$

$$M_{B} = \text{mass} = 7.141 \text{ lb}_{M}$$

$$\overline{\chi}_{B} - \chi_{B} - \chi_{B} - | = \frac{67.12528}{7.141}$$

$$I_{B} = \text{moment of inertia about B}$$

$$= 869.57078 \text{ lb}_{M} - \text{in}^{2}$$

$$B = \frac{67.12528}{7.141}$$

The shell has the following material properties

$$E = 10.6 \times 10^6 \, lb_F/in^2$$
 (11-78)

$$\nu = 0.3 \tag{II-79}$$

$$G = 4.0769 \times 10^6 \ lb_F/in^2$$
 (II-80)

$$\rho = 0.1 \, lb_{\rm M}/in^3$$
 (II-81)

The total mass of the shell is then

$$2 \pi b \tau \perp \rho = 10.0577 \ lb_{M}$$
 (II-82)

and the total mass of one outer lox tank is 23.2189  $\rm lb_{M}$ 

# 3.3.1.2 Outer Lox Tank Collocation Point Geometry and Generalized Coordinates

Fifteen points at equal intervals along the shell are shown in Figure 136.



FIGURE 136 COLLOCATION POINTS

Each point is allowed 3 degrees-of-freedom  $\xi_{\rm i},\,\eta_{\rm i}$  and  $\zeta_{\rm i}$  describing bending in two planes and longitudinal deformation.

In addition each end has five degrees-of-freedom as shown in Figure 137. These are





#### FIGURE 137 GENERALIZED COORDINATES FOR THE ENDS

Compatibility at the ends requires

$$\begin{aligned} \tilde{s}_{1} &= \tilde{s}_{T} & (II-83) \\ \tilde{s}_{15} &= \tilde{s}_{8} \\ \tilde{s}_{1} &= \tilde{s}_{T} \\ \tilde{s}_{15} &= \tilde{s}_{8} \\ \tilde{s}_{1} &= \tilde{s}_{T} \\ \tilde{s}_{1} &= \tilde{s}_{T} \\ \tilde{s}_{1} &= \tilde{s}_{T} \end{aligned}$$

The complete set of generalized coordinates for the outer lox tank is then



(II-84)

#### 3.3.1.3 Outer Lox Tank Influence Coefficients and Modes

The influence coefficients for the structure cantilevered at x = 333.857 were first calculated by complementary energy techniques



## FIGURE 138 CANTILEVERED OUTER LOX TANK

These influence coefficients were then transformed to influence coefficients on on simple supports as shown in Figure 139.



FIGURE 139 OUTER LOX TANK ON SIMPLE SUPPORTS

The modes in bending were then obtained from

$$[E_{\tau}^{(5)}][A_{\tau}^{(5)}][\psi] = \lambda\{\psi\}$$
 (iterate for first 3 modes) (II-85)

where  $[A_T^{(S)}]$  includes inertia properties of the ends of the tank and it is assumed that the modes are the same in both planes.

The longitudinal modes were obtained from

$$\begin{bmatrix} I_{5}^{(L)} \\ E_{\Gamma}^{(L)} \end{bmatrix} \begin{bmatrix} A_{\Gamma}^{(L)} \end{bmatrix} \{ \varphi \} = \lambda \{ \varphi \} \text{ (iterate for first 2 modes)}$$

where  $[E_{T}^{(L)}]$  is the longitudinal influence coefficient for the tank constrained at x = 350.717.

#### 3.3.1.4 Outer Lox Tank Modal Transformation Matrix

The complete set of modes for the lox tank are associated with the following generalized coordinates:

> > (II-87)

The complete modal transformation is

 $\begin{array}{c} \frac{1}{2} \rho_{T} \frac{1}{2} = \begin{bmatrix} \frac{1}{2} \frac{1}{2} \\ \frac{1}{2} \rho_{T} \frac{1}{2} \end{bmatrix} = \begin{bmatrix} \frac{1}{2} \frac{1}{2} \frac{1}{2} \\ \frac{1}{2} \frac{1}{2} \frac{1}{2} \end{bmatrix} \\ \begin{array}{c} \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \\ \frac{1}{2} \frac$ 

642

which defines the 49 x 9 matrix [ $\varphi_{\rm T}$ ].

.

The outer lox tanks are numbered as in Figure 140.



FIGURE 140 ARRANGEMENT OF FOUR OUTER LOX TANKS

The total displacements of the outer lox tanks are

(II-88)

$$\{ b_{T}^{(n)} \} = [\varphi_{T}] \{ q_{T}^{(n)} \} + [T_{TL}^{(n)}] \{ b_{L} \}$$

$$\{ b_{T}^{(n)} \} = [\varphi_{T}] \{ q_{T}^{(n)} \} + [T_{TL}^{(n)}] \{ b_{L} \}$$
(II-89)

motion of	motion of rigid
outer lox	outer lox tanks
tanks relative	due to motion of
to center lox	center lox tank
tank	

Due to symmetry:

$$\{\dot{p}_{i}^{(i)}\} = \{\dot{p}_{i}^{(i)}\}$$
(II-90)

The geometric transformations can be considered as

$$[\tau_{\tau_{L}}^{\prime}] = [\tau_{\tau_{R}}^{\prime}] [\tau_{\tau_{L}}]$$
(II-92)

where the displacements are first transformed to outrigger and then from outrigger to center lox tank.

#### 3.3.2 <u>Geometric Transformation Relating Motion of Rigid</u> Outer Lox Tank to Outrigger

In deriving the geometric relations the tanks are assumed rigid and cantilivered to the outrigger



FIGURE 141 GEOMETRY OF OUTER LOX TANK - OUTRIGGER

At the joint the following is true (for the rigid tank with rigid joint)

$$\begin{aligned} z_{8}^{(i)} &= \underline{z}_{8} - (z_{8}^{(i)} - z_{8}) \hat{z}_{8} \\ z_{B}^{(i)} &= (z_{8} + (x_{8} - x_{8})) \hat{z}_{8} \\ \hat{z}_{B}^{(i)} &= \hat{z}_{8} \end{aligned}$$
(II-93)

 $\operatorname{or}$ 

•

$$\begin{bmatrix} \mathbf{J}_{\mathbf{g}} \\ \mathbf{J}_{\mathbf{g}} \end{bmatrix} = \begin{bmatrix} \mathbf{J} & \mathbf{J} & \mathbf{J}_{\mathbf{g}} \end{bmatrix} \begin{bmatrix} \mathbf{J}_{\mathbf{g}} \\ \mathbf{J}_{\mathbf{g}} \end{bmatrix} \begin{bmatrix} \mathbf{J} & \mathbf{J}_{\mathbf{g}} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J}_{\mathbf{g}} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J}_{\mathbf{g}} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J}_{\mathbf{g}} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J}_{\mathbf{g}} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{J} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{J} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix} \end{bmatrix} \begin{bmatrix} \mathbf{J} \end{bmatrix} \begin{bmatrix} \mathbf{J} \\ \mathbf{J} \end{bmatrix}$$

Also for the rigid tank

$$\{ s \} = \{ i \} s_{g}^{(i)}$$
(II-95)  

$$\Theta_{T} = \Theta_{g}^{(i)}$$
  

$$\{ s \} = \{ i \} s_{g}^{(i)} + \{ x_{b} - x \} \Rightarrow_{g}^{(i)}$$
  

$$\varphi_{g} = \Theta_{g}^{(i)}$$
  

$$\psi_{T} = 0$$
  

$$\{ y \} = \{ 0 \}$$
  

$$\psi_{g} = 0$$

or

•

•

$$\frac{1}{2} \frac{1}{r}; = \frac{1}{r}; = 0 \quad 0 \quad \frac{1}{s}; = \frac{1}{s}; \quad (II-96)$$

(11-97)

$$\{ b_{T}^{(n)} \} = \begin{bmatrix} \{ 1 \} & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 1 & 0 \\ 0 & 1 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 1 & 0 & -(z_{0} - z_{0}^{n}) \\ 0 & 1 & (x_{R} - x_{0}) \\ 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & (x_{R} - x_{0}) \\ 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

## 3.3.3 Spider-Outer Lox Tank Constraints

.



FIGURE 142 SPIDER BEAM - OUTER LOX TANK COMPATIBILITY

The compatibility relations at the joint can be written as

$$\psi_{1}^{(i)} - (\lambda_{1} - \lambda_{0}) \psi_{1}^{(i)} = 0 \qquad (II-98)$$

$$S_{T}^{(i)} + (x_{T} - x_{0}) \Theta_{T}^{(i)} = \beta_{S_{10}} - (x_{0} - x_{S}) \beta_{S_{11}}$$
(II-99)

$$\begin{split} \vec{s}_{T}^{(1)} + (\vec{z}_{1} - \vec{z}_{1}^{(0)}) \theta_{T}^{(0)} - (y_{1} - y_{1}^{(0)}) \psi_{T}^{(1)} \\ &= \left(\frac{y_{1}\vec{z}_{S_{2}} - \vec{z}_{1}y_{S_{2}}}{y_{S_{1}}\vec{z}_{S_{2}} - y_{S_{1}}\vec{z}_{S_{1}}}\right) \beta_{S_{1}} + \frac{(-y_{1}\vec{z}_{S_{1}} + \vec{z}_{1}y_{S_{1}})}{y_{S_{1}}\vec{z}_{S_{2}} - y_{S_{1}}\vec{z}_{S_{1}}}\right) \beta_{S_{2}} \end{split}$$
(II-100)

$$\begin{split} \xi_{T}^{(h)} + (z_{2} - z_{T}^{(h)}) \xi_{T}^{(h)} - (y_{2} - y_{T}^{(h)}) \psi_{T}^{(h)} \\ &= \left( \frac{y_{1} z_{5_{1}} - z_{1} y_{5_{2}}}{y_{5_{1}} z_{5_{1}} - y_{5_{1}} z_{5_{1}}} \right) \beta_{S_{1}} + \left( \frac{-y_{1} z_{5_{1}} + z_{2} y_{5_{1}}}{y_{5_{1}} z_{5_{2}} - y_{5_{2}} z_{5_{1}}} \right) \beta_{S_{2}} \end{split}$$
(II-101)

or in matrix form

.

.

.

$$[L_{T}^{(i)}]_{i}b_{T}^{(i)}] = [L_{S}^{(i)}]_{i}b_{S}]$$
 (II-LO2)

$$[L_{\tau}^{(2)}]_{\xi} b_{\tau}^{(2)} = [L_{\delta}^{(2)}]_{\xi} b_{\delta}$$
 (II-103)

The total displacement of the spider beam is assumed to be

$$\{b_s\} = [\tau_c]\{b_c\} + [\psi_s]\{q_s\}$$
 (II-104)

ø

where the spider beam "modes" are

(II-105)

which describe & symmetric deformation defined by

$$\begin{array}{l} p_{s}^{(1)} = p_{s}^{(0)} \\ p_{s}^{(2)} = p_{s}^{(2)} \\ p_{s}^{(2)} = p_{s}^{(2)} \\ p_{s}^{(1)} = p_{s}^{(6)} \\ p_{s}^{(4)} = p_{s}^{(5)} \end{array}$$

$$\dot{\beta}_{S}^{(1)} = \dot{\beta}_{S}^{(n)} = \dot{\beta}_{S}^{(n)} = 0$$
(II-107)

#### 3.3.4 Modal Coupling for Natural Modes of Complete Vehicle

Rigid tanks - geometric transformation:

$$t b_{r}^{\infty} \} = [T_{rc}^{(m)}] \{b_{c}\}$$
 (II-108)

$$\{\flat_{r}^{(2)}\} = [\tau_{rL}^{(2)}]\{\flat_{L}\}$$
 (II-109)

Spider beam-outer lox end constraints:

$$[L_{\tau}^{(n)}] \{p_{\tau}^{(n)}\} = [L_{s}^{(n)}] \{p_{s}\}$$
(II-110)

$$[r_{2}, f_{2}, f_{3}] = [r_{2}, f_{2}, f_{2}]$$
(II-III)

Modal coordinate transformations:

$$\{t_{r}^{(i)}\} = [\{t_{r}^{(i)}, t_{r}^{(i)}\} + [t_{r}^{(i)}, t_{p_{L}}]\}$$
 (II-112)

$$\{ b_{1}^{(1)} \} = [i_{1}^{(1)}] \{ b_{1}^{(1)} \} + [i_{n}^{(1)}] \{ b_{1} \}$$
(II-113)

$$\{b_{i}\} = [\tilde{2}_{i}][\tau_{i}]\{q_{i}\}$$
 (II-114)

$$\{p_{s}\} = \{T_{s}, \{p_{s}\} + \{p_{s}\}, \{p_{s}\}\}$$
(II-115)

Introduce

$$\frac{2x}{2x} = \begin{bmatrix} -2x \\ -2x \end{bmatrix} = \begin{bmatrix} -2x \\ -2x \end{bmatrix}$$

$$\frac{4x}{2x}$$

$$\frac{4x}{2x}$$

$$\frac{4x}{2x}$$
(II-116)

Then we can write

$$\begin{cases} p_{L} \hat{s} &= \left\{ \begin{array}{c} \Phi_{LN} \\ p_{T} \end{array} \right\} \\ \left\{ p_{T} \right\} \\ \left\{ p_{S} \right\} \\ \left\{ \begin{array}{c} \Phi_{TN} \\ p_{T} \end{array} \right\} \\ \left\{ p_{S} \right\} \\ \left\{ \begin{array}{c} \Phi_{S} \\ p_{T} \end{array} \right\} \\ \\ \left\{ \begin{array}{c} \Phi_{S} \\ p_{T} \end{array} \right\} \\ \\ \left\{ \begin{array}{c} \Phi_{S} \\ p_{T} \end{array} \right\} \\ \\ \left\{ \begin{array}{c} \Phi_{S} \\ p_{T} \end{array} \right\} \\ \\ \left\{ \begin{array}{c} \Phi_{S} \\ p_{T} \end{array} \right\} \\ \\ \left\{ \begin{array}{c} \Phi_{S} \\ p_{T} \end{array} \right\} \\ \\ \left\{ \begin{array}{c} \Phi_{S} \end{array} \right$$

.

.

$$\begin{bmatrix} \oint_{1-N} \end{bmatrix} = \left[ \begin{bmatrix} p_{1}^{(1)} \\ p_{2}^{(1)} \end{bmatrix}, \begin{bmatrix} 0 \\ 0 \end{bmatrix}, \begin{bmatrix} 0$$

## 3.3.4.1 Constraint-Compatibility Matrix

.

$$[-]_{m} = [-]_{m} = [-]_{m} [ \frac{1}{2} ]_{m}$$
(II-119)

$$(II-120)$$

or

$$\{11-121\}$$

•

$$\begin{bmatrix} -1 \\ -1 \end{bmatrix} = \begin{bmatrix} -1 \\ -1 \\ -1 \end{bmatrix} \begin{bmatrix} -1 \\ -$$

•

.

.

$$\frac{32\times1}{2} = [\tau_{c}] \{ \frac{2\times1}{2} \} + [\tau_{v}] \frac{2\times1}{2}$$
 (II-123)

.

.

where  $\{q_{\rho}\}$  = coordinates to be eliminated by use of constraint conditions

 $[L][T_{0}]{}_{2_{0}} + [L][T_{v}]{}_{q_{v}} = for$  (II-126)

$$\{y_{c}\} = -([L][T_{0}])^{-}[L][T_{v}] \{y_{v}\}$$
 (II-127)

$$i q_{w} i = ([T_v] - [T_o]([L][T_o])^{-1}[L][T_v]) i q_{y} i$$

$$(II-128)$$

$$i q_{w} i = [T_v] i q_{y} i$$
compatibility matrix (II-129)

.

$$T = \frac{1}{2} \left\{ \dot{q}_{c} \right\} \left[ M_{c} \right] \left\{ \dot{q}_{c} \right\} + 2 \left\{ \dot{q}_{T}^{(1)} \right\} \left[ M_{T} \right] \left\{ \dot{q}_{T}^{(2)} \right\}$$

$$+ 2 \left\{ \dot{q}_{T}^{(3)} \right\} \left[ M_{T} \right] \left\{ \dot{q}_{T}^{(3)} \right\}$$

$$+ 2 \left\{ \dot{q}_{T}^{(3)} \right\} \left[ M_{T} \right] \left\{ \dot{q}_{T}^{(3)} \right\}$$

$$+ 2 \left\{ \dot{q}_{T}^{(3)} \right\} \left[ M_{T} \right] \left\{ \dot{q}_{T}^{(3)} \right\}$$

$$+ 2 \left\{ \dot{q}_{T}^{(3)} \right\} \left[ M_{T} \right] \left\{ \dot{q}_{T}^{(3)} \right\}$$

$$+ 2 \left\{ \dot{q}_{T}^{(3)} \right\} \left[ M_{T} \right] \left\{ \dot{q}_{T}^{(3)} \right\}$$

$$+ 2 \left\{ \dot{q}_{T}^{(3)} \right\} \left[ M_{T} \right] \left\{ \dot{q}_{T}^{(3)} \right\}$$

$$+ 2 \left\{ \dot{q}_{T}^{(3)} \right\} \left[ M_{T} \right] \left\{ \dot{q}_{T}^{(3)} \right\}$$

$$\begin{bmatrix} M_{c} \end{bmatrix} - \begin{bmatrix} \pi_{c} \end{bmatrix}^{\prime} \begin{bmatrix} M_{B} \end{bmatrix} \begin{bmatrix} \pi_{c} \end{bmatrix}$$

$$\begin{bmatrix} M_{T} \end{bmatrix} = \begin{bmatrix} \Gamma_{1} \end{bmatrix}^{\prime} \begin{bmatrix} \frac{1}{2} \chi_{e}, \chi_{1} & \chi_{1} \\ \vdots & \chi_{e}, \chi_{1} & \chi_{1} \end{bmatrix} \begin{bmatrix} \Lambda_{T}^{(s)} \end{bmatrix} \begin{bmatrix} \frac{1}{2} \chi_{e}, \chi_{1} & \chi_{1} \\ \vdots & \chi_{e}, \chi_{1} & \chi_{1} \end{bmatrix}$$

$$(II-132)$$

$$\tau = \frac{1}{2} \left\{ \dot{q}_{v} \right\}^{\prime} [M_{v}] \left\{ \dot{q}_{v} \right\}$$
(II-133)  
$$[M_{v}] = [\tau]^{\prime} \begin{bmatrix} [M_{c}] \\ & & \\ & & \\ & & \\ & & \\ & & & & \\ & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & &$$

.

3.3.4.3 Strain Energy

$$U = \frac{1}{2} \left( \{q_{c}\}^{\prime} \Gamma_{\lambda_{c}}^{\prime} \} \{q_{c}\} + 2\{q_{T}^{\prime\prime}\}^{\prime} \Gamma_{\lambda_{T}}^{\prime} \} \{q_{s}^{\prime\prime\prime}\}^{\prime\prime} \right)$$
(II-135)  
$$2\{q_{T}^{\prime\prime\prime}\}^{\prime} \Gamma_{\lambda_{T}}^{\prime} \} \{q_{s}\}^{\prime} [F_{s}] \{q_{s}\}^{\prime} \right)$$

$$\begin{bmatrix} \Gamma_{\lambda_{c}}^{L} \end{bmatrix} = \begin{bmatrix} 0 & 0 & 0 \\ 0 & \lambda_{c}^{(n)} & \lambda_{c}^{(n)} & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$
(II-136)

,

 $\begin{bmatrix} {}^{L}_{X_{1}} \end{bmatrix} = \begin{bmatrix} {}^{L}_{X_{1}^{(1)}} & & \text{longitudinal} \\ 0 & \text{flapping stiffness} \\ & {}^{L}_{S}^{(n)} & & \\$ 

$$\begin{bmatrix} x_{s} \\ x_{s} \end{bmatrix} = \begin{bmatrix} v \end{bmatrix} / \begin{bmatrix} v \\ v \end{bmatrix} / \begin{bmatrix} E_{s} \\ v \end{bmatrix} / \begin{bmatrix} v \\ v$$

 $U = \frac{1}{2} \{q_{V}\}' [F_{V}] \{q_{V}\}$  (II-140)

$$[F_{v}] = [T]' \begin{bmatrix} F_{\lambda_{c}} \\ F_{\lambda_{T}} \end{bmatrix} \begin{bmatrix} F_{\lambda_{T}} \\ F_{\lambda_{T}} \end{bmatrix} \begin{bmatrix} T \end{bmatrix}$$
(II-141)

$$[G_{v}] = [\Gamma]'[S]([S]'[F_{v}][S])^{-1}[S]'[\Gamma]$$
 (II-142)

$$\begin{bmatrix} 24 \times 2^{2} \\ \begin{bmatrix} S \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ \vdots \\ \vdots \\ \vdots \end{bmatrix}$$
 (II-143)

$$[\Gamma] = [1] - [M_{\nu}][\pi_{R}]'[\pi_{R}]'[M_{\nu}][\pi_{R}]'] \qquad (II-144)$$

$$\begin{bmatrix} \mathbf{x}_{R} \\ \mathbf{x}_{R} \end{bmatrix} = \begin{bmatrix} \mathbf{i} & \mathbf{0} \\ \mathbf{0} & \mathbf{i} \\ \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \mathbf{0} \end{bmatrix}$$
(II-145)

## 3.3.4.4 Vibration Modes and Frequencies

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Natural vibration modes and frequencies are obtained by iteration of

.

$$[G_{Y}][M_{Y}]{\pi} = \lambda \{\pi\}$$
(II-146)

$$\{ p_{L} \} = [ \oint_{LW} ] [ \top ] [ \pi ] \{ q \} = [ \varphi^{(L)} ] \{ q \}$$

$$\{ p_{1}^{(L)} \} = [ \oint_{TW}^{(L)} ] [ \top ] [ \pi ] \{ q \} = [ \varphi^{(T)} ] \{ q \}$$

$$\{ p_{1}^{(L)} \} = [ \oint_{TW}^{(L)} ] [ \top ] [ \pi ] \{ q \} = [ \varphi^{(T2)} ] \{ q \}$$

$$\{ p_{1}^{(L)} \} = [ \oint_{F}^{(L)} ] [ \top ] [ \pi ] \{ q \} = [ \varphi^{(T2)} ] \{ q \}$$

$$\{ p_{F}^{(T)} \} = [ \oint_{F}^{(L)} ] [ \top ] [ \top ] [ \pi ] \{ q \} = [ \varphi^{(F1)} ] \{ q \}$$

$$\{ p_{F}^{(L)} \} = [ \oint_{F}^{(L)} ] [ \top ] [ \top ] [ \pi ] \{ q \} = [ \varphi^{(F1)} ] \{ q \}$$

$$\{ p_{F}^{(L)} \} = [ \oint_{F}^{(L)} ] [ \top ] [ \top ] [ \pi ] \{ q \} = [ \varphi^{(F2)} ] \{ q \}$$

$$\{ p_{F}^{(L)} \} = [ \oint_{F}^{(L)} ] [ \top ] [ \top ] [ \pi ] \{ q \} = [ \varphi^{(F1)} ] \{ q \}$$

$$\{ p_{K} \} = [ \oint_{K}_{M} ] [ \top ] [ \top ] [ \pi ] \{ q \} = [ \varphi^{(M)} ] \{ q \}$$

$$\{ p_{M} \} = [ \oint_{M} ] [ \neg ] [ \top ] [ \top ] [ \neg ] \{ q \} = [ \varphi^{(M)} ] \{ q \}$$

$$\{ p_{U} \} = [ \oint_{U} ] [ \neg ] [ \neg ] [ \neg ] \{ q \} = [ \varphi^{(M)} ] \{ q \}$$

.

Mode shapes:

(II-147)

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### APPENDIX III

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COMPUTATIONAL PROCEDURES FOR FINITE DEGREE-OF-FREEDOM EIGENVALUE PROBLEMS

#### 1.0 INTRODUCTION

In this appendix we shall consider some numerical methods for solving the general eigenvalue problem. We shall consider the equations

$$[N/\lambda]{\varphi} = \{0\}$$
(III-1)

where  $[N(\lambda)]$  is a function of the parameter  $\lambda$  such that  $[N(\lambda)]$  is real when  $\lambda$  is real. Important special cases are

$$(1) \qquad \qquad [\lambda, \lambda] = [A] - \lambda [K] \qquad (III-2)$$

(2) 
$$\left\{ k_{1} \right\}_{k=1}^{\infty} = \lambda \left\{ k_{1} - \left\{ k_{1} \right\} \right\}$$
(III-3)

(3) 
$$\left[ \sum_{i} \sum_{j=1}^{\infty} \left[ \sum_{j=1$$

The first example is the eigenvalue problem of the general theory of vibrations. In the case of restrained systems, where [K] exists, it can be put in the same form as (2). It is shown in Section 2.2.3.4 that it can be put in the same form as (2) even when  $[K]^{-1}$  does not exist. Case (2) represents, then, an important sub-case of the general problem:

$$[\kappa] \{ \varphi \} = \chi \{ \varphi \}$$
 (III-5)

When a real eigenvalue exists and it is the largest then the eigenvalue and eigenvector may be obtained from Equation III-5 by iteration.

When the problem indicates that complex eigenvalues may be present, the general case, Equation III-1, must be considered. The case of interest in this report, is expressed in Equation III-4 which has, in general, complex eigenvalues. Further examples are

(4) 
$$[N_{1\lambda}] = \lambda^{2}[A] + \lambda [B] + [K]; [B]' = [B]$$
 (III-6)

which arises in the general theory of damped vibrations (see Section 2.2.3.5) and

(5) 
$$[N(\lambda)] = \lambda^2 [A] + \lambda [G] + [K]; [G]' = -[G]$$
 (III-7)

which arises in the general theory of vibrations about a point of steady motion

(examples are afforded by the vibrations of wings having large rotating machinery, vibrations of helicopter blades, perturbations of rotating space stations, perturbations of the 3-body problem, etc., etc.).

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2.0 SOLUTIONS BY MATRIX ITERATION

When this method applies, the equation

$$\{N\}\{\varphi\} = \lambda\{\varphi\}$$
(III-B)

is solved by assuming a trial  $\{\phi\},$  say  $\{\phi\}_o$  and computing

$$\{\varphi\}_{I} = [N] \{\varphi\}_{0}$$
(III-9)

and

 $\{\phi\}$  is normalized, for example by dividing by  $\{\phi\}'\{\phi\}$  so that

Then

$$\{\varphi\}_2 = [N H \varphi]_1$$
 (III-12)

and the procedure is repeated. Proof of the convergence of the process in the case of a dominant real eigenvalue is given by Frazer, Duncan, and Collar<sup>1</sup>.

# 3.0 SOLUTIONS TO THE GENERAL PROBLEM BY THE TAYLOR'S SERIES METHOD OF WIELANDT

#### 3.1 Computational Procedure

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The method to be described depends on having an estimate of the eigenvalue and eigenvectors of the problem

$$\{N \land Y\} = \{0\}$$
 (III-13)

<sup>&</sup>lt;sup>1</sup>Frazer, Duncan, and Collar, Elementary Matrices, Cambridge.

The methods to obtain these estimates will be considered in Section 3.2 of this appendix. Let us denote the estimate of the eigenvalue by  $\lambda_i$  and the estimate of the eigenvector by  $\{\varphi\}_i$ . Then, if  $\lambda$  is an eigenvalue of III-13, let

$$\lambda = \lambda_i + \Delta \tag{III-14}$$

where  $\Delta$  is a correction which is to be made zero.

Expand N( $\lambda$ ) in a Taylor's series about the point  $\lambda = \lambda_i$ 

$$[N(\lambda)] = [N(\lambda_{i})] + \left[\frac{dN}{d\lambda}(\lambda_{i})\right](\lambda - \lambda_{i}) + \frac{1}{2!}\left[\frac{d^{2}N}{d\lambda^{2}}(\lambda_{i})\right](\lambda - \lambda_{i})^{2}$$
(III-15)  
+ ...

or

$$[N(\lambda_{i}+\Delta)] = [N(\lambda_{i})] + [\frac{dN}{d\lambda}(\lambda_{i})] \Delta + \dots$$
 (III-16)

te	erms	of
order	of	$\nabla_{\Sigma}$

Iet

.

$$\{\varphi\} = \{\varphi\}_{i} + \begin{bmatrix} 0\\ \{\Delta\} \end{bmatrix}$$
 (III-17)  
(First element is uncorrected)

•

where  $\{\phi\}$  is an eigenvector of III-13 Then

$$[N(\lambda)]{\{\varphi\}} = \left( [N(\lambda_{i})] + [\frac{dN}{d\lambda}(\lambda_{i})] \Delta \right) \left( \{\varphi\}_{1} + \begin{bmatrix} 0\\ [\Delta] \end{bmatrix} \right)$$

$$= [N(\lambda_{i})]{\{\varphi\}}_{i}$$

$$+ \left( \frac{dN}{d\lambda}(\lambda_{i}) \right) [\varphi]_{i} \Delta$$

$$+ [N(\lambda_{i})] \begin{bmatrix} 0\\ [\Delta] \end{bmatrix}$$

$$+ \left[ \frac{dN}{d\lambda}(\lambda_{i}) \right] \begin{bmatrix} 0\\ [\Delta] \end{bmatrix} \Delta$$

This term is second order

$$\left[N(\lambda_{i}) \right] \left\{\varphi_{\lambda}^{i} + \left[\frac{d\lambda}{d\lambda}(\lambda_{i})\right] \left\{\varphi_{\lambda}^{i} \Delta + \left[N(\lambda_{i})\right]\right] \left[\begin{array}{c} 0\\ \left\{\Delta\right\}\end{array}\right] = \left\{0\right\}$$
(III-19)

$$[\mathsf{T}] = \begin{bmatrix} \{\mathsf{o}\}'\\ \mathsf{r}_{\mathsf{i}}\end{bmatrix}$$
(III-20)

then we can write

$$[N(\lambda_{i})]\{\varphi\}_{i} + [\frac{dN}{d\lambda}(\lambda_{i})]\{\varphi\}_{i} \Delta + [N(\lambda_{i})][\top]\{\Delta\} = \{0\}$$
(III-21)

These equations may be solved for the corrections

$$\begin{bmatrix} \begin{bmatrix} d_{11} \\ d\lambda \end{pmatrix} \langle \lambda_i \rangle f \phi_{i} & [N(\lambda_i)][\tau] \end{bmatrix} \begin{bmatrix} \Delta \\ \{\Delta\} \end{bmatrix} = - [N(\lambda_i)] f \phi_{i} & (\text{III-22}) \end{bmatrix}$$

$$\begin{bmatrix} \Delta \\ \{\Delta\} \end{bmatrix} = \left[ \left[ \frac{dN}{d\lambda} (\lambda_{i}) \right] \left\{ \varphi \right\}_{i}, \left[ N(\lambda_{i}) \right] \left[ \top \right] \right]^{-1} \left[ N(\lambda_{i}) \right] \left\{ \varphi \right\}_{i}$$
 (III-23)

Then compute

$$\{\varphi\}_{i} - \{\varphi\}_{i} + \{\tau\} \Delta\}$$
(III-24)

$$\lambda_{i} \rightarrow \lambda_{i} + \Delta \qquad (III-25)$$

and iterate the procedure. For most problems the process converges satisfactorily in five to ten iterations. The process is repeated for the problem

$$[N(\lambda)]' \{ \eta \} = \{ o \}$$
 (III-26)

starting with a "trial"  $\lambda_1$  which is the converged value for the iteration for  $\{\phi\}_i$ . The solutions can be normalized by the condition

$$\{\eta\}_i'\{\varphi\}_i = 1$$
 (III-27)

The above process fails for a repeated root.

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#### 3.2 Methods to Obtain Estimates for Use in Wielandt's Procedure

The common method for obtaining the eigenvalues from the solution of a polynominal is available when the problem is in the special form

$$[N(\lambda)] = \lambda [1] - [N]$$
 (III-28)

If

Most problems can be reduced to this "canonical" form by increasing the number of equations. For example, the third case (Equation III-4) can be reduced by taking [N] as

$$\begin{bmatrix} N \end{bmatrix} = \begin{bmatrix} -\frac{\partial_{\mathbf{w}} \nabla_{\mathbf{w}}}{2} \begin{bmatrix} M \end{bmatrix}^{\mathbf{i}} \begin{bmatrix} C_{\mathbf{I}} \end{bmatrix} & -\begin{bmatrix} M \end{bmatrix}^{\mathbf{i}} \begin{pmatrix} [\mathbf{F}] + \frac{\partial_{\mathbf{w}} \nabla_{\mathbf{w}}}{2} \end{bmatrix} \begin{bmatrix} C_{\mathbf{R}} \end{bmatrix} \end{pmatrix}$$
(III-29)

In the canonical form, the determinant

$$\Delta(\lambda) = |\lambda|^{r_{1}} - [N]$$
 (III-30)

can be expanded into a polynominal by the method of Danielewski, and the resulting polynominal solved by Newton's method for the  $\lambda_i$ ,  $i = 1, 2, \dots N$ .

Approximate eigenvectors may be obtained, when the eigenvalues are known, by the following procedure

then

$$\{\varphi_{i}\}_{i} = -[N_{22}(\lambda_{i})]^{-1}\{N_{2i}(\lambda_{i})\}$$
(III-32)

and

$$\{\varphi\}_{i} = \begin{bmatrix} 1 \\ -[N_{22}(\lambda_{i})]^{-1} \{N_{\lambda_{i}}(\lambda_{i})\} \end{bmatrix}$$
 (III-33)

This procedure is not generally very accurate<sup>1</sup>, but it suffices to obtain estimates for Weilandt's method.

<sup>1</sup>This is true because of Rayleigh's theorem which says that eigenvalues are insensitive to errors in the eigenvectors they are calculated from. Conversely, the eigenvectors are poorly determined from the eigenvalues. A routine incorporating Danielewski expansion and Wielandt's method has been used with considerable success at LTV Astronautics. The choice of this combination over other possible methods was largely based on a survey article by Paul A. White<sup>1</sup> and suggestions by Mario Rheinfurth of the NASA Marshall Space Flight Center<sup>2</sup>.

<sup>&</sup>lt;sup>1</sup>See White, Paul A. The Computation of Eigenvalues and Eigen Vectors of a Matrix Journal of the Society of Industrial and Applied Mathematics, Vol. 6, No. 4, December, 1958.

<sup>&</sup>lt;sup>2</sup>Rheinfurth, Mario <u>Control-Feedback Stability Analysis</u> ABMA Report No. DA-TR-2-60 January 1960.

## APPENDIX IV

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INTERPOLATION AND INTEGRATION COEFFICIENTS FOR DIPARABOLIC INTERPOLATION

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TWO-DIMENSIONAL DIPARABOLIC	INTERPOLATION COEFFICIENTS FO	A RECION ON THE UPPER EDGE
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13 14 15 1		0 	*0		0 0 0 0 0 0 0 0 0 0 0 0 0 0	5*2000000E-01 0*	. 50000005-01 -7.5000000E-01 0.			0. 					5.00000005-01 -J.0000005-01 -J.0000005-01 2.500	-2*2000006+01 **2000006-AT -********
12		-0				••		••	-1.5000006 00	,	•0	1.50000006 00	••	•0	1.00000005	-1.0000000E 00
11		•				1.5000006 00		4.5000000£ 00	-3.000000E 00		-3.0000000E 00	1.500000E 00	•	-1.00000006 00	5.0000006-01	
5		1.500000E 00				••		-4-500000E 00	1.50000016 00		3.0000000£ 00	-1.5000003 00	-5.0000006-01	۰.	5.00000005-01	
•		••				-1.50000005 00		.0	3.00000006 00		•	-1,5000006 00	•	1.00000006 00	-2.0000000E 00	
-	•	-7.49999996-02	1,50000005-01	•0		••	-5.0000005-01	6.74999965-91	-9,99999136-02	5.0000006-01	-4.49999986-01	-3-50000046-01	-7.49999996-02		-7.5000043E-02	
۲	•	3.0000006-01	-5.99999996-01	5.0000000E-01		-1-2500002 00	2.0000000€ 00	-5.7749999E DD	6.549997E 00	-1.500000E 00	3.84999995 00	-3.9499997E 00	3-0000006-01	7.5000006-01	-2.17499996 00	
•	1.0000000 00	-1.62500006 00	7.4999997E-01	••		•0	-2.5000006 00	5.77493996 00	-5.299999BE 00	1.5000000E 00	-3.850000E 00	3.9439999E JD	12-36666642*1-	•	2.024999E 00	
~		1.50000006-01	-3.0000006-01	-5.00000065-01		1.2500006 00	1.0000000€ 00	-6.7499997E-01	-1.15000016 00	-5.00000006-01	4.49999996-01	3.500002E-01	1.5000000E-01	-7.500000K-01	\$*2500002E-01	

																	15			.5000006-01	- 5000000 - 01		•	10-3000000						
																	15	o.	2-5000006-01 0	.00000006 00 2	- 5000005-01 -2			2. 50000005-01	1-00000006 00 -0	1				
	,	-	•	•	0	-	-5.000000E-01	1.250000E 00	5.00000000-01	-1.2500000E 00	•	••	-7.500000E-01	7.500000E-01		COLUMN	14	5.0000000E-01 C		- 00 3000000		1*20000000-01	5.0000005-01	•	1.250000E 00	7.5000001E-01 -				
		T	-0	••	5.0000006-01	-1.250000E 00	2.0000000E 00	-\$.0000000E 00	-1.5000006 00	3.750000E 00	•	7.500000E-01	3.000000E 00	-2.250000E 00			2	ï	. *000001-01		* 000000 - 01	2.5000000E-01 -	••	2.5000006-01	5.00000006-01 -	2.5000000E-01				
	COL UNIT	•	1.00000001 00	-2.500000E 00	••	••	-2.5000000€ 00	6.2500000E 30	1.5000006 00	-3.750000E 00	1.500000E 00	••	-3.7500000E 00	2.250000E 00		NOM				•	τ σ	12	<b>1</b>	1 1	51	16 -				
NOIS		'n	••	••	-5.0000006-01	1.250000E 00	1.0000000E 00	-2.5000000E 00	-5.0000006-01	1.25000006 00	••	-7.5000006-01	1.5000000E 00	-7.5000000E-01			51	:					5000006-01	.00000000 00	.5000300E-01	• 0000000E 00			.\$0000006-01	.500000E-01
AN INTERIOR REC	Ŋ		1	¢	•	a	1	•	10	12	ต	11	15	16			:	:		•	.5000000E-01 3.	.00000000 00 0.	.00000000 00 -2.	.00000000 00 -1.	.50000006-01 2.	.0000000E 00 1.	•	*\$000000E-01 0	1.000000£ 00 7	2.250000E 00 -7
			••	••	•	••	2.5000001E-01	-5,000000E-01	-2.5000006-01	5.0000036-01	•	ů.	2.5000005-01	-2.500000E-01		COLUMN	:	10	.0000000E-01 0.	.0000000E 00 0.	~ .	-	.25000006 00 1	+ 00 3C00000.	- 10-300000.	.0000000£ 00 -3			.750000E 00 -3	t.z500000E 00 2
		r	••	.0	-Z.500000E-01	5.00000005-01	-1.0000000 00	2-000000E 00	7.5000006-01	-1.500000€ 00	.0	-2.5000006-01	-1.0000006 00	7.5000006-01				6	•	~	.5000006-01 0	.0000000E 00 0		.00000006 00 -5	5000006-01 7	.00000006 00 3	ī	.50000006-01	. 50000006 00	- 10-3000001-01
	COLUMN	2	-5.000000E-01	1.000000E 00		••	1.25000016 00	-2.5000006 00	-7.5000006-01	1.5000005 00	-5.00000016-01		1.25000006 00	-7.5000006-01		700			2 2	0 1	ې ۳	1			· ·		13 0		15 -1	1.4
		-1	.0	••	2.5000006~01	-5.00000006-01	-5.0000006-01	1.0000006 00	2.5000006-01	-5.00000005-01	••	2.500000E-01	-5.0000000E+01	2 *000000F=01																
	ROM		2						=	12	9	1	1	2	2															

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TWO-DIMENSIONAL DIPARABOLIC Interpolation coefficients for Am intertor section

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ROM						C D T MINH						
	-	~	m	•	~		۲	<b>90</b>	*	20	11	12
-					•	1.0000006 00	•	4				c
2	<b>.</b> .	-5.000000E-01	••	<b>.</b>					•	-annonne-e		:
÷		1.499999996-01	.0,	۰.	•0	-1.500000E 00	••		1.5000006-01	3.750001E-01	3.0000005-01	~7.\$00000E-02
					-5.0000006-01		5.0000006-01	•				
ł			-2 \$00000F-01	•0					-2.500000E-01	•	2.5000000E-01	۰.
<b>n</b> .	2.5000000E-01	•	4.9999995-01	0.	1.500000E DD	•	-1.5000006 00		-9,9999995-01	. •0	1.0000006 00	••
•		5			1.0000000€ 00	~2.5000000E 00	2.0000000E 00	-5.000000E-01				
-		00 30000000 .	-1-000000F 00	2.5000005-01					5.00000005-01	-1.2500006 00	1.00000006 00	-2.500000E-01
8	-5.00000006-01	1.25000016 00 -1 76000016 00	1.25000006 00	-4.999998E-01	-3.00000006 00	2.9999995E 0D	-1.4999996 00	1.5000005 00	1.3250000E 00	7.7500029£-01	-1.7750002E D0	-3.25000006-01
c	ng 30000001	1000001-11-			-5.0000006-01	1.5000005 00	-1.5000000E 0D	5.200000E-01				
61									-2.5000000E-01	7.500000E-01	-7.500000E-01	2.5000000E-01
11	2.5500050E-C1	-1.5020205-01	7.5000C00F-01	-2.5000003E-01						10-3000000	()-36100009 a	5.499999F-D1
12	-5.99000006-01	10-306666666	-9.9999966-01	10-36666666	1.5000006 00	-1.49999976 00	1.49999996 00	-1.5000000E 00	->*>0000005-01	10-30200006-0-		
13	0.	-2.49999996-01	•	<b>.</b> .	<b>.</b> .	4.9999998E-01	۰.	•	-1.5000006-01	1.24999996-01	-3.0000000E-01	
71	7.50000356-01	••	-2.5000006-01	•0	-9.9999998E-01	<b>.</b>	1.00000006 00	••	7.49999996-01	••	-7.5000000E-U1	•
; :	-5.0000006-01	5.0000026-01	-2.49999965-01	2.4999996-01	2.000000€ 00	-4.9999956E-01	-5-0000006-01	-1.0000000€ 00	-8.2500004E-01	-2.0250003£ 00	2.7750002E DO	7.50003046-02
91	2.5000796-51	-2.5000016-01	10-3899996-01	-2.4999995-01	-1.00000005 00	-2.9802322E-07	5.9604645E-08	1.00000005 00	3.000003E-01	1.6000002E 00	-1.600001E 00	-2.99999995-01

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TWO-DIMENSIONAL DIPARABOLIC Interdiation coefficients for a recion om the lower ede

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#### TABLE 35 TWO-DIMENSIONAL DIPARABOLIC INTEGRATION MATRICES

#### TWO-DIMENSIONAL DIPARABOLIC INTEGRATION COEFFICIENTS FOR USE IN PLATE AND SHELL ANALYSES

# [Г]

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### COLUMN

	1	2	3	.4
1	1.0000000E 00	5.0000000E-01	3.3333333E-01	5.000000E-01
2	5.0000000E-01	3.3333333E-01	2.500000E-01	2.5000000E-01
3	3.3333333E-01	2.5000000E-01	2.0000000E-01	1.6666667E-01
4	5.000C000E-01	2.5000000E-01	1.6666667E-01	3.3333333E-01
5	2.5000000E-01	1.6666667E-01	1.2500000E-01	1.6666667E-01
6	1.6666667E-01	1.2500000E-01	9.99999998-02	l.1111111E-01
7	3.3333333E-01	1,6666667E-01	1.1111111E-01	2.500000E-01
8	1.6666667E-01	1.1111111E-01	8.3333333E-02	1.2500000E-01
9	1.1111111E-01	8.3333333E-02	6.666666E-02	8.3333333E-02
10	2.5000000E-01	1.2500000E-01	8.3333333E-02	2.000000E-01
11	1.2500000E-01	8.3333333E-02	6.2500000E-02	9.99999999E-02
12	8.3333333E-02	6.2500C00E-02	4 <b>.</b> 99999998-02	6.6666666E-02
13	2.500000E-01	2.000000E-01	1.6666667E-01	1.2500000E-01
14	1.2500000E-01	9.9999999E-02	8.3333333E-02	8.3333333E-02
15	8.3333333E-02	6.666666E-02	5.555555E-02	6.2500000E-02.
16	6.2500000E-02	4.9999999E-02	4.16666666-02	4.99999999E-02

	5	6	7	8
	2 5000005-01	1.66666675-01	3.3333333E-01	1.6666667E-01
1	2.500000000000	1.000000000000	1.6666667E-01	1.1111111E-01
2	1.6666667E-01	1.25000002 01		8.3333333E-02
3	1.2500000E-01	9.9999999E-02	1.1111111111111111111111111111111111111	1 250000E-01
4	1.6666667E-01	1.1111111E-01	2.5000000E-01	1.25000002 01
5	1.1111111E-01	8.3333333E-02	1.2500000E-01	8.3333333E-02
,	9 2233333F-02	6.6666666E-02	8.3333333E-02	6.2500000E-02
0	0.JJJJJJJJJ	8-3333333E-02	2.0000000E-01	9.9999999E-02
7	1.25000002-01	( <u>25000005-0</u> 2	9,9999999E-02	6.6666666E-02
8	8.3333333E-02	6.25000002-02		4-9999999E-02
9	6.2500000E-02	4.9999999E-02	6.66665662-02	0.22222325-02
10	9.9999999E-02	6.6666666E-02	1.6666667E-01	8.33333352-02
11	6-6666666E-02	4.99999999E-02	8.3333333E-02	5.5555555E-02
	4 0000000F-02	3.9999999E-02	5.5555555E-02	4.1666666E-02
12	4.99999992 02	• 22233333F-02	8.3333333E-02	6.6666666E-02
13	9.999999995-02	0.33333355 02	6 2500000E-02	4.99999999E-02
14	6.6666666E-02	5.5555555E-02	8.25000002 02	2 0000000F-02
15	4.9999999E-02	4.1666666E-02	4,999999945-02	J. 77777777 02
16	3.9999999E-02	3.3333333E-02	4.1666666E-02	3.3333333E-02

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COLUMN

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COLUMN

	9	10	11	12
1	1.1111111E-01	2.5000000E-01	1.2500000E-01	8.3333333E-02
• •	8.333333F-02	1.2500000E-01	8.3333333E-02	6.2500000E-02
2	6 6666666E=02	8.3333333E-02	6.2500000E-02	4.99999999E-02
د ر	0 22223333E-02	2.0000000E-01	9.9999999E-02	6.6666666E-02
4	( )500000E=02	9,9999999E-02	6.666666E-02	4.99999999E-02
5	6.250000000000	6-666666E-02	4.99999999E-02	3.99999999E-02
6	4.99999999	1 6666667E-01	8.3333333E-02	5.5555555E-02
7	6.66666666	e 22222333E-02	5.5555555E-02	4.1666666E-02
8	4.99999999E-02	6.55555555 c.55555555-02	4-1666666E-02	3.3333333E-02
9	3.9999999E-02	5.5355555 02	7.14285716-02	4.7619047E-02
10	5.5555555E-02	1.42857142-01	4 7619047E-02	3.5714285E-02
11	4.1666666E-02	7.1428571E-02	4.78190472 02	2.8571428E-02
12	3.3333333E-02	4.7619047E-02	3.51142896-02	4.1666666E=02
13	5.55555556-02	6.2500000E-02	4.99999999=02	
14	4.1666666E-02	4.99999999E-02	3.99999992-02	2. 777778E-02
15	3.3333333E-02	4.1666666E-02	3.3333333E-02	2.20005245-02
16	2.777778E-02	3.5714285E-02	2.8571428E-02	2.30093240-02

ROW

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## COLUMN

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	13	14	15	16
1	2.5000000E-01	1.2500000E-01	8.3333333E-02	6.2500000E-02
2	2.0000000E-01	9.9999999E-02	6.666666E-02	4.99999999E-02
3	1.6666667E-01	8.33333338-02	5.5555555E-02	4.1666666E-02
4	1.2500000E-01	8.3333333E-02	6.2500000E-02	4.99999998-02
5	9.9999999E-02	6.6666666E-02	4.99999999E-02	3.99999999E-02
6	8.3333333E-02	5.5555555E-02	4.1666666E-02	3•3333333E-02
7	8.3333333E-02	6.2500000E-02	4.99999999E-02	4.1666666E-02
8	6.6666666E-02	4.9999999E-02	3.9999999E-02	3.3333333E-02
9	5.5555555E-02	4.1666666E-02	3.3333333E-02	2.7777778E-02
10	6.2500000E-02	4.9999999E-02	4.1666666E-02	3.5714285E-02
11	4.9999999E-02	3.9999999E-02	3.3333333E-02	2.8571428E-02
12	4.1666666E-02	3.3333333E-02	2.7777778E-02	2 <b>.3809524E-0</b> 2
13	1.4285714E-01	7.1428571E-02	4.7619047E-02	3.5714285E-02
14	7.1428571E-02	4.7619047E-02	3.5714285E-02	2.8571428E-02
15	4.7619047E-02	3.5714285E-02	2.8571428E-02	2.3809524E-02
16	3.5714285E-02	2.8571428E-02	2.3809524E-02	2.0408163E-02

## TWO-DIMENSIONAL DIPARABOLIC INTEGRATION COEFFICIENTS FOR USE IN PLATE AND SHELL ANALYSES

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## COLUMN

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	1	í	2 3
3	0.	0.	4.000000E 00 0.
6	0.	0.	2.0000000E 00 0.
9	0.	0.	1.3333333E 00 0.
12	0.	0.	1.0000000E 00 0.
13	0.	0.	6.000000E 00 0.
14	0.	0.	3.0000000E 00 0.
15	0.	0.	2.0000000E 00 0.
16	0.	0.	1.5000000E 00 0.

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ROW

COLUMN

	5	6	7	8
3	0.	2.0000000E 00	0.	0.
6	0.	1.3333333E 00	·0•	0.
9	0.	1.000000E 00	0.	0.
12	0.	7.9999999E-01	0.	0.
13	G.	3.0000000E 00	0.	0.
14	0.	2.0000000E 00	0.	0.
15	0.	1.5000000E 00	0.	0.
16	0.	1.200000E 00	0.	0.

COLUMN	
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	_	1	0 11	12
	9	1	<u> </u>	1.0000000E 00
3	1.3333333E 00	0.	0.	7 0000099F-01
6	1.000000E 00	0.	0.	(.9999977)2 02
U	0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 -	0.	0.	6.6666666E-01
9	7.999999992-01		0 -	5.7142857E-01
12	6.6668666E-01	0.	0.	1 5000000E 00
	2.0000000E 00	0.	0.	1.50000000
	1 500000E 00	0.	0.	1.20000000 00
14	1.50000002 00	0	0.	9.9999999E-01
15	1.2000000E 00	0.		8-5714284E-01
16	9.9999999E-01	0.	0.	<b>~ - - -</b>

COLUMN

		14	15	16
	13		2 000000E 00	1.5000000E 00
3	6.0000000E 00	3.0000000E 00	2.00000000	1 2000000E 00
6	3.0000000E 00	2.0000000E 00	1.5000000E 00	1.20000000
J	2 000000E 00	1.5000000E 00	1.2000000E 00	9.9999999E-01
9	2.00000002 00	1 200000E 00	9.9999999E-01	8.5714284E-01
12	1.5000000E 00	1.20000002 00	4 000000F 00	3.0000000E 00
13	1.2000000E 01	5.9999999E 00	4.0000002 00	2 400000E 00
14	5,9999999E 00	4.0000000E 00	3.0000000E 00	2.40000002 00
14		3.0000000E 00	2.4000000E 00	2.0000000E 00
15	4.0000002 00		2.0000000E 00	1.7142857E 00
16	3.0000000E 00	2.4000000000000000000000000000000000000		

ROW

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#### COLUMN

	5	6	7	8
7	0.	0.	4.0000000E 00	2.0000000E 00
8	0.	0.	2.0000000E 00	1.3333333E 00
9	0.	0.	1.3333333E 00	1.0000000E 00
10	0.	0.	6.0000000E 00	3.0000000E 00
11	0.	0.	3.0000000E 00	2.0000000E 00
12	0.	0.	2.0000000E 00	1.5000000E 00
15	0.	0.	1.0000000E 00	7.99999999E-01
16	0.	0.	1.5000000E 00	1.2000000E 00

#### COLUMN

	9	10	11	12
7	1.3333333E 00	6.0000000E 00	3.0000000E 00	2.0000000E 00
8	1.0000000E 00	3.0000000E 00	2.0000000E 00	1.5000000E 00
9	7.9999999E-01	2.0000000E 00	1.500C000E 00	1.2000000E 00
10	2.0000000E 00	1:2000000E 01	5.9999999E 00	4.0000000E 00
11	1.5000000E 00	5.9999999E 00	4.0000000E 00	3.0000000E 00
12	1.2000000E 00	4.0000000E 00	3.0000000E 00	2.4000000E 00
15	6.6666666E-01	1,5000000E 00	1.2000000E 00	9.99999999E-01
16	9.99999998-01	3.0000000E 00	2.40000000 00	2.0000000E 00

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ROW		CULVAN		
	13	14	15	16
-	0	0.	1.0000000E 00	1.5000000E 00
(	0.	9.	7.9999999E-01	1.2000000E 00
8	0.	0.	6.666666E-01	9.99999999E-01
9	0.	0.	1.5000000E 00	3.0000000E 00
10	0.	0.	1.2000000E 00	2.4000000E 00
11	0.	0	9.9999999E-01	2.0000000E 00
12	0.	0	5.7142857E-01	8.5714284E-01
15	0.	0.	8.5714284E-01	1.7142857E 00
16	0.	0.		

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# TWO-DIMENSIONAL DIPARABOLIC INTEGRATION COEFFICIENTS FOR USE IN PLATE AND SHELL ANALYSES $\left[ \Gamma_{\mathbf{q}} \right]$

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#### COLUMN

	1	2	3	4
7	0.	0.	2.0000000E 00	0.
8	0.	0.	1.0000000E 00	0.
9	0.	0.	6.666666E-01	0.
10	0.	0.	3.0000000E 00	0.
11	0.	0.	1.5000000E 00	0.
12	0.	0.	9.9999999E-01	0.
15	0.	0.	5.000000E-01	0.
16	0.	0.	7.5000000E-01	0.

C	n	1	11	MA	1
U.	U	L	v	r.u.	

	5	6	7	8
3	0.	0.	2.0000000E 00	1.0000000E 00
6	0.	0, •	1.0000000E 00	5.000000E-01
7	0.	1.0000000E 00	0.	0.
8	0.	5.000000E-01	0.	0.
9	0.	3.3333333E-01	6.6666666E-01	3.3333333E-01
10	0.	2.0000000E 00	0.	0.
11	0.	9.9999999E-01	0.	0.
12	0.	6.6666666E-01	5.000000E-01	2.5000000E-01
13	0.	0.	3.0000000E 00	2.0000000E 00
14	0.	0.	1.5000000E 00	9.99999998-01
15	0.	2.500000E-01	9.9999999E-01	6.6666666E-01
16	0.	4.9999999E-01	7.5000000E-01	4.99999999E-01

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9	10	11	12
6 6666666F-01	3.0000000E 00	1.5000000E 00	9.99999999E-01
2 22222333E-01	2.0000000E 00	9.9999999E-01	6.6666666E-01
5.5555555 01	0.	0.	5.000000E-01
0.0000000000000000000000000000000000000	0.	0.	2.5000000E-01
3.33333350-01	1 E00000E 00	7.5000000E-01	6.6666666E-01
4.444444E-01	1.3000000000000	0.	1.2000000E 00
1.5000000E 00		0.	5.9999999E-01
7.5000000E-01	0.	5 999999F-01	7.99999998-01
6.6666666E-01	1.2000000000000000000000000000000000000	3.000000E 00	2.2500000E 00
1.5000000E 00	4.5000000E 00	3.00000000 00	1.5000000E 00
7.500000E-01	3.0000000E 00	2.00000002 00	1 2500000E 00
6.6666666E-01	2.2500000E 00	1.500000E 00	1.2000000 000
7.5000000E-01	1.8000000E 00 Column	1.2000000E 00	1.20000001 00
	9 6.66666666E-01 3.3333333E-01 6.66666666E-01 3.33333333E-01 4.4444444E-01 1.5000000E 00 7.5000000E-01 6.66666666E-01 1.5000000E-01 6.66666666E-01 7.5000000E-01	9106.66666666E-013.0000000E 003.333333E-012.000000E 006.6666666E-010.3.333333E-010.4.444444E-011.5000000E 001.5000000E 000.7.5000000E-010.6.6666666E-011.2000000E 001.5000000E-013.000000E 007.5000000E-013.000000E 006.6666666E-012.2500000E 007.5000000E-011.8000000E 007.5000000E-011.000000E 00	910116.666666667013.0000000E 001.5000000E 003.333333E-012.0000000E 009.9999999E-016.666666667010.0.3.333333E-010.0.4.44444447011.5000000E 007.500000E-011.5000000E 000.0.7.5000000E-010.0.6.66666666E-011.200000E 005.999999E-011.5000000E-013.000000E 002.000000E 007.500000E-013.000000E 001.5000000E 007.500000E-013.000000E 001.2000000E 007.500000E-011.8000000E 001.2000000E 007.500000E-011.8000000E 001.2000000E 00

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	13	14	15	16
~	0	0.	5.0000000E-01	7.5000000E-01
3	0.	0	2.5000000E-01	4.99999999E-01
6	0.	·	9,9999999E-01	7.5000000E-01
7	3.0000000E 00	1.30000000 00	6 6666666F-01	4.99999999E-01
8	2.0000000E 00	9.9999999E-01	6.00000002 01	7.500000E-01
9	1.5000000E 00	7.5000000E-01	6.6666666E=01	1 800000E 00
10	4.5000000E 00	3.0000000E 00	2.2500000E 00	1.0000002 00
11	3.0000000E 00	2.0000000E 00	1.5000000E 00	1.20000002 00
12	2.2500000E 00	1.5000000E 00	1.2500000E 00	1.2000000E 00
12	0	0.	1.2000000E 00	1.8000000E 00
15	0	0.	5.9999999E-01	1.2000000E 00
14	0. 	5,9999999E-01	7.9999999E-01	1.2000000E 00
15	1.20000002 00	1 2000000E 00	1.2000000E 00	1.4400000E 00
16	1.80000000 00	1.20000000 00		

 $[r_{5}]$ COLUMN

ROW

	5	6	7	8
5	1.0000000E 00	1.0000000E 00	0.	1.0000000E 00
6	1.0000000E 00	1.3333333E 00	0.	1.000000E 00
. 8	1.0000000E 00	1.0000000Ė 00	0.	1.3333333E 00
9	1.0000000E 00	1.3333333E 00	0.	1.3333333E 00
11	9.9999999E-01	9.9999999E-01	0.	1.500000E 00
12	9.99999999E-01	1.3333333E 00	0.	1.500000E 00
14	9.99999999E-01	1.5000000E 00	0.	9.9999999E-01
15	9.9999999E-01	1.5000000E 00	0.	1.3333333E 00
16	9.99999998-01	1.5000000E 00	0.	1.5000000E 00

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ROW.

	9	10	11	12
5	1.0000000E 00	0.	9.99999998-01	9.9999999E-01
6	1.3333333E 00	0.	9.9999999E-01	1.33333333E 00
'8	1.3333333E 00	0.	1.5000000E 00	1.5000000E 00
9	1.777778E 00	0.	1.5000000E 00	2.0000000E 00
11	1.5000000E 00	0.	1.8000000E 00	1.8000000E 00
12	2.0000000E 00	0.	1.8000000E 00	2.4000000E 00
14	1.5000000E 00	0.	9.9999999E-01	1.5000000E 00
15	2.0000000E 00	0.	1.5000000E 00	2.2500000E 00
16	2.2500000E 00	0.	1,8000000E 00	2.7000000E 00

	13	14	15	16
F	0	9.9999999E-01	9.9999999E-01	9.99999999E-01
2	0.	1-5000000E 00	1.5000000E 00	1.5000000E 00
6	0.	9,9999999E-01	1.3333333E 00	1.5000000E 00
8	0.	1 500000E 00	2.0000000E 00	2.2500000E 00
. 9	0.	0.0000000E-01	1.5000000E 00	1.8000000E 00
11	0.	9.99999992 01	2.2500000E 00	2.7000000E 00
12	0•	1.50000000 00	2.2900000E 00	1.8000000E 00
14	0 •	1.800000E 00	1.80000002 00	2 700000E 00
1.5	0.	1.8000000E 00	2.4000000000000000000000000000000000000	2.1000000E 01
16	0.	1.8000000E 00	2.7000000E 00	3.2344442 00

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ROW

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		1	2	3	
1	0.		1.0000000E 00	1.0000000E 00	-·O.
2	0.		5.000000E-01	6.6666666E-01	-0.
3	0.		3.3333333E-01	5.000000E-01	-0.
4	0.		5.000000E-01	5.000000E-01	-0.
5	0.		2.500000E-01	3.3333333E-01	-0.
6	0.		1.6666667E-01	2.500000E-01	-0.
7	0.		3.3333333E-01	3.3333333E-01	-0.
8	0.		1.6666667E-01	2.2222222E-01	-0.
9	0.		1.1111111E-01	1.6666667E-01	-0.
010	0.		2.5000000E-01	2.5000000E-01	-0.
011	0.		1.2500000E-01	1.6666667E-01	-0.
012	0.		8.3333333E-02	1.2500000E-01	-0.
013	0.		2.500000E-01	4.0000000E-01	-0.
014	0.		1.2500000E-01	2.0000000E-01	-0.
015	.0.		8.3333333E-02	1.3333333E-01	-0.
016	0.		6.2500000E-02	9.9999999E-02	-0.

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	5	6	7	8
1	5.0000000E-01	5.000000E-01	0.	3.3333333E-01
2	2.5000000E-01	3.3333333E-01	0.	1.6666667E-01
3	1.6666667E-01	2.5000000E-01	0.	1.1111111E-01
4	3.3333333E-01	3.3333333E-01	0.	2.5000000E-01
5	1.6666667E-01	2.2222222E-01	0.	1.2500000E-01
6	1.1111111E-01	1.6666667E-01	0.	8.3333333E-02
7	2.5000000E-01	2.5000000E-01	0.	2.0000000E-01
8	1.2500000E-01	1.6666667E-01	0.	9.99999999E-02
9	8.3333333E-02	1.2500000E-01	0.	6.6666666E-02
10	2.000000E-01	2.000000E-01	0.	1.6666667E-01
11	9.9999999E-02	1.3333333E-01	0.	8.3333333E-02
12	6.6666666E-02	9,99999999E-02	0.	5.5555555E-02
13	1.2500000E-01	2.0000000E-01	0.	8.3333333E-02
14	8.3333333E-02	1.3333333E-01	0.	6.2500000E-02
15	6.2500000E-02	9.99999999E-02	0.	4.99999998-02
16	4 <b>.9</b> 9999998-02	7.9999999E-02	0.	4.1666666E-02

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	9	10	11	12
1	3.3333333E-01	0.	2.500000E-01	2.5000000E-01
2	2.2222222E-01	· 0 •	1.2500000E-01	1.6666667E-01
3	1.6666667E-01	-0.	8.3333333E-02	1.2500000E-01
4	2.5000000E-01	0.	2.000000E-01	2.0000000E-01
5	1.6666667E-01	·0.	9.9999999E-02	1.3333333E-01
6	1.250C000E-01	0.	6.6666666E-02	9.99999998-02
71	2.0000000E-01	0.	1.6666667E-01	1.6666667E-01
8	1.3333333E-01	0.	8.3333333E-02	1.1111111E-01
9	9.9999999E-02	0.	5.55555555-02	8.3333333E-02
10	1.6666667E-01	0.	1.4285714E-01	1.42857148-01
11	1.11111116-01	0.	7.1428571E-02	9.5238094E-02
12	8.33333335-02	0.	4.7619047E-02	7.1428571E-02
13	1.3333333E-01	0.	6.2500000E-02	9.9999999E-02
14	9.99999998-02	0.	4.9999999E-02	7.9999999E-02
15	7.9999999E-02	0.	4.1666666E-02	6.6666666E-02
16	6.6666666E-02	0.	3.5714285E-02	5.7142857E-02

	13	14	15	16
	0 0000000F-01	4.9999999E-01	3.3333333E-01	2.500000E-01
1	9.9999999	3.7500000E-01	2.5000000E-01	1.8750000E-01
2	7.50000002-01	3 000000E=0]	2.0000000E-01	1.5000000E-01
3	5.9999999	2 22223333E=01	2.500000E-01	2.0000000E-01
4	4.99999999E-01		1-8750000E-01	1.5000000E-01
5	3.7500000E-01	2.5000000000000	1.500000E-01	1.2000000E-01
6	3.0000000E-01	2.000000000000		1-6666667E-01
7	3.3333333E-01	2.5000000E-01	2.0000000000000000000000000000000000000	1 2500000F-01
8	2.5000000E-01	1.8750000E-01	1.500000000000	0.000000E
9	2.0000000E-01	1.5000000E-01	1.200000000-01	9.99999997
10	2.5000000E-01	2.0000000E-01	1.6666667E-01	1.4285714E-01
11	1.8750000E-01	1.5000000E-01	1.2500000E-01	1.0714286E-01
12	1.5000000E-01	1.2000000E-01	9 <b>.</b> 99999998-02	8.5714284E-02
13	4.99999999E-01	2.5000000E-01	1.6666667E-01	1.2500000E-01
14	2.5000000E-01	1.6666667E-01	1.2500000E-01	9.9999999E-02
15	1.6666667E-01	1.2500000E-01	9 <b>.9</b> 9999999E-02	8.3333333E-02
16	1.2500000E-01	9.99999999E-02	8.3333333E-02	7.1428571E-02
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ROW

		2	3	4
2	0.	1.0000000E 00	1.0000000E 00	0.
3	0.	1.0000000E 00	1.3333333E 00	0.
5	0.	5.000000E-01	5.0000000E-01	0.
6	0.	5.000000E-01	6.6666666E-01	0.
8	0.	3.3333333E-01	3.3333333E-01	0.
9	0.	3.3333333E-01	4.444444E-01	0.
11	0.	2.500000E-01	2.5000000E-01	0.
12	0.	2.500000E-01	3.3333333E-01	0.
13	0.	1.0000000E 00	1.5000000E 00	0.
14	0.	5.000000E-01	7.5000000E-01	0.
15	0.	3.3333333E-01	5.0000000E-01	0.
16	0.	2.500000E-01	3.7500000E-01	0.
10	••			

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	5	. 6	7	8
2	5.0000000E-01	5.000000E-01	0.	3.3333333E-01
3	5.0000000E-01	6.6666666E-01	0.	3.3333333E-01
5	3.3333333E-01	3.3333333E-01	Ũ.	2.5000000E-01
6	3.3333333E-01	4.444444E-01	0.	2.5000000E-01
8	2.5000000E-01	2.5000000E-01	0.	2.0000000E-01
9	2.5000000E-01	3.3333333E-01	0.	2.000000E-01
11	2.0000000E-01	2.0000000E-01	0.	1.6666667E-01
12	2.0000000E-01	2.6666667E-01	0.	1.6666667E-01
13	5.000000E-01	7.5000000E-01	э.	3.3333333E-01
14	3.3333333E-01	5.000000E-01	0.	2.5000000E-01
15	2.5000000E-01	3.7500000E-01	0.	2.000000E-01
16	2.0000000E-01	3.0000000E-01	0.	1.6666667E-01

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	9	10	11	12
2	3,3333333E-01	0.	2.5000000E-01	2.5000000E-01
2	4-444444E-01	0.	2.5000000E-01	3.3333333E-01
5	2-5000000E-01	0.	2.000000E-01	2.0000000E-01
ر د	2.3333333F-01	0.	2.000000E-01	2.6666667E-01
0	2 000000E-01	0.	1.6666667E-01	1.6666667E-01
0	2.6666667E-01	0.	1.6666667E-01	2.2222222E-01
	2.00000072 01	0.	1.4285714E-01	1.4285714E-01
11	1.00000070 01	0.	1.4285714E-01	1.9047619E-01
12	2.22222220	0	2.5000000E-01	3.7500000E-01
13	5.00000002-01	0	2.000000E-01	3.0000000E-01
14	3.7500000E-01	0.	1.6666667E-01	2.50000008-01
15	3.000000E-01	U• ,	1 4285714F-01	2.1428571E-01
16	2.5000000E-01	0•	1.42031146 01	

	13	14	15	16
-	13	5.000000E-01	3.3333333E-01	2.5000000E-01
2	1.0000000000000000000000000000000000000	7 5000000E-01	5.0000000E-01	3.7500000E-01
3	1.50000002 00	2 22222333E-01	2.5000000E-01	2.0000000E-01
5	5.000000E-01	5.000000E-01	3.7500000E-01	3.0000000E-01
6	7.500000E-01	5.00000000000000	2-0000000E-01	1.6666667E-01
8	3.3333333E-01	2.50000002-01	3.0000000E-01	2.5000000E-01
9	5.000C000E-01	3.750000000-01	1. 6666667E-01	1.4285714E-01
11	2.5000000E-01	2.0000000000000		2.1428571E-01
12	3.7500000E-01	3.000000000000	2.3000000000000	4.49999998-01
13	1.8000000E 00	8.9999999E-01	5.99999992-01	3 600000E-01
14	8.9999999E-01	5.9999999E-01	4.49999998-01	3 000000E-01
15	5.9999999E-01	4.4999999E-01	3.600000000000	2 57142865-01
16	4.4999999E-01	3.600000E-01	3.0000000000000	2.JIL 42000 01

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		1	2	3	4
4	c	).	0.	0.	1.000000E 00
5	(	D.	0.	0.	5.000000E-01
6	(	0.	0.	0.	3.3333333E-01
7	. (	D <b>.</b>	0.	0.	1.0000000E 00
8	. (	0.	0.	0.	5.000000E-01
9	(	0.	0.	0.	3.3333333E-01
10	(	0.	0.	0.	1.0000000E 00
11	I	0.	0.	0.	5.000000E-01
12		0.	0.	0.	3.3333333E-01
14	i	0.	0.	0.	2.5000000E-01
15		0.	0.	0.	2.5000000E-01
16		0.	.0.	0.	2.5000000E-01

	5	6	7	8
4	5.000000E-01	3.3333333E-01	1.0000000E 00	5.000000E-01
5	3.3333333E-01	2.5000000E-01	5.000000E-01	3.3333333E-01
6	2.5000000E-01	2.0000000E-01	3.3333333E-01	2.5000000E-01
7	5.000000E-01	3.3333333E-01	1.3333333E 00	6.6666666E-01
8	3.3333333E-01	2.5000000E-01	6.6666666E-01	4.444444E-01
9	2.J000000E-01	2.000000E-01	4.444444E-01	3.3333333E-01
10	5.000000E-01	3.3333333E-01	1.5000000E 00	7.500000E-01
11	3.3333333E-01	2.500000E-01	7.500000E-01	5.000000E-01
12	2.5000000E-01	2.0000000E-01	5.000000E-01	3.7500000E-01
14	2.0000000E-01	1.6666667E-01	2.500000E-01	2.000000E-01
15	2.0000000E-01	1.6666667E-01	3.3333333E-01	2.6666667E-01
16	2.0000000E-01	1.6666667E-01	3.7500000E-01	3.000000E-01
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	٩	10	11	12
	2 2222333F-01	1.0000000E 00	5.0000000E-01	3.3333333E-01
4	3.53555555 01	5-000000E-01	3.3333333E-01	2.5000000E-01
5	2.30000002-01	a aaaaaaaF-01	2.5000000E-01	2.0000000E-01
6	2.0000000000000	1.500000E 00	7.5000000E-01	5.000000E-01
ד	4.444444E-01	7.50000002 00	5.000000E-01	3.7500000E-01
8	3.3333333E-01	7.3000000E=01	3.7500000E-01	3.0000000E-01
9	2.6666667E-01	5.000000000000	a aaaaaaaa=-01	5.9999999E-01
10	5.000000E-01	1.8000000000000000000000000000000000000	5.0000999E-01	4_4999999E-01
11	3.7500000E-01	8.9999999E-01	5.99999997	3.6000000E-01
12	3.0000000E-01	5.9999999E-01	4.49999992-01	1 6666667E-01
14	1.6666667E-01	2.5000000E-01	2.000000E-01	
15	2.2222222E-01	3.7500000E-01	3.0000000E-01	2.50000000000000
16	2.5000000E-01	4.49999999E-01	3.600000E-01	3.0000000E-01

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	13	14	15	16
4	0.	2.500000E-01.	2.500000E-01	2.500000E-01
5	0.	2.000000E-01	2.000000E-01	2.000000E-01
6	0.	1.6666667E-01	1.6666667E-01	1.6666667E-01
7	0.	2.500000E-01	3.3333333E-01	3.7500000E-01
8	0.	2.000000E-01	2.6666667E-01	3.0000000E-01
9	0.	1.6666667E-01	2.2222222E-01	2.5000000E-01
10	0.	2.500000E-01	3.7500000E-01	4.49999999E-01
11	0.	2.000000E-01	3.000000E-01	3.600000E-01
12	0.	1.6666667E-01	2.5000000E-01	3.000000E-01
14	0.	1.4285714E-01	1.4285714E-01	1.4285714E-01
15	0.	1.4285714E-01	1.9047619E-01	2.1428571E-01
16	0.	1.4285714E-01	2.1428571E-01	2.5714286E-01

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### [79] COLUMN

ROW

	1	2	3	4
2	0.	0.	0.	5.000000E-01
· 3	0.	0.	0.	5.000000E-01
4	0.	5.000000E-01	5.000000E-01	0.
5	0.	2.5000000E-01	3.3333333E-01	2.500000E-01
6	0.	1.6666666E-01	2.5000000E-01	2.5000000E-01
٦	0.	5.000000E-01	5.000000E-01	0.
8	0.	2.5000000E-01	3.3333333E-01	1.6666666E-01
9	0.	1.6666666E-01	2.5000000E-01	1.6666666E-01
10	0.	5.000000E-01	5.000000E-01	0.
11	0.	2.5000000E-01	3.3333333E-01	1.2500000E-01
12	0.	1.6666666E-01	2.5000000E-01	1.2500000E-01
13	0.	0.	0.	5.000000E-01
14	0.	1.2500000E-01	2.0000000E-01	2.500000E-01
15	0.	1.2500000E-01	2.000000E-01	1.6666666E-01
16	0.	1.2500000E-01	2.000000E-01	1.2500000E-01

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	5	6	7	8
2	2.5000000E-01	1.6666666E-01	5.0000000E-01	2.5000000E-01
3	3.3333333E-01	2.5000000E-01	5.000000E-01	3.3333333E-01
4	2.5000000E-01	2.500000E-01	0.	1.6666666E-01
·5	2.5000000E-01	2.5000000E-01	3.3333333E-01	2.500000E-01
6	2.5000000E-01	2.5000000E-01	3.3333333E-01	2.7777777E-01
7	3.3333333E-01	3.3333333E-01	0.	2.5000000E-01
8	2.5000000E-01	2.7777777E-01	2.5000000E-01	2.5000000E-01
9	2.222222E-01	2.5000000E-01	2.5000000E-01	2.500000E-01
10	3.7500000E-01	3.7500000E-01	0.	3.000000E-01
11	2.5000000E-01	2.9166666E-01	2.000000E-01	2.5000000E-01
12	2.0833333E-01	2.5000000E-01	2.000C000E-01	2.333333335-01
13	3.7500000E-01	3.0000000E-01	5.000000E-01	3.7500000E-01
14	2.5000000E-01	2.5000000E-01	3.3333333E-01	2.9166666E-01
15	2.0833333E-01	2.3333333E-01	2.5000000E-01	2.5000000E-01
16	1.8750000E-01	2.2500000E-01	2.0000000E-01	2.2500000E-01

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	9	10	11	12
2	1.6666666E-01	5.000000E-01	2.5000000E-01	1.6666666E-01
3	2.5000000E-01	5.0000000E-01	3.3333333E-01	2.500000E-01
4	1.6666666E-01	0.	1.2500000E-01	1.2500000E-01
5	2.22222228-01	3.7500000E-01	2.5000000E-01	2.0833333E-01
6	2.5000000E-01	3.7500000E-01	2.9166566E-01	2.5000000E-01
7	2.5000000E-01	0.	2.0000000E-01	2.000000E-01
8	2.5000000E-01	3.0000000E-01	2.5000000E-01	2.33333333E-01
9	2.5000000E-01	3.0000000E-01	2.6666666E-01	2.5000000E-01
0	3.0000000E-01	0.	2.5000000E-01	2.5000000E-01
11	2.6666666E-01	2.5000000E-01	2.5000000E-01	2.5000000E-01
12	2.5000000E-01	2.5000000E-01	2.5000000E-01	2.5000000E-01
13	3.0000000E-01	5.000000E-01	3.7500000E-01	3.0000000E-01
14	2.6666666E-01	3.7500000E-01	3.1250000E-01	2.7500000E-01
15	2.5000000E-01	3.0000000E-01	2.7500000E-01	2.6000000E-01
16	2.4000000E-01	2.5000000E-01	2.5000000E-01	2.5000000E-01

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COLUMN

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	13	14	15	16
2	0.	1.2500000E-01	1.2500000E-01	1.2500000E-01
2	9.	2.0000000E-01	2.0000000E-01	2.000000E-01
4	5,0000000E-01	2.5000000E-01	1.6666666E-01	1.2500000E-01
5	3.7500000E-01	2.5000000E-01	2.0833333E-01	1.8750000E-01
5	3-000000E-01	2.5000000E-01	2.3333333E-01	2.2500000E-01
7	5-000000E-01	3.3333333E-01	2.5000000E-01	2.0000000E-01
' 9	3.7500000E-01	2.9166666E-01	2.5000000E-01	2.2500000E-01
0	3.000000E-01	2.6666666E-01	2.5000000E-01	2.4000000E-01
7	5.0000000E-01	3.7500000E-01	3.0000000E-01	2.5000000E-01
10	3 7500000000000	3.1250000E-01	2.7500000E-01	2.5000000E-01
11	3.0000000000000	2.7500000E-01	2.600000E-01	2.5000000E-01
12	3.00000002 01	2,5000000E-01	2.5000000E-01	2.5000000E-01
13		2-500000E-01	2.5000000E-01	2.5000000E-01
14	2.500000000000	2 5000000E-01	2.500000E-01	2.5000000E-01
15	2.50000000-01	2.500000000000	2-5000000E-01	2.5000000E-01
16	2.500000E-01	2.500000E-01	2.900000000000000	



#### ROW

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COLUMN

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	5	6	7	8
		a aaaaaaaE-01	1.0000000E 00	5.000000E-01
1	5.000000000000	5.500000E=01	5,000000E-01	3.3333333E-01
2	3.3333333E-01	2.50000002-01	2 222222E-01	2.500000E-01
3	2.5000000E-01	2.0000000E-01	3.33333350	2 2222333F-01
4	2.5000000E-01	1.6666667E-01	6.6666666E-01	3.33333555 01
5	1.6666667E-01	1.2500000E-01	3.3333333E-Cl	2.22222222
	1 2500000E-01	9.9999999E-02	2.222/22/F-01	1.6666667E-01
0	1.2.3.0.4.4.4.7E=01	1.111111E-01	5.0000000E-01	2.5000000E-01
7	1.00000070 01	0 22233295-02	2.506.6001-01	1.66666671-01
8	1.11111111-01	0.33333272 02	1 66666676-01	1.25C0000E-01
9	8.3333329E-02	6.6666669E-02	[.00000011 01	2.0000001-01
10	1.2500000E-01	8.3333329E-02	4.000000E-01	2,000000000
11	8.3333329E-02	6.2500000E-02	2.000000E-01	1.33333352-01
12	6.2500000E-02	4.9999999E-02	1.333333332-61	9.999999995-02
	2.000000E=01	1.6666667E-01	2.50000008-01	2.0000000E-01
13	2.0000000000000000000000000000000000000	8.333329E-02	1.6666667E-01	1.3333333i-01
14	9.999999995-02		1-2500000E-01	9.939393946-02
15	6,6666669E-02	5.5555557740-02	0.000000000000	7.9999999E-02
16	4.9999999E-02	4.1666669E-02	A. AAAAAAAC_02	

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COLUMN

ROW

	9	10	11	12
1	3.3333333E-01	1.0000000E 00	5.0000008-01	3.3333333E-01
2	2.5000000E-01	5.000000E-01	3.3333333E-01	2.500000E-01
3	2.0000000E-01	3.3333333E-01	2.500000E-01	2.000000E-01
4	2.2222222E-01	7.5000000E-01	3.7500000E-01	2.5000000E-01
5	1.6666667E-01	3.7500000E-01	2.5000000E-01	1.8750000E-01
6	1.3333333E-01	2.5000000E-01	1.8750000E-01	1.500000E-01
7	1.6666667E-01	5.9999999E-01	3.0000000E-01	2.000000E-01
8	1.2500000E-01	3.000000E-01	2.000C000E-01	1.500000E-01
9	9.9999999E-02	2.000000E-01	1.500000E-01	1.200000E-01
10	1.33333338-01	5.000000E-01	2.5000000E-01	1.6666667E-01
11	9.9999999E-02	2.5000000E-01	1.6666667E-01	1.2500000E-01
12	7.9999999E-02	l.6666667E-01	1.2500000E-01	9.99999999E-02
13	1.6666667E-01	2.500000E-01	2.00000008-01	1.6666667E-01
14	1.1111111E-01	1.8750000E-01	1.5000000E-01	1.2500000E-01
15	8.3333329E-02	1.5000000E-01	1.200000E-01	9.999999995-02
16	6.6666669E-02	1.2500000E-01	9.99999999E-02	8.3333329E-02

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COLUMN

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	13	14	15	16
1	0.	2.5000000E-01	2.5000000E-01	2.5000000E-01
2	0.	2.0000000E-01	2.000000E-01	2.0000000E-01
3	0.	1.6666667E-01	1.6666667E-01	1.6666667E-01
4	0.	1.2500000E-01	1.6666667E-01	1.8750000E-01
5	0.	9.9999999E-02	1.3333333E-01	1.500000E-01
16	0.	8.3333329E-02	1.111111E-01	1.2500000E-01
.7	0.	8.3333329E-02	1.2500000E-01	1.500000E-01
8	0.	6.6666669E-02	9.9999999E-02	1.200000E-01
9	0.	5.555559E-02	8.3333329E-02	9.9999999E-02
10	0.	6.2500000E-02	9 <b>.9</b> 9999999E-02	1.2500000E-01
11	0.	4.9999999E-02	7.9999999E-02	9.99999999E-02
12	0.	4.1666669E-02	6.6666669E-02	8.3333329E-02
13	0.	1.4285714E-01	1.4285714E-01	1.4285714E-01
14	0.	7.1428570E-02	9.5238099E-02	1.0714286E-01
15	0.	4.7619050E-02	7.1428570E-02	8.5714289E-02
16	0.	3.5714290E-02	5.7142860E-02	7.1428570E-02

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#### COLUMN

ROW

	1	2	3	4
2	0.	0.	0.	1.0000000E 00
3	0.	0.	0.	1.0000000E 00
5	0.	0.	0.	5.000000E-01
6	0.	0.	0.	5.000000E-01
8	0.	0.	0.	3.3333333E-01
9	0.	0.	0.	3.33333333E-01
11	0.	0.	0.	2.5000000E-01
12	0.	0.	0.	2.5000000E-01
13	0.	0.	0.	1.0000000E 00
14	0.	0.	0.	5.000000E-01
15	0.	0.	0.	3.3333333E-01
16	0.	0.	0.	2.500000E-01

#### 8 7 6 5 5.000000E-01 1.000000E 00 3.3333333E-01 2 5.000000E-01 6.6666666E-01 5.000000E-01 1.000000E 00 6.666666E-01 3 3.3333333E-01 6.6666666E-01 1.6666667E-01 5 2.500000E-01 4.444444E-01 6.666666E-01 3.3333333E-01 2.500000E-01 6 2.500000E-01 5.000000E-01 1.6666667E-01 1.1111111E-01 8 3.3333333E-01 5.000000E-01 1.6666667E-01 2.2222222E-01 ' 9 2.000000E-01 4.000000E-01 1.250000E-01 8.3333329E-02 11 2.6666667E-01 4.000000E-01 1.2500000E-01 1.6666667E-01 12 7.500000E-01 1.000000E 00 5,9999999E-01 7.500000E-01 13 5.000000E-01 6.6666666E-01 -3.000000E-01 14 3.750000E-01 3.7500000E-01 5.000000E-01 2.000000E-01 2.500000E-01 15 4.000000E-01 3.000000E-01 1.500000E-01 1.8750000E-01 16

ROW

	9	10	11	1.2
2	3.3333333E-01	1.0000000E 00	5.0000000E-01	3.3333333E-01
3	5.0000000E-01	1.0000000E 00	6.6666666E-01	5.0000000E-01
5	2.2222222E-01	7.5000000E-01	3.7500000E-01	2.500000E-01
6	3.3333333E-01	7.5000000E-01	5.000000E-01	3.7500000E-01
8	1.6666667E-01	5.99999999E-01	3.000000E-01	2.000000E-01
9	2.5000000E-01	5.9999999E-01	4.000000E-01	3.000000E-01
11	1.3333333E-01	5.0000000E-01	2.500000E-01	1.6666667E-01
12	2.0000000E-01	5.0000000E-01	3.3333333E-01	2.5000000E-01
13	5.9999999E-01	1.0000000E 00	7.500000E-01	5.99999999E-01
14	4.0000000E-01	7.5000000E-01	5.6250000E-01	4.49999999E-01
15	3.0000000E-01	5.9999999E-01	4.4999999E-01	3.600000E-01
16	2.4000000E-01	5.000000E-01	3.7500000E-01	3.000000E-01

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COLUMN

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	13	14	15	16
2	0	2.500000E-01	2.5000000E-01	2.5000000E-01
2	0.	4.000000E-01	4.0000000E-01	4.0000000E-01
3	0.	1.2500000E-01	1.6666667E-01	1.8750000E-01
5	0.	2.000000E-01	2.6666667E-01	3.0000000E-01
·6	0.	9 3333329E-02	1.2500000E-01	1.5000000E-01
8	0.	0.55555272 02	2.0000000E-01	2.4000000E-01
9	0.	1.3333335501	a_9999999E-02	1.2500000E-01
11	0.	6.2500000E-02	1 600000E-01	2.0000000E-01
12	0.	9.9999999E-02	1.000000000000	5 000000E-01
13	0.	5.000000E-01	5.0000000000000	3.75000000000
14	0.	2.5000000E-01	3.3333333E-01	3.75000000 01
15	0.	1.6666667E-01	2.5000000E-01	3.00000002-01
16	0.	1.2500000E-C1	2.000GQ00E-01	2.5000000000000

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ROW

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#### ROW

		1		2	3	4	
7	0.		0.		0.	2.000000E	00
8	0.		0.		0.	1.000000E	00
9	0.		0.		0.	6.6666666E-	-01
10	0.		0.		0.	3.000000E	00
11	0.		0.		0.	1.5000000E	00
12	0.		0.		0.	1.000000E	00
15	0.		0.		0.	5.0000000E	-01
16	0.		0.		0.	7.5000000E-	-01
ROW				COLUMN			

	5	6	7	8
7	1.0000000E 00	6.6666666E-01	2.0000000E 00	1.0000000E 00
.8	6.6666666E-01	5.000000E-01	1.0000000E 00	6.6666666E-01
9	5.0000000E-01	4.0000000E-01	6.6666666E-01	5.0000000E-01
10	1.5000000E 00	1.0000000E 00	4.0000000E 00	2.0000000E 00
11	1.0000000E 00	7.5000000E-01	2.0000000E 00	1.3333333E 00
12	7.500000E-01	5.9999999E-01	1.3333333E 00	1.00000005 00
15	4.0000000E-01	3,3333333E-01	5.000000E-01	4.0000000E-01
16	5.9999999E-01	5.000000E-01	1.0000000E 00	7.99999998-01

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9 10 11 12 7 6.666666E-01 2.000000E 00 1.000000E 00 6.6666666E-01 8 5.000000E-01 1.0000000E 00 6.6666666E-01 5.000000E-01 9 4.000000E-01 6.666666E-01 5.000000E-01 4.000000E-01 10 1.3333333E 00 4.500000E 00 2.2500000E 00 1.500000E 00 11 1.000000E 00 2.2500000E 00 1.5000000E 00 1.1250000E 00 12 7.9999999E-01 1.500000E 00 1.1250000E 00 8.9999999E-01 15 3.3333333E-01 5.000000E-01 4.000000E-01 3.3333333E-01 16 6.6666666E-01 1.1250000E 00 8.9999999E-01 7.500000E-01

ROW

COLUMN

		13	•	14	15	16
7	0.			5.000000E-01	5.0000000E-01	5.000000E-01
8	0.			4.000000E-01	4.000000E-01	4.0000000E-01
9	0.			3.3333333E-01	3.3333333E-01	3.3333333E-01
10	0.			7.5000000E-Q1	1.0000000E 00	1.1250000E 00
11	0.			5.9999999E-01	7.9999999E-01	8.9999999E-01
12	0.			5.000000E-01	6.6666666E-01	7.500000E-01
15	0.			2.8571429E-01	2.8571429E-01	2.8571428E-01
16	0.			4.2857143E-01	5.7142857E-01	6.4285713F-0)

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# [<sub>*I*3</sub>]

#### COLUMN

	1	2	3	4
3	0.	0.	0.	2.0000000E 00
6	0.	0.	0.	1.0000000E 00
9	0.	0.	0.	6.6666666E-01
12	0.	0.	0.	5.000000E-01
13	0.	0.	0.	3.0000000E 00
14	0.	0.	0.	1.500000E 00
15	0.	0.	0.	1.0000000E 00
16	0.	0.	0.	7.500000E-01
ROW		COLUMN		

	5	6	7	8
3	1.0000000E 00	6.6666666E-01	2.0000000E 00	1.0000000E 00
6	5.000000E-01	3.3333333E-01	1.3333333E 00	6.6666666E-01
9	3.3333333E-01	2.2222222E-01	1.0000000E 00	5.000000E-01
12	2.500000E-01	1.6666667E-01	7.9999999E-01	4.000000E-01
13	2.0000000E 00	1.5000000E 00	3.000000E 00	2.0000000E 00
14	1.0000000E 00	7.500000E-01	2.0000000E 00	1.3333333E 00
15	6.6666666E-01	5.000000E-01	1.5000000E 00	1.000000E 00
16	5.000000E-01	3.7500000E-01	1.2000000E 00	7.99999998E-01

ROW

ROW

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	9	10	11	12
3.	6.6666666E-01	2.0000000E 00	1.0000000E 00	6.6666666E-01
6	4.444444E-01	1.500000E 00	7.500000E-01	5.000000E-01
:9	3.3333333E-01	1.2000000E 00	5.9999999E-01	4.000000E-01
12	2.6666667E-01	1.00000000 00	5.000000E-01	3.3333333E-01
13	1.5000000E 00	3.0000000E 00	2.0000000E 00	1.5000000E 00
.14	1.0000000E 00	2.2500000E 00	1.5000000E 00	1.1250000E 00
15	7.5000000E-01	1.8000000E 00	1.2000000E 00	8.99999998-01
16	5.9999999E-01	1.5000000E 00	1.0000000E 00	7.500000E-01

ROW

COLUMN

	13	14	15	16
3	0.	5.000000E-01	5.000000E-01	5.000000E-01
6	0.	2.5000000E-01	3.3333333E-01	3.7500000E-01
9	0.	1.6666667E-01	2.5000000E-01	3.000000E-01
12	0.	1.2500000E-01	2.000C000E-01	2.5000000E-01
13	0.	1.2000000E 00	1.2000000E 00	1.2000000E 00
14	0.	5.9999999E-01	7.9999999E-01	8.9999999E-01
15	0.	4.0000008-01	5.99999998-01	7.200000E-01
·16	0.	3.000000E-01	4.800000E-01	5.9999999E-01

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#### COLUMN

2 1 3 4 1.000000E 00 1.000000E 00 5 ο. 0. 6 0. 1.000000E 00 1.3333333E 00 0. 1.000000E 00 1.000000E 00 8 0. 0. 9 Ο. 1.000000E 00 1.3333333E 00 0. 1.000000E 00 1.000000E 00 0. 11 0. 1.000000E 00 1.3333333E 00 0. 12 0. 1.000000E 00 1.500000E 00 0. 14 0. 1.000000E 00 1.500000E 00 15 0. 0. 1.000000E 00 1.500000E 00 16 0. 0. COLUMN ROW

	5	6	7	8
5	5.000000E-01	5.000000E-01	0.	3.3333333E-01
6	5.000000E-01	6.6666666E-01	0.	3.3333333E-01
8	6.6666666E-01	6.6666666E-01	0.	5.000000E-01
9	6.6666666E-01	8.8888887E-01	0.	5.000000E-01
11	7.500000E-01	7.5000000E-01	0.	5.9999999E-01
12	7:5000000E-01	1.0000000E 00	0.	5.9999999E-01
14	5.000000E-01	7.5000000E-01	0.	3.3333333E-01
15	6.66666665-01	1.0000000E 00	0.	5.000000E-01
16	7.500000E-01	1.1250000E 00	0.	5.9999999E-0

ROW
	9	10	11	12
5	3.33333338-01	0.	2.500000E-01	2.5000000E-01
6	4,444444E-01	0.	2.5000000E-01	3.3333333E-01
8	5.0000000E-01	0.	4.0000000E-01	4.0000000E-01
9	6.6666666E-01	0.	4.000000E-01	5.3333332E-01
11	5,9999999E-01	0.	5.000000E-01	5.000000E-01
12	7.9999999E-01	0.	5.000000E-01	6.6666666E-01
14	5.000000E-01	0.	2.5000000E-01	3.7500000E-01
15	7.5000000E-01	0.	4.0000000E-01	5.9999999E-01
16	8.9999999E-01	0.	5.000000E-01	7.5000000E-01

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ROW

COLUMN

13	14	15	16
1.0000000E 00	5.000000E-01	3.3333333E-01	2.500000E-01
1.5000000E 00	7.5000000E-01	5.0000000E-01	3.7500000E-01
1.0000000E 00	6.6666666E-01	5.0000000E-01	4.000000E-01
1.5000000E 00	1.0000000E 00	7.5000000E-01	5.9999999E-01
1.0000000E 00	7.5000000E-01	5.9999999E-01	5.0000000E-01
1.5000000E 00	1.1250000E 00	8.99999998-01	7.500000E-01
1.8000000E 00	8.9999999E-01	5.9999999E-01	4.49999999E-01
1.8000000E 00	1.2000000E 00	8.9999999E-01	7.200000E-01
1.8000000E 00	1.3500000E 00	1.0800000E 00	8.99999998-01
	13 1.0000000E 00 1.5000000E 00 1.0000000E 00 1.5000000E 00 1.5000000E 00 1.8000000E 00 1.8000000E 00	13141.0000000E 005.000000E-011.5000000E 007.500000E-011.0000000E 006.66666666E-011.5000000E 001.0000000E 001.0000000E 007.5000000E-011.5000000E 001.1250000E-011.8000000E 001.2000000E 001.8000000E 001.3500000E 00	1314151.0000000E 005.000000E-013.333333E-011.5000000E 007.500000E-015.000000E-011.0000000E 006.66666666E-015.000000E-011.5000000E 001.000000E 007.500000E-011.0000000E 007.500000E-015.999999E-011.5000000E 001.1250000E 008.999999E-011.8000000E 001.200000E 008.999999E-011.8000000E 001.3500000E 001.0800000E 00

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# TWO-DIMENSIONAL DIPARABOLIC INTEGRATION COEFFICIENTS FOR USE IN PLATE AND SHELL ANALYSES $\left[\Gamma_{15}\right]$

# COLUMN

RO₩

	1	2	3	4
1	0.	0.	0.	5.000000E-01
2	0.	0.	Ģ.	2.5000000E-01
3	0.	0.	0.	1.6666666E-01
4	-5.000000E-01	-2.5000000E-01	-1.6666666E-01	0.
5	-2.5000000E-01	-1.6666666E÷01	-1.2500000E-01	0.
6	-1.6666666E-01	-1.2500000E-01	-9.9999999E-02	0.
7	-5.0000000E-01	-2.5000000E-01	-1.6666666E-01	-1.6666667E-01
3	-2.5000000E-01	-1.66666666E-01	-1.2500000E-01	-8.3333329E-02
9	-1.6666666E-01	-1.2500000E-01	-9.9999999E-02	~5.5555554E-02
10	-5.0000000E-01	-2.5000000E-01	-1.6666666E-01	-2.500000E-01
11	-2.5000000E-01	-1.6666666E-01	-1.2500000E-01	-1.2500000E-01
12	-1.6666666E-01	-1.2500000E-01	-9.9999999E-02	-8.3333334E-02
13	0.	0.	0.	1.2500000E-01
14	-1.2500000E-01	-9.9999999E-02	-8.3333334E-02	0.
15	-1.2500000E-01	~9.9999999E-02	-8.3333334E-02	-4.1666670E-02
16	-1.2500000E-01	-9.9999999E-02	-8.3333334E-02	-6.2500000E-02

OW		COLUMN		
	5	6	7	8
1	2.5000000E-01	1.666666666-01	5.0000000E-01	2.5000000E-01
2	1.65666666-01	1.2500000E-01	2.5000000E-01	1.6666666E-01
3	1.2500000E-01	9.9999999E-02	1.6666666E-01	1.2500000E-01
4	0.	0.	1.6666667E-J1	8.3333329E-02
5	0.	0.	8.3333329E-02	5.5555554E-02
6	0.	0.	5.5555554E-02	4.1666670E-02
7	-8.3333329E-02	-5.5555554E-02	0.	0.
8	-5.5555554E-02	-4.1666670E-02	0.	0.
9	-4.1666670E-02	-3.3333330E-02	0.	0.
10	-1.2500000E-01	-8.3333334E-02	-9.9999998E-02	-4.9999999E-02
11	-8.3333334E-02	-6.2500000E-02	-4.99999999E-02	-3.3333335E-02
12	-6.2500000E-02	-4.9999999E-02	-3.3333335E-02	-2.49999999E-02
13	9.9999999E-02	8.3333334E-02	1.2500000E-01	9.99999999E-02
14	0.	0.	4.1666670E-02	3.3333330E-02
15	-3.3333330E-02	-2.777775E-02	0.	0.
16	-4.9999999E-02	-4.1666665E-02	-2.4999999E-02	-2.0000000E-02

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	9	10	11	12
-	1 46666665-01	5.000000E-01	2.5000000E-01	1.6666666E-01
1	1.0000000000000	2 5000000E-01	1.6666666E-01	1.2500000E-01
2	1.25000002-01	2.50000000 11	1.2500000E-01	9.99999999E-02
3	9.999999945-02		1.250000E-01	8.3333334E-02
4	5.5555554E-02	2.500000000000	0.2223334E-02	6.2500000E-02
5	4.1666670E-02	1.2500000E-01	6.95959942 02	4_9999999E-02
6	3.3333330E-02	8.3333334E-02	6.25000002-02	2 2223335E-02
7	0.	9.9999998E-02	4.9999999E-U2	3.33333332
8	0.	4.99999999E-02	3.3333335E-02	2.49999992-02
9	0.	3.3333335E-02	2.49999999E-02	2.0000000000000
10	-3.333335E-02	0.	0.	0.
11	-2,4999999E-02	0•.	0.	0.
12	-2-000000E-02	0.	0.	0.
12	g 3333334F-02	1.2500000E-01	9.9999999E-02	8.3333334E-02
15	2.777775E-02	6.2500000E-02	4.99999998E-02	4.1666665E-02
14	~	2.4999999E-02	2.0000000E-02	1.6666665E-02
15	U•	0	0.	0.
16	-1.666666555-02	<b>↓</b>		

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COLUMN

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	13	14	15	16
1	0.	1.2500000E-01	1.2500000E-01	1.2500000E-01
2	0.	9 <b>.99999</b> 998-02	9.99999998-02	9 <b>.99999999E</b> -02
3	0.	8.3333334E-02	8.3333334E-02	8.3333334E-02
4	-1.2500000E-01	0.	4.1666670E-02	6.2500000E-02
5	-9.9999999E-02	0.	3.3333330E-02	4.99999998-02
6	-8.3333334E-02	0.	2.7777775E-02	<b>4.1666665E-02</b>
7	-1.2500000E-01	-4.1666670E-02	0.	2.4999999E-02
8	-9,9999999E-02	-3.3333330E-02	0.	2.0000000E-02
9	-8.3333334E-02	-2.777775E-02	0.	1.6666665E-02
10	-1.250C000E-01	-6.2500000E-02	-2.4999999E-02	0.
11	-9.9999999E-02	-4.9999999E-02	-2.0000000E-02	0.
12	-8.3333334E-02	-4.1666665E-02	-1.6666665E-02	0.
13	0.	7.1428570E-02	7.1428570E-02	7.1428570E-02
14	-7.1428570E-02	0.	2.3809525E-02	3.5714284E-02
15	-7.1428570E-02	-2.3809525E-02	0.	1.4285715E-02
16	-7.1428570E-02	-3.5714284E-02	-1.4285715E-02	0.

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#### TWO-DIMENSIONAL DIPARABOLIC INTEGRATION COEFFICIENTS FOR USE IN PLATE AND SHELL ANALYSES

# [r<sub>/6</sub>]

#### COLUMN

3 2 1 5.000000E-01 0. 5.000000E-01 0. 1 -2.500000E-01 1.6666667E-01 0. -5.000000E-01 2 -2.500000E-01 -1.6666667E-01 0. -5.000000E-01 3 0. 2.500000E-01 2.500000E-01 4 0. -1.6666667E-01 8.333333E-02 -2.5000000E-01 0. 5 -1.6666667E-01 -8.3333333E-02 Ο. -2.5000000E-01 6 1.6666667E-01 0. 1.6666667E-01 7 0. 5.55555558-02 -1.2500000E-01 -1.6666667E-01 0. 8 -1.2500000E-01 -5.5555555E-02 0. -1.6666667E-01 9 1.250000E-01 1.250000E-01 0. 0. 10 -9.9999999E-02 4.1666666E-02 0. -1.2500000E-01 11 -9.9999999E-02 -4.1666666E-02 0. -1.2500000E-01 12 -2.500000E-01 -9.9999998E-02 -2.500000E-01 -4.9999999E-01 13 -1.6666667E-01 -4.9999999E-02 -1.2500000E-01 -2.5000000E-01 14 -1.2500000E-01 -3.3333333E-02 -8.3333331E-02 -1.6666667E-01 15 -9.9999999E-02 -6.2500000E-02 -2.49999998-02 -1.2500000E-01 16

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ROW

COLUMN

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	5	6	7	8
1	2.5000000E-01	2.5000000E-01	0.	1.6666667E-01
2	0.	8.3333333E-02	-1.6666667E-01	0.
3	-8.3333333E-02	0.	-1.6666667E-01	-5.5555555E-02
4	1.6666667E-01	1.6666667E-01	0.	1.2500000E-01
5	0.	5.5555555E-02	-1.2500000E-01	0•
6	-5.5555555E-02	0.	-1.2500000E-01	-4.1666666E-02
7	1.2500000E-01	1.2500000E-01	0.	9.99999999E-02
8	0.	4.1666666E-02	-9.9999999E-02	0.
9	-4.1666666E-02	0.	-9.9999999E-02	-3.3333333E-02
10	9.9999999E-02	9.9999999E-02	0.	8.3333333E-02
11	0.	3.3333333E-02	-8.3333333E-02	0.
12	-3.3333333E-02	0.	-8.3333333E-02	-2.7777778E-02
13	-1.2500000E-01	-4.9999999E-02	-1.6666667E-01	-8.3333331E-02
14	-8.3333331E-02	-3.3333333E-02	-1.2500000E-01	-6.2500000E-02
15	-6.2500000E-02	-2.4999999E-02	-9.9999999E-02	-4.99999999E-02
16	-4,9999999E-02	-2.0000000E-02	-8.3333333E-02	-4.1666666E-02

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COLUMN

	9	10	11	12
1	1.6666667E-01	0.	1.2500000E-01	1.2500000E-01
2	5.5555555E-02	-1.2500000E-01	0.	4.1666666E-02
3	0.	-1.2500000E-01	-4.1666666E-02	0.
4	1.2500000E-01	0.	9.99999999E-02	9.9999999E-02
5	4.1666666E-02	-9.9999999E-02	0.	3.3333333E-02
6	0.	-9.9999999E-02	-3.3333333E-02	0.
ט ד	9,99999995-02	0.	8.3333333E-02	8.3333333E-02
, 8	3,3333333E-02	-8.3333333E-02	0.	2.7777778E-02
0	0.	-8.3333333E-02	-2.777778E-02	0.
,	8.333333E-02	0.	7.1428571E-02	7.1428571E-02
10	2.777778E-02	-7.1428571E-02	0.	2.3809524E-02
11	0	-7.1428571E-02	-2.3809524E-02	0.
12	2 22233335-02	-1-2500000E-01	-6.2500000E-02	-2.49999999E-02
13	2 4000000E-02	-9,9999999E-02	-4.9999999E-02	-2.0000000E-02
14	-2.49999995 02	-8.3333333E-02	-4.1666666E-02	-1.6666667E-02
15	-2.0000000E-02	-7.1428571E-02	-3.5714285E-02	-1.4285714E-02
10				

	13	14	15	16
1	4.99999999E-01	2.5000000E-01	1.6666667E-01	1.2500000E-01
2	2.5000000E-01	1.2500000E-01	8.3333331E-02	6.2500000E-02
3	9.9999998E-02	4.9999999E-02	3.3333333E-02	2.4999999E-02
4	2.5000000E-01	1.6666667E-01	1.2500000E-01	9.99999998-02
5	1.2500000E-01	8.3333331E-02	6.2500000E-02	4.99999999E-02
6	4.9999999E-02	3.3333333E-02	2.4999999E-02	2.0000000E-02
7	1.6666667E-01	1.2500000E-01	9.9999999E-02	8.3333333E-02
8	8.3333331E-02	6.2500000E-02	4.99999999E-02	4.1666666E-02
9	3.3333333E-02	2.4999999E-02	2.0000000E-02	1.6666667E-02
10	1.2500000E-01	9.9999999E-02	8.3333333E-02	7.1428571E-02
11	6.2500000E-02	4.99999998-02	4.1666666E-02	3.5714285E-02
12	2.4999999E-02	2.000000E-02	1.6666667E-02	1.4285714E-02

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## TWO-DIMENSIONAL DIPARABOLIC INTEGRATION COEFFICIENTS FOR USE IN PLATE AND SHELL ANALYSES

# $[\Gamma_{j7}]$

# COLUMN

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		1	2	3	
4	• 0•		1.0000000E 00	1.0000000E 00	0.
5	0.		5.000000E-01	6.666666E-01	0.
6	0.		3.3333333E-01	5.000000E-01	0.
7	0.		1.0000000E 00	1.0000000E 00	0.
8	0.		5.000000E-01	6.666666E-01	0.
9	0.		3.3333333E-01	5.000000E-01	0.
10	0.		1.0000000E 00	1.0000000E 00	0.
11	0.		5.000000E-01	6.6666666E-01	0.
12	0.		3.3333333E-01	5.000000E-01	0.
14	0.		2.500000E-01	4.000000E-01	0.
15	0.		2.500000E-01	4.000000E-01	0.
16	٥.		2.500000E-01	4.000000E-01	0.

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	5	6	7	8
4	5.000000E-01	5.000000E-01	0.	3.3333333E-01
5	2.5000000E-01	3.3333333E-01	0.	1.6666667E-01
6	1.6666667E-01	2.5000000E-01	0.	1.1111111E-01
7	6.6666666E-01	6.6666666E-01	0.	5.0000000E-01
8	3.3333333E-01	4.444444E-01	0.	2.5000000E-01
9	2.22222228-01	3.3333333E-01	0.	1.6666667E-01
10	7.5000000E-01	7.5000000E-01	0.	5.9999999E-01
11	3.7500000E-01	5.0000000E-01	0.	3.0000000E-01
12	2.5000000E-01	3.7500000E-01	0.	2.0000000E-01
14	1.2500000E-01	2.000000E-01	0.	8.3333329E-02
15	1.6666667E-01	2.6666667E-01	0.	1.2500000E-01
16	1.8750000E-01	3.000000E-01	0.	1.500000E-01

10

	9	
4	3.3333333E-01	0.
5	2.2222222E-01	0.
6	1.6666667E-01	0.
7	5.000000E-01	0.
8	3.3333333E-01	0.
9	2.5000000E-01	0.
10	5.9999999E-01	0.
11	4.000000E-01	0.
12	3.000000E-01	0.
14	1.3333333E-01	0.
15	2.0000000E-01	0.
16	2.4000000E-01	0.

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11	12
2.5000000E-01	2.500000E-01
1.2500000E-01	1.6666667E-01
8.3333329E-02	1.2500000E-01
4.000000E-01	4.000000E-01
2.000000E-01	2.6666667E-01
1.3333333E-01	2.000000E-01
5.000000E-01	5.000000E-01
2.5000000E-01	3.3333333E-01
1.6666667E-01	2.500000E-01
6.2500000E-02	9 <b>.99999999E-</b> 02
9.9999999E-02	1.600000E-01
1.2500000E-01	2.0000000E-01

ROW

	13	14	15	16
4	1.0000000E 00	5.0000000E-01	3.3333333E-01	2.5000000E-01
•5	7.5000000E-01	3.7500000E-01	2.5000000E-01	1.8750000E-01
6	5.9999999E-01	3.0000000E-01	2.0000000E-01	1.500000E-01
7	1.0000000E 00	6.6666666E-01	5.0000000E-01	4.0000000E-01
8	7.5000000E-01	5.000000E-01	3.7500000E-01	3.0000000E-01
9	5.9999999E-01	4.000000E-01	3.0000000E-01	2.4000000E-01
10	1.0000000E 00	7.5000000E-01	5.9999999E-01	5.000000E-01
11	7.5000000E-01	5.6250000E-01	4.49999998-01	3.7500000E-01
12	5.9999999E-01	4.4999999E-01	3.6000000E-01	3.000000E-01
14	5.000000E-01	2.5000000E-01	1.6666667E-01	1.2500000E-01
15	5.000000E-01	3.3333333E-01	2.5000000E-01	2.0000000E-01
16	5.000000E-01	3.7500000E-01	3.0000000E-01	2.5000000E-01

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### APPENDIX V

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GLOSSARY OF TERMS USED IN THE TEXT

#### MATRIX NOTATIONS

L J	rectangular or square matrix
{ }	column matrix
{}`	row matrix
- -	diagonal matrix
[]	matrix transpose
[]-'	matrix inverse
[0],{0}	null matrix
「1」	unity matrix

#### DEFINITION OF TERMS IN THE TEXT

(I,J,IK) inertial set of right-handed, orthogonal, unit vectors (X, Y, z) Lagrangian particle variables  $(5, \gamma, 5)$  Eulerian variables for a particle л position vector for the x-y-z particle at time, t  $V = \frac{\delta m}{\delta t}$  particle velocity  $\Omega = \frac{\partial^2 m}{\partial t^2}$  particle acceleration e density P body force  $\Sigma$ stress dyadic  $\mathbb{G}_{i,j}$  ,  $\mathbb{G}_{xx}$  ,  $\mathbb{G}_{xy}$  ,  $\mathbb{G}_{\mu k}$  ,  $\mathbb{G}_{\mu \kappa}$  various notations for stress components specific internal energy U specific internal dissipation function ル U total internal energy R Rayliegh's dissipation function Т total kinetic energy

v potential of body forces

V	total potential of body forces
$\lambda_i$	Lagrange's undetermined multipliers
Þ; , i = 1,2	N; { p} generalized coordinates
[A]	inertia matrix referred to b;
[K]	stiffness matrix referred to p;
Pi	generalized forces corresponding to b;
[E]	influence matrix referred to p:
[B]	damping matrix corresponding to p;
$\{\mathcal{G}\}_{i}$	vibration mode
[4]	modal matrix
$\omega_i$ .	vibration frequency
λ;	vibration eigenvalue
[L]	generic notation for coefficients in linear constrants expressed in implicit form
[5]	transformation matrix which constrains rigid body motion
[4e]	rigid body modal matrix
[٢]	rigid body "sweeping" metrix
[r][E][r]	free body influence coefficients
[G(t)]	Green's function
€;,{ <b>8</b> }	generic notation for more specialized generalized coordinates, modal coordinates or internal coordinates (Section 5.1.1)
[M]	inertia matrix referred to $q;$
[F]	stiffness matrix referred to Q;
$Q_{i}$	generalized forces corresponding to $q_i$
[G].	influence matrix referred to $Q_i$
[R]	damping matrix corresponding to $q_i$
(-)	

 $\{\pi\}_{i}$  vibration mode in terms of a set of  $q_{i}$  's

[π] modal matrix for  $q_i$ 's 3; modal damping factor ľþ displacement vector for x-y-z particle  $[h_i = \frac{\partial [p]}{\partial p_i} | p_i = p_2 = ... = 0$  generalized Rayliegh-Ritz functions, sometimes called "generalized displace-ments" corresponding to the generalized coordinates.  $(\xi, Y)$ local, non-dimensional, particle coordinates used in interpolation methods [3]: interpolation coefficients for the ; th region [Λ] aerodynamic influence coefficients corresponding to p;  $\left[ \Delta \right]$ streamwise "differentiation" matrix ξ rigid body longitudinal translation coordinate 3 rigid body lateral translation coordinate θ rigid body pitch coordinate 4 control coordinate [R] generic notation for transformation from external loads to internal loads [N] axial load coefficient matrix referred to b;  $[L_{R}], [L_{I}], \{L_{o}\}$ quasi-steady aerodynamic matrices referred to b; [H], [H.) thrust force matrices referred to b;  $[C_R], [C_T], \{C_o\}$ quasi-steady aerodynamic matrices referred to  $\mathscr{G}$ ; {4,},{4,},{4,},{4,} unit, orthogonal, rigid body modes. general unsteady aerodynamic matrix referred to q; [C(H, M\_m)] [N] generic notation for coefficient matrix in the "standard" eigenvalue problem IR · position vector for the center of mass  $V = \frac{d R}{dt}$  velocity of center of mass  $L_{i}$ ,  $\{L\}$  generic notation for internal loads, stress resultants, stresses, or "member" loads

 $(\dot{v}, \dot{U}, \mathbf{k})$  body set of right-handed, orthogonal, unit vectors

**\mathbf{\Omega}** angular velocity of  $(\hat{\mathbf{y}}, \hat{\mathbf{y}}, \mathbf{k})$  reference system

(F total force

Gr total moment of forces

H total angular momentum

**[Te]** generic form of geometric transformations used in the method of modal coupling

modal matrix in terms of system generalized coordinates

 $(\xi, \Upsilon, 5, \Psi, \Theta, \Psi, \mu, \tau, \lambda)$  rigid body generalized coordinates

- $(\xi, \gamma, 5)$  inertial coordinates of center of mass
- $(\varphi, \Theta, \psi)$  Euler angles for (v, j, k) reference system

 $(\mathcal{U},\mathcal{Y},\lambda)$  primary control displacements

#### GENERAL MATHEMATICAL NOTATIONS

J()dv	volume integration
ff().ds	closed surface integral
δ() ( <sup>m</sup>	virtual change or variation
$\overline{f}(s) = \int_{0}^{\infty} f(t) e^{-st} dt$	t Laplace transform
N2 (t)	Dirac's "delta" function
)\$ (t)	Heaviside's unit step function
$f(t) = \int_{-\infty}^{\infty} \tilde{f}(\omega) e^{i\omega t}$	dw ]
$\bar{f}(\omega) = \frac{i}{2\pi} \int_{-\infty}^{\infty} f(t) e^{-t}$	dt Fourier transform pair
	-

### APPENDIX VI

# THE GILL-RUNGE-KUTTA SCHEME OF NUMERICAL INTEGRATION

#### 1.0 COMPUTER SUBROUTINE FOR NUMERICAL INTEGRATION

The method for the numerical integration of ordinary differential equations described here is the method of Runge-Kutta which has been adapted for use on a digital computer by Stanley Gill.

The numerical integration of equations of the type

$$\frac{dy}{dx} = f(x,y)$$
 (VI-1)

is accomplished by the Runge-Kutta method as follows.

Let the interval be of length h so that the range of x is divided by the points  $x_0, x_1, \ldots$  where

$$\begin{array}{l} x_1 = x_0 + k \\ x_2 = x_0 + 2k \\ \vdots \\ x_2 = x_0 + nk \end{array}$$
(VI-2)

Each increment  $\Delta y$  of y is calculated as follows

 $\Delta q_{2} = \frac{1}{2} (k_{1} + 2k_{2} + k_{3})$  (VI-3)

where

$$\begin{aligned} k_{1} &= h f(x_{1}, y_{1}) \\ k_{2} &= h f'(x_{1} + k_{2}, y_{2} + \frac{k_{1}}{x}) \\ k_{3} &= h f'(x_{1} + k_{2}, y_{2} + \frac{k_{3}}{x}) \\ k_{4} &= h f'(x_{1} + h, y_{1} + k_{3}) \end{aligned}$$

$$(VI-4)$$

A weighted average of the four k's affords a good estimate of  $\Delta y$  and the error is of the order of h<sup>5</sup>, for a given interval\*.

If Equation VI-1 is of the form

$$\frac{du}{dx} = f(x) \qquad (VI-5)$$

the Runge-Kutta method reduces to Simpson's rule. In this case  $\Delta \mathbf{y}$  is accurately given by -

$$\Delta y = \int_{x_{n}}^{x_{n+1}} f(x) dx \qquad (VI-6)$$

\*The derivation of the above formulas is given in Ince, E. L., Ordinary Differential Equations, Dover, 1944, pp. 540 to 547. See also Lery, H. and Baggott, E. A., <u>Numerical Solutions to Differential Equations</u>, Watts and Co. (London) 1934, pp. 96 to 110. The Runge-Kutta formulas give

$$\Delta y = \frac{h}{6} \left( f(x_n) + 2 f(x_n + \frac{h}{2}) + 2 f(x_n + \frac{h}{2}) + f(x_n + h) \right)$$

$$= \frac{h}{12} \left( f(x_n) + 4 f(x_n + \frac{h}{2}) + f(x_n + h) \right)$$
(VI-7)

which is seen to be the Simpson rule approximation for the integral in VI-6.

The computation form displayed below is probably the most suitable if a hand calculator is being used for solution. The calculation of  $\Delta y$  is broken up into the following steps:

$$\begin{aligned} y_{12}^{*} &= f(x_{11}, y_{12}) \\ x_{12} &= x_{12} + hy_{22} \\ y_{12}^{*} &= y_{12} + hy_{22} \\ y_{12}^{*} &= y_{12} + hy_{22} \\ \end{aligned}$$
(VI-8)

$$y_{n_{2}} = \frac{1}{(x_{n_{2}}, y_{n_{2}})}$$
(VI-9)  
$$x_{n_{3}} = x_{n_{1}} + h_{2}$$
$$y_{n_{3}} - y_{n_{2}} + h_{2}y'_{n_{2}}$$

$$u_{1x_{3}} = \frac{1}{2} $

(VI-11)

$$\frac{2q_{x} - \frac{k}{r}}{q_{x}} \left( \frac{q_{x}}{r} + \frac{k}{r} \frac{q_{x}}{r} + \frac{q_{x}}{r} + \frac{q_{x}}{r} \right)$$
(VI-12)

If it is assumed that a differential equation can be solved for the derivative of highest order in the dependent variable, it is seen that the Runge-Kutta Equations VI-3 are applicable to higher order equations since these may be reduced to a system of first order equations.

 $4x_4 = 7 x_{24} \cdot 4x_4$ 

The Runge-Kutta method of integration has several desirable features which may be summarized as follows.

1) This method is generally considered to have good convergence qualities. Forward integration and iteration procedures can sometimes be unstable so that a calculated solution oscillates with rapidly increasing amplitude about the true solution. The Runge-Kutta method does not seem to be so susceptible to this difficulty. 2) The Runge-Kutta method allows the use of fairly large intervals compared to other methods of numerical integration.

3) Each integration interval is complete within itself, i.e., the only quantities necessary to proceed from one step to the next are those which would also be supplied as initial conditions to start the integration procedure. This feature allows a change of the interval size at any point. The integration may also be re-started at any point with ease.

The Gill modification to the Runge-Kutta process produces identical results but simplifies the labor involved when a system of simultaneous differential equations are to be integrated on a digital computer\*. The Gill-Runge-Kutta process is defined by the following equations. In these relations each equation contains terms defined by preceding equations.

	Equivalent Fortran Statement Number
	(See Table 36)
$k_{n_0} = h f_x (\eta_{\infty}, \eta_{\omega})$	40
$n_{\mathbf{x}_1} = \frac{1}{2} k_{\mathbf{x}_0} - \omega q_{\mathbf{x}_0}$	70
y, ≖ y <sub>*t</sub> + η <sub>*t</sub>	80
Za, - gro + 3 Mai - 2 kno	90
$k_{x_1} = h f_x(y_{01}, y_{11})$	120
$n_{22} = (1 - \int_{2}^{2}) (k_{x_{1}} - y_{x_{2}})$	140
$y_{n_2} = y_{n_1} + n_{n_2} - l_1 - JS \} k_{n_1}$	150
$q_{n_1} \neq q_{n_1} + 2n_{n_2}$	160
$k_{12} = -k f_{12} (y_{12}, y_{12},)$	190
$n_{13} = (1 + 1 \overline{5} + k_{m_2} - q_{m_2})$	210
Jws == Jws + 1×3	220
$f_{x_3} = q_{m_2} + 3 \pi_{x_3} - (1 + J \overline{\xi}) k_{x_3}$	230
kas - h fa (yes, yes)	250
724 - 2 (Kx3 - 27x3)	280
Yng, = yns + 724	290
	300

<sup>\*</sup>Wheeler, D. J., and Gill, S., The Preparation of Programs for an Electronic Digital Computer Addison-Wesley, 1951.

The coefficient  $\omega$  appearing in the expression for  $\mathcal{n}_{n_i}$  is not critical. The best value is actually 1, as it simplifies the program.

Table 36 is an IBM 7090 Fortran II listing of the integration scheme used in the results documented in this report. It is called Subroutine RK (Runge-Kutta). The definitions below will be helpful in its interpretation.

Subroutine DYDXS - Forms an expression for derivatives Subroutine INPUT - reads data in Subroutine OUTPUT - outputs results of integration Y(I) = dependent variables DYDX(I) = time derivatives of dependent variables Y(1) = independent variable DYDX(1) = 1.0

P(1) = integration step size

2.0 INTEGRATION OF THE GENERAL LINEAR TRANSIENT RESPONSE EQUATIONS

The Runge-Kutta integration can be used to obtain time-histories of transient stresses and displacements by solving a very general set of equations which arise when the assumption of small displacements is made. The form of the differential equation is:

$$[M]{\dot{q}} + [R]{\dot{q}} + [R]{\dot{q}} = \{Q_0\} + [\frac{2Q}{\delta E}]{F(L)}$$
(VI-13)

with the stresses and/or internal loads given by the general expression:

$$\{L\} = \{L_0\} + [\frac{\partial L}{\partial q}\} + [\frac{\partial L}{\partial q}] $

and the displacements and accelerations given by

••

$$\{b\} = [\phi]\{q_i\}$$
 (VT-15)

$$\{p\} = [\phi]\{\hat{g}\}$$
 (VI-16)

In these expressions, the following coefficient matrices are assumed to be independent of time and are supplied as input to the numerical integration scheme:

- [M], the mass matrix
- [R], dissipation matrix for the structure and the damping of quasi-steady aerodynamic forces
- [F], structural stiffness matrix and quasi-steady aerodynamic stiffness
- {Q},  $[\frac{\partial Q}{\partial F}]$  constant coefficients in time dependent generalized forces

 $\{ {}^{L_0} \}, [\frac{\partial L}{\partial q}], [\frac{\partial L}{\partial \dot q}], [\frac{\partial L}{\partial \dot q}], [\frac{\partial L}{\partial F}]$ 

constant coefficients relating stresses to time dependent quantities

 $[\boldsymbol{\phi}],$  relation of displacements to modal generalized coordinates

Also supplied as input, is a table of functions of time,  $F_i(t)$ , used to describe the transient generalized forces.

# TABLE 36 FORTRAN SOURCE PROGRAM LISTING OF RUNGE-KUTTA SUBROUTINE

LIST \* SYMBOL TABLE CRK SUBROUTINE RK COMMON VAR DIMENSION VAR(24000), DYDX(75), Y(75), Q(75), D(75), 1 NTEGER (225) .P(23400) EQUIVALENCE (VAR(1), Y(1)), (VAR(76), DYDX(1)), (VAR(151),Q(1)),(VAR(301),D(1)),(VAR(376),NTEGER(1)), 1 (VAR(601),P(1)),(NTEGER(6),N) 2 LOAD INPUT DATA INTO MACHINE. CALL INPUT С 10 CALCULATE THE DELTAY(J) AT Y(1) = 0.0. с 20 CALL DYDXS DO 40 I = 1+N 30 D(I) = DYDX(I)\*P(1)40 DETERMINE THE OUTPUT OF THE INTEGRATION. с CALL OUTPUT 50 CALCULATE THE Y(J) AT Y(1) = 0.0. С DO 90 J = 1+N 60 R = .5\*(D(J) - Q(J))70 Y(J) = Y(J) + R80 Q(J) = Q(J) + 3.0\*R - .5\*D(J)90 CALCULATE THE DELTAY(J) AT Y(1) = HALF STEP SIZE. С 100 CALL DYDXS DO 120 I = 1.N 110 D(1) = DYDX(1)\*P(1)120 CALCULATE THE Y(J) AT Y(1) = HALF STEP SIZE. ¢ 130 DO 160 J = 1, NR = .292893219\*(D(J) - Q(J)) 140 Y(J) = Y(J) + R Q(J) = Q(J) + 3.0\*R - .292893219\*D(J) 150 160 CALCULATE THE DELTAY(J) AT Y(1) = HALF STEP SIZE AGAIN. c 170 CALL DYDXS DO 190 I = 1.N 180 190 D(I) = DYDX(I)\*P(1)CALCULATE THE Y(J) AT Y(1) = HALF STEP SIZE AGAIN. С DO 230 J = 1.N 200 R = 1.70710678 \* (D(J) - Q(J))210 Y(J) = Y(J) + R Q(J) = Q(J) + 3.0\*R - 1.70710678\*D(J)220 230 CALCULATE THE DELTAY(J) AT Y(1) = STEP SIZE. Ċ 240 CALL DYDXS DO 260 I = 1+N 250 D(I) = DYDX(I)\*P(1)260 CALCULATE THE Y(J) AT Y(1) = STEP SIZE. C 270 DO 300 J = 1 .N R = .1656666667\*(D(J) - 2.0\*Q(J))280 Y(J) = Y(J) + R290 Q(J) = Q(J) + 3.0\*R - .5\*D(J)300 PROCEED TO THE NEXT INTEGRATION STEP. C NGO = 1 GO TO (20,330),NGO 310 320 RETURN 330 END

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