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Technical Report No. 10

RESPONSE OF A SIMPLY-SUPPORTED TIMOSHENKO BEAM TO A PURELY RANDOM GAUSSIAN PROCESS

by

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and
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to

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Reproduction in whole or in part is permitted for any purpose of the United States Government.
The generalized Fourier analysis is applied to the damped Timoshenko beam equation to calculate the mean square values of displacements and bending stress, resulting from purely random loading. Compared with the calculations based on the classical beam theory [4], it was found that the displacement correlations of both theories were in excellent agreement. Moreover the mean square of the bending stress, contrary to the results of the classical beam theory, was found to be convergent. Computations carried out with a digital computer are plotted for both theories.
1. Introduction

The response of linear elastic beams to random forcing functions has been studied by Ornstein [1], Houdijk [2], Van Leer and Uhlenbeck [3], and Eringen [4]. Eringen calculated the mean square displacements as well as the mean square stresses produced in a beam subject to a purely random loading, and found that the series for the mean square stresses diverge. He then suggested that the cause of this divergence may be traced to the inadequateness of the classical beam theory. In order to ascertain this point, one is then led to consider a more refined theory containing a mechanism which will favorably respond to such a loading. The present study is the result of such a consideration.

At first, the Timoshenko beam theory modified with the addition of a translatory velocity damping appeared to be adequate. However, a close examination showed that not only this, but the addition of Voigt-Serway type of internal damping did not produce an adequate theory leading to converging series for the mean square bending stress. Introduction of a linear damping to rotatory motion, however, produced the expected result. With the addition of such a mechanism to Timoshenko beam, it was no longer necessary to complicate the analysis by using internal damping. The analysis of this model is carried out for a simply supported beam subject to random pressure and purely random concentrated load.

An electronic computer was used to compute the displacement and bending stress correlation functions.
2. Differential Equations

Equations of rotatory and translatory motions of an element of the Timoshenko beam respectively are (Fig. 1)

\( \frac{dM}{dx} + Q - \frac{p}{2} (\tau_1 + \tau_2) = \rho I \frac{d^2 \varphi}{dx^2} \)

\( \frac{dQ}{dx} + P + (\tau_2 - \tau_1) \frac{d\varphi}{dx} = \rho A \frac{d^2 \psi}{dt^2} + \beta_0 \frac{d\psi}{dt} \)

where \( M, Q, P \) are the bending moment, vertical shearing force and the vertical applied load; \( \tau_1, \tau_2 \) are surface shearing stresses which will play the role of a damping due to rotation; \( h, A, I \) are the thickness, the cross section area, and the moment of inertia about the neutral axis, \( \rho \) is the mass density, \( \varphi \) and \( \psi \) are the bending angle and the deflection. Coordinate \( x \) locates the cross section and \( t \) is the time.

In the Timoshenko beam theory we have
(3) \( \frac{\partial^2 \psi}{\partial x^2} = \frac{\partial}{\partial x} \left( \frac{\partial \psi}{\partial x} \right) \)

(4) \( K = -EI \frac{\partial^2 \psi}{\partial x^2} - EI \frac{\partial^2 \nu_3}{\partial x^2} \)

(5) \( Q = kGA \gamma \)

Where \( \gamma \) is the shearing angle, \( E \) and \( G \) are Young's and shear moduli respectively, and \( k \) is a constant which is adjustable to take into account the effect of the shape of the cross section. Here \( \nu_3 \) is termed the bending deflection \( [3] \). For \( k \), Timoshenko and E. Reissner respectively use \( \frac{2}{3} \) and \( \frac{5}{6} \).

In order to arrive at a rotatory damping similar to linear velocity damping, we set

(6) \( \tau_1 = \tau_2 = \frac{1}{E} \frac{\partial \psi}{\partial t} \beta_1 \frac{\partial \psi}{\partial t} \), \( \beta_1 \) constant.

This merely introduces a mechanism for the rotatory damping and is no more or less a sacrifice than the universally accepted idea of linear damping \( \beta_0 \frac{\partial \psi}{\partial t} \) present in (2). It may be thought of as being the air friction combined with all the frictional resistance against the rotation of the element of the beam.

If we use (3) - (6) in (1) and (2), we obtain:

(7) \( EI \frac{\partial^2 \psi}{\partial x^2} + kGA \phi + kGA \frac{\partial \psi}{\partial x} = \rho I \frac{\partial^2 \psi}{\partial t^2} + \beta_1 \frac{\partial \psi}{\partial t} \)

\( kGA \frac{\partial \psi}{\partial x} + P + kGA \frac{\partial^2 \psi}{\partial x^2} = \beta_0 \frac{\partial \psi}{\partial t} \)

Differentiating the first of (7) with respect to \( x \) and using the second of (7) to eliminate \( \psi \), we find
A simple relation exists between \( w \) and \( w_B \). This is found by combining (2) - (5), integrating the result with respect to \( x \) and excluding a rigid body displacement from the result. Hence

\[
(10) \quad w_s = w - w_B = \frac{1}{k_G A} \left( \rho I \frac{\partial^2 w_B}{\partial t^2} + \beta_1 \frac{\partial w_B}{\partial t} - EI \frac{\partial^2 w_B}{\partial x^2} \right)
\]

Sometimes \( w_s \) is called the shear deflection [5].

With the introduction of (10), equation (9) may be simplified to

\[
(11) \quad Lw_B = p
\]

Equation (8) is the Timoshenko beam equation modified with linear and rotatory damping and external loading. In what follows, we also need the boundary conditions for a simply-supported beam:

\[
(12) \quad w = \frac{\partial \varphi}{\partial x} = 0 \quad \text{at} \quad x = 0, L
\]

From (10), (3), and (4), it is clear that if we make

\[
(13) \quad w_B = \frac{\partial^2 w_B}{\partial x^2} = 0 \quad \text{at} \quad x = 0, L
\]

then the boundary conditions (12) would be satisfied. Hence we may first solve (11) subject to (13) to obtain \( w_B \). Equation (10) then gives \( w \).
3. Formal Solution

In order to determine the mean square values of various quantities such as the displacements and the stresses, we must first find a steady-state solution of (8) satisfying (12). This is the same thing as solving (11) subject to (12). Generalized harmonic analysis [6] can then be used to complete the problem. A formal solution of (11) satisfying (12) has the form

\[ w_n(x,t) = \sum_{n=1}^{\infty} w_n(t) \sin \left( \frac{n \pi x}{L} \right) \]

If we use (14) and a similar expression for \( P \) with \( P_n(t) \) replacing \( w_n(t) \) in (11), we obtain an ordinary differential equation for \( w_n(t) \). The Fourier transform of this equation with respect to \( t \) gives:

\[ \mathcal{F}_n(\xi) = (c^2 \lambda/EI) \mathcal{P}_n(\xi)/D_n(\xi) \]

where

\[ D_n(\xi) = \xi^4 - a \xi^2 - b_n \xi + c_n d_n \]

and

\[ a = \beta_o A_0, \quad b_n = B_n + \beta_0 P_0^2, \quad c_n = \beta_0 C_n, \quad d_n = \lambda \lambda_n \]

\[ A_0 = (1 + \kappa)/\beta_A, \quad B = \kappa(\beta_A)^{-2}, \quad B_n = c^2[(1 + \lambda)\lambda_n^2 + \lambda r^{-2}] \]

\[ C_n = (c^2/\beta_A) [(1 + \kappa)\lambda^2 + \lambda r^{-2}], \quad \lambda = \kappa G/E, \quad c = (E/\rho)^{1/2} \]

\[ r = (1/\lambda)^{1/2}, \quad \kappa = \beta_1/\beta_2, \quad \lambda_n = n \pi /L \]
Further a bar on top of a letter represents the Fourier transform, e.g.,

\[(19) \quad \hat{w}_n(\xi) = \int_{-\infty}^{\infty} w_n(t) e^{-2\pi i t \xi} dt\]

From the expression of \(F(x,t)\) of the form (14), the Fourier coefficient \(f_n(\xi)\) may be found to be

\[(20) \quad f_n(\xi) = \frac{2}{L} \int_{0}^{L} F(t,\xi) \sin(\pi n / L) dt\]

With the use of (20) in (15) we may invert (15) to obtain

\[(21) \quad w_n(t) = \frac{20 \bar{\lambda}/(2\pi)}{\pi} \int_{0}^{L} \left[ \int_{-\infty}^{\infty} w_n(t-\tau) P(t,\tau) d\tau \right] \sin(\pi n / L) d\xi\]

where

\[(22) \quad \omega_n(t) = \frac{1}{2\pi} \int_{-\infty}^{\infty} \sin t / D_n(\xi) d\xi\]

is the weighting function of the system for the \(n\)th mode of vibration.

Through equation (14) we now have \(w_n(x,t)\). If we further use this result in (10), we obtain \(v(x,t)\), thus completing a steady-state solution of (8) subject to (12).

4. Evaluation of \(W_n(t)\)

Contour integration may conveniently be used to evaluate (21).

For \(t > 0\) the appropriate contour C for this integral is the upper half of a circle of radius \(R\) in the \(\xi\)-plane having the origin at \(\xi = 0\). It can be shown that the integral
\[ W_n(t) = \frac{1}{2\pi i} \oint_C e^{it\xi}/\beta_n(\xi) \, d\xi \]

along the semi-circular arc vanishes as \( R \to 0 \), and that along the real axis gives (22), which by the theory of residues will be \( 2m \) times the sum of the residues within the contour. To obtain the residues, we need to locate the poles of \( \beta_n(\xi) = 0 \), which in view of (16) is the same thing as finding the roots of the frequency equation:

\[ \left( z + \beta_0 \right) \sum_{n=0}^{\infty} a_n z^n = 0, \quad z = \pm i \xi \]

The roots of the quartic (23) can be determined exactly by using the known formulae for roots. However, the complexity of these formulae would make the result difficult to interpret. Thus we use an approximate method which, for small damping, introduces negligible errors in the final results. We take the transverse and rotatory damping coefficients \( \beta_0 \) and \( \beta_1 \) to be the same order of magnitude. This means \( \kappa = O(1) \).

For \( \beta_0 \) small a natural method for finding the roots of (16) is to use a perturbation procedure in which \( \beta_0 \) is the perturbation parameter. Thus we write

\[ \zeta = \zeta_0 + \beta_0 \zeta_1 + \beta_0^2 \zeta_2 + \ldots \]

Upon substitution of this into (23), using (17) and setting the coefficients of various powers of \( \beta_0 \) equal to zero, we obtain

1. \[ z_0^4 + 2z_0^2 \beta_0^2 + d_n = 0 \]

3. \[ 4z_0^2 \beta_0 z_0 + 2z_0 \beta_0^3 \beta_0^2 + A_\infty z_0^3 + C_n z_0 = 0 \]

...
These equations may be solved very simply since \( z_0 \) satisfies a biquadratic while all the other \( z_i \) satisfy linear equations. Hence

\[
z_0 = i_0, \quad \Omega = \omega_0, -\omega_0, \quad \Omega_n = i_n, -i_n
\]

(25) \[
\omega_n = \left( \frac{3}{4} B_n^2 + \frac{3}{4} (B_n^2 - 4d_n^2)^{\frac{3}{2}} \right)
\]

(26) \[
\mu_n = \left( \frac{1}{4} B_n^2 - \frac{1}{4} (B_n^2 - 4d_n^2)^{\frac{3}{2}} \right)
\]

and

(26) \[
z_1 = -(A_{o}^2 + C_n)(\omega_0^2 + 2B_n)^{-1}
\]

We likewise obtain \( z_2, \ldots \). However in the present work, it will not be necessary to obtain the higher order terms.

We note that the dependence of \( C_n \) on \( n \) does not violate the perturbation since for large \( n \) we have \( z = O(n) \), thus making \( C_n z \) and \( nz^2 \) \( O(n^3) \) while the remaining terms are \( O(n^4) \).

An examination will show that \( B_n^2 - 4d_n > 0 \). Hence \( \omega_n \) and \( \mu_n \) are real. Using (24) - (26) we may now write \( D_n(\xi) \) to a linear approximation in \( \beta_0 \) as

\[
D_n(\xi) \approx (\xi - \xi_1)(\xi + \xi_1^*)(\xi - \xi_2)(\xi + \xi_2^*)
\]

\[
\xi_1 = \omega_n + i\beta_n, \quad \xi_2 = \mu_n + i\beta_n
\]

(27) \[
b_n = \beta_0(\omega_n^2 - C_n)(\omega_n^2 - 2B_n)^{-1}
\]

\[
\alpha_n = \beta_0(\omega_n^2 - C_n)(\mu_n^2 - 2B_n)^{-1}
\]
and an asterisk represents complex conjugate e.g. \( \zeta_1^* = \omega_n - i\eta_n \).

By use of Cauchy's theorem of residues, we find that \( W_n(t) = 0 \) for \( t < 0 \) and

\[
W_n(t) = \text{Im} \left( P_1 e^{it_1 t} + P_2 e^{it_2 t} \right) \quad \text{for } t > 0
\]

(28) \[ P_1 = \omega_n^{-1}(t_1 - t_2)^{-1}(\zeta_1 + \zeta_2^*)^{-1} \]

\[ P_2 = \omega_n^{-1}(t_2 - t_1)^{-1}(\zeta_2 + \zeta_1^*)^{-1} \]

Here \( \text{Im} \) represents the imaginary part of the complex quantity. Substituting the first of (28) into (21) and the result into (14), we find

(29) \[ w_B(x,t) = -(2\lambda /EIL) \sum_{n=1}^{\infty} \left( \int_0^L \int_0^\infty \text{Im}(P_1 e^{it_1(t-t')} + P_2 e^{it_2(t-t')}) \right) P(t,t')dt' \sin (n\pi x/L) \sin (n\pi x/L) \]

This completes the solution of the deterministic problem. For through (10), we can calculate \( w(x,t) \).

(30) \[ w(x,t) = -(2\lambda /EIL) \sum_{n=1}^{\infty} \left( \int_0^L \int_0^\infty \text{Im}(Q_1 e^{it_1(t-t')} + Q_2 e^{it_2(t-t')}) \right) P(t,t')dt' \sin (n\pi x/L) \sin (n\pi x/L) \]

\[ P(t,t')dt' \sin (n\pi x/L) \sin (n\pi x/L) = Q_j \]

\[ = 1 - (r^2 /\lambda c^2) \xi_j^2 + (k\pi^2 /\lambda c^2(1+k)^{-1}) \xi_j + (r^2 /\lambda c^2(1+k)) \xi_j \]

\[ (j = 1,2) \]

5. **The Autocorrelation Function of the Displacement and the Bending Moment**

The time average of a function \( w(x,t) \) is defined by

(31) \[ \langle w(x,t) \rangle = \lim_{T\to\infty} \frac{1}{2T} \int_{-T}^{T} w(t,t + \tau) d\tau. \]
For an ergodic process this average is known to be equal to the expectation (ensemble average) $E[w(x,t)]$ of the function $w(x,t)$. If the origin selected for time $t$ does not effect the expectation, i.e.

$$E[x(x,t + \tau)] = E[w(x,t)]$$

we say that the process is stationary. For a stationary ergodic process, therefore, we may write:

$$E[w(w,t)] = \lim_{T \to \infty} \frac{1}{2T} \int_{-T}^{T} w(x,t) w(t + \tau) \, dt$$

(31)

$$E[w(x,t) w(t + \tau)] = R_w(x,t,\tau) = \lim_{T \to \infty} \frac{1}{2T} \int_{-T}^{T} w(x,t) w(t + \tau) \, dt$$

Here $R_w$ is called the autocorrelation function of $w(x,t)$. We note that the mean square value of $w(x,t)$ follows from the second of (31) by setting $\tau = 0$. The aim of the present paper is to determine the autocorrelation functions of the displacement $w$ and that of the bending stress in terms of the autocorrelation function $R_p(x,t,\tau)$ of the applied load, i.e.

$$R_p(x,t,\tau) = E[P(x,t) P(t, t + \tau)]$$

(32)

This latter quantity is obtained from a record of the time history of the load $P(x,t)$. Later for this quantity, we shall select the fundamental case of purely random process in time with temporal correlation in two space points, vanishing where the points are not coincident. More precisely.
(34) \( R_p(x,t,\tau) = D \delta(x-t) \delta(\tau) \)

where \( \delta \) is the Dirac delta function. This form is commonly used in the treatment of the Brownian motion, but it is doubtful if any real function could comply with it exactly.

Using (14) with \( w_n(t) \) given by (21) in place of \( w \) in the second of (32), we obtain

(35) \[ R_p(x,t,\tau + r - s) \mathrm{d}r \mathrm{d}s = \int_0^L \int_0^L \int_{-\infty}^\infty \int_{-\infty}^\infty W_m(r) W_n(s) \mathrm{d}x \mathrm{d}t \sin(n\pi x/L) \sin(m\pi t/L) \]

Upon substitution of (34) this gives

(36) \[ R_p(x,t,\tau + r - s) \mathrm{d}r \mathrm{d}s \int_0^L W_m(r) W_n(r + \tau) \mathrm{d}r \sin(n\pi x/L) \sin(m\pi t/L) \]

where

(37) \[ S_n(\tau) = 2 \int_{-\infty}^{\infty} W_n(r) W_n(r + \tau) \mathrm{d}r \sin(n\pi x/L) \sin(m\pi t/L) \]

We may now use (28) in (37) to evaluate this integral

(38) \[ S_n(\tau) = \text{Real} \left( M_1 e^{i\xi_1^0 \tau} + M_2 e^{i\xi_2^0 \tau} \right) \]

where

\[ M_1 = -i(2\xi_1^0)^{-1} p_1^2 + i(\xi_1^0 - \xi_1^0)^{-1} p_{1*}^2 - i(\xi_1^0 + \xi_2^0)^{-1} p_{1*} p_{2*} + i(\xi_1^0 - \xi_2^0)^{-1} p_{1*} p_2 \]

\[ M_2 = -i(2\xi_2^0)^{-1} p_2^2 + i(\xi_2^0 - \xi_2^0)^{-1} p_{2*} p_2 - i(\xi_2^0 + \xi_1^0)^{-1} p_{2*} p_1 + i(\xi_2^0 - \xi_1^0)^{-1} p_{2*} p_{1*} \]
The real form of (58) is

\[ g_n(\tau) = e^{-\alpha_n \tau} (E_n \cos \omega_n \tau + F_n \sin \omega_n \tau) + e^{-\alpha_n \tau} (G_n \cos \mu_n \tau + H_n \sin \mu_n \tau) \]

(39)

\[ 2F_n = M_1 + M_2^* \quad , \quad 2F_n = i(M_1 - M_2^*) \]
\[ 2G_n = M_2 + M_1^* \quad , \quad 2G_n = i(M_2 - M_1^*) \]

For small \( \beta \), these may be simplified to

\[ E_n \sim (2\omega_n)^{-1} \omega_n^{-2} (\omega_n^2 - \mu_n^2)^{-2} \]

(40) \[ F_n \sim (2\omega_n)^{-1} \omega_n^{-2} (\omega_n^2 - \mu_n^2)^{-2} + 2\omega_n^{-1} (\omega_n^2 - \mu_n^2)^{-3} \]
\[ G_n \sim (2\mu_n)^{-1} \mu_n^{-2} (\omega_n^2 - \mu_n^2)^{-2} \]
\[ H_n \sim (2\mu_n)^{-1} \mu_n^{-2} (\omega_n^2 - \mu_n^2)^{-2} - 2\mu_n^{-1} (\omega_n^2 - \mu_n^2)^{-3} \]

The total displacement correlation \( R_v \) is obtained by substituting the first of (39) into the second of (31). The result is identical in form to (36) and (38) with \( M_1 \) and \( M_2 \) calculated by replacing \( P_1 \) and \( P_2 \) by \( Q_1 \) and \( Q_2 \) respectively. The real form of \( R_v \) may be written as

(41) \[ R_v = \frac{D}{L} \frac{c_0}{\lambda T} 2 \sum_{n=1}^{\infty} e^{-\alpha_n \tau} \left[ (E_n' \cos \omega_n \tau + F_n' \sin \omega_n \tau) + e^{-\alpha_n \tau} \right] \sin \frac{n \pi x}{L} \sin \frac{n \pi y}{L} \sin \frac{n \pi l}{L} \]
\[ 2E'_n = M_1 + M_1^* \quad \text{and} \quad 2F'_n = i(M_1 - M_1^*) \]
\[ 2G'_n = M_2 + M_2^* \quad \text{and} \quad 2H'_n = i(M_2 - M_2^*) \]

which, for small \( \beta_0 \), simplify to

\[ 2E'_n \approx \beta_n^{-1} \omega_n^{-2}(\omega_n^2 - \mu_n^2) q_{1n}^2 \]
\[ 2F'_n \approx \omega_n^{-3}(\omega_n^2 - \mu_n^2)^2 q_{2n}^2 + 4\omega_n^{-1}(\omega_n^2 - \mu_n^2)^3 q_{1n} q_{2n} \]
\[ 2G'_n \approx \alpha_n^{-1} \mu_n^{-2}(\omega_n^2 - \mu_n^2) q_{1n}^2 \]
\[ 2H'_n \approx \mu_n^{-3}(\omega_n^2 - \mu_n^2)^2 q_{2n}^2 + 4\mu_n^{-1}(\omega_n^2 - \mu_n^2)^3 q_{1n} q_{2n} \]

\[ q_{1n} = 1 - \lambda c^2 \rho_n^2 + \lambda r \rho_n^2 \]
\[ q_{2n} = 1 - \lambda c^2 \rho_n^2 + \lambda r \rho_n^2 \]

In order to calculate the correlation function for the bending stress, we first calculate the bending moment \( \mu(x,t) \):
Thus the bending moment correlation \( R_M \) may be calculated from

\[
R_M = E\{M(x,t)M(t,t + \tau)\} = (EI)^2 \frac{\partial^2 w_B}{\partial x^2} \frac{\partial^2 w_B}{\partial t^2} = (EI)^2 \frac{\partial^4 R_{WB}}{\partial x^2 \partial t^2}
\]

When (35) is substituted, this gives

\[
R_M = (Dc \lambda^2/L) \sum_{n=1}^{\infty} S_n(\tau)(n\pi/L)^4 \sin (n\pi x/L) \sin (n\pi t/L)
\]

An examination of \( S_n(\tau) \) shows that it is \( O(1/n^6) \). Hence

\[
(n\pi/L)^4 S_n(\tau) = O(1/n^2)
\]

Therefore, we may conclude that (44) converges though possibly quite slowly. The maximum bending stress is given by \( \sigma_x = M/E \) where \( E \) is the section modulus of the beam. The bending stress correlation would therefore be

\[
R_{s_x} = 2\sigma_x^2 E\{M(x,t)M(t,t + \tau)\} = (Dc \lambda^2/L^2) \sum_{n=1}^{\infty} (n\pi/L)^4 S_n(\tau) \sin (n\pi x/L) \sin (n\pi t/L)
\]

6. Stress Correlations for Other Random Loadings

Two other special types of loadings are of practical importance. They are the purely random pressure and purely random concentrated load of which the first is represented by a pressure correlation of the form
where $D_1$ is the constant spectral density of pressure $P(x,t)$. The maximum bending stress correlation in this case, calculated on the basis of the classical (Bernoulli-Euler) theory, is given by

$$R_{\sigma \lambda} (x) = (4\pi^2 /4L^2)^2 \sum_{m,n=1}^{\infty} T_{mn}(\tau) \sin (m\pi L) \sin (n\pi L),$$

where

$$T_{mn}(\tau) = \frac{1}{2} \rho \frac{\omega}{\pi} - \frac{1}{2} e^{-\frac{\pi^2}{L^2}} \left[ (4\pi^2 + (\omega_m - \omega_n)^2)^{-1} [2(\cos \omega_n \tau + (\omega_m - \omega_n) \sin \omega_n \tau] \right].$$

In the case of purely random concentrated load acting at $x = a$, the load correlation is

$$R_p = D_2 \delta(x - a) \delta(t - a) \delta(t)$$

The maximum bending stress correlation, based on the classical theory, is given in [4]. The results based on the Timoshenko model is as follows:
(50) \( R_B(x) = (\beta \delta_2 \lambda_2^2 \frac{2 \beta}{L^2} \gamma_2^2) \sum_{m,n=1}^{\infty} S_{m,n}(\tau) (\frac{mn}{L})^2 (\frac{m}{L})^2 \sin \left( \frac{mn}{L} \right) \sin \left( \frac{m}{L} \right) \)

where

\( S_{m,n}(\tau) = \int_{-\infty}^{\infty} W_m(r) W_n(r + \tau) \, dr \)

with \( W_n \) given by the first of (28). For other types of load correlations, see [4].

7. Numerical Example and Discussion

An electronic digital computer was used to calculate the displacement on maximum stress correlations at \( x = \frac{1}{2} \) (the mean square values) for the case of purely random loading. The steel beam used was neither thick nor thin (\( L/r = 20 \)) and had the following characteristics

- \( E = 30 \times 10^6 \) psi
- \( G = 12 \times 10^6 \) psi
- \( k = 5/6 \)
- \( \rho = 0.285 \) lb/in\(^3\)
- \( L/r = 20 \)
- \( \mu_1/2\pi = 151.5 \) cps
- \( \omega_1/\omega_n = 3900 \) cps
- \( \kappa = 1.0 \)

\( \alpha_n = \beta_n = 95.8; 700. \)

The series for \( R_B \) and \( R_H \) converge extremely rapidly, thus requiring
the calculations of only the first few terms. The series for $R_{yx}$ is however very slowly convergent. For a fair accuracy, it was necessary to take into account 100 terms of the series. This, of course, was to be expected, since the Bernoulli-Euler Theory led to divergence for this quantity [4]. The results of these computations are plotted in Figures 2 - 5. For the purpose of comparison, the results of the Bernoulli-Euler Theory is also plotted in Figures 2 and 3. It is clear from these curves that Bernoulli-Euler beam theory is satisfactory for calculations of the mean square of the displacement. The curves of Figures 4 and 5 were, on the other hand, not obtainable from this theory. Figure 4 shows that the mean square of the bending stress is almost constant along the beam. According to Figure 5, this quantity, in time, is nearly purely random, as effectively indicated by the calculations based on the classical beam theory [4]. The damping has the effect of reducing the sharpness of the correlation at $\beta t = 0$.

In conclusion, the Bernoulli-Euler beam theory represents an adequate model for studying random vibrations of beams, when only the mean displacements are sought. If the mean bending stress is desired, it is necessary to use a more improved theory. The Timoshenko beam theory, as indicated by the present study, appears to be adequate for this purpose. The series obtained for the mean square bending stress is, however, slowly convergent. It would seem desirable to incorporate some form of internal damping to the theory so as to improve the convergence and to bring the theory into closer agreement with reality.
FIGURE 3. DISPLACEMENT CORRELATION FUNCTION (Unnormalized) vs. CORRELATION FUNCTION
\[ R_\sigma = R_{\sigma}(0) = \frac{\overline{\sigma}^2}{\sigma^2} \]

\[ c_0^2 = \frac{Dc_L}{2\pi^2 \beta} \]

**FIGURE 4. MEAN SQUARE STRESS vs. POSITION ALONG BEAM**
\[ \begin{align*} 
L & \frac{B}{L^3} = 0.1 \text{ if } \frac{x}{L} = 1/2 \\
2 & \frac{B}{L^3} = 0.736 \\
R_\sigma & = \frac{\sigma^2}{\sigma_0^2} \\
\sigma_0^2 & = \frac{DCL}{2\pi^2 L x^2} 
\end{align*} \]

**FIGURE 5. STRESS CORRELATION (Unnormalized) vs. CORRELATION INTERVAL**
\[ \omega^2 = \frac{EI}{\pi^2 A (1 + \nu)} \]

**FIGURE 2. MEAN SQUARE DISPLACEMENT ALONG BEAM**
REFERENCES


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